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DRAWN BY :	T.E. NEAL	DATE :	05/2023
CHECKED BY :	J.C. MORRISON	DATE :	06/2023
DESIGN ENGINE	R OF RECORD:D. TUTTLE	DATE :	06/2023
DESIGN ENGINE	R OF RECORD:D.TUTTLE	DATE :	06/2

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NOTES:

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT THE MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

ALL REINFORCING STEEL IN THE CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

THE D1, D2, D3, S1 AND S2 BARS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO PROVIDE 2" CLEARANCE TO THE $\frac{1}{2}$ " EXPANSION JOINT AND ALL OPEN JOINTS IN THE BARRIER.

PROVIDE A 4" HIGH X 8" WIDE FORMED DRAINAGE SLOT IN THE LEFT SIDE BARRIER RAIL. THIS SLOT SHALL BE PLACED AT 4'-0" SPACING ALONG THE FULL LENGTH OF THE BRIDGE. PROVIDE 2" MIN. CLR TO THE "S" BARS IN THE BARRIER TO THE SLOT. THE SLOT SHALL BE PLUGGED AT THE COMPLETION OF THE PROJECT AFTER TRAFFIC IS PLACED IN THE PERMANENT CONDITION. SEE SPECIAL PROVISIONS. PLUGGING OF DECK DRAINS IS CONSIDERED INCIDENTAL TO THE COST OF THE "CONCRETE BARRIER RAIL".

SEE CONSTRUCTION SEQUENCE AND TYPICAL SECTION SHEETS FOR ADDITIONAL STAGING INFORMATION.

#5D1 AND #5D2 BARS SHALL BE MACHINE THREADED TO FIT TO COUPLER AND DELIVERED CONNECTED, THREADS SHALL BE GREASED TO PERMIT EASY SEPARATION AFTER INSTALLATION. COUPLERS ARE CONSIDERED INCIDENTAL TO THE COST OF CONCRETE BARRIER RAIL

THE #5D1 COUPLED TO #5D2 OR #5D3 SHALL BE PUSHED INTO GREEN CONCRETE AFTER POURING AND SCREEDING THE DECK. THE COUPLER SHALL BE INSTALLED TO BE $\frac{3}{16}$ " BELOW THE TOP OF THE FINISHED CONCRETE DECK. ONCE DECK HAS CURED, #5D2 AND #5D3 SHALL BE UNTHREADED AND SET ASIDE. A TEMPORARY BOLT SHALL BE INSTALLED IN THE COUPLER AND SEALED TO PREVENT DEBRIS INTRUSION AND PROTECT THE THREADS. SEE DETAIL "D" WHEN CONSTRUCTING CONCRETE MEDIAN BARRIER, REMOVE SEALER AND TEMPORARY BOLT AND THREAD #5D2 AND #5D3 IN TO COUPLER.

PRIOR TO CURING, THE AREA UNDERNEATH THE STAGE III RIGHT CONCRETE BARRIER SHALL BE RAKE FINISHED TO A SURFACE ROUGHNESS OF $\frac{3}{16}$ ".

THE CONTRACTOR MAY SUBMIT ALTERNATIVE DETAILS FOR ANCHORING THE STAGE III CONCRETE BARRIER RAIL INTO THE DECK TO THE ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION OF THE DECK. ANY ALTERNATIVE METHODS APPROVED FOR CONSTRUCTION SHALL BE AT NO ADDITIONAL COST TO THE DEPARTMENT.

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS FOR THE D1, D2, D3, AND COUPLER PRIOR TO FABRICATION TO ENSURE PROPER FIT.

FOR SECTIONS AND DETAILS, SEE SHEET 2.

PROJECT NO. B-3186 / B-5898 HAYWOOD _ COUNTY

24+70.00 -L_LT-STATION:_

SHEET 1 OF 2



STD.NO.CBR1

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	TEC-	BILL OF MATERIAL										
UVI		CONCRETE BARRIER RAIL										
. BOLI	SHALL NOT	BAR	AR NO. SIZE TYPE LENGTH WEIGH									
		* B1	330	5	STR	19'-8"	6,769					
OW FO	DR REMOVAL	* B2	66	5	STR	12'-6"	860					
L APP	ROVE	₩ 83	66	5	STR	9'-0"	620					
		* SI	4/4	5		4'-/"	2,266					
LING T	HE BOLT	* 52 * D1	/11 /7/	5 5	2 د	/ '-0" 1' כיי	2,191 619					
			4/4 227	5	ح ⊿	ב- ד י_חי_י	<u>1010</u>					
			237	5		<u>ר איר</u> איר	803					
			201				005					
		* EPO>	(Y COAT	ED REIN	FORCING	G STEEL 17	7,621 LBS.					
		CLAS	SS AA CO	ONCRETE	-		96.3 C.Y.					
		CON	CRETE B	ARRIER	RAIL		708.1 L.F.					
				BA	R TYP	ES						
		8"11 ^{%16} "	8"	1'-0 ⁴ /2" 8 ⁷ /16" 5 2 ¹ /4" RAD.			4 					
		COUF	D1 PLER END				-0 ¹ / ₂ " - FIELD BEND TT. TO JPLER S END					
		•										
		PR ST	OJEC HA ATIO	T NO. AYWC N:	B-31 DOD 24+7	86 / B-{ C(0.00 -L_	5898 DUNTY _LT-					
EAS				۷								
PAINT	AECOM TECHNICAL SERVICES OF NC, IN 5438 WADE PARK BOULEVARD, SUITE 20 RALEIGH, NC 27607	AC.	DEPAR		TE OF NORTH OF TR RALEIGH	CAROLINA CANSPORTA	TION					
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	Docusigned by: CARO Docusigned by: CARO Docusigned by: CARO Docusigned by: CARO Docusigned by: CARO Docusigned by: CARO		C	DNCRI RAI		BARRIE TAILS						
	SHANE (NO.	BY:		NO. RY.	DATF:	<u>53-26</u>					
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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS. RUBRAIL. AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS. NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 ¹/₄" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE ³/₄" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

SKETCH SHOWING POINTS OF ATTACHMENTS

***** DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>B-3186 / B-5898</u> HAYWOOD _ COUNTY

APPROACH

SLAB

└── @ JT. @ END BENT 2

STATION: 24+70.00 -L_LT-

	AECOM TECHNICAL SERVICES OF NC, INC. 5438 WADE PARK BOULEVARD, SUITE 200 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342		DEPA	TION				
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	N SHANE			SHEET NO.				
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	10/13/2023	1			3			TOTAL SHEETS
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CHECKED BY : CRK 10/87

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GENERAL NOTES

1. FOR EXPANSION IOINT SEALS, SEE SPECIAL PROVISIONS.

2. ALL PLATES AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36 STEEL OR APPROVED EQUAL. ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. ALL STUD ANCHORS SHALL CONFORM TO AASHTO M169, GRADES 1010 THRU 1020 OR APPROVED EQUAL. ALL CONCRETE INSERTS SHALL BE CLOSED END AND SHALL CONFORM TO AASHTO M169, GRADE 12L14. TENSILE CAPACITY SHALL BE 3000 LBS. MINIMUM.

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3. A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°. FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED. THE GLAND SHALL BE CONTINUOUS AT THE BREAKBACK. SUBMIT DETAILS FOR THE GLAND IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

4. CLOSED END FERRULES AND STUD ANCHORS SHALL BE SHOP WELDED AND ALL HOLES SHALL BE SHOP DRILLED AS SHOWN ON PLANS. STUD ANCHORS SHALL BE ELECTRIC ARC END WELDED WITH COMPLETE FUSION.

