

REFERENCE: U-5896

PROJECT: 44674

SEE SHEET 3 FOR PLAN SHEET LAYOUT
AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5896	1	40

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

J. BLYTHE

H. CAMP

J. WHITE

T. WHITEHEAD

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S. BOWMAN

INVESTIGATED BY S&ME, Inc.

DRAWN BY J.R. SWARTLEY

CHECKED BY S.S. LANEY

SUBMITTED BY S.S. LANEY

DATE SEPTEMBER 2019

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LINE	STATION	PLAN	PROFILE
-L-	6+40 TO 10+50	4	8
-L-	10+50 TO 39+00	5	8 TO 10
-L-	39+00 TO 44+25	6	10
-Y-	10+50 TO 23+50	5	11
-Y-	23+50 TO 25+80	7	11
-YI-	13+90 TO 15+12	7	14
-RPA-	10+00 TO 24+05	5	12
-RPB-	10+00 TO 21+37	5	12
-RPC-	10+00 TO 24+12	5	13
-RPD-	10+00 TO 26+15	5	13 TO 14

CROSS SECTIONS

LINE	STATION	SHEETS
-L-	10+00	15
-L-	12+50 TO 13+50	16 TO 17
-L-	19+00 TO 22+50	18 TO 21
-L-	24+50 TO 32+50	22 TO 27
-Y-	14+00 TO 17+50	28 TO 30
-RPA-	16+00 TO 17+50	31 TO 32
-RPC-	16+50 TO 21+50	33 TO 36
-RPD-	16+50 TO 17+50	37
-RPD-	24+00 TO 25+50	38

TITLE	SHEETS
SOIL TEST RESULTS	39 TO 40

ROADWAY
SUBSURFACE INVESTIGATION

COUNTY GUILFORD

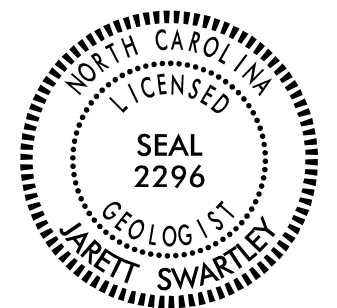
PROJECT DESCRIPTION US 29, US 70 AND SOUTH MAIN STREET (SR 1009 AND US 311 BUSINESS)

INVENTORY

Prepared in the Office of:



3201 SPRING FOREST ROAD
RALEIGH, NC 27616
(919) 872-2660



DocuSigned by:

Jarett Swartley

SIGNATURE 919459487BA3471... DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.									
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS										WEATHERED ROCK (WR)										CRYSTALLINE ROCK (CR)									
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS										THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.										FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.									
MINERALOGICAL COMPOSITION										COMPRESSION										NON-CRYSTALLINE ROCK (NCR)										COASTAL PLAIN SEDIMENTARY ROCK (CP)									
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.										SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50										FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.										COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.									
PERCENTAGE OF MATERIAL										GROUND WATER										WEATHERING																			
ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL										WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING										FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.																			
TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC > 10%										STATIC WATER LEVEL AFTER 24 HOURS										VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.																			
										PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA										SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.																			
										SPRING OR SEEP										MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.																			
MISCELLANEOUS SYMBOLS										RECOMMENDATION SYMBOLS										SEVERE (MOD. SEV.)										SEVERE (SEV.)									
ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION										UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF									
SOIL SYMBOL										UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF										ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. FABRIC MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.									
ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT										DIP & DIP DIRECTION OF ROCK STRUCTURES										VERY SEVERE (IV SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF										ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. FABRIC MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.									
INFERRED SOIL BOUNDARY										SLOPE INDICATOR INSTALLATION										COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. FABRIC MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.																			
INFERRED ROCK LINE										CONE PENETROMETER TEST																													
ALLUVIAL SOIL BOUNDARY										SOUNDING ROD																													
										TEST BORING WITH CORE																													
										SPT N-VALUE																													
TEXTURE OR GRAIN SIZE										ABBREVIATIONS										ROCK HARDNESS																			
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270										AR - AUGER REFUSAL MED. - MEDIUM VST - VANE SHEAR TEST										VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.																			
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)										BT - BORING TERMINATED MICA - MICACEOUS WEA. - WEATHERED										HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.																			
GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005										CL. - CLAY MOD. - MODERATELY UNIT WEIGHT										MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.																			
SOIL MOISTURE - CORRELATION OF TERMS										CPT - CONE PENETRATION TEST NP - NON PLASTIC										MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.																			
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION										ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY										SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.																			
LL - LIQUID LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE										SL. - SLIGHTLY TCR - TRIAXIAL REFUSAL										VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.																			
PL - PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE										W - MOISTURE CONTENT										FRACTURE SPACING										BEDDING									
OM - OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE										V - VERY										TERM SPACING										TERM THICKNESS									
DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE																				VERY WIDE MORE THAN 10 FEET										VERY THICKLY BEDDED 4 FEET									
PLASTICITY										EQUIPMENT USED ON SUBJECT PROJECT										WIDE 3 TO 10 FEET										THICKLY BEDDED 1.5 - 4 FEET									
NON PLASTIC 0-5 VERY LOW										ADVANCING TOOLS: DRILL UNITS: HAMMER TYPE:										MODERATELY CLOSE 1 TO 3 FEET										THINLY BEDDED 0.16 - 1.5 FEET									
SLIGHTLY PLASTIC 6-15 SLIGHT										CME-45C CLAY BITS										CLOSE 0.16 TO 1 FOOT										VERY THINLY BEDDED 0.03 - 0.16 FEET									
MODERATELY PLASTIC 16-25 MEDIUM										CME-55 6" CONTINUOUS FLIGHT AUGER										VERY CLOSE LESS THAN 0.16 FEET										THICKLY LAMINATED 0.008 - 0.03 FEET									
HIGHLY PLASTIC 26 OR MORE HIGH										CME-550 8" HOLLOW AUGERS																				THINLY LAMINATED < 0.008 FEET									
										VANE SHEAR TEST																													
										PORTABLE HOIST																													
										CME-550X																													
COLOR																																							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																																							

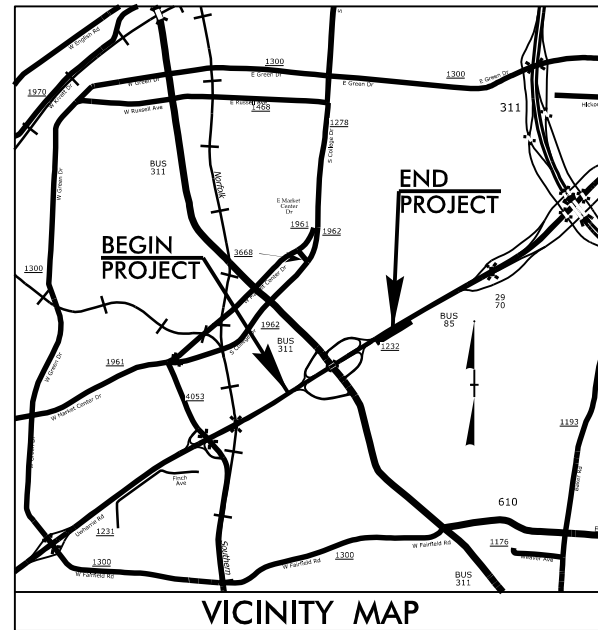
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5896	3	40
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
44674.1.1	NHP-0029(068)	PE(U-5896)	

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

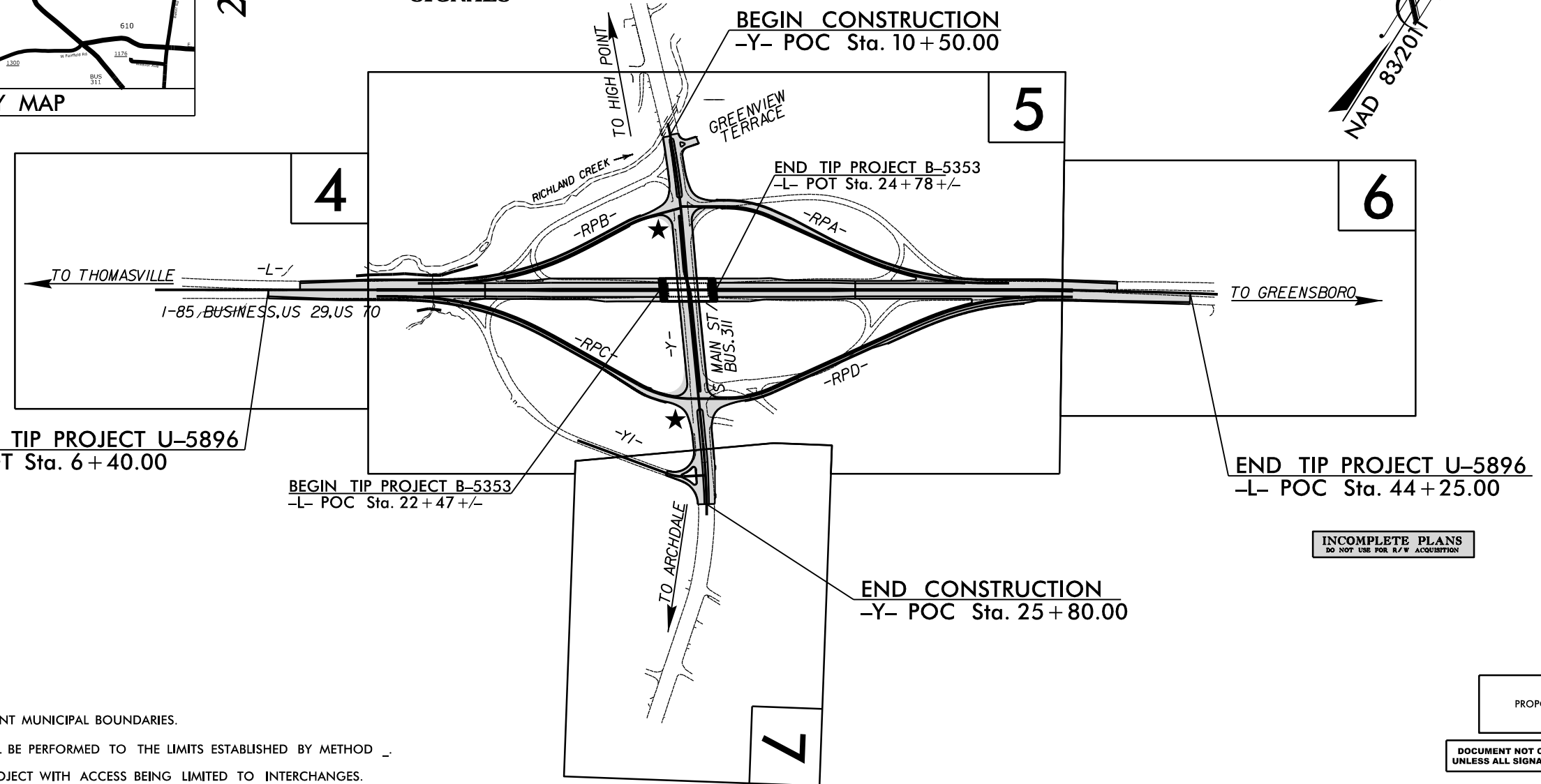
GUILFORD COUNTY

LOCATION: INTERCHANGE AT US 29, US 70 AND S MAIN STREET
(SR 1009 & US 311 BUSINESS), NEAR HIGH POINT

TYPE OF WORK: GRADING, PAVING, DRAINAGE, STRUCTURE,
SIGNALS



25% PLANS



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

PROPOSED SIGNAL

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

THIS PROJECT IS WITHIN HIGH POINT MUNICIPAL BOUNDARIES.

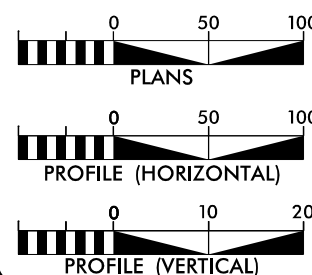
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD _____.

THIS IS A CONTROLLED-ACCESS PROJECT WITH ACCESS BEING LIMITED TO INTERCHANGES.

CONTRACT: TIP PROJECT: U-5896

CONTRACT:

GRAPHIC SCALES



DESIGN DATA

ADT 2020 = 38,070
ADT 2040 = 42,700
K = 10 %
D = 60 %
T = 9 % *
V = 60 MPH
* TTST = 4% DUAL = 5%
FUNC CLASS =
FREEWAY
STATEWIDE TIER

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT U-5896 = 0.683 MI
LENGTH OF STRUCTURE TIP PROJECT B-5353 = 0.034 MI
TOTAL LENGTH TIP PROJECTS U-5896 & B-5353 = 0.717 MI

PLANS PREPARED FOR THE NCDOT BY:

M MOTT MACDONALD
2018 STANDARD SPECIFICATIONS

VHB Vanasse Hangen Brustlin, Inc.
PO Box 700
Fayetteville, NC 27526
(919) 552-2253
(919) 552-2254 (Fax)
www.mottmac.com/americas
LICENSE NO. F-0669

RIGHT OF WAY DATE: MAY 31, 2019

LETTING DATE: AUGUST 18, 2020

TIM JORDAN, PE
PROJECT ENGINEER

JORDAN WOODARD, PE
PROJECT DESIGN ENGINEER

BRYAN C. KEY, PE
NCDOT CONTACT DIVISION 1

HYDRAULICS ENGINEER

SIGNATURE: _____ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



16-SEP-2019 12:57 Z:\Projects\2019\Trans\6235-19-003 - NCDOT US 29 & I-85 (U-5896) (GEU)\U5896_GEO_RDWY\CADD_GEOTECH\Site&Sub\U-5896_r_dy_tsh.dgn \$\$\$USERNAME\$\$\$



September 5, 2019

STATE PROJECT: 44674.1.2 (U-5896)
 FEDERAL PROJECT: N/A
 COUNTY: Guilford
 DESCRIPTION: US 29, US 70 and South Main Street (SR 1009 and US 311 Business)

SUBJECT: Geotechnical Report – Inventory

S&ME, Inc. has completed a reconnaissance and subsurface investigation for the above roadway project and presents the following inventory. Plans, profiles and cross-sections are included in this report.

Project Description

The project corridor is located in Guilford County near the town of High Point. The project consists of improving the existing interchange of US 29/US 70 (-L-) and S. Main St./US 311 Business (-Y-). The work consists of grading, drainage, minor widening, and the partial realignment of the on and off-ramps associated with this interchange. The existing structure (Bridge No. 147) over S. Main St./US 311 Business will be replaced and was also investigated during this project. There are two proposed retaining walls near the beginning of the mainline (-L-) that were investigated during this project. The mainline (-L-) starts at the southwestern end of the project and continues northeast for approximately 0.68 miles. The -Y- alignment passes underneath -L- and -Y- stations increase as the alignment trends from the North to the South.

The geotechnical field investigation was conducted during the period of March and April of 2019. Two S&ME drill crews were used to drill, sample, and log the borings in this report. S&ME rigs used for the drilling include two ATV mounted CME-550X drill machines. Standard Penetration Tests were performed at selected locations and additional borings were advanced using continuous flight augers or hand augers. Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by the S&ME soils lab. Soil results are referenced back to the original alignment the boring was assigned to during layout and drilling.

The following alignments, excluding the bridge, totaling 1.82 miles, were investigated. Subsurface profiles and/or cross-sections of these alignments are included in this report.

<u>Line</u>	<u>Station</u>
-L-	8+03 to 42+00
-Y-	11+20 to 25+50
-RPA-	12+00 to 23+00
-Y1-	13+90 to 15+12
-RPB-	14+00 to 20+71
-RPC-	14+25 to 23+50
-RPD-	11+50 to 24+50

Areas of Special Geotechnical Interest

- 1) The following station ranges encountered soft, cohesive soils which have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station</u>
-L-	22+50
-L-	25+00

- 2) Highly Plastic Clays: Highly plastic clays (PI > 25) were encountered on the project at the following station ranges:

<u>Line</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	10+00 to 15+00	LT & RT
-L-	21+50 to 22+80	LT & RT
-L-	24+50 to 31+00	LT & RT
-Y-	10+00 to 11+50	LT & RT
-Y-	14+50 to 17+00	LT & RT
-Y-	19+00 to 21+00	LT & RT
-RPA-	15+50 to 17+00	LT & RT
-RPC-	16+50 to 21+00	LT & RT
-RPD-	15+00 to 17+00	LT & RT

- 3) High Groundwater: High groundwater within 6' of grade or less was encountered at the following station ranges:

<u>Line</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-L-	9+75 to 10+25	RT
-RPA-	20+00 to 22+25	LT & RT
-RPC-	20+00 to 22+50	LT & RT

- 4) Crystalline Rock: Crystalline rock was encountered above or within 6' of grade at the following locations:

<u>Line</u>	<u>Stations</u>	<u>Offsets (ft)</u>
-RPD-	24+43 to 24+50	23 LT to 25 LT

Physiography and Geology

The project corridor is located in central North Carolina in the Piedmont Physiographic Province of North Carolina. A mixture of fields, wooded areas and streams lie within the project area. The project corridor is predominately urban with commercial businesses and few single family homes. Topography along the project is flat to gently sloping. Elevations along the project range from 804± to 848± feet above sea level.

Geologically the surficial soils in the area are generally classified as residual soils. These soils were formed by the in place weathering of the underlying bedrock in the area. There are some alluvial soils in the area adjacent to streambeds. The crystalline bedrock in this area is classified as Biotite Gneiss. This rock is metamorphosed granitic rock that formed in the late Proterozoic to late Cambrian periods during the Taconic Orogeny and is part of the Carolina Slate Belt suite of rocks.

Water Bodies

There is one creek that runs through the project corridor. The name of this creek is Richland Creek and it is part of the larger Cape Fear drainage basin. Richland Creek passes underneath US 29/70 at -L- sta. 13+15 through a single barrel culvert then flows to the north adjacent to -RPB- before passing underneath S. Main Street. at -Y- sta. 10+00 through a triple barrel culvert. This creeks flow from south to north in the vicinity of the project. It is anticipated that the single barrel culvert will need to be extended on the southern/inlet side but not the northern side due to the retaining wall that is proposed there. No modifications are anticipated for to the culvert passing underneath -Y- alignment.

Soil Properties

Soils encountered during this investigation are separated into 3 categories: Roadway Embankment, Alluvial soils and Residual soils.

Roadway Embankment soils are predominately cohesive in nature and may be derived from nearby sources. These soils consist of orange, brown, tan, red, and gray, soft to hard, sandy silt (A-4), clayey silt, (A-5), sandy clay (A-6), and silty clay (A-7-5/A-7-6) and loose to dense, sand with gravel (A-1-a/A-1-b), clayey sand (A-2-6), and silty sand (A-2-4).

Alluvial soils are found near the channels and floodplains from the nearby rivers and streams in the area. These soils consist of gray, brown, tan, orange and green, very loose to dense, sand (A-3), silty sand (A-2-4) and clayey sand (A-2-6) and very soft to stiff, sandy clay (A-6), silty clay (A-7-6), sandy silt (A-4), clayey silt (A-5) and organic clay.

Residual soils underlie the alluvial soils in some areas and are exposed at the surface in most locations. These soils consist of brown, tan, orange, white and black, soft to hard, sandy clay (A-6), sandy silt (A-4) and silty clay (A-7-6) and loose to very dense, sand (A-3), silty sand (A-2-4) and clayey sand (A-2-6).

Rock Properties

Beneath residual soils, Weathered Rock and Crystalline Rock were encountered. Crystalline rock is classified as metamorphosed granitic rock, and was first encountered at elevations ranging from 800± to 830± feet above sea level. Rock coring was performed near the bridge. RQD values ranged between 80% and 100% and recovery values were generally 90% and above.


