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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY WAKE

PROJECT DESCRIPTION BRIDGE NO. 126 ON -L-(LIGON MILL ROAD) OVER SMITH CREEK BETWEEN MAIN STREET AND US 401

STATE PROJECT REFERENCE NO. B-531816

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR NSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IMP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NINCLATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MOY LAVE CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:

1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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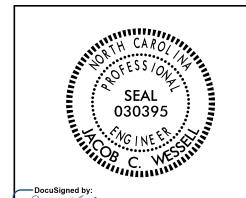
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DATE __**JULY 2021**



7/28/2021

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PROJECT REPERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, ASAFTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOUL YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AOUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF, GRAV, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS, (≤ 35; PASSING *200) (> 35; PASSING *200) (> 35; PASSING *200)	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	WEATHERED ROCK (WR) NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 1000 BLOWS PER FOOT IF TESTED. CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-2-6 A-2-7 A-3 A-6, A-7 D000000000000000000000000000000000000	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	ROCK (CR) ONEISS, GABRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) ROCK (NCR) ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	- <u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <u>COLLUYIUM</u> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPIT REFUSAL ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC. WEATHERING	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
288 15 MX 25 MX 18 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN SOMN 36 MN 36	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS SOILS TRACE OF ORGANIC MATTER 2 - 3%, 3 - 5%, TRACE 1 - 10%, LITTLE ORGANIC MATTER 3 - 5%, 5 - 12%, LITTLE 10 - 20%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LL 48 MX 41 MN 18 MX 11 MN MODERATE HIGHLY GROWN AND AN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE GROUND WATER	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	<u>OIP DIRECTION (OIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. <u>FAULT</u> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. OF MAJOR GRAYEL, AND MATERIALS SAND GRAYEL AND SAND SOILS SOILS MATERIALS SAND GRAYEL AND SAND SOILS SOILS	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS ■ MATER LEVEL AFTER 25 HOURS ■	(SLI.) 1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS, IN	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN.RATING	✓ PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA ○-∭ SPRING OR SEEP	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	MISCELLANEOUS SYMBOLS ROADWAY EMBANKMENT (RE) 25/02/5 DIP & DIP DIRECTION DIP & DIP DIRECTION	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
CUNSISTENLY (N-VALUE) (TONS/FT ²) GENERALLY VERY LOOSE (4 TO 10 GRANULAR MEDIUM DENSE 10 TO 30 N/A MATERIAL DENSE 30 TO 50	WITH SOIL DESCRIPTION OF ROCK STRUCTURES SOIL SYMBOL OF ROCK STRUCTURES SLOPE INDICATOR INSTALLATION ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER TEST	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRAVITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NUN-CUHESIVE) VERY DENSE > 50 VERY SOFT < 2	INFERRED SOIL BOUNDARY	VERY ALL ROCK EXCEPT QUARITZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	MONITORING WELL INFERRED ROCK LINE MONITORING WELL IEST BURING WITH CORE INSTALLATION SPT N-VALUE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053 BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - EXCEPTABLE BUT NOT TO BE UNCLASSIFIED EXCAVATION - EXCEPTABLE DEGRADABLE ROCK UNDERCUT UNDERCUT UNDERCUT UNDERCUT UNDERCUT UNDERCUT ON TO BACKFILL	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	ABBREVIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 7/6 - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION - SATURATED - USUALLY LIQUID; VERY WET, USUALLY	CSE COARSE	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	STRATA CORE RECOVERY (SPEC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK OUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN A INCHES DIVIDED BY
PLASTIC RANGE - WET - (W) SEMISOLID: REQUIRES DRYING TO	F - FINE SL SILT, SILTY ST - SHELBY TUBE FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
(PI) PLPLASTIC LIMITATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	BENCH MARK: BM-2, N 793495 E 2139827 -BL- STA 14+65.00 OFFSET 76'LT, R/R SPIKE IN II" GUM
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 201.04 FEET
SL SHRINKAGE LIMIT	ORILL UNITS: ADVANCING TOOLS: HAMMER TYPE: X CME-45C	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.003 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES: FIAD = FILLED IN AFTER DRILLING
ATTAIN UPTIMUM MUISTURE	CME-55 CONTINUOUS FLIGHT AUGER CORE SIZE: 8 HOLLOW AUGERS -B -H -H	THINLY LAMINATED < 0.008 FEET INDURATION	-
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	1
NON PLASTIC 0-5 VERY LOW	TUNG,-CARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS: FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	VANE SHEAR TEST X CASING W/ ADVANCER HAND TOOLS: POST HOLE DIGGER POST HOLE DIGGER HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; INDURATED DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY), MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1:

