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Ö REFERENCE

40163

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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<u>LINE</u>	<b>STATION</b>	<u>PLAN</u>	<b>PROFILE</b>
-L-	15+00.00 to 43+00.00	4-5	N/A
-DRV-	10+00 00 +0 12+41 88	Δ	N/A

#### CROSS SECTIONS

<u>LINE</u>	<u>STATION</u>	<b>SHEETS</b>
-L-	15+00.00 to 23+50.00	6-13
-L-	28+00.00 to 34+00.00	14-20
-L-	36+00.00 to 43+00.00	21-25
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STATE OF NORTH CAROLINA

**DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **ROADWAY** SUBSURFACE INVESTIGATION

COUNTY **LENOIR** 

PROJECT DESCRIPTION BRIDGE NO. 20 AND BRIDGE NO. 34 ON NC 55 OVER THE NEUSE RIVER

**INVENTORY** 

STATE PROJECT REFERENCE NO. 27 B-4926

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6805. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INCLORDED TO CLIMATIC CONDITIONS INCLOURS CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLOURING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO IT THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

  1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

J. HOWARD M. CRUZ K. PLUMMER S. HARDEE B. POWELL L. GONZALEZ INVESTIGATED BY \_WOOD E&IS, INC. DRAWN BY \_R. RAHIE CHECKED BY \_\_C.T. TANG SUBMITTED BY M. LEAR DATE \_\_MARCH, 2022

**PERSONNEL** 

NC Engineering F-1253 NC Geology C-247 4/21/2022 SIGNATURE

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

B-4926

SHEET NO. **2** 

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (MASHITO T 206, ASTM DI586), SOIL CLASSIFICATION	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PERETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS  THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
LLASS. ( \( \sigma .30\) PASSING "200) ( > 30\) PASSING "200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.  CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-6 A-2-7 A-4, A-5 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	ROCK (NCR)  ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK BUT MAY NOT YIELD	OF SLOPE.
7. PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
#10 50 MX GRANULAR CLAY MUCK,	PERCENTAGE OF MATERIAL  GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.  VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL — — 40 MX 41 NN 46 MX 41 MN 46 MX 41 MN 47 MX 41 MN 40 MX 41 MN LITTLE OR HIGHLY	MODERATELY ORGANIC         5 - 10%         12 - 20%         SOME         20 - 35%           HIGHLY ORGANIC         > 10%         > 20%         HIGHLY         35%         AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS.  OF MAJOR   CRAYEL, AND   SILTY OR CLAYEY   SILTY   CLAYEY   MATTER		(SLI,)  1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS,	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.  FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.  FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBCROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBCROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK.  MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK,	FIELD.   <u>JOINT</u> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD RANGE OF UNCONFINED COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-YHLUE) (TUNSZET-)	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4 TO 10 GRANULAR LOOSE 4 TO 10	SOIL SYMBOL  Open on test boring  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10 TO 30 N/A  MATERIAL DENSE 30 TO 50  (NON-COHESIVE) MEDIUM DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERT DENSE 2 20	INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT < 2 < 0.25  GENERALLY SOFT 2 TO 4 0.25 TO 0.5	MW C TEST POPING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY         MEDIUM STIFF         4 TO 8         0.5 TO 1.0           MATERIAL         STIFF         8 TO 15         1 TO 2	INFERRED ROCK LINE MONITORING WELL WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4  HARD > 30 > 4	TTTT ALLUVIAL SOIL BOUNDARY ALLUVIAL SOIL BOUNDARY INSTALLATION SPT N-VALUE	ALSO AN EXAMPLE.	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS  VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STO. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER   COBBLE   GRAVEL   SAND   SAND   SILT   CLAY   SAND   CSE. SD.)   (F SD.)	ABBRE VIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE FIELD MOISTURE COURS FOR THE A MAINTURE OFFICE OF THE A MAINTURE OFFICE OF THE A MAINTURE OFFICE OF THE A MAINTURE OF THE A MAINTURE OFFICE OF THE A MAINTURE OF T	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS)  OESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.  VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.  TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID: REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BORING ELEVATIONS OBTAINED FROM TIN FILE PROVIDED
(P]) PL PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BY NCDOT (b4926_Is_tin_171205.tin).
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT  DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: N/A FEET
SL SHRINKAGE LIMIT	X CME-45C CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6' CONTINUOUS FLIGHT AUGER CORE SIZE	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY	8* HOLLOW AUGERS	INDURATION	1
PLASTICITY INDEX (PI) DRY STRENGTH	X CME-550 HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINCER FREES NUMEROUS GRAINS: GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	1 — TOYONG AND I A THING ROOM	CRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PRORE.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CME-450   INICONE - TUNG, CARB.   SOUNDING ROD   VANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	X CME-550X X MUD ROTARY X MUCK PROBE	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14
			52.0 10 1.