Groundwater

Groundwater measurements were taken in March and April of 2019. Groundwater is typically between 7 feet and 23 feet below the ground surface. Ground water is expected to cause some minor impacts during construction depending on rainfall conditions at the time.

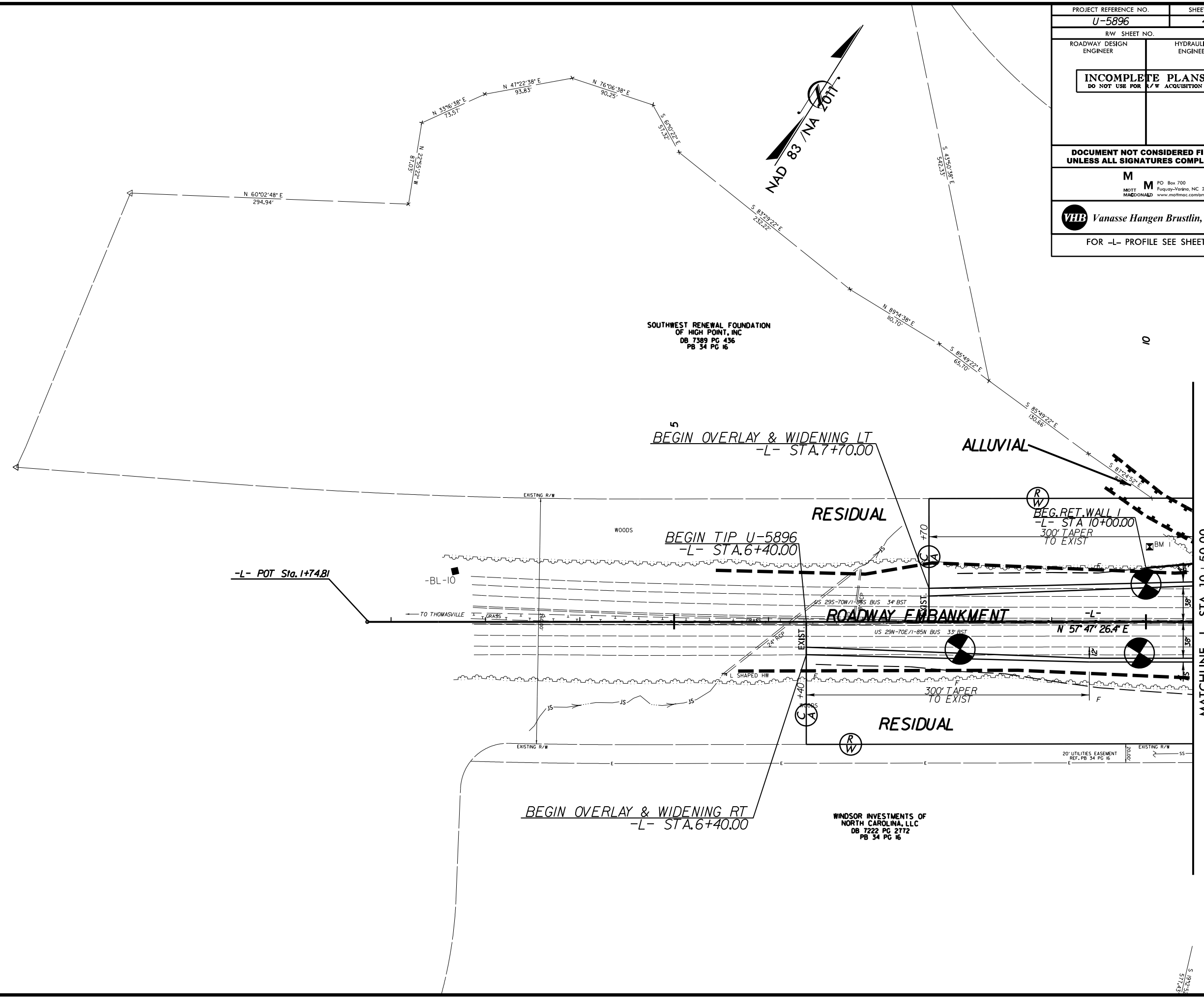
Respectfully Submitted,




Jarett Swartley, PG
Senior Geologist

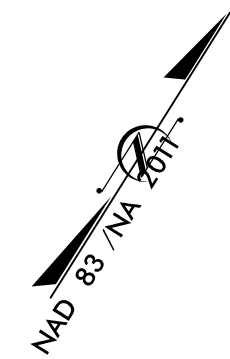
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M MOTT MACDONALD PO Box 700 Fuquay-Varina, NC 27526 www.mottmac.com/america	
 Vanasse Hangen Brustlin, Inc.	
FOR -L- PROFILE SEE SHEET 8	

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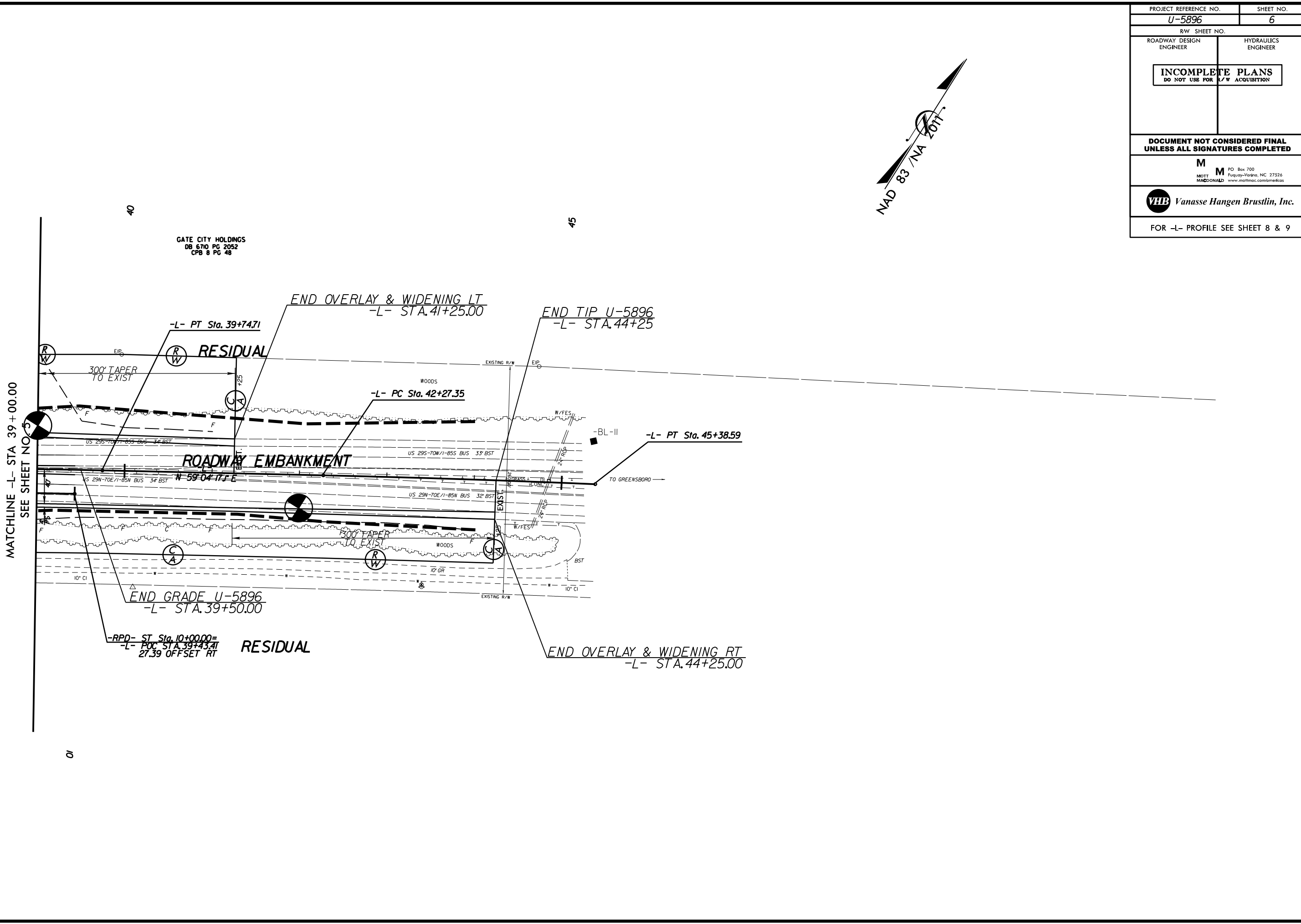


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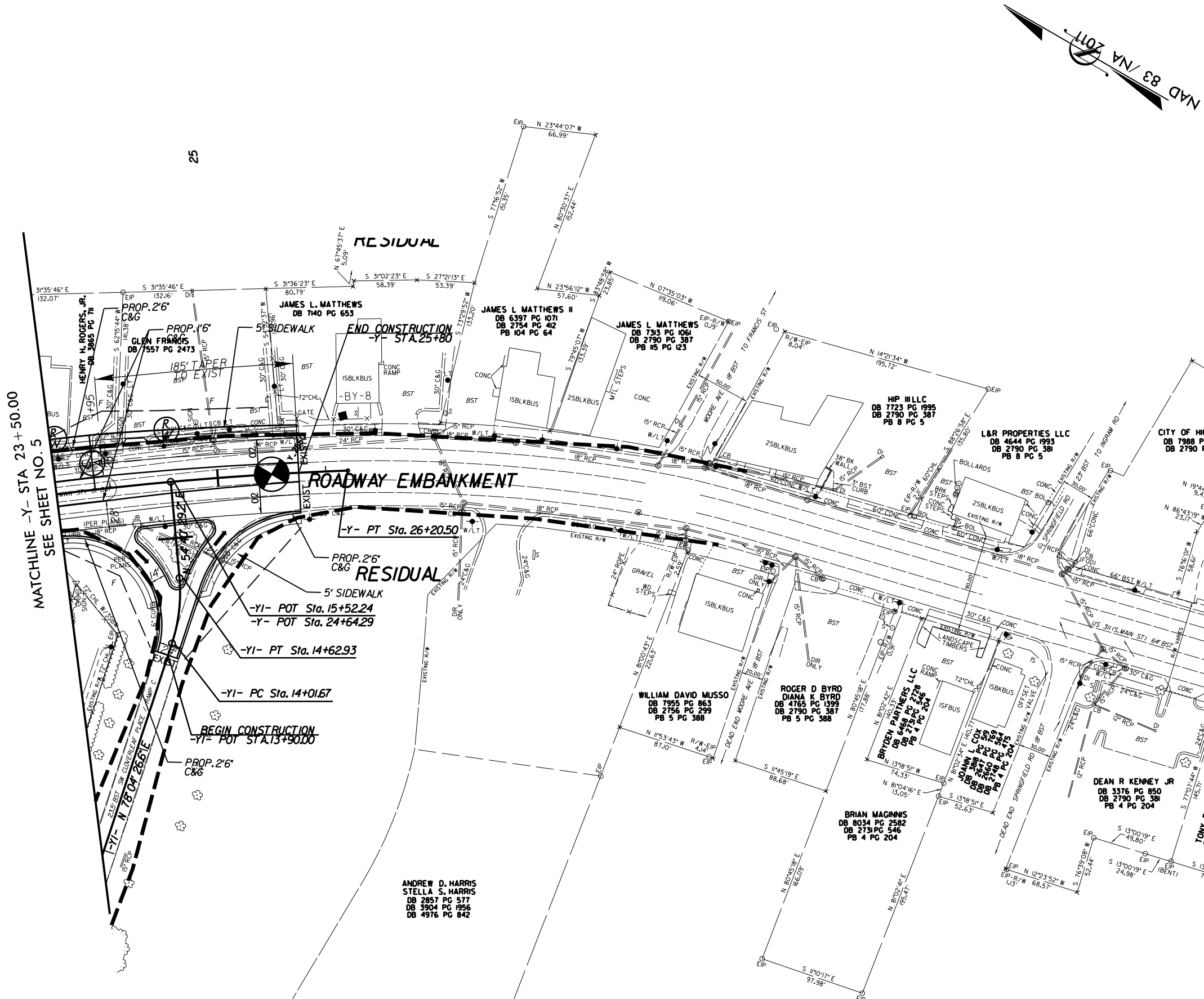
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ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
M MOTT MACDONALD <small>PO Box 700 Fuquay-Varina, NC 27526 www.mottmac.com/america</small>	
 Vanasse Hangen Brustlin, Inc.	
FOR -L- PROFILE SEE SHEET 8 & 9	



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PROJECT REFERENCE NO. U-5896	SHEET NO. 7
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	
M MOTT MACDONALD PO Box 700 Fuquay-Varina, NC 27526 www.mottmac.com/mactexas	
VHB Vanasse Hangen Brustlin, Inc.	
FOR -Y- PROFILE SEE SHEET 10	



MATCHLINE -Y- STA 23+50.00
 SEE SHEET NO. 5

25

RESIDUAL

ROADWAY EMBANKMENT

BEGIN CONSTRUCTION
 -YI- POT STA. 13+90.00

RESIDUAL

END CONSTRUCTION
 -Y- STA. 25+80.00

5' SIDEWALK

PROF. 2'6" C&G

18.5" TAPER

18" RCP

15" RCP

12" RCP

10" RCP

8" RCP

6" RCP

4" RCP

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2" RCP

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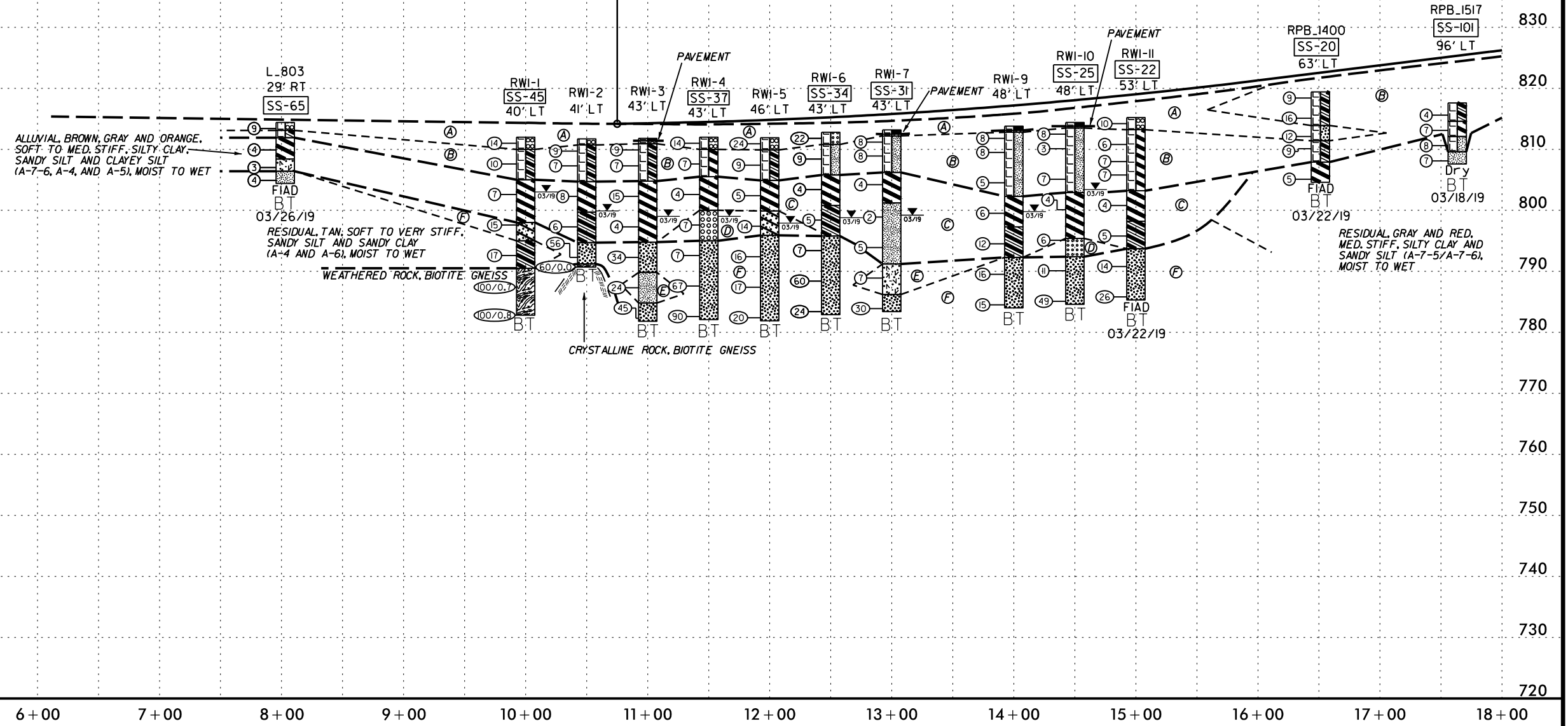
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS-20	CL	14+00	3.3-4.8	A-6(3)	32	11	22	30	32	16	92	80	51	16.3	-
SS-22	53 LT	15+00	3.4-4.9	A-7-5(12)	49	13	9	25	46	20	100	96	76	29.0	-
SS-25	48 LT	14+50	0.9-2.4	A-4(3)	33	7	19	25	34	22	95	84	60	19.7	-
SS-31	43 LT	13+00	13.3-14.8	A-4(1)	22	6	12	33	37	18	100	96	63	ND	-
SS-34	43 LT	12+50	8.4-9.9	A-7-6(37)	70	41	8	14	23	56	100	97	82	ND	-
SS-37	43 LT	11+50	3.4-4.9	A-7-5(12)	48	13	8	20	36	36	98	94	77	36.4	-
SS-45	40 LT	10+00	8.4-9.9	A-7-6(17)	47	21	8	16	31	45	98	94	78	ND	-
SS-65	29 RT	8+03	6.4-7.9	A-5(7)	44	7	10	25	47	18	100	94	73	38.0	-
SS-101	CL	15+17	1.0-2.5	A-6(8)	40	17	16	24	39	22	88	79	59	ND	-

