9304 PROJEC

(RUSSEL CHAPEL CHURCH RD)

C2048

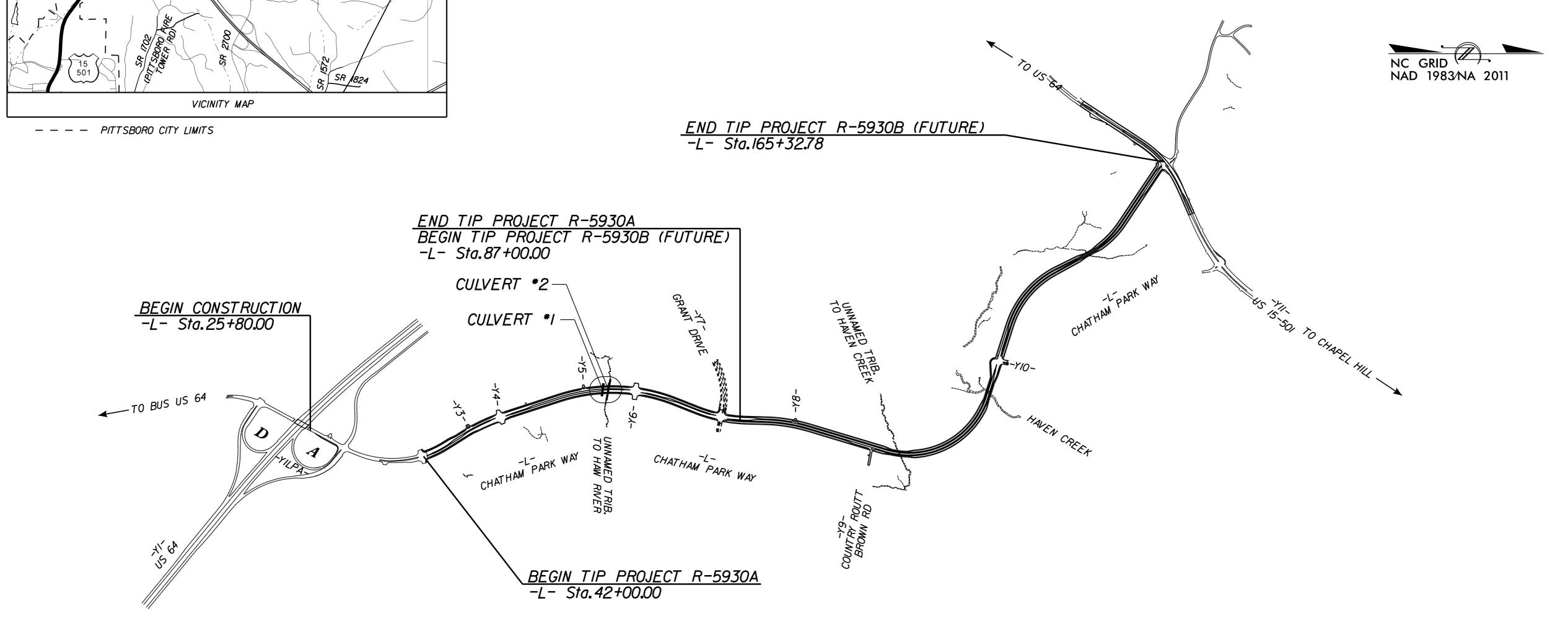
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CHATHAM COUNTY

N.C. R-5930A STATE PROJ. NO. F. A. PROJ. NO. DESCRIPTION 48548.1.1 48548.2.1 RW 48548.3.2 CONST

LOCATION: CHATHAM PARK WAY FROM NORTH OF US 64 TO NORTH OF FUTURE GRANT DRIVE

TYPE OF WORK: DRAINAGE, GRADING, PAVING, AND CULVERTS



STRUCTURES

R-5930A DESIGN DATA

END PROJECT

BEGIN PROJECT

ADT 2025 = 0ADT 2045 = 30000

K = 8%

D = 65 $T = 5\%^*$

V = 50 MPH

* (TTST 2% + DUAL 3%) **FUNCTIONAL CLASSIFICATION:** URBAN ARTERIAL SUB_REGIONAL TIER

PROJECT LENGTH

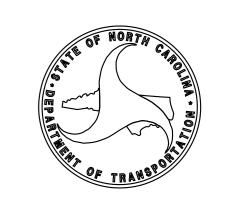
LENGTH ROADWAY TIP PROJECT R-5930A TOTAL LENGTH TIP PROJECT R-5930A

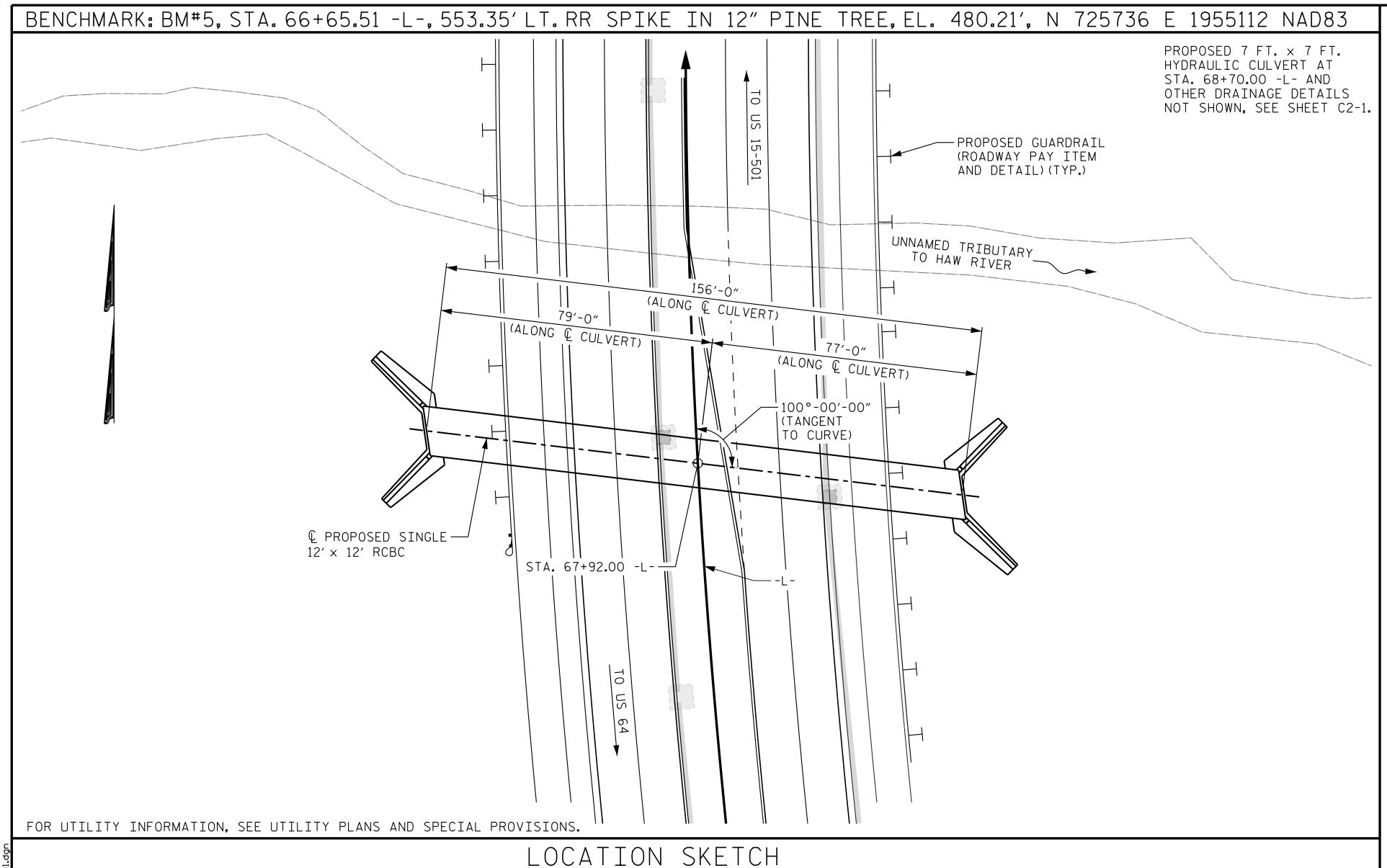
0.852 MILES 0.852 MILES

Kimley » Horn PLANS PREPARED FOR THE NCDOT BY: 2018 STANDARD SPECIFICATIONS ANDREW L. PHILLIPS, P.E.

LETTING DATE: AUGUST 15, 2023

PROJECT ENGINEER CLAY T. POOLE, P.E. PROJECT DESIGN ENGINEER

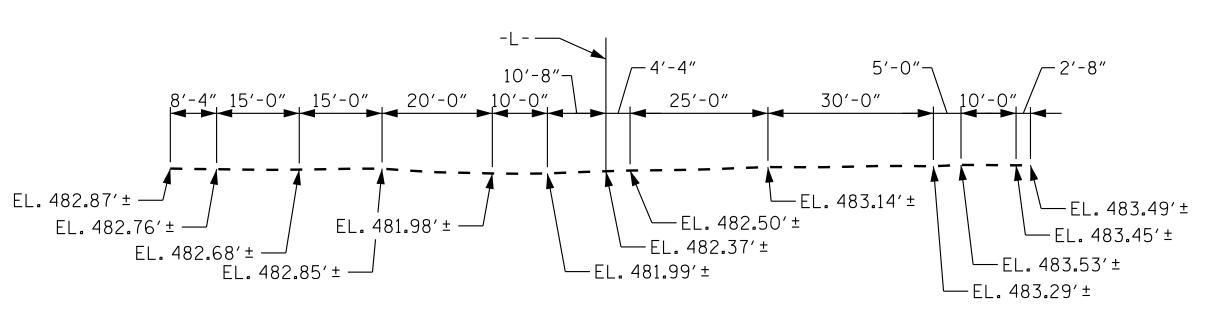




ROADWAY DATA

GRADE POINT EL. @ STA. 67+92.00 -L- = 500.38' BED ELEVATION @ STA. 67+92.00 -L- = 480.71' ROADWAY SLOPES 2:1

TOTAL STRUCTURE QUANTITIES CLASS A CONCRETE BARREL @ ____1.930 ___CY/FT ____301.1 ___C.Y. 66.0 WINGS ETC.____ TOTAL _____ REINFORCING STEEL 55,845 Barrel _____ 10,825 WINGS ETC. ____ TOTAL ____ 66,670 LUMP SUM CULVERT EXCAVATION STA. 67+92.00 -L-FOUNDATION CONDITIONING MATERIAL 199 TONS



DRAWN BY: <u>D.D. LOWERY</u> DATE: 01/2023 CHECKED BY: C. T. POOLE DATE: 01/2023 DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 01/2023 PROFILE ALONG & CULVERT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

DESIGN FILL ----- 8'-6" (MAX.)

