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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY _**DAVIDSON**

PROJECT DESCRIPTION REPLACE BRIDGES 164 AND 168 ON US 29/70 OVER NORFOLK SOUTHERN RAILWAY STATE PROJECT REFERENCE NO. 29 B-5783

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSUFFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE CINVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS, AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO PERFORM INDEPENDENT SUBSURFACE INVESTIGATIONS AND MAKE INTERPRETATIONS AS NECESSARY TO CONFIRM CONDITIONS ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUYIUM (ALLUY,) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALUGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED WISCHARD NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-7-6 A-7-7	COMPRESSIBILITY	NON-CHISTALLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING GRANULAR SILT-MUCK.	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
#40 30 MX 50 MX 51 MN SOILS SOILS SOILS PEAT		- WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
*200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS TRACE OF ORGANIC MATTER 2 - 3%, 3 - 5%, TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
PASSING *40 SOILS WITH	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL 48 MX 41 MN LITTLE OR LITTLE OR HIGHLY MODERATE HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. FINE SHITY OR CLAYEY SHITY CLAYEY MATTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING AS SUPCRAPS EXCELLENT TO GOOD FAIR TO POOR POOR POOR POOR POOR POOR POOR POO	<u> </u>	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBGRADE P1 OF A-7-5 SUBGROUP IS < LL - 30 ; P1 OF A-7-6 SUBGROUP IS > LL - 30	- O-MG SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED	III 25 (425	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK, IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (TONS/FT ²)	ROADWAY EMBANKMENT (RE) ROADWAY EMBANKMENT (RE) DIP & DIP DIRECTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL SPT DEF COMT DEST DEFINITION SECURITY SPT DEFINITION SECURITY S	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LOOSE 4 TO 10 GRANULAR MEDIUM DENSE 10 TO 30 MATERIAL MEDIUM DENSE 10 TO 30 N/A	The service of the se	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE > 50	THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETRUMETER	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	— INFERRED SOIL BOUNDARY — CORE BORING ■ SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	A SULVIVIAL SOLV POLYPORY A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 → 4	INSTREERTION	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SILT CLAY (BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (FL.) (CL.)	ABBRE VIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY γ - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	_ CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; YERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(P) ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING	BENCH MARK: B5783-2: N-788319.2227, E-1689384.5867
	EQUIPMENT USED ON SUBJECT PROJECT	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION, 903 25 EEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	ELEVATION: 903.25 FEET
SL _ SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:
- DRY - (D) ATTAIN OPTIMUM MOISTURE	CME-55 G'CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED 4.008 FEET	F.I.A.D. = FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY	1	INDURATION	ROADWAY DESIGN AND SURVEY INFORMATION PROVIDED BY NCDOT CT = CORING TERMINATED
PLASTICITY INDEX (PI) DRY STRENGTH	L CME-550 HARD FACED FINGER BITS X-N Q	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS;	5. 55mm (Figure 1
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST UNGCARBIDE INSERTS	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING X W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER X TRICONE 2-3/4 TUNG-CARB. SOUNDING ROD	BREAKS EASILY WHEN HIT WITH HAMMER. GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X DIEDRICH D-50 X INICONE 2-374 TUNG-CARB. SOUNDING ROD X CORE BIT VANE SHEAR TEST	INDURATED ORALINS HAE DIFFICULT TO SCHARLE WITH STEEL PROBE;	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	

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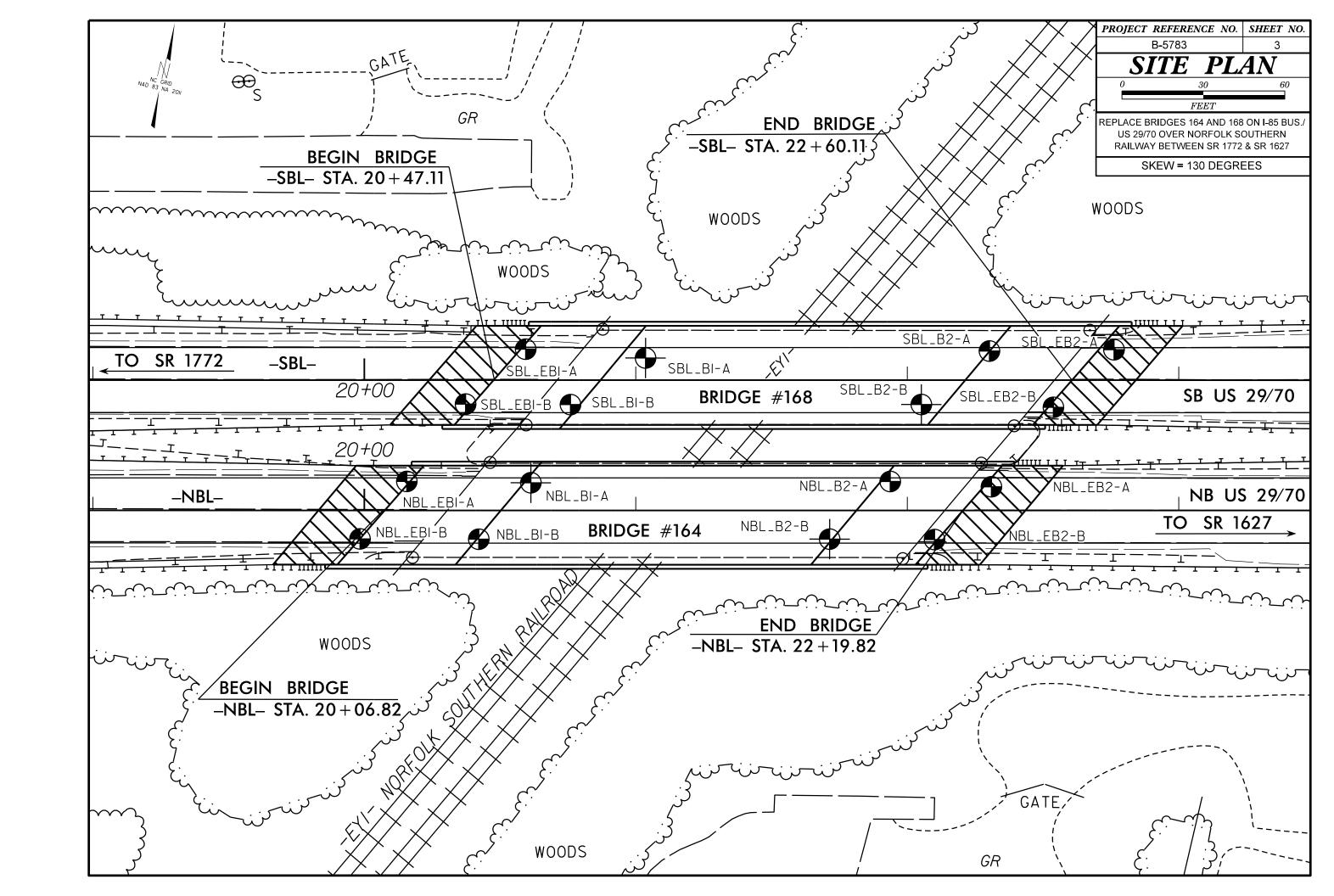
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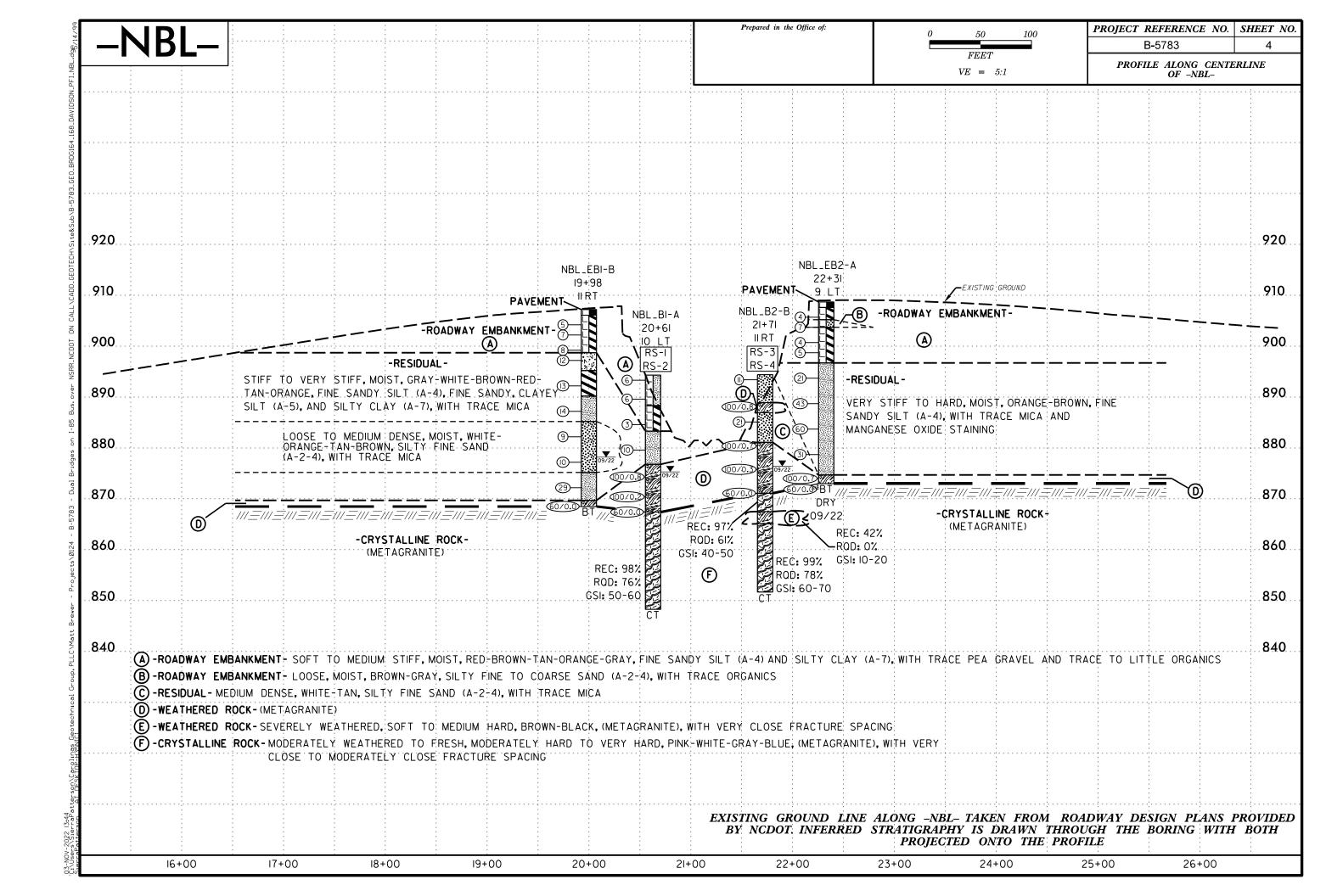
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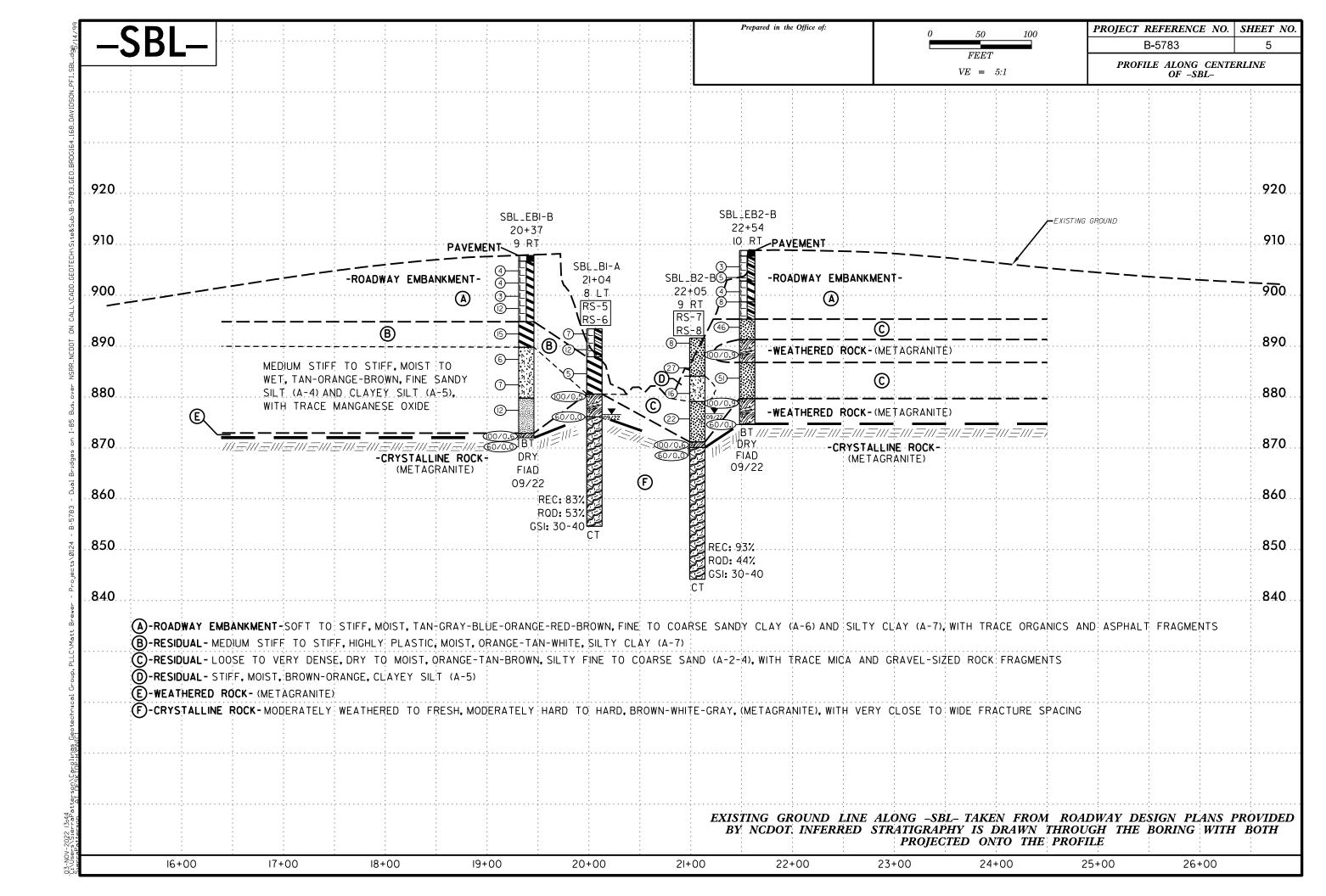
SUBSURFACE INVESTIGATION