-L-

BEGIN GRADE
STA. 10+75.00
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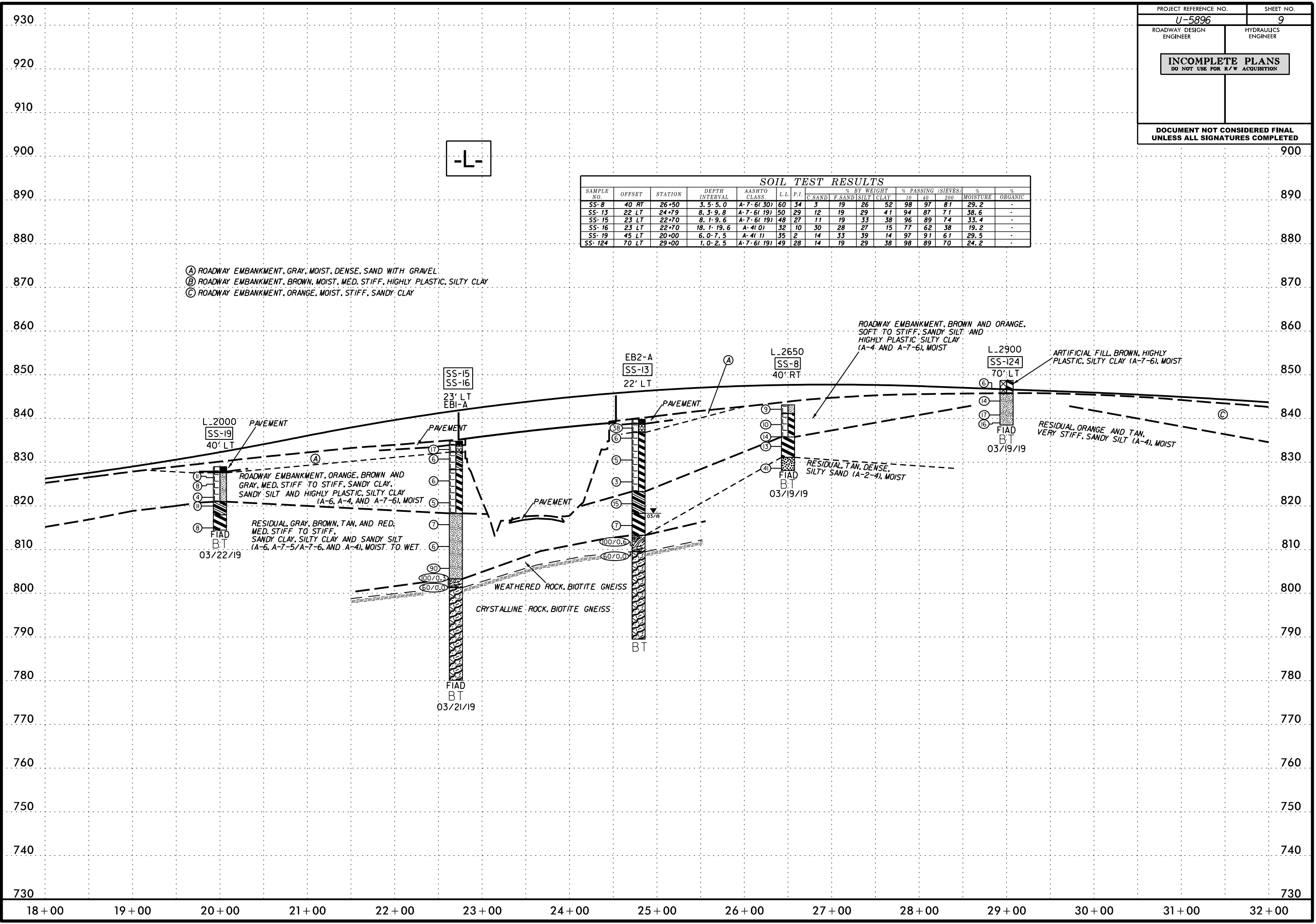
- (A) ROADWAY EMBANKMENT, BLACK AND GRAY, LOOSE TO MED. DENSE, SILTY SAND, CLAYEY SAND AND SAND WITH GRAVEL (A-2-4, A-2-6, AND A-1-b), DRY TO MOIST
- (B) ROADWAY EMBANKMENT, ORANGE, GRAY AND BROWN, MED. STIFF TO STIFF, SANDY SILT, SANDY CLAY AND SILTY CLAY (A-4, A-6, AND A-7-5), MOIST
- (C) ALLUVIAL BROWN, GRAY, RED AND ORANGE, SOFT TO STIFF, SANDY SILT, SANDY CLAY AND HIGHLY PLASTIC, SILTY CLAY WITH TRACE ORGANIC MATTER (A-4, A-6, AND A-7-6), MOIST TO WET
- (D) ALLUVIAL TAN, RED AND GRAY, COARSE SAND, SAND AND CLAYEY SAND WITH GRAVEL (A-1-b, A-3, AND A-2-6), MOIST TO SATURATED
- (E) RESIDUAL TAN AND GRAY, MED. STIFF TO VERY STIFF, SANDY SILT AND CLAYEY SILT (A-4 AND A-5), WET
- (F) RESIDUAL GRAY AND TAN, LOOSE TO VERY DENSE, SILTY SAND AND CLAYEY SAND (A-2-4 AND A-2-6), SATURATED



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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G SAND	F SAND	SILT	CLAY	10	40	200		
SS-8	40 RT	26+50	3.5-5.0	A-7-6(30)	60	34	3	19	26	52	98	97	81	29.2	-
SS-13	22 LT	24+79	8.3-9.8	A-7-6(19)	50	29	12	19	29	41	94	87	71	38.6	-
SS-15	23 LT	22+70	8.1-9.6	A-7-6(19)	48	27	11	19	33	38	96	89	74	33.4	-
SS-16	23 LT	22+70	18.1-19.6	A-4(0)	32	10	30	28	27	15	77	62	38	19.2	-
SS-19	45 LT	20+00	6.0-7.5	A-4(1)	35	2	14	33	39	14	97	91	61	29.5	-
SS-124	70 LT	29+00	1.0-2.5	A-7-6(19)	49	28	14	19	29	38	98	89	70	24.2	-

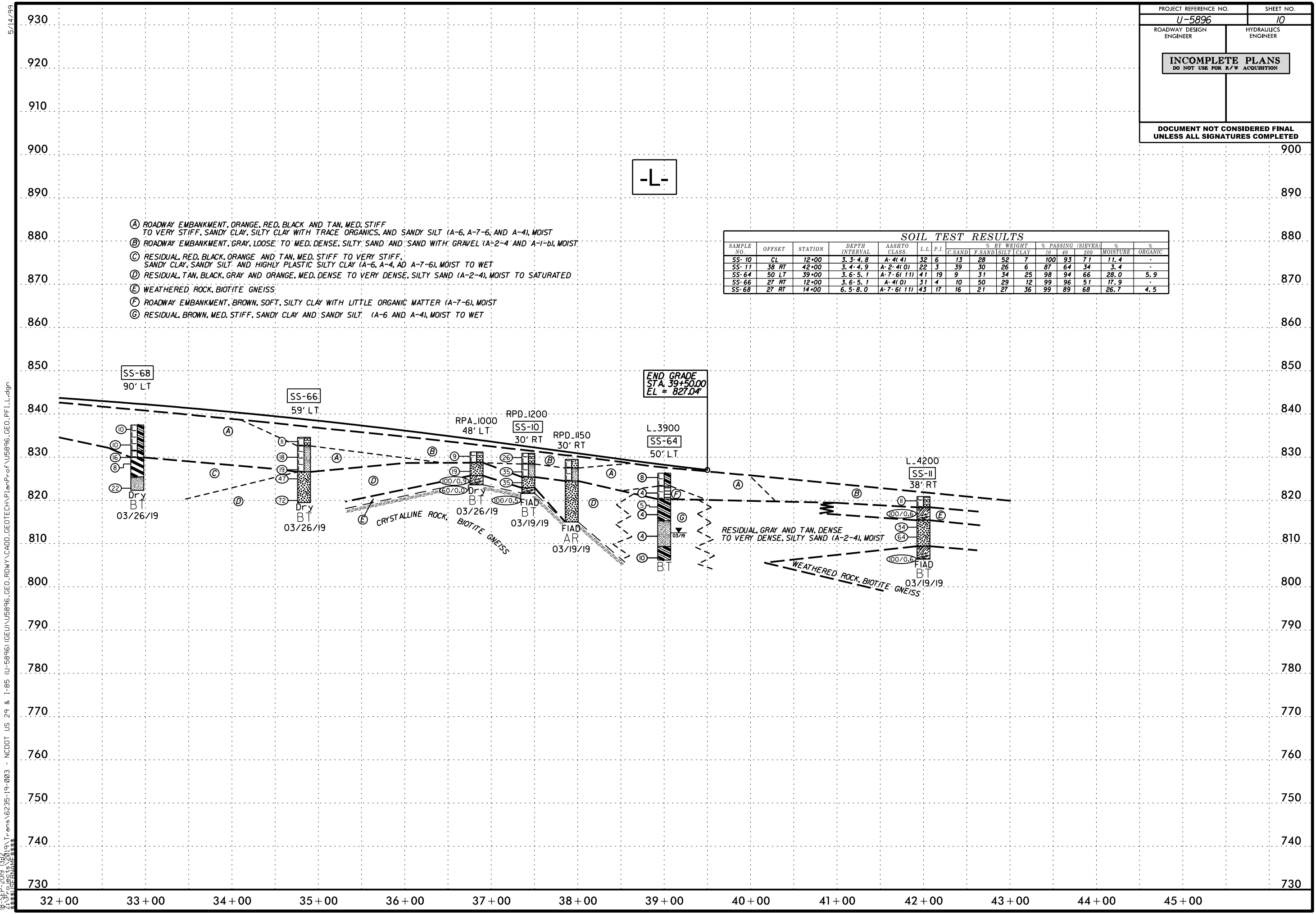
- (A) ROADWAY EMBANKMENT, GRAY, MOIST, DENSE, SAND WITH GRAVEL.
- (B) ROADWAY EMBANKMENT, BROWN, MOIST, MED. STIFF, HIGHLY PLASTIC, SILTY CLAY
- (C) ROADWAY EMBANKMENT, ORANGE, MOIST, STIFF, SANDY CLAY



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- (A) ROADWAY EMBANKMENT, ORANGE, RED, BLACK AND TAN, MED. STIFF TO VERY STIFF, SANDY CLAY, SILTY CLAY WITH TRACE ORGANICS, AND SANDY SILT (A-6, A-7-6, AND A-4), MOIST
- (B) ROADWAY EMBANKMENT, GRAY, LOOSE TO MED. DENSE, SILTY SAND AND SAND WITH GRAVEL (A-2-4 AND A-1-b), MOIST
- (C) RESIDUAL, RED, BLACK, ORANGE AND TAN, MED. STIFF TO VERY STIFF, SANDY CLAY, SANDY SILT AND HIGHLY PLASTIC SILTY CLAY (A-6, A-4, AND A-7-6), MOIST TO WET
- (D) RESIDUAL, TAN, BLACK, GRAY AND ORANGE, MED. DENSE TO VERY DENSE, SILTY SAND (A-2-4), MOIST TO SATURATED
- (E) WEATHERED ROCK, BIOTITE GNEISS
- (F) ROADWAY EMBANKMENT, BROWN, SOFT, SILTY CLAY WITH LITTLE ORGANIC MATTER (A-7-6), MOIST
- (G) RESIDUAL, BROWN, MED. STIFF, SANDY CLAY AND SANDY SILT (A-6 AND A-4), MOIST TO WET

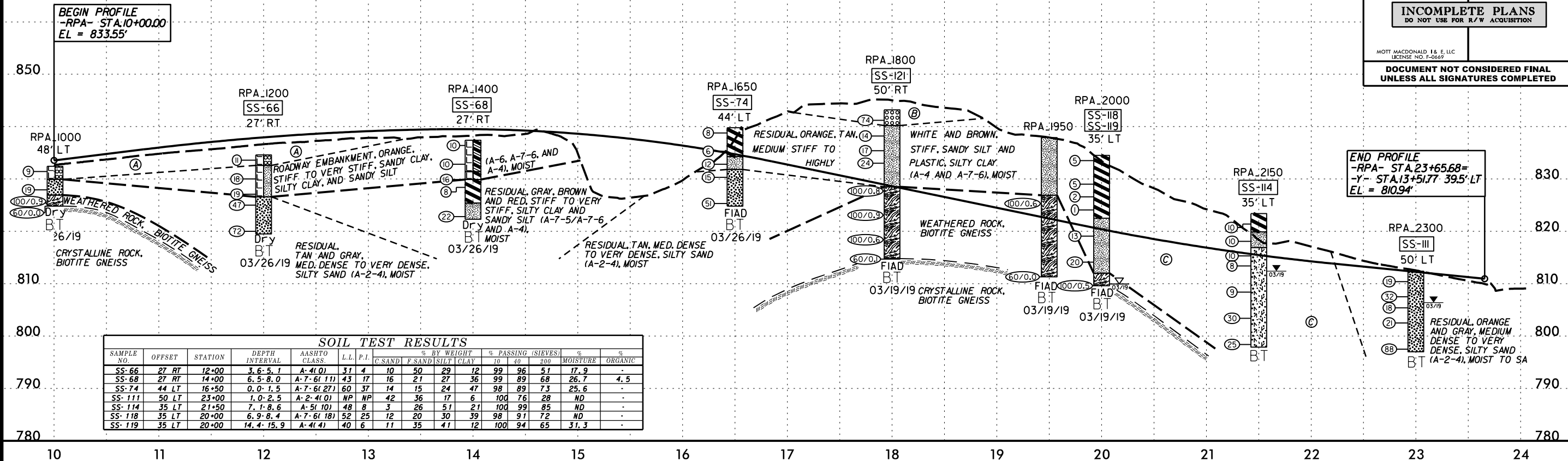
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		% MOISTURE		
							C SAND	F SAND	SILT	CLAY	10	40	200	W	ORGANIC
SS-10	CL	12+00	3.3-4.8	A-4(1)	32	6	13	28	52	7	100	93	71	11.4	-
SS-11	38 RT	42+00	3.4-4.9	A-2-4(1)	22	3	39	30	26	6	87	64	34	3.4	-
SS-64	50 LT	39+00	3.6-5.1	A-7-6(1)	41	19	9	31	34	25	98	94	66	28.0	5.9
SS-66	27 RT	12+00	3.6-5.1	A-4(1)	31	4	10	50	29	12	99	96	51	17.9	-
SS-68	27 RT	14+00	6.5-8.0	A-7-6(1)	43	17	16	21	27	36	99	89	68	26.7	4.5



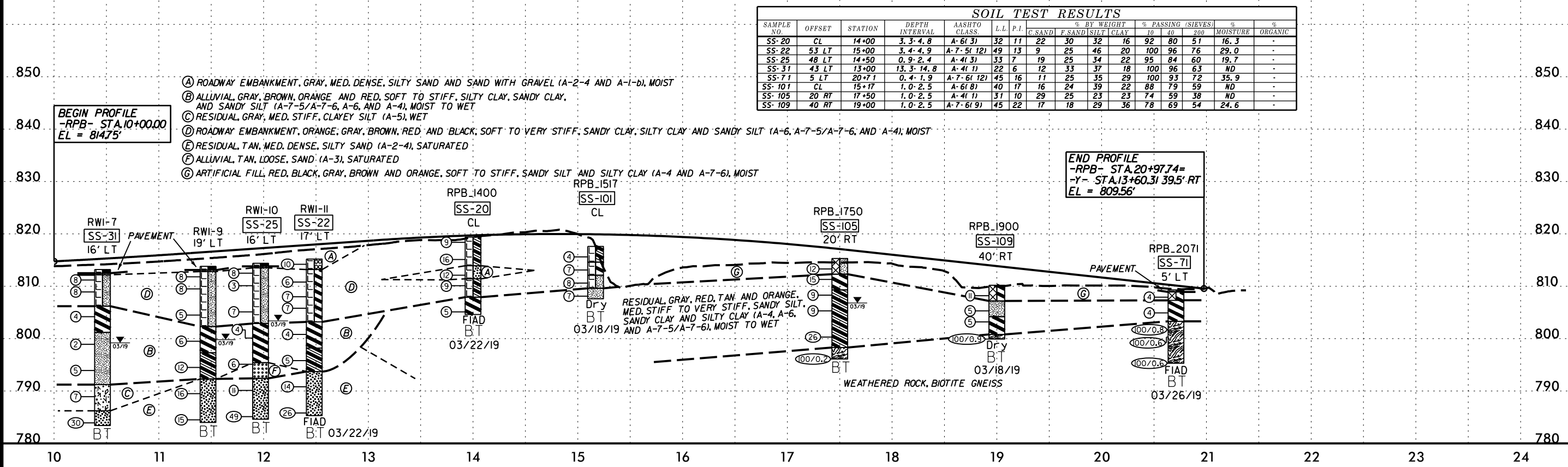
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- (A) ROADWAY EMBANKMENT, GRAY, LOOSE TO MED. DENSE, SAND WITH GRAVEL (A-1-b), MOIST
- (B) RESIDUAL, WHITE, VERY DENSE, SAND WITH GRAVEL (A-1-b), MOIST
- (C) RESIDUAL, RED, GRAY AND TAN, VERY SOFT TO VERY STIFF, SILTY CLAY; SANDY SILT AND CLAYEY SILT (A-7-6, A-4, AND A-5), MOIST TO WET