5. SURFACES COMING IN CONTACT WITH NEOPRENE SHALL BE GROUND SMOOTH PRIOR TO METALLIZING.

6. UPON COMPLETION OF SHOP FABRICATION, THE HOLD-DOWN PLATE AND BASE ANGLE ASSEMBLY, AS SHOWN IN THE "TYPICAL SECTION OF BASE ANGLE ASSEMBLY" SHALL BE METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

7. THE COVER PLATES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

8. BASE ANGLE ASSEMBLY SHALL BE CONTINUOUS FOR THE LENGTH OF THE JOINT IN EACH STAGE. UPON INSTALLATION OF EACH SUBSEQUENT STAGE, FIELD WELD THE BASE ANGLE IN ACCORDANCE WITH THE DETAIL. AT CROWN BREAKS, THE ENDS OF THE BASE ANGLE ASSEMBLY SHALL BE CUT PARALLEL TO THE BRIDGE CENTERLINE FOR SKEWS LESS THAN 80° AND GREATER THAN 100°. FINISHED WELD SHALL BE REPAIRED IN ACCORDANCE WITH THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

9. FIELD SPLICES OF HOLD-DOWN PLATES SHALL BE KEPT TO A MINIMUM. CONTRACTOR SHALL FURNISH DETAILED PLANS SHOWING PROPOSED SPLICE LOCATIONS FOR APPROVAL. HOLD-DOWN PLATES SHALL NOT EXCEED 20' LENGTHS UNLESS APPROVED BY THE ENGINEER.

10. NO ALTERNATE IOINT DETAILS SHALL BE PERMITTED IN LIEU OF THOSE SHOWN ON

11. THE CONTRACTOR MAY, AT HIS OPTION, USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF CONCRETE INSERTS FOR COVER PLATES. THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT

12. THE FABRICATOR SHALL PROVIDE ¹/₂" Ø THREADED HOLES IN THE HOLD-DOWN PLATES TO ASSIST IN LIFTING AND PLACING. THE HOLES SHALL BE 3/4" DEEP AT 6'-0" MAXIMUM SPACING AND A MINIMUM OF TWO HOLES PER PLATE.

13. DURING THE TEMPORARY STAGES, UTILIZE TEMPORARY HOLD-DOWN PLATES AND A TEMPORARY GLAND IN THE REGION OF THE JOINT SEAL. NO BLOCKOUT OR HOLD-DOWN PLATES WILL BE PRESENT IN THE OVERBUILD PORTION. UPON CONSTRUCTION OF THE FINAL STAGE AND RIGHT-SIDE PERMANENT BARRIER RAIL, REMOVE THE TEMPORARY HOLD-DOWN PLATES AND GLAND AND INSTALL PERMANENT GLAND AND HOLD-DOWN PLATES. TEMPORARY PLATES, GLANDS, AND INSTALLATION AND REMOVAL OF TEMPORARY ITEMS IS CONSIDERED INCIDENTAL TO THE COST OF THE JOINT. ANY EXPOSED, CUT OR DAMAGED SURFACES, AND FIELD WELDS SHALL BE COATED WITH A ZINC-RICH PAINT.

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PROJECT	NO. B-3186	6 / B-5898
HAY	WOOD	

COUNTY 24+70.00 -L LT-STATION:

SHEET 1 OF 2

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STD. NO. EJS2

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DRAWN BY :	M.L. (DATE :	06/202	
CHECKED BY :	D. T	DATE :	06/202	
DESIGN ENGINEER	OF RECORD:	D. TUTTLE	DATE :	06/202

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NOTES:

GROOVING IS NOT PERMITTED UNDERNEATH THE STAGE III RIGHT CONCRETE BARRIER. THIS AREA HAS BEEN EXCLUDED FROM THE GROOVING AREA QUANTITIES SHOWN HERE. THIS AREA SHALL BE RAKE FINISHED TO A SURFACE ROUGHNESS OF $\frac{3}{16}$ ".

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GROOVING BRI	DGE FLOORS										
STAGE I											
APPROACH SLABS	<u>971_</u> SQ. FT.										
BRIDGE DECK	<u>6,098</u> SQ. FT.										
<u>TOTAL</u>	<u>7,069</u> SQ. FT.										
STAGE	II										
APPROACH SLABS	<u>768 S</u> Q. FT.										
BRIDGE DECK	<u>3,782</u> SQ. FT.										
TOTAL	<u>4,550</u> SQ. FT.										
ΤΟΤΑ	AL.										
APPROACH SLABS	<u>1,739</u> SQ. FT.										
BRIDGE DECK	<u>9,880</u> SQ. FT.										
TOTAL	<u>11,619</u> SQ. FT.										

