

REFERENCE: U-5748

PROJECT: 50168

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5748	1	99

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ROADWAY SUBSURFACE INVESTIGATION

COUNTY WAKE
PROJECT DESCRIPTION US 401 (LOUISBURG RD) AT
SR 2044 (LIGON MILL RD)/SR 2224 (MITCHELL MILL RD)
AND SR 2006 (PERRY CREEK ROAD) INTERSECTION
IMPROVEMENTS

INVENTORY

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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DATE NOVEMBER 2022



SIGNATURE

DATE

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

GRADATION
WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.
ANGULARITY OF GRAINS
THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.
MINERALOGICAL COMPOSITION
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.
COMPRESSIBILITY
SLIGHTLY COMPRESSIBLE LL < 31
MODERATELY COMPRESSIBLE LL = 31 - 50
HIGHLY COMPRESSIBLE LL > 50
PERCENTAGE OF MATERIAL
ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL
TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%
LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%
MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%
HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE

ROCK DESCRIPTION
HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:
WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR) FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR) FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP) COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.
WEATHERING
FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL
SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF
VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF
COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

TERMS AND DEFINITIONS
ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
AQUIFER - A WATER BEARING FORMATION OR STRATA.
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOROUS STRATUM.
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
STRATA CORE RECOVERY (SCRC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

SOIL LEGEND AND AASHTO CLASSIFICATION
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS
GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7
SYMBOL
% PASSING #10 #40 #200
MATERIAL PASSING #40 LL PI
GROUP INDEX
USUAL TYPES OF MAJOR MATERIALS
GEN. RATING AS SUBGRADE

MISCELLANEOUS SYMBOLS
ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
SOIL SYMBOL
ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
INFERRED SOIL BOUNDARY
INFERRED ROCK LINE
ALLUVIAL SOIL BOUNDARY
DIP & DIP DIRECTION OF ROCK STRUCTURES
TEST BORING
AUGER BORING
CORE BORING
MONITORING WELL
PIEZOMETER INSTALLATION
SLOPE INDICATOR INSTALLATION
CONE PENETROMETER TEST
SOUNDING ROD
TEST BORING WITH CORE
SPT N-VALUE

TEXTURE OR GRAIN SIZE
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 0.42 0.25 0.075 0.053
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)
GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3

RECOMMENDATION SYMBOLS
UNDERCUT
SHALLOW UNDERCUT
UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

SOIL MOISTURE - CORRELATION OF TERMS
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION
- SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
- WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

ABBREVIATIONS
AR - AUGER REFUSAL
BT - BORING TERMINATED
CL - CLAY
CPT - CONE PENETRATION TEST
CSE - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HL - HIGHLY
MED. - MEDIUM
MICA - MICAEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLL - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
% - UNIT WEIGHT
%g - DRY UNIT WEIGHT
SAMPLE ABBREVIATIONS
SS - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

ROCK HARDNESS
VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

ROCK HARDNESS (CONT.)
BENCH MARK:
ELEVATION: FEET
NOTES:
ADDITIONAL ABBREVIATIONS:
WOH - WEIGHT OF HAMMER
FIAD - FILLED IMMEDIATELY AFTER DRILLING
ROADWAY BORING ELEVATIONS BASED OFF u5748_ls.tin.tin FILE DATED 7/15/2019

PLASTICITY
PLASTICITY INDEX (PI) DRY STRENGTH
NON PLASTIC 0-5 VERY LOW
SLIGHTLY PLASTIC 6-15 SLIGHT
MODERATELY PLASTIC 16-25 MEDIUM
HIGHLY PLASTIC 26 OR MORE HIGH

EQUIPMENT USED ON SUBJECT PROJECT
DRILL UNITS:
CME-45C
CME-55
CME-550
VANE SHEAR TEST
PORTABLE HOIST
DIEDRICH D-50
CME 450
ADVANCING TOOLS:
CLAY BITS
4" CONTINUOUS FLIGHT AUGER
8" HOLLOW AUGERS
HARD FACED FINGER BITS
TUNG-CARBIDE INSERTS
CASING w/ ADVANCER
TRICONE 3-7/8" STEEL TEETH
TRICONE TUNG-CARB.
CORE BIT
2-1/4" ID HOLLOW-STEM AUGER
HAMMER TYPE:
AUTOMATIC
MANUAL
CORE SIZE:
B
H
N
HAND TOOLS:
POST HOLE DIGGER
HAND AUGER
SOUNDING ROD
VANE SHEAR TEST

ROCK HARDNESS (CONT.)
FRACTURE SPACING BEDDING
TERM SPACING TERM THICKNESS
VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET
WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET
MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET
CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET
VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET
THINLY LAMINATED < 0.008 FEET

INDURATION
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.
FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

COLOR
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

EQUIPMENT USED ON SUBJECT PROJECT (CONT.)
DIEDRICH D-50
CME 450

INDURATION (CONT.)
EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION

**SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES
 FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS**

AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		SURFACE CONDITIONS					GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos, P and Hoek E., 2000)		SURFACE CONDITIONS OF DISCONTINUITIES (Predominantly bedding planes)				
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.		VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slackensided, highly weathered surfaces with compact coatings or fillings or angular fragments	VERY POOR Slackensided, highly weathered surfaces with soft clay coatings or fillings	From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.		VERY GOOD - Very Rough, fresh unweathered surfaces	GOOD - Rough, slightly weathered surfaces	FAIR - Smooth, moderately weathered and altered surfaces	POOR - Very smooth, occasionally slackensided surfaces with compact coatings or fillings with angular fragments	VERY POOR - Very smooth, slackensided or highly weathered surfaces with soft clay coatings or fillings
STRUCTURE		DECREASING SURFACE QUALITY →					COMPOSITION AND STRUCTURE						
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90				N/A	N/A	A. Thick bedded, very blocky sandstone. The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.	70					
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	80		70				B. Sandstone with thin inter-layers of siltstone	60					
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets			60				C. Sandstone and siltstone in similar amounts		50				
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity				50			D. Siltstone or silty shale with sandstone layers			40			
DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces					40		E. Weak siltstone or clayey shale with sandstone layers				30		
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes						30	F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure					20	
						20	G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers						10
						10	H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.						
		N/A	N/A										

→ Means deformation after tectonic disturbance

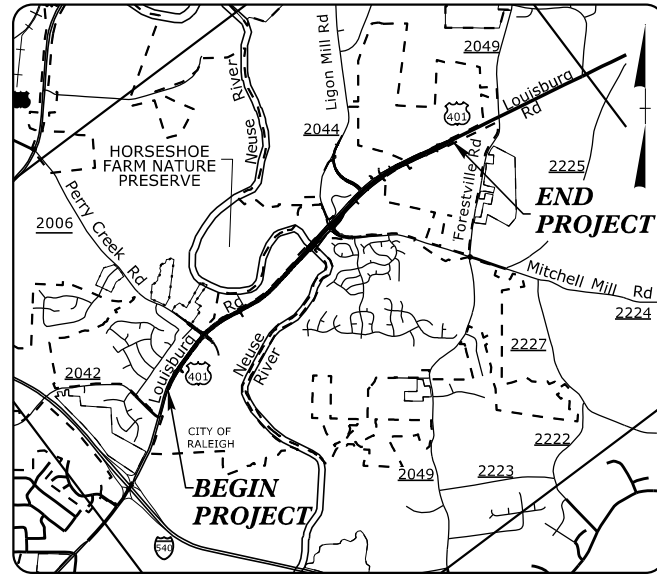
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5748	3	99
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
50168.1.1		PE	
50168.2.1		RW	

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

WAKE COUNTY

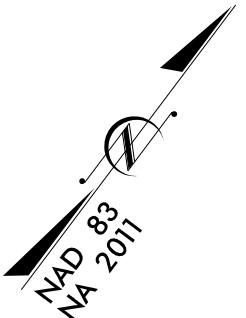
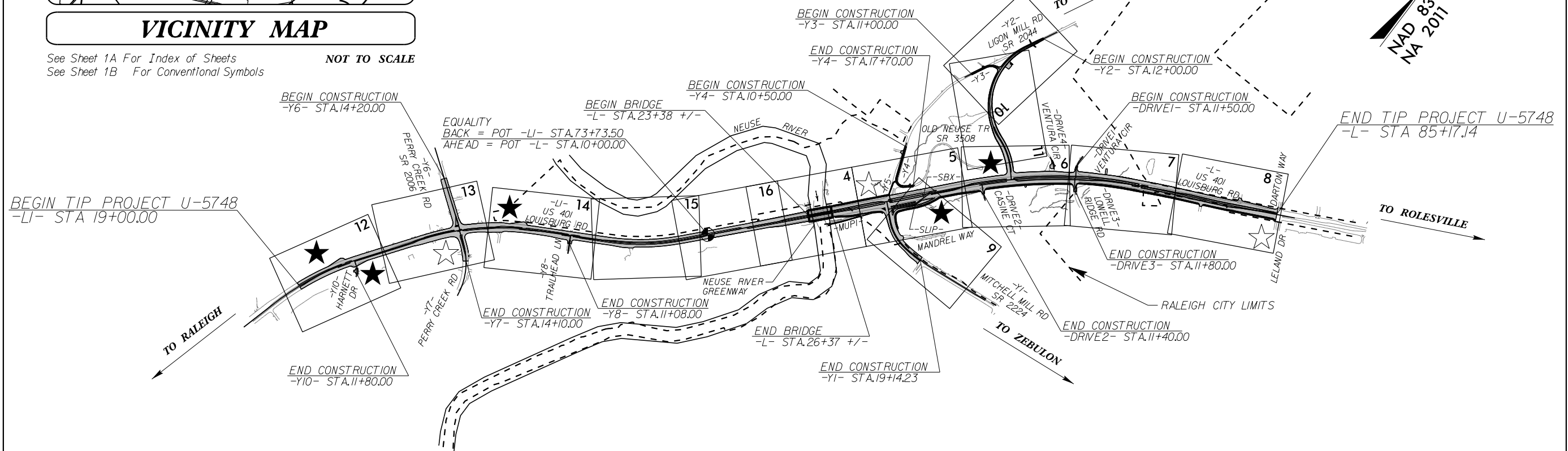
LOCATION: US 401 (LOUISBURG RD) AT SR 2044 (LIGON MILL RD) / SR 224 (MITCHELL MILL RD) AND SR 2006 (PERRY CREEK ROAD) INTERSECTION IMPROVEMENTS
TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNALS, AND STRUCTURES

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED



VICINITY MAP

See Sheet 1A For Index of Sheets
See Sheet 1B For Conventional Symbols
NOT TO SCALE



THIS IS A PARTIAL CONTROLLED-ACCESS PROJECT WITH ACCESS LIMITED TO POINTS AS SHOWN ON THE PLANS.

CLEARING ON THE PROJECT SHALL BE TO THE LIMITS ESTABLISHED USING METHOD III.

A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF THE CITY OF RALEIGH.

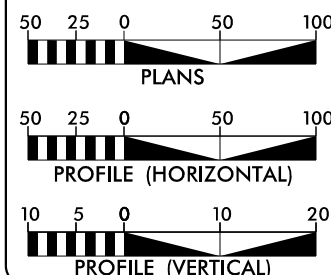
SUBMITTAL: 75% ROADWAY PLANS
DATE: MARCH 4, 2022

- ★ PROPOSED SIGNAL
- ☆ EXISTING SIGNAL

TIP PROJECT: U-5748

CONTRACT:

GRAPHIC SCALES



DESIGN DATA

ADT 2022 = 38,425
ADT 2042 = 58,842
K = 10 %
D = 55 %
T = 3% % *
V = 50 MPH
* TTST = 1% DUAL 2%
FUNC CLASS =
PRINCIPAL ARTERIAL
REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5748 = 2.403 MI
LENGTH STRUCTURE TIP PROJECT U-5748 = 0.057 MI
TOTAL LENGTH TIP PROJECT U-5748 = 2.460 MI



Prepared In the Office of:
NC FIRM LICENSE No: F-0342
5438 Wade Park Blvd., Suite 200
Raleigh, NC 27607
(919) 854-6200 - (919) 854-6259(FAX)

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:
JANUARY 8, 2021

LETTING DATE:
MARCH 21, 2023

ED EDENS, P.E.
PROJECT ENGINEER

ELIZABETH WARGO, P.E.
PROJECT DESIGN ENGINEER

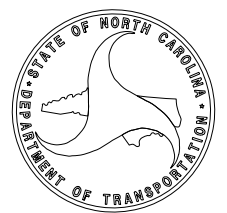
JENNIFER EVANS, P.E.
NCDOT PROJECT ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.





AECOM – North Carolina
1600 Perimeter Park Drive, Suite 400
Morrisville, NC 27560
Tel: 919-461-1100
Fax: 919-461-1415

October 4, 2022

WBS NO.: 50168.1.1
NCDOT TIP: U-5748
COUNTY: Wake

DESCRIPTION: US 401 (Louisburg Rd) from South of SR 2044 (Ligon Mill Rd)/SR 2224 (Michell Mill Rd) to Leland Dr & US 401 at SR 2006 (Perry Creek Rd) Intersection Improvements

SUBJECT: Geotechnical Report – Roadway Inventory

Project Description

The proposed project consists of widening 2.44 miles of US-401 starting from about 1/2 mile south of Perry Creek Road and ending at the intersection of US-401/Leland Drive in Wake Forest, Wake County, North Carolina. The intersection of US 401 and Perry Creek Road will be upgraded to a superstreet configuration. Also, to improve traffic flow near the existing US-401/Ligon Mill Rd/Mitchell Mill Rd intersection, this project includes realigning Ligon Mill Rd so that it intersects US-401 about 0.30 miles northeast from its current location and reconfiguring the Mitchell Mill Rd side of the intersection. Maximum cut and fill thicknesses on the project are approximately 25-ft and 30-ft, respectively.

Two geotechnical investigations were conducted. The initial investigation was conducted from December 9, 2019 through March 12, 2020 and the second investigation was conducted from January 11, 2022 to January 14, 2022. Summit Design and Engineering Services from Hillsborough, North Carolina were contracted for both investigations to provide drilling services.. A total of 94 hollow stem auger borings and 32 hand-auger borings were conducted. Standard penetration testing (SPT) was performed in the majority of the hollow stem auger borings. Representative soil samples were collected for visual classification in the field and select samples were submitted for laboratory analysis. AECOM subcontracted Geotechnics, Inc. of Raleigh, North Carolina to provide laboratory services for this project. The following alignments were investigated:

<u>Line</u>	<u>Station(±)</u>
-L-	15+50 to 85+17
-L1-	19+00 to 73+74
-Y1-	10+00 to 13+74
-Y2-	12+00 to 34+17
-Y3-	11+00 to 15+11
-Y4-	10+50 to 17+70

Physiography and Geology

The project is located in the Piedmont Physiographic Province. Along the project corridor the terrain is relatively flat to slightly undulated and land use mostly consists of existing roadways and adjacent commercial and private properties. According to the US Geological Survey¹, the near surface geology consists primarily of foliated to massive granitic rock. Foliated to massive granitic rock is described as megacrystic to equigranular. According to the North Carolina Geological Survey², the igneous units along the project corridor are Late-Paleozoic-aged Rolesville granitoid (*Prg*). *Prg* is described as medium-to coarse-grained to megacrystic monzogranite, granite, and granodiorite. Alluvial material is also located within the project limits as the Neuse River crosses near the southern part of the project. Alluvium in this area is described as tan to light gray, unconsolidated, with poorly sorted and stratified deposits of angular to subrounded gravel, sand, silt, and clay.

Soil Properties

The project encountered the following soil types: Roadway Embankment, Alluvium, Residuum, and Artificial Fill.

Roadway Embankment soils consisted of brown to orange to gray, dry to wet, sand and fine sand (A-1-b, A-3), silty to clayey sand (A-2-4, A-2-6), sandy silt (A-4), and sandy clay (A-6). N-values ranged from 2 to 19 blows per foot (bpf), indicating very loose to medium dense conditions for granular soils and soft to very stiff consistencies for fine soils. The average N-value is 11 bpf.

Alluvial deposits were mainly found in the area around the Neuse River. These deposits consisted of brown to tan to gray, moist to saturated, silty to clayey sand (A-2-4, A-2-6), sandy to clayey silt (A-4, A-5), and sandy to silty clay (A-6, A-7). N-values ranged from 2 to 25 bpf, indicating very loose to medium dense conditions for granular deposits and soft to very stiff consistencies for fine soils. The average N-value is 9 bpf.

Residual soils were encountered throughout the entire project and consisted of brown to orange to gray, dry to wet, sand (A-1-b), silty to clayey sand (A-2-4, A-2-6, A-2-7), sandy silty (A-4), and sandy to silty clay (A-6, A-7-5, A-7-6). SPT N-values indicate the consistency and densities of the soils ranged from soft to hard for fines and very loose to very dense for granular soils. These soils appeared to be broken down Rolesville granite.

Artificial Fill was encountered in few areas across the project: (1) a man-made soil mound near the proposed -Y2-/-Y3- intersection consists of brown and gray, saturated, sandy clay (A-6); (2) the right side of US 401, near STA 77+00, consists of brown to orange, dry to moist, silty sand (A-2-4); and (3) the right side of US 401, near STA 82+80, consists of dark brown to olive gray, moist to saturated, silty to clayey sand (A-2-4, A-2-6).

¹<https://mrdata.usgs.gov/geology/state/sgmc-unit.php?unit=NCPAmg%3B8>

²https://files.nc.gov/ncdeq/Energy%20Mineral%20and%20Land%20Resources/Geological%20Survey/OFRs_Geological_Survey/NCGS_OFR_2004-02_Raleigh_100k_bedrock_geopdf.pdf

Rock Properties

Rolesville granite was the predominant rock type encountered across the project. Rolesville granite generally consists of highly competent, fine-grained granite which is often exposed at the surface and weathers to sandy soils.

-Y2-	17+25 to 22+00	LT to RT
-Y2-	25+75 to 31+50	LT to RT
-Y3-	14+00 to 15+00	LT to RT
-Y4-	11+75 to 15+75	LT

Groundwater Properties

Groundwater was encountered in 19 of the borings. In these borings, depth to groundwater ranged from 0.6-ft to 22.9-ft below ground surface (bgs) with an average depth of 7.2-ft bgs.

The Neuse River crosses the -L- alignment at STA 25+00. A drainage ditch, running parallel to the -L- alignment on the right side, flows back station towards the Neuse River beginning at STA 45+48 and ending at STA 37+17 where it flows into an existing culvert, which transports it underneath US-401 and towards the Neuse River. A stream that flows south, into an existing culvert crosses US-401 (-L-) near STA 64+50 where the toe of the embankment is expected to widen approximately 70-ft. A stream that flows into a farm pond crosses the proposed -Y2- alignment near STA 25+00.

Areas of Special Geotechnical Interest

- 1) **Shallow Rock:** The following sections were found to contain rock above or within 6 feet of grade:

<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>
-L-	31+50 to 34+00	RT
-L-	38+50 to 40+00	LT
-L-	70+50 to 75+50	LT
-L-	76+50 to 77+50	RT
-L-	78+75 to 79+25	LT

- 2) **Highly Plastic Soils:** Highly plastic soils were encountered in few areas and at various individual borings throughout the project. Atterberg limit tests for 24 samples had plasticity indices greater than or equal to 26. The following alignments were found to contain highly plastic soils:

<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>
-L-	30+25 to 31+75	RT
-L-	39+25 to 41+75	LT
-L-	43+25 to 46+75	LT
-L-	49+25 to 56+50	RT
-L-	51+75 to 55+75	LT
-L-	58+95 to 60+10	LT
-L-	65+50 to 67+50	LT
-L-	75+50 to 77+75	LT
-L-	78+00 to 80+75	RT
-L-	80+50 to 81+50	LT
-L-	82+00 to 83+75	RT
-L1-	28+75 to 30+75	RT
-L1-	55+75 to 57+25	RT
-Y1-	12+25 to 14+25	LT

- 3) **Soft Soils:** Soft soils were found in the following alignments:

<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>
-L-	27+00 to 29+50	LT
-L-	34+50 to 37+43	RT
-L-	38+00 to 40+00	RT
-L-	58+95 to 60+10	LT
-L-	82+00 to 83+75	RT

- 4) **Loose Sands:** Loose sands were found in the following alignments:

<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>
-L-	17+00 to 19+50	LT to RT
-L-	26+41 to 29+50	RT
-L-	29+50 to 31+50	LT to RT
-L-	34+00 to 44+25	LT
-L-	35+50 to 44+50	RT
-L-	46+00 to 49+75	LT
-L-	47+50 to 48+50	RT
-L-	51+25 to 52+75	LT
-L-	54+25 to 61+50	LT
-L-	54+50 to 56+25	RT
-L-	62+50 to 66+50	LT
-L-	67+50 to 69+50	LT
-L-	73+75 to 77+50	LT
-L-	76+75 to 77+25	RT
-L-	79+50 to 82+00	LT
-L-	80+25 to 83+75	RT
-Y1-	12+25 to 14+25	LT
-Y2-	17+25 to 33+50	LT to RT

- 5) **High Water:** The following areas encountered water within 6-ft of proposed grade:

<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>
-L-	35+25 to 35+75	RT
-L-	36+25 to 36+50	LT
-L-	40+50 to 41+50	LT to RT
-L-	46+00 to 47+50	LT

- 6) **Wet Soils:** The following areas had wet to saturated soils near the surface that could potentially affect construction of proposed embankment widening.

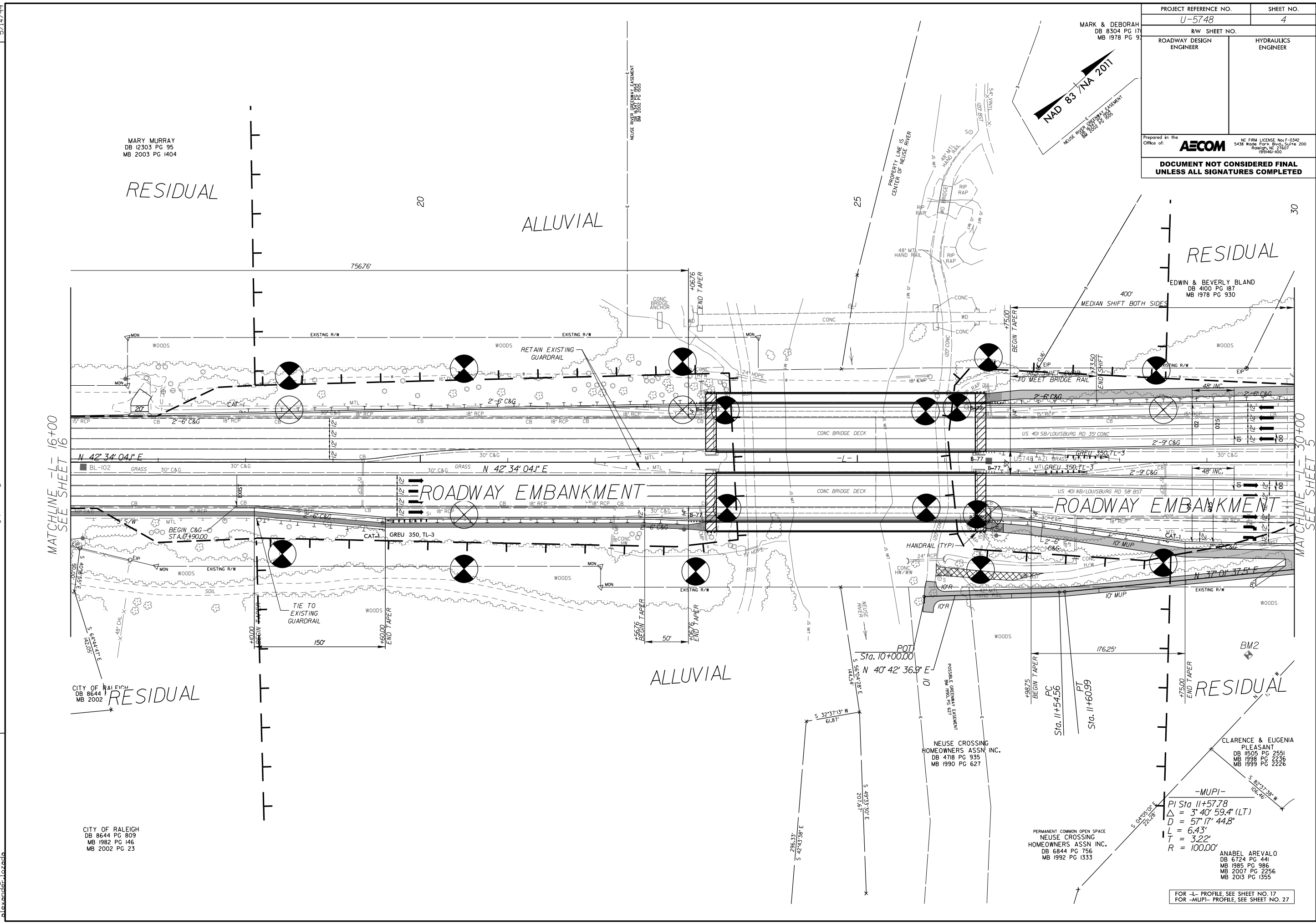
<u>Line</u>	<u>Stations (±)</u>	<u>Offset</u>	<u>Moisture</u>
-L-	38+75 to 39+75	RT	W-Sat.
-L-	41+75 to 44+75	RT	M-W

Bulk Samples

Bulk samples were collected at the following locations and submitted for testing.

<u>Line</u>	<u>Sample No.</u>	<u>Station</u>	<u>Offset</u>	<u>Depth (ft)</u>	<u>Test</u>
-L-	BULK-02	48+00	95' RT	0.5-2.0	Standard Proctor/CBR
-L-	BULK-03	77+00	70' LT	0.5-2.0	Standard Proctor/CBR
-L-	BULK-04	78+00	95' LT	0.1-1.0	Standard Proctor
-L-	BULK-05	52+00	108' LT	0.5-2.0	Standard Proctor/CBR
-L-	BULK-06	36+32	123' LT	0.1-1.0	Standard Proctor/CBR
-L1-	BS-001	49+46	70' LT	0.5-2.0	Standard Proctor/CBR
-L1-	BS-002	26+48	65' RT	0.5-2.0	Standard Proctor/CBR
-Y2-	BULK-01	28+50	0' CL	1.3-3.0	Standard Proctor
-Y3-	BULK-07	14+13	36' RT	0.5-2.0	Standard Proctor/CBR

PROJECT REFERENCE NO.	SHEET NO.
U-5748	4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
Prepared in the Office of:	
AECOM <small>NC FIRM LICENSE No. F-0342 5438 Wade Park Blvd., Suite 200 Raleigh, NC 27607 919461-800</small>	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



REVISIONS
 ROW REV. - March 11, 2021 - Parcel 1: Rev. owner info, rev. PUE, add RW, Parcel 2: Rev. owner info, rev. PUE, rev. TCE, add RW, Parcel 4: Rev. PUE, add RW, Parcel 4: Rev. PUE, add RW, Parcel 5: Rev. R.W.
 ROW REV. - July 14, 2021 - Parcel 5: Combined Parcels 49 and 50. Eliminated Parcel 49.
 11/21/2022 U-5748-11-400-Technical\431-Geotechnical\U5748-11-400-Technical.dgn
 alexander

MATCHLINE -L- 16+00 SEE SHEET 16

MATCHLINE -R- 30+00 SEE SHEET 5

CITY OF RALEIGH
 DB 8644 PG 809
 MB 1982 PG 146
 MB 2002 PG 23

CITY OF RALEIGH
 DB 8644 PG 809
 MB 1982 PG 146
 MB 2002 PG 23


NEUSE CROSSING
 HOMEOWNERS ASSN. INC.
 DB 4718 PG 935
 MB 1990 PG 627

PERMANENT COMMON OPEN SPACE
 NEUSE CROSSING
 HOMEOWNERS ASSN. INC.
 DB 6844 PG 756
 MB 1992 PG 1333

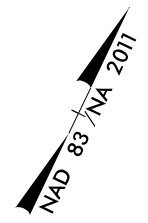
CLARENCE & EUGENIA
 PLEASANT
 DB 1505 PG 2551
 MB 1998 PG 2236
 MB 1999 PG 2226

-MUPI-
 PI Sta 11+57.78
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 $D = 57' 17" 44.8"$
 $L = 6.43'$
 $T = 3.22'$
 $R = 100.00'$

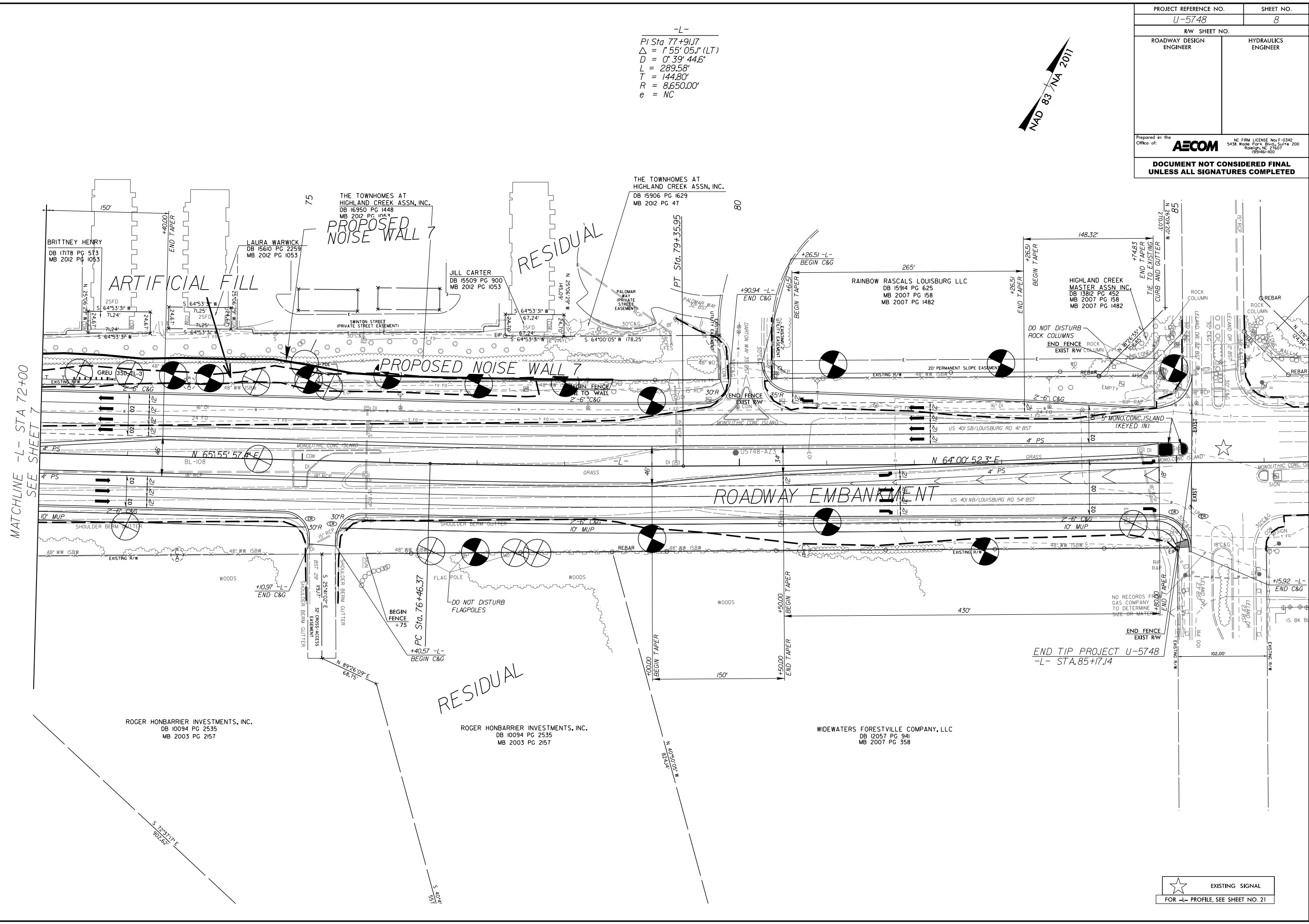
FOR -L- PROFILE, SEE SHEET NO. 17
 FOR -MUPI- PROFILE, SEE SHEET NO. 27


PROJECT REFERENCE NO.	SHEET NO.
U-5748	8
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
Prepared in the Office of:	
	
<small>NC FIRM LICENSE No. F-0342 5438 Wade Park Blvd., Suite 200 Raleigh, NC 27607 199461-000</small>	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

-L-
 PI Sta 77+91.7
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 $D = 0^{\circ}39'44.6''$
 $L = 289.58'$
 $T = 144.80'$
 $R = 8,650.00'$
 $e = NC$

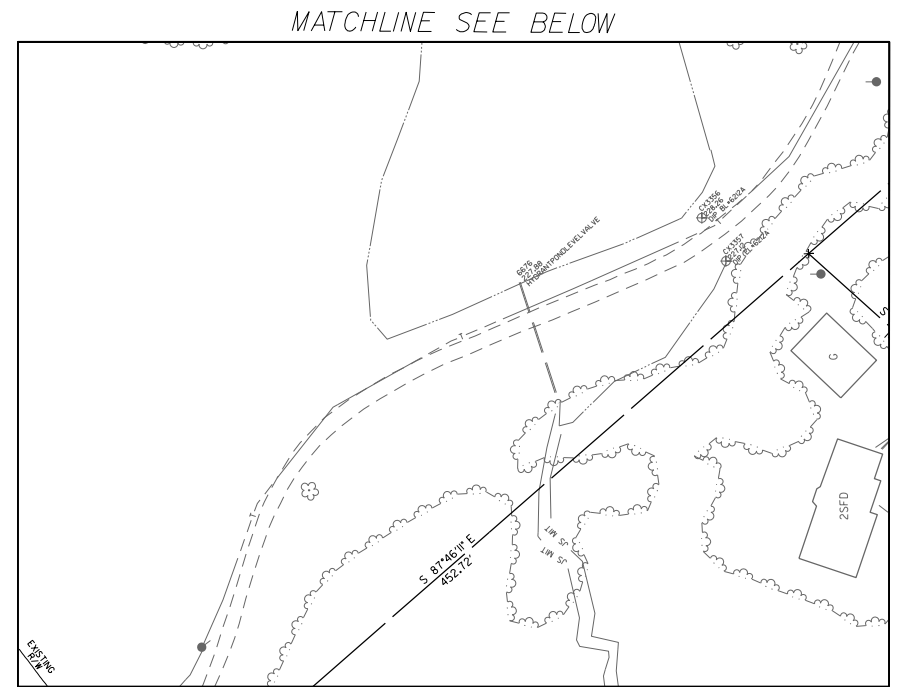
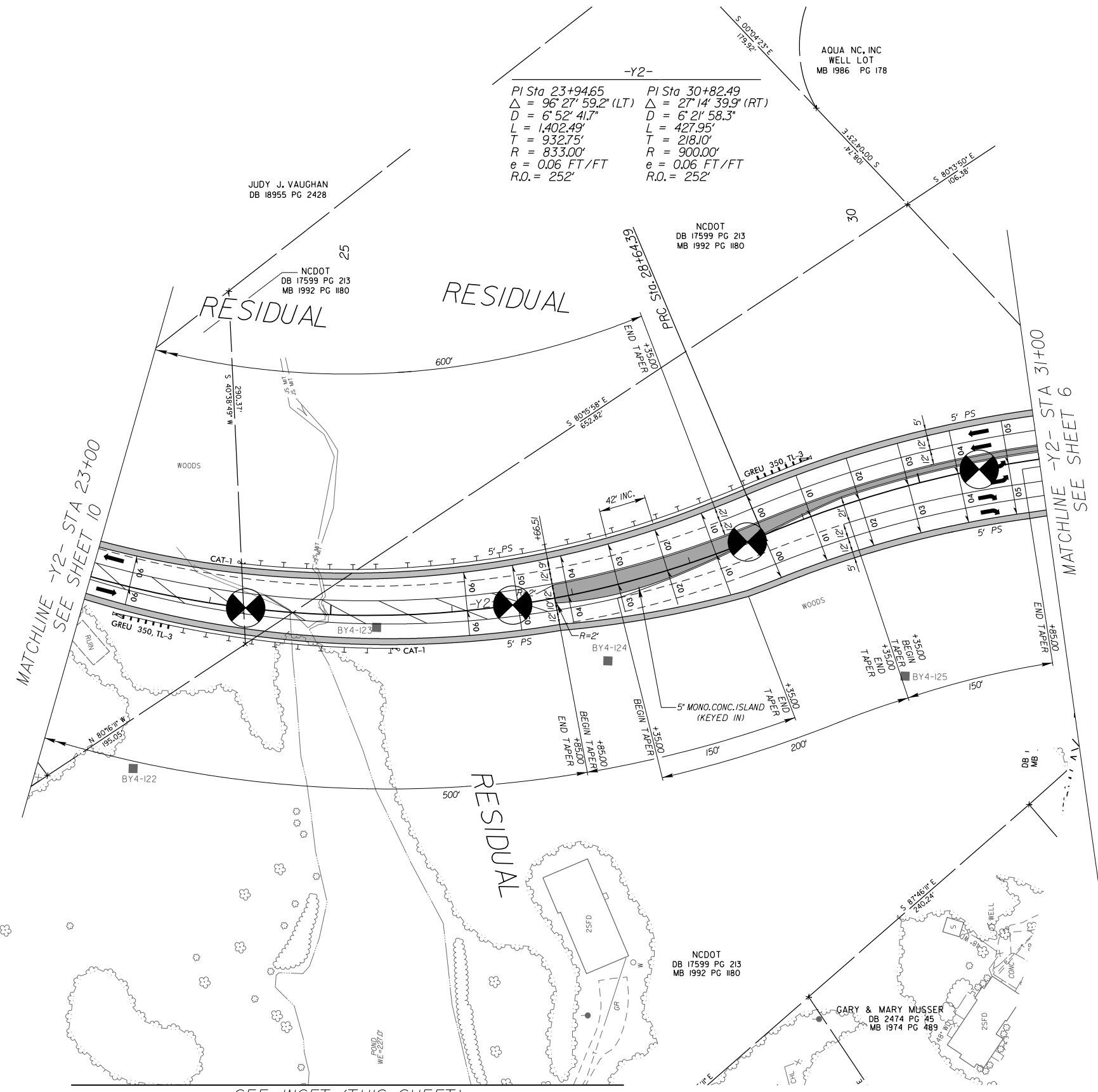
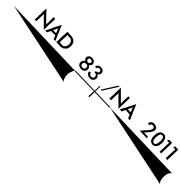


REVISIONS
 ROW REV. - March 11, 2021 - Parcel 20. Rev. R/W, rev. TCE. Eliminated Parcel 21. Parcel 22. Rev. R/W, rev. TCE.
 ROW REV. - July 14, 2021 - Parcel 33. Combined Parcels 33 and 34. Eliminated Parcel 34. Parcel 51. Added previously omitted Parcel 51. Added proposed noise wall.
 10/4/2022 U-5748-01-01-000-Technical\431-Geotechnical\431-Geotechnical\U5748-01-01-000-Technical\431-Geotechnical\431-Geotechnical\U5748-01-01-000-Technical.dgn
 5/14/99



 EXISTING SIGNAL
 FOR -L- PROFILE, SEE SHEET NO. 21

PROJECT REFERENCE NO.	SHEET NO.
U-5748	11
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
Prepared in the Office of:	
AECOM	
<small>NC FIRM LICENSE No. F-0342 5438 Wade Park Blvd., Suite 200 Raleigh, NC 27607 199461-000</small>	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