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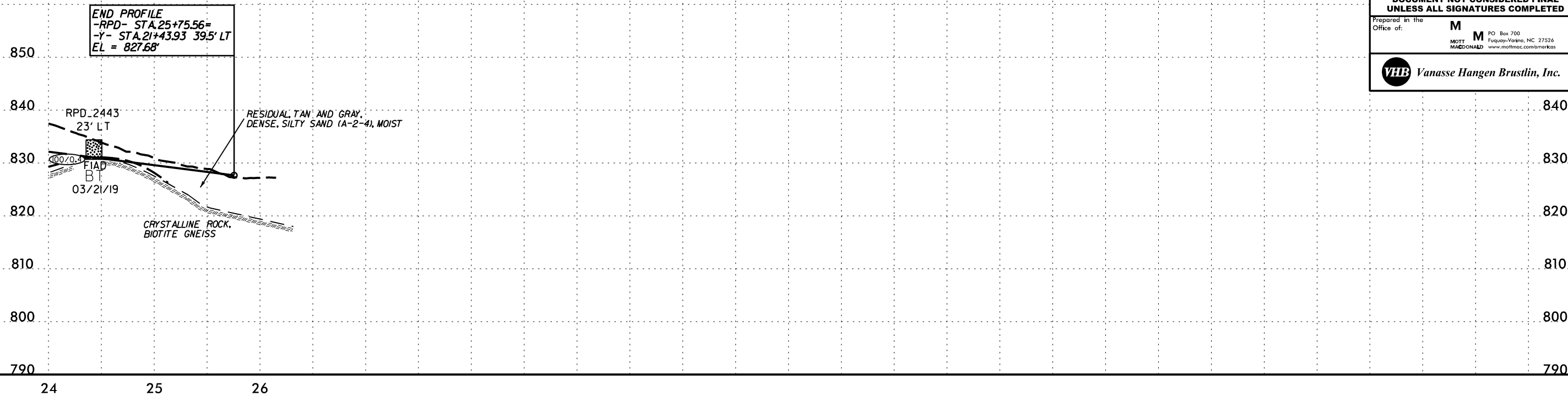
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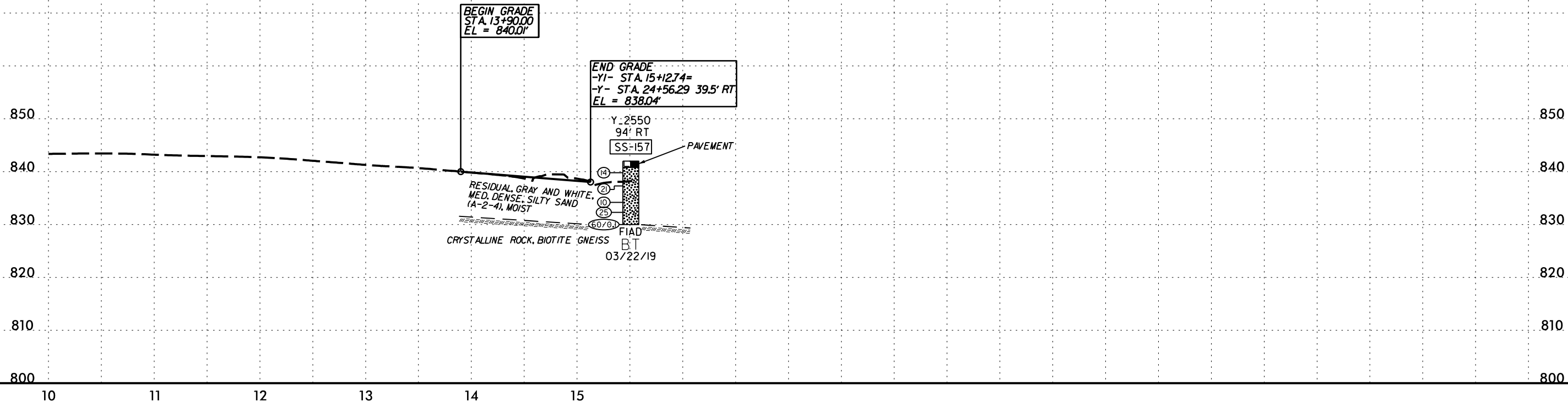
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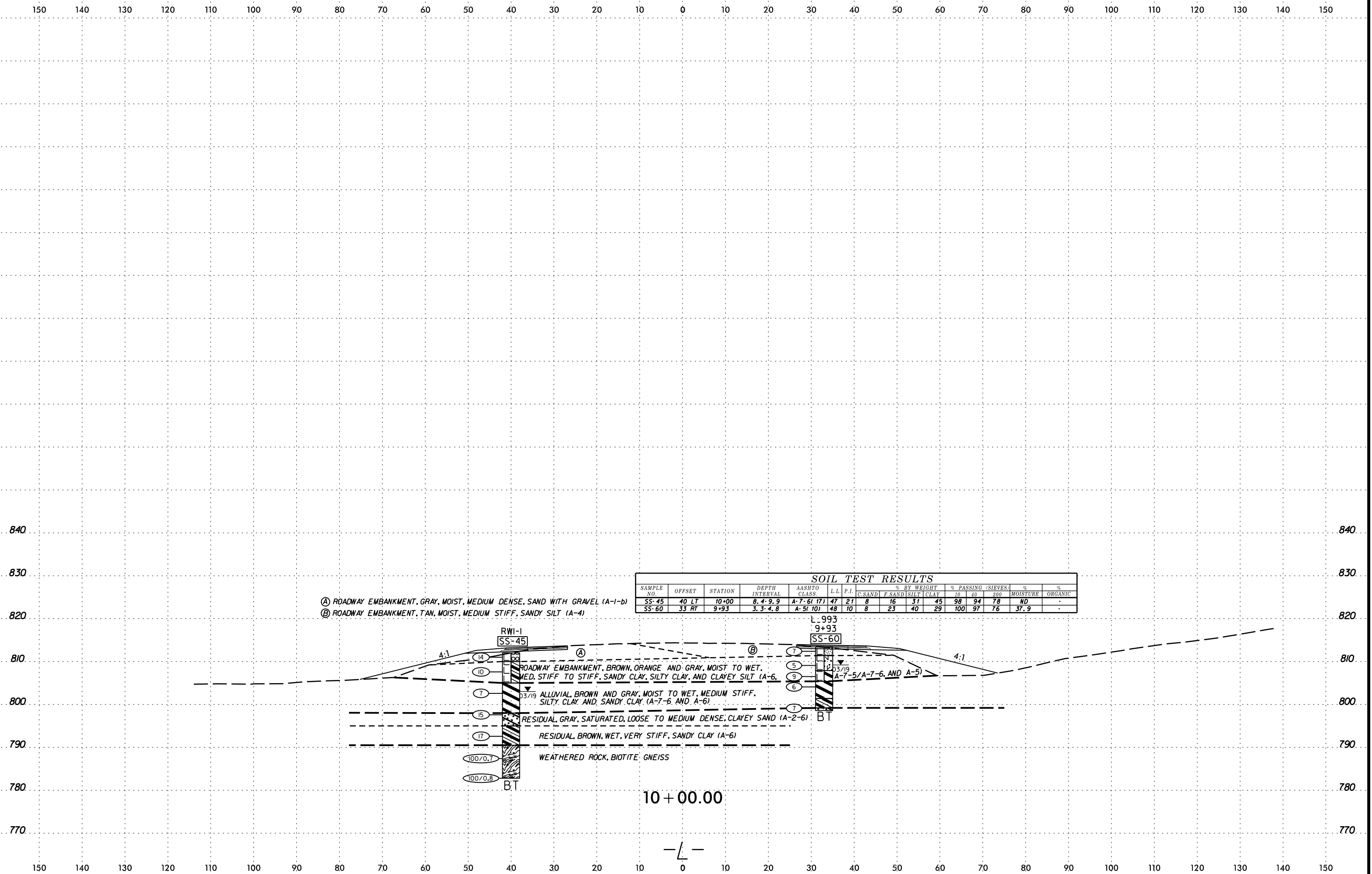
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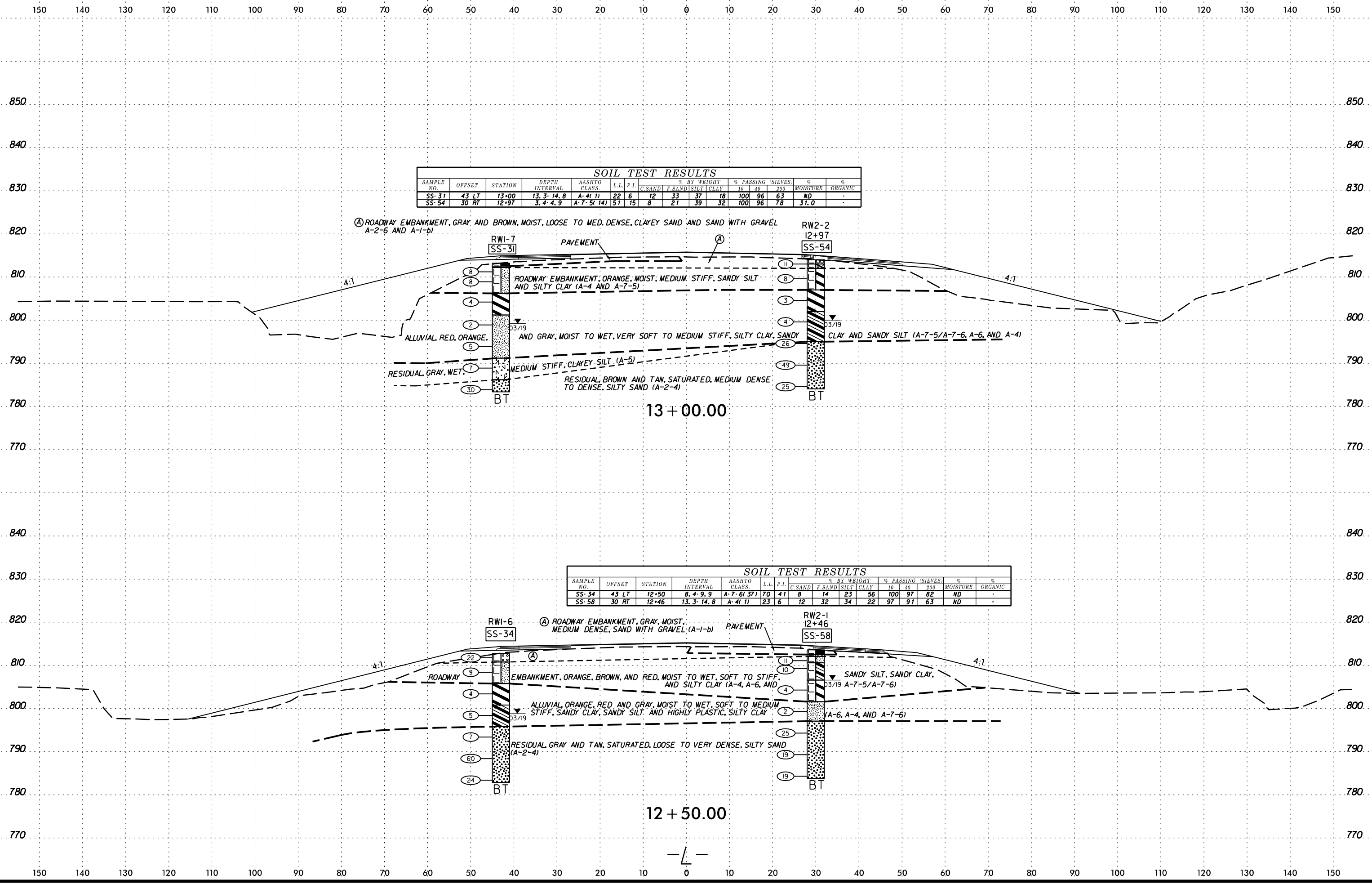
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VHB Vanasse Hangen Brustlin, Inc.	

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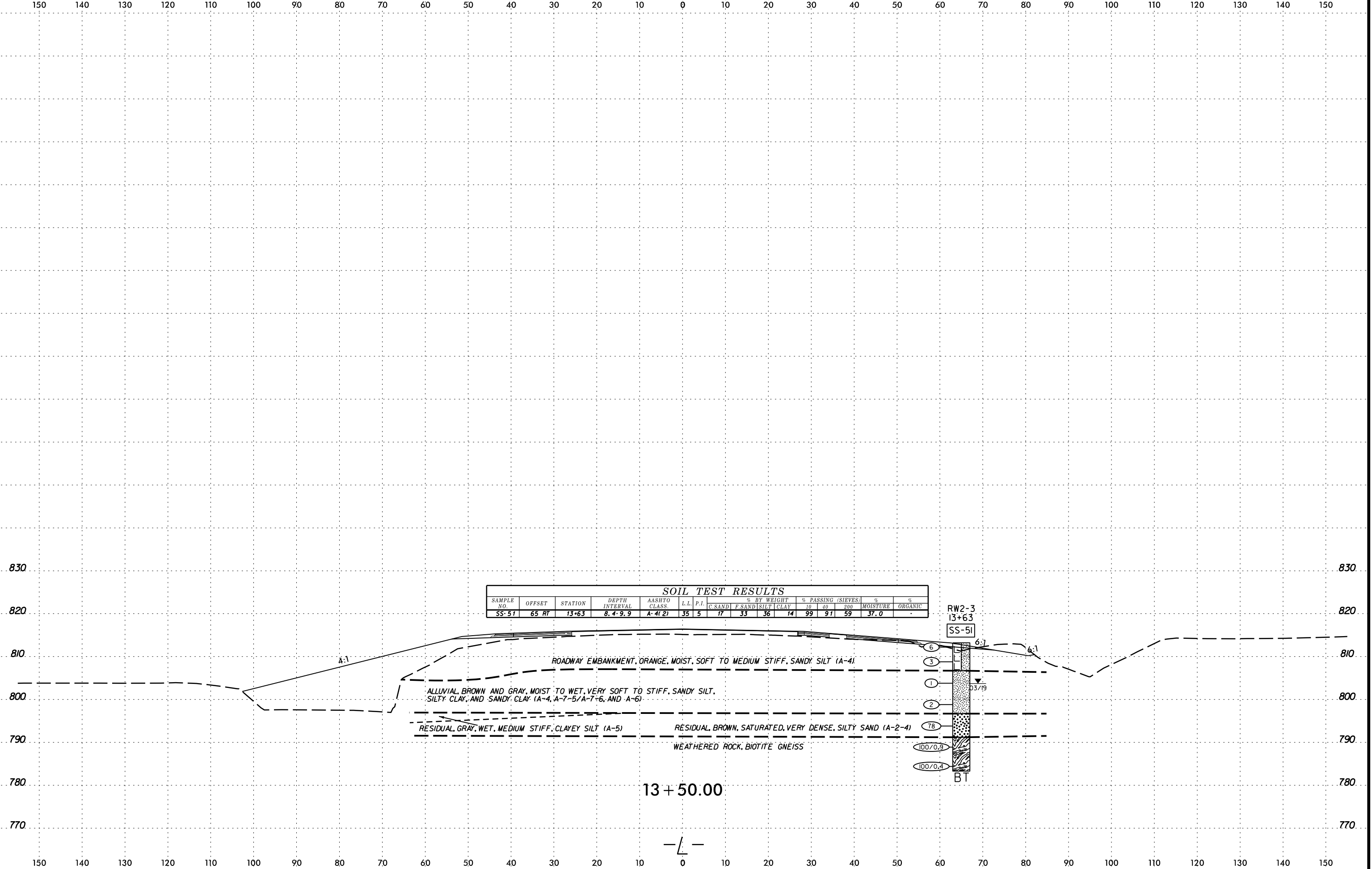
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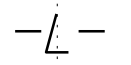


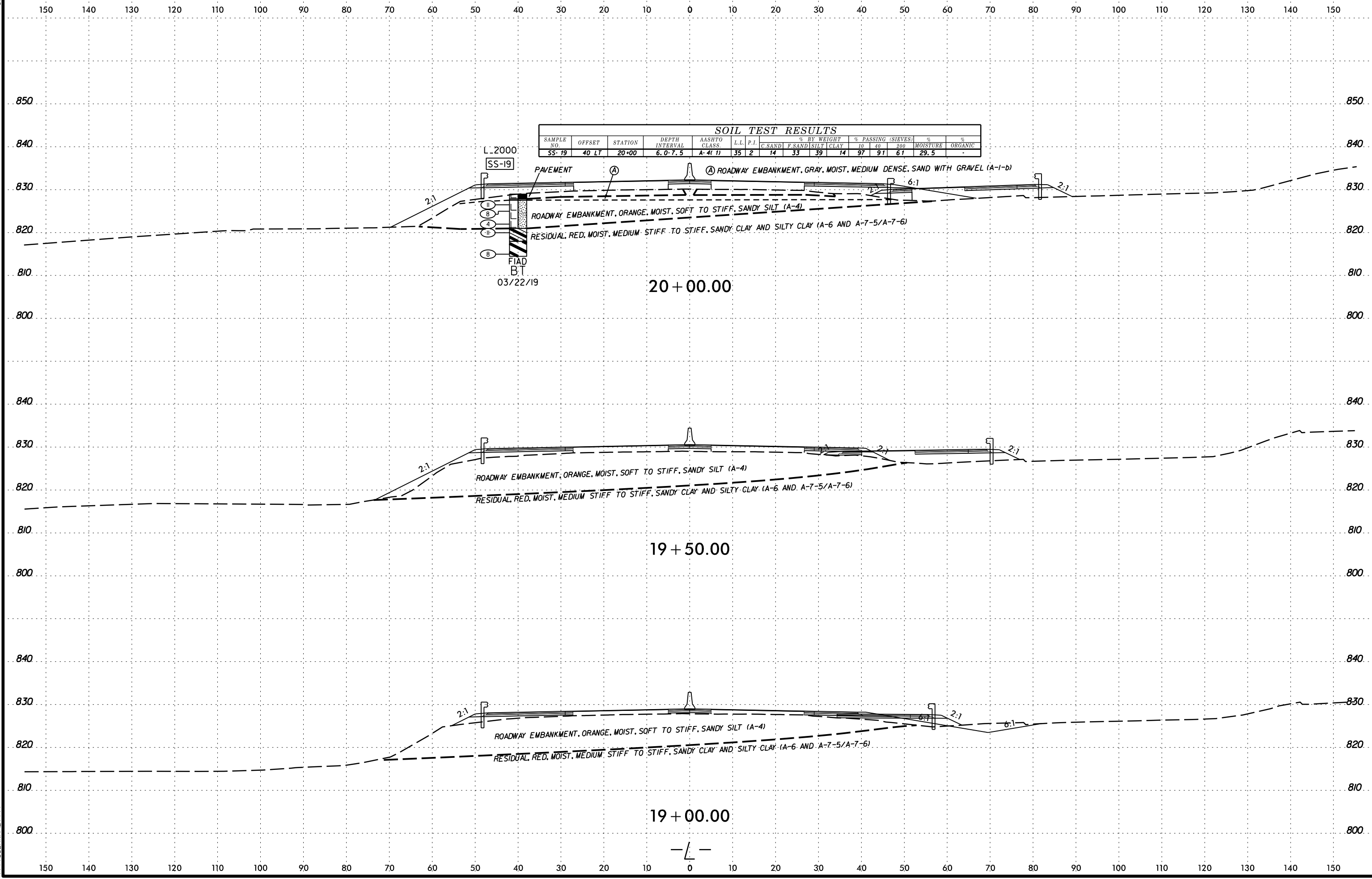
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		MOISTURE	ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40			200
SS-51	65 RT	13+63	8.4-9.9	A-4(2)	35	5	17	33	36	14	99	91	59	37.0	-

