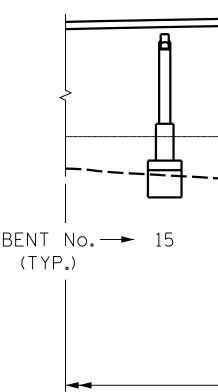
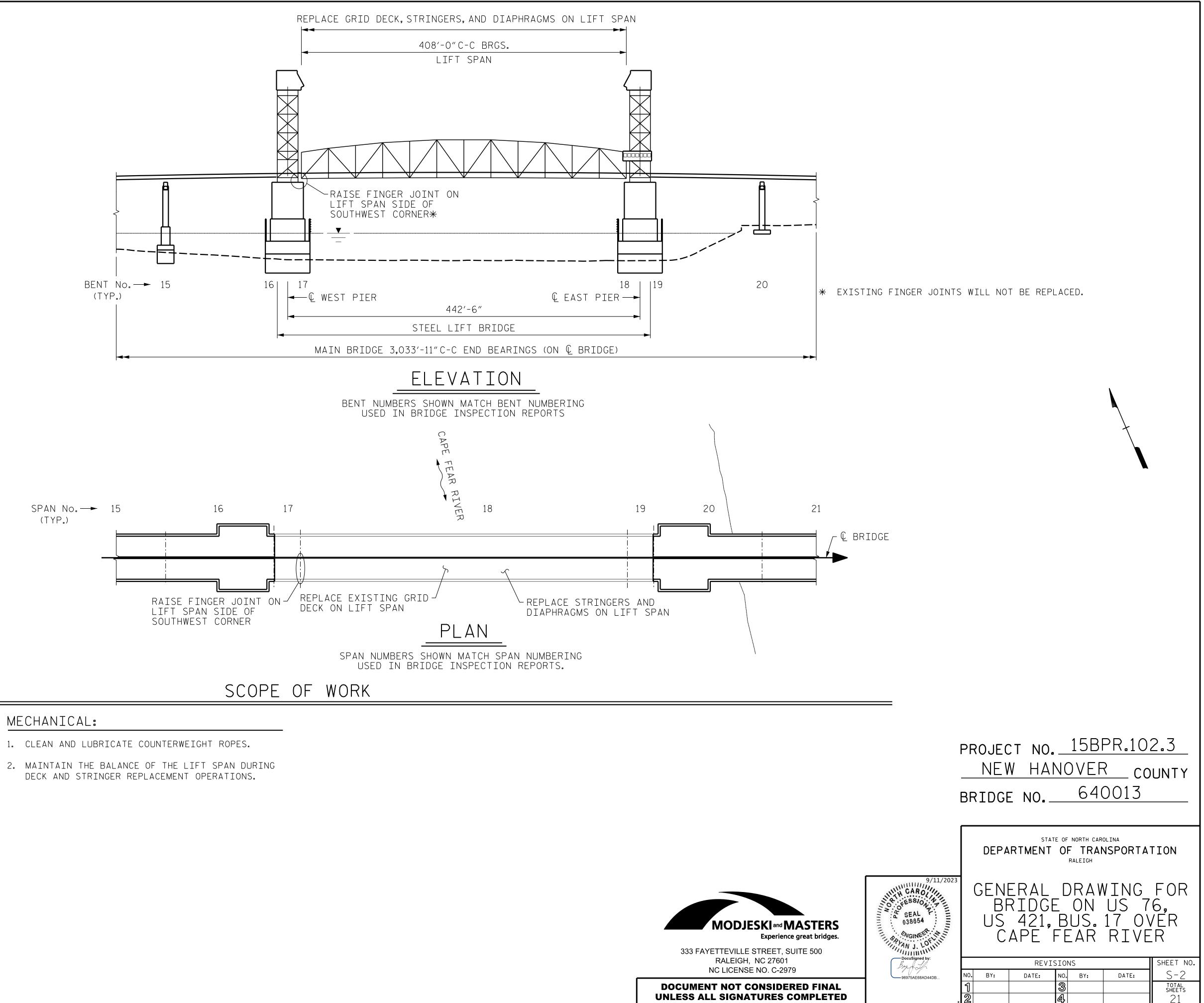


state N.C.	state 151	SHEET NO.	total sheets 21		
STAT	E PROJ. NO.	F. A. PROJ. NO.		DESCRIPT	ION
15B	PR.102.1			P.E.	
15BI	PR.102.3		C	ONSTRU	CTION







STRUCTURAL:

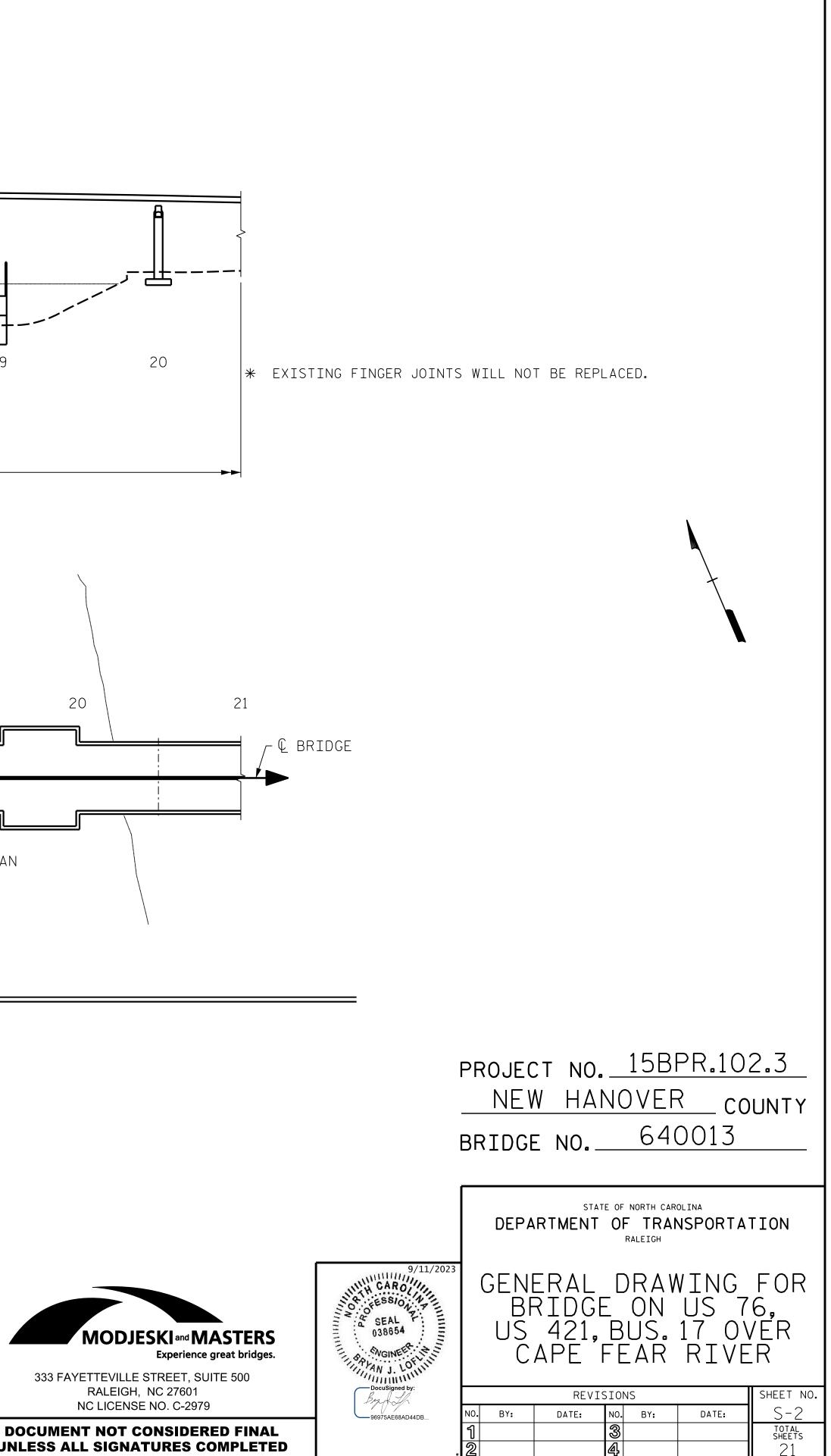
- 1. GRID DECK REPLACEMENT ON LIFT SPAN.
- 2. REPLACE STRINGERS AND DIAPHRAGMS ON LIFT SPAN.
- 3. RAISE FINGER JOINT ON LIFT SPAN SIDE AT SOUTHWEST CORNER OF THE LIFT SPAN.

MECHANICAL:



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.15BPF	DESIGNED BY: _	M.NIFONG	DATE : <u>JUL 2023</u>
3_1		R. JOHNSON	DATE : JUL 2023
, 8 1	DRAWN BY: _ Checked By: _	B.LOFLIN	DATE : <u>AUG 2023</u>
401_	DESIGN ENGINEE OF RECORD: _	R B.LOFLIN	DATE : <u>SEP 2023</u>





LOCATION SKETCH

INFORMATION ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY.

TOTAL BILL OF MATERIAL					
	REPLACEMENT OF STRUCTURAL STEEL	REPLACEMENT OF STEEL GRID DECK	UNDER STRUCTURE WORK PLATFORM	WIRE ROPE CLEANING AND RELUBRICATION	
	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	

3 5BPR . 102_SMU			
10/6/2023 401_005_15B	DESIGNED BY: DRAWN BY: CHECKED BY:	M.NIFONG R.JOHNSON B.LOFLIN	DATE : <u>JUL 2023</u> DATE : <u>JUL 2023</u> DATE : <u>AUG 2023</u>
10/(401.	DESIGN ENGINE	R B.LOFLIN	DATE : <u>SEP 2023</u>

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GENERAL NOTES:

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD V THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFE

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW AL AND FEDERAL SAFETY REQUIREMENTS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLAN.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR COAST GUARD COORDINATION, SEE SPECIAL PROVISION

ACCESS TO OPERATOR'S HOUSE FOR BRIDGE TENDER SHALL MAINTAINED AT ALL TIMES DURING CONSTRUCTION.

FOR MAINTENANCE OF WATER TRAFFIC, SEE SPECIAL PROVI

FOR SECURING OF VESSELS, SEE SPECIAL PROVISIONS.

WORK ON BRIDGE SHALL BE PERFORMED SO AS NOT TO ALL DEBRIS TO FALL BELOW. THE CONTRACTOR SHALL SUBMIT P FOR CONSTRUCTION IN ACCORDANCE TO ARTICLE 402-2 OF STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS.

PRIOR TO BEGINNING WORK, CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS F EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

PERFORM CHAPRY V-NOTCH TESTING ON NEW STRINGERS IN ACCORDANCE WITH ARTICLE 1072-7 OF THE STANDARD SPECIFICATIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PRON

FOR WORK IN, OVER, OR ADJACENT TO NAVIGABLE WATERS, SPECIAL PROVISIONS.

FOR STEEL TEMPORARY BARRIER RAIL, SEE TRAFFIC MANAC PLAN.

FOR WIRE ROPE CLEANING AND RELUBRICATION, SEE SPECI PROVISIONS.

ANY PAINT REMOVED DURING CONSTRUCTION OR PAINT DA DURING CONSTRUCTION SHALL BE CLEANED AND PAINTED I ACCORDANCE WITH SYSTEM 1 OF SECTION 442-8 OF THE ST SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS PAYMENT FOR THIS CLEANING AND PAINTING SHALL BE IN IN THE REPLACEMENT OF STRUCTURAL STEEL PAY ITEM.

THE CONTRACTOR'S ATTENTION IS BROUGHT TO THE FACT THERE IS CONDUIT CLAMPED TO THE BOTTOM FLANGE OF T STRINGERS NEAR FB2, FB6, AND FB10 THAT CARRIES THE W FOR THE NAVIGATION LIGHTS. CONTRACTOR SHALL SUBMIT REVIEW AND APPROVAL A METHOD TO TEMPORARILY SUPPO CONDUIT, TEMPORARILY REROUTE THE CONDUIT, OR PROVIDE TEMPORARY NAVIGATION LIGHTING. NAVIGATION LIGHTS S FUNCTIONAL AT ALL TIMES. CONDUIT SHALL BE RE-CLAMPE THE PROPOSED STRINGERS AFTER THEY ARE INSTALLED.

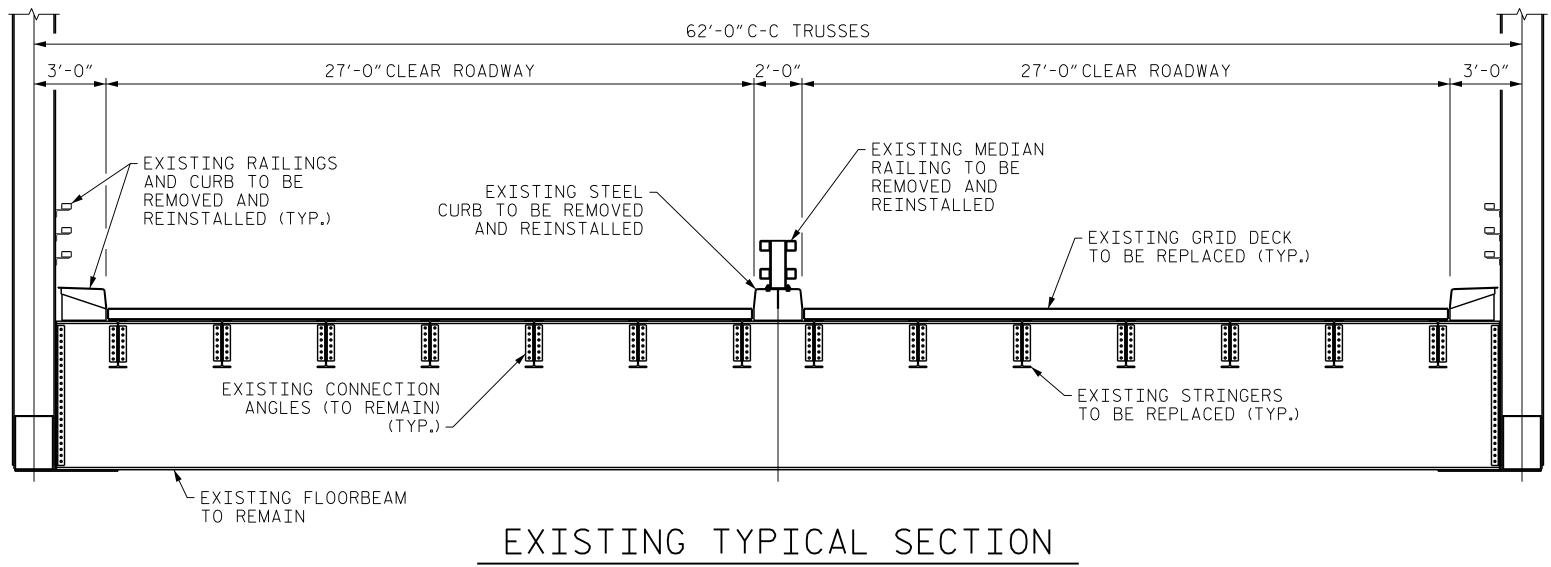
ALL FAYING SURFACES EXPOSED DURING CONSTRUCTION SH CLEANED AND PAINTED IN ACCORDANCE WITH THE STANDAR SPECIFICATIONS AND NCDOT STRUCTURAL STEEL SHOP COA PROGRAM.

