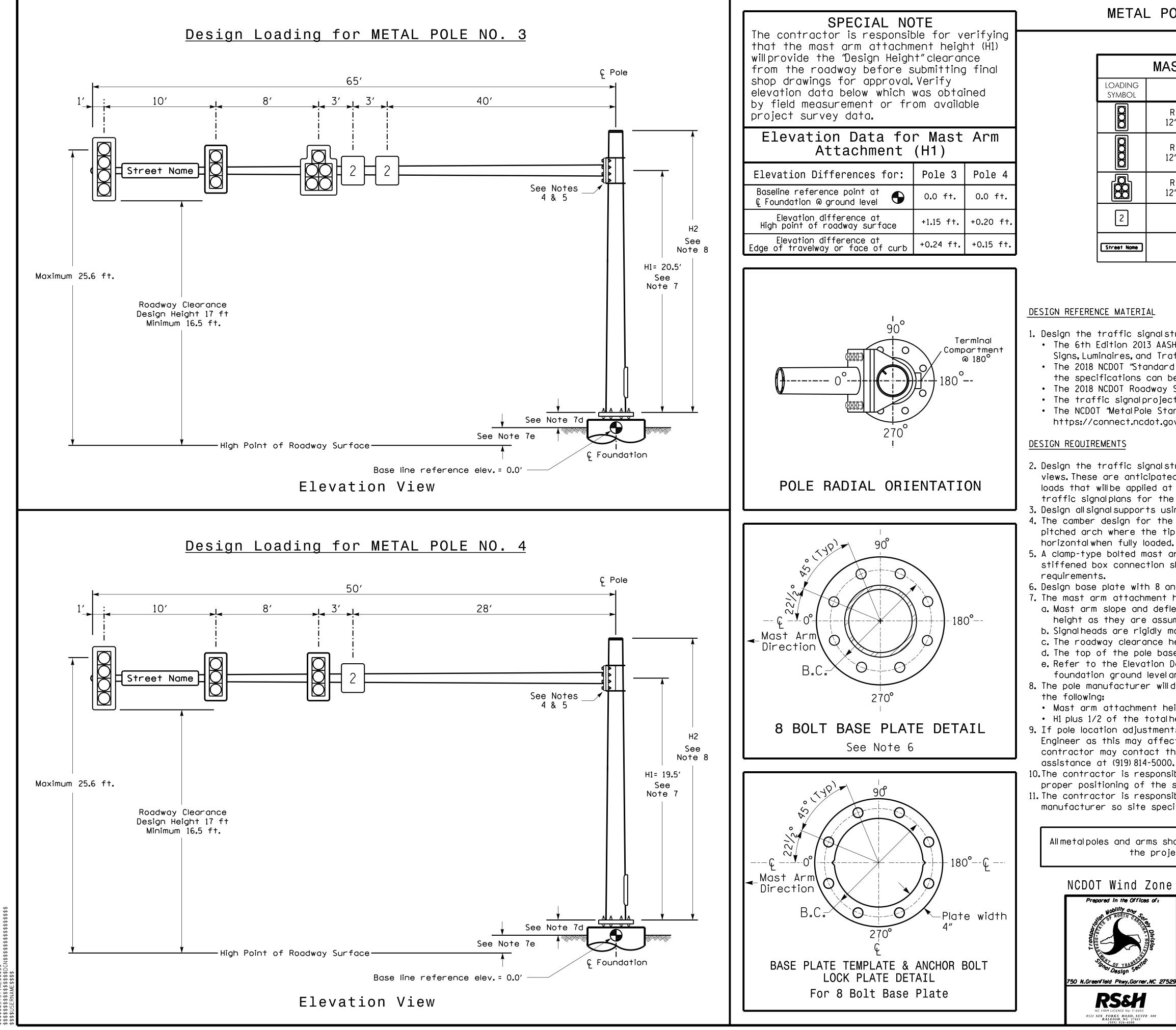
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## METAL POLES No. 3 and 4