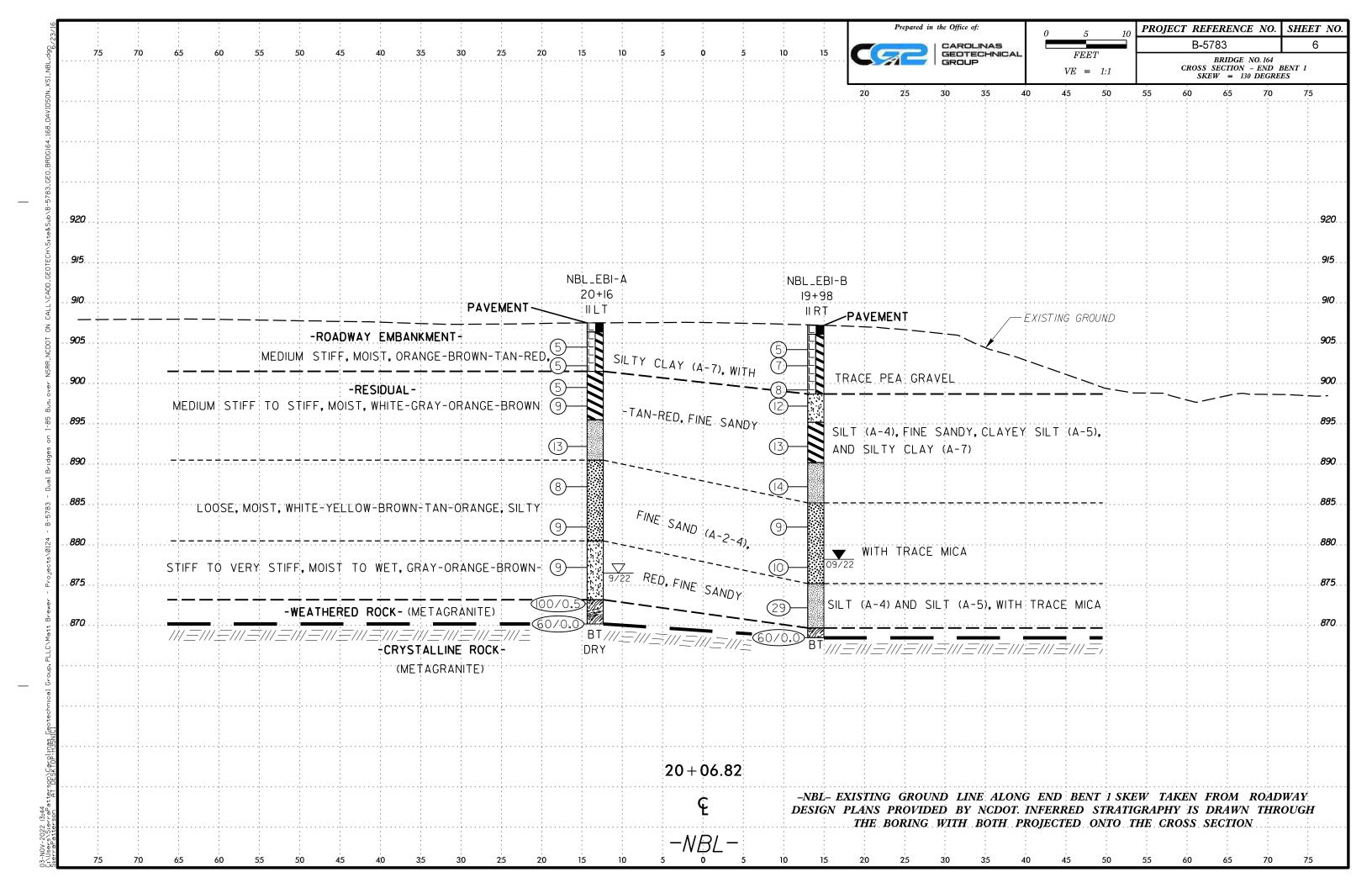
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

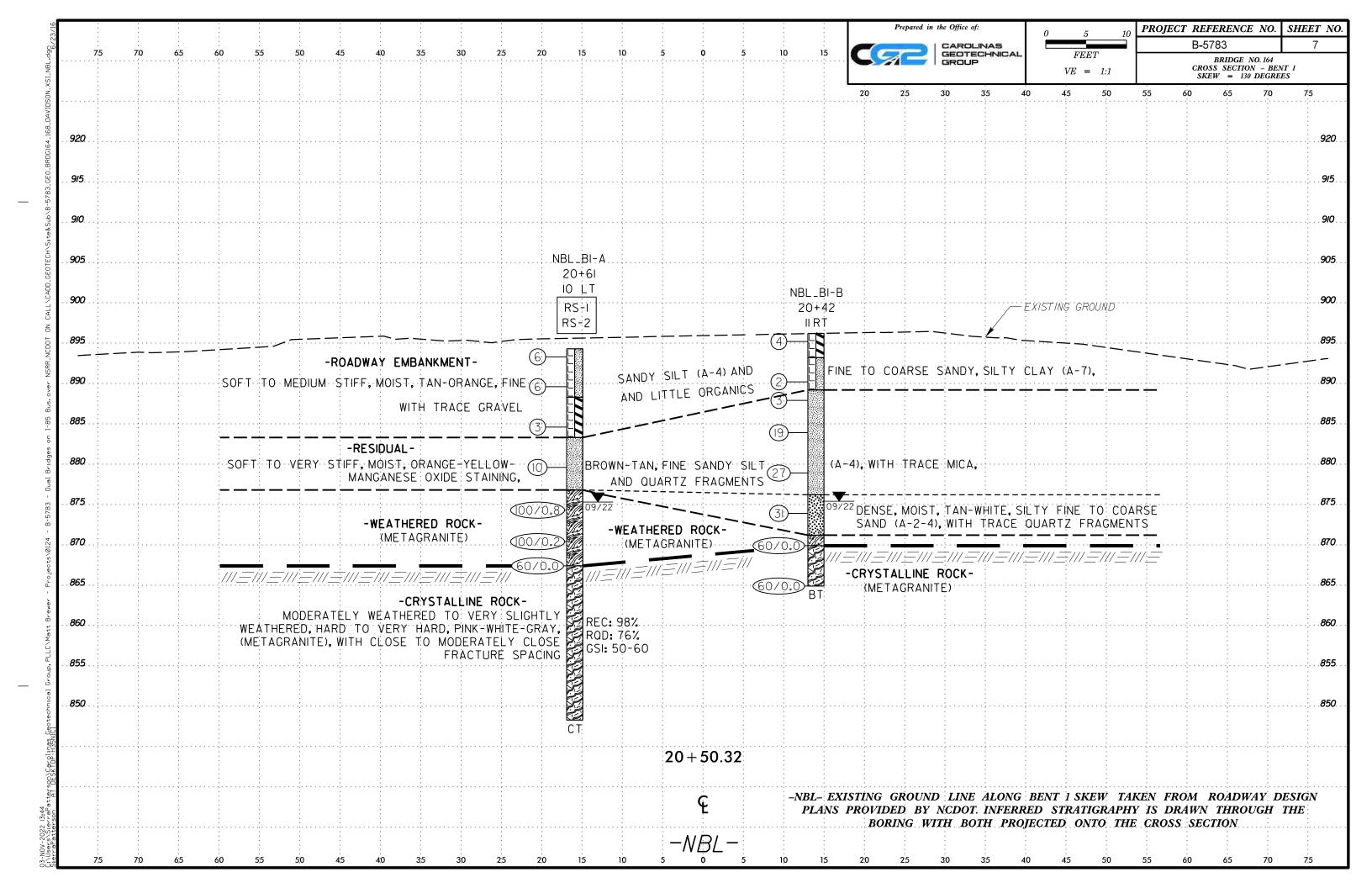
FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000) AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000) GEOLOGICAL STRENGTH INDEX (GSI) FOR GSI FOR HETEROGENEOUS ROCK MASSES SUCH JOINTED ROCKS (Hoek and Marinos, 2000) AS FLYSCH (Marinos, P and Hoek E., 2000) From a description of the lithology, structure and ed surfé fillings POOR - Very smooth, slicken-l or highly weathered surfaces soft clay coatings or fillings From the lithology, structure and surface athered surf or fillings smooth, occasionally surfaces with compac fillings with angular and conditions of the discontinuities, estimate the average value of GSI. Do not try to surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not planes) weather position in the box that corresponds to the condition weathered of the discontinuities and estimate the average value ther of GSI from the contours. Do not attempt to be too eq. apply to structurally controlled failures. Where weak planar structural planes are precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the highly wea coatings ragments weather slightly present in an unfavorable orientation SURFACE CONDITIONS (DISCONTINUITIES) Hoek-Brown criterion does not apply to structurally with respect to the excavation face, CONDITIONS these will dominate the rock mass controlled failures. Where unfavourably oriented behaviour. The shear strength of surfaces continuous weak planar discontinuities are present, in rocks that are prone to deterioration slightly es POOR Slickensided, h with compact c these will dominate the behaviour of the rock mass. Rough, as a result of changes in moisture content will be reduced if water is GOOD -thered - Very : ensided ings or f The strength of some rock masses is reduced by the **G00D** rough, presence of groundwater and this can be allowed for present. When working with rocks in the by a slight shift to the right in the columns for fair, fair to very poor categories, a shift to th, r FAIR - weather poor and very poor conditions. Water pressure does the right may be made for wet conditions. GOOD Rough, s surface VERY R sided with s VERY I POOR slicke coatir FAIR Smoot alter VERY Slick With Water pressure is dealt with by effective VERY Very not change the value of GSI and it is dealt with by stress analysis. using effective stress analysis. COMPOSITION AND STRUCTURE STRUCTURE DECREASING SURFACE QUALITY INTACT OR MASSIVE - intact A. Thick bedded, very blocky sandstone 90 rock specimens or massive in 7Ó N/A N/A The effect of pelitic coatings on the bedding situ rock with few widely spaced planes is minimized by the confinement of PIECES discontinuities the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally 80 controlled instability. 60 BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets 50 B. Sand C. Sand-D. Siltstone F. Weak 60 or silty shale si/tstone stone with stone and or clayey С thin inter siltstone with sandshale with layers of in similar stone layers VERY BLOCKY - interlocked. amounts sands tone siltstone 40 partially disturbed mass with 50 multi-faceted angular blocks formed by 4 or more joint sets INTERL C. D. E. and G - may be more or F. Tectonically deformed, BLOCKY/DISTURBED/SEAMY -30 less folded than illustrated but intensively folded/faulted, folded with angular blocks this does not change the strength. sheared clayey shale or siltstone formed by many intersecting Tectonic deformation, faulting and with broken and deformed CREASING loss of continuity moves these discontinuity sets. Persistence sandstone layers forming an 30 categories to F and H. of bedding planes or schistosity almost chaotic structure 20 DISINTEGRATED - poorly interlocked, heavily broken rock mass H 20 G. Undisturbed silty H. Tectonically deformed silty with mixture of angular and or clayey shale with or clayey shale forming a 10 rounded rock pieces or without a few very chaotic structure with pockets thin sandstone layers of clay. Thin layers of sandstone are transformed nto small rock pieces. 10 LAMINATED/SHEARED - Lack of blockiness due to close spacing N/A N/A → Means deformation after tectonic disturbance of weak schistosity or shear planes