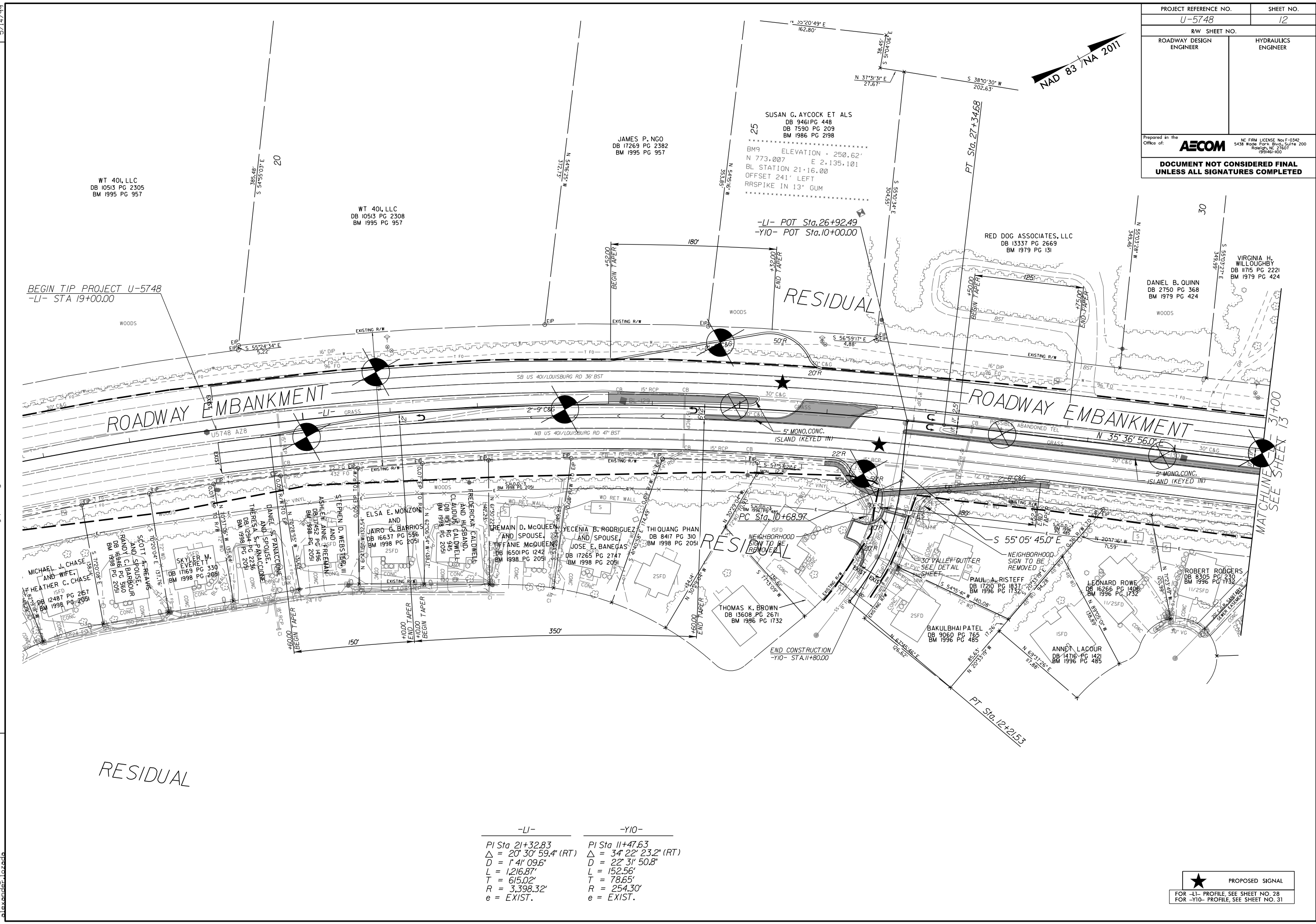
REVISIONS
 ROW REV. - March 11, 2021 - Parcel 41: Rev. owner info, Parcel 42: Corrected Parcel 42 to 41, eliminated parcel 42, Parcel 44: Rev. owner info, added PDE.
 ROW REV. - July 14, 2021 - Parcel 39: Combined Parcels 39 and 40, Eliminated Parcel 40.
 ROW REV. - July 14, 2021 - Parcel 39: Combined Parcels 39 and 40, Eliminated Parcel 40.
 10/4/2022 M:\11\400-Technical\431-Geotechnical\U5748.GEO.RDWAY\CADD.GEOTECH\PlanPr\of\U5748_r.dwg_psh11.dgn
 alexander@aec.com

5/14/99
 10/4/2022 M:\11\400-Technical\431-Geotechnical\U5748.GEO.RDWAY\CADD.GEOTECH\PlanPr\of\U5748_r.dwg_psh11.dgn
 alexander@aec.com

SEE INSET (THIS SHEET)

FOR -Y2- PROFILE, SEE SHEET NO. 23

PROJECT REFERENCE NO.		SHEET NO.	
U-5748		12	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
Prepared in the Office of: AECOM NC FIRM LICENSE No. F-0342 5438 Wade Park Blvd., Suite 200 Raleigh, NC 27607 199461-000			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED			



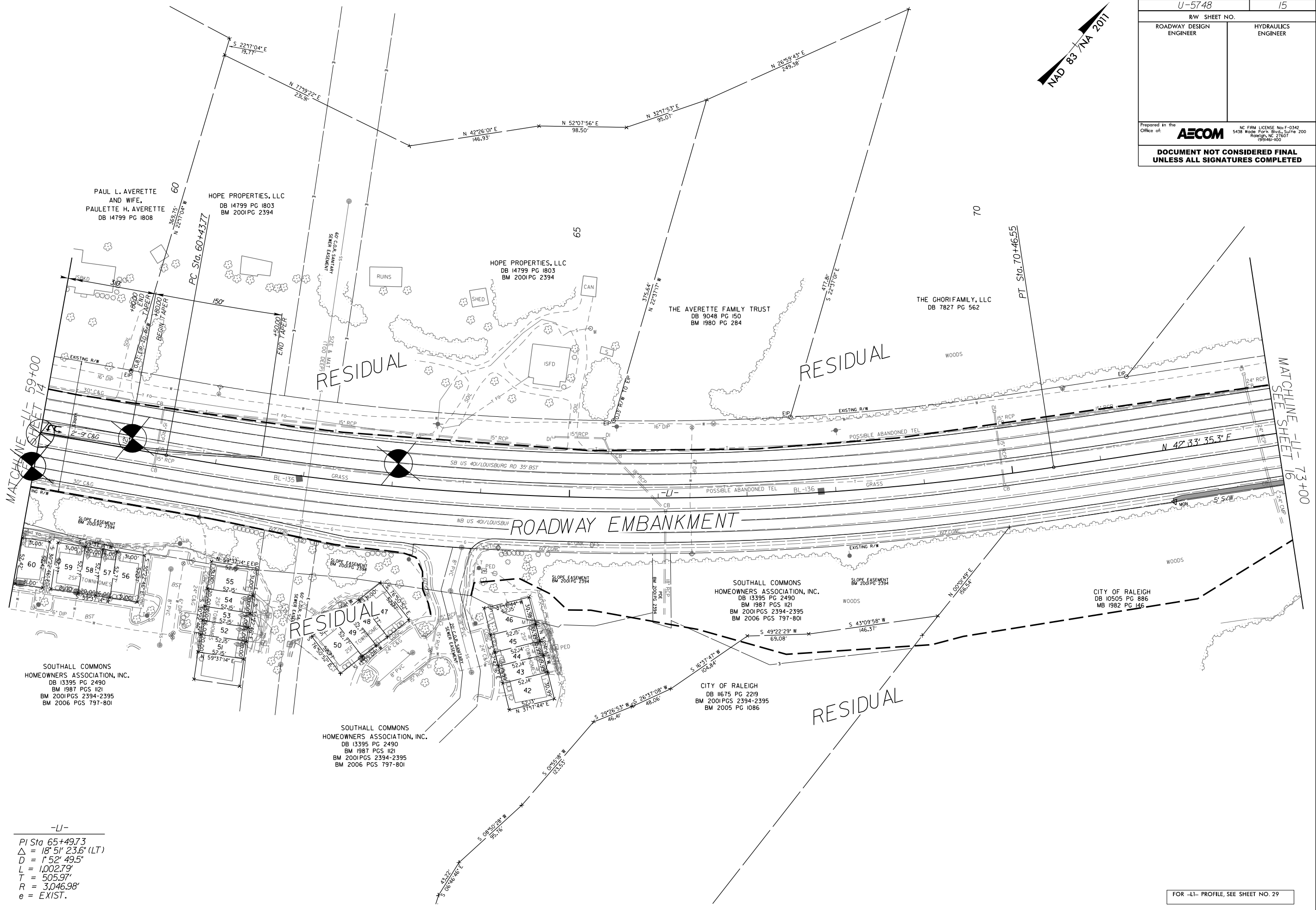
REVISIONS
 Design REV. - April 2022 - Changed property limits and added Sheet 12. Parcel 52: added CA and TCE. Parcel 53: added RW, CA, and TCE. Parcel 54: added CA and TCE. Parcel 61: added CA and TCE. Parcel 62: added CA and TCE. Parcel 63: added CA and TCE.
 ROW REV. - August 29, 2022 - Parcel 64: Removed PDE. Eliminated dam on Parcel 64.

-LI-	-YIO-
PI Sta 21+32.83	PI Sta 11+47.63
Δ = 20° 30' 59.4" (RT)	Δ = 34° 22' 23.2" (RT)
D = 1' 41" 09.6"	D = 22' 31" 50.8"
L = 1,216.87'	L = 152.56'
T = 615.02'	T = 78.65'
R = 3,398.32'	R = 254.30'
e = EXIST.	e = EXIST.

★ PROPOSED SIGNAL
 FOR -LI- PROFILE, SEE SHEET NO. 28
 FOR -YIO- PROFILE, SEE SHEET NO. 31

5/14/99
 10/4/2022
 9754-U-5748-GeoTechnical\431-GeoTechnical\431-GeoTechnical\U5748.dgn
 9754-U-5748-GeoTechnical\431-GeoTechnical\431-GeoTechnical\U5748.dgn

PROJECT REFERENCE NO.	SHEET NO.
U-5748	15
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
Prepared in the Office of:	NC FIRM LICENSE No. F-0342 5438 Wade Park Blvd., Suite 200 Raleigh, NC 27607 19946-000
AECOM	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



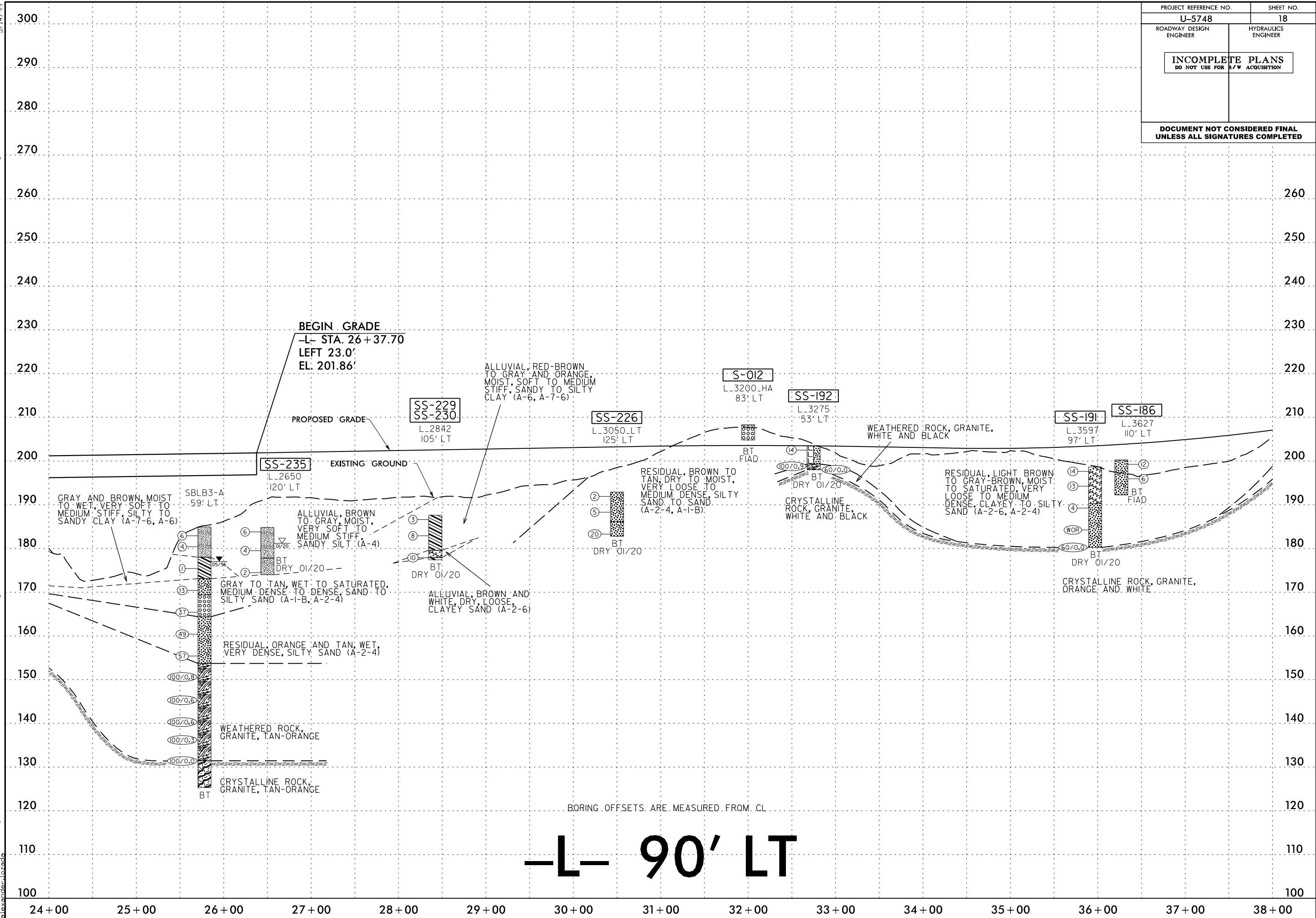
-LI-
 PI Sta 65+497.3
 $\Delta = 18' 51'' 23.6'' (LT)$
 $D = 1' 52'' 49.5''$
 $L = 1,002.79'$
 $T = 505.97'$
 $R = 3,046.98'$
 $e = EXIST.$

FOR -LI- PROFILE, SEE SHEET NO. 29

REVISIONS
 Design REV. - April, 2022 - Changed project limits and added Sheet 15. Parcel 59, added TCE.

5/14/99
 I:\4\2022\U-5748\Geotech\431-Technical\431-Geotech\431\400-Technical\431-Geotech\431\U5748_GEO.CADD\RDWY\CADD_GEO\TECH\Plan\U5748_rdy_psh15.dgn
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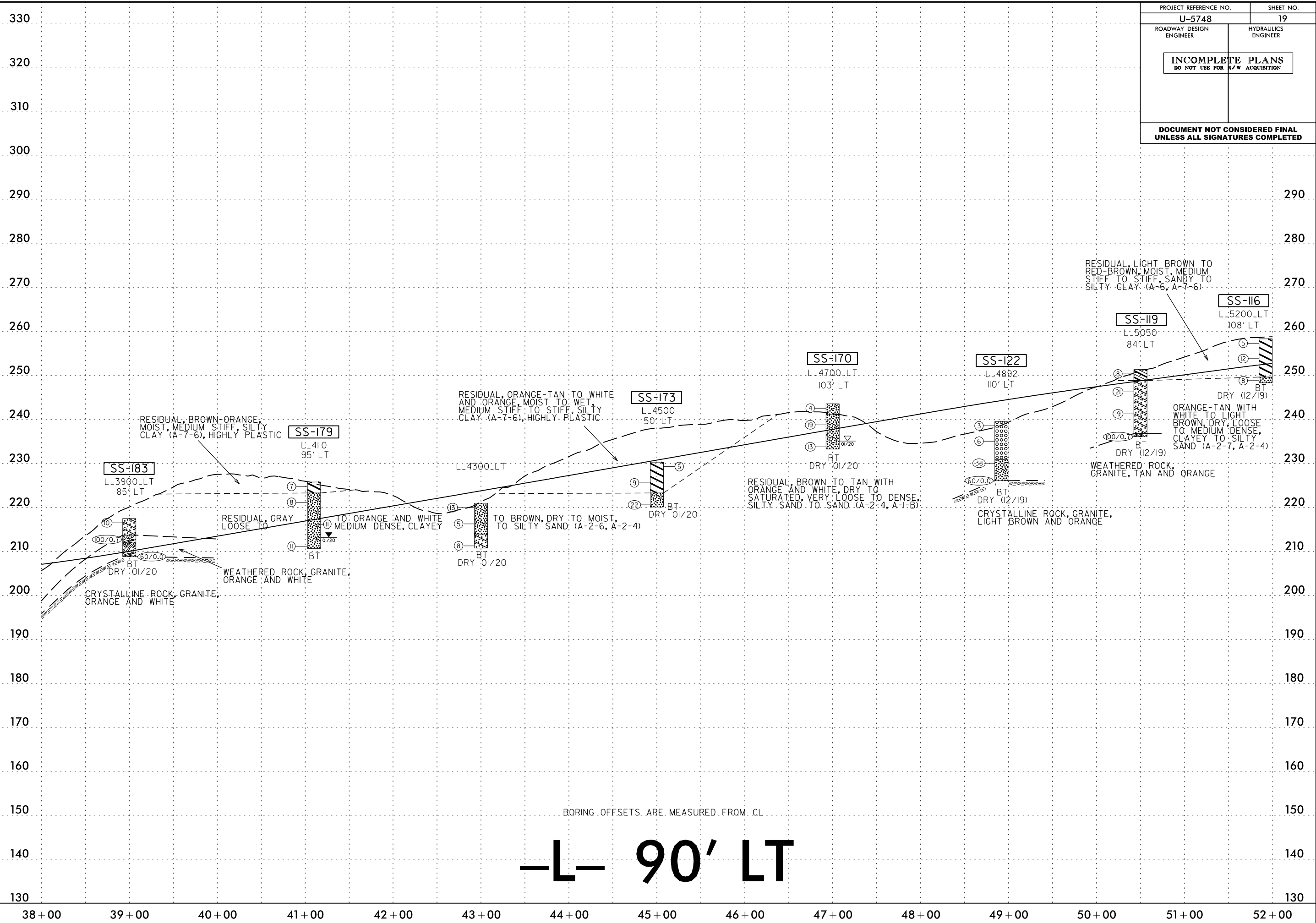
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



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 5/14/99

PROJECT REFERENCE NO.	SHEET NO.
U-5748	19
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

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 5/14/99



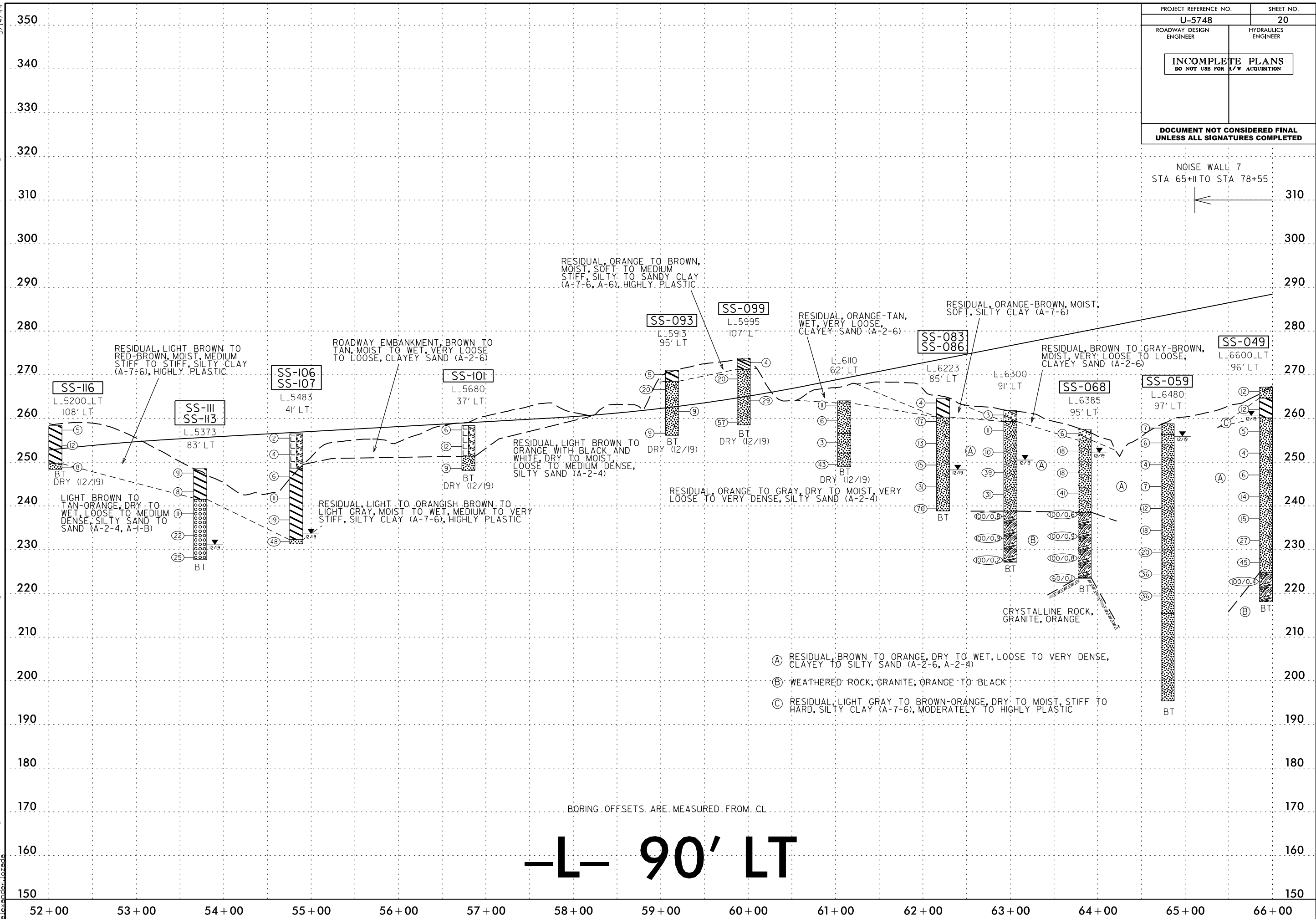
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38+00 39+00 40+00 41+00 42+00 43+00 44+00 45+00 46+00 47+00 48+00 49+00 50+00 51+00 52+00

PROJECT REFERENCE NO.	SHEET NO.
U-5748	20
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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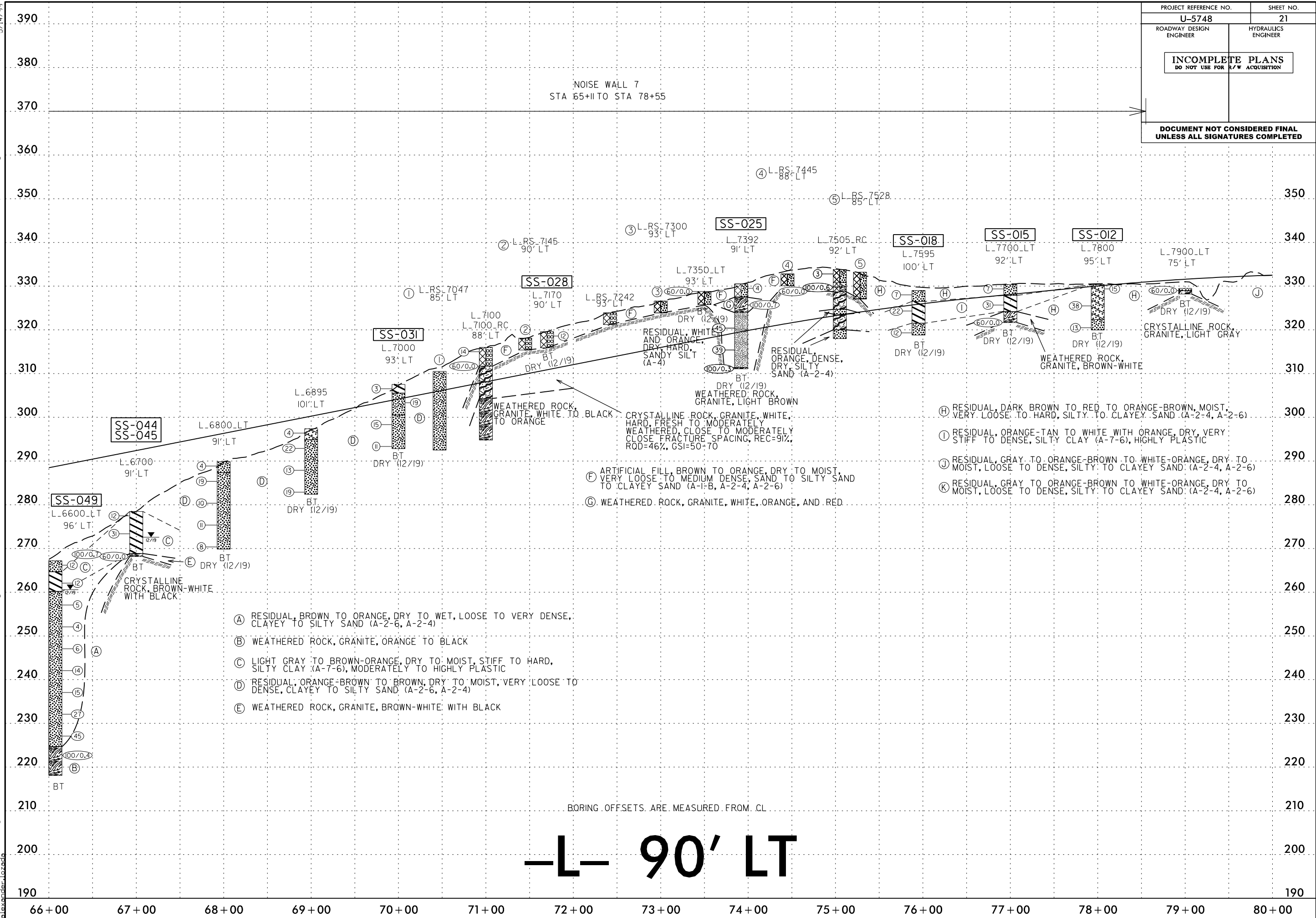
NOISE WALL 7
STA 65+11 TO STA 78+55



BORING OFFSETS ARE MEASURED FROM CL

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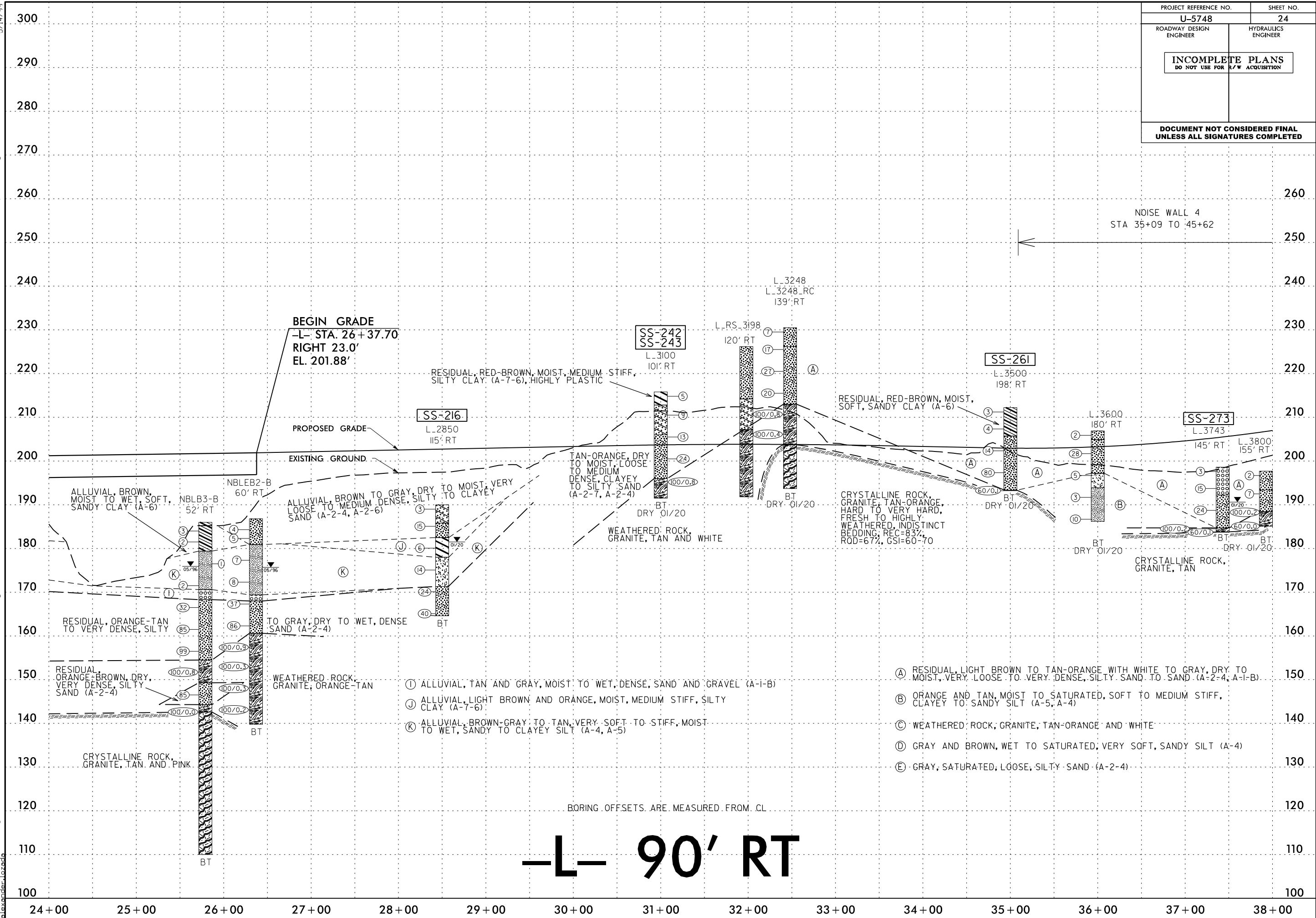
- (A) RESIDUAL, BROWN TO ORANGE, DRY TO WET, LOOSE TO VERY DENSE, CLAYEY TO SILTY SAND (A-2-6, A-2-4)
- (B) WEATHERED ROCK, GRANITE, ORANGE TO BLACK
- (C) LIGHT GRAY TO BROWN-ORANGE, DRY TO MOIST, STIFF TO HARD, SILTY CLAY (A-7-6), MODERATELY TO HIGHLY PLASTIC
- (D) RESIDUAL, ORANGE-BROWN TO BROWN, DRY TO MOIST, VERY LOOSE TO DENSE, CLAYEY TO SILTY SAND (A-2-6, A-2-4)
- (E) WEATHERED ROCK, GRANITE, BROWN-WHITE WITH BLACK

- (H) RESIDUAL, DARK BROWN TO RED TO ORANGE-BROWN, MOIST, VERY LOOSE TO HARD, SILTY TO CLAYEY SAND (A-2-4, A-2-6)
- (I) RESIDUAL, ORANGE-TAN TO WHITE WITH ORANGE, DRY, VERY STIFF TO DENSE, SILTY CLAY (A-7-6), HIGHLY PLASTIC
- (J) RESIDUAL, GRAY TO ORANGE-BROWN TO WHITE-ORANGE, DRY TO MOIST, LOOSE TO DENSE, SILTY TO CLAYEY SAND (A-2-4, A-2-6)
- (K) RESIDUAL, GRAY TO ORANGE-BROWN TO WHITE-ORANGE, DRY TO MOIST, LOOSE TO DENSE, SILTY TO CLAYEY SAND (A-2-4, A-2-6)

BORING OFFSETS ARE MEASURED FROM CL

-L- 90' LT

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-L- 90' RT

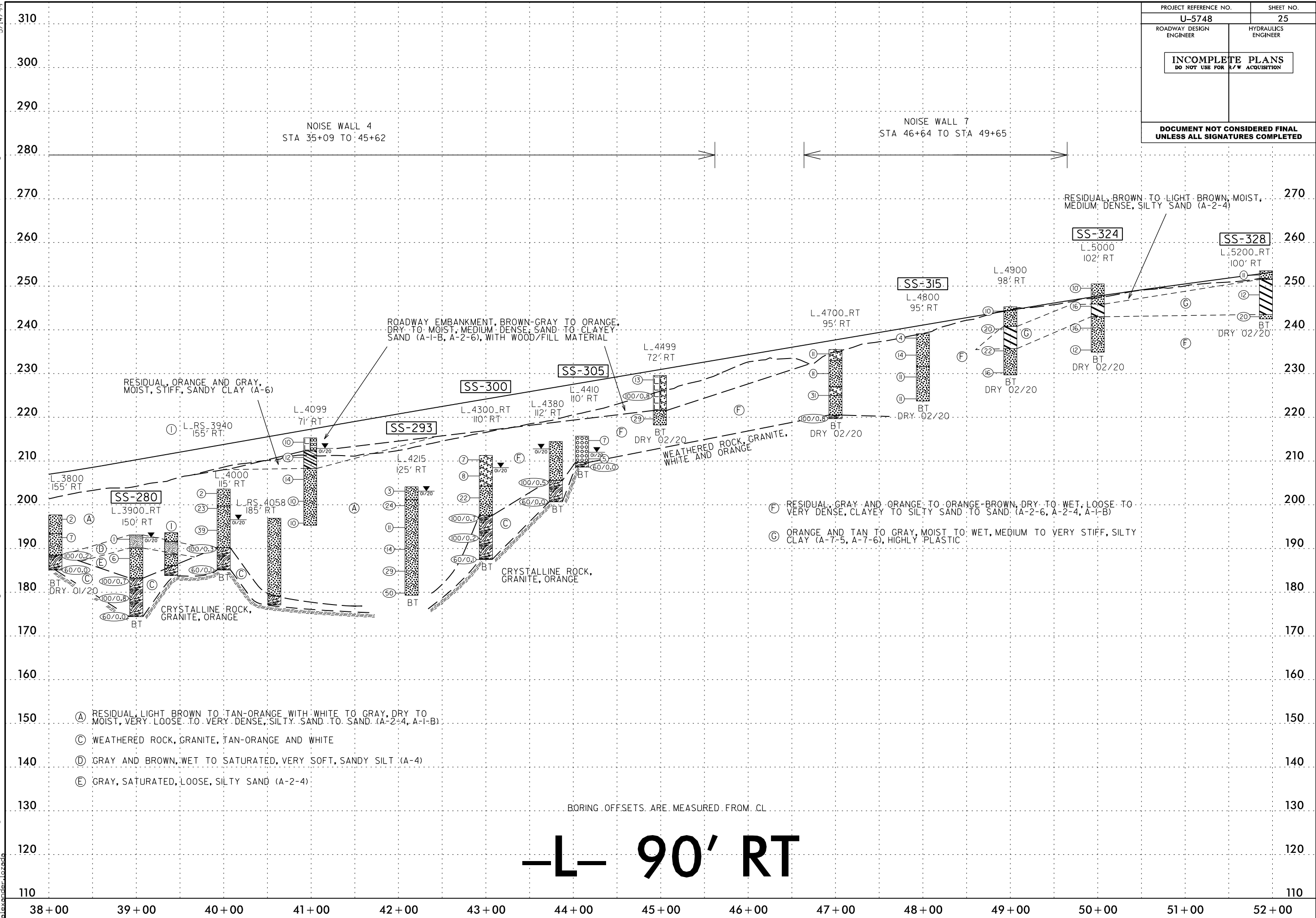
- ① ALLUVIAL, TAN AND GRAY, MOIST TO WET, DENSE, SAND AND GRAVEL (A-I-B)
- ② ALLUVIAL, LIGHT BROWN AND ORANGE, MOIST, MEDIUM STIFF, SILTY CLAY (A-7-6)
- ③ ALLUVIAL, BROWN-GRAY TO TAN, VERY SOFT TO STIFF, MOIST TO WET, SANDY TO CLAYEY SILT (A-4, A-5)

- Ⓐ RESIDUAL, LIGHT BROWN TO TAN-ORANGE WITH WHITE TO GRAY, DRY TO MOIST, VERY LOOSE TO VERY DENSE, SILTY SAND TO SAND (A-2-4, A-I-B)
- Ⓑ ORANGE AND TAN, MOIST TO SATURATED, SOFT TO MEDIUM STIFF, CLAYEY TO SANDY SILT (A-5, A-4)
- Ⓒ WEATHERED ROCK, GRANITE, TAN-ORANGE AND WHITE
- Ⓓ GRAY AND BROWN, WET TO SATURATED, VERY SOFT, SANDY SILT (A-4)
- Ⓔ GRAY, SATURATED, LOOSE, SILTY SAND (A-2-4)

BORING OFFSETS ARE MEASURED FROM CL

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 5/14/99

PROJECT REFERENCE NO.	SHEET NO.
U-5748	25
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



- (A) RESIDUAL, LIGHT BROWN TO TAN-ORANGE WITH WHITE TO GRAY, DRY TO MOIST, VERY LOOSE TO VERY DENSE, SILTY SAND TO SAND (A-2-4, A-1-B)
- (C) WEATHERED ROCK, GRANITE, TAN-ORANGE AND WHITE
- (D) GRAY AND BROWN, WET TO SATURATED, VERY SOFT, SANDY SILT (A-4)
- (E) GRAY, SATURATED, LOOSE, SILTY SAND (A-2-4)

- (F) RESIDUAL, GRAY AND ORANGE TO ORANGE-BROWN, DRY TO WET, LOOSE TO VERY DENSE, CLAYEY TO SILTY SAND TO SAND (A-2-6, A-2-4, A-1-B)
- (G) ORANGE AND TAN TO GRAY, MOIST TO WET, MEDIUM TO VERY STIFF, SILTY CLAY (A-7-5, A-7-6), HIGHLY PLASTIC

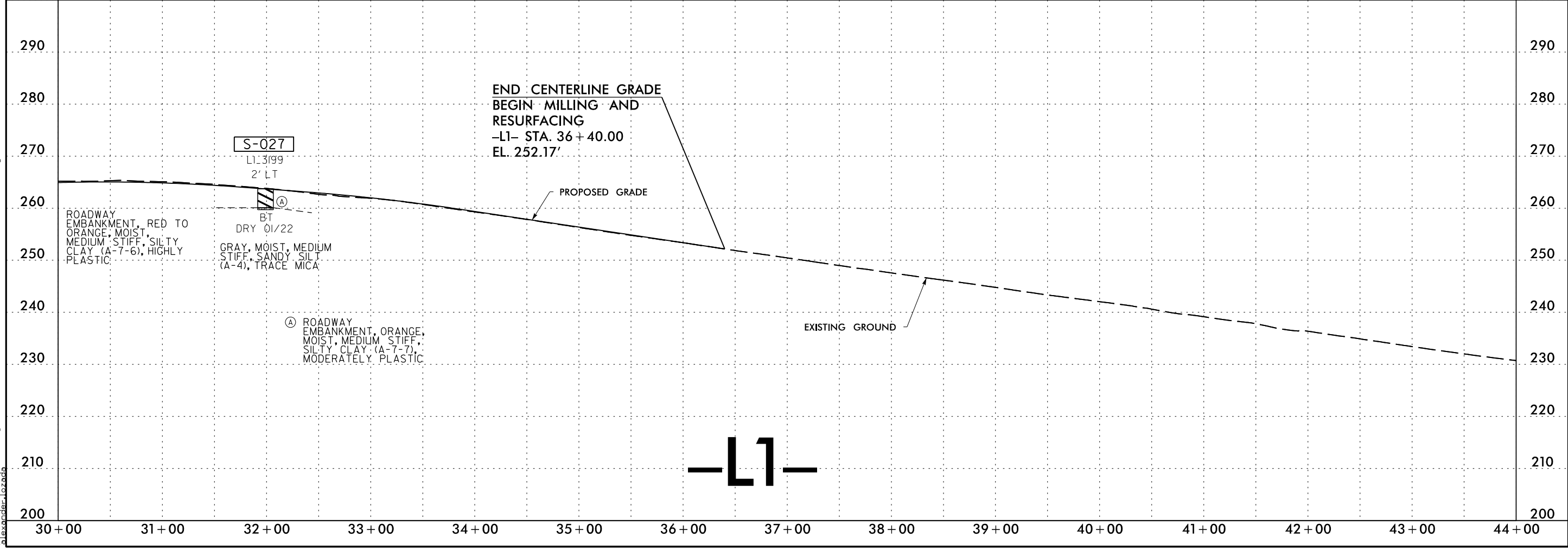
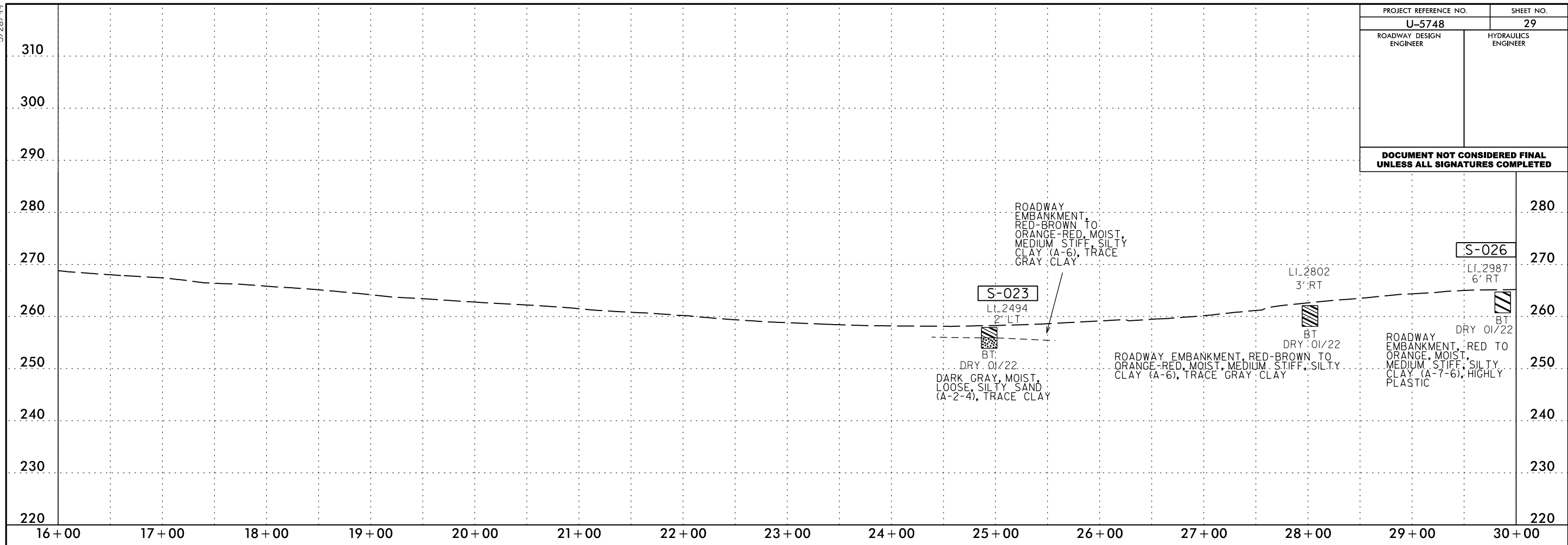
BORING OFFSETS ARE MEASURED FROM CL