PROJECT REFERENCE NO.	SHEET NO.
U - 5808	Sig 5.5

	MAST ARM LOADING SCHEDULE						
loading Symbol	DESCRIPTION	AREA	SIZE	WEIGHT			
	RIGID MOUNTED SIGNAL HEAD 12"-3 SECTION-WITH BACKPLATE	9.3 S.F.	25 <b>.</b> 5″₩ X 52 <b>.</b> 5″L	60 LBS			
	RIGID MOUNTED SIGNAL HEAD 12"-4 SECTION-WITH BACKPLATE	11.7 S.F.	25 <b>.</b> 5" W X 66 <b>.</b> 0" L	74 LBS			
	RIGID MOUNTED SIGNAL HEAD 12"-5 SECTION-WITH BACKPLATE	16.3 S.F.	42 <b>.</b> 0″₩ X 56 <b>.</b> 0″L	103 LBS			
2	SIGN RIGID MOUNTED	7.5 S.F.	30.0″W X 36.0″L	14 LBS			
Street Name	STREET NAME SIGN RIGID MOUNTED	16.0 S.F.	24.0″W X 96.0″L	36 LBS			

## NOTES

## DESIGN REFERENCE MATERIAL

1. Design the traffic signal structure and foundation in accordance with: • The 6th Edition 2013 AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, including all of the latest interim revisions.

• The 2018 NCDOT "Standard Specifications for Roads and Structures." The latest addenda to the specifications can be found in the traffic signalproject special provisions.

• The 2018 NCDOT Roadway Standard Drawings.

• The traffic signal project plans and special provisions. • The NCDOT "MetalPole Standards" located at the following NCDOT website:

https://connect.ncdot.gov/resources/safety/Pages/ITS-Design-Resources.aspx

2. Design the traffic signal structure using the loading conditions shown in the elevation views. These are anticipated worst case "design loads" and may not represent the actual loads that will be applied at the time of the installation. The contractor should refer to the traffic signal plans for the actual loads that will be applied at the time of the installation. 3. Design all signal supports using stress ratios that do not exceed 0.9.

4. The camber design for the mast arm deflection should provide an appearance of a low pitched arch where the tip or the free end of the mast arm does not deflect below horizontal when fully loaded.

5. A clamp-type bolted mast arm-to-pole connection may be used instead of the welded ring stiffened box connection shown as long as the connection meets all of the design

6. Design base plate with 8 anchor bolt holes. Provide 2 inch x 60 inch anchor bolts.

7. The mast arm attachment height (H1) shown is based on the following design assumptions: a. Mast arm slope and deflection are not considered in determining the arm attachment height as they are assumed to offset each other.

b. Signalheads are rigidly mounted and vertically centered on the mast arm.

c. The roadway clearance height for design is as shown in the elevation views.

d. The top of the pole base plate is 0.75 feet above the ground elevation.

e. Refer to the Elevation Data Chart for the elevation differences between the proposed foundation ground leveland the high point of the roadway.

8. The pole manufacturer will determine the total height (H2) of each pole using the greater of

• Mast arm attachment height (H1) plus 2 feet, or

• H1 plus 1/2 of the total height of the mast arm attachment assembly plus 1 foot. 9. If pole location adjustments are required, the contractor must gain approval from the Engineer as this may affect the mast arm lengths and arm attachment heights. The contractor may contact the Signal Design Section Senior Structural Engineer for

10. The contractor is responsible for verifying that the mast arm length shown will allow proper positioning of the signalheads over the roadway.

11. The contractor is responsible for providing soil penetration testing data (SPT) to the pole manufacturer so site specific foundations can be designed.

All metalpoles and arms should be black in color as specified in the project special provisions.



OT Wind Zone	4 (90 mph)			MENT NOT CONSIDERED FINAL S ALL SIGNATURES COMPLETED
NODILITY ON THE OFFICES OF:	SR 1362 (Chestnut Parkway Conr Chestnut Parkway) at SR 1367 (Matthews-Indian Trai Division 10 Union County In			SEAL OFESSION SEAL 029531
Design Sect	PLAN DATE: September 2023	REVIEWED BY: S.G. Ha	ynie	KAN KAN
enfield Pkwy.Garner.NC 27529	prepared by: O. Drobny	REVIEWED BY:		DocuSigned by:
RSSH NC FIRM LICENSE NO: F-0493	REVISIONS	INIT.	DATE	Sturn G. Hayny 8/2023 D633DCC9A5D44B6 SIGNATURE DATE
1 SIX FORKS ROAD, SUITE 400 RALEIGH, NC 27615		• • • • • • • • • • • • • • • • • • • •	.	Sig Inventory No. 10-2336