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH NCDOT STANDARD SPECIFICATIONS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

CONCRETE IN THE CULVERT TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS, CURTAIN WALLS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS. 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON THE WING SHEETS.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF THE EXTERIOR WALLS ABOVE THE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT POURS TO A MAXIMUM OF 70 FEET. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

EXCAVATE A MINIMUM OF 1 FOOT BELOW CULVERT BEARING ELEVATION AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL PER SECTION 414 OF THE STANDARD SPECIFICATIONS.

UNDERCUT ANY SOFT/LOOSE ALLUVIAL SOILS THAT MAY BE ENCOUNTERED BENEATH THE BOTTOM OF THE FOUNDATION CONDITIONING MATERIAL. BACKFILL UNDERCUT AREAS WITH FOUNDATION CONDITIONING MATERIAL (SELECT MATERIAL CLASS VI: NO. 57 STONE).

INSTALL TYPE 4 GEOTEXTILE AT THE BOTTOM OF EXCAVATION PRIOR TO PLACING FOUNDATION CONDITIONING MATERIAL. THE GEOTEXTILE SHOULD BE PLACED AT THE BOTTOM OF THE EXCAVATION AND WRAPPED UP THE SIDE WALLS OF THE EXCAVATION.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

-L- HORIZONTAL CURVE DATA

PI STA. 70+27.00 $\triangle = 39^{\circ} - 35' - 18.7'' (RT)$

 $D = 2^{\circ}-29'-28.0''$

L = 1,589.19' T = 827.79'

R = 2,300.00'

Phone (919) 677-2000

PROJECT NO. R-5930A CHATHAM _ COUNTY STATION: 67+92.00 -L-

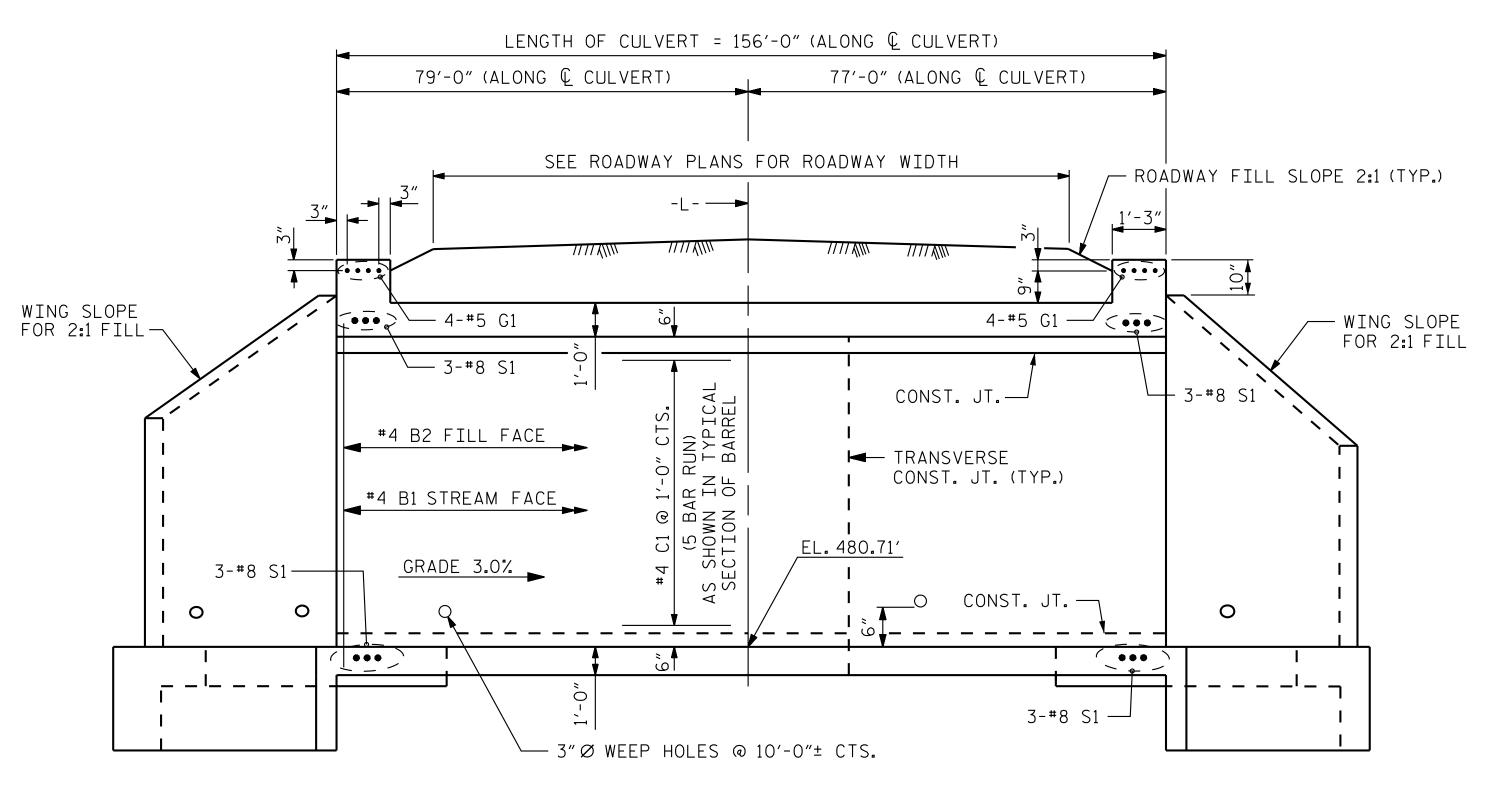
SHEET 1 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

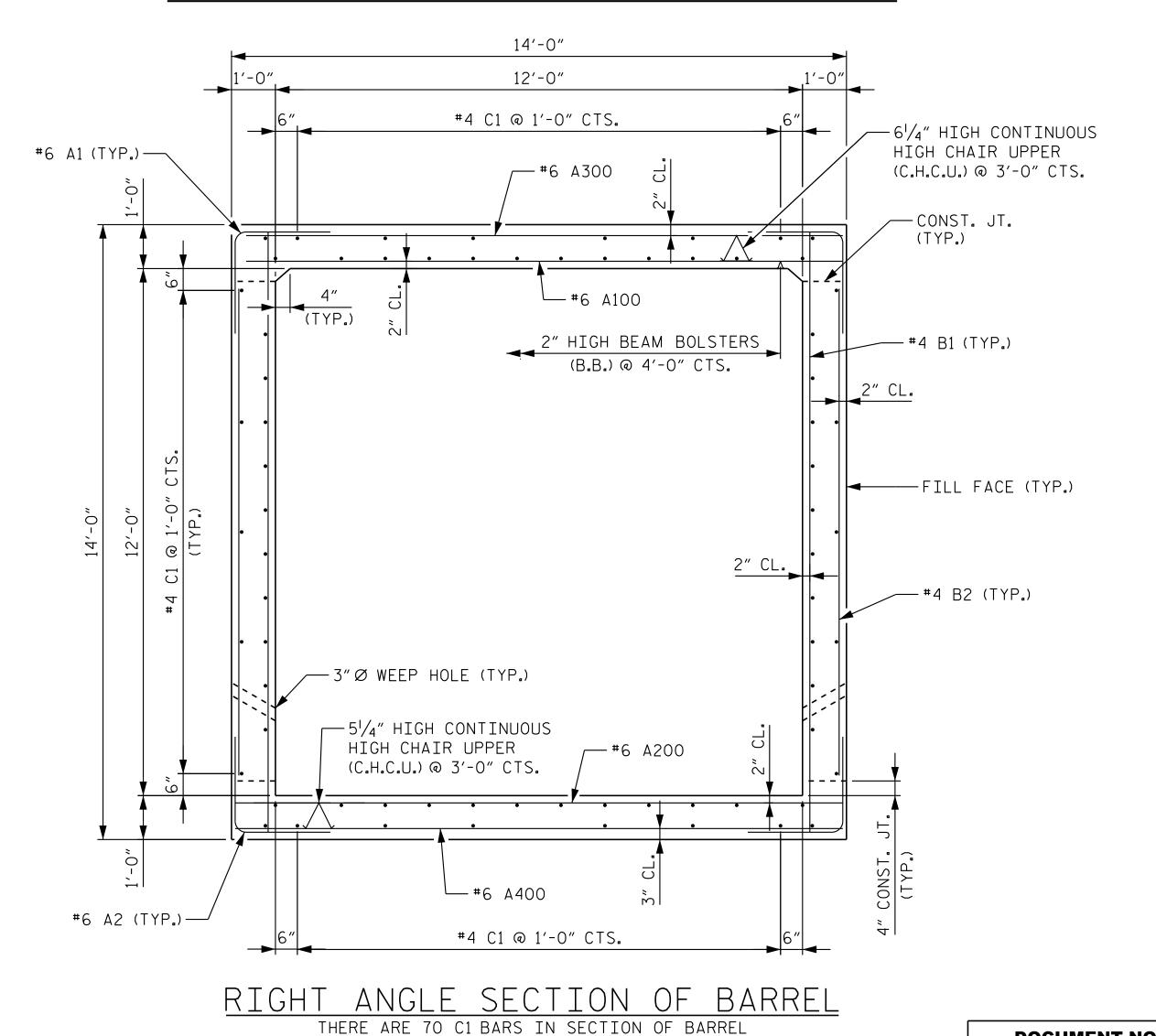
CONCRETE BOX CULVERT 100° SKEW

(PEDESTRIAN)