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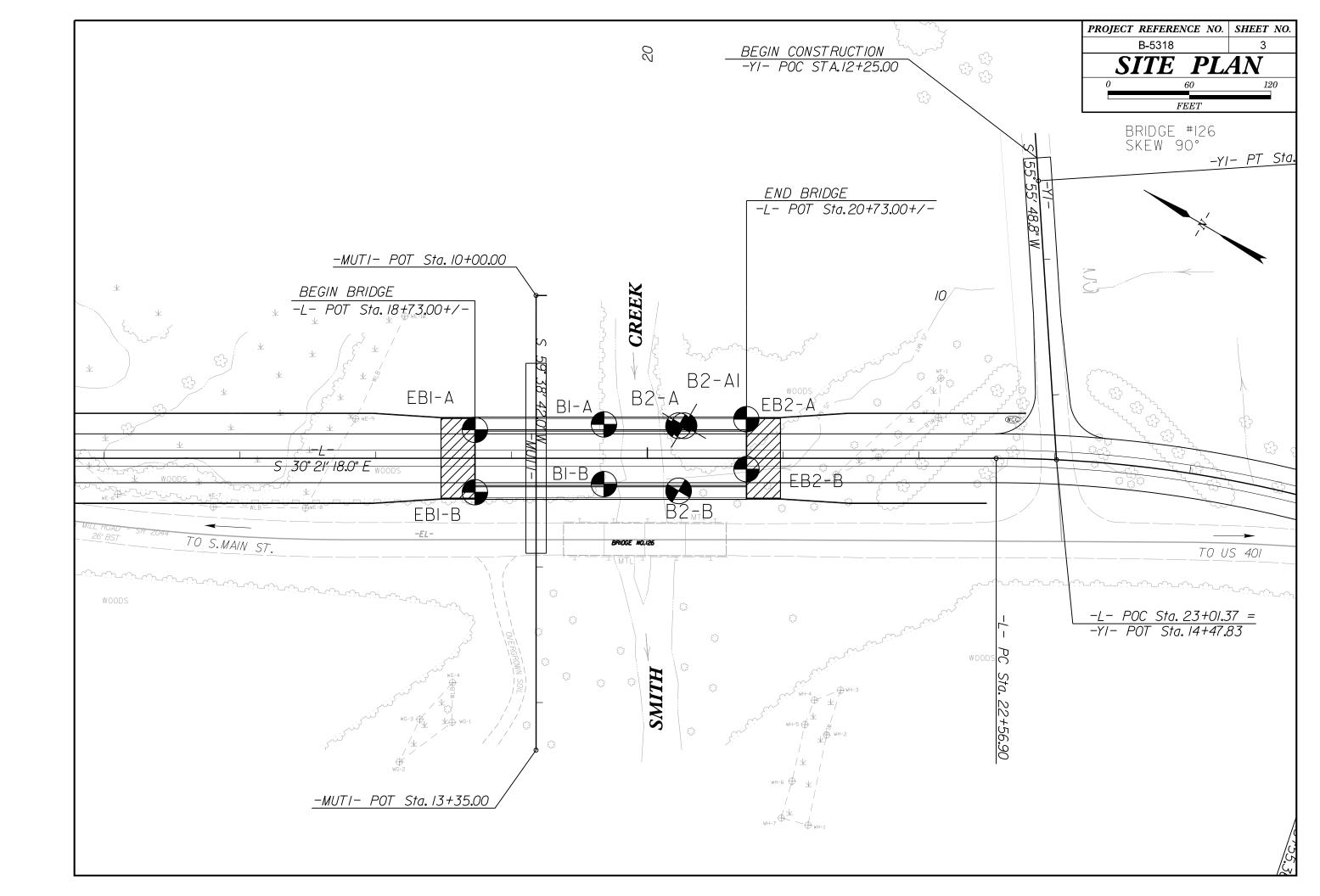
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