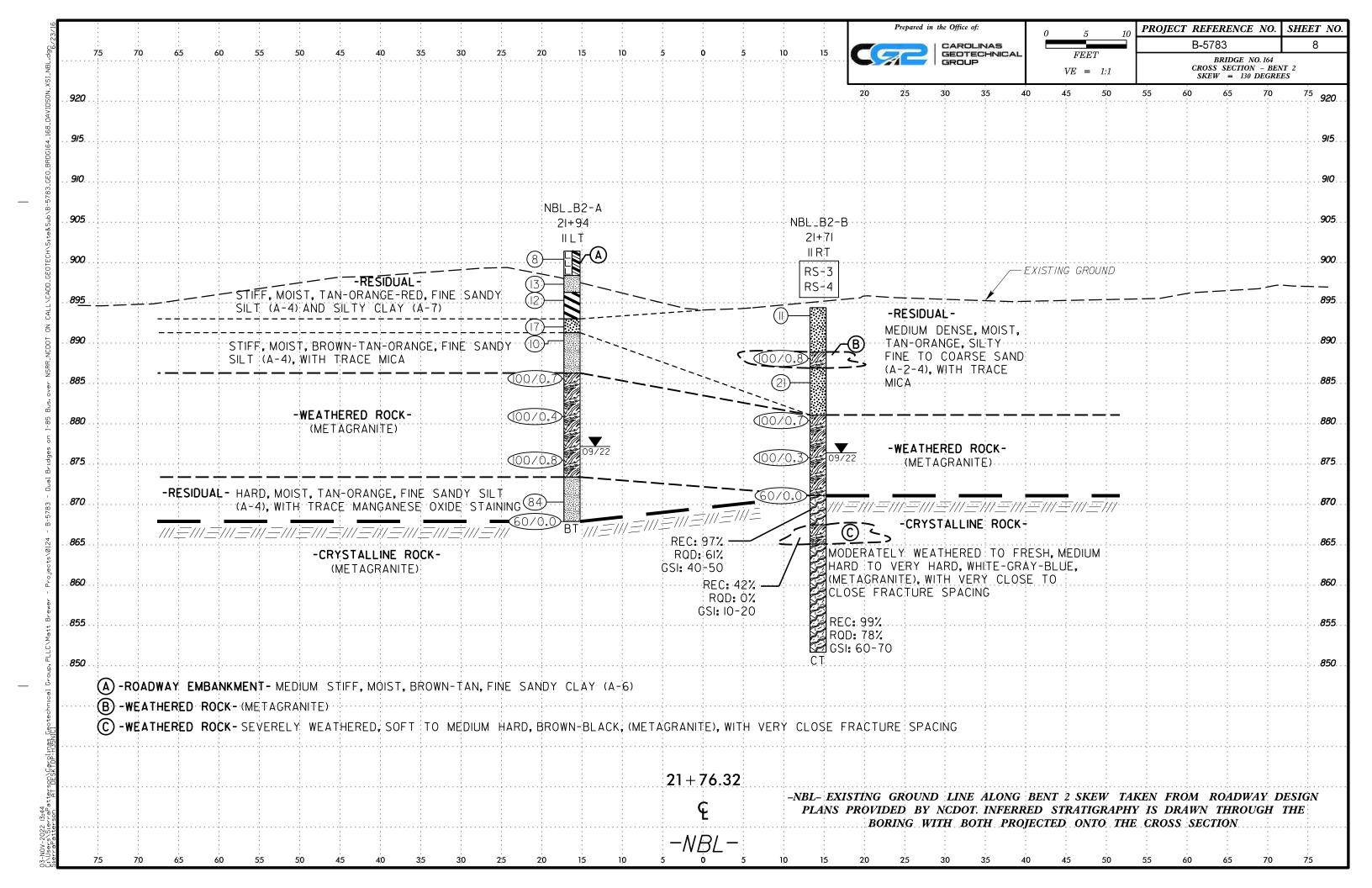


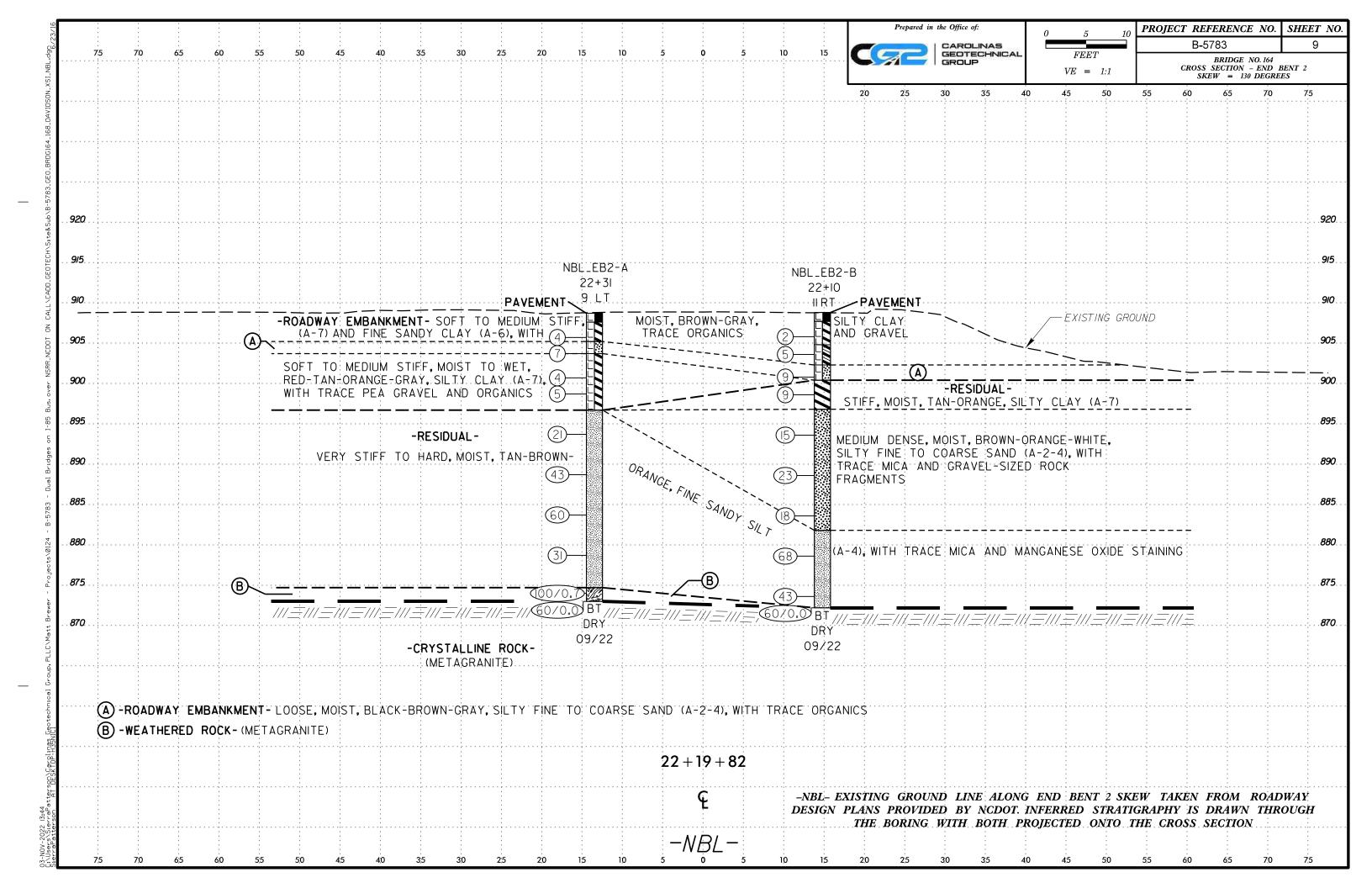


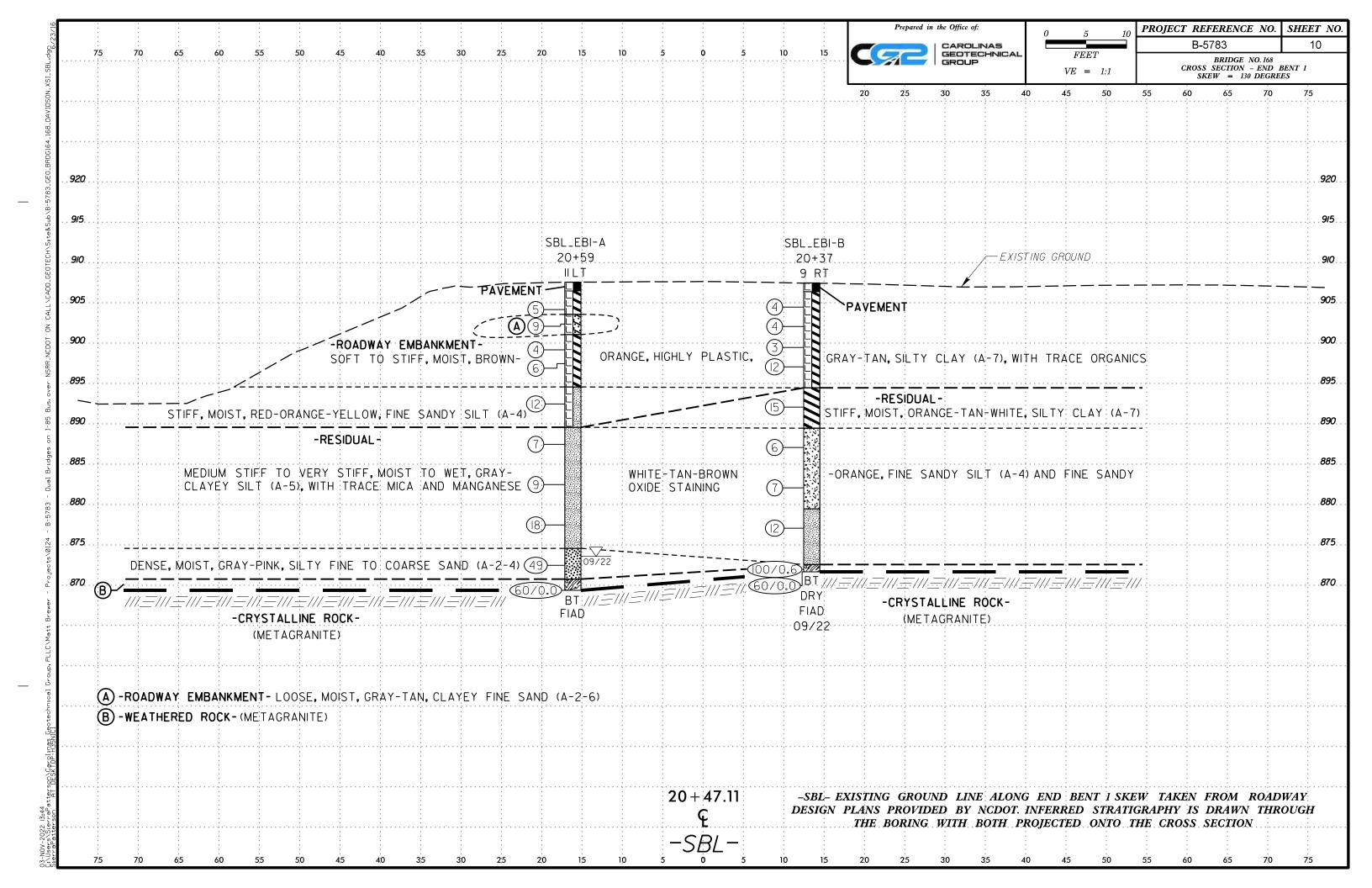


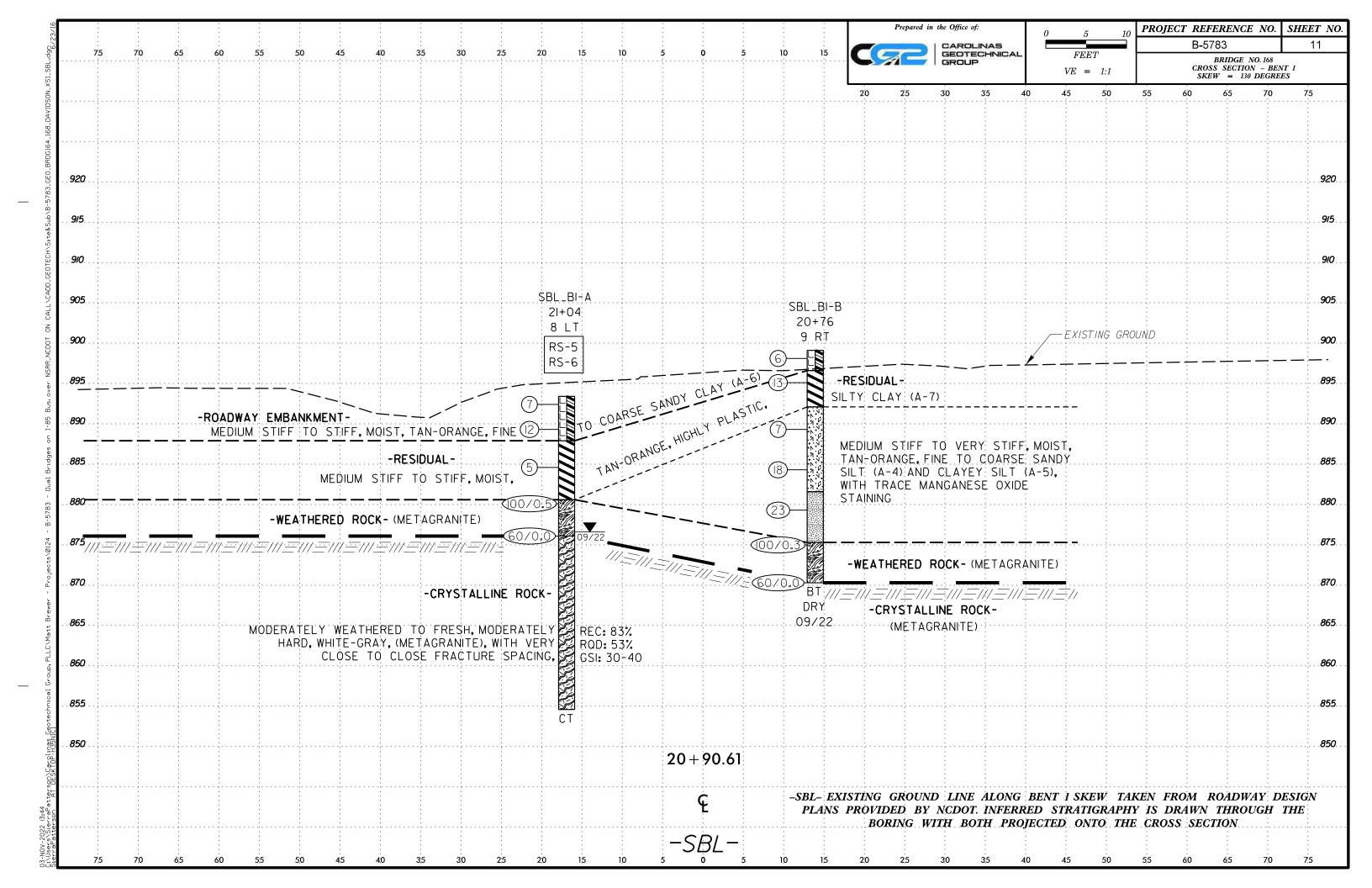


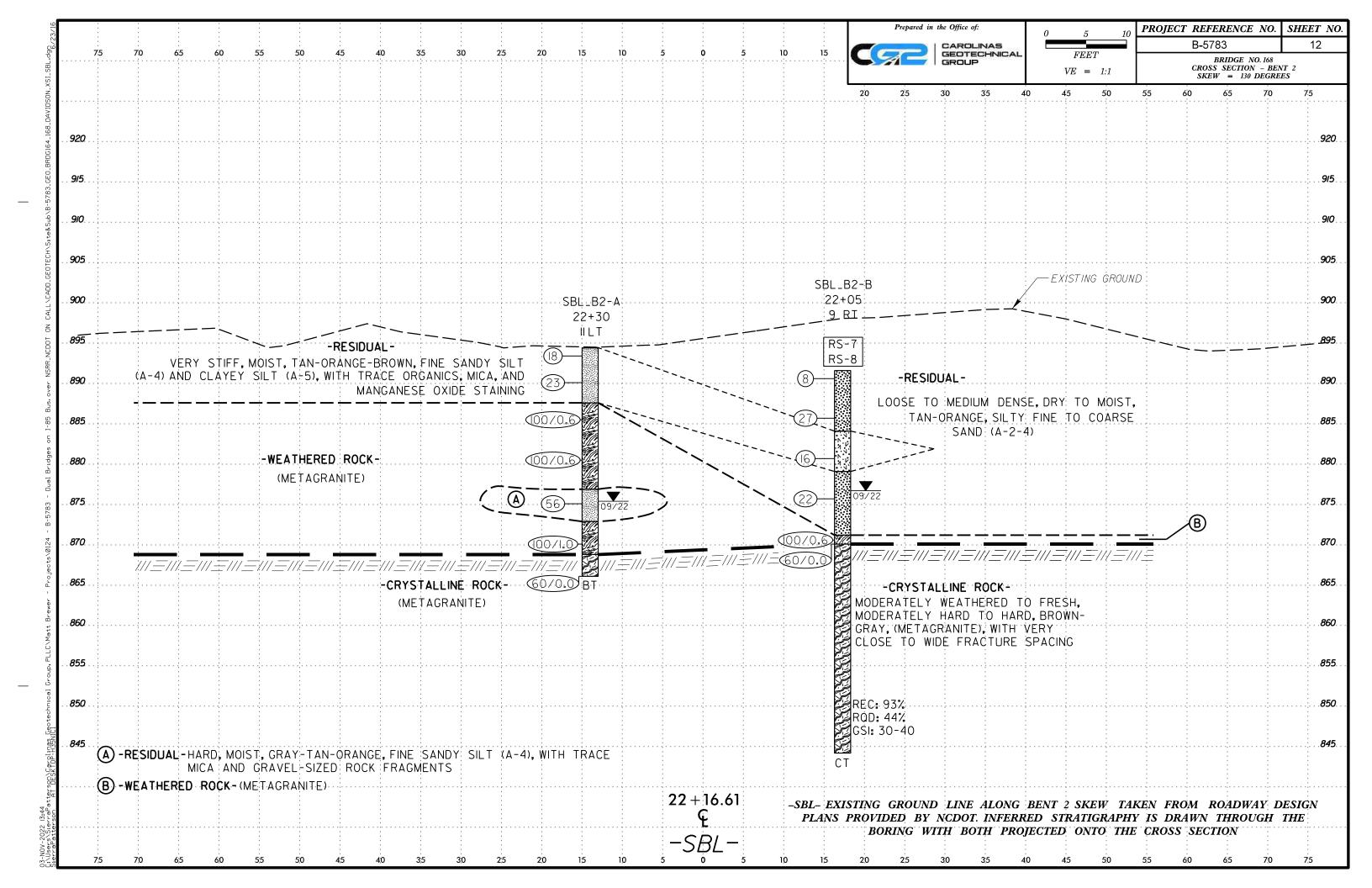


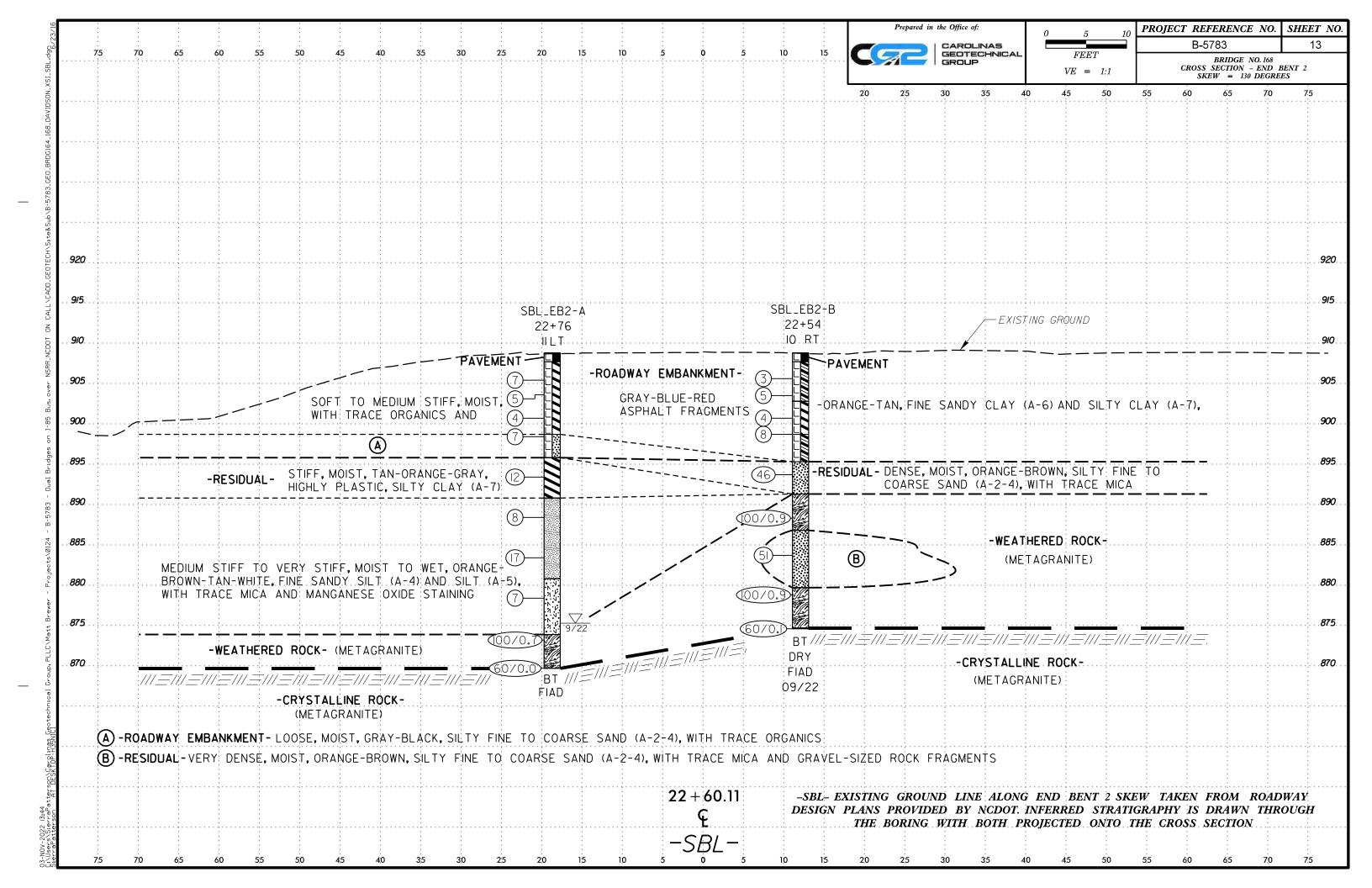


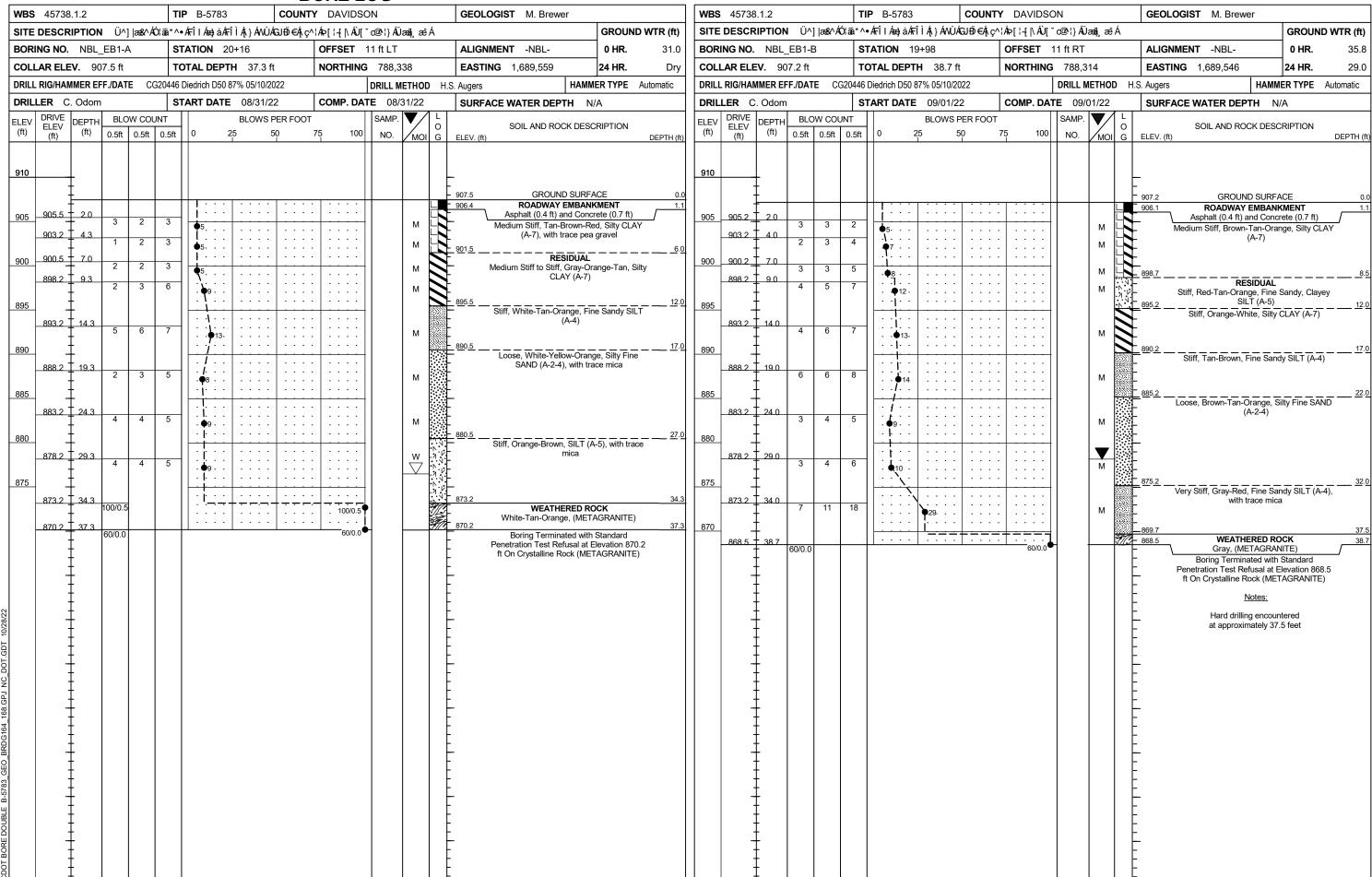












		BORE LO	G										CC	ORE LO	G			
WBS 45738.1.2	TIP B-5783	COUNTY DAVIDSON		GEOLOGIST R. Kral		-	S 45738.1.2			TIP B-578				DAVIDSON		GEOLOGIST R. Kral		
SITE DESCRIPTION Ü^] a	±&^ÁÓ¦ãã,*^•ÁFÎIÁsa)åÁFÎÌÁį}ÁNÙÁG.	ÜİÜ €Á,ç^¦Á⇔[¦-{ \ÁÛ[˘o@)	¦}ÁÜæajjæêÁ		GROUND WTR (ft)	SITI	E DESCRIPT	ion ü^	`] æ&^ÁÓ¦ãã	å*^∙Á√FÎIÁsa)	åÆfîÌÆ[}Á	NÙÁGJEÏ	•(^¦.ç^	ÁÞ[¦-{ \ÁÛ[˘o@	∖¦}ÁÜæaj́æêÁ		GROUND	WTR (ft)
BORING NO. NBL_B1-A	STATION 20+61	OFFSET 10 ft	LT	ALIGNMENT -NBL-	0 HR. N/A	BOF	ring no. Ni	BL_B1-A		STATION	20+61			OFFSET 10 ft	:LT	ALIGNMENT -NBL-	0 HR.	N/A
COLLAR ELEV. 894.3 ft	TOTAL DEPTH 46.0 ft	NORTHING 78		<u> </u>	24 HR. 19.0	COL	LAR ELEV.	894.3 ft		TOTAL DE	PTH 46.	0 ft	1	NORTHING 78		EASTING 1,689,604	24 HR.	19.0
DRILL RIG/HAMMER EFF./DATE	CG20446 Diedrich D50 87% 05/10/202		LL METHOD SF	-	IER TYPE Automatic	DRIL	L RIG/HAMMER	R EFF./DAT	re CG2044	46 Diedrich D50	0 87% 05/10	/2022		DRI	ILL METHOD SP	· · · · · · · · · · · · · · · · · · ·	HAMMER TYPE A	Automatic
DRILLER C. Odom	START DATE 09/06/22			SURFACE WATER DEPTH N	/A	-	LLER C. Od			START DA			- 1	COMP. DATE	09/07/22	SURFACE WATER DEF	TH N/A	
F F(/ 3-:	COUNT		MP. L O	SOIL AND ROCK DESC		l 	RE SIZE NQ		I DBILL	TOTAL RU			ΑΤΑ					