REVISIONS SHEET NO C1-1 DATE: DATE: NO. BY: BY: TOTAL SHEETS



CULVERT SECTION NORMAL TO ROADWAY



BAR SIZE	SPLICE LENGTH			
#4 B1	1'-10"			
#4 C1	2′-5″			

PROJECT NO. R-5930A CHATHAM __ COUNTY STATION: 67+92.00 -L-

BILL OF MATERIAL

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

A1 622 6 1 6'-3" 5,839

A100 | 305 | 6 | STR | 13'-8" | 6,261

STR 11'-9"

STR | 9<u>'-11"</u>

STR | 8'-0"

|STR| 6'-2"

|STR| 11'-9"

STR | 9'-11"

|STR| 8'-0"

STR | 6'-2"

STR 4'-3"

|STR| 9'-11"

|STR| 8'-0" |

6 | STR | 2'-5"

A300 | 305 | 6 | STR | 13'-8" | 6,261

6 | STR | 6'-2"

A400 305 6 STR 13'-8" 6,261

6 STR 11'-9"

6 | STR | 6'-2"

B1 622 4 STR 13'-7" 5,644 B2 | 622 | 4 | STR | 11'-4" | 4,709

C1 350 4 STR 33'-1" 7,735

STR 9'-11"

STR | 8'-0"

6 | STR | 4'-3" |

6 STR 2'-5"

A200 305 6 STR 13'-8"

A301 2 | 6 |STR| 11'-9"

A305 2 6 STR 4'-3"

A306 4 6 STR 2'-5"

A405| 2 | 6 |STR| 4'-3" |

G1 | 8 | 5 | STR | 14'-1" |

S1 | 12 | 8 | STR | 14'-1"

REINFORCING STEEL

A406 4 6 STR 2'-5"

A2 | 622 | 6 | 1 |

A106 4

A206 4

A302| 2 | 6 |

6'-2" | 5,761

24

13

15

35

24

15

24

13

15

24

13

118

SHEET NO

DATE:

C1-2

TOTAL SHEETS

LBS. 55,845

SHEET 2 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

CONCRETE BOX CULVERT 100° SKEW

NO. BY:

REVISIONS

BAR TYPE

ALL BAR DIMENSIONS ARE OUT TO OUT.

VERTICAL LEG

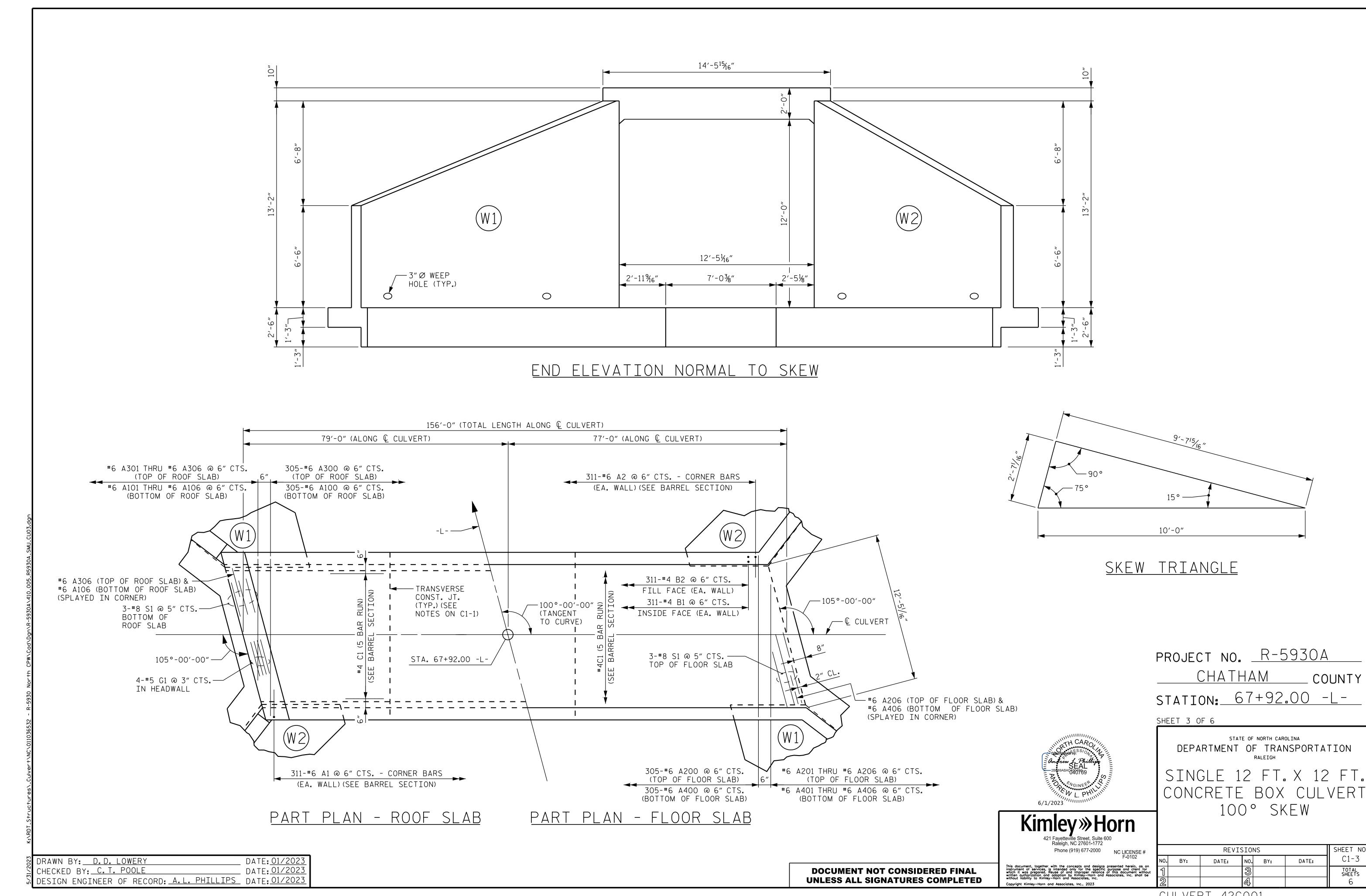
6"RAD.—

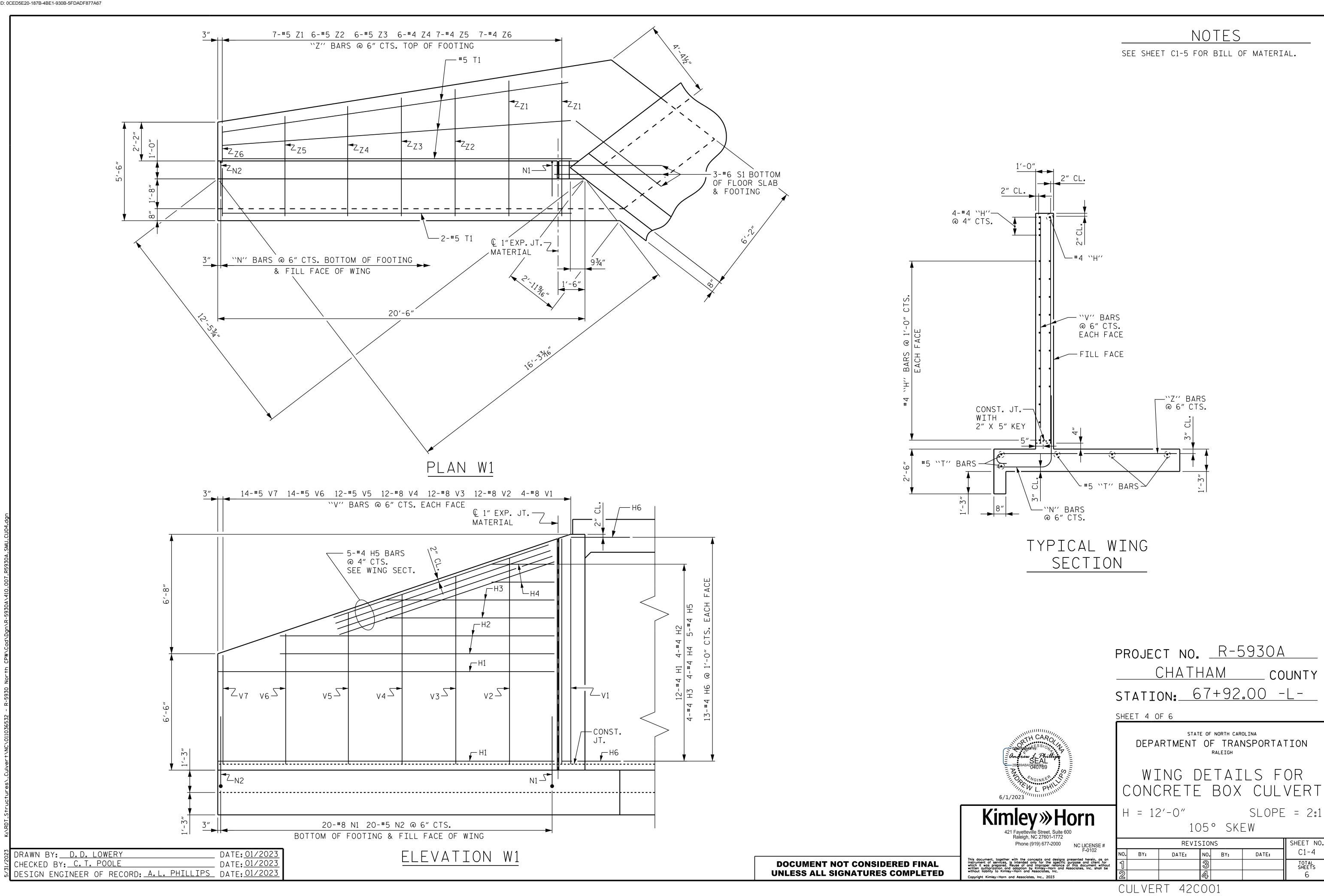
DRAWN BY: D.D. LOWERY	DATE: 01/2023
CHECKED BY: C. T. POOLE	DATE: 01/2023
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS	DATE: 01/2023