-L- 90' RT

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 5/14/99

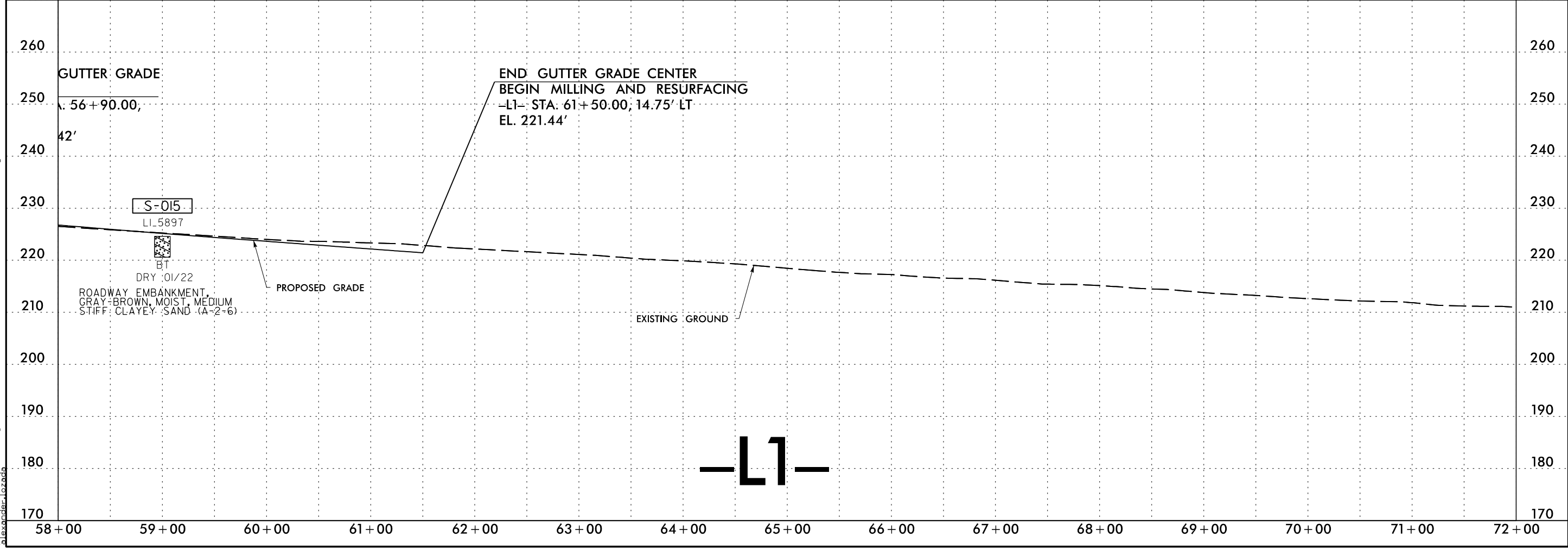
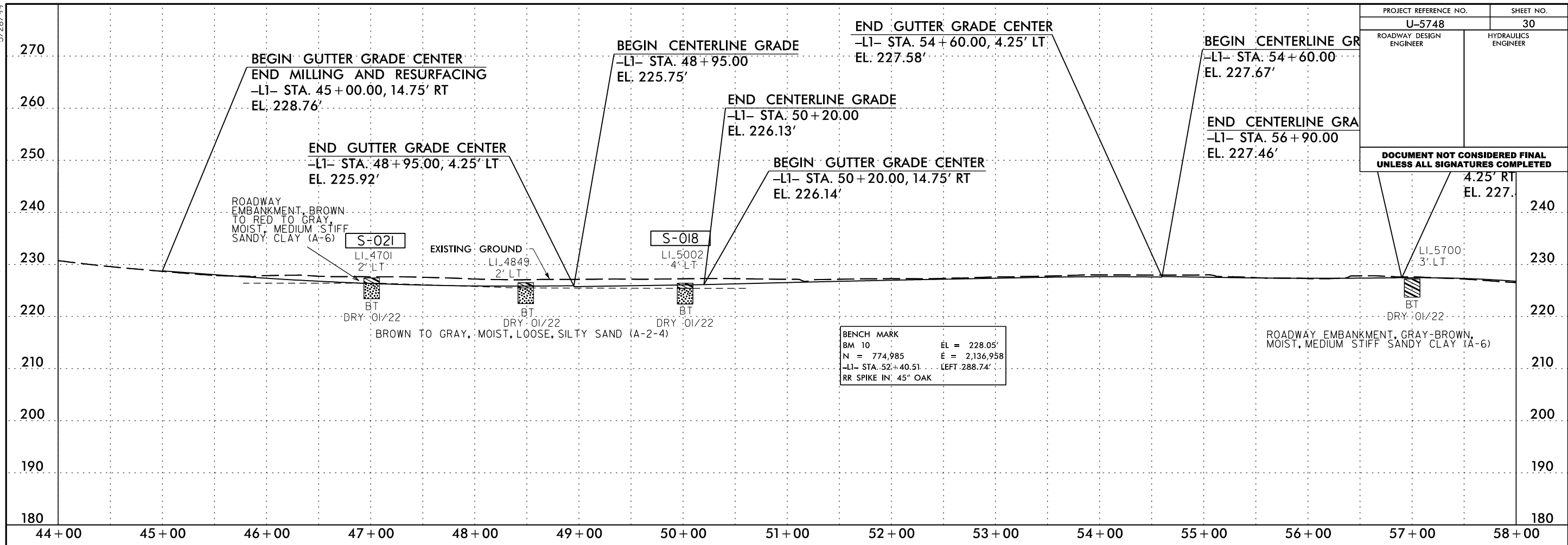
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 5/28/99

PROJECT REFERENCE NO. U-5748	SHEET NO. 29
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	



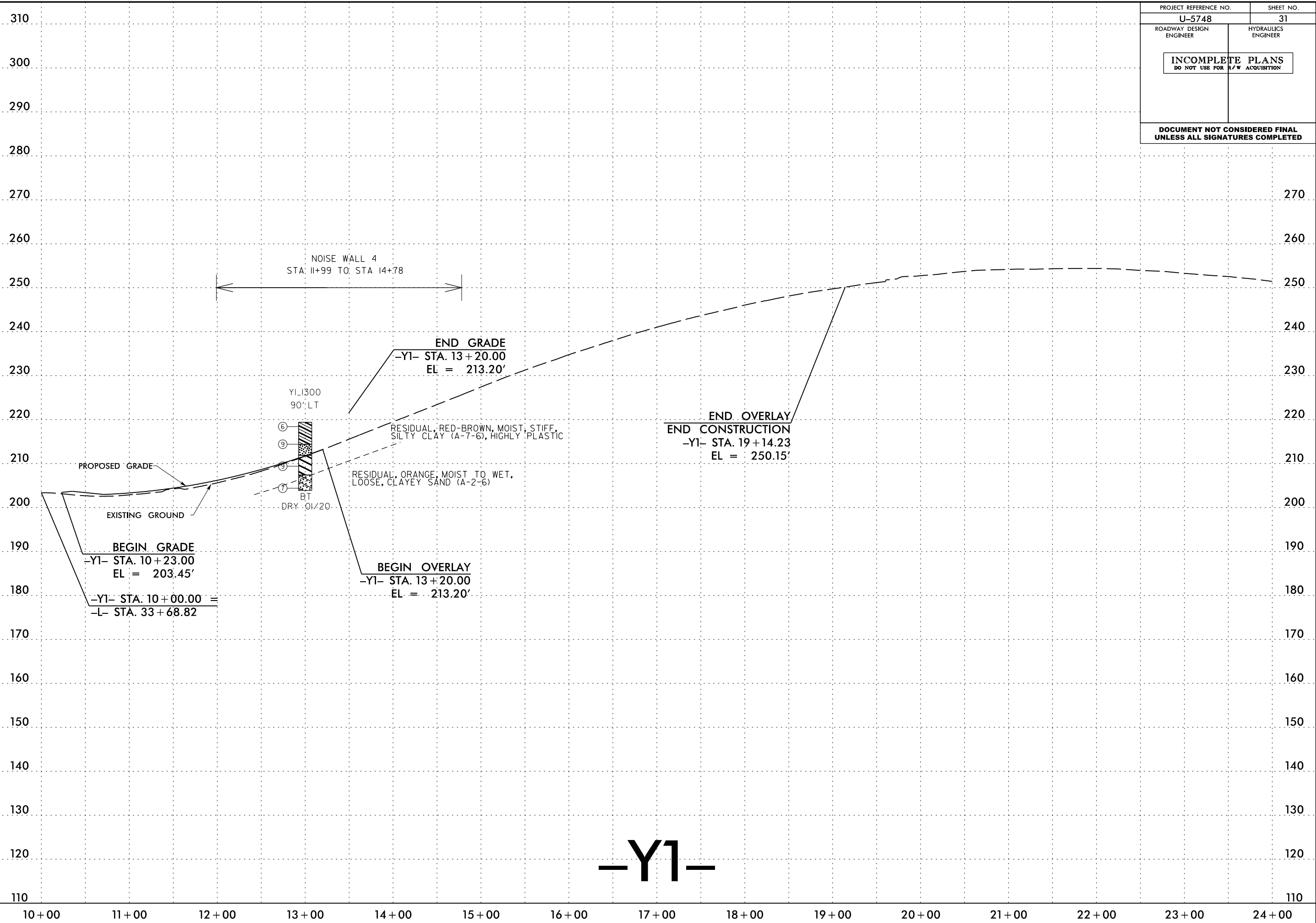
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PROJECT REFERENCE NO.	SHEET NO.
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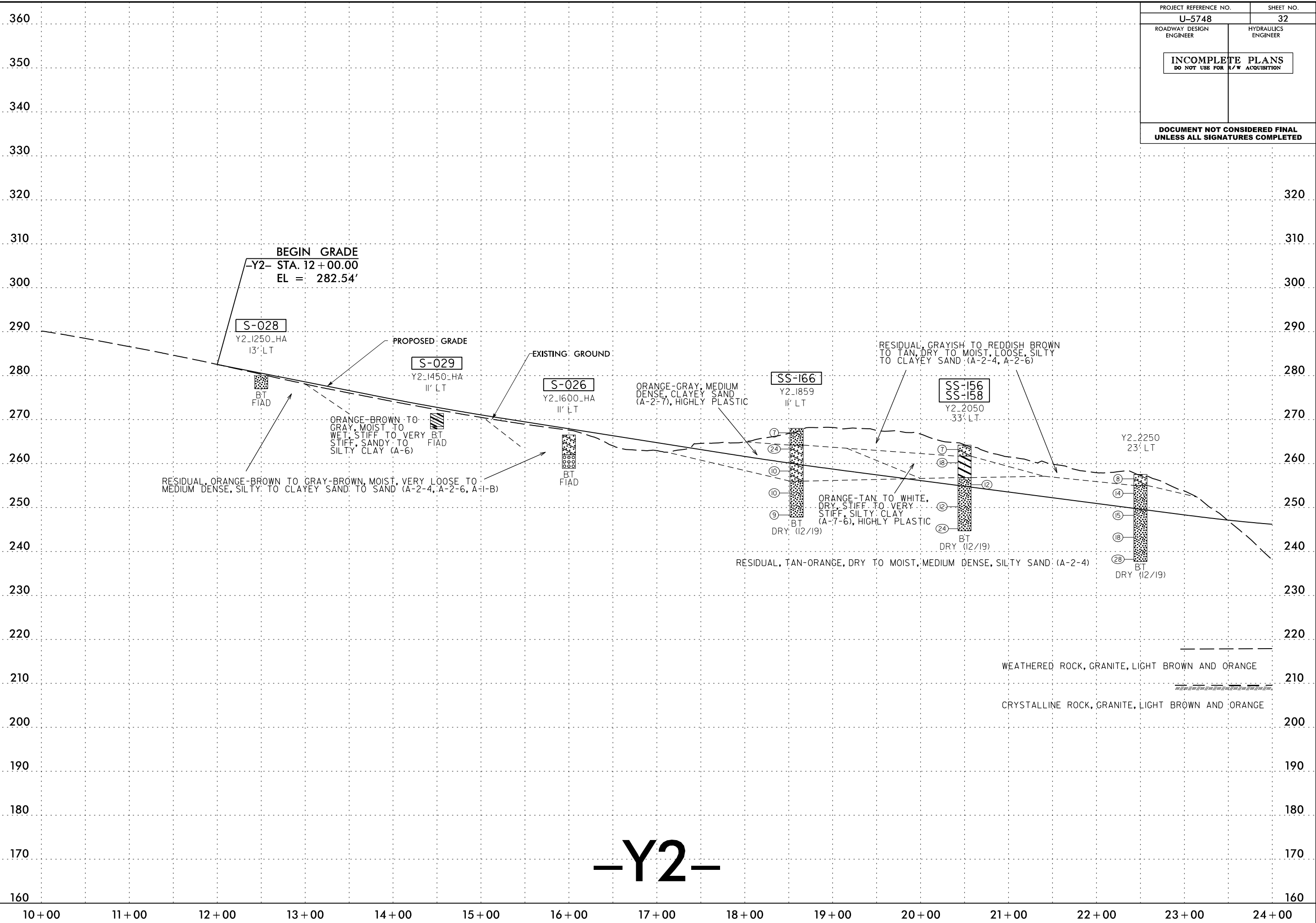
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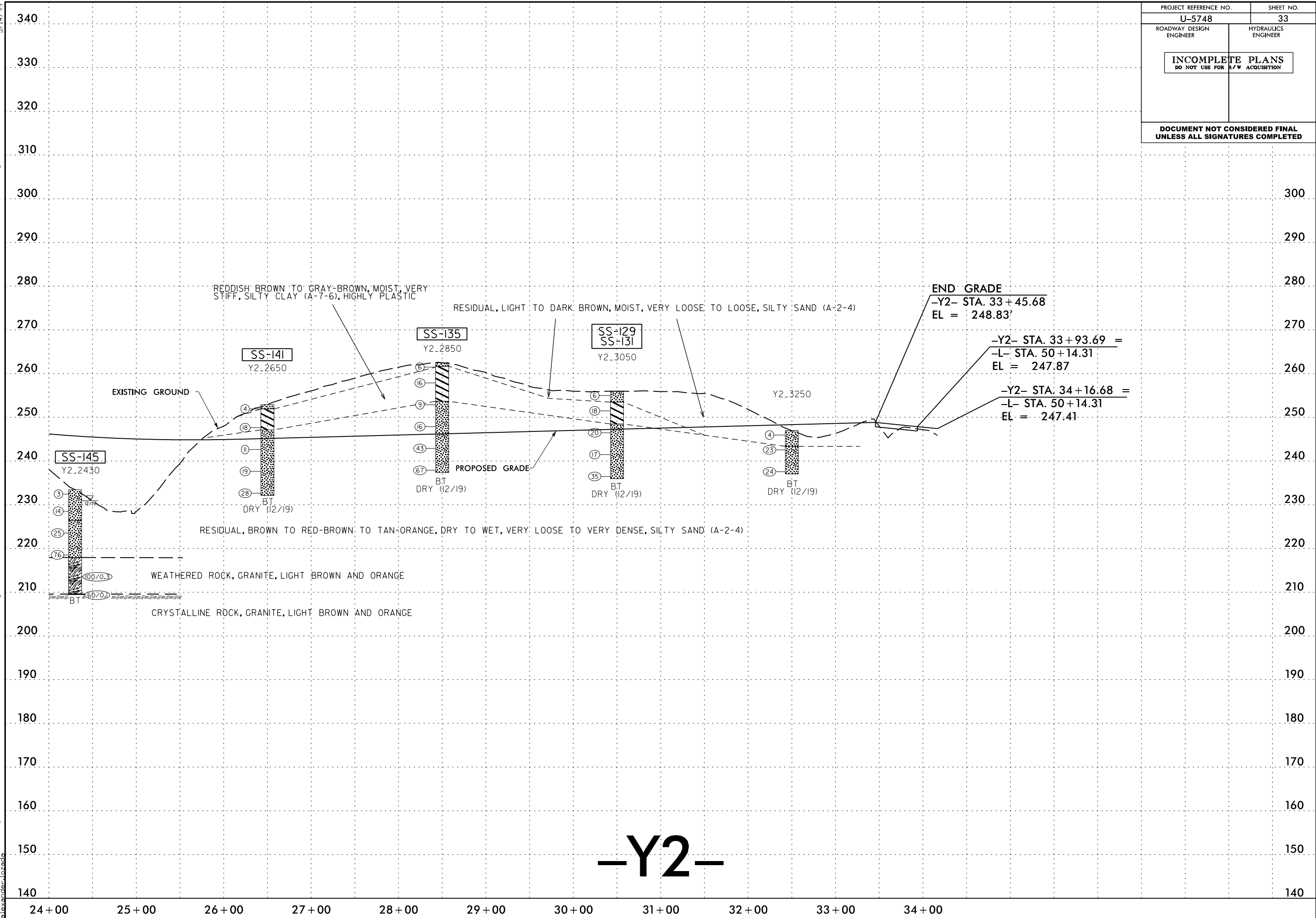


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 5/14/99



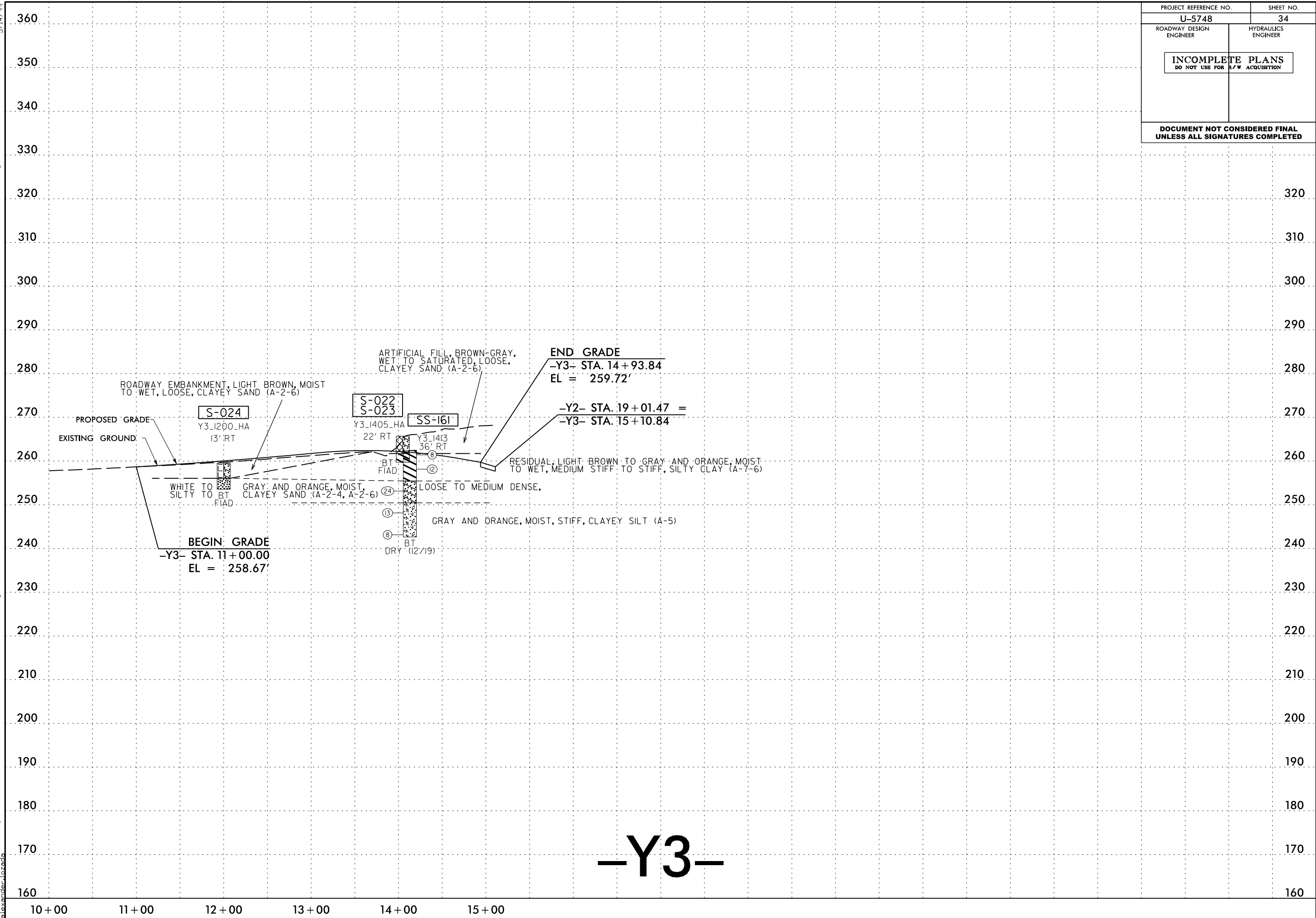
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PROJECT REFERENCE NO.	SHEET NO.
U-5748	34
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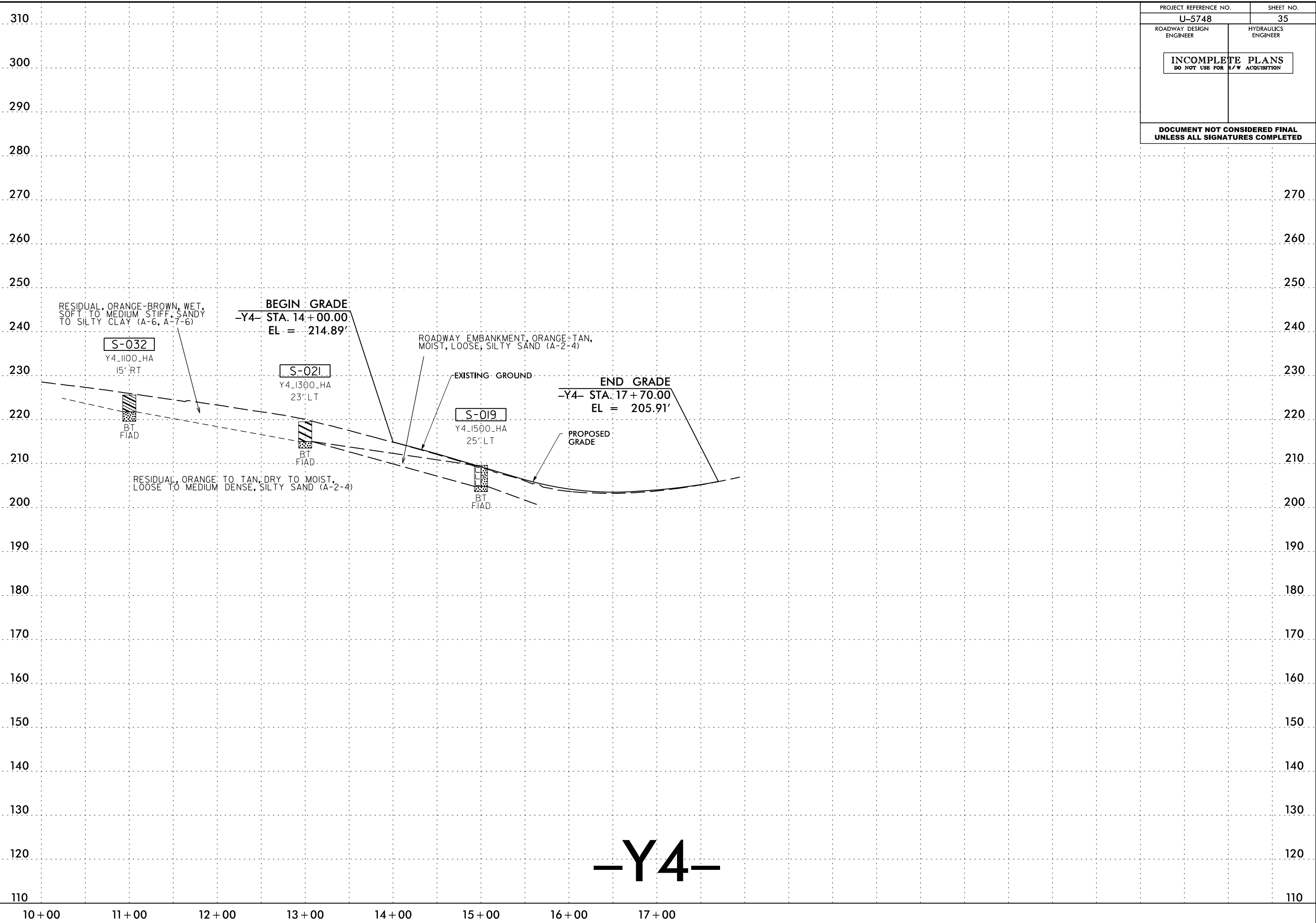


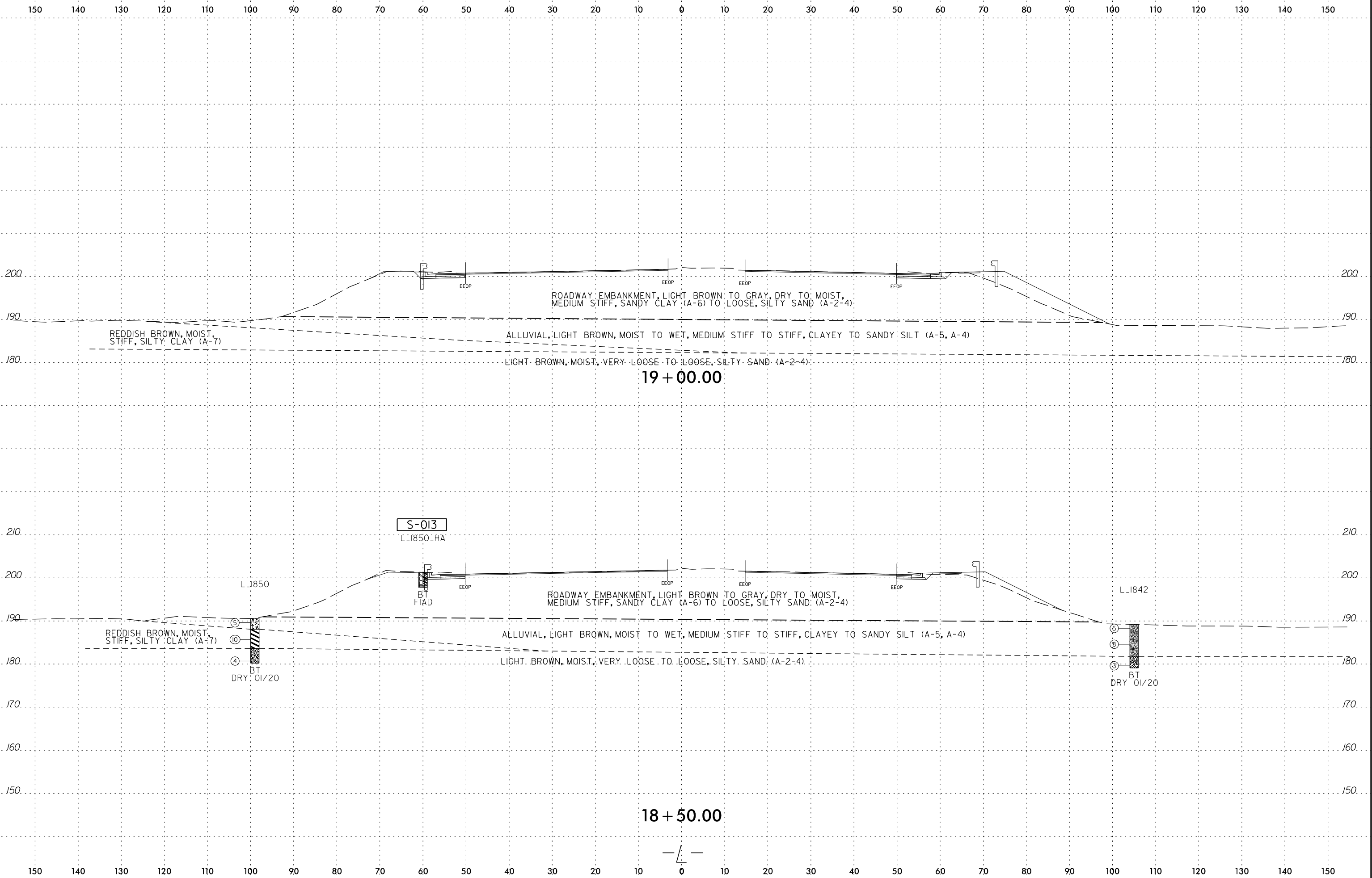
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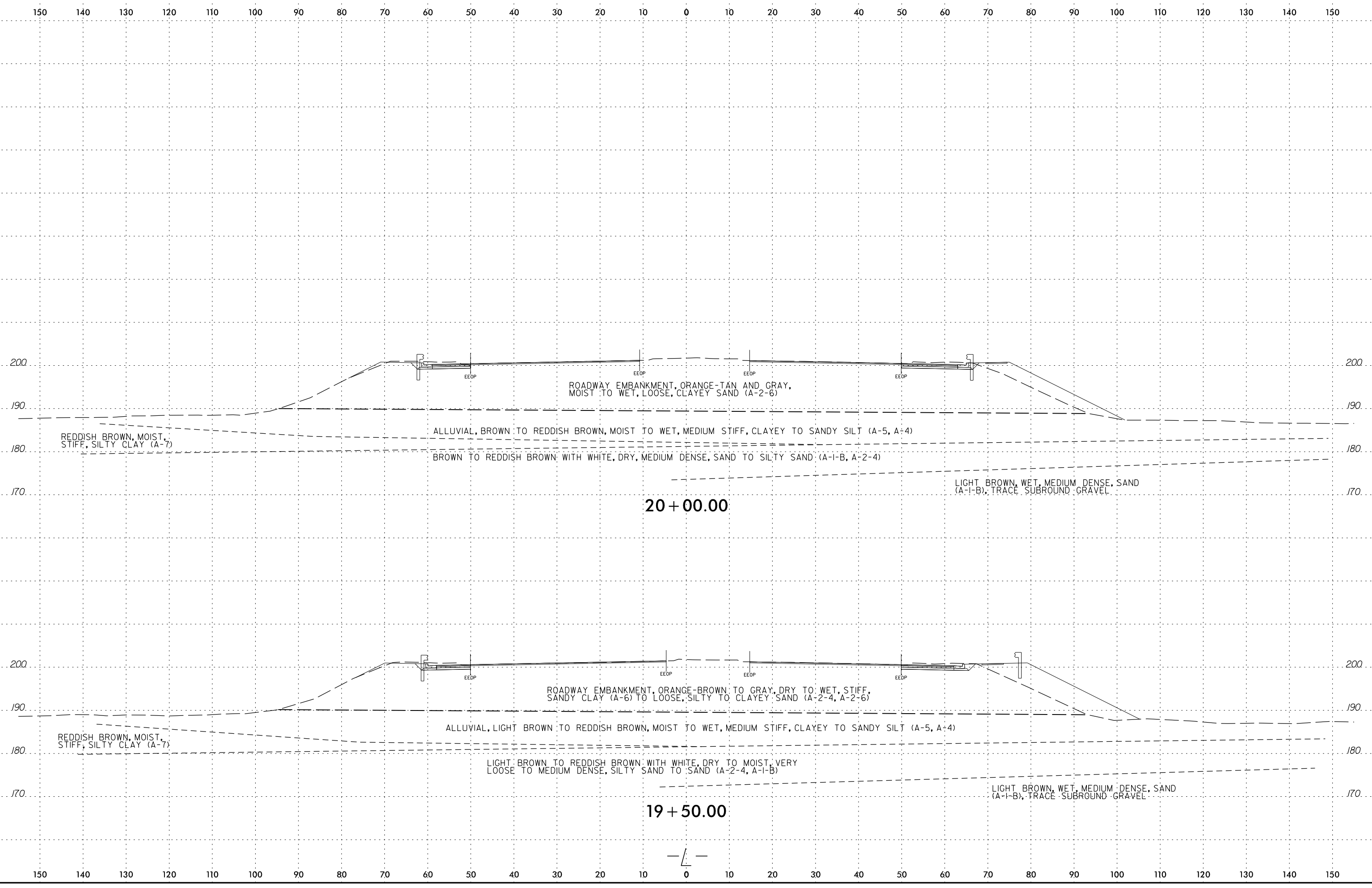
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U-5748	35
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	

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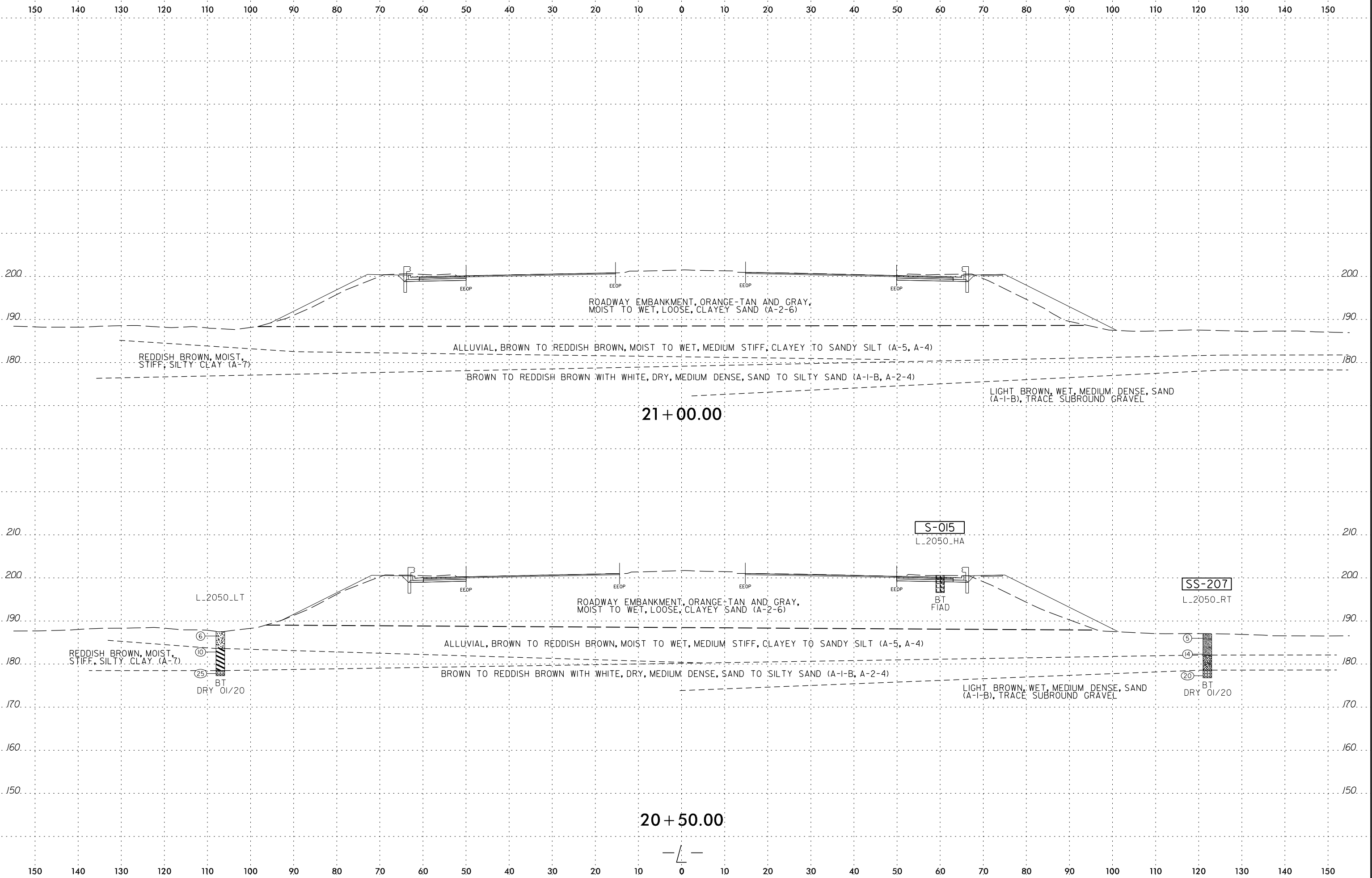




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 10/4/2022
 alexander.bozada

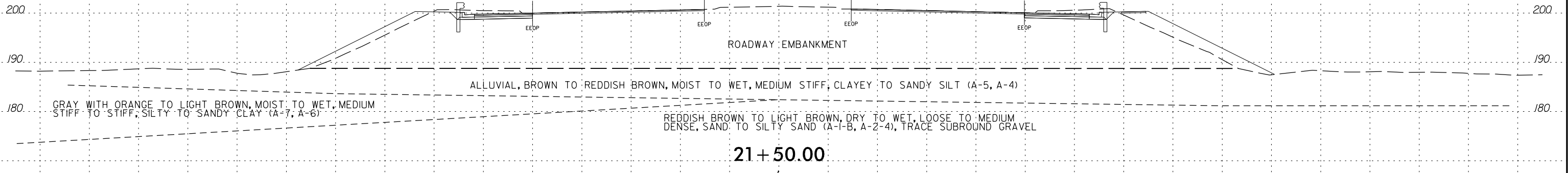
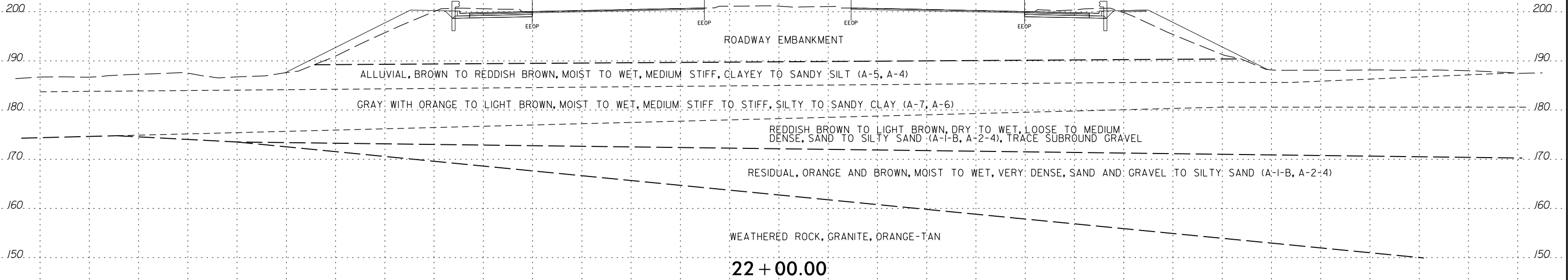


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 alexander.bozada



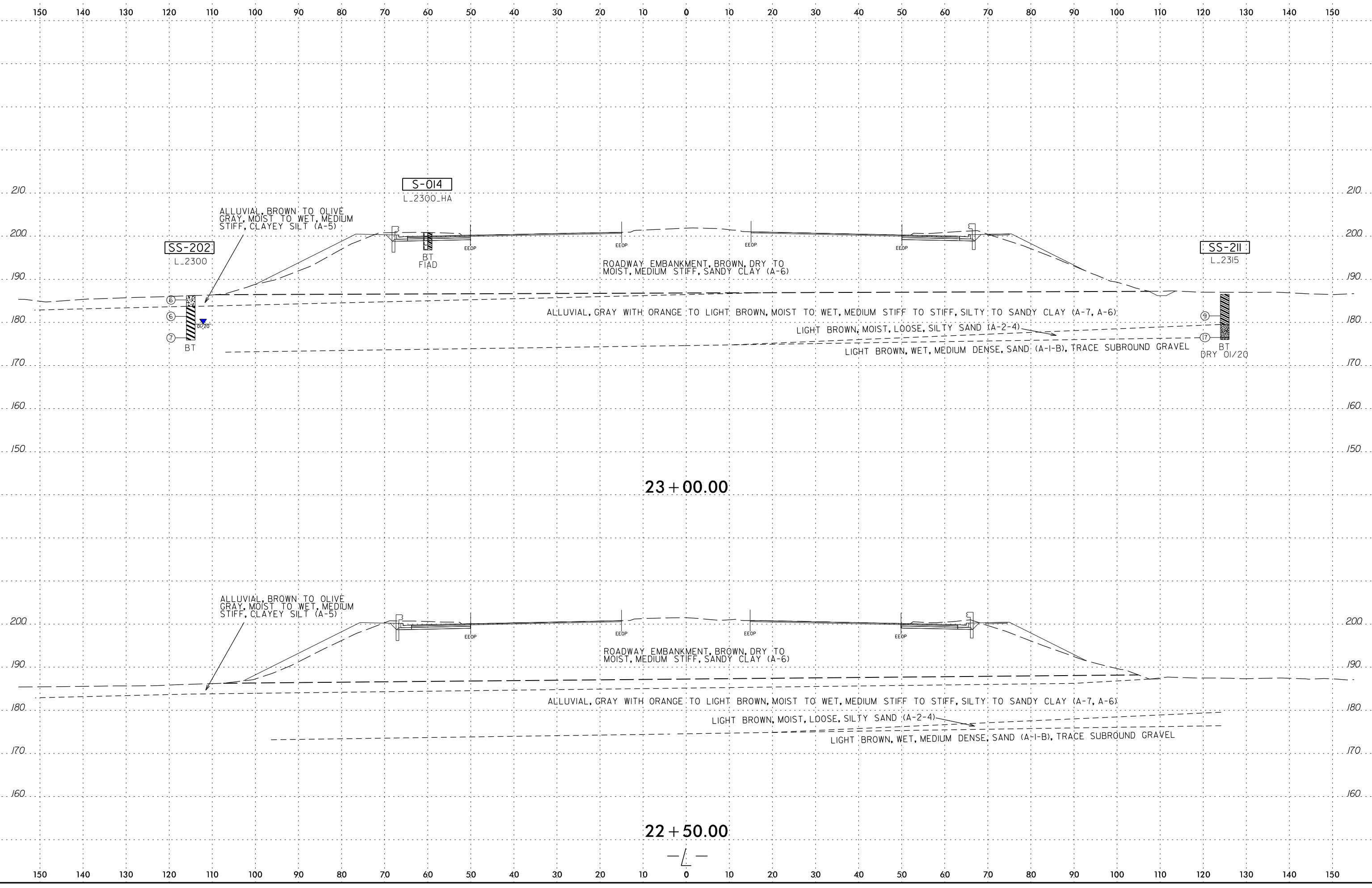
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 alexander.bozada