13 + 50.00

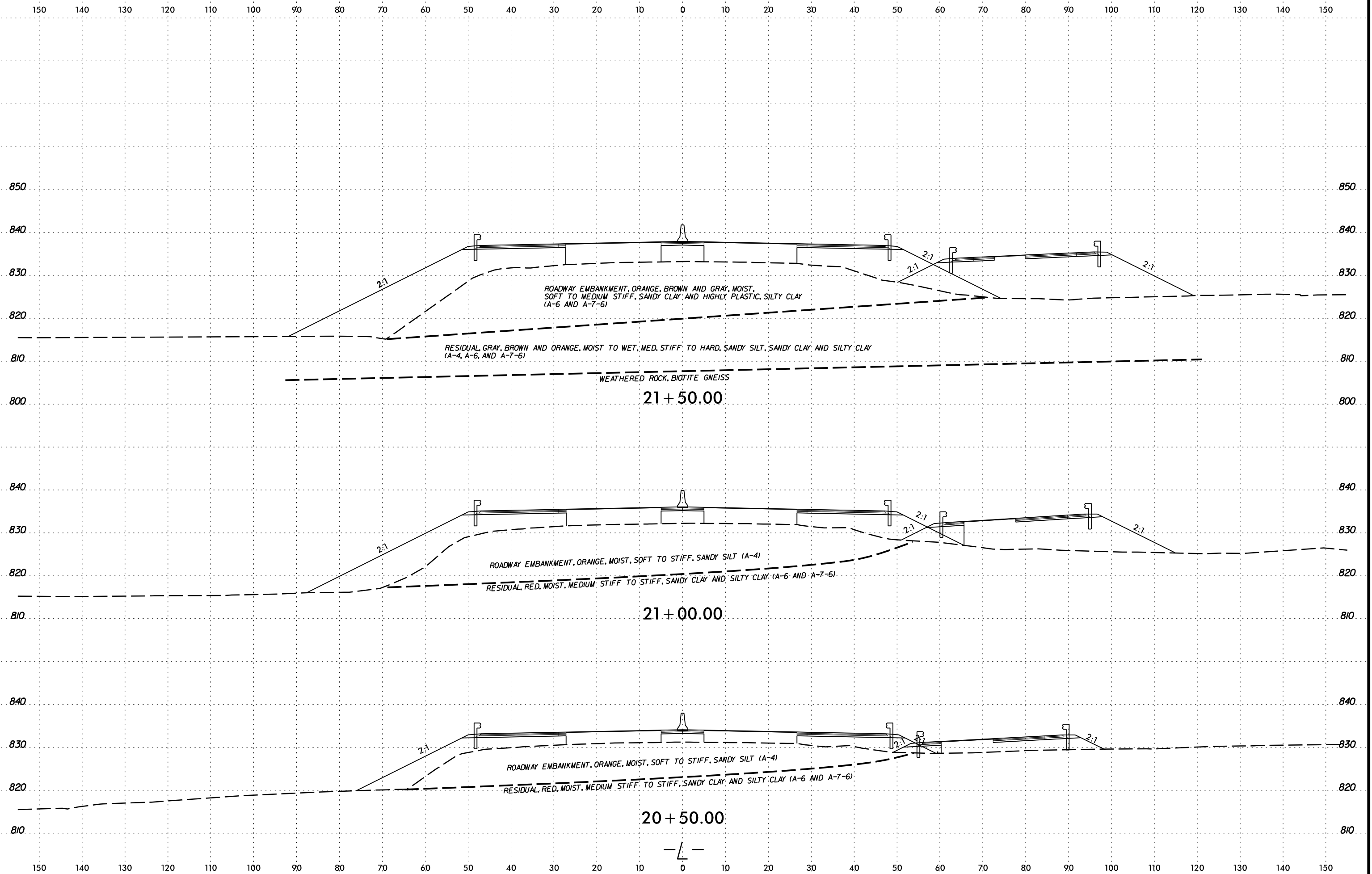


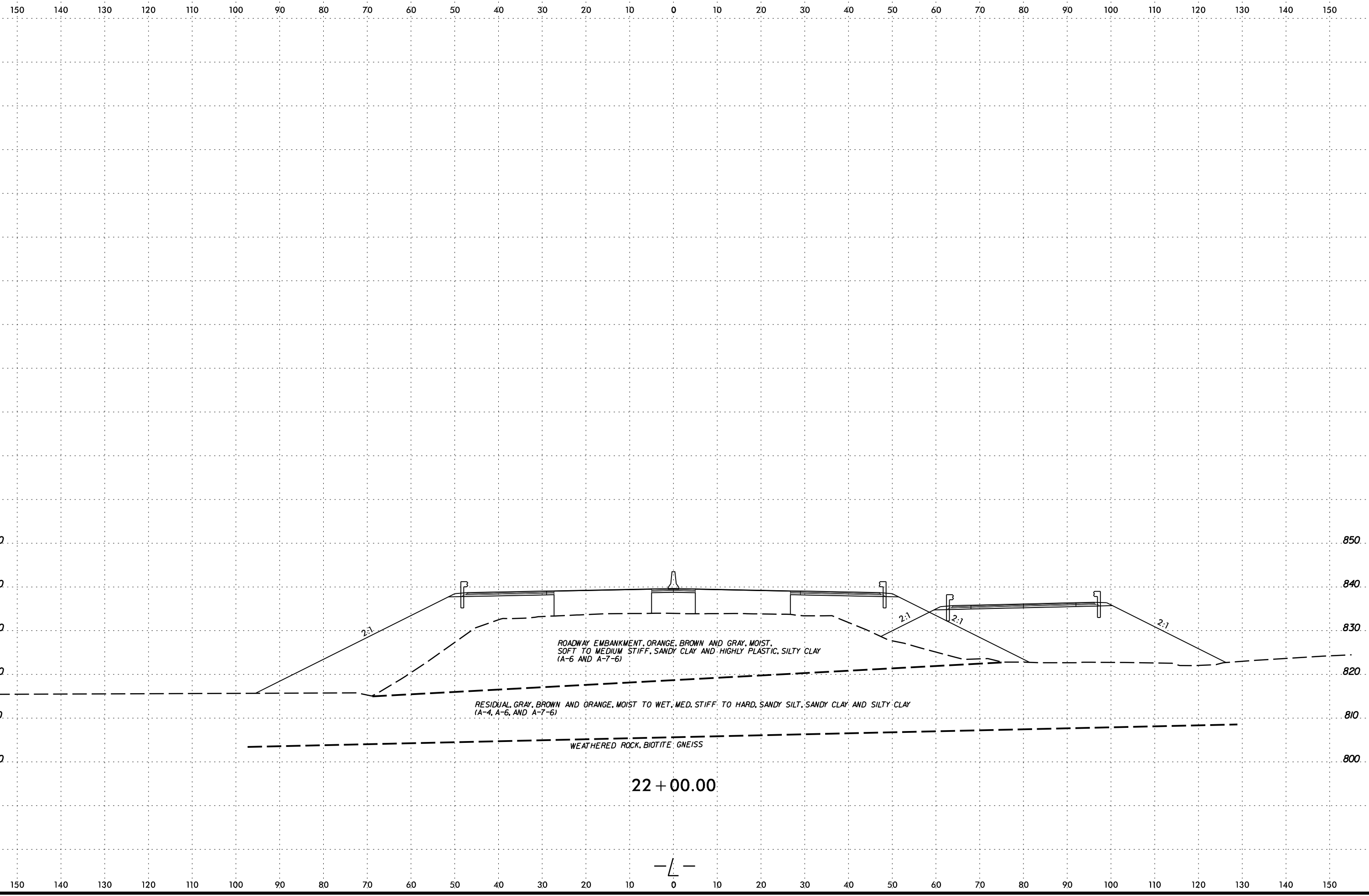


SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		% MOISTURE ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40	200	
SS-19	40 LT	20+00	6.0-7.5	A-4(1)	35	2	14	33	39	14	97	91	61	29.5

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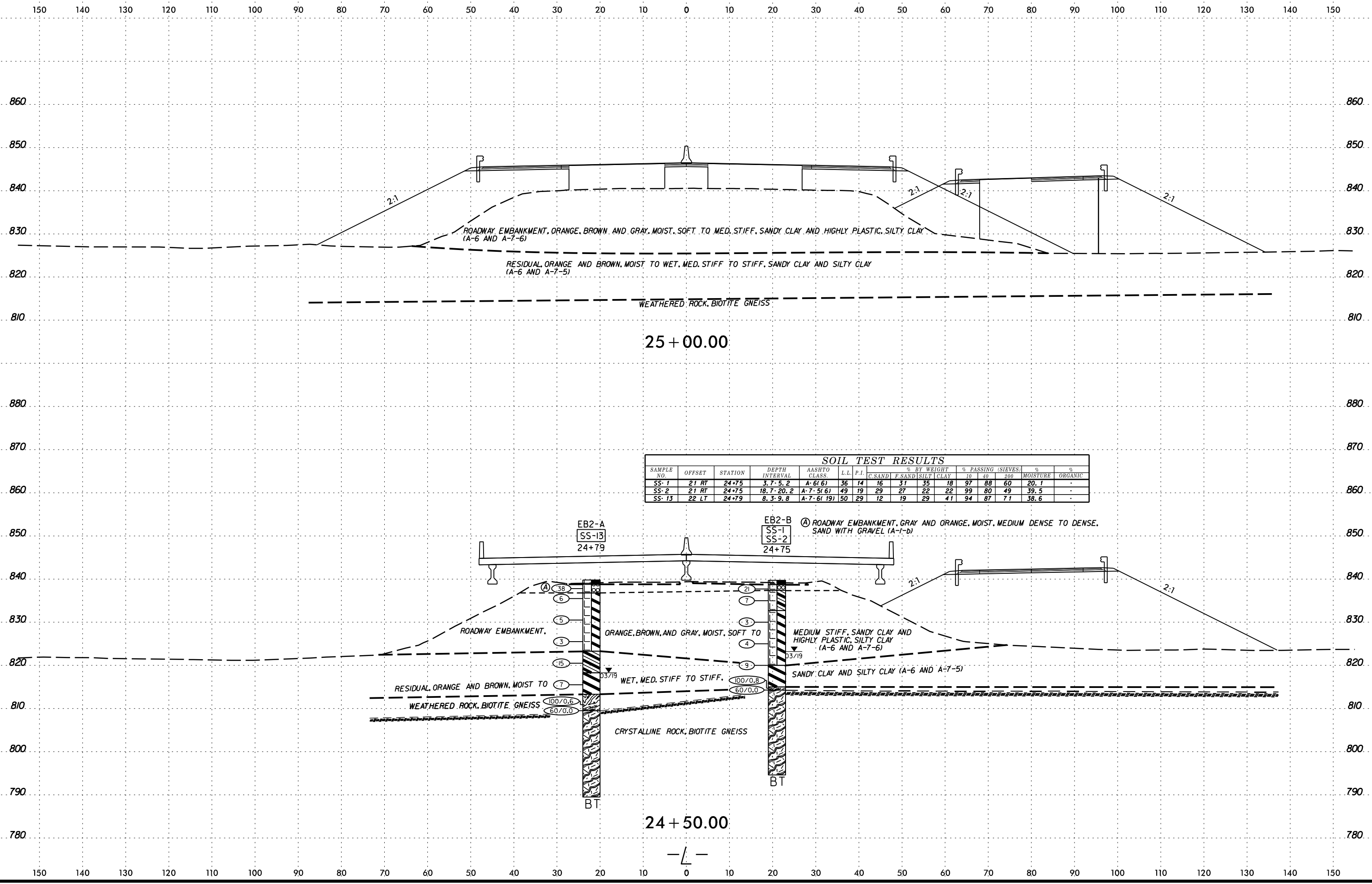
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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT	CLAY	10	40			200
SS-1	21 RT	24+75	3.7-5.2	A-6(6)	36	14	16	31	35	18	97	88	60	20.1	-
SS-2	21 RT	24+75	18.7-20.2	A-7-5(6)	49	19	29	27	22	22	99	80	49	39.5	-
SS-13	22 LT	24+79	8.3-9.8	A-7-6(19)	50	29	12	19	29	41	94	87	71	38.6	-

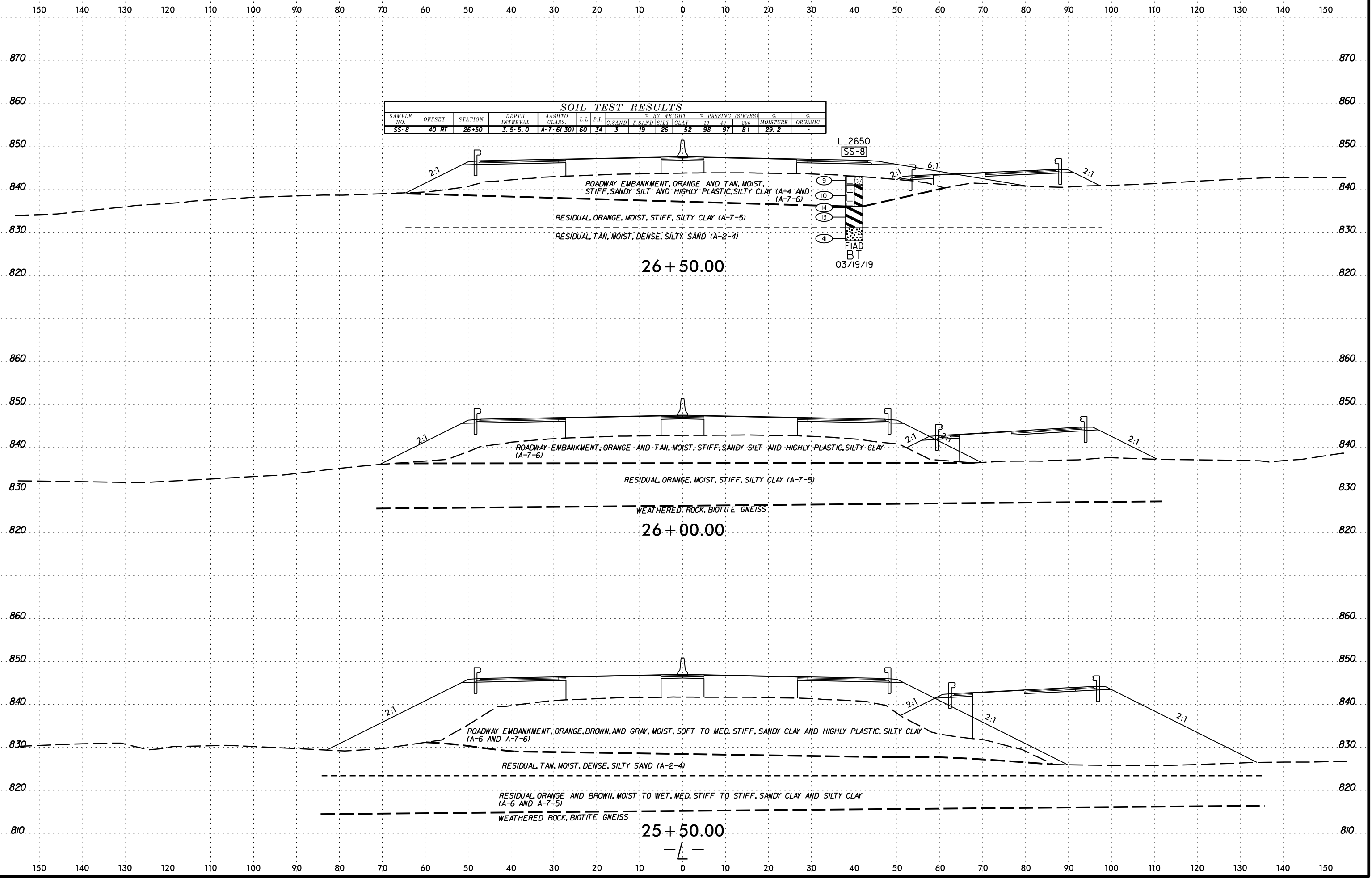
EB2-A
 SS-13
 24+79

EB2-B
 SS-1
 SS-2
 24+75

(A) ROADWAY EMBANKMENT, GRAY AND ORANGE, MOIST, MEDIUM DENSE TO DENSE, SAND WITH GRAVEL (A-1-b)

24 + 50.00

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SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)		% MOISTURE	% ORGANIC	
							C.SAND	F.SAND	SILT CLAY	10	40			200
SS-8	40 RT	26+50	3.5-5.0	A-7-6(30)	60	34	3	19	26	52	98	97	81	29.2

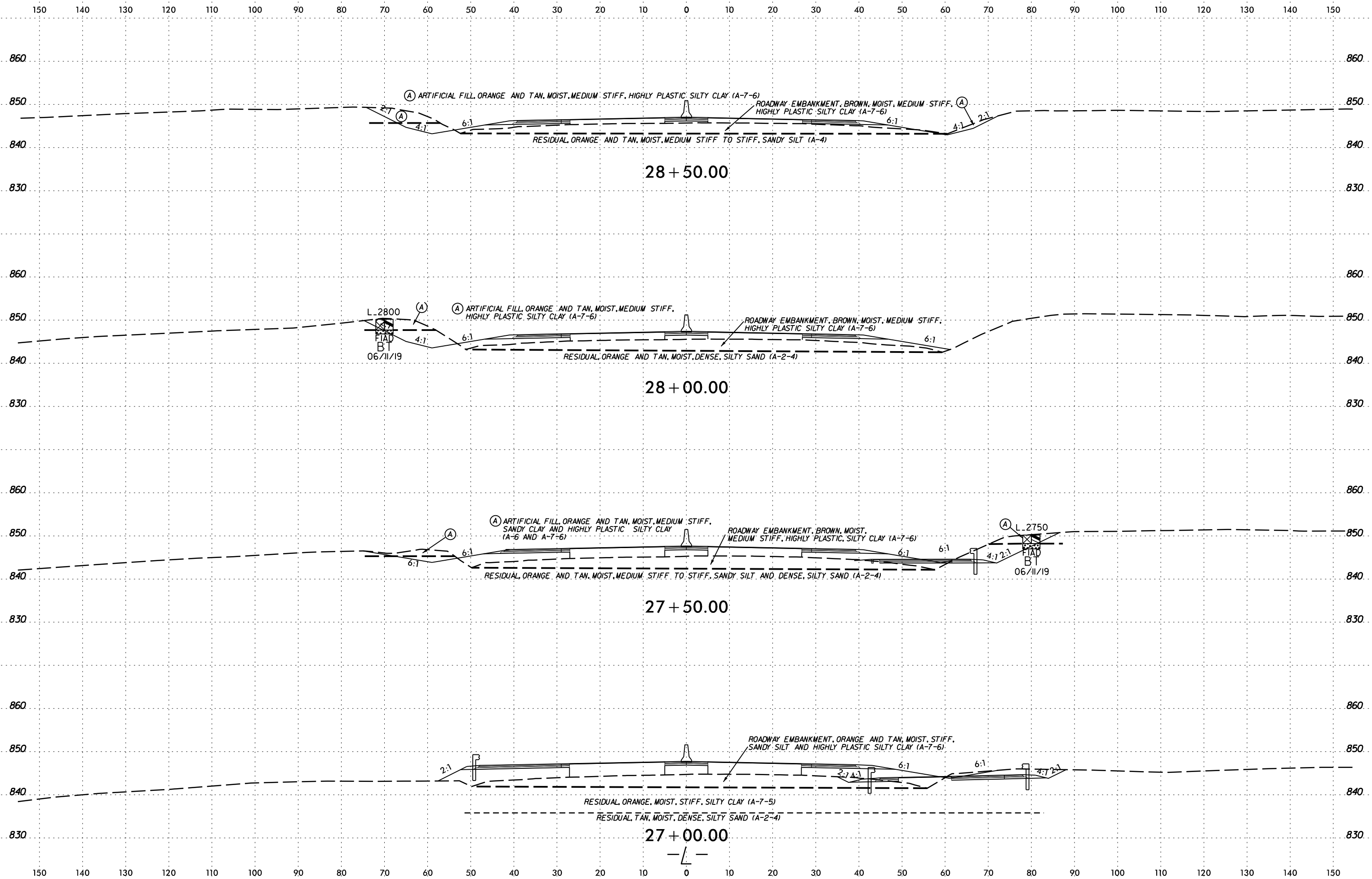
L-2650
 [SS-8]
 9
 10
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26 + 50.00

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25 + 50.00

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28 + 50.00

28 + 00.00

27 + 50.00

27 + 00.00

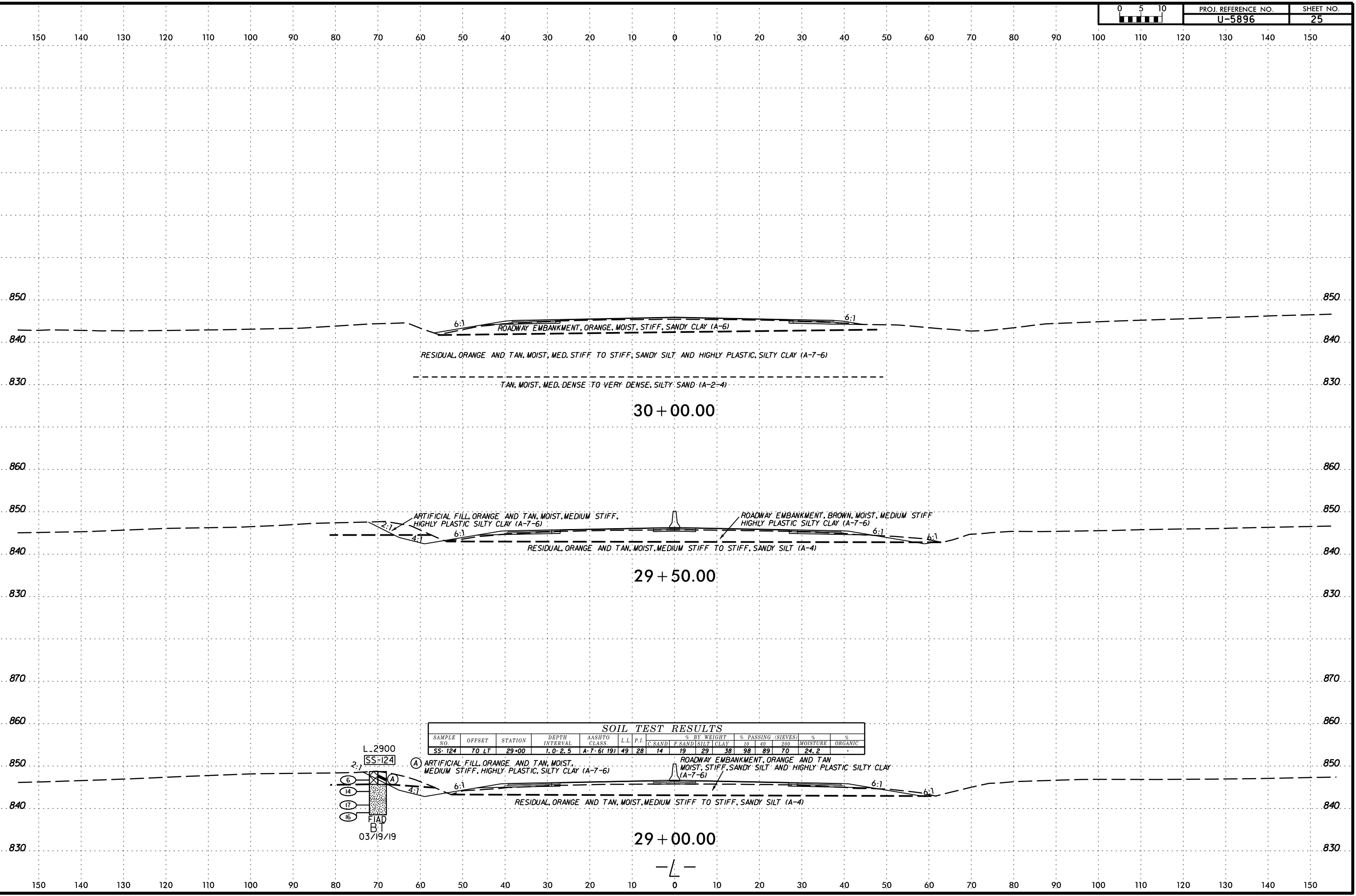
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RESIDUAL, ORANGE AND TAN, MOIST, MEDIUM STIFF TO STIFF, SANDY SILT (A-4)