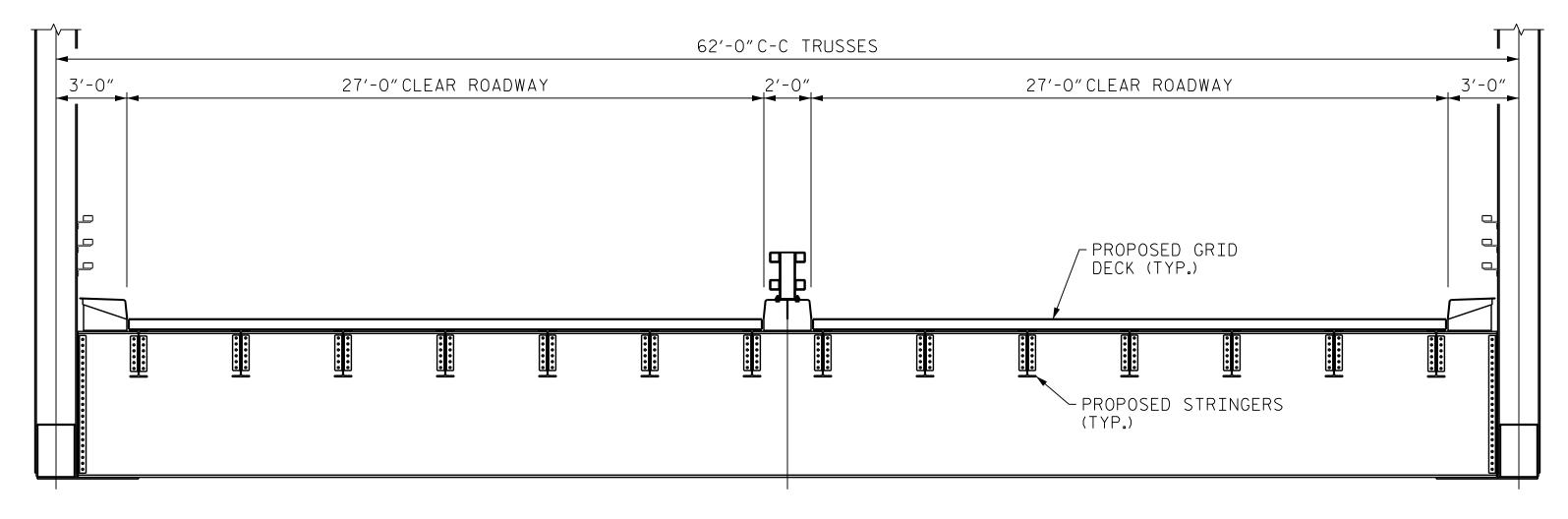


333 FAYETTEVILLE STREET, SU RALEIGH, NC 27601 NC LICENSE NO. C-2979

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

UITE 500 79 RED FINAL COMPLETED	NO. BY: DATE: NO. BY: DATE: S-3 96975AE68AD44DB 1 3 TOTAL SHEETS
ASTERS re great bridges.	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL NOTES AND TOTAL BILL OF MATERIAL DOCUSIONE DOCUSIONED IN DOCUSIONED IN DOCUSIONE I
ATINGS	BRIDGE NO. 640013
PED TO Shall be Ard	PROJECT NO. 15BPR.102.3 NEW HANOVER COUNTY
T FOR ORT THE DE SHALL BE	PROVISIONS.
THE	POSITION STRINGER MILL CAMBER UP. FOR REPLACEMENT OF STRUCTURAL STEEL, SEE SPECIAL
IS.	BECAUSE THE PROFILE OF THE GRID DECK IS DETERMINED BY THE VERTICAL LOCATION OF THE STRINGERS, EXTREME CARE SHOULD BE TAKEN WHEN LOCATING THE FIELD DRILLED HOLES FOR THE STRINGER CONNECTIONS TO ENSURE A SMOOTH GRID DECK PROFILE.
AMAGED IN	CONTRACTOR SHALL NOTIFY ENGINEER IF ANY CONNECTION ANGLES, OR OTHER EXISTING MEMBERS TO REMAIN, HAVE SECTION LOSS OF 25% OR GREATER. THE DEPARTMENT WILL SUPPLY (24) BLANK L4×4×3/8 STRINGER TO FLOORBEAM CONNECTION ANGLES.
AGEMENT	REMOVAL AND DISPOSAL OF EXISTING STRUCTURAL STEEL SHALL BE INCLUDED IN THE REPLACEMENT OF STRUCTURAL STEEL PAY ITEM.
DVISIONS. , SEE	THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE WILL NOT BE DAMAGED.IF THE CONTRACTOR DAMAGES ANY PART OF THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE, THE DAMAGED AREA SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE ENGINEER AT NO ADDITIONAL COST TO THE DEPARTMENT.
IN	THE STRUCTURE DURING CONSTRUCTION FOR REVIEW AND APPROVAL.
FOR FOR R	THE COLOR OF THE FINAL PAINT COATING SHALL MATCH THE COLOR OF THE EXISTING STRUCTURAL STEEL ON THE LIFT SPAN AND SHALL BE SUBMITTED FOR REVIEW AND APPROVAL.
PLANS	FOR REQUIREMENTS OF MAINTAINING BALANCE OF THE LIFT SPAN DURING CONSTRUCTION, SEE MECHANICAL SPECIAL PROVISION. COST SHALL BE INCLUDED IN THE REPLACEMENT OF STRUCTURAL STEEL PAY ITEM.
	FOR UNDER STRUCTURE WORK PLATFORM, SEE SPECIAL PROVISIONS.
ONS. _ BE /ISIONS.	INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICES FOR ITEMS ASSOCIATED WITH REPLACEMENT OF STRUCTURAL STEEL.
.	ALL STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 50 AND PAINTED IN ACCORDANCE WITH SYSTEM 1 OF SECTION 442-8 OF THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.
ER. All state	STRUCTURAL STEEL WILL BE SUPPLIED BY THE DEPARTMENT FOR REPLACEMENT OF THE STRINGERS AND DIAPHRAGMS.CONTRACTORS COST FOR REPLACEMENT OF STRUCTURAL STEEL SHOULD NOT INCLUDE THE COST TO SUPPLY THE STRUCTURAL STEEL.
	GRID DECK WILL BE SUPPLIED BY THE DEPARTMENT.FOR REPLACEMENT OF STEEL GRID DECK,SEE SPECIAL PROVISIONS.





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./2023 _007_15BPR	DESIGNED BY:	M.NIFONG	DATE : JUL 2023
7_1	DRAWN BY:	R. JOHNSON	DATE : JUL 2023
00/20	CHECKED BY:	B.LOFLIN	DATE : AUG 2023
9/8, 401_	DESIGN ENGINEE OF RECORD:	R B.LOFLIN	DATE : <u>SEP 2023</u>

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

333 FAYETTEVILLE STREET, SL RALEIGH, NC 27601 NC LICENSE NO. C-2979



PROPOSED TYPICAL SECTION

NOTES:

FOR GRID DECK DETAILS, SEE SHEET NO.S-9 THRU S-11.

FOR STRINGER DETAILS, SEE SHEET NO. S-12 THRU S-18.

EXISTING RAILINGS ALONG THE EXTERIOR AND MEDIAN OF THE LIFT SPAN, WILL BE REMOVED DURING CONSTRUCTION, STORED, AND REATTACHED. SEE SHEET NO. S-19 FOR ADDITIONAL DETAILS AND S-5 AND S-6 FOR CONSTRUCTION STAGING REQUIREMENTS.

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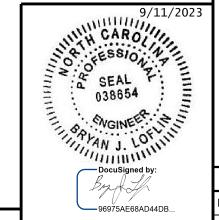
REFER TO TRAFFIC MANAGEMENT PLANS FOR TRAFFIC PHASING AND WORK ZONE INFORMATION.

PROJECT NO. 15BPR.102.3 NEW HANOVER COUNTY BRIDGE NO. 640013

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

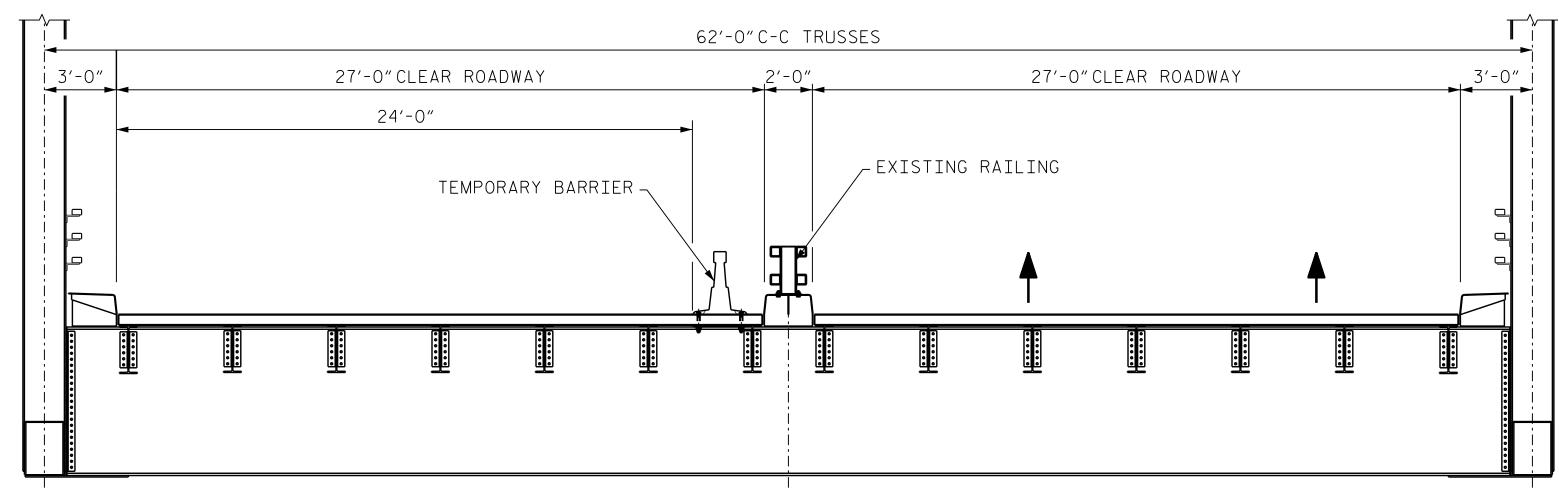
LIFT SPAN TYPICAL SECTION

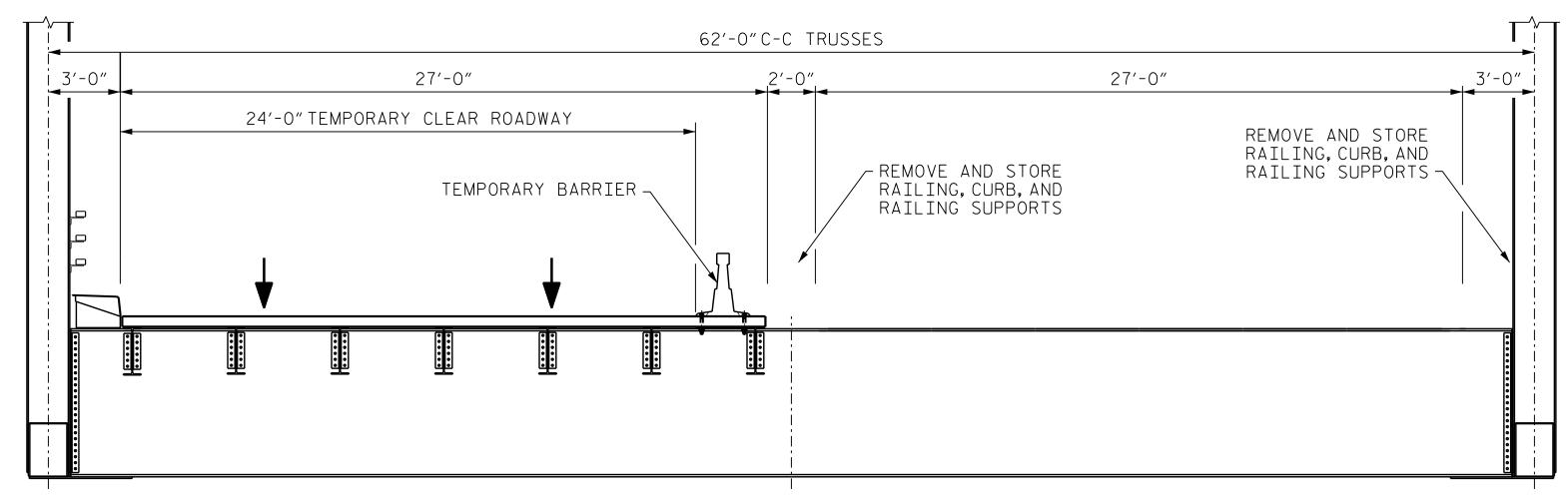
REVISIONS					SHEET NO.	
0.	BY:	DATE:	NO.	BY:	DATE:	S-4
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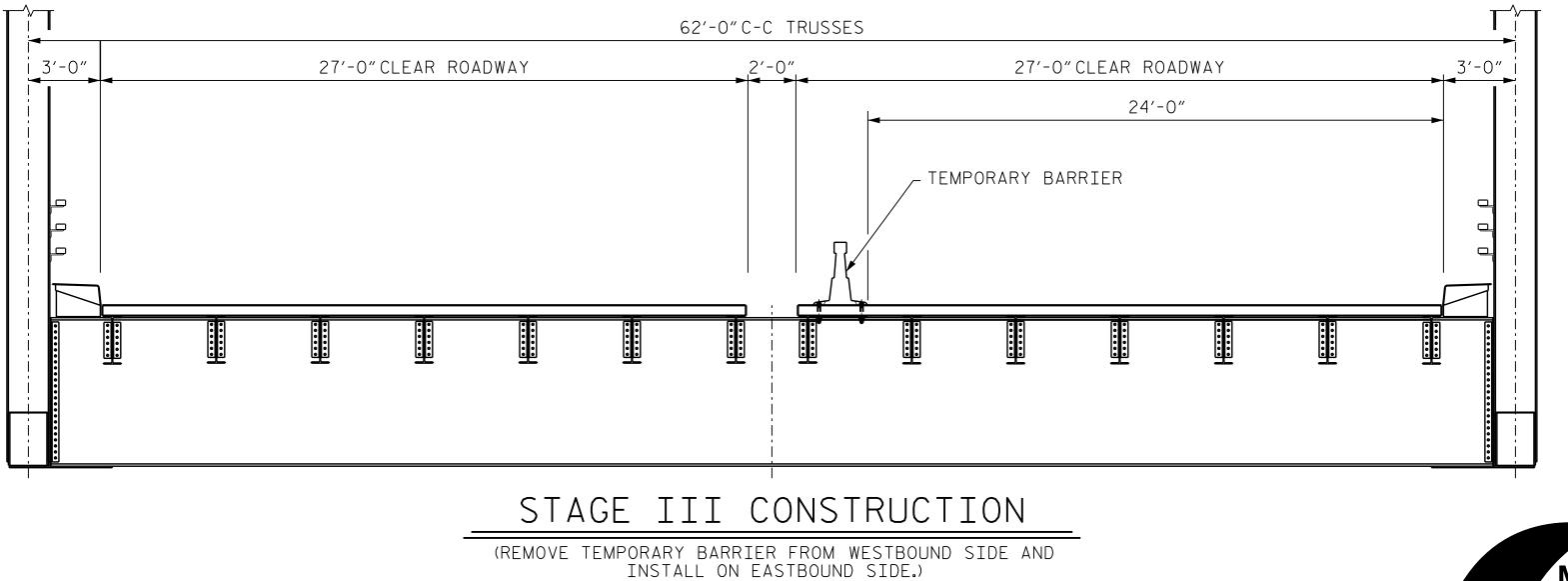
ASTERS great bridges.
JITE 500

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PR			
15B	DESIGNED BY:	M.NIFONG	DATE : JUL 2023
	DRAWN BY:	R. JOHNSON	DATE : JUL 2023
00.00	CHECKED BY:	B.LOFLIN	DATE : <u>AUG 2023</u>
401-	DESIGNED BY: DRAWN BY: CHECKED BY: DESIGN ENGINEE OF RECORD:	ER B.LOFLIN	DATE : <u>SEP 2023</u>

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STAGE I CONSTRUCTION

(INSTALL TEMPORARY BARRIER ON WESTBOUND DIRECTION.)

STAGE II CONSTRUCTION

(PERFORM EASTBOUND WORK.REMOVE AND STORE MEDIAN AND EASTBOUND EXTERIOR RAILINGS,CURBS,AND RAILING SUPPORTS.REPLACE STRINGERS AND GRID DECK. REINSTALL EXTERIOR EASTBOUND RAILING, CURB, AND RAILING SUPPORTS.)