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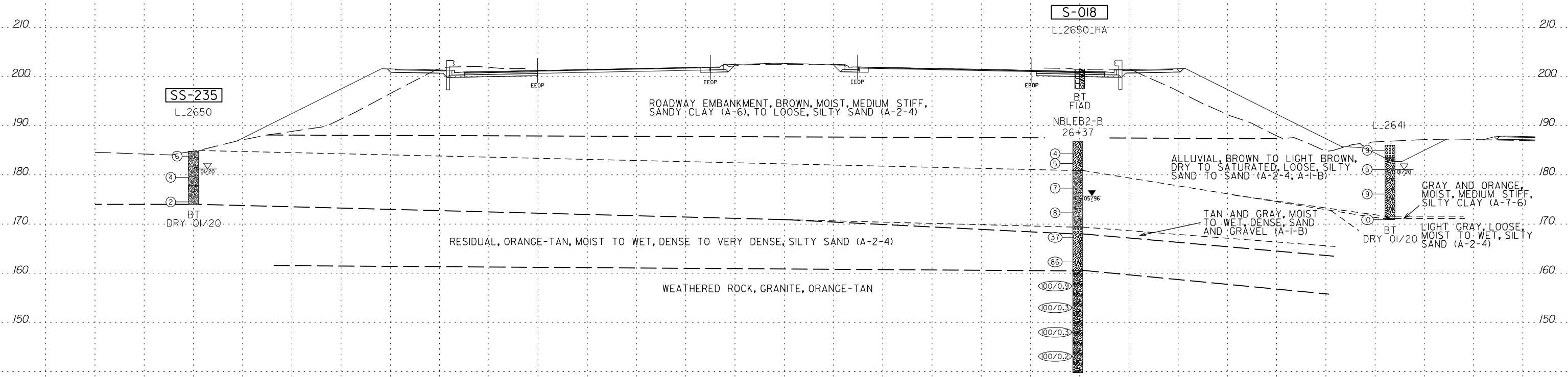
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 alexander.bozada



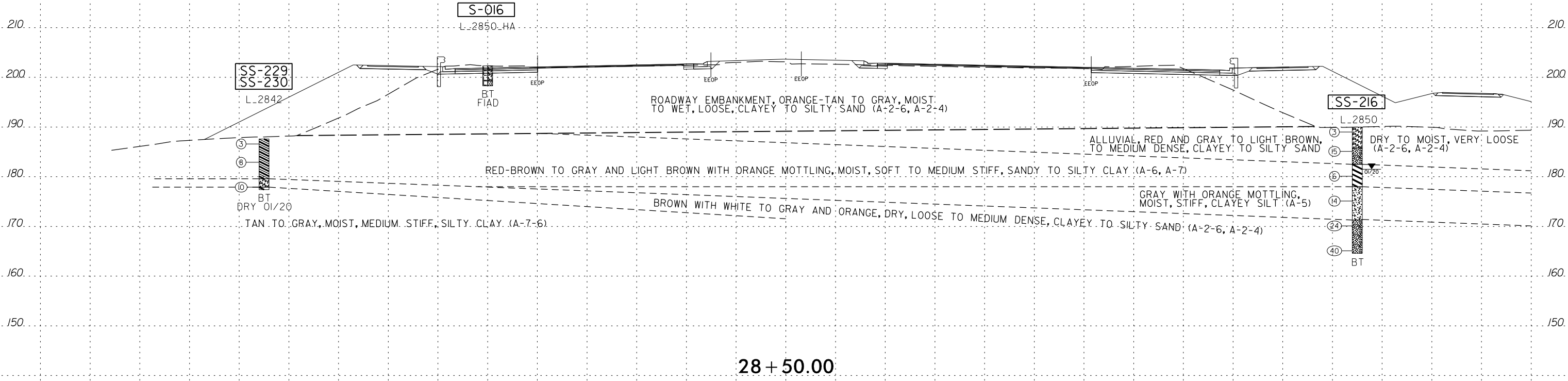
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 alexander.jozada

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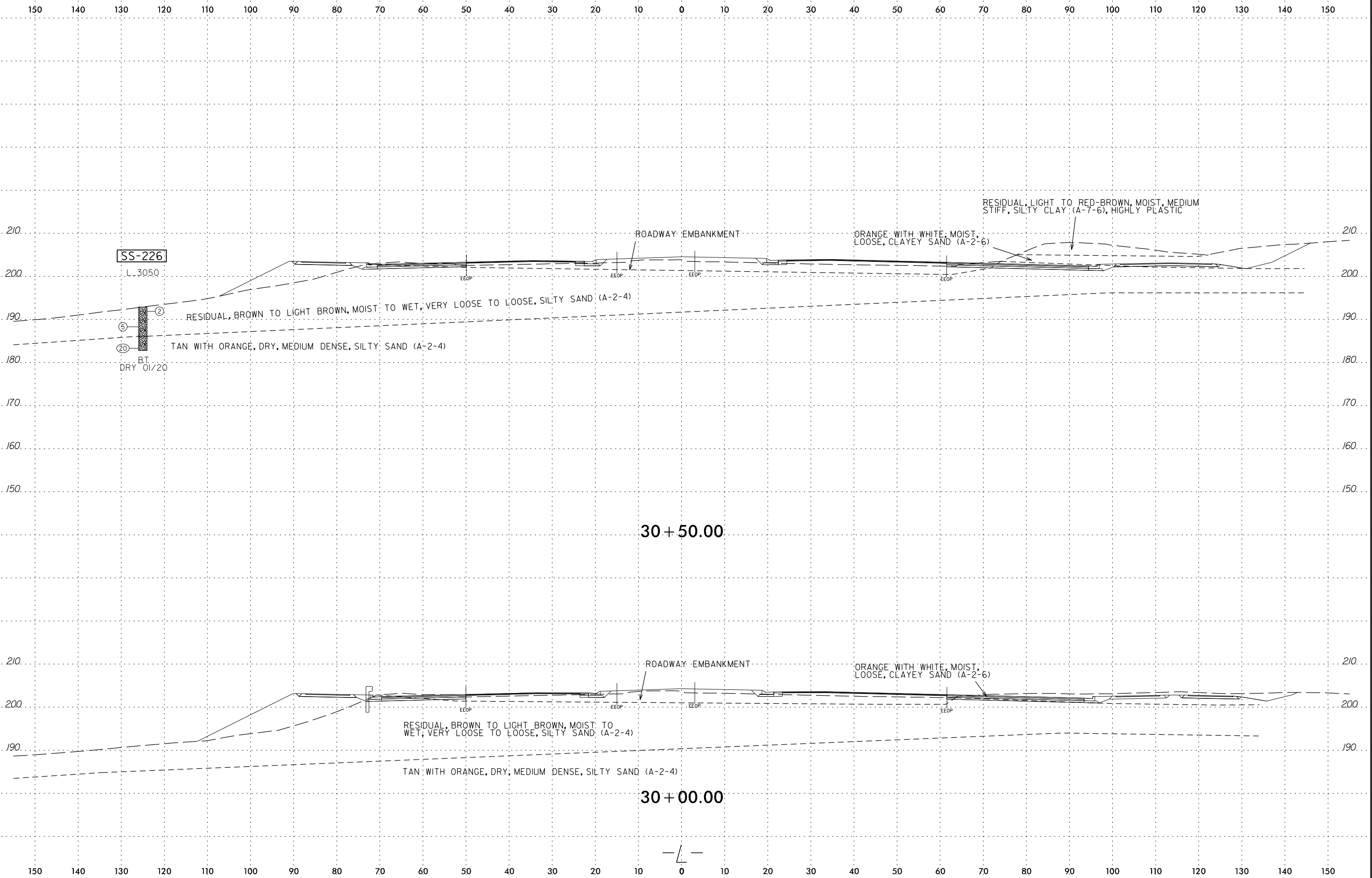
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 alexander.bozada

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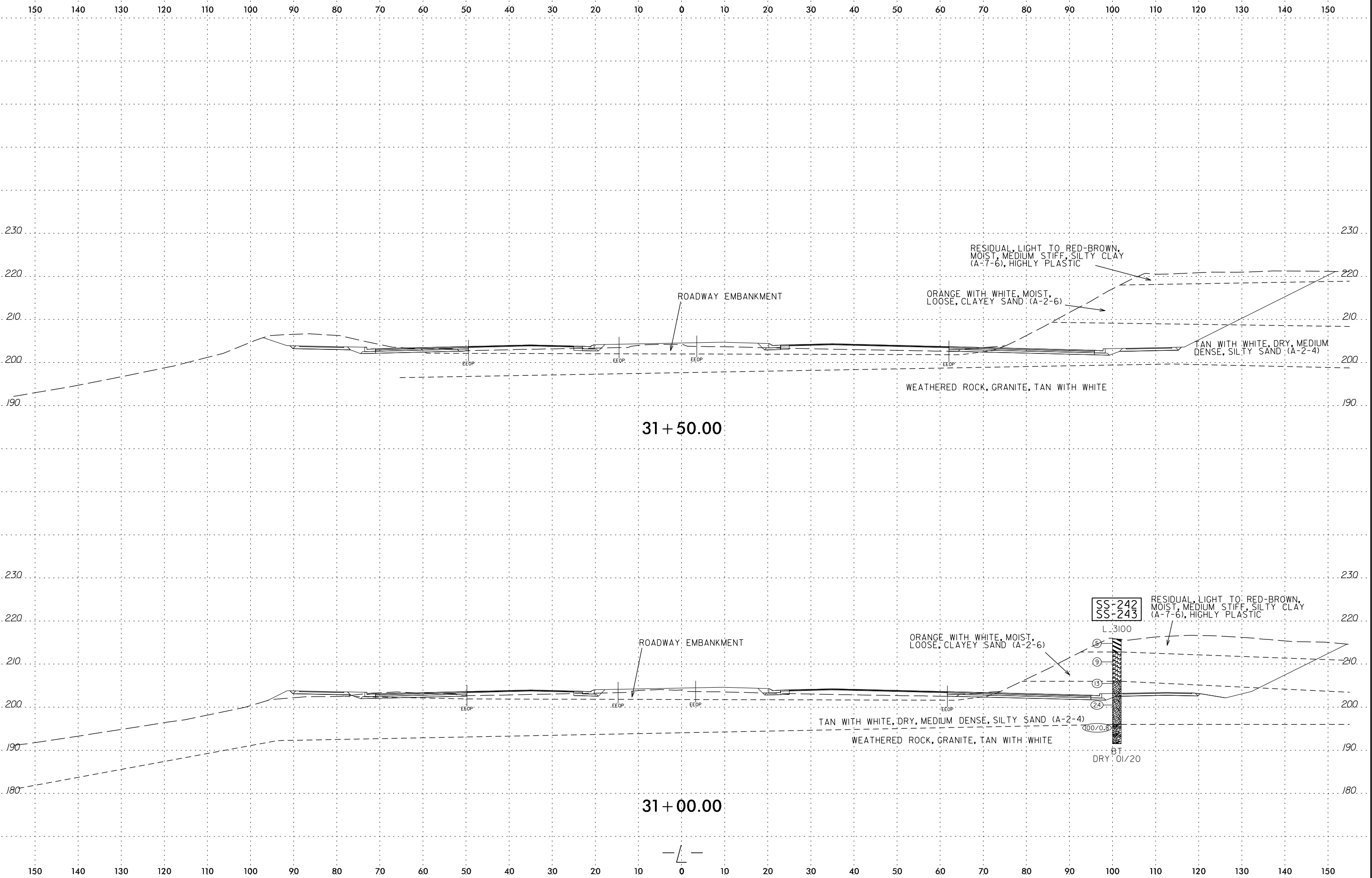


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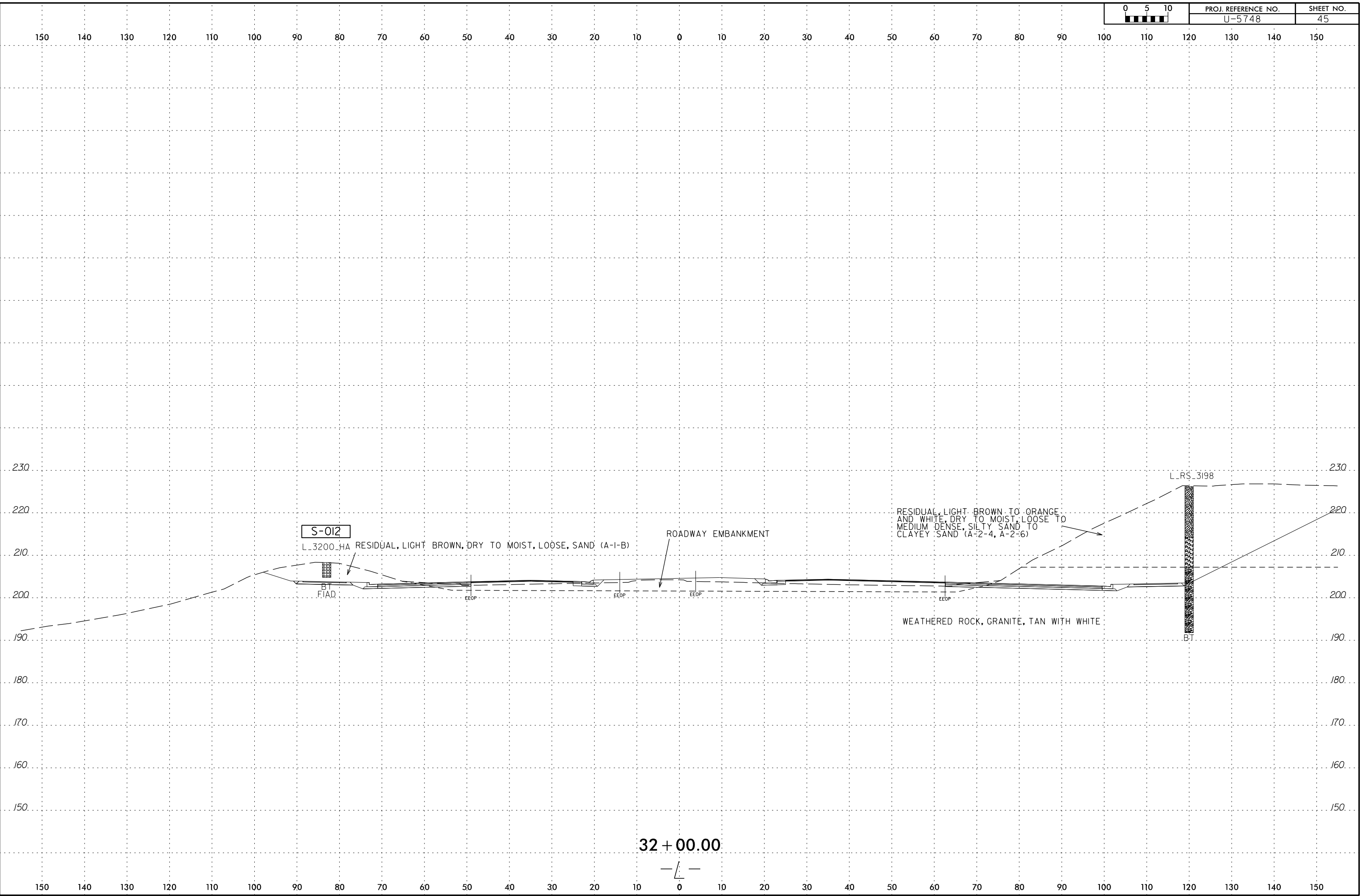
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10/4/2022
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alexander.bozada



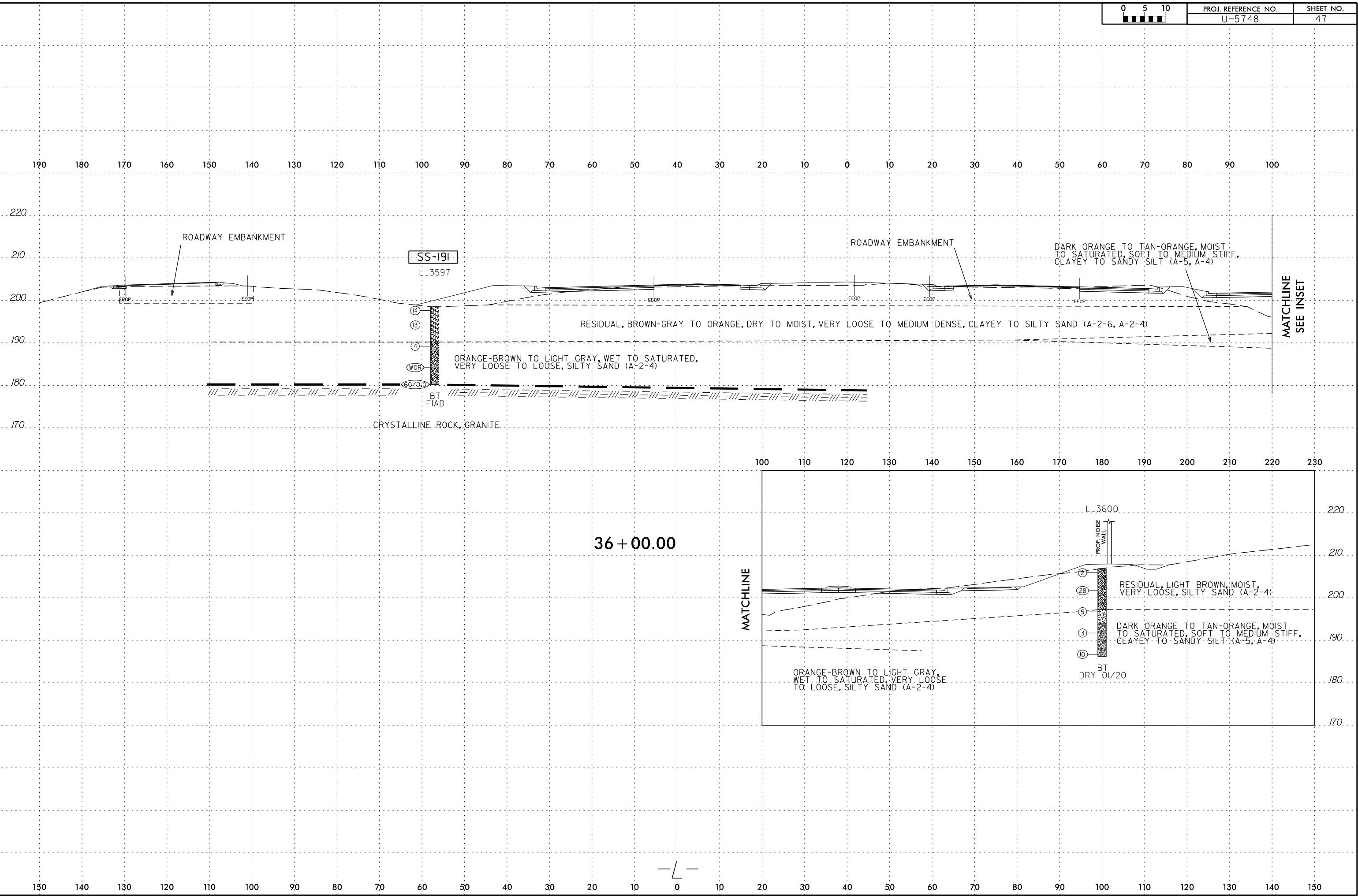
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 alexander.bozada



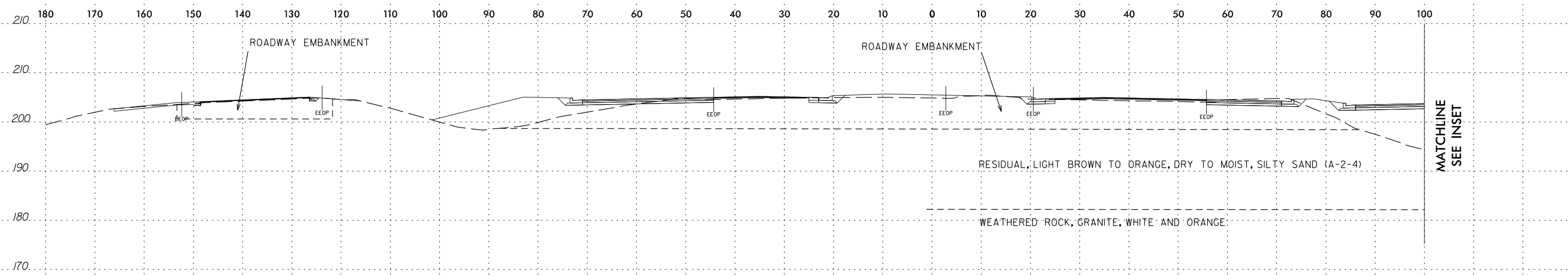
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1/2"

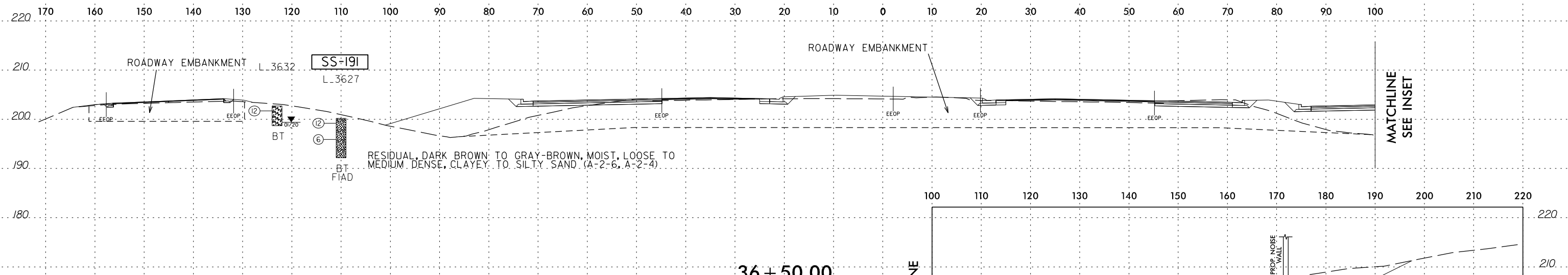
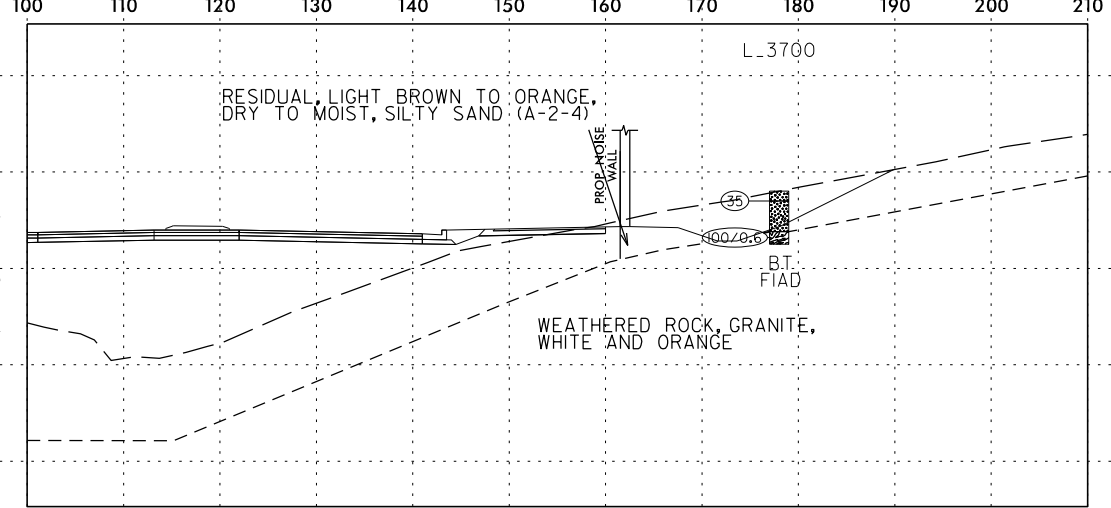
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alexander.bozada



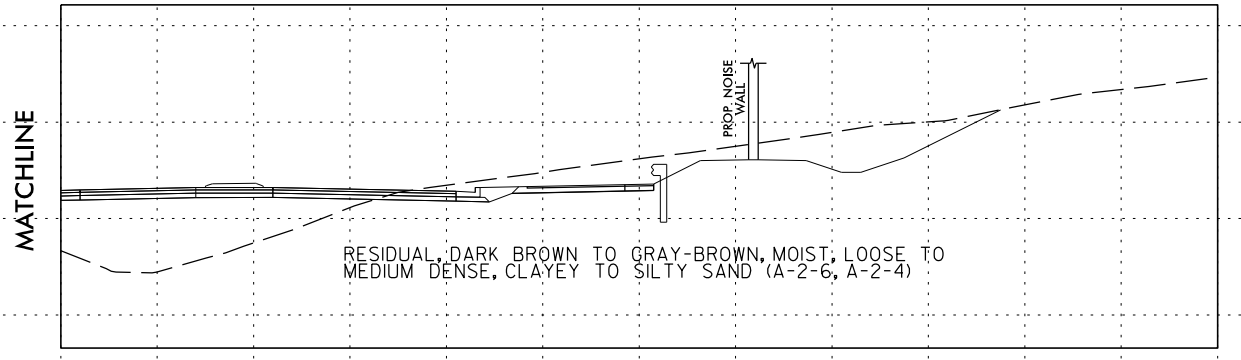
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 alexander.jozada



37 + 00.00

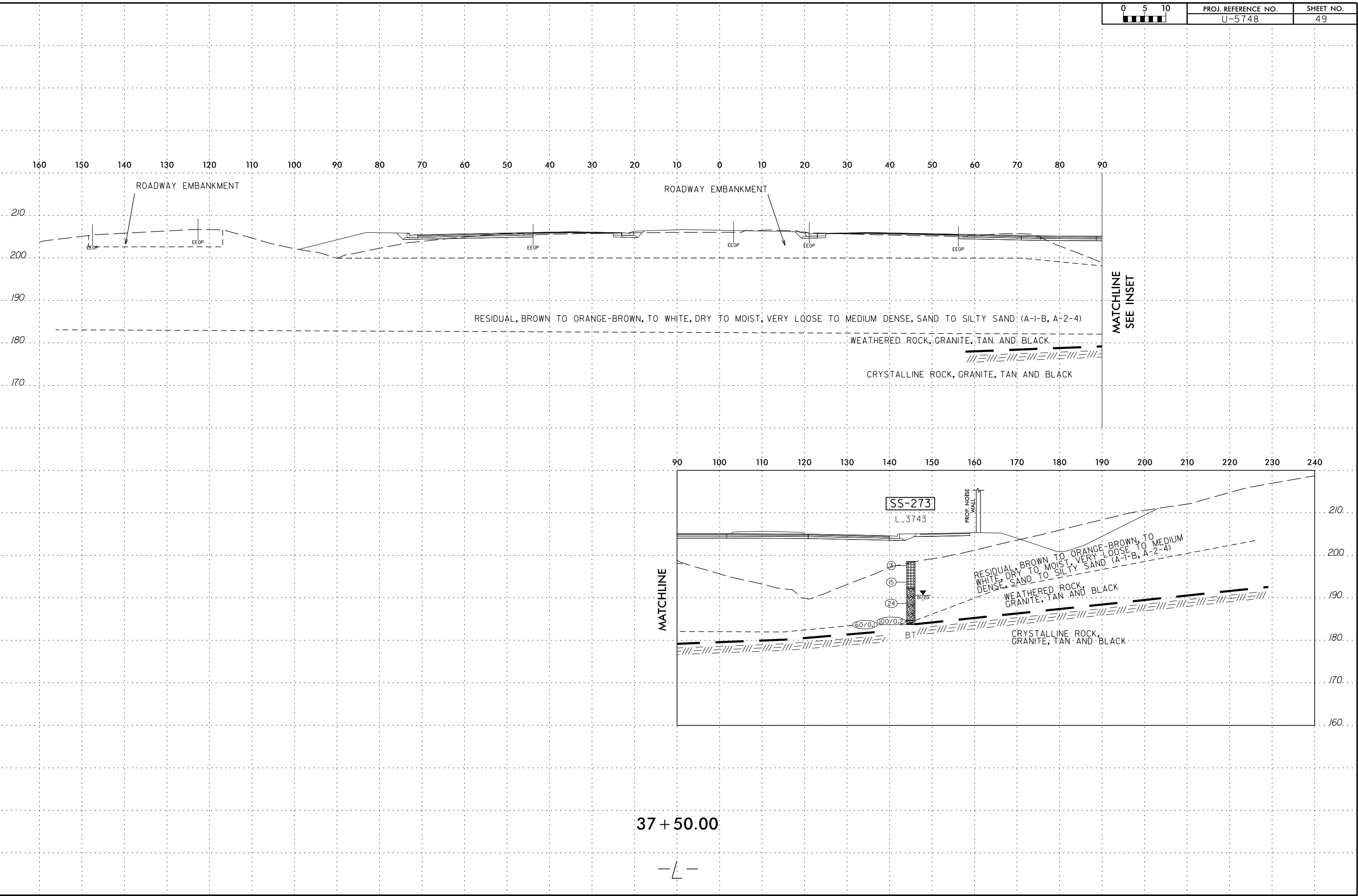


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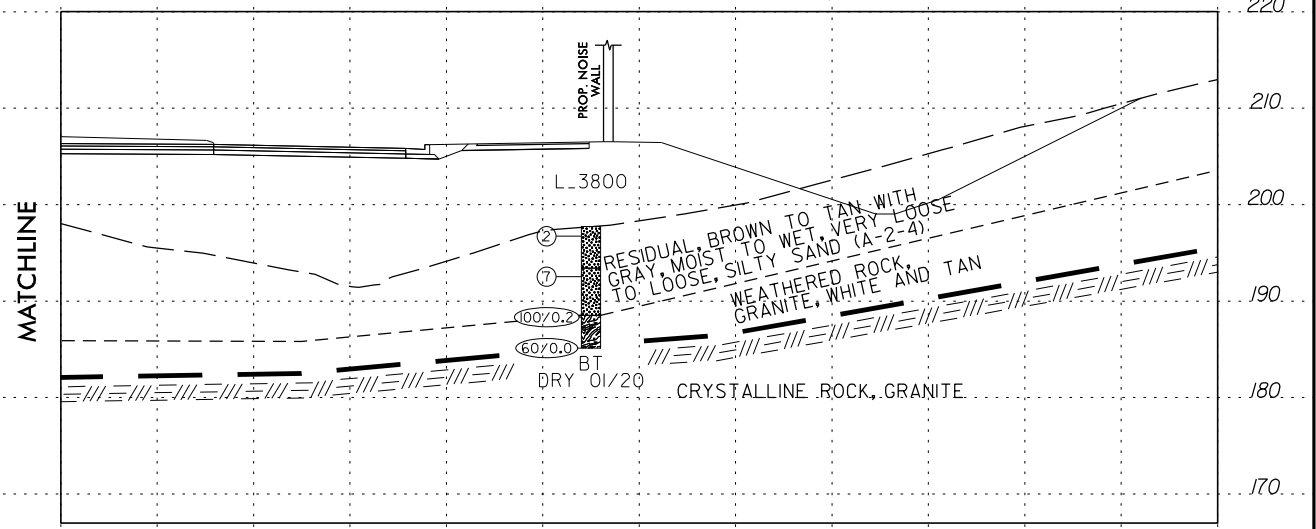
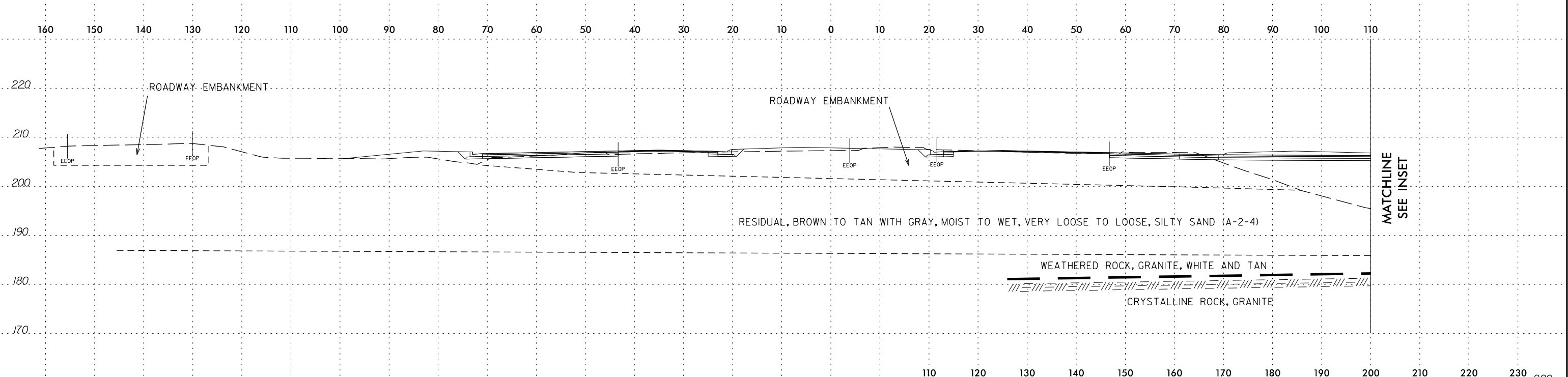
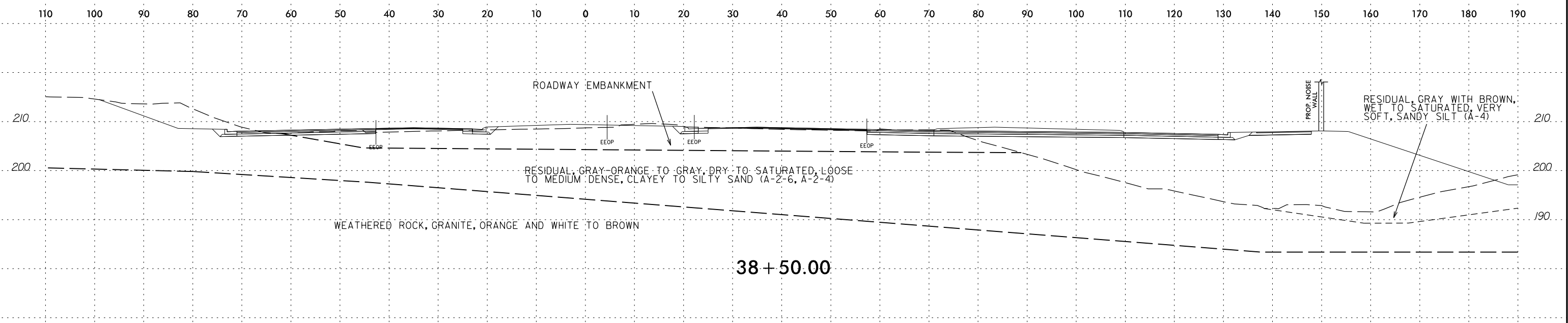
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37 + 50.00

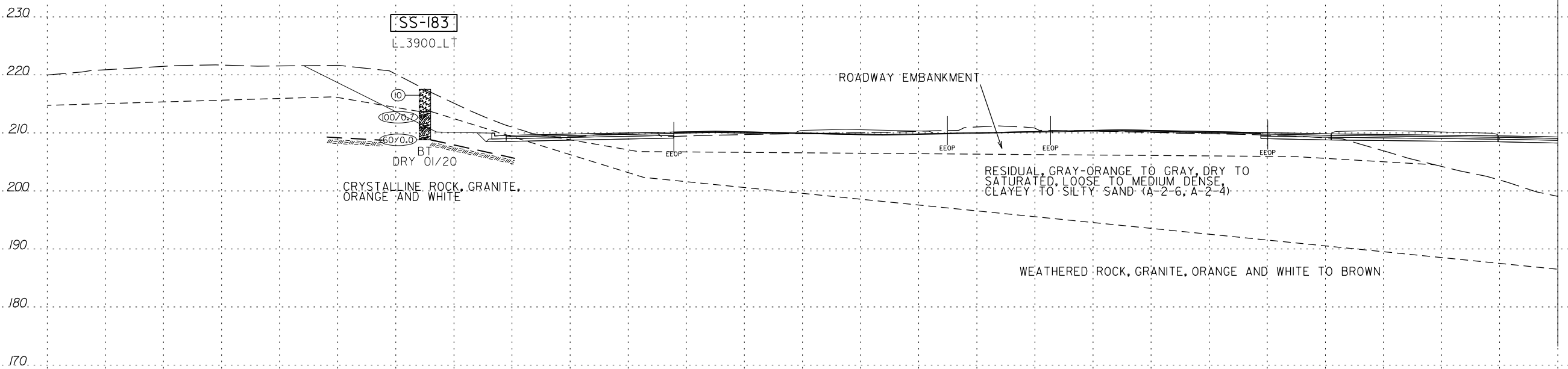
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6/23/16
10/4/2022
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alexander.bozada



