

**BAR TYPES**

1: 50'-0" length, 1'-5" end sections, HK.  
2: 9'-9" length, 0 7/8" top width, 10" height.  
3: 10'-4" length, 10" height, 0 7/8" top width.  
4: 3'-7" length, 5 1/2" top width, HK.  
5: 3'-5" length, 5 1/2" end sections, HK.  
6: 1'-8" diameter, 1'-3" lap.  
7: 3'-5" length, 1'-6" height.

ALL BAR DIMENSIONS ARE OUT TO OUT.

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#10	1	52'-10"	1819
B2	4	#10	STR	50'-2"	864
B3	24	#4	STR	26'-4"	423
B4	13	#4	STR	3'-5"	30
B5	16	#4	STR	3'-6"	38
B6	4	#4	STR	9'-0"	25
H1	44	#5	3	11'-2"	513
H2	28	#5	2	10'-7"	310
K1	12	#4	STR	3'-7"	29
K2	12	#4	STR	2'-9"	23
S1	51	#5	4	11'-6"	612
S2	51	#5	5	4'-4"	231
S3	32	#4	6	6'-6"	139
U1	18	#4	7	6'-5"	78
V1	4	#5	STR	9'-9"	41
V2	26	#5	STR	10'-1"	274
V3	24	#5	STR	9'-4"	234
V4	2	#5	STR	9'-0"	19
V5	64	#5	STR	7'-2"	479

REINFORCING STEEL 6,181 LBS.

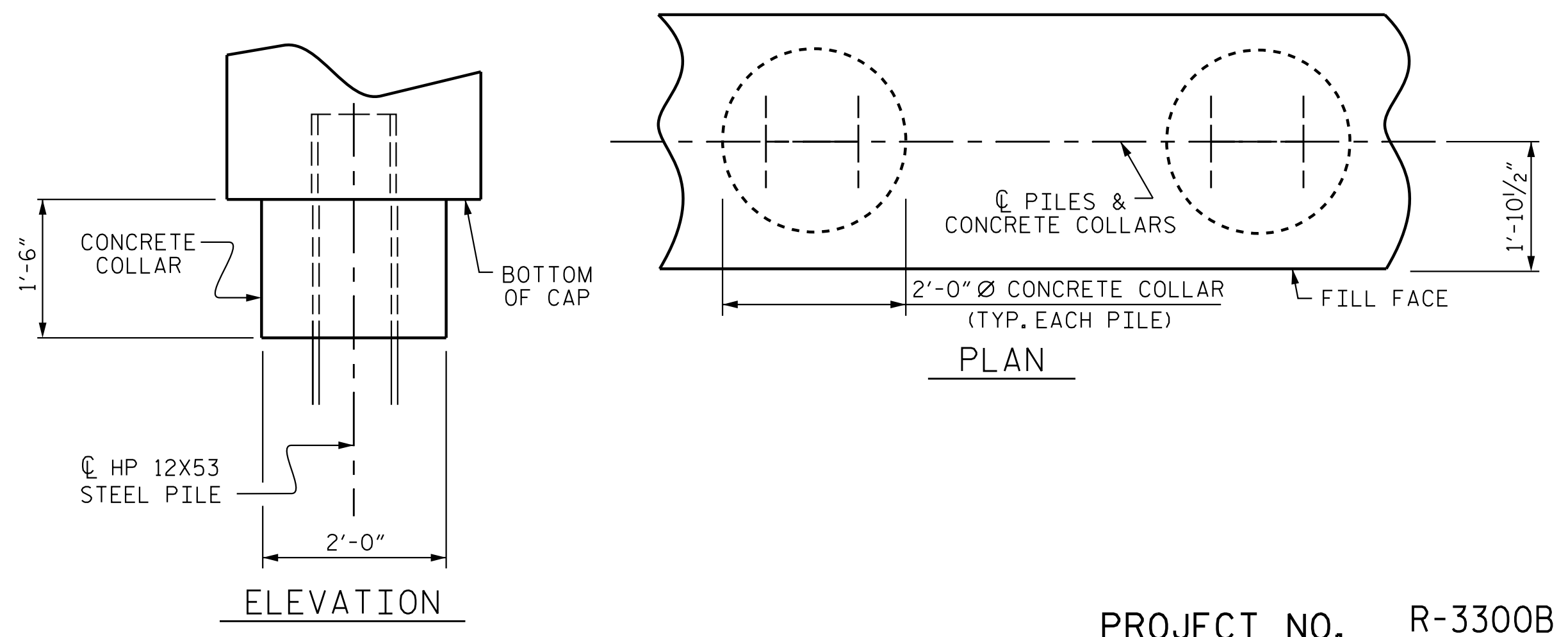
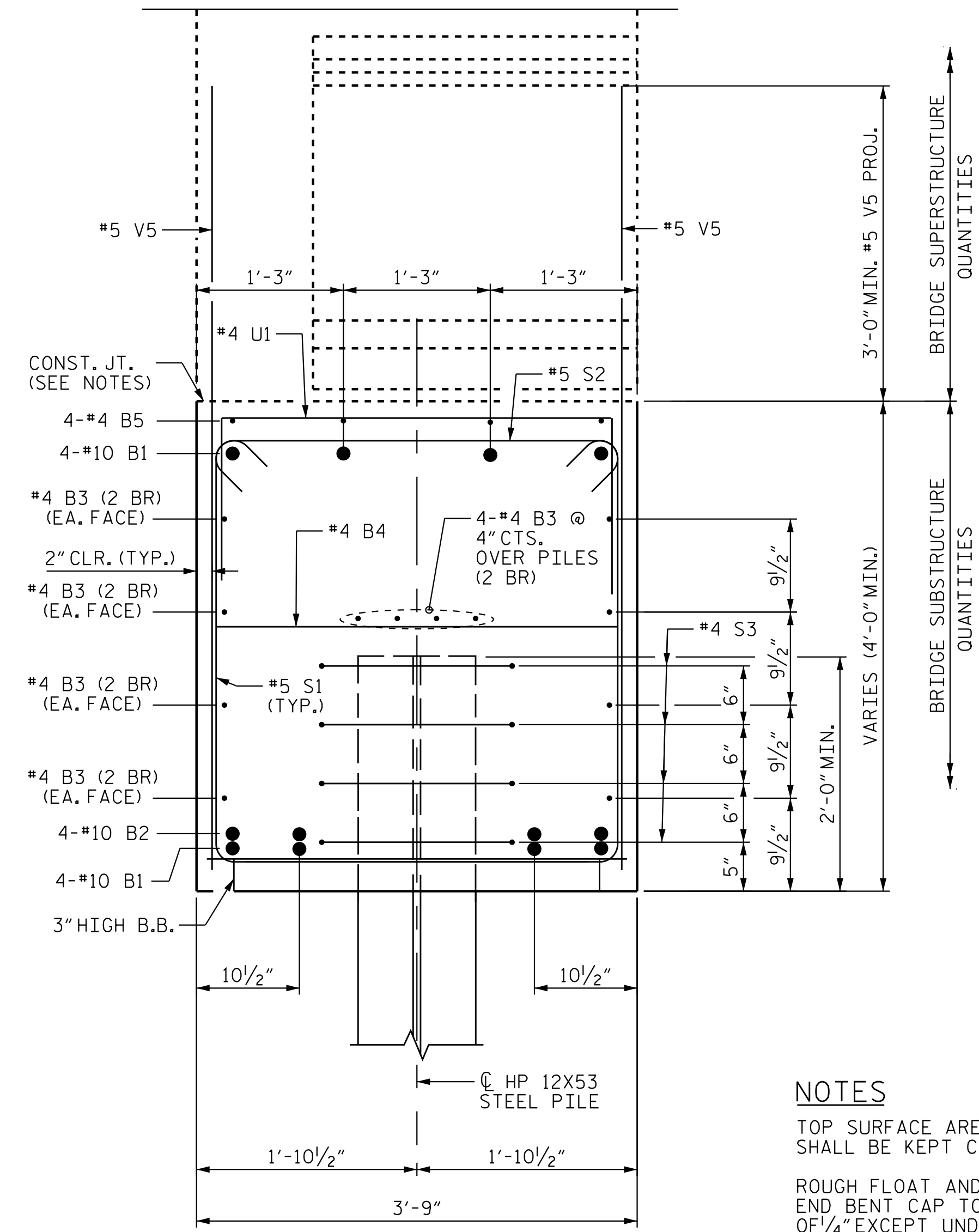
CLASS A CONCRETE BREAKDOWN

POUR #1 CAP, LOWER PART OF WINGS & COLLARS 34.0 C.Y.

POUR #2 UPPER PART OF WINGS 5.5 C.Y.

TOTAL CLASS A CONCRETE 39.5 C.Y.

HP 12X53 STEEL PILES	STEEL PILE POINTS	EA. 8	
NO: 8	LIN. FT. = 480.00	PILE REDRIVES	EA. 4
PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES		EA. 8	



**NOTES**

TOP SURFACE AREAS OF THE END BENT CAP SHALL BE KEPT CLEAN AND FREE OF LAITANCE.

ROUGH FLOAT AND ROUGHEN THE TOP OF THE END BENT CAP TO PROVIDE MIN. SURFACE AMPLITUDE OF 1/4" EXCEPT UNDER BEARING AREAS.

#10 B1 & #10 B2 BARS IN THE BOTTOM OF CAP ARE BUNDLED.

(2 BR) DENOTES 2 BAR RUN.

SECTION A-A  
SEE "END BENT 2" SHEET 1 OF 3.  
PILE COLLAR NOT SHOWN FOR CLARITY.

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DRAWN BY: J.B. GEILE DATE: 09/20/19  
CHECKED BY: N. D'AIUTO DATE: 09/23/19  
DESIGN ENGINEER OF RECORD: M. B. ISENHOUR DATE: 10/25/21

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 658+69.17 -L1-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

**END BENT 2 DETAILS (RIGHT LANE)**

REVISIONS						SHEET NO. S15-26
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 30
2			4			

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10/25/2021

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**NOTES:**

**WALL SUPPORT SYSTEM**

HP10X42 POSTS & ANGLES SHALL BE AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111 AND IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. REPAIR ANY DAMAGED GALVANIZATION IN ACCORDANCE WITH ARTICLE 1076-7 OF THE STANDARD SPECIFICATIONS.

HP14X73 POSTS, BENT PLATES & ANGLES/LAGGING STOPS SHALL MEET THE REQUIREMENTS OF AASHTO M270, GRADE 50 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111 AND IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. REPAIR ANY DAMAGED GALVANIZATION IN ACCORDANCE WITH ARTICLE 1076-7 OF THE STANDARD SPECIFICATIONS.

ALL BEARING PLATES, BOLTS, NUTS AND WASHERS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F3125 AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M232.

ALL STEEL HP10X42 & HP14X73 POSTS SHALL BE PLUMB.

**SOUND ABSORPTIVE BARRIER WALL**

SOUND ABSORPTIVE BARRIER WALL SHALL BE DESIGNED TO WITHSTAND A MINIMUM WIND VELOCITY OF 150 MPH AND A MINIMUM WIND PRESSURE OF 0.0689 KSF.

TOTAL WEIGHT OF THE SOUND ABSORPTIVE BARRIER WALL, COMPLETE AND IN PLACE, SHALL NOT EXCEED 300 LBS/FT.

SOUND ABSORPTIVE PANELS WILL BE INSTALLED FOR SOUND ABSORPTIVE BARRIER WALL. FOR SOUND ABSORPTIVE PANEL, SEE SPECIAL PROVISIONS.

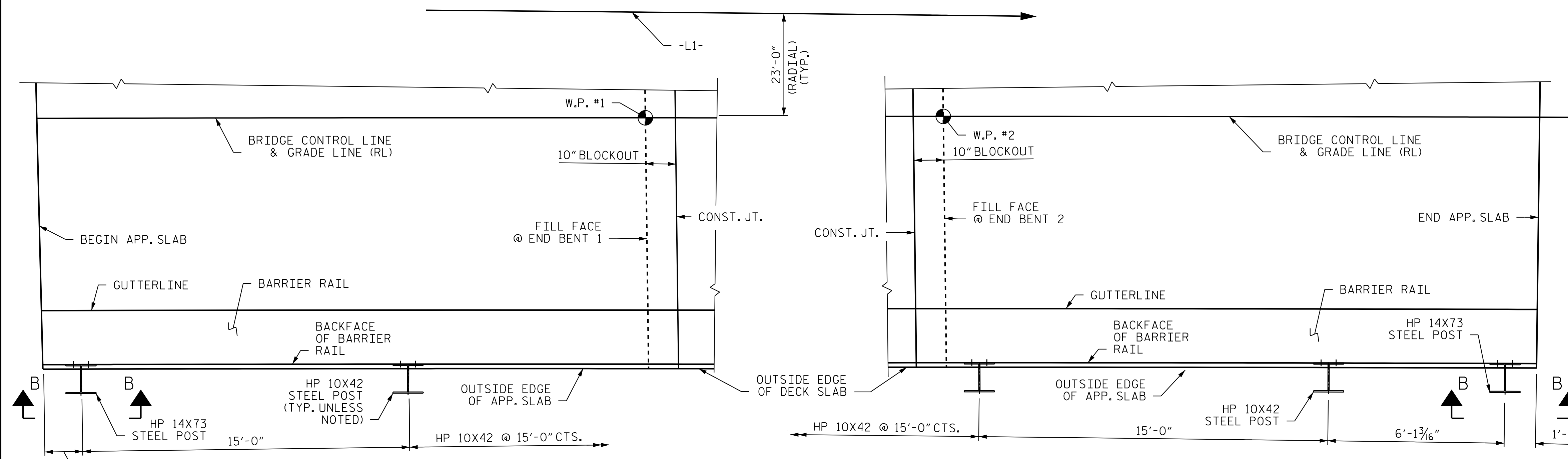
PANELS SHALL BE CUT SO THAT THE ENDS ARE SMOOTH AND PERPENDICULAR TO EACH PANEL BASE AND SHALL BE APPROVED BY THE ENGINEER.

EACH PANEL SHALL BE PLACED SO THAT THE TOP OF THE FINISHED PANEL MEETS FLUSH WITH THE TOP OF EACH SUPPORT POST.

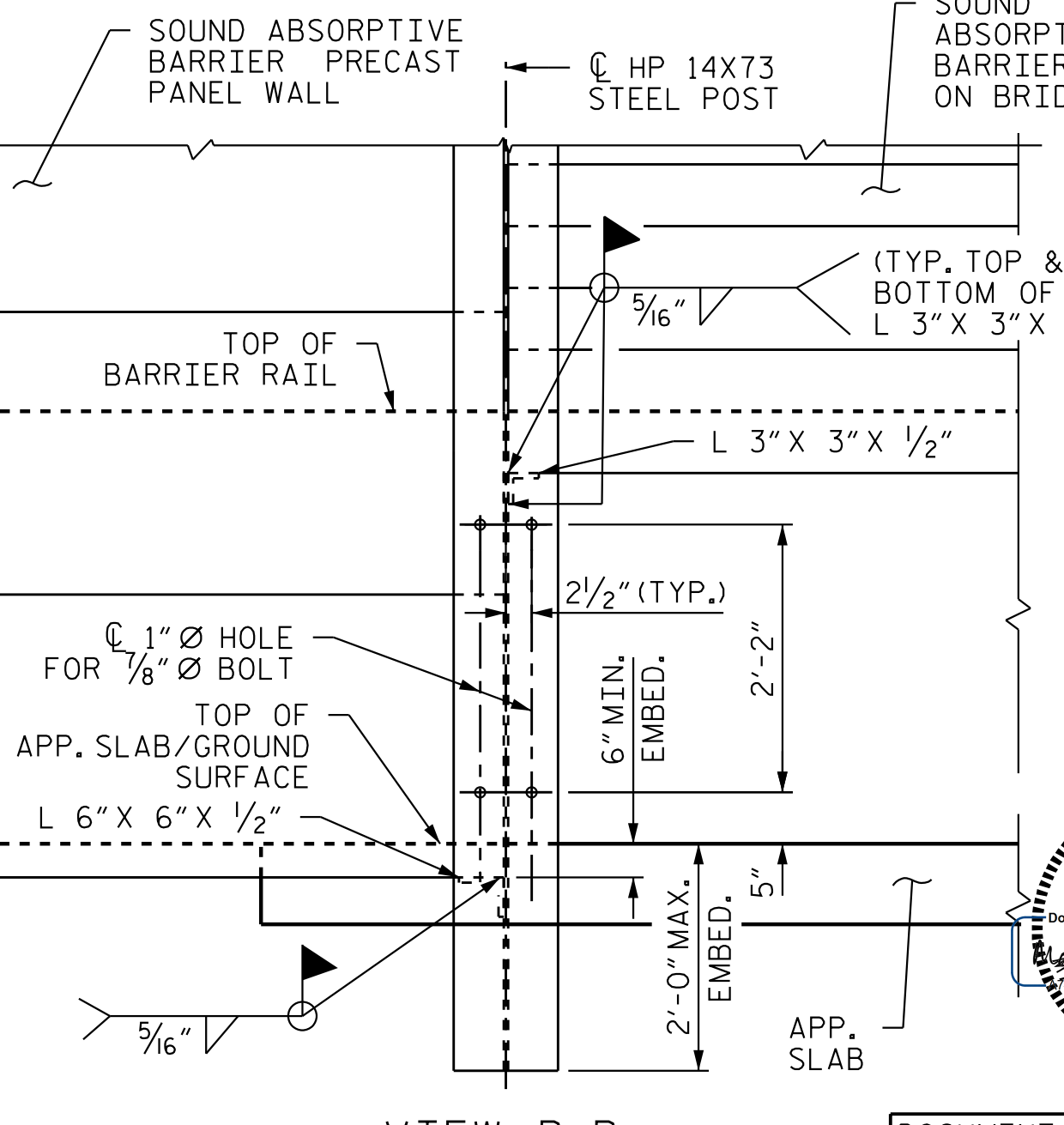
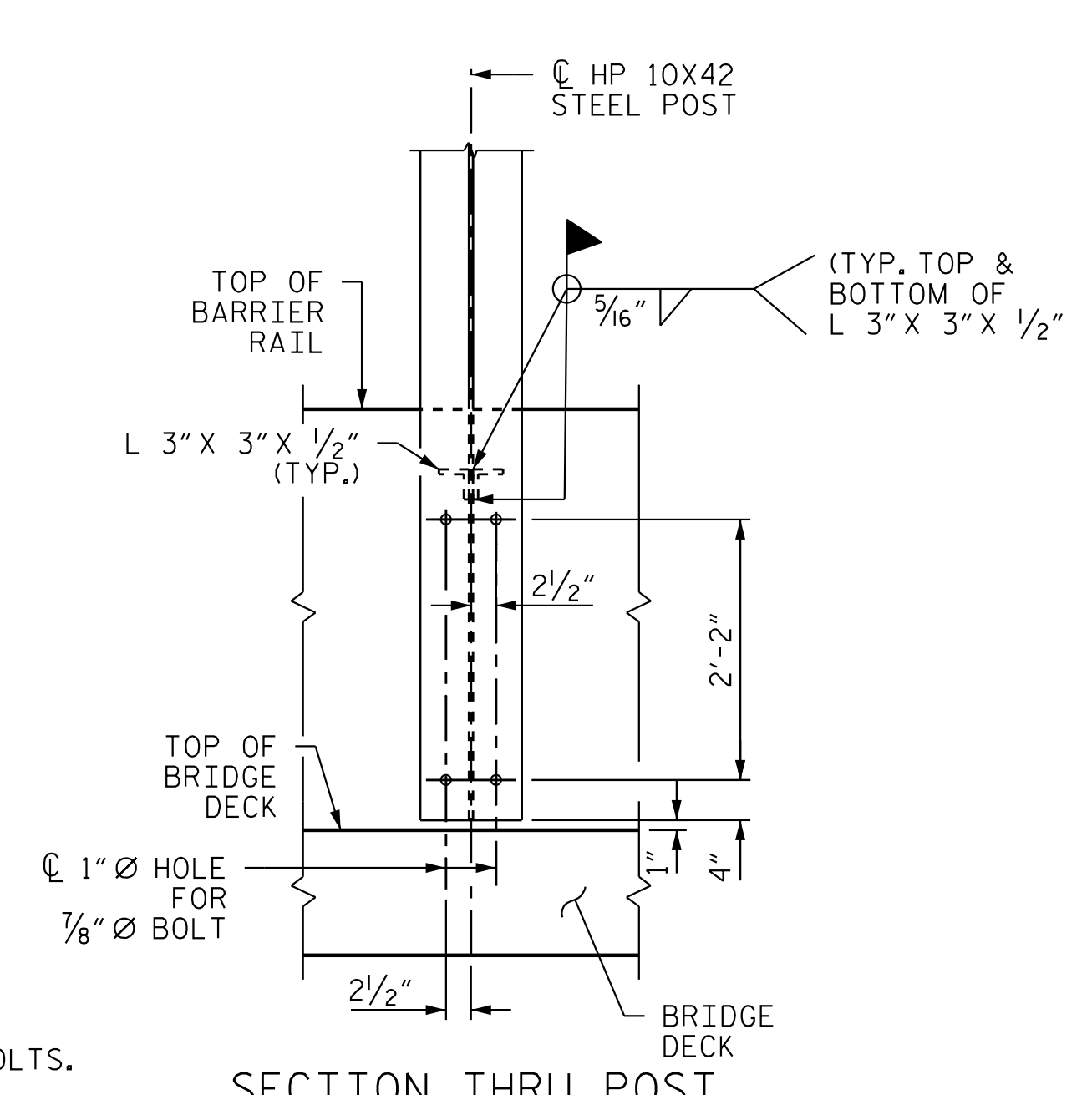
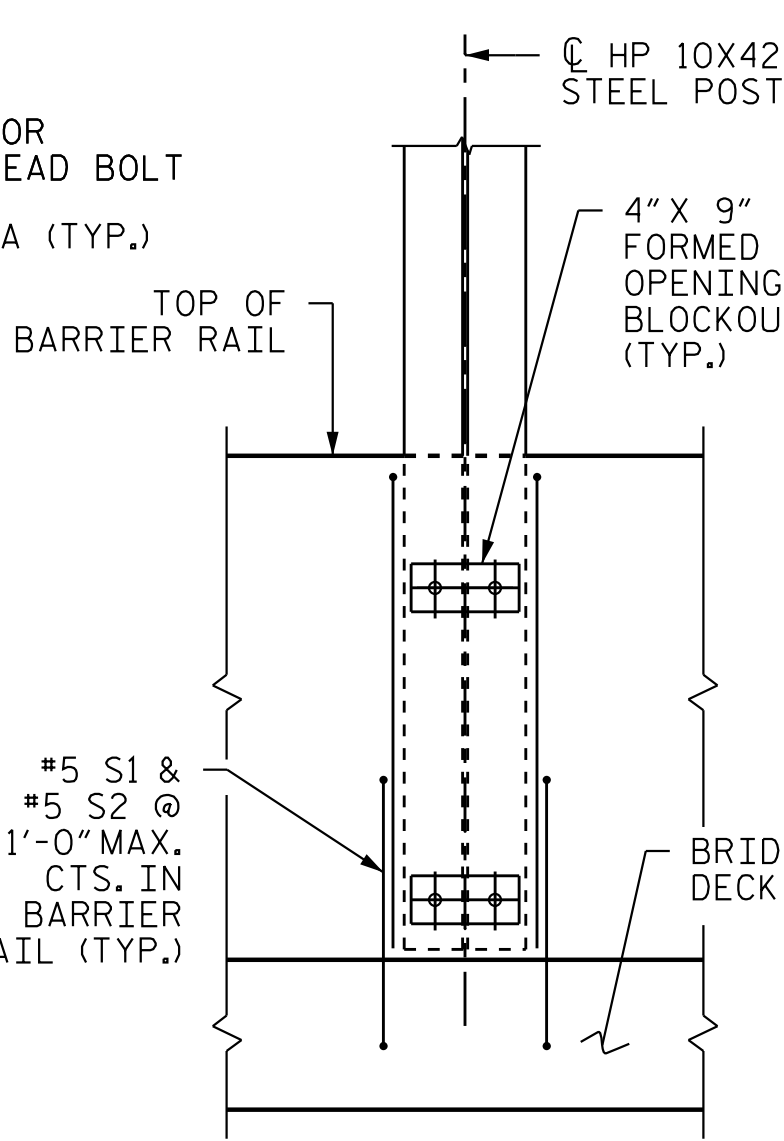
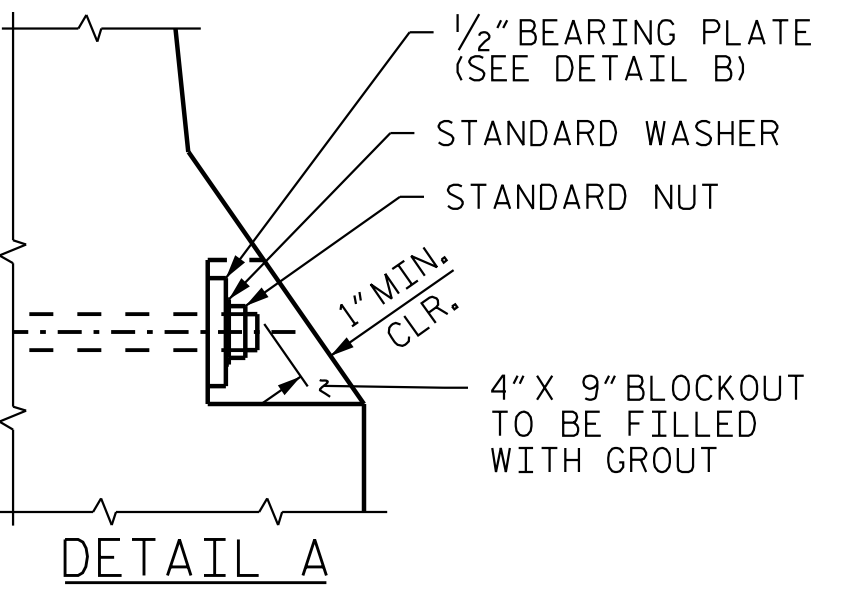
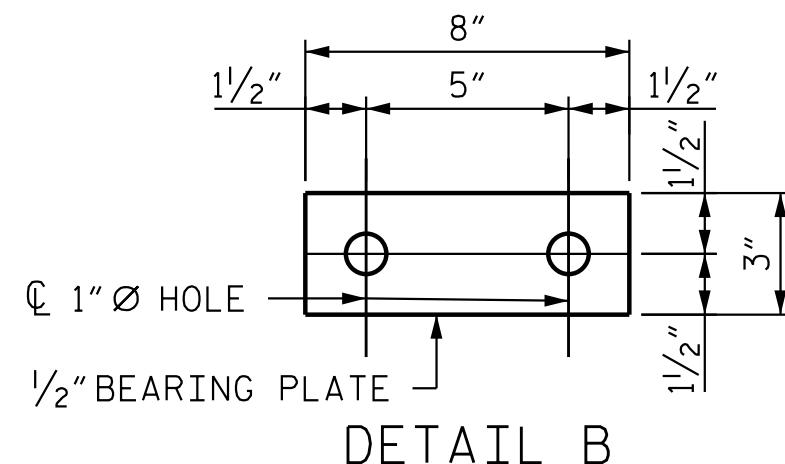
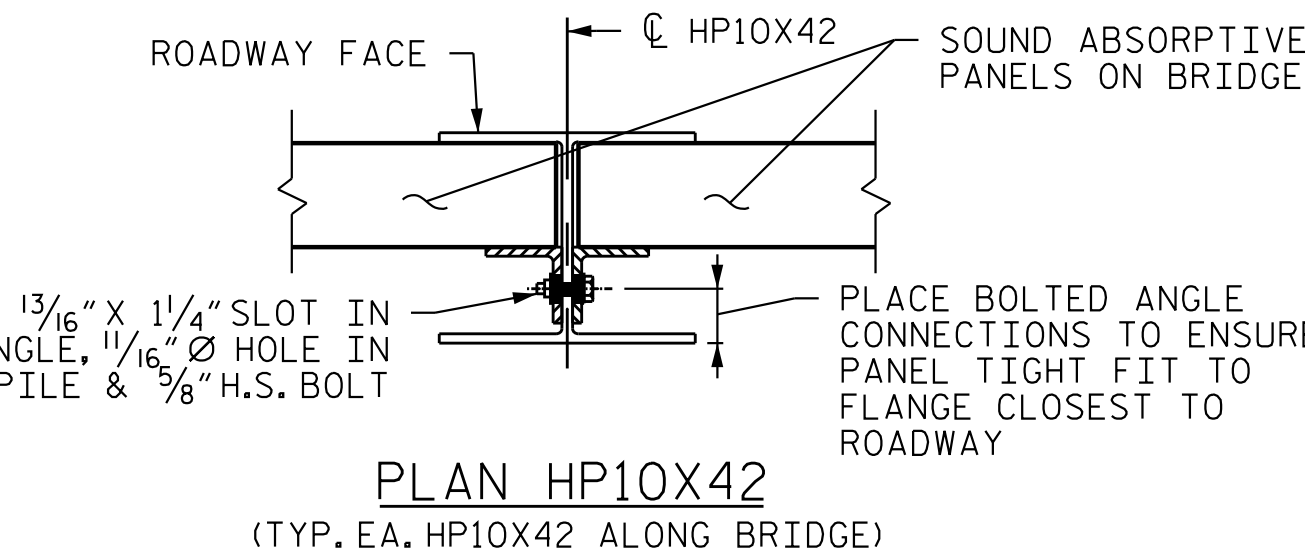
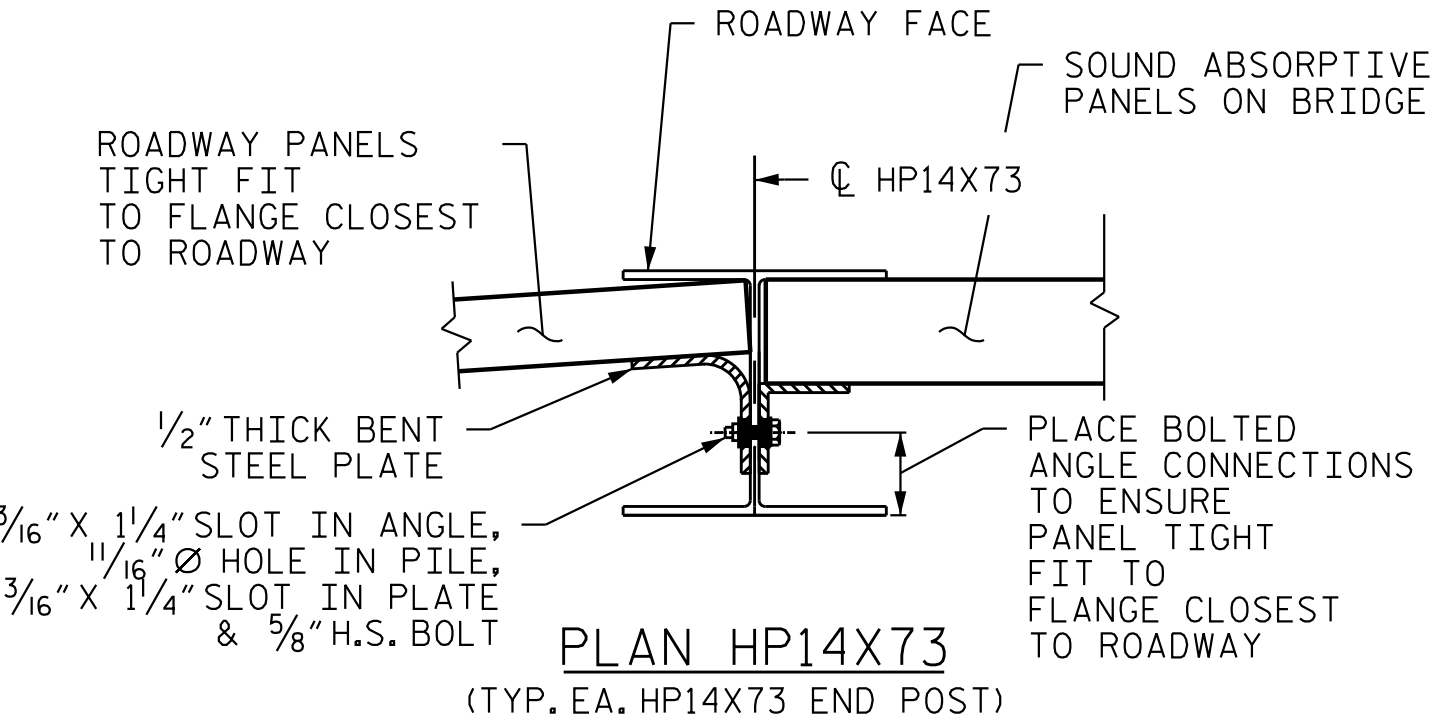
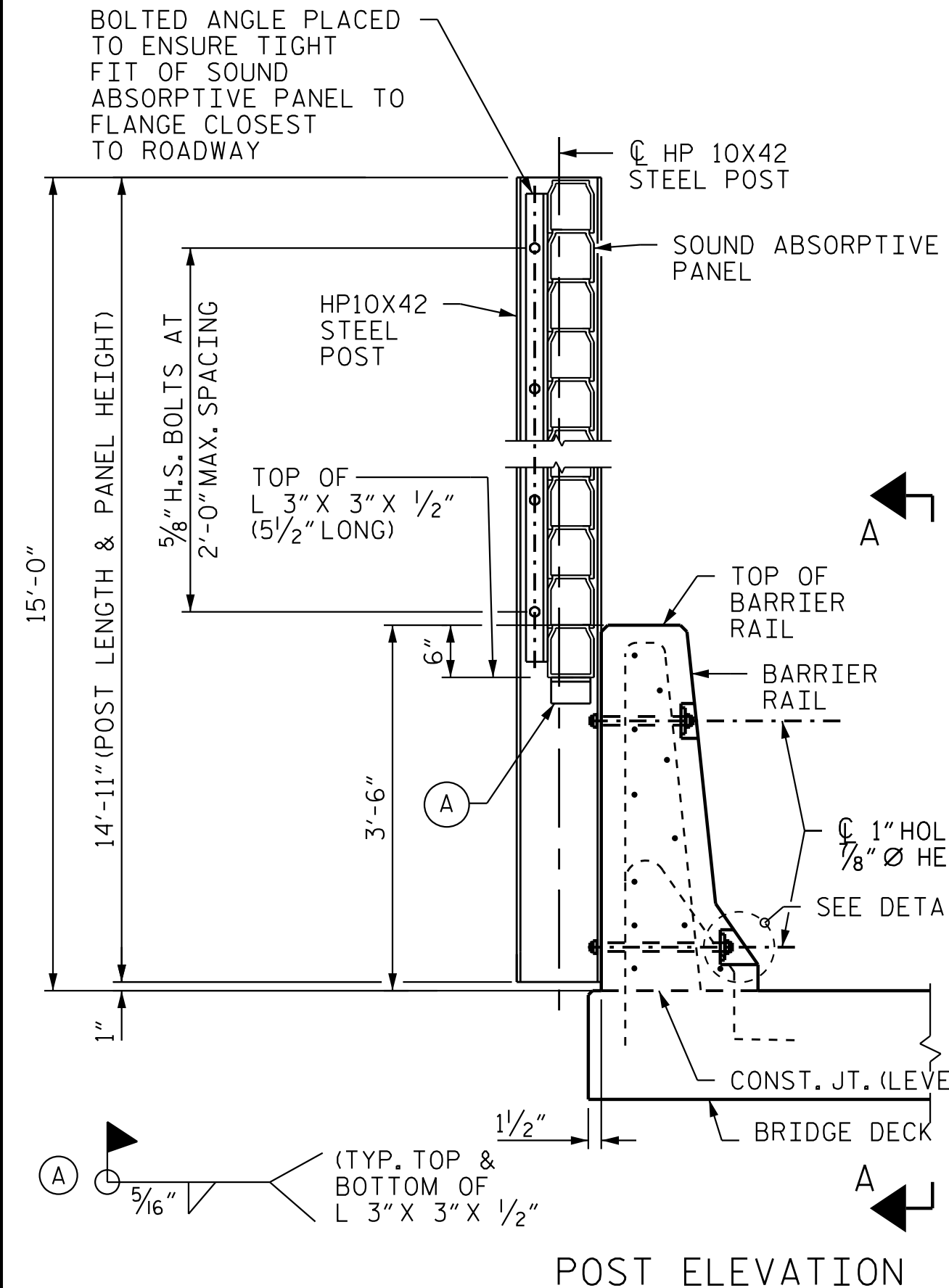
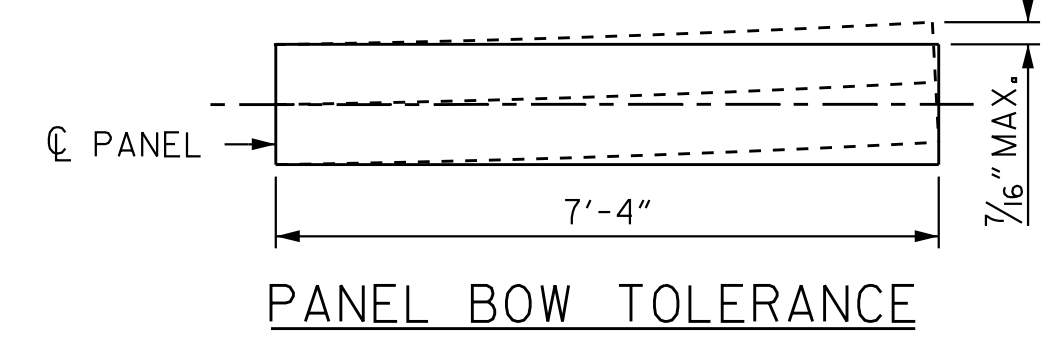
SOUND ABSORPTIVE BARRIER WALL & PANELS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE PROCEDURES OUTLINED BY THE MANUFACTURER.

FOR FILLET WELDS, E70 ELECTRODES SHALL BE USED.

FOR SOUND ABSORPTIVE BARRIER WALL DETAILS AND WALL TURN DETAILS, SEE ROADWAY PLANS.



**PLAN OF SOUND ABSORPTIVE BARRIER WALL LAYOUT**



GROUT SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS (LOWER BLOCKOUT SHOWN, UPPER BLOCKOUT SIMILAR)

ABSORPTIVE PANEL QUANTITY	
PANELS	1,745 SQ. FT.

**Stantec**  
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DRAWN BY: N. D'AIUTO DATE: 09/30/19  
CHECKED BY: M. B. ISENHOUR DATE: 12/16/19  
DESIGN ENGINEER OF RECORD: M. B. ISENHOUR DATE: 10/25/21

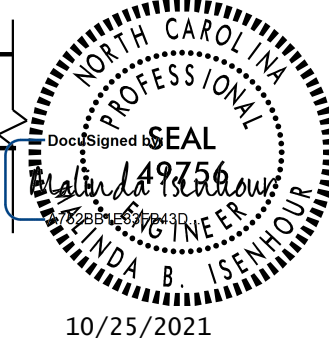
PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 658+69.17 -L1-

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

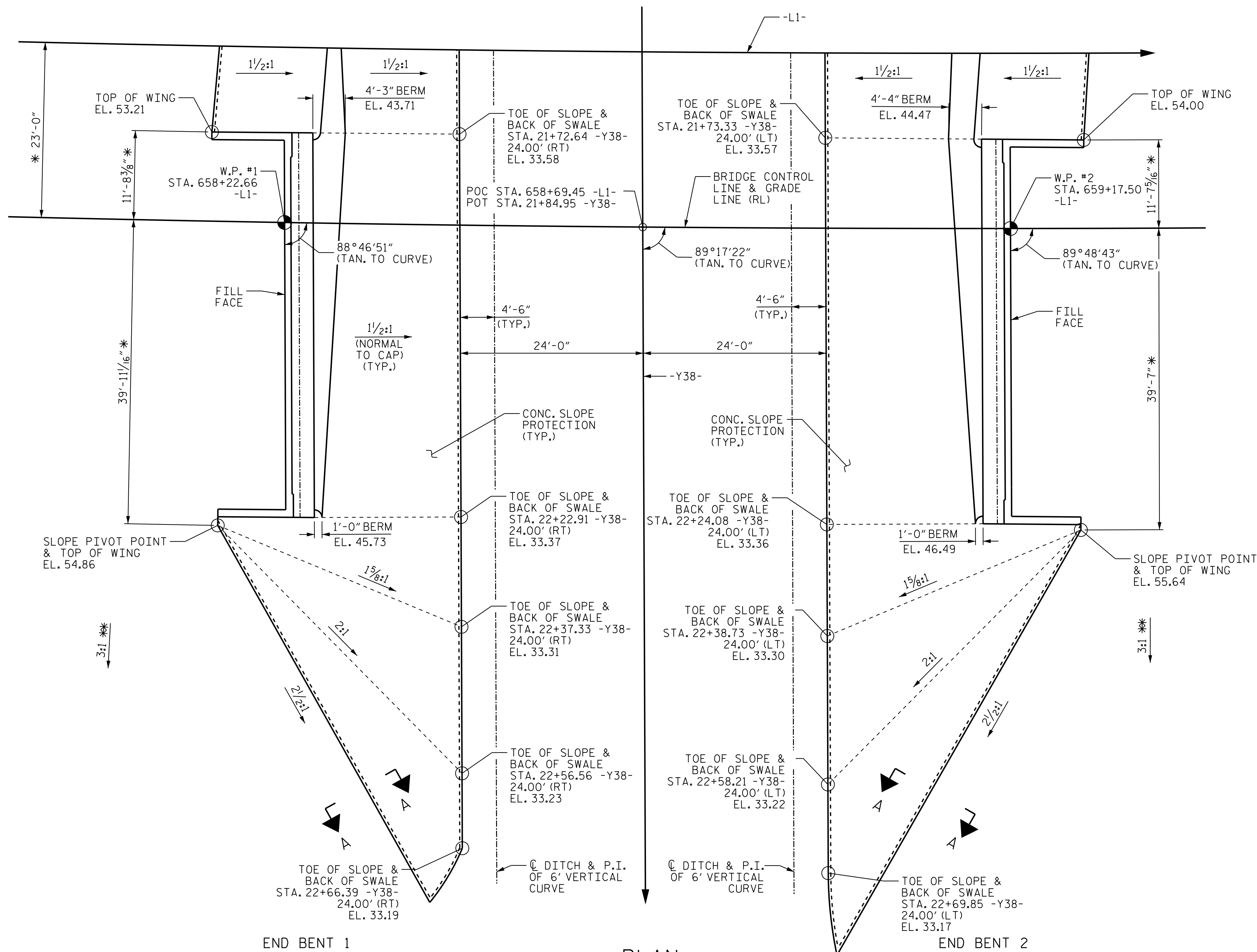
**SOUND ABSORPTIVE BARRIER WALL (BRIDGE MOUNTED)**  
(RIGHT LANE)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S15-27
1			3			TOTAL SHEETS
2			4			30

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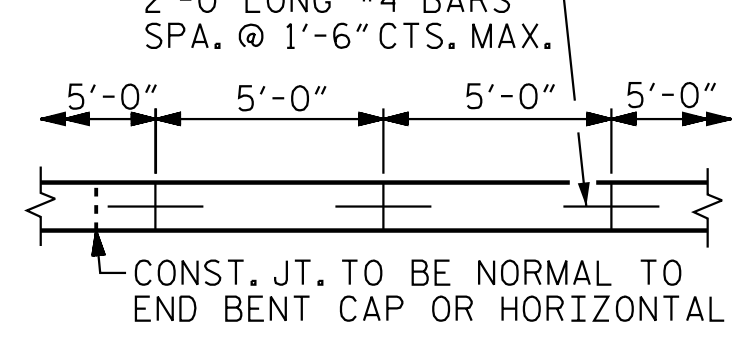
10/25/2021



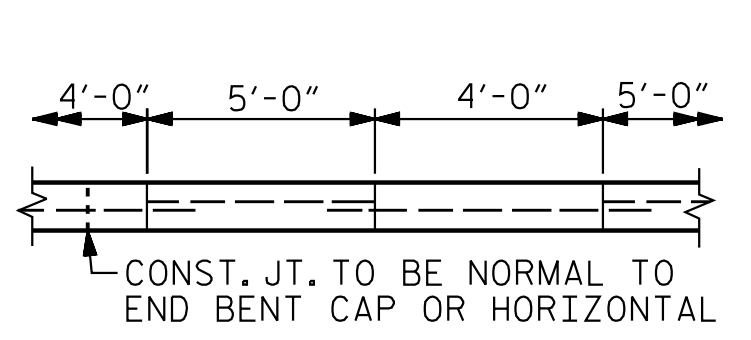
PLAN

ELEVATIONS SHOWN ARE AT TOP SURFACE OF SLOPE PAVING (SEE POINT A ON SECTION A-A WHEN FILL CATCHES DITCH) \* RADIAL DIMENSION

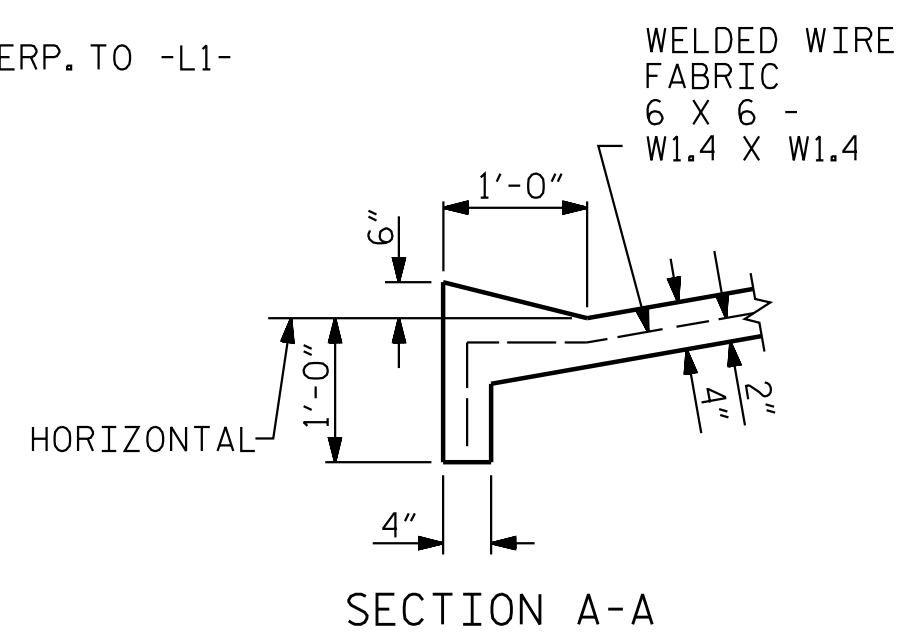
\*\* TRANSITION FROM 2 1/2:1 SLOPE PROTECTION TO MATCH 3:1 FILL SLOPES PERP. TO -L1-  
 2'-0" LONG #4 BARS SPA. @ 1'-6" CTS. MAX.



POURING DETAIL



OPTIONAL POURING DETAIL



SECTION A-A

GENERAL NOTES

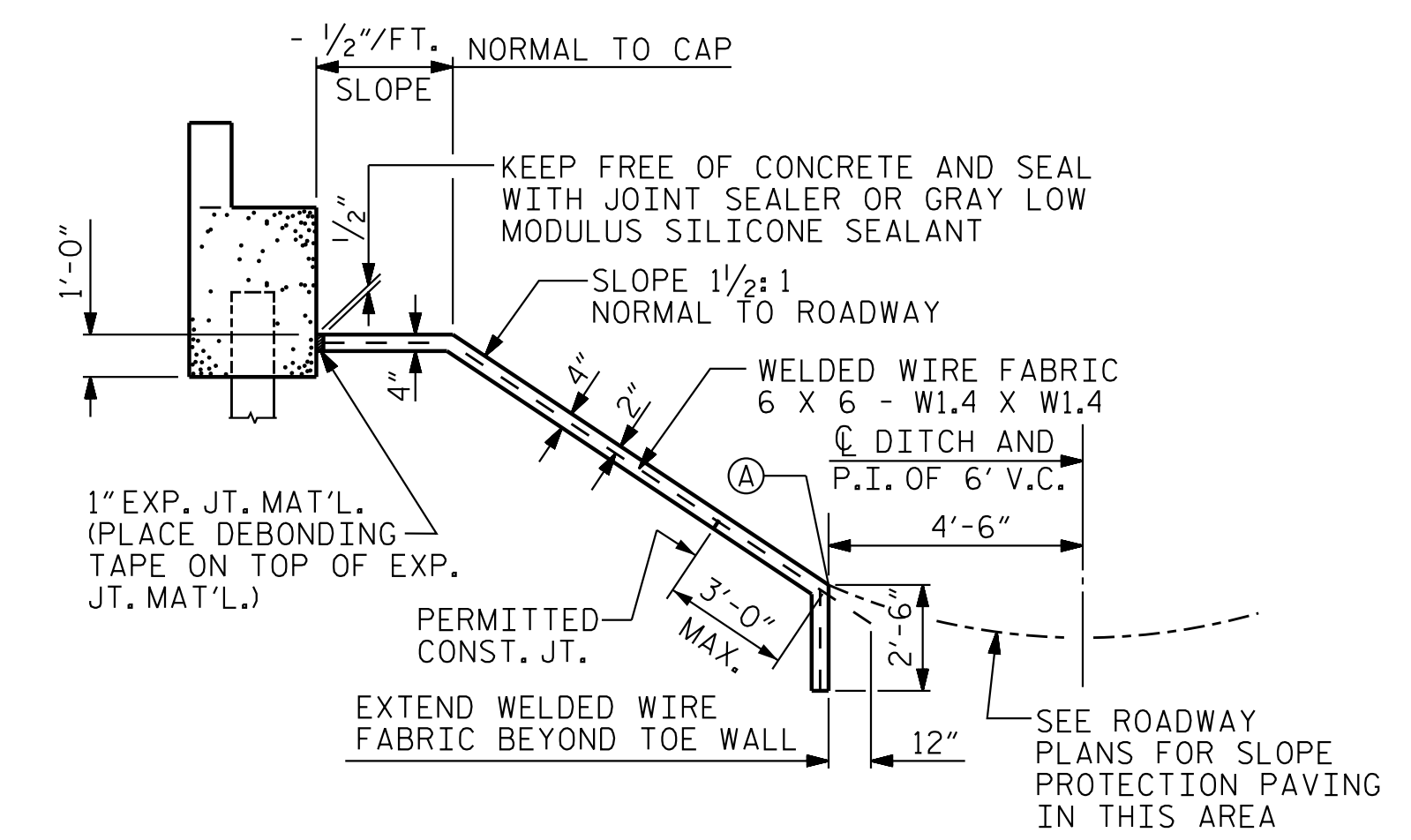
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

ALTERNATE "A"

ALTERNATE "A" SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 658+69.17 -L1-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	295	531
END BENT 2	320	576
TOTAL	615	1107

\* QUANTITY SHOWN IS BASED ON 5' POURS.



SECTION ALONG C SURVEY WHEN FILL CATCHES IN DITCH

DETAILS FOR ALTERNATE "A"

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 658+69.17 -L1-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SLOPE PROTECTION  
 DETAILS**

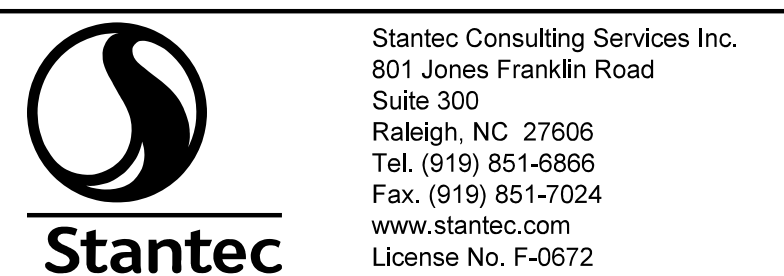
(RIGHT LANE)



10/25/2021

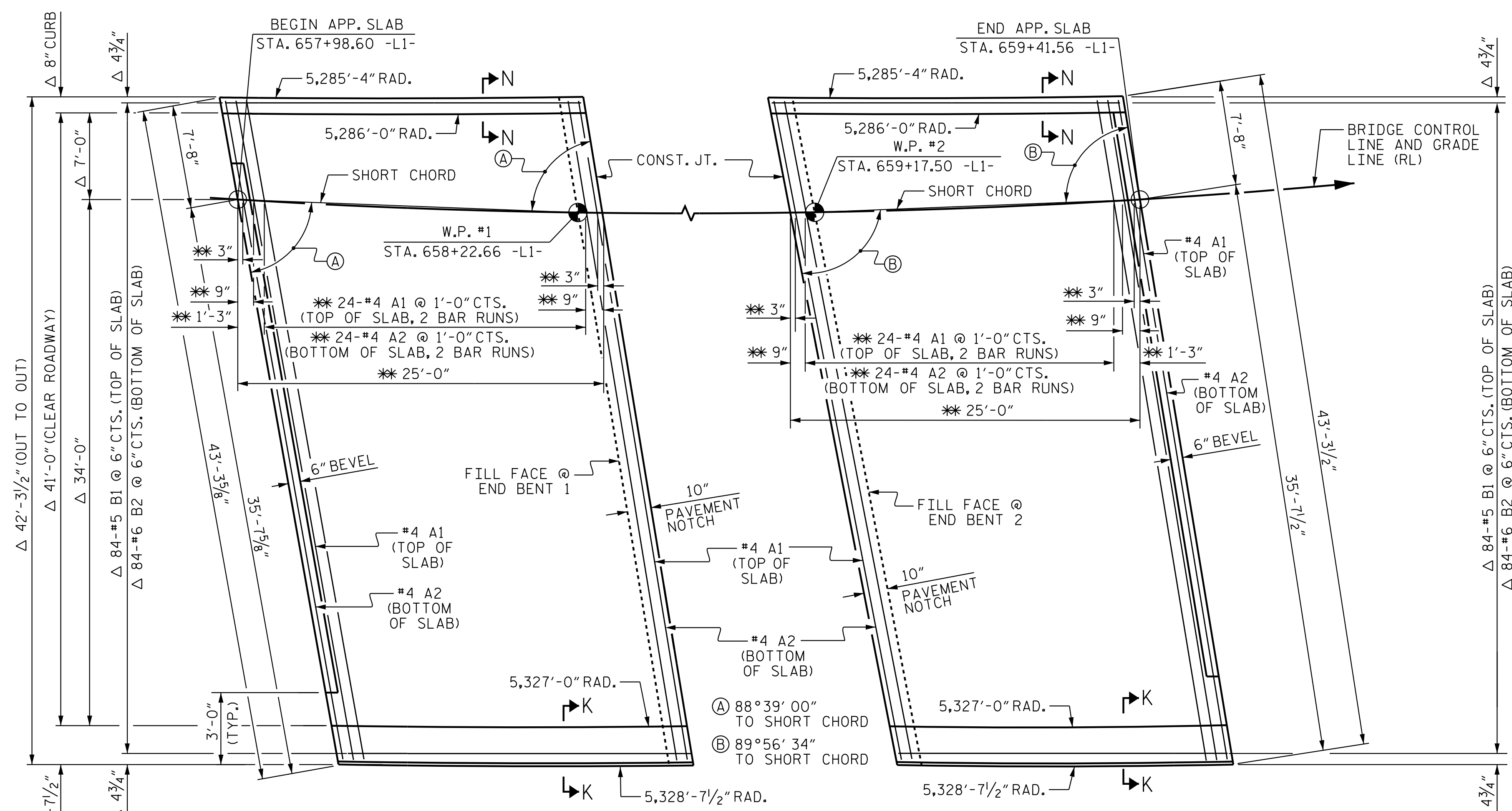
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1			3			TOTAL SHEETS 30
2			4			

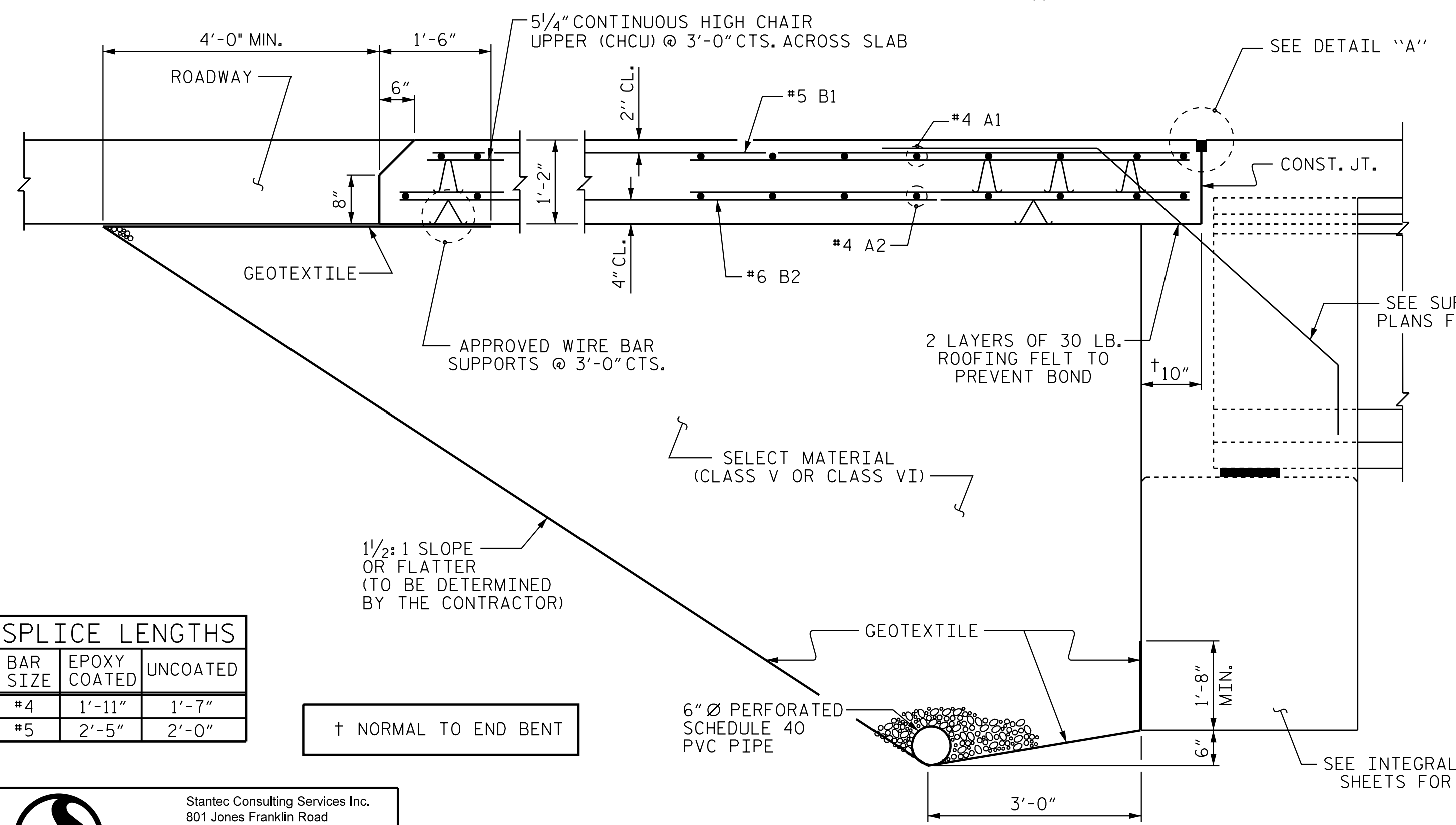


DRAWN BY: J. F. KENNEDY DATE: 09/23/19  
 CHECKED BY: N. D'AIUTO DATE: 09/24/19  
 DESIGN ENGINEER OF RECORD: M. B. ISENHOUR DATE: 10/25/21

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 10/25/2021  
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PLAN @ END BENT 1 PLAN @ END BENT 2  
DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS, UNLESS NOTED OTHERWISE.  
Δ RADIAL DIMENSION \*\* ALONG ARC



SECTION THRU SLAB  
(TYPE I - STANDARD APPROACH FILL)

SPlice LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"

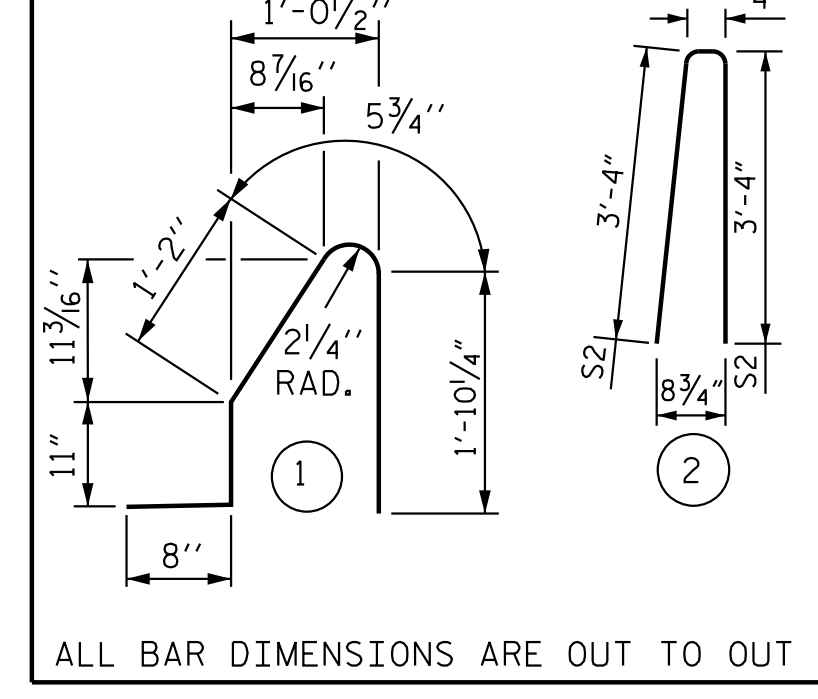
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DRAWN BY: J. F. KENNEDY DATE: 08/07/19  
CHECKED BY: N. D'AIUTO DATE: 08/28/19  
DESIGN ENGINEER OF RECORD: M. B. ISENHOUR DATE: 10/25/21

NOTES

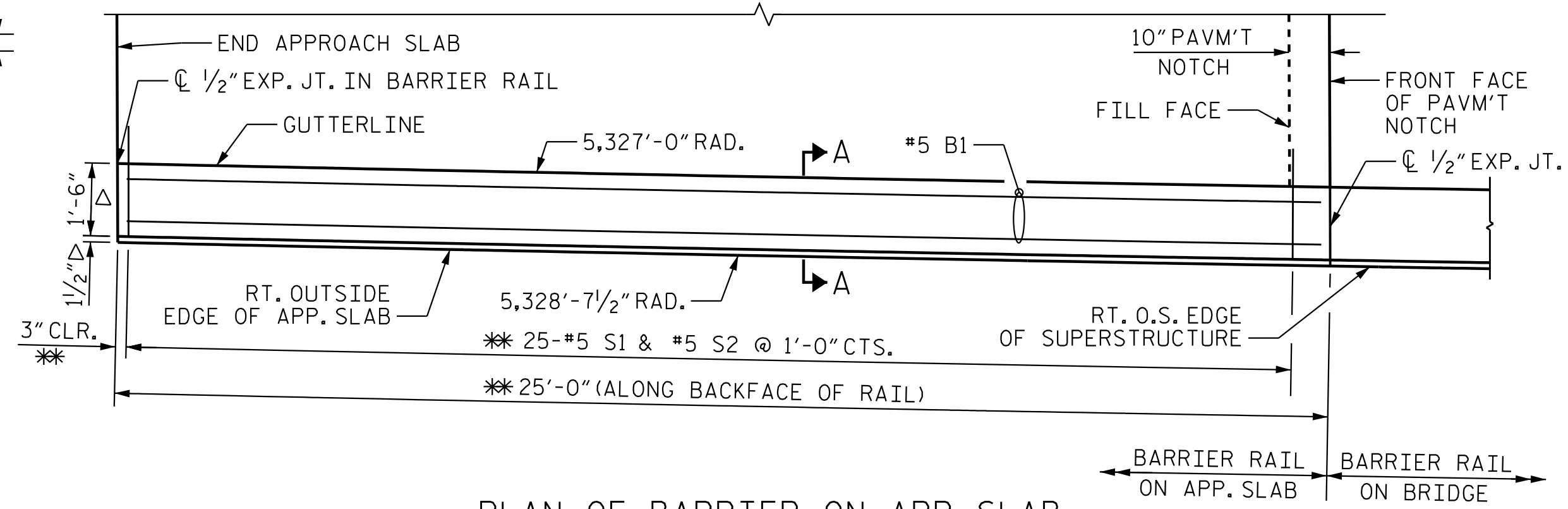
APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.  
FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.  
GEOTEXTILE SHALL BE TYPE I IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.  
SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.  
SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.  
FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.  
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.  
THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.  
AT THE CONTRACTORS OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.  
FOR ARC OFFSETS ON APPROACH SLABS, SEE "ARC OFFSETS" SHEET.  
FOR SECTION A-A, SEE SECTION THRU RAIL ON "CONCRETE BARRIER RAIL" SHEET.

BAR TYPES

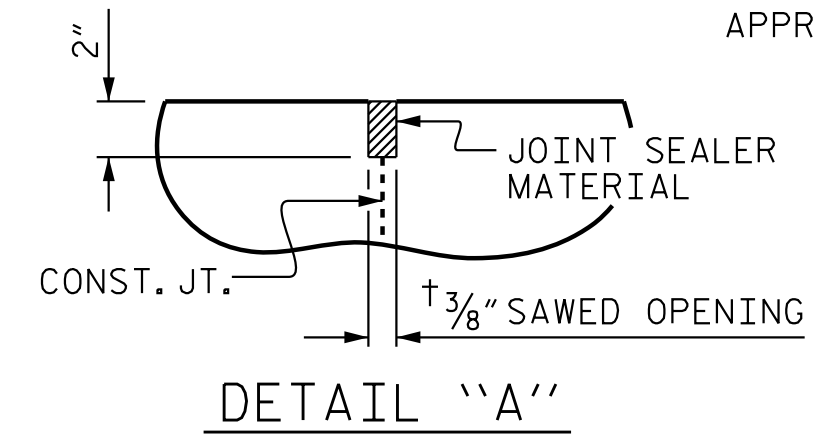


BILL OF MATERIAL

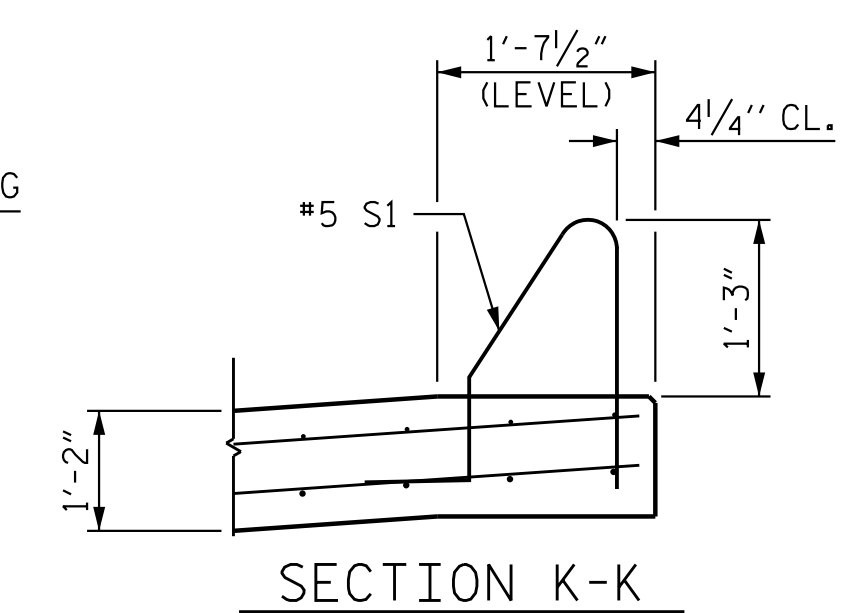
FOR ONE APPROACH SLAB (2 REQ'D)					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	52	#4	STR	22'-6"	782
A2	52	#4	STR	22'-4"	776
* B1	84	#5	STR	24'-1"	2,110
B2	84	#6	STR	24'-8"	3,113
REINFORCING STEEL				LBS.	3,889
* EPOXY COATED REINFORCING STEEL				LBS.	2,892
CLASS AA CONCRETE				C. Y.	46.7
FOR ONE BARRIER RAIL ON APPROACH SLAB (2 REQ'D)					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* B1	11	#5	STR	24'-7"	283
* S1	25	#5	1	5'-1"	133
* S2	25	#5	2	7'-0"	183
* EPOXY COATED REINFORCING STEEL				LBS.	599
CONCRETE BARRIER RAIL				LIN. FT.	25.0
CLASS AA CONCRETE				C. Y.	3.5



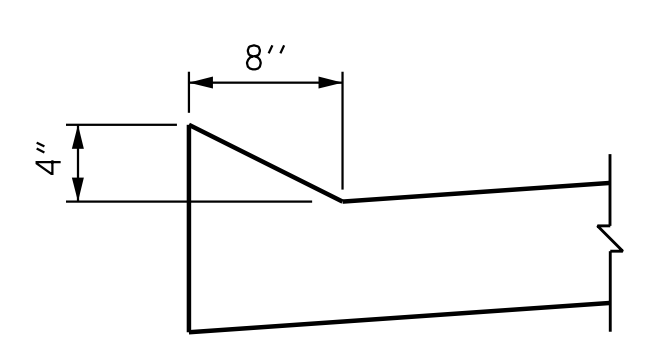
PLAN OF BARRIER ON APP. SLAB  
APPROACH SLAB @ END BENT 1 SHOWN. APPROACH SLAB @ END BENT 2 SIMILAR.