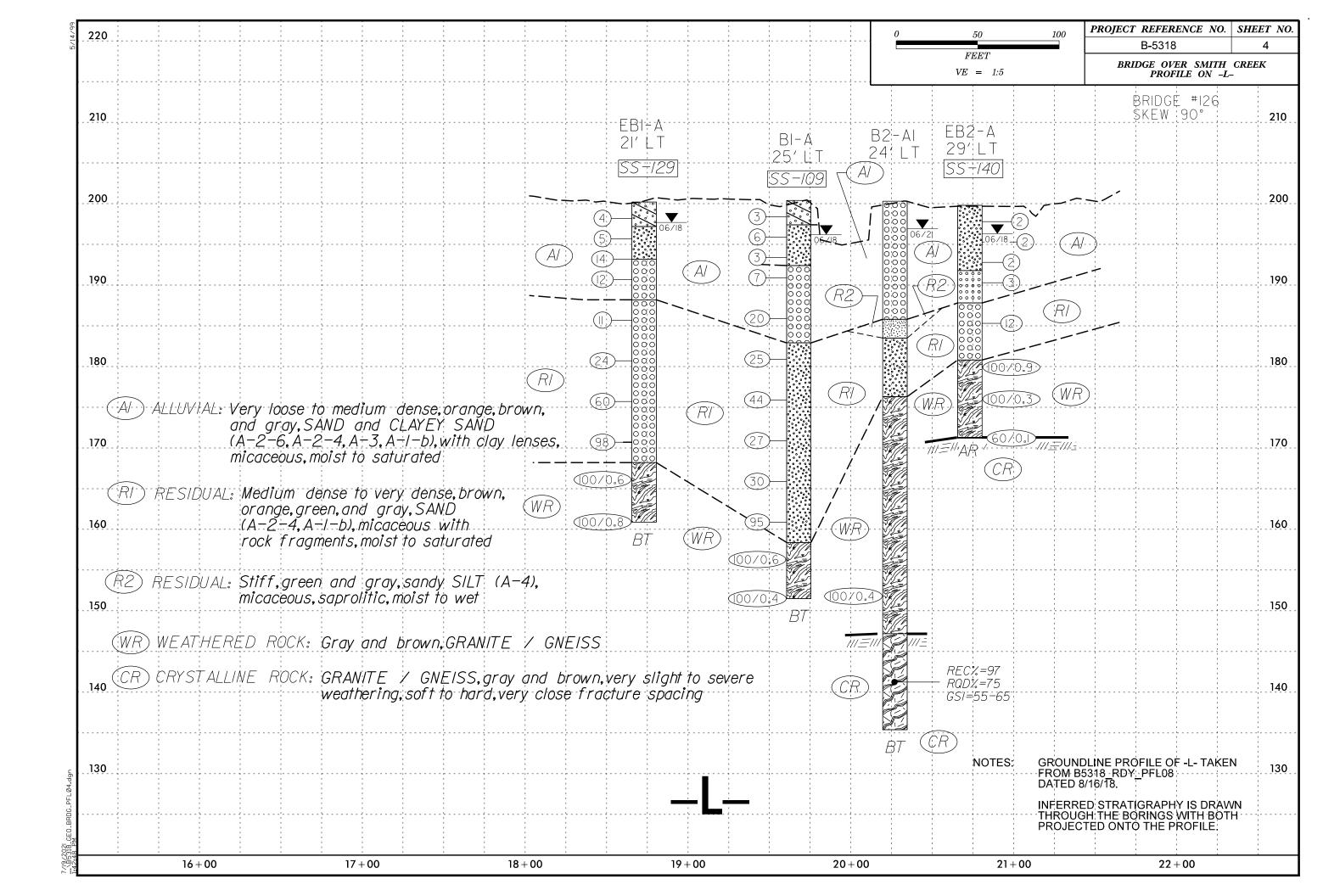
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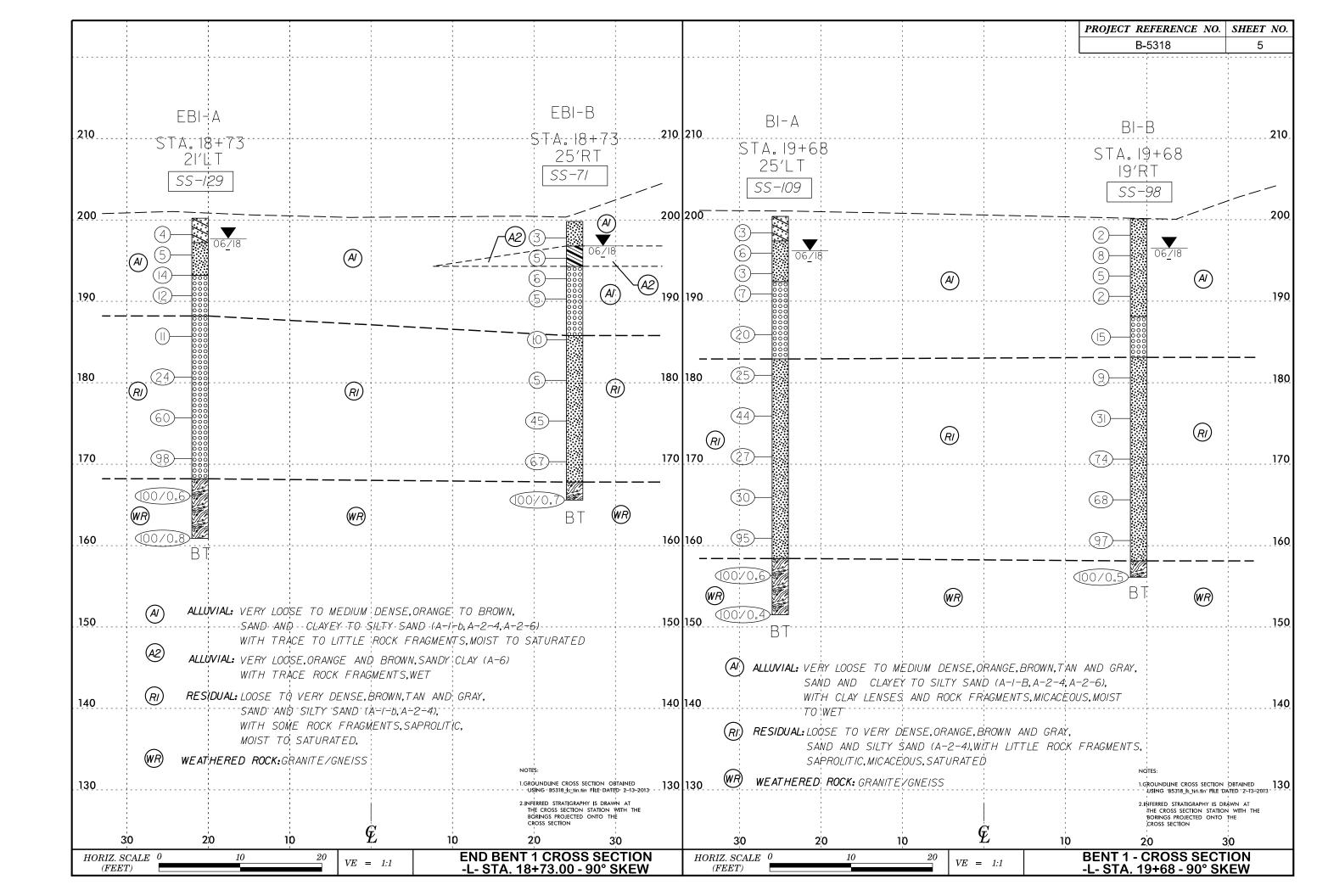
SUBSURFACE INVESTIGATION