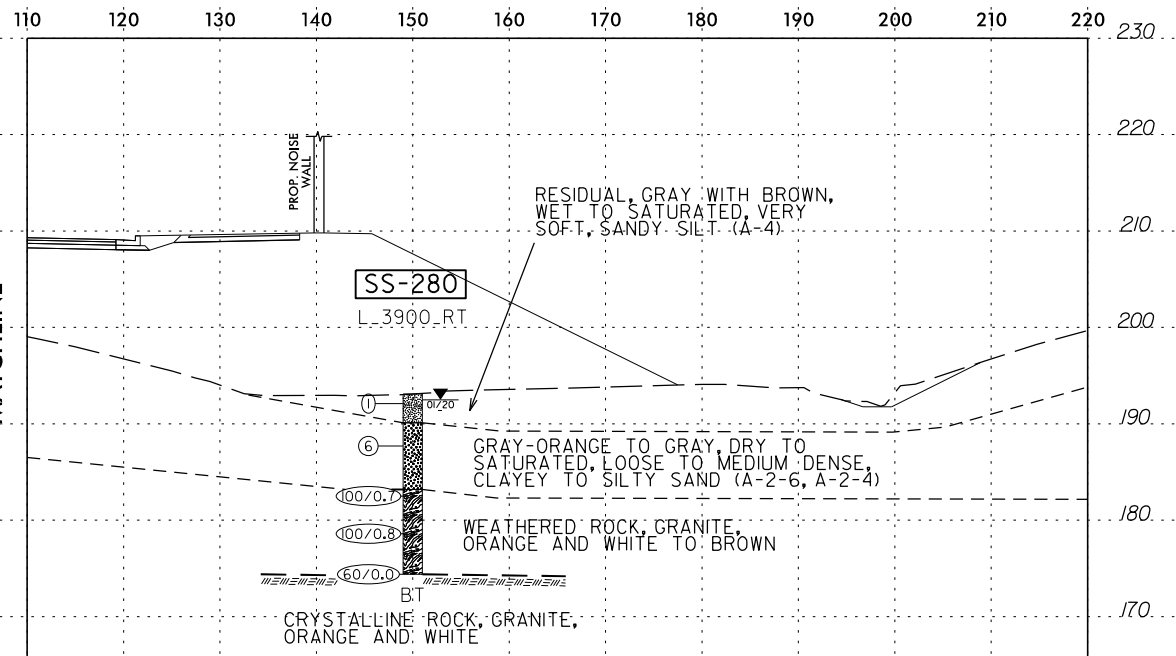
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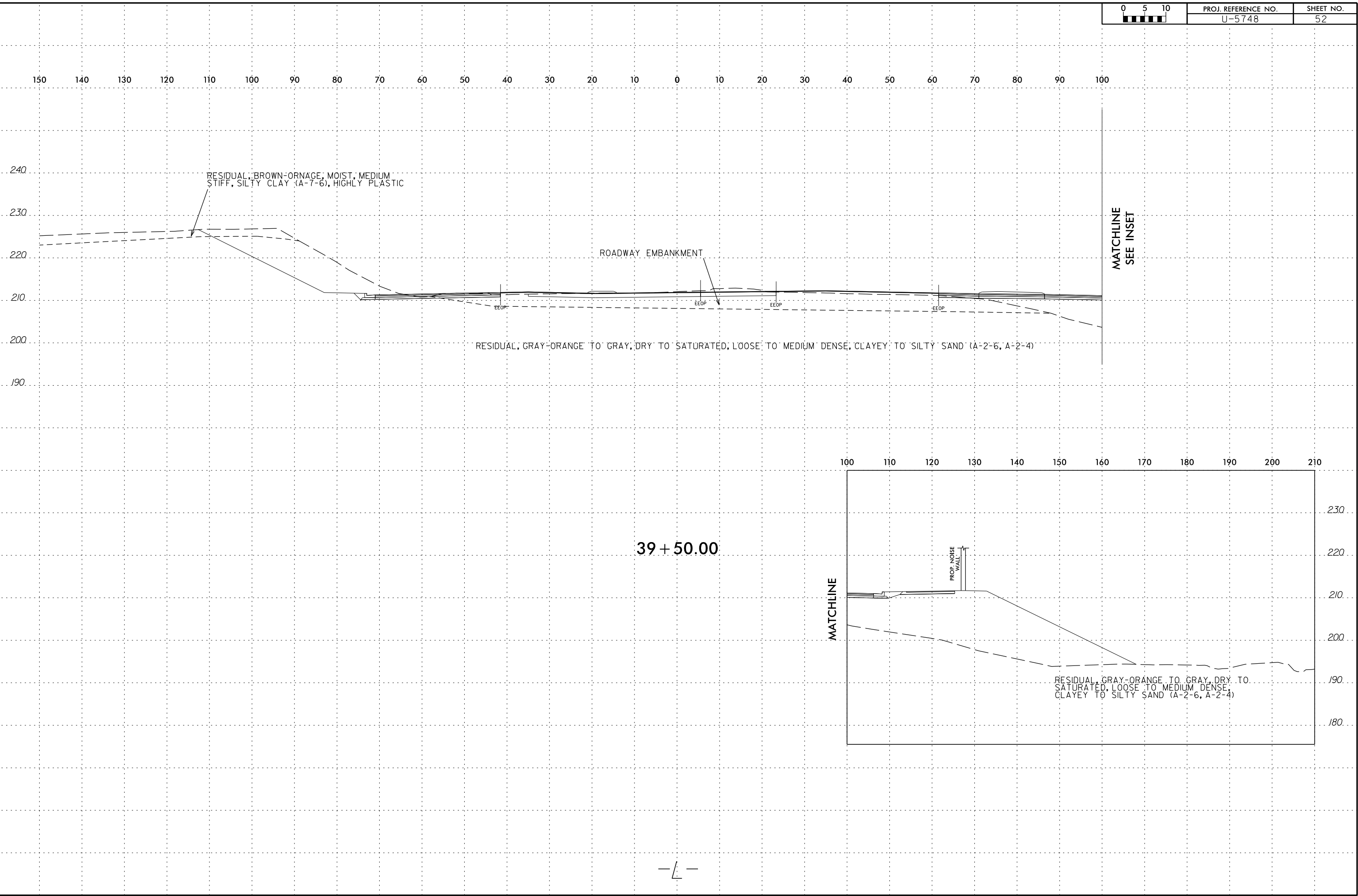
MATCHLINE
SEE INSET

39 + 00.00



MATCHLINE

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RESIDUAL, BROWN-ORANGE, MOIST, MEDIUM STIFF, SILTY CLAY (A-7-6), HIGHLY PLASTIC

ROADWAY EMBANKMENT

RESIDUAL, GRAY-ORANGE TO GRAY, DRY TO SATURATED, LOOSE TO MEDIUM DENSE, CLAYEY TO SILTY SAND (A-2-6, A-2-4)

MATCHLINE
SEE INSET

39 + 50.00

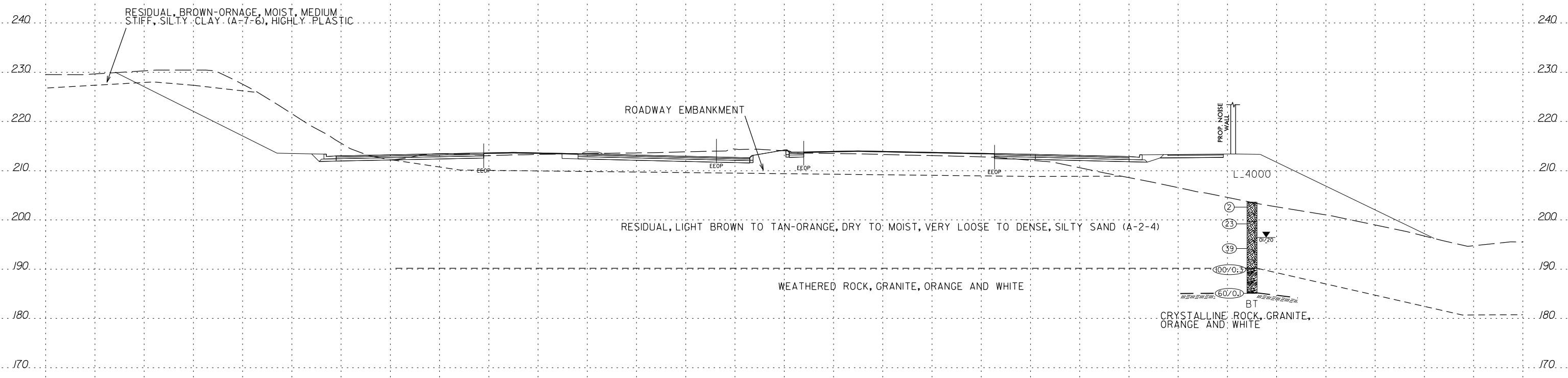
MATCHLINE

PROP. NOISE WALL

RESIDUAL, GRAY-ORANGE TO GRAY, DRY TO SATURATED, LOOSE TO MEDIUM DENSE, CLAYEY TO SILTY SAND (A-2-6, A-2-4)

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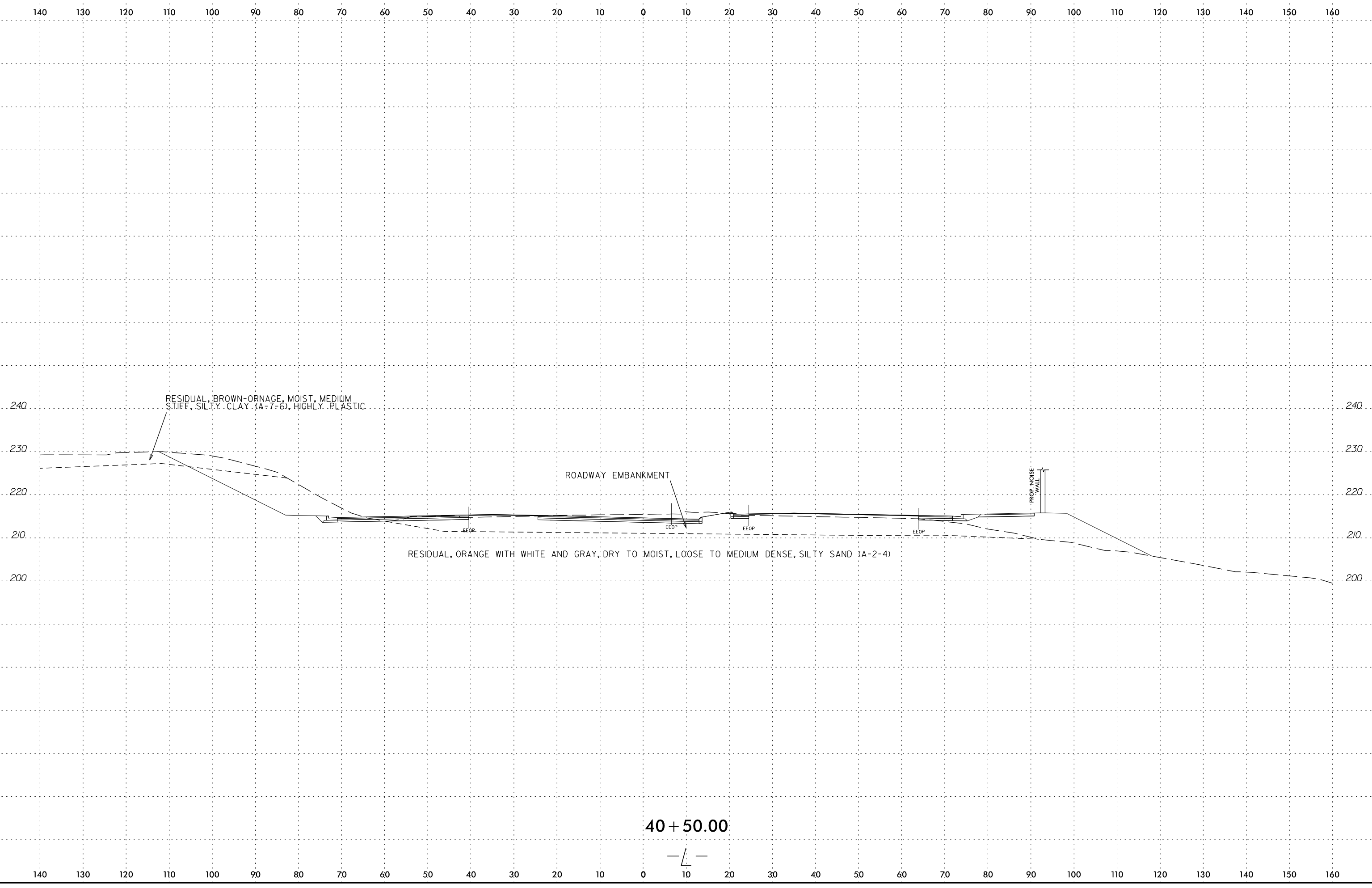
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 10/2/2022
 alexander.bozada



40 + 00.00

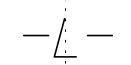
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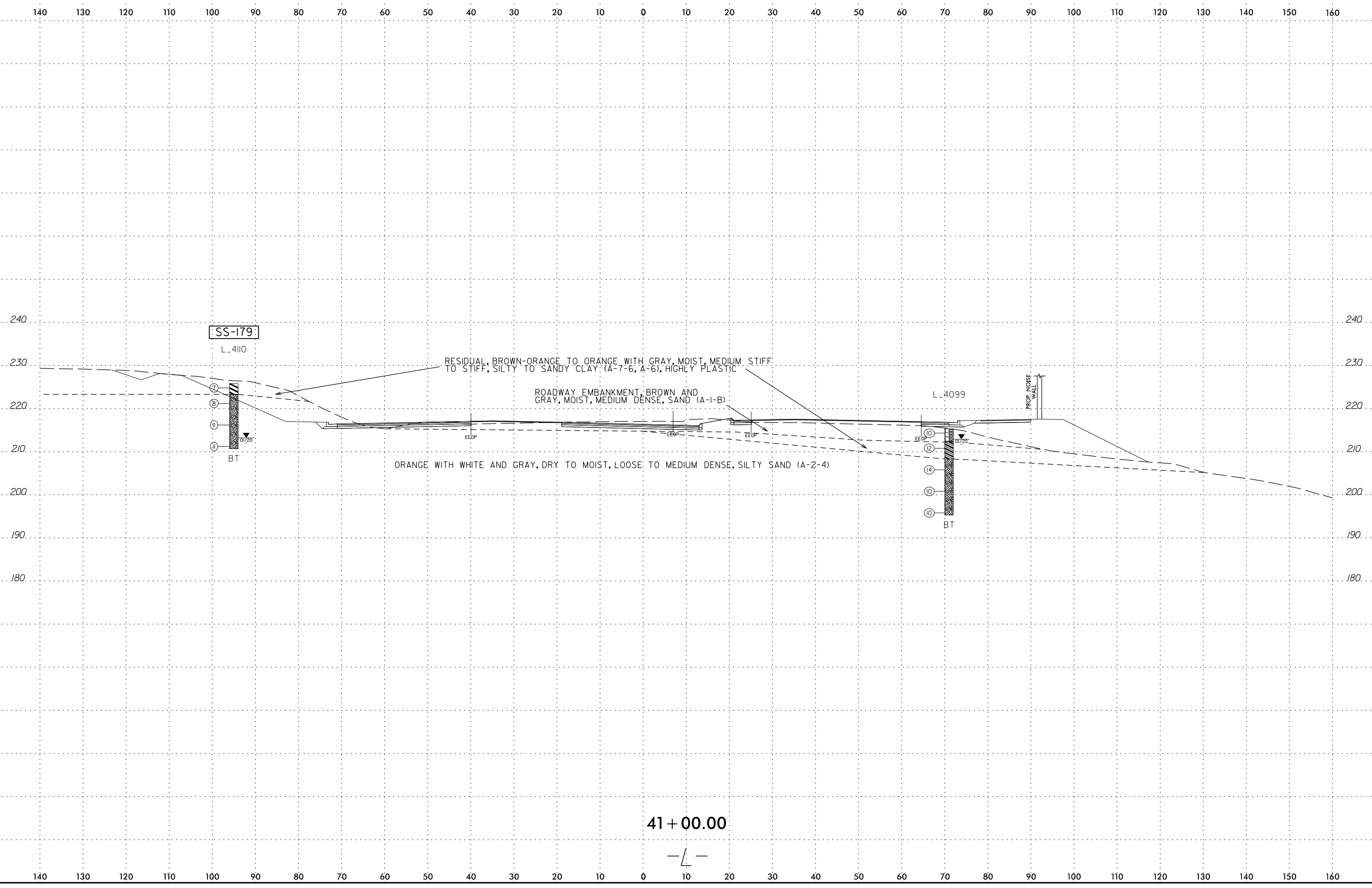
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 alexander.jozada

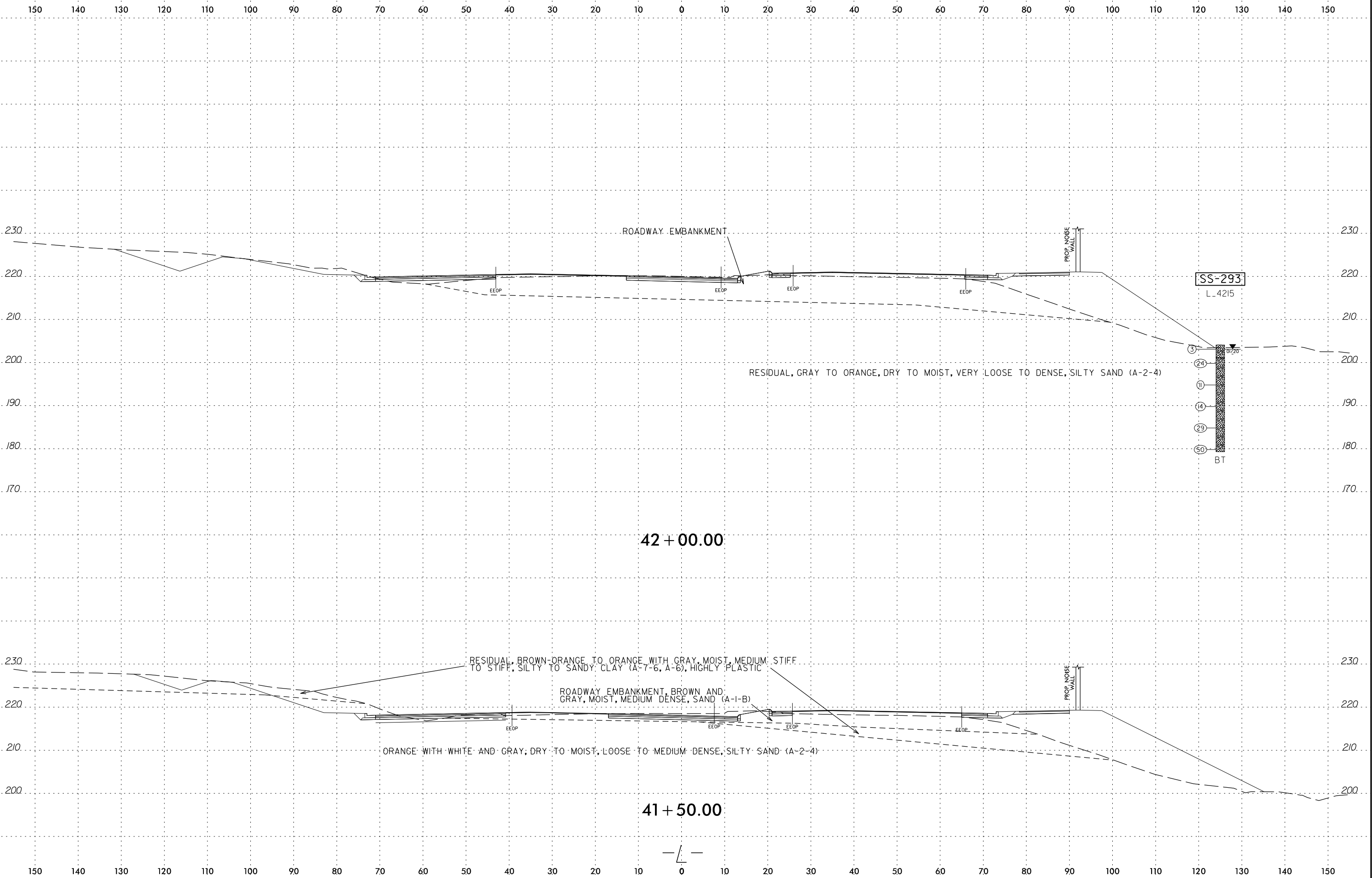
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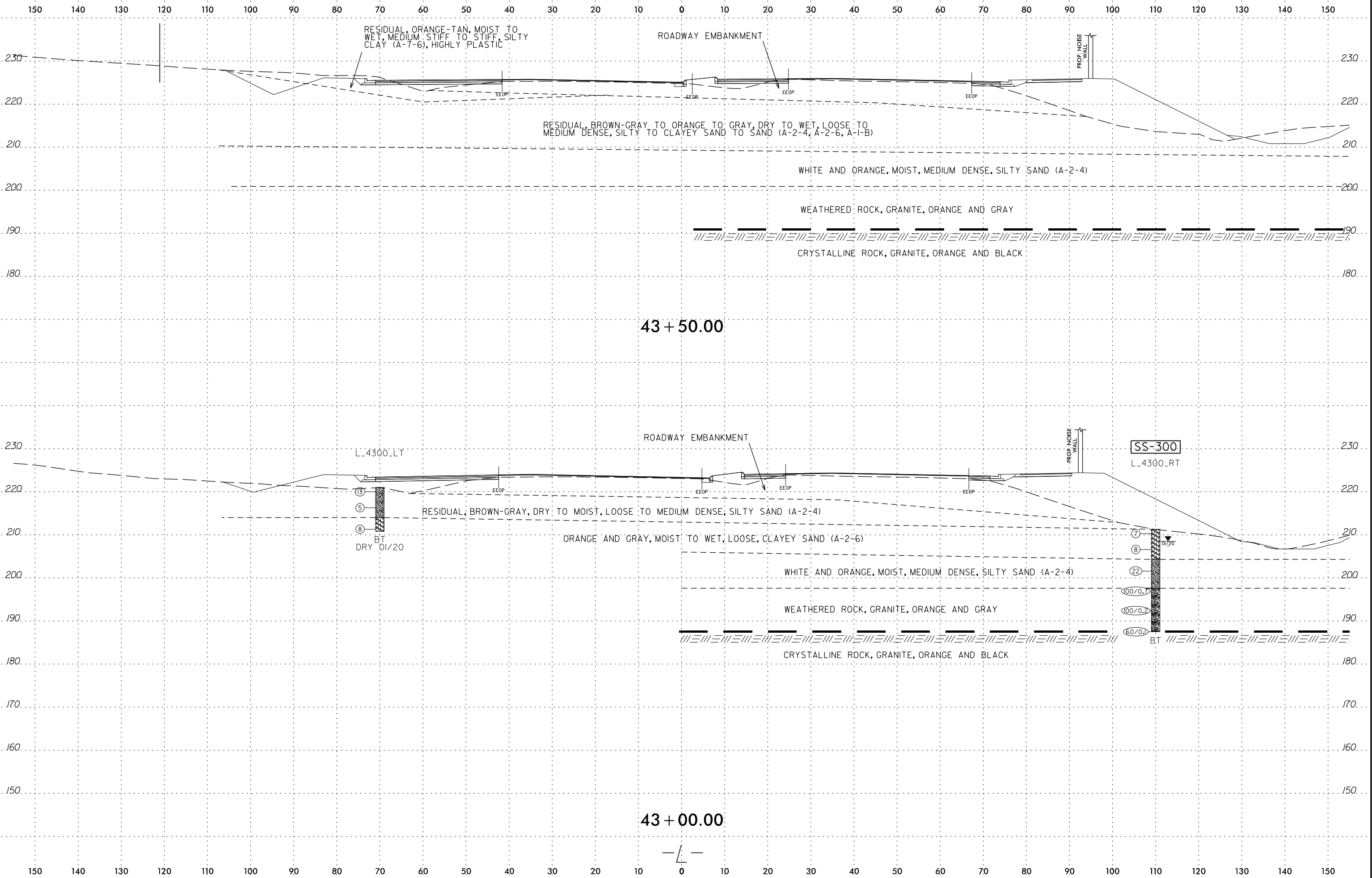




41 + 00.00

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 alexander.bozada





RESIDUAL, ORANGE-TAN, MOIST TO WET, MEDIUM STIFF TO STIFF, SILTY CLAY (A-7-6), HIGHLY PLASTIC

ROADWAY EMBANKMENT

PROP. NOISE WALL

RESIDUAL, BROWN-GRAY TO ORANGE TO GRAY, DRY TO WET, LOOSE TO MEDIUM DENSE, SILTY TO CLAYEY SAND TO SAND (A-2-4, A-2-6, A-1-B)

WHITE AND ORANGE, MOIST, MEDIUM DENSE, SILTY SAND (A-2-4)

WEATHERED ROCK, GRANITE, ORANGE AND GRAY

CRYSTALLINE ROCK, GRANITE, ORANGE AND BLACK

43 + 50.00

L_4300_LT

ROADWAY EMBANKMENT

PROP. NOISE WALL

SS-300

L_4300_RT

BT
DRY 01/20

RESIDUAL, BROWN-GRAY, DRY TO MOIST, LOOSE TO MEDIUM DENSE, SILTY SAND (A-2-4)

ORANGE AND GRAY, MOIST TO WET, LOOSE, CLAYEY SAND (A-2-6)

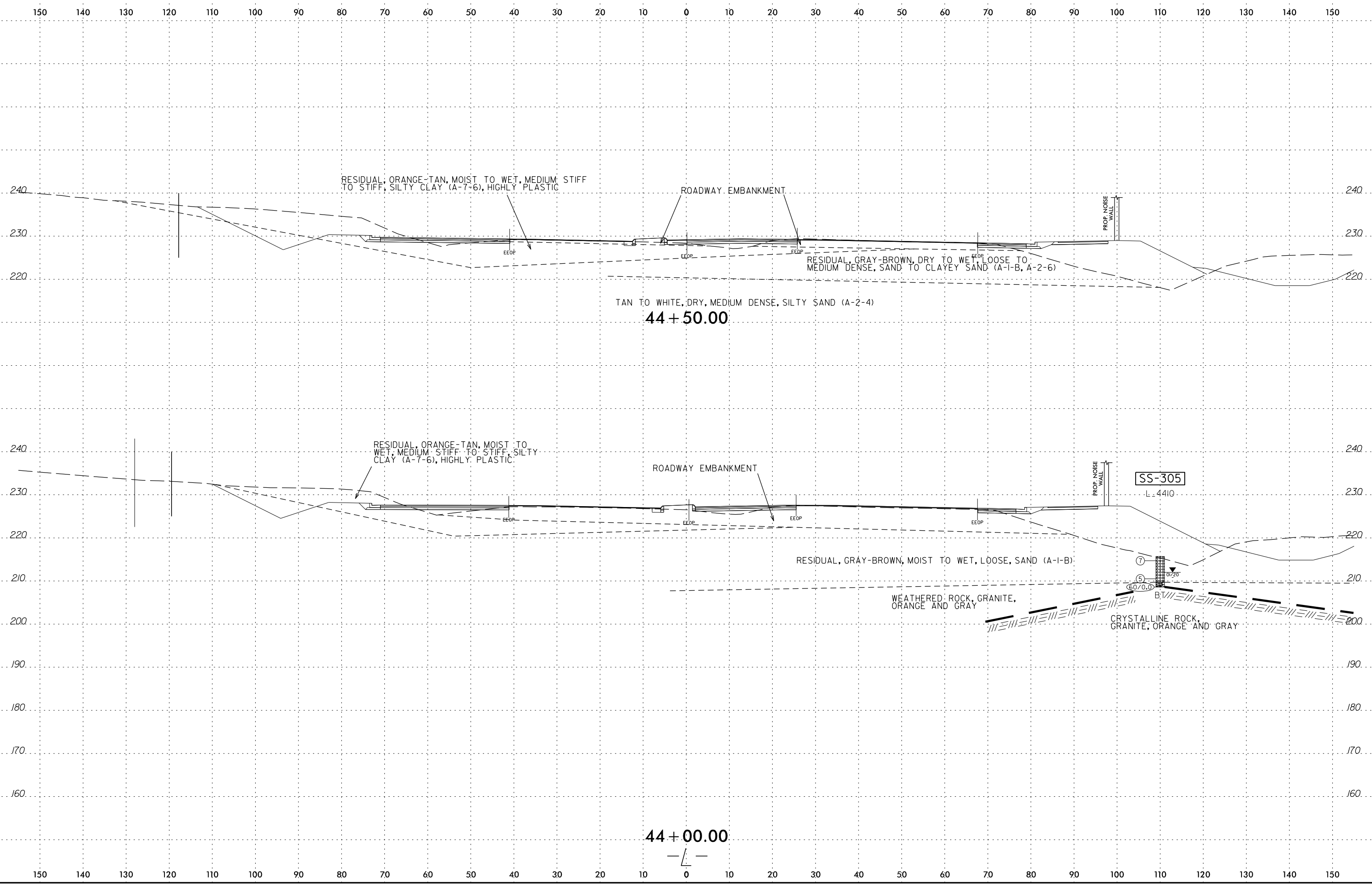
WHITE AND ORANGE, MOIST, MEDIUM DENSE, SILTY SAND (A-2-4)

WEATHERED ROCK, GRANITE, ORANGE AND GRAY

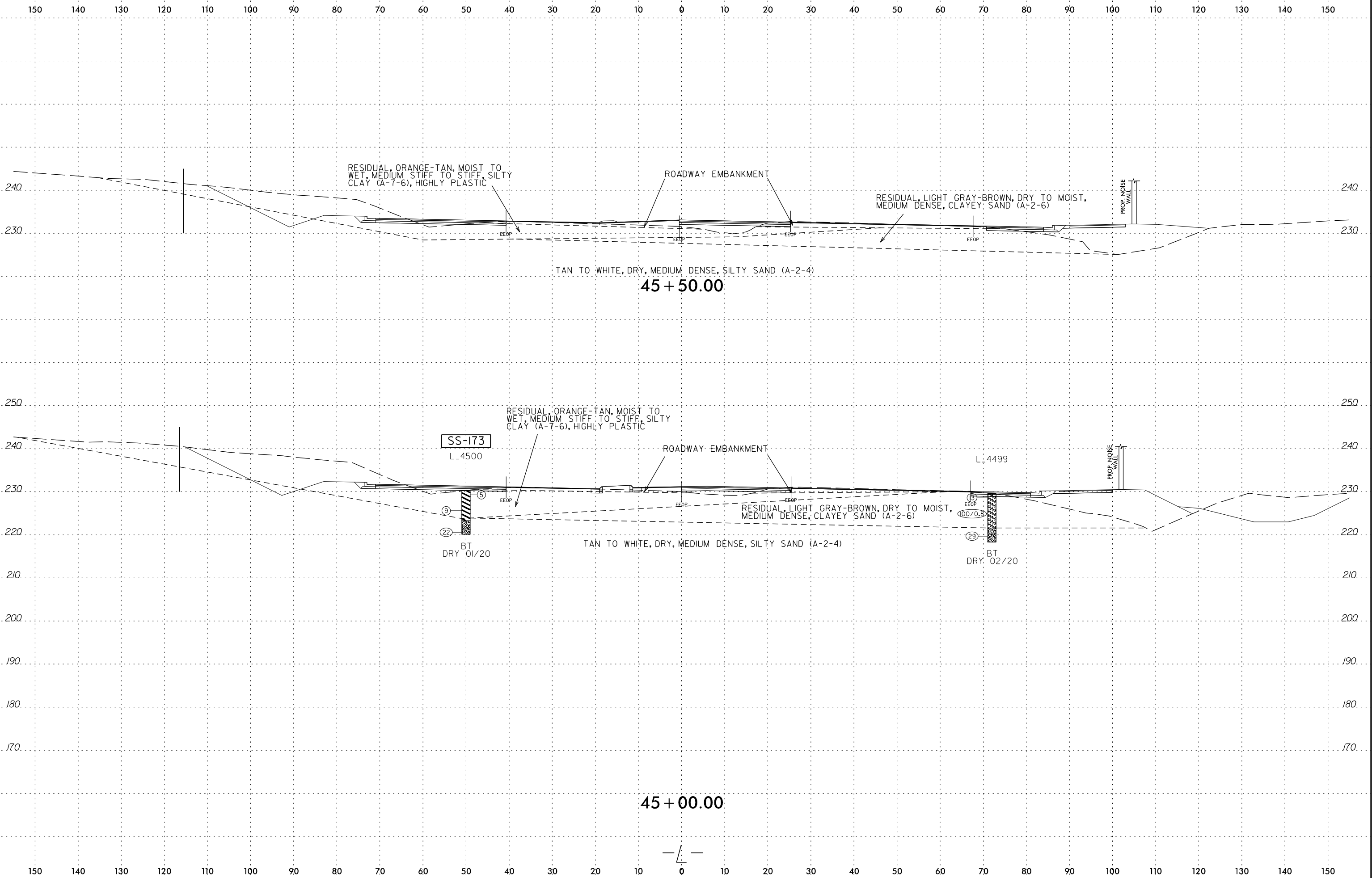
CRYSTALLINE ROCK, GRANITE, ORANGE AND BLACK

BT
DRY 01/20

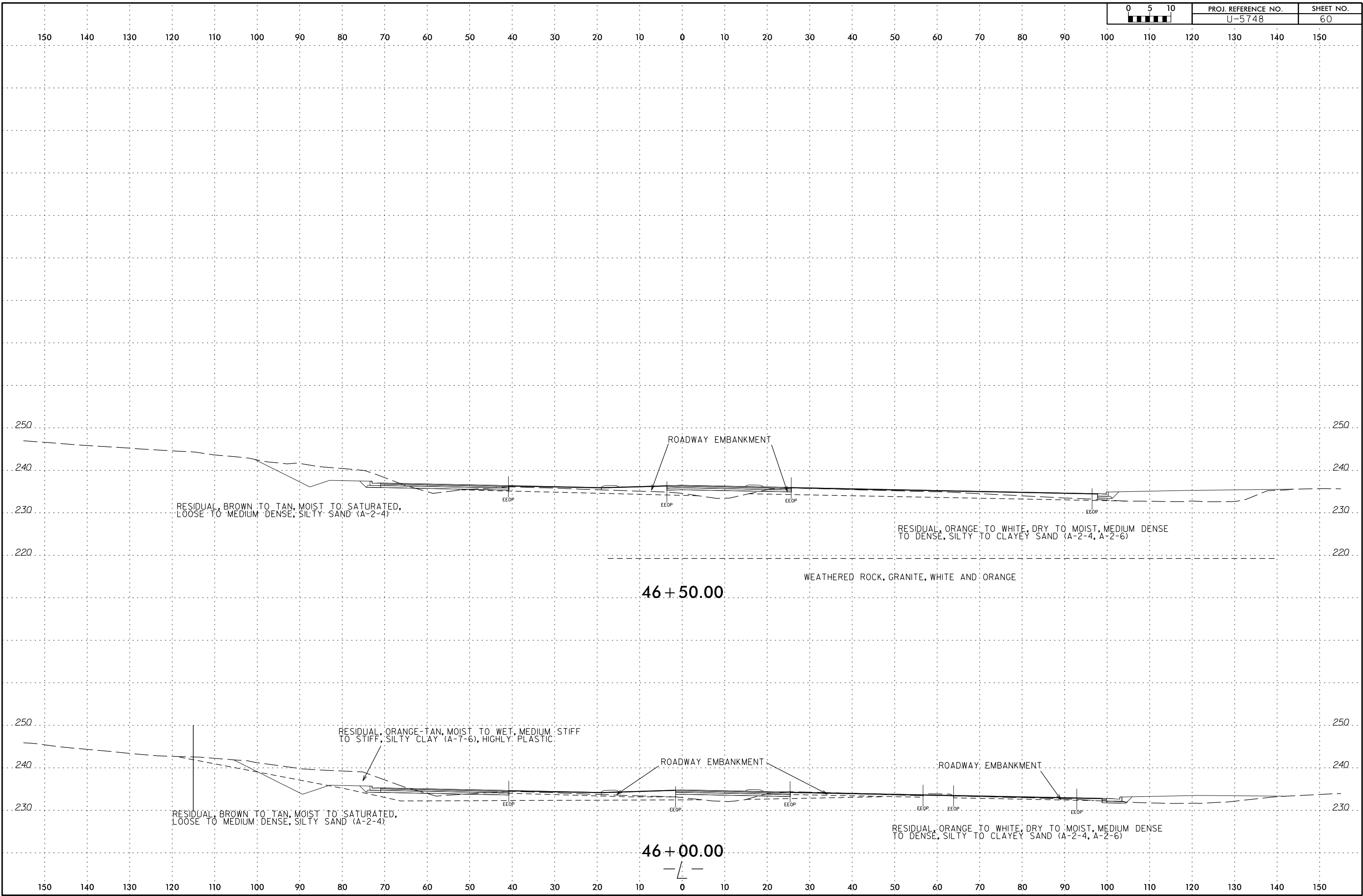
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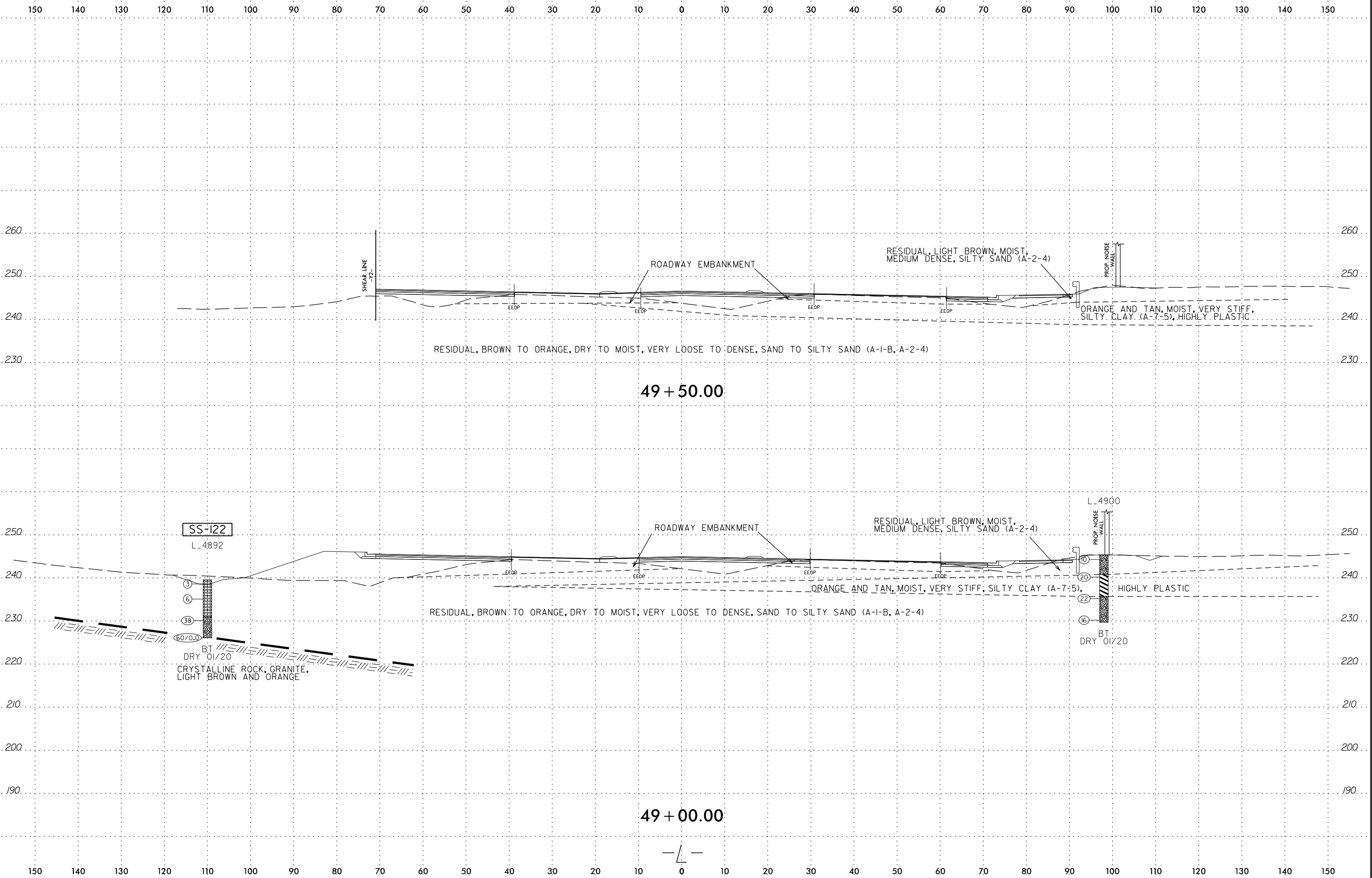


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 alexander.bozada

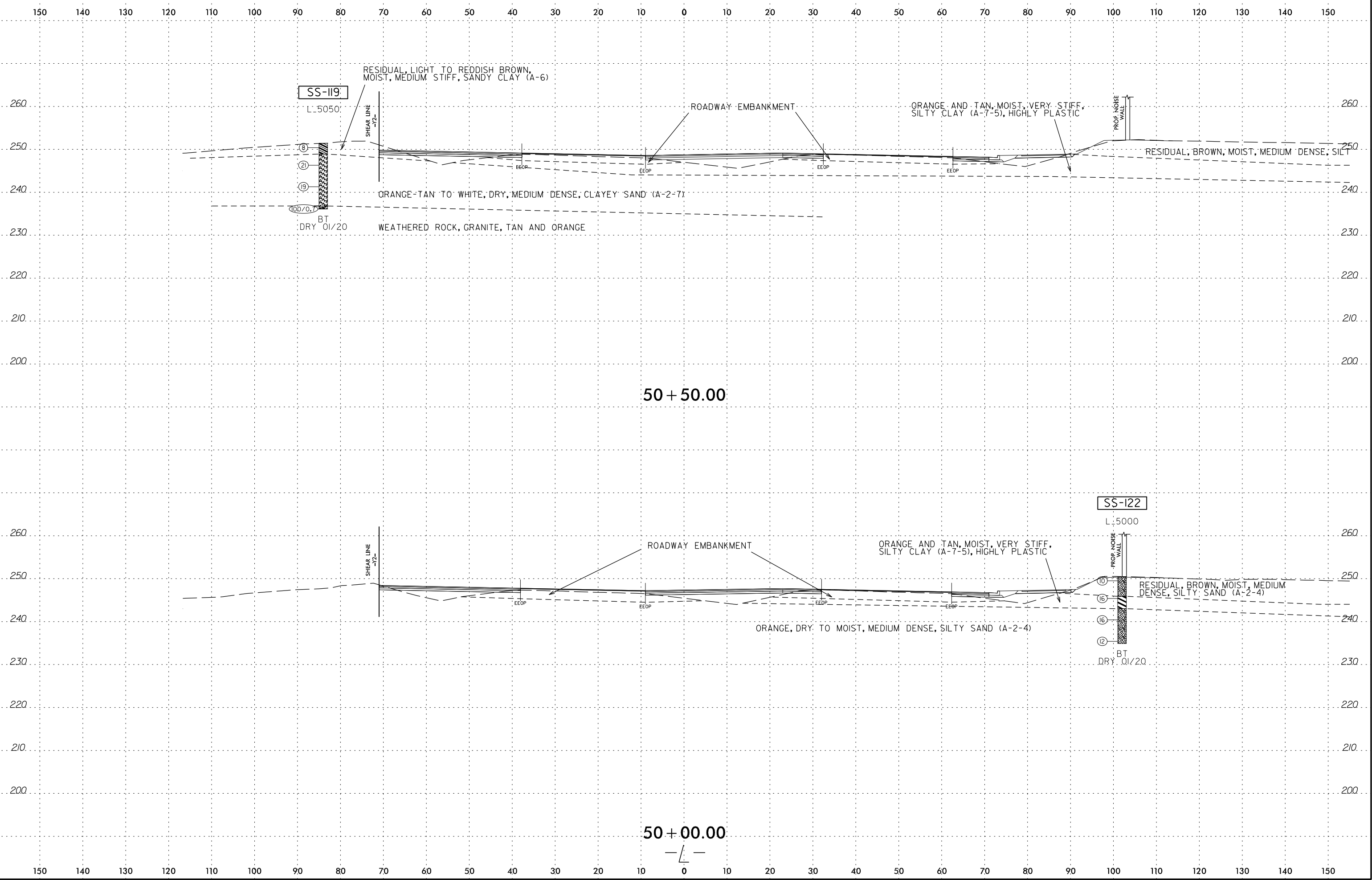


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 alexander.bozada



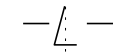
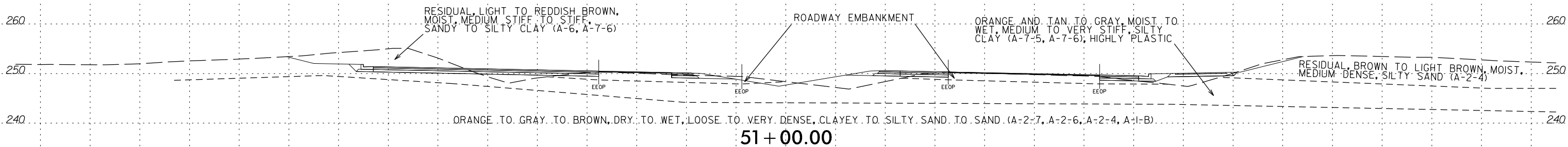
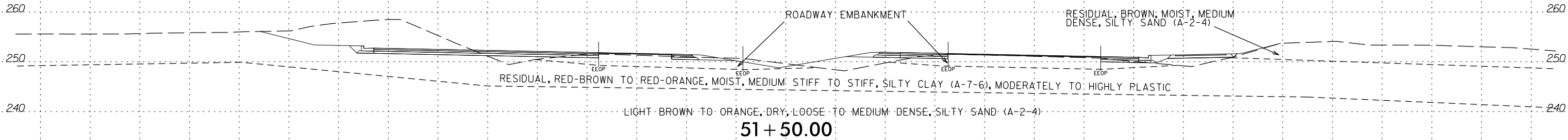