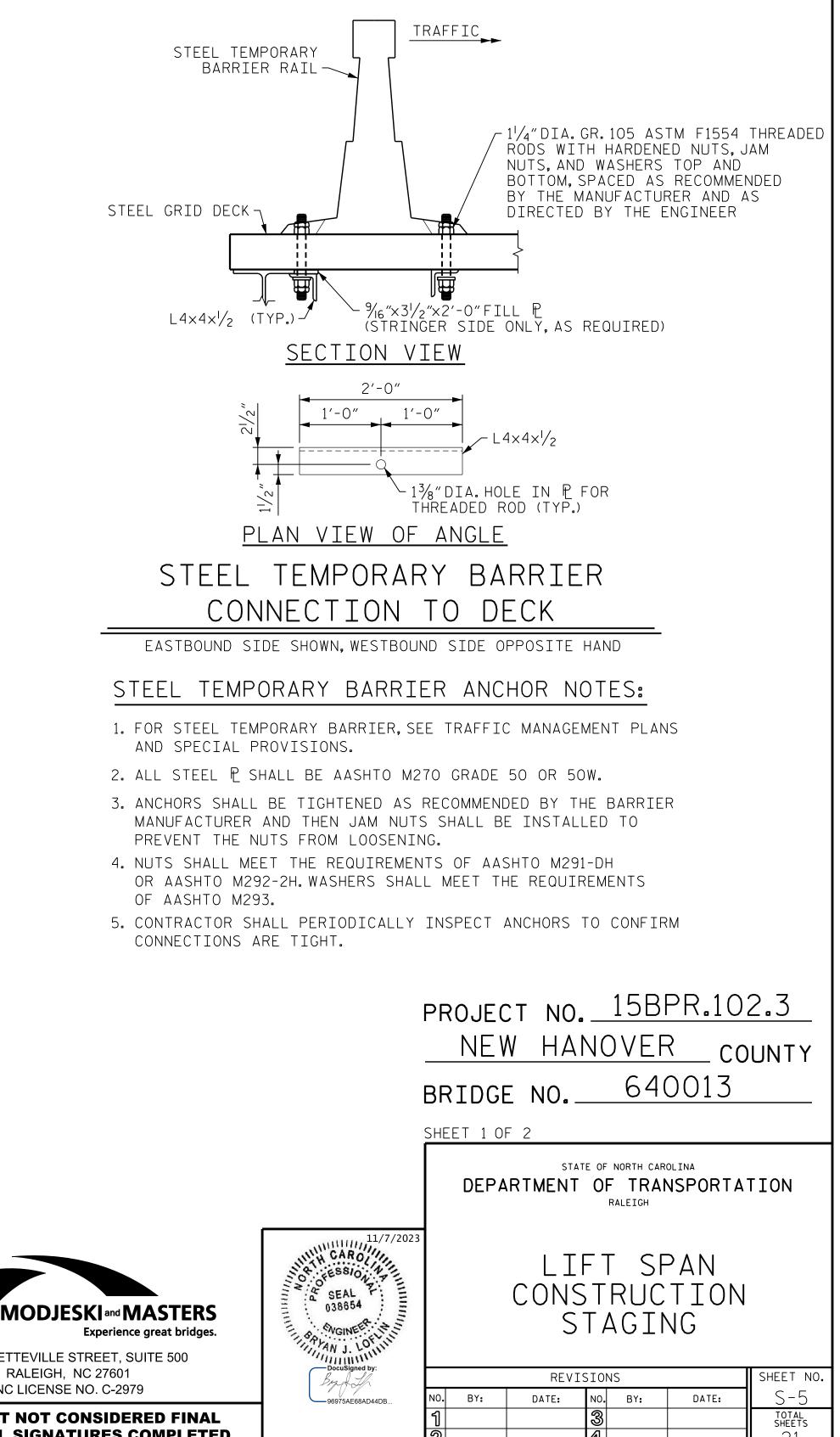
333 FAYETTEVILLE STREET, SUITE 500 RALEIGH, NC 27601 NC LICENSE NO. C-2979

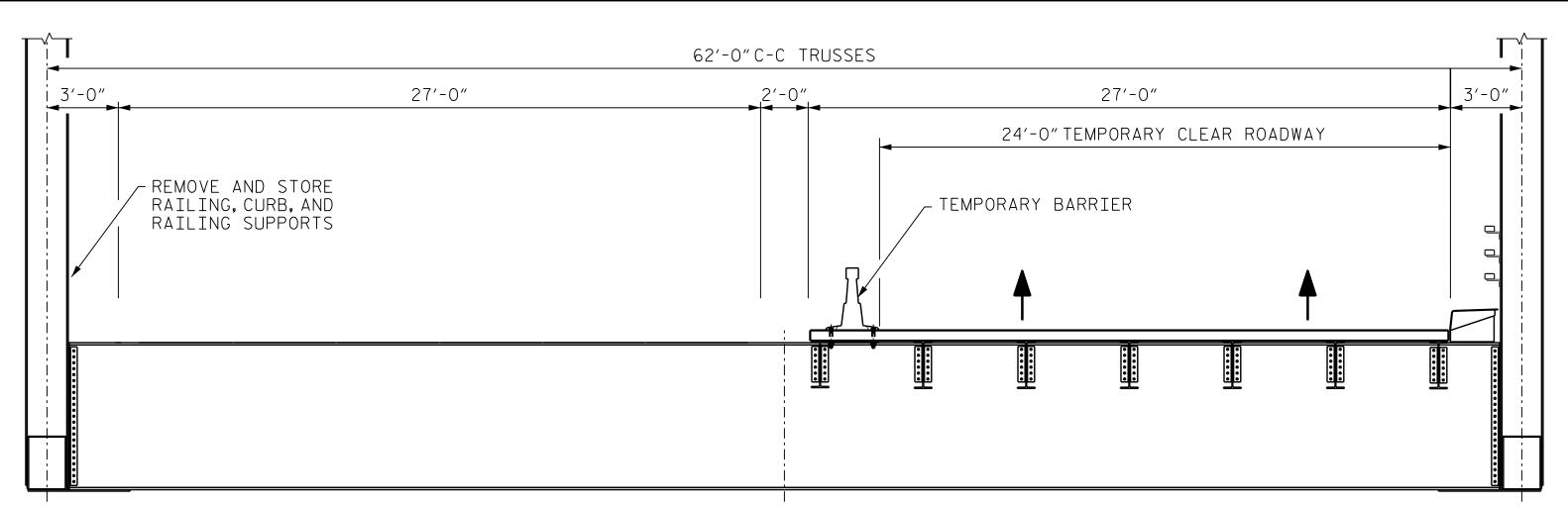
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

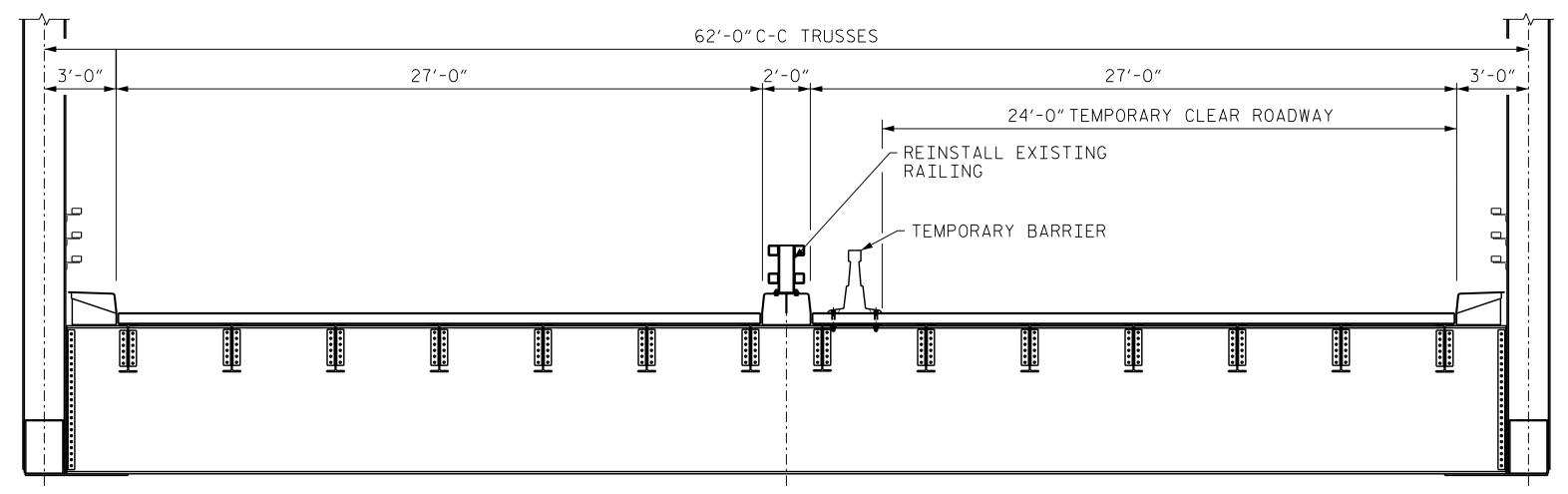
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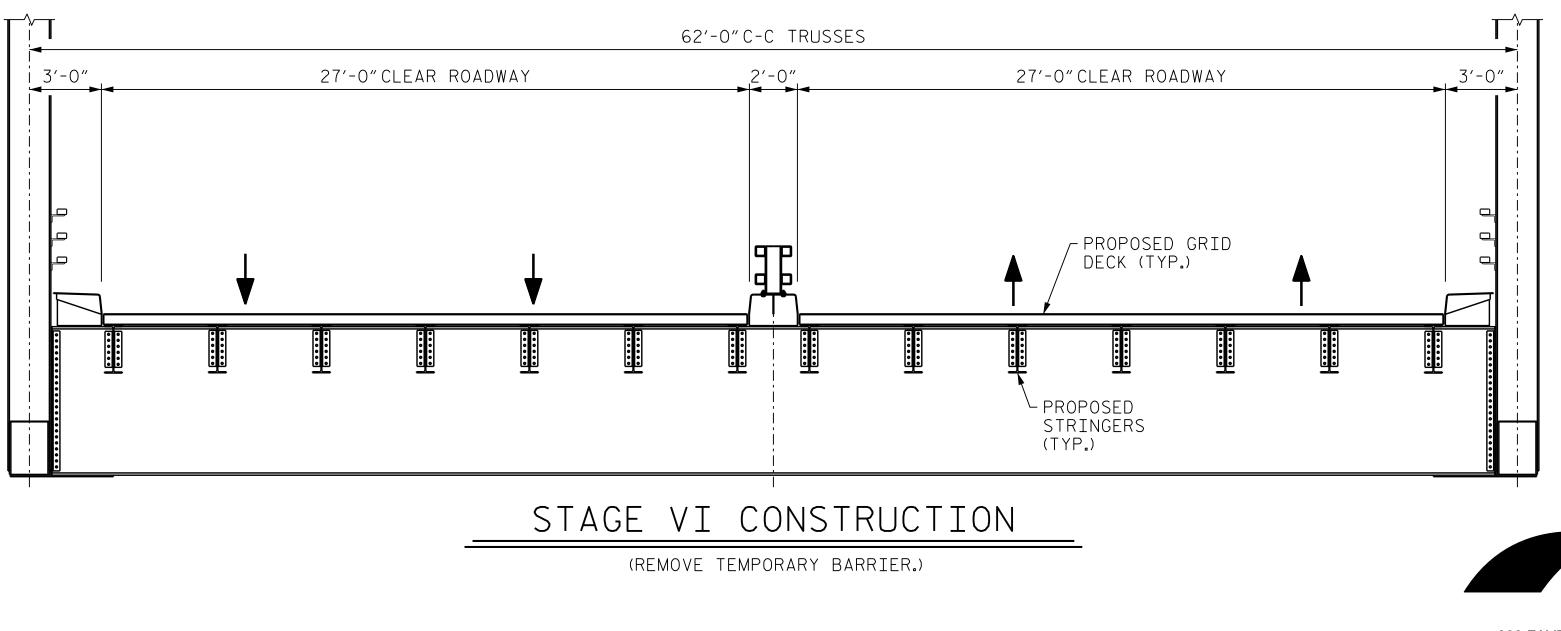
REFER TO TRAFFIC MANAGEMENT PLANS FOR TRAFFIC PHASING AND WORK ZONE INFORMATION.

CONTRACTOR SHALL HAVE NO MORE THAN TWO ADJACENT BAYS OF STRINGERS AND GRID DECK REMOVED AT ONE TIME.









BPR.102			
11/6/2023 401_011_15BF	DESIGNED BY: DRAWN BY: CHECKED BY:	M.NIFONG R.JOHNSON B.LOFLIN	DATE : <u>JUL 2023</u> DATE : <u>JUL 2023</u> DATE : <u>AUG 2023</u>
11/i 401	DESIGN ENGINER OF RECORD:	ER B.LOFLIN	_ DATE : <u>SEP 2023</u>

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STAGE IV CONSTRUCTION

(PERFORM WESTBOUND WORK.REMOVE AND STORE WESTBOUND EXTERIOR RAILING, CURBS,AND RAILING SUPPORTS.REPLACE STRINGERS AND GRID DECK.REINSTALL EXTERIOR WESTBOUND RAILING, CURB, AND RAILING SUPPORTS)

STAGE V CONSTRUCTION

(REINSTALL EXISTING MEDIAN RAILING, CURBS, AND RAILING SUPPORT.)

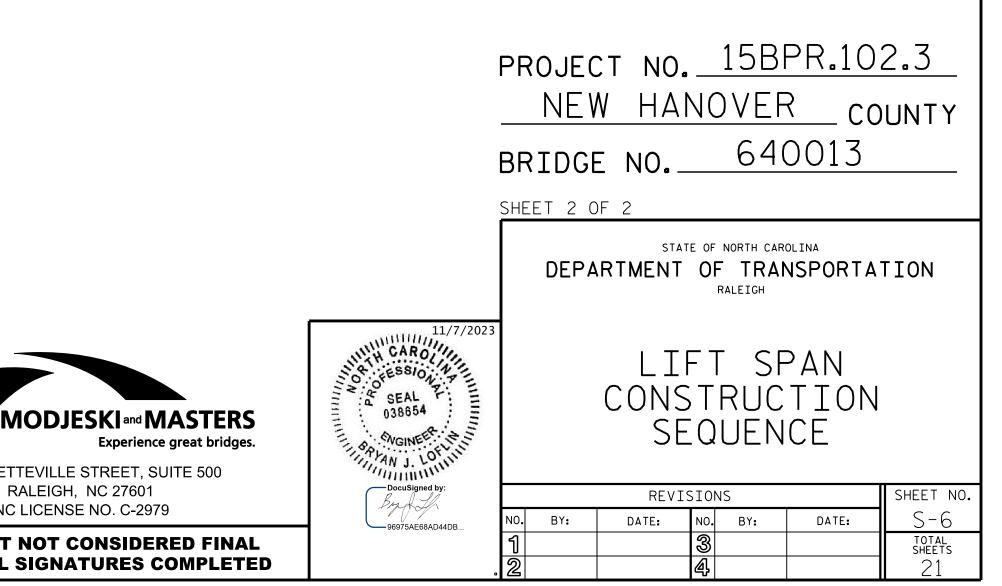
333 FAYETTEVILLE STREET, SUITE 500 RALEIGH, NC 27601 NC LICENSE NO. C-2979

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES:

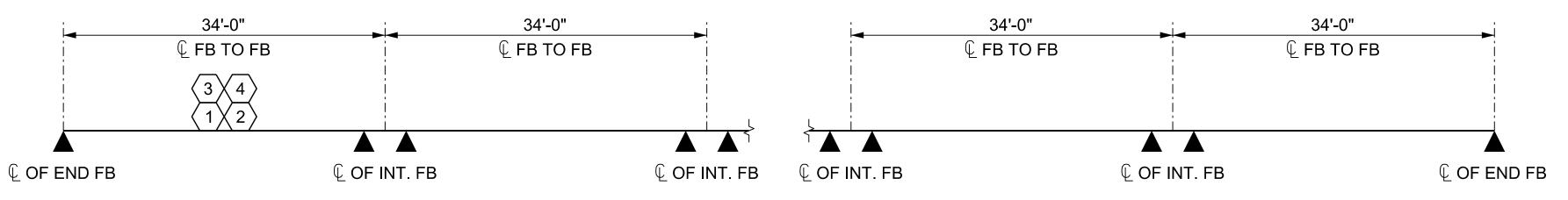
FOR TEMPORARY BARRIER ANCHOR DETAILS, SEE SHEET NO. S-5.

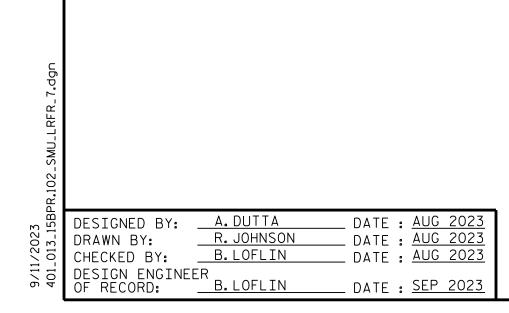
REFER TO TRAFFIC MANAGEMENT PLANS FOR TRAFFIC PHASING AND WORK ZONE INFORMATION.



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	LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR STEEL STRINGERS																						
										STF	RENG	TH I LIMIT	T STATE				SERVICE II LIMIT STATE				Ξ		
										MC	OMEN ⁻	Г		SHE	EAR					MOM	IENT		Ľ Ľ
I OAD TYPE		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W x RF	LIVE-LOAD FACTORS (7 LL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF) RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	(1)	1.33	-	1.75	0.433	1.33	1		17	0.6 2.10	1	I	0	1.30	0.433	1.43	1	I	17	1
DESI	GN	HL-93 (OPERATING)	N/A		1.72	-	1.35	0.433	1.72	1		17	0.6 2.72	1	I	0	N/A	-	-	-	-	-	1
LOA	۷D	HS-20 (INVENTORY)	36.000	2	1.72	62.1	1.75	0.433	1.72	1		17	0.6 2.42	1	I	0	1.30	0.433	1.86	1	I	17	
		HS-20 (OPERATING)	36.000		2.24	80.5	1.35	0.433	2.24	1		17	0.6 3.14	1	I	0	N/A	-	-	-	-	_	
		SNSH	13.500		3.26	44.0	1.40	0.433	3.78	1		17	0.6 6.35	1	I	0	1.30	0.433	3.26	1	I	17	
	Щ	SNGARBS2	20.000		2.72	54.5	1.40	0.433	3.16	1		17	0.6 4.77	1	I	0	1.30	0.433	2.72	1	I	17	
	VEHICLE SV)	SNAGRIS2	22.000		2.71	59.5	1.40	0.433	3.14	1	I	17	0.6 4.53	1	I	0	1.30	0.433	2.71	1	I	17	
		SNCOTTS3	27.250		1.65	44.9	1.40	0.433	1.91	1	I	17	0.6 3.21	1	I	0	1.30	0.433	1.65	1	I	17	
	GLE \ (S)	SNAGGRS4	34.925		1.50	52.5	1.40	0.433	1.75	1		17	0.6 2.83	1	I	0	1.30	0.433	1.50	1	I	17	
		SNS5A	35.550		1.47	52.4	1.40	0.433	1.71	1	I	17	0.6 2.97	1	I	0	1.30	0.433	1.47	1	I	17	
	SIN	SNS6A	39.950		1.42	56.6	1.40	0.433	1.64	1	I	17	0.6 2.80	1	I	0	1.30	0.433	1.42	1	I	17	
LEGAL		SNS7B	42.000	3	1.35	56.8	1.40	0.433	1.57	1		17	0.6 2.86	1	I	0	1.30	0.433	1.35	1	I	17	
LOAD		TNAGRIT3	33.000		1.76	58.0	1.40	0.433	2.04	1		17	0.6 3.26	1	I	0	1.30	0.433	1.76	1	I	17	
	R R R	TNT4A	33.075		1.75	58.0	1.40	0.433	2.04	1		17	0.6 3.09	1	I	0	1.30	0.433	1.75	1	I	17	
	CTC	TNT6A	41.600		1.53	63.6	1.40	0.433	1.77	1		17	0.6 3.05	1	I	0	1.30	0.433	1.53	1	I	17	
	RA(ST)	TNT7A	42.000		1.55	65.3	1.40	0.433	1.80	1		17	0.6 2.80	1	I	0	1.30	0.433	1.55	1	I	17	
		TNT7B	42.000		1.55	65.0	1.40	0.433	1.80	1		17	0.6 2.72	1	I	0	1.30	0.433	1.55	1	I	17	
	U U U U	TNAGRIT4	43.000		1.54	66.2	1.40	0.433	1.79	1		17	0.6 2.60	1	I	0	1.30	0.433	1.54	1	I	17	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGT5A	45.000		1.46	65.6	1.40	0.433	1.69	1		17	0.6 2.73	1	I	0	1.30	0.433	1.46	1	I	17	
		TNAGT5B	45.000		1.39	62.4	1.40	0.433	1.61	1		17	0.6 2.46	1	I	0	1.30	0.433	1.39	1		17	
FATI	GUE	HL-93 (INVENTORY)	γLL=0.75		1.05	-	-	0.361	1.05	1		17		-	-	-	-	-	-	-	-	_	2
EMERO	GENCY	EV2	28.750		2.45	70.5	1.30	0.433	2.45	1		17	0.6 3.63	1		0	-	-	-	-	-	=	
VEHICL	E (EV)	EV3	43.000	4	1.62	69.5	1.30	0.433	1.62	1		17	0.6 2.47	1	I	0	-	-	-	-	-	_	





LRFR SUMMARY



DOCUMENT NOT CONSIDERED UNLESS ALL SIGNATURES COMP

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γDC	γDW
	STRENGTH I	1.25	1.50
	SERVICE II	1.00	1.00

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NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE II LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE II LIMIT STATE ARE AS REQUIRED FOR DESIGN.