(II) (7 0.311 0	7.5it 0.5it 0 20 00	1 100 100	O. MOI G	ELEV. (ft)	DEPTH (ft)	ELE\ (ft)		PTH RUN t) (ft)	DRILL RATE (Min/ft)	RUN REC. RQD (ft) (ft) %	SAMP. NO.	REC.	ATA RQD (ft) %	O	С	DESCRIPTION AND REMARK	(S	DEDTI (#)
895						867.3	(1.1)		(IVIIII/IC)	% % 		%	%	G ELEV. (ft)		Begin Coring @ 26.9 ft		DEPTH (ft)
894.3 0.0	2 4 1 1		M	ROADWAY EMBAN			867.4 + 26	5.9 4.1	N=60/0.0 7:56/1.0 5:03/1.0	(3.9) (1.8) 95% 44%		(18.8)	(14.6)	867.4	Moderately to	CRYSTALLINE ROCK o Very Slightly Weathered, Ha	ard to Very Hard	26.9
				Medium Stiff, Tan-Orange, Fi (A-4), with trace gi		865	863.3 + 31	0	5:03/1.0 4:19/1.0	1	1	30 70	\$		Pink-White-Gray,	, (METAGRANITE), with mode fracture spacing	erately close to close	•
890 890.6 + 3.7	3 3		М	- -			-	5.0	0:54/0.1 2:23/1.0	(5.0) (4.2) 100% 84%	RS-1 /					RS-1: 31.6-32.2'		
				Soft, Tan-Orange, Fine to C		860	7 T		1 Z'10/1 ()	10070 0470					Unconfined	Unit Weight: 176.5 pcf d Compressive Strength: 5,23	0 pci (753 kcf)	
885 885.6 - 8.7	1 2			Silty CLAY (A-7), with litt	le organics		858.3 + 36	5.0	6:03/1.0 5:47/1.0 4:48/1.0	(5.0) (4.3)	RS-2				Oncomme	RS-2: 35.4-36.0'	o pai (700 kai)	
	$\left \begin{array}{c} - \\ - \\ - \\ \end{array} \right \left \begin{array}{c} \P^3 \\ - \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot \\ \cdot \\ \cdot \\ \cdot \\ \cdot \end{array} \right \left \begin{array}{c} \cdot \\ \cdot $			- 883.3 RESIDUAL	11.0	855	‡		2:30/1.0 2:46/1.0	(5.0) (4.3) 100% 86%					Unconfined	Unit Weight: 173.4 pcf Compressive Strength: 15,93	nei (2 294 kef)	