	PROJE	CT NO.	B	-318	6 / B-	5898
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AECOM TECHNICAL SERVICES OF NC, INC. 5438 WADE PARK BOULEVARD, SUITE 200 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	DEPA	stat RTMENT SUPI	ie of OI ERS	NORTH CAR F TRAN RALEIGH STRUC	NSPORT	ATION
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							REINF	ORCIN	IG BAR	SCHEDU	JLE										
		S	TAGE	I				STAGE	<u>I (CC</u>)NT'D)			S	TAGE	II (C	ONT'D)					5'-11 ¹ ⁄2" K21
	NO.	SIZE	TYPE	LENGTH	WT./LN. FT	BAR	NO.	SIZE	TYPE STP	LENGTH	WT./LN. FT	BAR	NO.	SIZE	TYPE STD	LENGTH W	Γ./LN. FT. 1147			-	2'-2 ¹ ⁄2" K19
D1 	374	5	STR	5'-10"	2275	* B3 * B4	54	4	STR	33'-0"	1190	* B1 * B2	34	6	STR	42'-3"	2158			-	4'-9" K4
						B5	95	5	STR	48'-10"	4839	* B3	30	6	STR	48'-10"	2200			-	
** A1	365	6	STR	12'-2"	4441	B6	15	5	STR	57'-9"	903	* B4	34	4	STR	33'-0"	749				
** A2 * A3	365	<u> </u>	STR	21'-2"	8045	ж К1	4	8	1	11'-7"	124	B5 B6	55	5	STR	48'-10" 57'-9"	422				
A4	356	5	STR	21'-8"	8045	* K2	4	8	2	17'-0"	182		,				122				
						* K3	4	8	2	21'-6"	230	* K14	2	6	STR	2'-8"	8			GIRDERS.	
**A101	2	6	STR STP	11'-1"	22	* K4	4	8	1 <u> </u>	13'-0" 5' 3"	139	* K15	2	6	STR STP	3'-8" 4' 2"	11				2'-4
**A102 **A103	2	6	STR	8'-11"	18	** K5 ** K6	2	6	STR	6'-3"	13	* K10 * K17	4	6	STR	6'-1"	37				6'-2
* *A104	2	6	STR	7'-10"	16	* * K7	2	6	STR	7'-3"	15	* K18	4	8	STR	6'-1"	65				3'-1
**A105	2	6	STR	6'-9"	14	* K8	4	6	STR	5'-1"	31	* K19	4	8	1	8'-2"	87				
**A106 **Δ107	2	6	STR	5'-8" 4'-7"	9	* K9 * K10	4 4	6	STR	6'-1" 7'-1"	<u> </u>	* K20 * K21	4 4	8	2	17'-0" 11'-11"	182				0 3'-7
**A108	1	6	STR	3'-6"	4	** K11	4	6	STR	8'-4"	33	* K21	4	6	STR	2'-8"	16				
**A109	1	6	STR	2'-5"	2	* * K12	4	8	STR	8'-4"	33	* K23	4	6	STR	3'-8"	22			(26	
<u>ΨΨΛΟ01</u>	2	6		111.0"	24	* K13	8	6	STR	8'-1"	97	* K24	4	6	STR	4'-8"	28			<u> </u>	
**A201 **A202	2	6	STR	10'-8"	24	* K20 * K27	31	5	5	<u> </u>	302	* K25 * K26	62	5	2	8'-3"	533			6 ^{3,4} "	
**A203	2	6	STR	9'-8"	19							* K27	31	5	5	9'-4"	302			-	
**A204	2	6	STR	8'-7"	17	* S1	26	5	3	6'-4"	172		10								 1י ד <u>ו</u> ע ער
**A205 **∆206	2	6		/'-6" 6'-5"	<u>15</u> 17	* S2 * S2	26 24	4 4	4 5	<u>6'-8"</u> י11"	116 63	* S1 * S7	12 12	5 4	3 4	6'-4"	<u>/9</u> 53			-	
**A207	2	6	STR	5'-4"	11							* S2 * S3	24	4	5	3'-11"	63			-	6'-1 ¹ ⁄2" K3
**A208	1	6	STR	4'-3"	4	* G1	2	5	STR	25'-1"	52										3'-10 ¹ ⁄ ₂ " K20
**A209	1	6	STR	3'-2"	3	* G3	2	5	STR	9'-6"	20	* G2	2	5	STR	21'-11"	46				1'-9" K26
* A301	2	5	STR	20'-10"	43	* 1	48	4	6	1'-5"	45	↓ ★ 1	40	4	6	1'-5"	38			◄	
* A302	2	5	STR	19'-9"	41		1	S	TAGE I	I											
* A303	2	5	STR	18'-8"	39	BAR	NO.	SIZE	TYPE	LENGTH	WT./LN. FT										11
* A304 * A305	2	5	STR	16'-6"	37	+ D3	375	5	STR	5'-10"	2282	┨╞━━━		A33 AA							
* A306	2	5	STR	15'-5"	32	U4	375	5	SIR	5'-10"	2282	11	STA	GE I		ST	AGE II				
* A307	2	5	STR	14'-4"	30	* A5	361	5	STR	14'-11"	5616		<u>#1</u>		20 F						
* A308 * A309	2	5	STR STR	13'-3"	28	A6	363	5	STR	14'-11"	5648	POUR	#1 #1A		6.2	POUR #4 POUR #4A	39.8				
* A310	2	5	STR	11'-1"	23	+ + 4501	2	5	STR	14'-7"	30	POUR	#2	1	43.2	POUR #5	70.8		*	EPOXY C	OATED
* A311	2	5	STR	10'-0"	21	* A501 * A502	2	5	STR	13'-11"	29	POUR	#2A		6.9	POUR #5A	3.4			REINFOR	CING STEEL
* A312	2	5	STR STP	8'-11"	19	* A503	2	5	STR	13'-3"	28		#3		25.9	POUR #6	37.5		ste ste	REINFOR	CING STEEL
* A313 * A314	2	5	STR	6'-9"	10	+ A504	2	5	STR	12'-1"	25						37.3		**	GFRP RE	INFORCEMENT
* A315	2	5	STR	5'-8"	12	* A505 * A506	2	5	STR	10'-0"	23	TOTAL	<u>S ***</u>	2	62.7	TOTALS ***	167.4				
* A316	2	5	STR	4'-7"	10	* A507	2	5	STR	8'-11"	19		*** FI	NAL TO	TAL = 43	0.1 CUBIC YAF		SUPERS	STRUCTURE	BTLL ()F MATERIAL
* A317		5		5-0	4	* A508	2	5	STR	7'-10"	16	***(VANTT	IES FOR	BARRIE	R RAILS ARE N	OTINCLUDED				
A401	2	5	STR	21'-6"	45	* A509 * A510	2	5	STR	5'-8"	14	-						GFRP REINFORCEME	NT REINFORCI	NG STEEL	EPOXY COATED
A402	2	5	STR	20'-5"	43	* A511	2	5	STR	4'-7"	10	1						(LIN. FT.)	(LB)	5.)	(LBS.)
A403 A404	2	5 5	STR	19'-4"	40 38	+ A512	2	5	STR	3'-6"	7	4					STAGE I	9,230	16,5	513	23,234
A405	2	5	STR	17'-2"	36	- * A513		5		2-5"	3	1						0			10.100
A406	2	5	STR	16'-1"	34	A601	2	5	STR	14'-8"	31	1					STAGE II	U	11,3	5/0	16,168
A407	2	5	STR STP	15'-0" 15'-11"	<u>31</u> 20	A602	2	5	STR	13'-11"	29	4								-	
A409	2	5	STR	12'-10"	27	A603	2	5	STR	12'-10" 11'-9"	27	-			SUP		JRE REINFORC	ING STEEL			
A410	2	5	STR	11'-9"	25	A605	2	5	STR	10'-8"	22	1			F	OLLOWING	MIN. SPLICE				
A411	2	5	STR	10'-8"	22	A606	2	5	STR	9'-7"	20									-	
A412 A413	2	5	STR	<u> </u>	18	A607	2	5	STR	8'-6"	18	-			SUPERST	FRUCTURE					
A414	2	5	STR	7'-5"	15	A608	2	5	STR	6'-4"	13	-	BAR	S	EXCEPT A	APPROACH RAPET, AND	APPROAC	H SLABS	BARRIER RAILS	5	
A415	2	5	STR	6'-4"	13	A610	2	5	STR	5'-3"	11		SIZE		BARRI	ER RAIL					
A416 Δ <i>4</i> 17	2	5		<u>5'-3"</u> <u>גי</u> -י		A611	2	5	STR	4'-2"	9	4		EPOX	(Y COATE	D UNCOATE	D EPOXY COATED	UNCOATED	EPOXY COATED	2	
					т	A612	1	5	STR	3'-1"	3	J	#4		1'-11"	1'-7"	1'-11"	1'-7"	2'-6"		
* B1	81	4	STR	33'-8"	1822]							#5		2'-5"	2'-0"	2'-5"	2'-0"	3'-1" י סיי	_	
* B2	54	6		42'-3"	<u> 3427</u>	J							#0		2 -10 4'-2"	2-5			o- د -	-1	
DRAWN BY : CHECKED BY :		M.L. CA D. TUT		DATE	06/2023								#8		4'-9"	3'-2"	-	-	-		UMENT NOT CONSID
DESIGN ENGIN	EER OF RE	CORD:	D. TUTTL	E DATE	. 06/2023															SI	GNATURES COMPLET

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NOTES:

MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #9 "B" BARS IN STAGE I WITH THE #9 "B" BARS IN STAGE II. REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 1FT EXTENSION BEYOND THE CONSTRUCTION JOINT. THE CONTRACTOR SHALL ADJUST FABRICATED DIMENSIONS AS NECESSARY TO ACCOMMODATE THE COUPLER USED. SEE MECHANICAL BUTT SPLICES FOR REINFORCING STEEL IN STANDARD SPECIFICATIONS. NO ADDITIONAL PAYMENT WILL BE MADE FOR ANY ADJUSTMENTS.

STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHORS.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE OF THE END BENT CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE FRONT FACE AT THE RATE OF 2%.

THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE JOINT BETWEEN THE DECK AND THE APPROACH SLAB HAS BEEN SAWED AND THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

DURING STAGE I CONSTRUCTION, A ³/₄" TRIANGULAR BLOCKOUT (SEE DETAIL "C" ON SHEET 4 OF 5) WILL BE PLACED IN THE FRONT FACE OF THE BACKWALL AND FRONT AND TOP FACES OF THE CAP AT THE SAW CUT LOCATION TO FACILITATE SAW CUT AND SUCH THAT FINAL END BENT FACES MAY BE CHAMFERED ACCORDING TO THE STANDARD NOTES.

THREADED MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #9 "B" BARS ON EITHER SIDE OF THE SAW-CUT LINE BETWEEN THE PERMANENT AND OVERBUILT PORTIONS OF STAGE I. THE CONTRACTOR SHALL ADJUST THE FABRICATED DIMENSIONS OF THE REINFORCING AS NECESSARY TO ACCOMMODATE FOR THE COUPLER USED WHILE PROVIDING AT LEAST 3" CLEAR FROM COUPLER TO THE SAW-CUT LINE REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 3" EXTENSION BEYOND THE SAW-CUT LINE TO FACE OF COUPLER. DEBOND THE BAR BETWEEN THE COUPLER AND THE SAW-CUT LINE. #5 "B" BARS SHALL EXTEND THROUGH THE SAW-CUT LINE WITHOUT DEBONDING AND SHALL BE DRILLED OUT TO A DEPTH OF 2" DURING OVERBUILD REMOVAL. SEE "TEMPORARY OVERBUILD AND REMOVAL" SPECIAL PROVISION.

DURING STAGE III CONSTRUCTION, THE CONTRACTOR SHALL SAW CUT THE CAP AND BACKWALL. THE CAP SHALL BE CUT PARALLEL TO THE SKEWED ENDS OF THE CAP AND THE BACKWALL SHALL BE CUT NORMAL TO THE FILL FACE AS SHOWN ON THE PLANS. AFTER SAW CUTTING THE CAP AND BACKWALL THE CONTRACTOR SHALL REMOVE THE OVERBUILD PORTION, UNTHREAD AND REMOVE OVERBUILD REINFORCEMENT FROM MECHANICAL COUPLERS IN STAGE I, DRILL OUT #5B11, #5B12, AND #4K1 BARS TO A DEPTH OF 2", AND PATCH EXTERIOR FACE WITH AN APPROVED GROUT. SEE SPECIAL PROVISIONS FOR DETAILS.

CONTRACTOR SHALL DRILL 1" DIAMETER HOLE IN THE PILES TO PERMIT #4B15, #4B23, & #4B24 BARS TO PASS THROUGH AT THE LOCATIONS SHOWN ON SECTION A-A. THE HOLE SHALL BE DRILLED AFTER PILE IS INSTALLED AND CUT-OFF. FLAME CUTTING SHALL NOT BE PERMITTED.

FOR DETAIL "A", SEE SHEET 2 OF 5. FOR WING DETAILS, SEE SHEET 3 OF 5. FOR SECTION A-A, SEE SHEET 4 OF 5. FOR SECTION B-B, SEE SHEET 4 OF 5. FOR PILE SPLICE DETAILS SEE SHEET 5 OF 5. FOR TEMPORARY DRAINAGE DETAILS, SEE SHEET 5 OF 5. FOR CONSTRUCTION JOINT DETAILS, SEE "KEYED CONSTRUCTION JOINT DETAIL" ON SHEET 4 OF 5.

- (1) MECHANICAL COUPLER, SEE NOTES
- (2) #5B11, #5B12, #4B13, & #4B14 EXTENDED 3'-2" MIN. INTO STAGE II. FIELD BEND AS NEEDED TO AVOID **TEMPORARY SHORING.**
- (3) 2-#5S1 & 2-#5S2 @ 5¹/₂" CTS.
- (4) 2-#5S1 & 2-#5S2 @ 6" CTS.
- (5) 5 BAR PAIRS: #5S1 & #5S2, 2-#5S3 & 2-#5S4, #5S5 & #5S6 AND #5S7 & #5S8 @ 11" MAX (AS SHOWN IN DETAIL "B")
- (6) 5 BAR PAIRS: 2-#5S1 & 2-#5S2, #5S3 & #5S4, #5S5 & #5S6 AND #5S7 & #5S8 @ 11" MAX (AS SHOWN IN DETAIL "B")
- (7) 4 BAR PAIRS: #5S3 & #5S2, #5S3 & #5S4, #5S5 & #5S6 AND #5S9 & #5S10 @ 11" MAX (AS SHOWN ABOVE IN PLAN)
- (8) THREADED MECHANICAL COUPLER REQ'D (SEE NOTES) (TYP. AT SAW CUT LOCATION)
- (9) #4U3, #4U4, AND #4U5 @ 1'-6" MAX (AS SHOWN IN DETAIL "D")

FOR LOCATION OF ELEVATIONS BETWEEN
BRIDGE SEAT BUILDUPS, SEE SECTION A-A
ON SHEET 4 OF 5.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJEC	т _{NO.} <mark>В-3186</mark>	6 / B-5898
HA	YWOOD	COUNTY

24+70.00 -L LT-STATION:

SHEET 1 OF 5

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DRAWN BY :	D. R	RITACCO	DATE :	05/2023
CHECKED BY :	D.	TUTTLE	DATE :	06/2023
DESIGN ENGINEER	OF RECORD:	D. TUTTLE		06/2023