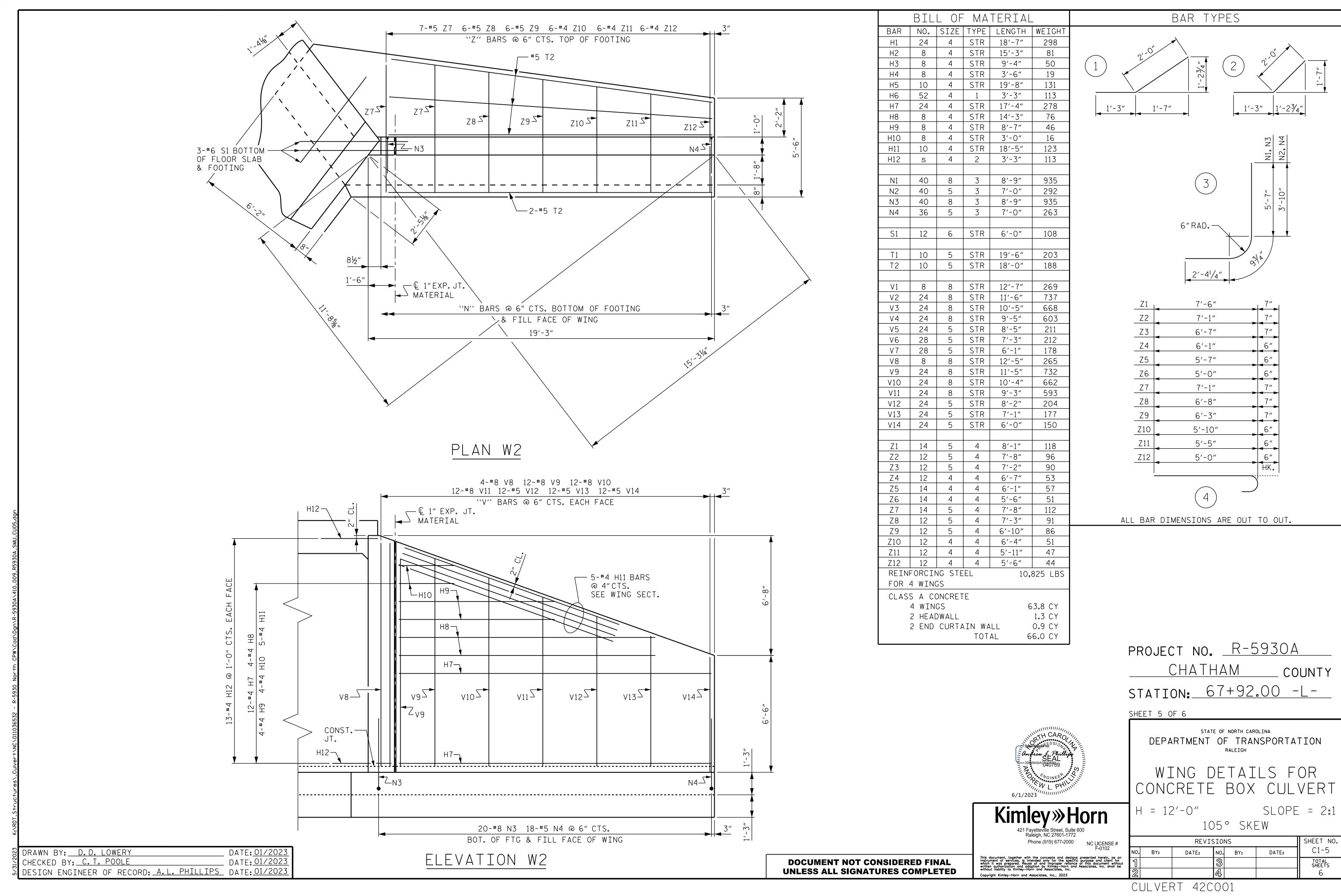
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CULVERT 42C001

DATE:

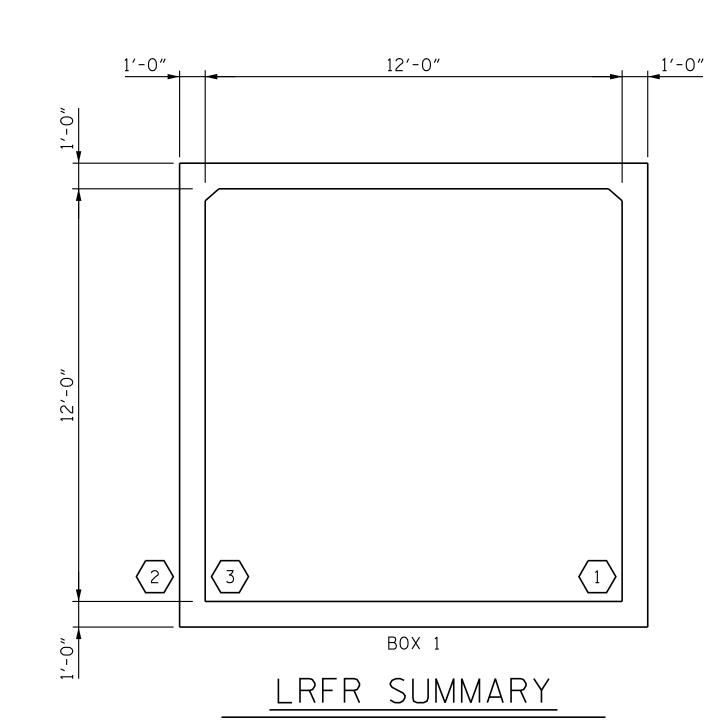






LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

							STRENGTH I LIMIT STATE									
										MOMENT				SHEAR		•
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.92		1.75	2.23	1	BOTTOM SLAB	7.00	1.92	1	BOTTOM SLAB	13.00	
DESIGN LOAD		HL-93 (OPERATING)	N/A		2.48		1.35	2.89	1	BOTTOM SLAB	7.00	2.48	1	BOTTOM SLAB	13.00	
RATING		HS-20 (INVENTORY)	36.000	2	2.25	81.00	1.75	2.25	1	EXT WALL	0.50	2.41	1	BOTTOM SLAB	1.00	
		HS-20 (OPERATING)	36.000		2.92	105.12	1.35	2.92	1	EXT WALL	0.50	3.13	1	BOTTOM SLAB	1.00	
		SNSH	13.500		2.81	37.94	1.40	2.81	1	EXT WALL	0.50	5.26	1	BOTTOM SLAB	1.00	
		SNGARBS2	20.000		2.81	56.20	1.40	2.81	1	EXT WALL	0.50	4.92	1	BOTTOM SLAB	1.00	
	CLE	SNAGRIS2	22.000		2.81	61.82	1.40	2.81	1	EXT WALL	0.50	5.26	1	BOTTOM SLAB	1.00	
	SINGLE VEHICLE (SV)	SNCOTTS3	27.250		2.45	66.76	1.40	2.81	1	EXT WALL	0.50	2.45	1	BOTTOM SLAB	1.00	
	SLE (S	SNAGGRS4	34.925		2 . 55	89.06	1.40	2.81	1	EXT WALL	0.50	2.55	1	BOTTOM SLAB	1.00	
LEGAL	NIS	SNS5A	35.550		2.36	83.90	1.40	2.76	1	BOTTOM SLAB	7.00	2.36	1	BOTTOM SLAB	13.00	
LOAD RATING		SNS6A	39.950		2.28	91.09	1.40	2.67	1	BOTTOM SLAB	7.00	2.28	1	BOTTOM SLAB	1.00	
NATING		SNS7B	42.000	3	2.24	94.08	1.40	2.63	1	BOTTOM SLAB	7.00	2.24	1	BOTTOM SLAB	1.00	
		TNAGRIT3	33.000		2.81	92.73	1.40	2.81	1	EXT WALL	0.50	3.52	1	BOTTOM SLAB	1.00	
	- IV	TNT4A	33.075		2.81	92.94	1.40	2.81	1	EXT WALL	0.50	2.83	1	BOTTOM SLAB	1.00	
	SEMI-	TNT6A	41.600		2.58	107.33	1.40	2.81	1	EXT WALL	0.50	2.58	1	BOTTOM SLAB	1.00	
	TRACTOR ILER (TT)	TNT7A	42.000		2.70	113.40	1.40	2.81	1	EXT WALL	0.50	2.70	1	BOTTOM SLAB	1.00	
	TRA(ILEF	TNT7B	42.000		2.49	104.58	1.40	2.81	1	EXT WALL	0.50	2.49	1	BOTTOM SLAB	1.00	
	TRUCK	TNAGRIT4	43.000		2.74	117.82	1.40	2.81	1	EXT WALL	0.50	2.74	1	BOTTOM SLAB	1.00	
	TRL	TNAGT5A	45.000		2.74	123.30	1.40	2.81	1	EXT WALL	0.50	2.74	1	BOTTOM SLAB	13.00	
		TNAGT5B	45.000		2.76	124.20	1.40	2.81	1	EXT WALL	0.50	2.76	1	BOTTOM SLAB	1.00	