880.6 + 13.7				Stiff, Brown-Tan, Fine Sandy trace mica	SILT (A-4), with		853.3 41	.0	3:12/1.0 4:08/1.0						Officoriffica	GSI=50-60	0 psi (2,204 ksi)	
880 2	3 7		м	trace mica -				5.0	3:45/1.0 4:04/1.0	(4.9) (4.3) 98% 86%				848.3		201-00-00		
		· · · · · · · ·		- 876.8	17.5	850	848.3 + 46	1	4:16/1.0 5:16/1.0 3:00/1.0					.40.2				46.0
875 875.6 + 18.7	52 48/0.3		V	- WEATHERED RO Brown-Gray, (METAGI	RANITE)		040.5 40	7.0	3.00/1.0					- 040.0	Boring Termi	inated at Elevation 848.3 ft In (METAGRANITE)	Crystalline Rock	40.0
			▼ 3.1 5.2	- -			‡							-		Notes:		
870.6 - 23.7		100/0.2		- - <u>-</u>			‡							-		Hard drilling encountered		
+				- - 867.4	26.9		‡							-		at approximately 17.5 feet		
867.4 † 26.9 60/0.0		· · · · · · · · · · · · · · · · · · ·		CRYSTALLINE RO	ОСК									F				
865				- REC=98%	L)									E				
		RS	S-1	RQD=76% GSI=50-60			1 +							-				
860				_			‡							-				
			S-2	- -			‡							-				
855				- - _			‡							-				
							‡							-				
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850				- - 848.3	46.0									E				
+ +				Boring Terminated at Elevat Crystalline Rock (META)	tion 848.3 ft In		 							E				
+				Notes:	OIV WITE)									-				
70 +				- <u>Notes.</u> - Hard drilling encour	ntered		‡							-				
G				at approximately 17			‡							-				
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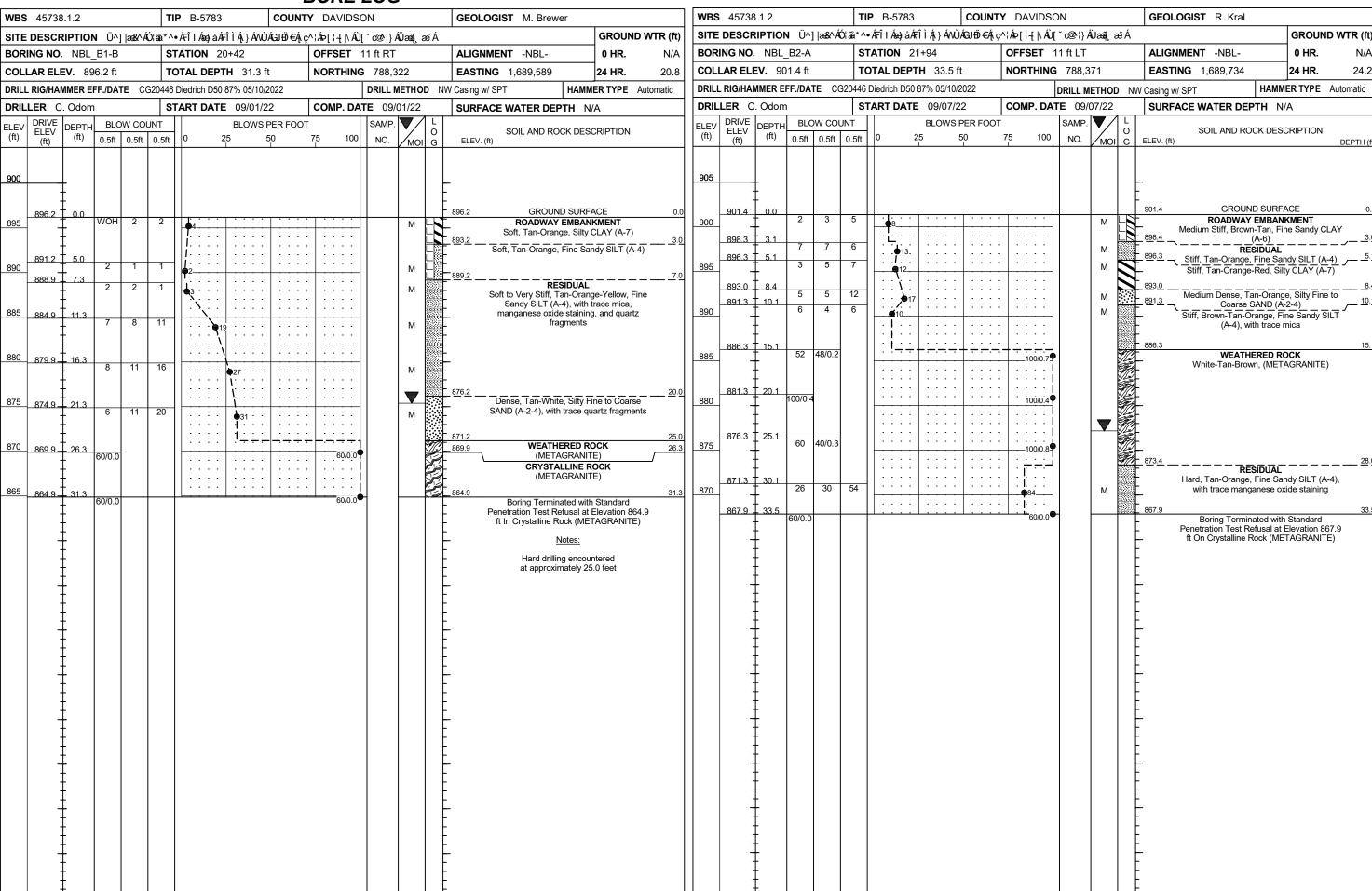


FYd`UWY'6f]X[Yg'% ('UbX'%, 'cb'l G'&- #\$'cj Yf'BcfZc`_'Gci l\ Yfb'FU]`k Um, Davidson County, NC Rock Core Photographs' Boring: NBL_B1-A'

26.9 to 46.0 FeetÁ

26.9 ft RS-1: 31.6-32.2' RS-2: 35.4-36.0'