	BILL OF MATERIAL										
					END B	ENT	1				
		ST	AGE	I				ST	AGE	II	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	2	9	STR	25'-6''	173	B16	14	4	STR	4'-8''	44
B2	2	9	STR	26'-7''	181	B25	2	9	1	26'-10''	182
B3	2	9	STR	27'-0''	184	B26	2	9	1	25'-10''	176
B4	2	9	STR	28'-1''	191	B27	2	9	1	25'-6''	173
B5	1	9	STR	25'-6''	87	B28	2	9	1	24'-4''	165
B6	1	9	STR	26'-0''	88	B29	1	9	1	26'-10''	91
B7	1	9	STR	26'-9''	91	B30	1	9	1	26'-5''	90
B8	1	9	STR	27'-6''	94	B31	1	9	1	25'-8''	87
B9	1	9	STR	28'-1''	95	B32	1	9	1	24'-11''	85
BIO	13	9		10'-7''	468	B33		9		24'-4"	83
BII	4	5		38'-2''	159	B34	4	5		26'-5''	110
	4	5					4 5	5		240	100
B17	4	4		20-02	<i>//</i> 80	B37	5 1	4		4-0 25' 0''	60
B15	4	4		ט-טכ ייניסנ	67	B38	4	4		25-9	65
B16	18	4		10-1 /'_8''	56	B30	4	4		24-5	17
B17	1	4 	STR		16	B35 B40	1	4 4	STR	23-11	16
B18	1	4	STR	24 4	17						10
B19	1	4	STR	25'-8''	17	Н1	13	4	3	17'-1''	148
B20	1	4	STR	26'-4''	18	H2	13	4	3	17'-5"	151
B21	1	4	STR	26'-11''	18						
B22	5	4	STR	10'-0''	33	К3	6	4	STR	24'-0''	96
B23	1	4	STR	25'-0''	17	K4	6	4	STR	24'-2''	97
B24	1	4	STR	26'-4''	18	K5	2	4	STR	2'-9''	4
						K6	2	4	STR	2'-11''	4
H3	11	4	2	2'-2''	16	K7	8	4	STR	2'-0''	11
K1	24	4	STR	21'-4''	342	S1	34	5	4	5'-7''	198
K2	2	4	STR	10'-2''	14	S2	34	5	5	12'-9''	452
						S3	1	5	4	4'-9''	5
<u>S1</u>	44	5	4	5'-7"	256	<u>54</u>	1	5	5	11'-11"	12
<u>S2</u>	44	5	5	12'-9''	585	<u>55</u>	1	5	4	5'-0''	5
53	4	5	4	4'-9''	20	56	1	5	5		13
54	4	5	5		50	59	32	4	/	/-10	167
55	2	5	4	ט- כ ייכ יכ ו	20	S10 S11	2	6	6	2 -4 10' 1''	<u> </u>
50 57	2	5		12-Z 6' /''	20 13	S12	2 1	5		10-1 6'-0''	45
58	2	5	5	13'_6''	28	S12 S13	1	5	5	13'_2''	1/
50	40	<u> </u>	7	<u>13-0</u> 7'-10''	20	515				13-2	14
S12	1	5	4	6'-0''	6	U1	22	4	7	4'-2''	61
S13	1	5	5	13'-2''	14	U2	3	4	7	9'-2''	18
							_				_
U1	40	4	7	4'-2''	111	V1	44	5	STR	8'-11''	409
U2	21	4	7	9'-2''	129	V5	42	5	STR	10'-7''	464
U3	3	4	7	8'-4''	17						
U4	3	4	7	8'-5''	17						
U5	3	4	7	9'-11''	20						
V1	18	5	STR	8'-11''	167						
V2	39	5	STR	9'-4''	380						
V3	22	5	STR	10'-11''	250						
V4	2	5	STR	6'-6''	14						
	ORCING	STEEL		5	,127 LBS.	REINF	ORCING	STEEL		3	,970 LBS.
		CLASS	A CONC	CRETE				CLASS	A CONC	CRETE	<u> </u>
	#1 COLLAR	2			33.9 C.Y.		#1	S			25.0 C.Y.
	VER WIN	lgwall)					WER WI	NGWALL)		
	# 2				0101		#0				0 2 6 1
BACK	. #∠ ⟨WALL &	UPPER	WINGWA	LL)	0.1 C.Y.	BACK	#∠ WALL &	UPPER	WINGWA	ALL)	9.2 C.Y.
	~			,			~		• •		
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NOTE: SEE GEOTECHNICAL FOUNDATION TABLES FOR ADDITIONAL PILE INFORMATION

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NOTES:

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MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #10 "B" OR #11 "B" BARS IN STAGE I WITH THE #10 "B" OR BARS, RESPECTIVELY, IN STAGE II. REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 1FT EXTENSION BEYONI CONSTRUCTION JOINT. THE CONTRACTOR SHALL ADJUST FABRICATED DIMENSIONS AS NECESSARY TO ACCOMMOD COUPLER USED. SEE MECHANICAL BUTT SPLICES FOR REINFORCING STEEL IN STANDARD SPECIFICATIONS.

STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHORS.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN **REINFORCING STEEL".**

FOR DRILLED PIERS AND PERMANENT STEEL CASING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

SEE PILE AND DRILLED PIER FOUNDATION TABLES FOR DRILLED PIER FOUNDATION DATA. THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS ARE BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT AT A MINIMUM OF ONE FOOT BELOW THE GROUND LINE.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRIL PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

TEMPORARY OVERBUILD NOTES:

SEE BRIDGE 155 PLANS FOR COORDINATION. DURING THE CONSTRUCTION AND DEMOLITION OF BRIDGE 158 OVER PORTION OF BENT 1 OVER PROPOSED BRIDGE 155, THE CONTRACTOR SHALL ENSURE THAT NO DAMAGE SHALL OC THE PROPOSED BENT 1 OF BRIDGE 155. THE TEMPORARY SUPPORT SHALL BE REMOVED PRIOR TO THE ERECTION GIRDERS. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE SPECIAL PROVISION "TEMPORARY OVERBUILD AN REMOVAL".

THE TEMPORARY SUPPORT SHALL BE CAPABLE OF TRANSFERRING A VERTICAL LOAD OF (509 KIPS STRENGTH) TO 1 SUBSTRUCTURE. TRANSFER OF MOMENT IS NOT REQUIRED. POSITIVE RESTRAINT SHALL BE PROVIDED. SEE "TEMP OVERBUILD AND REMOVAL" SPECIAL PROVISION.

DURING STAGE I CONSTRUCTION, A 3/4" TRIANGULAR BLOCKOUT (SEE DETAIL "B" ON SHEET 1 OF 3) WILL BE PLAC THE SIDE AND BOTTOM FACES OF THE CAP AT THE SAW CUT LOCATION TO FACILITATE SAW-CUT AND SUCH THAT F BENT FACES MAY BE CHAMFERED ACCORDING TO THE STANDARD NOTES.

THREADED MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #10 "B" OR #11 "B" BARS ON EITHER SIDE OF T SAW-CUT LINE BETWEEN THE PERMANENT AND OVERBUILT PORTIONS OF STAGE 1. REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 1'-3" EXTENSION BEYOND THE SAW-CUT LINE. THE CONTRACTOR SHALL ADJUST THE FABR DIMENSIONS OF THE REINFORCING SUCH THAT THE COUPLER PROVIDES AT LEAST 3" CLEAR TO THE SAW-CUT LINE DEBOND THE BAR BETWEEN THE COUPLER AND THE SAW-CUT LINE. #5 "B" BARS SHALL EXTEND THROUGH THE SAW-CUT LINE. #5 LINE WITHOUT DEBONDING AND SHALL BE DRILLED OUT TO A DEPTH OF 2" DURING OVERBUILD REMOVAL. SEE "TEMPORARY OVERBUILD AND REMOVAL" SPECIAL PROVISION.