FB = FLOORBEAM

COMMENTS:

- 1. HL-93 IS CONTROLLED BY THE DESIGN TANDEM FOR MOMENT AND THE DESIGN TRUCK FOR SHEAR.
- 2. NOTE THAT THE STRINGER FATIGUE I STRESS IS BELOW THE INFINITE FATIGUE THRESHOLD.

 $\langle \# \rangle$

CONTROLLING LOAD RATING

- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- $\langle 3 \rangle$ LEGAL LOAD RATING * *
- $\langle 4 \rangle$ EMERGENCY VEHICLE LOAD RATING

* * SEE CHART FOR VEHICLE TYPE

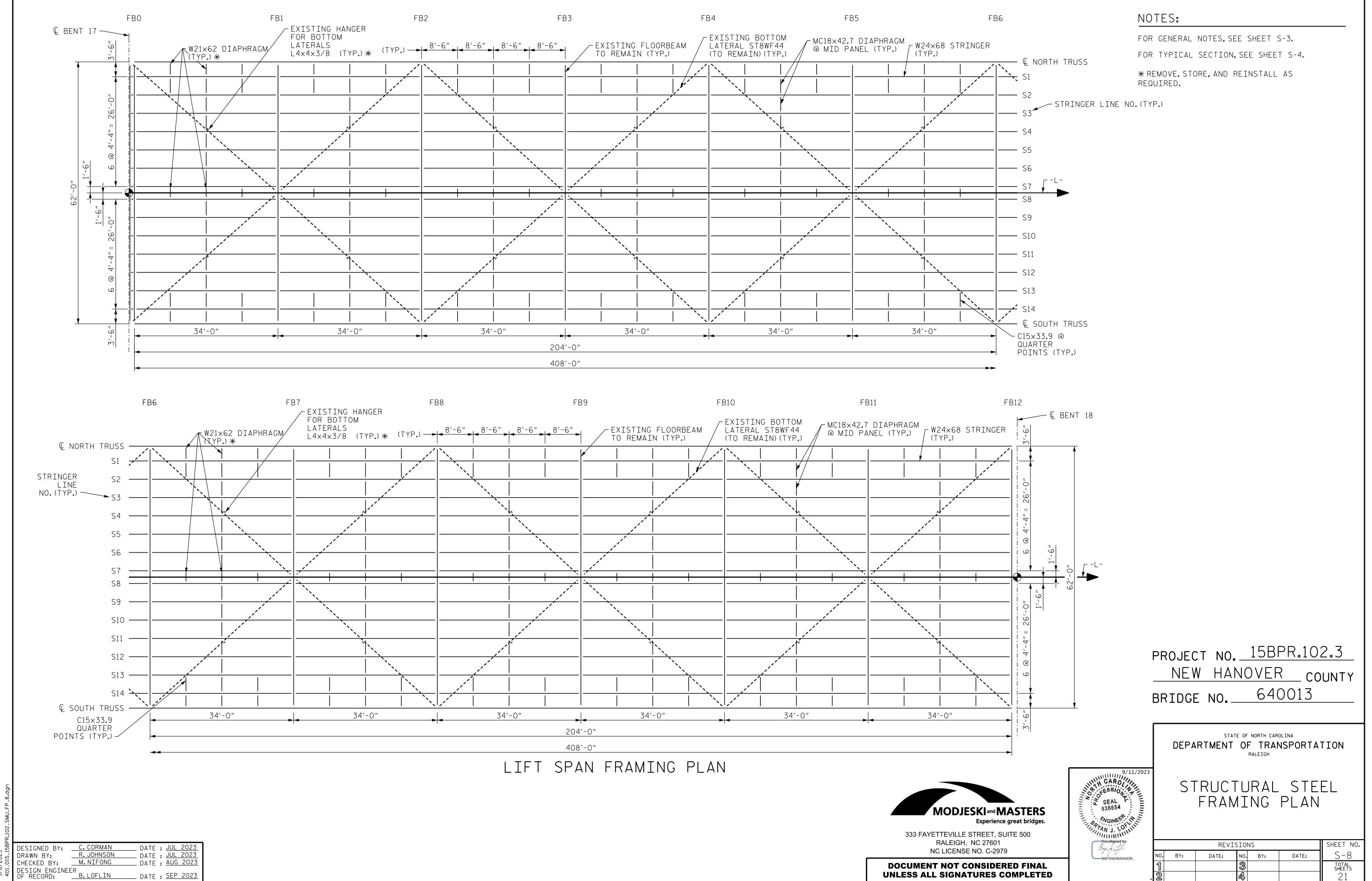
STRINGER LOCATION

- I INTERIOR STRINGER
- EL EXTERIOR LEFT STRINGER
- ER- EXTERIOR RIGHT STRINGER

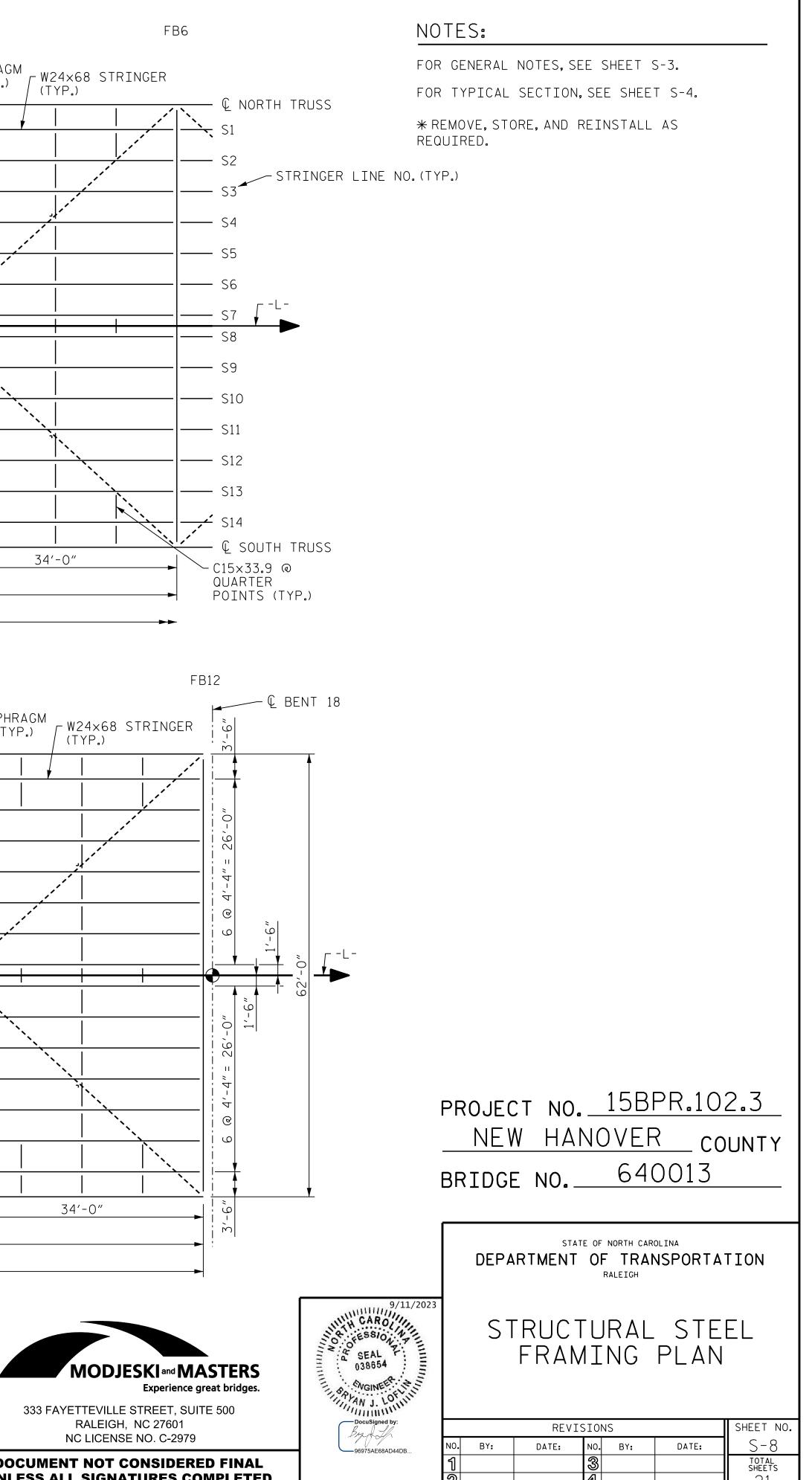
		PROJEC <u>NEV</u> BRIDGE	N HAI			2.3 UNTY
FERS bridges.	9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023 9/11/2023	LR S	RTMENT SFR S TEEL	RALEIGH STANDAF UMMA STR	NSPORTA	OR S
00	DocuSigned by:		REVIS	SIONS		SHEET NO.
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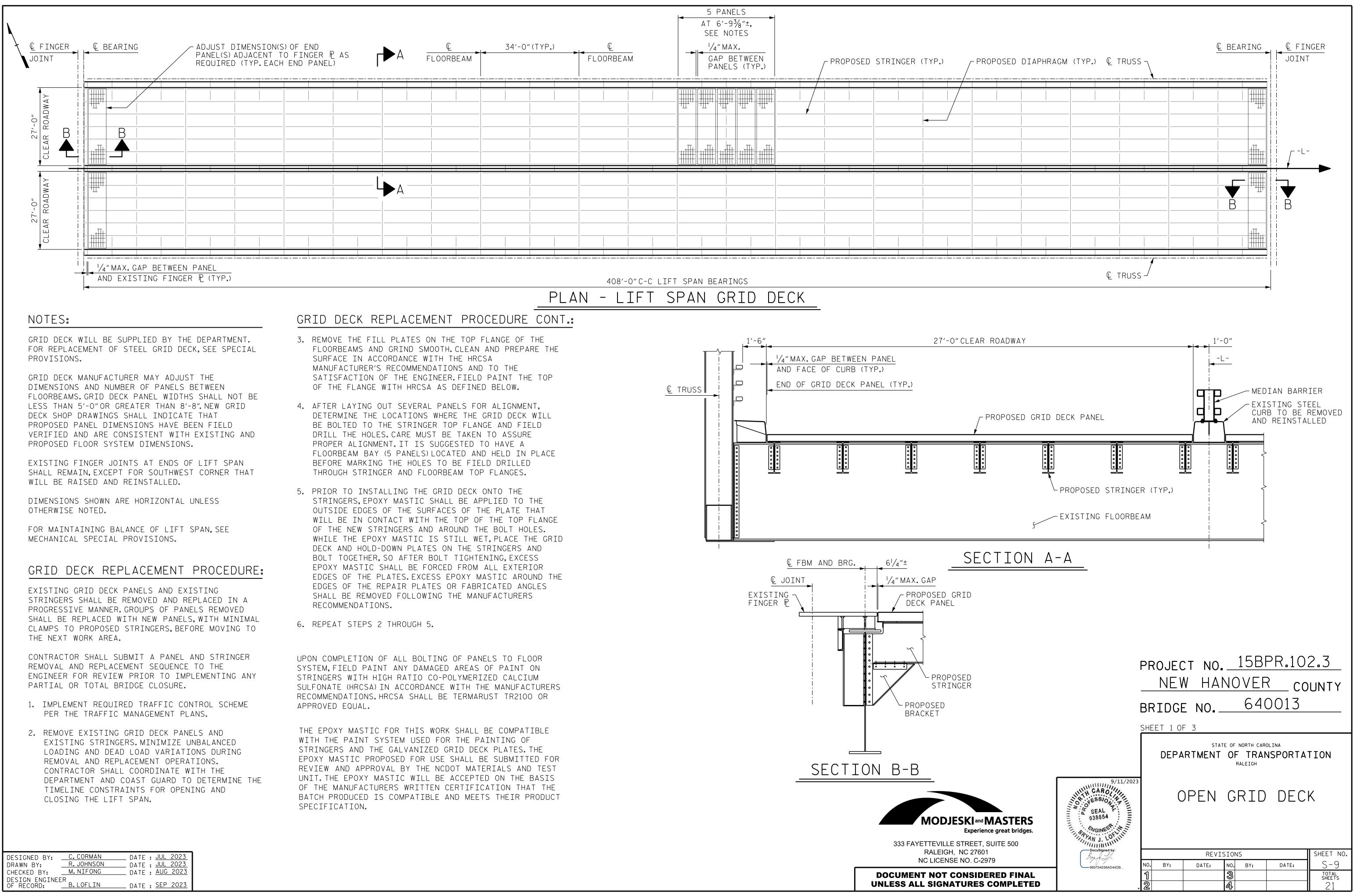


FILES MODJESKI & MASTERS





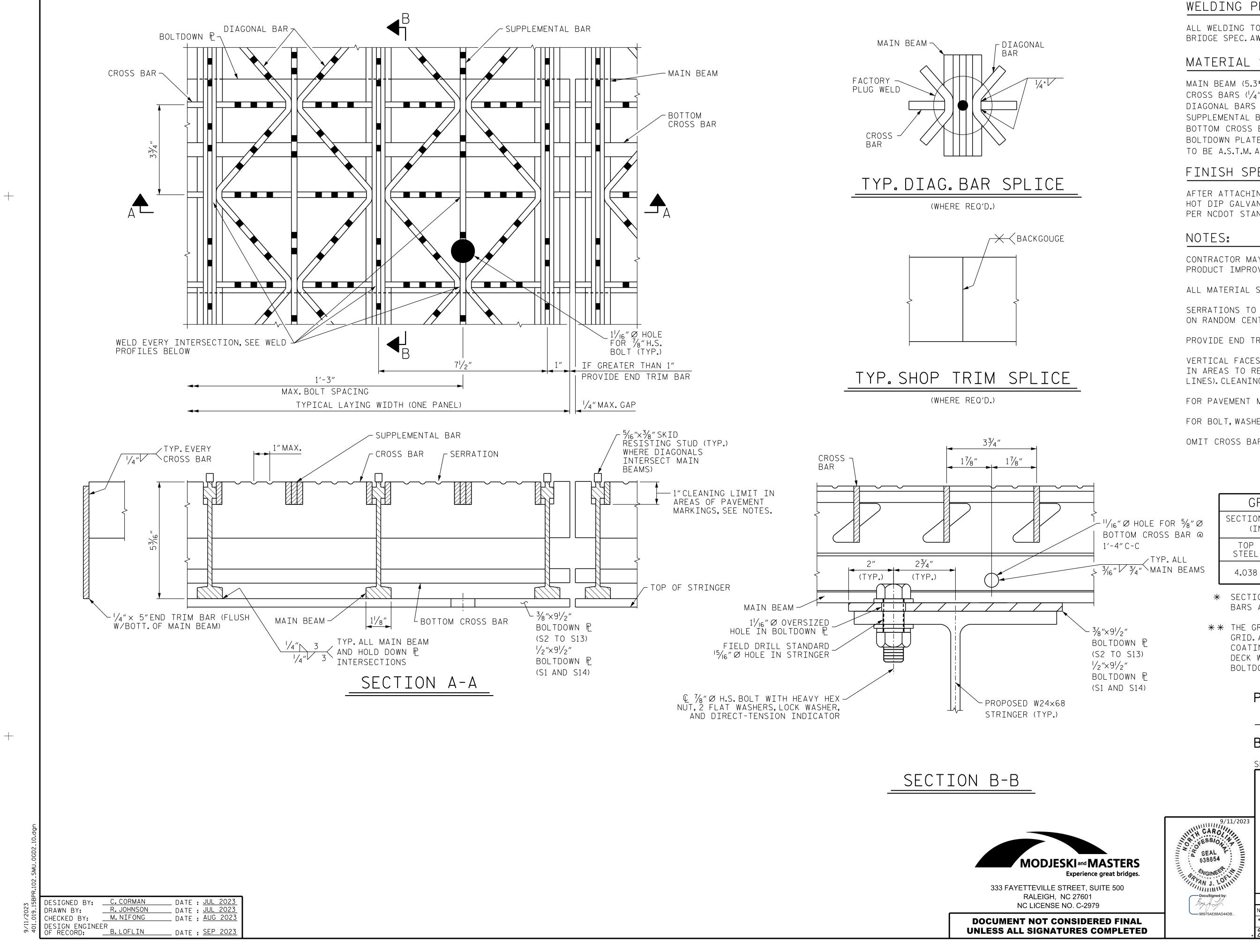
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FILES MODJESKI & MASTERS

34'-0" (TYP.)	<u>Ç</u> Loorbeam	5 PANELS AT 6'-9 ³ / ₈ "±, SEE NOTES 1/4" MAX. GAP BETWEEN PANELS (TYP.)	- PROPOSED STRINGER (TYP.) - PR	}0





WELDING PROCESS

ALL WELDING TO BE DONE IN ACCORDANCE WITH AASHTO BRIDGE SPEC. AWS D1.5 LATEST VERSION.

MATERIAL SPECIFICATIONS

MAIN BEAM (5.3#/FT.) TO BE A.S.T.M. A588 CROSS BARS $(\frac{1}{4}" \times 2^{1}/2")$ to be a.s.t.m. A588 DIAGONAL BARS $(\frac{1}{4}'' \times 1'')$ TO BE A.S.T.M. A588 SUPPLEMENTAL BARS $(\frac{1}{4}'' \times 1'')$ to be a.s.t.m. A588 BOTTOM CROSS BARS (5/8" Ø) TO BE A.S.T.M. A588 BOLTDOWN PLATES (3/8"×91/2", 1"×91/2" AND 1/2"×91/2") TO BE A.S.T.M. A588 OR APPROVED EQUAL

FINISH SPECIFICATIONS

AFTER ATTACHING BOLTDOWN PLATE, GRID DECK SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 PER NCDOT STANDARD SPECIFICATIONS.