# 49 IE

BEGIN PROJECT B-4926

### STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

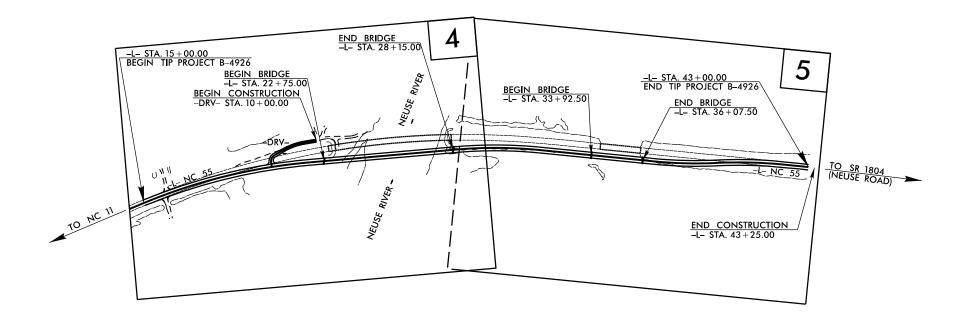
# LENOIR COUNTY

LOCATION: BRIDGE NO. 20 AND BRIDGE NO. 34 ON NC 55 OVER THE NEUSE RIVER

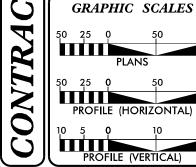
TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURES

0.7.5	0	N	Ο.	SHEETS				
N.C.	E	3–4926		. 0	3	27		
STATE PROJ	. NO.	P. A. PROJ. NO.		TON				
40163.1	.2	N/A		PE				
40163.2	2.1	N/A	R/W & UTILITIES					
			1					





DESIGN EXCEPTION REQUIRED FOR SUPERELEVATION. THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II. INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



#### DESIGN DATA ADT 2020 = 3020

END PROJECT

MAP

VICINITY

ADT 2040 = 3900K = 9 % D = 60 %T = 8 % \*

V = 60 MPH\* TTST = 3% DUAL = 5% FUNC CLASS = MAJOR COLLECTOR

"REGIONAL TIER"

#### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4926 = 0.387 MILES LENGTH STRUCTURES TIP PROJECT B-4926 = 0.143 MILES TOTAL LENGTH OF TIP PROJECT B-4926 = 0.530 MILES

# 2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:

AUGUST 16, 2019

LETTING DATE:

OCTOBER 3, 2022

RAJIT RAMKUMAR, PE

DANIEL W. GARDNER, JR., PE

CASEY K. WHITLEY, PE, PLS

## HYDRAULICS ENGINEER

SIGNATURE:

ROADWAY DESIGN **ENGINEER** 





March 24, 2022

WBS Number: 40163.1.2
TIP Number: B-4926
COUNTY: Lenoir

DESCRIPTION: Bridge No. 20 and Bridge No. 34 on NC 55 Over the Neuse River

WOOD E&IS Number: 6468199027

SUBJECT: Geotechnical Roadway Inventory Report

#### **Project Description**

The project consists of replacing the bridges on NC 55 over the Neuse River (Bridge No. 20) and the Neuse River Overflow (Bridge No. 34). We understand that the Bridge over the Neuse River will be a 6-span bridge and the Neuse River Overflow Bridge will be a 3-span bridge. To maintain traffic on NC 55 during construction, the horizontal alignment of NC 55 will be shifted south into the wetland area. The project site is located as described above, approximately 5 miles northeast of Kinston, NC. Most of the proposed roadway alignment (-L-) passes through forested swamp with standing water. The remaining portion of the proposed alignment consists of wooded areas.

The geotechnical field investigation for the roadway portion of the project was conducted in two phases, with most of the field work completed during April and May of 2019, and the remaining field work completed in October of 2021. This interruption to the field work was the result of the NCDOT placing the project on hold in June of 2019, and direction to resume work in the spring of 2021. A CME-45C drill rig mounted on a swamp buggy ATV carrier was used to access and perform borings in portions the swamp section of the project. Two CME-550X ATV drill rigs were utilized to perform borings within the existing roadway. The three rigs were equipped with automatic hammers. Mud rotary drilling procedures were used to advance borings to the required depths. Standard Penetration Tests (SPT) were performed at approximately 2.5-foot to 5.0-foot intervals to termination in selected borings. Muck probes and/or hand auger borings were also performed throughout the swamp section of the project. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis.

The following alignments, totalling approximately 0.4 miles, were explored. Selected subsurface cross sections of these alignments are included in this report.

 Alignment
 Station (±)

 -L 15+00 to 43+00

 -DRV 10+00 to 12+41.88

#### **Areas of Special Geotechnical Interest**

1) The entire project contains fine grained cohesive soils which have the potential to cause embankment/subgrade and/or slope stability problems during construction.

- 2) The entire project was found to exhibit seasonal high ground water.
- 3) <u>Organic Soils:</u> The following sections contain soils with little to high organic content, which have the potential to cause embankment/subgrade and/or slope stability problems during construction.

<u>Line</u>	Stations (±)	Offsets (ft.)
-L-	15+00 to 15+75	RT
-L-	16+25 to 19+75	RT
-L-	27+75 to 31+25	RT
-L-	32+25 to 34+25	RT
-L-	35+75 to 42+75	RT

#### Physiography and Geology

The project is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping and generally exhibits poor surface drainage. Natural ground elevations along the project alignments ranged from 12± to 19± feet above sea level. Surface waters from this area are generally collected in shallow ditches along the roadway and flow into the Neuse River. The project site is located within the existing floodplain of the Neuse River and as such is subject to periodic flooding from the river. The site consists primarily of the existing roadway, woods, and forested swamp.

Geologically, the project is located within the Coastal Plain Province. Alluvial soils are present throughout the project site. The alluvial soils are underlain at depth by the Cretaceous aged marine sediments of the Peedee Formation.

#### **Ground Water**

Ground water data was collected during our subsurface investigations in April and May of 2019 and October of 2021, in almost all borings throughout the project corridor. Ground water elevations ranged from approximately 10.5 to 23.4 feet above sea level. The surface water elevation of the wooded swamp was measured several times during the field investigation and ranged from 8.7 to 19.1 feet above sea level. Ground water and surface water levels may fluctuate with seasonal precipitation and localized storm events within the Neuse River Basin.

#### **Soil Properties**

Soils encountered during this investigation have been divided into four categories based on origin, including roadway embankment, artificial fill, alluvial soils, and coastal plain formational strata.