ASSEMBLED BY: D.D. LOWERY DATE: 01/2023 CHECKED BY: C.T. POOLE DATE: 01/2023 DRAWN BY: WMC 7/II REV. 10/1/II REV. 12/17

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1.50	0.65		
EV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

PROJECT NO. R-5930A

CHATHAM COUNTY

STATION: 67+92.00 -L-

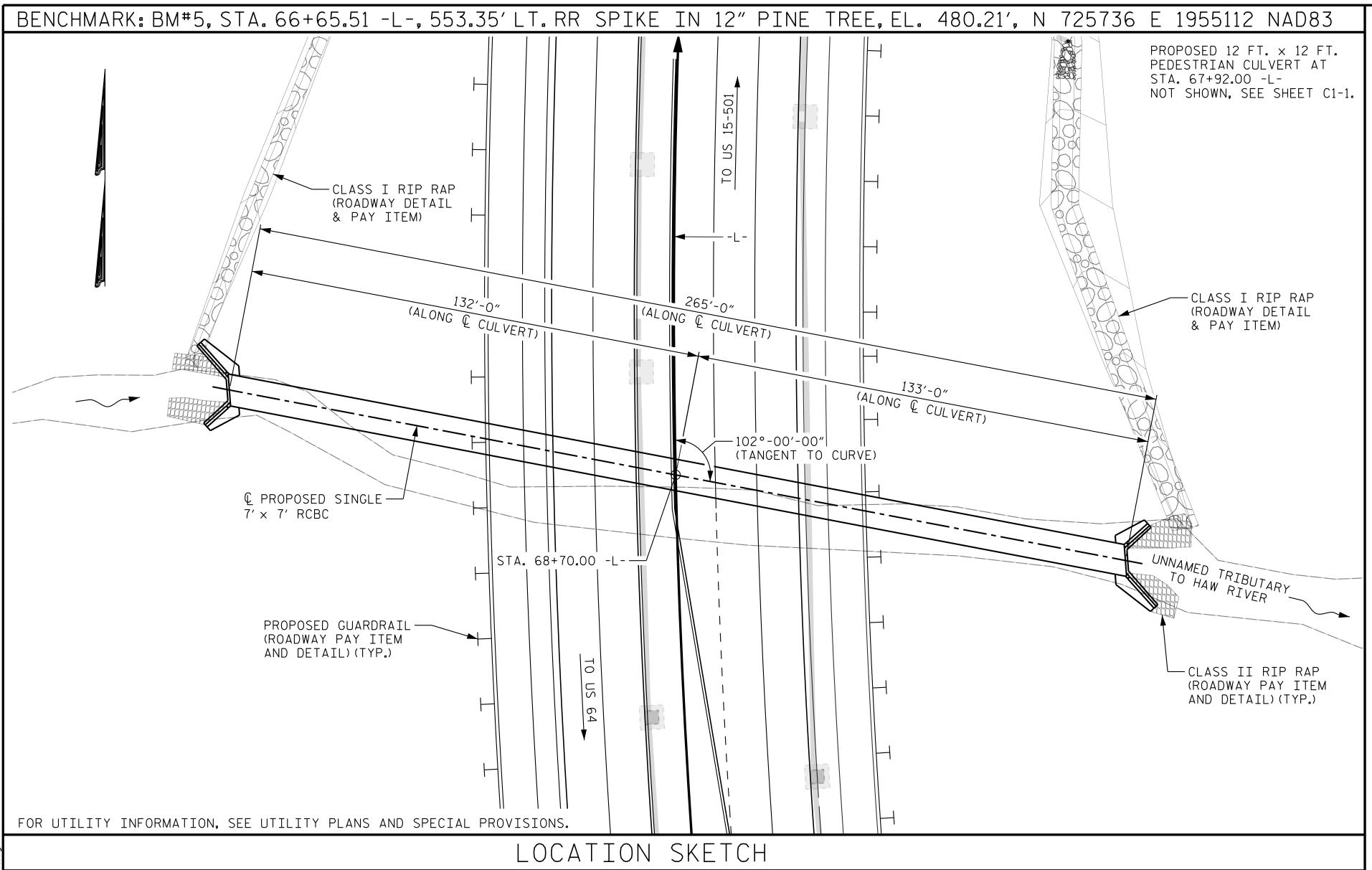
SHEET 6 OF 6



DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)

STATE OF NORTH CAROLINA

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	C1-6
		3			TOTAL SHEETS
		<u> </u>			6



HYDRAULIC DATA

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE ------1100 CFS FREQUENCY OF OVERTOPPING FLOOD --->500 YR. OVERTOPPING FLOOD ELEVATION -----498.3 *
*OVERTOPPING WILL OCCUR AT STA. 70+27.28
AND DOES NOT TAKE INTO ACCOUNT THE PEDESTRIAN CULVERT

ROADWAY DATA

GRADE POINT EL. @ STA. 68+70.00 -L- = 499.70' INVERT ELEVATION @ STA. 68+70.00 -L- = 458.20' ROADWAY SLOPES 2:1

DRAWN BY: D.D. LOWERY

CHECKED BY: C.T. POOLE

DATE: 01/2023

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS

DATE: 01/2023

-L- HORIZONTAL CURVE DATA

PI STA. 70+27.00 △ = 39°-35′-18.7″(RT)

D = 2°-29'-28.0" L = 1,589.19' T = 827.79'

R = 2,300.00'

4'-3"— *─*9′-2″ 9'-0"-12′-8″¬ <u>___14'-11"</u> −4′-0″ 30'-2" .17'-2". 41'-0" .16'-8".21'-1". 27'-3". 25'-3". EL. 462.31′± — .458.99′± — EL. 456.35′± EL. 461.38′± — EL. 458.85′± — — EL. 456.36′± EL. 461.30′± — -EL.456.67′± EL.460.68′± — └─EL.459.80′± -EL.457.34′± EL.460.74′± — —EL.460.36′± EL. 457.88′± EL. 460.76′± — — EL. 458.76′±

PROFILE ALONG & CULVERT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

DESIGN FILL ----- 34'-3" (MAX.)

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH NCDOT STANDARD SPECIFICATIONS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

CONCRETE IN THE CULVERT TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS. CURTAIN WALLS AND FLOOR SLAB INCLUDING 4"OF ALL VERTICAL

1. WING FOOTINGS, CURTAIN WALLS AND FLOOR SLAB INCLUDING 4"OF ALL VERTICAL WALLS.
2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY SILLS/BAFFLES,
ROOF SLAB AND HEADWALLS.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON THE WING SHEET.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF THE EXTERIOR WALLS ABOVE THE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT POURS TO A MAXIMUM OF 70 FEET. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM. SEE EROSION CONTROL PLANS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.

THE ENGINEER, IN CONSULTATION WITH DEO STAFF, SHALL REVIEW ALL MATERIAL TO BE USED AS BACKFILL PRIOR TO CONDUCTING THE BACKFILL ACTIVITY. BACKFILL SHALL CONSIST OF NATIVE MATERIAL ONLY UNLESS THE ENGINEER, IN CONSULTATION WITH DEO STAFF, DETERMINES THAT (1) THE NATIVE MATERIAL IS UNSUITABLE, OR (2) ADDITIONAL MATERIAL IS REQUIRED TO SUPPLEMENT THE NATIVE MATERIAL. THE CHOSEN BACKFILL MATERIAL SHALL NOT HAVE ADVERSE EFFECTS TO AQUATIC LIFE, AQUATIC LIFE PASSAGE, OR WATER QUALITY. NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED OR FLOODPLAIN AT THE PROJECT SITE DURING CULVERT CONSTRUCTION.

THE ENTIRE COST OF WORK REQUIRED TO PLACE EXCAVATED OR SUPPLEMENTAL MATERIAL AS SHOWN ON THE PLANS SHALL BE INCLUDED IN THE LUMP SUM PRICE FOR CULVERT EXCAVATION.

EXCAVATE A MINIMUM OF 1 FOOT BELOW CULVERT BEARING ELEVATION AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL PER SECTION 414 OF THE STANDARD SPECIFICATIONS.

UNDERCUT ANY SOFT/LOOSE ALLUVIAL SOILS THAT MAY BE ENCOUNTERED BENEATH THE BOTTOM OF THE FOUNDATION CONDITIONING MATERIAL. BACKFILL UNDERCUT AREAS WITH FOUNDATION CONDITIONING MATERIAL (SELECT MATERIAL CLASS VI; NO. 57 STONE).

INSTALL TYPE 4 GEOTEXTILE AT THE BOTTOM OF EXCAVATION PRIOR TO PLACING FOUNDATION CONDITIONING MATERIAL. THE GEOTEXTILE SHOULD BE PLACED AT THE BOTTOM OF THE EXCAVATION AND WRAPPED UP THE SIDE WALLS OF THE EXCAVATION.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

TOTAL STRUCTURE QUANTITIES

CLASS A CONCRETE

BARREL @ 1.245 CY/FT 329.9 C.Y.

WINGS ETC. 20.2 C.Y.

SILLS 3.4 C.Y.

TOTAL 353.5 C.Y.

REINFORCING STEEL

 BARREL
 66,759
 LBS.

 WINGS ETC.
 1,277
 LBS.

 TOTAL
 68,071
 LBS.