NOTES:

CONTRACTOR MAY REQUEST TO MAKE MINOR CHANGES FOR PRODUCT IMPROVEMENT.

ALL MATERIAL SUBJECT TO MILL/SHOP TOLERANCES.

SERRATIONS TO BE APPROXIMATELY $\frac{3}{6}$ " DEEP BY $\frac{3}{8}$ " WIDE ON RANDOM CENTERS, 1" MAX. C-C SPACING.

PROVIDE END TRIM BAR AT ALL MAIN BEAM ENDS.

VERTICAL FACES OF TRANSVERSE BARS SHALL BE CLEANED IN AREAS TO RECEIVE PAVEMENT MARKINGS (PAINT LINES). CLEANING SHALL EXTEND 1"BELOW TOP OF GRID.

FOR PAVEMENT MARKINGS, SEE PAVEMENT MARKING PLANS.

FOR BOLT, WASHER, AND NUT DETAILS SEE SHEET 3 OF 3.

OMIT CROSS BARS AT BOLT LOCATIONS.

	GRI	D DECK	PROPERTIES
ک م	SECTION N (IN ³ /		APPROX.GRID WEIGHT (LBS / SF)**
	TOP STEEL	BOTTOM STEEL	20.0
IS	4.038	4.321	20.0

* SECTION MODULUS BASED ON 50% OF DIAGONAL BARS ACTIVE.

** THE GRID WEIGHT IS BASED ON AN UNCOATED GRID. ACTUAL WEIGHTS MAY VARY DUE TO COATING WEIGHT AND DECK ATTACHMENTS.GRID DECK WEIGHT SHOWN DOES NOT INCLUDE BOLTDOWN PLATES.

	PROJECT NO. 158PR.102.3
	NEW HANOVER COUNTY
	BRIDGE NO. 640013
	SHEET 2 OF 3
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
9/11/2023 RO	OPEN GRID DECK

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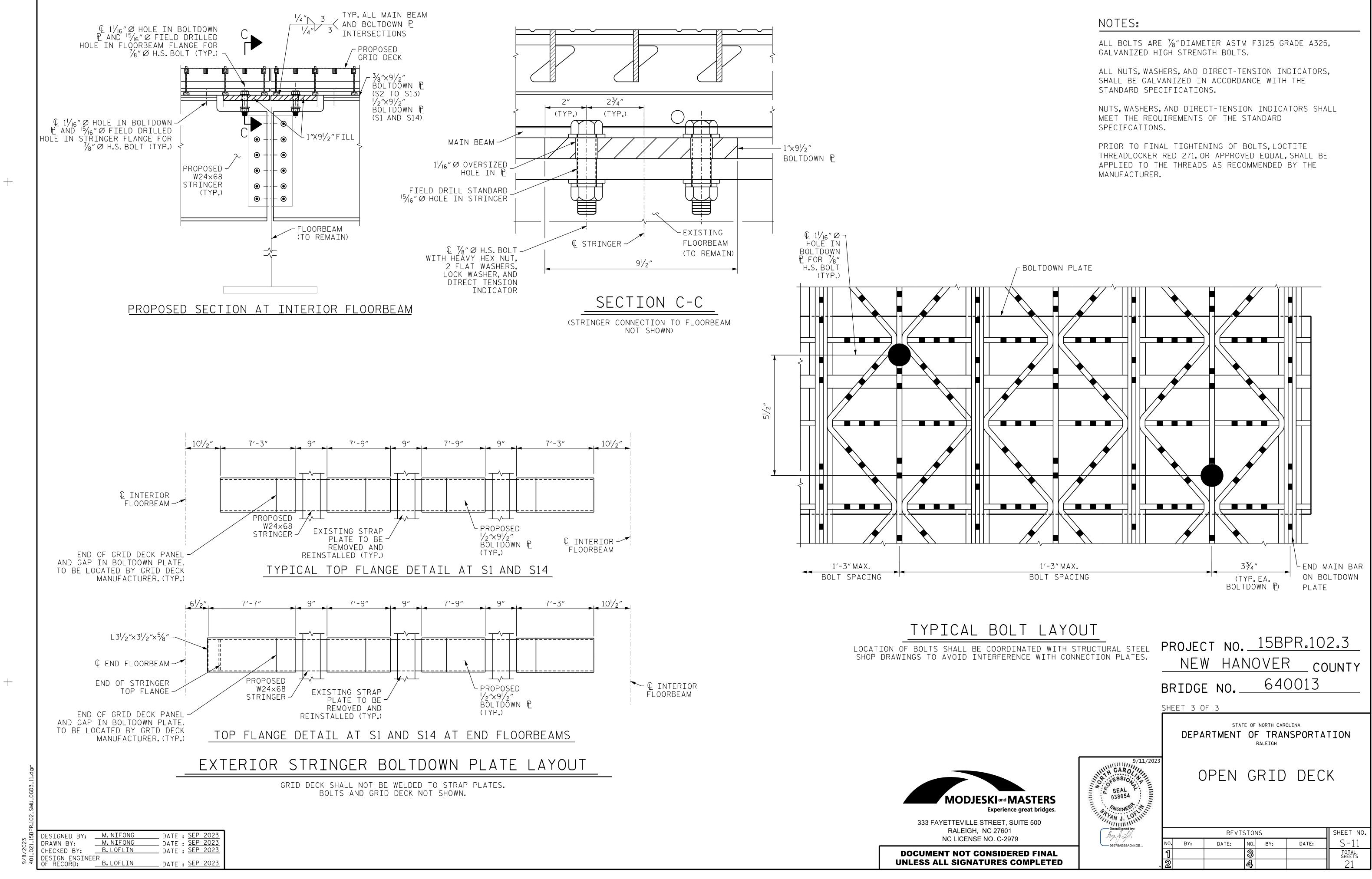
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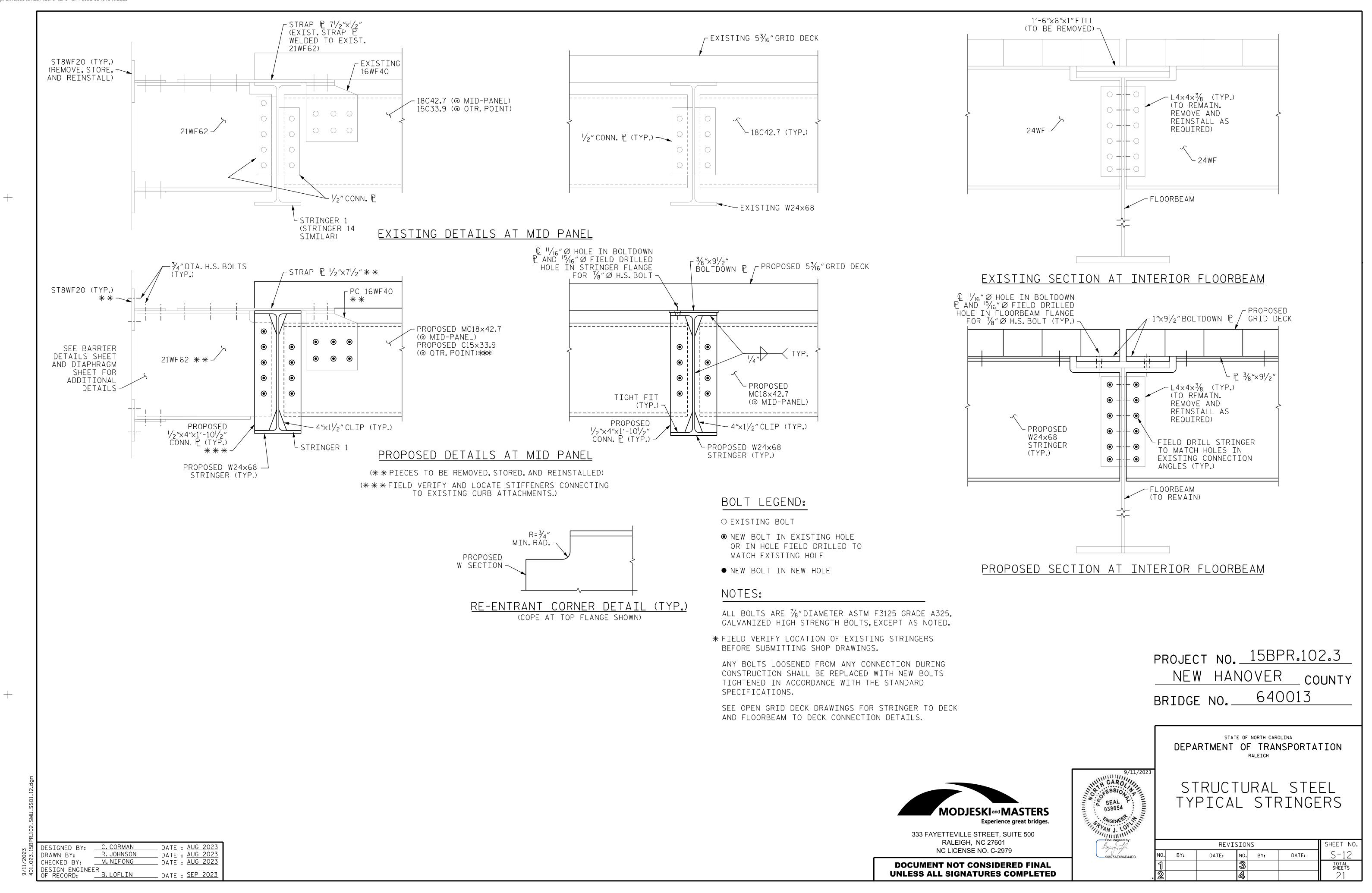
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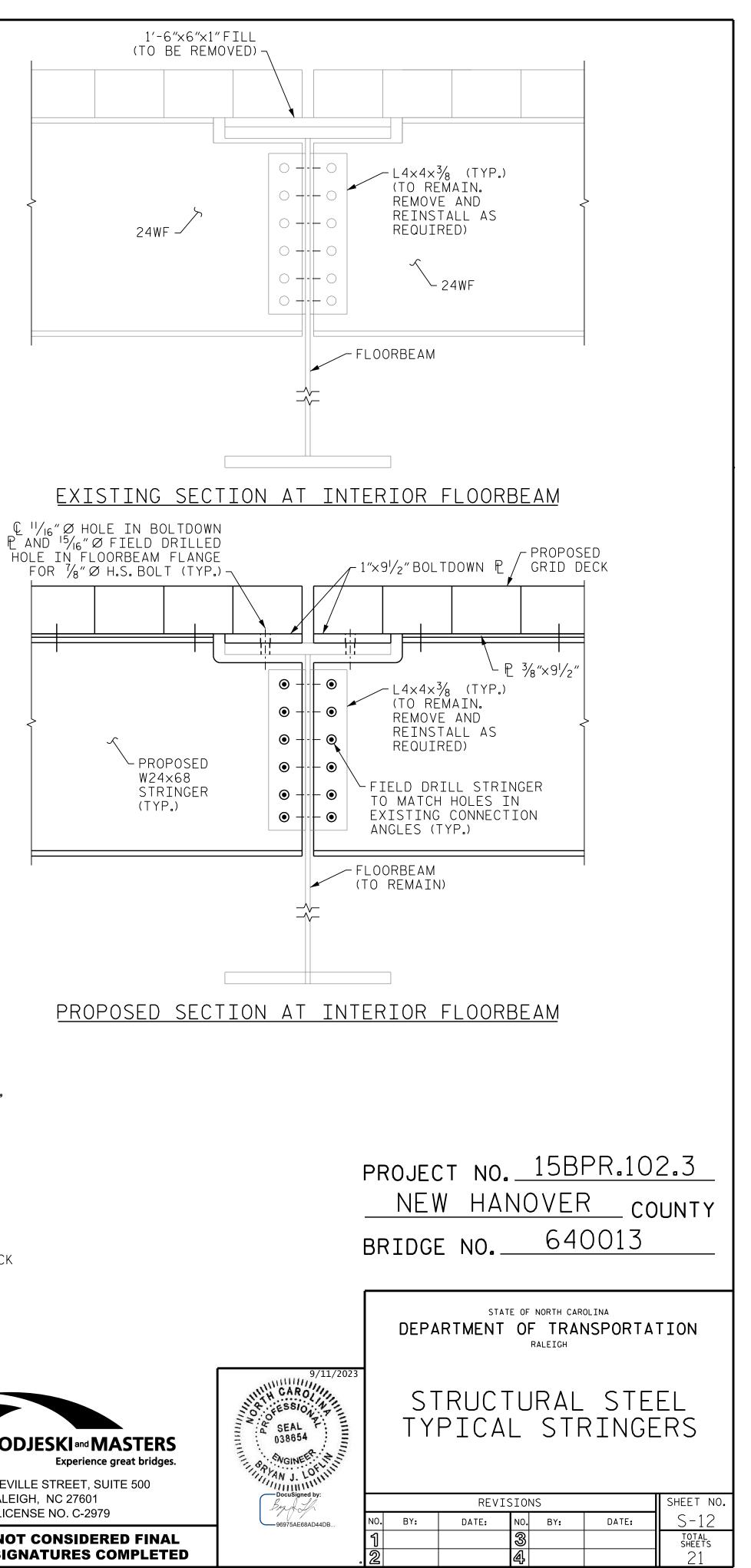
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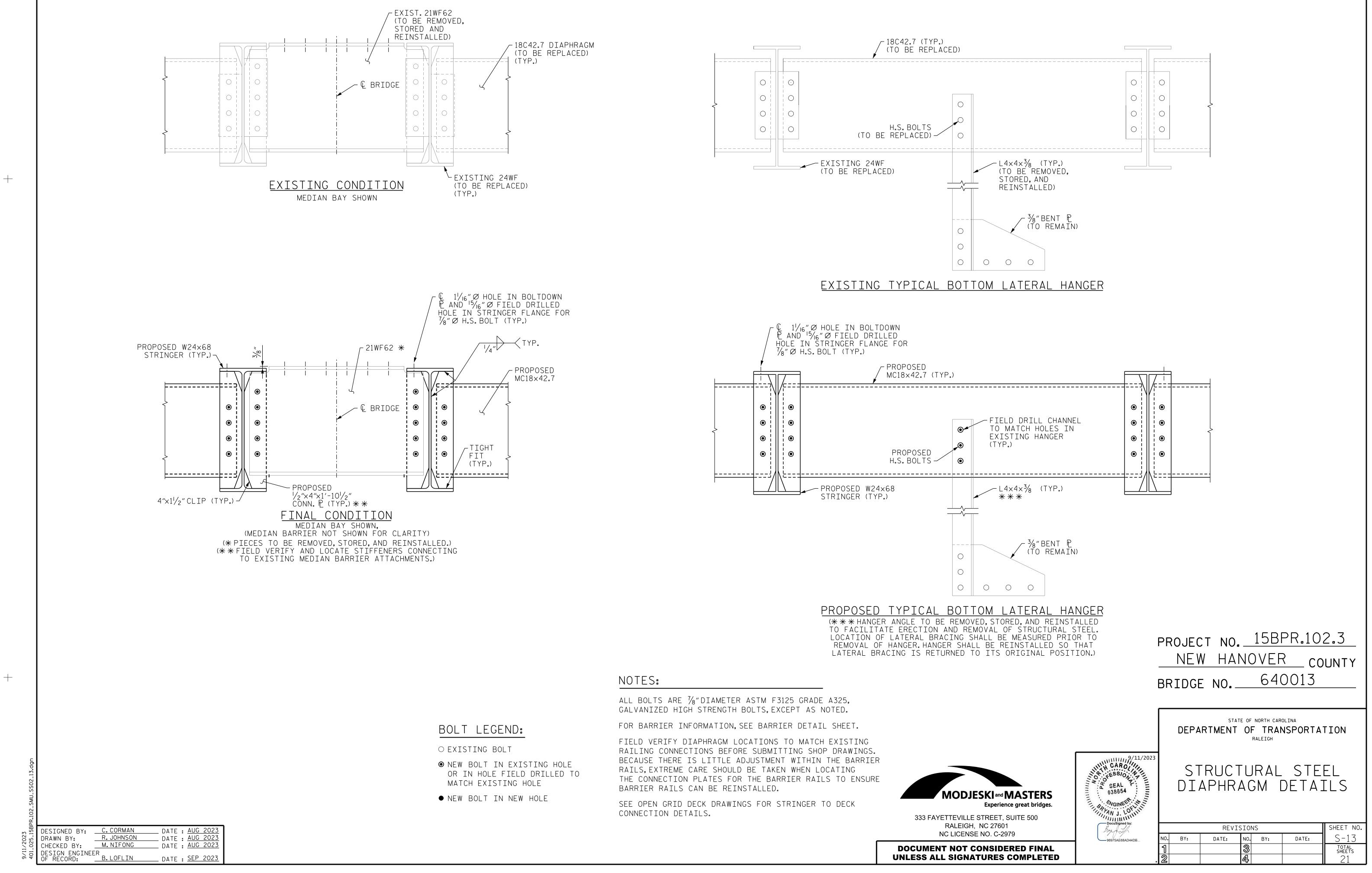
-"/₁₆"ØHOLE FOR 5/8"Ø BOTTOM CROSS BAR @