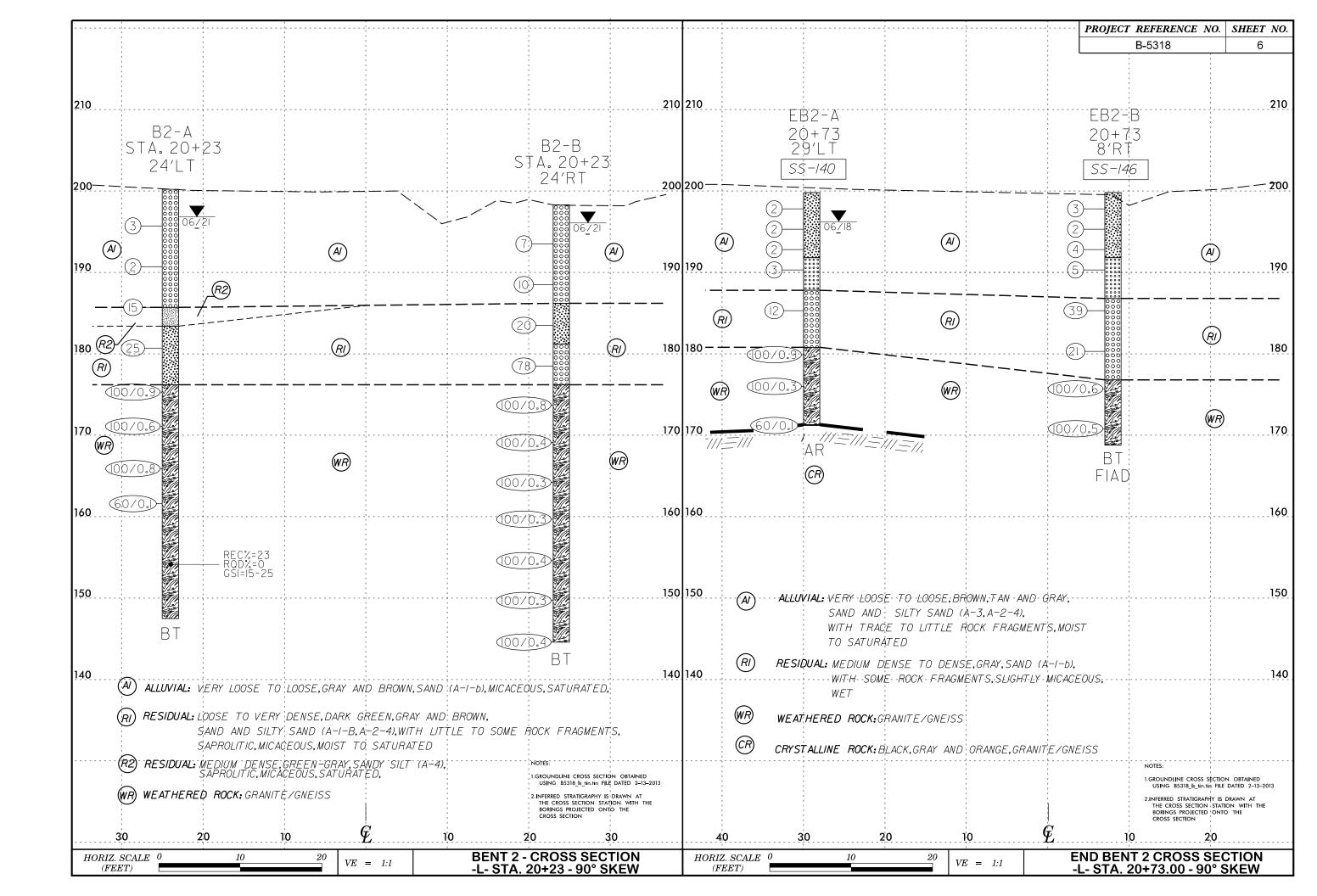
SUPPLEMENTAL LEGEND GEOLOGICAL STRENGTH INDEX (GSI) TABLES

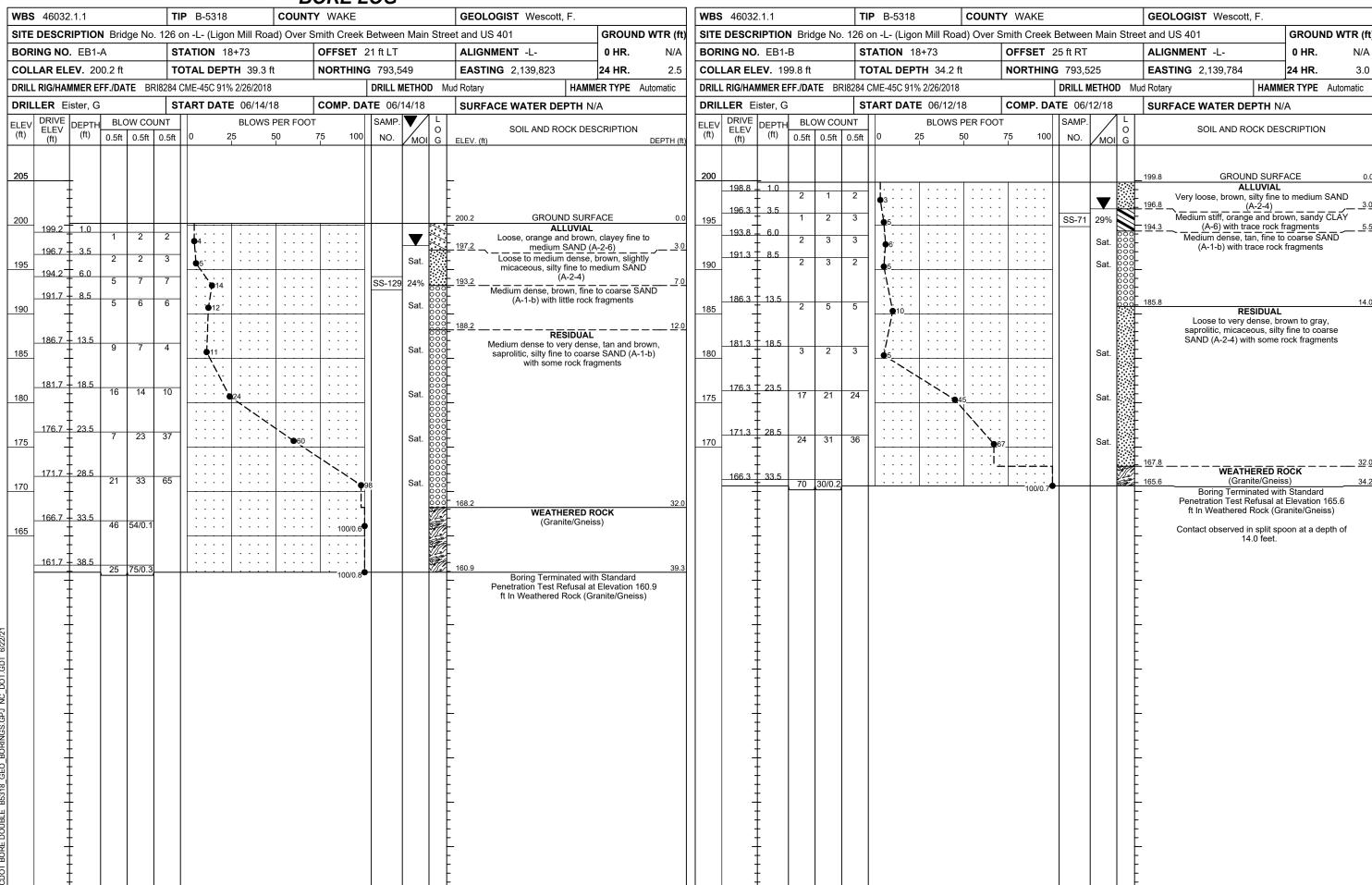
	S	UPPLEME FRO	ENTAL LI OM AAS	EGEND, G HTO LR	EOLOGIC FD BRID	AL STRENGTH INDEX (GSI) TABLES GE DESIGN SPECIFICATIONS
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	nted Rock Mass (Marı	nos and Hoek, 2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)	f oces	p		faces	S O O	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS VERY GOOD Very rough, fresh unweathered surface	GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surface with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surface with soft clay coatings or fillings	Exercise of the distriction of the lithology, structure and surface conditions (barticularly of the pedding planes), choose a pox in the chart. Tocate the bosition in the pox that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too brease. Growing a range from 33 to 31 is more realistic than giving the presence of dillings with moderately slightly weathered couting a surface south, and altered surfaces with comply the presence of dillings with conditions. WERY GOOD - Rough, slightly weathered couting weathered surfaces with conditions with any of total and this can be allowed for by a slight shift to the right in the columns to team, slickens of surface of surface of surface of surface of surface of total and the search of the playly weathers and this can be allowed for by a slight to the right in the columns for the same of the playly of weathers and this dead the value of Surface of the playly of weathers and this can be allowed for by a slight of weathers and this dead the value of Surface of the playly of weathers and the same of the playly of the playly of weathers and the same of the playly of weathers and the same of the playly of weathers and the same of the playly of
STRUCTURE	DEC	REASING SU	JRFACE QU	,	⇒	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90 80 80			N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability. A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	ROCK	70 60				B. Sand- stone with stone with stone and stitutore stitutore stitutore stitutore stitutore stitutore stitutore stitutore
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	OCKING OF	5	60			thin inter- layers of siltstone siltstone amounts sultstone sultstone sultstone sultstone stone layers
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL		40	30		C.D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H. F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECRE			20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small rock pieces.
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	N/A	N/A			10	sandstone are transformed into small rock pieces. Means deformation after tectonic disturbance