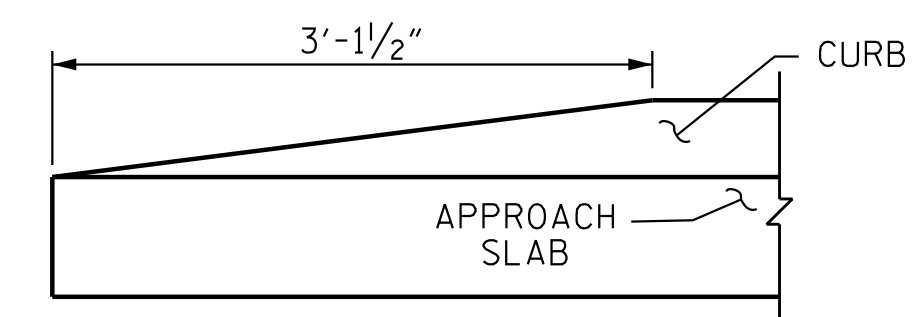
DETAIL "A"



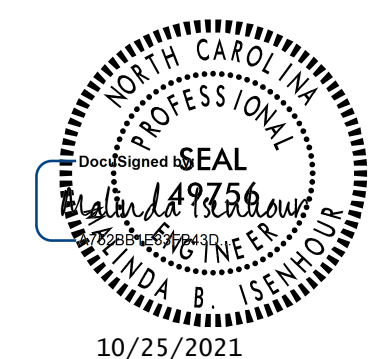
SECTION K-K



SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER



10/25/2021

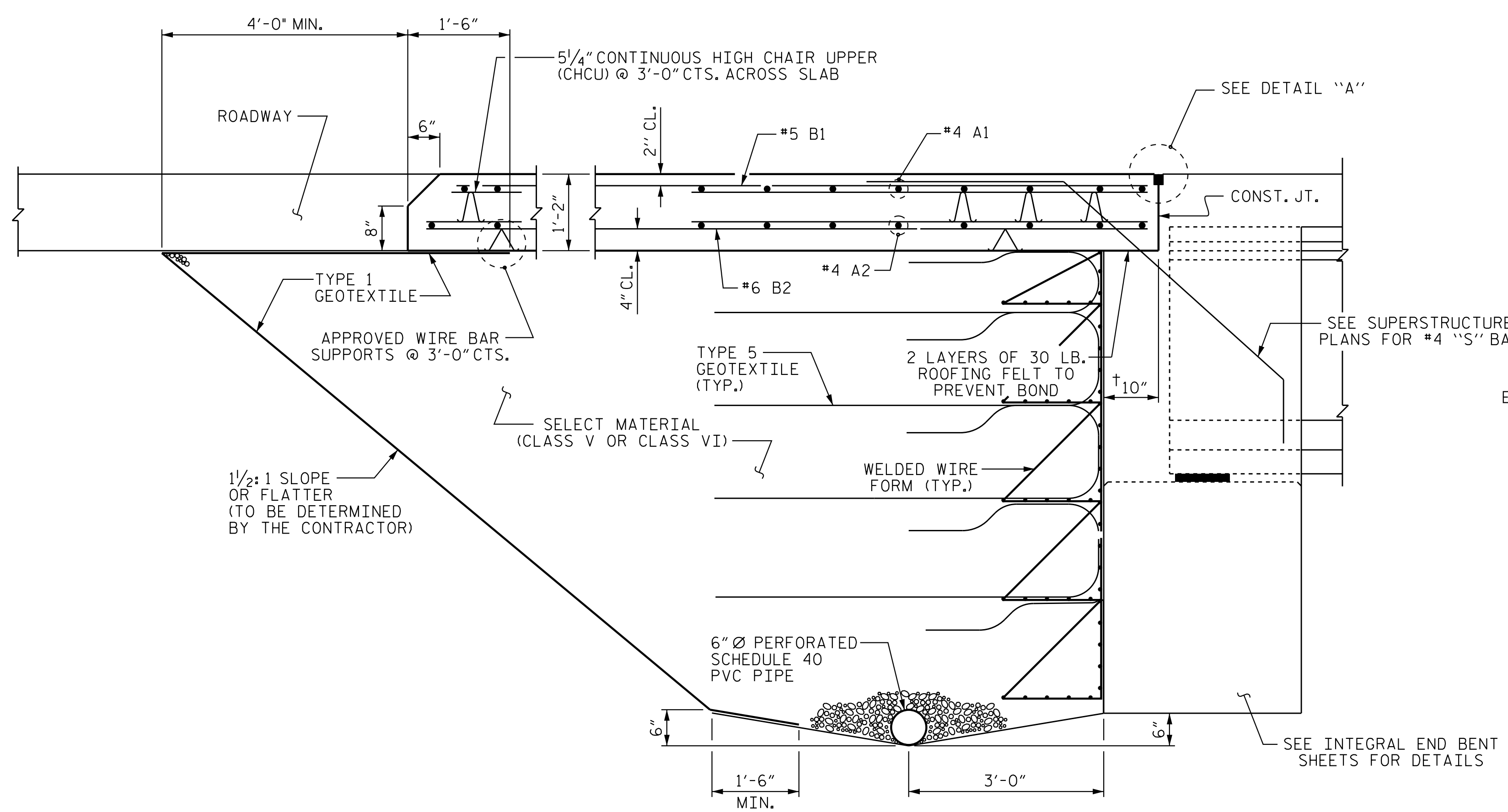
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PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 658+69.17 -L1-

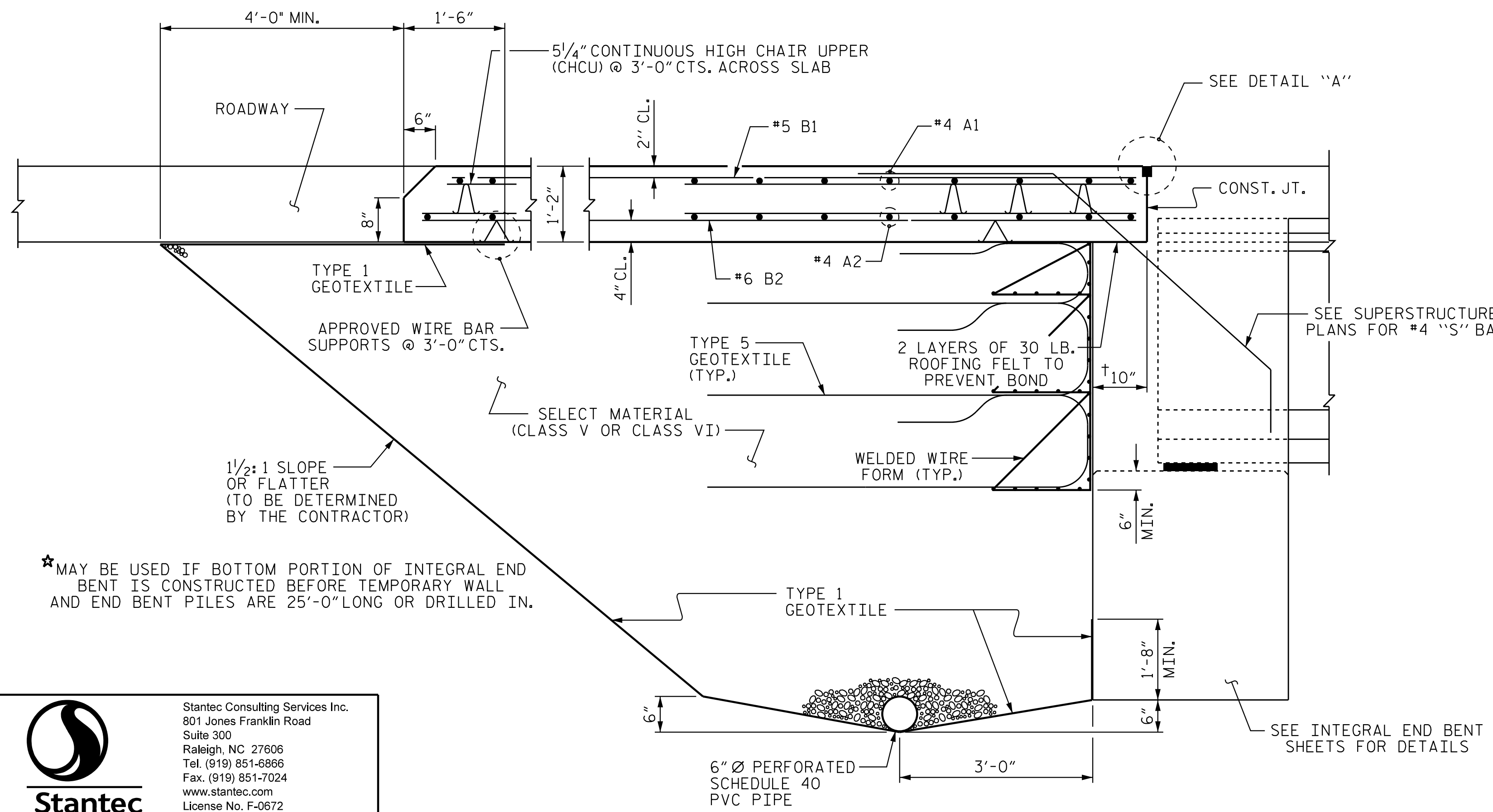
SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT WITH FLEXIBLE PAVEMENT (RIGHT LANE)					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

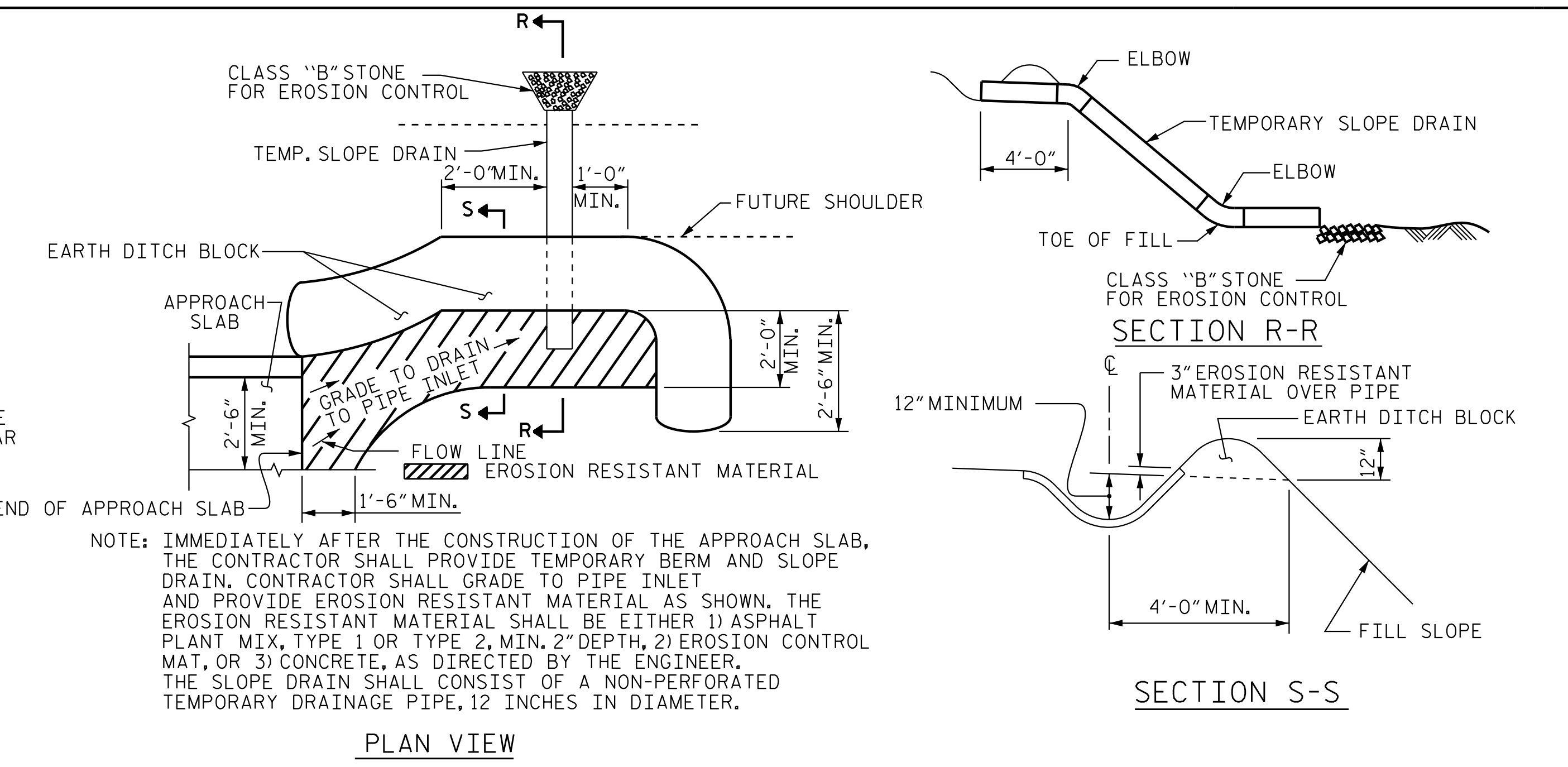
SHEET NO. S15-29	TOTAL SHEETS 30
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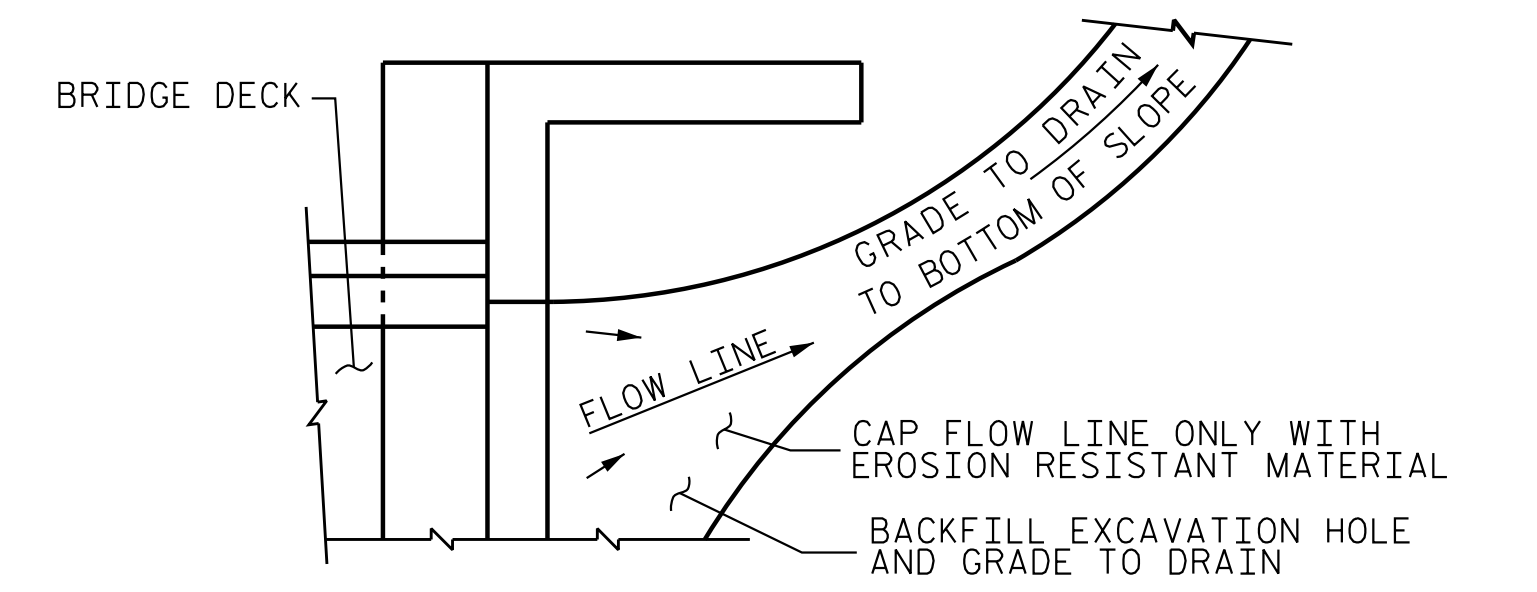
**SECTION THRU SLAB**  
(TYPE A - ALTERNATE APPROACH FILL)



**SECTION THRU SLAB**  
★(TYPE A - ALTERNATE APPROACH FILL)



**TEMPORARY BERM AND SLOPE DRAIN DETAILS**  
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

**TEMPORARY DRAINAGE DETAIL**

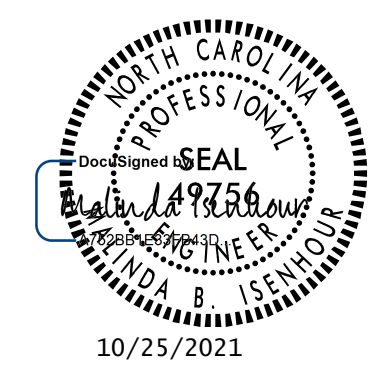
**NOTES**

- APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.
- FOR TEMPORARY GEOTEXTILE WALL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, WELDED WIRE FORM, AND SELECT MATERIAL, SEE ROADWAY PLANS.
- GEOTEXTILE (TYPE 1 OR TYPE 5) SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.
- SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.
- SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.
- FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.
- AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.
- THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 658+69.17 -L1-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
STANDARD BRIDGE APPROACH SLAB DETAILS (RIGHT LANE)					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S15-30					TOTAL SHEETS 30



DOCUMENT NOT CONSIDERED  
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SIGNATURES COMPLETED

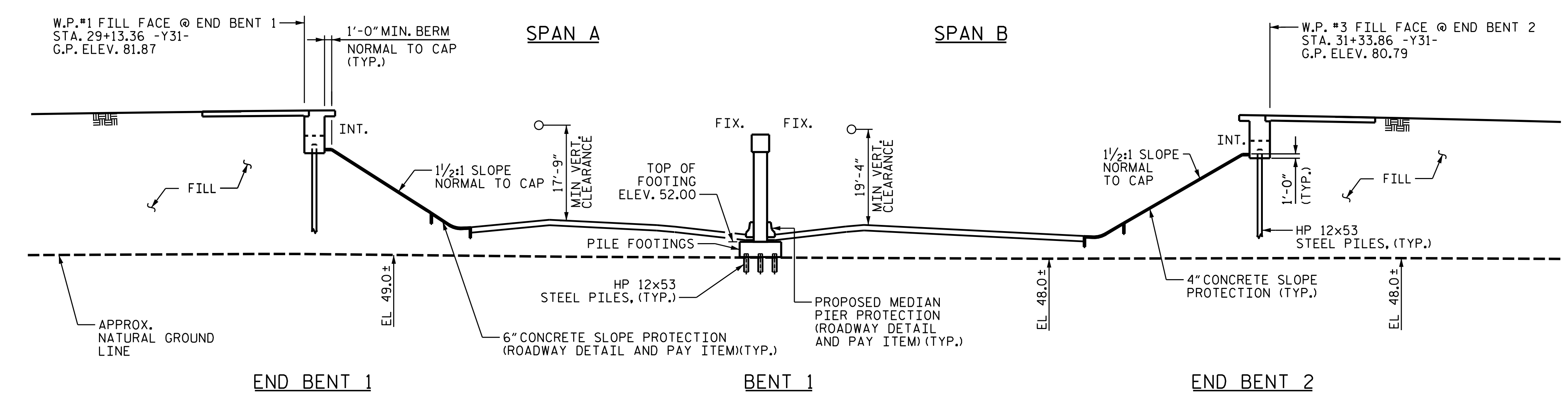
10/25/2021 jgelle U:\Structures\B15 - Y38 - \Dr-off\fin\Fin\R3300B.SMU.AS02\_700259.dgn

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Raleigh, NC 27606  
Tel. (919) 851-6866  
Fax. (919) 851-7024  
www.stantec.com  
License No. F-0672

ASSEMBLED BY : J. F. KENNEDY	DATE : 08/07/19	DESIGN ENGINEER OF RECORD: M. B. ISENHOUR	DATE : 10/25/21
CHECKED BY : N. D'AIUTO	DATE : 08/28/19		
DRAWN BY : TLA	10/05	REV. 12/21/11	MAA/GM
CHECKED BY : GM	5/06	REV. 6/13	MAA/GM
		REV. 12/17	MAA/THC

28+50 29+00 29+50 30+00 30+50 31+00 31+50 32+00

(+13.8046% Δ (-14.2209%)  
 PI = 30+00.00 -Y31-  
 EL = 88.95  
 VC = 680'  
 GRADE DATA -Y31-

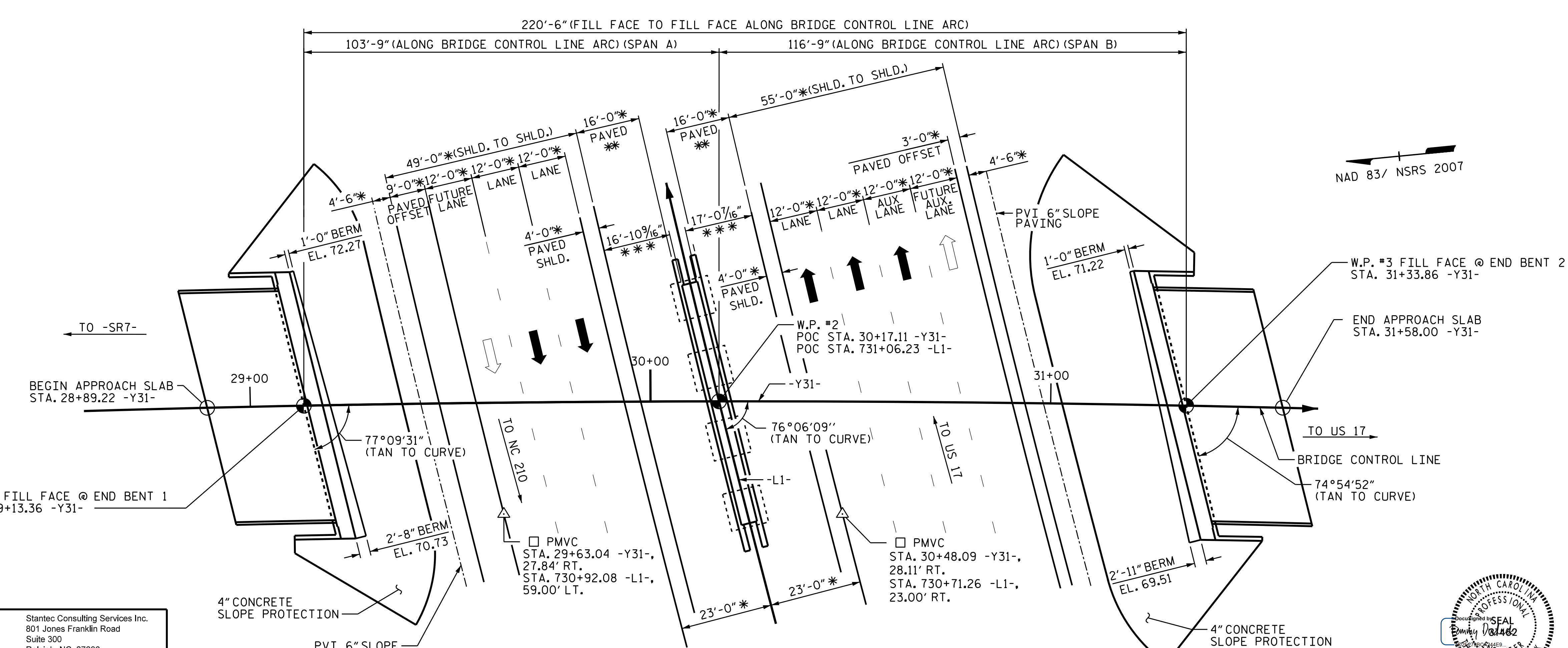


PI STA. = 31+36.77 -Y31-  
 Δ = 8°36'11.50" (RT.)  
 D = 1°01'03.67"  
 L = 845.37'  
 T = 423.48'  
 R = 5,630.00'  
 HORIZONTAL CURVE DATA -Y31-

PI STA. = 724+07.56 -L1-  
 Δ = 30°19'31.69" (RT.)  
 D = 1°22'50.24"  
 L = 2,196.51'  
 T = 1,124.63'  
 R = 4,150.00'  
 HORIZONTAL CURVE DATA -L1-

↑ DENOTES FUTURE TRAVEL LANE

SECTION ALONG -Y31-  
 (SECTION AT END BENTS AND BENTS ARE SHOWN AT RIGHT ANGLES)



PLAN  
 (PILES NOT SHOWN FOR CLARITY)

- \* RADIAL DIMENSION
- PMVC DENOTES PT. OF MIN. VERTICAL CLEARANCE
- \*\* MIN. HORZ. CLEARANCE TO FACE OF BARRIER PROTECTION
- \*\*\* MIN. HORZ. CLEARANCE TO FACE OF CAP



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PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 30+17.11 -Y31-  
 731+06.23 -L1-  
 SHEET 1 OF 4 BRIDGE NO. 700260

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-01
1			3			TOTAL SHEETS
2			4			38

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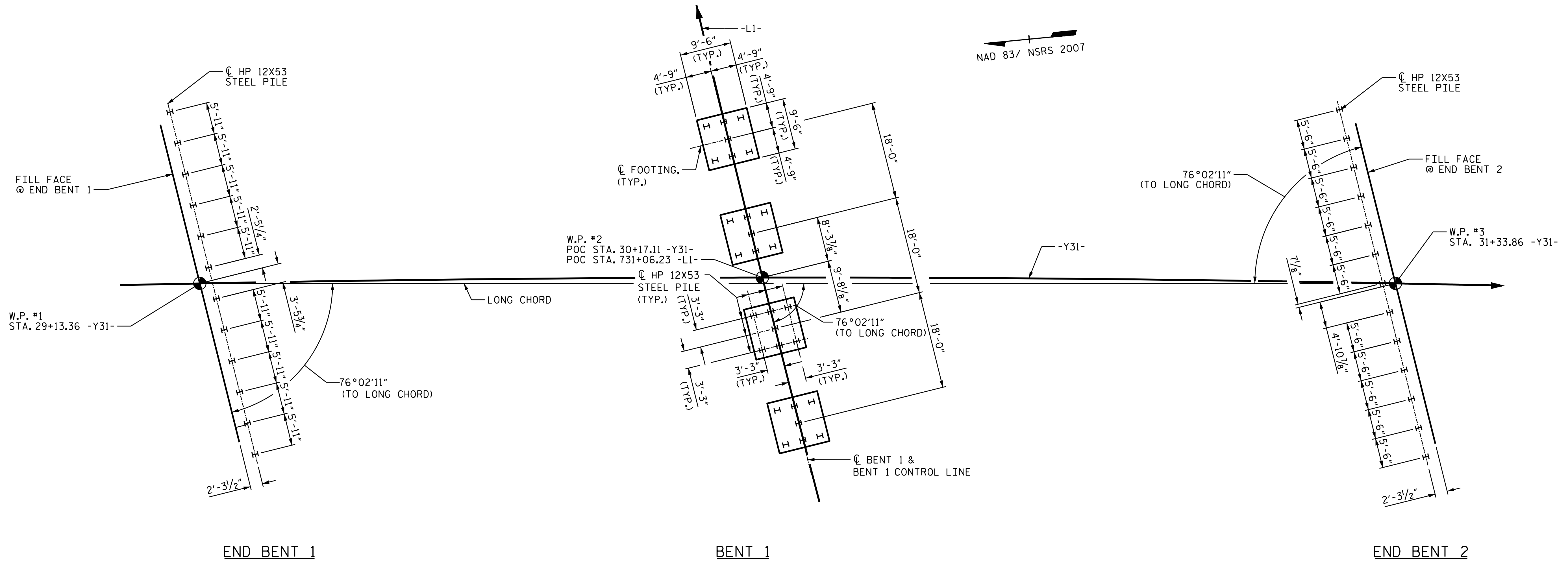
Stantec Consulting Services Inc.  
 801 Jones Franklin Road  
 Suite 300  
 Raleigh, NC 27606  
 Tel. (919) 851-8866  
 Fax. (919) 851-7024  
 www.stantec.com  
 License No. F-0672

DRAWN BY: J. B. GEILE DATE: 11/21/17  
 CHECKED BY: R. DeCOLA DATE: 01/09/18

DESIGN OF RECORD ENGINEER: T. R. DUDECK DATE: 10/25/21

10/25/2021

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### FOUNDATION LAYOUT

#### NOTES:

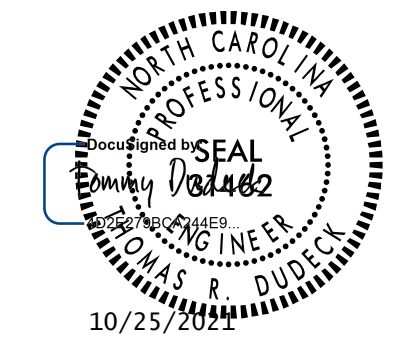
1. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
2. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
3. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 160 TONS PER PILE.
4. PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
5. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 160 TONS PER PILE.
6. STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
7. PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
8. DRIVE PILES AT BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 270 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG.
9. INSTALL PILES AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN -8.0 FT.
10. STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT BENT NO.1. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
11. IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 55 TO 75 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT NO.1. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.
12. TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING, OR REDRIVING IS REQUIRED AT END BENT NO.1 AND BENT NO.1. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
13. THE END BENT NO.1 AND 2 EMBANKMENTS ARE TO BE CONSTRUCTED IN STAGES. OBSERVE A 2.5 MONTH WAITING PERIOD FOR STAGE 1 AFTER CONSTRUCTING EMBANKMENT FILL TO EL. 68 FT BEFORE CONTINUING FILL PLACEMENT. OBSERVE ADDITIONAL 2.5 MONTH WAITING PERIOD FOR STAGE 2 AFTER CONSTRUCTING EMBANKMENT FILL TO EL. 77 FT BEFORE CONTINUING FILL PLACEMENT. OBSERVE AN ADDITIONAL 1 MONTH WAITING PERIOD FOR STAGE 3 AFTER CONSTRUCTING THE EMBANKMENT TO FINAL GRADE AND PLACING SURCHARGE BEFORE BEGINNING END BENT CONSTRUCTION AT END BENTS NO.1 AND 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS.
14. SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS FOR THE SETTLEMENT GAUGES, STAGED CONSTRUCTION, AND SURCHARGE REQUIRED AT END BENTS NO.1 AND 2.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

GENERAL DRAWING  
 FOR BRIDGE OVER NC 417  
 ON SR 1569 (HOOVER RD.)  
 BETWEEN US 17 & NC 210



DOCUMENT NOT CONSIDERED  
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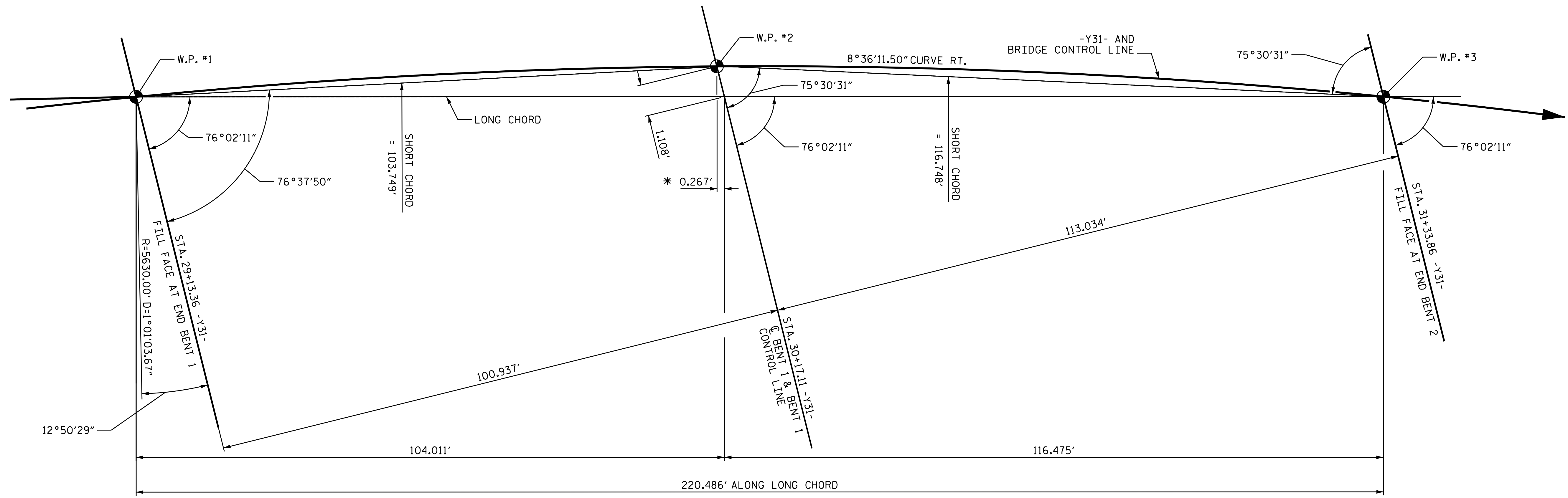
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-02
1			3			TOTAL SHEETS
2			4			38

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DRAWN BY : J. B. GEILE DATE : 11/28/17  
 CHECKED BY : R. DeCOLA DATE : 01/08/18  
 DESIGN ENGINEER OF RECORD : T. R. DUDECK DATE : 10/25/21

10/25/2021

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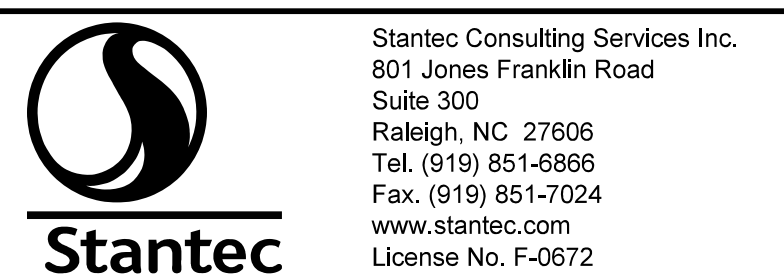
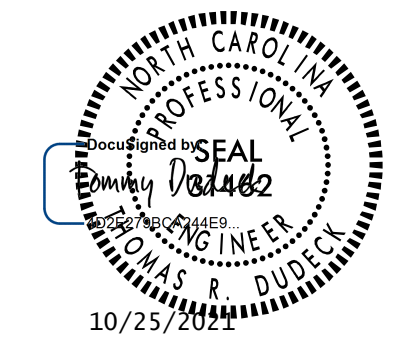
NOTES:  
 ALL BENTS ARE PARALLEL.  
 \* POINT PROJECTED ON LONG CHORD.

### LONG CHORD LAYOUT

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 3 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**GENERAL DRAWING**  
 FOR BRIDGE OVER NC 417  
 ON SR 1569 (HOOVER RD.)  
 BETWEEN US 17 & NC 210



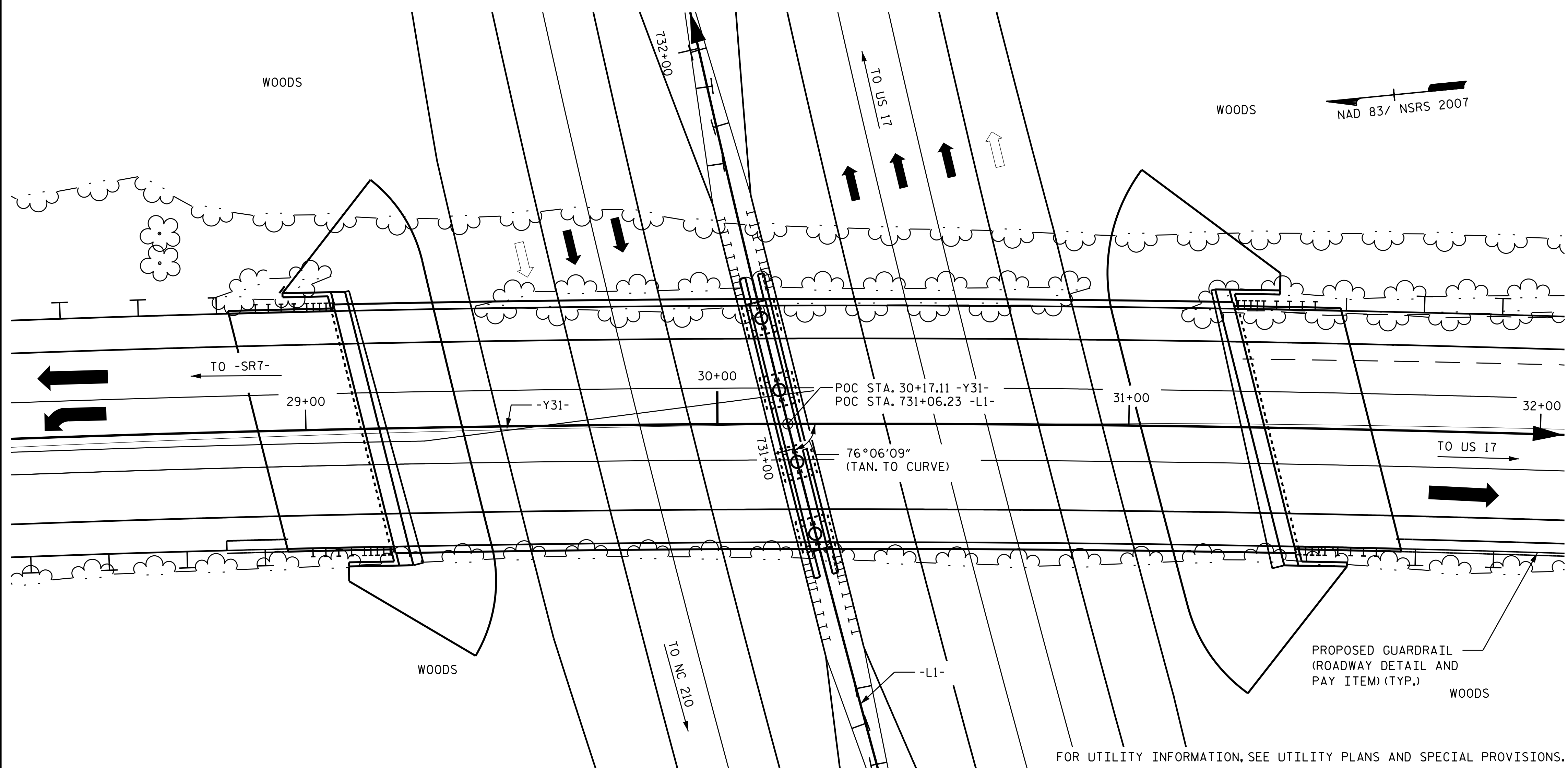
DRAWN BY : J. B. GEILE DATE : 12/20/17  
 CHECKED BY : R. DeCOLA DATE : 02/08/18  
 DESIGN ENGINEER OF RECORD : T. R. DUDECK DATE : 10/25/21

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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-03
1			3			TOTAL SHEETS
2			4			38



BM #29: R/R SPIKE SET IN 6" SWEET GUM. N 234830 E 2388303, STA. 719+28.86 -L1-, 593.20' RT. EL. 44.00



LOCATION SKETCH

- NOTES:**
- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
  - THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
  - THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
  - FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
  - FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
  - FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
  - FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
  - FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
  - PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
  - REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
  - FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
  - NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
  - FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

TOTAL BILL OF MATERIAL

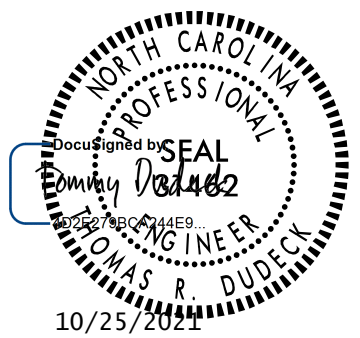
	FOUNDATION EXCAVATION FOR BENT	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS, STA. 30+17.11 -Y31-	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	MODIFIED 72" PRESTRESSED CONC. GIRDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	HP 12X53 STEEL PILES	STEEL PILE POINTS	PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS
	LUMP SUM	EACH	SO.FT.	SO.FT.	CU.YDS.	LUMP SUM	LBS.	LBS.	NO. LIN.FT.	EA.	NO. LIN.FT.	EA.	EA.	LIN.FT.	SO.YDS.	LUMP SUM
SUPERSTRUCTURE			13,291	10,627		LUMP SUM			12 1,298.06					437.56		LUMP SUM
END BENT NO.1					62.1		8,038			12	12 960.00	0	6		587	
BENT NO.1	LUMP SUM				123.6		18,349	1,604		28	28 1680.00	28	14			
END BENT NO.2					66.0		8,253			13	13 1040.00	13	7		788	
TOTAL	LUMP SUM	2	13,291	10,627	251.7	LUMP SUM	34,640	1,604	12 1,298.06	53	53 3680.00	41	27	437.56	1,375	LUMP SUM

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 4 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
 FOR BRIDGE OVER NC 17  
 ON SR 1569 (HOOVER RD.)  
 BETWEEN US 17 & NC 210



NO.	BY:	DATE:	REVISIONS			SHEET NO.
			NO.	BY:	DATE:	
1			3			S16-04
2			4			TOTAL SHEETS 38

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 www.stantec.com  
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DRAWN BY: J. B. GEILE DATE: 11/28/17  
 CHECKED BY: R. DeCOLA DATE: 01/09/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

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LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	$\gamma_{DC}$	$\gamma_{DW}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																								
LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.08	--	1.75	0.82	1.53	A	I	50.30	0.796	1.31	B	I	22.20	0.80	0.817	1.08	B	I	56.50		
	HL-93 (OPERATING)	N/A		1.71	--	1.35	0.82	1.98	A	I	50.30	0.796	1.71	B	I	22.20	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.44	51.840	1.75	0.82	2.11	A	I	50.30	0.796	1.86	B	I	22.20	0.80	0.752	1.44	A	ER	100.50		
	HS-20 (OPERATING)	36.000		2.43	87.480	1.35	0.82	2.74	A	I	50.30	0.796	2.43	B	I	22.20	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.64	49.140	1.40	0.82	6.22	A	I	50.30	0.796	6.22	B	I	22.20	0.80	0.817	3.64	B	I	56.50	
		SNGARBS2	20.000		2.55	51.000	1.40	0.82	4.50	A	I	50.30	0.796	4.31	B	I	22.20	0.80	0.752	2.55	A	ER	100.50	
		SNAGRIS2	22.000		1.28	28.160	1.40	0.82	2.27	A	I	50.30	0.796	2.09	B	I	22.20	0.80	0.752	1.28	A	ER	100.50	
		SNCOTTS3	27.250		1.43	38.968	1.40	0.82	2.49	A	I	50.30	0.796	2.28	B	I	22.20	0.80	0.752	1.43	A	ER	100.50	
		SNAGGRS4	34.925		1.22	42.609	1.40	0.82	2.16	A	I	50.30	0.796	2.01	B	I	22.20	0.80	0.752	1.22	A	ER	100.50	
		SNS5A	35.550		1.80	63.990	1.40	0.82	3.08	A	I	50.30	0.796	3.00	B	I	22.20	0.80	0.817	1.80	B	I	56.50	
		SNS6A	39.950		2.32	92.684	1.40	0.82	4.21	A	I	50.30	0.796	3.97	B	I	22.20	0.80	0.752	2.32	A	ER	100.50	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.55	51.150	1.40	0.82	2.77	A	I	50.30	0.796	2.49	B	I	22.20	0.80	0.752	1.55	A	ER	100.50	
		TNT4A	33.075		1.24	41.013	1.40	0.82	2.29	A	I	50.30	0.796	1.96	B	I	22.20	0.80	0.752	1.24	A	ER	100.50	
		TNT6A	41.600		1.20	49.920	1.40	0.82	2.21	A	I	50.30	0.796	1.88	B	I	22.20	0.80	0.752	1.20	A	ER	100.50	
		TNT7A	42.000		1.15	48.300	1.40	0.82	2.09	A	I	50.30	0.796	1.83	B	I	22.20	0.80	0.752	1.15	A	ER	100.50	
		TNT7B	42.000	③	1.14	47.880	1.40	0.82	2.07	A	I	50.30	0.796	1.89	B	I	22.20	0.80	0.752	1.14	A	ER	100.50	
		TNAGRIT4	43.000		1.55	66.650	1.40	0.82	2.76	A	I	50.30	0.796	2.41	B	I	22.20	0.80	0.752	1.55	A	ER	100.50	
		TNAGT5A	45.000		1.24	54.800	1.40	0.82	2.24	A	I	50.30	0.796	2.07	B	I	22.20	0.80	0.752	1.24	A	ER	100.50	
TNAGT5B	45.000		1.23	53.350	1.40	0.82	2.25	A	I	50.30	0.796	2.04	B	I	22.20	0.80	0.752	1.23	A	ER	100.50			

NOTES:

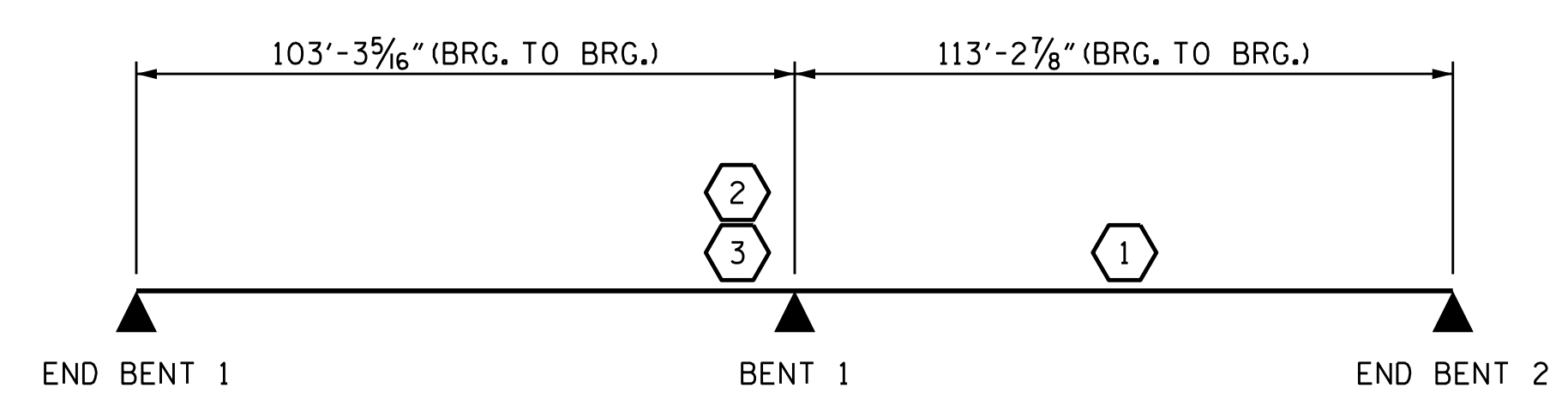
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- FOR LOAD RATING PURPOSE, IT IS ASSUMED A TIME PERIOD AT LEAST 40 DAYS SHALL ELAPSE FROM THE TRANSFER OF PRESTRESSING FORCE INTO ANY GIRDER AND THE PLACEMENT OF DECK SLAB CONCRETE IN THE SPANS.

#	CONTROLLING LOAD RATING
①	DESIGN LOAD RATING (HL-93)
②	DESIGN LOAD RATING (HS-20)
③	LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE	
GIRDER LOCATION	
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER	

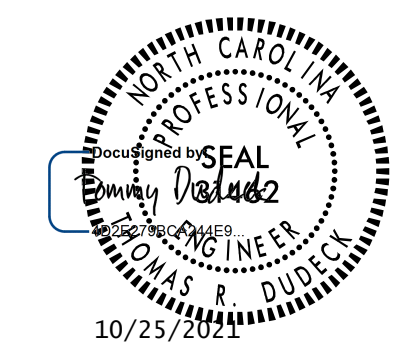


LRFR SUMMARY

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

Stantec Consulting Services Inc.  
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 www.stantec.com  
 License No. F-0672

ASSEMBLED BY : J. HAGENBUSH DATE : 11/30/17  
 CHECKED BY : R. DeCOLA DATE : 03/18/17  
 DRAWN BY : MAA 1/08 REV. 11/2/08RR MAA/GM DESIGN ENGINEER  
 CHECKED BY : GM/DI 2/08 REV. 10/1/11 MAA/GM ENGINEER  
 OF RECORD: T. R. DUDECK DATE : 10/25/21



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 LRFR SUMMARY FOR  
 PRESTRESSED  
 CONCRETE GIRDERS  
 (NON-INTERSTATE TRAFFIC)

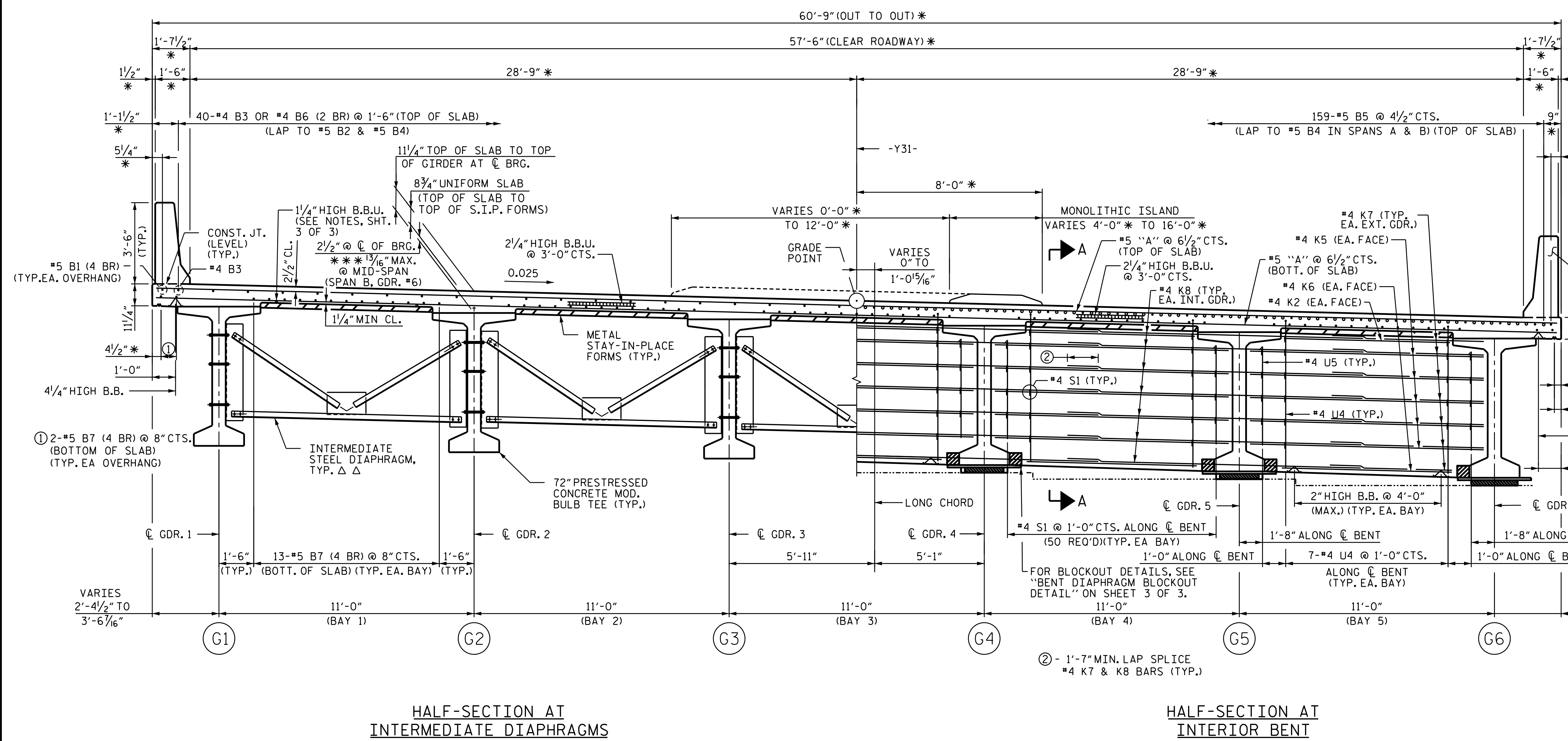
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	NO.	BY:	DATE:	NO.	BY:	DATE:	
	1			3			
	2			4			

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**NOTES**

SEE "TYPICAL SECTION DETAILS", SHEET 3 OF 3 FOR ADDITIONAL NOTES.

- DENOTES CONTINUOUS LONGITUDINAL DECK REINFORCEMENT.
- DENOTES NON-CONTINUOUS LONGITUDINAL DECK REINFORCEMENT.
- 2 BR DENOTES 2 BAR RUN.
- 4 BR DENOTES 4 BAR RUN.
- \* DENOTES RADIAL DIMENSION.
- Δ Δ FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.
- \*\*\* BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS.
- FOR CONCRETE BARRIER RAIL REINFORCING STEEL AND DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET, (TYP.)



HALF-SECTION AT INTERMEDIATE DIAPHRAGMS

HALF-SECTION AT INTERIOR BENT

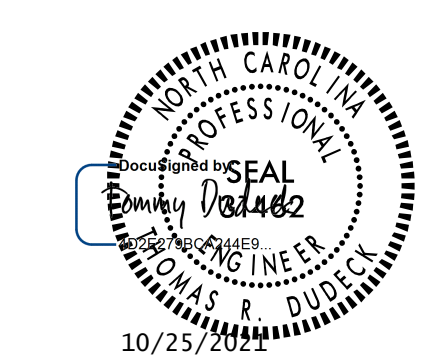
TYPICAL SECTION

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE

TYPICAL SECTION



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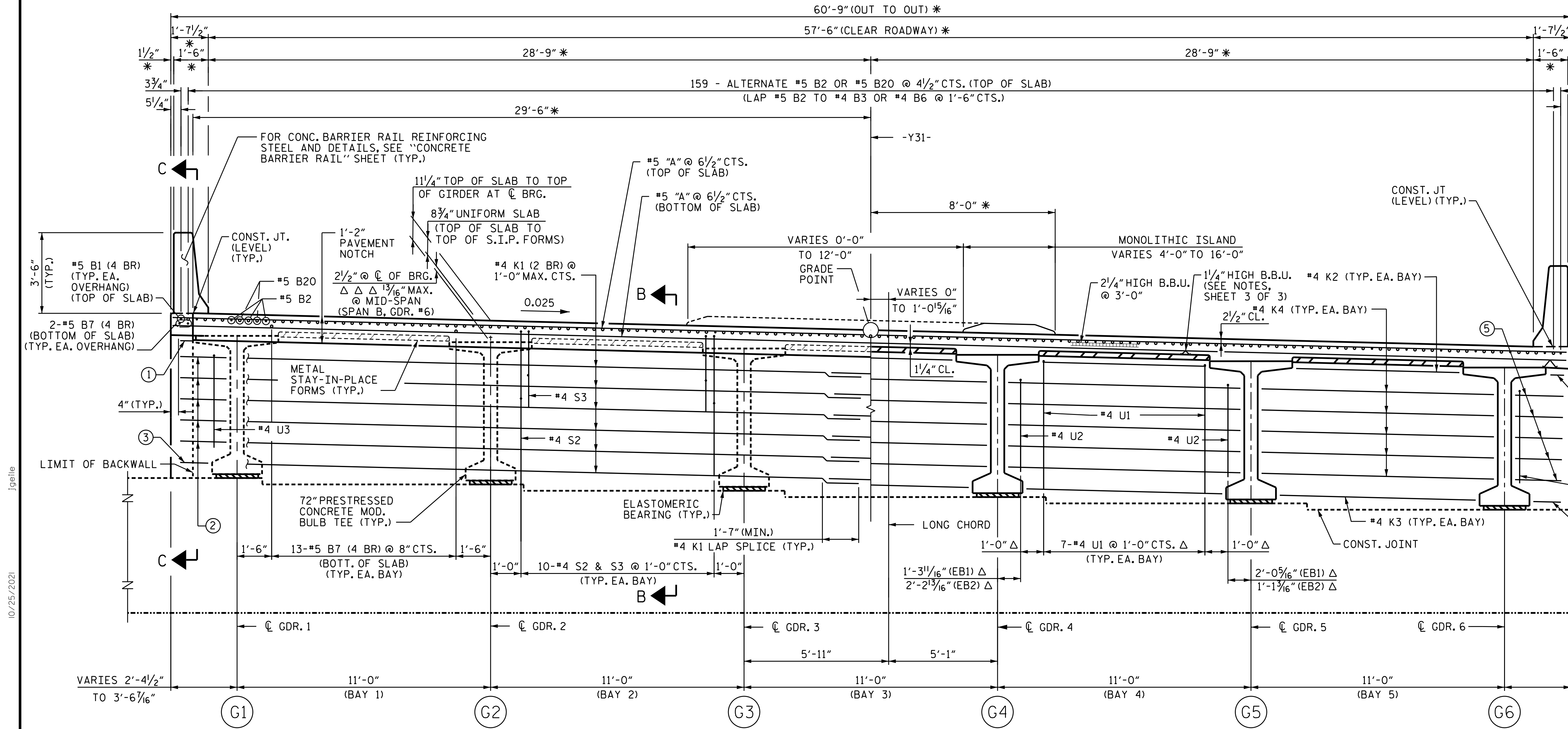
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NO.	BY:	DATE:	NO.	BY:	DATE:	S16-06
1			3			TOTAL SHEETS
2			4			38

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 Tel. (919) 851-6866  
 Fax. (919) 851-7024  
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DRAWN BY: J. B. GEILE DATE: 12/11/17  
 CHECKED BY: R. DeCOLA DATE: 01/17/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

10/25/2021

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**NOTES**

SEE "TYPICAL SECTION DETAILS", SHEET 3 OF 3 FOR ADDITIONAL NOTES.

- DENOTES CONTINUOUS LONGITUDINAL DECK REINFORCEMENT.
- DENOTES NON-CONTINUOUS LONGITUDINAL DECK REINFORCEMENT.
- 3 BR DENOTES 3 BAR RUN.
- 4 BR DENOTES 4 BAR RUN.
- \* DENOTES RADIAL DIMENSION.
- \*\*\*EB1 DENOTES END BENT 1 EB2 DENOTES END BENT 2.
- △ DENOTES MEASURED ALONG FRONT FACE, SEE END BENT SHEETS.
- △ △ △ BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS.

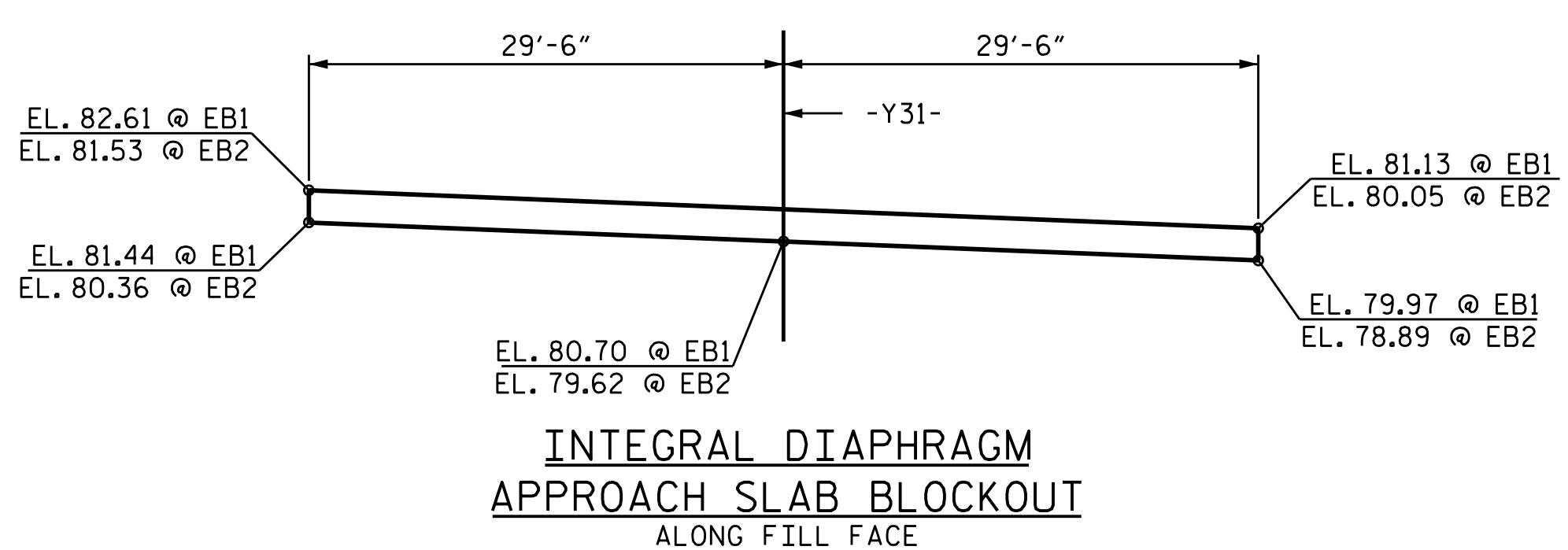
**INTEGRAL DIAPHRAGM  
HALF-SECTION REINFORCEMENT  
AT FILL FACE**

**INTEGRAL DIAPHRAGM  
HALF-SECTION REINFORCEMENT  
AT FRONT FACE**

**TYPICAL SECTION**

- ① #4 K15 (EB2) FRONT FACE \*\*\*
- ② #4 K9 (EB1) FILL FACE \*\*\*  
#4 K11 (EB1) FRONT FACE  
#4 K12 (EB2) FILL FACE  
#4 K14 (EB2) FRONT FACE
- ③ #4 K9 (EB1) FILL FACE \*\*\*  
#4 K10 (EB1) FRONT FACE  
#4 K12 (EB2) FILL FACE  
#4 K13 (EB2) FRONT FACE

- ④ #4 K15 (EB1) FRONT FACE \*\*\*  
#4 K10 (EB2) FRONT FACE
- ⑤ #4 K12 (EB1) FILL FACE \*\*\*  
#4 K14 (EB1) FRONT FACE  
#4 K9 (EB2) FILL FACE  
#4 K16 (EB2) FRONT FACE
- ⑥ #4 K12 (EB1) FILL FACE \*\*\*  
#4 K13 (EB1) FRONT FACE  
#4 K9 (EB2) FILL FACE  
#4 K11 (EB2) FRONT FACE



**INTEGRAL DIAPHRAGM  
APPROACH SLAB BLOCKOUT  
ALONG FILL FACE**

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 3

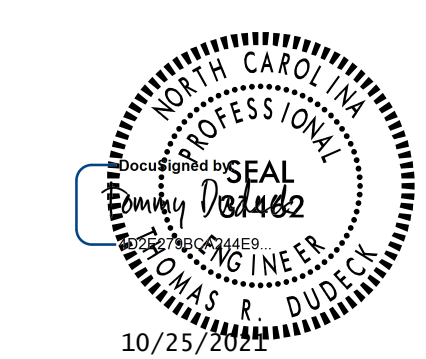
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 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE

**TYPICAL SECTION**

REVISIONS						SHEET NO.
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1			3			TOTAL SHEETS
2			4			38

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DRAWN BY: J. B. GEILE DATE: 11/08/17  
 CHECKED BY: R. DeCOLA DATE: 01/17/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21



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10/25/2021 10:25:20 AM I:\Projects\1700260.dgn

**NOTES**

PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS, WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

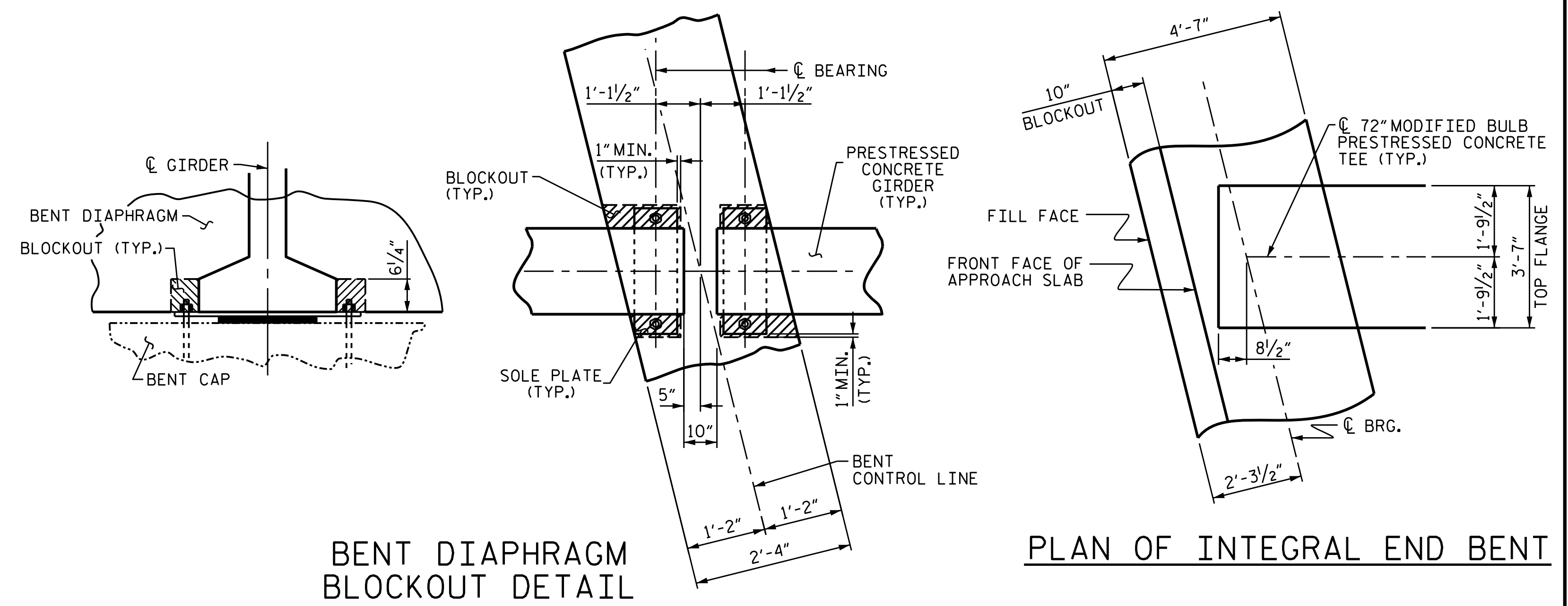
NO CHAMFER IS REQUIRED ON CORNERS OF GIRDER BUILDUPS.

PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT EXCEPT AS NOTED IN THE PLANS.

ALL REINFORCING STEEL IN CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

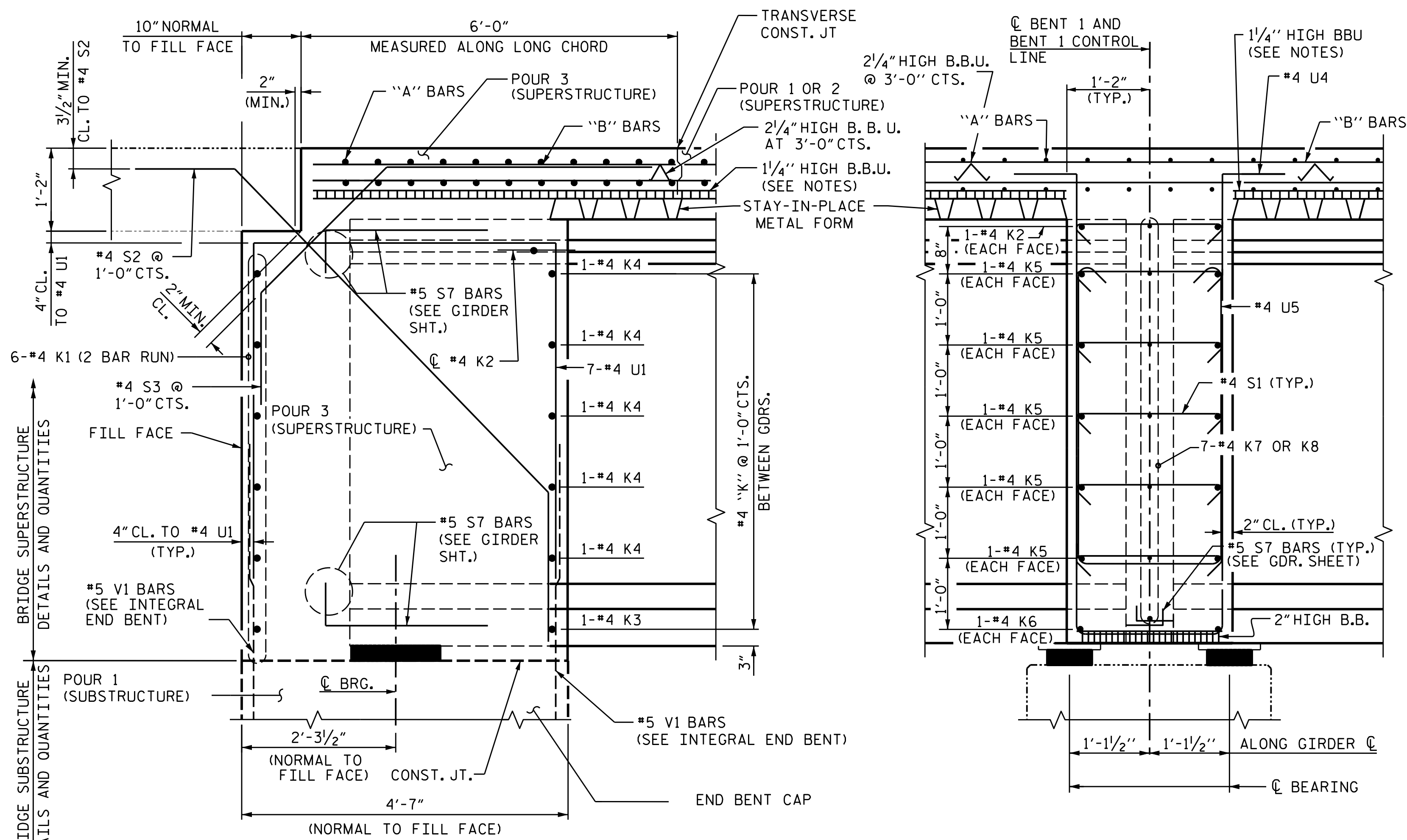
A TIME PERIOD OF AT LEAST 40 DAYS SHALL ELAPSE FROM THE TRANSFER OF PRESTRESSING FORCE INTO ANY GIRDER AND THE PLACEMENT OF DECK SLAB CONCRETE IN THE SPANS.

ALL REINFORCING STEEL IN CONCRETE MEDIANS SHALL BE EPOXY COATED.