	BAR TYPES					В	ILL OF	MATEF	RIAL				
#11 "B"							BEN	T 1					
D THE DATE THE		S	TAGE	IA	ND C	VERBU	ILD			ST	AGE	II	
	1'-5" 20'-3" B3	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
		B4	8	<u> </u>	STR	25'-10" 42'-8"	1098	B1 B2	8	11 5	STR	20'-3"	861
J		B5 B6	6	10	1	27'-4"	706	B3	6	10	1	21'-1	559
		B7	6	4	STR	17'-0"	68	5.4.7	14	1.1	CTD		2000
		B8 B9	8	11	STR	15'-8" 15'-8"	666 404	IMT	14		SIR	28'-1"	2089
	$1\frac{1}{2}$ EXTRA TURNS \neg	B10	6	4	STR	14'-3"	57	V1	14	11	STR	18'-5"	1370
		B11	6	10	STR	6'-0"	155	S1	96	5	2	14'-7"	1460
		M2	14	11	STR	37'-1"	2758				_	,	
LED		M3	14	11	STR	46'-1"	3428	U1	33	4	3	8'-5"	186
		V1	28	11	STR	18'-5"	2740	U3	8	4	3	7'-3"	39
RBUILT		61	126										
CCUR TO OF		51	136	5	2	14'-7"	2069						
ND		U1	61	4	3	8'-5"	343						
	4 SPACERS	U3	16	4	3	7'-3"	77						6 927 PC
ORARY	3'-8" Ø	04	10	4	5	0 -4	09	SP-1		*	4	688'-2"	460
								SP-2	1	**	5	561'-8"	586
CED IN INAL		REINF	ORCING	STEEL *	4	1 688'-2"	5,103 LBS. 919	SPIRA REINI	AL COLUI FORCING	MN 5 STEEL			1 046 LBS
		SP-3	1	**	5	806'-9"	841	* -	THE SP-1	L SPIRAL	REINFC	RCING ST	EL SHALL
HE E		SP-4		**	5	1051'-10"	1097		BE W20	OR D-20	COLD I	DRAWN WI	RE OR #4
		REINF		STEEL			2,857 LBS.	<u>ykyk</u> -					
SAW-CUT		+ ⊤	HE SP-1	SPIRAL	. REINFC	RCING STE	EL SHALL	不不 (STEEL SH	HALL BE	W31 OF	R D-31 CO	LD DRAWN
		E				DRAWN WIF	RE OR #4			#5 PLA			BAR
	\sim	י אַאַ ד						POUR	#2 (COL	UMNS)	CREIEE	BREAKDOV	6.7 C.Y.
			TEEL SH	IALL BE	W31 OI	RAL REINF	D DRAWN	POUR	#3 (CAP	?)			21.2 C.Y.
	OF DRILLED	V		#5 PLA				ΤΟΤΑΙ					27 9 C V
	4 SPACERS - V	POUR	#2 (COL	UMNS)	CRETEI	BREAKDOW	13.5 C.Y.	TUTAL	CLASS /			RS:	27.9 C.T.
		POUR	#3 (CAP)			41.1 C.Y.		ED PIER		TE		12.1 C.Y.
		TOTAL	CLASS /	<u>a conc</u> Drii	RETE LED PIE	RS:	54.6 C.Y.	FOOR					
	ALL BAR DIMENSIONS ARE OUT TO OUT	DRILLE			ETE		40.1 C.Y.						
L		POUR	#1 (DRII	LED PI	ERS)								
								DE		τ ΝΟ	B-31	86 / B-	5898
ı								ГГ	UJEC HZ	1 1NU. 1V\ <i>\/(</i>			
		BENT 1 (CONTRO	L LINE.					1 17				
LUMN & LED PIER 2	COLUMN & DRILLED PIER 3		INS &	,				ST	OITA	N:	24+/\	0.00 -L	
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Ì						AECOM TECH 5438 WADE F	INICAL SERVICES OF NC, IN ARK BOULEVARD, SUITE 200 ALEIGH, NC 27607	C. 0	ULI AN				
_ <u>1</u>						(919) 854-6200 AEC	www.aecom. DM License No. F-0342	.com		SUE -	5 KU(LIUKE	
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- 15	-y ->					- CzcynA	SHANE			REVIS	SIONS		SHEET NO.

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TOTAL SHEETS

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NOTES:

MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #9 "B" BARS IN STAGE I WITH THE #9 "B" BARS IN STAGE II REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 1FT EXTENSION BEYOND THE CONSTRUCTION JOINT. THE CONTRACTOR SHALL ADJUST FABRICATED DIMENSIONS AS NECESSARY TO ACCOMMODATE THE COUPLER USED. SEE MECHANICAL BUTT SPLICES FOR REINFORCING STEEL IN STANDARD SPECIFICATIONS. NO ADDITIONAL PAYMENT WILL BE MADE FOR ANY ADJUSTMENTS.

STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHORS.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

THE TOP SURFACE OF THE END BENT CAP EXCEPT THE BRIDGE SEAT BUILDUPS SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE FRONT FACE AT THE RATE OF 2%.

THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE JOINT BETWEEN THE DECK AND THE APPROACH SLAB HAS BEEN SAWED AND THE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

DURING STAGE I CONSTRUCTION, A 3/4" TRIANGULAR BLOCKOUT (SEE DETAIL "B") WILL BE PLACED IN THE FRONT FACE OF THE BACKWALL AND FRONT AND TOP FACES OF THE CAP AT THE SAW CUT LOCATION TO FACILITATE SAW CUT AND SUCH THAT FINAL END BENT FACES MAY BE CHAMFERED ACCORDING TO THE STANDARD NOTES.

THREADED MECHANICAL COUPLERS SHALL BE USED TO JOIN THE #9 "B" BARS ON EITHER SIDE OF THE SAW-CUT LINE BETWEEN THE PERMANENT AND OVERBUILT PORTIONS OF STAGE I. THE CONTRACTOR SHALL ADJUST THE FABRICATED DIMENSIONS OF THE REINFORCING AS NECESSARY TO ACCOMMODATE FOR THE COUPLER USED WHILE PROVIDING AT LEAST 3" CLEAR FROM COUPLER TO THE SAW-CUT LINE. REINFORCING DIMENSIONS ARE PROVIDED ASSUMING A 3" EXTENSION BEYOND THE SAW-CUT LINE TO FACE OF COUPLER. DEBOND THE BAR BETWEEN THE COUPLER AND THE SAW-CUT LINE. #5 "B" BARS SHALL EXTEND THROUGH THE SAW-CUT LINE WITHOUT DEBONDING AND SHALL BE DRILLED OUT TO A DEPTH OF 2" DURING OVERBUILD REMOVAL. SEE "TEMPORARY OVERBUILD AND REMOVAL" SPECIAL PROVISION.

DURING STAGE III CONSTRUCTION, THE CONTRACTOR SHALL SAW CUT THE CAP AND BACKWALL. THE CAP AND THE BACKWALL SHALL BE CUT NORMAL TO THE FILL FACE AS SHOWN ON THE PLANS. AFTER SAW CUTTING THE CAP AND BACKWALL, THE CONTRACTOR SHALL REMOVE THE OVERBUILD PORTION, UNTHREAD AND REMOVE OVERBUILD REINFORCEMENT FROM MECHANICAL COUPLERS IN STAGE I, DRILL OUT #5B3, AND #4K1, #4K2, AND #4K3 TO A DEPTH OF 2", AND PATCH EXTERIOR FACE WITH AN APPROVED GROUT, FOR DETAILS OF STAGE III REMOVAL, SEE SPECIAL PROVISIONS.

CONTRACTOR SHALL DRILL 1" DIAMETER HOLE IN THE PILES TO PERMIT #4B4, #4B6, #4B9 & #4B10 BARS TO PASS THROUGH AT THE LOCATIONS SHOWN ON SECTION A-A. THE HOLE SHALL BE DRILLED AFTER PILE IS INSTALLED AND CUT-OFF. FLAME CUTTING SHALL NOT BE PERMITTED.

FOR WING DETAILS, SEE SHEET 3 OF 5. FOR SECTION A-A, SEE SHEET 4 OF 5. FOR SECTION B-B, SEE SHEET 4 OF 5 FOR PILE SPLICE DETAILS SEE SHEET 5 OF 5. FOR TEMPORARY DRAINAGE DETAILS, SEE SHEET 5 OF 5.

⊊ BRG.