Roadway embankment soils are present along existing NC 55 (-L-) and the intersecting driveway (-DRV-). These soils consist of ±3 to ±20 feet of tan, orange, gray, and brown, very loose to very dense, dry to saturated, silty fine to coarse sand (A-2-4, A-3), locally with trace gravel and asphalt fragments; and tan, orange, gray, and brown, very soft to soft, wet, fine sandy, silt and clay (A-4, A-6). Laboratory test results from selected samples show plastic indices of non-plastic to 22. The natural moisture content ranges from 8 to 35 percent in these samples.

Artificial fill soils are present along the existing driveway (-DRV-). These soils consist of ±7 feet of tan, orange, and brown, medium dense, dry to wet, silty fine to coarse sand (A-2-4).

Alluvial soils are present at the ground surface within the low laying areas in and around the wooded swamp and below the existing roadway embankment soils for NC 55 where it passes through the Neuse River floodplain. The surficial alluvial soils primarily consist of ±1 to ±6 feet of gray and brown, very soft, saturated, sandy, and clayey, Muck; and brown, gray, and tan, very soft to medium stiff, moist to saturated, fine sandy, silt and clay (A-4, A-6, A-7-6) with trace to moderate organics, locally with trace gravel. Below the surficial alluvial sediments the alluvial soils consists of ±7 to ±19 feet of brown, tan, orange, gray and green, very loose to medium dense, wet to saturated, silty and clayey, fine to coarse sand (A-2-4, A-2-6, A-3, A-1-b) with trace gravel and organics; and locally interbedded with gray and tan, very soft to stiff, wet to saturated, fine to coarse sandy and silty clay and clayey silt (A-6, A-7-6, A-4) with trace gravel, organics, and wood fragments. The natural moisture content of the alluvial soils ranges from 13 to 52 percent. Laboratory test results from selected samples of the surficial alluvial clay and silt show trace to high organic content (4.7% to 57.7%) and slight to high plasticity with plastic indices from 10 to 38. Alluvial soils extended to the top of the coastal plain formational strata at elevations ranging from approximately 10 to -3 feet.

Coastal plain formational strata belonging to the Peedee Formation where encountered below the alluvial soils in the deeper subsurface and consist mostly of gray and green, medium stiff to hard, moist to saturated, locally glauconitic, fine to coarse sandy and silty, clay and sandy silt (A-6, A-7-6, A-4) with trace shell fragments and fine sandy partings; green and gray, medium dense to very dense, moist to wet, glauconitic, silty and clayey fine sand (A-2-4, A-2-6) with trace shell fragments and cemented sand lenses, gravel, clay lenses, and very thinly to thinly bedded cemented sand lenses. Laboratory test results from selected clay samples show liquid limits ranging from 32 to 42 and slight to high plasticity, with plastic indices of 14 to 26. The natural moisture content ranges from 23 to 32 percent in these samples.