**BENT DIAPHRAGM BLOCKOUT DETAIL**

**PLAN OF INTEGRAL END BENT**

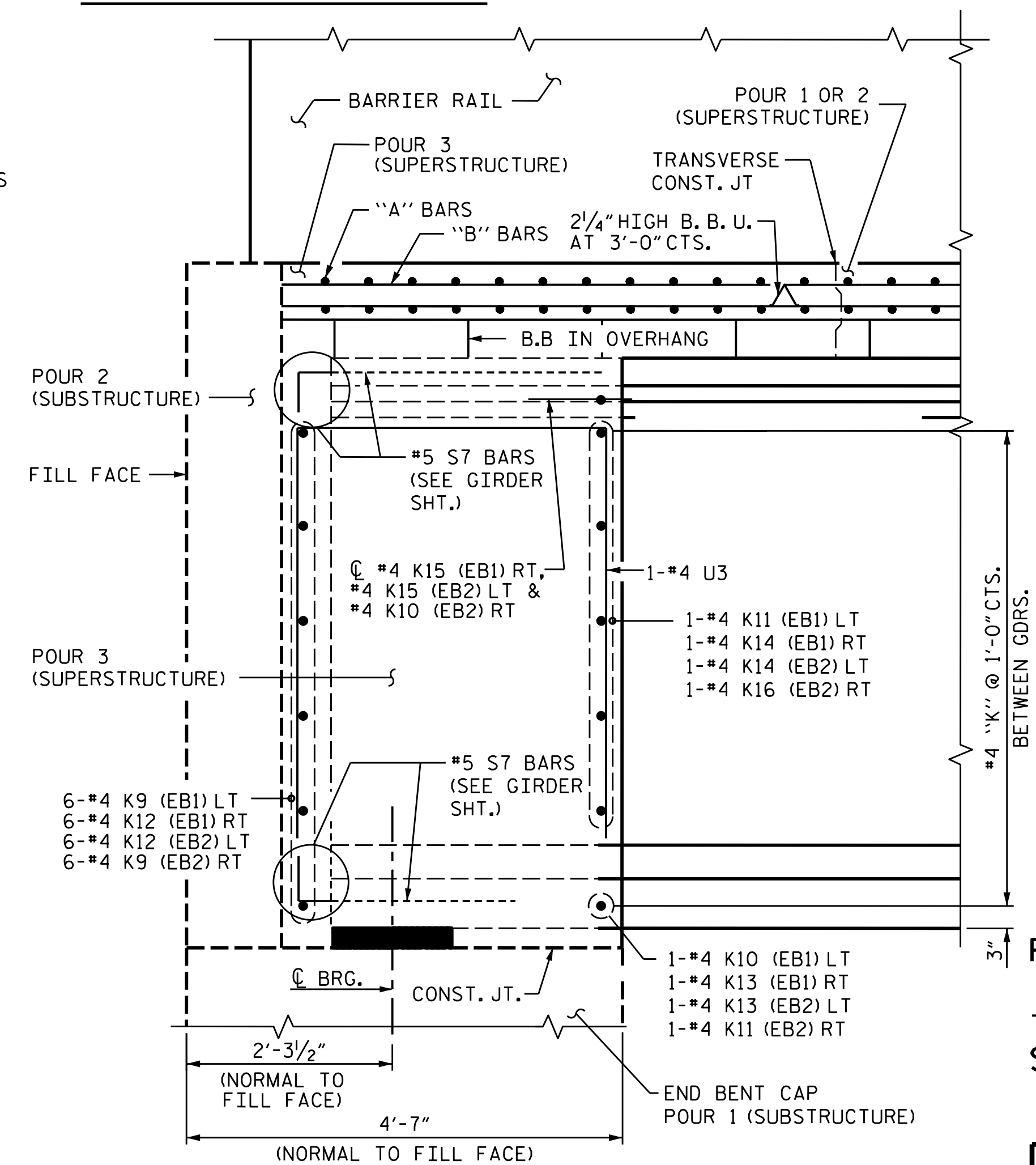


**SECTION B-B**

SECTION THRU INTEGRAL END BENT DIAPHRAGM  
WORK WITH "PLAN OF SPAN DETAILS - DIAPHRAGMS", SH. 3 OF 3"

**SECTION A-A**

SECTION THRU DIAPHRAGM @ INTERIOR BENT  
WORK WITH "PLAN OF SPAN DETAILS - DIAPHRAGMS", SH. 3 OF 3"



**SECTION C-C**

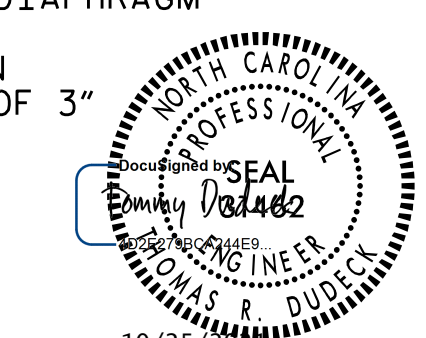
SECTION THRU INTEGRAL END BENT DIAPHRAGM  
BEYOND EXTERIOR GIRDER  
WORK WITH "PLAN OF SPAN  
DETAILS - DIAPHRAGMS", SH. 3 OF 3"

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
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RALEIGH  
SUPERSTRUCTURE

TYPICAL SECTION  
(DETAILS)



10/25/2021

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REVISIONS						SHEET NO.
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2			4			38

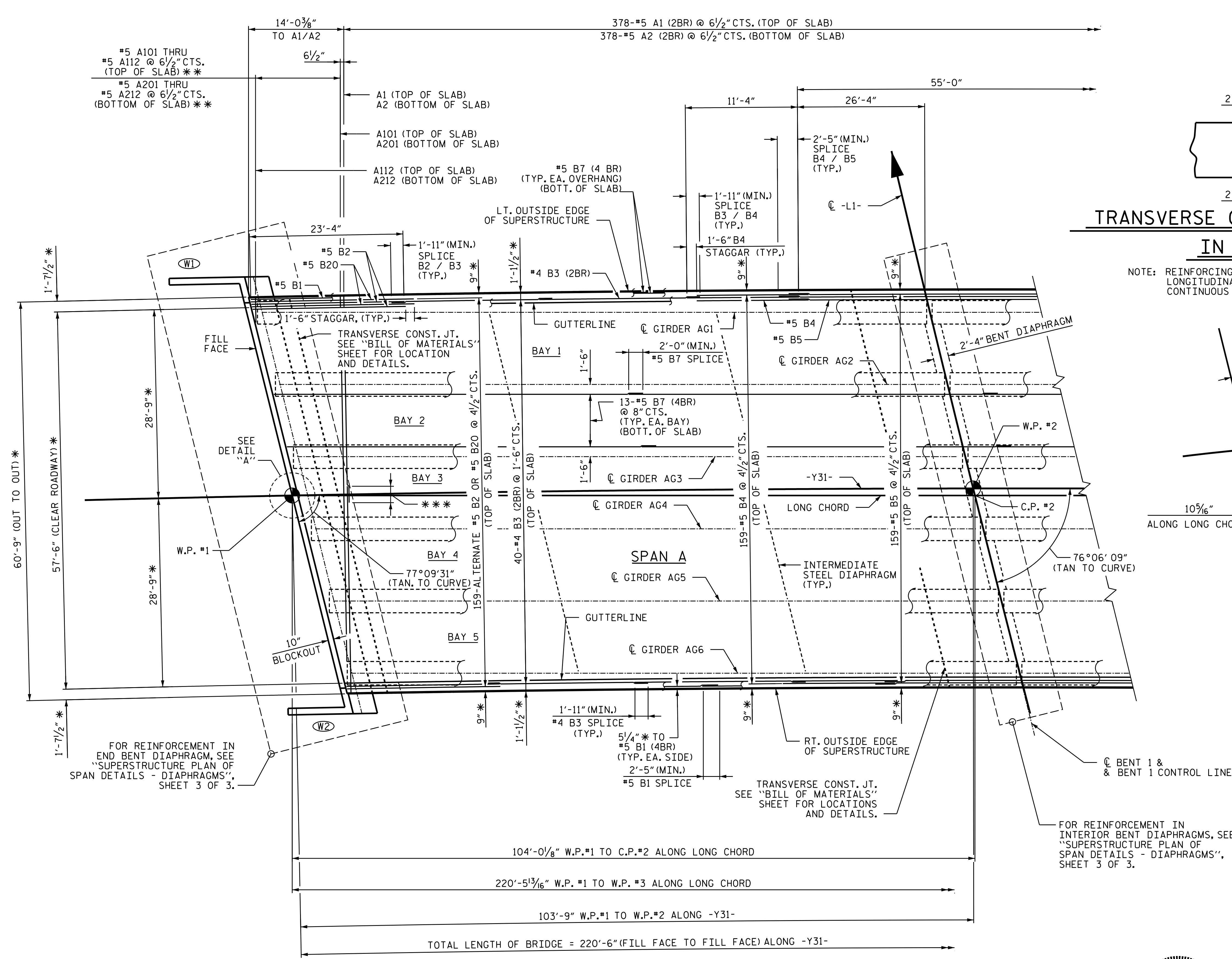
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DRAWN BY: J. B. GEILE DATE: 12/12/17  
CHECKED BY: R. DeCOLA DATE: 1/25/18  
DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

jgeile

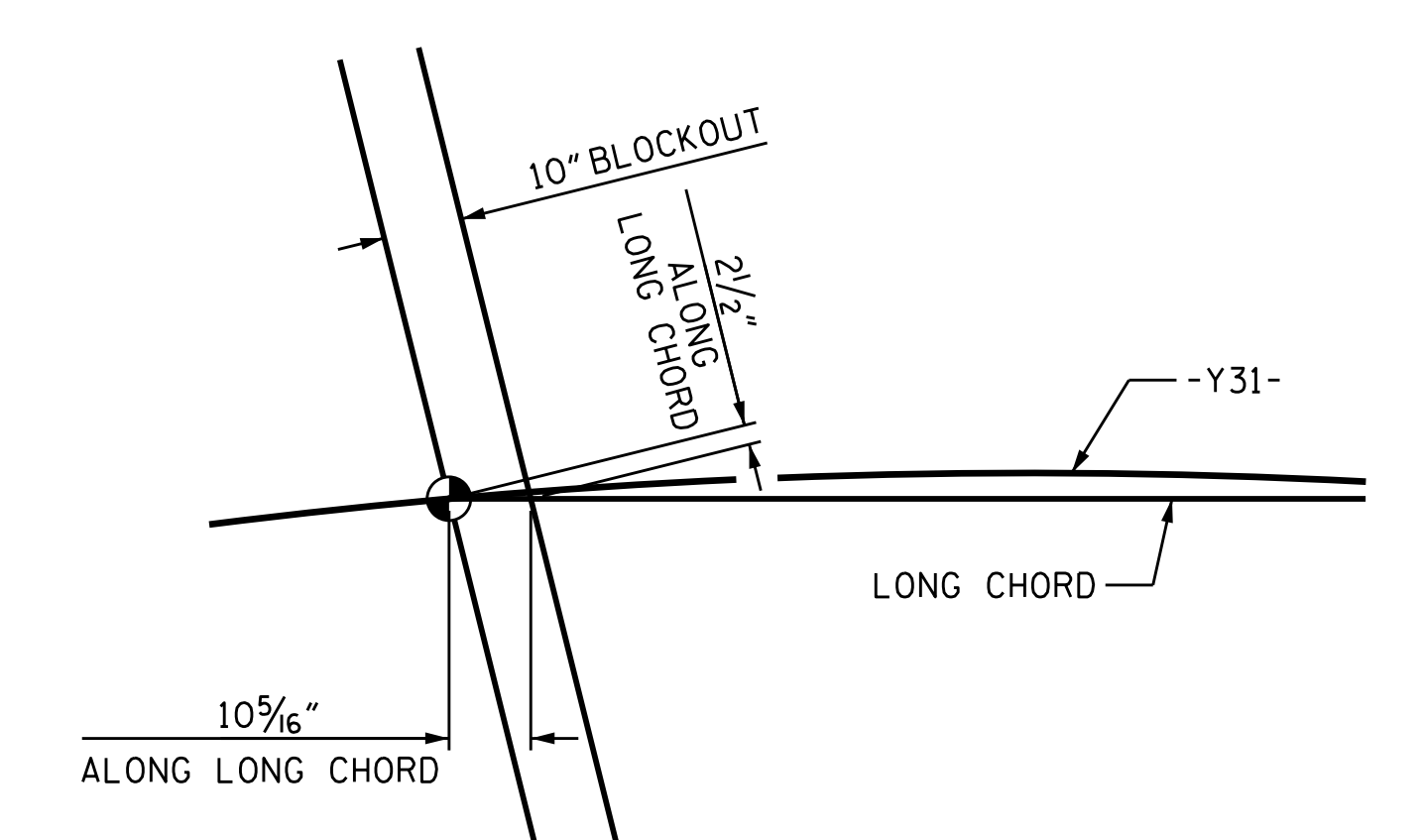
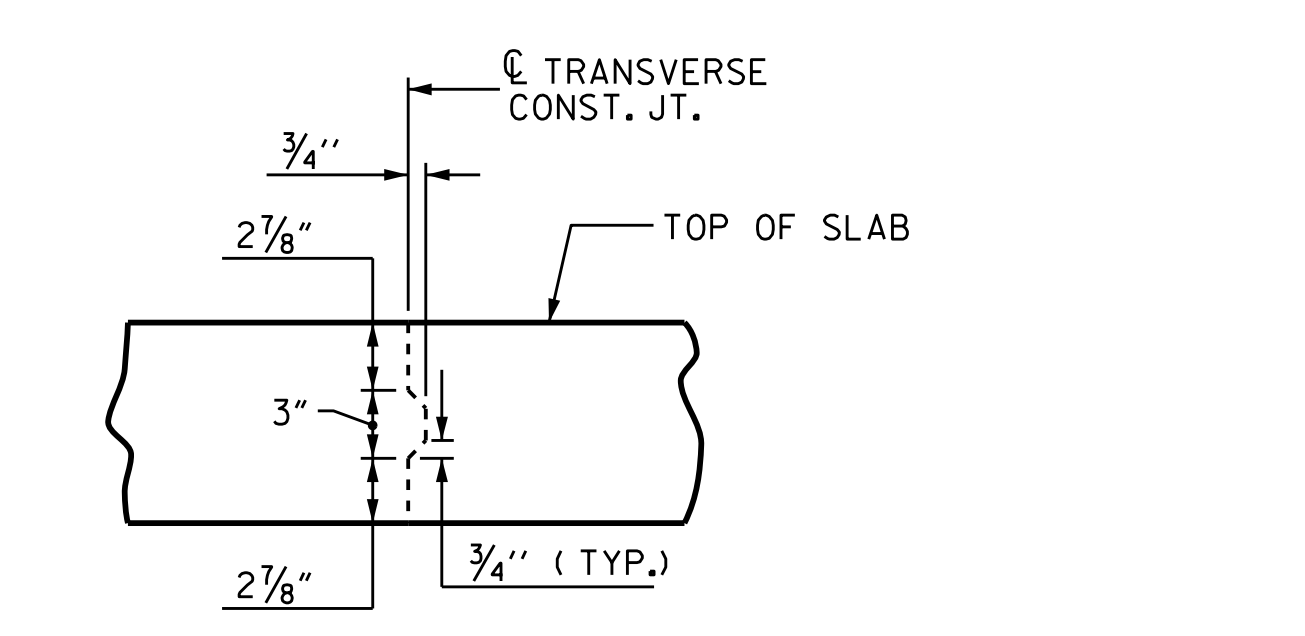
10/25/2021

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### TRANSVERSE CONSTRUCTION JOINT IN DECK SLAB

NOTE: REINFORCING STEEL IN DECK SLAB NOT SHOWN.  
LONGITUDINAL REINFORCING STEEL SHALL BE  
CONTINUOUS THRU JOINT.



### DETAIL "A"

### PLAN OF SPAN SPAN A

NOTE: (2BR) DENOTES TWO BAR RUN  
(4BR) DENOTES FOUR BAR RUN

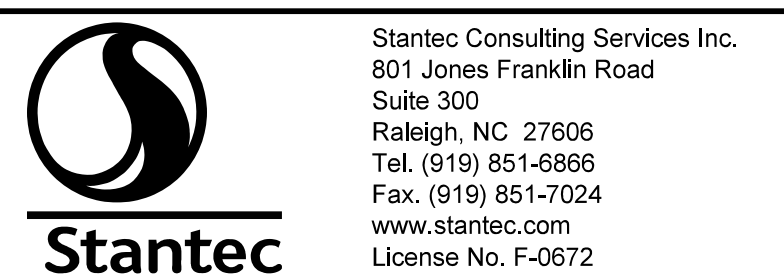
PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
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RALEIGH  
SUPERSTRUCTURE

### PLAN OF SPAN (SPAN A)

REVISIONS						SHEET NO. S16-09
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 38
2			4			



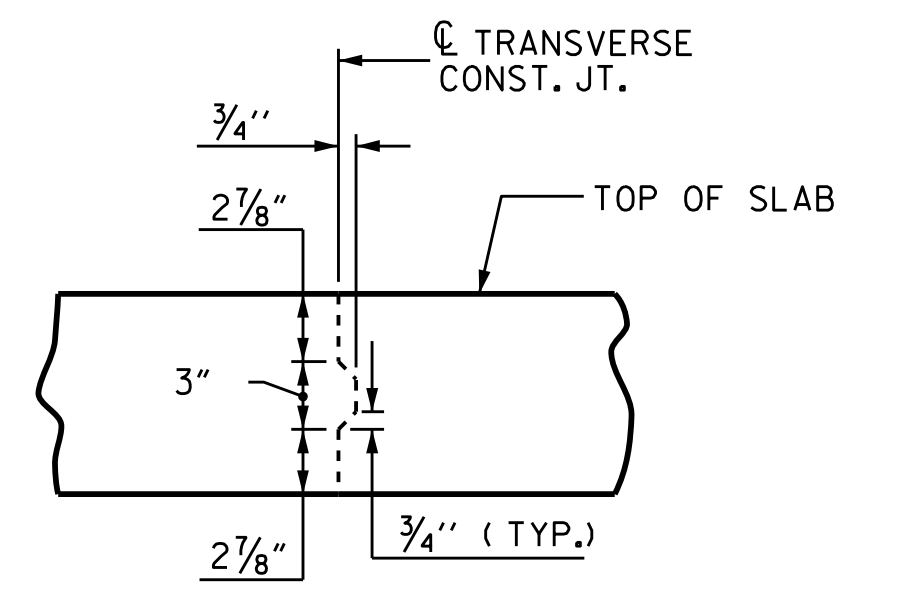
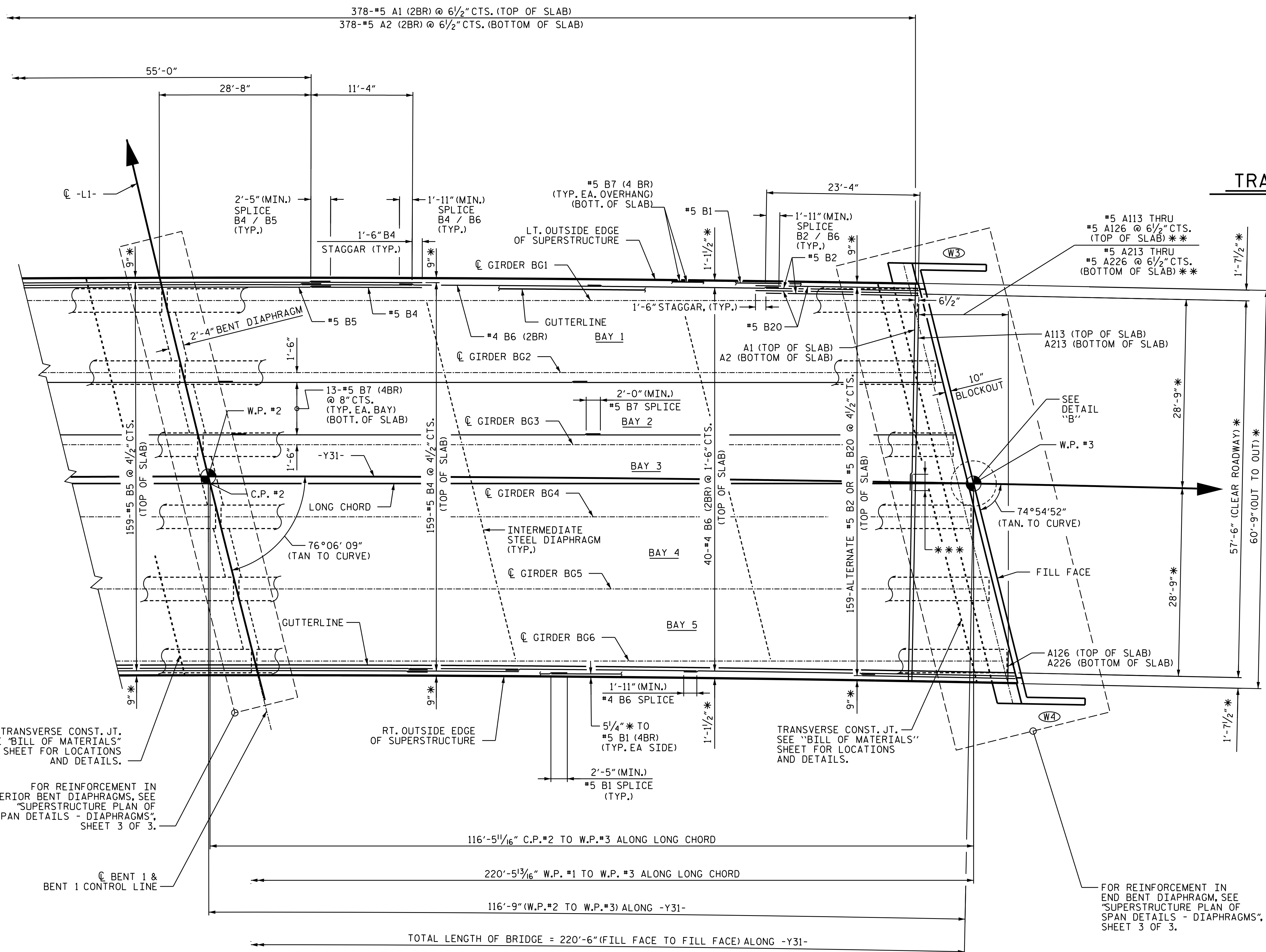
- \* - DENOTES RADIAL DIMENSION
- \*\* - DENOTES TWO BARS / MARK
- \*\*\* - 2'-5" (MIN.) #5 A1 LAP SPLICE
- \*\*\*\* - 2'-0" (MIN.) #5 A2 LAP SPLICE

DRAWN BY: J. E. HAGENBUSH DATE: 12/20/17  
CHECKED BY: R. DeCOLA DATE: 03/15/18  
DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

10/25/2021  
10/25/2021  
10/25/2021

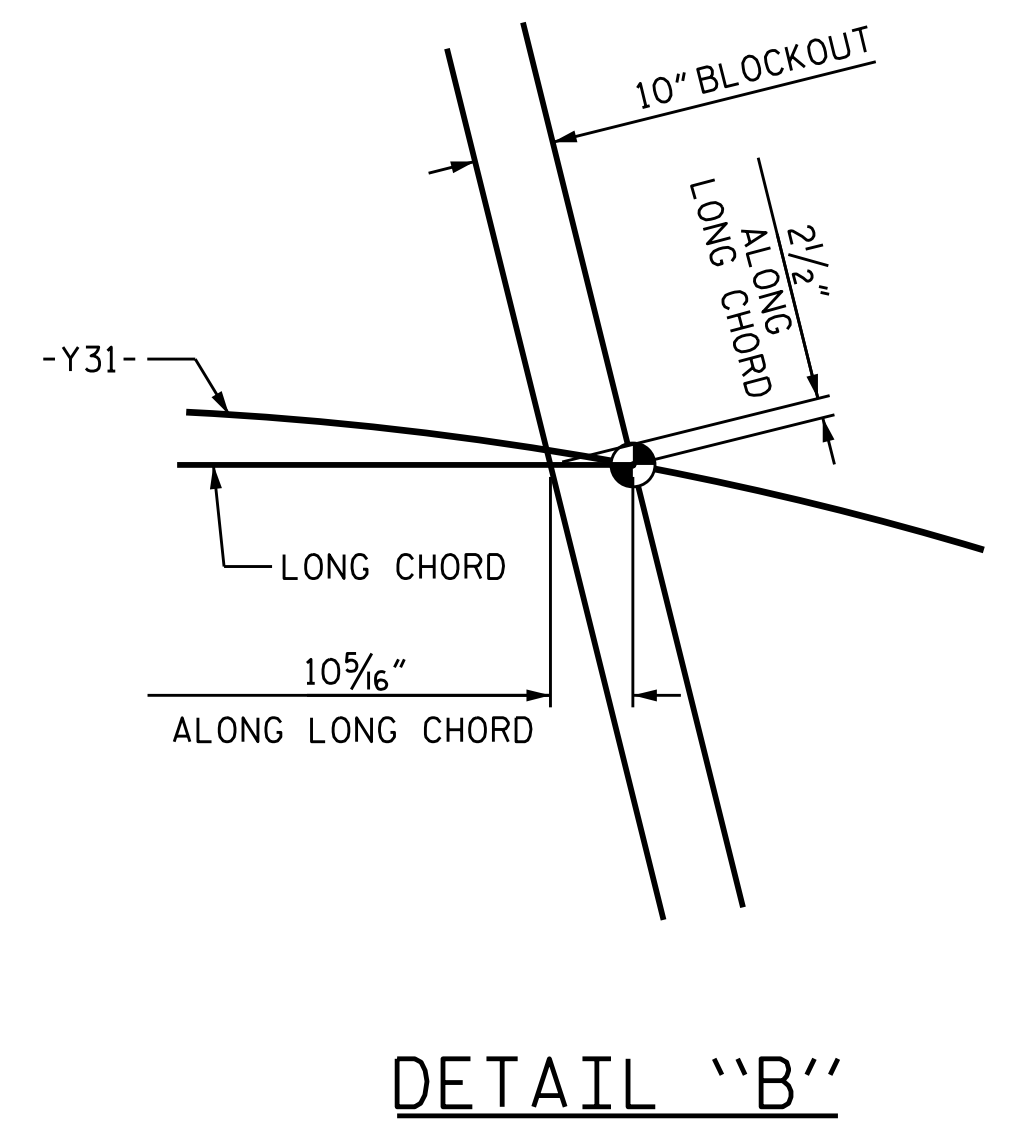
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**TRANSVERSE CONSTRUCTION JOINT  
IN DECK SLAB**

NOTE: REINFORCING STEEL IN DECK SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT.



**DETAIL "B"**

TRANSVERSE CONST. JT. SEE "BILL OF MATERIALS" SHEET FOR LOCATIONS AND DETAILS.

FOR REINFORCEMENT IN INTERIOR BENT DIAPHRAGMS, SEE "SUPERSTRUCTURE PLAN OF SPAN DETAILS - DIAPHRAGMS", SHEET 3 OF 3.

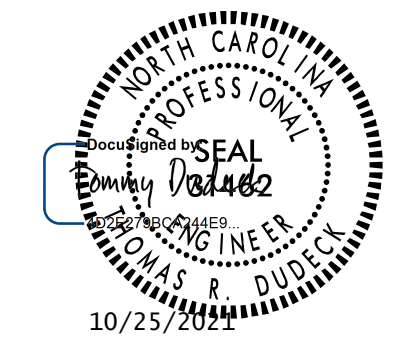
TRANSVERSE CONST. JT. SEE "BILL OF MATERIALS" SHEET FOR LOCATIONS AND DETAILS.

FOR REINFORCEMENT IN END BENT DIAPHRAGM, SEE "SUPERSTRUCTURE PLAN OF SPAN DETAILS - DIAPHRAGMS", SHEET 3 OF 3.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
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 SUPERSTRUCTURE  
 PLAN OF SPAN  
 (SPAN B)



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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-10
1			3			TOTAL SHEETS
2			4			38

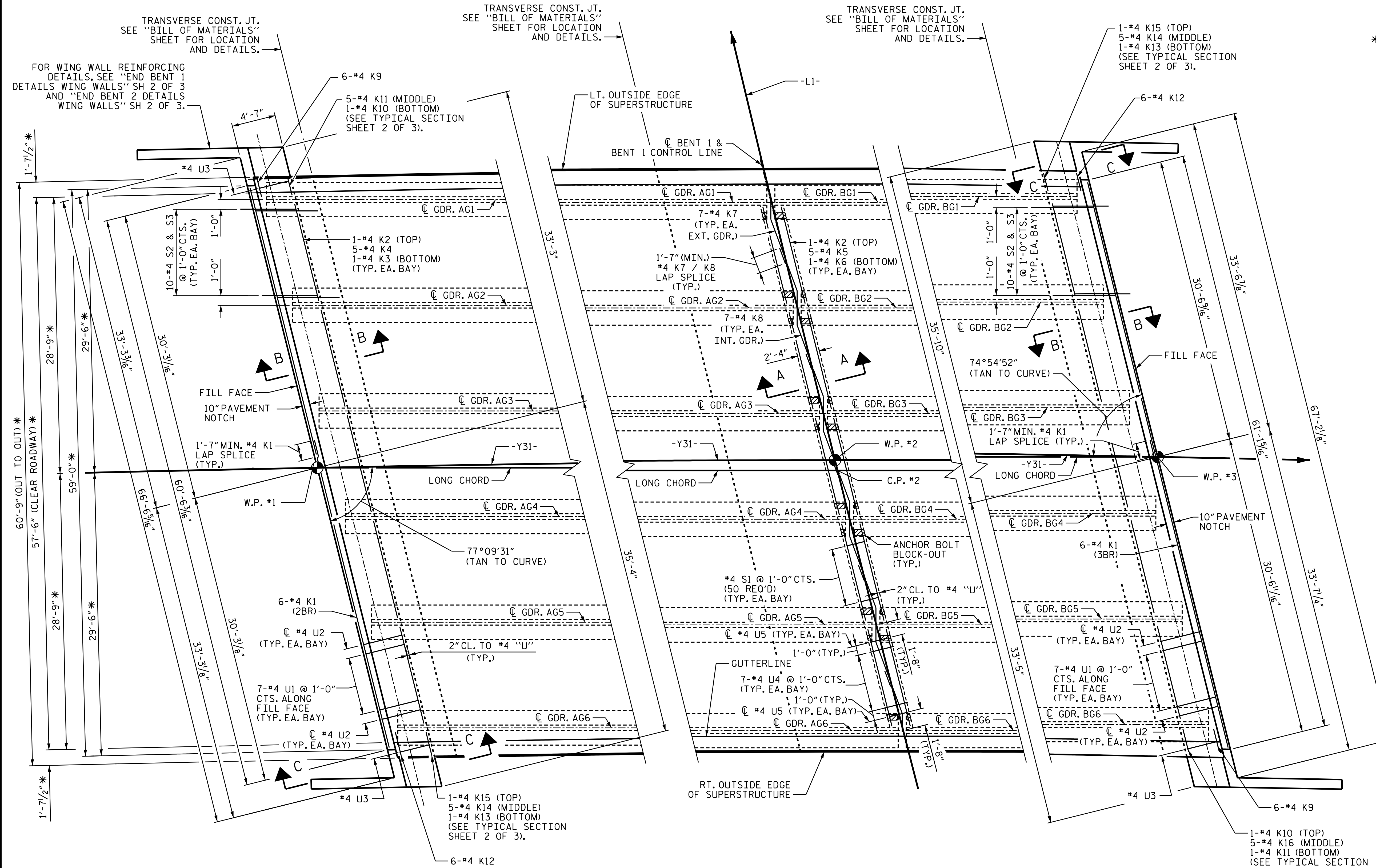
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\* - DENOTES RADIAL DIMENSION  
 \*\* - DENOTES TWO BARS / MARK  
 \*\*\* - 2'-5" (MIN.) #5 A1 LAP SPLICE  
 2'-0" (MIN.) #5 A2 LAP SPLICE

**PLAN OF SPAN  
SPAN B**

NOTE: (2BR) DENOTES TWO BAR RUN  
 (4BR) DENOTES FOUR BAR RUN

DRAWN BY: J. E. HAGENBUSH DATE: 12/20/17  
 CHECKED BY: R. DeCOLA DATE: 03/15/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21



**NOTES**

- \* - DENOTES RADIAL DIMENSION
- REINFORCEMENT IN DECK AND BARRIER RAIL NOT SHOWN FOR CLARITY.
- FOR SECTION A-A, SECTION B-B & SECTION C-C, SEE "TYPICAL SECTION (DETAILS)" SHEET 3 OF 3.
- (3BR) DENOTES THREE BAR RUN
- CONCRETE ISLAND NOT SHOWN FOR CLARITY.

**END BENT 1  
DIAPHRAGM REINFORCING  
DETAIL**

**BENT DIAPHRAGM  
REINFORCING  
DETAIL**

**END BENT 2  
DIAPHRAGM REINFORCING  
DETAIL**

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 3 OF 3

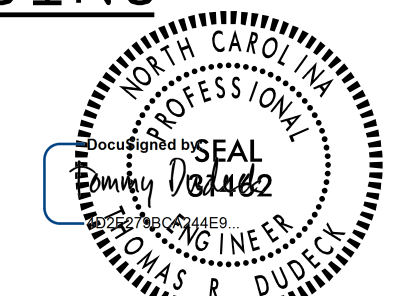
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**PLAN OF SPANS  
 DETAILS - DIAPHRAGMS**



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DRAWN BY: J. E. HAGENBUSH DATE: 01/05/18  
 CHECKED BY: R. DeCOLA DATE: 03/28/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21



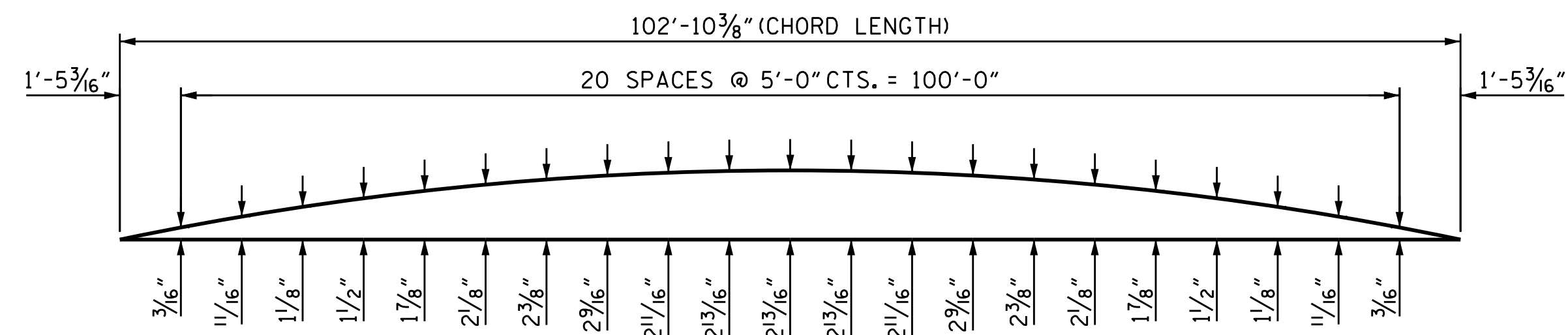
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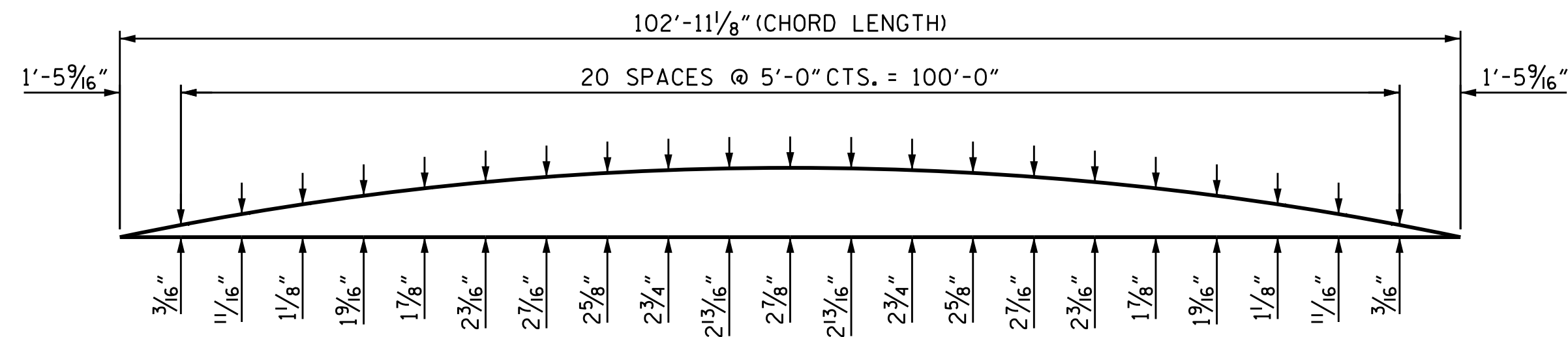
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NO.	BY:	DATE:	NO.	BY:	DATE:	S16-11
1			3			TOTAL SHEETS
2			4			38



NOTE:  
ARC OFFSETS ARE TAKEN ALONG THE OUTSIDE EDGE OF SUPERSTRUCTURE.

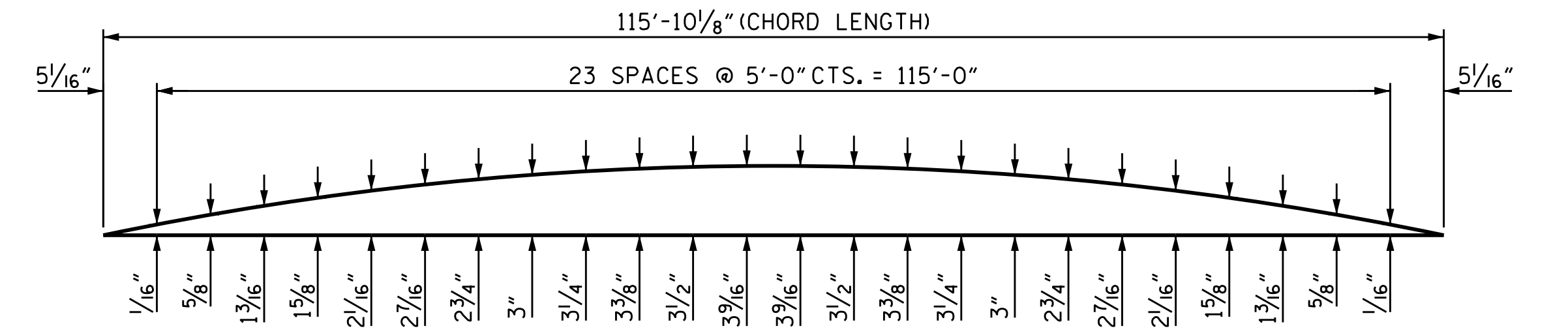


LEFT SIDE

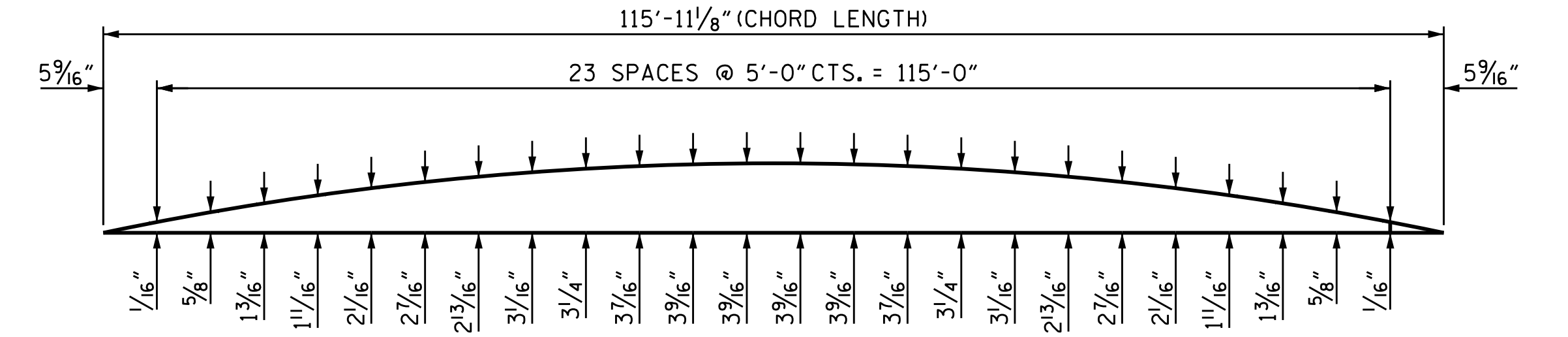


RIGHT SIDE

ARC OFFSETS- SPAN A



LEFT SIDE



RIGHT SIDE

ARC OFFSETS- SPAN B

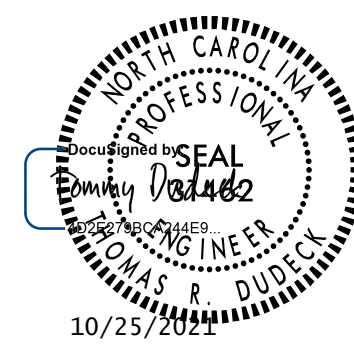
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PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 1 OF 1

STATE OF NORTH CAROLINA  
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RALEIGH  
SUPERSTRUCTURE  
ARC OFFSETS



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CHECKED BY : R. DeCOLA DATE : 01/23/18  
DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE : 10/25/21

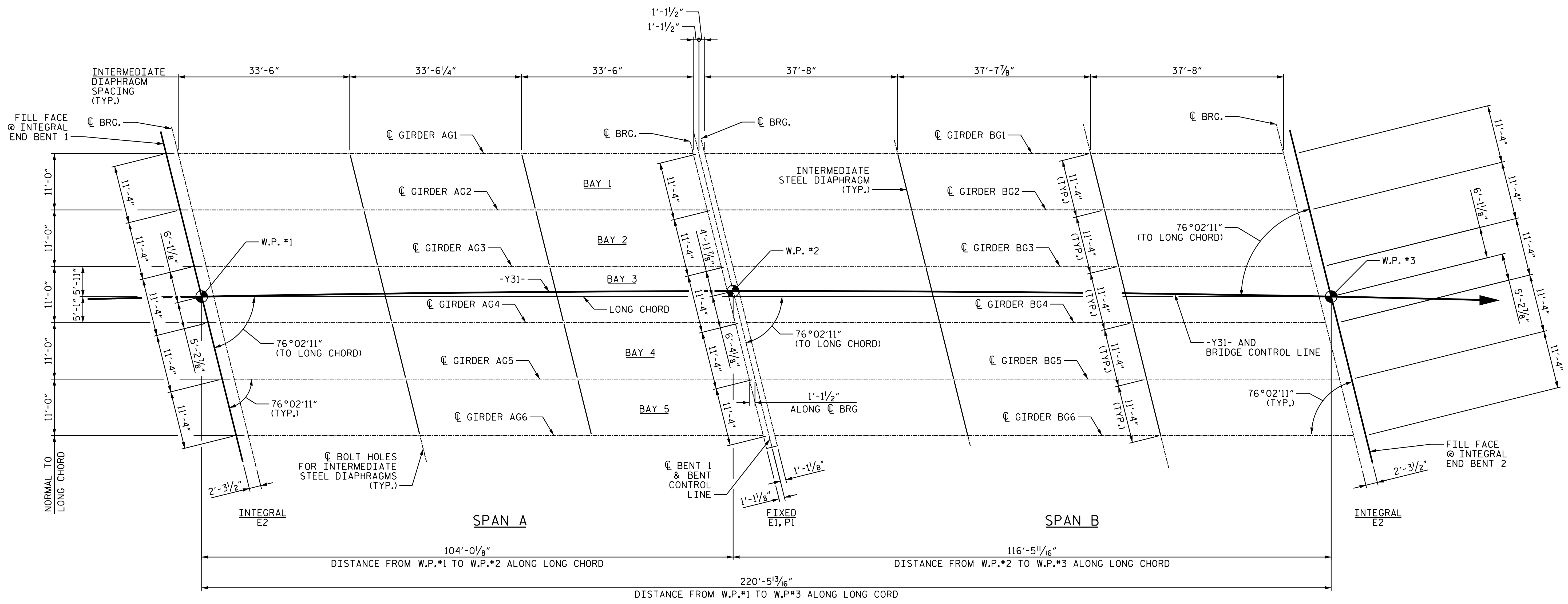
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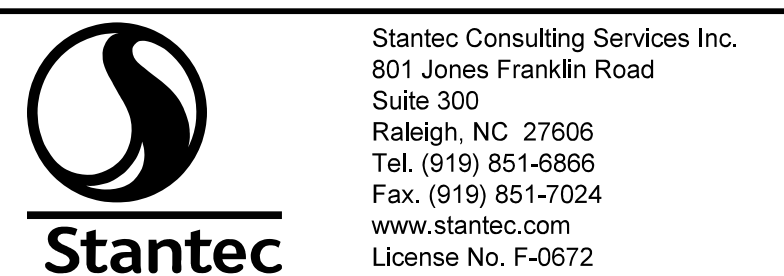
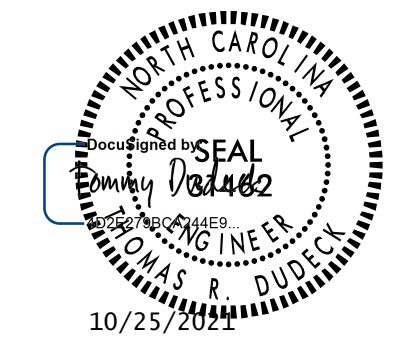


### FRAMING PLAN

PROJECT NO. R-3300B  
         PENDER          COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 1

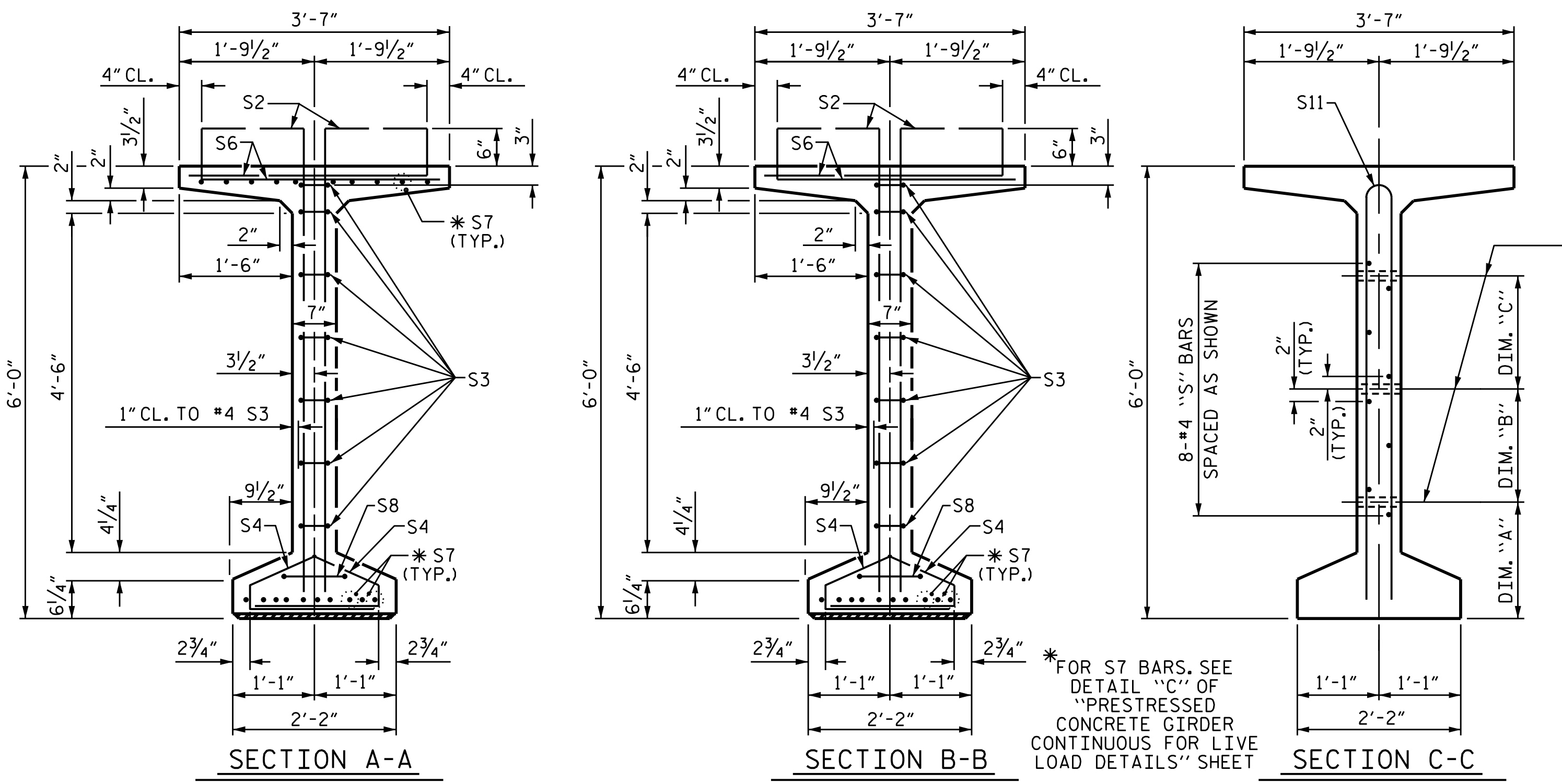
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 FRAMING PLAN



DRAWN BY : J. E. HAGENBUSH DATE : 12/05/17  
 CHECKED BY : R. DeCOLA DATE : 01/03/18  
 DESIGN ENGINEER OF RECORD : T. R. DUDECK DATE : 10/25/21

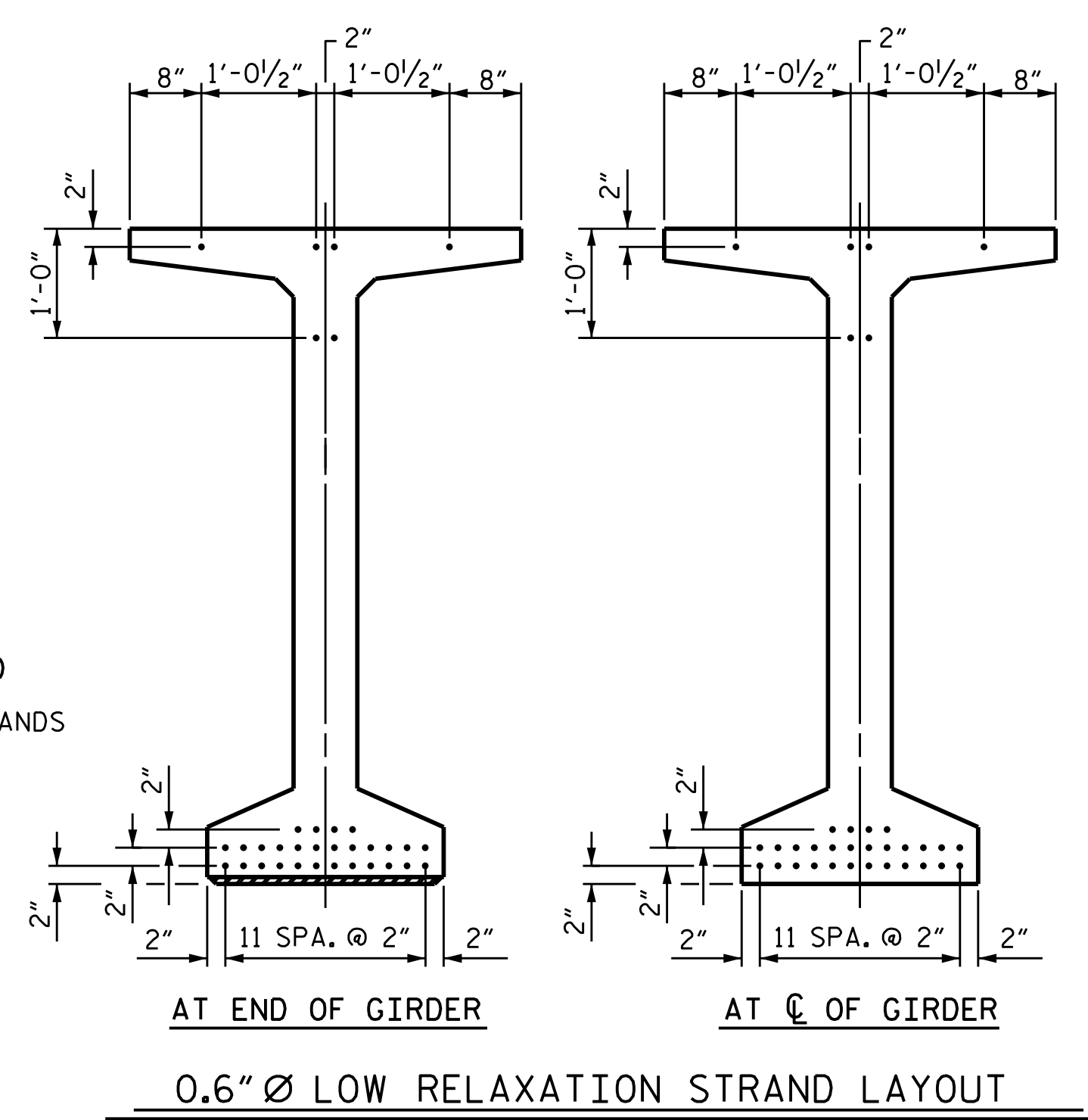
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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-13
1			3			TOTAL SHEETS
2			4			38



1/2"  $\phi$  FORMED HOLE. SEE ELEVATION FOR LOCATION. FOR DIM. "A", "B" & "C" SEE "INTERMEDIATE STEEL DIAPHRAGMS" SHEET.

DEBONDING LEGEND  
● FULLY BONDED STRANDS



0.6"  $\phi$  L. R. GRADE 270 STRANDS

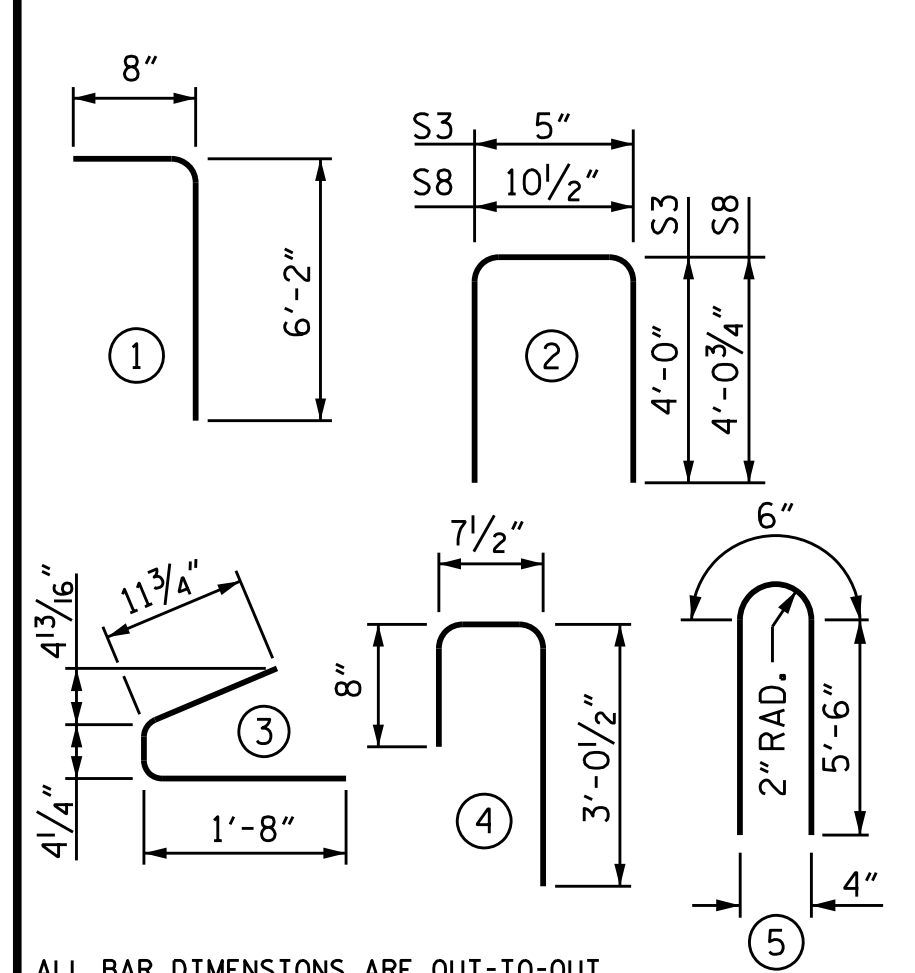
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	210	#5	1	6'-10"	1387
S2	24	#6	1	6'-10"	246
S3	14	#4	2	8'-5"	79
S4	144	#4	3	3'-0"	289
S6	234	#5	4	4'-4"	1058
*S7	30	#5	STR	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	22	#5	STR	3'-3"	75
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	11'-6"	96
S12	16	#4	STR	8'-0"	86

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

BAR TYPES



ALL BAR DIMENSIONS ARE OUT-TO-OUT

QUANTITIES FOR ONE GIRDER

REINFORCING STEEL	8,000 PSI CONCRETE		0.6" $\phi$ L. R. STRANDS
	LB.	C.Y.	
3451		21.8	32

GIRDERS REQUIRED

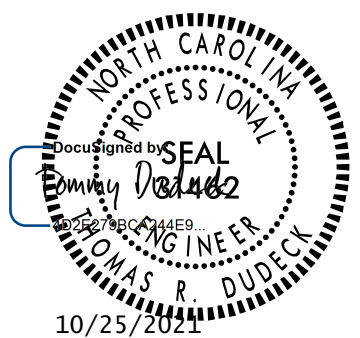
NUMBER	LENGTH	TOTAL LENGTH
6	101'-11 1/4"	611'-7 1/2"

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

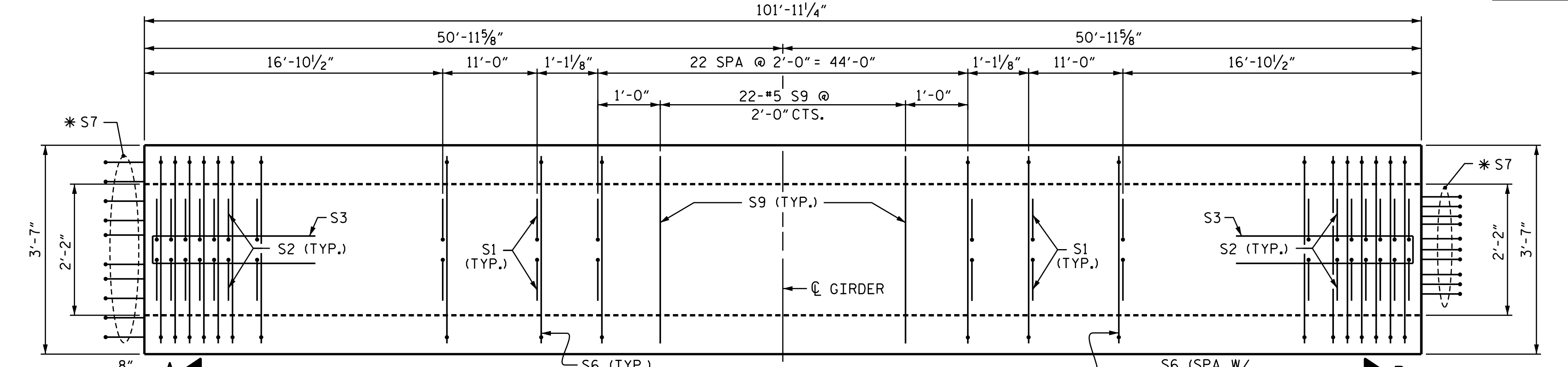
SHEET 1 OF 4

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
  
STANDARD  
72" PRESTRESSED CONCRETE  
MODIFIED BULB TEE  
CONTINUOUS FOR LIVE LOAD  
SPAN A

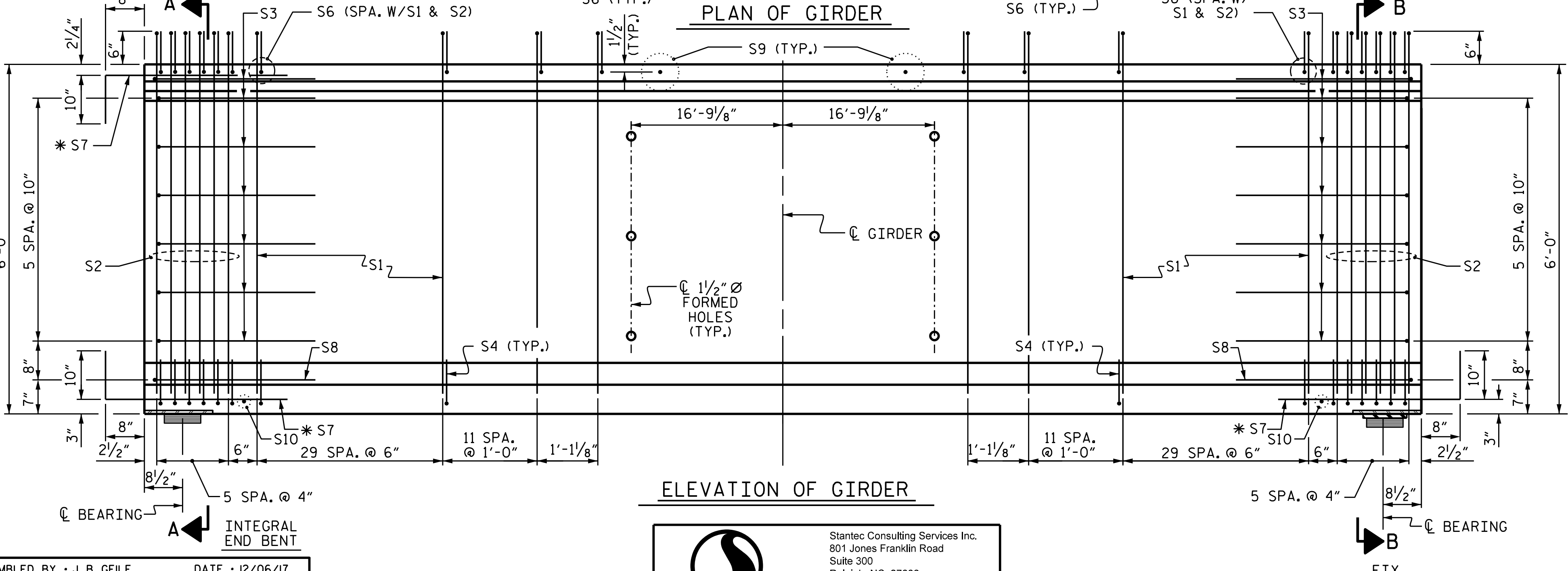
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	BY:	DATE:	NO.	BY:	DATE:		
1			3				
2			4				



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PLAN OF GIRDER



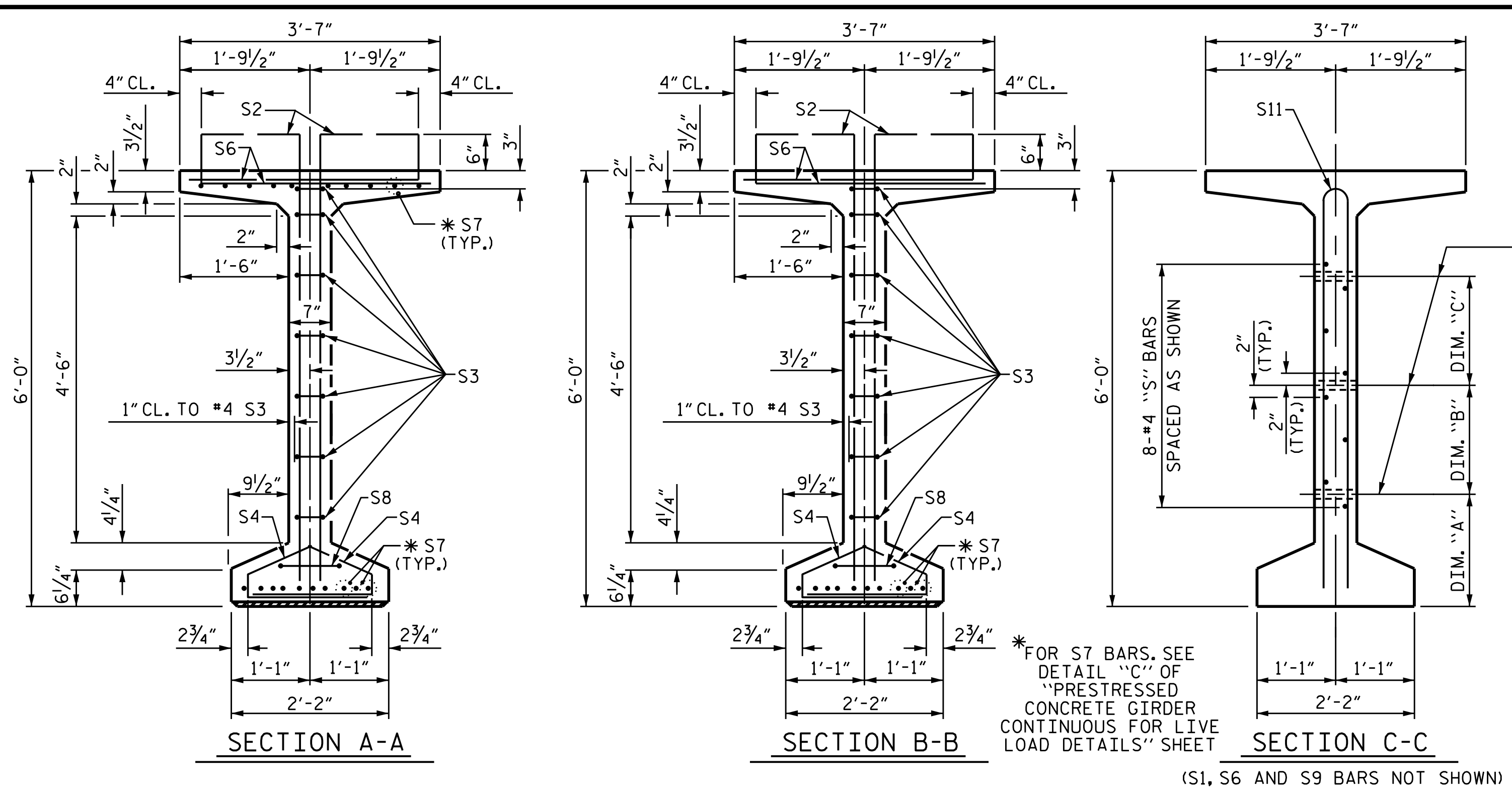
ELEVATION OF GIRDER

SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. A1-A6

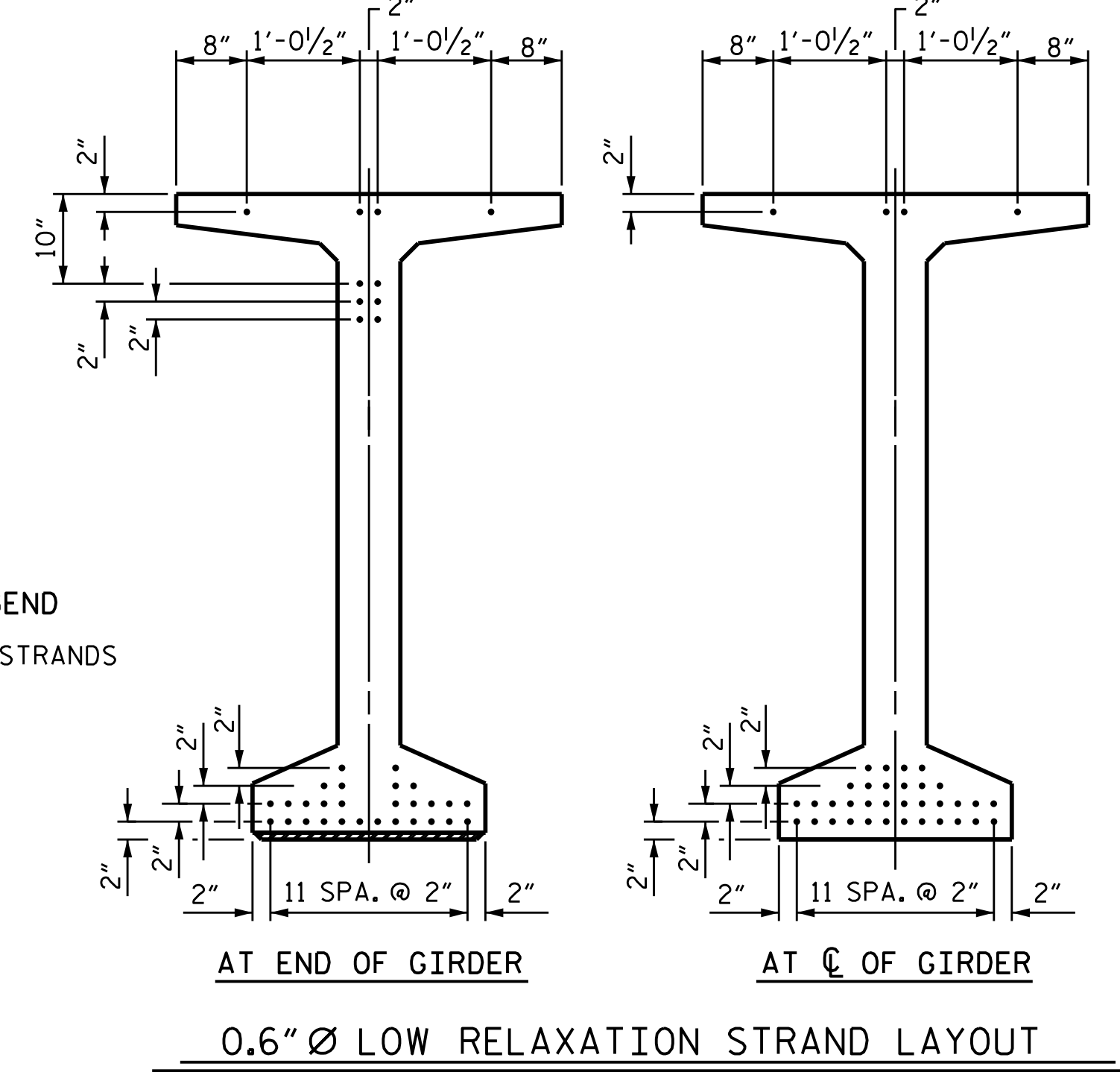
PARTIAL ELEVATION

ASSEMBLED BY : J. B. GEILE DATE : 12/06/17  
CHECKED BY : R.F. DECOLA DATE : 01/05/18  
DRAWN BY : EEM 2/6/97 REV. 6/13 MAA/GM DESIGN  
CHECKED BY : VAP 2/6/97 REV. 12/17 MAA/TMG ENGINEER  
OF RECORD: T. R. DUDECK DATE : 10/25/21