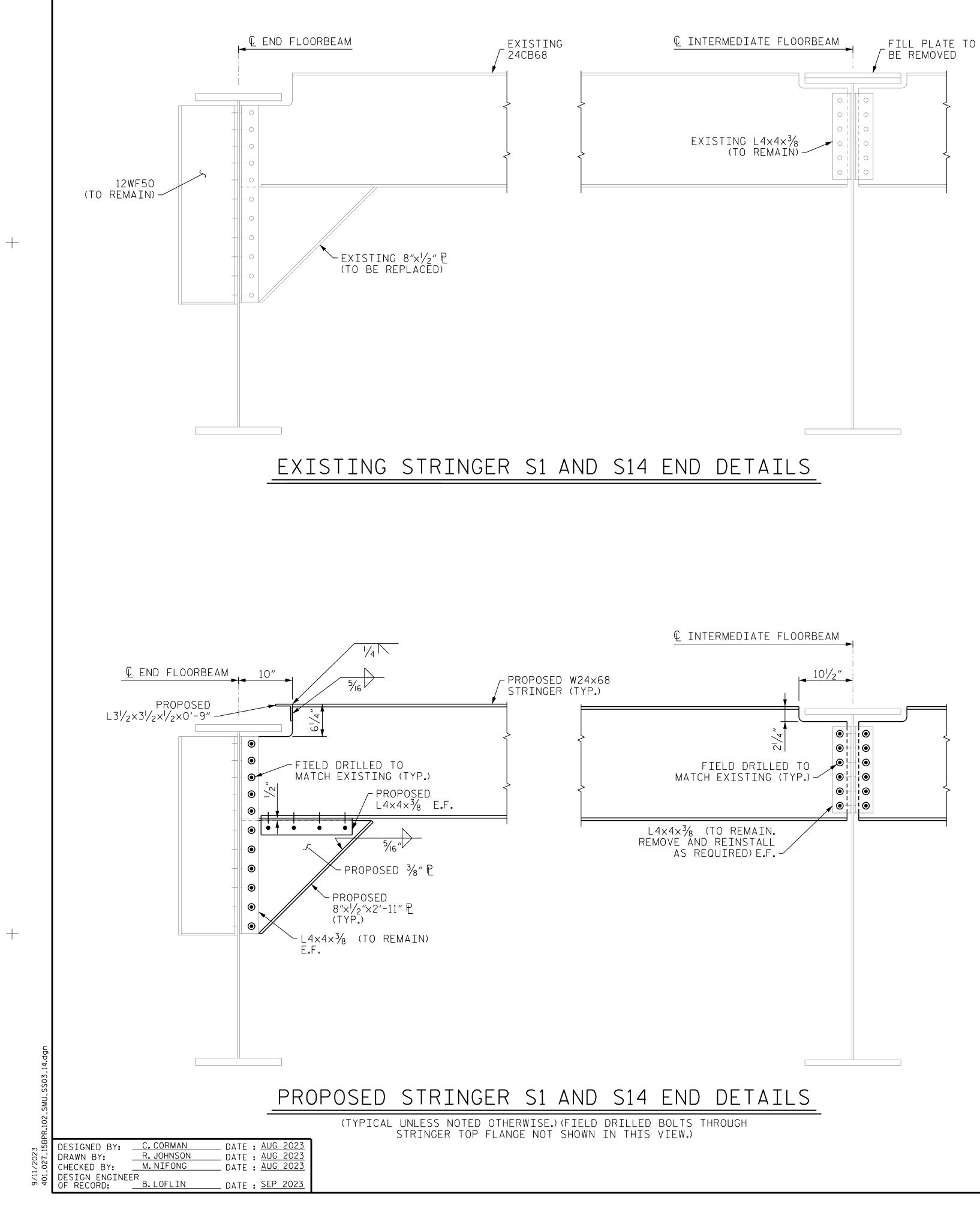
, 3/16" / 3/4" MAIN BEAMS

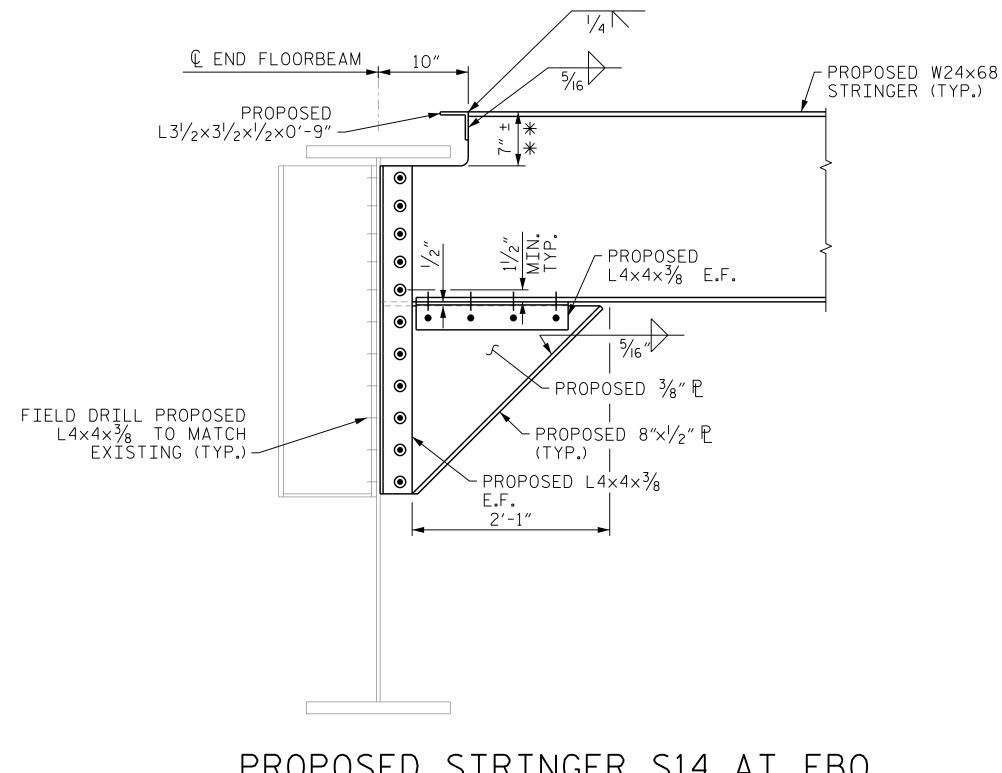












** FIELD VERIFY THIS DIMENSION AS THIS DIMENSION IS APPROXIMATED ASSUMING THAT THE LIFT SPAN FINGER JOINT WILL NEED TO BE RAISED $\frac{3}{4}$ " ± AT EACH SOUTHWEST END STRINGER LOCATION (S8-S14). (FIELD DRILLED BOLTS THROUGH STRINGER TOP FLANGE NOT SHOWN IN THIS VIEW.)

BOLT LEGEND:

O EXISTING BOLT

- NEW BOLT IN EXISTING HOLE OR IN HOLE FIELD DRILLED TO MATCH EXISTING HOLE
- NEW BOLT IN NEW HOLE

NOTES:

ALL BOLTS ARE 7/8"DI A325 GALVANIZED HI OTHERWISE SHOWN.

* FIELD VERIFY LOCAT BEFORE SUBMITTING ANY BOLTS LOOSENED DURING CONSTRUCTIO WITH NEW BOLTS TIC WITH THE STANDARD THE CONTRACTOR'S A FACT THAT A FEW OF CONNECTIONS AT THE RIVETS.

SEE OPEN GRID DECK DECK CONNECTION DE



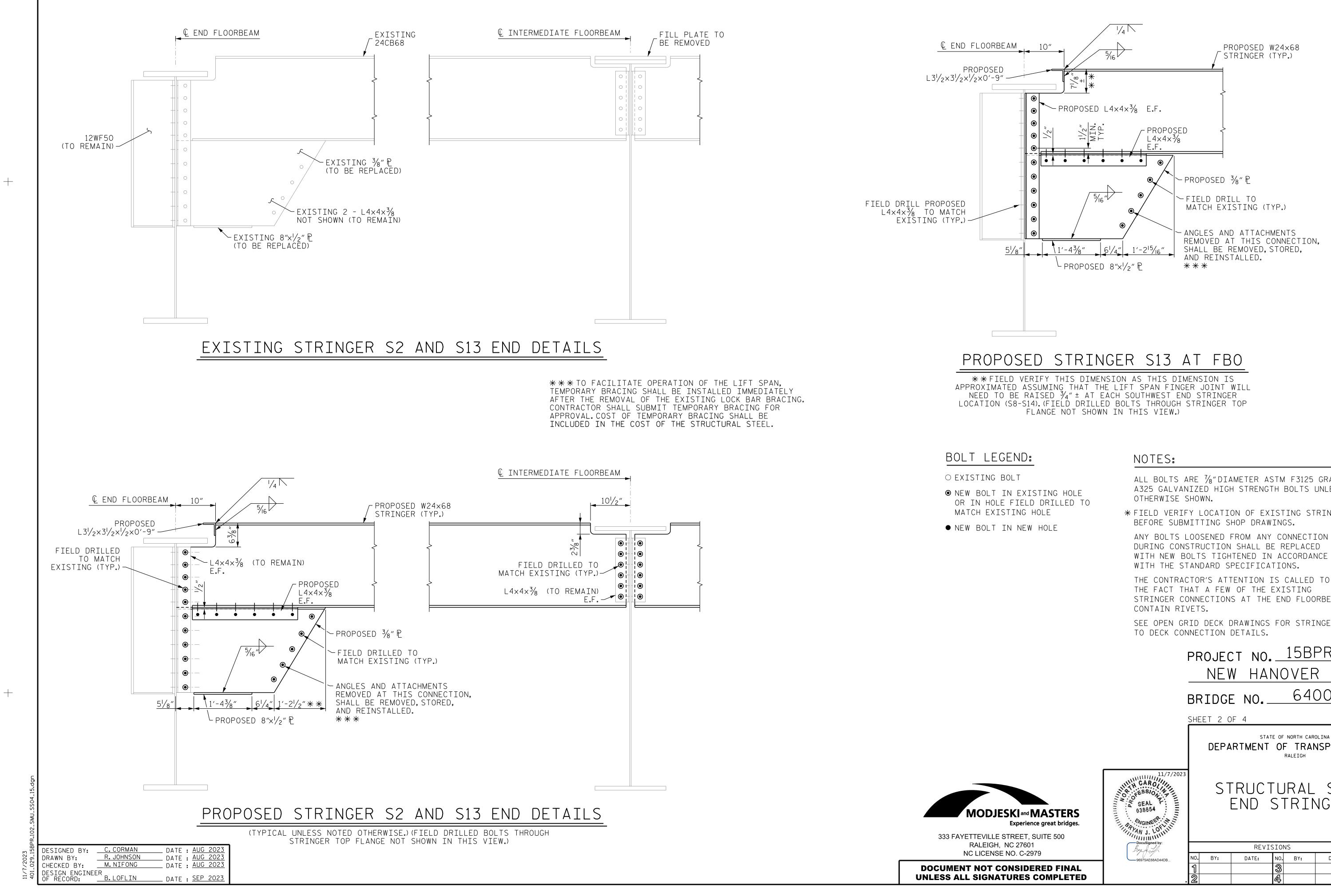
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PROPOSED STRINGER S14 AT FBO

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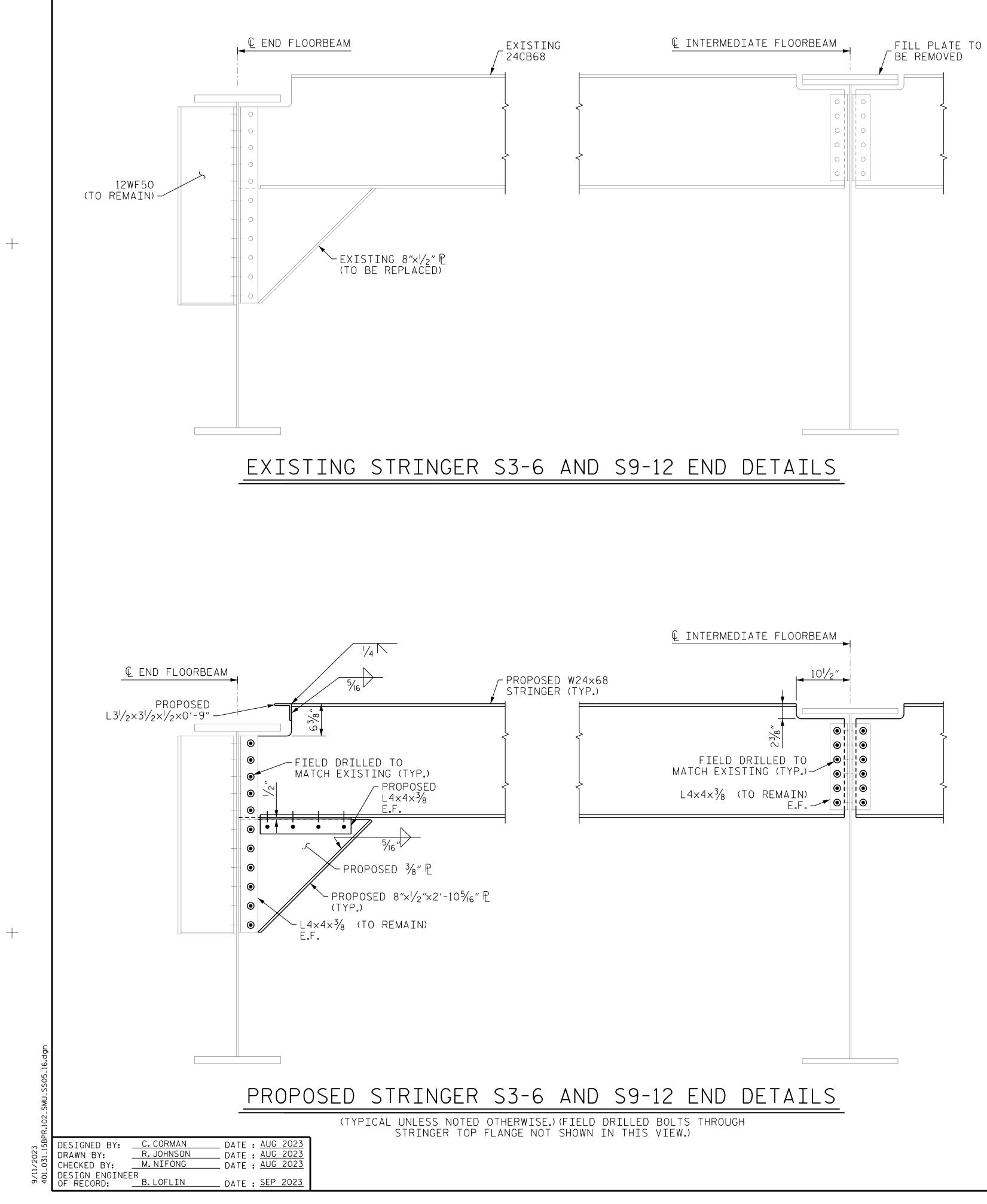
ALL BOLTS ARE 7/8" DIAMETER ASTM F3125 GRADE A325 GALVANIZED HIGH STRENGTH BOLTS UNLESS

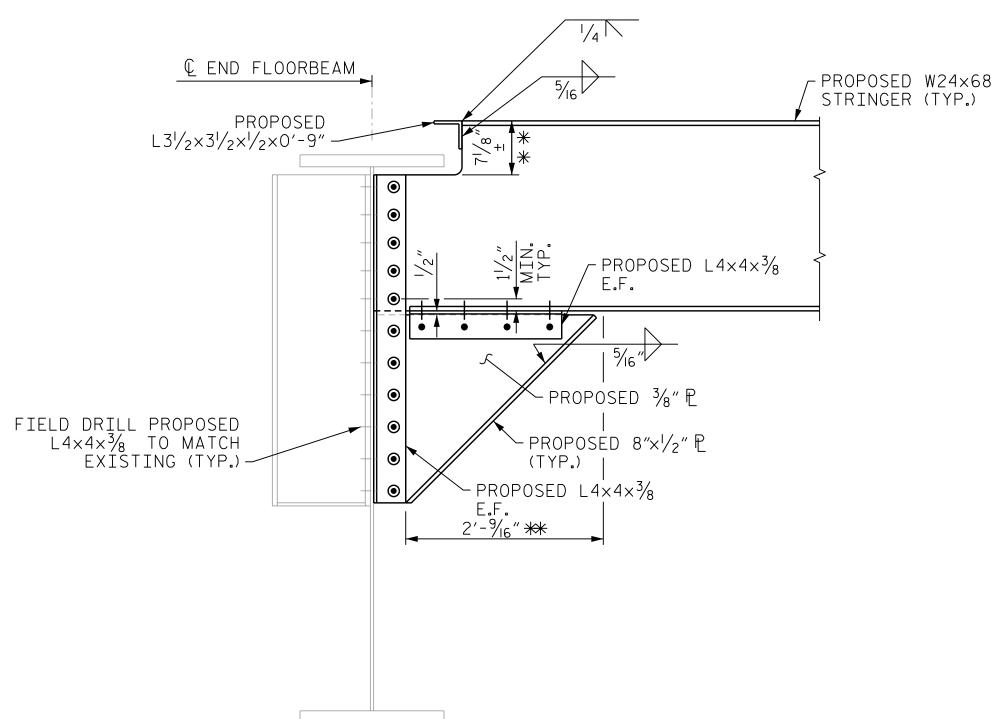
* FIELD VERIFY LOCATION OF EXISTING STRINGERS

THE CONTRACTOR'S ATTENTION IS CALLED TO STRINGER CONNECTIONS AT THE END FLOORBEAMS

SEE OPEN GRID DECK DRAWINGS FOR STRINGER

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** FIELD VERIFY THIS DIMENSION AS THIS DIMENSION IS APPROXIMATED ASSUMING THAT THE LIFT SPAN FINGER JOINT WILL NEED TO BE RAISED $\frac{3}{4}$ " ± AT EACH SOUTHWEST END STRINGER LOCATION (S8-S14). (FIELD DRILLED BOLTS THROUGH STRINGER TOP FLANGE NOT SHOWN IN THIS VIEW.)