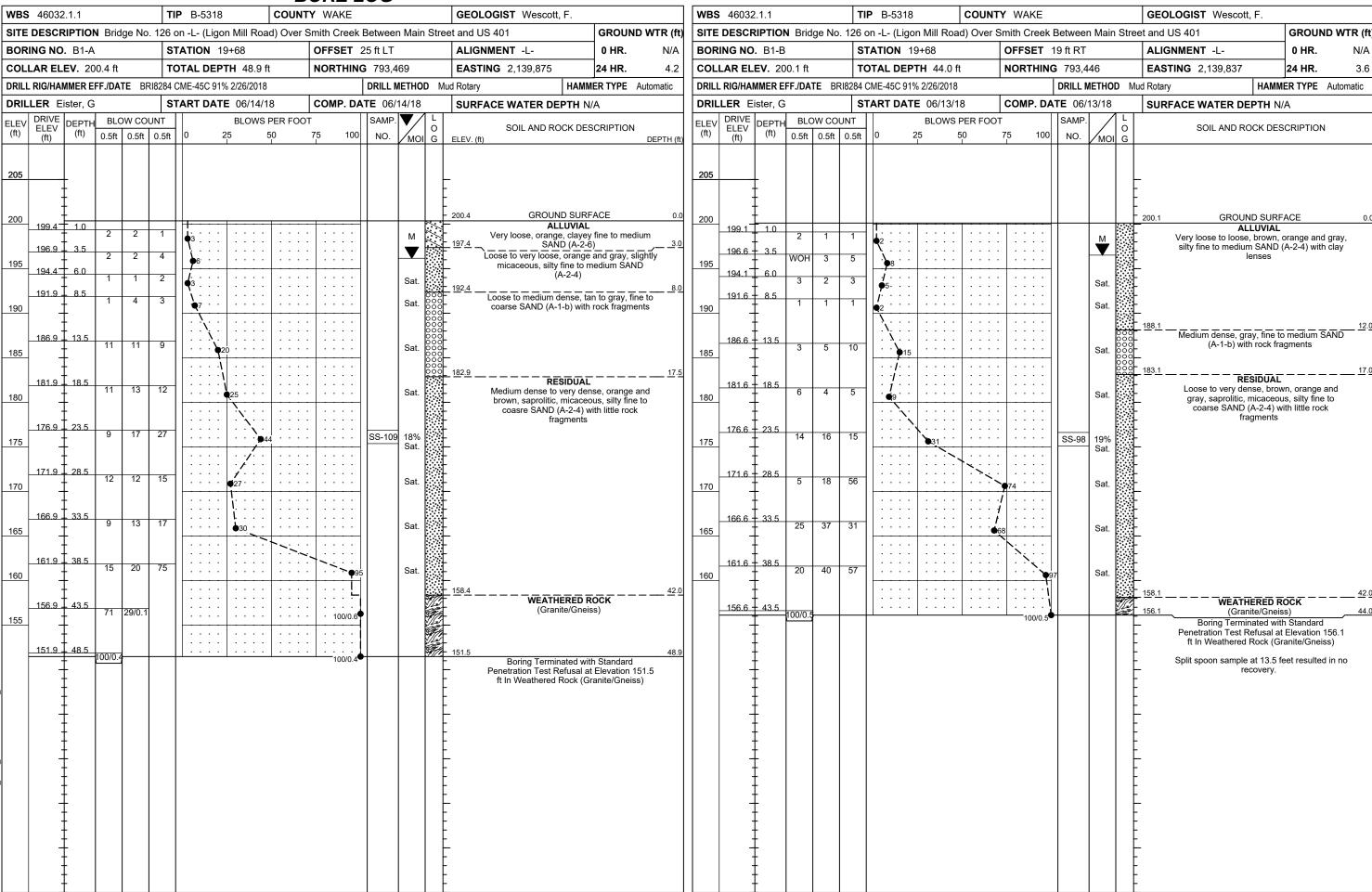










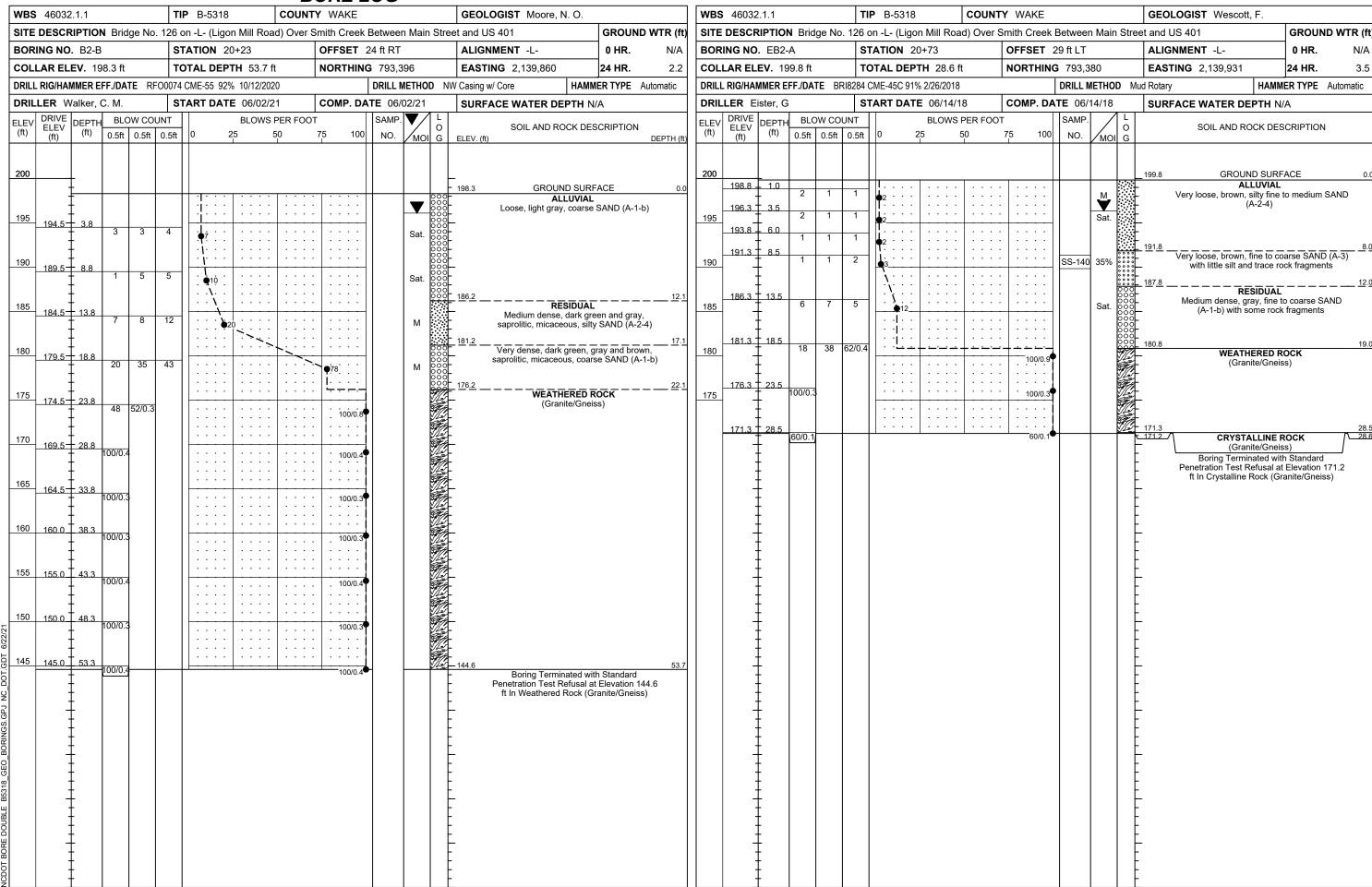


GEOTECHNICAL BORING REPORT CORE LOG

									KE L	UG											_					RE LOG				
WBS 4603				1	3-5318			V YTV						IST Moore	e, N. O.	1		-	S 46032.1.1			B-531				WAKE	GEOLOGIST M	oore, N. O.		
SITE DESC	RIPTIO	N Brido	je No. 1		`		ad) Ove									GROUND	WTR (ft)	SITE	E DESCRIPTIO	N Bridge No. 1				oad) Over		h Creek Between Main			GROUN	ID WTR
BORING NO). B2-A	١		STAT	ION 20	0+23		OF	FSET 2	24 ft LT			ALIGNME	ENT -L-		0 HR.	N/A	BOF	RING NO. B2-A		STA	TION	20+23		0	FFSET 24 ft LT	ALIGNMENT -L	-	0 HR.	N
COLLAR EL						TH 52.7		NO	RTHING					2,139,90	1	24 HR.	N/A		LLAR ELEV. 20				PTH 52.		N	ORTHING 793,420	EASTING 2,139	9,901	24 HR.	N
DRILL RIG/HA	MMER E	FF./DAT	E RFO	0074 CME	E-55 92%	6 10/12/20	020			DRILL	METHO	D NW	Casing w/ Co	ore	HAMM	IER TYPE A	utomatic	DRIL	L RIG/HAMMER E	FF./DATE RFOO	074 CME	-55 92	% 10/12/20	20		DRILL METHOD	NW Casing w/ Core	HAN	MMER TYPE	Automation
DRILLER V					T DATE	E 05/26/	/21	СО	MP. DA				SURFACI	E WATER	DEPTH N	/A		DRII	LLER Walker,	C. M.	STAI	RT DA	TE 05/20	6/21	C	OMP. DATE 05/27/21	SURFACE WAT	ER DEPTH	N/A	
ELEV DRIVE			W COUN				S PER FO		400		. ▼/	0		SOIL AND	ROCK DES	SCRIPTION		COF	RE SIZE NQ				N 14.1 ft			_				
(ft) (ft)	(ft)	0.5ft	0.5ft 0	.5ft 0	- 2	25 	50	75 	100	NO.	MOI	G	ELEV. (ft)				DEPTH (ft)	ELEV (ft)	CLC V /ft\	I /#\ KAIE	REC.	JN RQD (ft) %	SAMP. NO.	STRATA REC. RQI (ft) (ft) % %			DESCRIPTION AND I	REMARKS		
																		(11)	(ft)	(III) (Min/ft)) (11)	(II) %	NO.	% %	G	ELEV. (ft)				DEPT
205	+											-						161.6	6 161.6 38.6	2.6 00:26/0	6 (0.1)	(0.0)		(3.2) (0.0)) <i>F</i>	<u> </u> 7-	Begin Coring @ : WEATHERED ROCK	38.6 ft		
	Ŧ											l F						160	159.0 41.2	00:43/1	0 4%	0%		(3.2) (0.0 23% 0%		Granite/Gneis	s, gray-brown, very slight to very close fracture	o severe weat	hering, soft to	hard,
200	Ŧ												200.2		OUND SURF	ACE	0.0			5.0 01:11/1 01:02/1	.0 (0.0) .0 0%	(0.0) 0%				<u></u>	GSI = 15-2	, ,		
	Ŧ											0000	\	Very loose to	ALLUVIAL medium de	ense, gray and		155	1,510 + 100	00:47/1 00:36/1	.0						GSI = 13-2	.5		
196.7	3.5	2	2	į:				.				000	bro	own, micace	ous, coarse with trace sil	SAND (A-1-b),		154.0 46.2	01:17/1 5.0 00:57/1 01:08/1	.0 (3.1)	(0.0)								