10/25/21



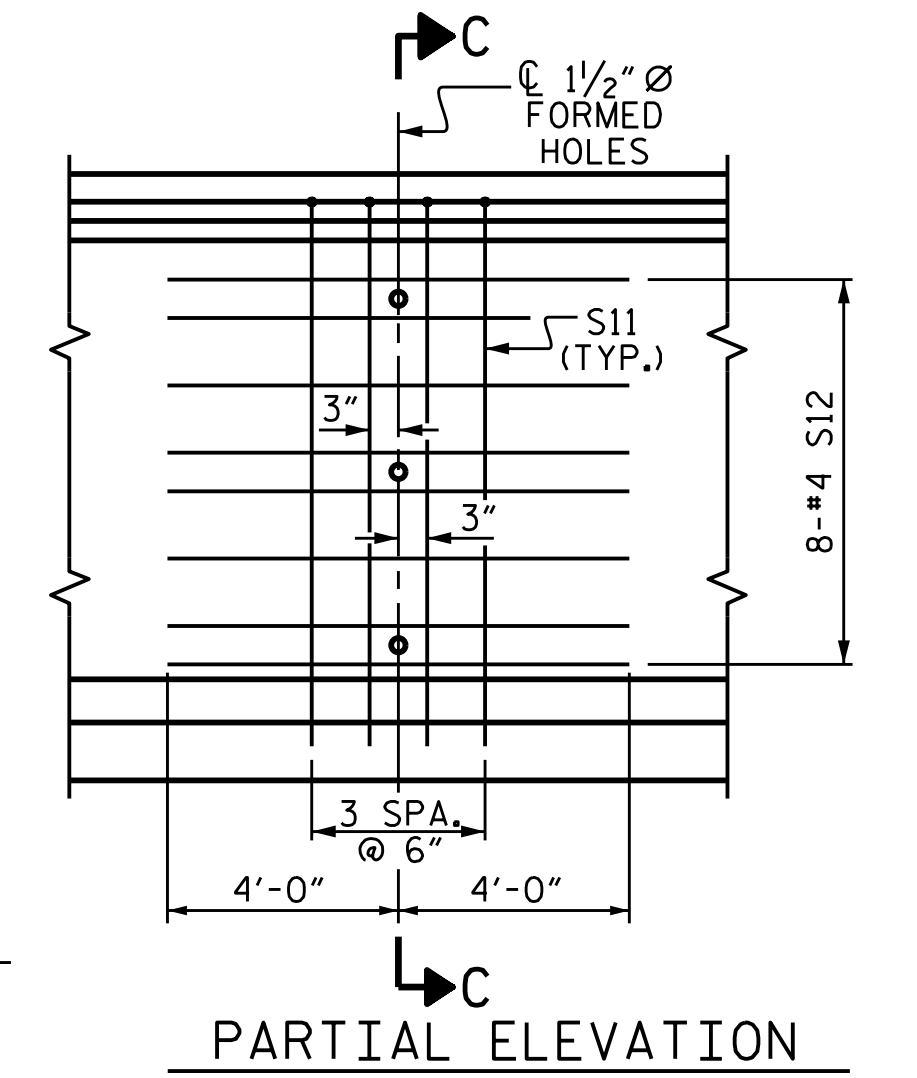
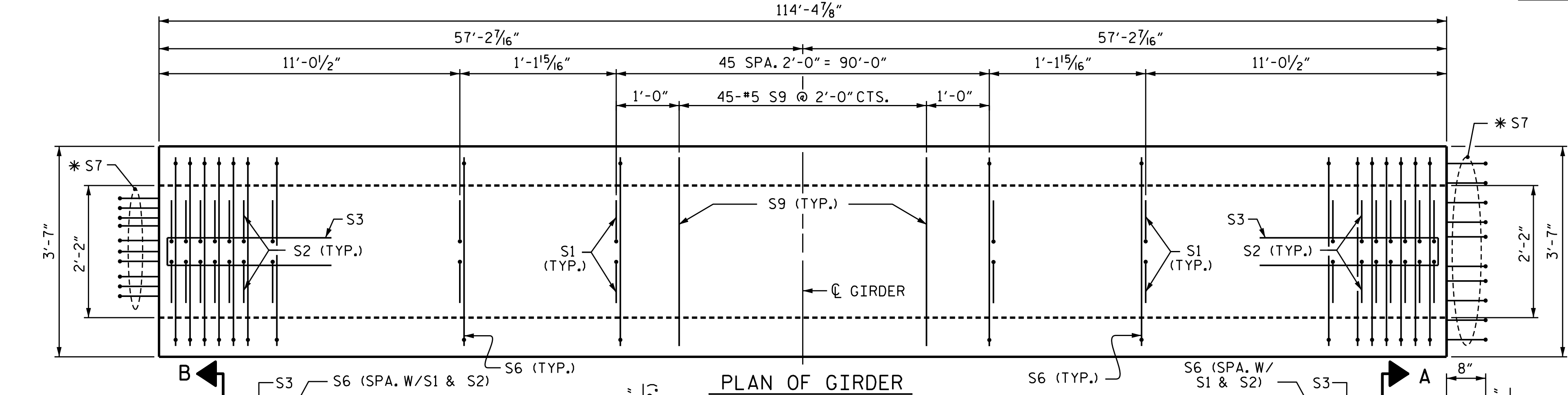
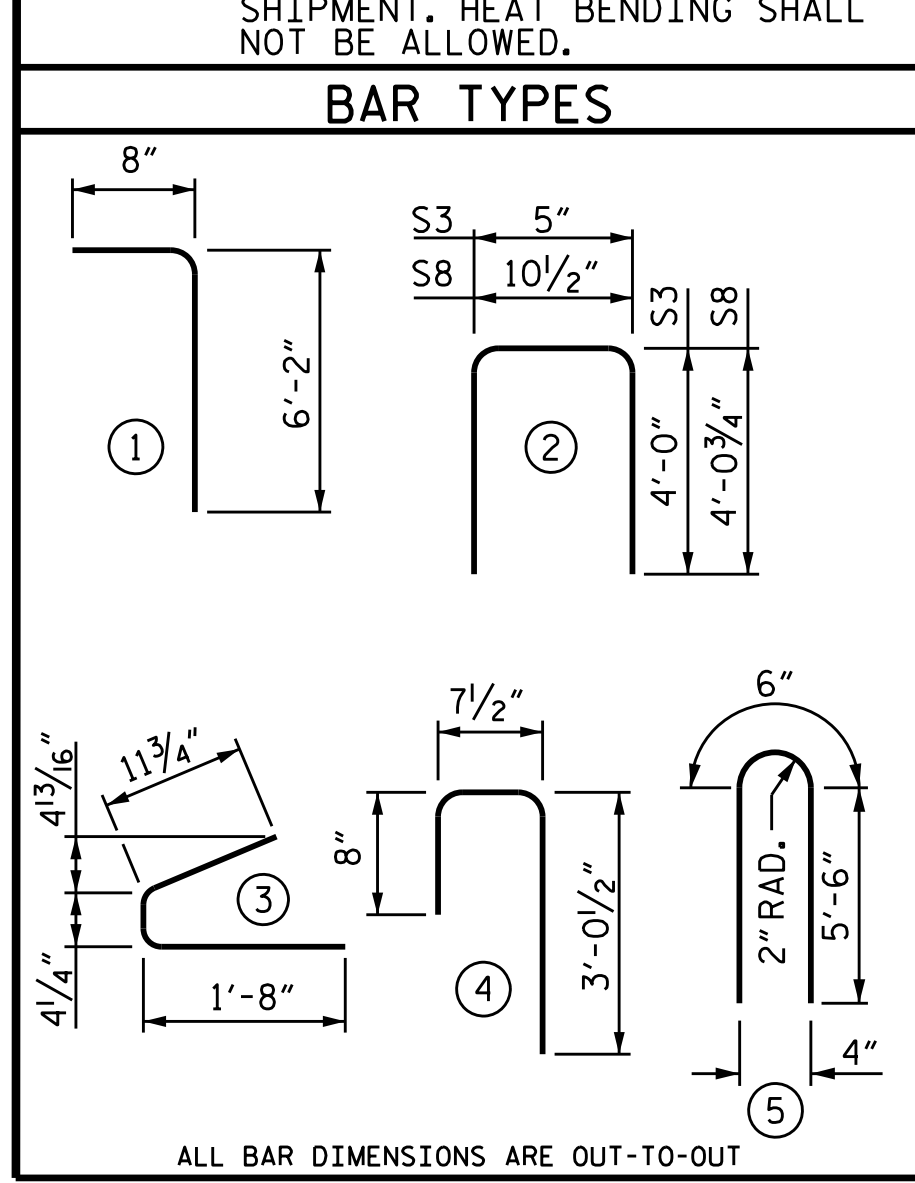
DEBONDING LEGEND  
 • FULLY BONDED STRANDS



0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR						
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	
S1	180	#5	1	6'-10"	1283	
S2	24	#6	1	6'-10"	246	
S3	14	#4	2	8'-5"	79	
S4	112	#4	3	3'-0"	224	
S6	204	#5	4	4'-4"	922	
*S7	30	#5	STR	3'-8"	115	
S8	2	#5	2	9'-0"	19	
S9	45	#5	STR	3'-3"	153	
S10	2	#3	STR	1'-10"	1	
S11	8	#5	5	11'-6"	96	
S12	16	#4	STR	8'-0"	86	

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



QUANTITIES FOR ONE GIRDER			
REINFORCING STEEL	8,500 PSI CONCRETE		0.6" Ø L. R. STRANDS
	LB.	C.Y.	No.
	3224	24.5	38

GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
6	114'-4 7/8"	686'-5 1/4"

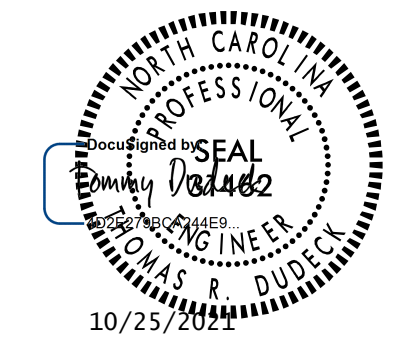
NOTE: THE UPLIFT FORCE DUE TO DRAPED STRANDS IS 23 KIPS (KN)

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 4

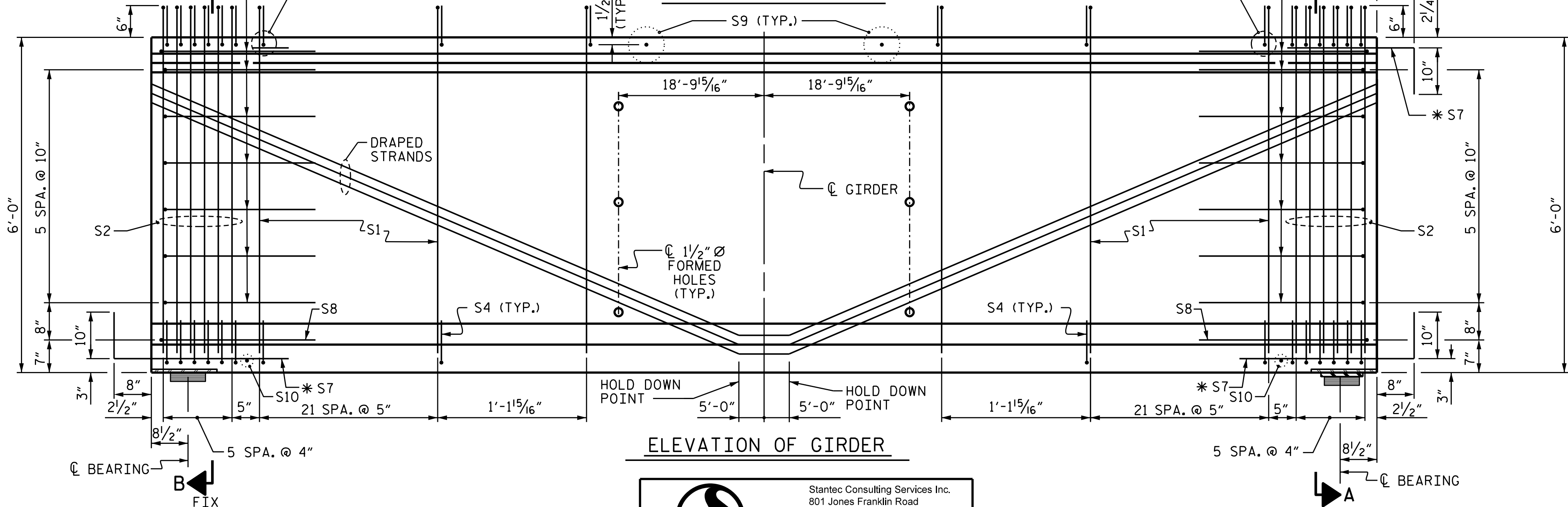
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD  
 72" PRESTRESSED CONCRETE  
 MODIFIED BULB TEE  
 CONTINUOUS FOR LIVE LOAD  
 SPAN B



REVISIONS						SHEET NO. S16-15 TOTAL SHEETS 38
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			
2			4			

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ASSEMBLED BY: J. B. CEILE DATE: 12/7/17  
 CHECKED BY: R. DeCOLA DATE: 01/08/18

REV. 6/13 MAA/GM DATE: 12/7/17  
 REV. 1/15 MAA/TMG DATE: 01/08/18  
 REV. 12/17 MAA/THC

DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

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10/25/2021

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6,500 PSI SPAN A AND 7,000 PSI FOR SPAN B.

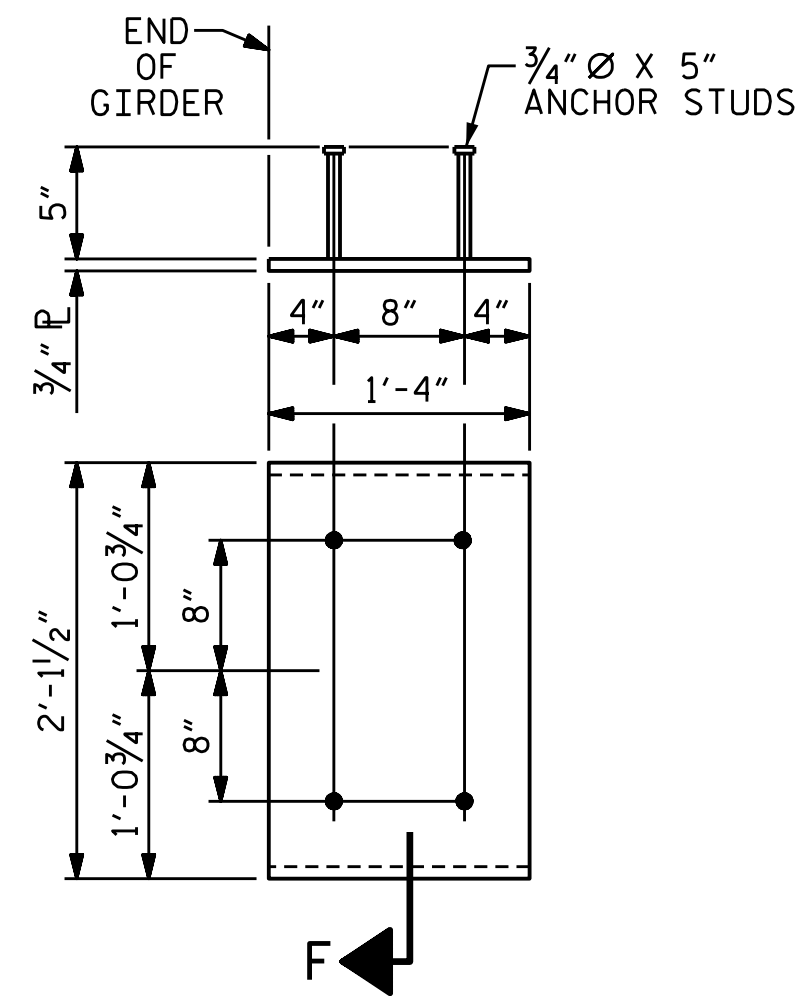
DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 72" MODIFIED BULB TEES ONLY.

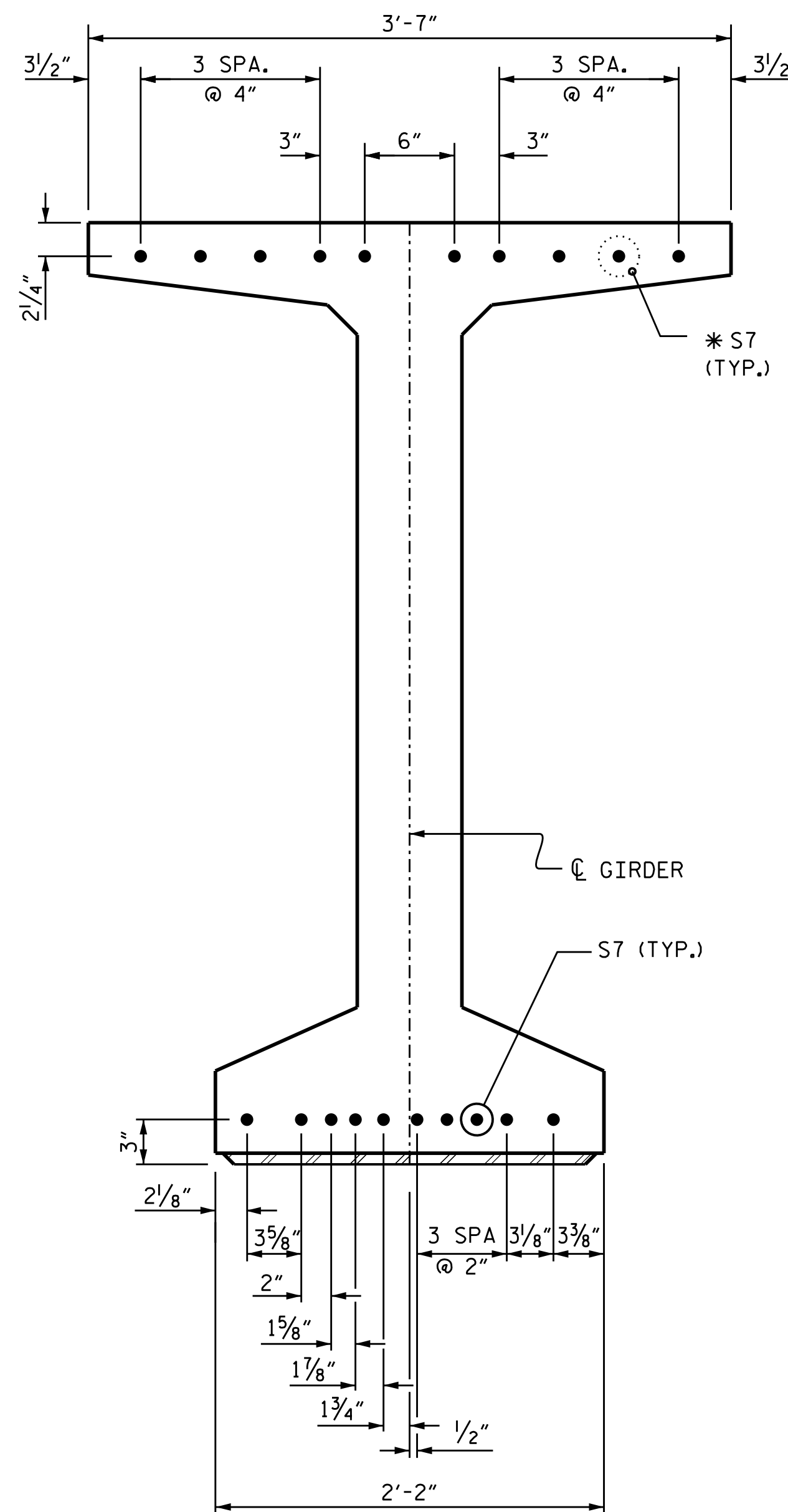
THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4500 lbs.

PRESTRESSED CONCRETE GIRDERS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR. SEE STANDARD SPECIFICATIONS FOR CALCIUM NITRITE CORROSION INHIBITOR.

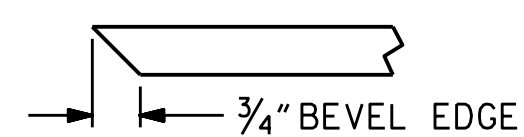


EMBEDDED PLATE "B-1" DETAILS FOR 72" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)



DETAIL "C"



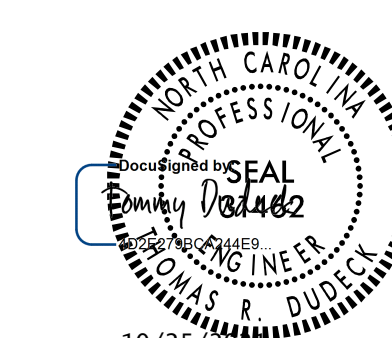
SECTION "F"

(SEE NOTES)

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 3 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 PRESTRESSED CONCRETE GIRDER  
 CONTINUOUS FOR LIVE LOAD  
 DETAILS



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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-16
1			3			TOTAL SHEETS
2			4			38



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ASSEMBLED BY : J. B. GEILE DATE : 12/07/17  
 CHECKED BY : R. DeCOLA DATE : 01/03/18

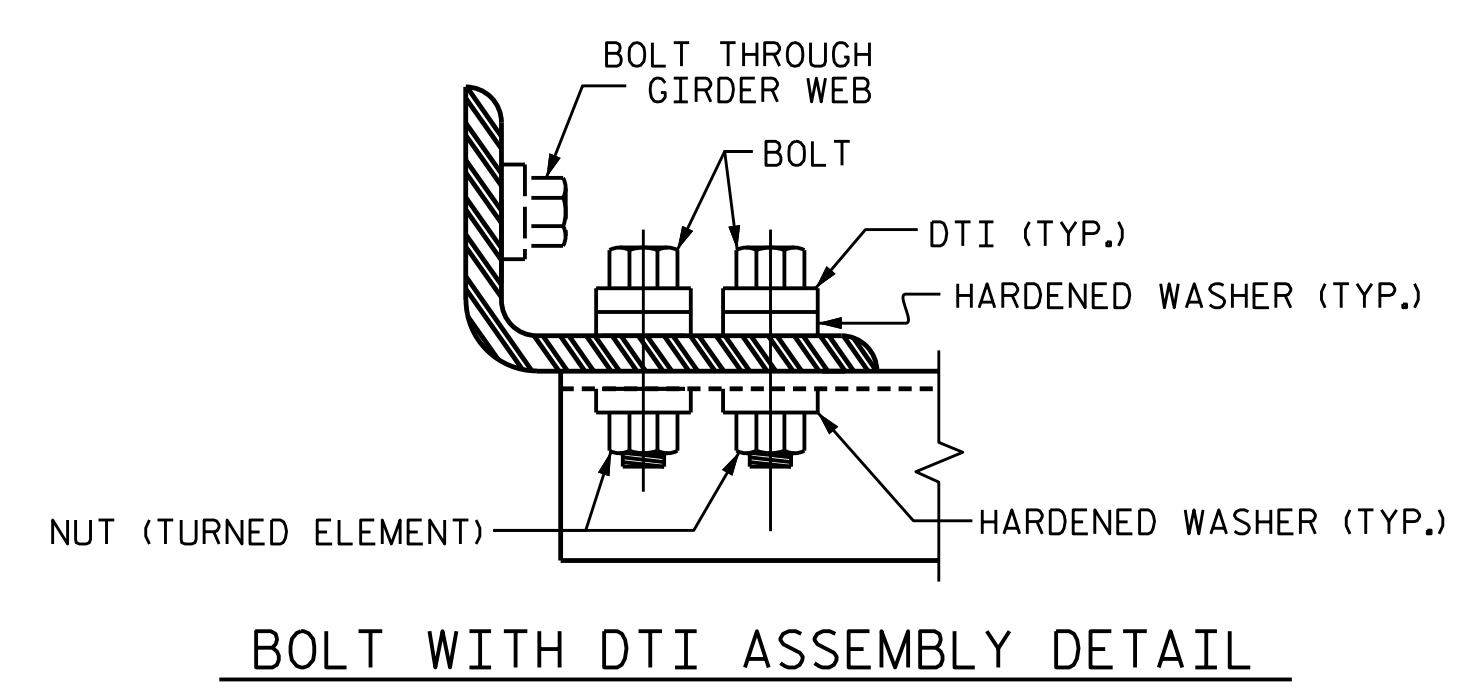
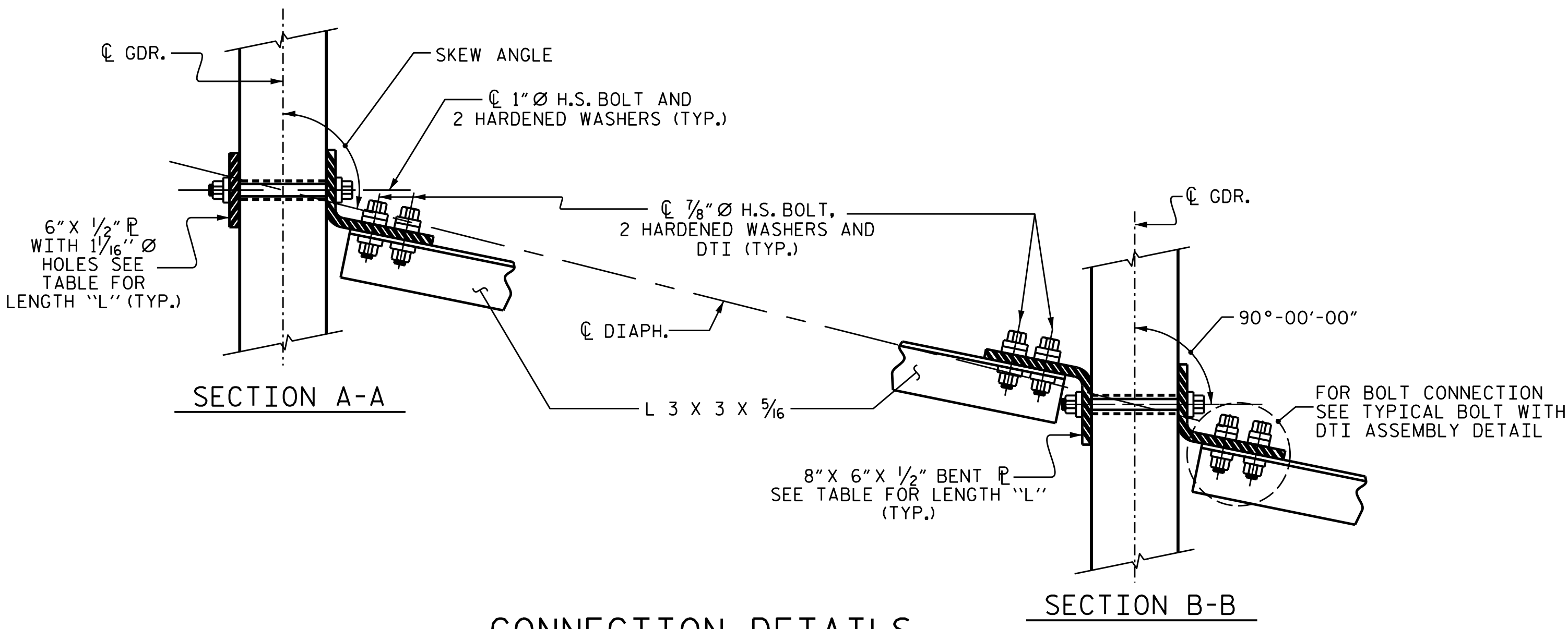
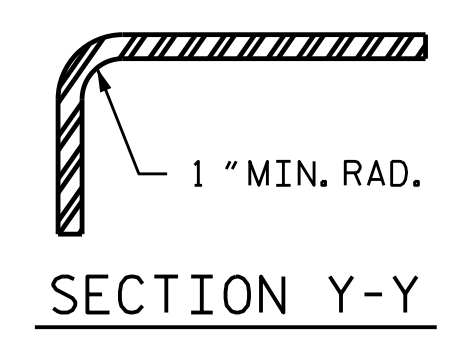
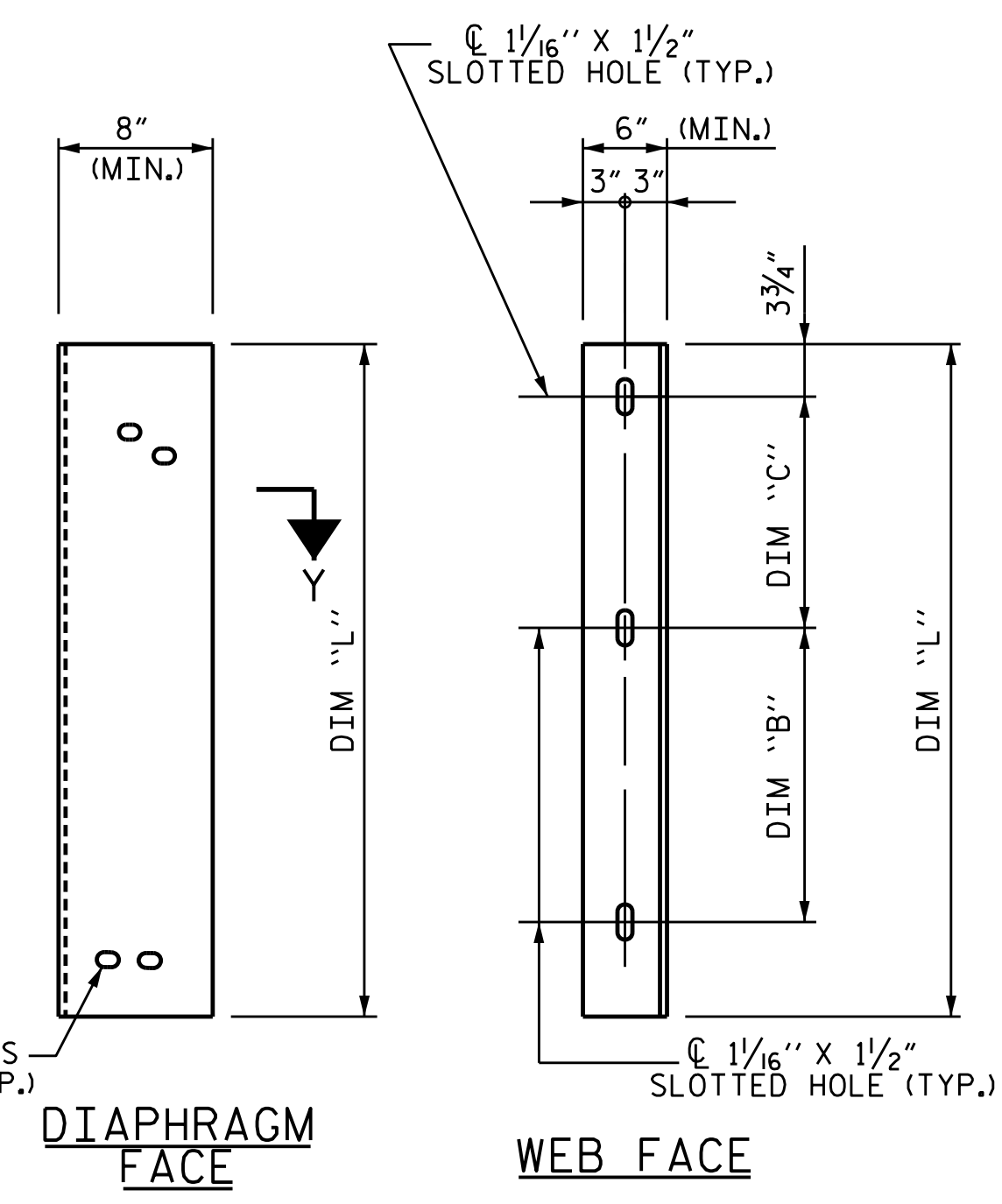
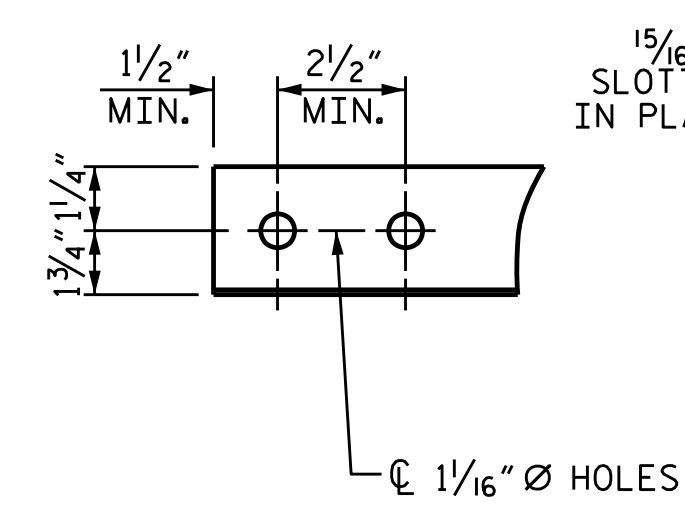
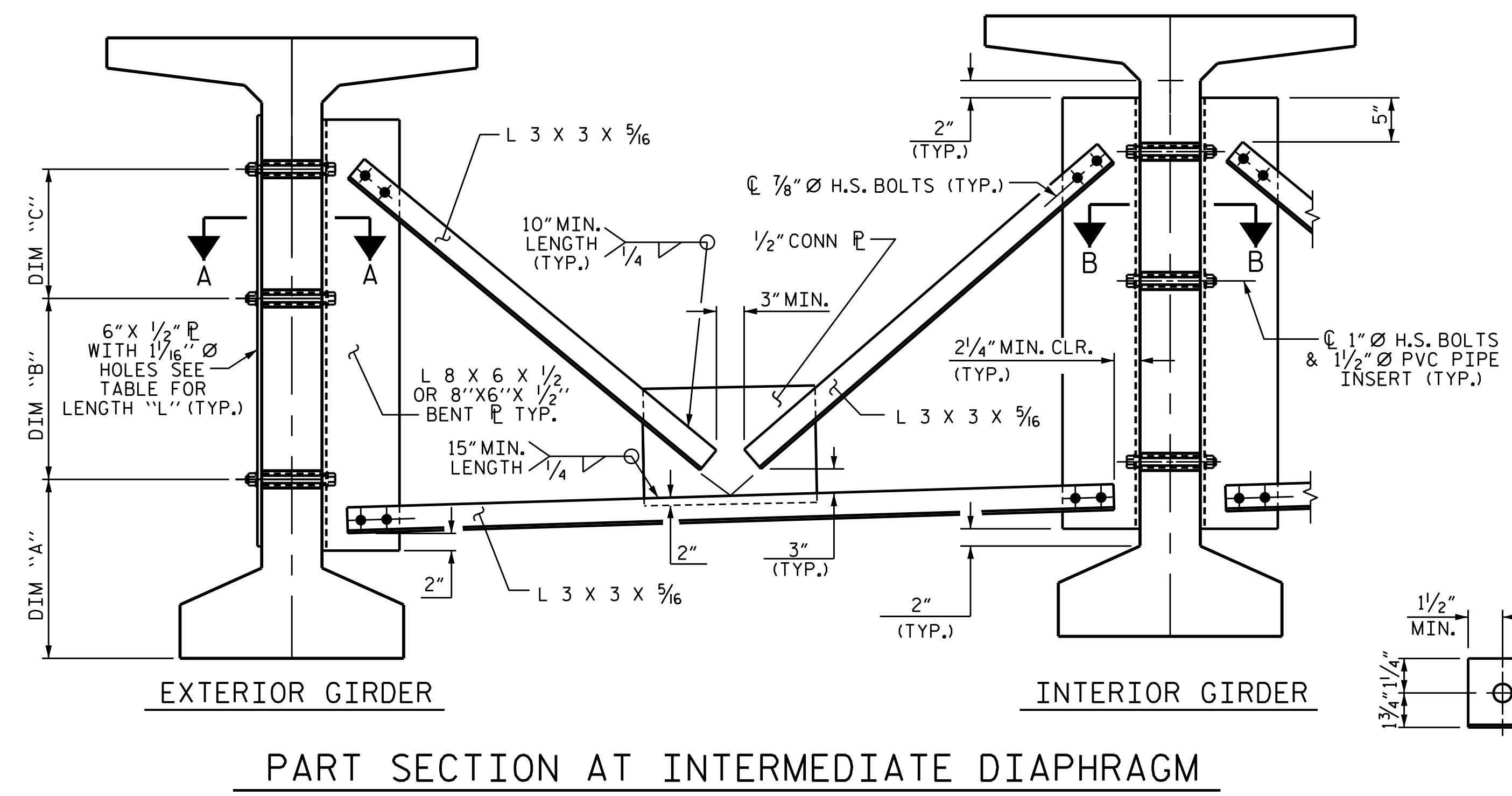
DRAWN BY : ELR 11/91  
 CHECKED BY : GRP 11/91

DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE : 10/25/21

REV. 1/15 MAA/TMG  
 REV. 2/15 MAA/TMG  
 REV. 12/17 MAA/THC

### STRUCTURAL STEEL NOTES

- ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.
- TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
- TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.
- THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.
- FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM. THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.
- GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
- USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.
- FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.
- INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.
- SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.
- IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.
- THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



### TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
72" BULB TEE	1'-3"	2'-0"	1'-5 3/4"	4'-2"

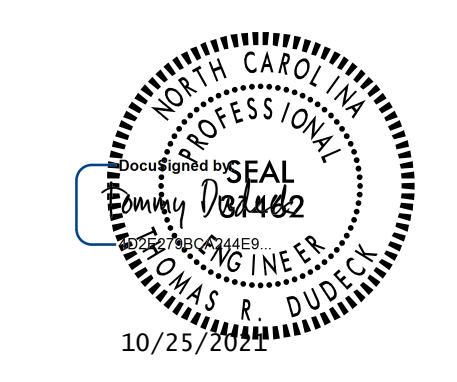
PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 4 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 72" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS**

REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		



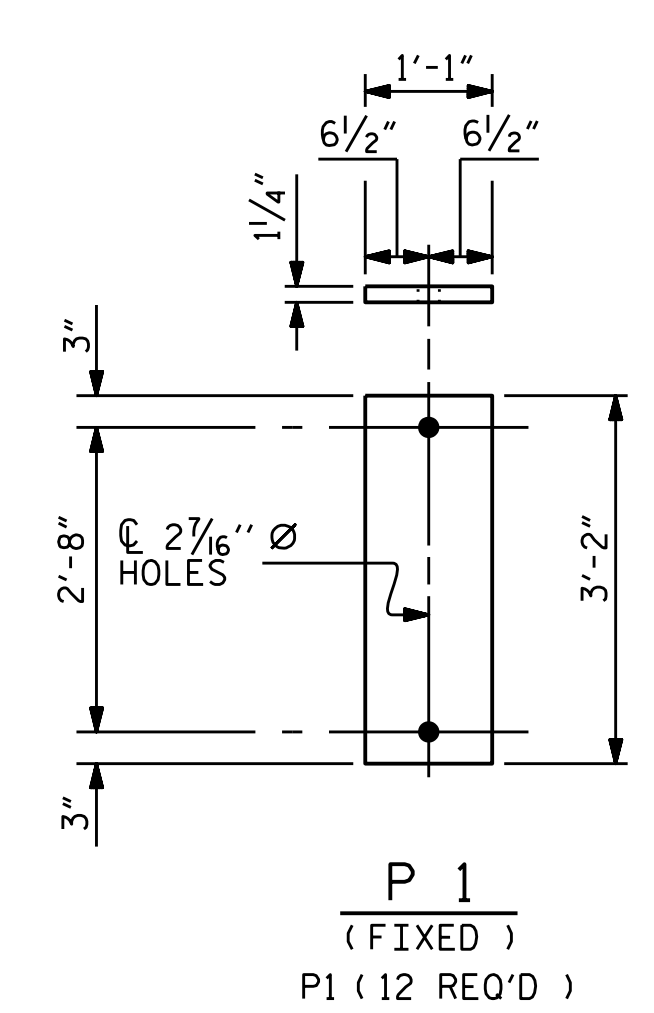
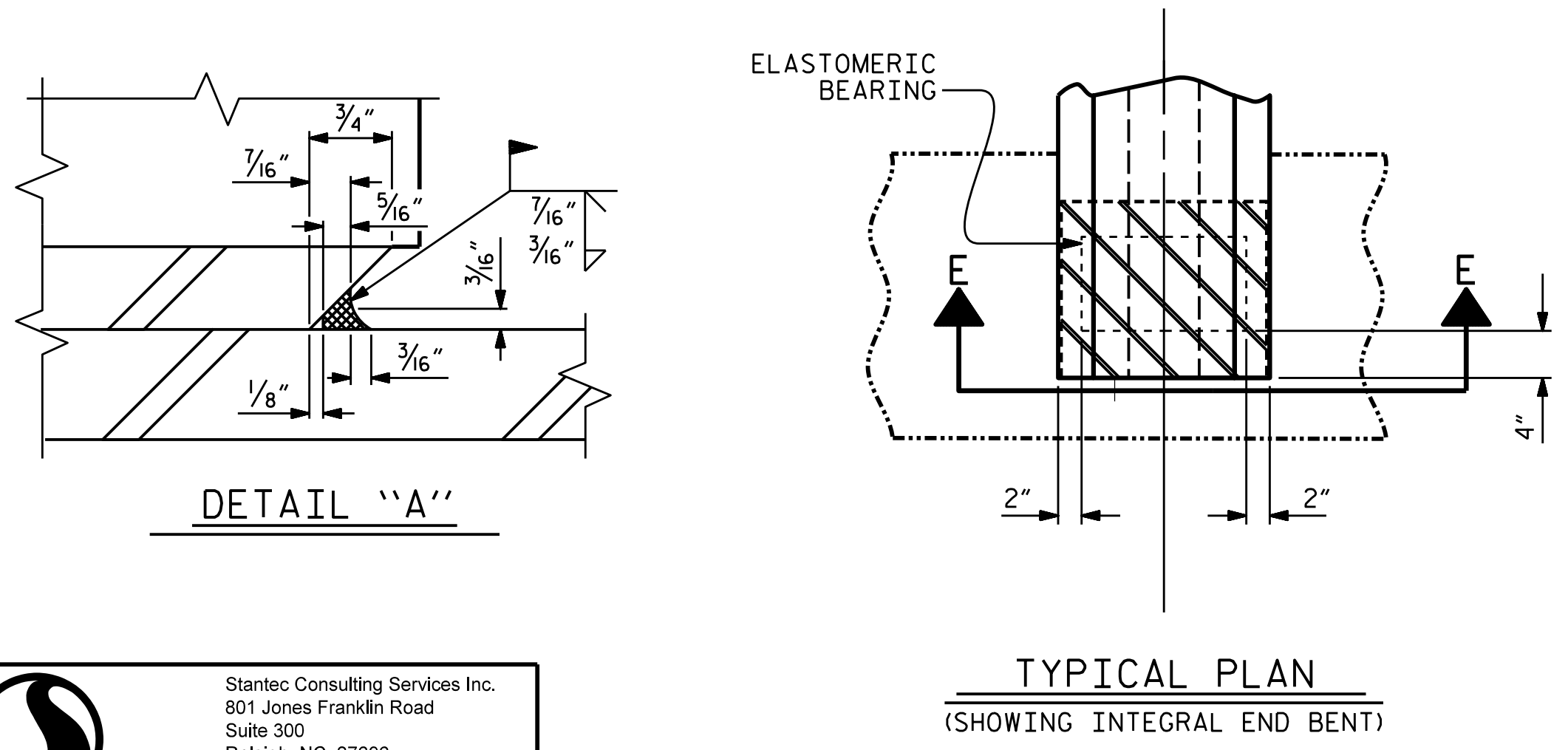
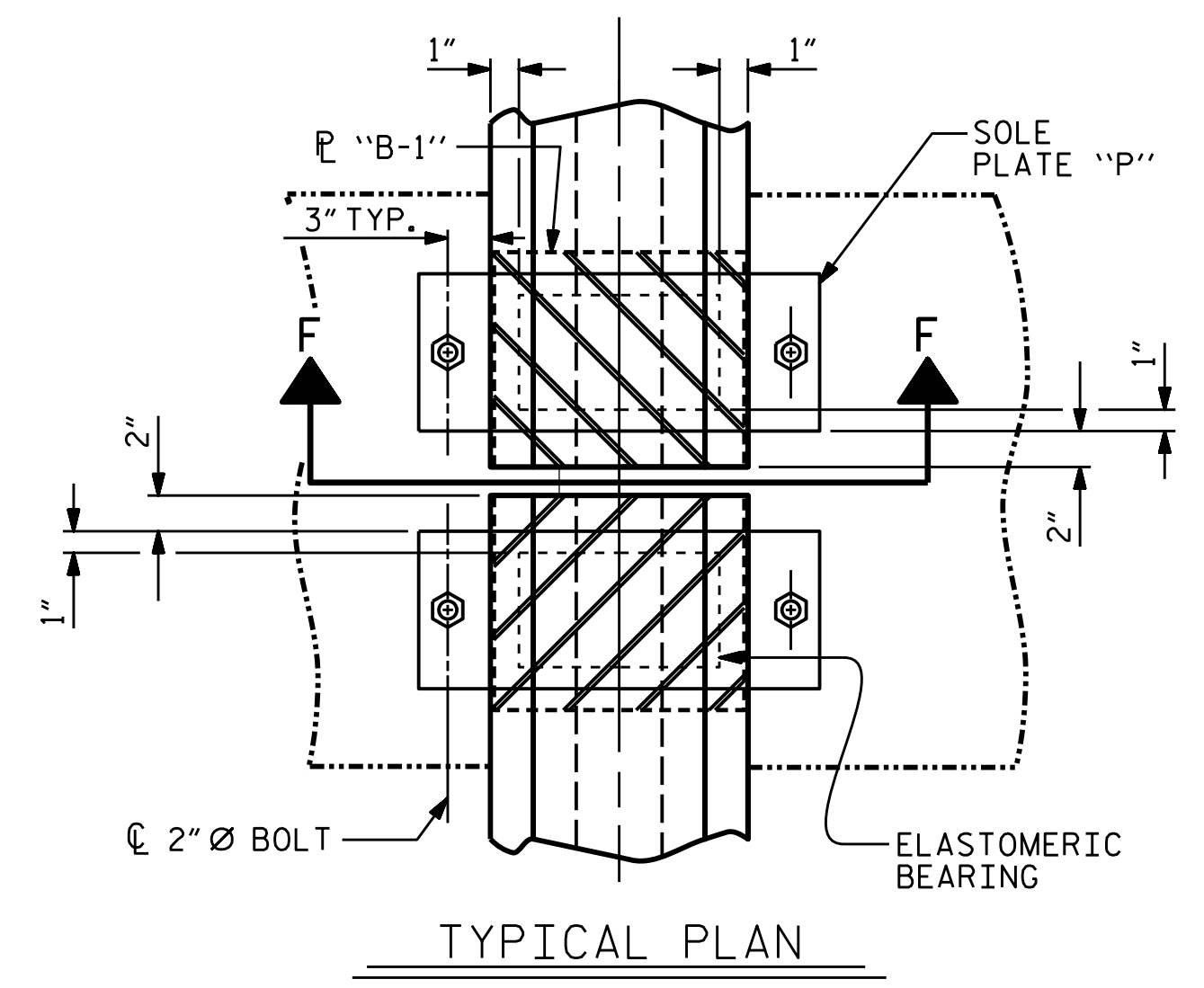
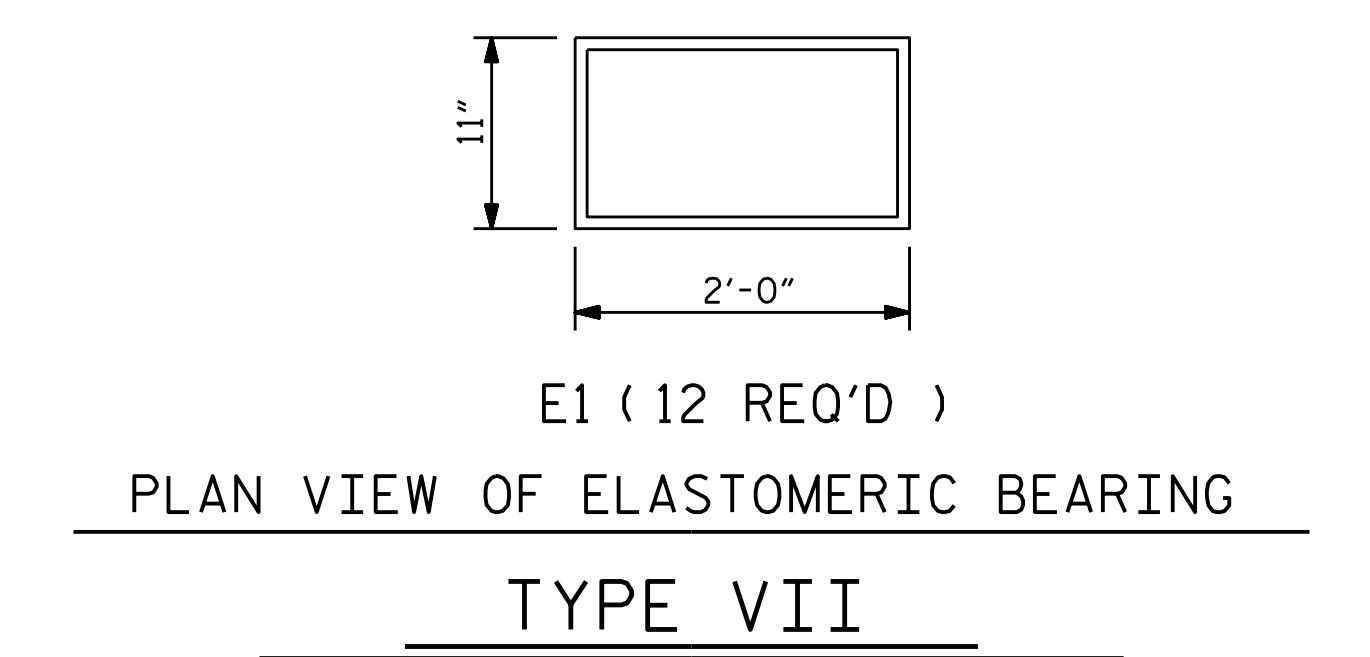
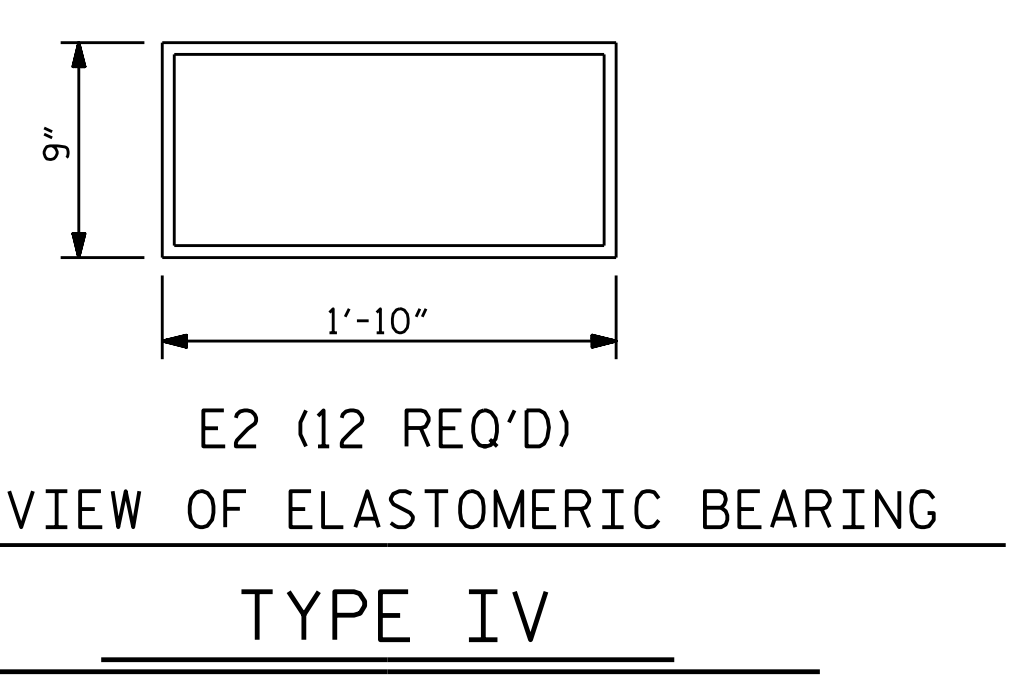
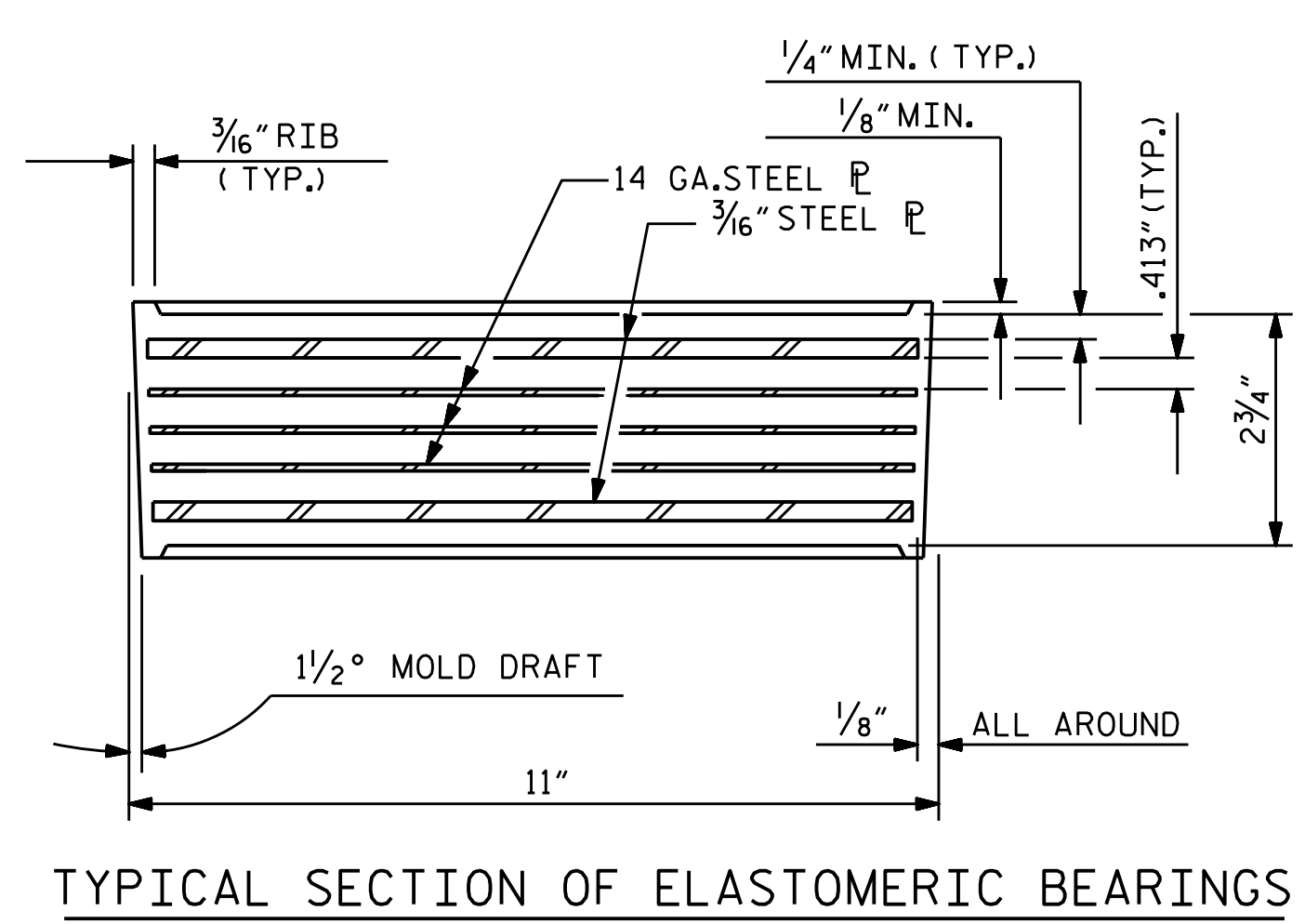
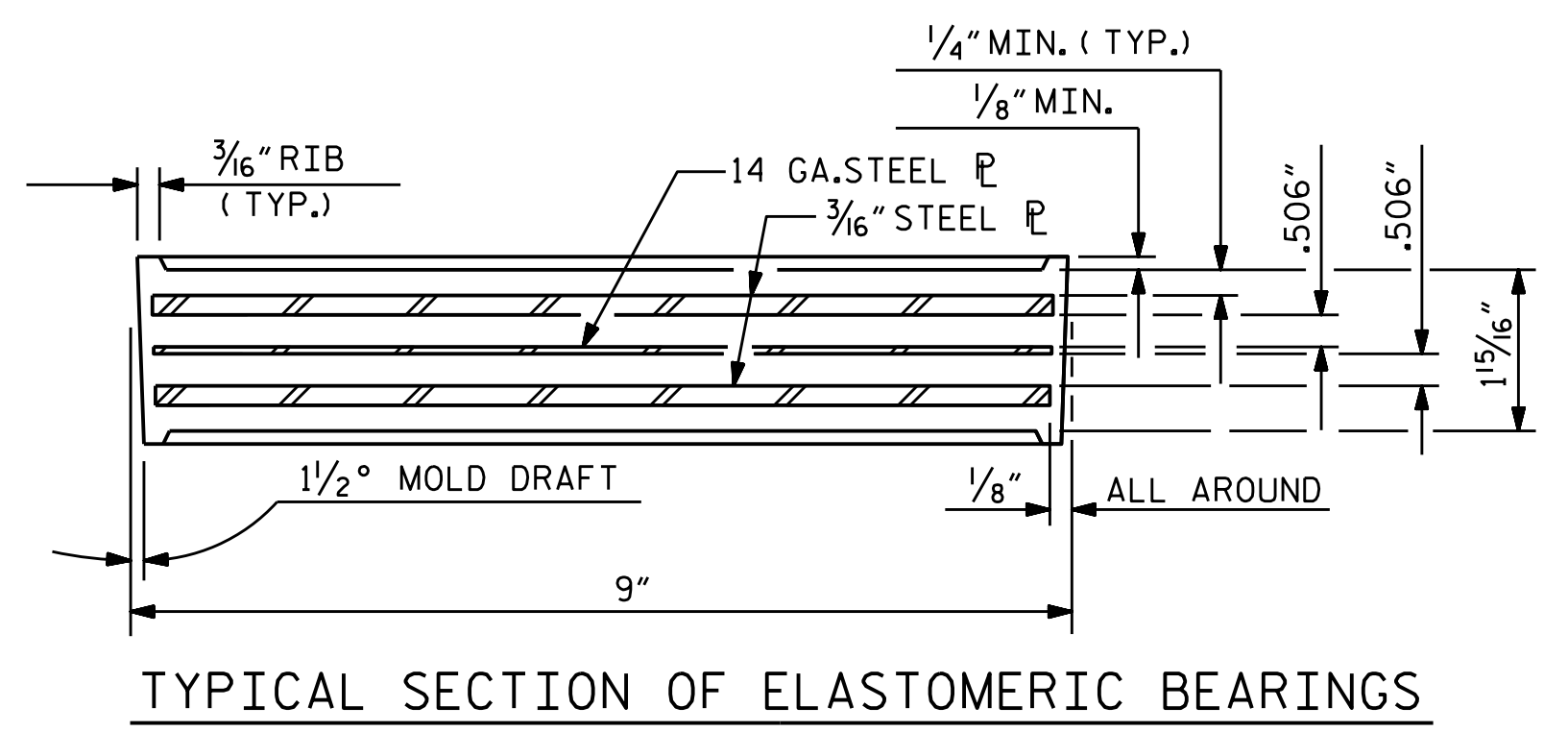
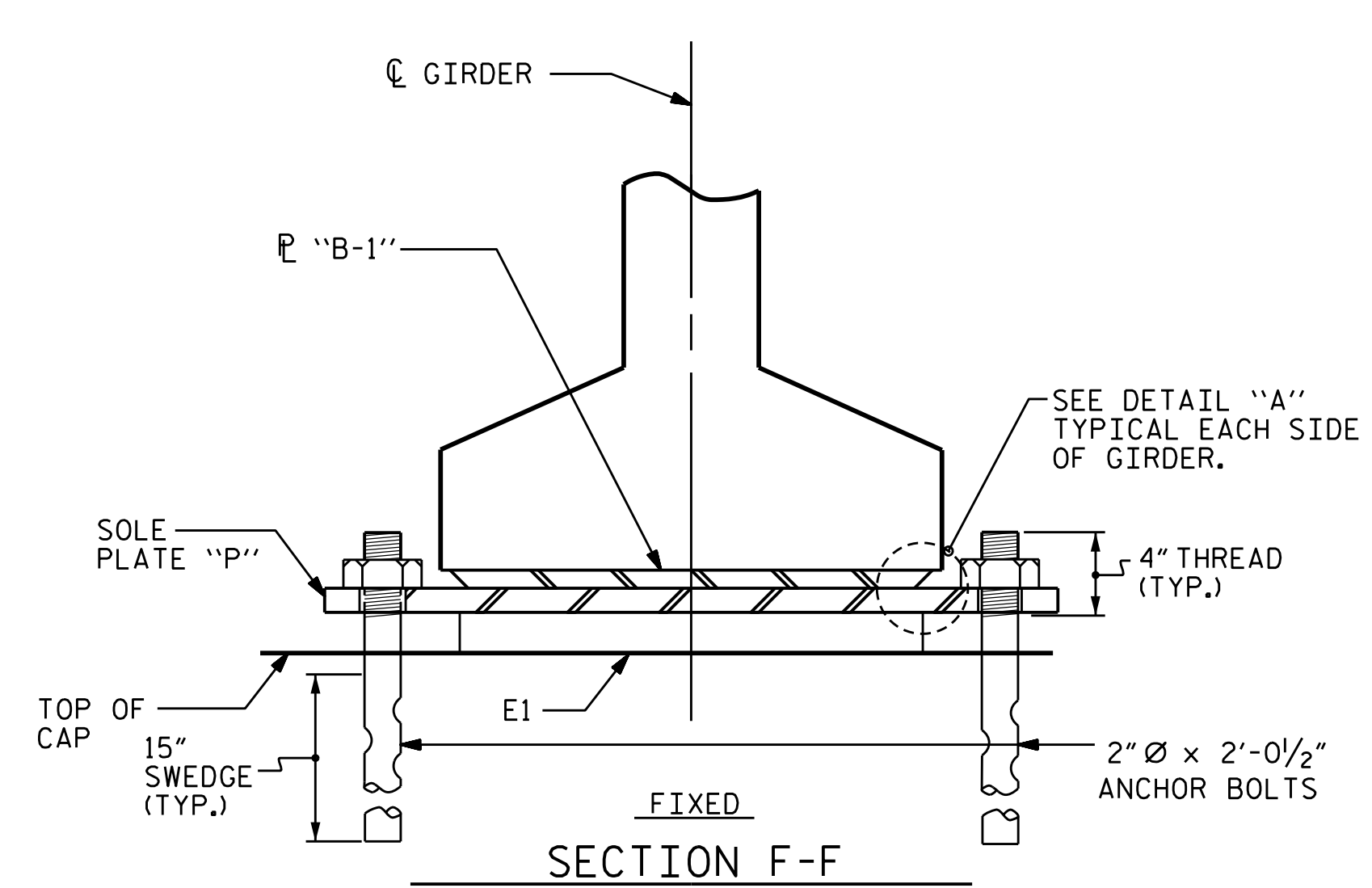
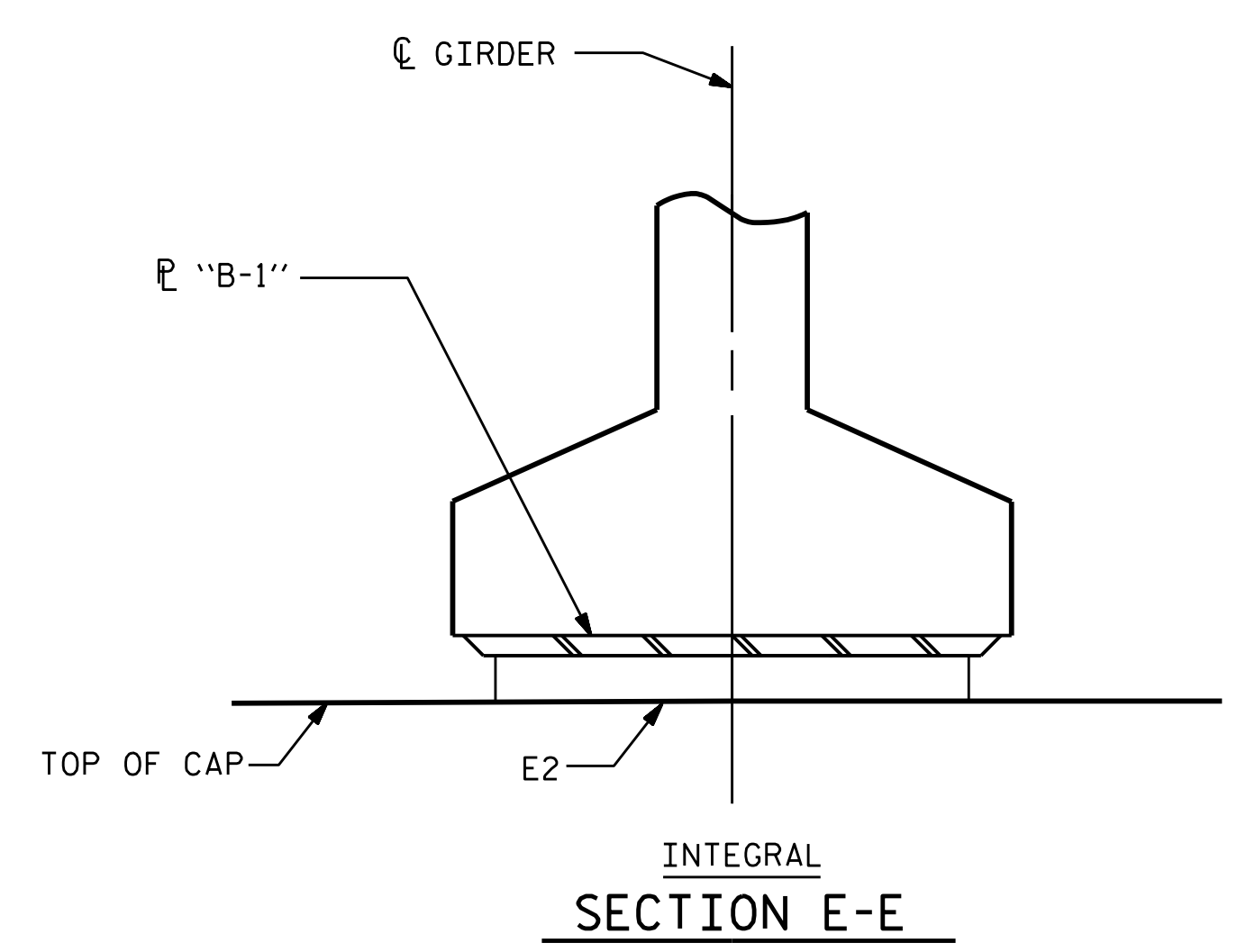
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ASSEMBLED BY : J. B. GEILE DATE : 12/15/17  
 CHECKED BY : R.F. DECOLA DATE : 03/09/18  
 DRAWN BY : RWW 11/09 REV. 10/11/11 MAA/GM  
 CHECKED BY : GM 11/09 REV. 12/17 MAA/THC  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE : 10/25/21

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TOTAL SHEETS	38

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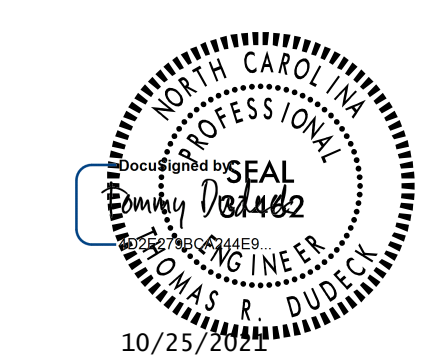


MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k
TYPE VII	395 k

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 1 OF 1

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
STANDARD  
ELASTOMERIC BEARING  
DETAILS  
PRESTRESSED CONCRETE GIRDER  
SUPERSTRUCTURE



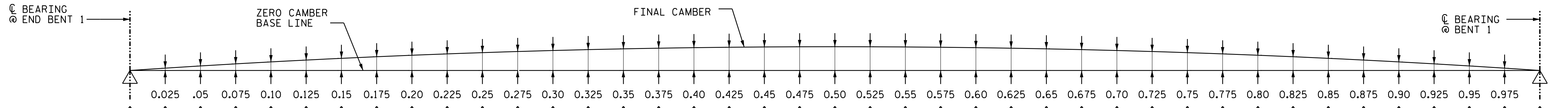
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NO.	BY:	DATE:	NO.	BY:	DATE:	S16-18
1			3			TOTAL SHEETS
2			4			38

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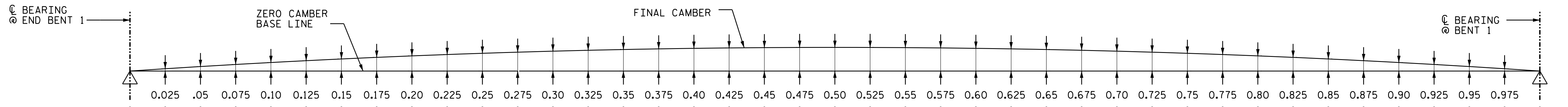
DRAWN BY: E. DAVIS DATE: 12/8/17  
CHECKED BY: R. DeCOLA DATE: 01/02/18  
DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21



GIRDERS 1 & 6

FORTIETH POINTS BETWEEN BRGS.	0.000	0.013	0.025	0.038	0.050	0.062	0.074	0.085	0.095	0.105	0.114	0.122	0.130	0.137	0.143	0.148	0.152	0.155	0.158	0.159	0.160	0.159	0.158	0.155	0.152	0.148	0.143	0.137	0.130	0.122	0.114	0.105	0.095	0.085	0.074	0.062	0.050	0.038	0.025	0.013	0.000			
CAMBER (GIRDER ALONE IN PLACE)	↑ 0.000	↑ 0.013	↑ 0.025	↑ 0.038	↑ 0.050	↑ 0.062	↑ 0.074	↑ 0.085	↑ 0.095	↑ 0.105	↑ 0.114	↑ 0.122	↑ 0.130	↑ 0.137	↑ 0.143	↑ 0.148	↑ 0.152	↑ 0.155	↑ 0.158	↑ 0.159	↑ 0.160	↑ 0.159	↑ 0.158	↑ 0.155	↑ 0.152	↑ 0.148	↑ 0.143	↑ 0.137	↑ 0.130	↑ 0.122	↑ 0.114	↑ 0.105	↑ 0.095	↑ 0.085	↑ 0.074	↑ 0.062	↑ 0.050	↑ 0.038	↑ 0.025	↑ 0.013	↑ 0.000			
DEFLEC. DUE TO SUPERIMPOSED DL *	↓ 0.000	↓ 0.006	↓ 0.012	↓ 0.018	↓ 0.023	↓ 0.029	↓ 0.035	↓ 0.040	↓ 0.046	↓ 0.050	↓ 0.055	↓ 0.059	↓ 0.063	↓ 0.066	↓ 0.069	↓ 0.072	↓ 0.074	↓ 0.075	↓ 0.076	↓ 0.077	↓ 0.078	↓ 0.077	↓ 0.075	↓ 0.074	↓ 0.073	↓ 0.070	↓ 0.067	↓ 0.064	↓ 0.062	↓ 0.057	↓ 0.053	↓ 0.048	↓ 0.044	↓ 0.038	↓ 0.033	↓ 0.027	↓ 0.022	↓ 0.016	↓ 0.011	↓ 0.005	↓ 0.000			
FINAL CAMBER (OR DEFLECTION)	↑ 0"	↑ 1/16"	↑ 3/16"	↑ 1/4"	↑ 5/16"	↑ 3/8"	↑ 1/2"	↑ 1/2"	↑ 9/16"	↑ 5/8"	↑ 11/16"	↑ 3/4"	↑ 13/16"	↑ 7/8"	↑ 7/8"	↑ 15/16"	↑ 15/16"	↑ 15/16"	↑ 1"	↑ 1"	↑ 1"	↑ 1"	↑ 1"	↑ 15/16"	↑ 15/16"	↑ 15/16"	↑ 15/16"	↑ 7/8"	↑ 13/16"	↑ 3/4"	↑ 3/4"	↑ 11/16"	↑ 5/8"	↑ 9/16"	↑ 1/2"	↑ 7/16"	↑ 7/16"	↑ 3/8"	↑ 5/16"	↑ 3/16"	↑ 1/4"	↑ 3/16"	↑ 1/16"	↑ 0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.