BOLT LEGEND:

○ EXISTING BOLT

- NEW BOLT IN EXISTING HOLE OR IN HOLE FIELD DRILLED MATCH EXISTING HOLE
- NEW BOLT IN NEW HOLE

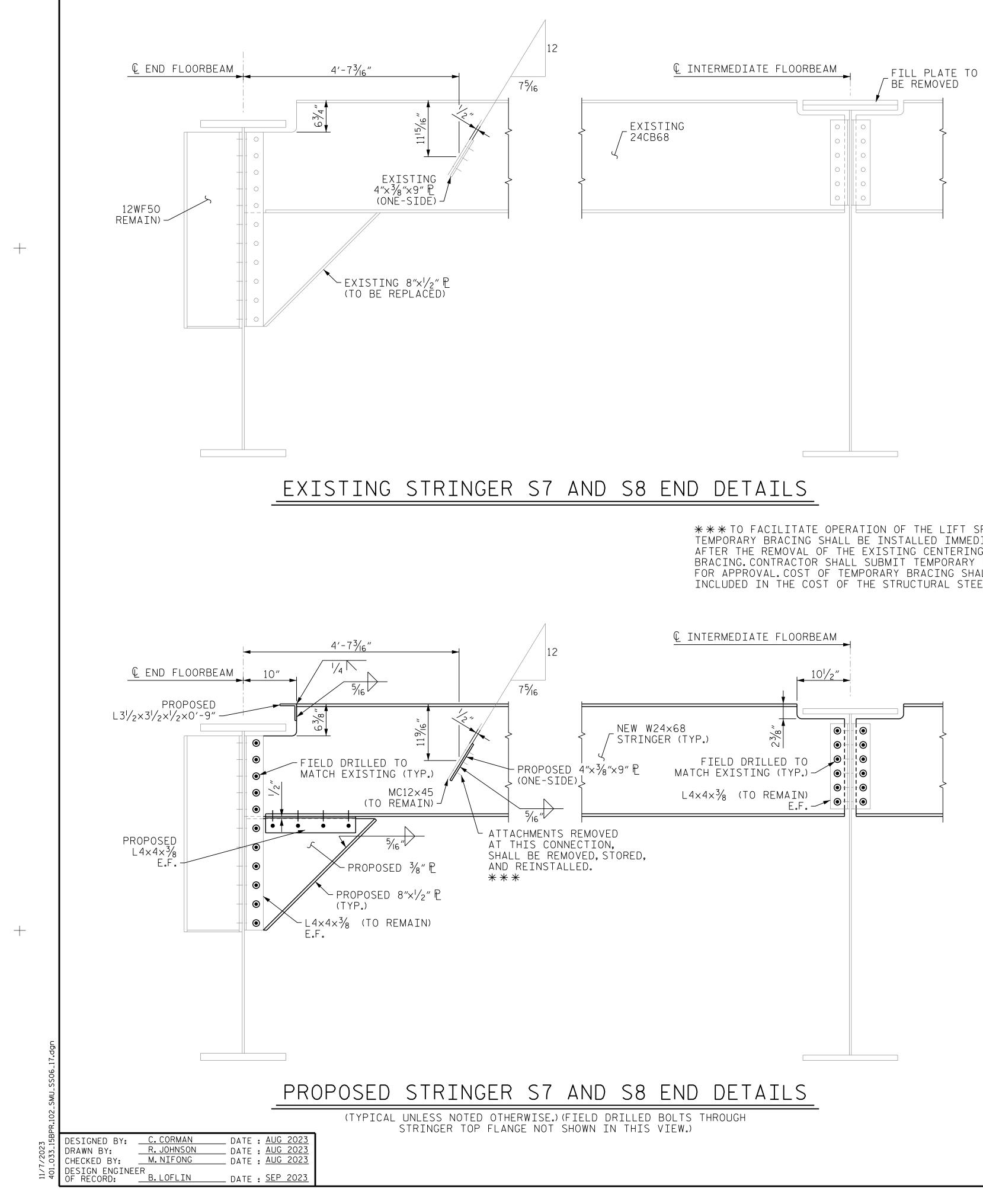


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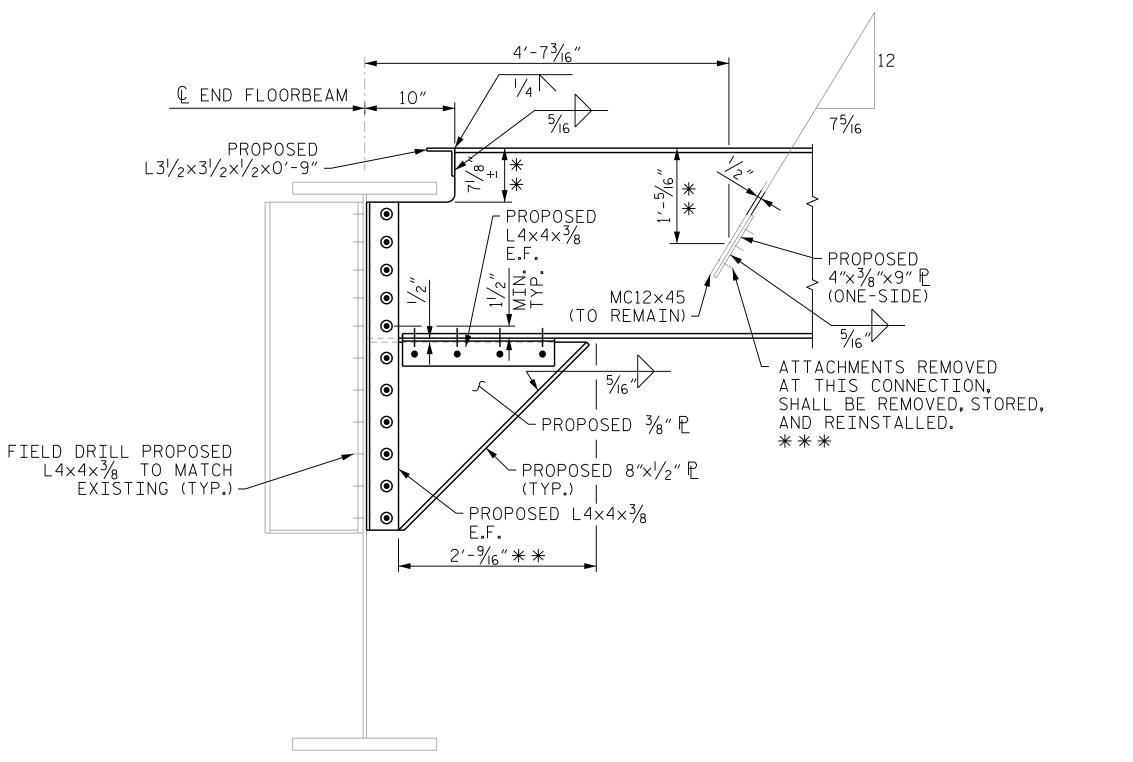
PROPOSED STRINGER S9-12 AT FBO

NOTES:

BOLT IN EXISTING HOLE	IAMETER ASTM F3125 GRADE GH STRENGTH BOLTS UNLESS					
E FIELD DRILLED TO STING HOLE IN NEW HOLE	OTHERWISE SHOWN. * FIELD VERIFY LOCAT BEFORE SUBMITTING	ION OF EXISTING STRINGERS SHOP DRAWINGS.				
IN NEW HOLL	DURING CONSTRUCTIO	FROM ANY CONNECTION ON SHALL BE REPLACED GHTENED IN ACCORDANCE SPECIFICATIONS.				
	FACT THAT A FEW OF	TTENTION IS CALLED TO THE F THE EXISTING STRINGER E END FLOORBEAMS CONTAIN				
	SEE OPEN GRID DECK TO DECK CONNECTION	DRAWINGS FOR STRINGER DETAILS.				
		PROJECT NO. <u>15BPR.102.3</u> <u>NEW HANOVER</u> county BRIDGE NO. <u>640013</u>				
		SHEET 3 OF 4				
		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
MODJESKI and MASTERS Experience great bridges. YETTEVILLE STREET, SUITE 500	9/11/2023 HILL CAROL CAROL HILL CAROL SEAL 038654 038654 CVGINEER	STRUCTURAL STEEL END STRINGERS				
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* * * TO FACILITATE OPERATION OF THE LIFT SPAN, TEMPORARY BRACING SHALL BE INSTALLED IMMEDIATELY AFTER THE REMOVAL OF THE EXISTING CENTERING DEVICE BRACING. CONTRACTOR SHALL SUBMIT TEMPORARY BRACING FOR APPROVAL.COST OF TEMPORARY BRACING SHALL BE INCLUDED IN THE COST OF THE STRUCTURAL STEEL.

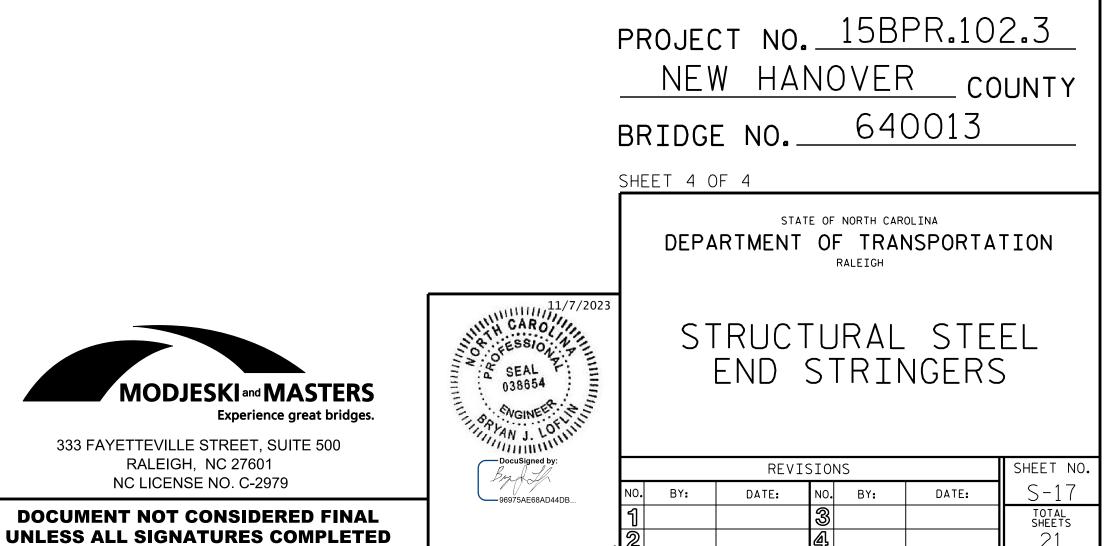


** FIELD VERIFY THIS DIMENSION AS THIS DIMENSION IS APPROXIMATED ASSUMING THAT THE LIFT SPAN FINGER JOINT WILL NEED TO BE RAISED $\frac{3}{4}$ " ± AT EACH SOUTHWEST END STRINGER LOCATION (S8-S14). (FIELD DRILLED BOLTS THROUGH STRINGER TOP FLANGE NOT SHOWN IN THIS VIEW.)

BOLT LEGEND:

O EXISTING BOLT

- NEW BOLT IN EXISTING HOLE OR IN HOLE FIELD DRILLED TO MATCH EXISTING HOLE
- NEW BOLT IN NEW HOLE



PROPOSED STRINGER S8 AT FBO

NOTES:

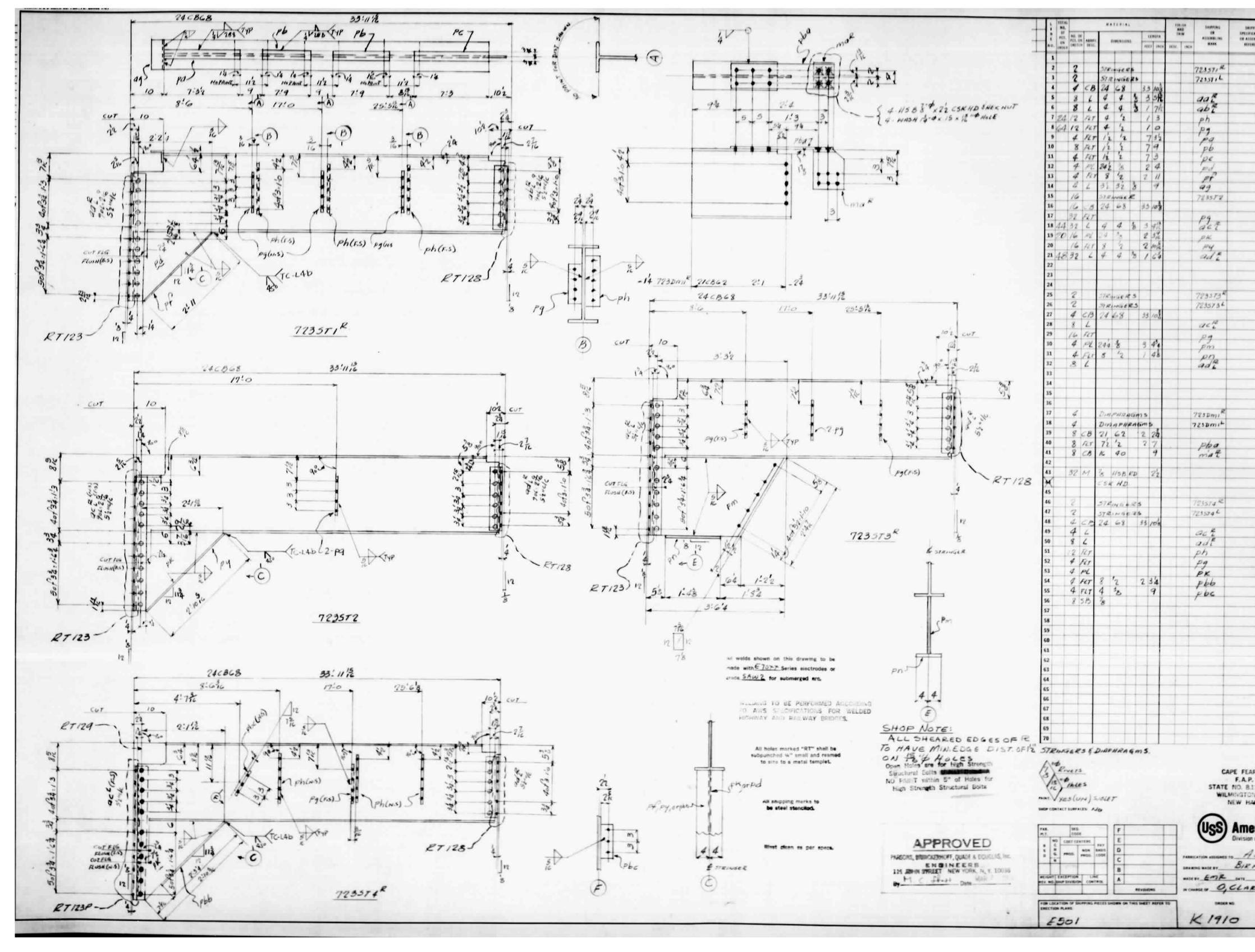
ALL BOLTS ARE $\frac{7}{8}$ " DIAMETER ASTM F3125 GRADE A325 GALVANIZED HIGH STRENGTH BOLTS UNLESS OTHERWISE SHOWN.

* FIELD VERIFY LOCATION OF EXISTING STRINGERS BEFORE SUBMITTING SHOP DRAWINGS.

ANY BOLTS LOOSENED FROM ANY CONNECTION DURING CONSTRUCTION SHALL BE REPLACED WITH NEW BOLTS TIGHTENED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT A FEW OF THE EXISTING STRINGER CONNECTIONS AT THE END FLOORBEAMS CONTAIN RIVETS.

SEE OPEN GRID DECK DRAWINGS FOR STRINGER TO DECK CONNECTION DETAILS.



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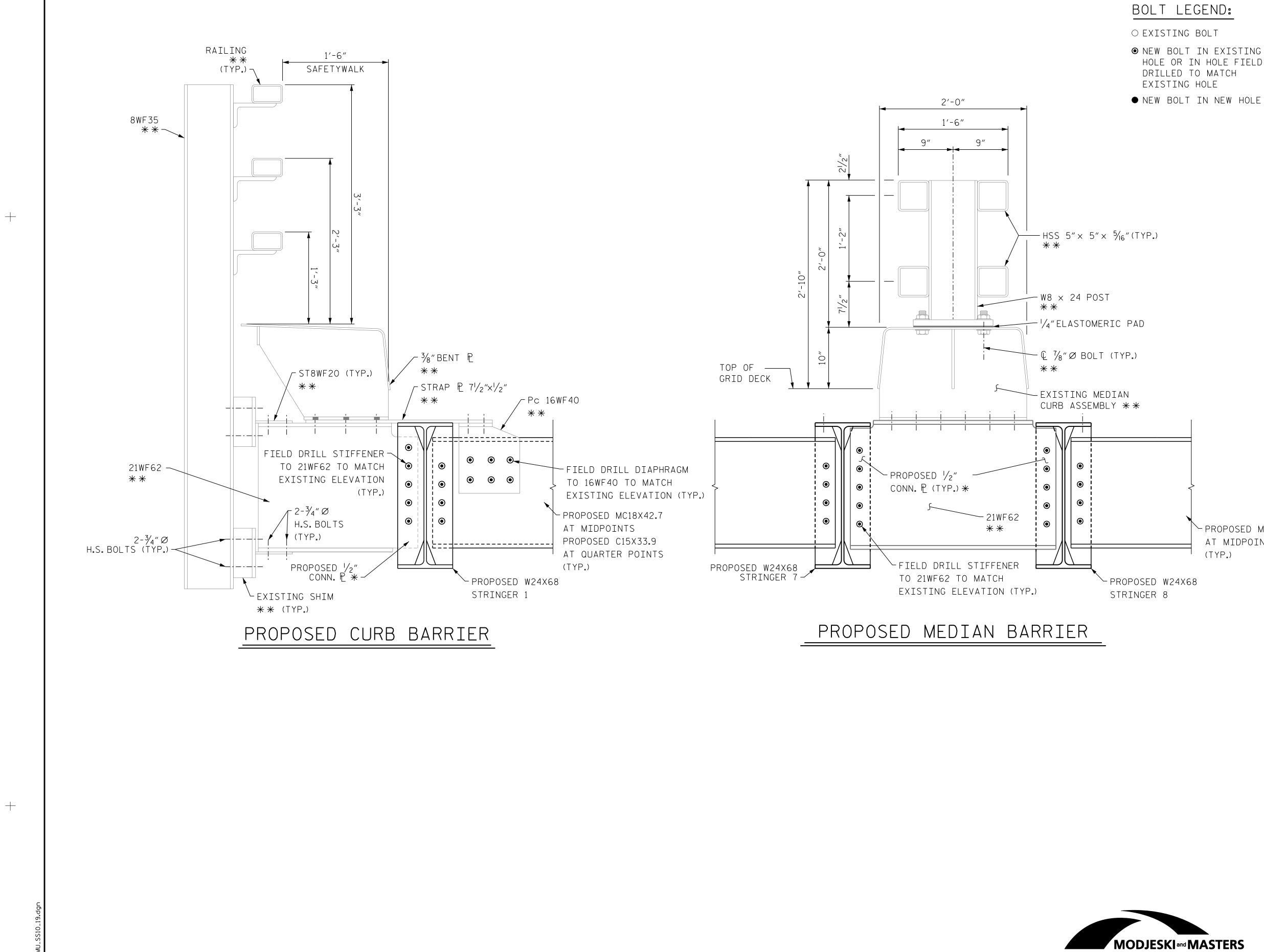
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PROJECT NO. <u>15BPR.102.3</u> <u>NEW HANOVER</u> COUNTY BRIDGE NO. <u>640013</u>

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STRUCT	URAL	STEEL
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● NEW BOLT IN EXISTING HOLE OR IN HOLE FIELD BARRIER AND CURB ASSEMBLY TO BE REMOVED, STORED DURING STRINGER AND DECK REPLACEMENT, AND REASSEMBLED.