CULVERT EXCAVATION STA. 68+70.00 -L- LUMP SUM FOUNDATION CONDITIONING MATERIAL 243 TONS

PROJECT NO. R-5930A

CHATHAM COUNTY

STATION: 68+70.00 -L-

SHEET 1 OF 6

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SINGLE 7 FT.X 7 FT. CONCRETE BOX CULVERT 102° SKEW

REVISIONS

SHEET NO
C 27601-1772

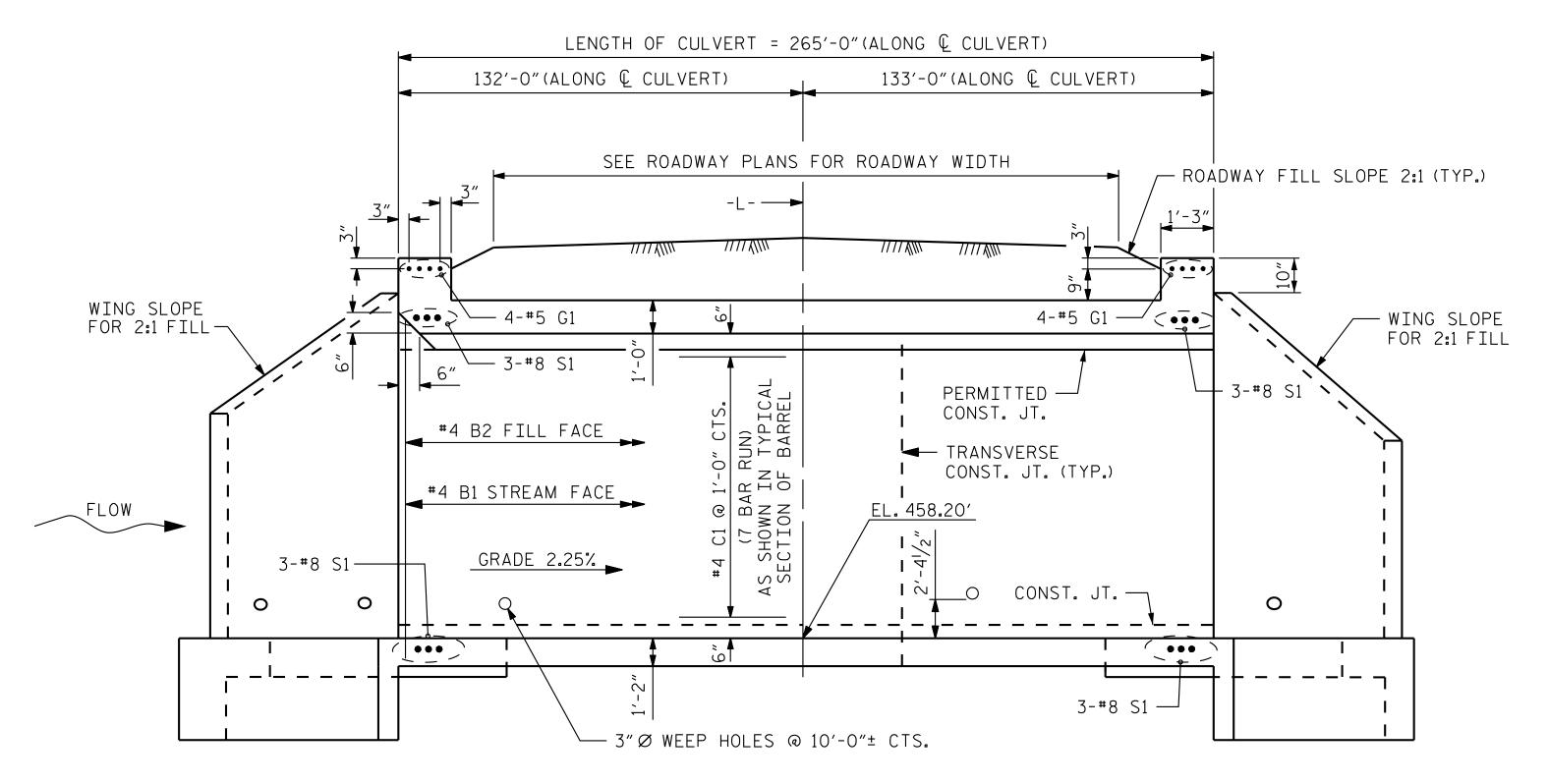
19) 677-2000 NC LICENSE #
F-0102

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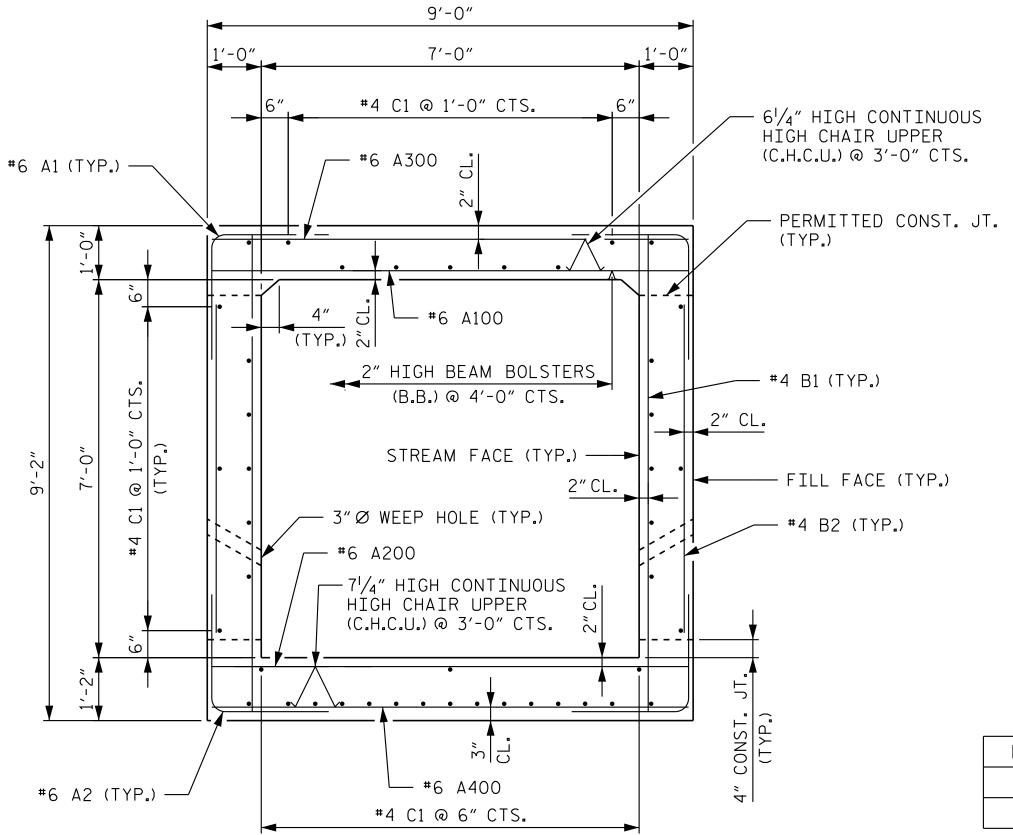
No. BY: DATE:

10 DATE: No. BY: DATE:

11 DATE: NO. BY: DAT



CULVERT SECTION NORMAL TO ROADWAY



BAR SIZE	SPLICE LENGTH		
#4 B1	1'-10"		
#4 C1	2′-5″		

RIGHT ANGLE SECTION OF BARREL THERE ARE 43 "C" BARS IN SECTION OF BARREL

> **DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**

A1 1058 6 1 6'-4" 10,06 A2 1058 6 1 6'-4" 10,06 A100 525 6 STR 8'-8" 6,83 A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 3'-8" 11 A104 4 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A203 2 6 STR 3'-8" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 3'-8" 11 A304<				1 1 1 1 7	, , , , , , , , ,	_
A1 1058 6 1 6'-3" 9,93 A2 1058 6 1 6'-4" 10,06 A100 525 6 STR 8'-8" 6,83 A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 7'-5" 22 A202 2 6 STR 5'-6" 17 A203 2 6 STR 7'-5" 22 A202 2 6 STR 5'-6" 17 A203 2 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 B1 1058 4 STR 8'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 137 G1 8 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,79 BAR TYPE	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A2 1058 6 1 6'-4" 10,06 A100 525 6 STR 8'-8" 6,83 A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A203 2 6 STR 3'-8" 11 A300 525 6 STR 3'-8" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A303 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 7'-5" 22 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-4" 4,47 C1 301 4 STR 39'-11" 8,02 REINFORCING STEEL LBS. 66,79 BAR TYPE					6′-3″	9,932
A100 525 6 STR 8'-8" 6,83 A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 3'-8" 11 A104 4 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A202 2 6 STR 7'-5" 22 A203 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 11 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 74 REINFORCING STEEL LBS. 66,79 BAR TYPE					6′-4″	10,064
A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 3'-8" 11 A104 4 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 5'-6" 17 A203 2 6 STR 7'-5" 22 A202 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A303 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 24 A403 2 6 STR 1'-9" 11 B1 1058 4 STR 8'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 13 G1 8 S STR 8'-11" 74 S1 12 8 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE						
A101 2 6 STR 7'-5" 22 A102 2 6 STR 5'-6" 17 A103 2 6 STR 3'-8" 11 A104 4 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 5'-6" 17 A203 2 6 STR 7'-5" 22 A202 2 6 STR 3'-8" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A302 2 6 STR 7'-5" 22 A303 2 6 STR 5'-6" 17 A303 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 1 6 STR 1'-9" 11 B1 1058 4 STR 8'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 11 STR 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 8 STR 8'-11" 74 STR 8'-11" 286 BAR TYPE	A100	525	6	STR	8′-8″	6,834
A102 2 6 STR 5'-6" 17 A103 2 6 STR 3'-8" 11 A104 4 6 STR 1'-9" 11 A200 525 6 STR 8'-8" 6,83 A201 2 6 STR 7'-5" 22 A202 2 6 STR 5'-6" 17 A203 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 5'-6" 17 A304 4 6 STR 1'-9" 11 A400 525 6 STR 3'-8" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 4,47 C1 301 4 STR 39'-11" 8,02 REINFORCING STEEL LBS. 66,75 BAR TYPE					7′-5″	22
A104		2	6		5′-6″	17
A104		2	6		3′-8″	11
A201 2 6 STR 7'-5" 22 A202 2 6 STR 5'-6" 17 A203 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 5'-6" 17 A303 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-9" 11 B1 1058 4 STR 8'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A104	4	6	STR	1'-9"	11
A201 2 6 STR 7'-5" 22 A202 2 6 STR 5'-6" 17 A203 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 5'-6" 17 A303 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 5'-6" 17 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 8'-9" 11 S1 12 8 STR 8'-11" 74 S1 12 8 STR 8'-11" 74 REINFORCING STEEL LBS. 66,79 BAR TYPE						
A202 2 6 STR 5'-6" 17 A203 2 6 STR 3'-8" 11 A204 4 6 STR 1'-9" 11 A300 525 6 STR 1'-9" 11 A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5	A200	525	6	STR	8′-8″	6,834
A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 5'-6" 17 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 74 REINFORCING STEEL LBS. 66,75	A201	2		STR	7′-5″	
A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 5'-6" 17 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 74 REINFORCING STEEL LBS. 66,75	A202	2		STR	5′-6″	
A300 525 6 STR 8'-8" 6,83 A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 5'-6" 17 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 74 REINFORCING STEEL LBS. 66,75		2				11
A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A204	4	6	STR	1'-9"	11
A301 2 6 STR 7'-5" 22 A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 B1 1058 4 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE						
A302 2 6 STR 5'-6" 17 A303 2 6 STR 3'-8" 11 A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE					8'-8"	6 , 834
A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE		2			7′-5″	
A304 4 6 STR 1'-9" 11 A400 525 6 STR 8'-8" 6,83 A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75	A302	2	6		5′-6″	
A304	A303	2	6		3′-8″	
A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A304	4	6	STR	1'-9"	11
A401 2 6 STR 7'-5" 22 A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS 66,75 BAR TYPE	A 400		6	CTD	0/ 0//	6.074
A402 2 6 STR 5'-6" 17 A403 2 6 STR 3'-8" 11 A404 4 6 STR 1'-9" 11 B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE						
B1 1058 4 STR 8'-9" 6,18 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE		2	6		('-5"	
B1 1058 4 STR 8'-9" 6,188 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A402	2	6		5'-6"	
B1 1058 4 STR 8'-9" 6,188 B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A403		b			
B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	A404	4	Ь	SIR	1,-9,,	11
B2 1058 4 STR 6'-4" 4,47 C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	R1	1058	Δ	STR	8'-9"	6 184
C1 301 4 STR 39'-11" 8,02 D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE						i
D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	טע	1000	7	J 111	U 7	ا ا ا ا
D1 52 6 STR 1'-9" 137 G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	C1	301	4	STR	39'-11"	8,026
G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE			•			,
G1 8 5 STR 8'-11" 74 S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	D1	52	6	STR	1'-9"	137
S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE						
S1 12 8 STR 8'-11" 286 REINFORCING STEEL LBS. 66,75 BAR TYPE	G1	8	5	STR	8'-11"	74
REINFORCING STEEL LBS. 66,75 BAR TYPE						
BAR TYPE	S1	12	8	STR	8'-11"	286
BAR TYPE						
BAR TYPE	D= + 1 · ·	<u> </u>	FN10 07		. 5.0	66 356
	KFTN	FURC.				66,759
A A 1			ВА	<u>R T</u>	YPE	
					A1	A A A
l † †					1	†

BILL OF MATERIAL

VERTICAL LEG — 6″RAD.─ ALL BAR DIMENSIONS ARE OUT TO OUT.