GIRDERS 2-5

FORTIETH POINTS BETWEEN BRGS.	0.000	0.013	0.025	0.038	0.050	0.062	0.074	0.085	0.095	0.105	0.114	0.122	0.130	0.137	0.143	0.148	0.152	0.155	0.158	0.159	0.160	0.159	0.158	0.155	0.152	0.148	0.143	0.137	0.130	0.122	0.114	0.105	0.095	0.085	0.074	0.062	0.050	0.038	0.025	0.013	0.000	
CAMBER (GIRDER ALONE IN PLACE)	↑ 0.000	↑ 0.013	↑ 0.025	↑ 0.038	↑ 0.050	↑ 0.062	↑ 0.074	↑ 0.085	↑ 0.095	↑ 0.105	↑ 0.114	↑ 0.122	↑ 0.130	↑ 0.137	↑ 0.143	↑ 0.148	↑ 0.152	↑ 0.155	↑ 0.158	↑ 0.159	↑ 0.160	↑ 0.159	↑ 0.158	↑ 0.155	↑ 0.152	↑ 0.148	↑ 0.143	↑ 0.137	↑ 0.130	↑ 0.122	↑ 0.114	↑ 0.105	↑ 0.095	↑ 0.085	↑ 0.074	↑ 0.062	↑ 0.050	↑ 0.038	↑ 0.025	↑ 0.013	↑ 0.000	
DEFLEC. DUE TO SUPERIMPOSED DL *	↓ 0.000	↓ 0.007	↓ 0.014	↓ 0.021	↓ 0.028	↓ 0.035	↓ 0.042	↓ 0.049	↓ 0.055	↓ 0.061	↓ 0.066	↓ 0.071	↓ 0.076	↓ 0.080	↓ 0.083	↓ 0.086	↓ 0.090	↓ 0.091	↓ 0.092	↓ 0.093	↓ 0.094	↓ 0.092	↓ 0.091	↓ 0.090	↓ 0.088	↓ 0.085	↓ 0.081	↓ 0.078	↓ 0.074	↓ 0.069	↓ 0.063	↓ 0.058	↓ 0.053	↓ 0.046	↓ 0.039	↓ 0.033	↓ 0.026	↓ 0.020	↓ 0.013	↓ 0.007	↓ 0.000	
FINAL CAMBER (OR DEFLECTION)	↑ 0"	↑ 1/16"	↑ 1/8"	↑ 3/16"	↑ 1/4"	↑ 5/16"	↑ 3/8"	↑ 7/16"	↑ 1/2"	↑ 1/2"	↑ 9/16"	↑ 5/8"	↑ 5/8"	↑ 11/16"	↑ 3/4"	↑ 3/4"	↑ 3/4"	↑ 3/4"	↑ 13/16"	↑ 13/16"	↑ 13/16"	↑ 13/16"	↑ 13/16"	↑ 13/16"	↑ 13/16"	↑ 3/4"	↑ 3/4"	↑ 3/4"	↑ 11/16"	↑ 11/16"	↑ 5/8"	↑ 5/8"	↑ 9/16"	↑ 1/2"	↑ 7/16"	↑ 7/16"	↑ 3/8"	↑ 5/16"	↑ 3/16"	↑ 1/8"	↑ 1/16"	↑ 0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.

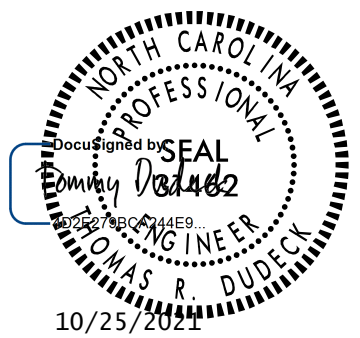
SCHMATIC CAMBER ORDINATES SPAN A

ALL VALUES ARE SHOWN IN DECIMALS OF A FOOT EXCEPT "FINAL CAMBER (OR DEFLECTION)" WHICH IS SHOWN IN INCHES.  
 (+) FINAL CAMBER INDICATES NET UPWARD DISPLACEMENT.  
 (-) FINAL CAMBER INDICATES NET DOWNWARD DISPLACEMENT.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 DEAD LOAD  
 DEFLECTIONS  
 SPAN A



REVISIONS						SHEET NO. S16-19
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 38
2			4			

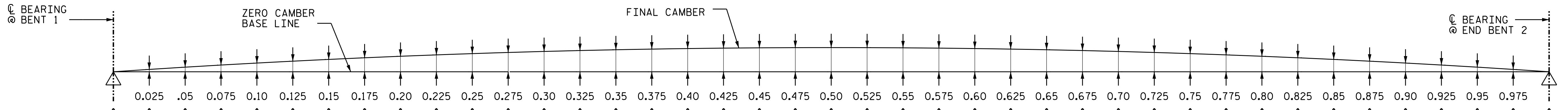
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DRAWN BY: J. B. GEILE DATE: 08/14/19  
 CHECKED BY: T. R. DUDECK DATE: 04/10/20  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

10/25/2021  
 10/25/2021  
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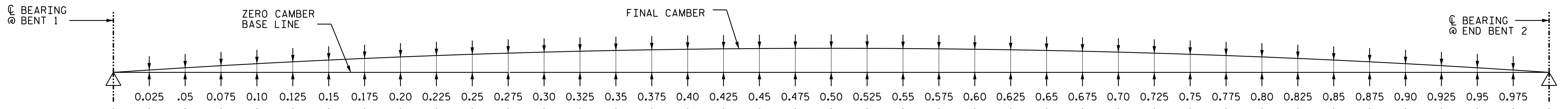




GIRDERS 1 & 6

FORTIETH POINTS BETWEEN BRGS.																																										
CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.013	0.026	0.038	0.050	0.062	0.074	0.085	0.095	0.105	0.115	0.123	0.131	0.138	0.144	0.149	0.153	0.156	0.159	0.159	0.161	0.160	0.159	0.156	0.153	0.149	0.144	0.138	0.131	0.123	0.115	0.105	0.095	0.085	0.074	0.062	0.050	0.038	0.026	0.013	0.000
DEFLEC. DUE TO SUPERIMPOSED DL *	↓	0.000	0.009	0.018	0.027	0.036	0.045	0.054	0.062	0.071	0.078	0.085	0.092	0.099	0.104	0.108	0.112	0.117	0.119	0.120	0.122	0.123	0.122	0.121	0.120	0.118	0.114	0.110	0.106	0.102	0.095	0.088	0.081	0.074	0.065	0.056	0.047	0.038	0.029	0.019	0.010	0.000
FINAL CAMBER (OR DEFLECTION)	↑	0"	1/16"	1/8"	1/8"	3/16"	3/16"	1/4"	1/4"	5/16"	5/16"	3/8"	3/8"	3/8"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	7/16"	3/8"	3/8"	5/16"	5/16"	5/16"	1/4"	1/4"	3/16"	3/16"	1/8"	1/8"	1/16"	1/16"	0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.



GIRDERS 2-5

FORTIETH POINTS BETWEEN BRGS.																																										
CAMBER (GIRDER ALONE IN PLACE)	↑	0.000	0.013	0.026	0.038	0.050	0.062	0.074	0.085	0.095	0.105	0.115	0.123	0.131	0.138	0.144	0.149	0.153	0.156	0.159	0.159	0.161	0.160	0.159	0.156	0.153	0.149	0.144	0.138	0.131	0.123	0.115	0.105	0.095	0.085	0.074	0.062	0.050	0.038	0.026	0.013	0.000
DEFLEC. DUE TO SUPERIMPOSED DL *	↑	0.000	0.011	0.022	0.033	0.043	0.054	0.065	0.075	0.086	0.094	0.102	0.111	0.119	0.125	0.130	0.135	0.141	0.143	0.145	0.147	0.149	0.147	0.146	0.144	0.143	0.138	0.132	0.127	0.122	0.114	0.106	0.097	0.089	0.078	0.068	0.057	0.046	0.034	0.023	0.011	0.000
FINAL CAMBER (OR DEFLECTION)	↑	0"	0"	1/16"	1/16"	1/16"	1/8"	1/8"	1/8"	1/8"	1/8"	1/8"	1/8"	1/8"	3/16"	3/16"	1/8"	3/16"	3/16"	1/8"	1/8"	1/8"	1/8"	1/8"	3/16"	1/8"	1/8"	1/8"	1/8"	1/8"	1/8"	1/16"	1/16"	1/16"	1/16"	1/16"	1/16"	1/16"	1/16"	1/16"	0"	0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.

### SCHMATIC CAMBER ORDINATES SPAN B

ALL VALUES ARE SHOWN IN DECIMALS OF A FOOT EXCEPT "FINAL CAMBER (OR DEFLECTION)" WHICH IS SHOWN IN INCHES.

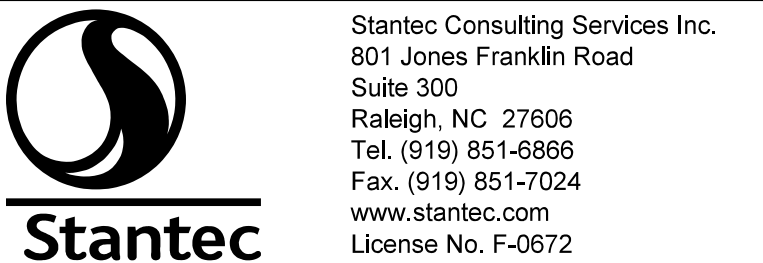
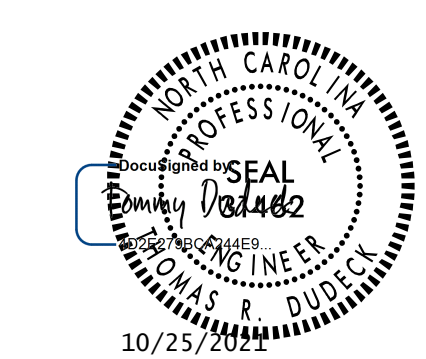
(+) FINAL CAMBER INDICATES NET UPWARD DISPLACEMENT.

(-) FINAL CAMBER INDICATES NET DOWNWARD DISPLACEMENT.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 2

STATE OF NORTH CAROLINA  
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 RALEIGH  
 SUPERSTRUCTURE  
 DEAD LOAD  
 DEFLECTIONS  
 SPAN B

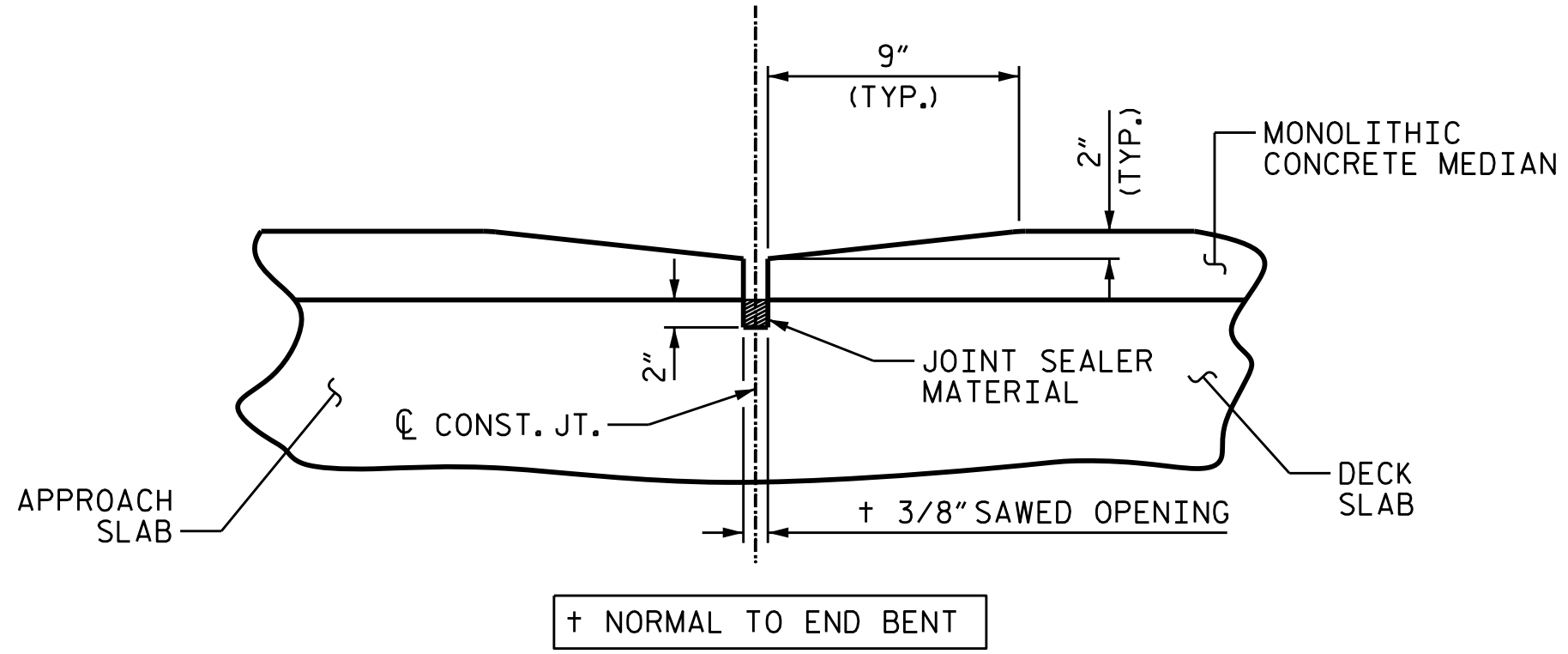


DRAWN BY: J. B. GEILE DATE: 12/12/17  
 CHECKED BY: R. DeCOLA DATE: 02/12/17  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

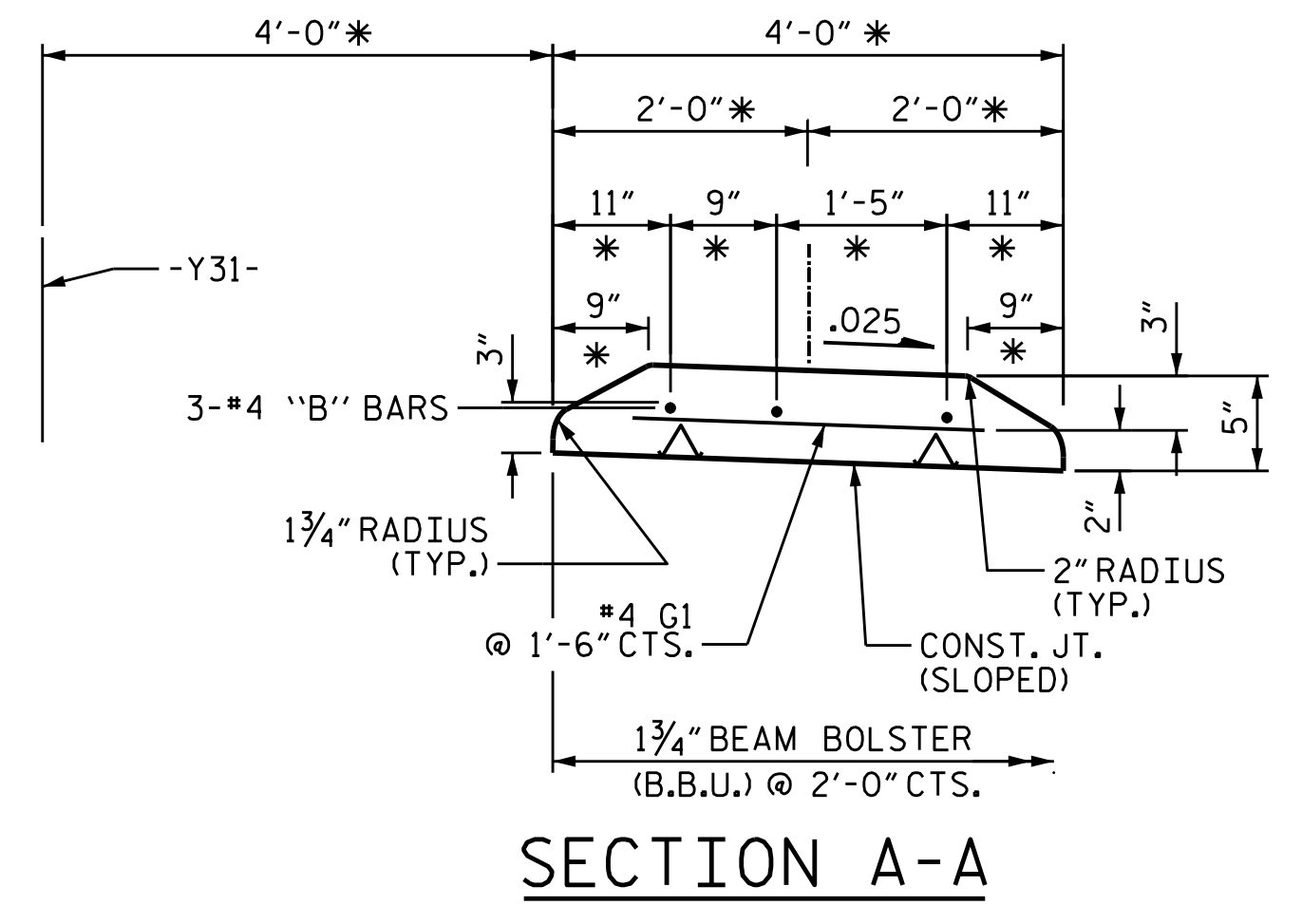
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2			4			38

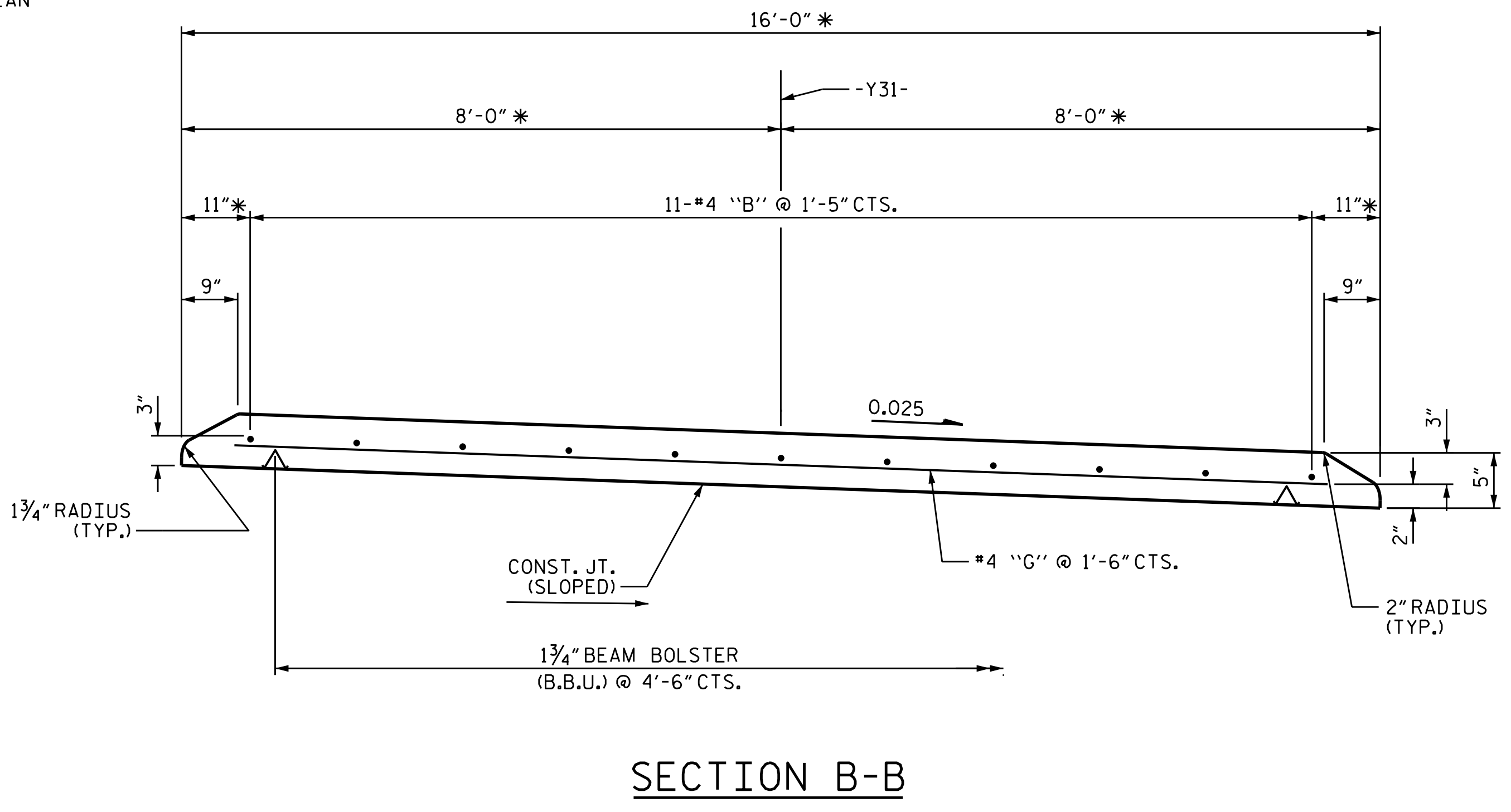
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**SECTION THROUGH MONOLITHIC CONCRETE MEDIAN AT INTEGRAL END BENT**  
END BENT 1 SHOWN, END BENT 2 SIMILAR



**SECTION A-A**



**SECTION B-B**

**NOTES**

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE MONOLITHIC CONCRETE MEDIAN IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT A SPACING OF 8 FT. TO 10 FT. BETWEEN EXPANSION JOINTS. NO CONTRACTION JOINT WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FT. IN LENGTH.

ALL REINFORCING STEEL IN MONOLITHIC CONCRETE MEDIAN SHALL BE EPOXY COATED.

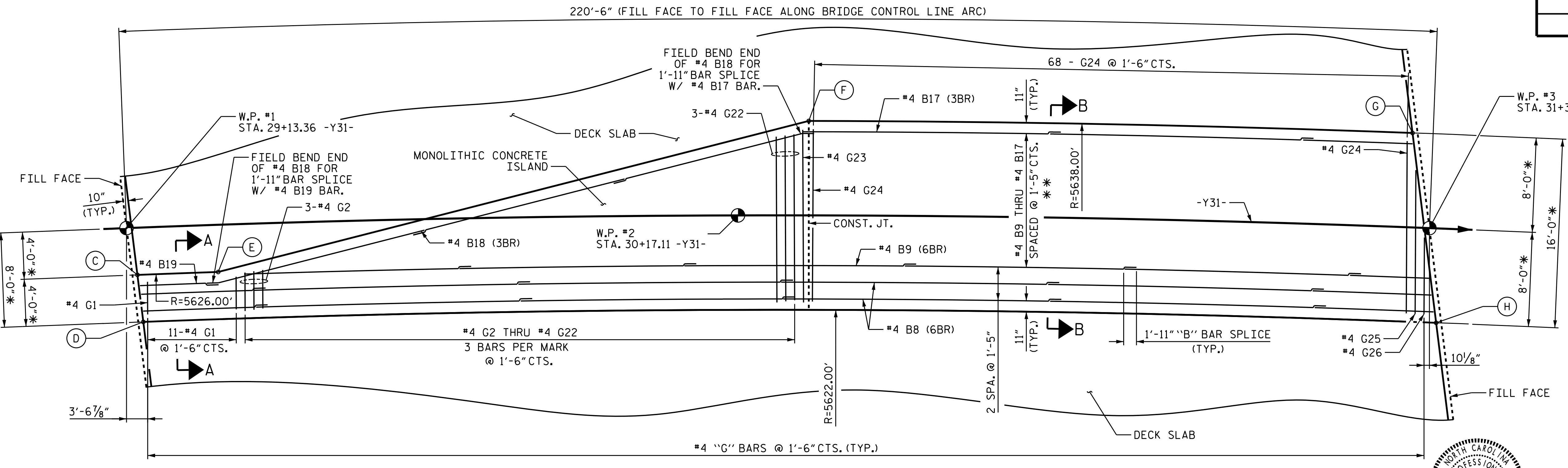
FOR MONOLITHIC CONCRETE MEDIAN ON APPROACH SLAB, SEE "BRIDGE APPROACH SLAB" SHEETS.

NO SEPARATE MEASUREMENT OR PAYMENT WILL BE MADE FOR MATERIALS OR LABOR REQUIRED TO CONSTRUCT THE CONCRETE MEDIAN. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR REINFORCED CONCRETE DECK SLAB.

THE CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONCRETE MEDIAN IS INCLUDED IN THE "SUPERSTRUCTURE BILL OF MATERIAL" AND INCLUDED IN THE SQUARE FOOT PRICE FOR "REINFORCED CONCRETE DECK SLAB."

CONCRETE MEDIAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

MONOLITHIC CONCRETE MEDIAN LAYOUT		
WORK POINT	STATION	OFFSET
(C)	29+15.13 -Y31-	4.00' RT.
(D)	29+16.05 -Y31-	8.00' RT.
(E)	29+28.81 -Y31-	4.00' RT.
(F)	30+28.82 -Y31-	8.00' L.T.
(G)	31+30.85 -Y31-	8.00' L.T.
(H)	31+35.16 -Y31-	8.00' RT.

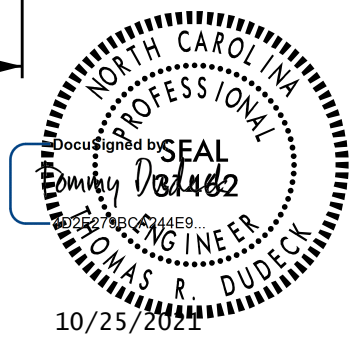


**PLAN OF MONOLITHIC CONCRETE MEDIAN**

\* RADIAL DIMENSIONS  
 \* # B10-B12 (5BR)  
 B13-B15 (4BR)  
 B16 (3BR)

PROJECT NO. R-3300B  
 PENDER \_\_\_\_\_ COUNTY  
 STATION: 30+17.11 -Y31-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 MONOLITHIC  
 CONCRETE MEDIAN  
 DETAILS



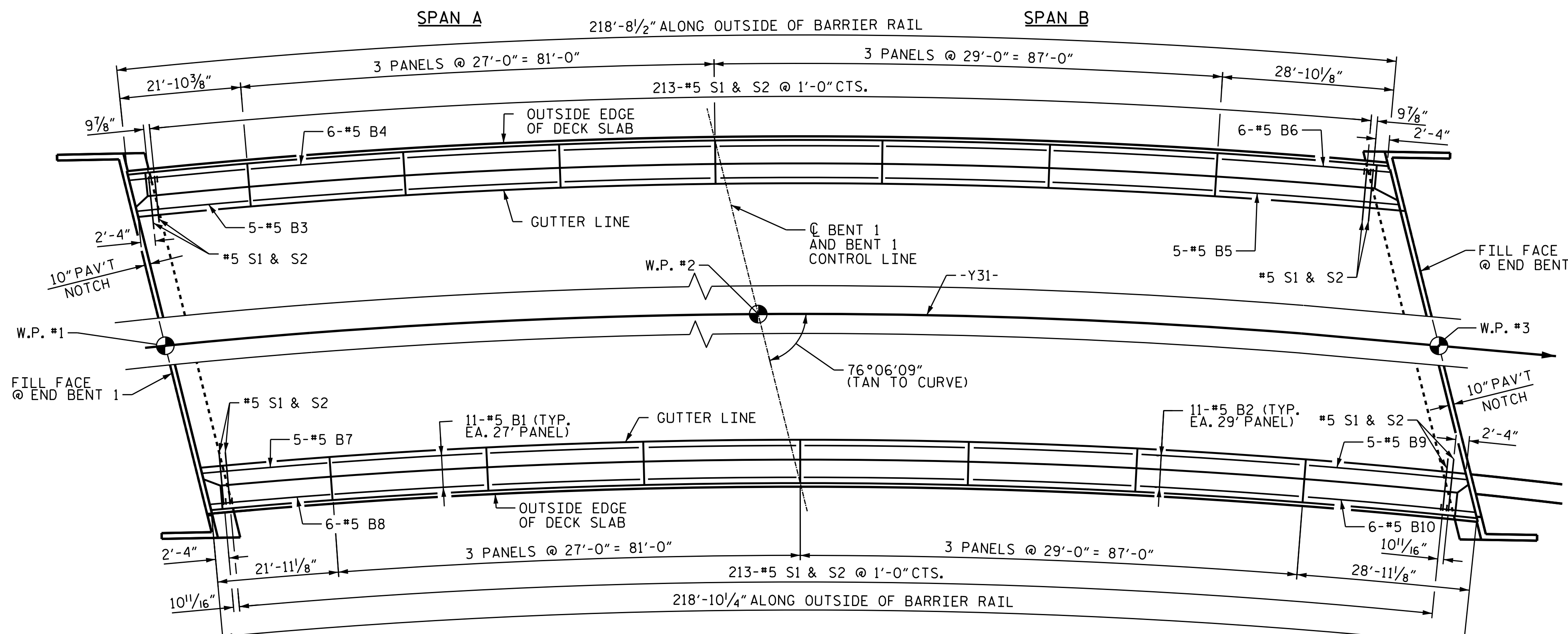
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	BY:	DATE:			BY:	DATE:		
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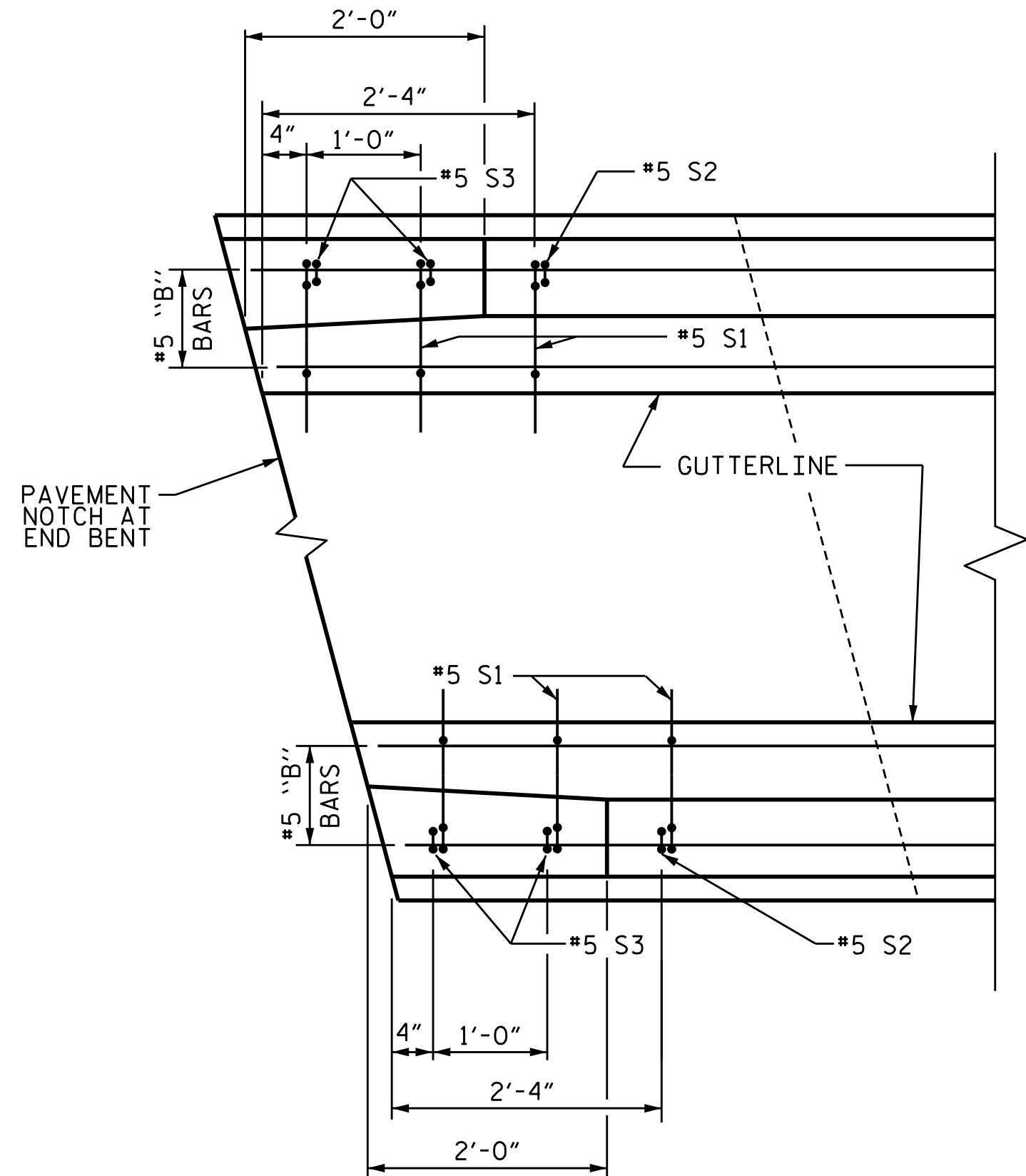
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 CHECKED BY: R. DeCOLA DATE: 04/02/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

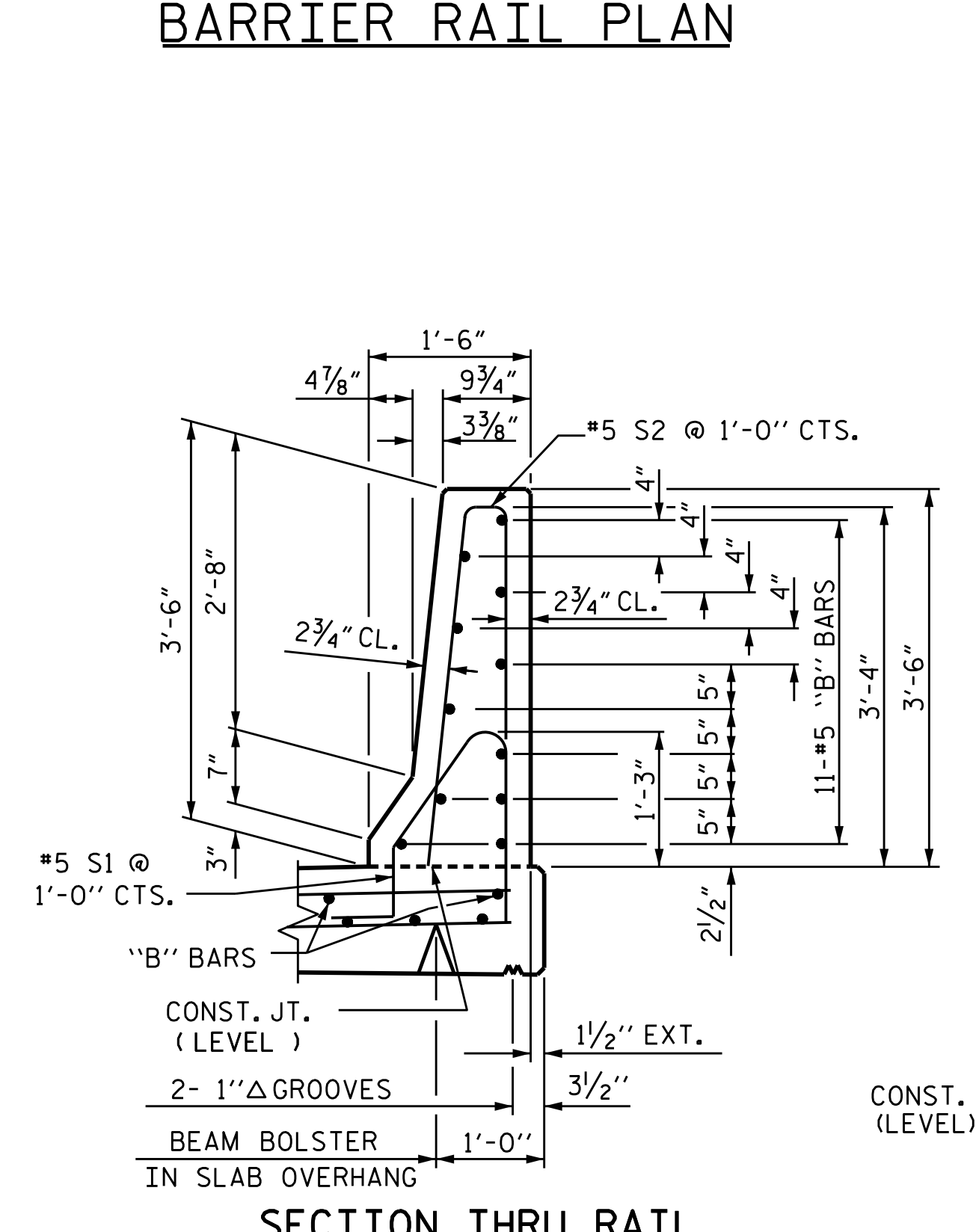
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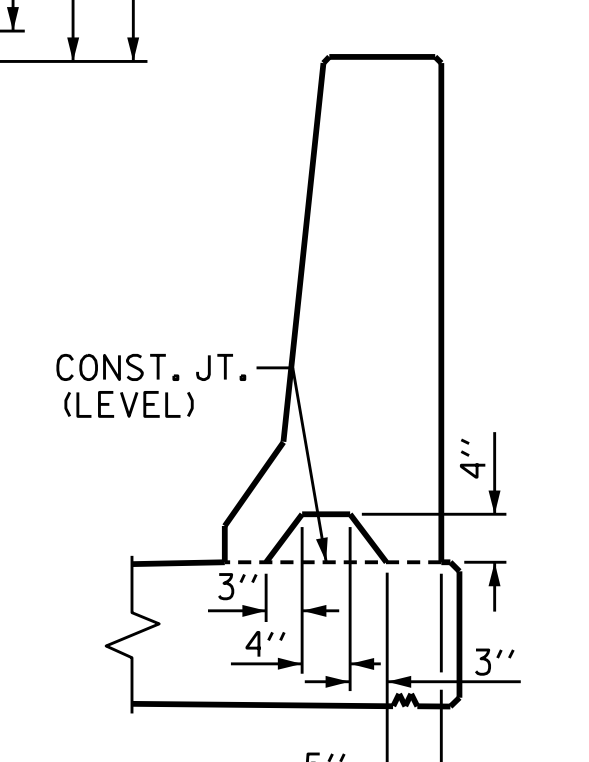
**BARRIER RAIL PLAN**



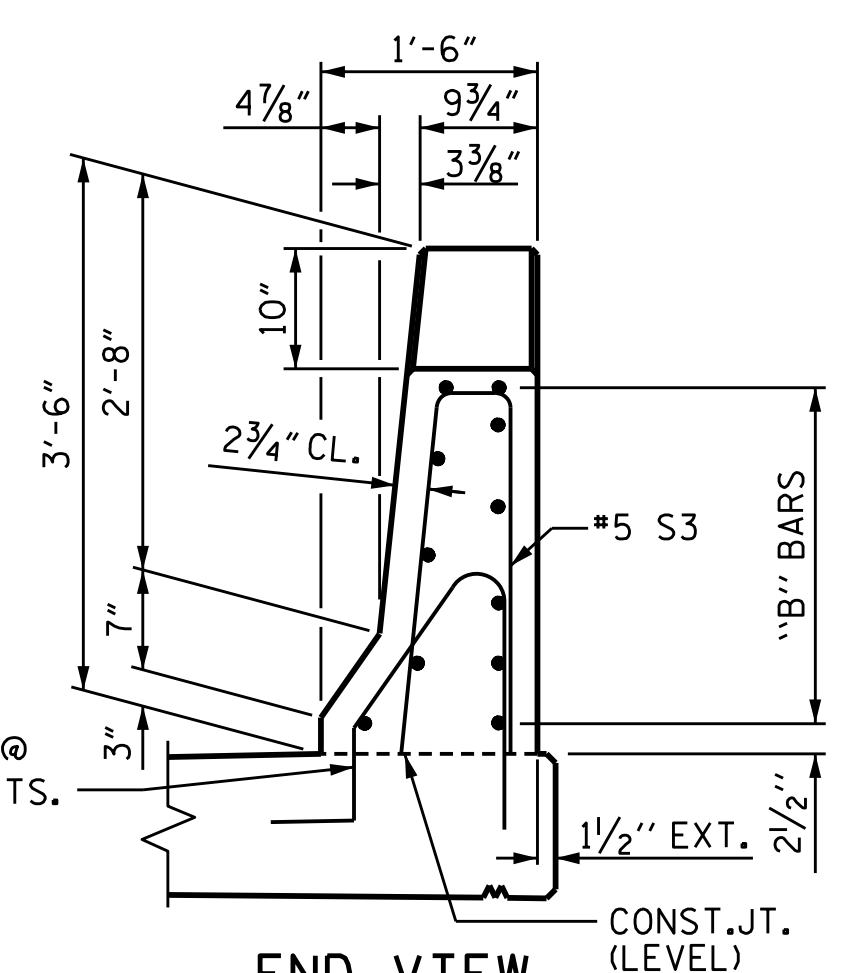
**PLAN**



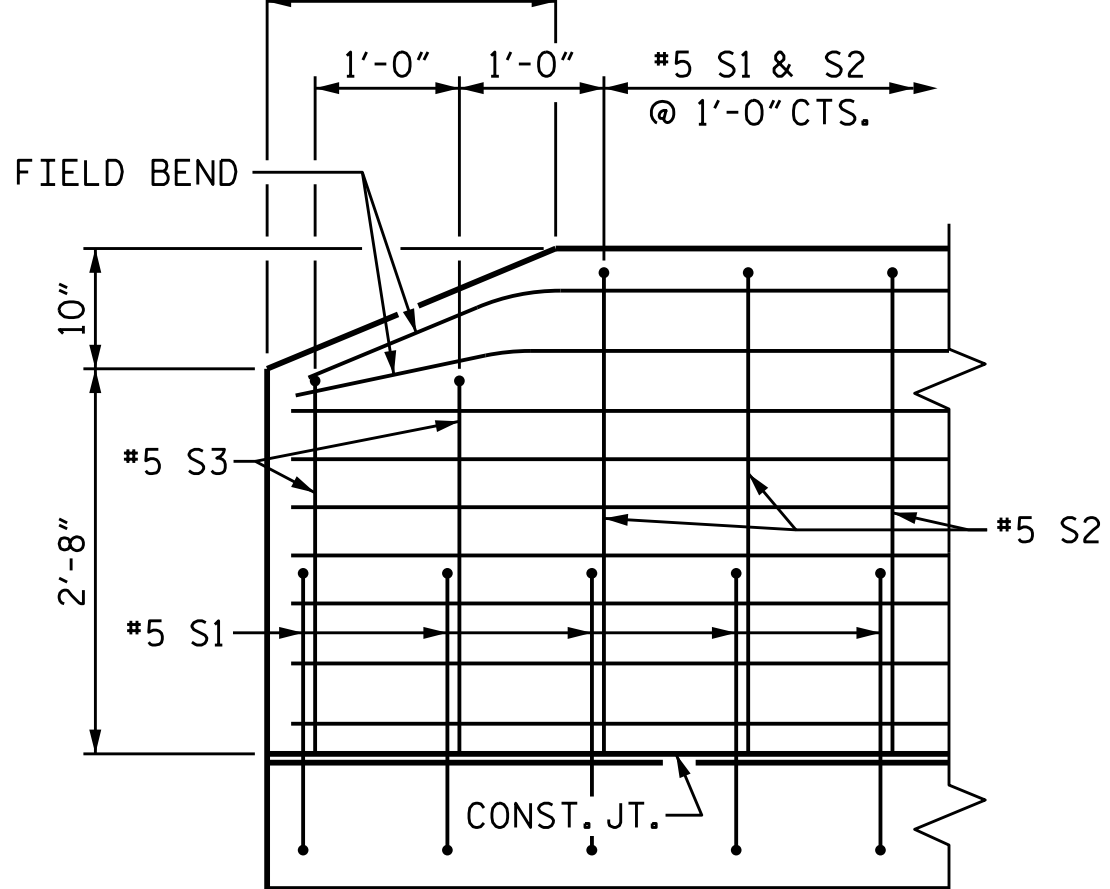
**SECTION THRU RAIL**



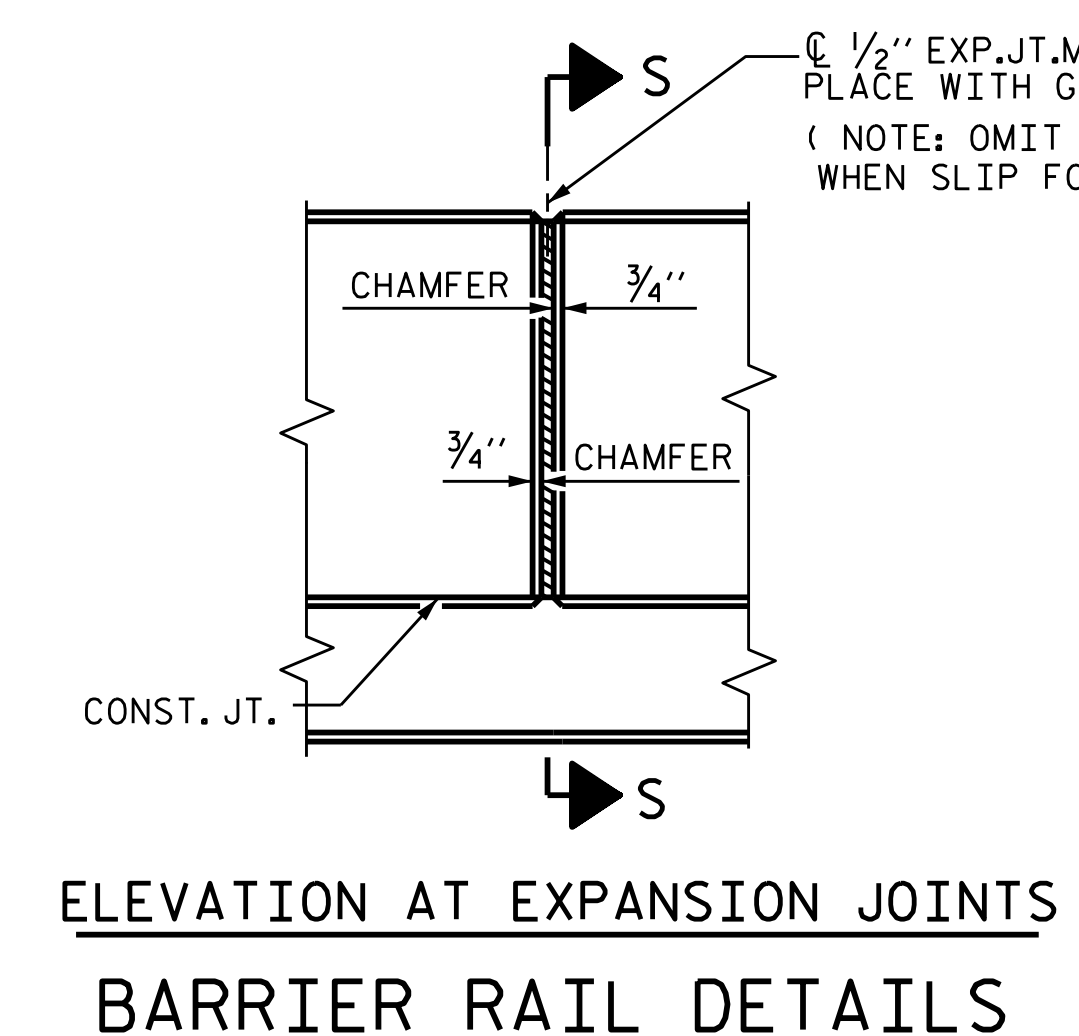
**SECTION S-S**  
AT DAM IN OPEN JOINT  
(THIS IS TO BE USED ONLY  
WHEN SLIP FORM IS USED)



**END VIEW**



**SIDE VIEW**



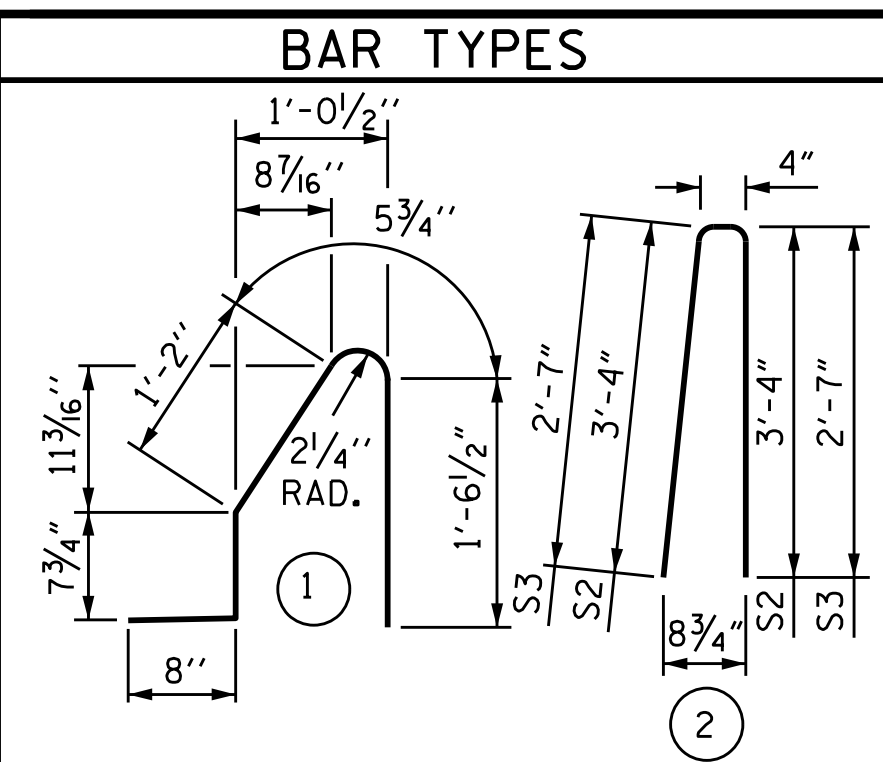
**ELEVATION AT EXPANSION JOINTS**  
**BARRIER RAIL DETAILS**

**NOTES**

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS.



ALL BAR DIMENSIONS ARE OUT TO OUT

**BILL OF MATERIAL**

FOR CONCRETE BARRIER RAIL ONLY					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* B1	66	#5	STR	26'-7"	1830
* B2	66	#5	STR	28'-7"	1968
* B3	5	#5	STR	21'-1"	110
* B4	6	#5	STR	21'-4"	134
* B5	5	#5	STR	28'-10"	150
* B6	6	#5	STR	28'-7"	179
* B7	5	#5	STR	21'-10"	114
* B8	6	#5	STR	21'-7"	135
* B9	5	#5	STR	28'-3"	147
* B10	6	#5	STR	28'-6"	178
* S1	438	#5	1	4'-6"	2056
* S2	430	#5	2	7'-0"	3139
* S3	8	#5	2	5'-6"	46

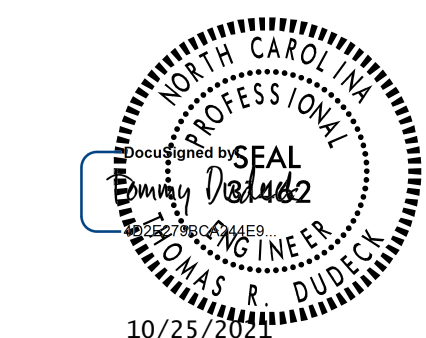
\* EPOXY COATED REINFORCING STEEL 10186 LBS.  
CLASS AA CONCRETE 59.5 CU. YDS.  
CONCRETE BARRIER RAIL 437.56 LIN. FT.

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
STANDARD

**CONCRETE BARRIER RAIL**

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ASSEMBLED BY : J. B. GEILE DATE : 12/15/17  
CHECKED BY : R. DeCOLA DATE : 03/09/18

DRAWN BY : ARB 5/87  
CHECKED BY : SJD 9/87

REV. 7/12  
REV. 6/13  
REV. 12/17

MAA/GM  
MAA/GM  
MAA/GM

DESIGN ENGINEER  
OF RECORD: T. R. DUDECK DATE : 10/25/21

10/25/2021 10/25/2021

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

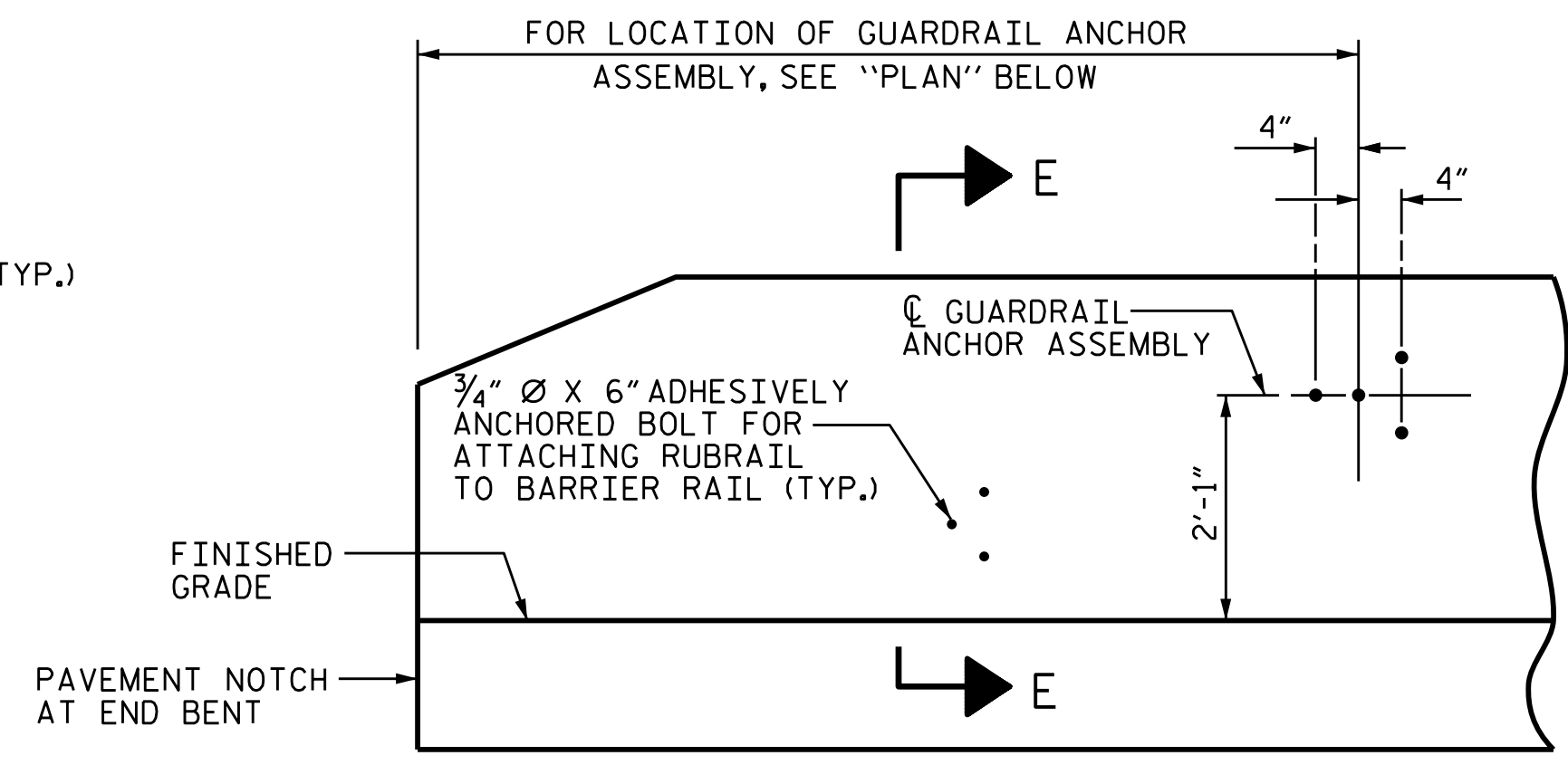
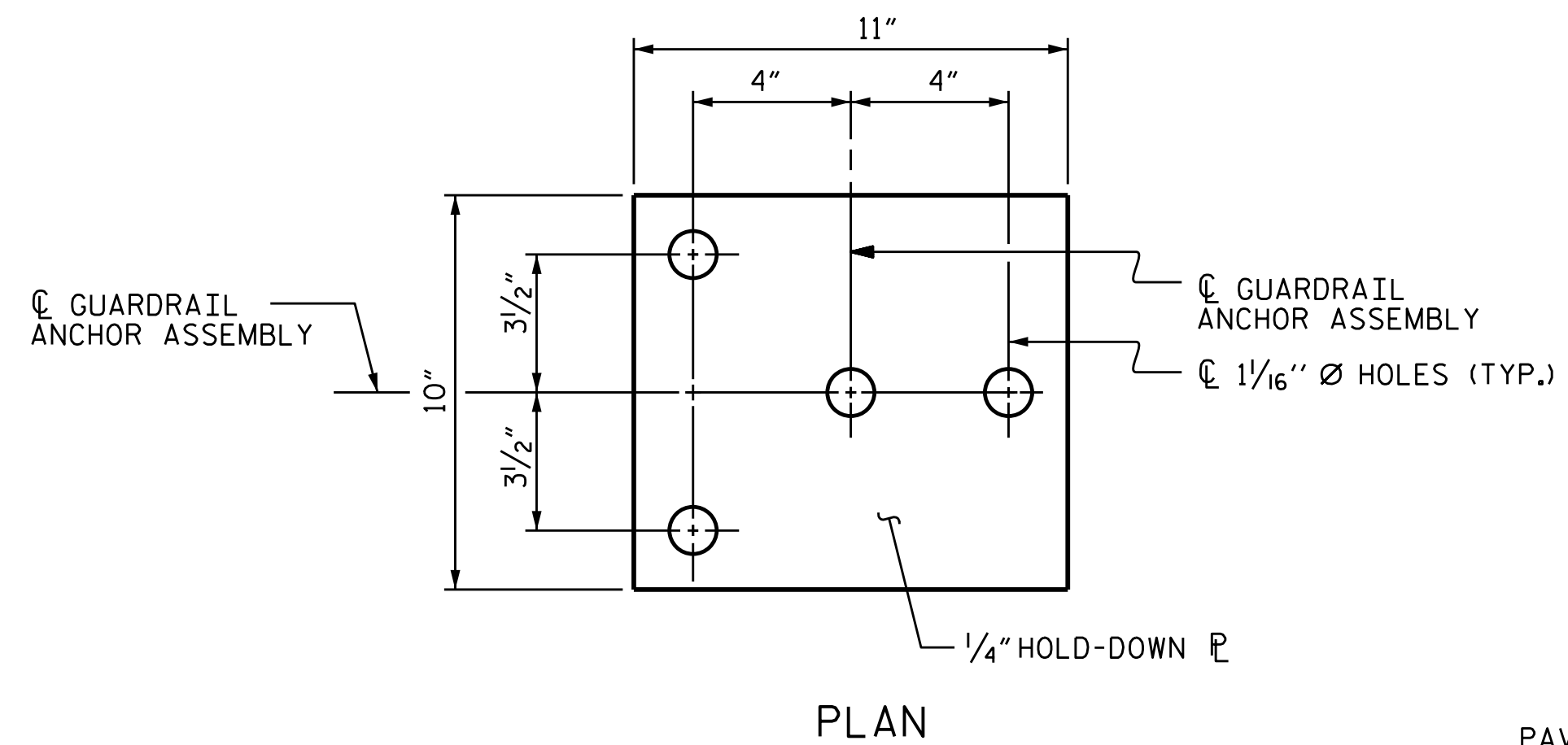
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

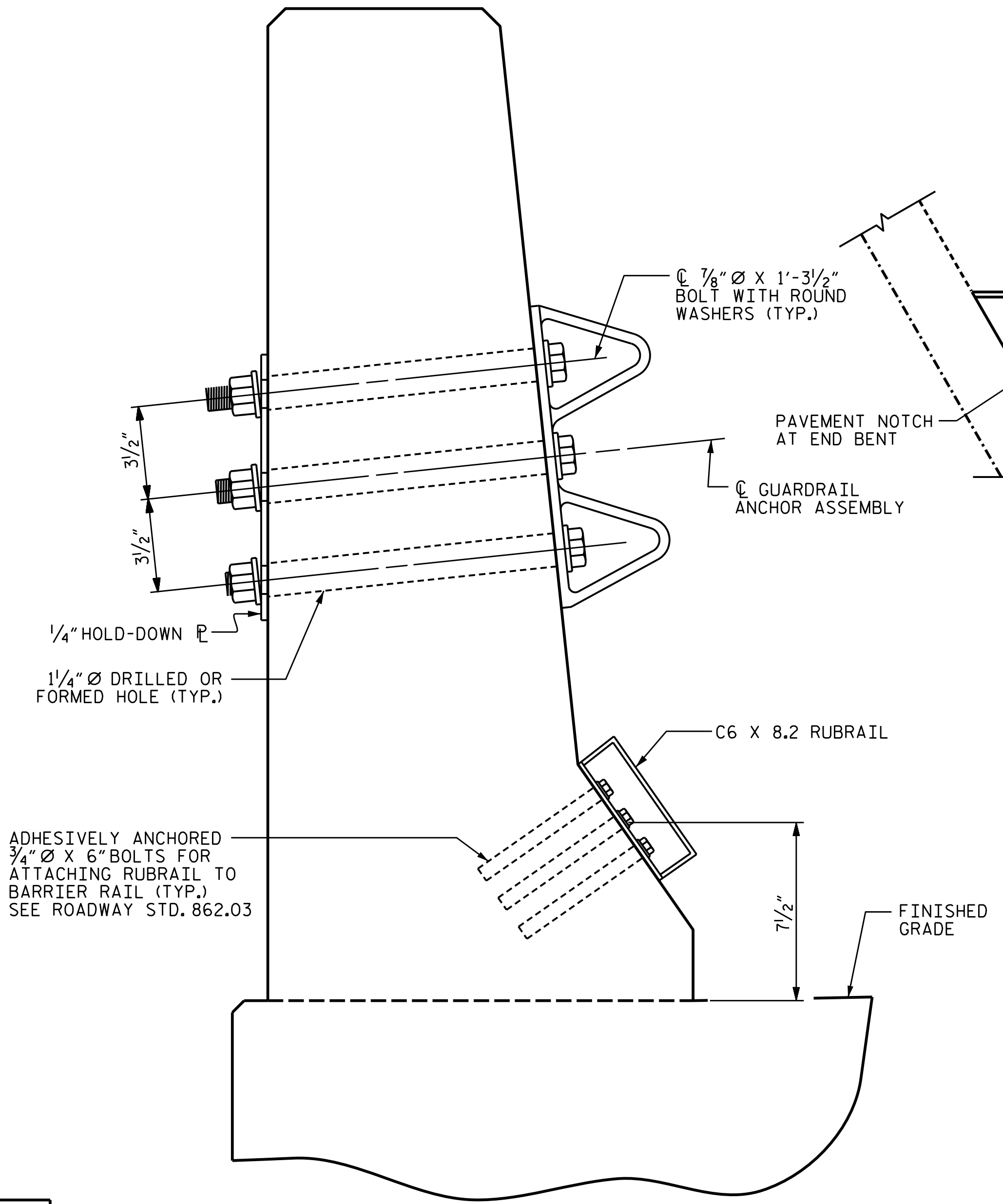
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

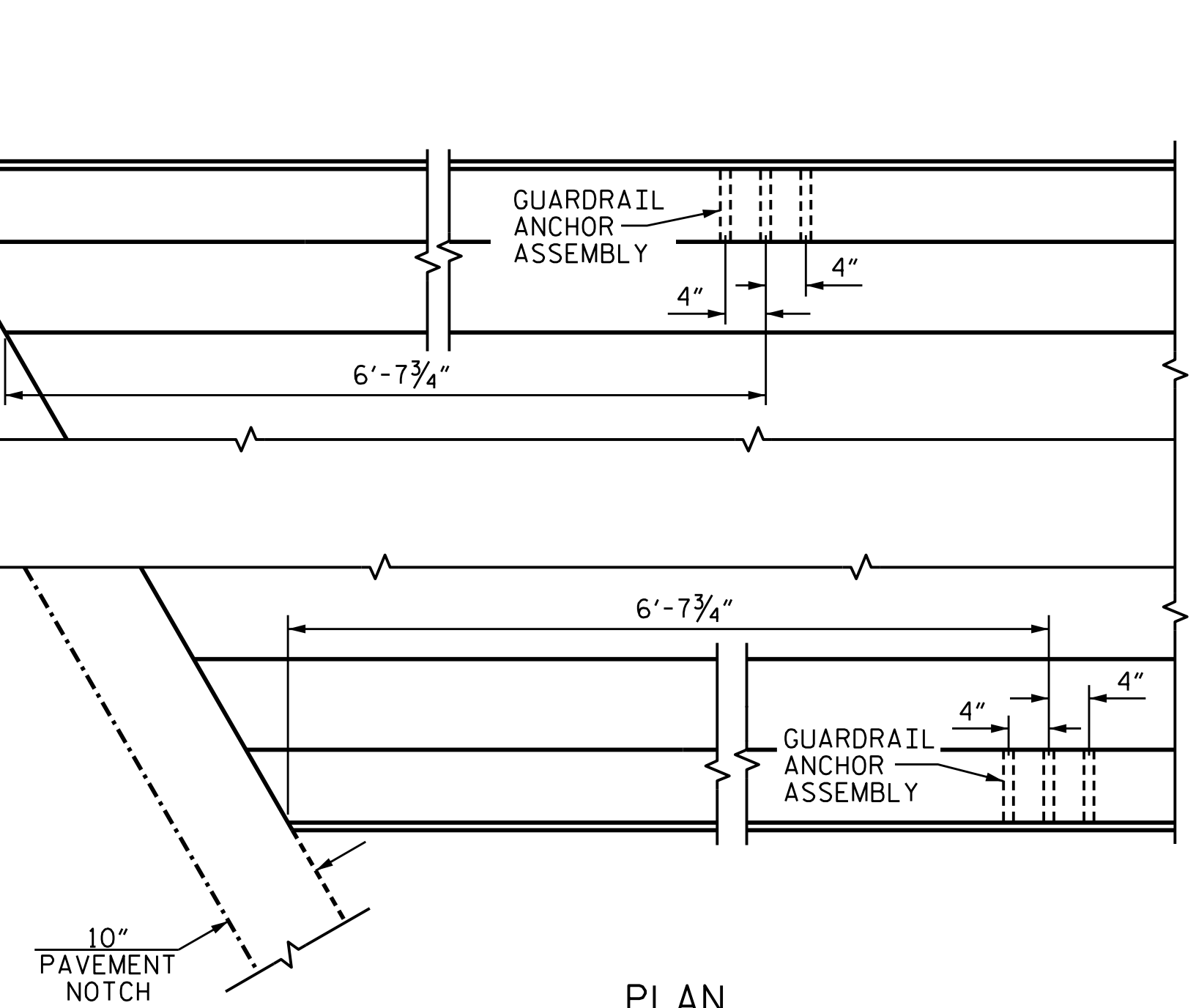
THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