195	Ŧ	-		' •							Sat.							450	‡	01:32/1	.0	0%								
101.7	8.5			;			.					000						150	□ 149.0	01:30/1 01:26/1	0l	(2.2)								
191.7	T 0.5	1	1	1 2							Sat.	0000							147.5 - 52.7	1.5 03:19/1 03:19/0	.U (0.0) <u>.5</u> /_ <u>0%</u> /	(0.0) 0%			//		inated due to core bit brea	king off at Fle	vation 147.5	ft In
	Ŧ				<u> </u>							0000							‡							<u> </u>	Weathered Rock (Gra	anite/Gneiss)	117.0	
186.7	13.5	2	5	10 .	./		.	.				000	185.7				14 5		‡							_	Hard drilling encountered	ed at 20.5 feet	.	
185	Ŧ		١	" -	15						Sat.	- E			RESIDUAL		14.0		‡							-				
181.7	18.5				/		.					F		sai	ndy SILT (A	olitic, micaceo -4)										-				
180	10.5	6	12	13 .	: : :}	25	.				М		Me	edium dense micaceou	e, green and us, silty SAN	gray, saprolit ID (A-2-4)	ic,		‡							-				
	Ŧ											_							‡							_				
176.7	23.5	10	28 72	- 11.					_: _: _:			<u> </u>	176.2				24.0									_				
175	Ŧ	"	20 / 2	1	· · ·		4		—100/0.9	,			Gı	ranite/Gneiss	THERED R s, gray-brow	n, very slight	to									-				
474.7	28.5							.					se		ring, soft to acture spaci	hard, very clo na	se									-				
170	7 20.5	82	18/0.1				.		100/0.6	,				RE	EC=23% (3.	2')			‡							-				
	Ŧ								: : :					R	QD=0% (0.0 GSI = 15-25	D') 5			‡							-				
166.7	33.5	38	32/0.3	-				.	- : : <u> </u>										‡							-				
165	Ŧ		32/0.0	1					100/0.8	<u>'</u>									‡							-				
161.7	38.5			- -			.	.											‡							-				
160	1	60/0.1		11					. 60/0.1	'									‡							‡				
	ŧ			11			. .												‡							_				
	‡			•			.	.											‡							-				
155	‡																	<u>.</u>	‡							-				
	‡			11			I											8/22/2	‡							-				
150	‡																		‡							-				
	‡			11			. .						147.5				52.7).TO	‡							-				
	‡								*					oring Termina at Elevation	ated due to o	core bit breaki Weathered Ro	na		‡							F				
-	‡											-	311	(G	ranite/Gnei	ss)	-	GBJ	‡							F				
	‡												I	Hard drilling	encountered	d at 20.5 feet.		NGS.	‡							F				
	‡											<u> </u>						BOR	‡							F				
	‡																		‡							F				
	‡																	318_(E				
-	‡											-						B2(‡							F				
	‡																	NGLE	‡							F				
150	‡											<u> </u>						B S	‡							F				
	‡																	8	‡							F				
	‡																	CDO	‡							F				
	1	1	1	1						1	1							zl	ı +	1 1	1	1		1		-				