PROJECT NO. R-5930A CHATHAM COUNTY STATION: 68+70.00 -L-

SHEET 2 OF 6

Phone (919) 677-2000 NC LICENSE # F-0102

DEPARTMENT OF TRANSPORTATION CONCRETE BOX CULVERT 102° SKEW

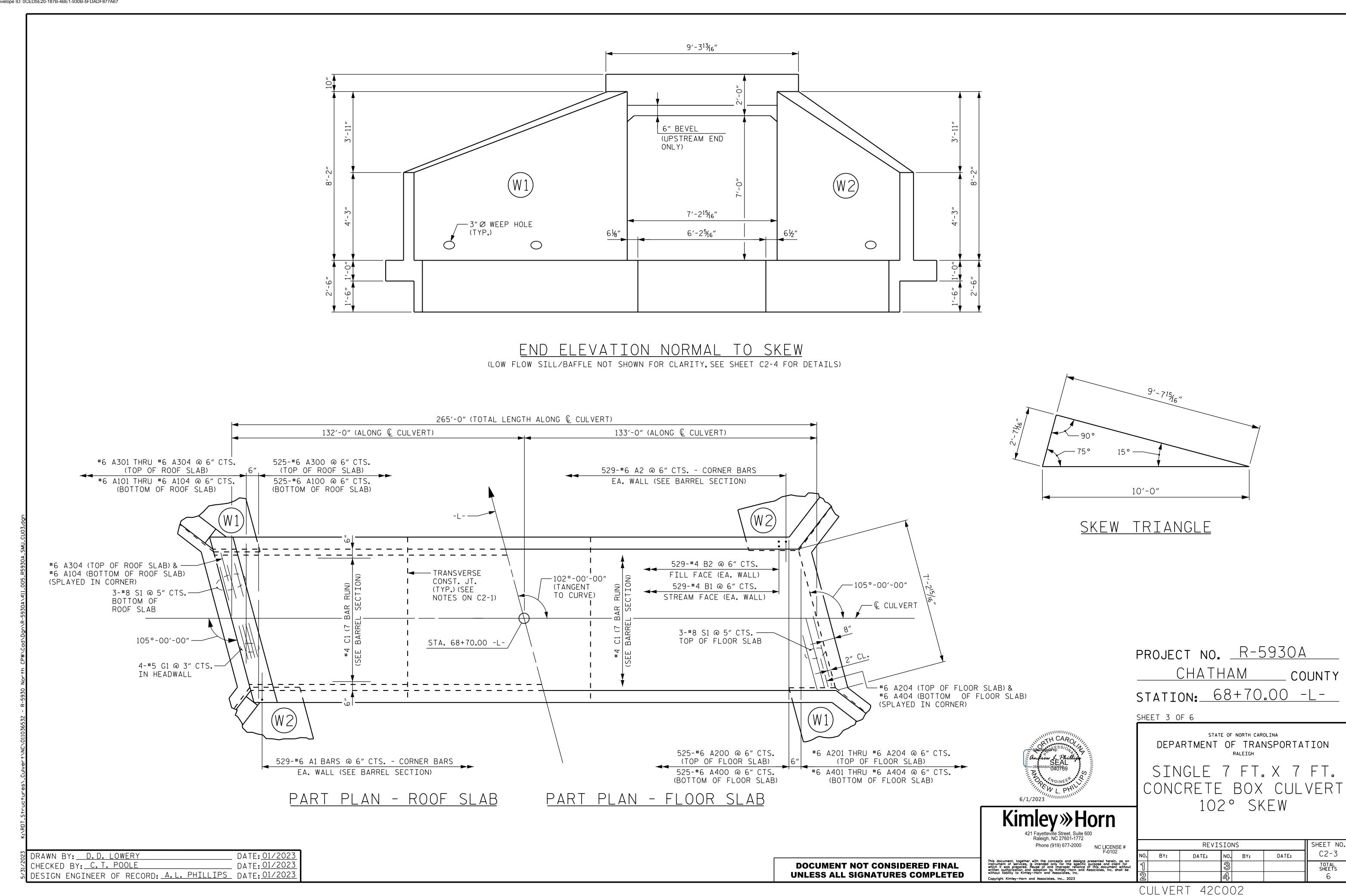
STATE OF NORTH CAROLINA

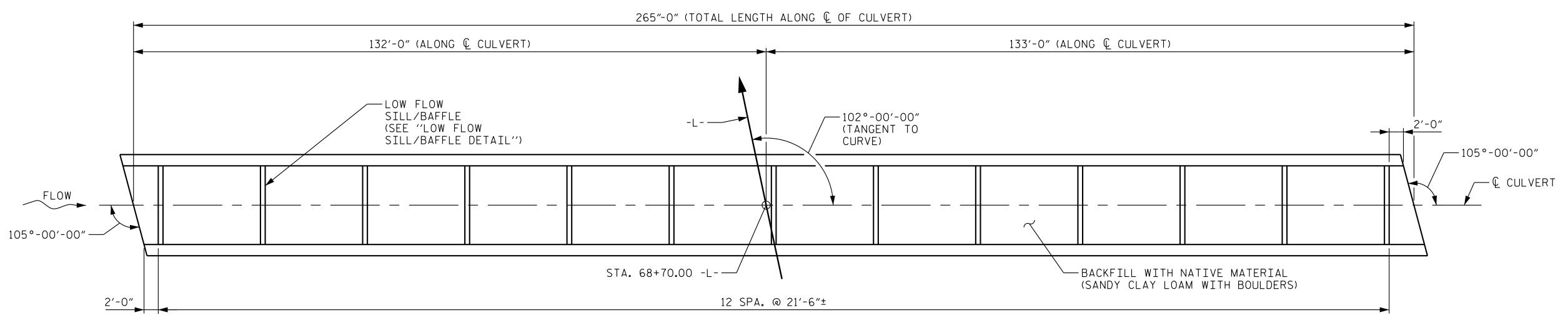
REVISIONS SHEET NO C2-2 DATE: NO. BY: DATE: BY: TOTAL SHEETS

CULVERT 42C002

DRAWN BY: <u>D.D. LOWERY</u> CHECKED BY: <u>C.T. POOLE</u>

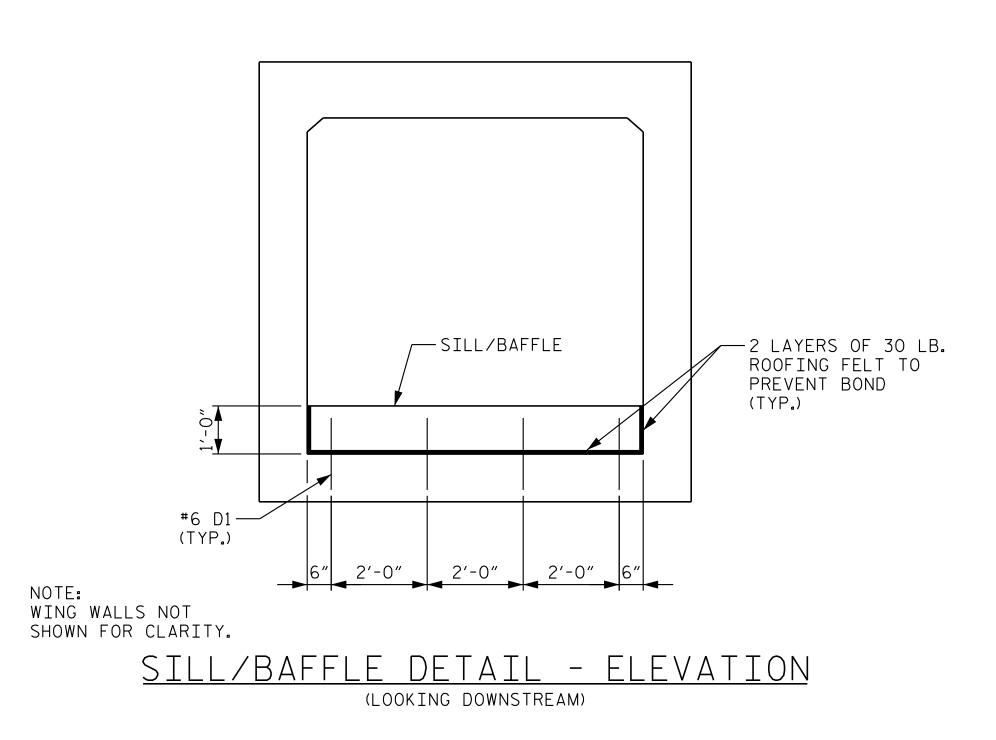
DATE: <u>01/2023</u> CHECKED BY: C.T. POOLE DATE: 01/2023
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 01/2023





PLAN VIEW SHOWING SILL/BAFFLE LOCATIONS

(FOR BACKFILL BETWEEN SILLS/BAFFLES, SEE NOTES ON SHEET C2-1.)



-∗#6 D1 D0WEL @ 2′-0″ CTS. INLET (BEGIN CULVERT)— — 2 LAYERS OF 30 LB.
ROOFING FELT TO
PREVENT BOND (TYP.)

SECTION THRU SILL (INLET END SHOWN, OUTLET END SIMILAR)

* DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOAT FINISHED.

NOTE: SILL/BAFFLES ARE TO BE CAST NORMAL TO CULVERT WALLS.

Phone (919) 677-2000 NC LICENSE # F-0102

CONCRETE BOX CULVERT 102° SKEW

SHEET 4 OF 6

PROJECT NO. R-5930A

STATION: 68+70.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

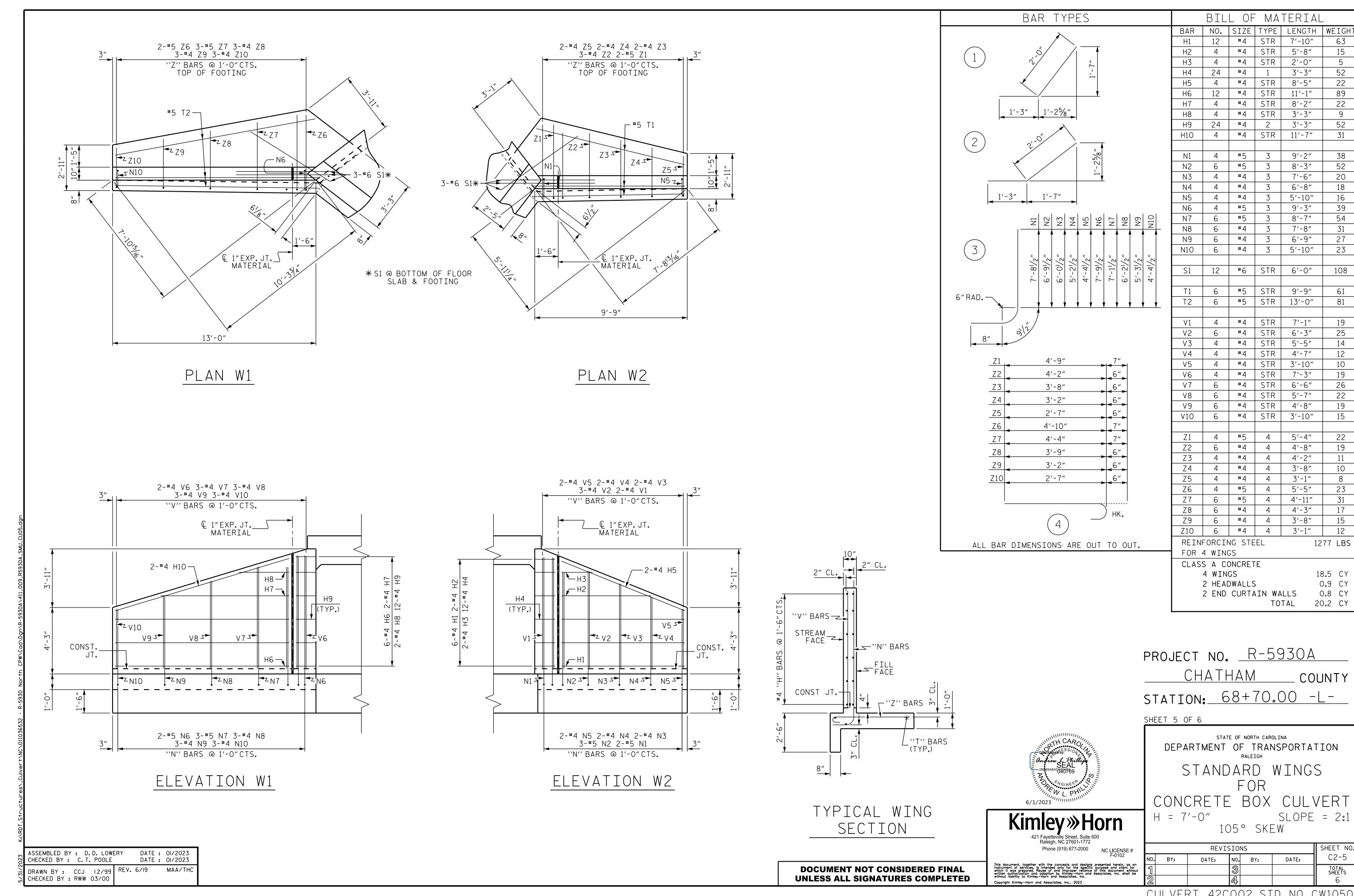
___ COUNTY

CHATHAM

SHEET NO REVISIONS C2-4 NO. BY: DATE: DATE: BY: TOTAL SHEETS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

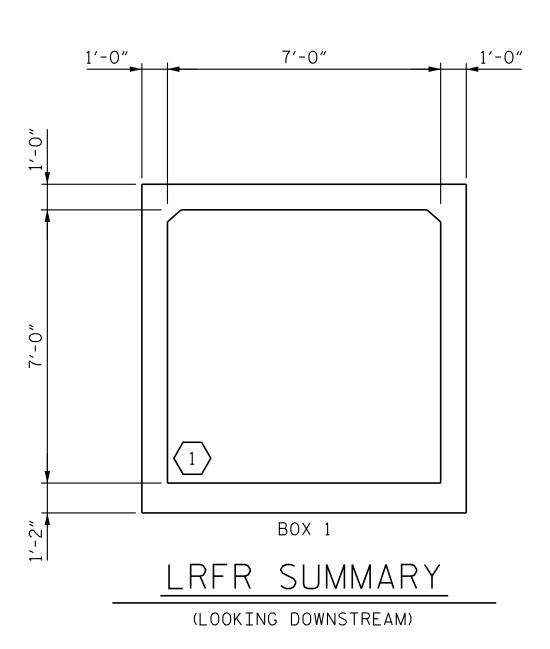
DRAWN BY: <u>D.D. LOWERY</u> CHECKED BY: <u>C.T. POOLE</u> DATE: <u>01/2023</u> CHECKED BY: C.T. POOLE DATE: 01/2023
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 01/2023



BOTTOM SLAB

1.33

PERMANENT LOAD RATING



0.58

1.10

0.58

BOTTOM SLAB

ASSEMBLED BY : D.D. LOWERY CHECKED BY : C.T. POOLE DATE : 01/2023 DATE : 01/2023 DRAWN BY: BNB 6/19 CHECKED BY: THC 6/19

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PERMANENT LOAD FACTORS:

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
WA	1.00	

NOTES:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

THE EFFECTS OF LIVE LOAD ON DESIGN AND LOAD RATING MAY BE NEGLECTED FOR CULVERTS WITH CERTAIN FILL DEPTHS DESCRIBED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

CULVERTS WITH NEGLIGIBLE LIVE LOAD SHOULD BE LOAD RATED FOR PERMANENT LOADS ONLY IN ACCORDANCE WITH THE AASHTO MANUAL FOR BRIDGE EVALUATION.

> PROJECT NO. R-5930A CHATHAM COUNTY

STATION: 68+70.00 -L-

SHEET 6 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (DEEP FILLS)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	C2-6
		3			TOTAL SHEETS
		ΔL			6

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

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