ELEVATION



SECTION E-E  
GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

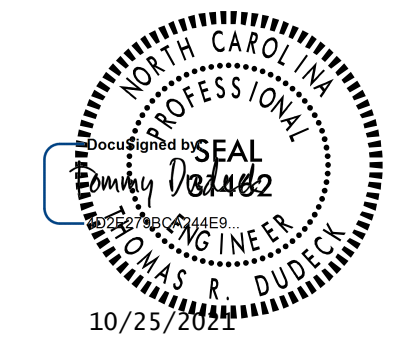


SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 GUARDRAIL ANCHORAGE  
 FOR BARRIER RAIL



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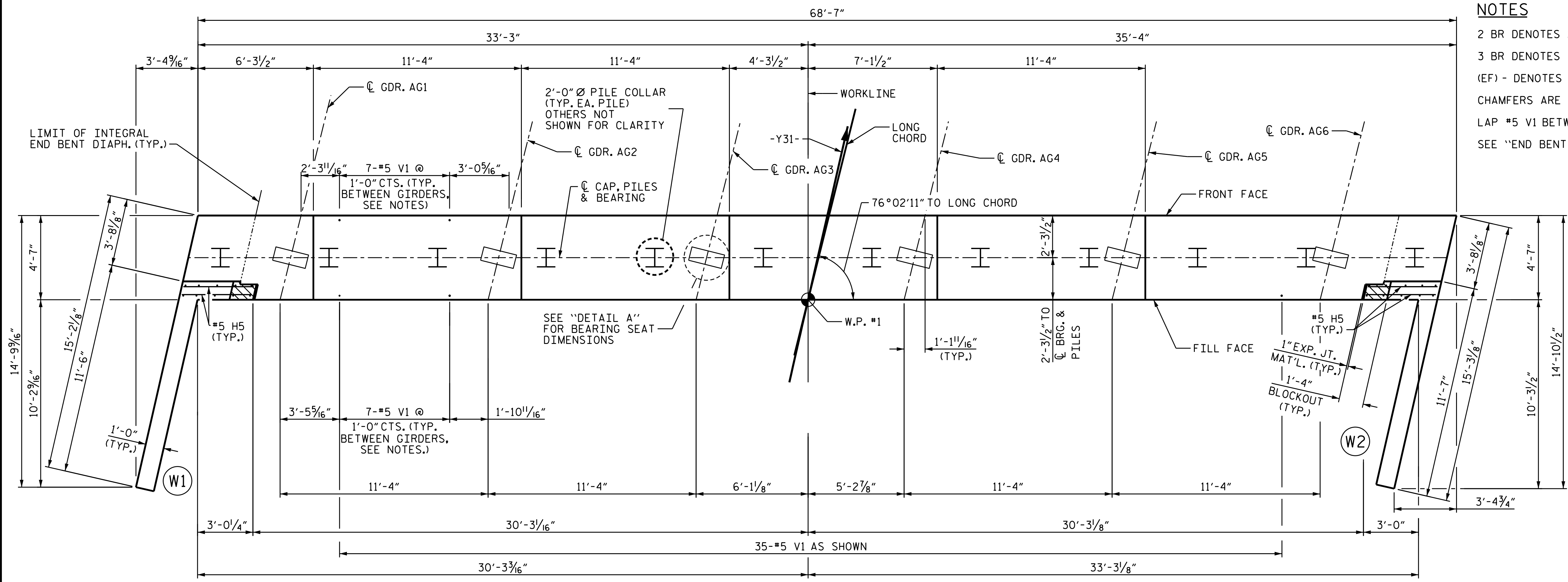
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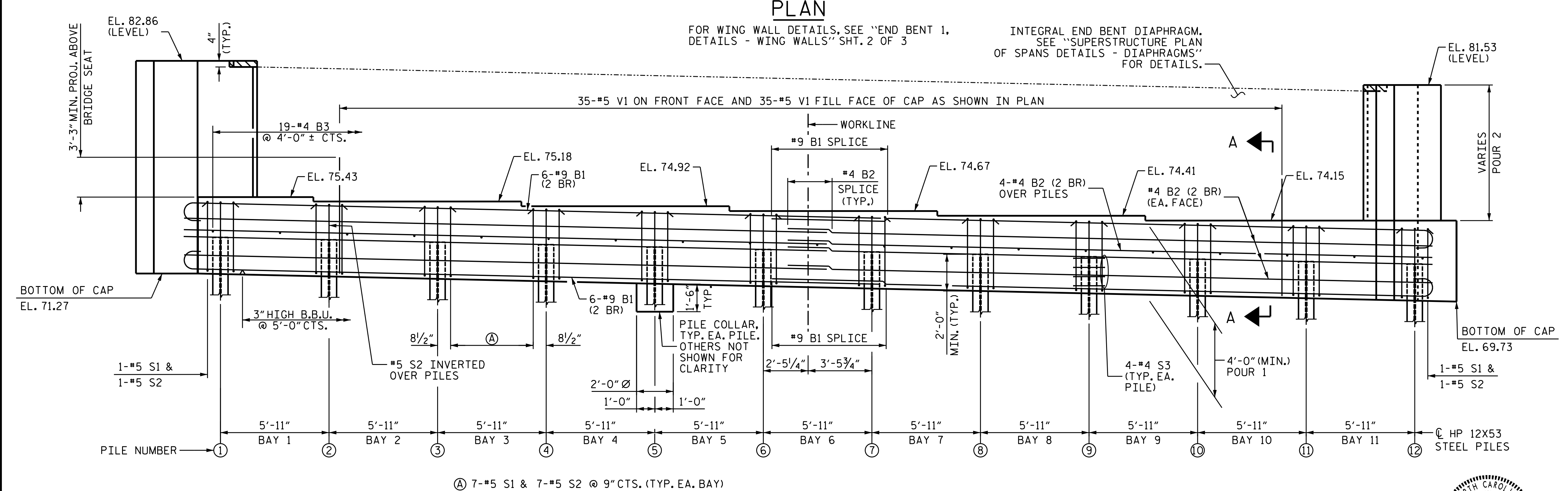
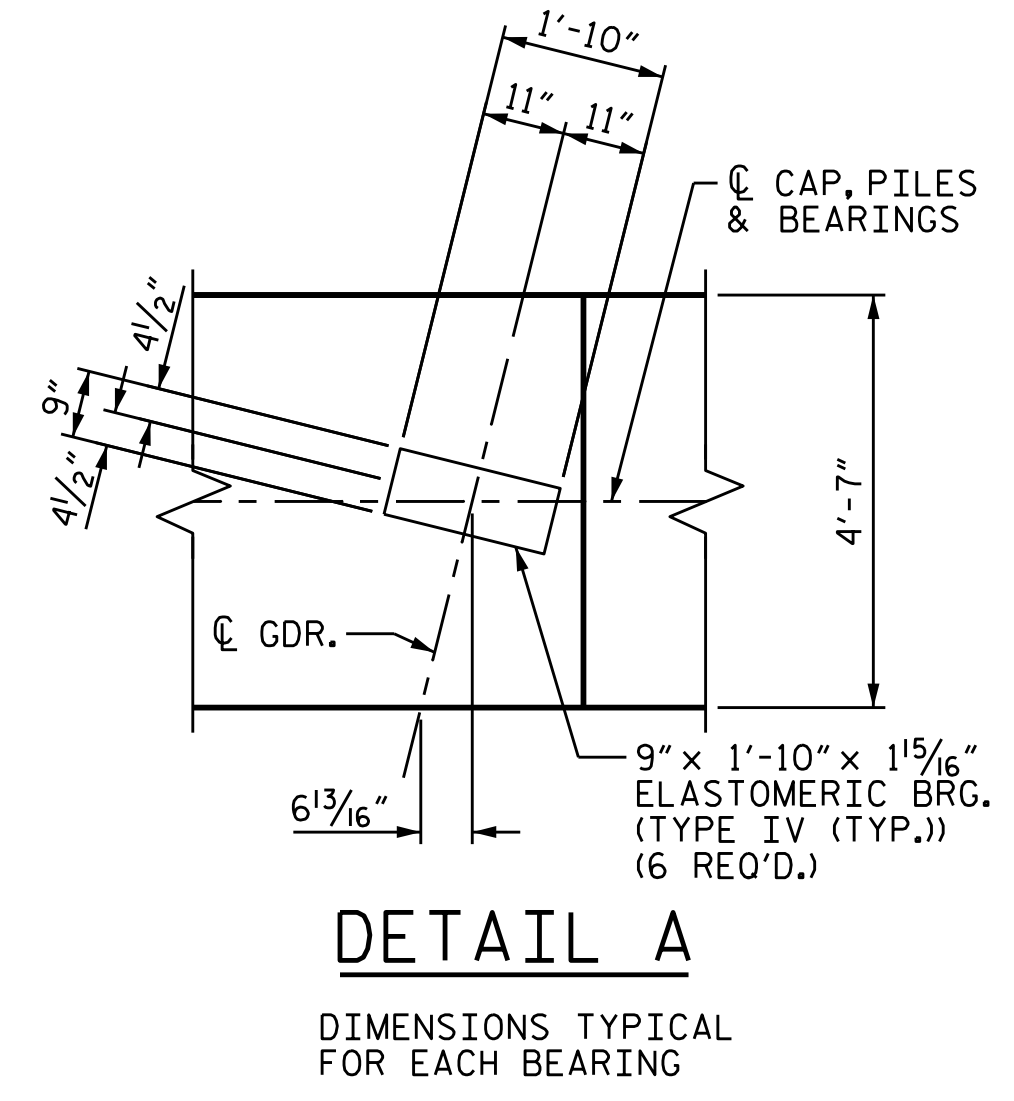
ASSEMBLED BY : J. B. GEILE	DATE : 12/20/17	MAA/GM	DESIGN ENGINEER
CHECKED BY : R. DeCOLA	DATE : 03/09/18	MAA/GM	OF RECORD: T. R. DUDECK
DRAWN BY : TLA 5/06	REV. 7/12	MAA/THC	DATE : 10/25/21
CHECKED BY : GM 5/06	REV. 6/13		
	REV. 12/17		





**NOTES**

2 BR DENOTES 2 BAR RUN  
 3 BR DENOTES 3 BAR RUN  
 (EF) - DENOTES EACH FACE  
 CHAMFERS ARE NOT REQUIRED EXCEPT AS NOTED.  
 LAP #5 V1 BETWEEN GIRDERS TO #4 U1  
 SEE "END BENT 1 DETAILS", SHEET 3 OF 3, FOR SECTION A-A.



TOP OF PILE ELEVATIONS	
NO.	EL.
1	73.22
2	73.09
3	72.96
4	72.83
5	72.70
6	72.57
7	72.43
8	72.30
9	72.17
10	72.04
11	71.91
12	71.78

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE  
 END BENT 1

REVISIONS						SHEET NO. S16-25
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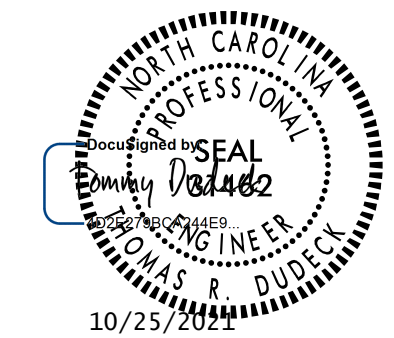
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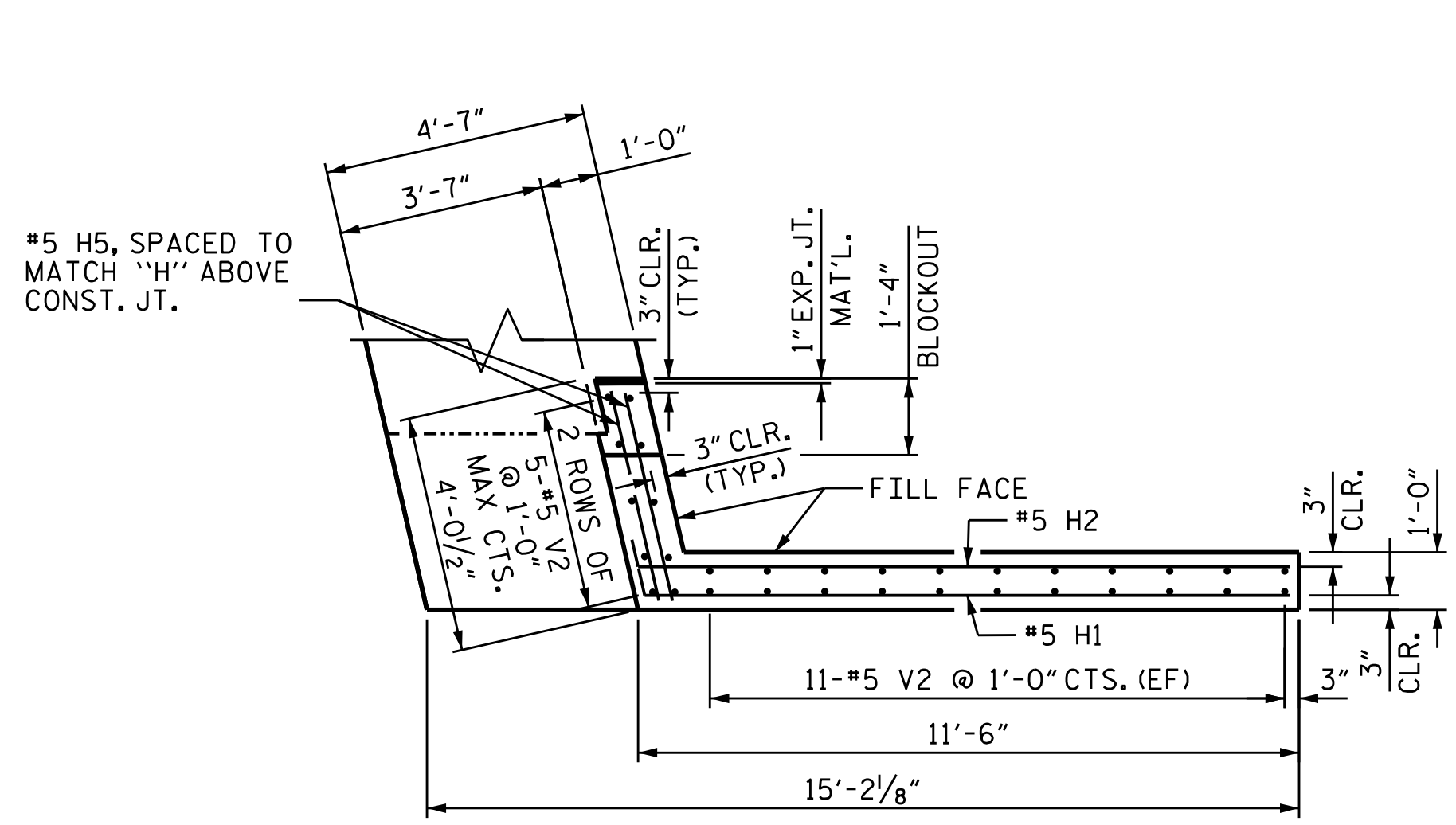
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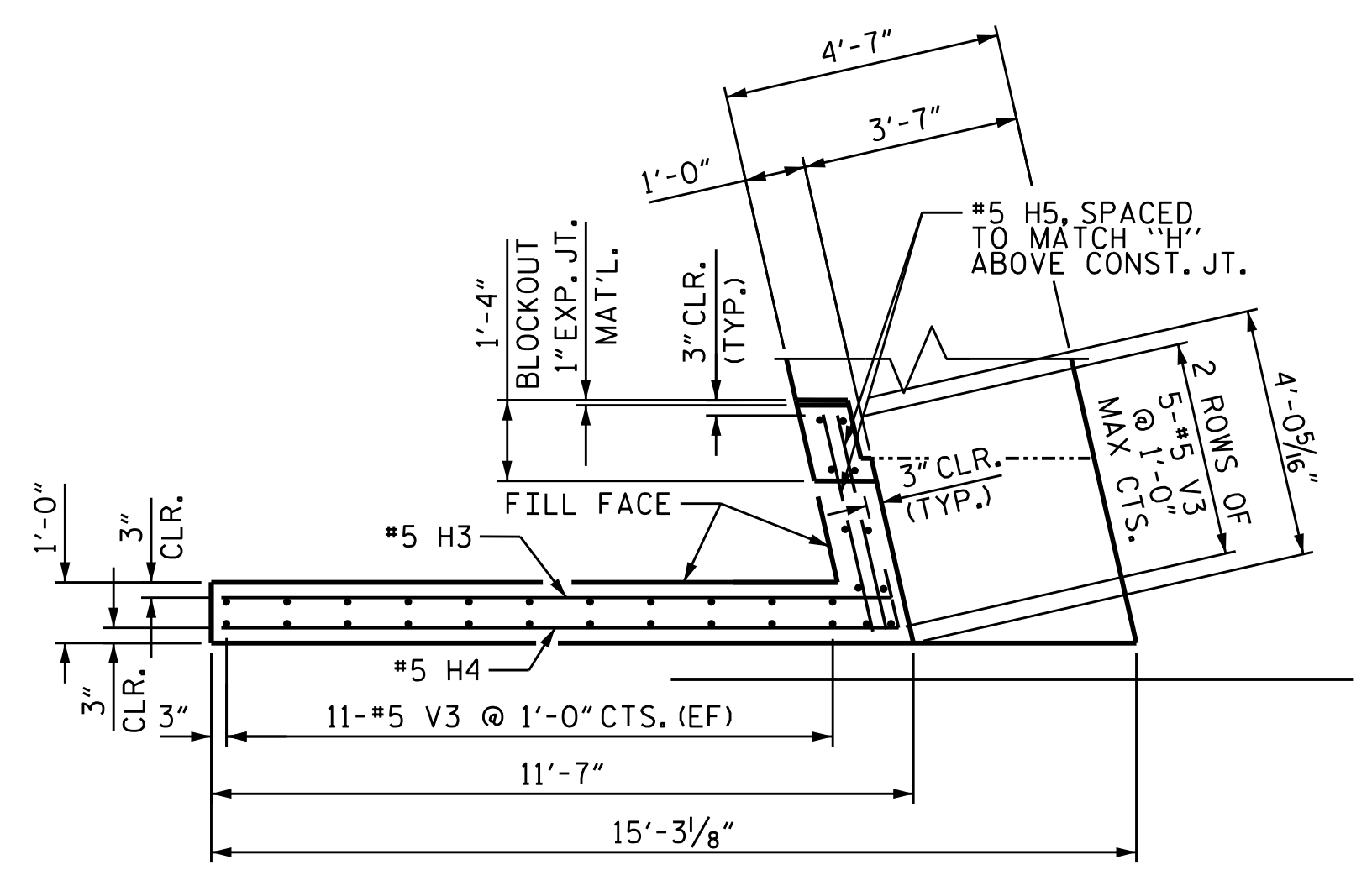
DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21



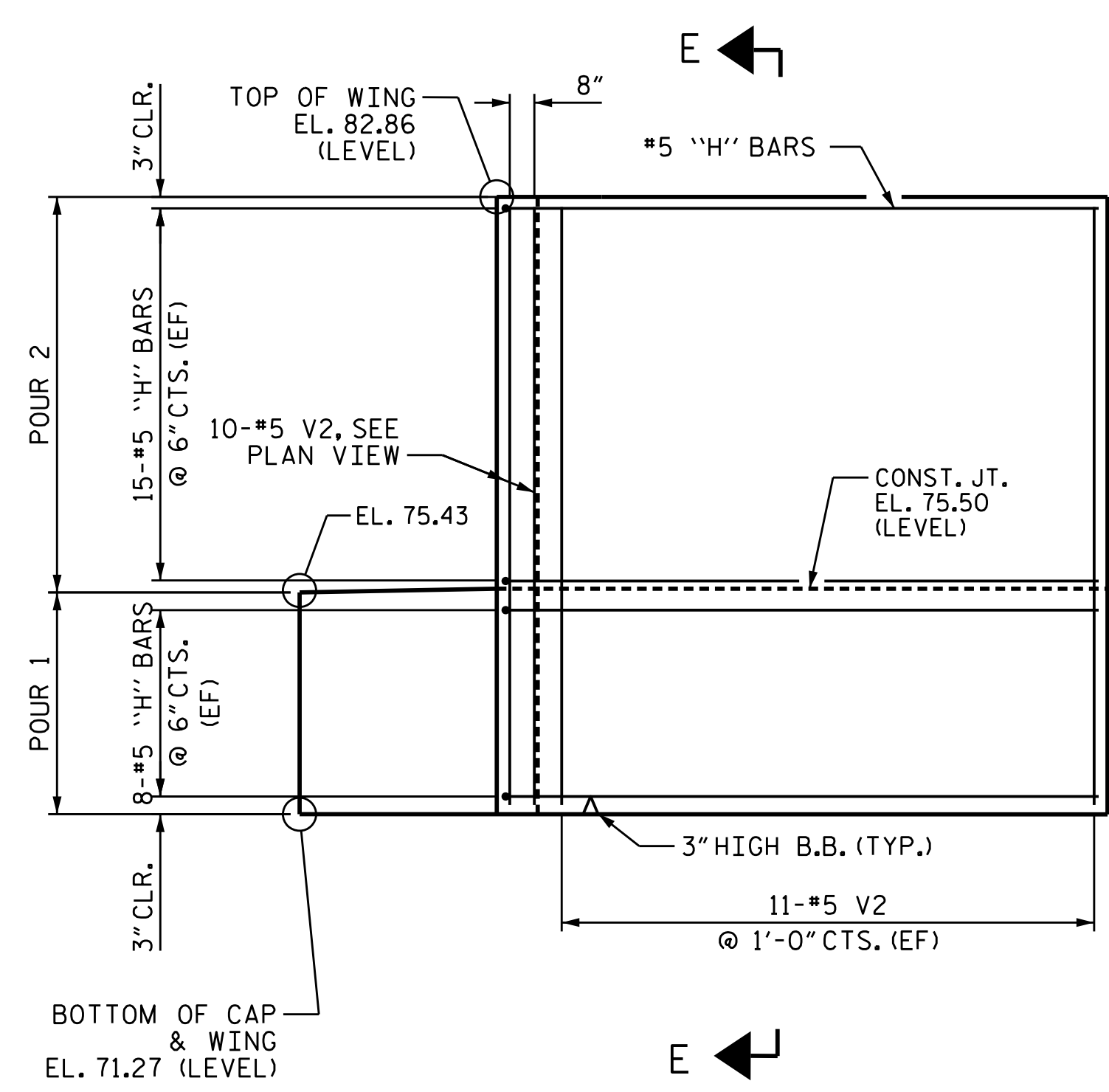
10/25/2021 10:25/2021



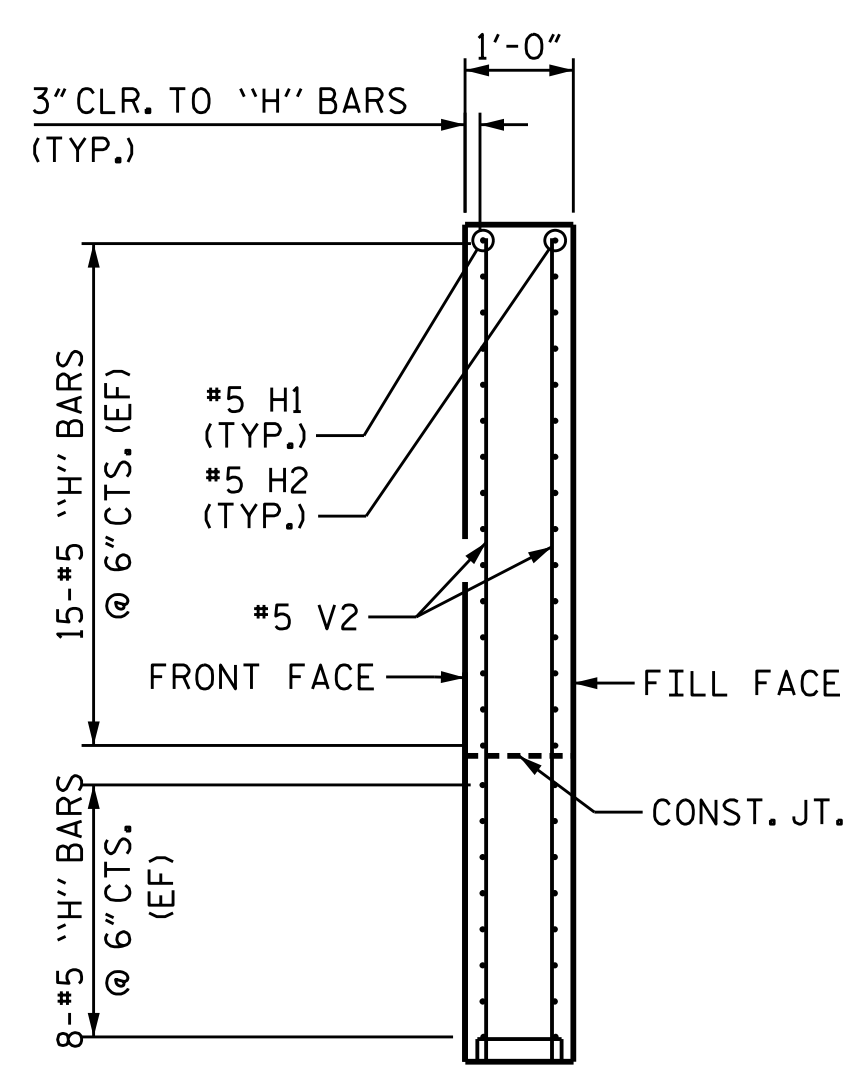
PLAN OF LEFT WING (W1)



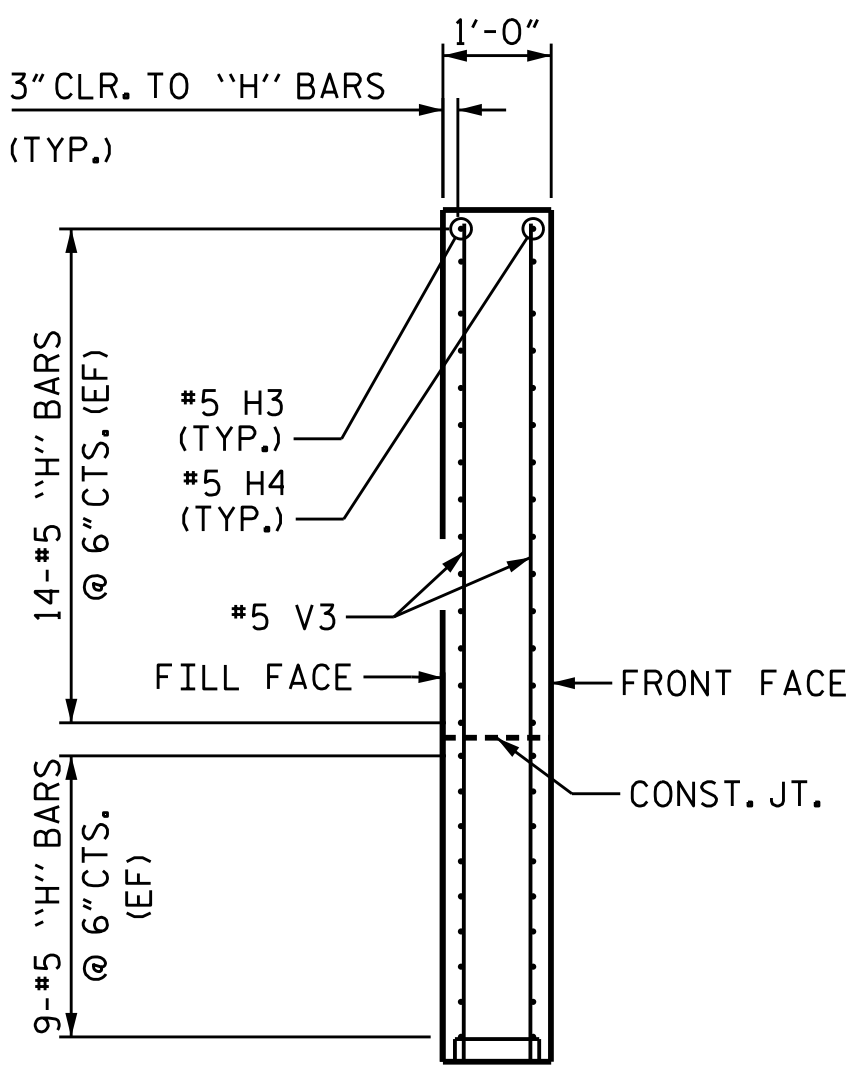
PLAN OF RIGHT WING (W2)



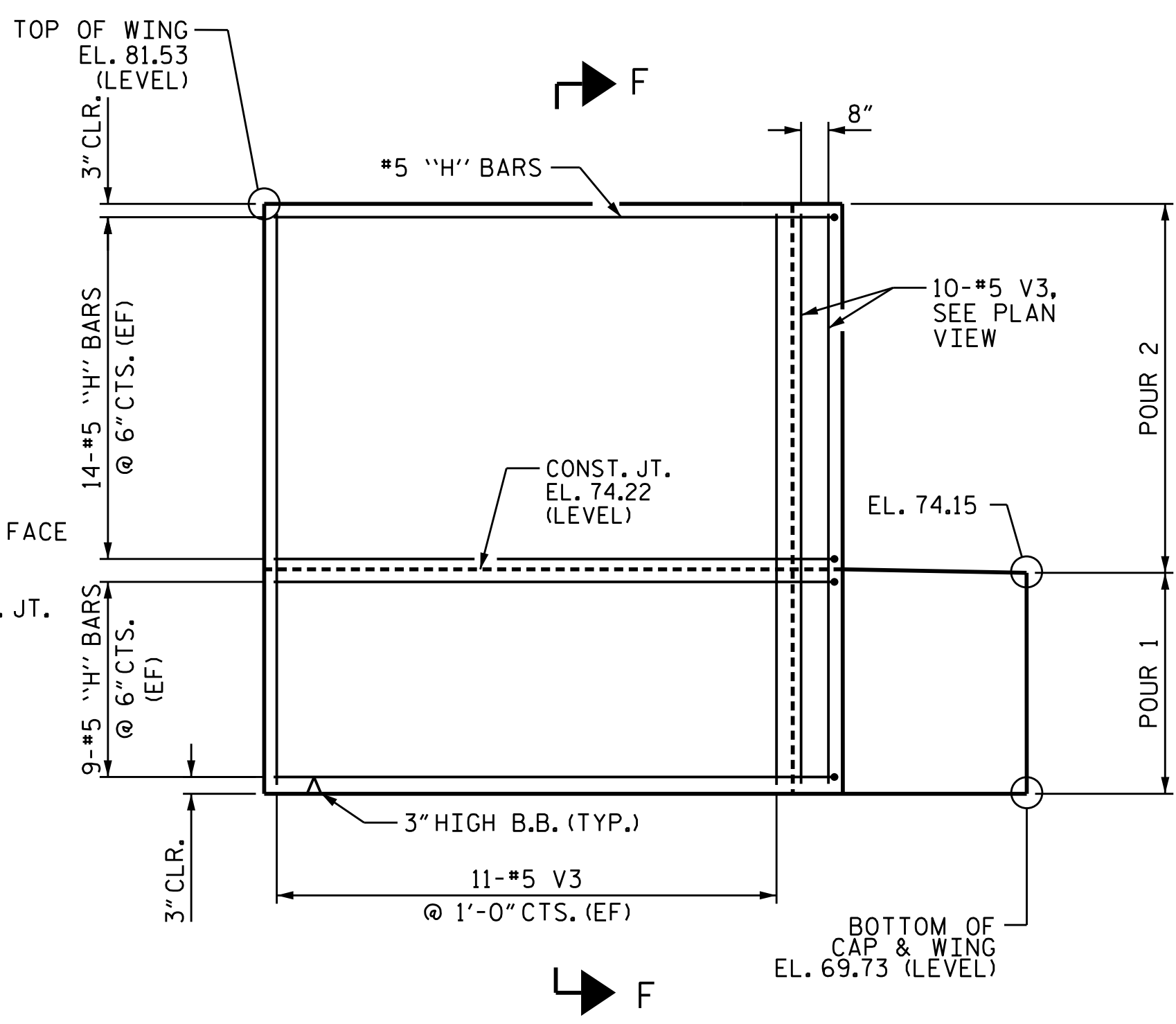
ELEVATION OF LEFT WING (W1)



SECTION E-E



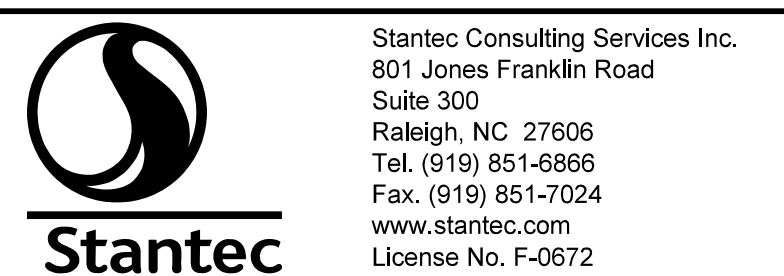
SECTION F-F



ELEVATION OF RIGHT WING (W2)

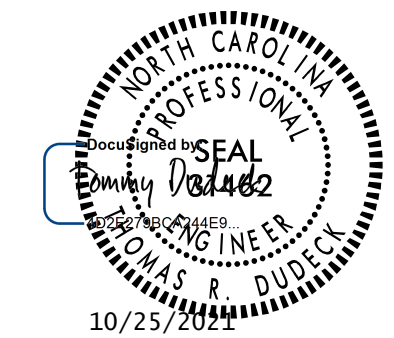
**NOTE:**  
TOP SURFACE OF END BENT CAP BETWEEN EDGE OF DECK SLAB AND END OF CAP SHALL BE SLOPED TRANSVERSELY FROM FRONT FACE TO FILL FACE AT A RATE OF 1/4\"/>

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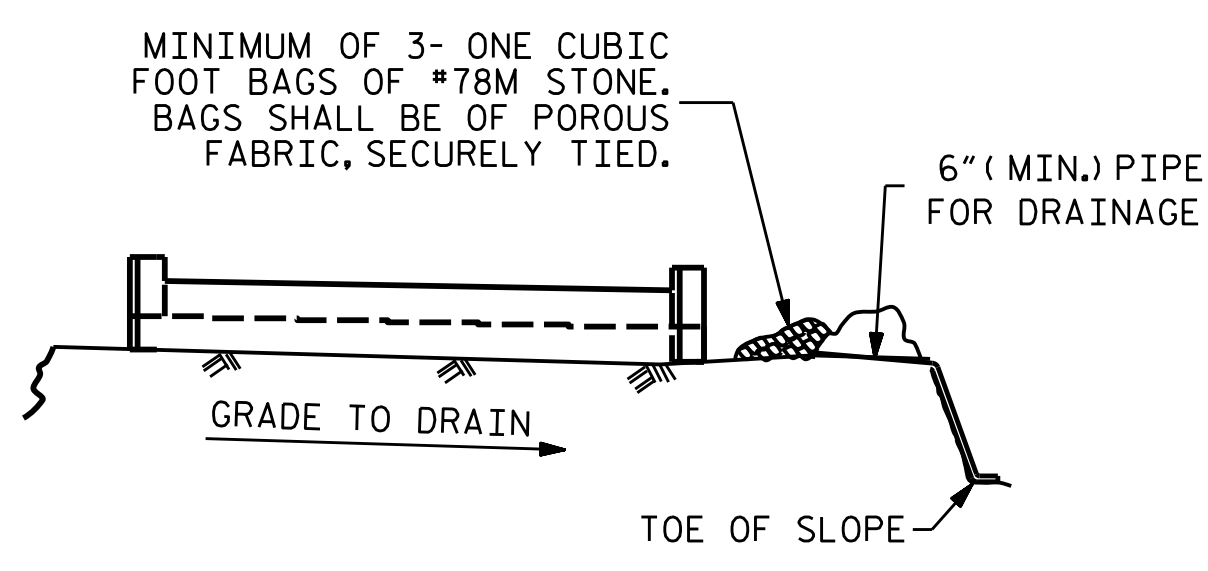
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PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 2 OF 3

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S16-26
1			3			TOTAL SHEETS
2			4			38

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUBSTRUCTURE  
END BENT 1 DETAILS  
WING WALLS

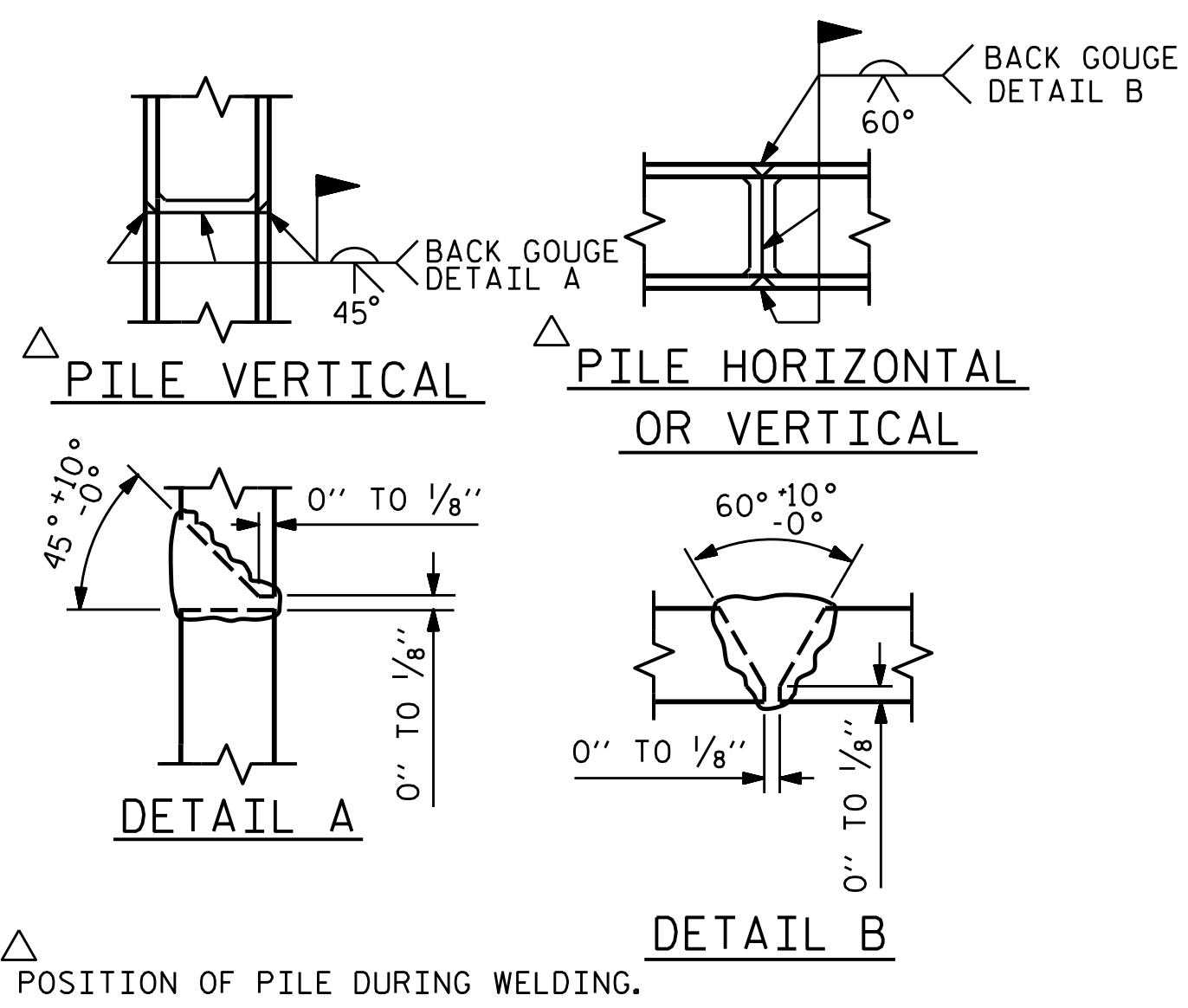


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

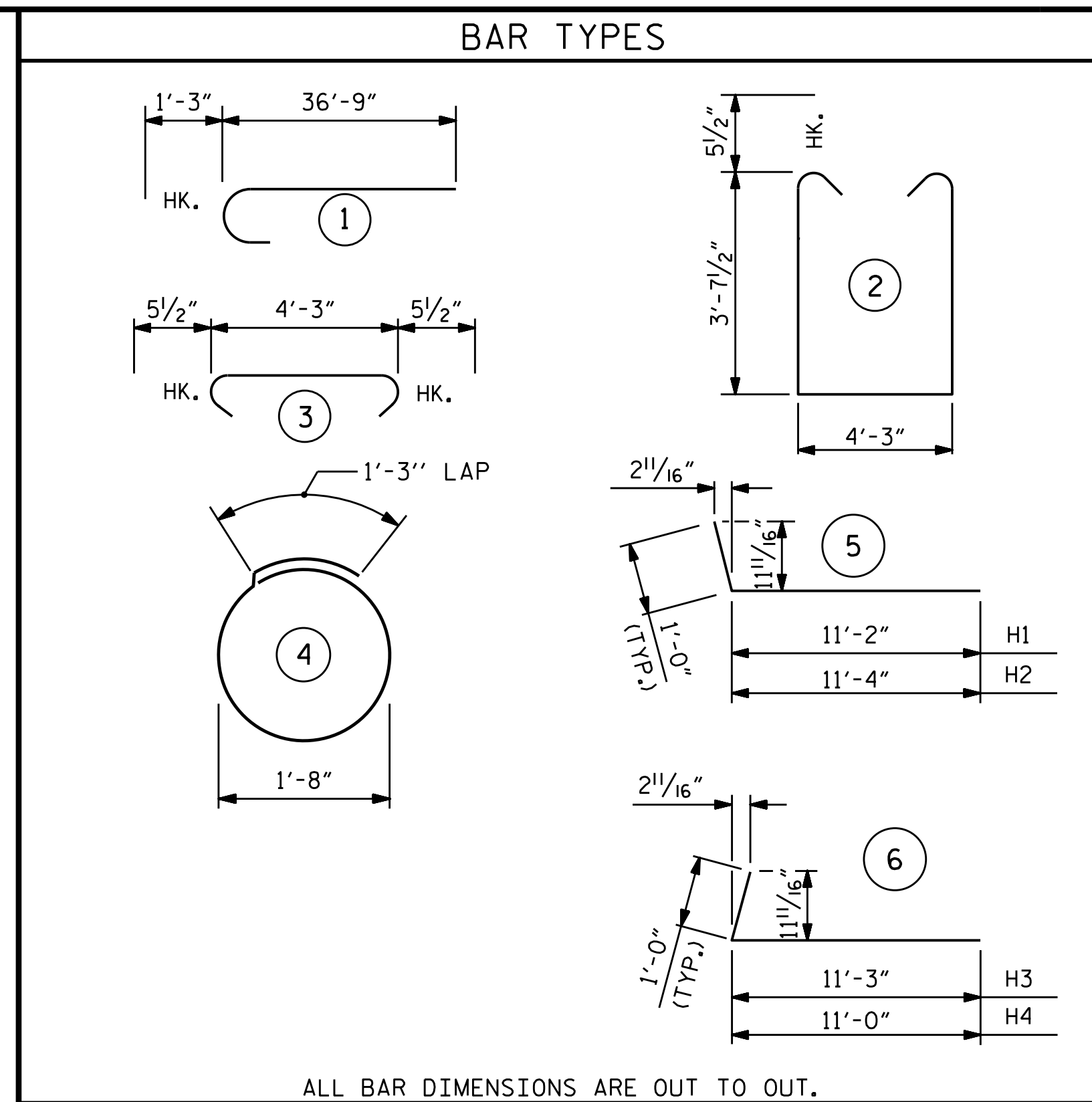
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

**TEMPORARY DRAINAGE AT END BENT**

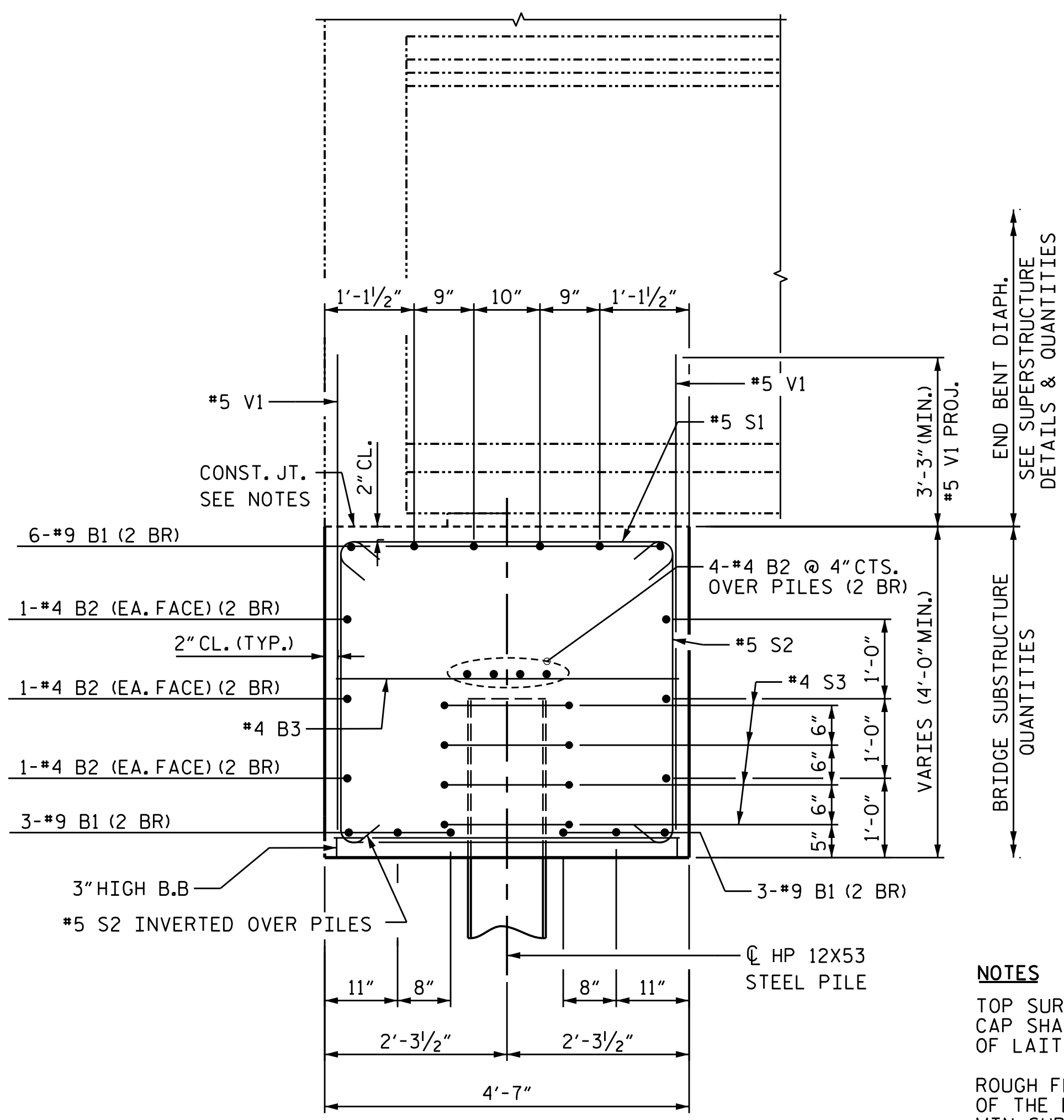


**PILE SPLICE DETAILS**



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL					
END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	24	#9	1	36'-10"	3006
B2	20	#4	STR	35'-4"	472
B3	19	#4	STR	4'-3"	54
H1	23	#5	5	12'-2"	292
H2	23	#5	5	12'-4"	296
H3	23	#5	6	12'-0"	288
H4	23	#5	6	12'-3"	294
H5	60	#5	STR	3'-6"	219
S1	79	#5	3	5'-2"	426
S2	91	#5	2	12'-5"	1179
S3	48	#4	4	6'-6"	208
V1	70	#5	STR	7'-6"	548
V2	32	#5	STR	11'-3"	375
V3	32	#5	STR	11'-5"	381
REINFORCING STEEL					LBS. 8,038
CLASS A CONCRETE BREAKDOWN:					
POUR #1: CAP, COLLARS, ETC.					C.Y. 54.2
POUR #2: UPPER WINGS					C.Y. 7.9
CLASS A CONCRETE TOTAL					C.Y. 62.1
HP12X53 PILES					
NO. 12					LIN. FT. 960.00
PILE REDRIVES					EA. 6
PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES					EA. 12



**SECTION A-A**  
SEE 'END BENT 1' SHEET 1 OF 3

**NOTES**  
TOP SURFACE AREAS OF THE END BENT CAP SHALL BE KEPT CLEAN AND FREE OF LAITANCE.

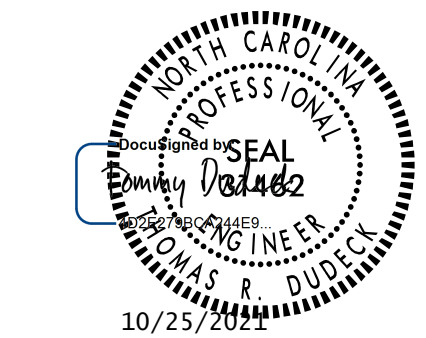
ROUGH FLOAT AND ROUGHEN THE TOP OF THE END BENT CAP TO PROVIDE MIN. SURFACE AMPLITUDE OF 1/4\"/>

2 BR DENOTES 2 BAR RUN.  
3 BR DENOTES 3 BAR RUN.

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUBSTRUCTURE  
**END BENT 1 DETAILS**



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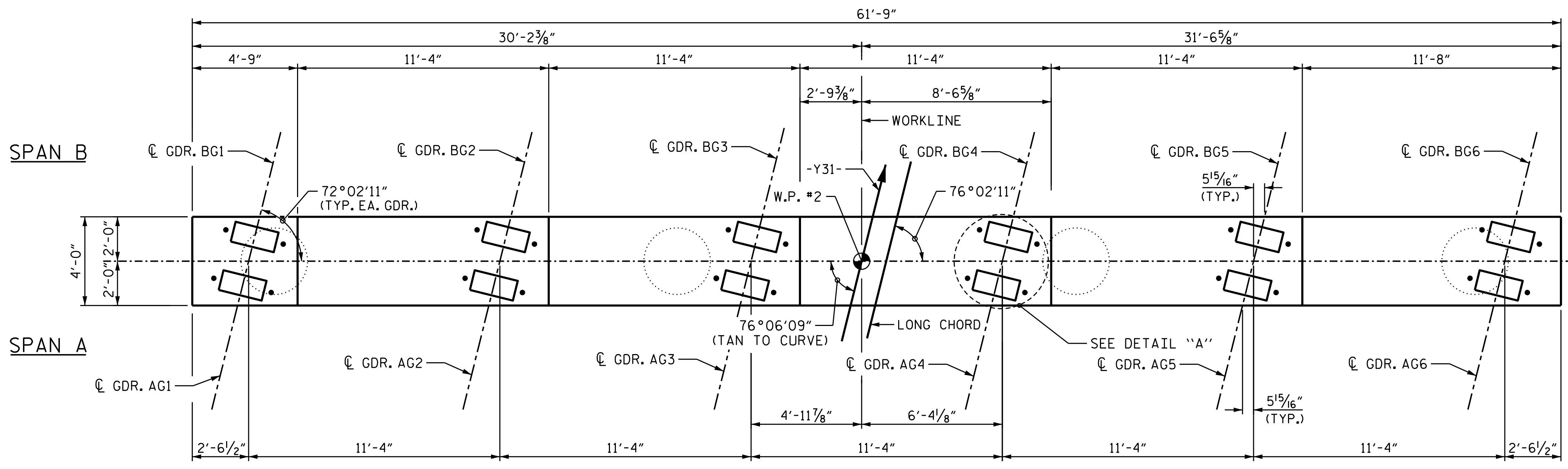
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1			3			TOTAL SHEETS
2			4			38

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DESIGN ENGINEER OF RECORD : T. R. DUDECK DATE : 10/25/21

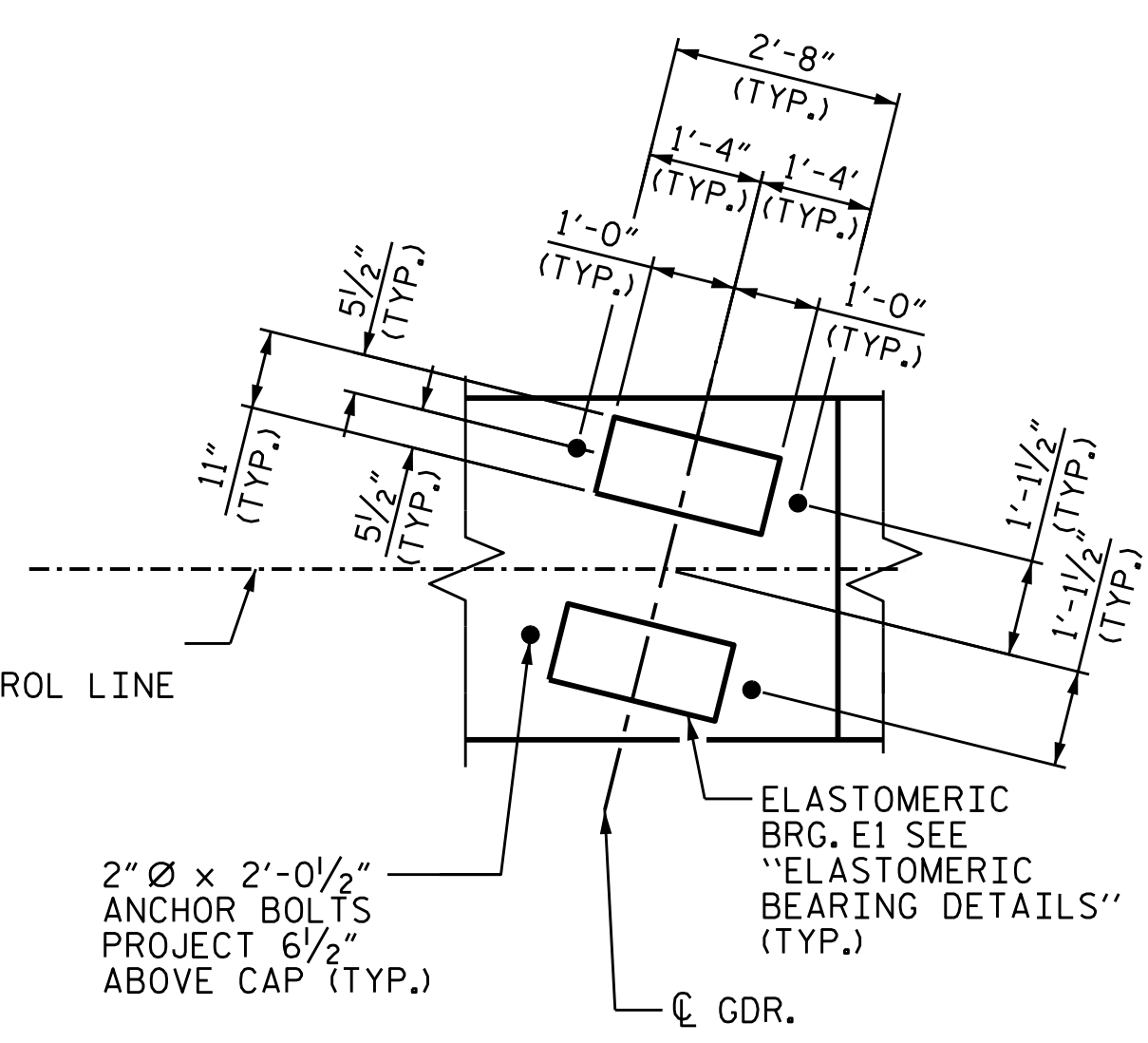
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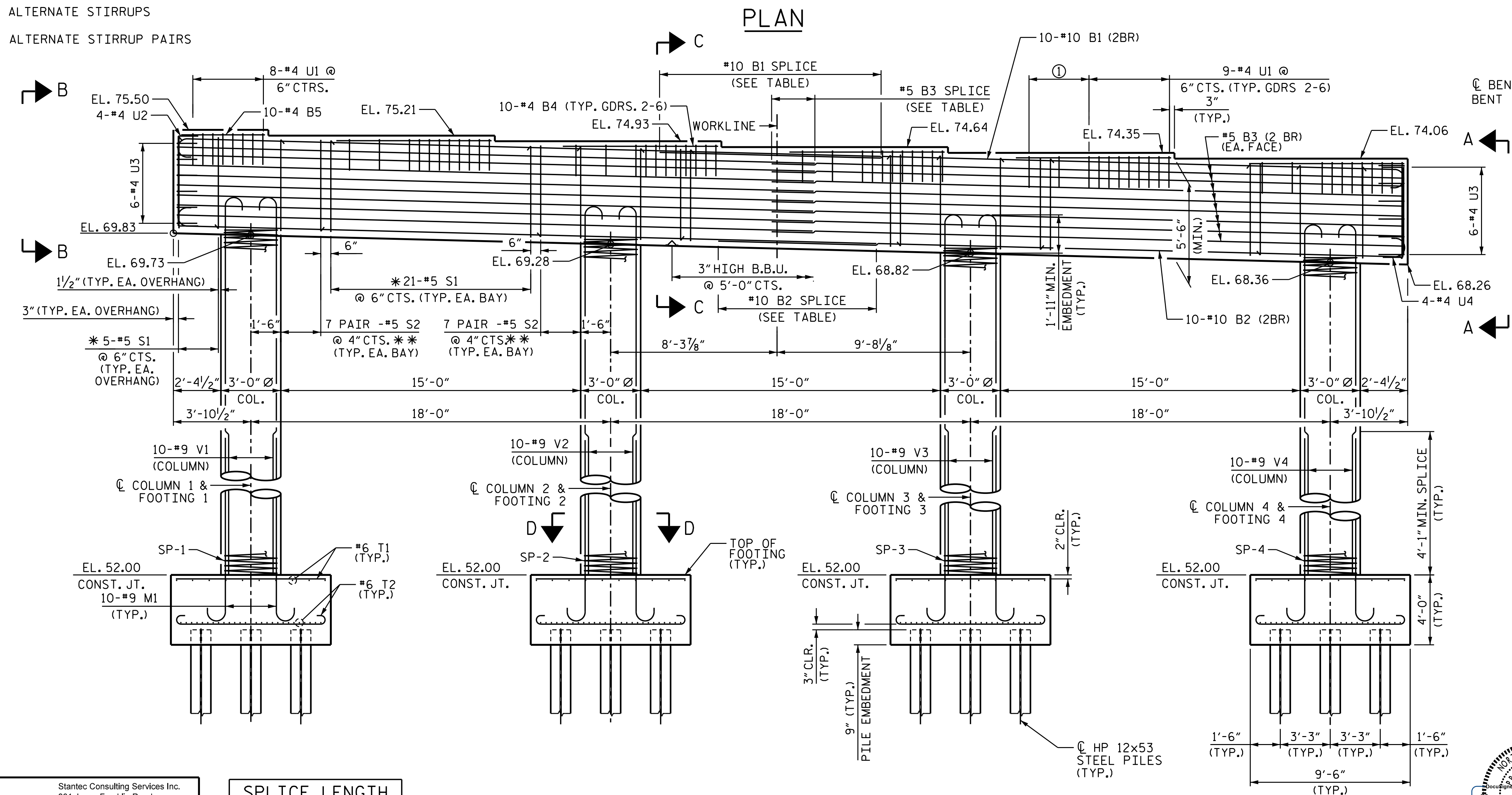


**NOTES:**  
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS AND COLUMN REINFORCEMENT.  
 HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINF. STEEL.  
 FOR VIEW A-A, VIEW B-B, AND SECTION C-C, SEE BENT 1 SHEET 3 OF 3.  
 2BR DENOTES 2 BAR RUN.  
 FOR SECTION D-D, SEE BENT 1 SHEET 2 OF 3.  
 ① 2-#4 U1 @ 1'-6" CTS. (TYP. GDRS 2-6)

\* INVERT ALTERNATE STIRRUPS  
 \*\* INVERT ALTERNATE STIRRUP PAIRS



**DETAIL "A"**  
 DIMENSIONS TYPICAL FOR EACH BEARING



**ELEVATION**

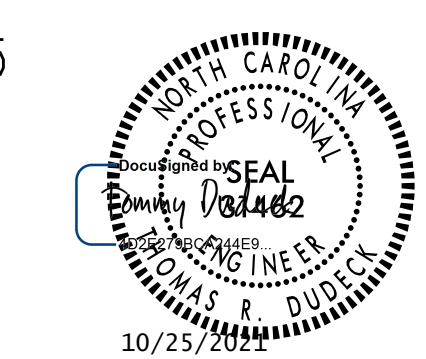
SPLICE LENGTH	
#10 B1	6'-0"
#10 B2	4'-7"
#5 B3	3'-0"

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**SUBSTRUCTURE**

**BENT 1**



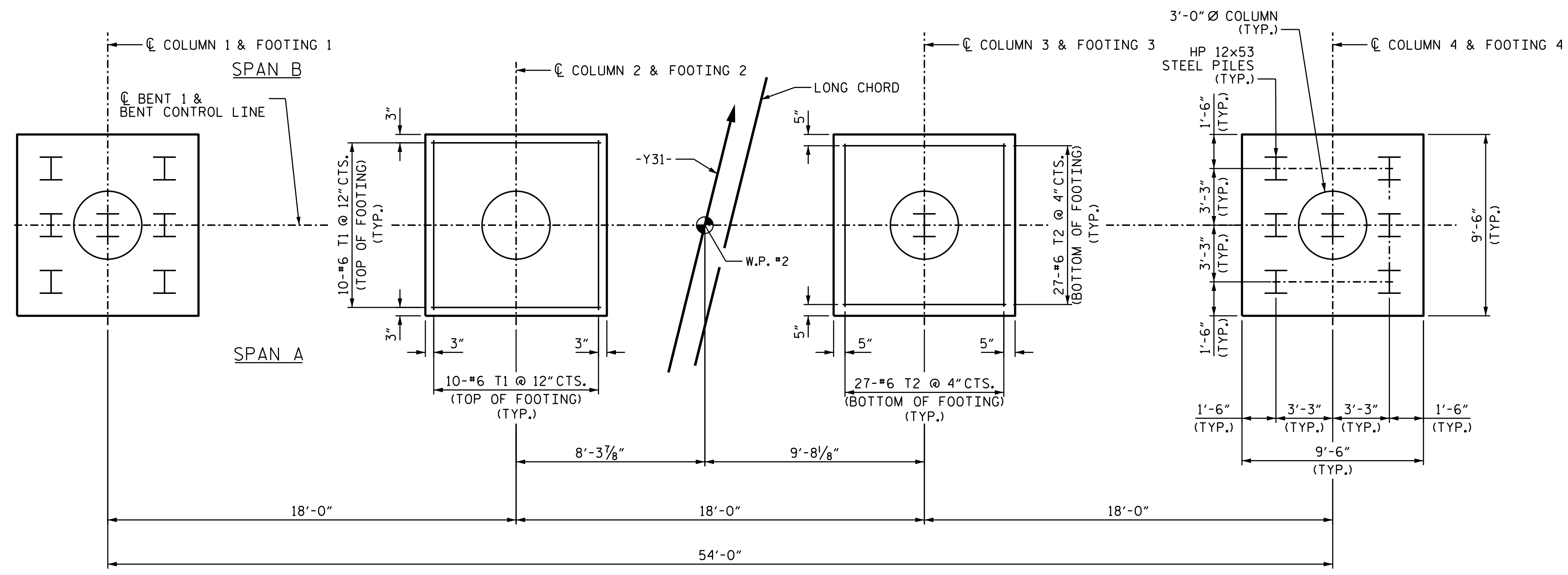
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2			4			

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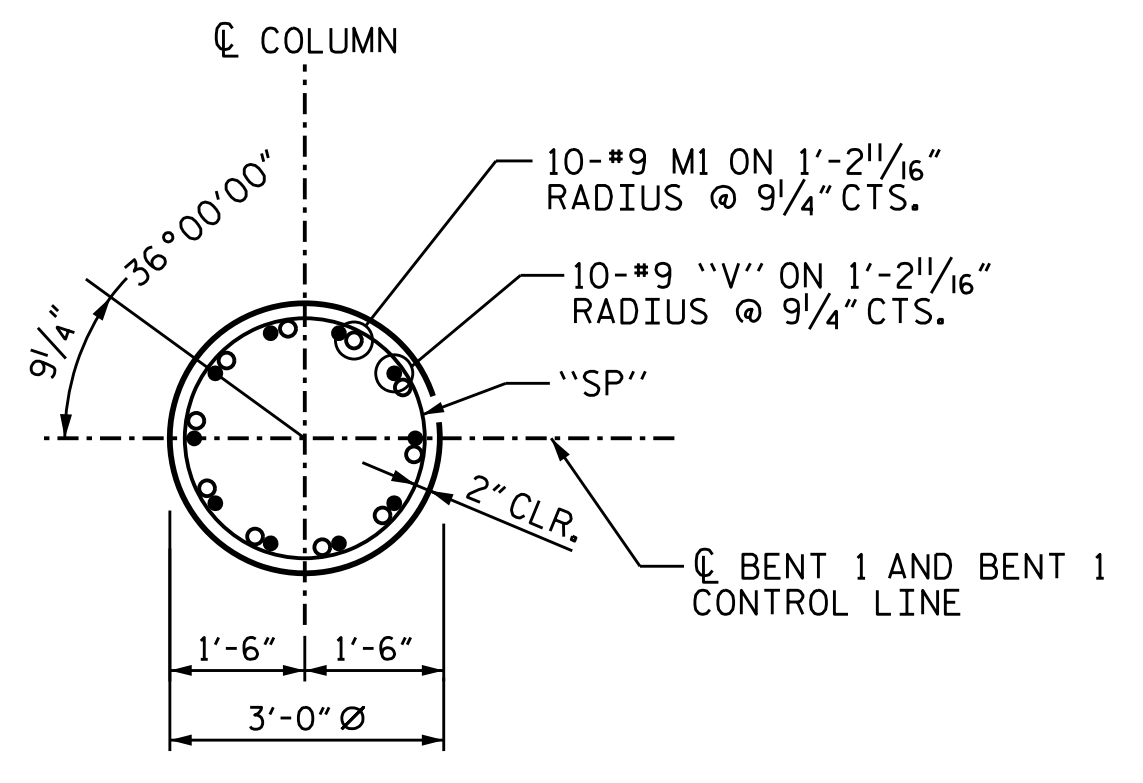
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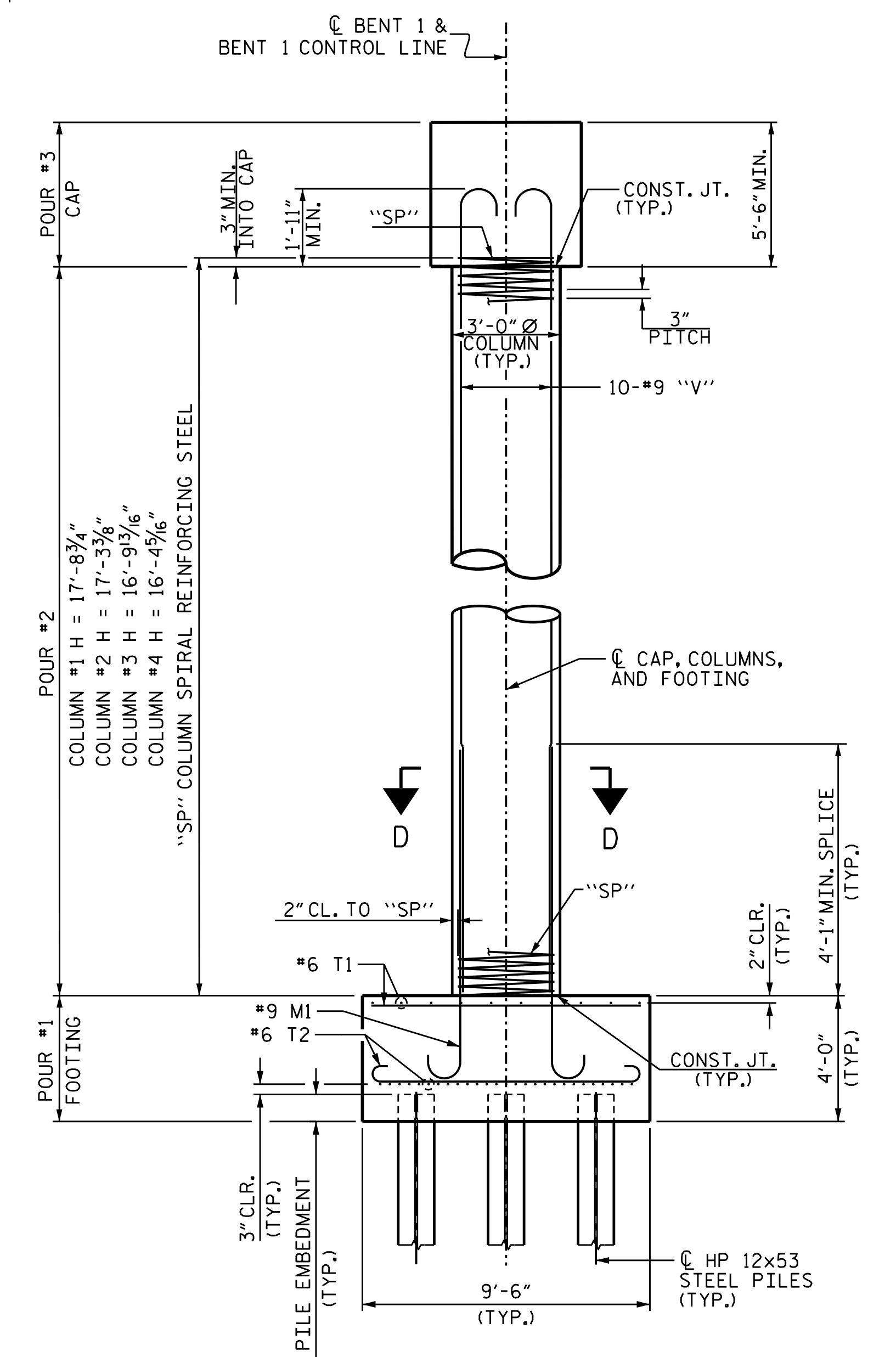


PLAN OF FOOTINGS

ALL DIMENSIONS AND DETAILS SHOWN FOR FOOTINGS ARE TYPICAL FOR EACH FOOTING UNLESS NOTED OTHERWISE.