L 2800
FIAD
BT
06/11/19
(A) ARTIFICIAL FILL, ORANGE AND TAN, MOIST, MEDIUM STIFF, HIGHLY PLASTIC SILTY CLAY (A-7-6)
ROADWAY EMBANKMENT, BROWN, MOIST, MEDIUM STIFF, HIGHLY PLASTIC SILTY CLAY (A-7-6)
RESIDUAL, ORANGE AND TAN, MOIST, DENSE, SILTY SAND (A-2-4)

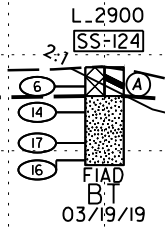
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ROADWAY EMBANKMENT, BROWN, MOIST, MEDIUM STIFF, HIGHLY PLASTIC, SILTY CLAY (A-7-6)
RESIDUAL, ORANGE AND TAN, MOIST, MEDIUM STIFF TO STIFF, SANDY SILT AND DENSE, SILTY SAND (A-2-4)
L 2750
FIAD
BT
06/11/19

ROADWAY EMBANKMENT, ORANGE AND TAN, MOIST, STIFF, SANDY SILT AND HIGHLY PLASTIC SILTY CLAY (A-7-6)
RESIDUAL, ORANGE, MOIST, STIFF, SILTY CLAY (A-7-5)
RESIDUAL, TAN, MOIST, DENSE, SILTY SAND (A-2-4)

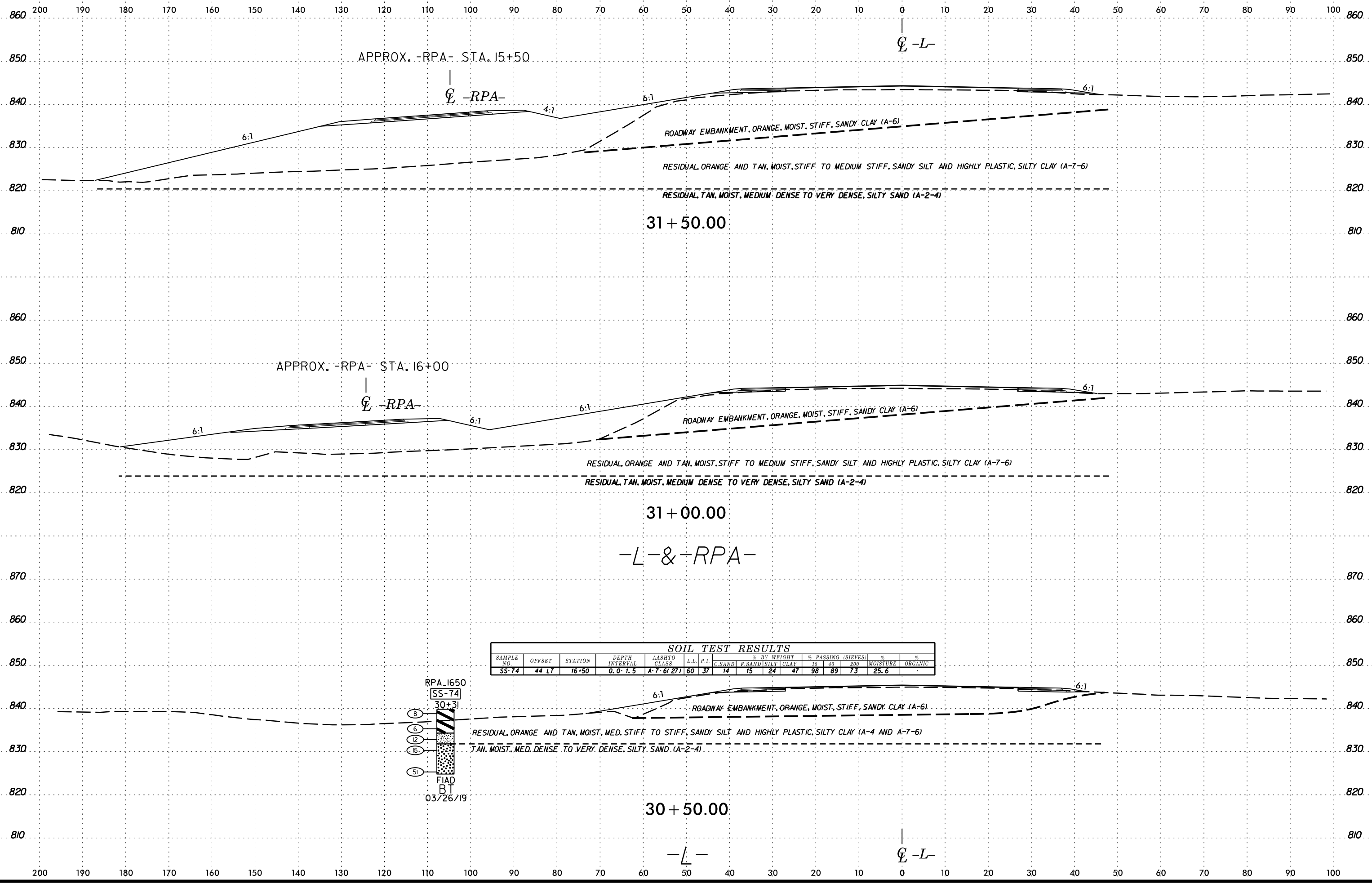
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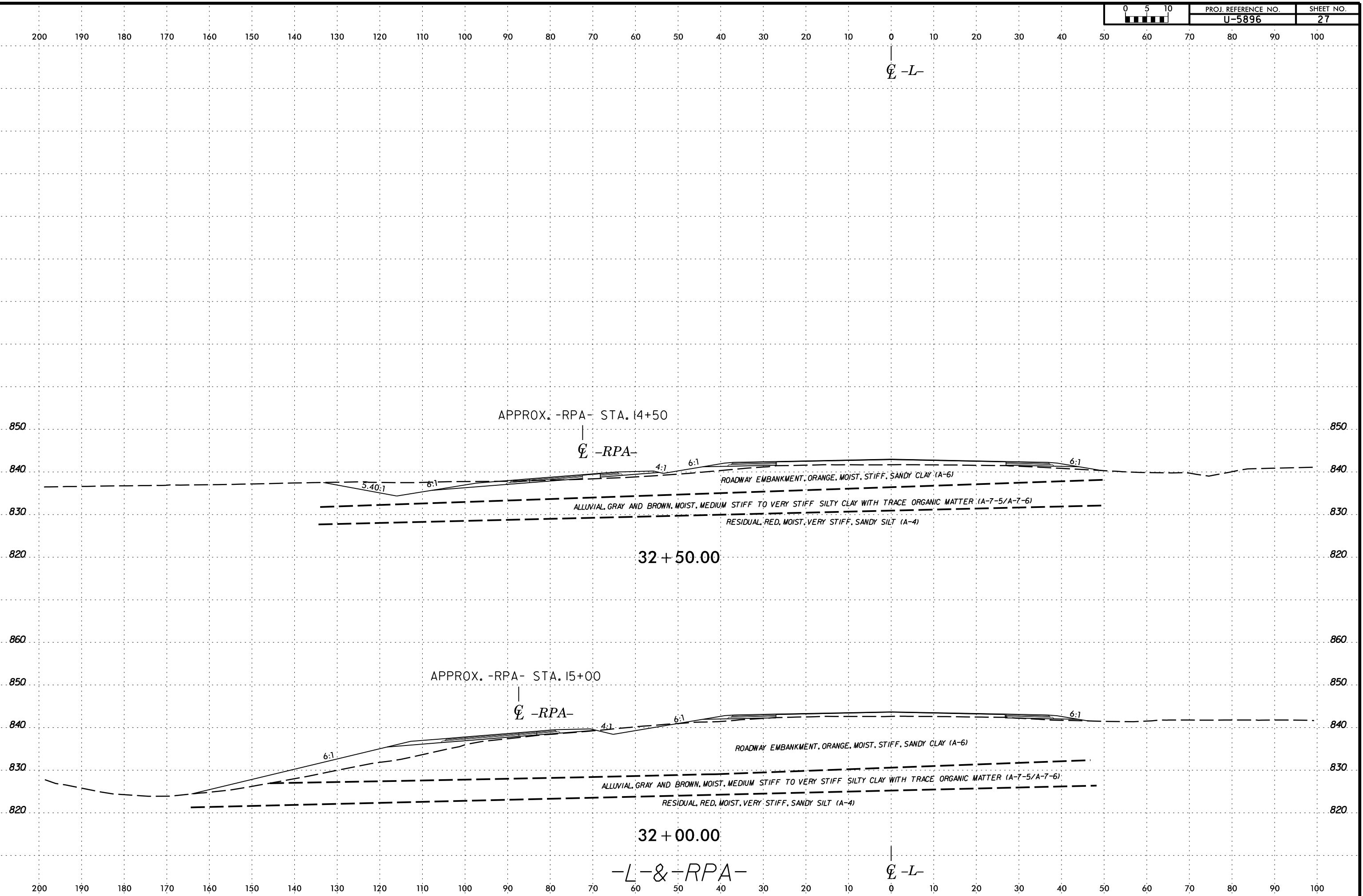
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SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)		% MOISTURE	% ORGANIC	
							C. SAND	F. SAND	SILT/CLAY	10	40			200
SS-124	70 LT	29+00	1.0-2.5	A-7-6(19)	49	28	14	19	29	38	98	89	70	24.2

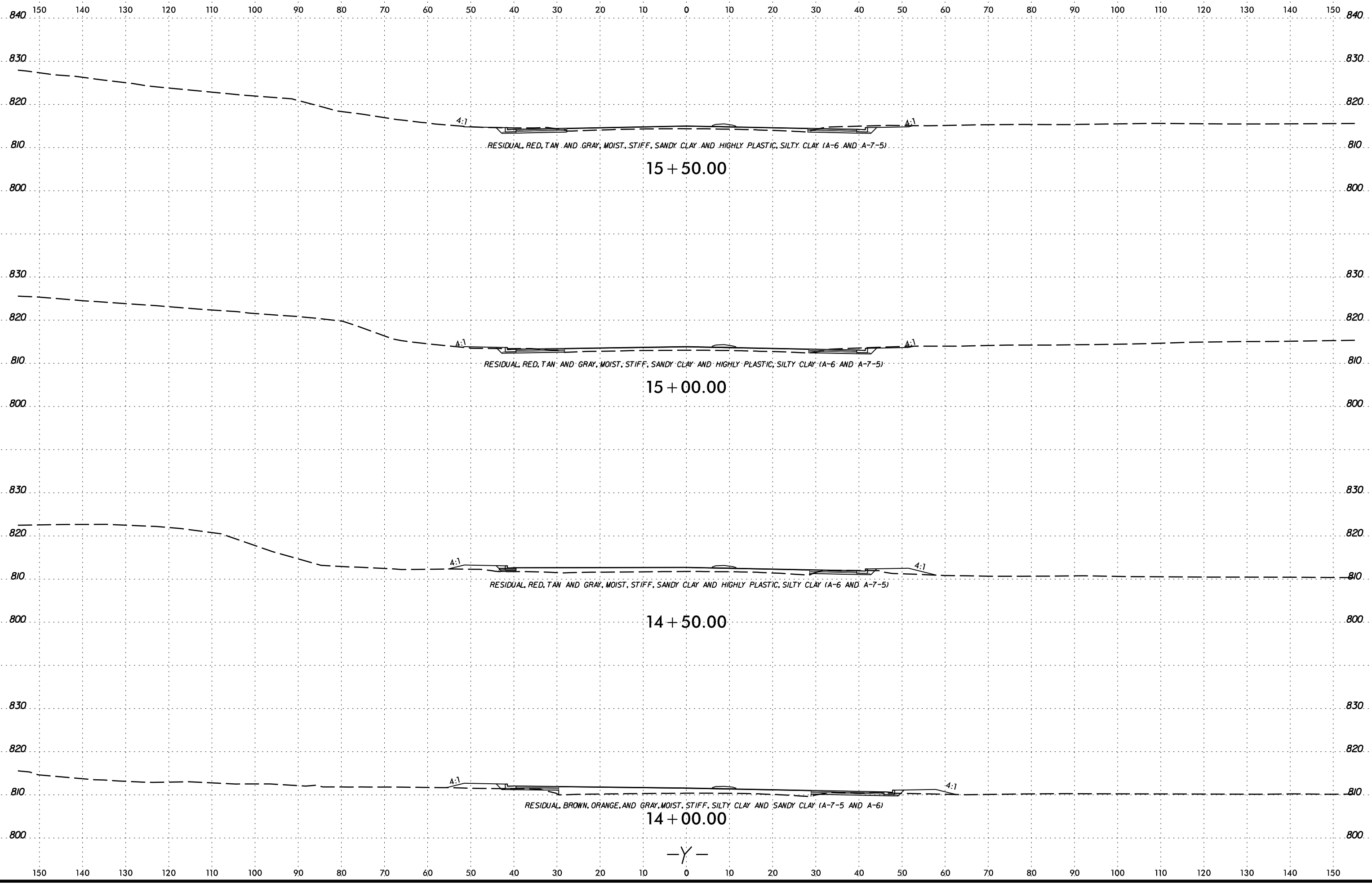


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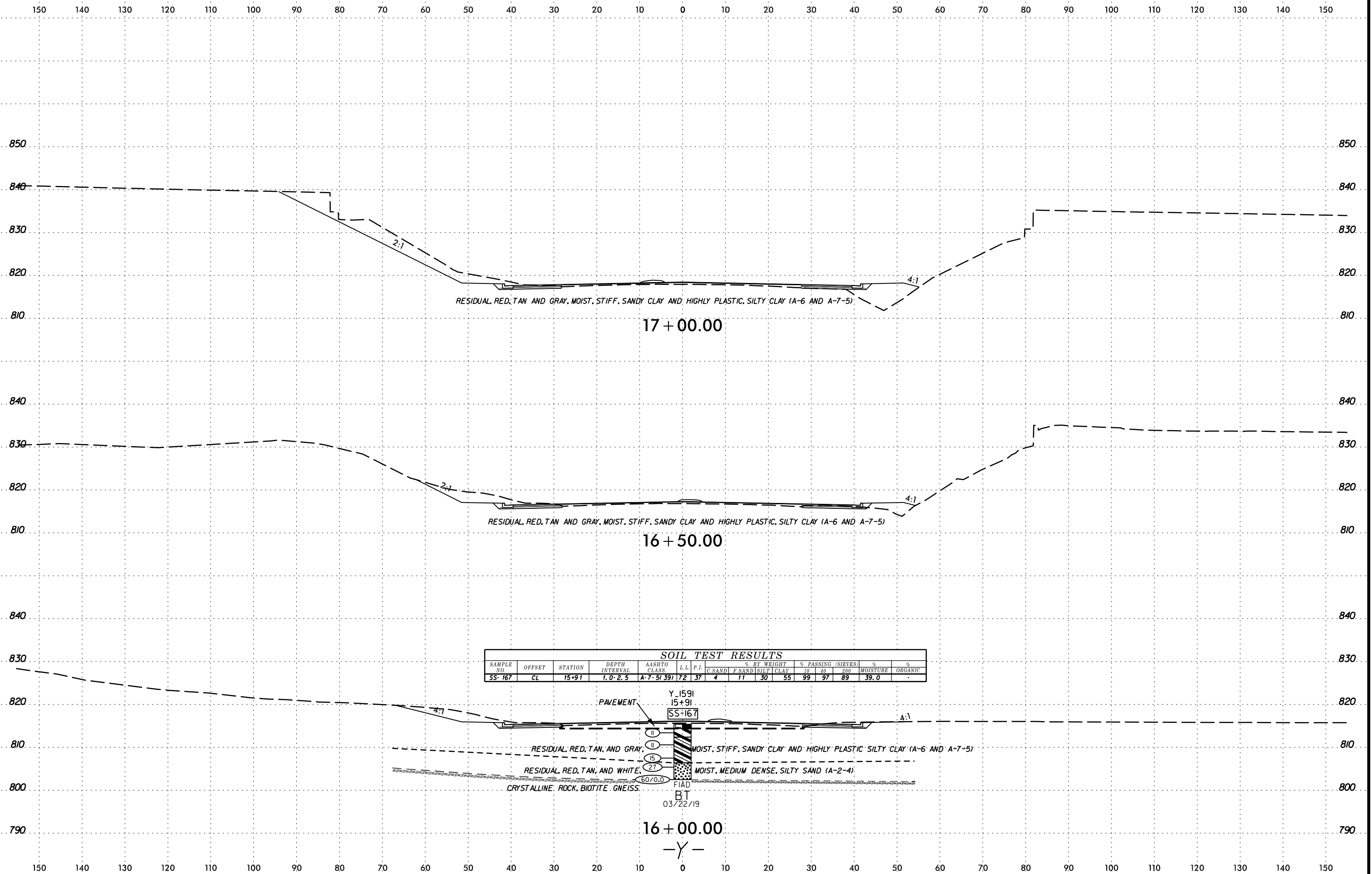


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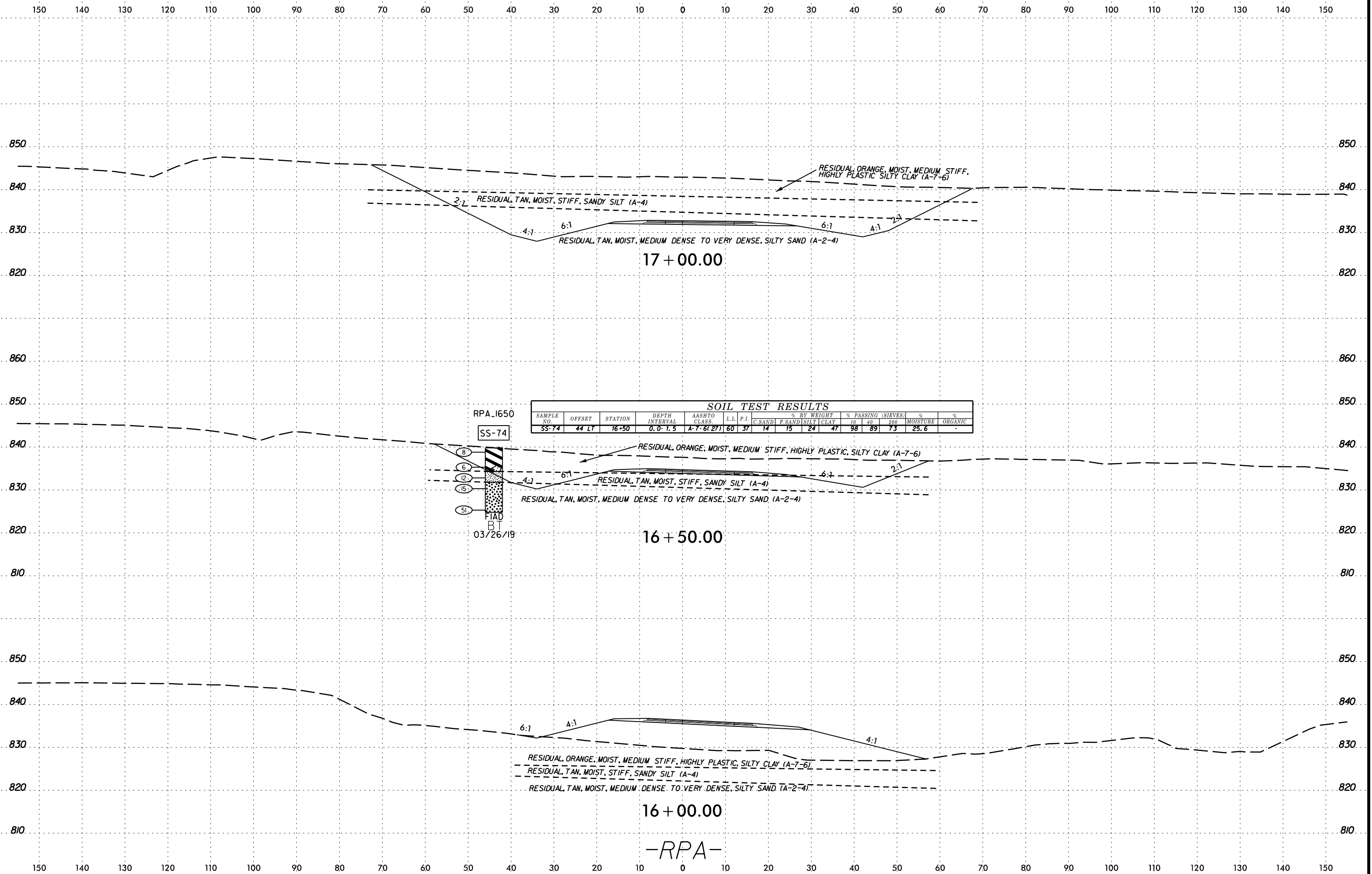