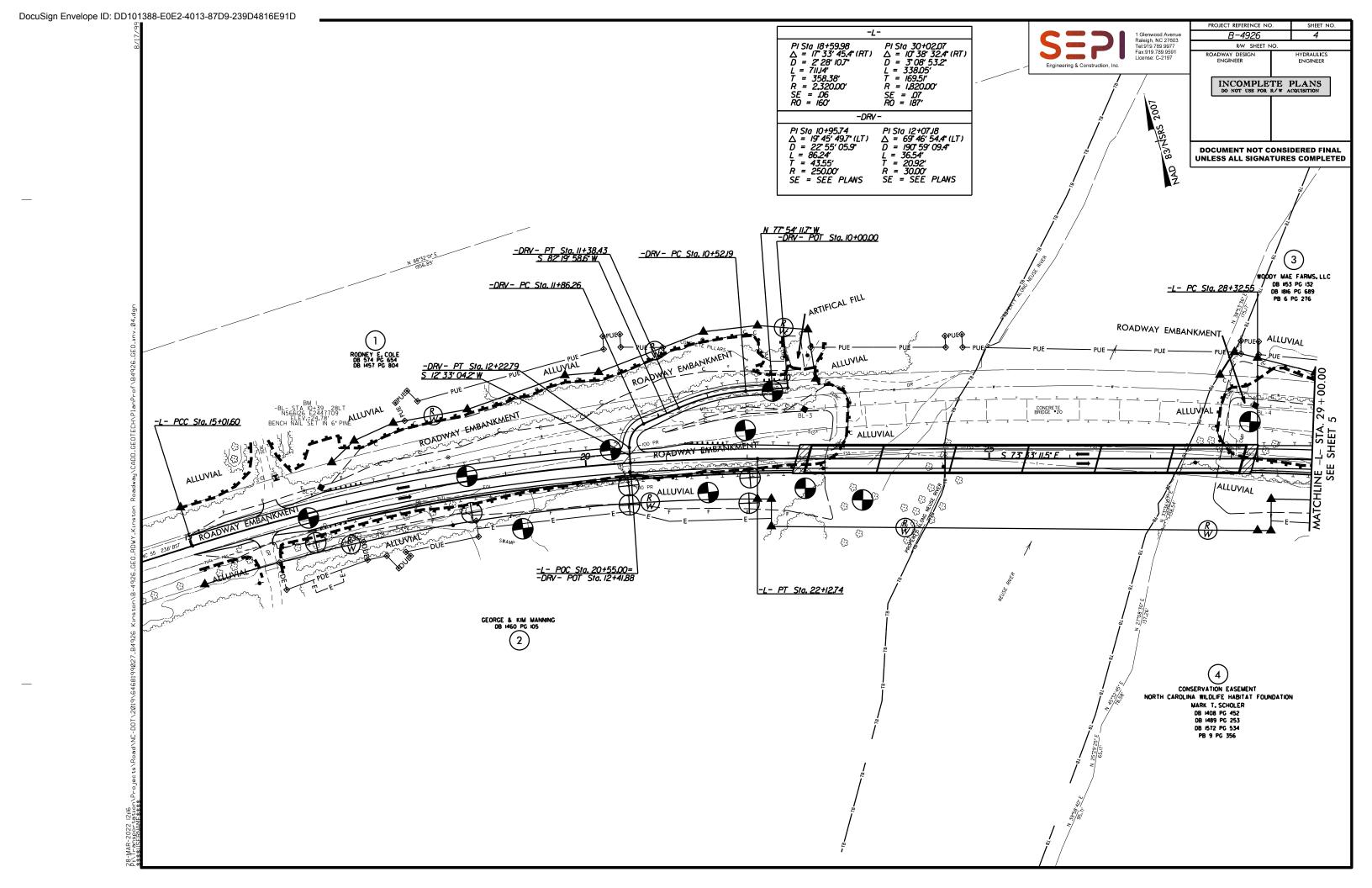
Prepared By,

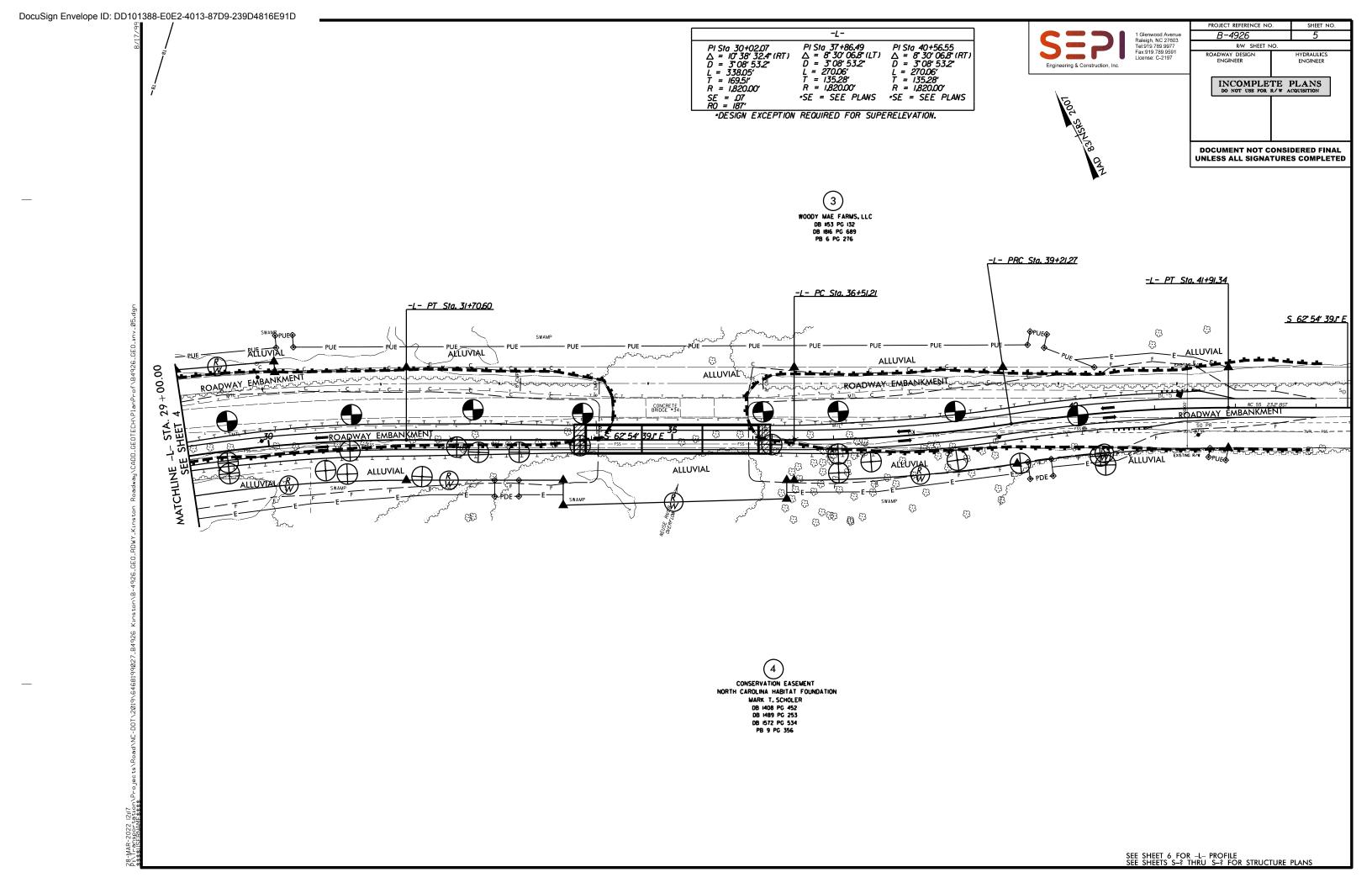
Michael B. Lear, PG Senior Geologist

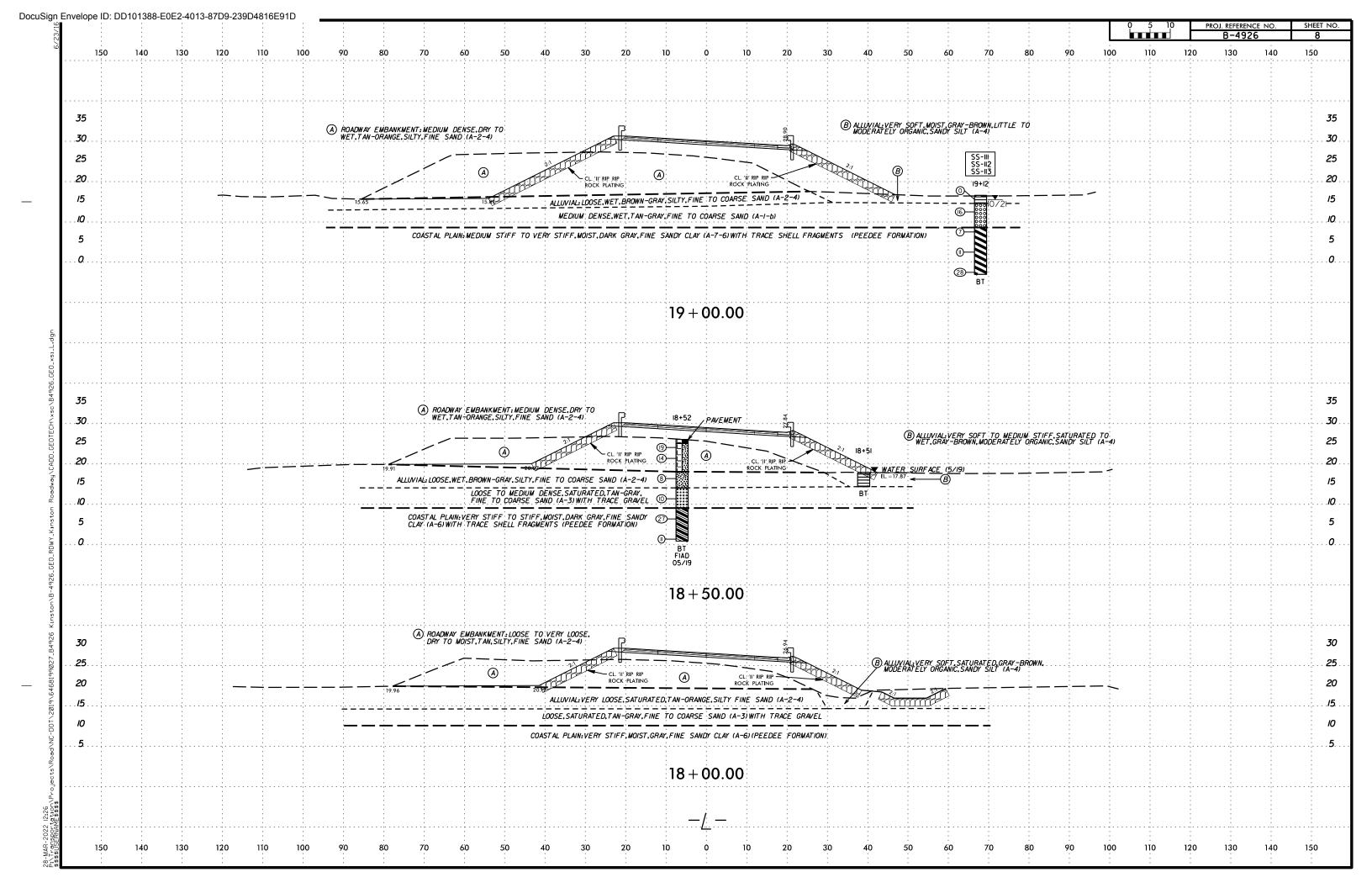
#### **UNDISTURBED SAMPLES**

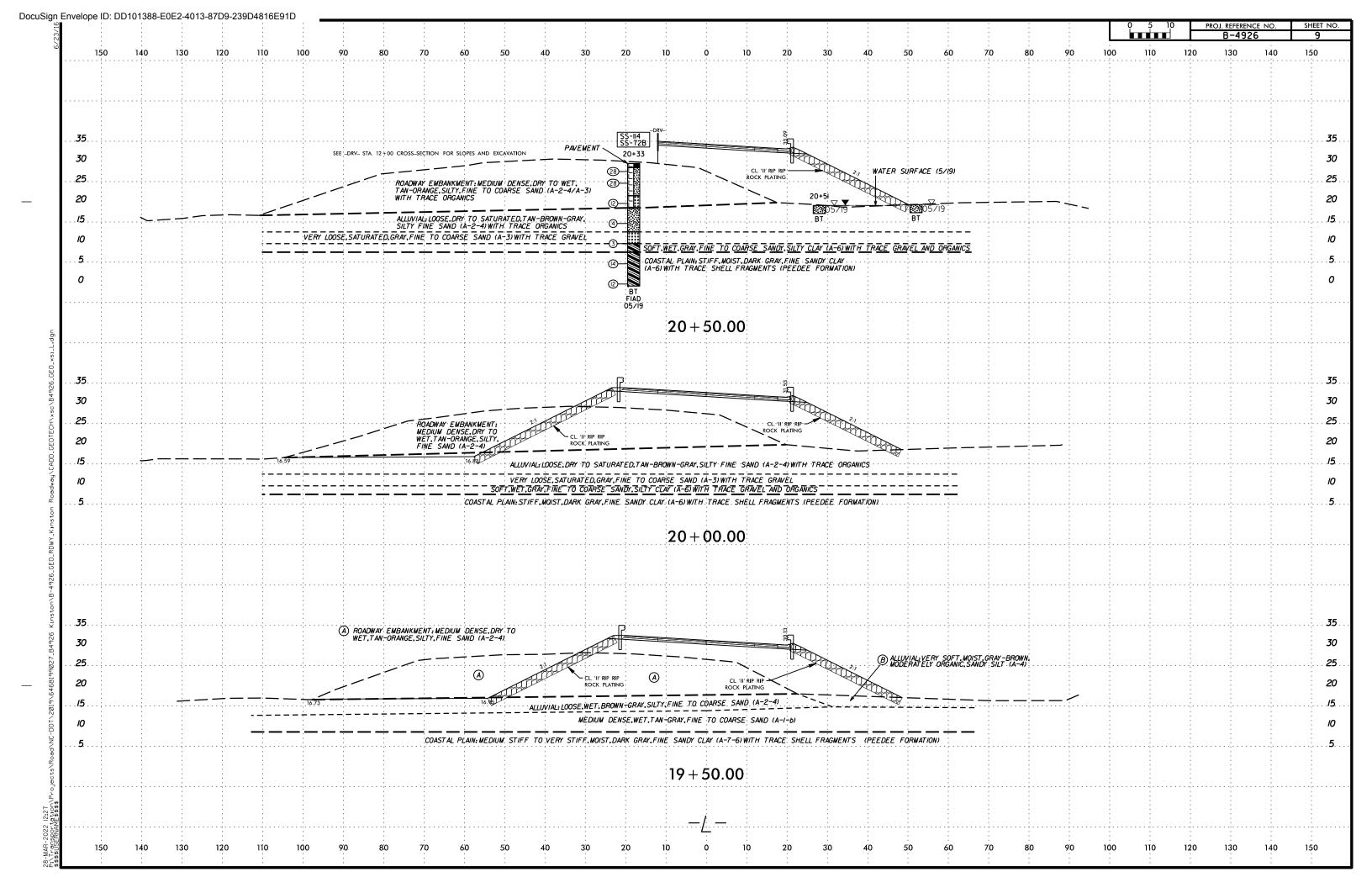
The following undisturbed samples were taken for tests to determine the engineering properties of the soil: (Note: Tubes pushed at offset locations shown and adjacent to SPT boring location. ST-1 and ST-2 not tested due to project hold as directed by NCDOT, ST-3 not tested due to damage to tube during sample collection.)