SECTION D-D



END ELEVATION

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE

**BENT 1 DETAILS**



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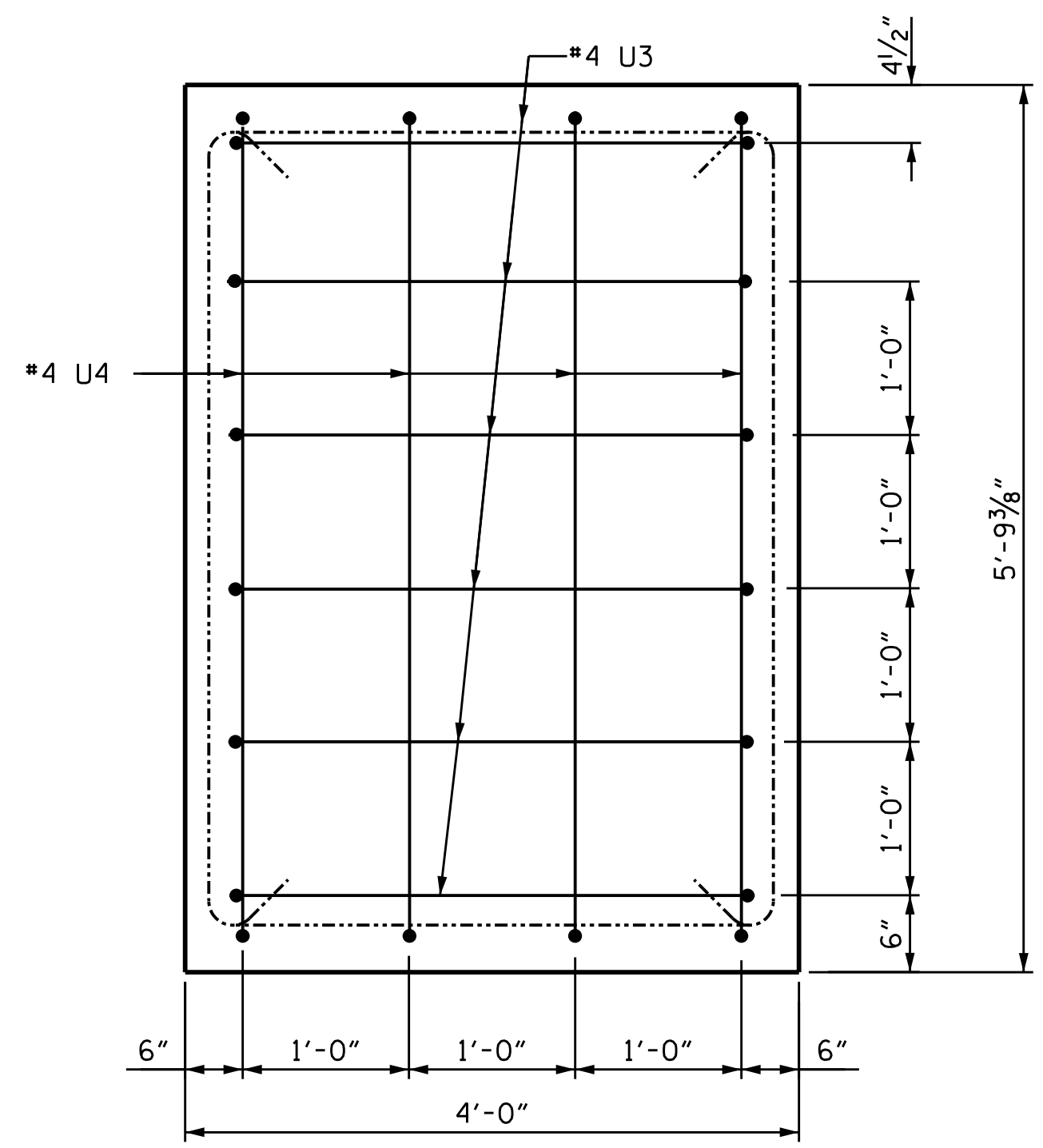
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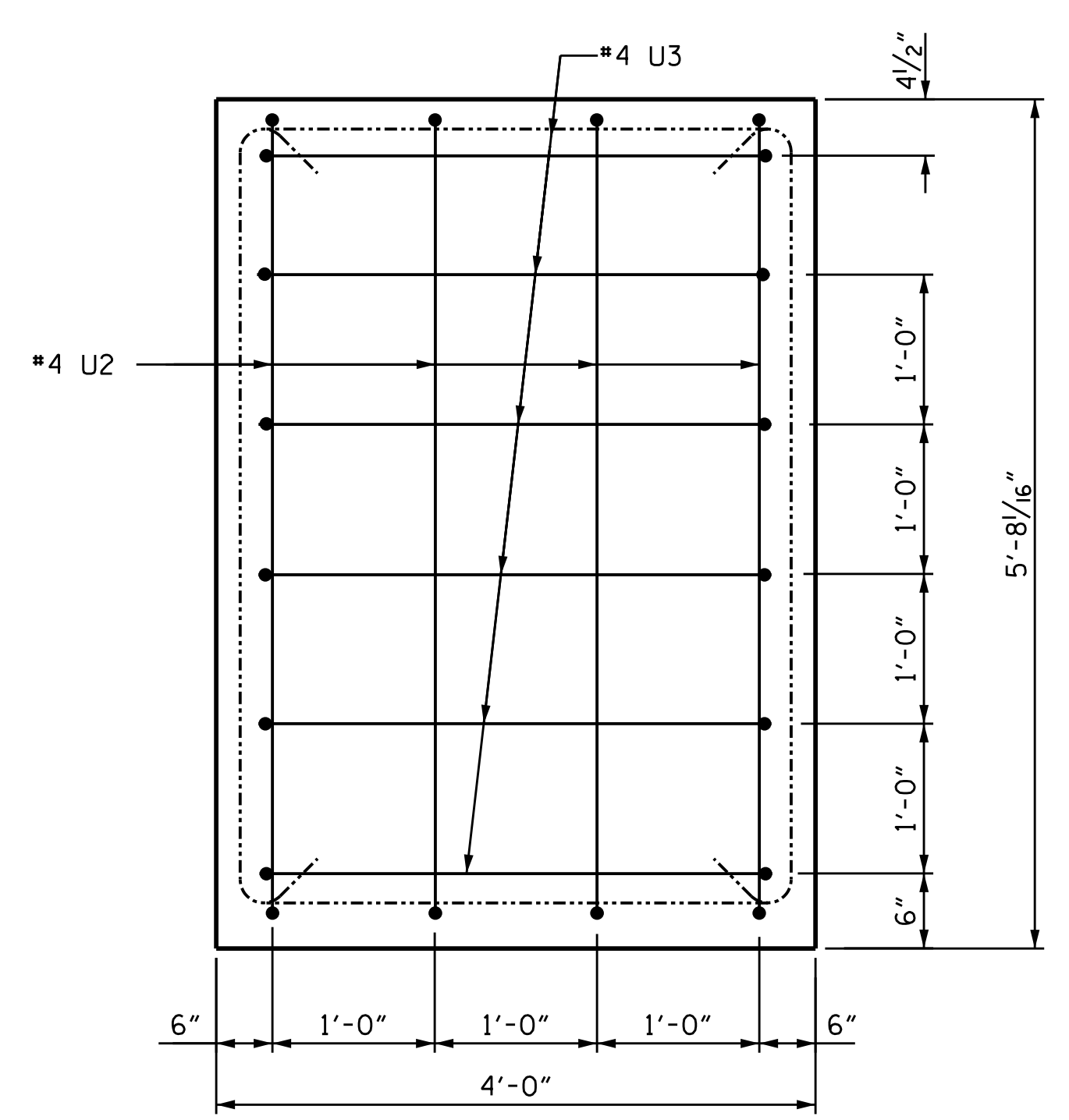
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 CHECKED BY : R. DeCOLA DATE : 03/20/18  
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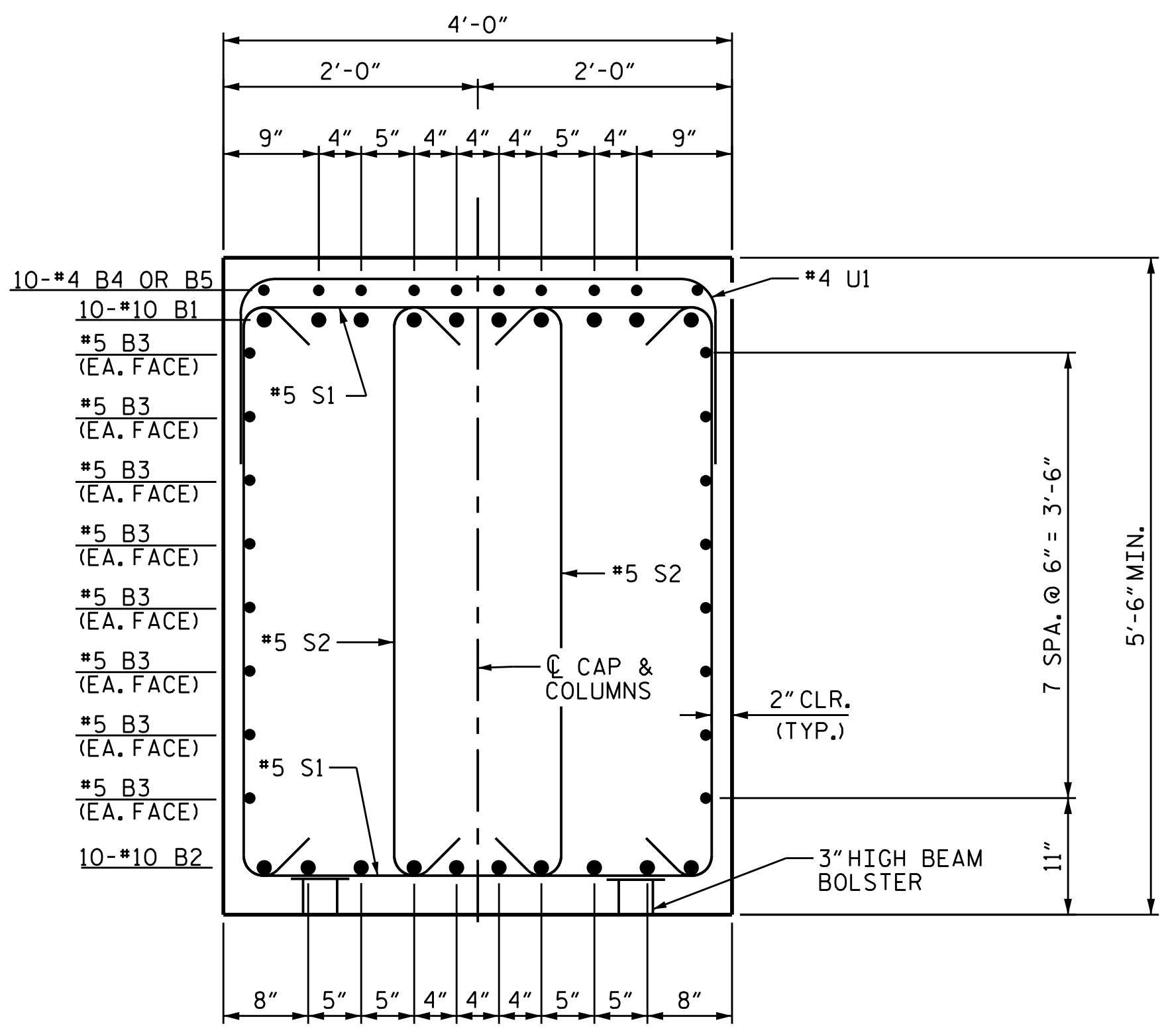
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VIEW A-A



VIEW B-B



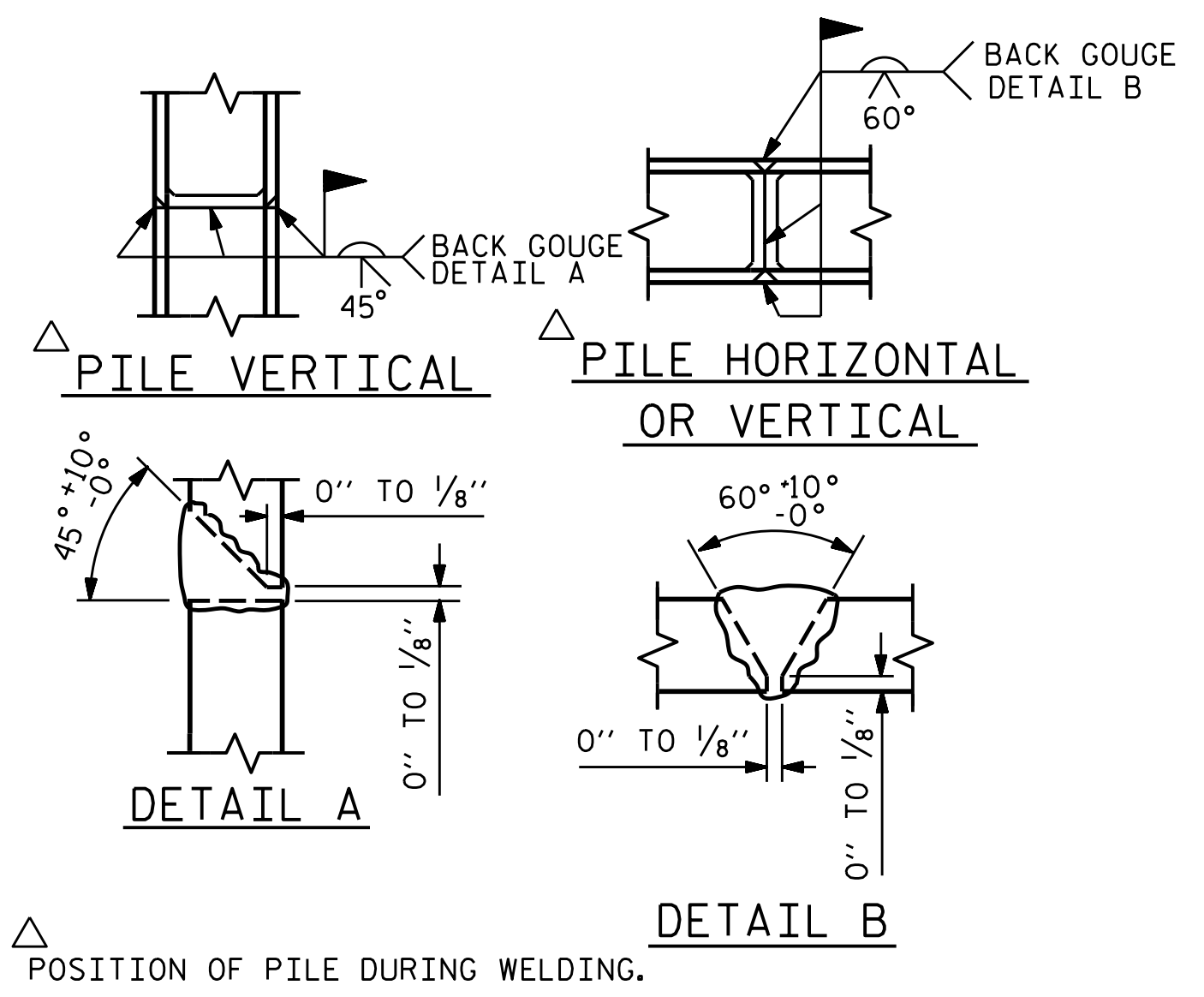
SECTION C-C

BAR TYPES

B1	1'-5"	33'-9"
B2	1'-5"	33'-0"
M1	1'-3"	6'-11"
V1	1'-3"	19'-8"
V2	1'-3"	19'-3"
V3	1'-3"	18'-9"
V4	1'-3"	18'-4"

ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL					
BENT #1					
BAR NO.	SIZE	TYPE	LENGTH	WEIGHT	
B1	20	#10	1	35'-2"	3026
B2	20	#10	1	34'-5"	2962
B3	32	#5	STR	32'-0"	1068
B4	50	#4	STR	7'-2"	239
B5	10	#4	STR	4'-5"	30
M1	40	#9	1	8'-2"	1111
S1	73	#5	2	14'-11"	1136
S2	84	#5	2	13'-9"	1205
T1	80	#6	STR	9'-0"	1081
T2	216	#6	4	10'-4"	3352
U1	63	#4	3	6'-8"	281
U2	4	#4	3	8'-5"	22
U3	12	#4	3	7'-6"	60
U4	4	#4	3	8'-5"	22
V1	10	#9	1	20'-11"	711
V2	10	#9	1	20'-6"	697
V3	10	#9	1	20'-0"	680
V4	10	#9	1	19'-7"	666
REINFORCING STEEL					LBS. 18,349
SPIRAL COLUMN					LBS. 1,604
CLASS A CONCRETE BREAKDOWN					
POUR #1 FOOTINGS				C. Y.	53.5
POUR #2 COLUMNS				C. Y.	18.4
POUR #3 CAP				C. Y.	51.7
TOTAL CLASS A CONC.				C. Y.	123.6
HP 12X53 STEEL PILES			NO.	28	
			LIN. FT.	1680.00	
PILE DRIVING EQUIPMENT			EA.	28	
SETUP FOR HP 12X53 STEEL PILES					
FOUNDATION EXCAVATION			LUMP SUM		
STEEL PILE POINTS			EA.	28	
PILE REDRIVES			EA.	14	



PILE SPLICE DETAILS

PROJECT NO. R-3300B  
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 STATION: 30+17.11 -Y31-

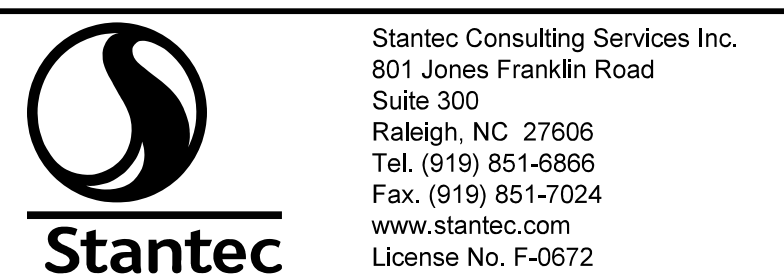
SHEET 3 OF 3

STATE OF NORTH CAROLINA  
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 RALEIGH  
 SUBSTRUCTURE

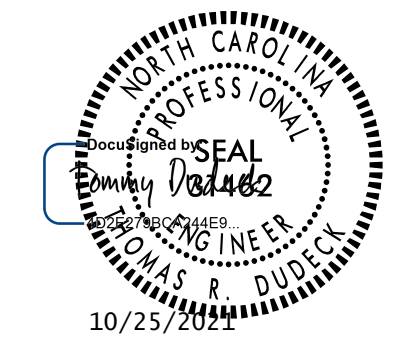
BENT 1 DETAILS

REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
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2			4		

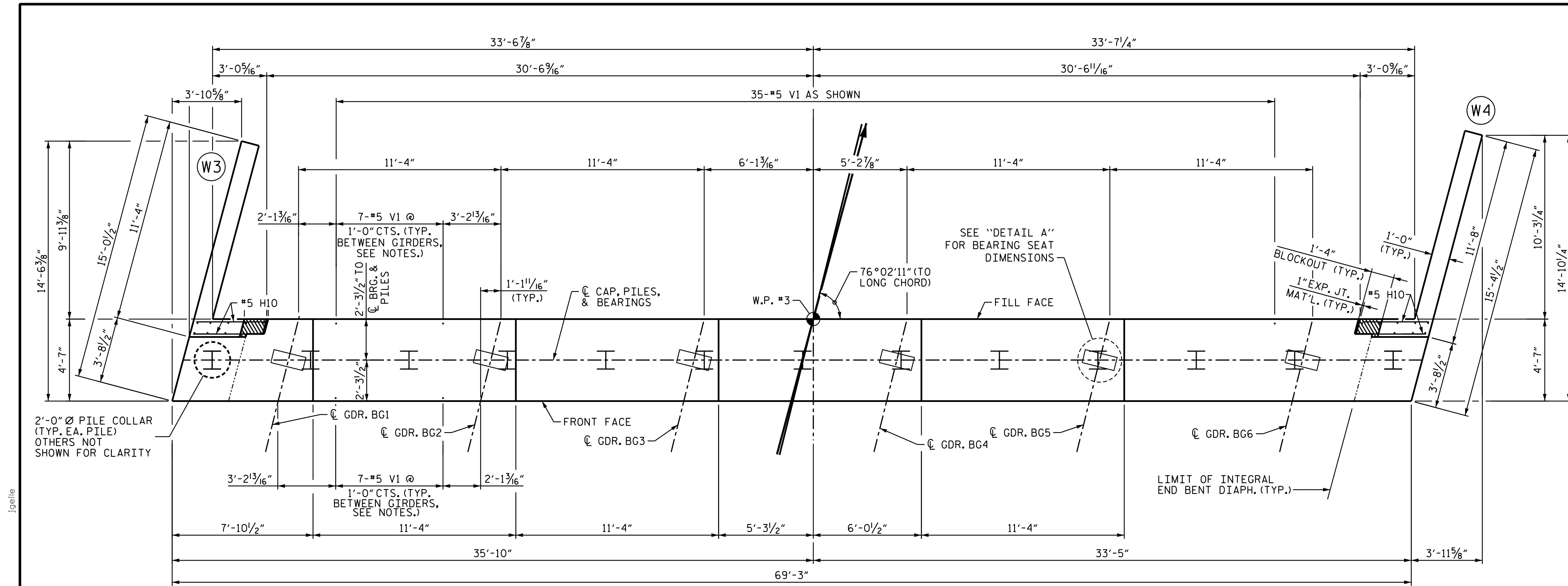
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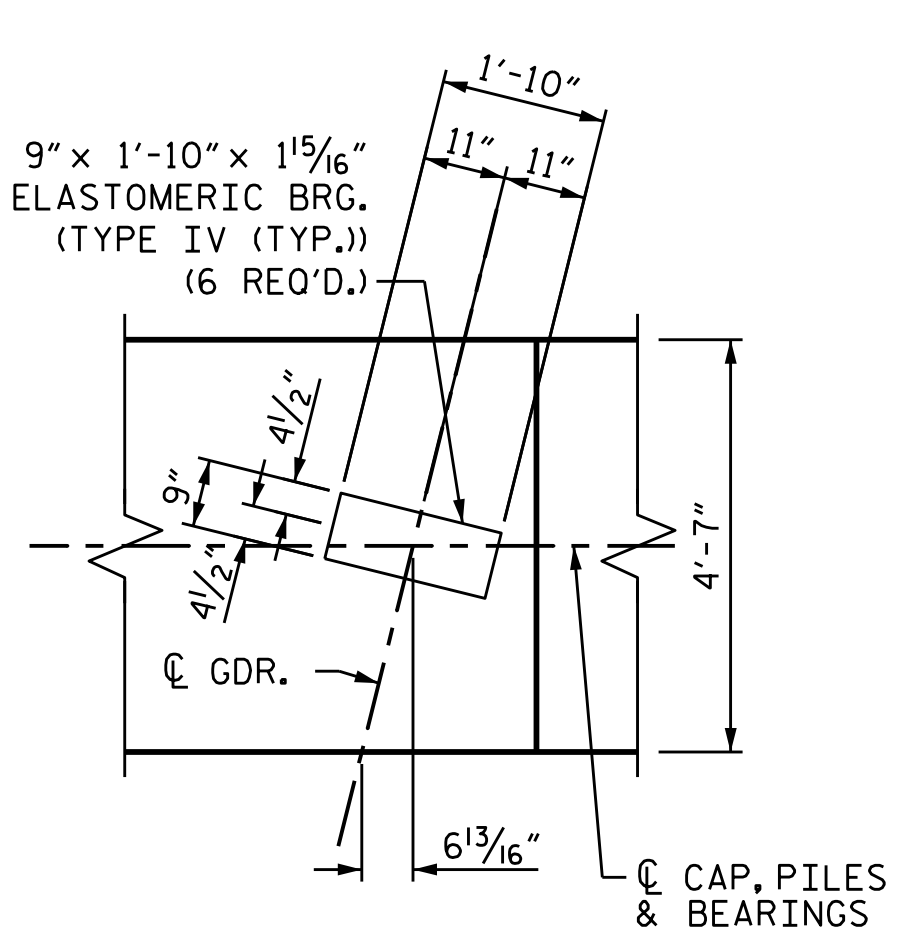
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 CHECKED BY: R. DeCOLA DATE: 03/20/18 OF RECORD: T. R. DUDECK DATE: 10/25/21



10/25/2021



**NOTES**  
 2 BR DENOTES 2 BAR RUN  
 3 BR DENOTES 3 BAR RUN  
 (EF) - DENOTES EACH FACE  
 CHAMFERS ARE NOT REQUIRED EXCEPT AS NOTED.  
 LAP #5 V1 BETWEEN GIRDERS TO #4 U1  
 TIE #5 V1 AT GIRDERS TO #4 "K" BARS  
 SEE "END BENT 2 DETAILS", SHEET 3 OF 3, FOR SECTION A-A.

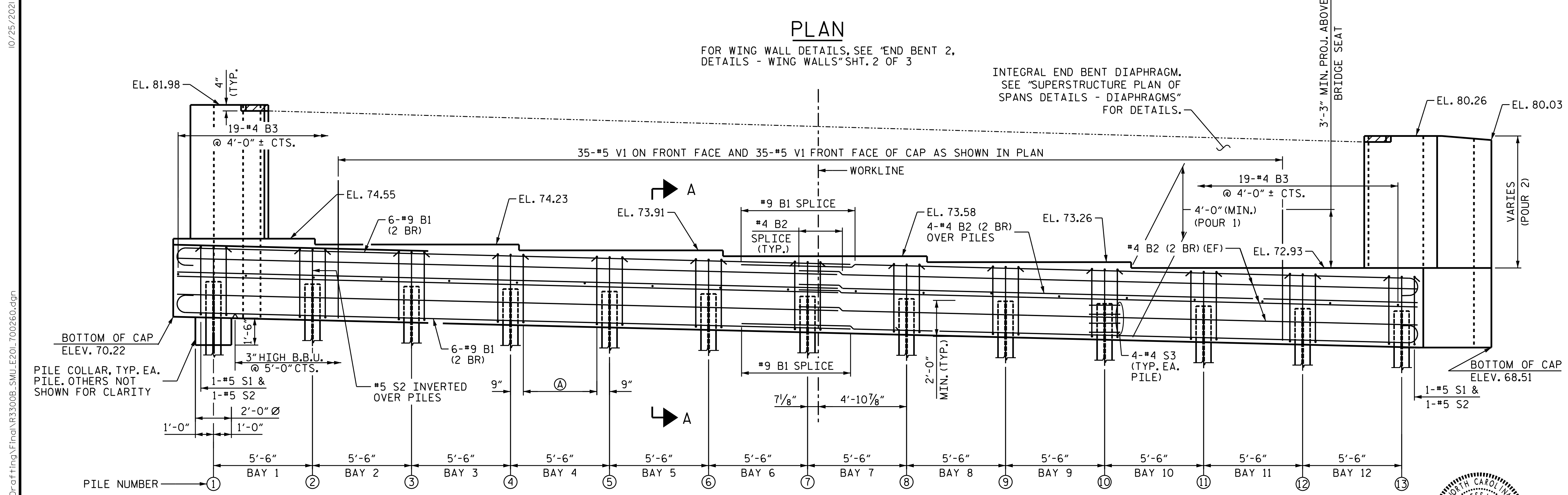


**PLAN**

FOR WING WALL DETAILS, SEE 'END BENT 2, DETAILS - WING WALLS' SHT. 2 OF 3

INTEGRAL END BENT DIAPHRAGM. SEE "SUPERSTRUCTURE PLAN OF SPANS DETAILS - DIAPHRAGMS" FOR DETAILS.

TOP OF PILE ELEVATIONS	
NO.	EL.
1	72.17
2	72.03
3	71.90
4	71.77
5	71.63
6	71.50
7	71.37
8	71.23
9	71.10
10	70.96
11	70.83
12	70.70
13	70.56



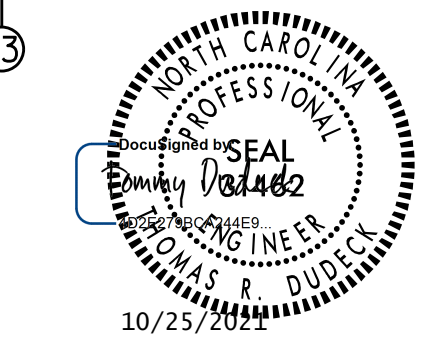
**ELEVATION**

SPlice LENGTH	
#9 B1	5'-4"
#4 B2	2'-5"

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 1 OF 3  
 STATE OF NORTH CAROLINA  
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**SUBSTRUCTURE**  
**END BENT 2**

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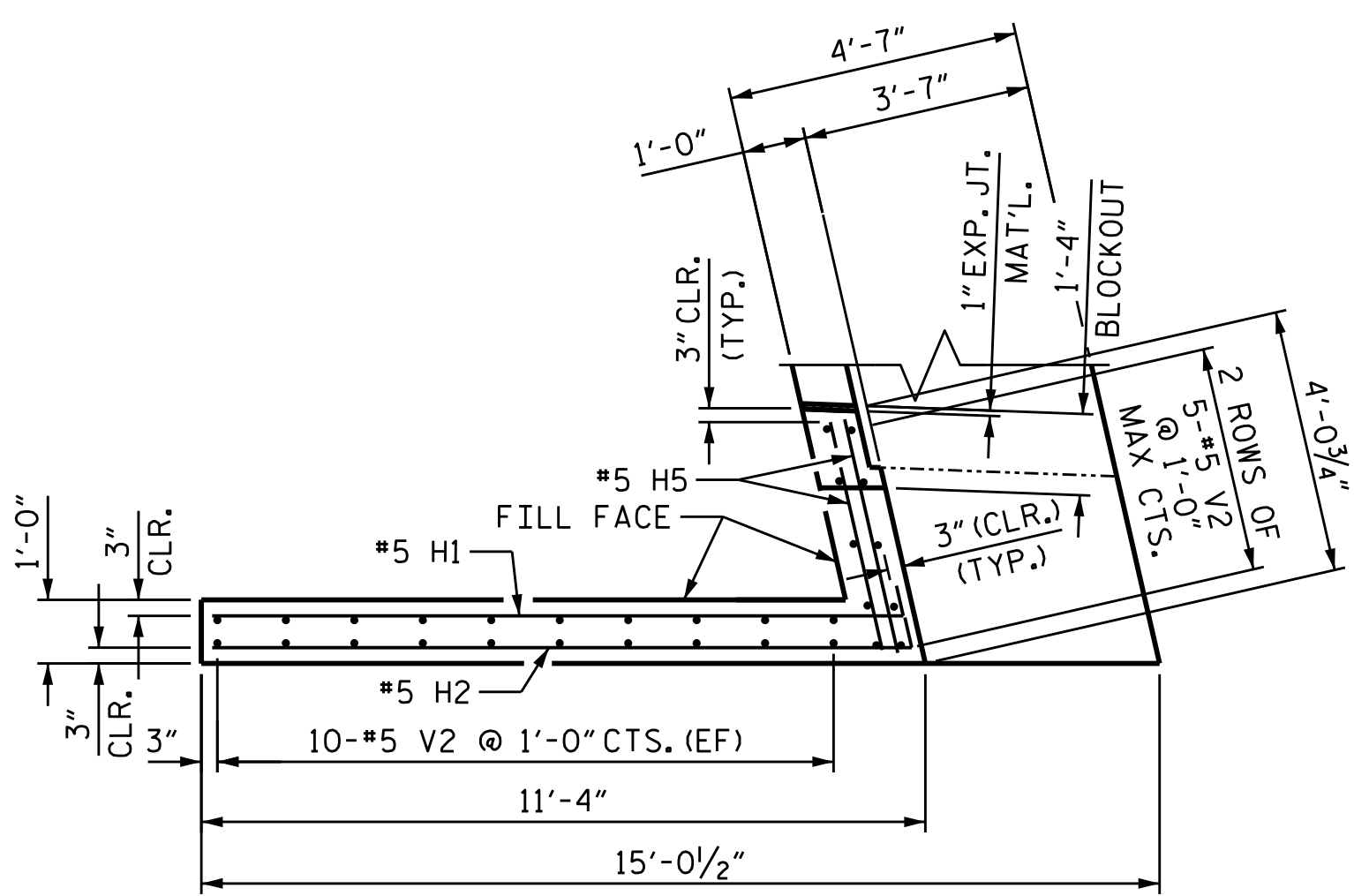


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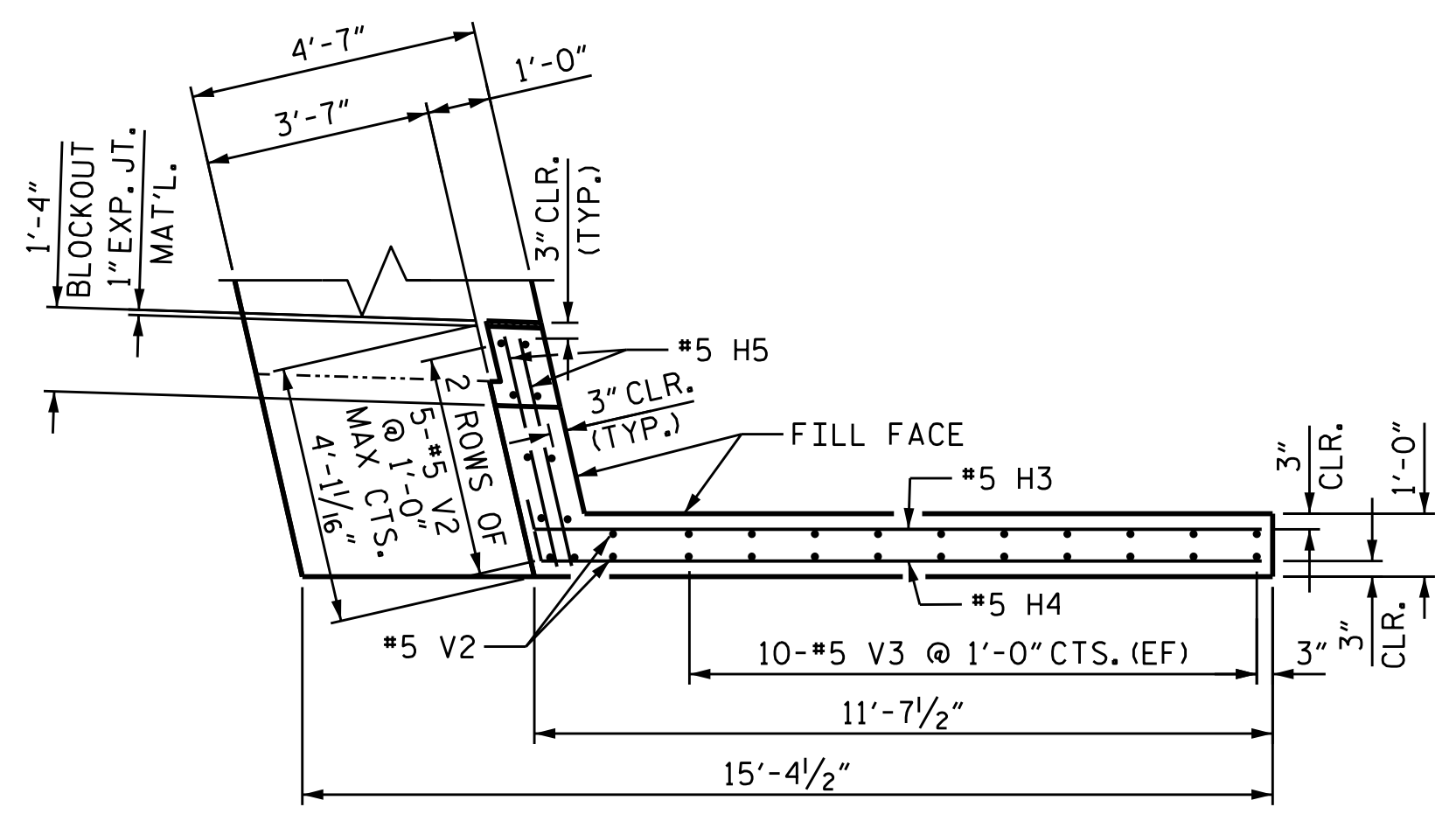
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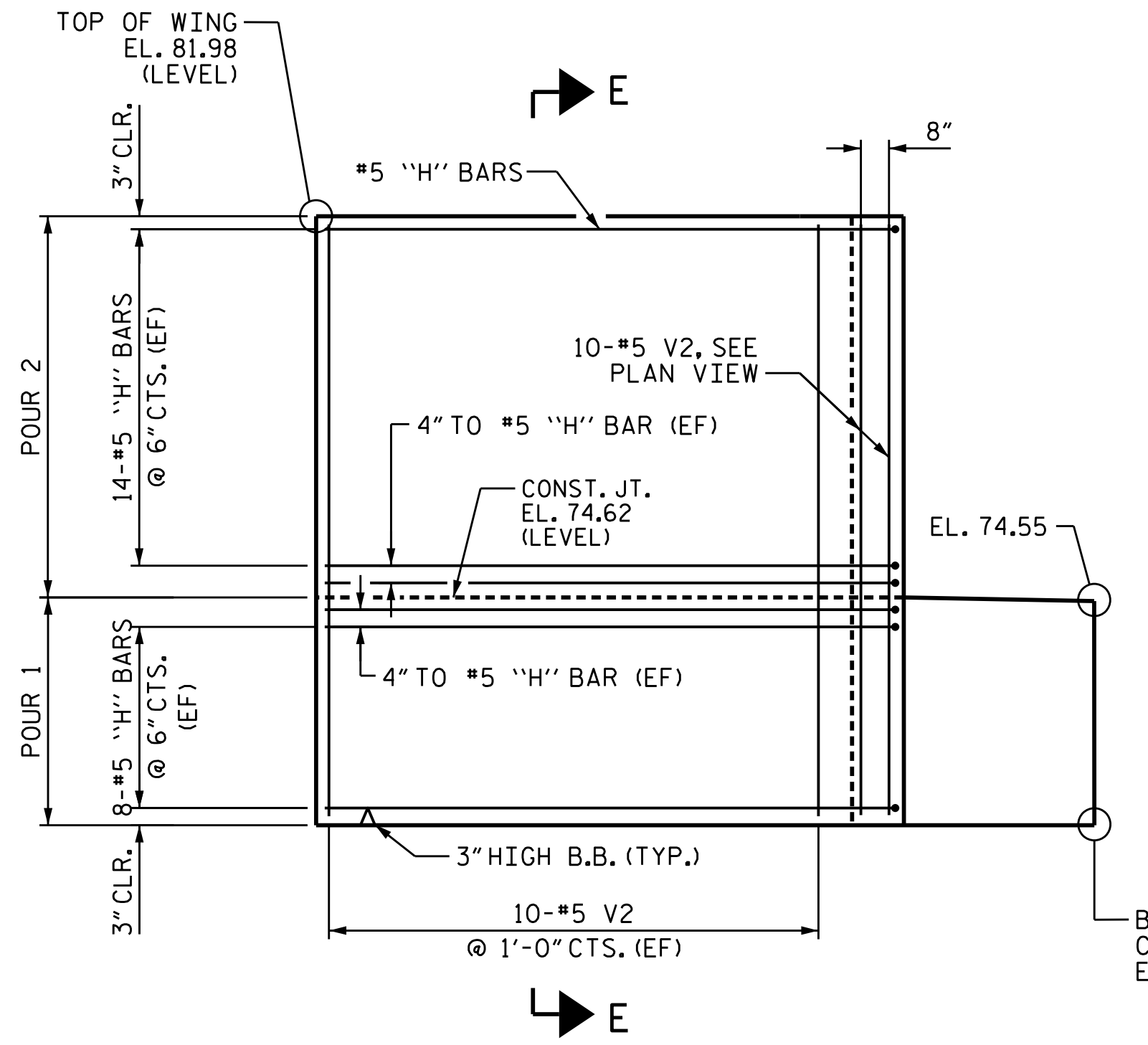
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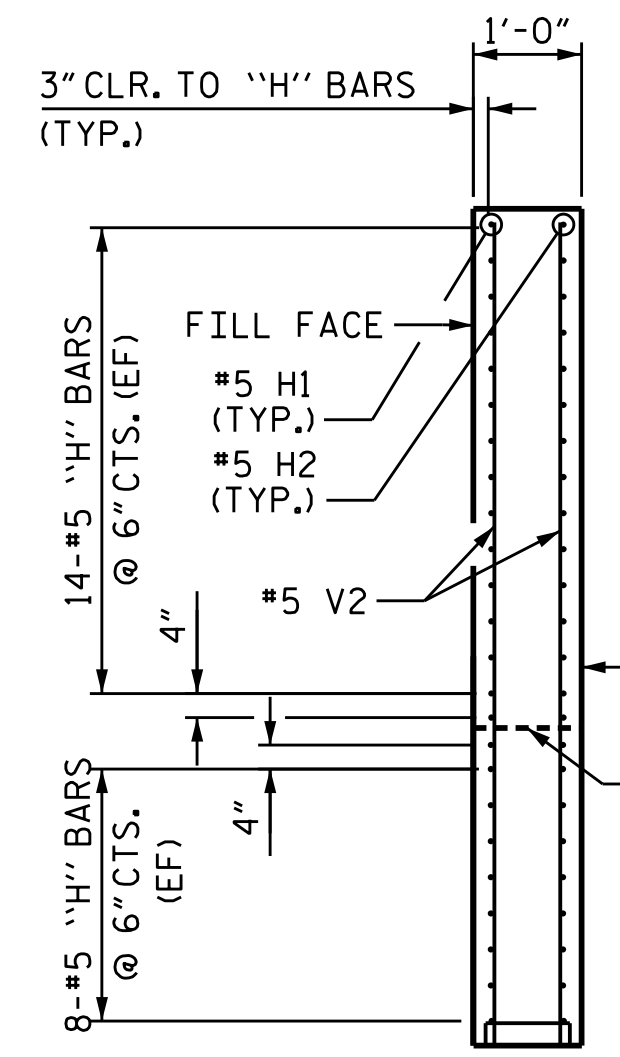
PLAN OF LEFT WING (W3)



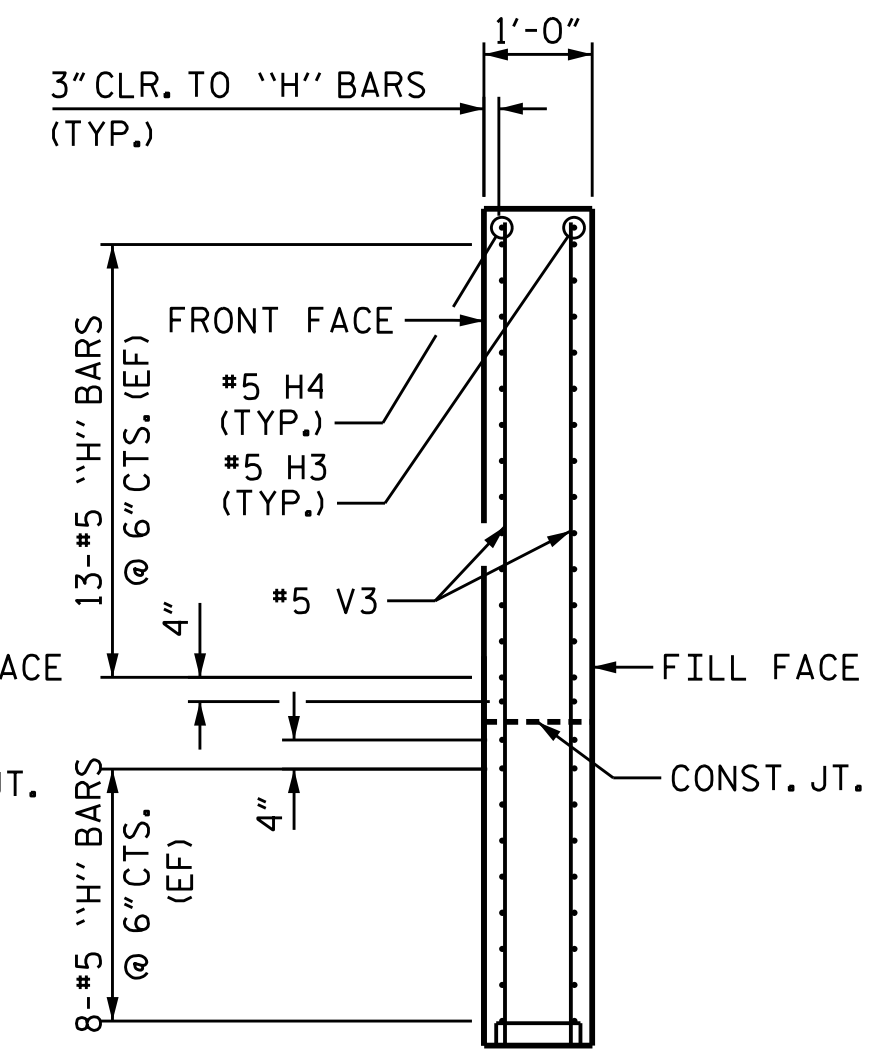
PLAN OF RIGHT WING (W4)



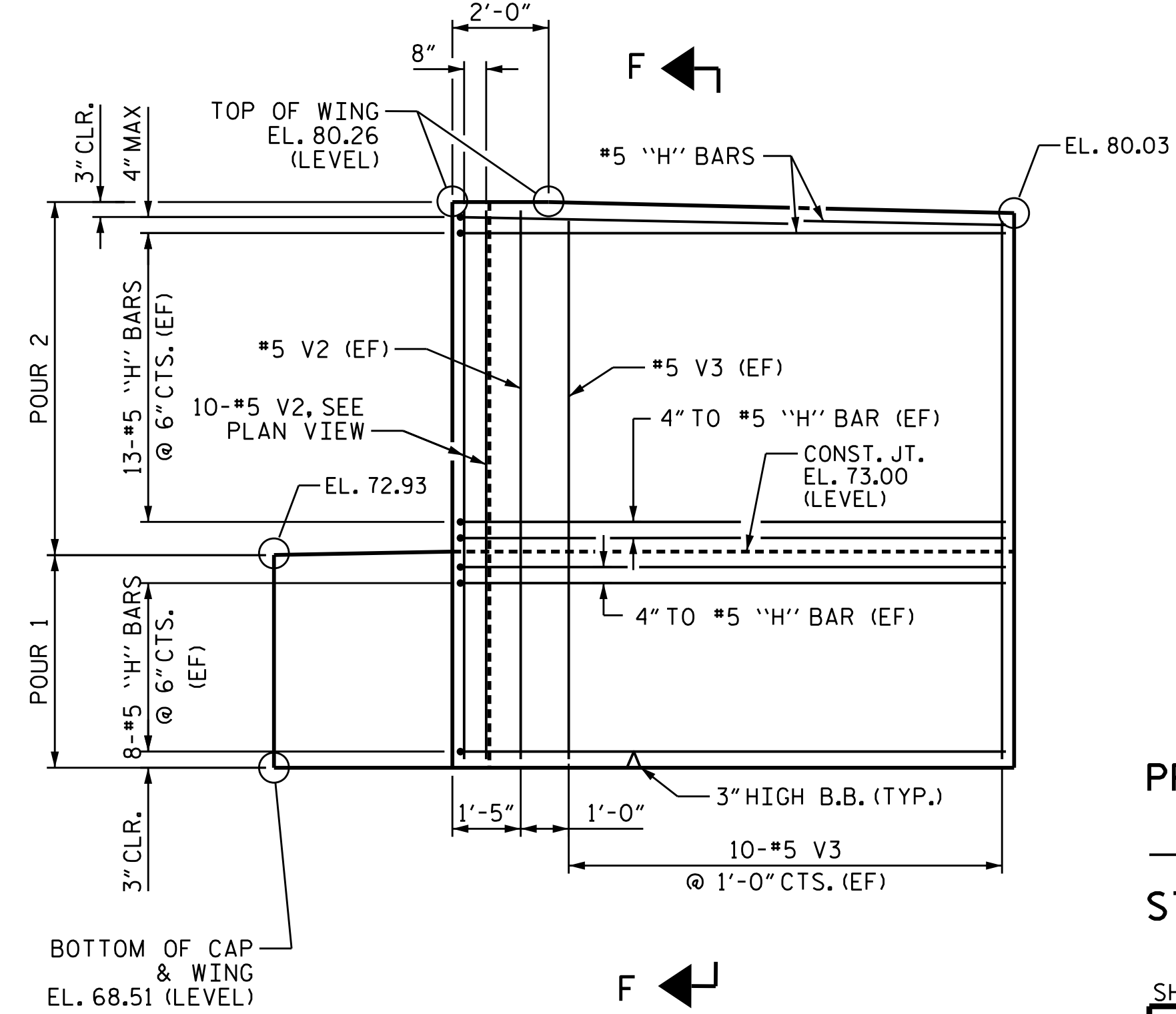
ELEVATION OF LEFT WING (W3)



SECTION E-E



SECTION F-F

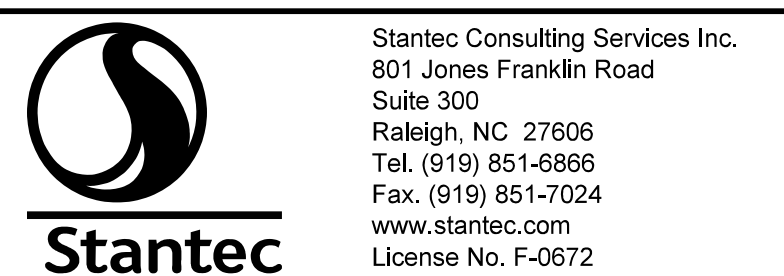
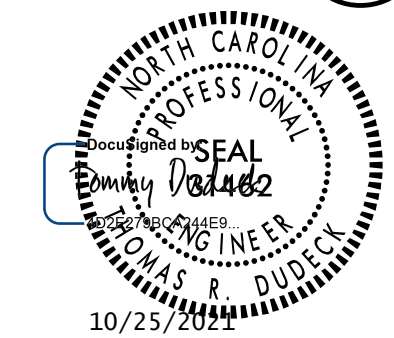


ELEVATION OF RIGHT WING (W4)

**NOTE:**  
 TOP SURFACE OF END BENT CAP BETWEEN EDGE OF DECK SLAB AND END OF CAP SHALL BE SLOPED TRANSVERSELY FROM FRONT FACE TO FILL FACE AT A RATE OF 1/4" / FT.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 3  
 STATE OF NORTH CAROLINA  
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 RALEIGH  
 SUBSTRUCTURE  
 END BENT 2 DETAILS  
 WING WALLS

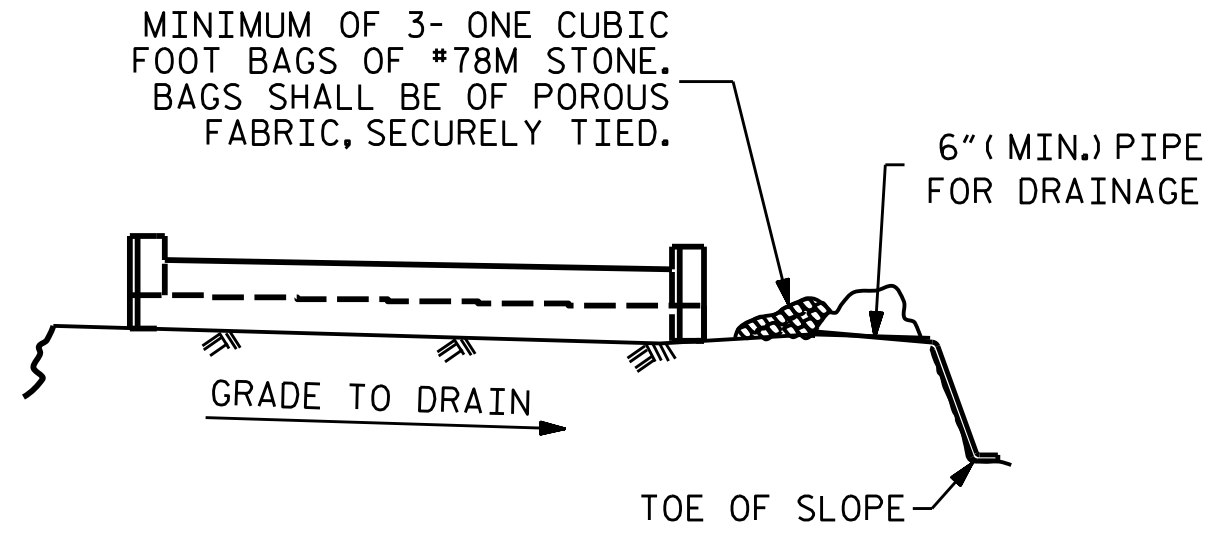


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 CHECKED BY: R. DeCOLA DATE: 02/07/18  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

REVISIONS						SHEET NO. S16-32
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 38
2			4			

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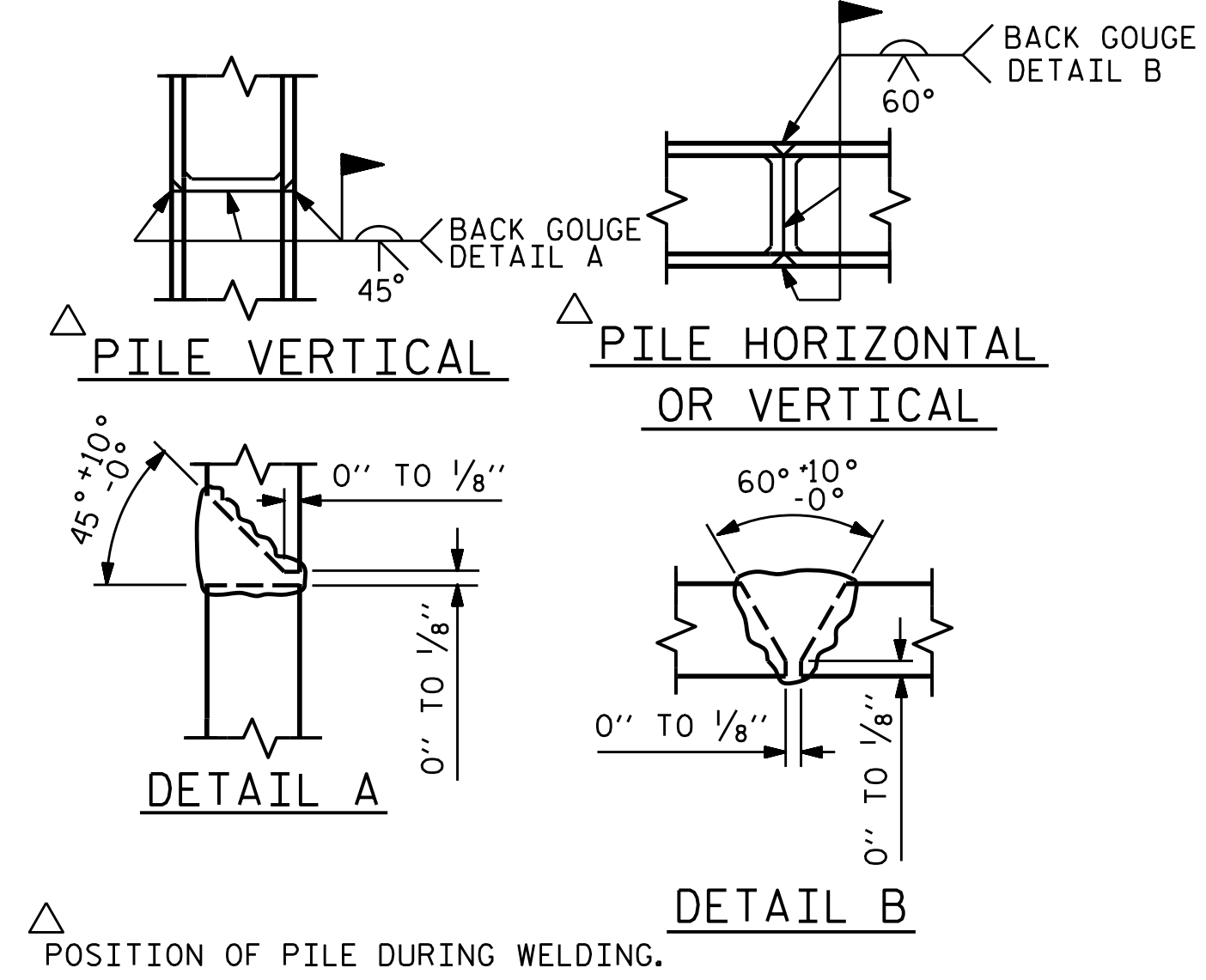


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

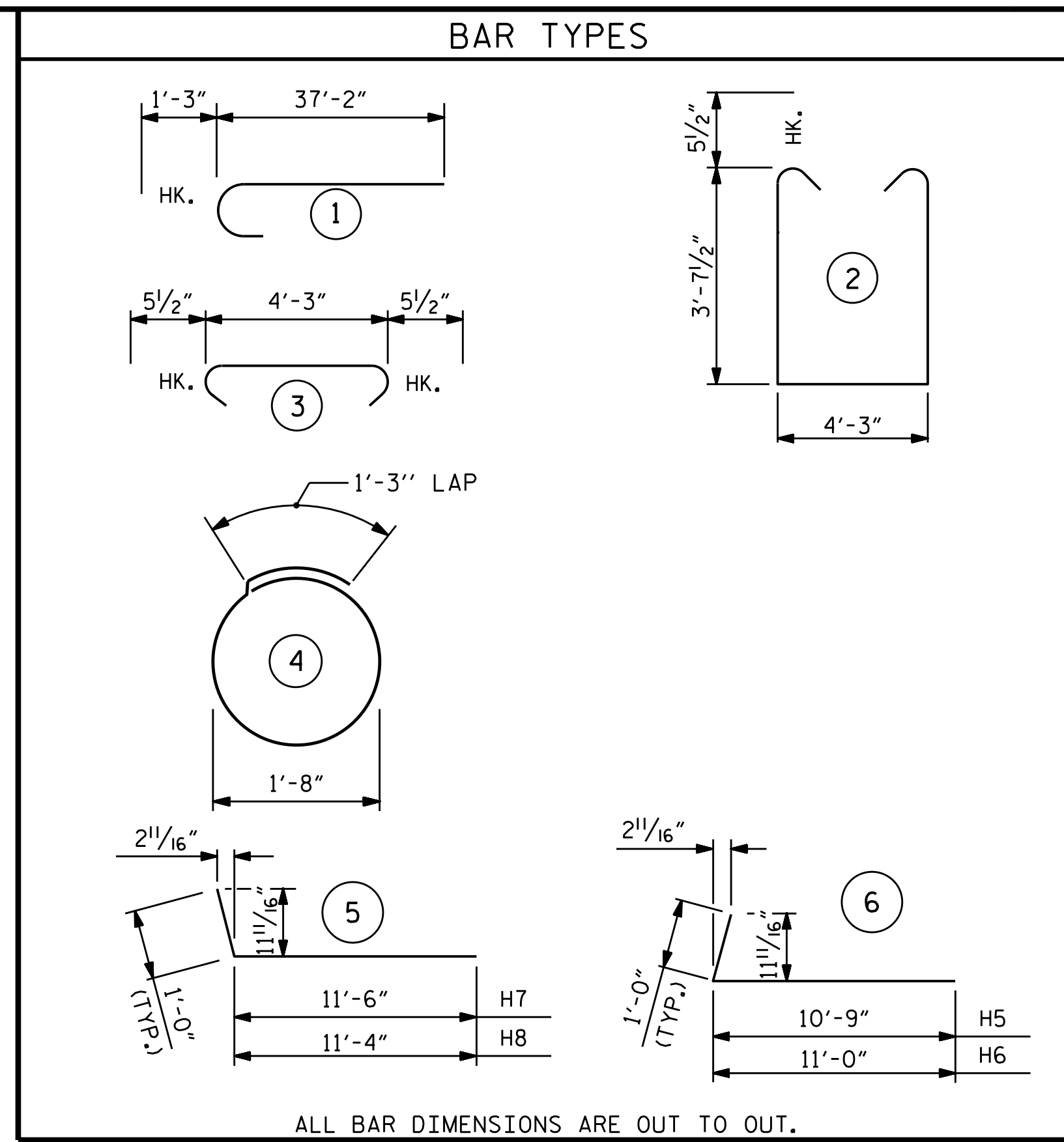
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

**TEMPORARY DRAINAGE AT END BENT**



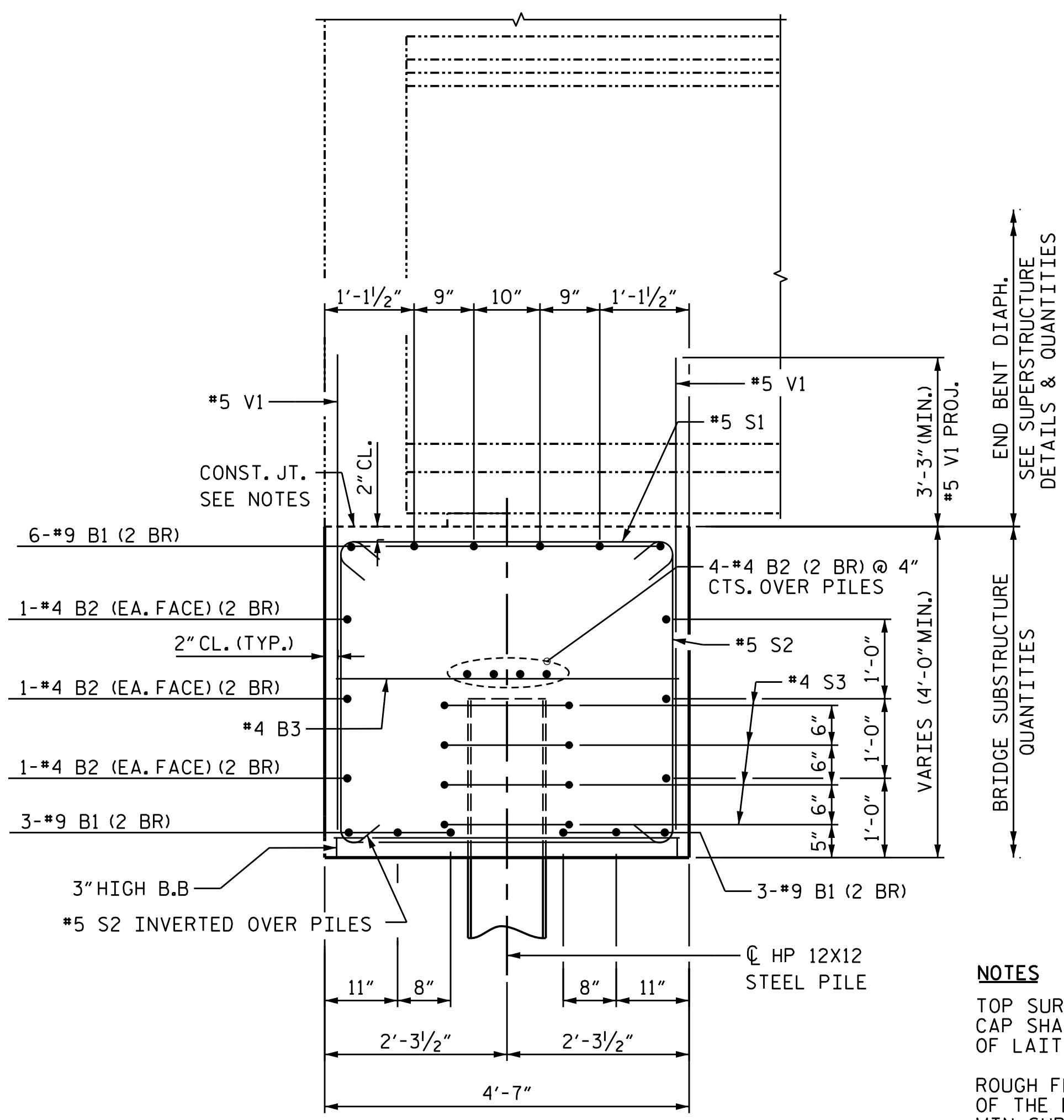
POSITION OF PILE DURING WELDING.

**PILE SPLICE DETAILS**



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL					
END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	24	#9	1	37'-2"	3033
B2	20	#4	STR	35'-8"	477
B3	19	#4	STR	4'-3"	54
H1	24	#5	6	11'-9"	294
H2	24	#5	6	12'-0"	300
H3	24	#5	5	12'-7"	315
H4	24	#5	5	12'-4"	309
H5	60	#5	STR	3'-6"	219
S1	86	#5	3	5'-2"	463
S2	99	#5	2	12'-5"	1282
S3	52	#4	4	6'-6"	226
V1	70	#5	STR	7'-6"	548
V2	42	#5	STR	11'-5"	500
V3	20	#5	STR	11'-2"	233
REINFORCING STEEL					LBS. 8,253
CLASS A CONCRETE BREAKDOWN:					
POUR #1: CAP, COLLARS, ETC.					C.Y. 55.8
POUR #2: UPPER WINGS					C.Y. 10.2
CLASS A CONCRETE TOTAL					C.Y. 66.0
HP12X53 PILES					
NO. 13					LIN FT. 1040.00
PILE REDRIVES					EA. 7
STEEL PILE POINTS					EA. 13
PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES					EA. 13



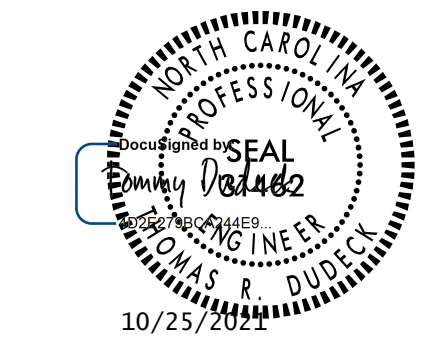
**SECTION A-A**  
SEE "END BENT 2" SHEET 1 OF 3

**NOTES**  
TOP SURFACE AREAS OF THE END BENT CAP SHALL BE KEPT CLEAN AND FREE OF LAITANCE.  
ROUGH FLOAT AND ROUGHEN THE TOP OF THE END BENT CAP TO PROVIDE MIN. SURFACE AMPLITUDE OF 1/4", EXCEPT UNDER BEARING AREAS.  
2 BR DENOTES 2 BAR RUN.  
3 BR DENOTES 3 BAR RUN.