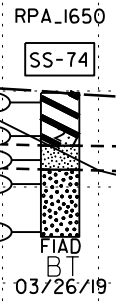
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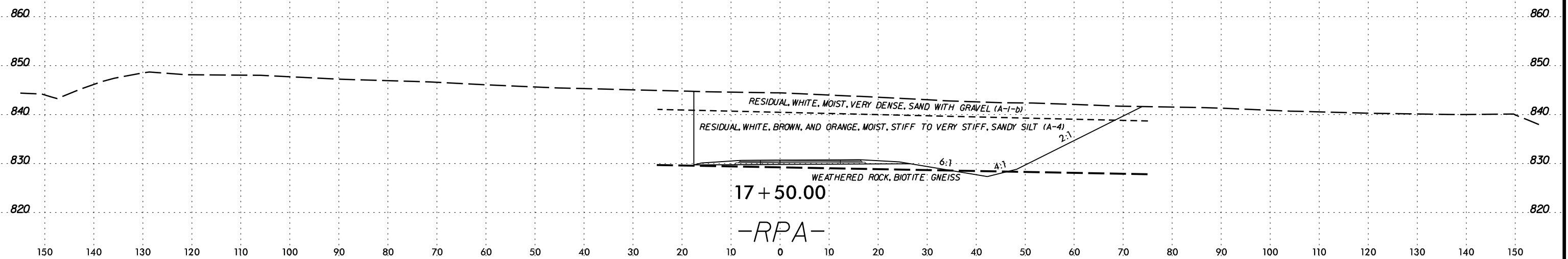
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							C. SAND	F. SAND	SILT	CLAY	10	40		
SS-74	44 LT	16+50	0.0 - 1.5	A-7-6(2)	60	37	14	15	24	47	98	89	73	25.6

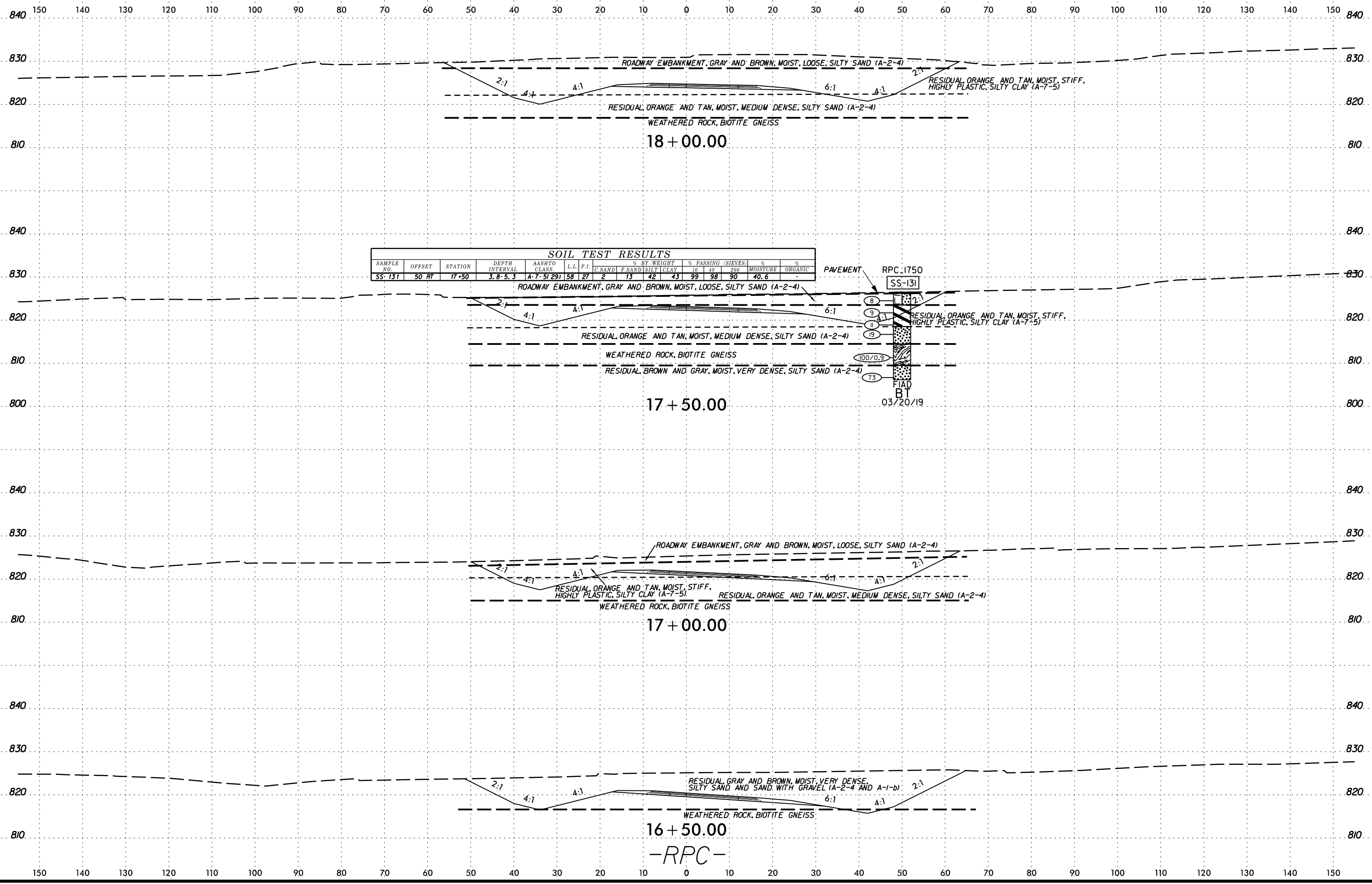




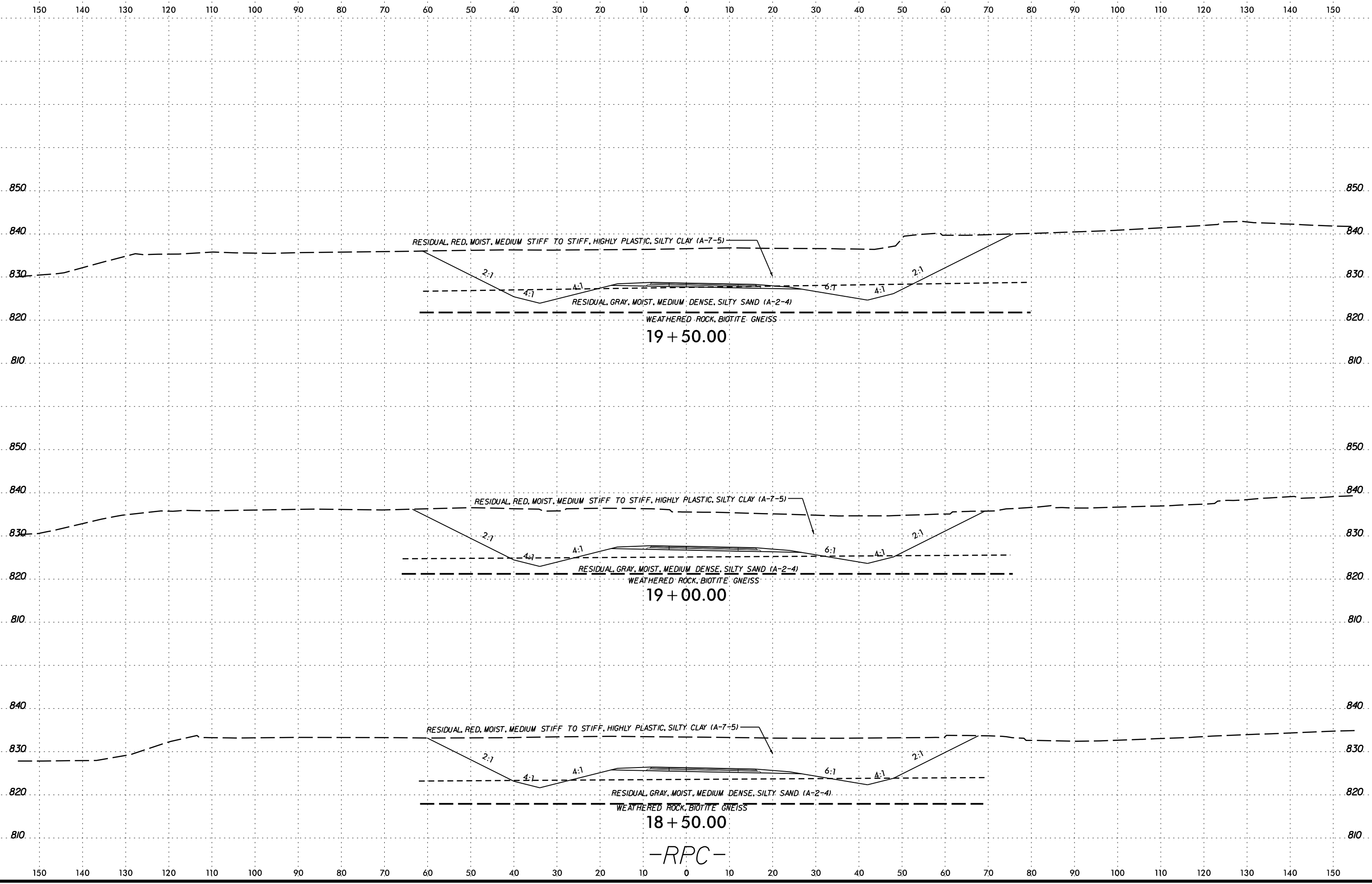
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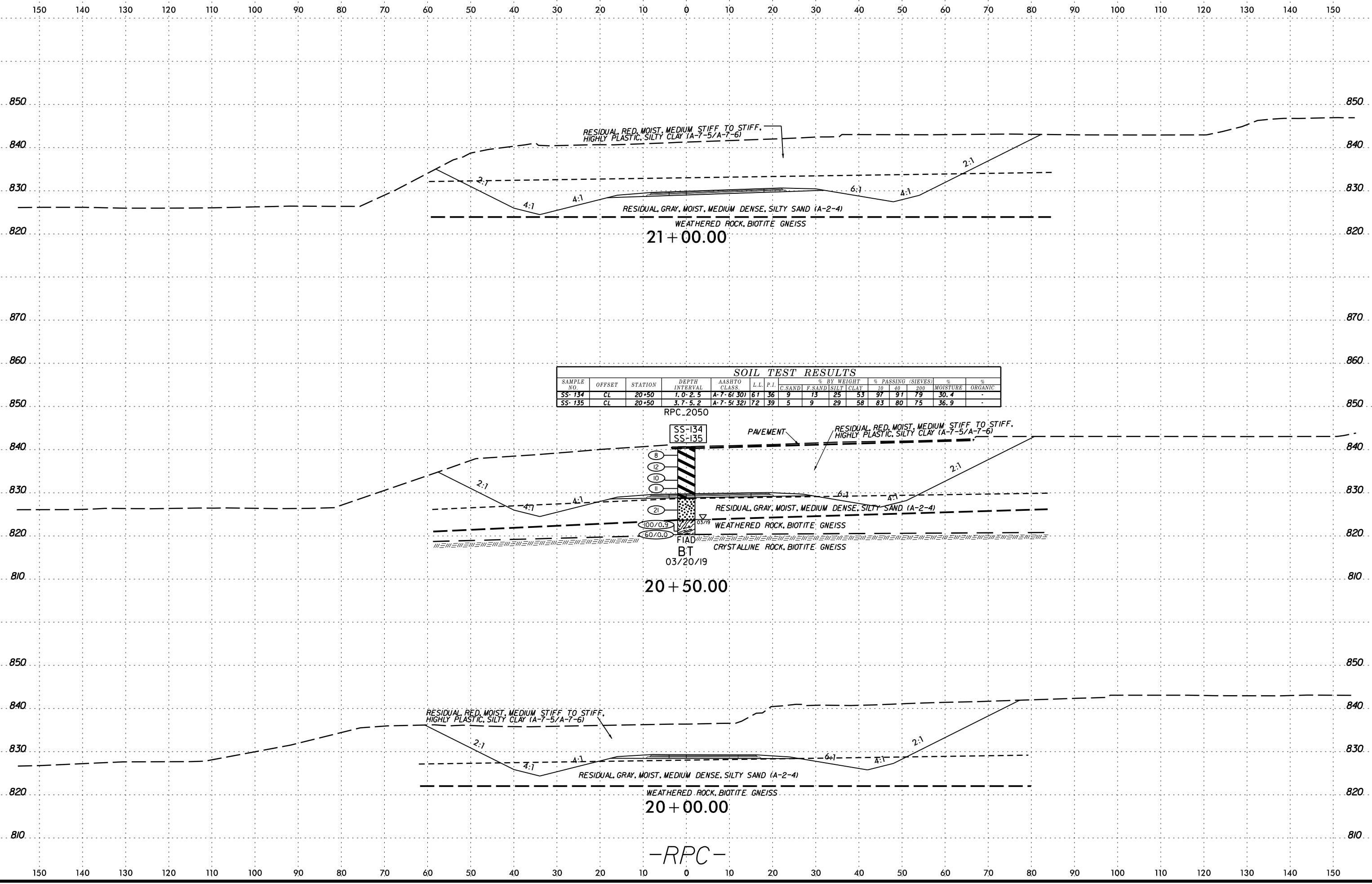


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SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-134	CL	20+50	1.0-2.5	A-7-6(30)	61	36	9	13	25	53	97	91	79	30.4	-
SS-135	CL	20+50	3.7-5.2	A-7-5(32)	72	39	5	9	29	58	83	80	75	36.9	-

RPC_2050

SS-134
SS-135

PAVEMENT

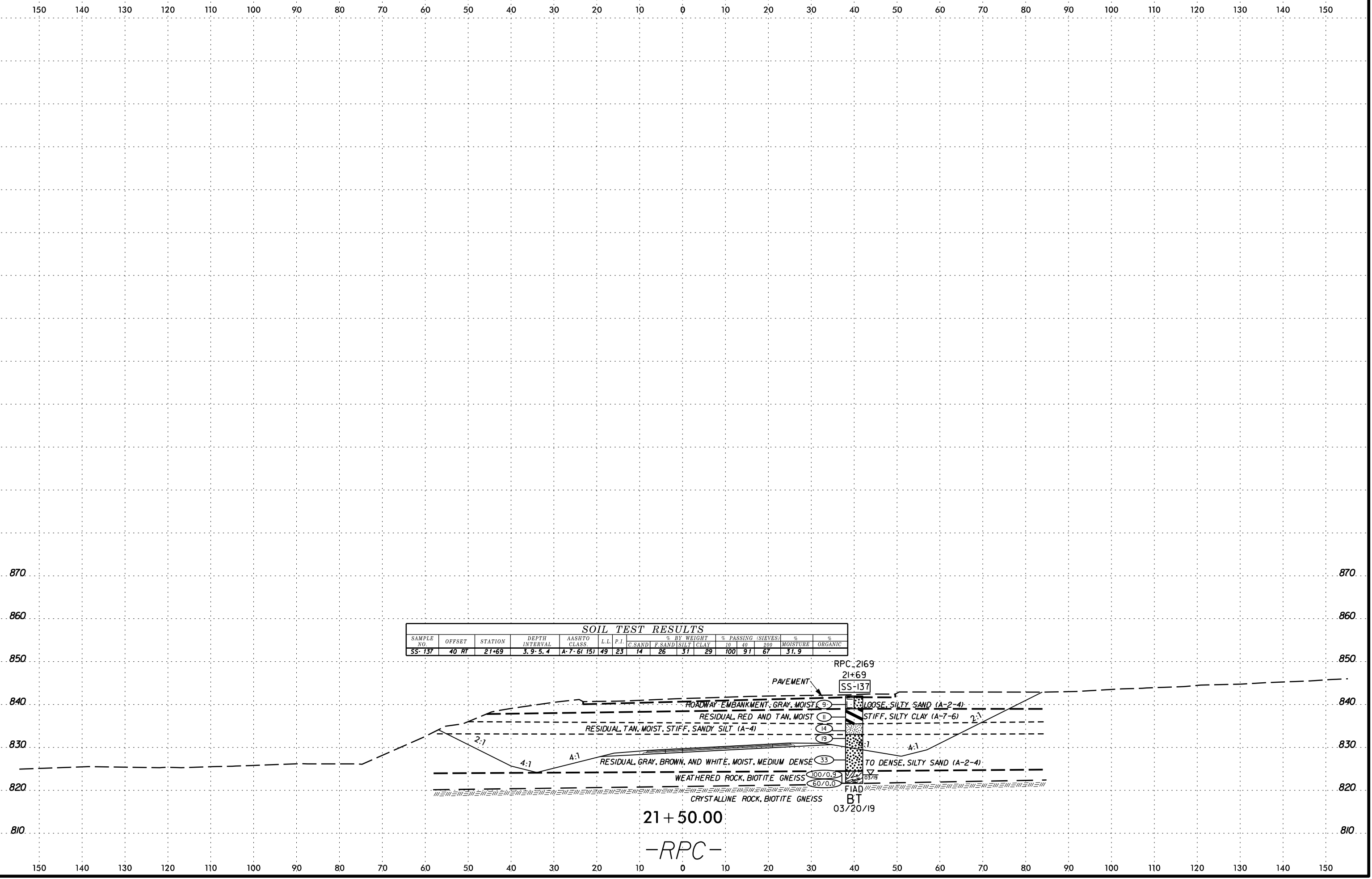
(8)
 (12)
 (10)
 (11)
 (21)
 (100/0.9)
 (60/0.0)
 FIAD
 BT
 03/20/19