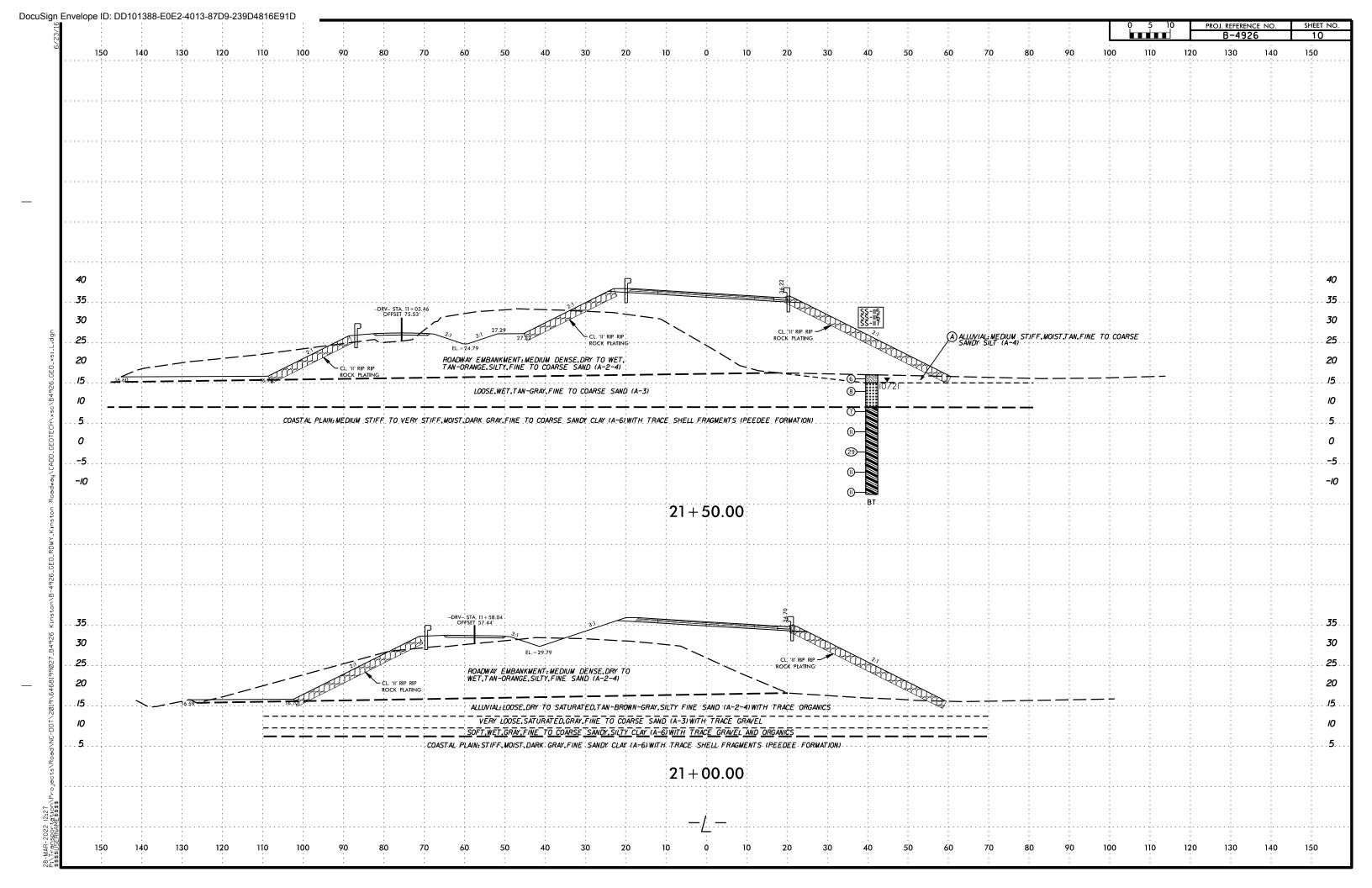
Sample No.	<u>Location</u>	Depth (ft)	<u>Test</u>
ST-1	-L-, 36+18, 35' LT	8.5 – 10.5	Not Tested
ST-2	-L-, 36+18, 35' LT	13.0 – 15.0	Not Tested
ST-3	-L-, 22+72, 40' RT	4.1 - 5.2	Not Tested