FEET



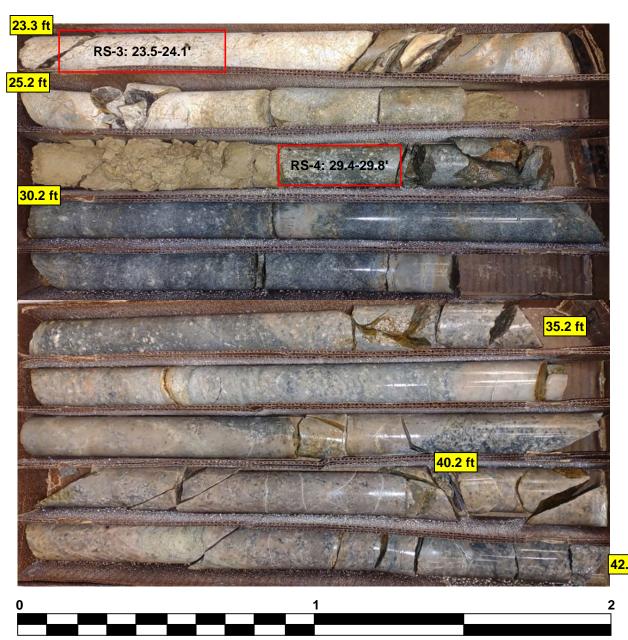
	BORE LOG				ORE LOG	
	COUNTY DAVIDSON	GEOLOGIST M. Brewer	WBS 45738.1.2		Y DAVIDSON	GEOLOGIST M. Brewer
SITE DESCRIPTION Ü^] æ&^ÁÓ¦ãá*^• ÆÎ I Æ; åÆÎ Ì Æ; ÁNÙÆGJ	Đi€Á,ç^¦Á≂[¦-[ÁÛ[ˇc@-¦}ÁÜæaj,æêÁ	GROUND WTR (ft)	SITE DESCRIPTION Ü^] æ&^ÁÓ¦â	nå*^•ÁrÎlÁ sa) åÁrÎÌÁş}ÁNÙÁGJÐİ€Áşç	^\AP[\-{ \AÛ[`c@\}AÜæaaj,æêÁ	GROUND WTR (
BORING NO. NBL_B2-B STATION 21+71	OFFSET 11 ft RT	ALIGNMENT -NBL- 0 HR. N/A	BORING NO. NBL_B2-B	STATION 21+71	OFFSET 11 ft RT	ALIGNMENT -NBL- 0 HR. NA
COLLAR ELEV. 894.4 ft TOTAL DEPTH 42.7 ft	NORTHING 788,346	EASTING 1,689,716 24 HR. 18.0	COLLAR ELEV. 894.4 ft	TOTAL DEPTH 42.7 ft	NORTHING 788,346	EASTING 1,689,716 24 HR . 18
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/10/2022	DRILL METHOD SE	T Core Boring HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE CG204	46 Diedrich D50 87% 05/10/2022	DRILL METHOD S	SPT Core Boring HAMMER TYPE Automatic
DRILLERC. OdomSTART DATE09/01/22	COMP. DATE 09/02/22	SURFACE WATER DEPTH N/A	DRILLER C. Odom	START DATE 09/01/22	COMP. DATE 09/02/22	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COUNT BLOWS PE BLOWS PE CHIP 400 7 0	SOIL AND ROCK DESCRIPTION	CORE SIZE NQ	TOTAL RUN 19.4 ft			
(ft) (ft) (ft) 0.5ft 0.5ft 0.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft) DEPTH (ft)	I I LEEV DEL RATE	TOTAL RUN	-	DESCRIPTION AND REMARKS
			$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	% % NO. % %	G ELEV. (ft)	DEPTH
895 894.4 0.0 2 5 6 2 4		894.4 GROUND SURFACE 0.0 RESIDUAL	871.13 870 871.1 7 23.3 1.9 N=60/0.0 869.2 7 25.2 03:43(0.9)	(1.8) (1.4) RS-3 (3.5) (2.2)	871.1	Begin Coring @ 23.3 ft CRYSTALLINE ROCK 2
<u> </u>	M	Medium Dense, White-Tan, Silty Fine SAND (A-2-4), with trace mica	 	(1.8) (1.4) RS-3 (3.5) (2.2) 95% 74% 97% 61%	Moderately to (METAGRA	o Slightly Weathered, Moderately Hard, White-Gray, ANITE), with close to very close fracture spacing
890		- -	865 004.0 00.0 00.0 00.0 00.0 00.0 00.0 00	(3.5) (1.3) 70% 26% (1.0) (0.0)	865.1	RS-3: 23.5-24.1'
888.9 5.5 45 55/0.3			865 864.2 30.2 03:43/1.0 06:09/1.0	42% 0% RS-4 (13.3) (10.4) 99% 78%	865.1 Unconfin	Unit Weight: 162.4 pcf ned Compressive Strength: 2,290 psi (330 ksf)
885 886.1 8.3 12 14 7		White-Orange-Tan, (METAGRANITE)7.5 - RESIDUAL		0 (5.0) (4.3) 99% 78%		GSI=40-50
885	M	Medium Dense, White-Tan, Silty Fine SAND (A-2-4), with trace mica	860 859.2 35.2 03:36/1.0 03:22/1.0 04:12/1.0		Savaraly V	WEATHERED ROCK Weathered, Medium Hard to Soft, Brown-Black,
		- 881.1 13.3	5.0 04:19/1.0	0 (5.0) (4.2)	Severely V (META Moderately V White-Gray-Blue	AGRANITE), with very close fracture spacing
880 881.1 13.3 69 31/0.2	100/0.7	WEATHERED ROCK Tan-Orange-Gray, (METAGRANITE)		0 100% 84%	Moderately \	CRYSTALLINE ROCK Weathered to Fresh, Medium Hard to Very Hard,
	· · · · · · · · · · 💯	- Tail-Offinge-Gray, (METAGRANITE)	855 854.2 40.2 06:36/1.0 07:03/1.0 2.5 08:47/1.0 11:04/1.1 45:04/0.6	(2.5) (1.4)	White-Gray-Blue	ue, (METAGRANITE), with very close to close fracture spacing
875 876.1 18.3 100/0.3	100/0.3	<u> </u>	851.7 42.7 11:04/1.0 15:29/0.5	100% 56%	851.7	RS-4: 29.4-29.8' 4
†		- -	+ (15.29/0.3			Unit Weight: 165.9 pcf ed Compressive Strength: 13,650 psi (1,966 ksf)
871.1 23.3		- - 871.1 23.3			-	GSI=60-70
870 60/0.0	RS-3	CRYSTALLINE ROCK (METAGRANITE)			Boring Terr	rminated at Elevation 851.7 ft In Crystalline Rock (METAGRANITE)
		- 867.5 REC=97%				
865		RQD=61% GSI=40-50]			
<u> </u>	RS-4	- WEATHERED ROCK - Brown-Black, (METAGRANITE)			 	
		- REC=42%			E	
860		RQD=0% GSI=10-20			<u> </u>	
		CRYSTALLINE ROCK (METAGRANITE)			-	
855		<u> </u>				
		REC=99% RQD=78% SSI=60-70 42.7				
+ + + + + + + + + + + + + + + + + + + +		Boring Terminated at Elevation 851.7 ft In				
		Crystalline Rock (METAGRANITE) -				
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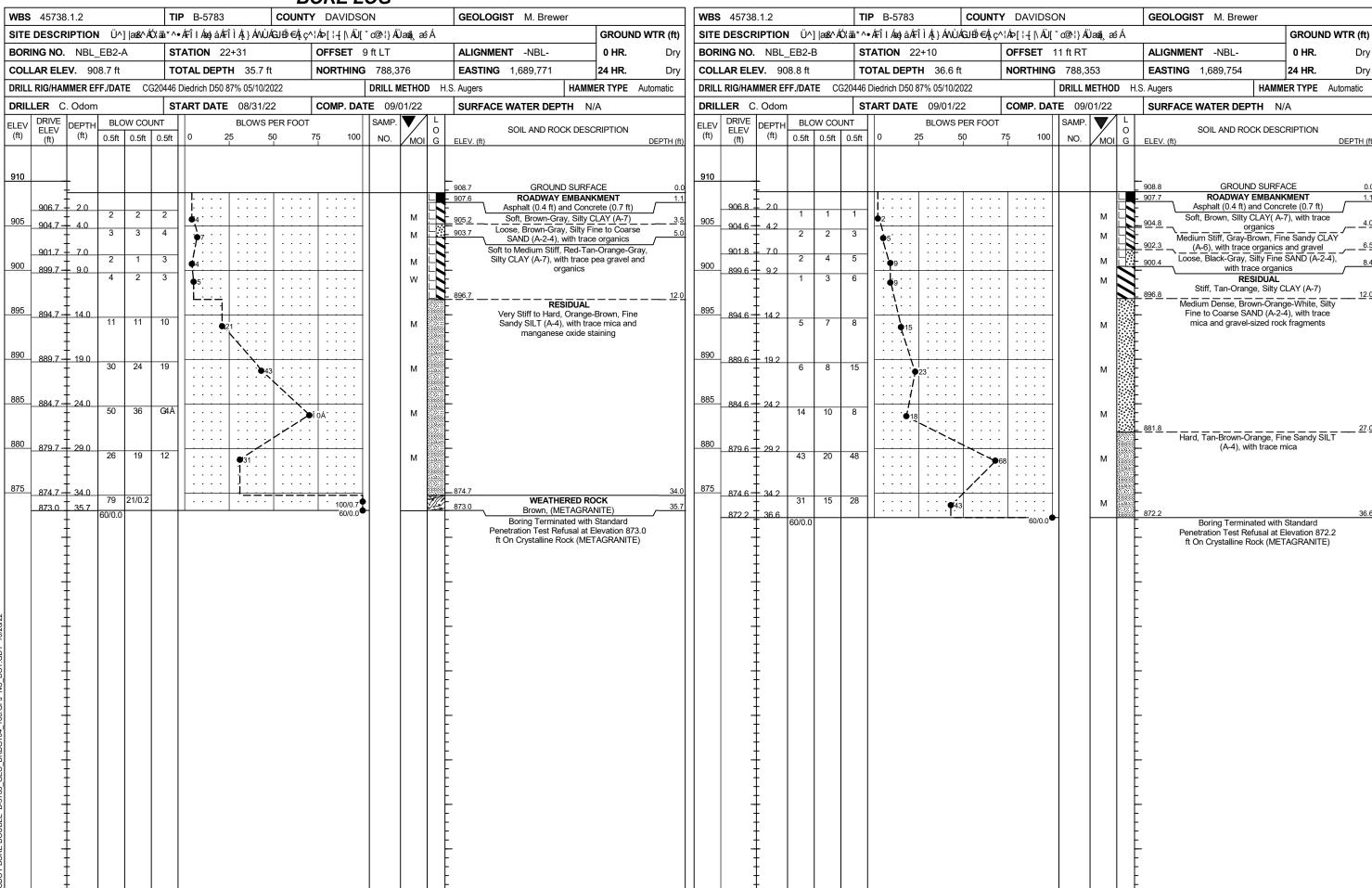
FYd`UWY'6f]X[Yg'% ('UbX'%, 'cb'l G'&-#\$'cjYf'BcfZc`_'Gci l\Yfb'FU]`k Um, Davidson County, NC'