2" Ø 2'-1" LONG ANCHOR BOLTS PROJECTING 7" ABOVE CAP (TYP.) (8 REQ'D IN STAGE I) (6 REQ'D IN STAGE II)

- **#9 "B" BARS MECHANICAL COUPLER**
- LOCATED 1'-0" FROM CONST. JT.
- #5B3 AND #4B5 EXTENDED 3'-2" INTO STAGE II.
- 2-#5S1 & 2-#5S2 @ 5" CTS.
- 2-#5S1 & 2-#5S2 @ 6" CTS.
- 3-#4V2 @ 11 1/2" C.T.S. (E.F.) 3-#4U1 @ 11 1/2" C.T.S.
- 9-#5V2 @ 1'-0" CTS. MAX (E.F.) 9-#5U1 @ 1'-0" CTS. MAX
- THREADED MECHANICAL COUPLER REQ'D (SEE NOTES) (TYP. AT SAW CUT LOCATION)
- ▲ FOR LOCATION OF ELEVATIONS BETWEEN BRIDGE SEAT BUILDUPS, SEE SECTION A-A ON SHEET 4 OF 5.

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THREADED BAR DETAIL

	PROJECT NO. <u>B-3186 / B-58</u> <u>HAYWOOD</u> COL STATION: <u>24+70.00 -L_L</u> SHEET 4 OF 5	398 JNTY _T-
AECOM TECHNICAL SERVICES OF NC, INC. 5438 WADE PARK BOULEVARD, SUITE 200 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	DEPARTMENT OF TRANSPORTAT RALEIGH SUBSTRUCTURE END BENT 2 SECTIONS & DETAILS	ION
SHANE	REVISIONS	SHEET NO.
ED 10/13/2023	NO. BY: DATE: NO. BY: DATE:	S3-43
)	1 3 2 <u>4</u>	SHEETS

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DRAWN BY :	D. R	ITACCO	DATE :	05/2023
CHECKED BY :	D.	TUTTLE	DATE :	06/2023
DESIGN ENGINEER	OF RECORD:	D. TUTTLE		06/2023

				E	BILL OF	MATER					
					END B	ENT	2				
		ST	AGE	I				ST	AGE	II	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	13	9	STR	29'-1''	1285	B7	14	4	STR	4'-8''	44
B2	13	9	STR	8'-4''	368	B9	1	4	STR	23'-2''	15
B3	8	5	STR	39'-7''	330	B10	1	4	STR	24'-6"	16
B4	2	5		28'-0''	58	BII B12	2	9		23'-1''	157
B5 B6	0 15	4 1	STR	→ → → → → → → → → → → → → → → → → → →	79	B12 B13	2	9 Q		24 -1	166
B7	18	4	STR	4'-8''	56	B13 B14	2	9	1	24-5	174
B9 B8	15	4	STR	4'-0''	40	B15	1	9	1	23'-1"	78
						B16	1	9	1	23'-6''	80
K1	12	4	STR	39'-0''	313	B17	1	9	1	24'-3''	82
K2	2	4	STR	10'-6''	14	B18	1	9	1	25'-0''	85
K3	2	4	STR	10'-1''	13	B19	1	9	1	25'-7''	87
						B20	4	5	STR	22'-7''	94
H3	11	4	3	2'-2''	16	B21	4	5	STR	25'-2''	105
						B22	4	4	STR	22'-11''	61
<u>S1</u>	51	5	4	5'-7''	297	B23	4	4	STR	24'-3''	65
52 50	51	5	5	12'-9''	6/8		6		СТР	241 101	165
- 29	40	4		/ -10	209	K4 K5	6	5		24 -10	155
LU1	20	Δ	7	/!_?"	100	K6		<u>З</u>		25-2 2'_11''	8
U2	15	4	7	9'-2''	92	K0 K7	8	4	STR	2'-11	11
	15		,		52						
V1	54	5	STR	8'-11''	502	H1	13	5	2	17'-8''	240
V2	25	5	STR	10'-6''	274	H2	13	5	2	18'-0''	244
						S1	31	5	4	5'-7''	181
						S2	31	5	5	12'-9''	412
						S3	1	5	4	4'-8''	5
						S4	1	5	5	11'-10"	12
						55		5		4'-11''	5
						50		5	5		13
						57		5	4 5	0-5 13י_5יי	1/
						50	32	<u> </u>	7	7'-10''	167
						S10	3	6	8	5'-4''	24
						S11	3	6	9	10'-1''	45
						U1	23	4	7	4'-2''	64
						V1	46	5	STR	8'-11''	428
						V3	42	5	STR	10'-7''	464
		<u>CTEEL</u>									100 1 50
	UKCING	SIEEL		4	,900 LBS.	KEINF	ORCING	SIEEL		ζ	129 LBS.
				DETE						DETE	
POUR (CAP,	#1 COLLAR	S & LOV	VER WIN	GWALL)	31.8 C.Y.	POUR (CAP,	#1 COLLAR	S & LOV	VER WIN	GWALL)	23.7 C.Y.
POUR (BACK	#2 WALL &	UPPER	WINGWA	NLL)	7.8 C.Y.	POUR (BACk	#2 (WALL &	UPPER	WINGWA	ALL)	9.4 C.Y.
<u> </u>				TOTAL =	39.6 C.Y.					TOTAL =	33.1 C.Y.
					-						

NOTE: SEE GEOTECHNICAL FOUNDATION TABLES FOR ADDITIONAL PILE INFORMATION

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FOR DETAILS NOT SHOWN, SEE GENERAL DRAWING.

ESTIMA	TED QUANTIT	[ES
BRIDGE @ STA. 24+70.00 -L_LT-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	489	543
END BENT 2	460	511
E	HAYWOO STATION: 24- SHEET 1 OF 1	D COUNTY +70.00 -L_LT-
AECOM TECHNICAL SERVICES OF NC, INC. 5438 WADE PARK BOULEVARD, SUITE 200 RALEIGH, NC 27607 (919) 854-6200 www.aecom.com AECOM License No. F-0342	STATE OF N DEPARTMENT OF RA STA	ORTH CAROLINA TRANSPORTATION ALEIGH ANDARD
CARO	RIP RA	P DETAILS

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DESIGN ENGINEER	OF RECORD:	D. TUTTLE	DATE :	06/2023
BESIGN ENGINEEN				

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DRAWN BY :	L. L	.EE	DATE :	06/2023
CHECKED BY :	S. NAT	ARAJAN	DATE :	06/2023
DESIGN ENGINEE	R OF RECORD:	D. TUTTLE	DATE :	06/2023

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NOTES

FOR NOTES, SEE SHEET 4 OF 4.

FOR SECTION N-N AND K-K, SEE SHEET 3 OF 4.