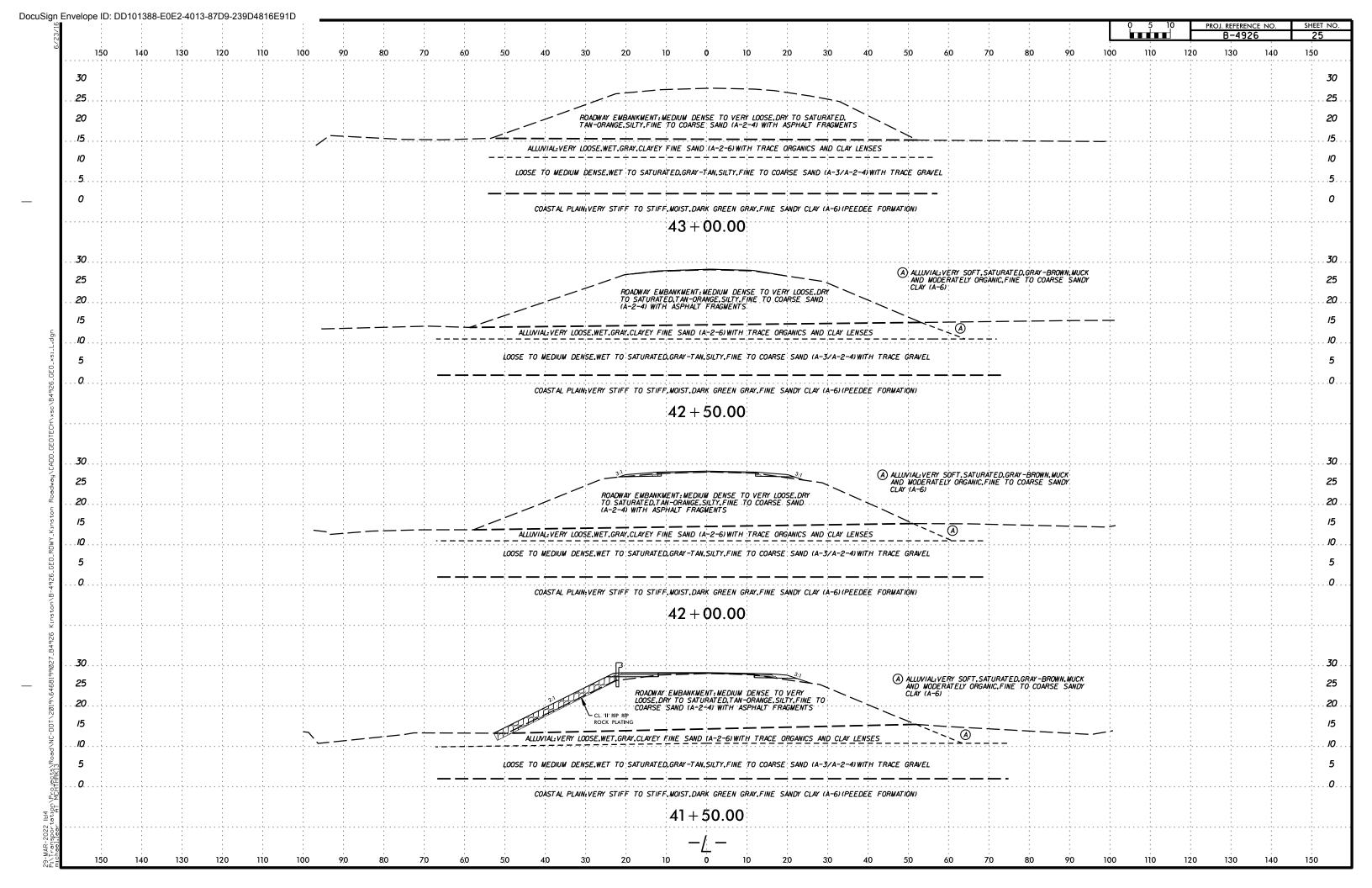












Wood E&IS Project No.: 6468-19-9027 Bridge Nos. 20 and 34 on NC 55 Over the Neuse River and Overflow