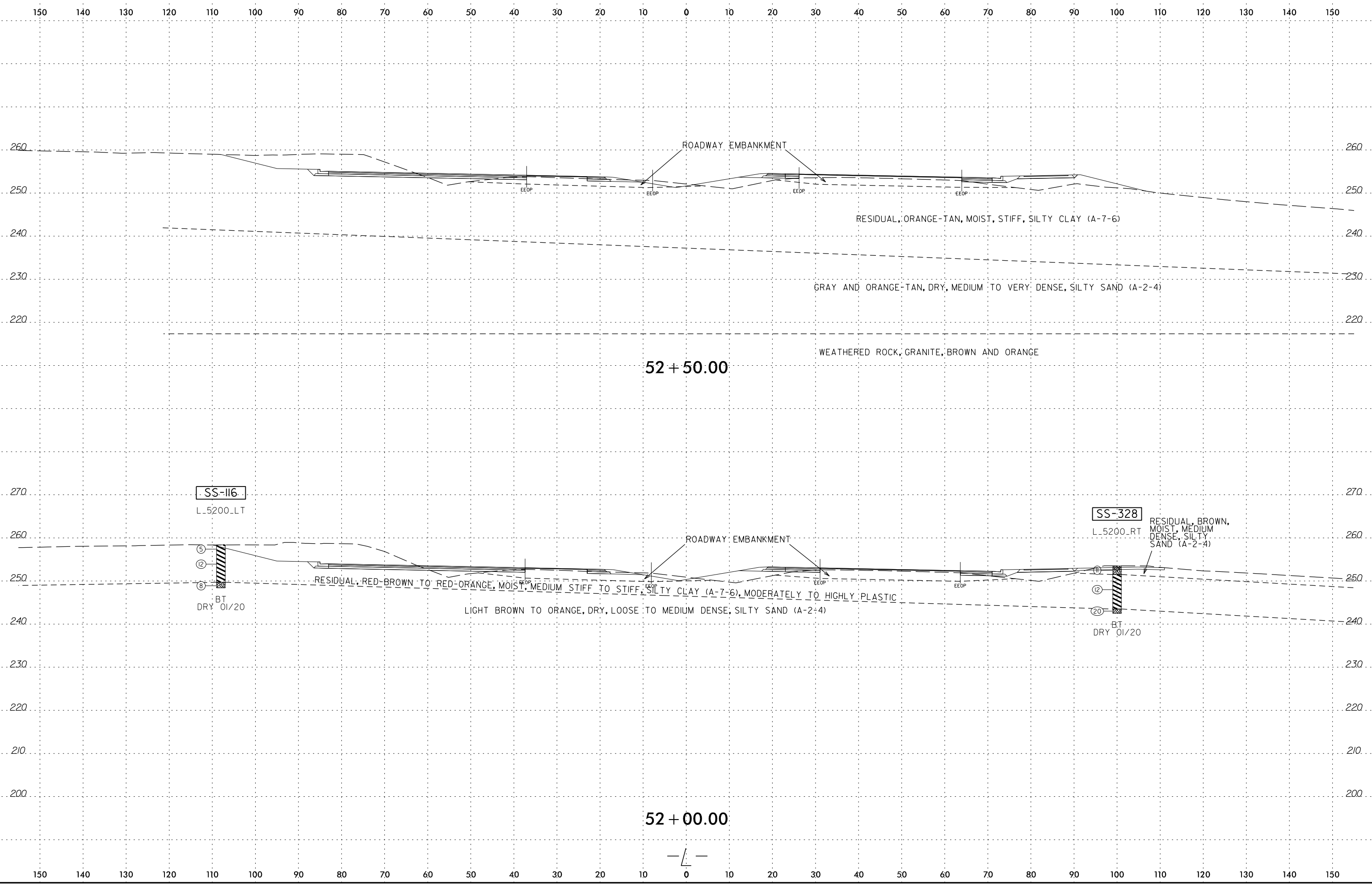
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 alexander.bozada



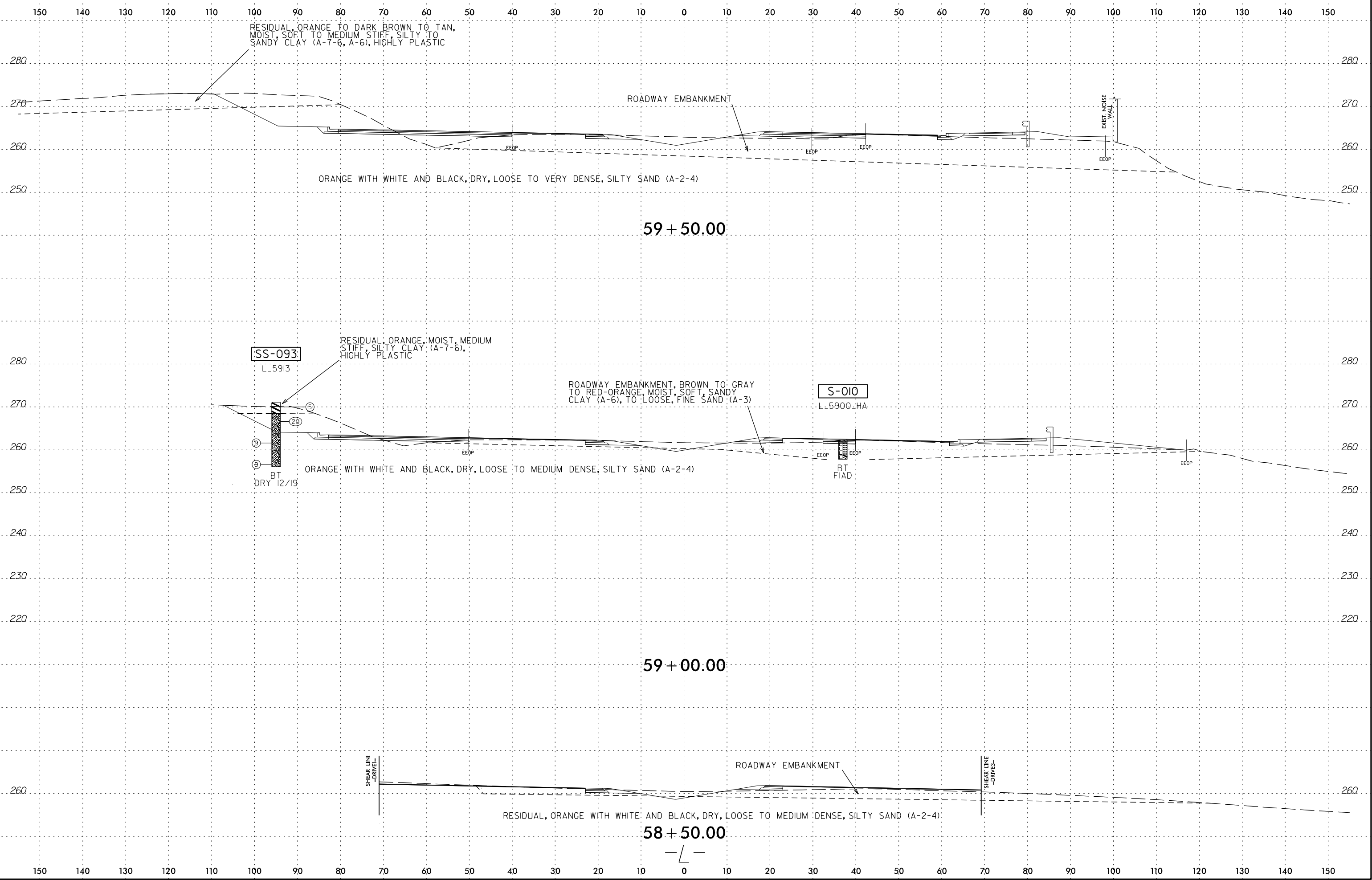
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 alexander.bozada

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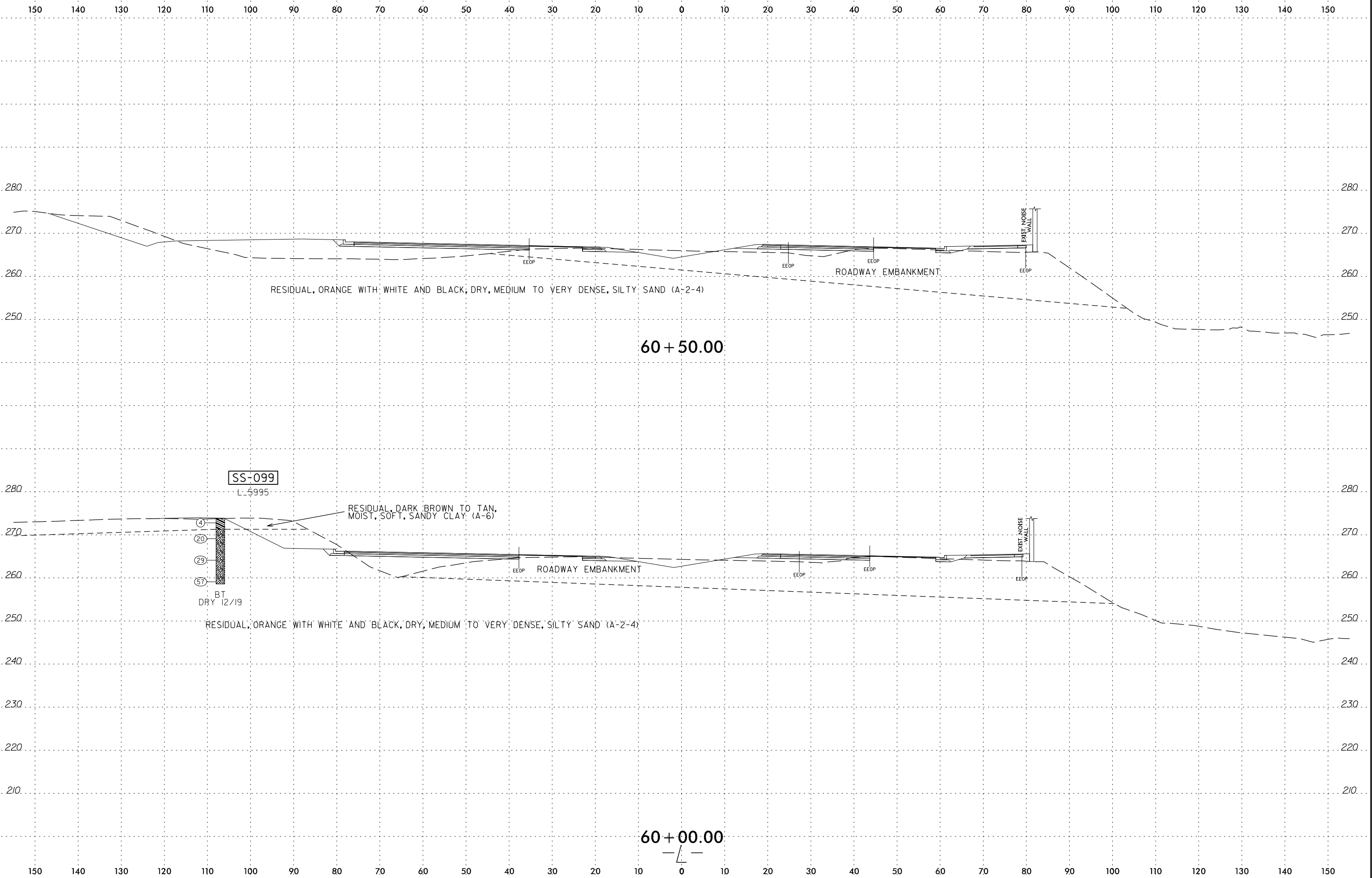


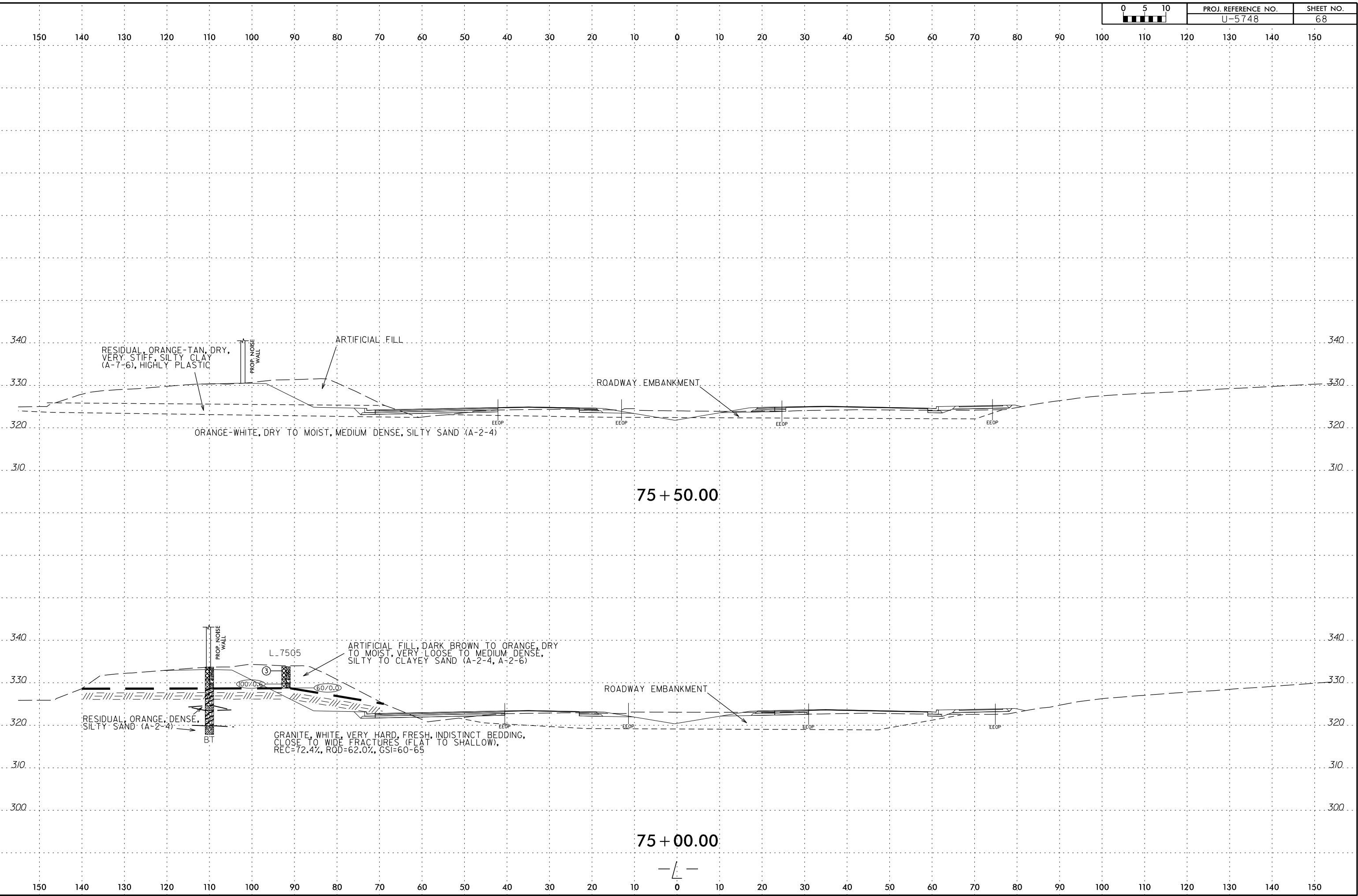


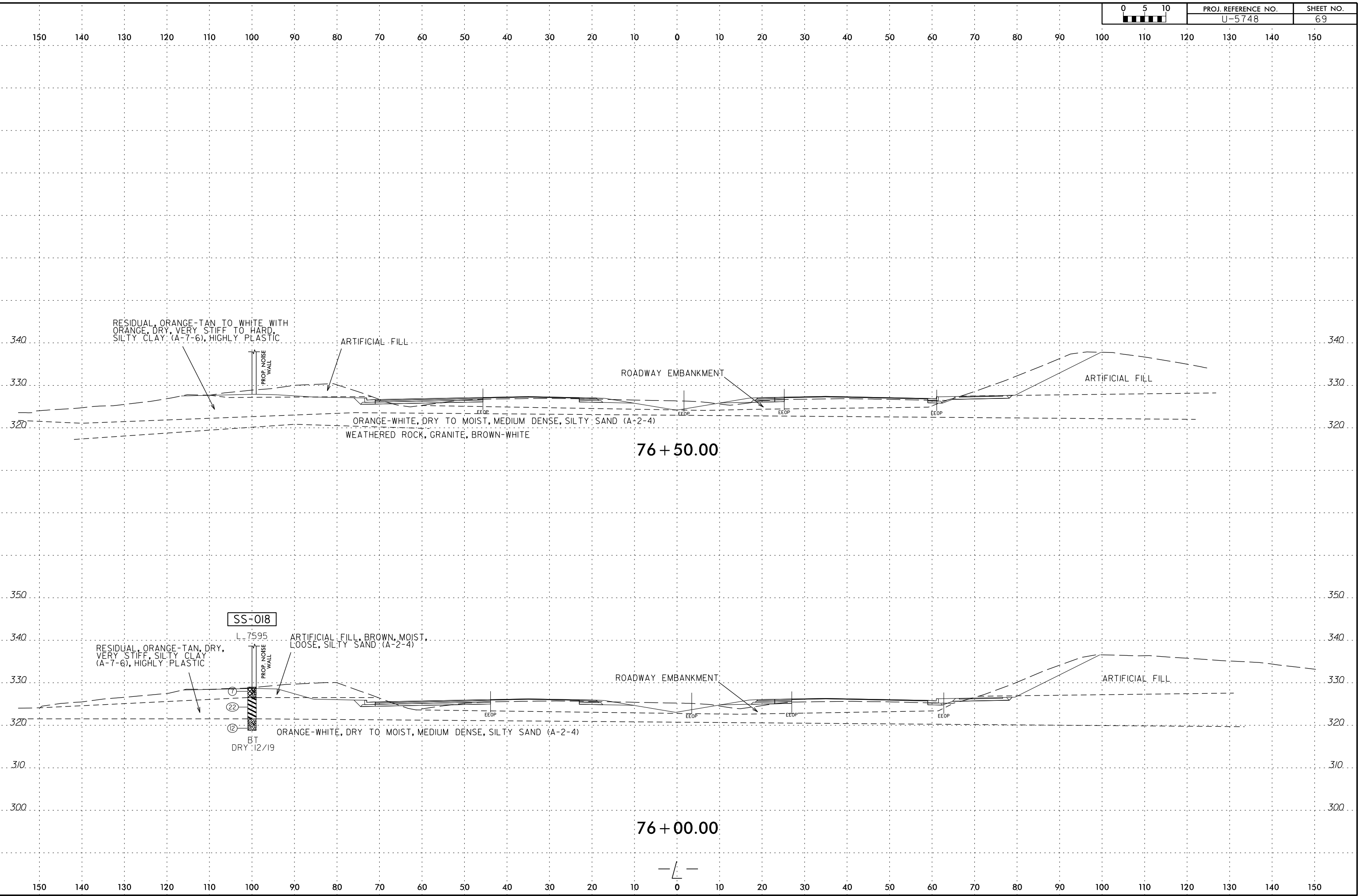
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 alexander.jozada

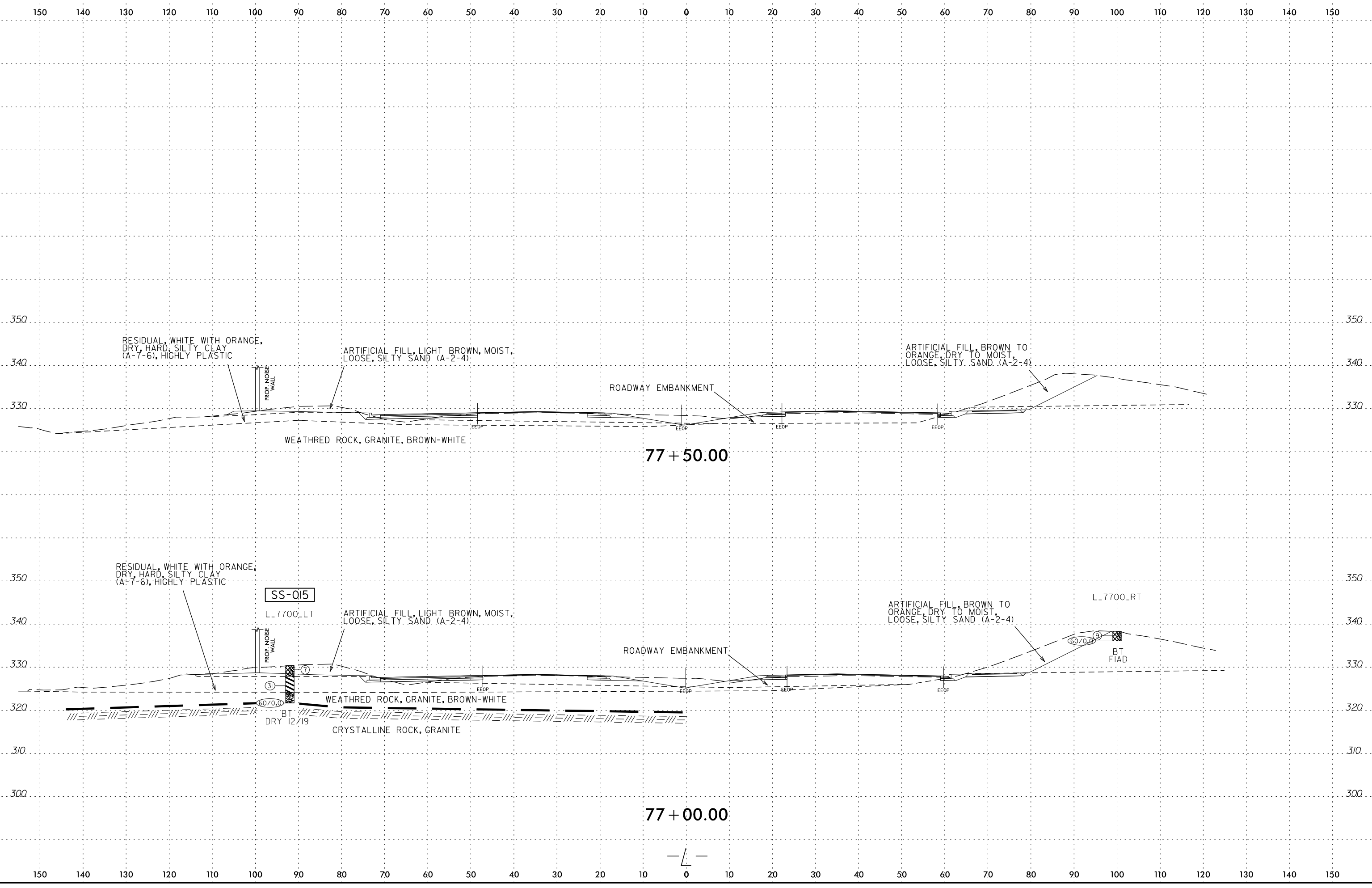


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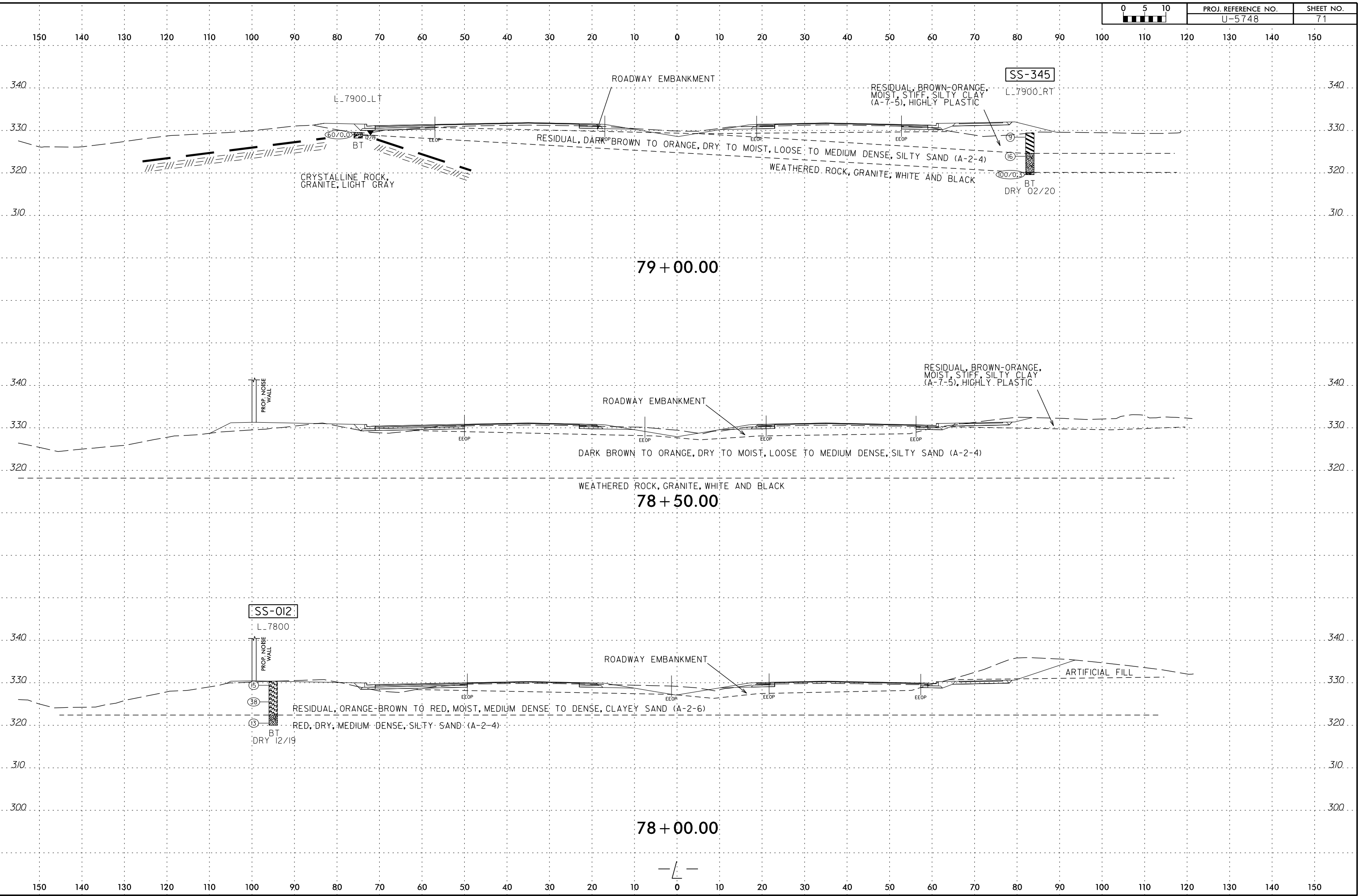


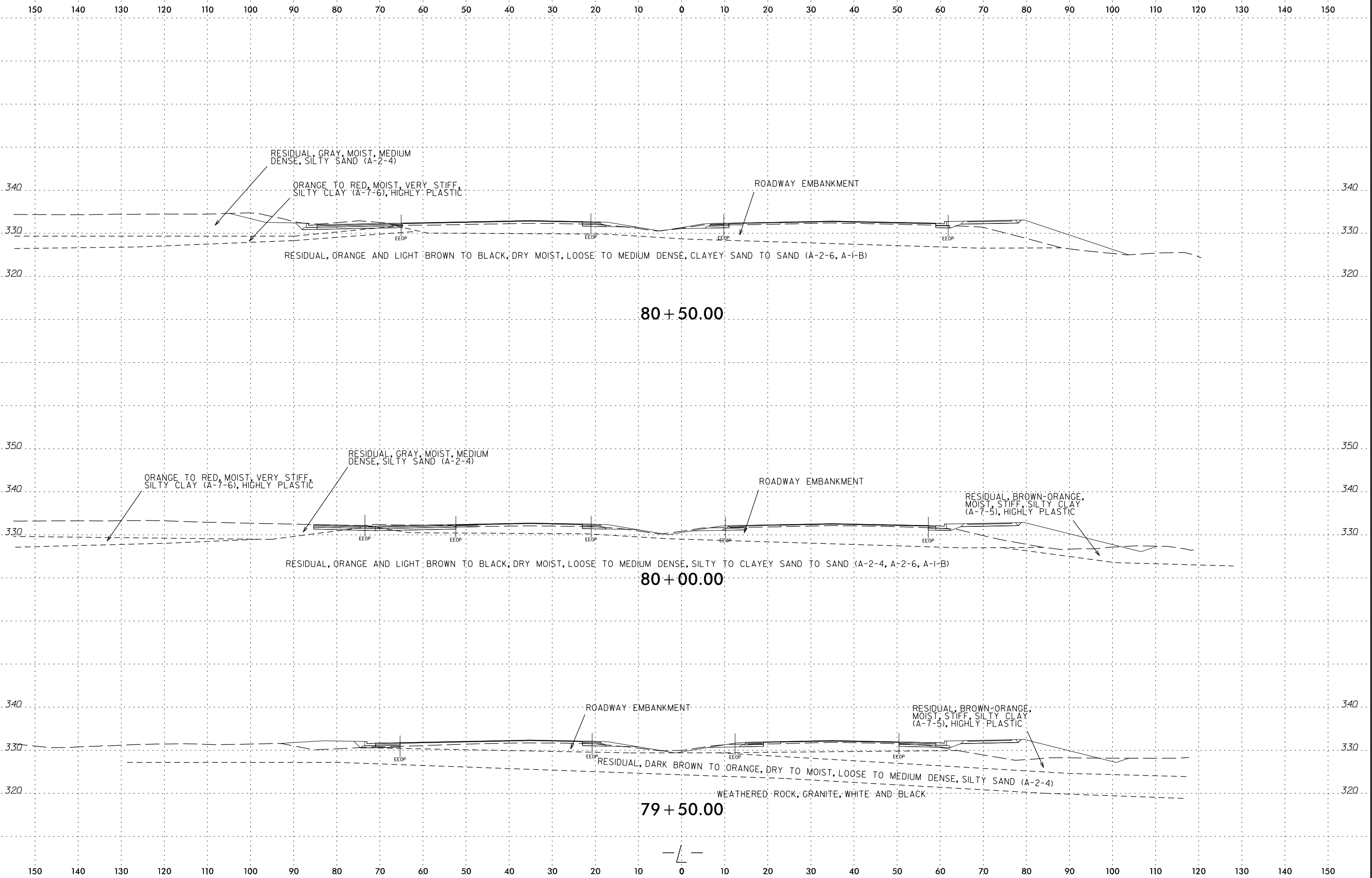




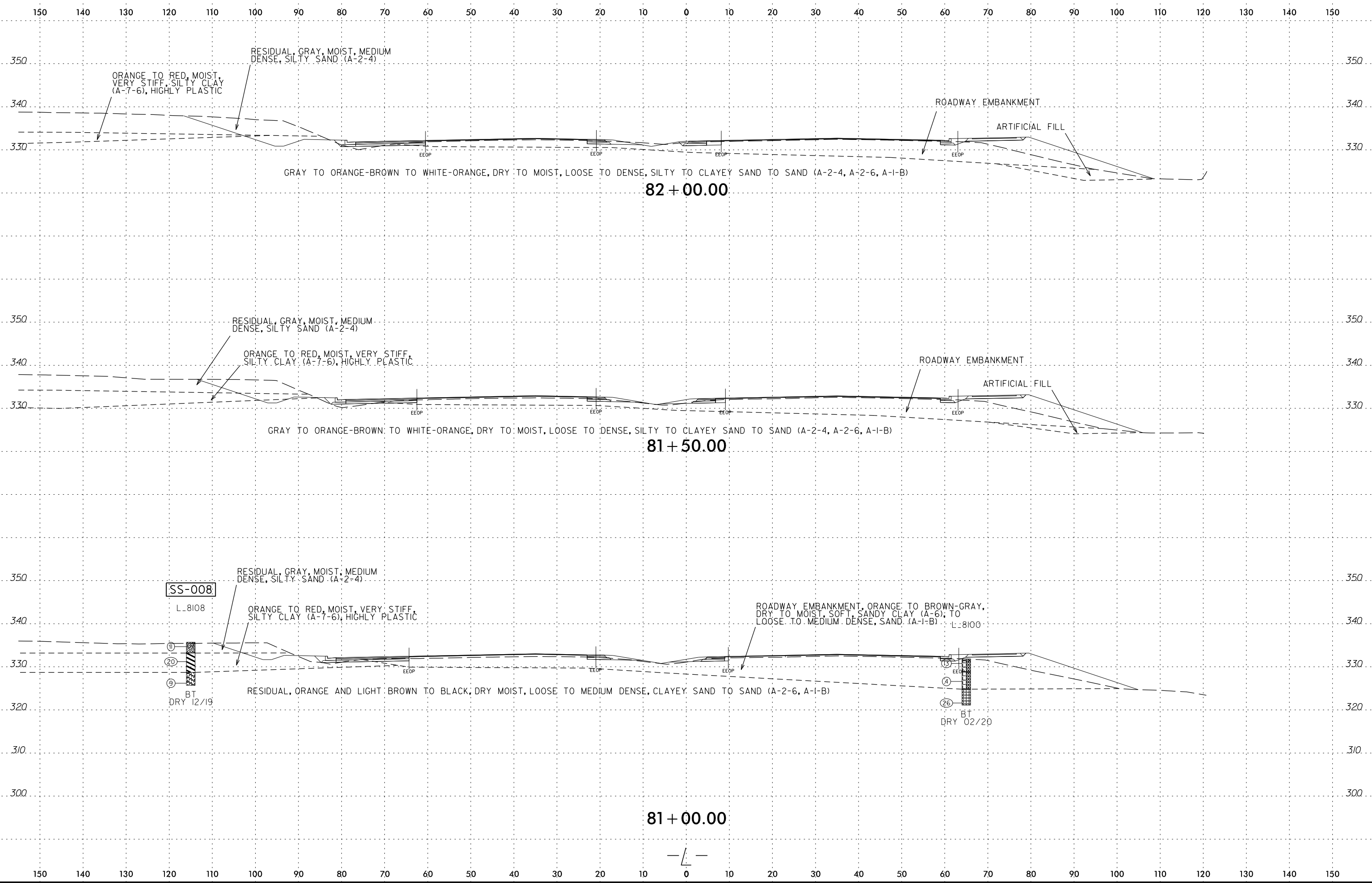


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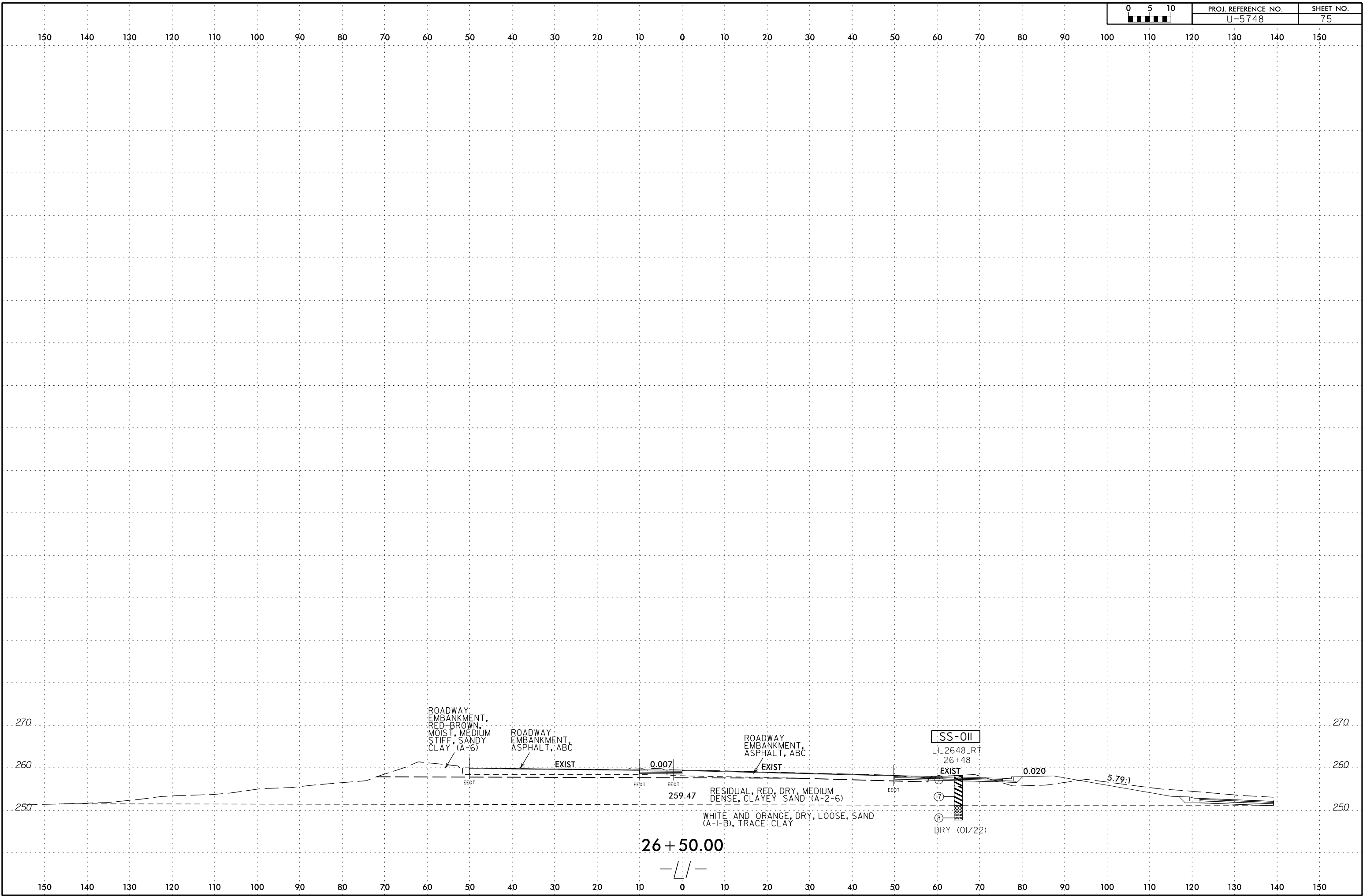