NOTES:

ALL BOLTS ARE $\frac{7}{8}$ " DIAMETER ASTM F3125 GRADE A325, GALVANIZED HIGH STRENGTH BOLTS, EXCEPT AS NOTED.

*FIELD VERIFY AND LOCATE STIFFENERS CONNECTING TO EXISTING CURB OR EXISTING MEDIAN BARRIER ATTACHMENTS. BECAUSE THERE IS LITTLE ADJUSTMENT WITHIN THE BARRIER RAILS, EXTREME CARE SHOULD BE TAKEN WHEN LOCATING THE CONNECTION PLATES FOR THE BARRIER RAILS TO ENSURE THE BARRIER RAILS CAN BE INSTALLED.

** PIECES TO BE REMOVED, STORED, AND REINSTALLED AS REQUIRED.

SEE DIAPHRAGM DETAILS SHEET FOR ADDITIONAL DETAILS.

IT IS SUGGESTED THAT BARRIER, RAILING, AND CURB COMPONENTS BE MARKED SUCH THAT THEY ARE REINSTALLED IN THEIR EXISTING LOCATIONS UPON COMMENCEMENT OF REINSTALLATION OPERATIONS.

SEE OPEN GRID DECK DRAWINGS FOR STRINGER TO DECK AND FLOORBEAM TO DECK CONNECTION DETAILS.

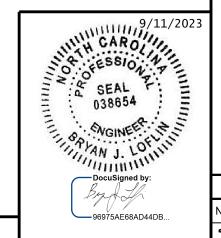
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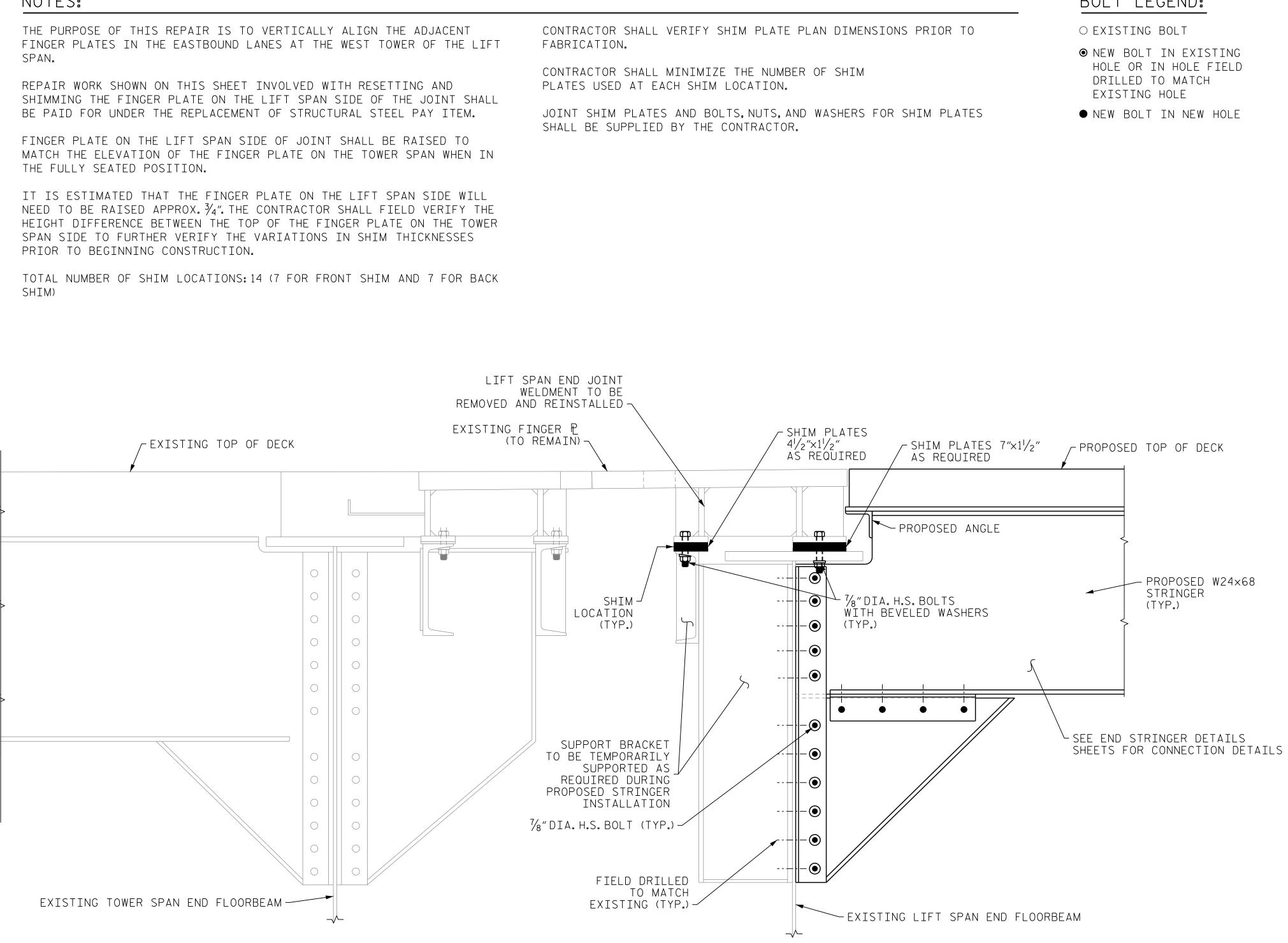
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BARRIER DETAILS

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NOTES:



SECTION THROUGH FINGER JOINT AT WEST TOWER

THIS REPAIR WORK SHALL BE PERFORMED ONLY ON THE SOUTHWEST HALF OF THE BRIDGE - EASTBOUND LANES ON THE LIFT SPAN SIDE OF THE JOINT.

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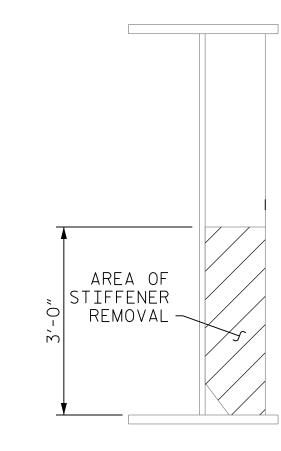


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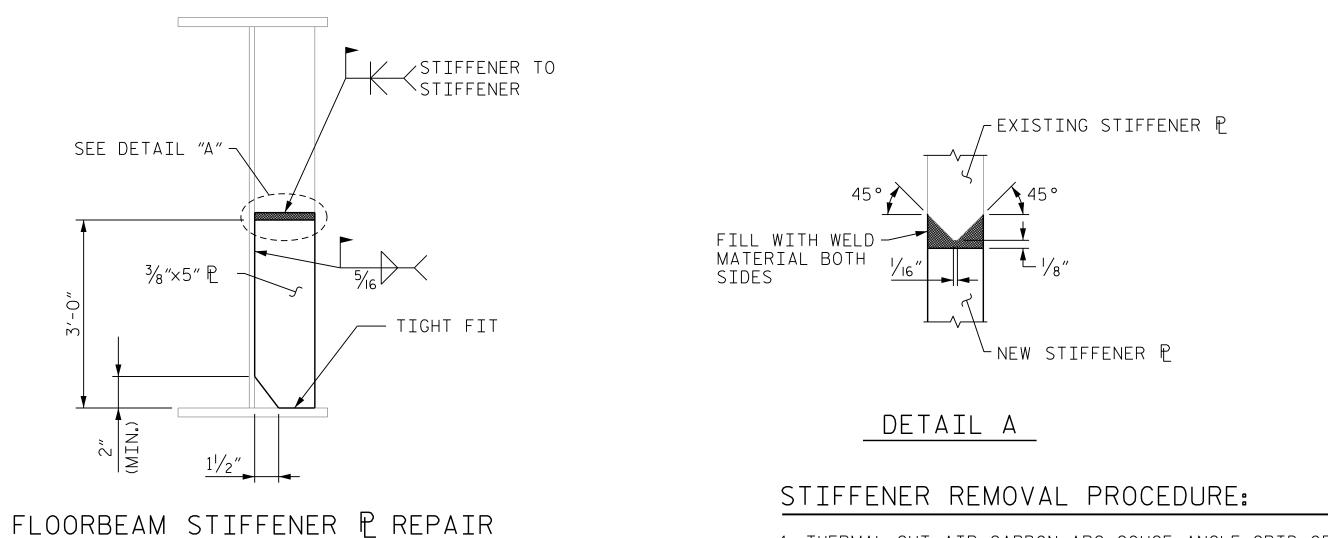
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FLOORBEAM STIFFENER 🕆 REMOVAL



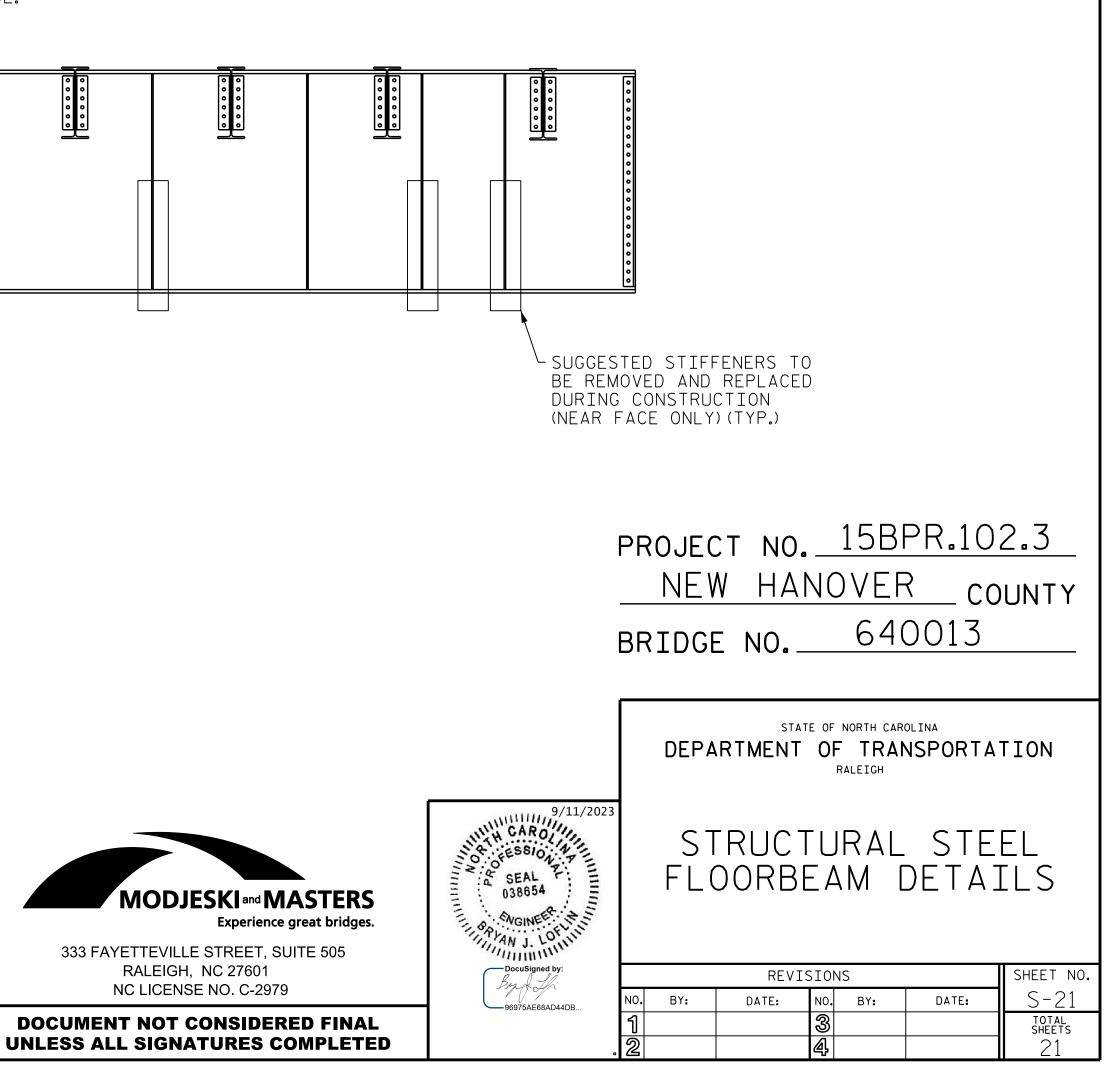
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9/11/2023 401_041_15	DESIGN ENGINEE OF RECORD:	R B.LOFLIN	DATE	<u>SEP</u>	2023

- 1. THERMAL CUT, AIR CARBON ARC GOUGE, ANGLE GRID OR SAW CUT THE STIFFENER TO WITHIN $\frac{3}{6}$ " of the flange or web plate. AVOIDING ANY DAMAGE TO THE EXISTING STEEL TO REMAIN. USE MECHANICAL GUIDE WHILE MAKING THE CUT.
- 2. AIR CARBON ARC GOUGE THE WELD AND REMAINING STIFFENER TO WITHIN $\frac{3}{16}$ " of the web plate avoiding any damage TO THE EXISTING STEEL TO REMAIN.
- 3. GRIND THE REMAINING WELD AND STIFFENER REMNANTS SMOOTH AND FLUSH WITH THE SURROUNDING BASE METAL. FINAL GRINDING SHALL BE IN THE DIRECTION OF THE FLANGE LENGTH. GROUND SURFACES SHALL HAVE A SURFACE ROUGHNESS OF RA = 1000 MICROINCHES OR LESS.
- 4. 100 PERCENT MAGNETIC PARTICLE TEST THE WELD REMOVAL AREAS.
- 5. ALIGN AND ATTACH THE NEW PORTION OF THE STIFFENER AND PLACE WELDS. TEST WELDS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND BRIDGE WELDING CODE.

SUGGESTED FLOORBEAM STIFFENER REMOVAL ELEVATION



NOTES:

FLOORBEAM STIFFENER REMOVAL AND REPLACEMENT IS NOT REQUIRED. FLOORBEAM STIFFENER PLATES MAY BE REMOVED AND REPLACED TO FACILITATE STRINGER REPLACEMENT OPERATIONS.

CONTRACTOR SHALL ONLY REMOVE STIFFENERS REQUIRED TO FACILITATE ERECTION OF NEW STRUCTURAL STEEL.A SUGGESTED STIFFENER REMOVAL LAYOUT IS SHOWN ON THIS DRAWING. CONTRACTOR MAY PROPOSE AN ALTERNATE REMOVAL LAYOUT FOR REVIEW AND APPROVAL. THE DEPARTMENT WILL SUPPLY 108 STIFFENER PLATES WITH THE DIMENSIONS SHOWN ON THIS SHEET. CONTRACTOR WILL BE RESPONSIBLE FOR SUPPLYING ANY ADDITIONAL STIFFENER PLATES OR STIFFENER PLATES OF OTHER SIZES REQUIRED BY A DIFFERENT PROPOSED METHOD OF REPLACING THE STRINGERS.

REMOVE ONLY THE STIFFENERS ON THE HALF OF THE BRIDGE WHERE WORK IS OCCURING.

ONLY REMOVE THE STIFFENER PORTIONS SHOWN ON ONE FACE OF THE FLOORBEAM WEB, LEAVING THE STIFFENER ON THE OPPOSING SIDE OF THE WEB.

IT IS SUGGESTED THAT, WITHIN A BAY (BETWEEN FLOORBEAMS) ONLY REMOVE FROM ONE OF THE FLOORBEAM WEB FACES IN THAT BAY TO FACILITATE THE REMOVAL AND INSTALLATION OF STRINGERS.

EXCEPT FOR THE STIFFENER AT CENTERLINE BRIDGE UNDER MEDIAN BARRIER, STIFFENERS REMOVED ON THE EASTBOUND OR WESTBOUND SIDES SHALL BE REPAIRED PRIOR TO ALLOWING TRAFFIC ONTO THAT SIDE.

NO LIVE LOAD SHALL BE PRESENT ON THE LIFT SPAN DURING WELDING.

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{16}$ " Ø STUDS ALONG THE BEAM, AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{16}$ " Ø STUDS FOR 4 - 🔏 Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

STD. NO. SN