GEOTECHNICAL BORING REPORT CORE LOG

		BURE LUG				URE LUG	
WBS 46032.1.1		NTY WAKE	GEOLOGIST Moore, N. O.	WBS 46032.1.1	TIP B-5318 COUNTY		GEOLOGIST Moore, N. O.
SITE DESCRIPTION Bridge No.	126 on -L- (Ligon Mill Road) Ove	er Smith Creek Between Main Str	eet and US 401 GROUND WTR (ft)	SITE DESCRIPTION Bridge No. 1	26 on -L- (Ligon Mill Road) Over Sr	mith Creek Between Main Stre	eet and US 401 GROUND WTR (
BORING NO. B2-A1	STATION 20+27	OFFSET 24 ft LT	ALIGNMENT -L- 0 HR. N/A	BORING NO. B2-A1	STATION 20+27	OFFSET 24 ft LT	ALIGNMENT -L- 0 HR. N//
COLLAR ELEV. 200.3 ft	TOTAL DEPTH 64.9 ft	NORTHING 793,417	EASTING 2,139,903 24 HR. 3.4	COLLAR ELEV. 200.3 ft	TOTAL DEPTH 64.9 ft	NORTHING 793,417	EASTING 2,139,903 24 HR. 3.4
DRILL RIG/HAMMER EFF./DATE RFC	00074 CME-55 92% 10/12/2020	DRILL METHOD	IW Casing w/ Core HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE RFOO	074 CME-55 92% 10/12/2020	DRILL METHOD N	W Casing w/ Core HAMMER TYPE Automatic
DRILLER Walker, C. M.	START DATE 05/27/21	COMP. DATE 06/01/21	SURFACE WATER DEPTH N/A	DRILLER Walker, C. M.	START DATE 05/27/21	COMP. DATE 06/01/21	SURFACE WATER DEPTH N/A
ELEV DRIVE DEPTH BLOW COU	NT BLOWS PER FO	DOT SAMP.	SOIL AND ROCK DESCRIPTION	CORE SIZE NQ	TOTAL RUN 11.8 ft		-
(ft) ELEV (ft) 0.5ft 0.5ft	0.5ft 0 25 50		SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV RUN DEPTH RUN DRILL	RUN SAMP. STRATA	L	DESCRIPTION AND DESCRIPTION
				ELEV RUN DEPTH RUN RATE (Min/ft)		O G ELEV. (ft)	DESCRIPTION AND REMARKS DEPTH
205				147.2			Begin Coring @ 53.1 ft
 			F	147.2 53.1 2.6 00:50/0.1	6 (2.6) (2.6) RS-1 (11.4) (8.9) 75% 75%	147.2 Granite/Gneiss hl:	CRYSTALLINE ROCK 5 ack, gray and orange, slight weathering, moderately hard
				144.6 55.7 02:29/1 - 4.2 01:59/1. - 02:04/1.	0 (4.2) (2.8)	to I	hard, moderate to close fracture spacing
200			200.3 GROUND SURFACE 0.0	+	0 100% 67% RS-2 RS-2		RS-1: 53.1'-54.1'
		- -	SEE B2-A FOR SOIL DESCRIPTIONS		0		RS-2: 57.7-58.1' GSI = 55-65
195			- -	140 140.4 59.9 5.0 01.130/1.	92% 70%	to I	Qu =
†			- -			135.4	2
				100.4 04.0 101.29/1.		Boring Terminated	at Elevation 135.4 ft In Crystalline Rock (Granite/Gneiss)
190						-	Hard drilling encountered at 21.2 feet.
		1 11 1				L	
			E			F	
185			-			F	
			-			Ė.	
180			-			‡	
			179.1 21.2			-	
			(Granite/Gneiss)			-	
175 🛨			-			_	
			-			<u>-</u>	
						_	
$\frac{170}{1}$			_			F	
			_			F	
165						‡	
			-			ļ.	
			-			-	
160			-			-	
			 -			<u> </u>	
155			-			_	
155			- -	[2] †		<u> </u>	
152.2 48.1			-	 		-	
150		100/0.4	-			E	
			- 4470			F	
<u> </u>		RS-1	147.2 53.1 53.1			F	
145			Granite/Gneiss, black, gray and orange, slight weathering, moderately hard to hard,			F	
			moderate to close fracture spacing			F	
140		RS-2	REC=97% (11.4') RQD=75% (8.9')			‡	
			GSI = 55-65			ļ.	
			-			-	
			- 135.4 Boring Terminated at Elevation 135.4 ft In	353.13 		E	
			Crystalline Rock (Granite/Gneiss)			Ł	
			Hard drilling encountered at 21.2 feet.			_	
			-			F	
			-			F	
ğ ‡			F			‡	