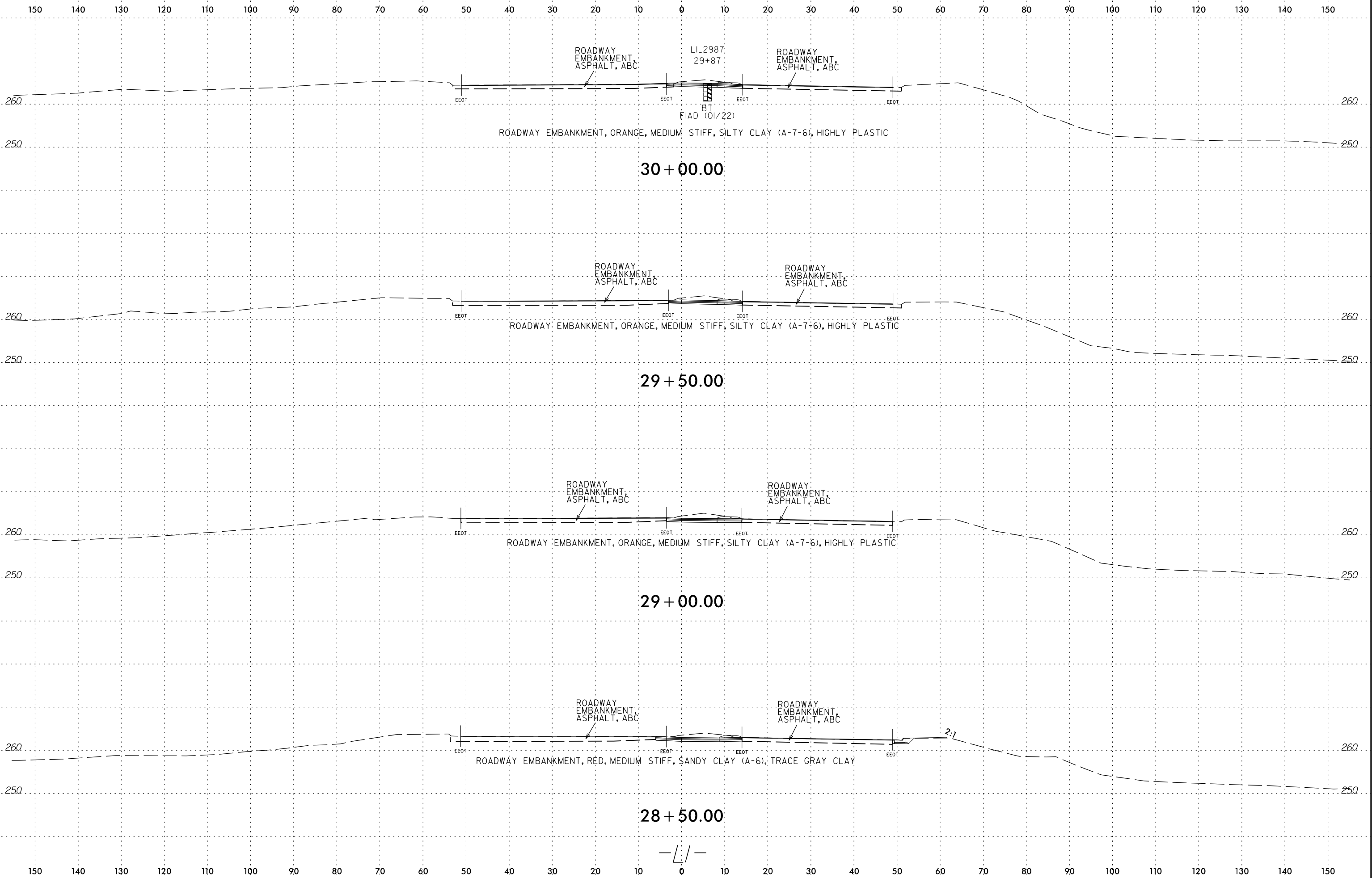


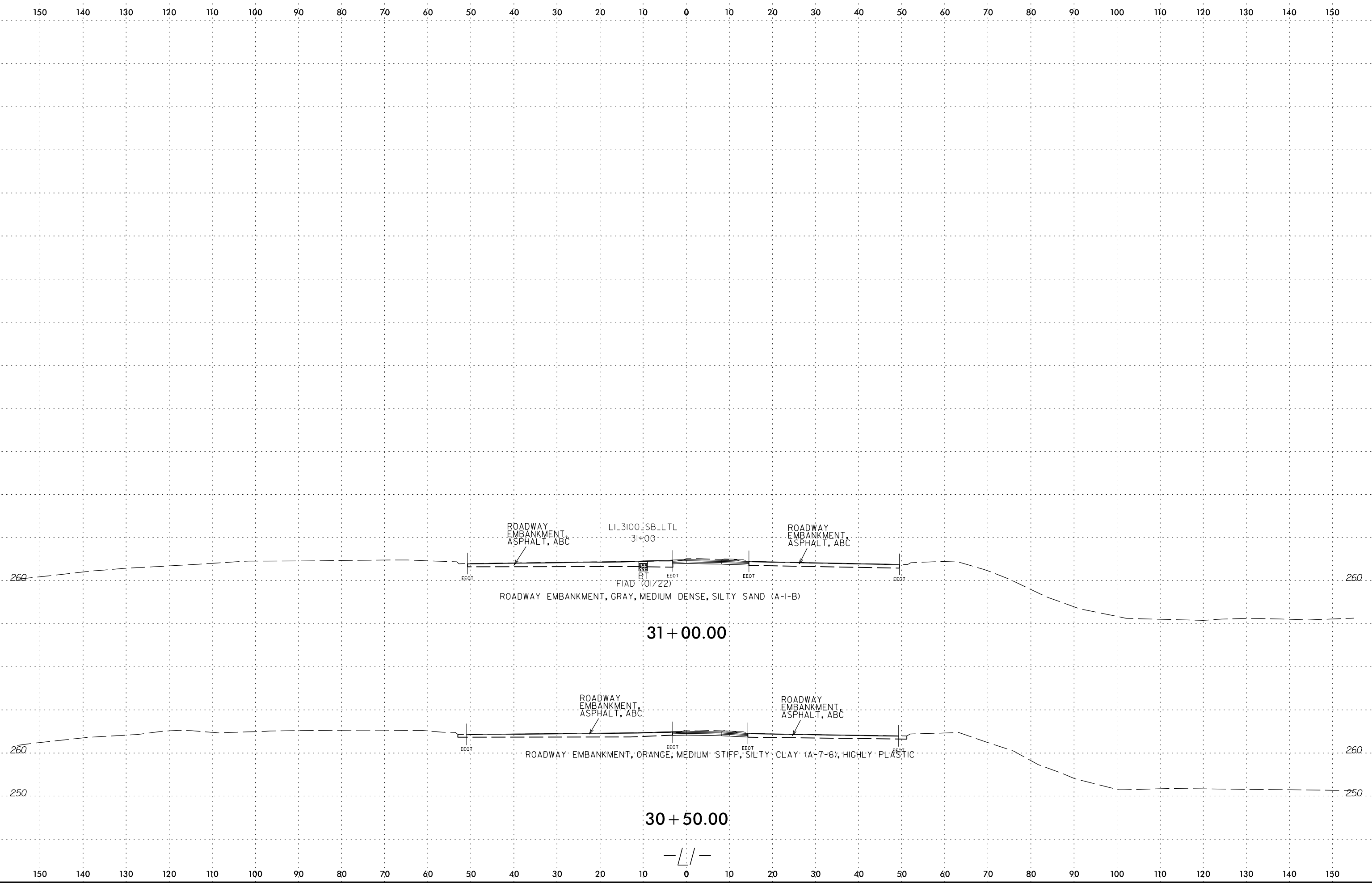
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 alexander.bozada



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 alexander.bozada







ROADWAY EMBANKMENT, ASPHALT, ABC

LI 3100 SB LTL
31+00

FIAD (01/22)

ROADWAY EMBANKMENT, ASPHALT, ABC

ROADWAY EMBANKMENT, GRAY, MEDIUM DENSE, SILTY SAND (A-I-B)

31 + 00.00

ROADWAY EMBANKMENT, ASPHALT, ABC

ROADWAY EMBANKMENT, ASPHALT, ABC

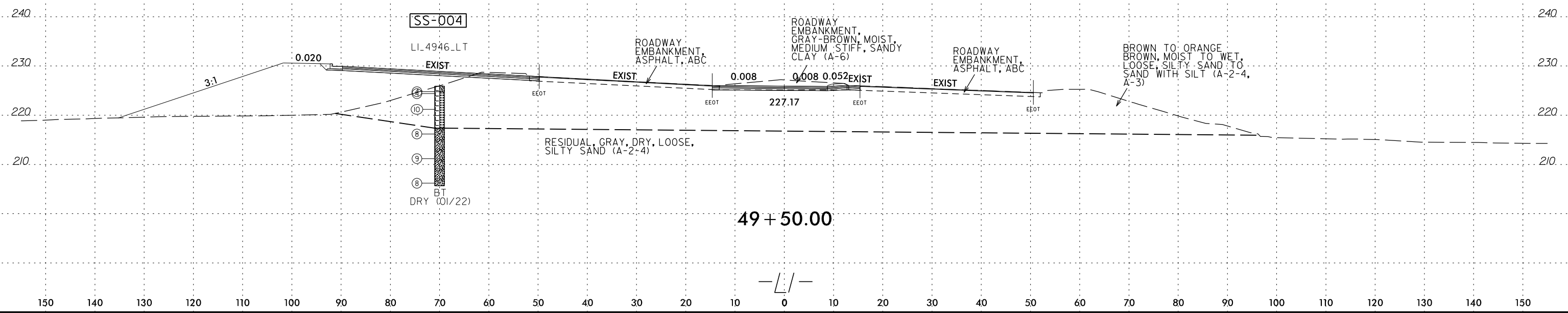
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30 + 50.00

— / —

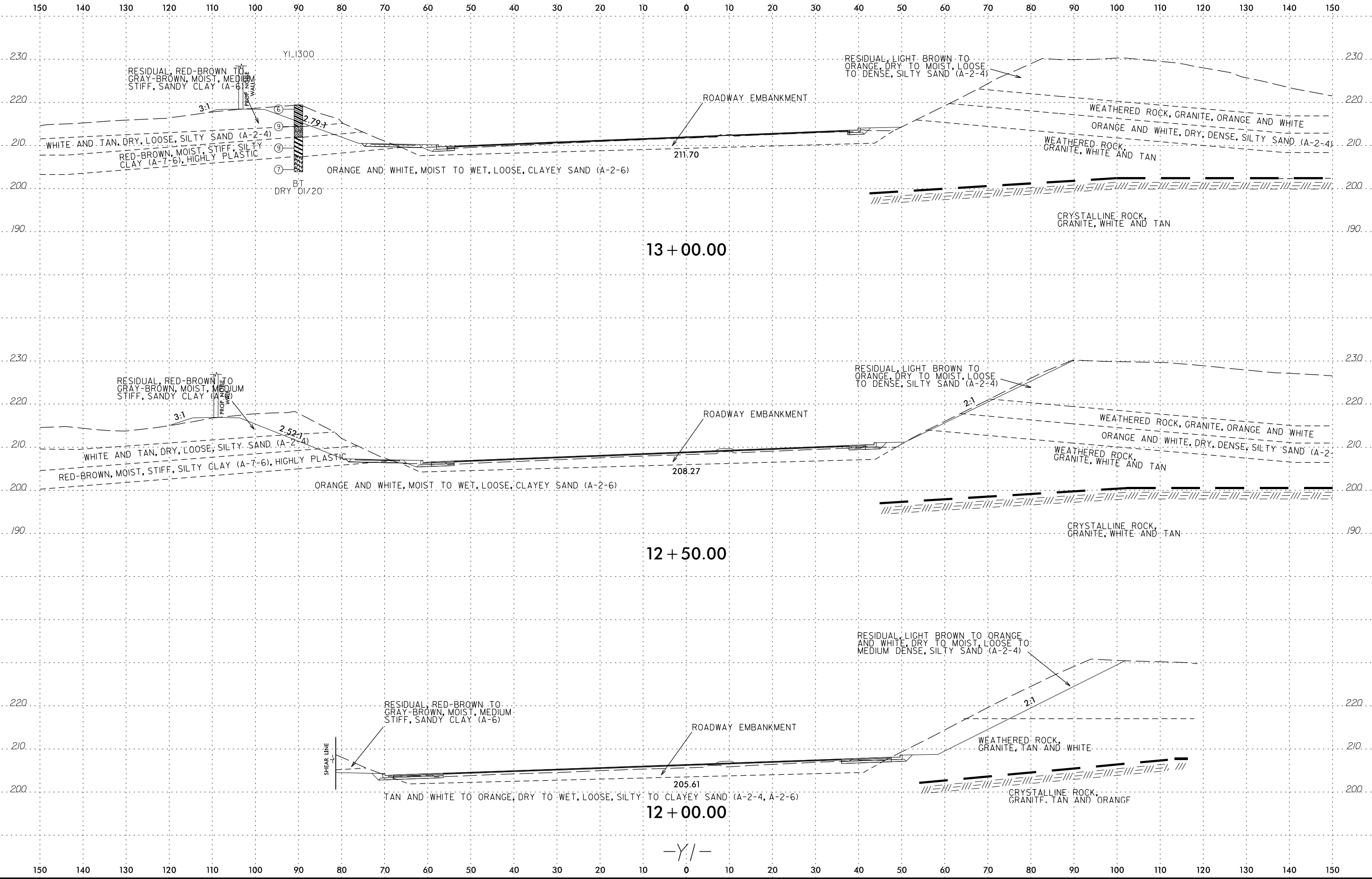
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 alexander_lozade



49 + 50.00

— / —



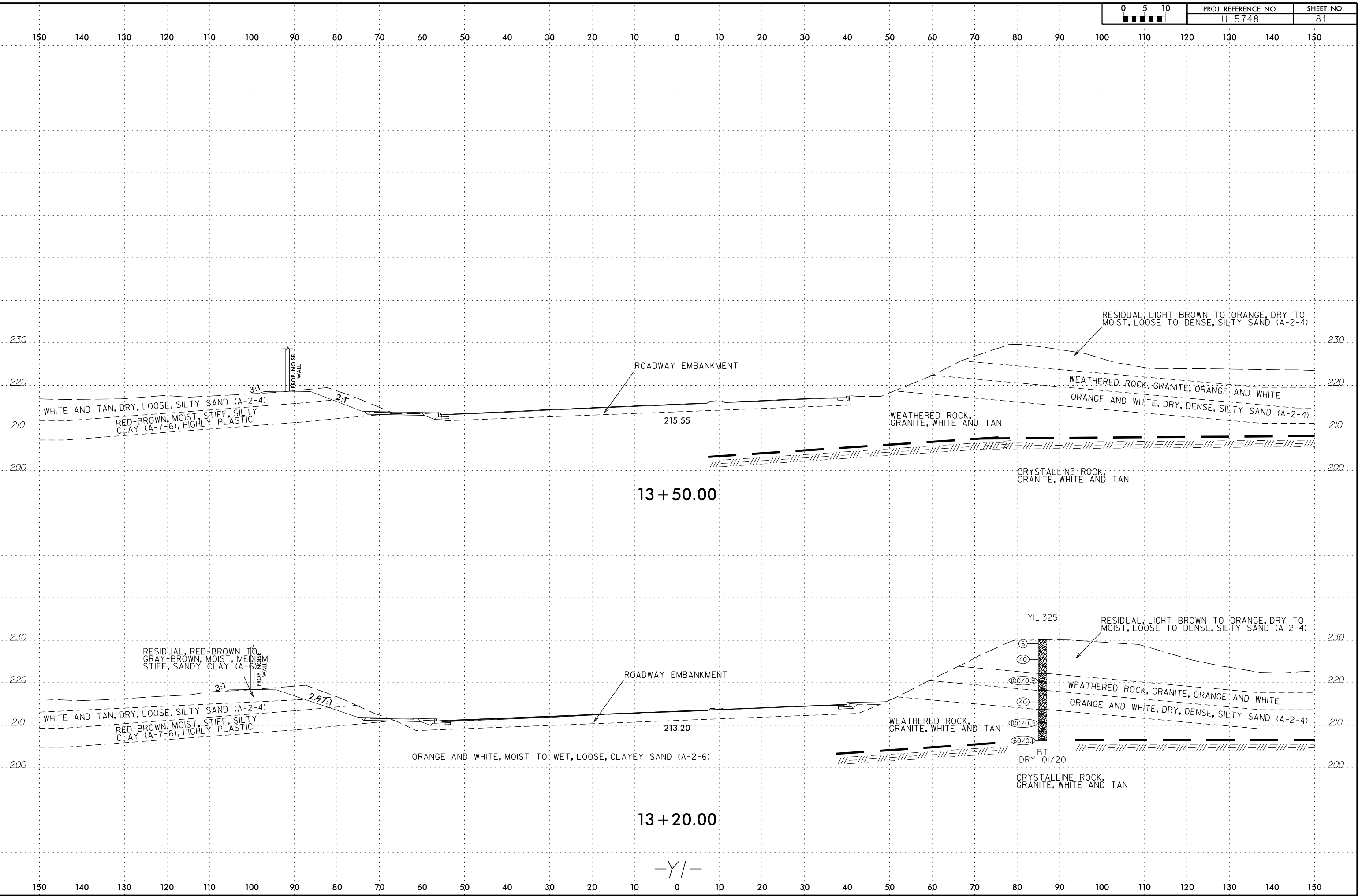
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 alexander.bozada

13 + 00.00

12 + 50.00

12 + 00.00

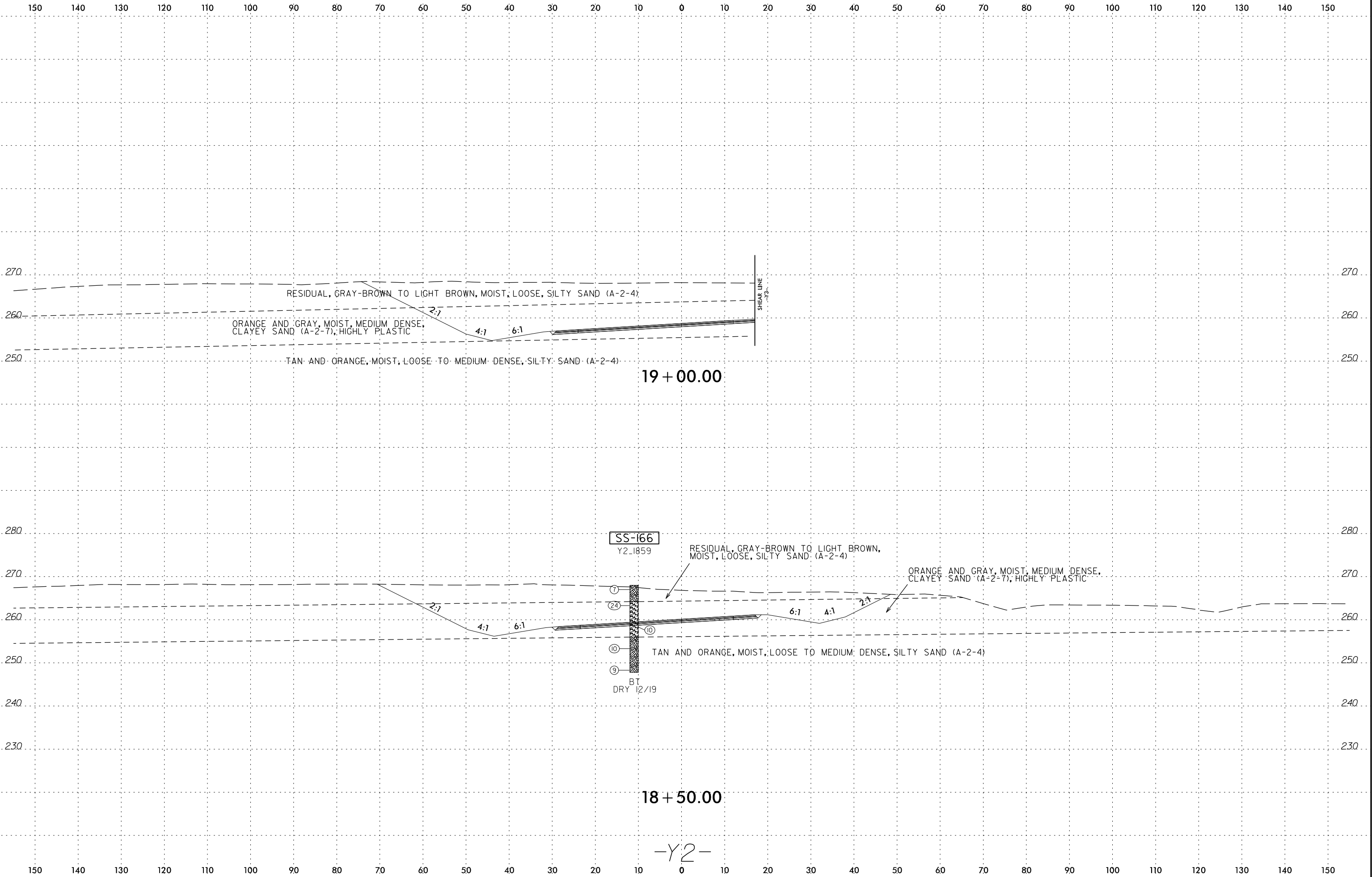
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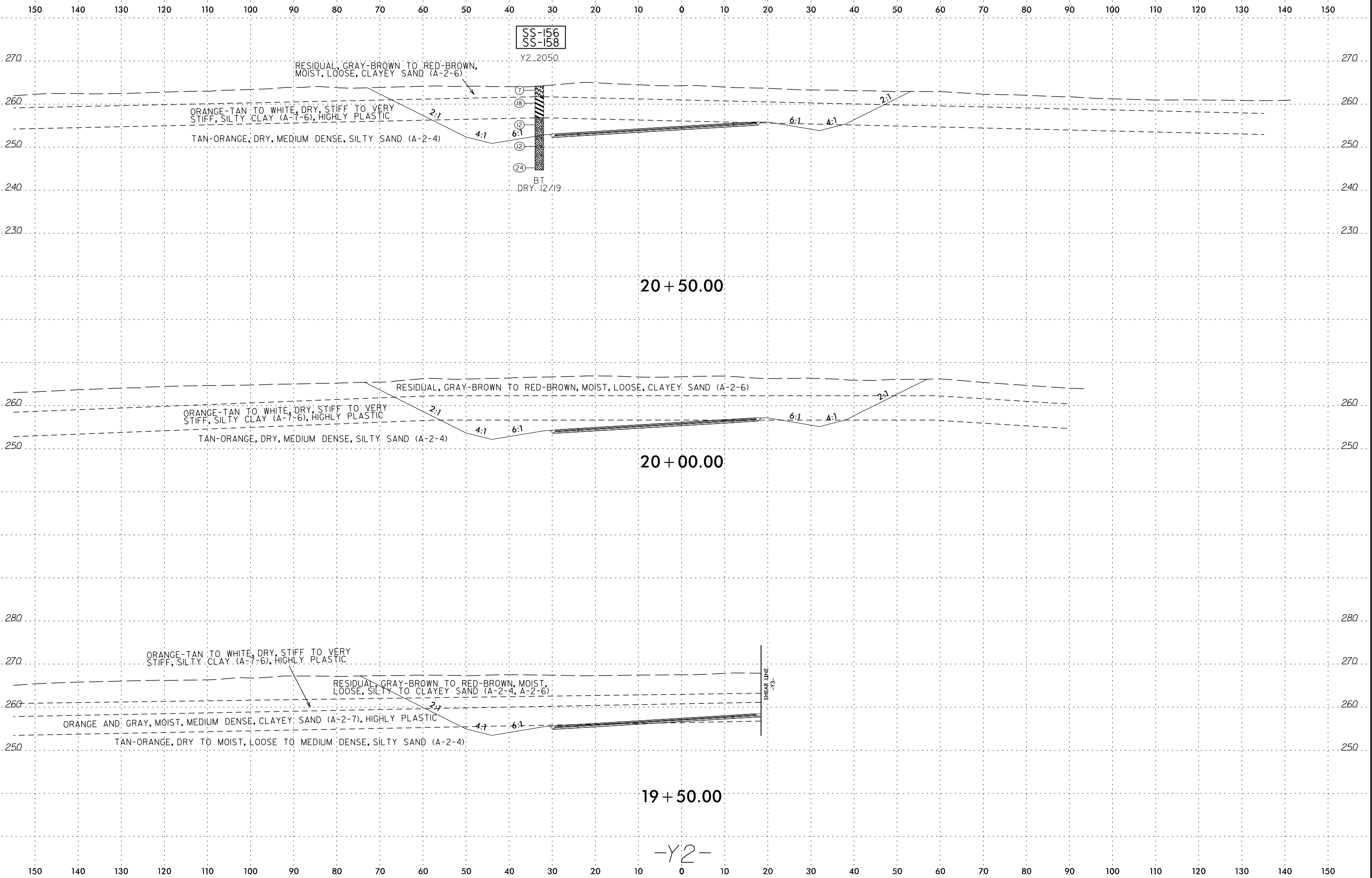


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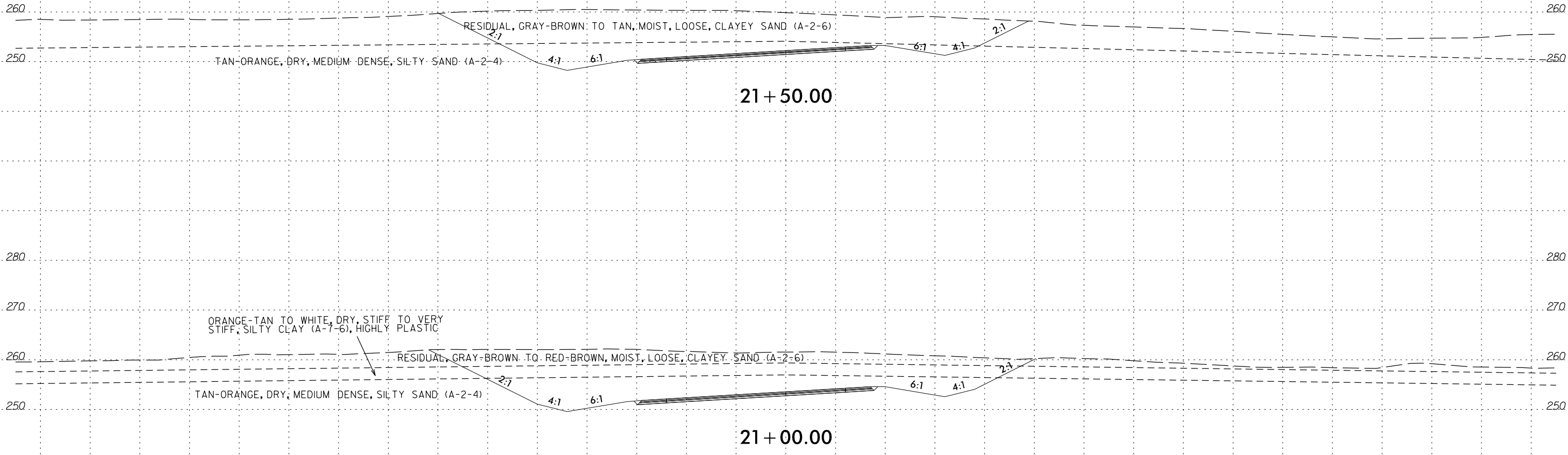
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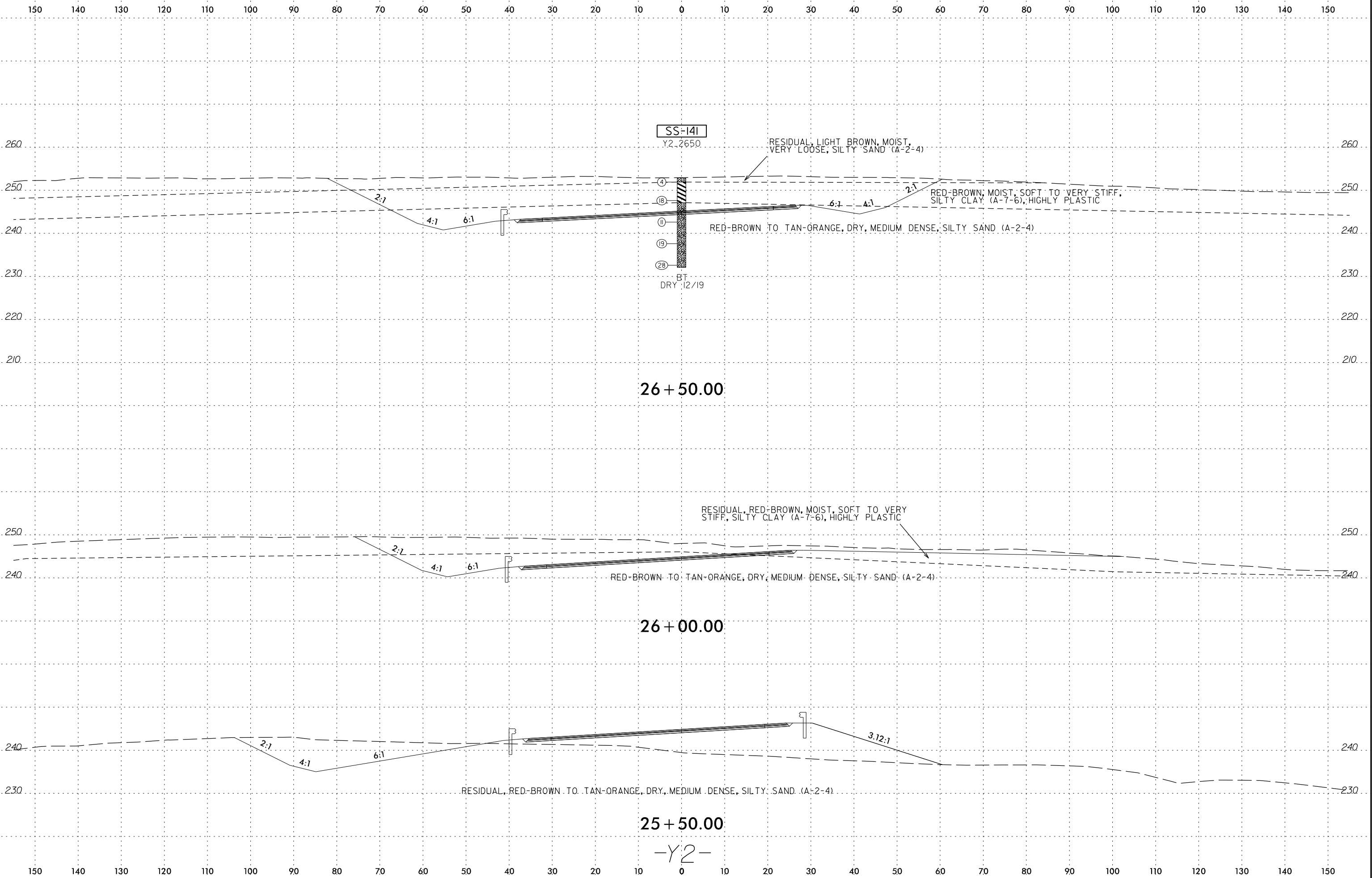


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 alexander.bozada



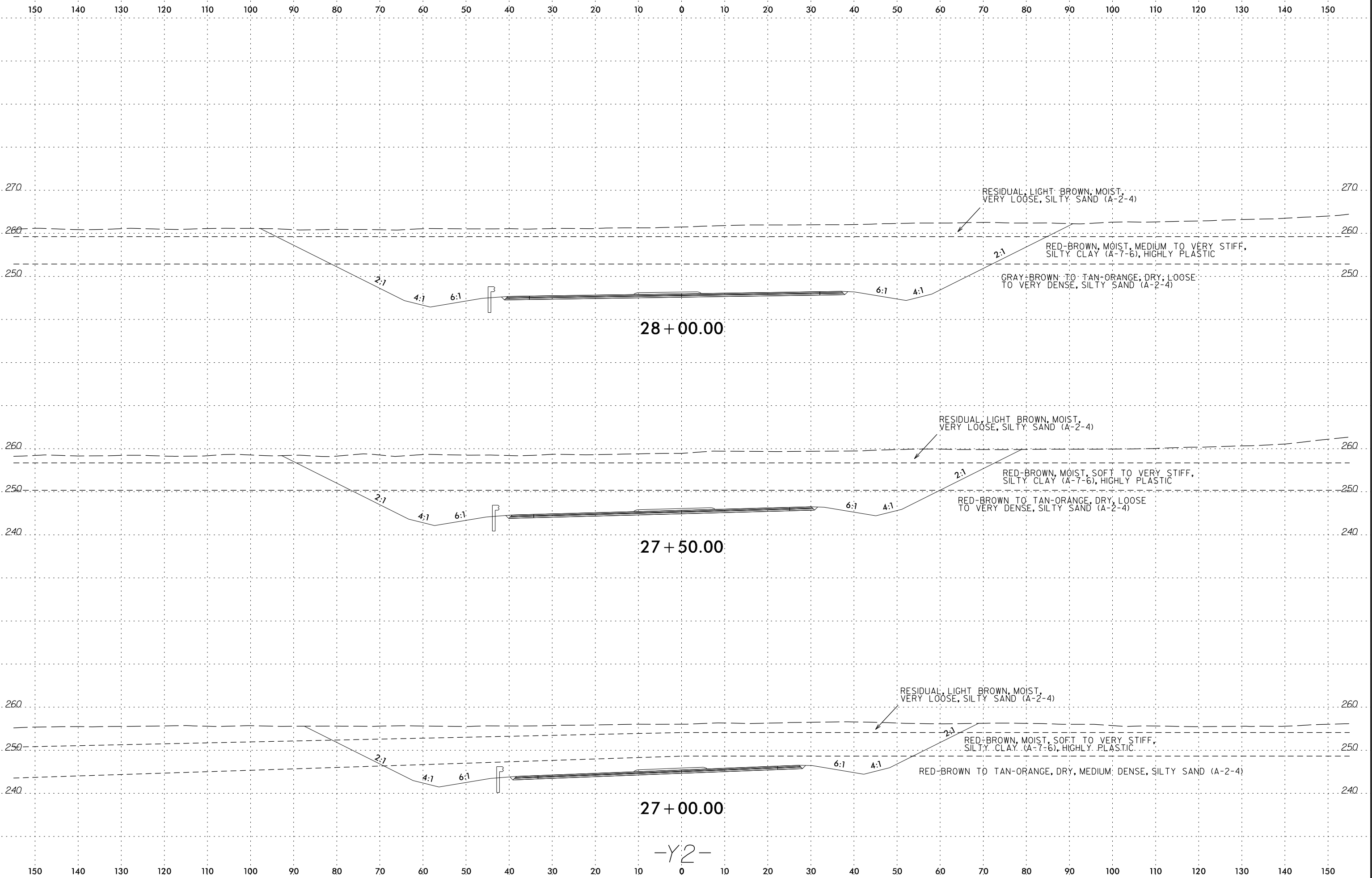
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 alexander.bozada