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NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTII MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE (TYPE 1 OR TYPE 4a) SHALL BE IN ACC THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI IN ACCORDANCE WITH STANDARD SPECIFICATIONS

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTS

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRI COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SL GRADED TO DRAIN THE WATER AWAY FROM THE FIL BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLAN

FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISI

CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL BACKFILL EXCAVATION HOLE AND GRADE TO DRAIN NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. **TEMPORARY DRAINAGE DETAIL**

DOCUMENT	NOT	CON	SIDERE	D
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SIGNATU	RES	COM	PLETED	

	BILL OF MATERIAL FOR ONE APPROACH SLAB (2 REQ'D)							
			S	TAGE	I			
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
	*A1	50	#4	STR	27'-0"	902		
LE AND SELECT	A2	52	#4	STR	27'-0"	938		
	*B1	43	#5	STR	24'-1"	1080		
ORDANCE WITH	B2	43	#6	STR	24'-8"	1593		
	* 11	24	#1	5	1' 5"	23		
SHALL BE	JT	24	#4			25		
SECTION 1010.		23	#5	6	3'-10"	92		
ALONG FILL	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							
SIDE EDGE OF	* EPOXY COATED BEINEORCING STEEL 2005 LBS							
IOR TO								
	CLASS AA CONCRETE 24.1 C. Y							
LAB SHALL BE	STAGE II							
NS.	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
	*A3	50	#4	STR	20'-11"	699		
IONS.	A4	52	#4	STR	20'-11"	727		
	*B1	38	#5	STR	24'-1"	955		
	B2	38	#6	STR	24'-8"	1408		
	*B3	4	#5	STR	6'-6"	27		
	B4	4	#6	STR	6'-6"	39		
	* J1	20	#4	5	1'-5"	19		
	REINF	ORCIN	NG STE	EL	2	174 LBS.		
	I * EP REINF	UXY C FORCII	OATED NG STF	EL	1	700 LBS		

CLASS AA CONCRETE

SECTION N-N

SPLICE LENGTHS						
BAR SIZE	EPOXY COATED	UNCOATED				
#4	1'-11"	1'-7"				
#5	2'-5"	2'-0"				
#6	3'-7"	2'-5"				

21.3 C. Y.

	PROJEC H STATIC	T NO. AYWC DN:	<u>B-</u>)O 24-	318 D +70.	6 / B (00 -L	-5898 COUNTY LT-
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THE COST OF THE BARRIER RAIL IN THE LINEAR FOOT CONTRACT

THE BARRIER RAIL ON EACH APP APPROACH SLAB CONCRETE HAS COMPRESSIVE STRENGTH OF 3,0

ALL REINFORCING STEEL IN BAR

SHIFT, BEND, OR CUT REINFORC BLOCKOUT.

#5D1 AND #5D2 SHALL BE MAC DELIVERED CONNECTED. THREA SEPARATION AFTER INSTALLATIC

THE #5D1 COUPLED TO #5D2 C CONCRETE AFTER POURING AND BE INSTALLED TO BE $\frac{3}{16}$ " BELOV ONCE DECK HAS CURED, #5D2 ASIDE. A TEMPORARY BOLT SHA TO PREVENT DEBRIS INTRUSION CONSTRUCTING THE STAGE III C AND TEMPORARY BOLT AND THR

PRIOR TO CURING, THE AREA UN BARRIER SHALL BE RAKE FINISH

THE CONTRACTOR MAY SUBMIT MEDIAN BARRIER INTO THE DEC CONSTRUCTION OF THE DECK. A CONSTRUCTION SHALL BE AT NO

S1, S2, D1, D2 AND D3 BARS IN SLIGHTLY, AS NECESSARY, TO PR JOINTS IN THE BARRIER.

THE CONTRACTOR SHALL SUBMI AND COUPLER PRIOR TO FABRIC

DO NOT GROOVE WITHIN 6" OF FOR MECHANICAL COUPLERS, SI

FINAL STAGE B

- INSTALL #5 "D" BARS (WIT LOCATIONS SHOWN ON "CO
- 2. INSTALL TEMPORARY BOLT PROJECT ABOVE THE TOP (
- 3. BOLT HEAD SHALL BE PRO CURING OF DECK. ENGINE
- 4. POUR AND SCREED DECK.
- 5. AT FINAL STAGE, REMOVE E THREADED END) FOR FINA

COST OF THE TEMPORARY BOL

	BILL OF MATERIAL							
NOTES		CON	CRETE	BARR	IER RAII			
	BAR	NO	SIZE	TYPE	I ENGTH	- WEIGHT		
PRICE BID FOR "CONCRETE BARRIER RAIL".	* B1	44	5	STR	9'-8"	444		
				511	5.0			
S BEEN CAST AND HAS REACHED A MINIMUM	* S1	66	5	1	5'-1"	350		
000 PSI.	* S2	62	5	2	7'-0"	453		
RIER RAILS SHALL BE EPOXY COATED.	* S3	4	5	2	5'-6"	23		
ING STEEL AS NECESSARY TO CLEAR IOINT								
ING STEELAS NECESSANT TO CELAN JOINT	* D1	96	5	4	2'-2"	217		
HINE THREADED TO FIT TO COUPLER AND	* D2	48	5	3	2'-0"	100		
DS SHALL BE GREASED TO PERMIT EASY	* D3	48	5	STR	3'-3"	163		
DN.	* EPOXY COATED 1750							
OR #5D3 SHALL BE PUSHED INTO GREEN	REI	NFORCING	G STEEL					
V THE TOP OF THE FINISHED CONCRETE DECK	CLA	SS AA CO	ONCRET	Ξ		19.0 C.Y.		
AND #5D3 SHALL BE UNTHREADED AND SET	CONCRETE BARRIER RAIL 70.0 L.F.							
LL BE INSTALLED IN THE COUPLER AND SEALED			BAI	ς τγρι	ES			
CONCRETE BARRIER, REMOVE SEALER								
READ #5D2 AND #5D3 IN TO COUPLER.		, 1'-	-0½",					
NDERNEATH THE STAGE III RIGHT CONCRETE		87/1	<u> </u>			<mark>⊢4"</mark>		
ED TO A SURFACE ROUGHNESS OF 716"		↓	5 ³ / ₄ "		T T			
ALTERNATIVE DETAILS FOR ANCHORING THE			$\overline{ }$	\mathbf{X}				
CK TO THE ENGINEER FOR REVIEW PRIOR TO ANY ALTERNATIVE METHODS APPROVED FOR		Ň.						
O ADD'L COST TO THE DEPARTMENT.		~~/_	-//)—	-		- 4		
THE BARRIER AND DECK SHALL BE SHIFTED	$1^{3/1}$	<	2 ¹ ⁄ ₄ "	14=				
ROVIDE 2" MIN. CLEARANCE TO ALL OPEN			RAD.	10				
					++!			
IT WORKING DRAWINGS FOR THE D1, D2, D3				S 🕂	83/4			
ATION TO ENSURE PROPER FIT.		8" (1			S3 S3		
EXPOSED BARRIER COUPLERS IN DECK SURFACE.					(\mathbf{r})	\mathbf{i}		
EE SPECIAL PROVISIONS.		1 /			Z)		
ADDTED COUDLED NOTEC		1'-0½" ◄	►	◀	2'-0"			
ARKIER COUPLER NUIES			ET:					
			t it					
ONCRETE BARRIER RAIL" SHEET.					6			
INTO COUPLER BOLT SHALL NOT	Ī					<u>T</u>		
OF THE DECK.	=	\tilde{g}^{γ}/\hbar	- FIELD					
TECTED TO ALLOW FOR REMOVAL AFTER	1^{1}	~~//	BEND		1.	1.		
ER SHALL APPROVE METHOD.								
	HK.							
				*				
L STAGE BARRIER.	7 ¹ /		ER <u>1'-</u>	5")		
T AND SEALING THE BOLT ARE INCIDENTAL					(
I AND SEALING THE BOEF ARE INCIDENTAL.		3)		(4)		5)		
ı ——▶				\bigcirc				
³ / ₄ "		ALL BA	R DIMEN	ISIONS A	ARE OUT TO	OUT.		
³ / ₈ " / / #5S2 @ 1'-0" CTS.	-							
						-000		
	PI	ROJEC	T NO.	B-31	80 / B-:	2898		
		ΗΔ				<u></u>		
		1 1/			C(JUNIY		
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