Date Reported: 10/29/2021 **County: LENOIR Date Tested: October 2021** 

NCDOT WBS No.: 40163.1.2 Tip No.: B-4926					County: LENOIR						Date Tested: October 2021							
						S	OIL T	EST RES	ULTS									
DEF					AASHTO				% BY W	/EIGHT		% PASSING SIEVES			%	%		
SAMPLE NO.	STATION	OFFSET	LINE	INTERVAL	CLASS.			L.L.	P.I.	C. SAND	F. SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS-110	16+50	6' RT	-L-	3.2-4.7'	A-2-4(0)	NP	NP	27.4	62.5	1.6	8.5	100.0	90.4	12.2	11.9	-		
SS-111	19+12	68' RT	-L-	0.0-1.5'	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	74.3	11.7		
SS-112	19+12	68' RT	-L-	3.1-4.6'	A-1-b	NP	NP	77.7	15.1	3.7	0.8	97.3	39.3	5.4	19.5	-		
SS-113	19+12	68' RT	-L-	8.1-9.6'	A-7-6(10)	42	24	2.2	49.1	14.4	34.3	100.0	98.8	54.7	30.9	-		
SS-114	20+33	18' LT	-L-	8.9-10.4'	A-3	NP	NP	68.4	21.0	5.1	2.7	97.2	53.7	9.0	8.4	-		
SS-72B	20+33	18' LT	-L-	19.9-21.0'	A-6(10)	34	15	14.3	12.2	52.7	20.0	99.2	87.7	73.8	35.4	-		
SS-115	21+50	41' RT	-L-	0.0-1.5'	A-4(0)	26	10	39.4	26.2	6.7	27.4	99.7	83.2	35.8	13.2	-		
SS-116	21+50	41' RT	-L-	3.1-4.6'	A-3	NP	NP	65.9	25.2	3.3	5.4	99.8	81.6	10.2	24.7	-		
SS-117	21+50	41' RT	-L-	8.1-9.6'	A-6(7)	36	19	35.3	11.7	25.4	27.6	100.0	73.5	54.3	31.5	-		
SS-118	22+72	37' RT	-L-	0.0-1.5'	A-6(1)	27	11	17.0	41.5	7.6	32.4	98.5	92.3	43.1	12.7	-		
SS-119	22+72	37' RT	-L-	2.9-4.4'	A-7-6(12)	56	38	1.6	55.4	17.9	23.1	98.0	97.1	45.5	27.6	-		
SS120	22+72	37' RT	-L-	7.9-9.4'	A-6(2)	32	14	1.7	62.9	12.2	23.2	100.0	99.1	41.9	28.9	-		
SS-121	28+23	46' LT	-L-	18.8-20.3'	A-6(5)	30	13	2.4	47.3	14.7	35.6	100.0	99.5	58.8	27.3	-		
SS-122	28+23	46' LT	-L-	23.6-25.1'	A-4(2)	24	10	5.6	47.8	15.9	30.7	100.0	99.7	51.7	26.8	-		
S-111	30+68	35' RT	-L-	0.0-0.5'	A-7-5 (vis)	ND	ND	8.4	13.0	35.8	41.3	98.5	93.6	79.5	302.6	16.9		
S-112	30+68	35' RT	-L-	1.0-2.5'	A-6(5)	33	16	22.1	29.1	13.0	35.8	100.0	89.1	51.6	32.4	-		
S-108	31+90	46' RT	-L-	0.5-1.5'	A-4(2)	32	9	5.4	48.6	16.8	26.2	97.0	95.3	46.6	51.9	4.7		
S-109	31+90	46' RT	-L-	2.0-2.5'	A-2-4(0)	16	1	5.3	72.6	7.7	14.4	100.0	99.6	25.7	26.2	-		
S-110	31+90	46' RT	-L-	2.8-3.3	A-2-4(0)	NP	NP	5.4	83.9	3.6	7.0	99.9	99.9	13.6	27.4	-		
SS-27	32+53	37' LT	-L-	13.5-15.0'	A-4(4)	24	9	7.3	20.1	42.0	30.5	99.9	97.5	73.2	24.8	-		
S-106	33+10	15' RT	-L-	0.5-1.5'	A-6 (vis)	ND	ND	10.9	26.4	20.9	32.9	91.1	85.2	57.1	171.8	12.2		
S-107	33+10	15' RT	-L-	3.5-4.0'	A-2-4(0)	NP	NP	48.2	40.6	6.0	3.4	98.2	73.5	11.8	20.6	-		
SS-103	33+89	33' LT	-L-	13.4-14.9'	A-6(12)	35	19	7.8	18.4	36.3	37.4	99.9	95.7	74.8	30.3	-		
SS-100	36+13	35' LT	-L-	8.4-9.9'	A-6(12)	37	22	27.5	6.8	33.7	31.4	99.4	77.1	65.6	35.2	-		
SS-101	36+13	35' LT	-L-	13.4-14.9'	A-6(8)	30	14	2.1	25.1	37.9	34.9	100.0	99.6	74.2	45.5	-		
SS-102	36+13	35' LT	-L-	28.4-29.9'	A-7-6(20)	41	26	12.6	7.5	49.4	30.3	99.8	91.3	80.8	23.4	-		
S-104	37+45	29' RT	-L-	0.5-1.5'	A-6 (vis)	ND	ND	16.9	27.3	21.9	26.4	92.5	85.5	51.4	160.3	57.7		
S-105	37+45	29' RT	-L-	3.0-3.5'	A-6(1)	27	12	15.0	44.3	12.4	25.3	97.0	90.3	40.4	28.7	-		
SS-4	38+54	24' LT	-L-	13.7-15.2'	A-6(7)	32	17	30.7	9.7	28.1	31.1	99.6	78.6	60.0	40.8	-		
S-102	39+26	50' RT	-L-	0.5-1.5'	ND	ND	ND	ND	ND	ND	ND	ND	ND	ND	277.7	21.2		
S-103	39+26	50' RT	-L-	2.5-3.0'	A-6(5)	36	17	22.1	29.1	12.5	35.7	99.4	88.2	50.8	29.0	-		
S-100	40+34	64' RT	-L-	0.5-1.0'	A-6 (vis)	ND	ND	22.0	11.7	15.2	44.5	93.4	80.3	61.2	265.6	17.2		
S-101	40+34	64' RT	-L-	2.8-3.3'	A-6(8)	32	16	18.5	15.3	35.9	30.0	99.7	97.8	67.4	25.0	-		
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ND = NOT DETERMINED

NV = NO VALUE NP = NON-PLASTIC

allat 2. Romo Signature

115-01-0504 Certification # Albert Romero Print Name