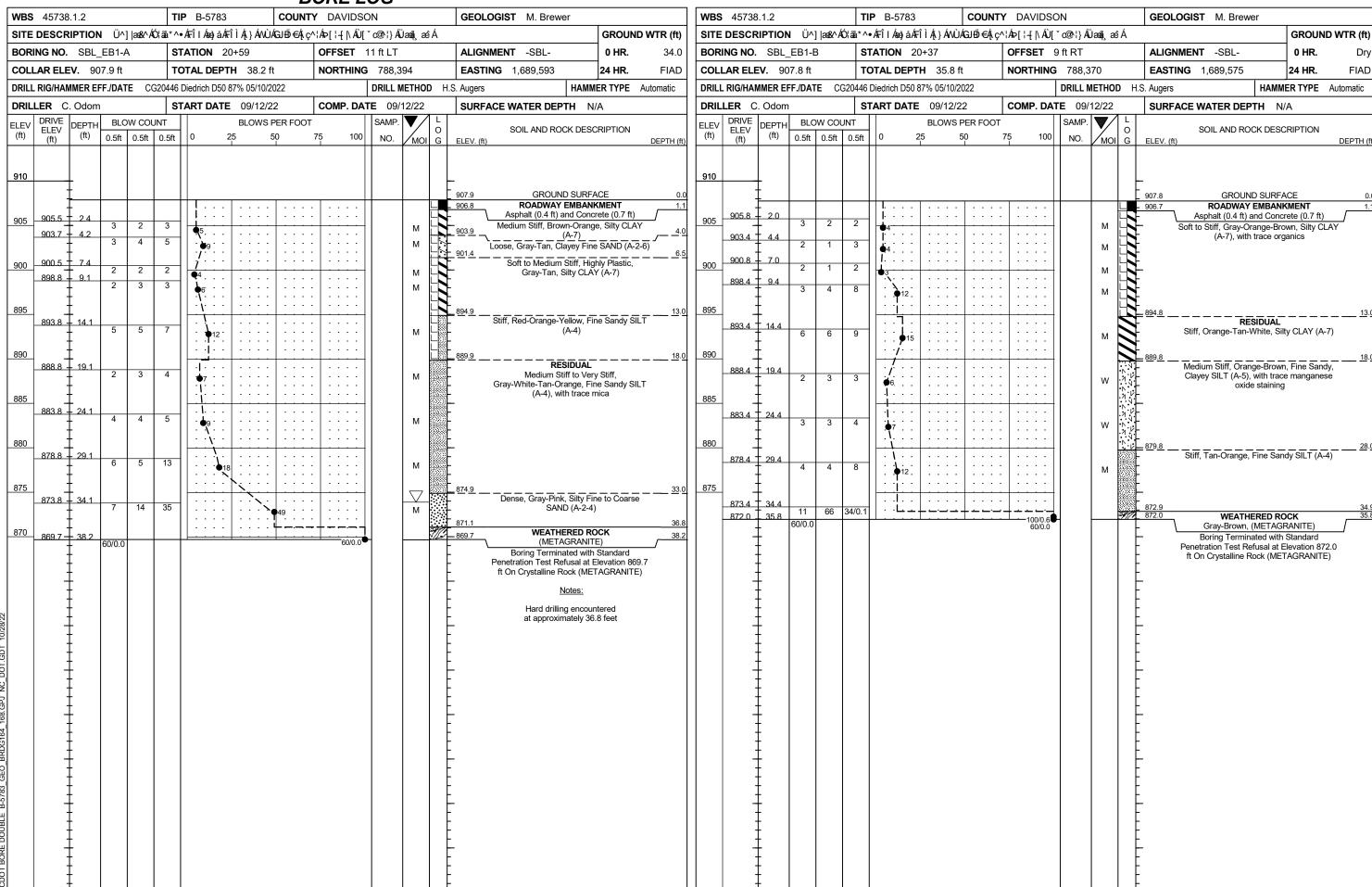
Rock Core Photographs Boring: NBL_B2-B

23.3 to 42.7 FeetÁ



FEET





WRS	45738	12			Т	TP B-5783			ORE L	ON			GEOLOGIST T. Wenner				
			J I"I^] 242∧ <i>ĥ</i> r		<u>⊪ Б-5763</u> \•ÁπÎlÁsa)åÁπÎ	ÌΔίιΔωνίν				ഷപ്പ	Á	JEGEOGIST 1. Weililei	GROUND WTR (ft)			
	ING NO.] aux ·A		STATION 21		on an eAr c∖.	OFFSET		ada, ad	A	ALIGNMENT -SBL-	0 HR. N/A			
					_						200			-			
	AR ELE			. 00		OTAL DEPTH			NORTHING			D 01	EASTING 1,689,637	24 HR. 16.8 IER TYPE Automatic			
	RIG/HAN			E ((Diedrich D50 8			COMP. DA	DRILL I		וט טו	<u> </u>				
	DRIVE	l		W COL		DATE		ER FOOT	COIVIP. DA	SAMP.	00/22	1 [SURFACE WATER DEPTH N	/A			
ELEV (ft)	ELEV (ft)	DEPTH (ft)	0.5ft		0.5ft	0 25			75 100	NO.	MOI	0	SOIL AND ROCK DESC	CRIPTION DEPTH (
	(11)					11 '					IVIOI		ELEV. (ft)	DEPTH			
895																	
000	893.4	- 0.0											- 893.4 GROUND SURFA				
	-	 	2	2	5						М		ROADWAY EMBANI Medium Stiff to Stiff, Tan-O				
890	890.3	3.1	7	6	6	12.	· · · ·		· · · · ·		М		Coarse Sandy CLA	(A-6)			
	-	-											887.9 	5			
885	885.6 -	7.8	2	2	3								Medium Stiff, Tan-Orange, Silty CLAY (A-	Highly Plastic,			
	-	F		2	3	6 5					М		- Silly CLAT (A-	()			
	-	100						: : : :						40			
880	880.6 -	12.8	100/0.5			'		+====	100/0.5				- 880.6 - WEATHERED RO				
	-	F											- Tan-Gray, (METAGR -	ANITE)			
875	876.1	17.3	60/0.0						- 60/0.0		_		876.1 CRYSTALLINE R	OCK 17			
	_	F											. (METAGRANIT	E)			
]	E								RS-5	1		REC=83% RQD=53%				
870	-									RS-6			GSI=30-40				
	-	E											•				
865	_												- - -				
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860	-	[+				<u> </u>				
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855	_												- 854.6	38.			
	-												- Boring Terminated at Elevat - Crystalline Rock (META	ion 854.6 ft In			
	-												- Orystalline Nock (IVIL 174	OIVANITE)			
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									C	OI	RE LOG				
WBS	45738	.1.2			TIP	B-578	3	C	OUNT	Y D	AVIDSON	GEOLOGIST T. Wenr	ner		
SITE	DESCR	IPTION	\ Ü^]	æ&^ÁÓ¦ã	*^• <i>Á</i> FÍ	ÌlÁse)å	ÁFÎÌÁ[}Á	WÙÁGJI	ÐİĄ∱ç	^¦Æp∣	¦-{ \ÁÛ[ઁc@^¦}ÁÜæ ā j,æÂÁ			GROUN	ND WTR (ft)
BOR	NG NO.	SBL_	B1-A		STAT	ΓΙΟΝ	21+04			OF	FSET 8 ft LT	ALIGNMENT -SBL-		0 HR.	N/A
COLI	AR ELE	V. 89	3.4 ft		TOTA	AL DE	PTH 38.	8 ft		NO	RTHING 788,399	EASTING 1,689,637		24 HR.	16.8
DRILL	RIG/HAM	MER EF	F./DATI	E CG2044	16 Diedr	ich D50	87% 05/10	/2022			DRILL METHOD S	SPT Core Boring	HAMN	IER TYPE	Automatic
DRIL	LER C.	Odom			STAF	RT DA	TE 09/0	8/22		CC	MP. DATE 09/08/22	SURFACE WATER DE	PTH N	/A	
COR	E SIZE	NQ			TOTA	AL RUI	V 21.5 f								
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft)	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RQD (ft) %	L O G	ELEV. (ft)	DESCRIPTION AND REMAR	KS		DEPTH (ft)
876.14		- 170										Begin Coring @ 17.3 ft			
875	876.1 <u> </u>	17.3 - - 19.8 -	2.5 5.0	N=60/0.0 2:15/1.0 1:34/1.0 0:14/0.5	(2.5) 100% (5.0)	(3.9)		(17.9) 83%	(11.3) 53%		(METAGRANITE	CRYSTALLINE ROCK Veathered to Fresh, Moderately), with very close to moderately	close fra	acture space	cing,
870	-	- - 		2:55/1.0 2:06/1.0 2:29/1.0 2:37/1.0	100%	78%	RS-5 RS-6				_ severely weathere - —	ed seam from 19.3 to 20.2 feet, to 27.3 feet	and soil	seam from	20.8
	868.6 - - -	- 24.8 - -	5.0	3:38/1.0 3:42/1.0 2:41/1.0	(4.4) 88%	(3.3) 66%					- - - Unconfir	RS-5: 20.6-21.2' Unit Weight: 172.5 pcf ned Compressive Strength: 4,8	20 psi (69	94 ksf)	
865	863.6 -	- - - 29.8	5.0	3:15/1.0 3:13/1.0 5:56/1.0	(3 0)	(1.9)					- - -	RS-6: 23.2-23.7' Unit Weight: 167.1 pcf	0 nc: /4 4	00 leaf)	
860	-	- - -	3.0	2:12/1.0 2:07/1.0 4:07/1.0 7:39/1.0	(3.8) 76%	38%					- Unconfine - -	ed Compressive Strength: 7,80 GSI=30-40	o psi (1,1	∠o KSI)	
	858.6 - -	- 34.8 - -	4.0	6:50/1.0 4:20/1.0 2:53/1.0 5:16/1.0	(2.2) 55%	(0.8) 20%					- - -				
855	854.6 –	- - 38.8		5:16/1.0 6:50/1.0						Z	854.6	minated at Elevation 854.6 ft Ir			38.8
												(METAGRANITE)			

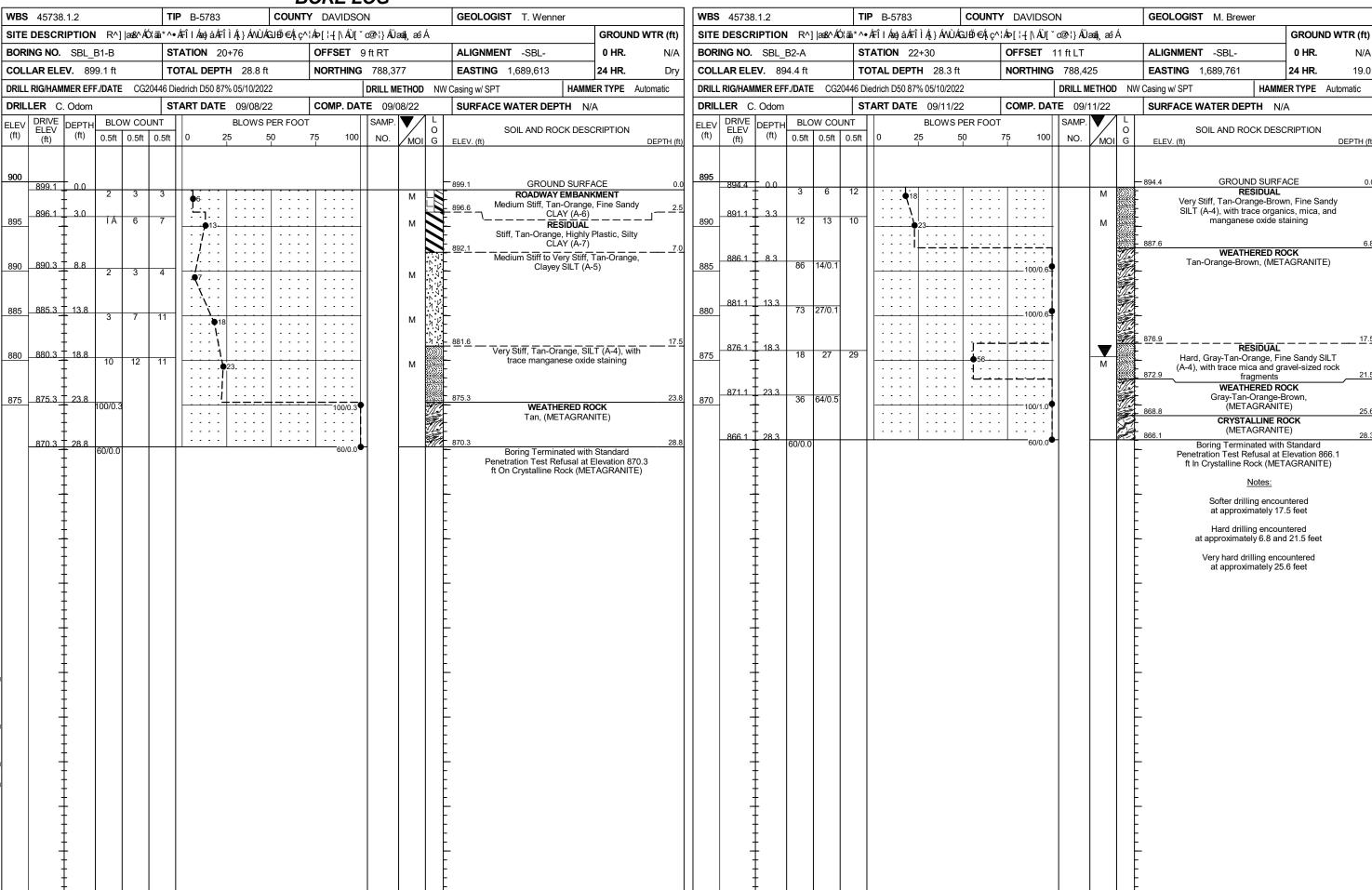


FYd`UWY'6f]X[Yg'% ('UbX'%, 'cb'l G'&-#+\$'cj Yf'B'cfZc`_'Gci l\ Yfb'FU]`k Um, Davidson County, NC' Rock Core Photographs' Boring: SBL_B1-A'