26 + 50.00

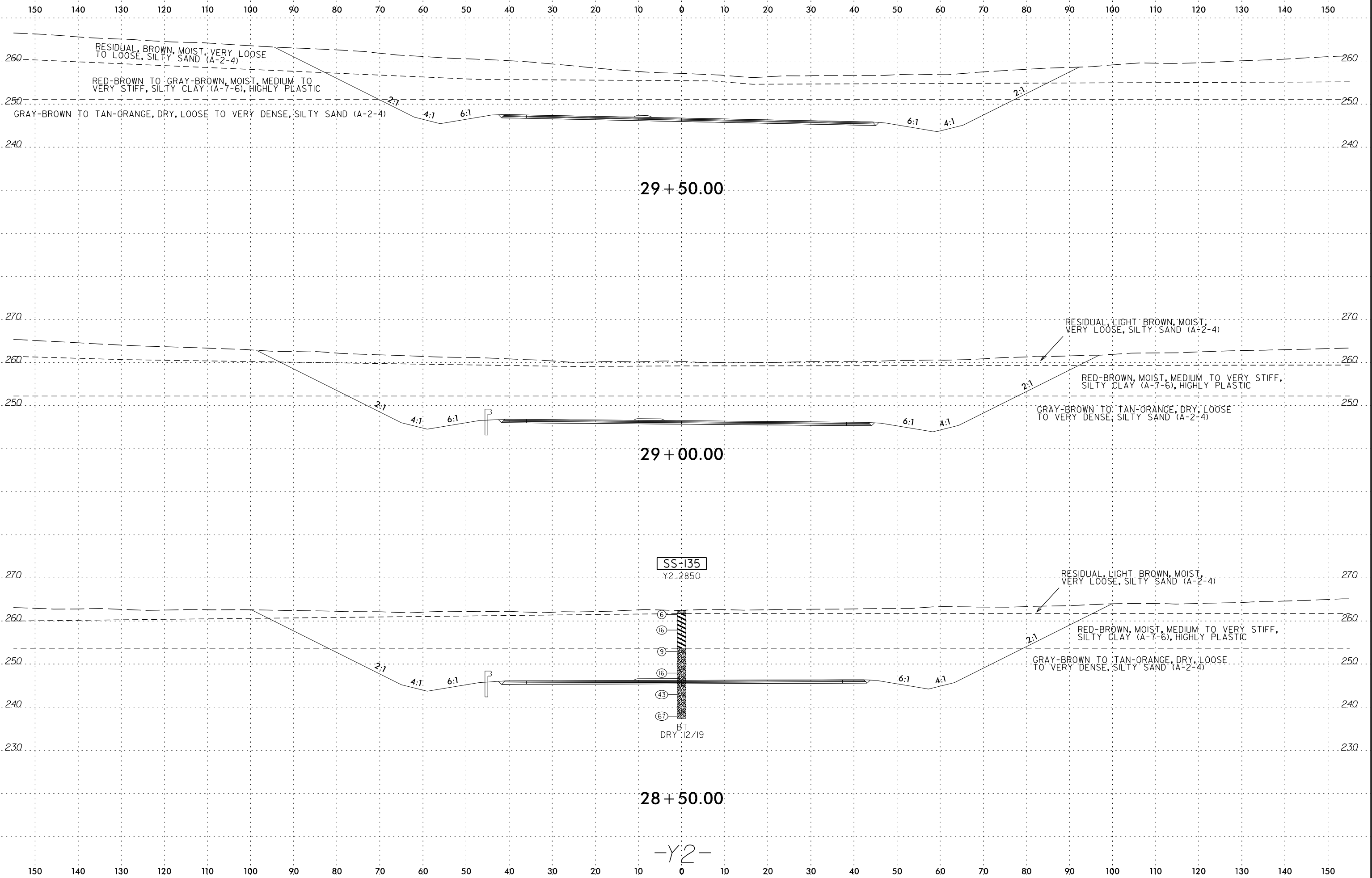
26 + 00.00

25 + 50.00

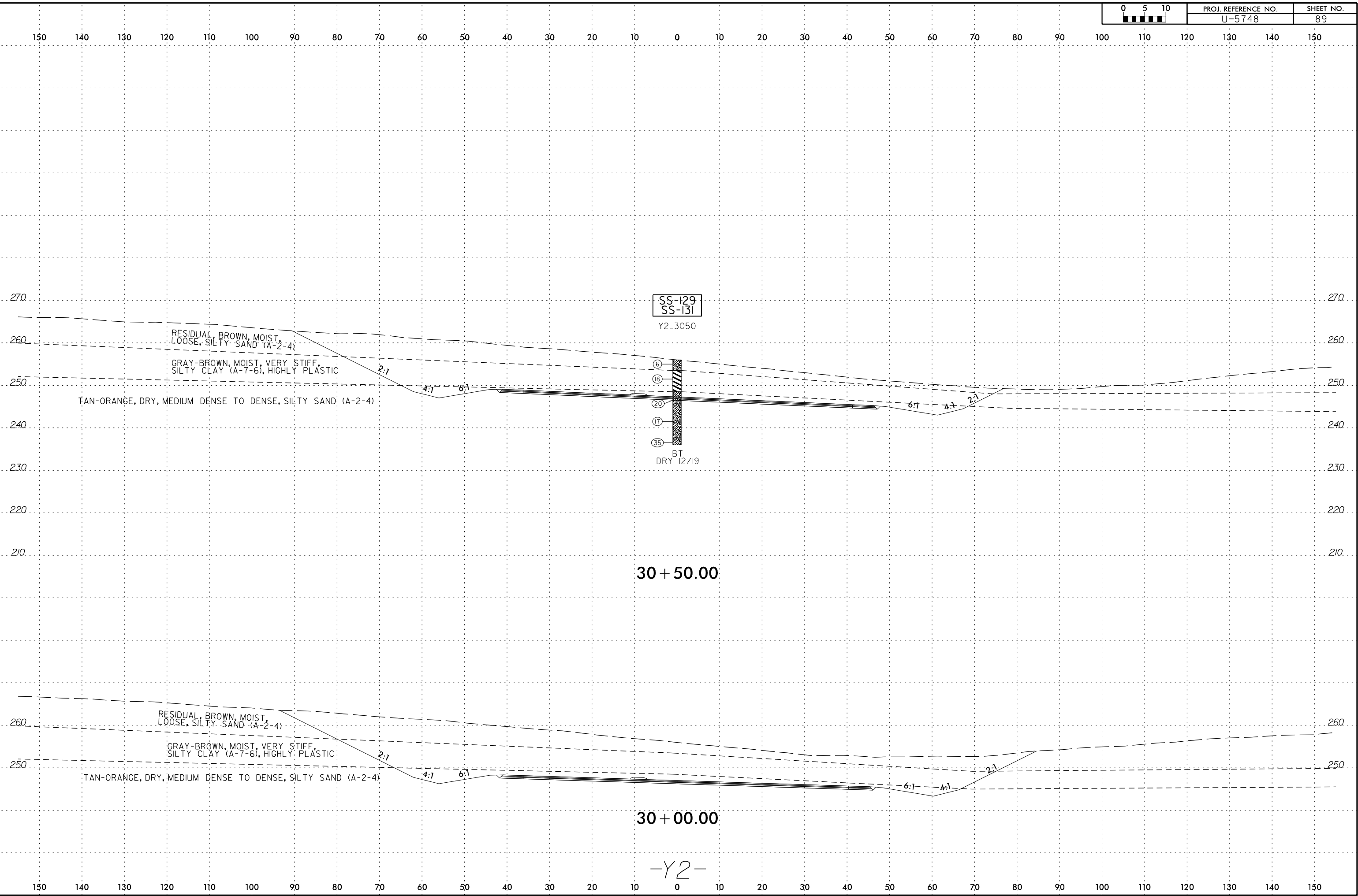
-Y2-



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 alexander.bozada



-Y2-

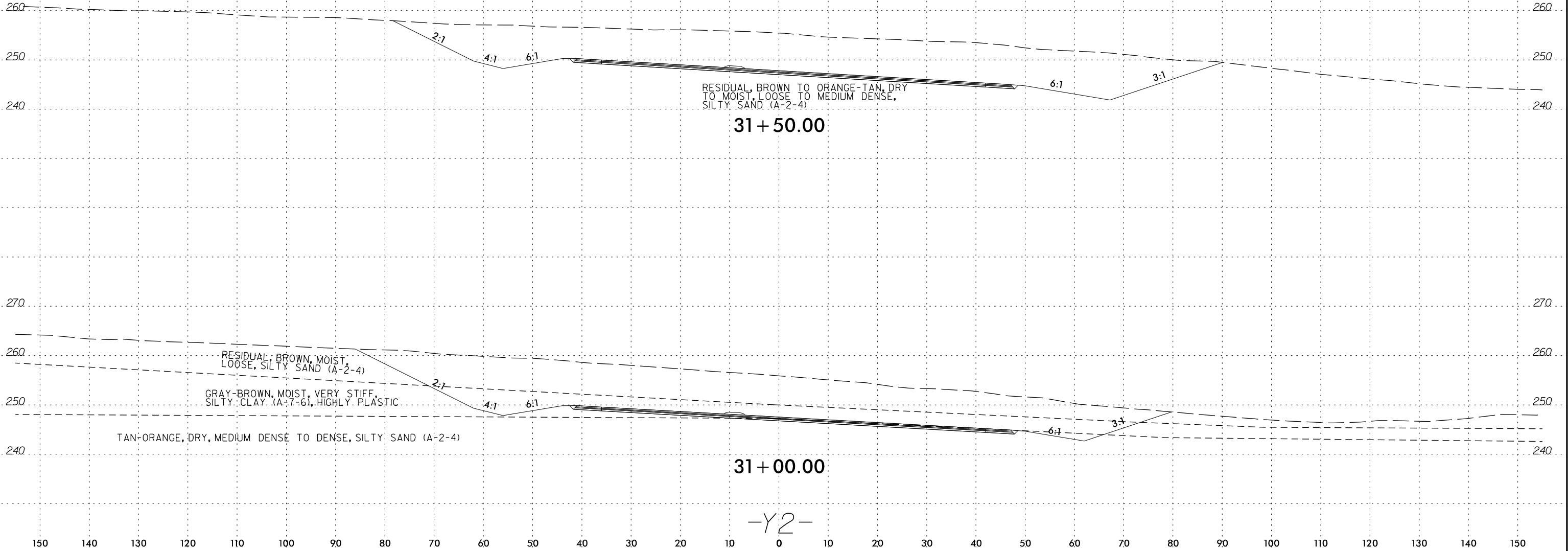


30 + 50.00

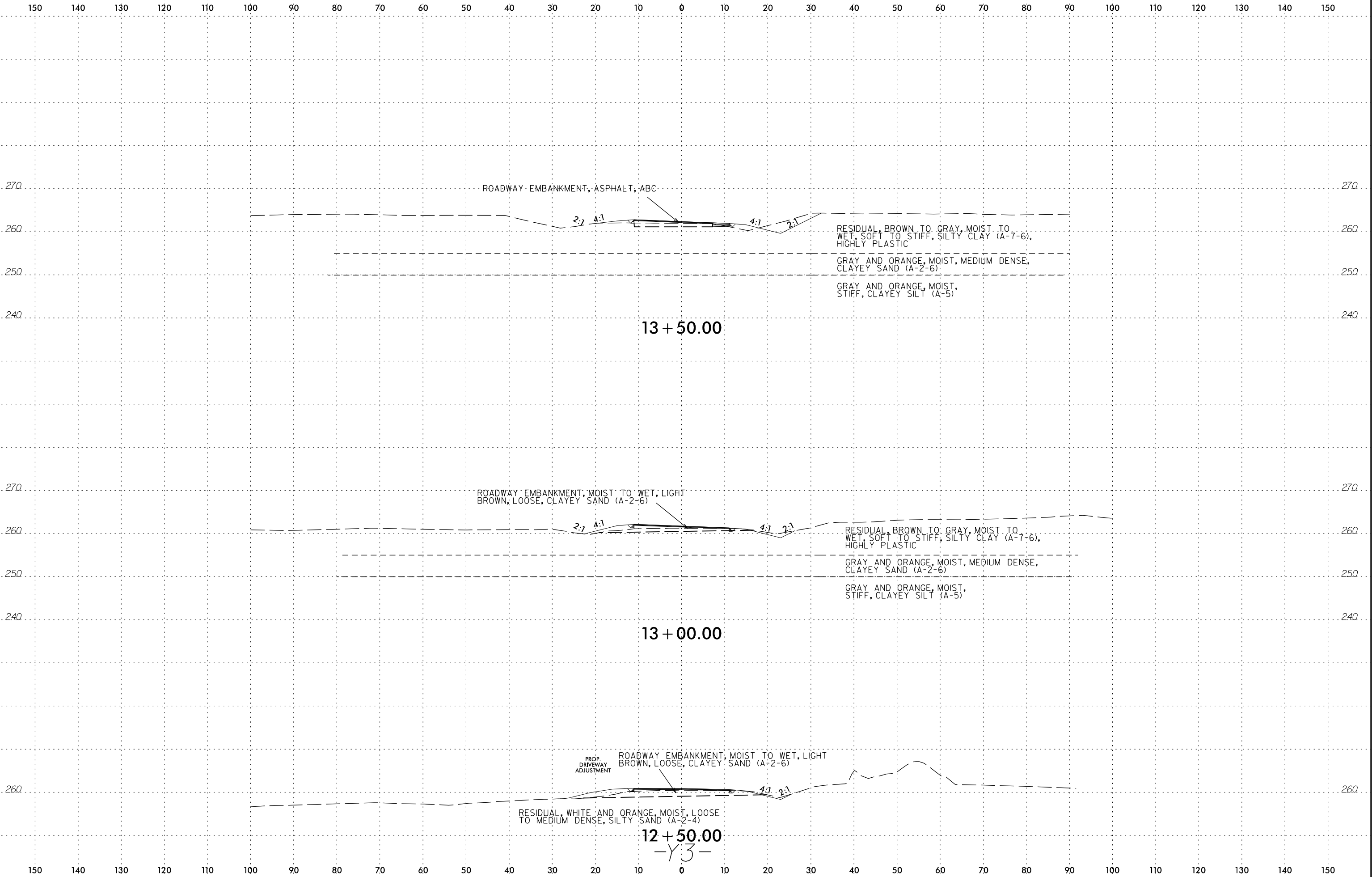
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-Y2-

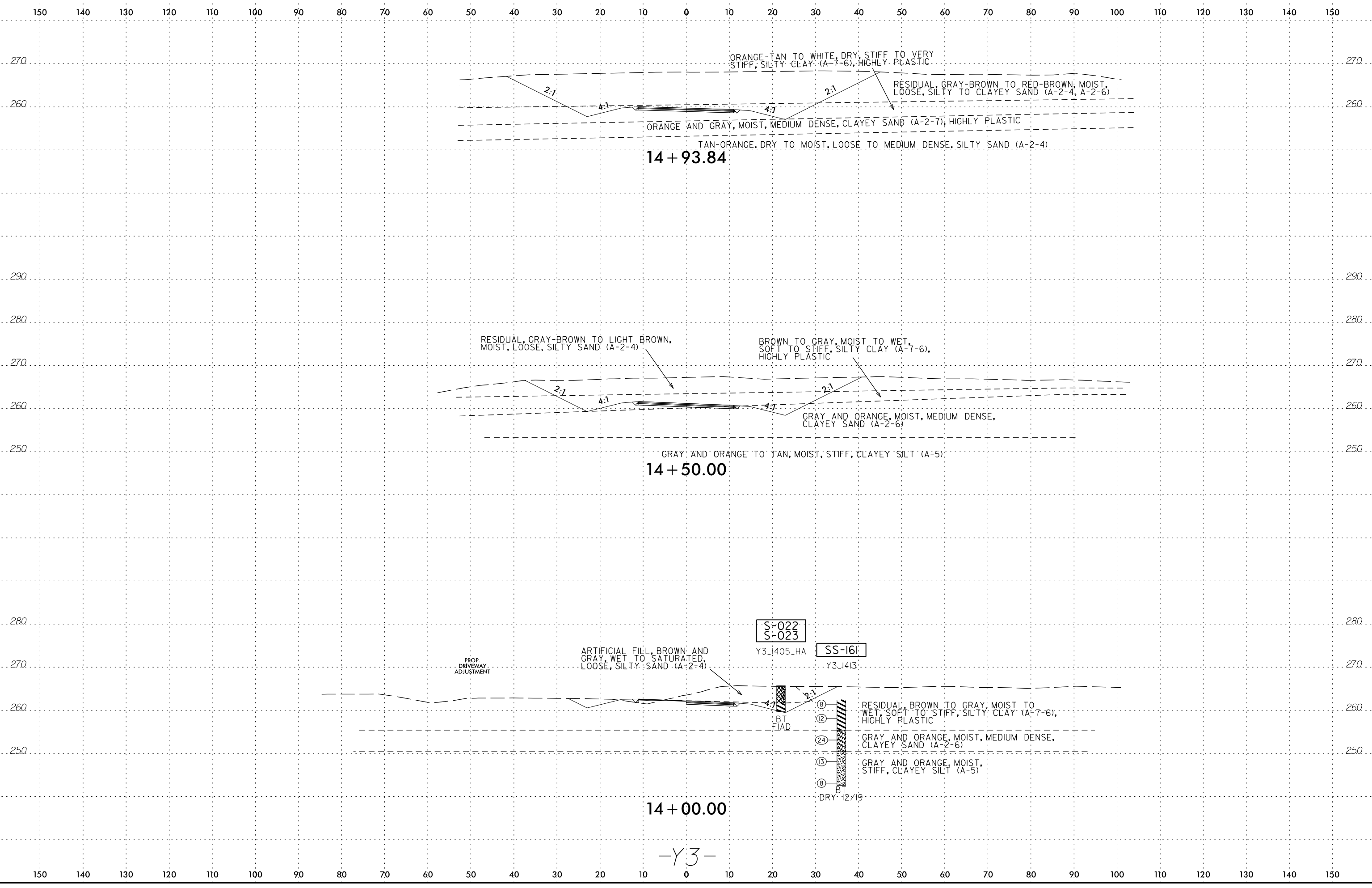
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-Y2-



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 alexander.jozada



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 alexander.bozada

-Y3-

*NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS*

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

***APPENDIX A
SOIL TEST RESULTS***

REFERENCE: U-5748

PROJECT: 50168

SOIL TEST RESULTS

Soil Classification and Gradation

5438 Wade Park Blvd Suite 200, Raleigh, NC 27607

WBS No.: 50168.1.1
 Description: US 401 at Ligon Mill Rd/Mitchell Mill Rd and Perry Creek Rd Intersection Improvements
 Client: NCDOT

County: Wake
 TIP No: U-5748
 Date(s) Tested: 2/3/2020 - 2/11/2022

BORING NO.	SAMPLE NO.	ALIGNMENT	STATION	OFFSET	DEPTH (FT)	AASHTO					% BY WEIGHT				% FINER (SIEVES)			% MOIST.	% ORG.
						CLASS	GI	LL	PL	PI	GRAVEL	C. SAND	F. SAND	FINES	10	40	200		
L_1850_HA	S-013	-L-	18+50	60' LT	1.0-2.5	A-6	2	36	17	19	11.3	23.8	29.3	35.6	88.7	64.9	35.6	14.5	--
L_2050_HA	S-015	-L-	20+50	60' RT	1.0-3.0	A-2-6	1	32	18	14	4.2	34.2	26.3	35.4	95.8	61.7	35.4	15.6	--
L_2050_RT	SS-207	-L-	20+50	122' RT	0.0-1.5	A-4	4	27	17	10	0.1	2.0	30.1	67.9	99.9	97.9	67.9	18.7	--
L_2300	SS-202	-L-	23+00	115' LT	3.8-5.3	A-7-6	4	45	22	23	0.0	0.1	60.3	39.6	100.0	99.9	39.6	25.2	--
L_2300_HA	S-014	-L-	23+00	60' LT	2.0-3.0	A-6	3	33	15	18	5.4	21.7	32.5	40.4	94.6	72.9	40.4	22.3	--
L_2315	SS-211	-L-	23+15	125' RT	4.0-5.5	A-6	4	34	18	16	0.0	0.8	53.6	45.6	100.0	99.2	45.6	23.2	--
L_2650	SS-235	-L-	26+50	120' LT	9.3-10.8	A-4	0	NP	NP	NP	0.1	0.4	50.8	48.8	100.0	99.6	48.8	24.9	--
L_2650_HA	S-018	-L-	26+50	60' RT	0.0-1.5	A-6	1	30	17	13	8.7	28.3	27.1	36.0	91.3	63.0	36.0	7.3	--
L_2842	SS-230	-L-	28+42	105' LT	3.7-5.2	A-6	13	36	16	20	0.1	1.7	25.3	72.9	99.9	98.2	72.9	18.4	3.1
L_2842	SS-229	-L-	28+42	105' LT	0.0-1.5	A-6	1	30	19	11	6.4	21.7	32.3	39.7	93.6	71.9	39.7	17.1	--
L_2850	SS-216	-L-	28+50	115' RT	8.9-10.4	A-7-6	18	41	21	20	0.1	1.3	12.6	86.0	99.9	98.6	86.0	24.7	3.5
L_2850_HA	S-016	-L-	28+50	60' LT	1.0-2.0	A-2-6	0	27	15	12	8.0	29.1	27.6	35.3	92.0	62.9	35.3	17.8	--
L_3050	SS-226	-L-	30+50	125' LT	0.0-1.5	A-2-4	0	21	17	4	8.8	37.1	27.9	26.2	91.2	54.1	26.2	10.9	--
L_3100	SS-243	-L-	31+00	101' RT	4.3-5.8	A-2-7	1	43	24	19	4.7	39.9	22.1	33.4	95.3	55.4	33.4	15.8	--
L_3100	SS-242	-L-	31+00	101' RT	0.0-1.5	A-7-6	12	51	22	29	2.5	21.3	22.2	54.0	97.5	76.2	54.0	14.0	--
L_3200_HA	S-012	-L-	32+00	83' LT	1.0-2.5	A-1-b	0	NP	NP	NP	19.6	31.1	31.9	17.4	80.4	49.4	17.4	9.0	--
L_3275	SS-192	-L-	32+75	53' LT	0.0-1.5	A-2-4	0	27	18	9	8.7	33.8	29.4	28.0	91.3	57.5	28.0	14.3	--
L_3500	SS-261	-L-	35+00	198' RT	0.0-1.5	A-6	2	37	18	19	10.1	29.1	23.6	37.2	89.9	60.9	37.2	16.4	--
L_3556	SS-256	-L-	35+56	63' RT	0.0-1.5	A-6	1	33	18	15	6.6	39.3	28.3	35.8	103.4	64.1	35.8	17.6	--
L_3597	SS-191	-L-	35+97	97' LT	13.4-14.9	A-2-4	0	NP	NP	NP	4.6	35.9	31.5	28.0	95.4	59.5	28.0	20.0	--
L_3627	SS-186	-L-	26+27	110' LT	3.3-4.8	A-2-4	0	28	18	10	13.2	33.6	24.8	28.4	86.8	53.2	28.4	13.6	--
L_3743	SS-273	-L-	37+43	145' RT	0.0-1.5	A-1-b	0	NP	NP	NP	8.5	44.1	27.6	19.8	91.5	47.4	19.8	16.3	--
L_3900_LT	SS-183	-L-	39+00	85' LT	0.0-1.5	A-2-6	0	33	17	16	6.9	35.6	27.8	29.8	93.1	57.5	29.8	15.4	--
L_3900_RT	SS-280	-L-	39+00	150' RT	0.0-1.5	A-4	0	35	30	5	2.3	26.5	32.2	39.0	97.7	71.2	39.0	60.2	--
L_4110	SS-179	-L-	41+00	95' LT	0.0-1.5	A-7-6	15	60	27	33	2.5	22.1	20.1	55.4	97.5	75.4	55.4	19.0	--
L_4215	SS-293	-L-	42+15	125' RT	0.0-1.5	A-2-4	0	29	24	5	4.2	35.5	28.5	31.8	95.8	60.3	31.8	18.4	--
L_4300_RT	SS-300	-L-	43+00	110' RT	3.7-5.2	A-2-6	0	40	24	16	8.5	42.0	23.3	26.2	91.5	49.5	26.2	20.7	--
L_4410	SS-305	-L-	44+10	110' RT	4.2-5.7	A-1-b	0	NP	NP	NP	7.5	48.9	25.1	18.5	92.5	43.6	18.5	15.1	--
L_4500	SS-173	-L-	45+00	50' LT	0.0-1.5	A-7-6	6	53	25	28	4.6	37.1	17.8	40.6	95.4	58.4	40.6	20.1	--
L_4700_LT	SS-170	-L-	47+00	103' LT	0.0-1.5	A-2-4	0	NP	NP	NP	4.3	41.5	29.3	24.9	95.7	54.2	24.9	12.4	--
L_4800	SS-315	-L-	48+00	95' RT	0.0-1.5	A-2-4	0	14	11	3	1.7	45.1	25.6	27.7	98.4	53.2	27.7	11.0	--
L_4892	SS-122	-L-	48+92	110' LT	0.0-1.5	A-1-b	0	NP	NP	NP	4.9	47.5	26.0	21.7	95.2	47.6	21.7	9.6	--
L_5000	SS-324	-L-	50+00	102' RT	4.6-5.6	A-7-5	9	65	31	34	2.3	31.1	24.0	42.6	97.7	66.6	42.6	21.6	--
L_5050	SS-119	-L-	50+50	84' LT	4.1-5.6	A-2-7	1	43	23	20	5.5	35.7	26.8	32.0	94.5	58.8	32.0	9.1	--
L_5200_LT	SS-116	-L-	52+00	108' LT	3.5-5.0	A-7-6	5	48	28	20	3.7	27.7	25.4	43.3	96.3	68.7	43.3	19.8	--

SOIL TEST RESULTS

Soil Classification and Gradation

5438 Wade Park Blvd Suite 200, Raleigh, NC 27607

WBS No.: 50168.1.1
 Description: US 401 at Ligon Mill Rd/Mitchell Mill Rd and Perry Creek Rd Intersection Improvements
 Client: NCDOT

County: Wake
 TIP No: U-5748
 Date(s) Tested: 2/3/2020 - 2/11/2022

BORING NO.	SAMPLE NO.	ALIGNMENT	STATION	OFFSET	DEPTH (FT)	AASHTO					% BY WEIGHT				% FINER (SIEVES)			% MOIST.	% ORG.
						CLASS	GI	LL	PL	PI	GRAVEL	C. SAND	F. SAND	FINES	10	40	200		
L_5200_RT	SS-328	-L-	52+00	100' RT	4.5-6.0	A-7-6	8	53	26	27	3.0	30.0	20.9	46.1	97.0	67.0	46.1	19.6	--
L_5320	SS-355	-L-	53+20	120' RT	3.5-5.0	A-7-6	3	44	25	19	13.0	32.1	17.6	37.3	87.0	54.9	37.3	17.7	--
L_5373	SS-113	-L-	53+73	83' LT	14.3-15.8	A-1-b	0	NP	NP	NP	11.3	39.7	26.4	22.6	88.7	49.1	22.6	11.9	--
L_5373	SS-111	-L-	53+73	83' LT	4.3-5.8	A-7-6	11	56	24	32	1.6	26.1	25.1	47.2	98.4	72.3	47.2	20.0	--
L_5483	SS-106	-L-	54+83	41' LT	8.7-10.2	A-7-6	9	56	24	32	5.5	29.4	20.4	44.8	94.6	65.2	44.8	20.3	--
L_5483	SS-107	-L-	54+83	41' LT	13.7-15.2	A-7-6	6	42	15	27	1.7	34.5	21.6	42.2	98.3	63.8	42.2	11.0	--
L_5547	SS-334	-L-	55+47	78' RT	9.5-10.5	A-7-6	7	53	25	28	5.2	31.3	19.8	43.7	94.8	63.5	43.7	19.7	--
L_5680	SS-101	-L-	56+80	37' LT	0.0-1.5	A-2-6	0	32	17	15	13.2	40.3	18.3	28.1	86.8	46.4	28.1	15.7	--
L_5900_HA	S-010	-L-	59+00	37' RT	0.0-0.5	A-6	1	33	19	14	7.3	35.4	21.5	35.8	92.7	57.3	35.8	14.7	--
L_5913	SS-093	-L-	59+13	95' LT	0.0-1.5	A-7-6	14	42	15	27	3.0	20.3	14.9	61.9	97.0	76.7	61.9	21.2	--
L_5995	SS-099	-L-	59+95	107' LT	8.7-10.2	A-2-4	0	28	24	4	7.7	37.7	30.2	24.5	92.3	54.7	24.5	5.7	--
L_6200_HA	S-009	-L-	62+00	36' RT	4.5-6.0	A-2-4	0	NP	NP	NP	17.0	22.8	45.7	14.5	83.1	60.2	14.5	11.3	--
L_6223	SS-086	-L-	62+23	85' LT	14.2-15.7	A-2-4	0	32	26	6	9.9	35.3	28.2	26.6	90.1	54.8	26.6	16.1	--
L_6223	SS-083	-L-	62+23	85' LT	0.0-1.5	A-7-6	5	48	23	25	4.6	28.3	26.6	40.5	95.4	67.0	40.5	21.9	--
L_6385	SS-068	-L-	63+85	95' LT	4.0-5.5	A-2-4	0	21	14	7	11.6	40.9	21.5	26.1	88.4	47.6	26.1	8.8	--
L_6400_HA	S-008	-L-	64+00	38' RT	0.0-2.5	A-2-6	1	36	19	17	27.9	19.6	19.4	33.1	72.1	52.5	33.1	11.3	--
L_6480	SS-059	-L-	64+80	97' LT	3.4-4.9	A-2-5	0	45	35	10	10.0	35.2	23.8	31.1	90.0	54.9	31.1	25.5	--
L_6600_HA	S-006	-L-	66+00	29' RT	1.0-2.0	A-2-6	0	28	16	12	14.9	29.7	24.0	31.5	85.2	55.5	31.5	10.1	--
L_6600_LT	SS-049	-L-	66+00	96' LT	4.1-5.6	A-7-6	4	46	23	23	3.0	34.0	25.0	38.0	97.0	63.0	38.0	13.3	--
L_6700	SS-044	-L-	67+00	91' LT	0.0-1.5	A-7-6	12	61	21	40	3.2	24.5	27.7	44.6	96.8	72.3	44.6	16.7	--
L_6700	SS-045	-L-	67+00	91' LT	4.1-5.9	A-7-6	6	46	22	24	4.5	24.6	28.0	43.0	95.5	71.0	43.0	13.7	--
L_6800_HA	S-005	-L-	68+00	31' RT	4.0-5.0	A-2-4	0	NP	NP	NP	16.1	25.4	43.2	15.3	83.9	58.5	15.3	9.2	--
L_7000	SS-031	-L-	70+00	93' LT	0.0-1.5	A-7-6	5	42	20	22	3.7	24.3	29.6	42.3	96.3	72.0	42.3	21.9	--
L_7170	SS-028	-L-	71+70	90' LT	0.0-1.5	A-1-b	0	21	17	4	23.7	32.3	26.3	17.7	76.3	44.0	17.7	7.9	--
L_7300_HA	S-002	-L-	73+00	81' RT	0.0-1.5	A-2-6	1	35	19	16	17.1	28.4	21.0	33.5	82.9	54.5	33.5	17.5	--
L_7392	SS-025	-L-	73+92	91' LT	14.1-15.6	A-4	0	29	22	7	1.2	31.8	30.6	36.4	98.8	67.0	36.4	12.3	--
L_7450_HA	S-001	-L-	74+50	0' CL	1.0-3.0	A-2-4	0	NP	NP	NP	16.9	24.3	44.2	14.6	83.1	58.8	14.6	7.4	--
L_7595	SS-018	-L-	75+95	100' LT	3.7-5.2	A-7-6	12	60	25	35	0.7	26.7	24.8	47.8	99.3	72.6	47.8	17.3	--
L_7700_LT	SS-015	-L-	77+00	92' LT	3.7-5.2	A-7-6	28	74	26	48	0.9	20.0	17.4	61.7	99.1	79.1	61.7	26.6	--
L_7800	SS-012	-L-	78+00	95' LT	3.9-5.4	A-2-6	0	39	25	14	2.9	36.6	28.6	31.9	97.1	60.5	31.9	27.3	--
L_7900_RT	SS-345	-L-	79+00	83' RT	0.0-1.5	A-7-5	38	75	30	45	0.4	11.8	10.7	77.1	99.6	87.8	77.1	29.5	--
L_8108	SS-008	-L-	81+08	115' LT	3.5-5.0	A-7-6	12	56	29	27	0.7	19.1	25.1	55.0	99.3	80.2	55.0	21.0	--
L_8280	SS-352	-L-	82+80	100' RT	5.3-5.8	A-7-6	4	50	21	29	3.8	39.0	18.4	38.8	96.2	57.2	38.8	18.7	--
L_8450_HA	S-031	-L-	84+50	70' RT	0.5-1.0	A-2-6	0	35	21	14	17.3	32.3	25.9	24.6	82.7	50.5	24.6	12.9	--
L_8498	SS-002	-L-	84+98	108' LT	3.7-5.2	A-7-6	5	50	27	23	3.1	28.1	28.4	40.3	96.9	68.7	40.3	25.0	--

SOIL TEST RESULTS

Soil Classification and Gradation

5438 Wade Park Blvd Suite 200, Raleigh, NC 27607

WBS No.: 50168.1.1
 Description: US 401 at Ligon Mill Rd/Mitchell Mill Rd and Perry Creek Rd Intersection Improvements
 Client: NCDOT

County: Wake
 TIP No: U-5748
 Date(s) Tested: 2/3/2020 - 2/11/2022

BORING NO.	SAMPLE NO.	ALIGNMENT	STATION	OFFSET	DEPTH (FT)	AASHTO					% BY WEIGHT				% FINER (SIEVES)			% MOIST.	% ORG.
						CLASS	GI	LL	PL	PI	GRAVEL	C. SAND	F. SAND	FINES	10	40	200		
L1_2475_LT	SS-007	-L1-	24+75	74 LT	0.0-1.5	A-1-b	0	0	0	0	33.4	22.6	25.7	18.3	66.6	44.1	18.3	15.5	--
L1_2494	SS-023	-L1-	24+94	2 LT	0.0-2.0	A-6	5	38	19	19	2.1	28.6	24.5	44.8	97.9	69.3	44.8	18.3	--
L1_2648_RT	SS-011	-L1-	26+48	65 RT	0.0-1.5	A-6	4	36	17	19	0.7	27.4	27.8	44.1	99.3	71.9	44.1	15.8	--
L1_2987	SS-026	-L1-	29+87	6 RT	0.0-4.0	A-7-6	13	59	27	32	0.2	26.0	22.8	51.0	99.8	73.8	51.0	21.7	--
L1_3199	SS-027	-L1-	31+99	2 LT	0.0-3.6	A-7-7	7	47	22	25	0.9	28.6	25.5	44.9	99.1	70.5	44.9	18.8	--
L1_4701	SS-021	-L1-	47+01	2 LT	0.0-1.0	A-6	3	36	19	17	0.7	23.7	34.4	41.3	99.3	75.7	41.3	21.2	--
L1_4946_LT	S-004	-L1-	49+46	70 LT	8.7-10.2	A-2-4	0	0	0	0	0.7	35.6	31.9	31.9	99.3	63.8	31.9	16.3	--
L1_5002	SS-018	-L1-	50+02	4 LT	1.0-4.0	A-2-4	0	0	0	0	7.7	34.0	32.1	26.3	92.3	58.4	26.3	16.5	--
L1_5644_RT	SS-015	-L1-	56+44	70 RT	3.8-5.2	A-7-6	9	54	28	26	0.9	28.6	25.5	44.9	99.1	70.5	44.9	24.5	--
L1_5897	SS-015	-L1-	58+97	1 RT	0.0-4.0	A-2-6	1	35	16	19	4.4	33.9	29.3	32.3	95.6	61.7	32.3	16.5	--
Y1_1300	SS-257	-Y1-	13+00	90' LT	0.0-1.5	A-6	2	37	19	18	5.5	34.0	22.4	38.1	94.5	60.5	38.1	15.1	--
Y1_1300	SS-259	-Y1-	13+00	90' LT	9.0-10.5	A-7-6	12	59	23	36	2.5	24.5	25.0	48.1	97.6	73.1	48.1	16.4	--
Y2_1250_HA	S-028	-Y2-	12+50	13' LT	1.0-1.5	A-2-4	0	20	13	7	1.3	31.7	35.1	31.9	98.7	67.0	31.9	15.4	--
Y2_1450_HA	S-029	-Y2-	14+50	14' RT	1.0-1.5	A-6	2	35	18	17	3.4	32.5	27.7	36.4	96.6	64.1	36.4	11.3	--
Y2_1600_HA	S-026	-Y2-	16+00	6' LT	1.0-2.5	A-2-6	0	28	15	13	4.1	33.0	34.2	28.7	95.9	62.9	28.7	24.0	--
Y2_1859	SS-166	-Y2-	18+59	11' LT	8.7-10.2	A-2-7	5	59	23	36	6.5	28.9	29.6	34.9	93.5	64.5	34.9	11.0	--
Y2_2050	SS-158	-Y2-	20+50	33' LT	13.0-14.5	A-2-4	0	35	30	5	6.3	35.7	26.4	31.6	93.8	58.1	31.6	16.2	--
Y2_2050	SS-156	-Y2-	20+50	33' LT	3.0-4.5	A-7-6	4	50	24	26	2.1	32.5	27.9	37.5	98.0	65.4	37.5	13.5	--
Y2_2430	SS-145	-Y2-	24+30	0' CL	0.0-1.5	A-2-4	0	NP	NP	NP	7.2	40.1	32.8	19.9	92.8	52.8	19.9	8.9	--
Y2_2650	SS-141	-Y2-	26+50	0' CL	4.3-5.8	A-7-6	6	51	22	29	2.8	33.8	22.5	40.9	97.2	63.4	40.9	13.3	--
Y2_2850	SS-135	-Y2-	28+50	0' CL	3.6-5.1	A-7-6	8	55	25	30	2.6	30.3	23.8	43.2	97.4	67.0	43.2	15.1	--
Y2_3050	SS-131	-Y2-	30+50	0' CL	13.5-15.0	A-2-4	0	33	25	8	8.0	34.3	27.0	30.7	92.0	57.7	30.7	12.5	--
Y2_3050	SS-129	-Y2-	30+50	0' CL	3.5-5.0	A-7-6	15	63	25	38	1.1	24.1	23.7	51.1	98.9	74.8	51.1	22.6	--
Y3_1200_HA	S-024	-Y3-	12+00	13' RT	1.0-2.5	A-2-6	0	27	15	12	14.3	36.6	21.4	27.6	85.7	49.1	27.6	13.1	--
Y3_1405_HA	S-022	-Y3-	14+05	22' RT	1.0-2.5	A-2-4	0	27	17	10	10.7	28.1	30.5	30.7	89.3	61.2	30.7	20.0	--
Y3_1405_HA	S-023	-Y3-	14+05	22' RT	4.0-5.5	A-7-6	49	78	26	52	0.5	7.8	6.6	85.2	99.6	91.7	85.2	33.4	--
Y3_1413	SS-161	-Y3-	14+13	36' RT	3.3-4.8	A-7-6	4	43	23	20	2.9	31.3	23.5	42.3	97.1	65.8	42.3	--	--
Y4_1100_HA	S-032	-Y4-	11+00	15' RT	1.0-2.0	A-6	3	40	21	19	8.5	26.3	26.3	38.9	91.5	65.2	38.9	10.4	--
Y4_1300_HA	S-021	-Y4-	13+00	23' LT	1.0-2.5	A-7-6	5	54	27	27	2.6	29.3	29.6	38.5	97.5	68.2	38.5	18.2	--
Y4_1500_HA	S-019	-Y4-	15+00	25' LT	1.0-2.5	A-2-4	0	20	17	3	12.6	29.5	26.6	31.3	87.4	57.9	31.3	16.0	--

TESTED BY: Michael P. Sum

NCDOT No.: 129-03-0411

*NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS*

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

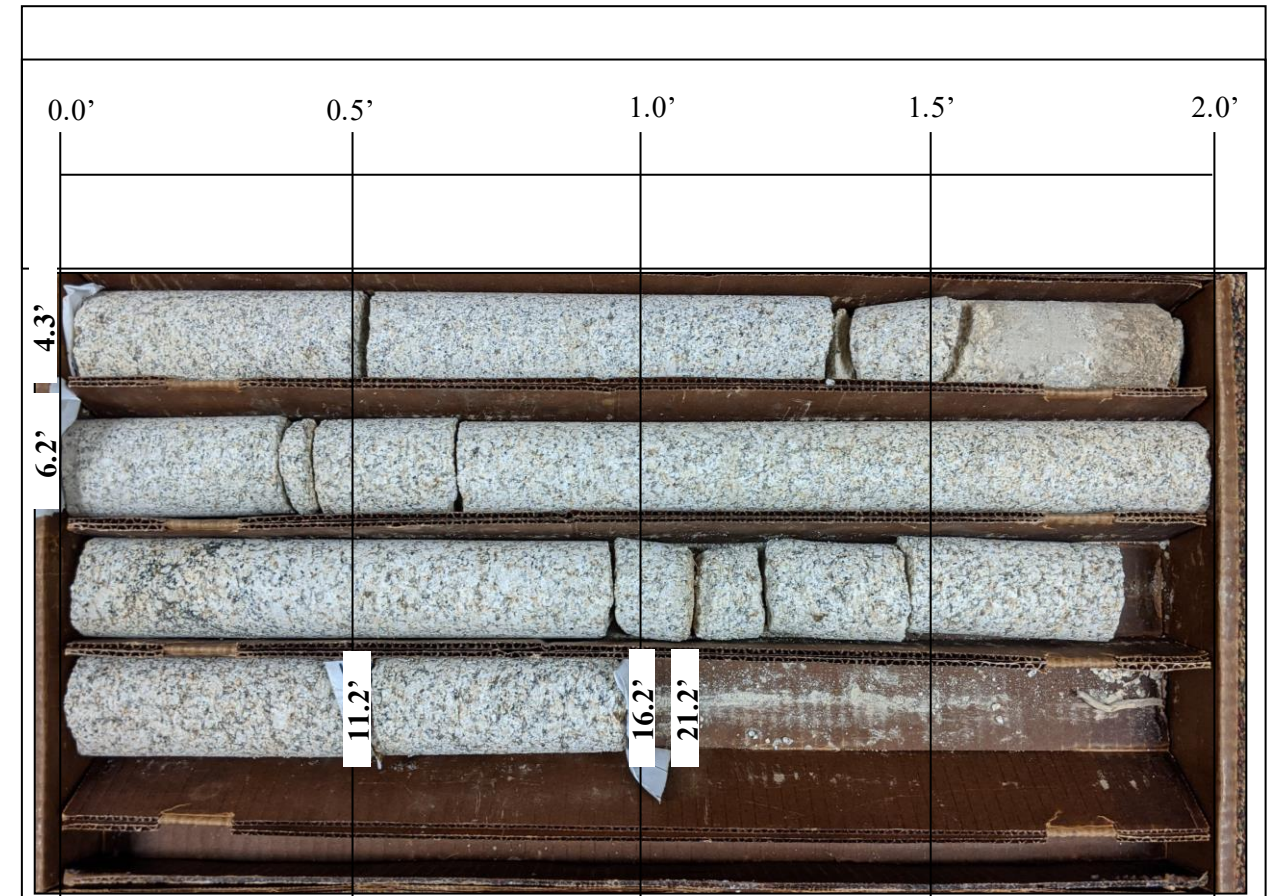
***APPENDIX B
ROCK CORE PHOTOGRAPHS***

REFERENCE: U-5748

PROJECT: 50168



Sta. 32+48 -L-, 139-ft RT, Box 1 of 1, 26.8-ft to 36.8-ft



Sta. 71+00 -L-, 92-ft LT, Box 1 of 1, 4.3-ft to 21.2-ft

SCALE 1:40 (1"=4")

ROCK CORE PHOTOGRAPHS

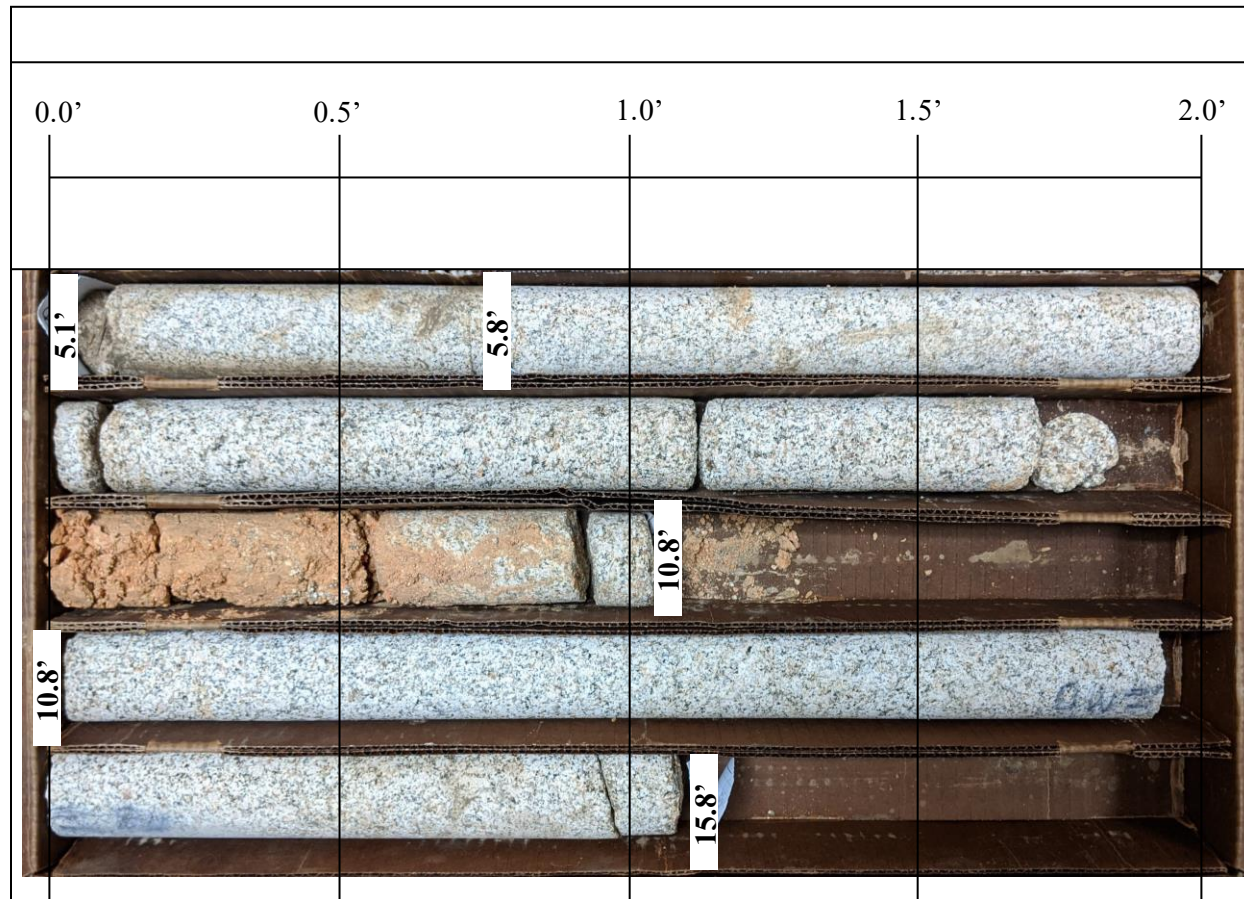
**U.S. 401 AND LIGON MILL-MITCHELL MILL ROAD
IMPROVEMENTS**

WAKE COUNTY, NORTH CAROLINA

WBS NO.: 50168.1.1, TIP NO.: U-5748

AECOM

AECOM - North Carolina
1600 Perimeter Park Drive, Suite 400
Morrisville, NC 27560
Tel: 919-461-1100 Fax: 919-46-1415



Sta. 75+00 -L-, 110-ft LT, Box 1 of 1, 5.1-ft to 15.8-ft

SCALE 1:40 (1"=4")

ROCK CORE PHOTOGRAPHS

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