20+50.00

20+00.00

-RPC-

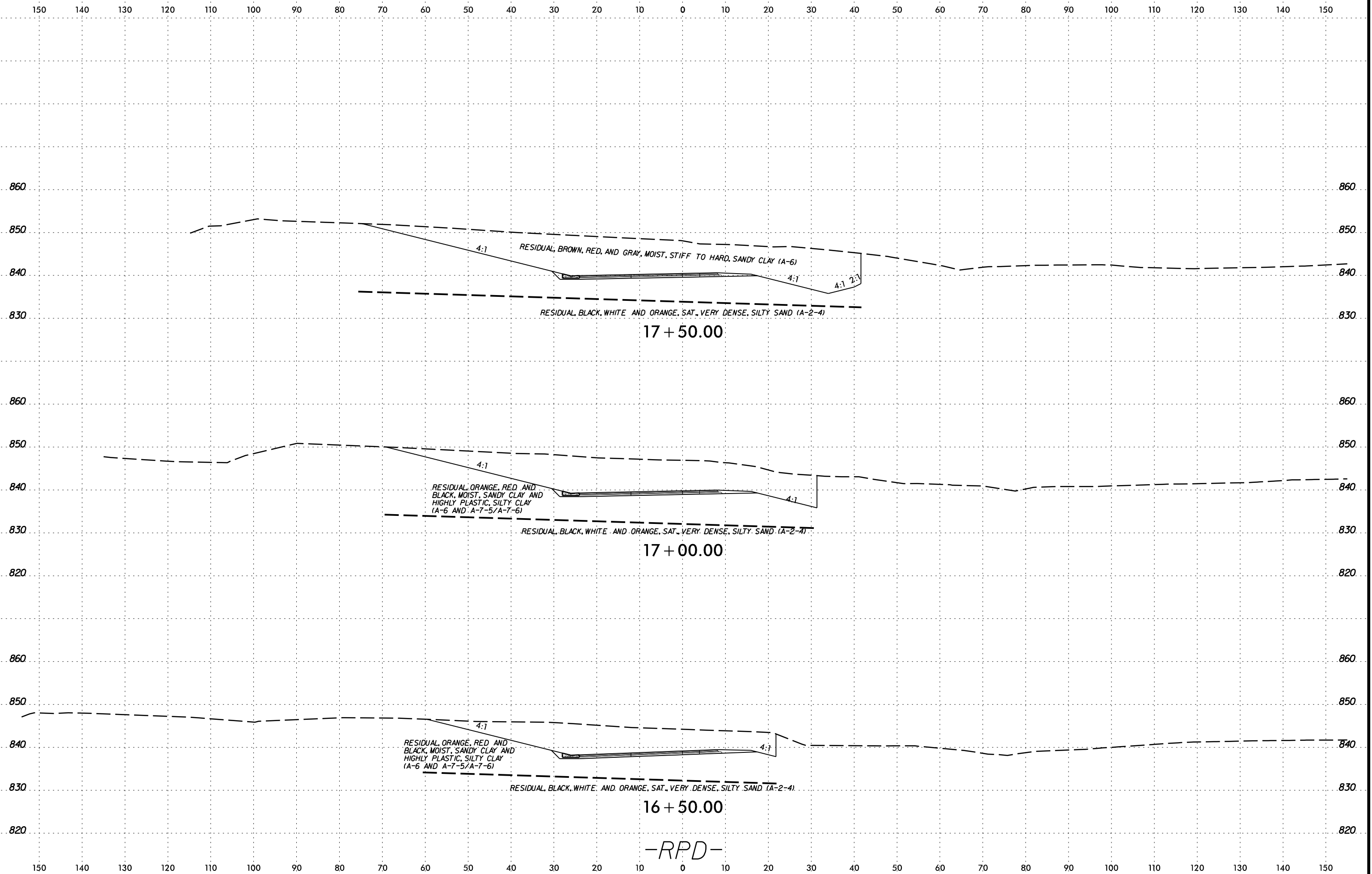
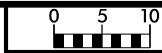
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)		% MOISTURE	% ORGANIC		
							C.SAND	F.SAND	SILT	CLAY	10			200	
SS-137	40 RT	21+69	3.9-5.4	A-7-6(15)	49	23	14	26	31	29	100	91	67	31.9	-

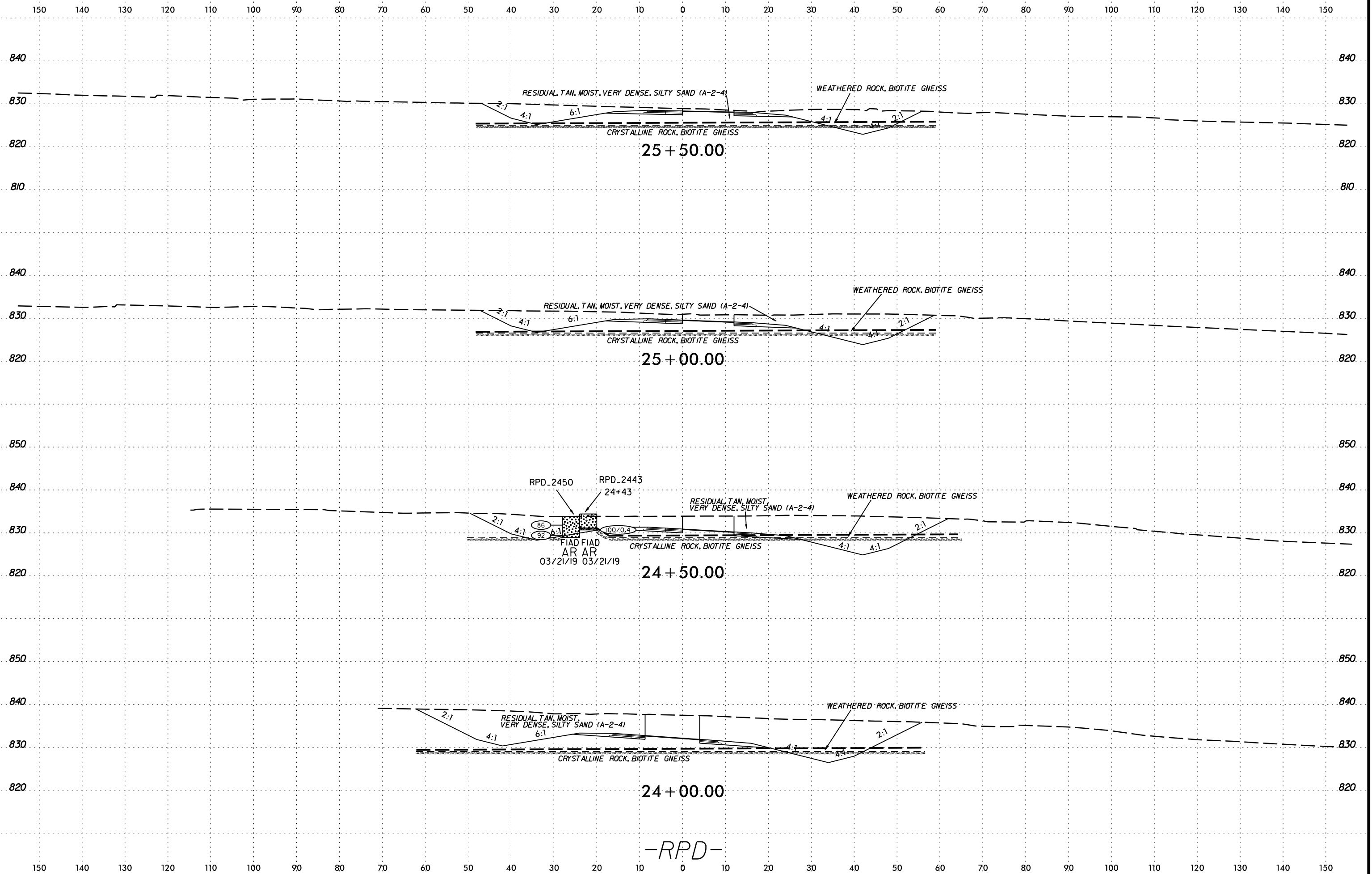
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 21+69
 SS-137
 BT
 03/20/19

21 + 50.00
 -RPC-



-RPD-

6/23/16
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\$\$\$\$\$SERIALNAME\$\$\$\$\$



25 + 50.00

25 + 00.00

24 + 50.00

24 + 00.00

-RPD-



SUMMARY OF LABORATORY TEST DATA

Soil Classification and Gradation

S&ME, Inc. Raleigh, 3201 Spring Forest Road, Raleigh, North Carolina 27616

S&ME Project #: 6235-19-003 Date Report: 4/16/2019

State Project No.: 44674.1.2 County: Guilford County Date Tested: 4/3/19-4/16/19

Federal ID No.: NHP-0029(068) TIP No.: U-5896

Project Name: US 29, US 70 and South Main St. (SR 1009 and US 311 Business)

Client Name: NCDOT GEU Client Address: Raleigh, NC

Sample No.	Boring No.	Station No.	Offset	Alignment	Sample Depth (ft)	AASHTO Classification	Total % Passing				Total Mortar Fraction (%)				LL	PL	PI	Organic	Moist. %
							Sieve #				Coarse Sand	Fine Sand	Silt	Clay					
							10	40	60	200									
SS-1	EB2-B	24+75	21 RT	-L-	3.7 - 5.2	A-6 (6)	97	88	82	60	16	31	35	18	36	22	14	-	20.1
SS-2	EB2-B	24+75	21 RT	-L-	18.7 - 20.2	A-7-5 (6)	99	80	70	49	29	27	22	22	49	30	19	-	39.5
SS-3	EB1-B	22+56	19 RT	-L-	3.6 - 5.1	A-6 (6)	94	83	77	57	19	29	25	27	35	19	16	-	24.6
SS-4	EB1-B	22+56	19 RT	-L-	13.6 - 15.1	A-7-6 (11)	100	96	91	74	9	26	37	29	41	26	15	-	31.6
SS-8	L_2650	26+50	40RT	-L-	3.5 - 5.0	A-7-6 (30)	98	97	95	81	3	19	26	52	60	26	34	-	29.2
SS-10	RPD_1200	12+00	CL	-RPD-	3.3 - 4.8	A-4 (4)	100	93	87	71	13	28	52	7	32	26	6	-	11.4
SS-11	L_4200	42+00	38 RT	-L-	3.4 - 4.9	A-2-4 (0)	87	64	53	34	39	30	26	6	22	19	3	-	3.4
SS-13	EB2-A	24+79	22 LT	-L-	8.3 - 9.8	A-7-6 (19)	94	87	83	71	12	19	29	41	50	21	29	-	38.6
SS-15	EB1-A	22+70	23 LT	-L-	8.1 - 9.6	A-7-6 (19)	96	89	86	74	11	19	33	38	48	21	27	-	33.4
SS-16	EB1-A	22+70	23 LT	-L-	18.1 - 19.6	A-4 (0)	77	62	54	38	30	28	27	15	32	22	10	-	19.2
SS-19	L_2000	20+00	40 LT	-L-	6.0 - 7.5	A-4 (1)	97	91	83	61	14	33	39	14	35	33	2	-	29.5
SS-20	RPB_1400	14+00	CL	-RPB-	3.3 - 4.8	A-6 (3)	92	80	71	51	22	30	32	16	32	21	11	-	16.3
SS-22	RW1-11	15+00	53 LT	-L-	3.4 - 4.9	A-7-5 (12)	100	96	92	76	9	25	46	20	49	36	13	-	29
SS-25	RW1-10	14+50	48 LT	-L-	0.9 - 2.4	A-4 (3)	95	84	77	60	19	25	34	22	33	26	7	-	19.7
SS-31	RW1-7	13+00	43 LT	-L-	13.3 - 14.8	A-4 (1)	100	96	88	63	12	33	37	18	22	16	6	-	ND
SS-34	RW1-6	12+50	43 LT	-L-	8.4 - 9.9	A-7-6 (37)	100	97	92	82	8	14	23	56	70	29	41	-	ND
SS-37	RW1-4	11+50	43 LT	-L-	3.4 - 4.9	A-7-5 (12)	98	94	90	77	8	20	36	36	48	35	13	-	36.4
SS-45	RW1-1	10+00	40 LT	-L-	8.4 - 9.9	A-7-6 (17)	98	94	90	78	8	16	31	45	47	26	21	-	ND
SS-49	RPC_1425	14+25	10 RT	-RPC-	9.1 - 10.6	A-4 (1)	80	69	64	49	20	25	34	21	28	21	7	-	25
SS-51	RW2-3	13+63	65 RT	-L-	8.4 - 9.9	A-4 (2)	99	91	82	59	17	33	36	14	35	30	5	-	37
SS-54	RW2-2	12+97	30 RT	-L-	3.4 - 4.9	A-7-5 (14)	100	96	92	78	8	21	39	32	51	36	15	-	31
SS-58	RW2-1	12+46	30 RT	-L-	13.3 - 14.8	A-4 (1)	97	91	85	63	12	32	34	22	23	17	6	-	ND
SS-60	L_993	9+93	33 RT	-L-	3.3 - 4.8	A-5 (10)	100	97	92	76	8	23	40	29	48	38	10	-	37.9
SS-64	L_3900	39+00	50 LT	-L-	3.6 - 5.1	A-7-6 (11)	98	94	89	66	9	31	34	25	41	22	19	-	28
SS-65	L_803	8+03	29 RT	-L-	6.4 - 7.9	A-5 (7)	100	94	90	73	10	25	47	18	44	37	7	-	38
SS-66	RPA_1200	12+00	27 RT	-RPA-	3.6 - 5.1	A-4 (0)	99	96	90	51	10	50	29	12	31	27	4	-	17.9
SS-68	RPA_1400	14+00	27 RT	-RPA-	6.5 - 8.0	A-7-6 (11)	99	89	84	68	16	21	27	36	43	26	17	4.5	26.7
SS-69	Y_1120	11+20	CL	-Y-	3.3 - 4.8	A-7-6 (22)	99	91	87	74	12	17	24	47	55	26	29	-	21.3
SS-70	Y_1200	12+00	CL	-Y-	3.0 - 4.5	A-7-6 (12)	98	90	85	67	14	23	27	36	43	24	19	-	22.9
SS-71	RPB_2071	20+71	5 LT	-RPB-	0.4 - 1.9	A-7-6 (12)	100	93	89	72	11	25	35	29	45	29	16	-	35.9
SS-74	RPA_1650	16+50	44 LT	-RPA-	0 - 1.5	A-7-6 (27)	98	89	85	73	14	15	24	47	60	23	37	-	25.6
SS-101	RPB_1517	15+17	CL	-RPB-	1.0 - 2.5	A-6 (8)	88	79	74	59	16	24	39	22	40	23	17	-	ND
SS-105	RPB_1750	17+50	20 RT	-RPB-	1.0 - 2.5	A-4 (1)	74	59	53	38	29	25	23	23	31	21	10	-	ND
SS-109	RPB_1900	19+00	40 RT	-RPB-	1.0 - 2.5	A-7-6 (9)	78	69	64	54	17	18	29	36	45	23	22	-	24.6
SS-111	RPA_2300	23+00	50 LT	-RPA-	1.0 - 2.5	A-2-4 (0)	100	76	58	28	42	36	17	6	NP	NP	NP	-	ND
SS-114	RPA_2150	21+50	35 LT	-RPA-	7.1 - 8.6	A-5 (10)	100	99	97	85	3	26	51	21	48	40	8	-	ND
SS-118	RPA_2000	20+00	35 LT	-RPA-	6.9 - 8.4	A-7-6 (18)	98	91	86	72	12	20	30	39	52	27	25	-	ND

SUMMARY OF LABORATORY TEST DATA

Soil Classification and Gradation



S&ME, Inc. Raleigh, 3201 Spring Forest Road, Raleigh, North Carolina 27616

S&ME Project #: 6235-19-003 Date Report: 4/16/2019

State Project No.: 44674.1.2 County: Guilford County Date Tested: 4/3/19-4/16/19

Federal ID No.: NHP-0029(068) TIP No.: U-5896

Project Name: US 29, US 70 and South Main St. (SR 1009 and US 311 Business)

Client Name: NCDOT GEU Client Address: Raleigh, NC

Sample No.	Boring No.	Station No.	Offset	Alignment	Sample Depth (ft)	AASHTO Classification	Total % Passing				Total Mortar Fraction (%)				LL	PL	PI	Organic	Moist. %
							Sieve #				Coarse Sand	Fine Sand	Silt	Clay					
							10	40	60	200									
SS-119	RPA_2000	20+00	35 LT	-RPA-	14.4 - 15.9	A-4 (4)	100	94	89	65	11	35	41	12	40	34	6	-	31.3
SS-121	RPA_1800	18+00	50 RT	-RPA-	4.0 - 5.5	A-4 (0)	82	64	56	37	31	32	22	15	31	22	9	-	18.6
SS-124	L_2900	29+00	70 LT	-L-	1.0 - 2.5	A-7-6 (19)	98	89	84	70	14	19	29	38	49	21	28	-	24.2
SS-127	RPC_2350	23+50	CL	-RPC-	3.5 - 5.0	A-7-6 (8)	75	66	61	51	18	18	30	33	49	28	21	-	ND
SS-128	RPC_2350	23+50	CL	-RPC-	6.2 - 7.7	A-6 (10)	94	84	76	59	19	24	24	33	37	15	22	5.1	26.8
SS-130	RPC_1600	16+00	40 RT	-RPC-	1.0 - 2.5	A-2-4 (0)	82	56	48	32	41	26	25	9	26	20	6	-	27.6
SS-131	RPC_1750	17+50	50 RT	-RPC-	3.8 - 5.3	A-7-5 (29)	99	98	97	90	2	13	42	43	58	31	27	-	40.6
SS-134	RPC_2050	20+50	CL	-RPC-	1.0 - 2.5	A-7-6 (30)	97	91	88	79	9	13	25	53	61	25	36	-	30.4
SS-135	RPC_2050	20+50	CL	-RPC-	3.7 - 5.2	A-7-5 (32)	83	80	79	75	5	9	29	58	72	33	39	-	36.9
SS-137	RPC_2169	21+69	40 RT	-RPC-	3.9 - 5.4	A-7-6 (15)	100	91	86	67	14	26	31	29	49	26	23	-	31.9
SS-142	RPD_2184	21+84	31 LT	-RPD-	7.0 - 8.5	A-7-5 (21)	100	100	99	88	1	19	51	29	57	39	18	-	39.3
SS-145	RPD_2000	20+00	60 LT	-RPD-	3.9 - 5.4	A-7-6 (13)	96	93	92	85	4	17	60	19	42	29	13	-	18
SS-149	RPD_1797	17+97	34 LT	-RPD-	1.0 - 2.5	A-6 (7)	100	90	79	57	21	30	29	21	38	20	18	-	ND
SS-152	RPD_1600	16+00	43 RT	-RPD-	3.7 - 5.2	A-7-5 (54)	100	98	96	90	4	9	24	63	85	34	51	-	ND
SS-155	RPD_1400	14+00	50 LT	-RPD-	8.7 - 10.2	A-4 (0)	96	87	77	40	20	48	24	8	34	32	2	-	ND
SS-157	Y_2550	25+50	CL	-Y-	1.2 - 2.7	A-2-4 (0)	99	78	64	35	35	37	20	8	27	23	4	-	16
SS-160	Y_2300	23+00	CL	-Y-	3.9 - 5.4	A-6 (3)	86	68	62	43	28	29	22	21	35	18	17	-	21.2
SS-161	Y_1980	19+80	CL	-Y-	1.3 - 2.8	A-7-6 (19)	98	89	84	70	14	19	23	43	50	21	29	-	21.5
SS-166	Y_1800	18+00	CL	-Y-	6.8 - 8.3	A-6 (8)	91	79	75	60	18	23	32	27	39	22	17	-	ND
SS-167	Y_1591	15+91	CL	-Y-	1.0 - 2.5	A-7-5 (39)	99	97	95	89	4	11	30	55	72	35	37	-	39

References / Comments / Deviations: ND=Not Determined.

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO M145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

Karen Warner

Technician Name:

Signature

NCDOT 118-06-0305

Certification #

Joey Daily

Technical Responsibility:

Project Manager

Position

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