17.3 to 38.8 FeetÁ

17.3 ft RS-5: 20.6-21,2 RS-6: 23.2-23.7' 38.8 ft

FEET

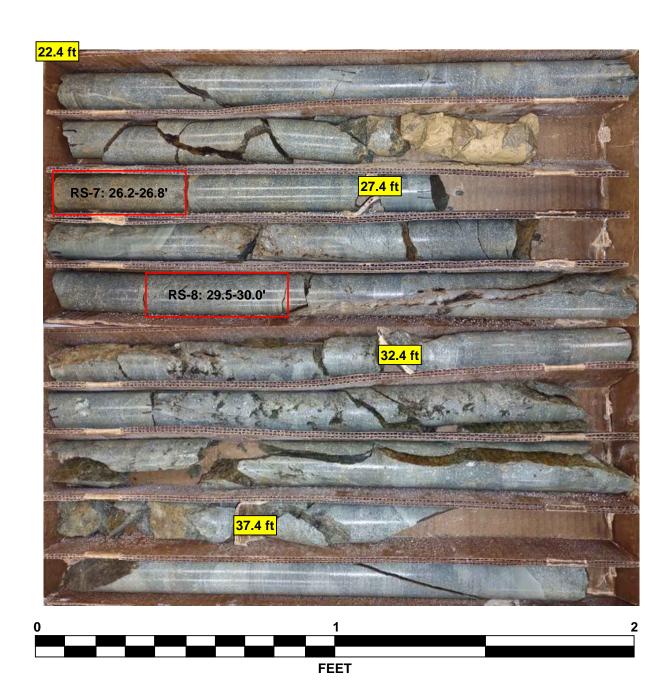


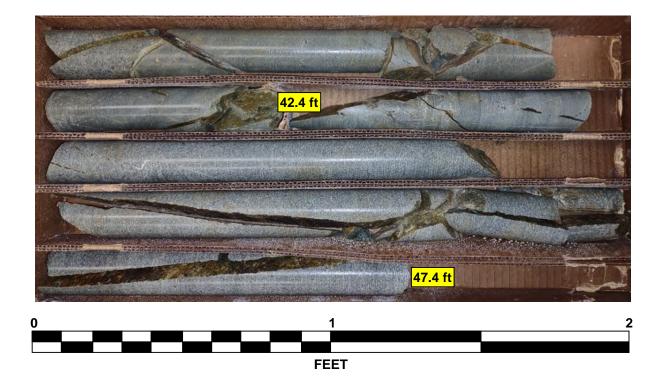
	BORE LOG								C	ORE L	OG		
WBS 45738.1.2 TIP B-5783 COU	INTY DAVIDSON	GEOLOGIST T. Wenner		WB	S 45738.1.2		TIP B-57	83	COUN	NTY DAVIDS	ON	GEOLOGIST T. Wenne	er _
SITE DESCRIPTION Ü^] æ&^ÁÓ¦ãå*^• ÁFÎ I Áæ) åÆÎ Ì Á;} ÁNÙÆGJÐ €	Áşç^¦Án⊳[¦-{ \ÁÛ[ĭc@^¦}ÁÜæánj,æêÁ		GROUND WTR (ft)	SIT	E DESCRIPTIO	N R^] æ&^ÁÓ¦	ãa*^∙ÁrÎlÁse)	åÆfîÌÆį}ÁNÙ	ÁGJÐÖ€Á	[iç^¦Á⊅[¦-{ \Áû	J[čo@\¦}ÁÜæajaêÁ		GROUND WTR (ft)
BORING NO. SBL_B2-B STATION 22+05	OFFSET 9 ft RT	ALIGNMENT -SBL-	0 HR. N/A	BOF	ring no. SBL_	B2-B	STATION	22+05		OFFSET	9 ft RT	ALIGNMENT -SBL-	0 HR . N/A
COLLAR ELEV. 891.6 ft TOTAL DEPTH 47.4 ft	NORTHING 788,401	EASTING 1,689,740	24 HR. 14.9	COI	LAR ELEV. 8	91.6 ft	TOTAL DI	EPTH 47.4 f	t	NORTHIN	G 788,401	EASTING 1,689,740	24 HR. 14.9
DRILL RIG/HAMMER EFF./DATE CG20446 Diedrich D50 87% 05/10/2022	DRILL METHOD S		MER TYPE Automatic	DRIL	L RIG/HAMMER E	F./DATE CG20					DRILL METHOD SP		HAMMER TYPE Automatic
DRILLER C. Odom START DATE 09/07/22	COMP. DATE 09/08/22	SURFACE WATER DEPTH N	/A	→	LLER C. Odom	l		ATE 09/07/2	2	COMP. DA	ATE 09/08/22	SURFACE WATER DEP	PTH N/A
ELEV (ft)	400 '/ 0	SOIL AND ROCK DES		I —	RE SIZE NQ	I I DDIII	RUN		STRATA	<u> </u>			
(ii) (ft) (iii) 0.5ft 0.5ft 0.5ft 0 25 50	75 100 NO. MOI G	ELEV. (ft)	DEPTH (ft)	ELE\		RUN RATE (ft) (Min/ft)	REC. RQD	SAMP. R	STRATA EC. RQ ft) (ft) %	A DD O G G	С	DESCRIPTION AND REMARK	S
895				870	1	(**************************************			%			Begin Coring @ 21.5 ft	
-		- -		087.0	869.2 22.5	0.9 N=60/0. 11:14/0.	0 (0.0) (0.0)	(2	4.0) (11. 3% 44°	.5) 870.1	Moderately Weathe	CRYSTALLINE ROCK ered to Fresh, Moderately Hard	21.4 d to Hard Brown-Gray
891.6 + 0.0		891.6 GROUND SURF			†	3:31/1.0 3:03/1.0 3:03/1.0	0% /(0% 0Å (4.8) (3.4) €A 96% 68% 0 0 (4.0)		370 44.			NITE), with very close to wide	fracture spacing
890	M M	− RESIDUAL Loose to Medium Dense, Ta Fine to Coarse SANI	an-Orange, Silty	865	864.2 27.4	1:24/1.0 2:40/1.0	0 96% 66% 0 0A	RS-7				RS-7: 26.2-26.8' Unit Weight: 185.3 pcf	
886.7 + 4.9		Fille to Coarse SAINL	D (A-2-4)		1 1	5.0 2:39/1.0	0 (4.9) (1.3) 0 98% 26%				Unconfined	d Compressive Strength: 3,88	0 psi (559 ksf)
885 11 13 14	D		7.5	860	859.2 32.4	2:17/1.0 2:14/1.0 1:55/1.0	0	RS-8				RS-8: 29.5-30.0' Unit Weight: 180.2 pcf	
		Very Stiff, Brown-Orange, (A-5)	, Clayey SILT		059.2 32.4	5.0 8:04/1.0	0 (4.9) (1.8) 0 98% 36%	1			Unconfined	d Compressive Strength: 3,55	0 psi (511 ksf)
880 4 8 8		(A-5)		855	‡	2:08/1.0 2:19/1.0	0					GSI=30-40	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		879.1 Medium Dense, Tan-Orang	ge. Silty Fine to	633	854.2 37.4	1 2.17/1 (n I	-					
876.7 14.9 20 13 9		Coarse SAND (A	A-2-4)		†	1:42/1.0	0 (4.6) (2.6) 0 92% 52% 0						
875	M	[850	849.2 42.4	4:44/1.0 2:41/1.0	0 0						
871 7 10 0			20.4			5.0 2:25/1.0	0 (4.8) (2.4) 0 96% 48%						
870 870.1 21.5 60/0.0	100/0.6	870.1 WEATHERED R	POCK 21.5	845	844.2 + 47.4	1:59/1.0 1:42/1.0 2:27/1.0	0			844.2			47.
	60/0.0	871.2 870.1 Brown-Tan, (METAG CRYSTALLINE R (METAGRANI) REC=93% RQD=44% GSI=30-40	ROCK		044.2	2.21/1.0		1		- 044.2		inated at Elevation 844.2 ft In (METAGRANITE)	
865		L (METAGRANII	•		‡					-		(INETACIONITE)	
T	RS-7	REC=93% RQD=44%								F			
	RS-8	GSI=30-40			†								
860		_			 					F			
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855		-			‡								
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850		-			‡					-			
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845		844.2 Boring Terminated at Eleva	47.4	4	+					-			
		Crystalline Rock (META	AGRANITE)		‡					-			
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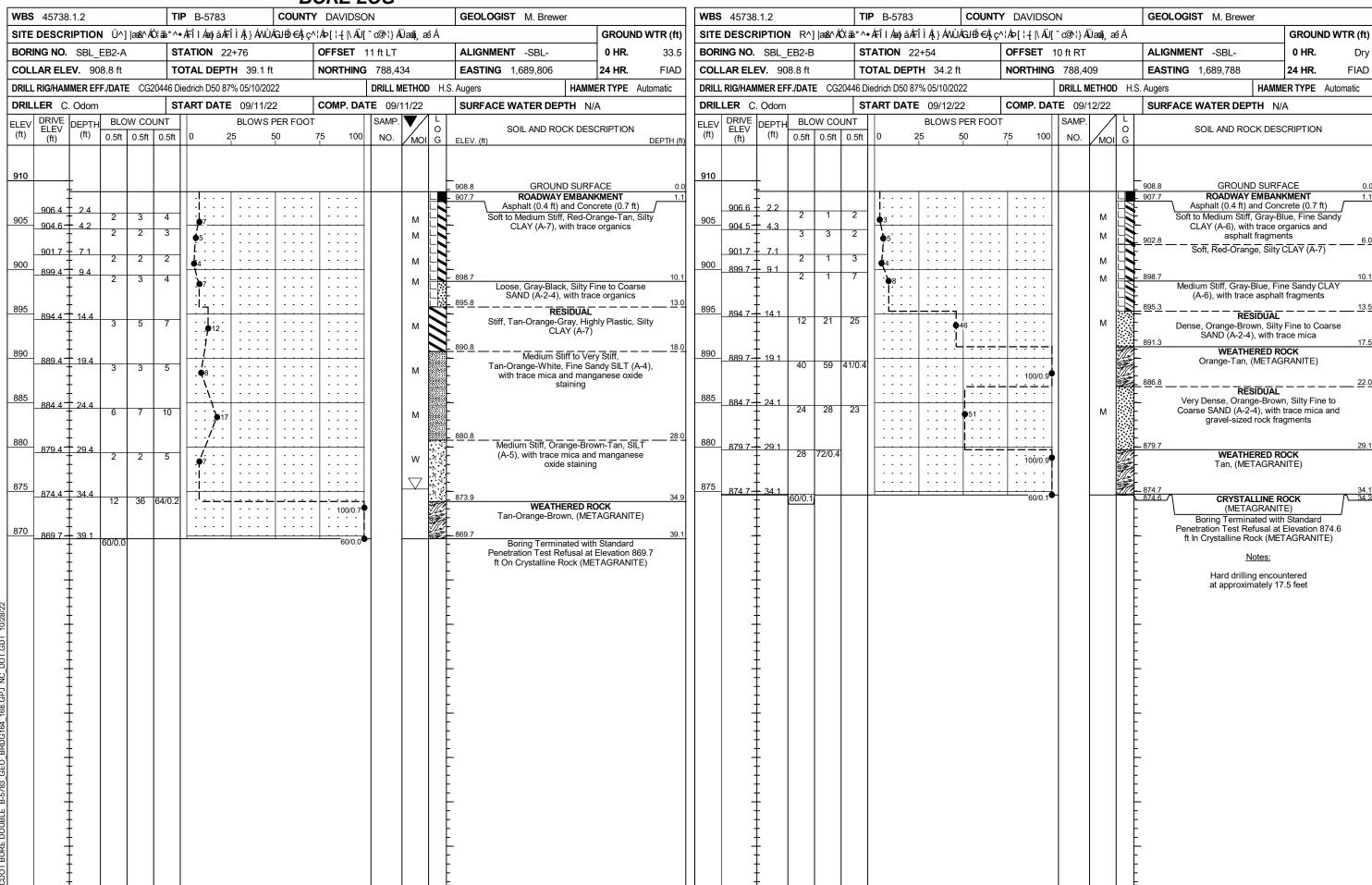


FYd`UWY'6f]X[Yg'% ('UbX'%, 'cb'l G'&- #+\$'cj Yf'B'cfZc`_'Gci l\ Yfb'FU]`k Um, Davidson County, NC' Rock Core Photographs' Boring: SBL_B2-B'

21.5 to 47.4 FeetÁ







PROJECT REFERENCE NO.	SHEET NO.
B-5783	28
LAB RESU	<i>ILTS</i>

	ROCK TEST RESULTS													
SAMPLE NO.	BORING	STATION	OFFSET	DEPTH INTERVAL	ROCK TYPE	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH							
RS-1	NBL_B1-A	20+61 -NBL-	10' LT	31.6 - 32.2'	METAGRANITE	176.5	5,230 psi/753 ksf							
RS–2	NBL_B1-A	20+61 -NBL-	10' LT	35.4 - 36.0'	METAGRANITE	173.4	15,930 psi/2,294 ksf							
RS–3	NBL_B2-B	21+71 -NBL-	11' RT	23.5 - 24.1'	METAGRANITE	162.4	2,290 psi/330 ksf							
RS–4	NBL_B2-B	21+71 -NBL-	11' RT	29.4 - 29.8'	METAGRANITE	165.9	13,650 psi/1,966 ksf							
RS–5	SBL_B1-A	21+04 -SBL-	8'LT	20.6 - 21.2'	METAGRANITE	172.5	4,820 psi/694 ksf							
RS–6	SBL_B1-A	21+04 -SBL-	8'LT	23.2 - 23.7'	METAGRANITE	167.1	7,800 psi/1,123 ksf							
RS-7	SBL_B2-B	22+05 -SBL-	9' RT	26.2 - 26.8'	METAGRANITE	185.3	3,880 psi/559 ksf							
RS-8	SBL_B2-B	22+05 -SBL-	9'RT	29.5 - 30.0'	METAGRANITE	180.2	3,550 psi/511 ksf							

LAB TESTING PERFORMED BY NCDOT LAB CERT NO. 117-1104

SITE PHOTOS



PHOTO #1: END BENT 2 OF EXISTING BRIDGE 164 LOOKING WEST (DOWNSTATION)



PHOTO #3: END BENT 1 OF EXISTING BRIDGE 168 LOOKING EAST (UPSTATION)



PHOTO #2: END BENT 1 OF EXISTING BRIDGES 164 AND 168 LOOKING EAST (UPSTATION)



PHOTO #4: END BENT 1 OF EXISTING BRIDGE 164 LOOKING EAST (UPSTATION)