											KE L						
	46032					P B-53′			COUN							GEOLOGIST Wescott, F.	
SITE	DESCF	RIPTIO	N Brid	dge No					d) Over	_				ain S	Stree	et and US 401	GROUND WTR (ft)
BOR	ING NO	. EB2-	-B		S	TATION	20+7	'3		OF	FSET	3 ft R	•			ALIGNMENT -L-	0 HR . N/A
COLI	LAR EL	EV. 19	99.8 ft		TO	OTAL DE	PTH	31.0 ft	t	NC	RTHIN	3 793	,361			EASTING 2,139,899	24 HR. FIAD
DRILL	. RIG/HAI	MMER E	FF./DA	TE BI	RI8284 (CME-45C	91% 2/2	26/2018				DRIL	_ MET	HOD	Mu	d Rotary HAMMI	ER TYPE Automatic
DRIL	LER E	ister, G	i		S	TART DA	TE C	06/15/1	8	CC	MP. DA	TE 0	6/15/	18		SURFACE WATER DEPTH N/	A
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	OW CO		0	25 		PER FOC	75	100	SAM	ーレン	/	L O G	SOIL AND ROCK DES	CRIPTION DEPTH (ft)
200	198.8 - 196.3	- 1.0 - - 3.5	2	1	2	1 43 · · ·	· · · · · · · · · · · · · · · · · · ·					SS-1	N	И		199.8 GROUND SURF. ALLUVIAL Very loose to loose, brown, to fine to medium SAND (A-2-4 with little rock frage)	an and gray, silty) with trace clay,
195	193.8 - - - 191.3	-	1	2	2	Q 2 Q 4 : :	 					33-1		at.		191.8	8.0
190	- 191.9 - -	- 0.5	2	3	2	5	· ·						S	at.		Loose, gray, fine to coarso	
185	186.3	13.5	12	21	18		:	7. 1 39					S	at. 000	000	RESIDUAL Dense to medium dense, micaceous, fine to coarse SA some rock fragm	AND (A-1-b) with
180	181.3	18.5	10	9	12		. /. . / . •21						S	at.	000	Some rock nagm	erits
175	176.3 -	23.5	79	21/0.1			<u>i</u> :	· · · · · · · · · · · · · · · · · · ·			100/0.6	,		9000k3		176.8 WEATHERED RO	
170	171.3	28.5	100/0.	5							100/0.5			330000000000000000000000000000000000000			
																Boring Terminated with Penetration Test Tricone Elevation 168.8 ft On Wes (Granite/Gneis	e Refusal at athered Rock

SHEET 12

CORE PHOTOGRAPHS

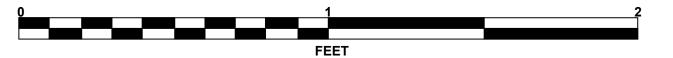
B2-ABOX 1: 38.6 - 52.7 FEET



B2-A1BOXES 1 & 2: 53.1 - 65.7 FEET







PROJECT REFERENCE NO.	SHEET NO.
B-5318	14

	SOIL TEST RESULTS														
SAMPLE OFFSET STATION DEPTH AASHTO L.L. P.I. GRAND FRANK GLAY 10												(EVES)	%	%	
NO.	OFFSET	BIATION	INTERVAL	CLASS.	L.D.	1 .1.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS- 129	21' LT	18 +7 3	6.0-7.5	A- 1- b	-	NP	82.9	8.5	3. 1	5. 5	86	28	10	23.8	-
SS-71	25′ RT	18 +7 3	3. 5- 5. 0	A-6(6)	37	16	9.5	42.4	21.0	27.1	99	96	54	28.4	-
SS- 109	25' LT	19+68	23. 5- 25. 0	A-2-4	-	NP	56.5	25.8	11.2	6.5	85	60	21	18.3	-
SS- 98	19' RT	19+68	23. 5- 25. 0	A-2-4	-	NP	49.5	29.7	14.5	6.3	89	65	25	19.0	-
SS- 140	29′ LT	20 +7 3	8.5-10.0	A- 3	-	NP	57.4	36.4	3. 4	2. 8	97	68	8	34.9	-
SS- 146	8′ RT	20 +7 3	3. 5- 5. 0	A-2-4	-	NP	48.8	28.8	9.6	12.7	98	64	25	29.0	-



PROJECT NO.: 43032.1.1 PROJECT ID: B-5318

REPORT ON SAMPLES OF: ROCK QUALITY

PROJECT DESCRIPTION: BRIDGE NO. 126 ON SR 2044 (LIGON MILL ROAD) OVER SMITH CREEK

DATE SAMPLED: 6/8/2021 COUNTY: WAKE

SUBMITTED BY: N.O. MOORE

BORING NO.	SAMPLE NO.	DEPTH (FT)	ROCK TYPE	GEOLOGIC MAP UNIT	LENGTH (IN)	DIAMETER (IN)	UNIT WEIGHT (PCF)	UNCONFINED COMPRESSIVE STRENGTH (PSI)	YOUNG'S MODULUS (PSI)	SPLITTING TENSILE STRENGTH (PSI)	REMARKS
B2-A1	RS-1	53.1-54.2	Injected Gneiss	CZig	13	1.86	164.2	29,700	-	-	-
B2-A1	RS-2	57.8-58.2	Injected Gneiss	CZig	5	1.86	154.2	6,430	-	-	-

SITE PHOTOGRAPHS BRIDGE NO. 126 OVER SMITH CREEK ON SR 2044



View of SR 2044 looking south.



View of Smith Creek looking west.