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 30+17.11 -Y31-

SHEET 3 OF 3

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RALEIGH  
SUBSTRUCTURE  
END BENT 2 DETAILS



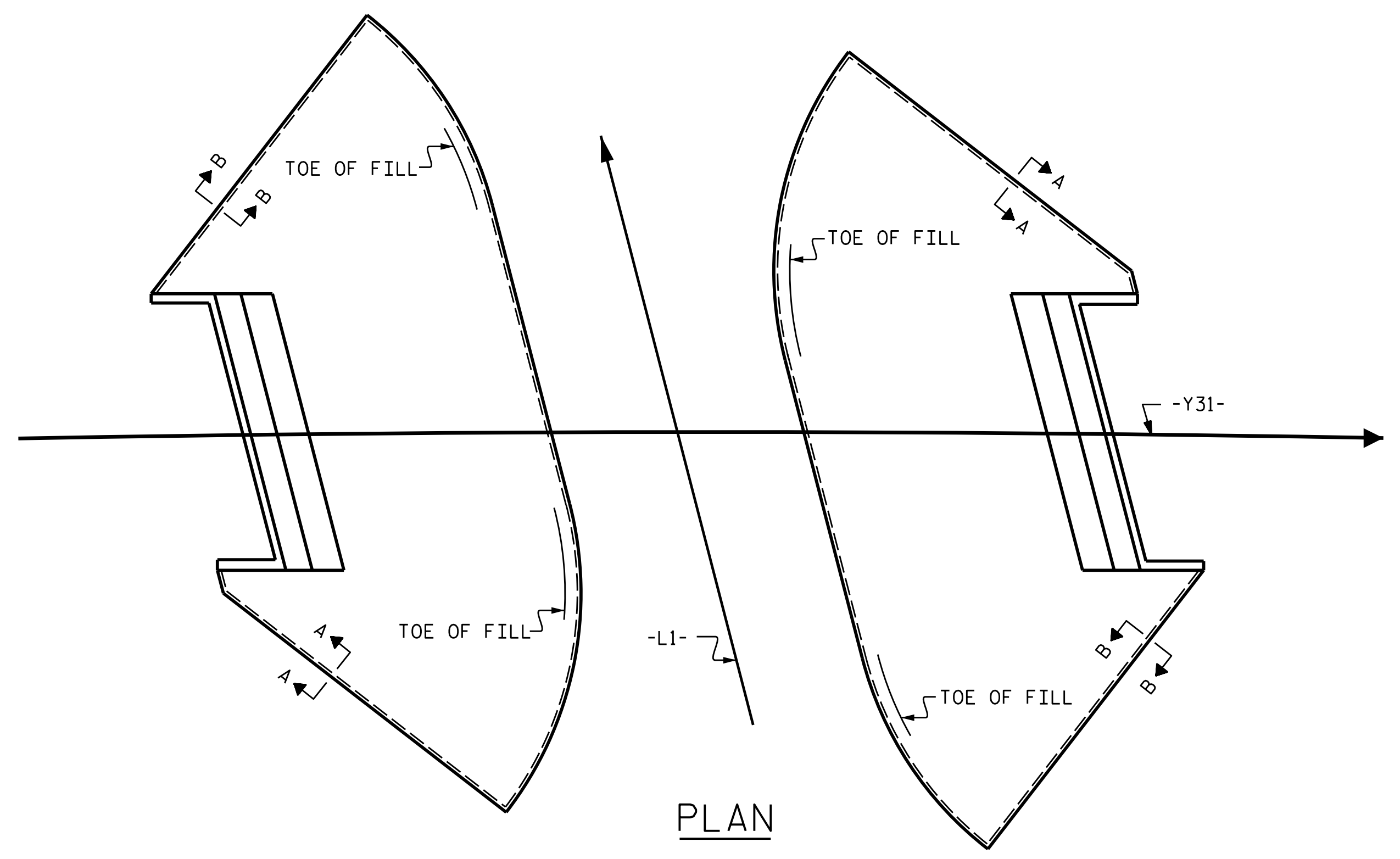
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CHECKED BY : R. DeCOLA DATE : 02/07/18  
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10/25/2021  
10/25/2021



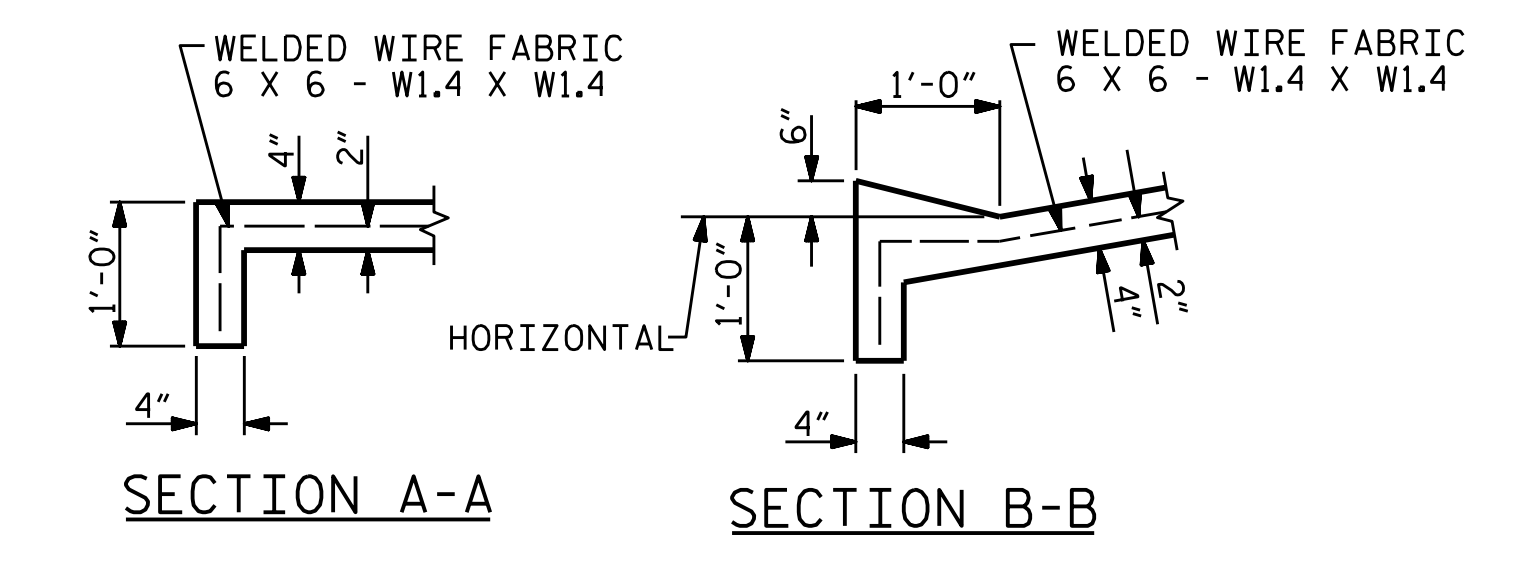
PLAN

GENERAL NOTES

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING. SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

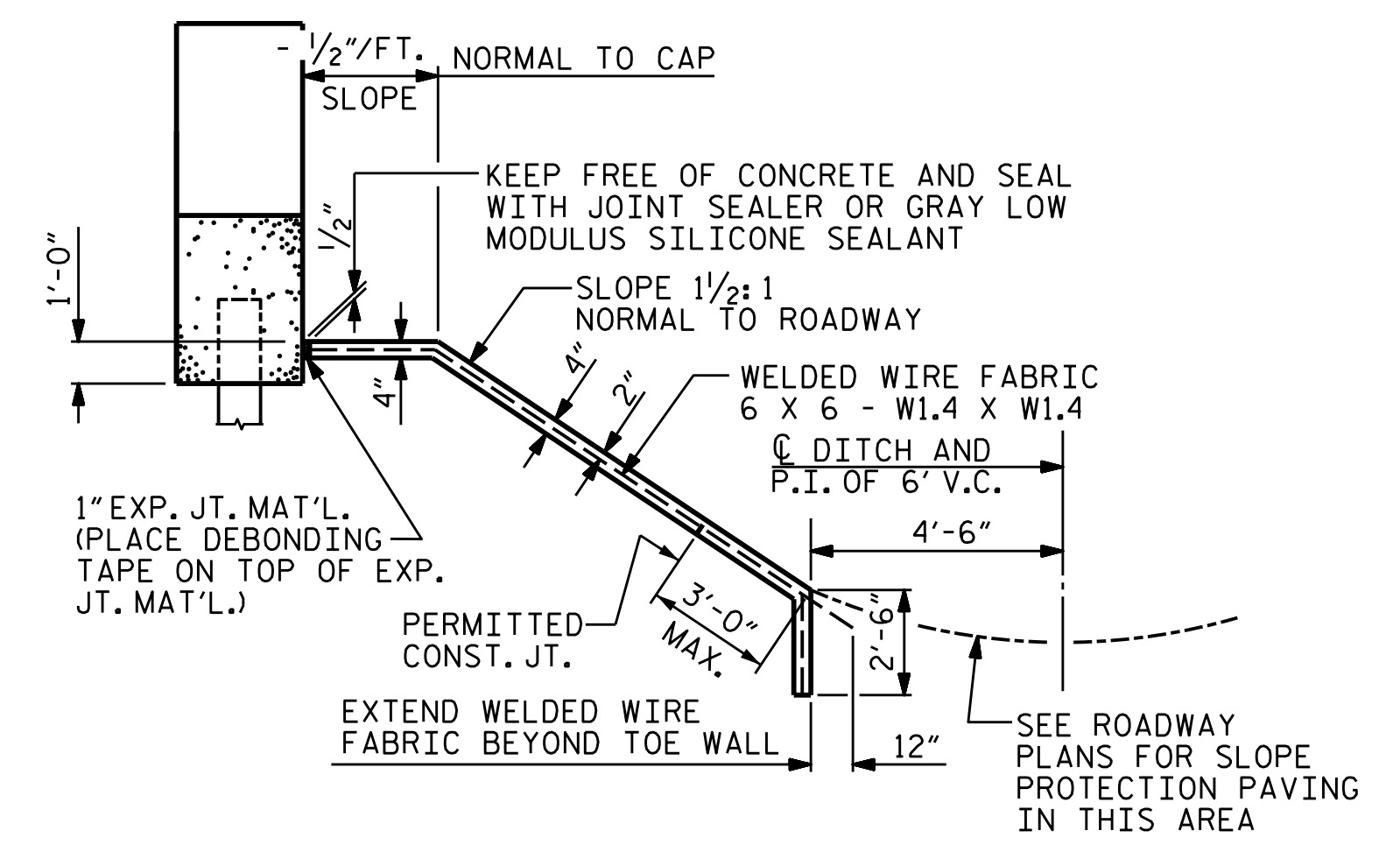
BRIDGE @ STA. 30+17.11 -Y31-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	587	1056
END BENT 2	788	1418
TOTAL	1375	2474

\* QUANTITY SHOWN IS BASED ON 5' POURS.

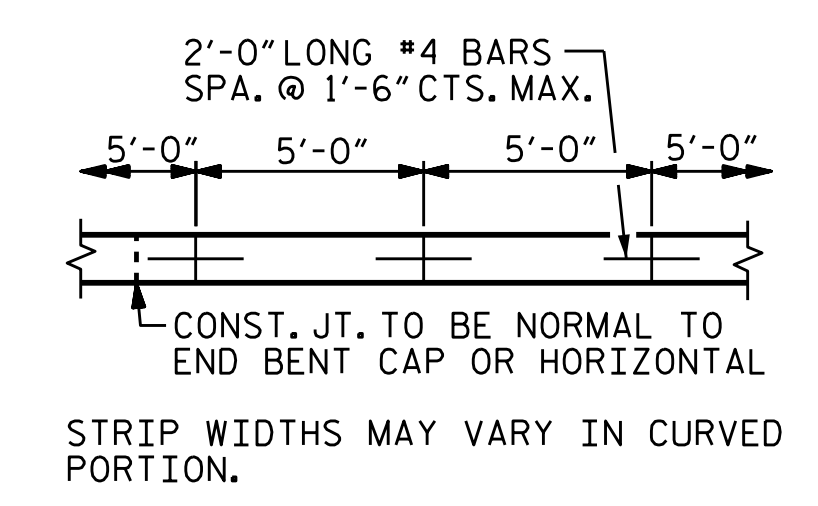


SECTION A-A

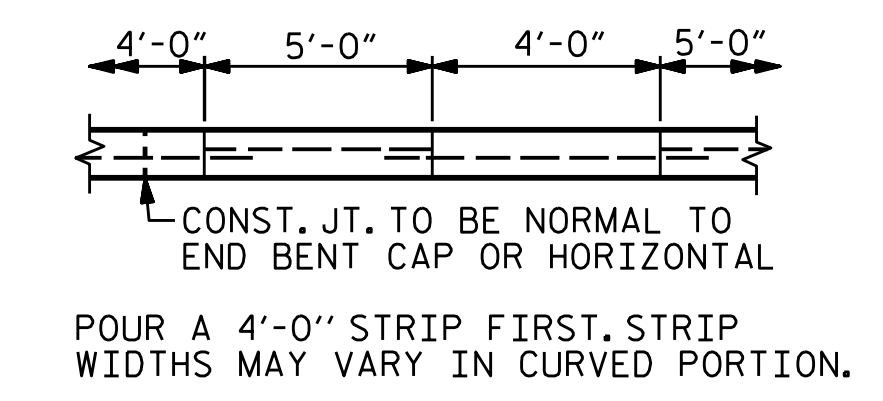
SECTION B-B



SECTION ALONG -L1- WHEN FILL CATCHES IN DITCH



POURING DETAIL



OPTIONAL POURING DETAIL



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SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SLOPE PROTECTION  
 DETAILS**

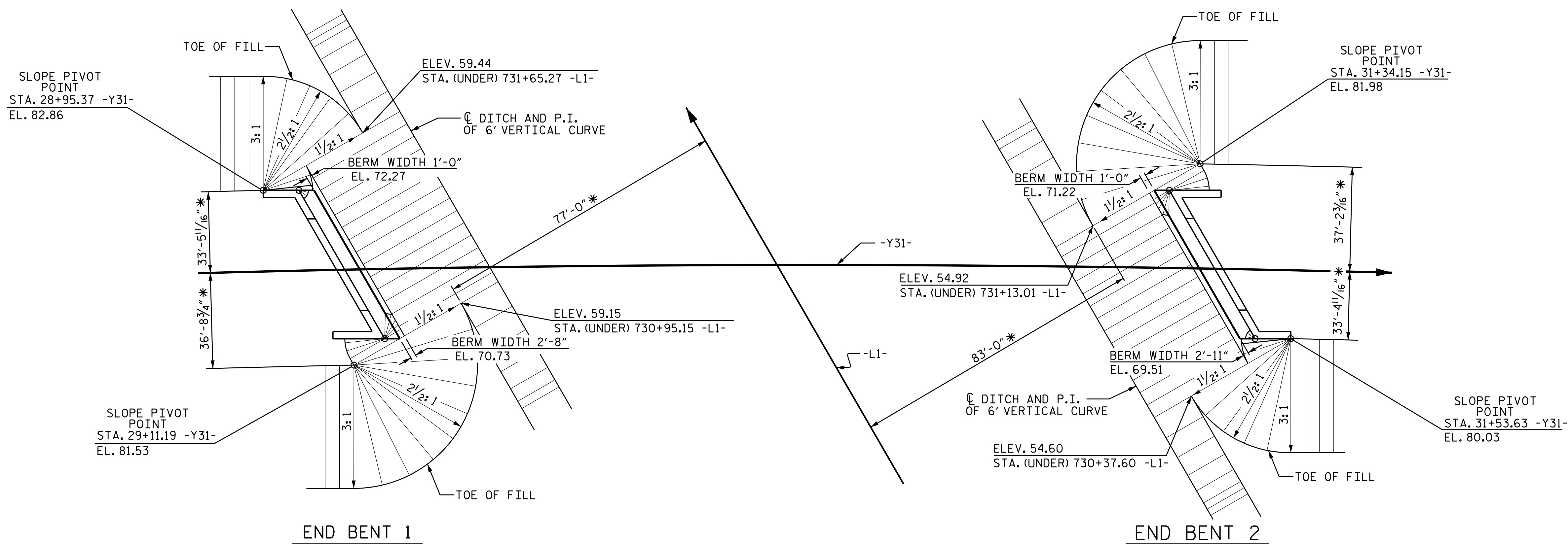
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 CHECKED BY : R. DeCOLA DATE : 02/09/18  
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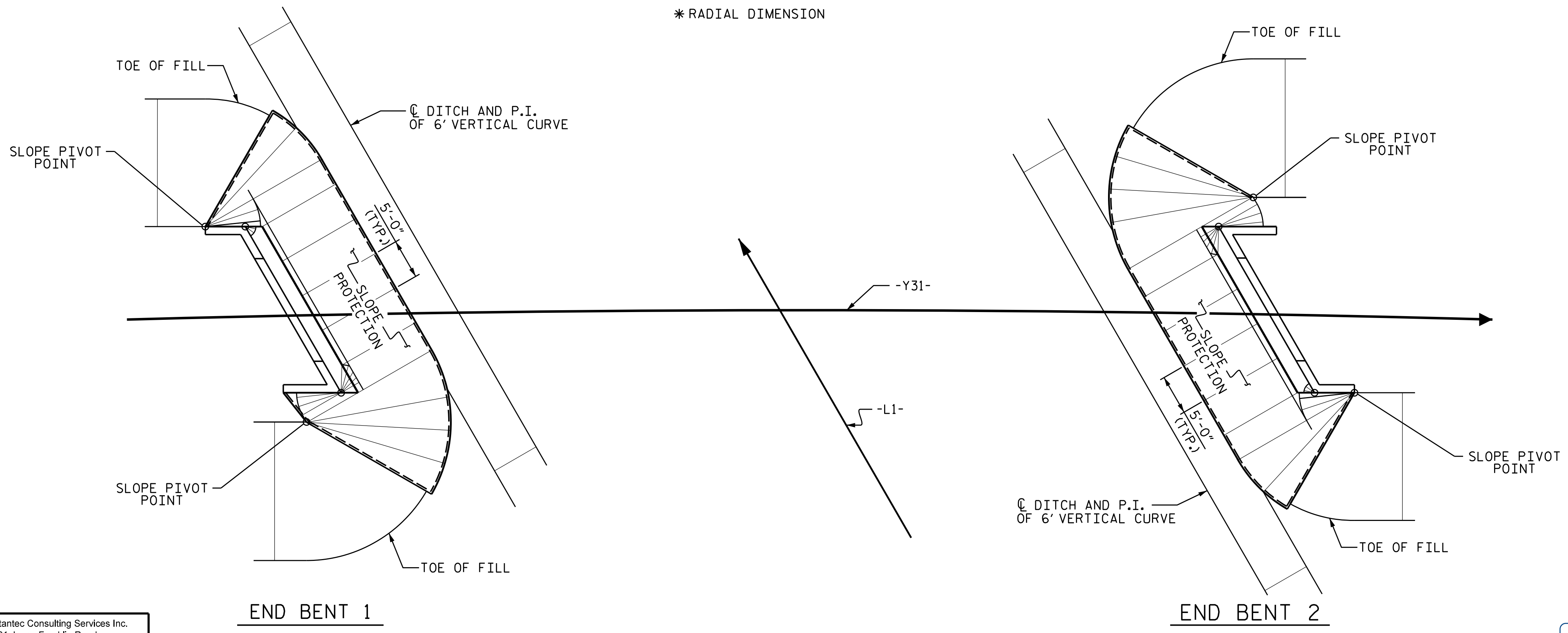
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**PLAN - GRADING**

ELEVATIONS SHOWN ARE AT TOP SURFACE OF SLOPE PAVING.

\* RADIAL DIMENSION



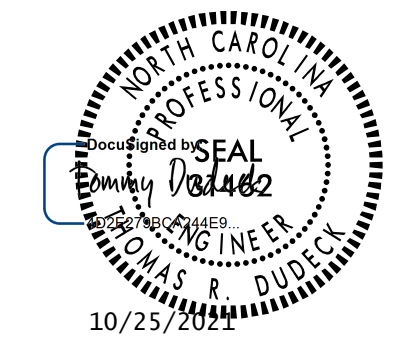
**PLAN - CONCRETE PLACEMENT**

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 2 OF 2

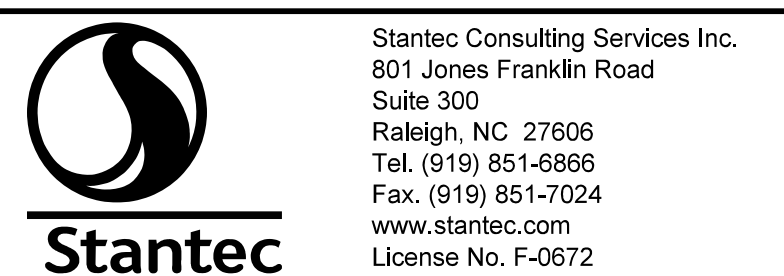
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SLOPE PROTECTION  
 DETAILS**



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DRAWN BY : J. E. HAGENBUSH DATE : 11/21/17  
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10/25/2021  
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### NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE I IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, "TYPE A - ALTERNATE APPROACH FILL" IN LIEU OF "TYPE I - STANDARD APPROACH FILL" MAY BE CONSTRUCTED AT NO ADDITIONAL COST TO THE DEPARTMENT, SEE SHEET 2 OF 2 FOR DETAILS AND NOTES.

CONTRACTOR MAY PLACE "B" BARS ALONG CURVATURE OF ROADWAY TO MATCH GEOMETRY OF APPROACH SLAB.

### REINFORCING BAR SCHEDULE

FOR APPROACH SLAB AT END BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	78	#4	STR	21'-3"	1107
A2	78	#4	STR	21'-1"	1099
* B1	117	#5	STR	24'-1"	2939
B2	117	#6	STR	24'-7"	4320
* B3	3	#4	STR	24'-7"	49
* G1	17	#4	STR	2'-7"	29
REINFORCING STEEL					5,419 LBS
* EPOXY COATED REINFORCING STEEL					4,124 LBS

### REINFORCING BAR SCHEDULE

FOR APPROACH SLAB AT END BENT 2					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	78	#4	STR	21'-3"	1107
A2	78	#4	STR	21'-1"	1099
* B1	117	#5	STR	24'-1"	2939
B2	117	#6	STR	24'-7"	4320
* B3	11	#4	STR	24'-7"	181
* G2	1	#4	STR	3'-9"	3
* G3	1	#4	STR	9'-3"	6
* G4	14	#4	STR	14'-7"	136
* G5	1	#4	STR	13'-0"	9
* G6	1	#4	STR	7'-5"	5
REINFORCING STEEL					5,419 LBS
* EPOXY COATED REINFORCING STEEL					4,386 LBS

### SPLICE LENGTHS

BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

### APPROACH SLAB #1 BILL OF MATERIAL

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
APPR. SLAB #1	63.4	5,419	4,122
CONCRETE MEDIAN	1.5	-	78
TOTALS **	64.9	5,419	4,200

\*\*QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

### APPROACH SLAB #2 BILL OF MATERIAL

	CLASS AA CONCRETE (CU. YDS.)	REINFORCING STEEL (LBS.)	EPOXY COATED REINFORCING STEEL (LBS.)
APPR. SLAB #2	63.4	5,432	4,399
CONCRETE MEDIAN	6.2	-	340
TOTALS **	69.6	5,432	4,739

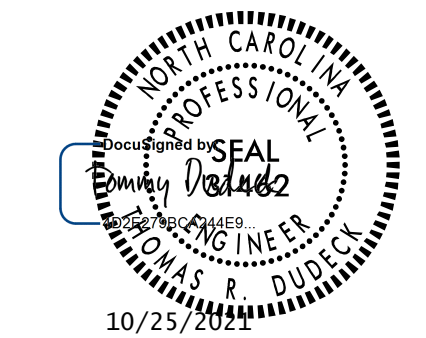
\*\*QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

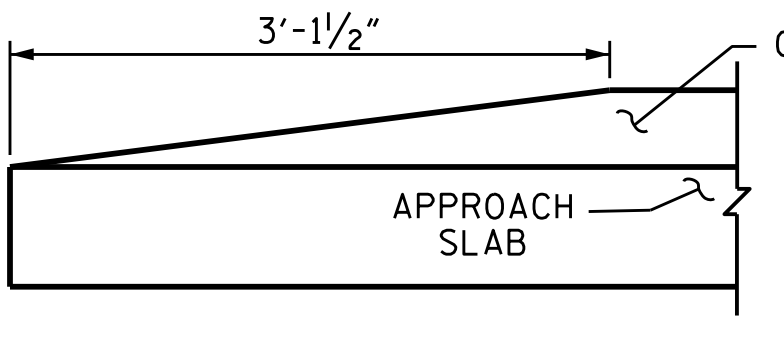
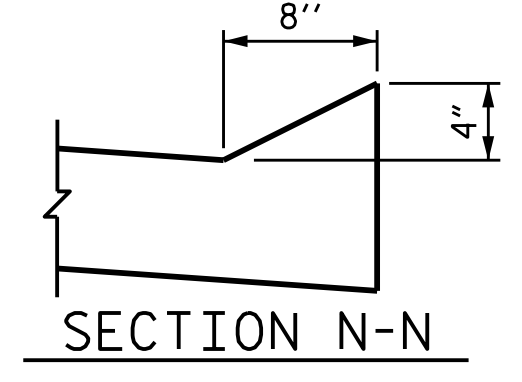
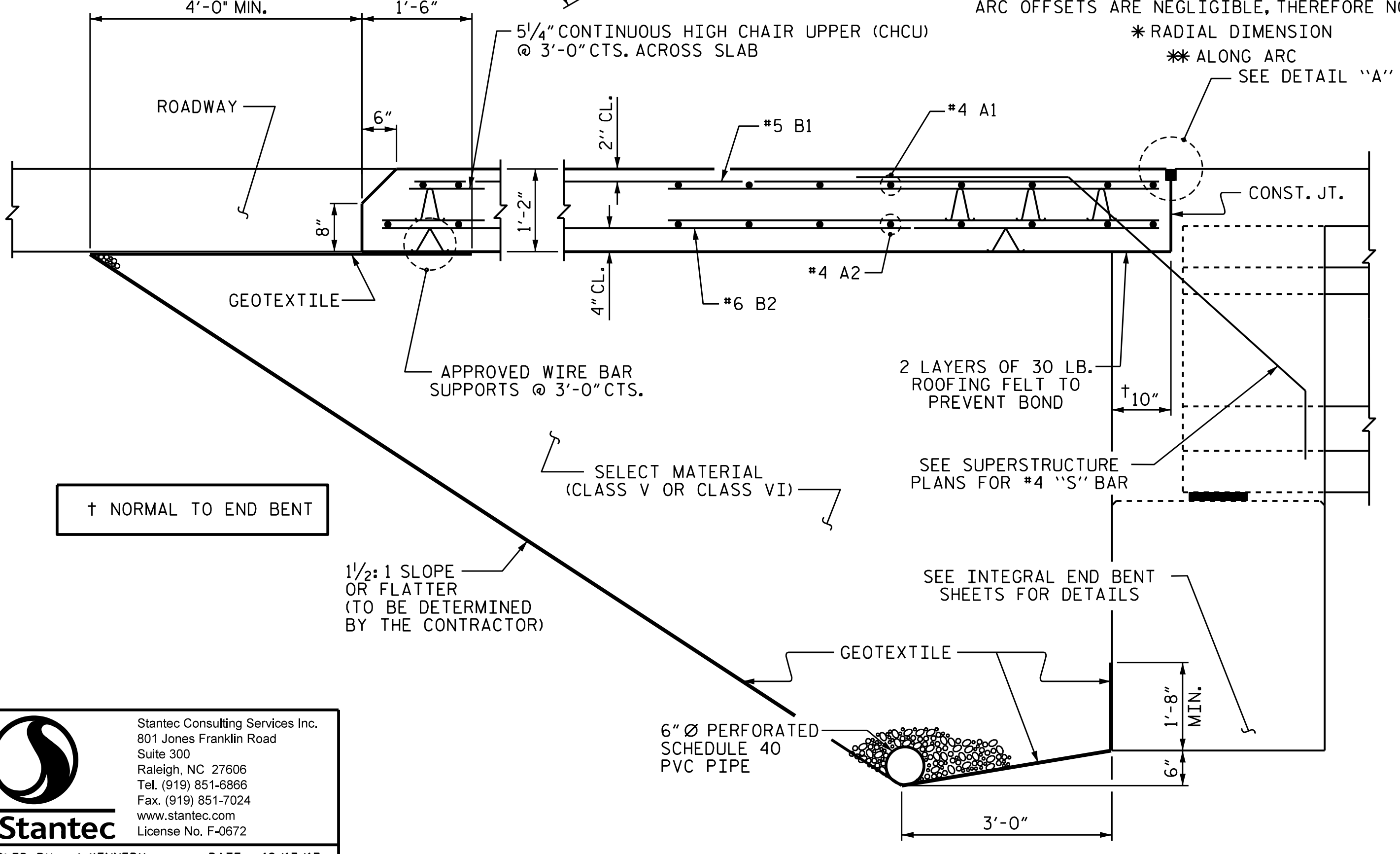
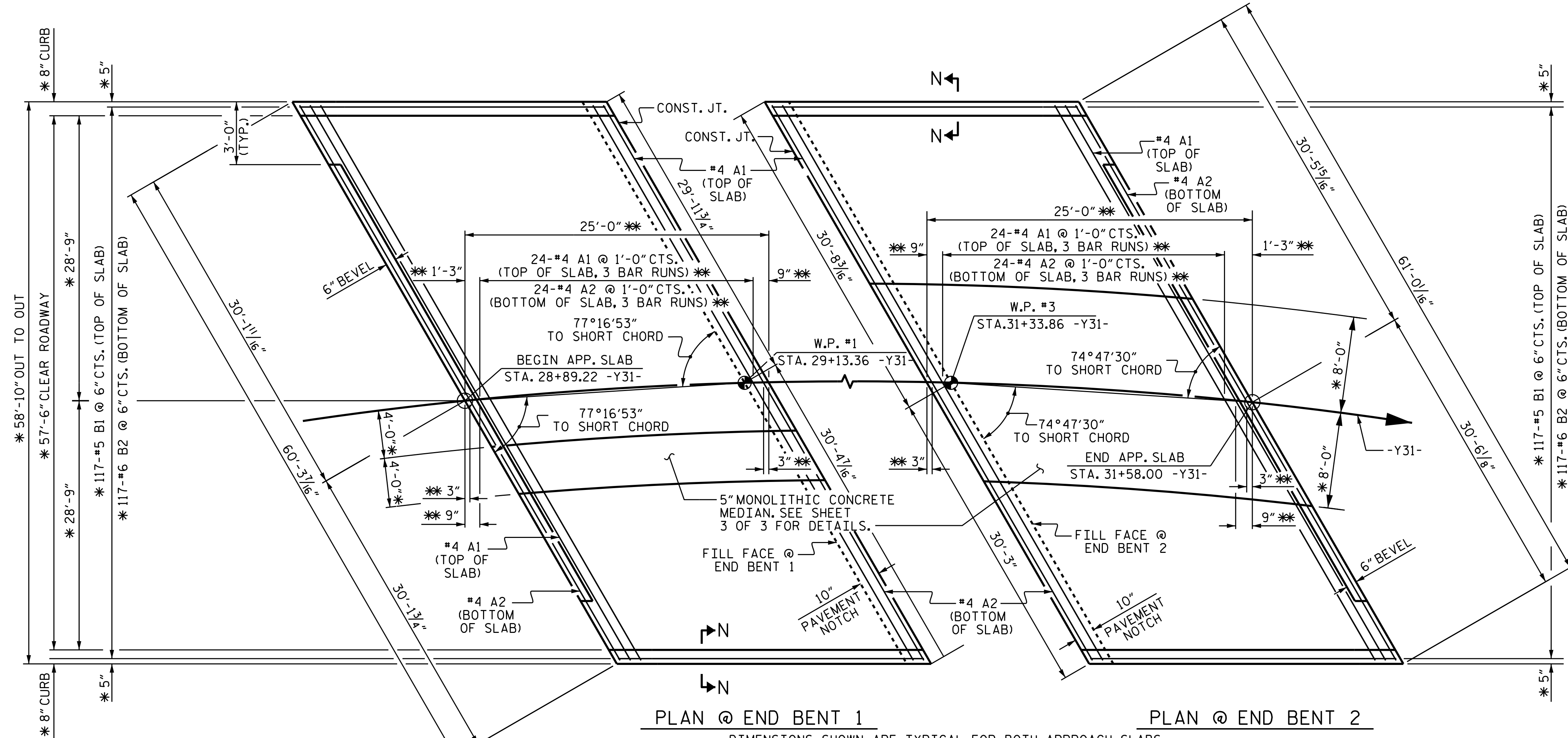
SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT WITH FLEXIBLE PAVEMENT

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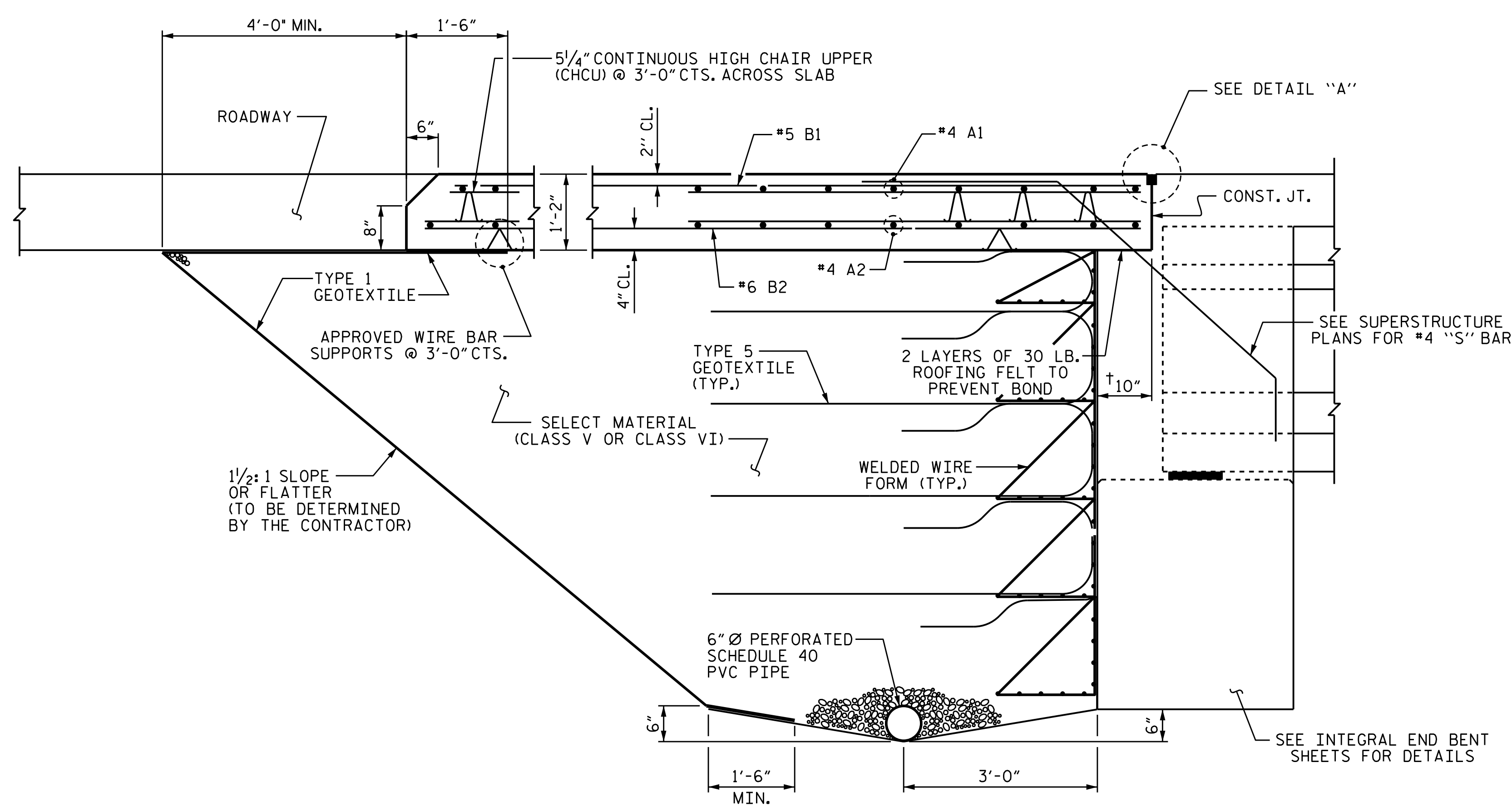
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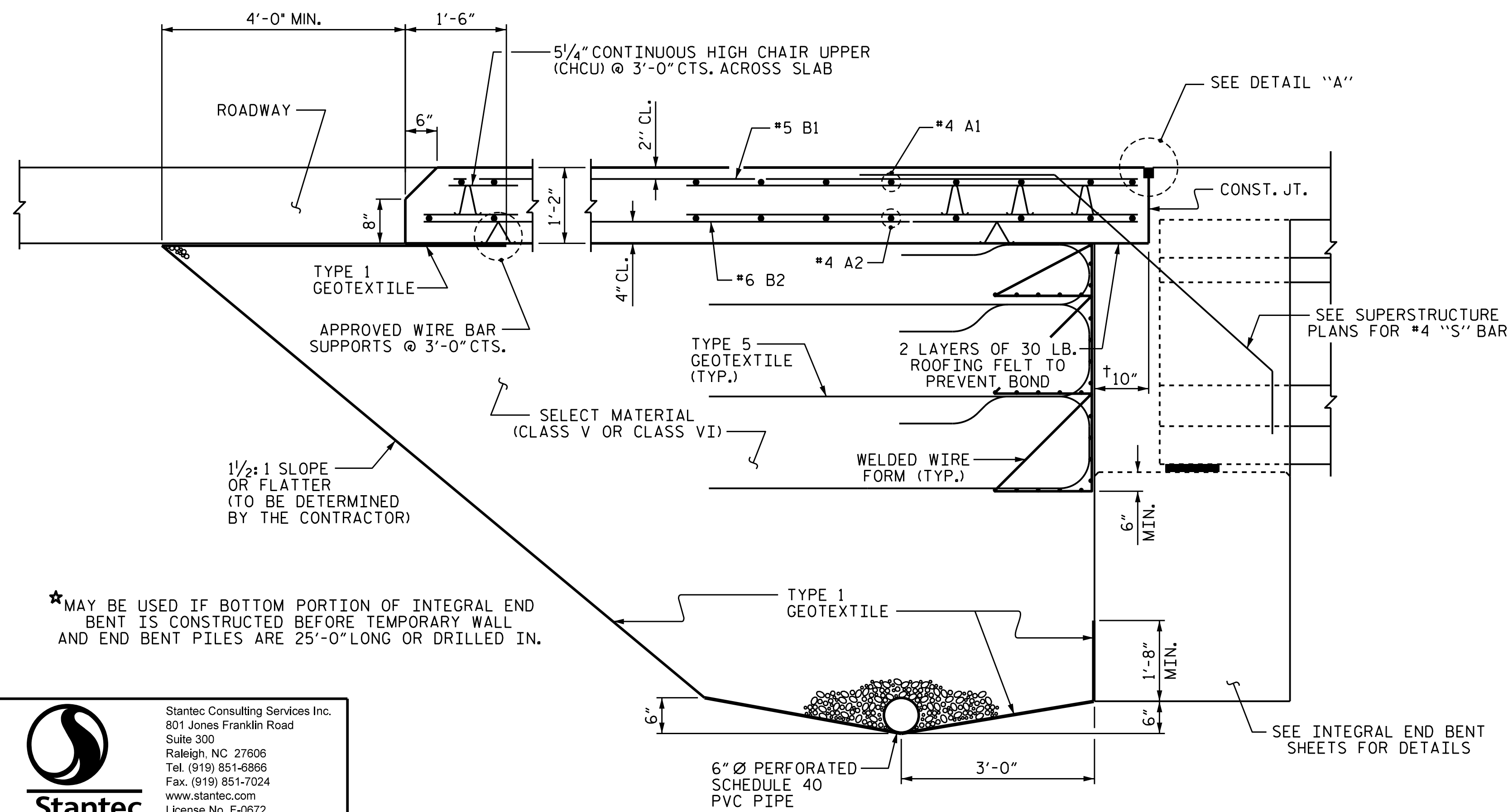
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ASSEMBLED BY: J. KENNEDY DATE: 12/17/17  
 CHECKED BY: R. DeCOLA DATE: 01/03/18  
 DRAWN BY: TLA 10/05 REV. 12/21/11 MAA/GM  
 CHECKED BY: GM 5/06 REV. 6/13 MAA/GM  
 REV. 12/17 MAA/THC  
 DESIGN ENGINEER OF RECORD: T. R. DUDECK DATE: 10/25/21

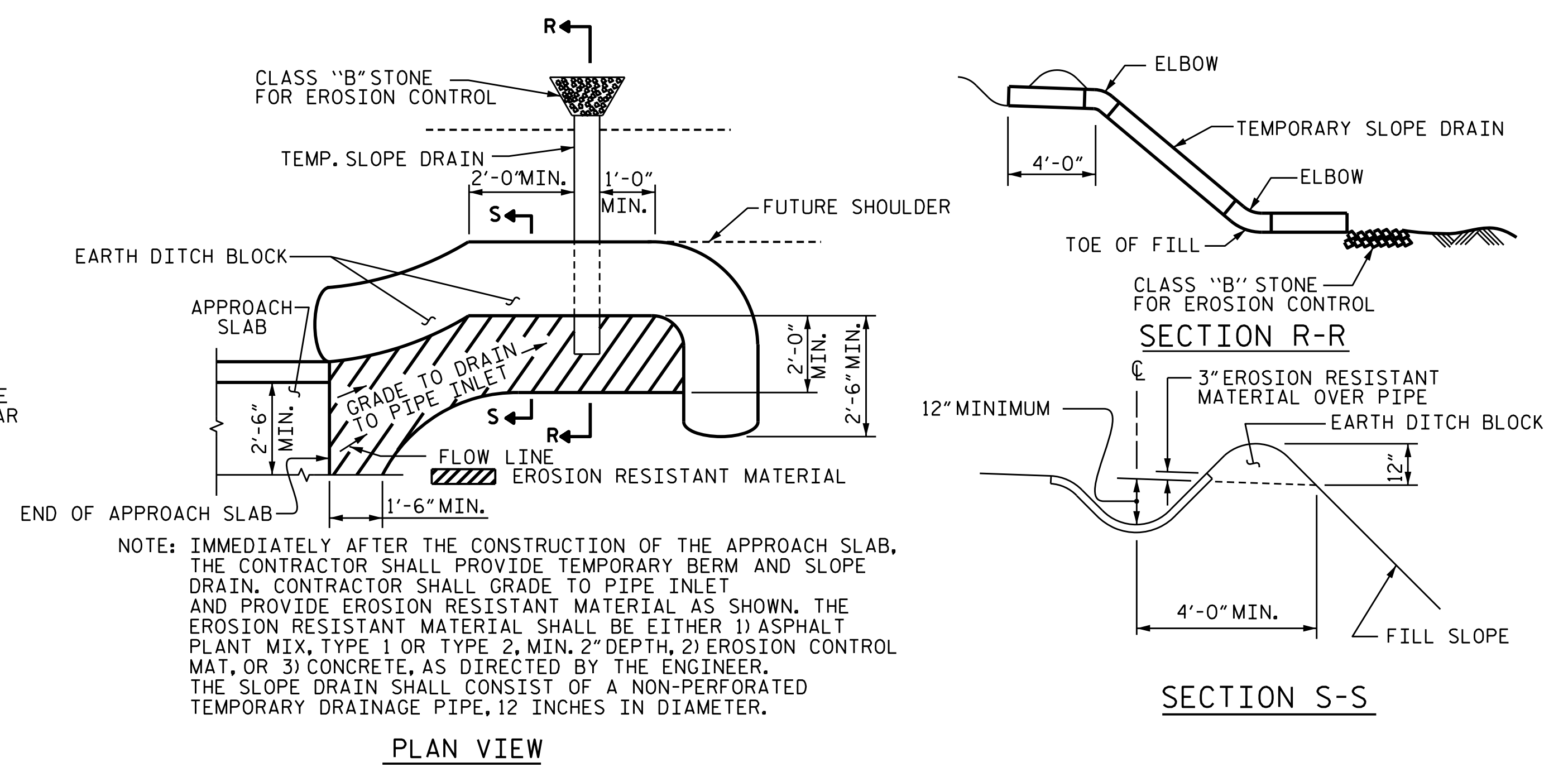
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**SECTION THRU SLAB**  
(TYPE A - ALTERNATE APPROACH FILL)



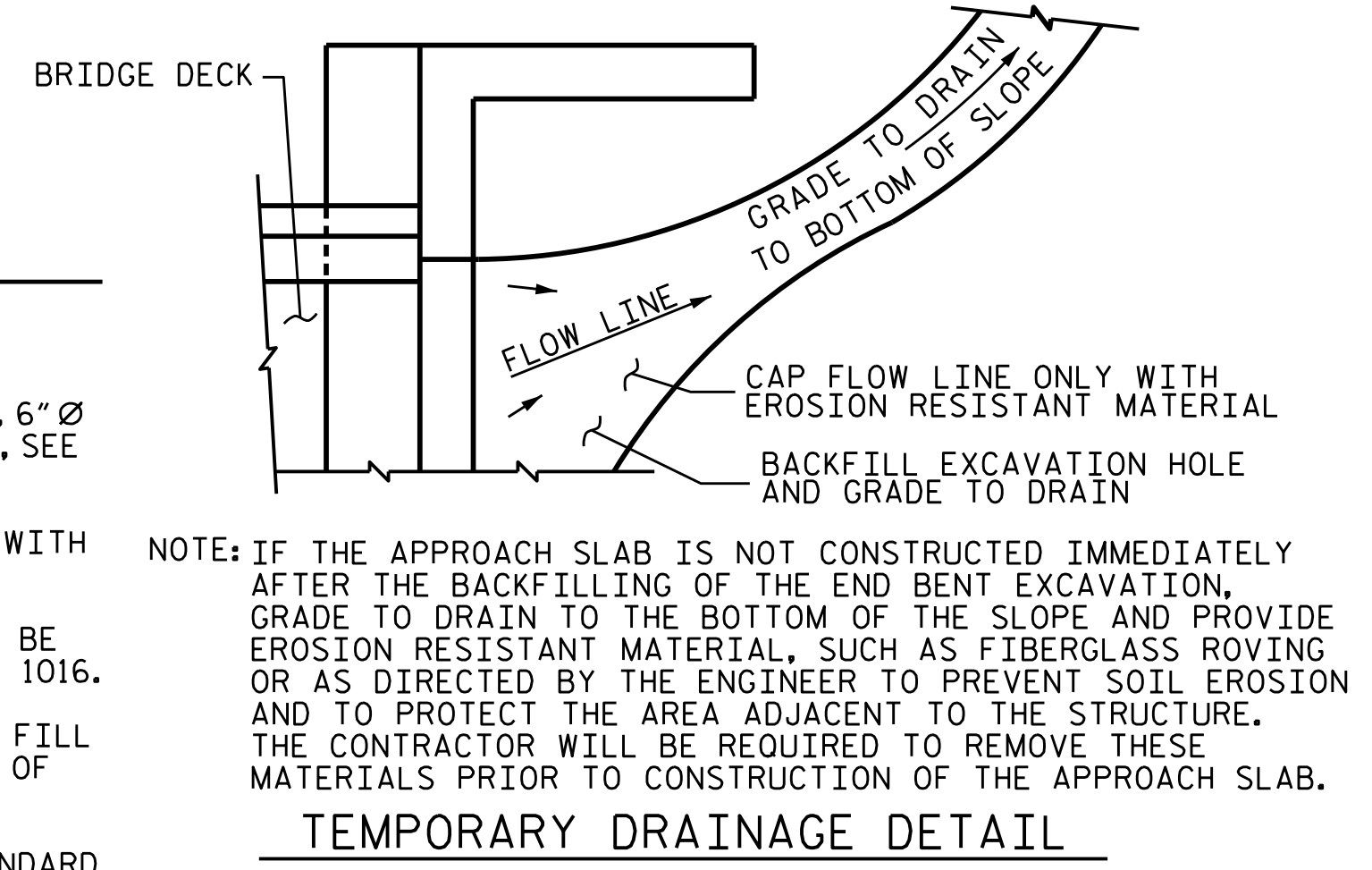
**SECTION THRU SLAB**  
★(TYPE A - ALTERNATE APPROACH FILL)



**TEMPORARY BERM AND SLOPE DRAIN DETAILS**  
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

**NOTES**

- APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.
- FOR TEMPORARY GEOTEXTILE WALL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, WELDED WIRE FORM, AND SELECT MATERIAL, SEE ROADWAY PLANS.
- GEOTEXTILE (TYPE 1 OR TYPE 5) SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.
- SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.
- SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.
- FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.
- AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.
- THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.



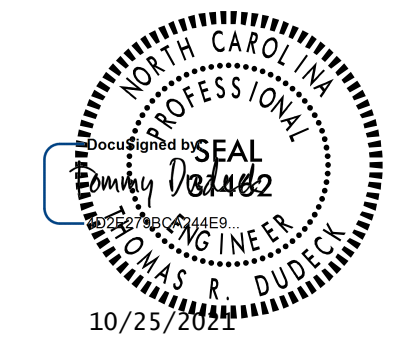
**TEMPORARY DRAINAGE DETAIL**

★ MAY BE USED IF BOTTOM PORTION OF INTEGRAL END BENT IS CONSTRUCTED BEFORE TEMPORARY WALL AND END BENT PILES ARE 25'-0" LONG OR DRILLED IN.



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ASSEMBLED BY : J. KENNEDY DATE : 12/17/17  
CHECKED BY : R. DeCOLA DATE : 01/03/18  
DRAWN BY : TLA 10/05 REV. 12/21/11 MAA/GM DESIGN  
CHECKED BY : GM 5/06 REV. 6/13 MAA/GM ENGINEER  
OF RECORD : T. R. DUDECK DATE : 10/25/21

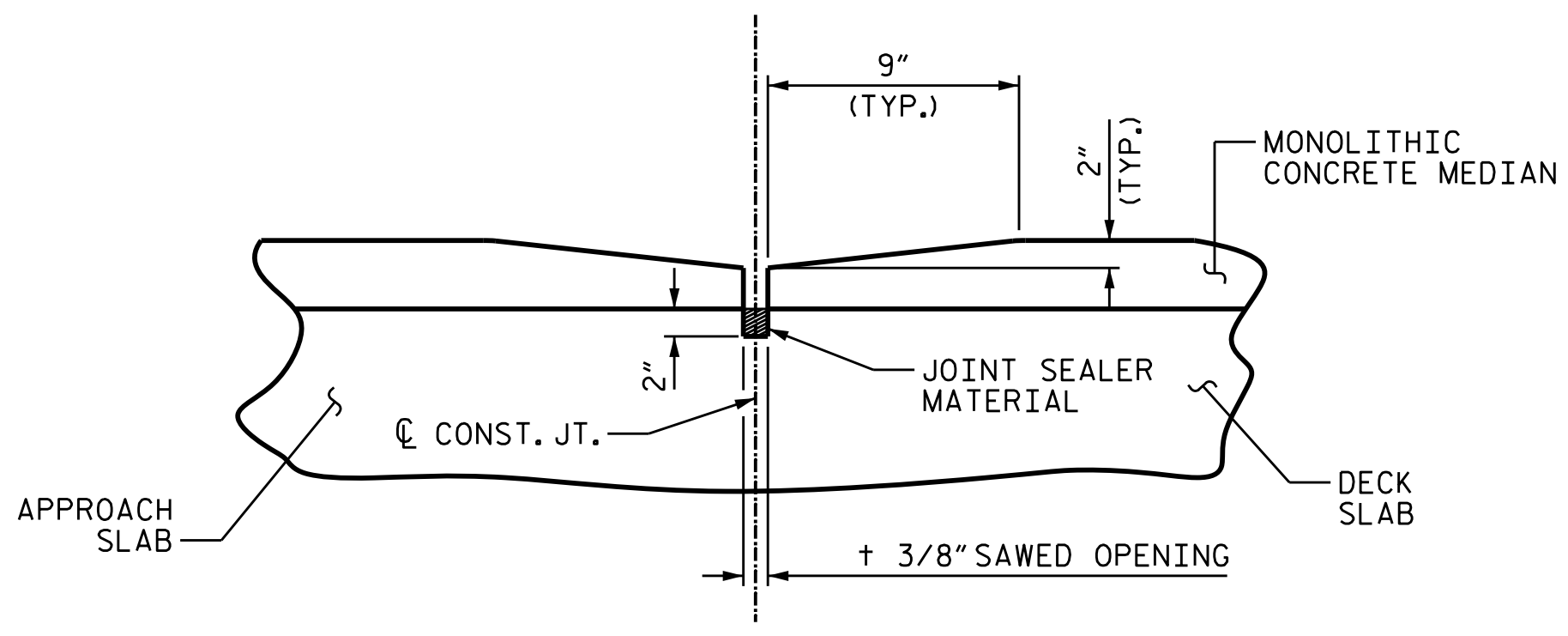


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STATION: 30+17.11 -Y31-

SHEET 2 OF 3

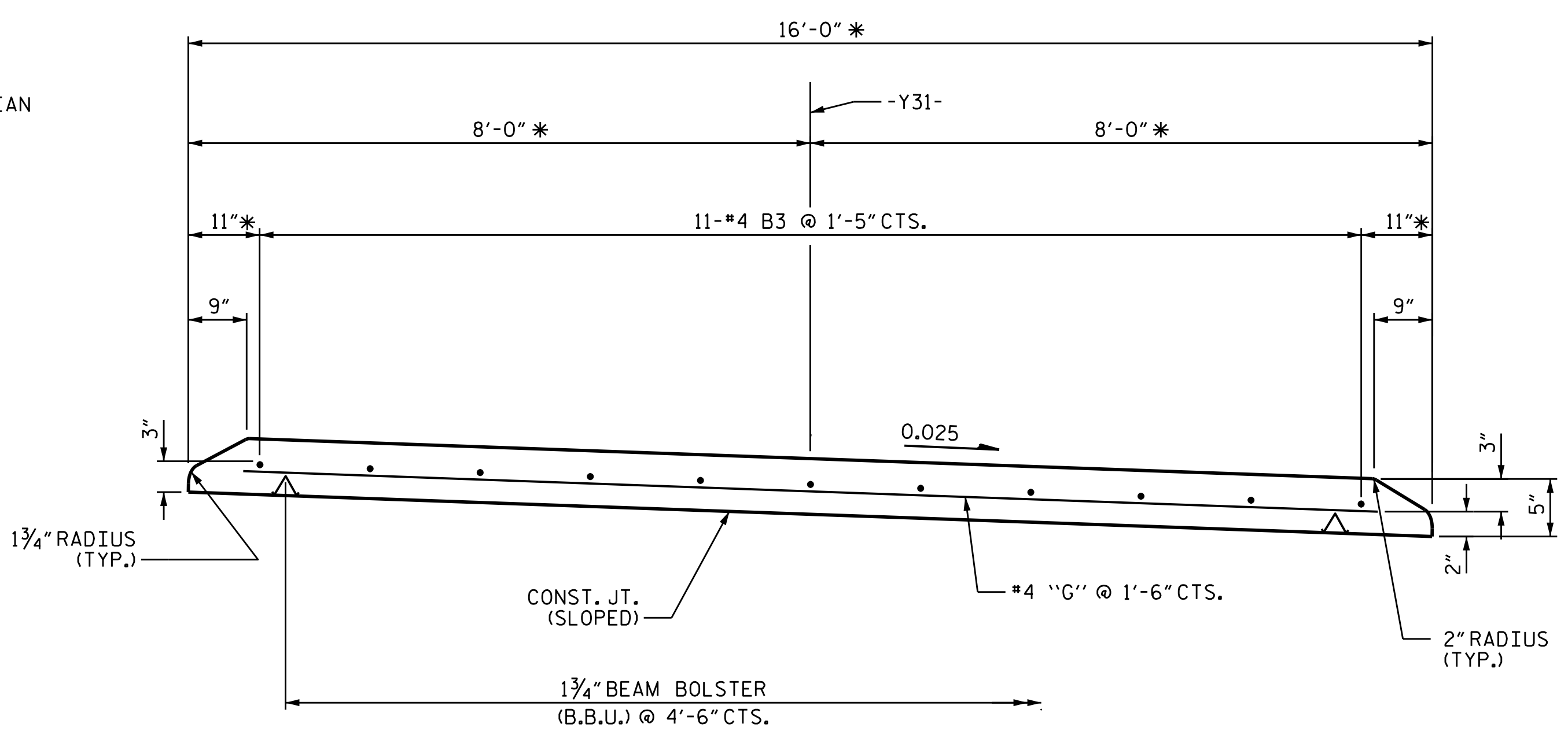
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† NORMAL TO END BENT

**SECTION THROUGH MONOLITHIC CONCRETE MEDIAN AT INTEGRAL END BENT**  
END BENT 1 SHOWN, END BENT 2 SIMILAR



**SECTION B-B**

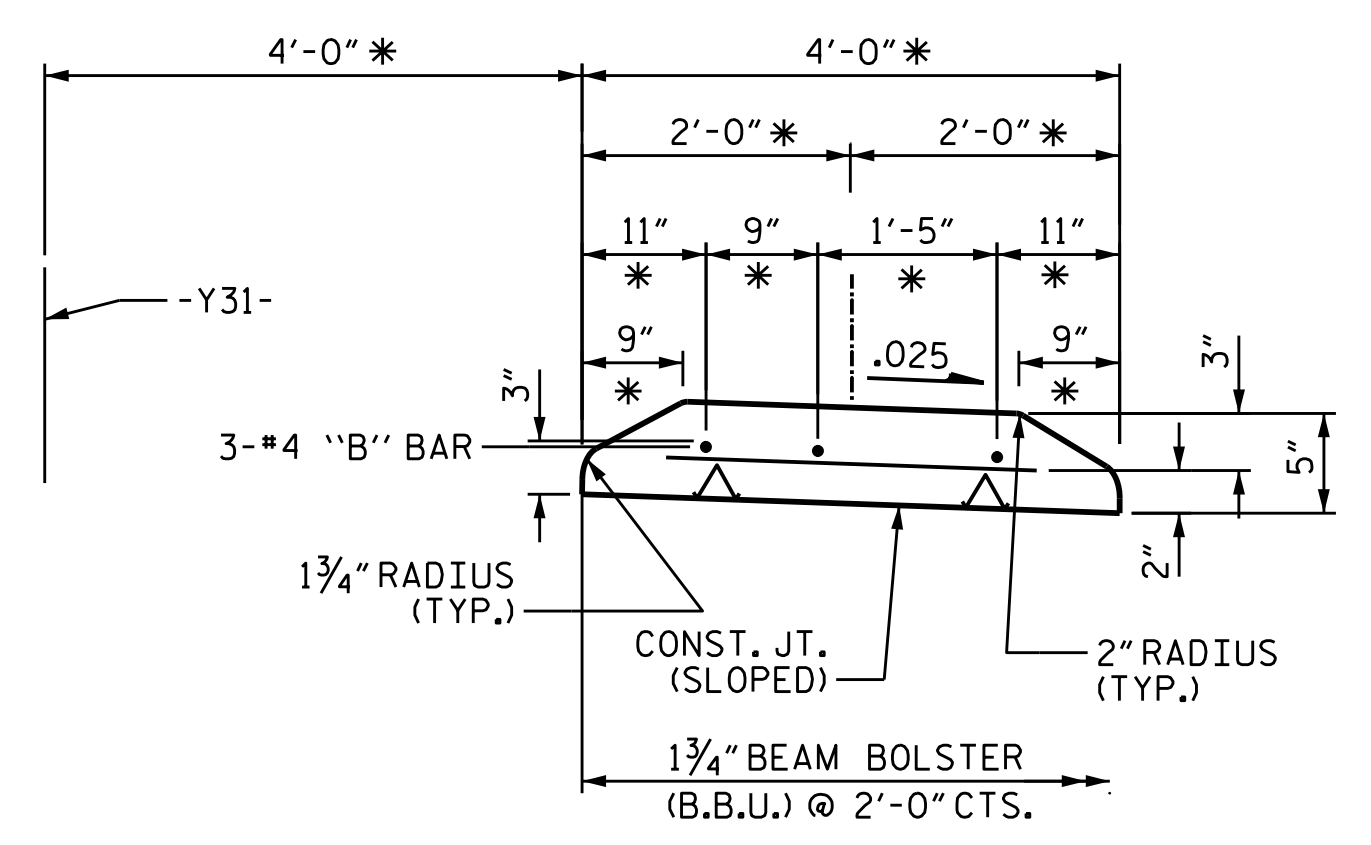
**NOTES**

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE MONOLITHIC CONCRETE MEDIAN IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT A SPACING OF 8 FT. TO 10 FT. BETWEEN EXPANSION JOINTS; NO CONTRACTION JOINT WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FT. IN LENGTH.

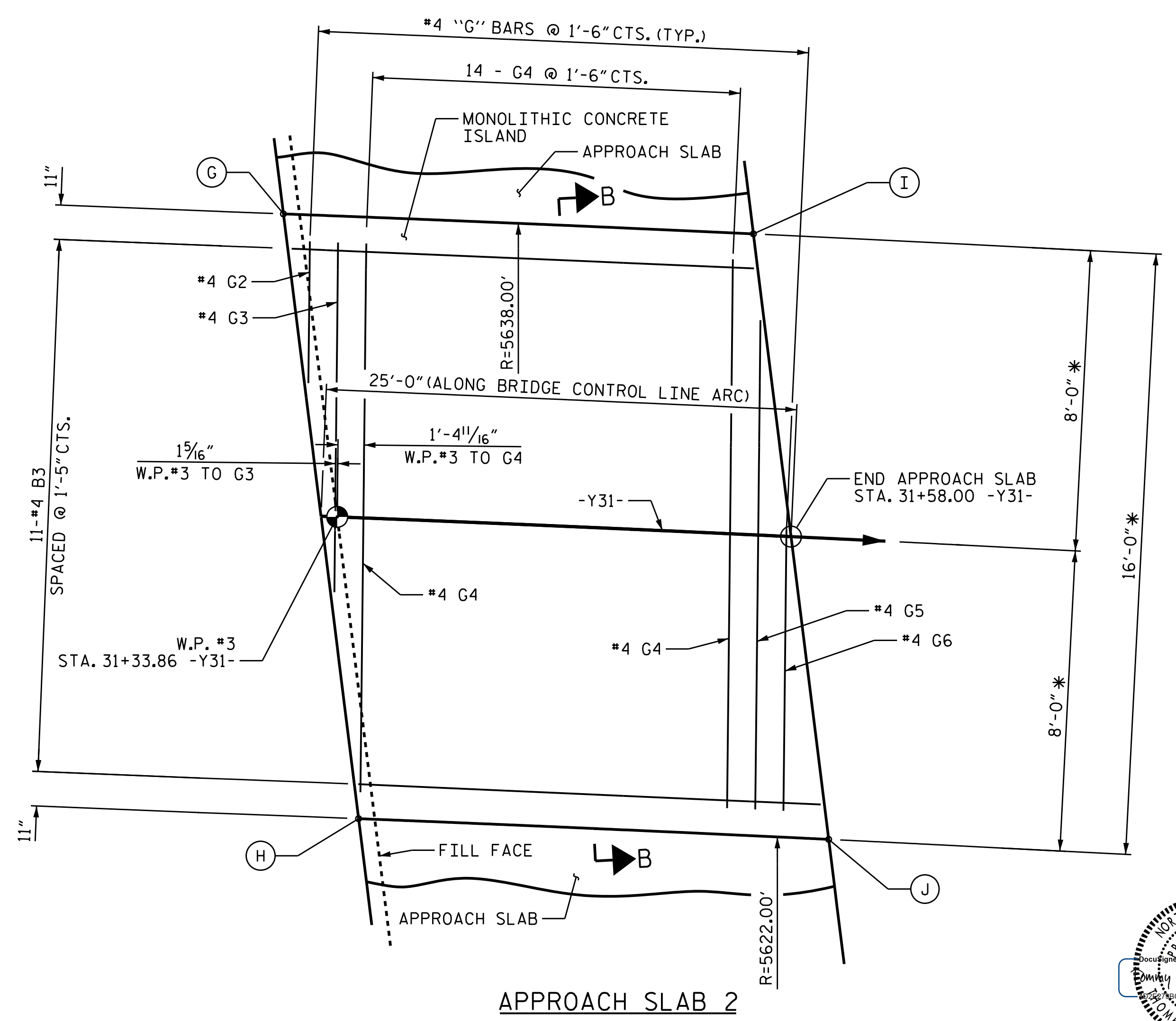
ALL REINFORCING STEEL IN MONOLITHIC CONCRETE MEDIAN SHALL BE EPOXY COATED.

NO SEPARATE MEASUREMENT OR PAYMENT WILL BE MADE FOR MATERIALS OR LABOR REQUIRED TO CONSTRUCT THE CONCRETE MEDIAN. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR BRIDGE APPROACH SLABS.

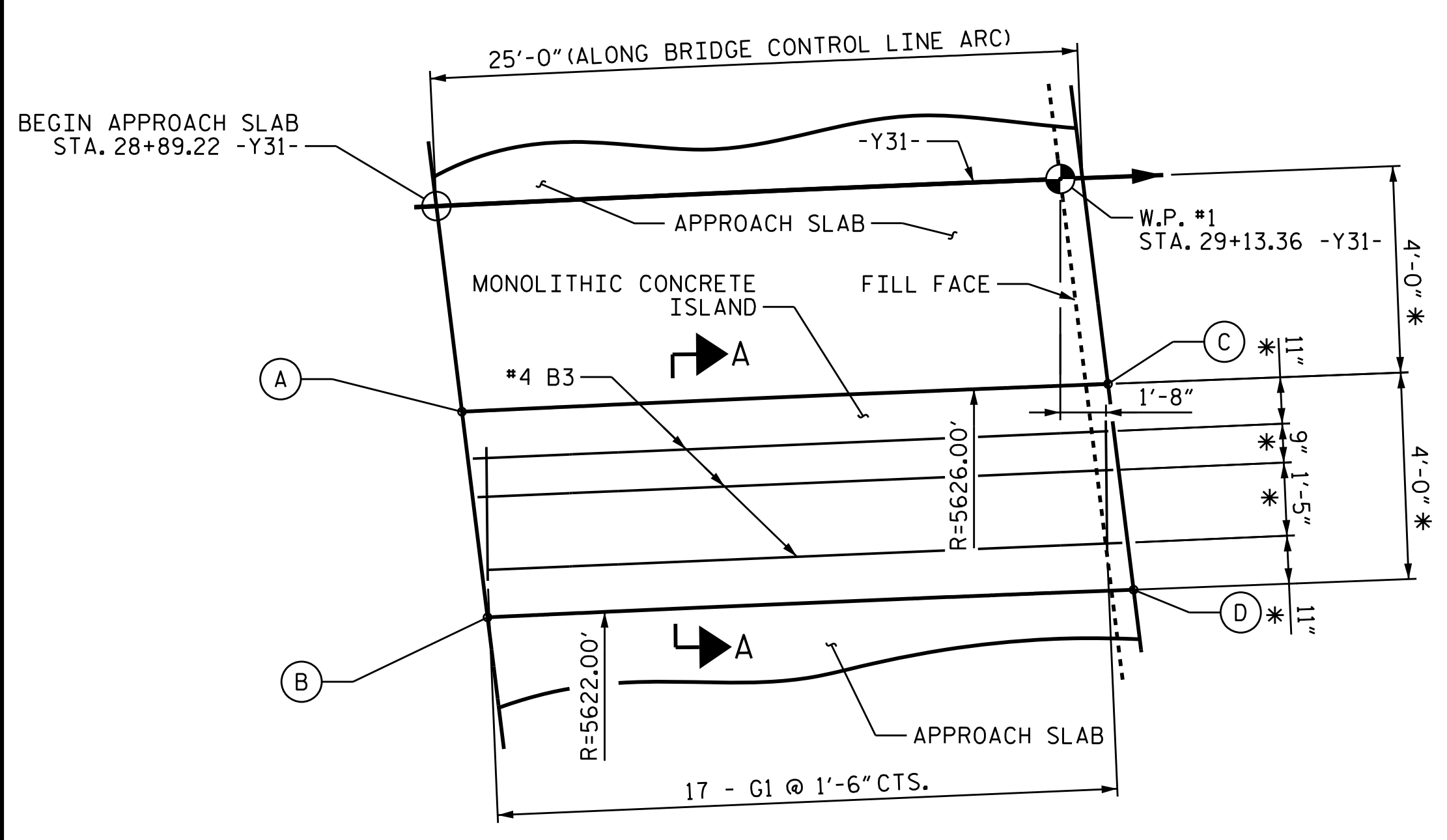
MONOLITHIC CONCRETE MEDIAN LAYOUT		
WORK POINT	STATION	OFFSET
(A)	28+90.11 -Y31-	4.00 RT.
(B)	28+90.01 -Y31-	8.00 RT.
(C)	29+15.13 -Y31-	4.00 RT.
(D)	29+16.05 -Y31-	8.00 RT.
(G)	31+30.85 -Y31-	8.00 LT.
(H)	31+35.16 -Y31-	8.00 RT.
(I)	31+55.81 -Y31-	8.00 LT.
(J)	31+60.20 -Y31-	8.00 RT.



**SECTION A-A**



**APPROACH SLAB 2**



**APPROACH SLAB 1**

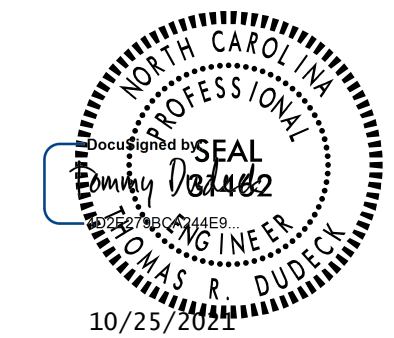
**PLAN OF MONOLITHIC CONCRETE MEDIAN**

\* RADIAL DIMENSION

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 30+17.11 -Y31-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
**APPROACH SLAB  
 MONOLITHIC CONCRETE  
 MEDIAN DETAILS**



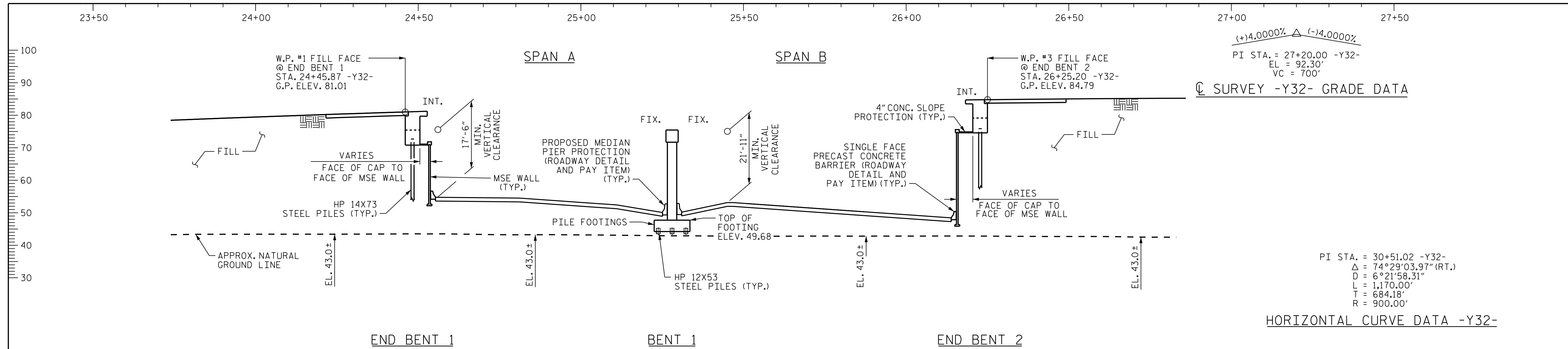
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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DRAWN BY : J. E. HAGENBUSH DATE : 03/22/18  
 CHECKED BY : R. DeCOLA DATE : 04/02/18  
 DESIGN ENGINEER OF RECORD : T. R. DUDECK DATE : 10/25/21

10/25/2021 10:25/2021 U:\Structures\B16 - Y31 - Drafting\Final\B3300B\_SML\_AS03\_700260.dgn



(+/-)4.0000% Δ (-)4.0000%

PI STA. = 27+20.00 -Y32-  
EL = 92.30'  
VC = 700'

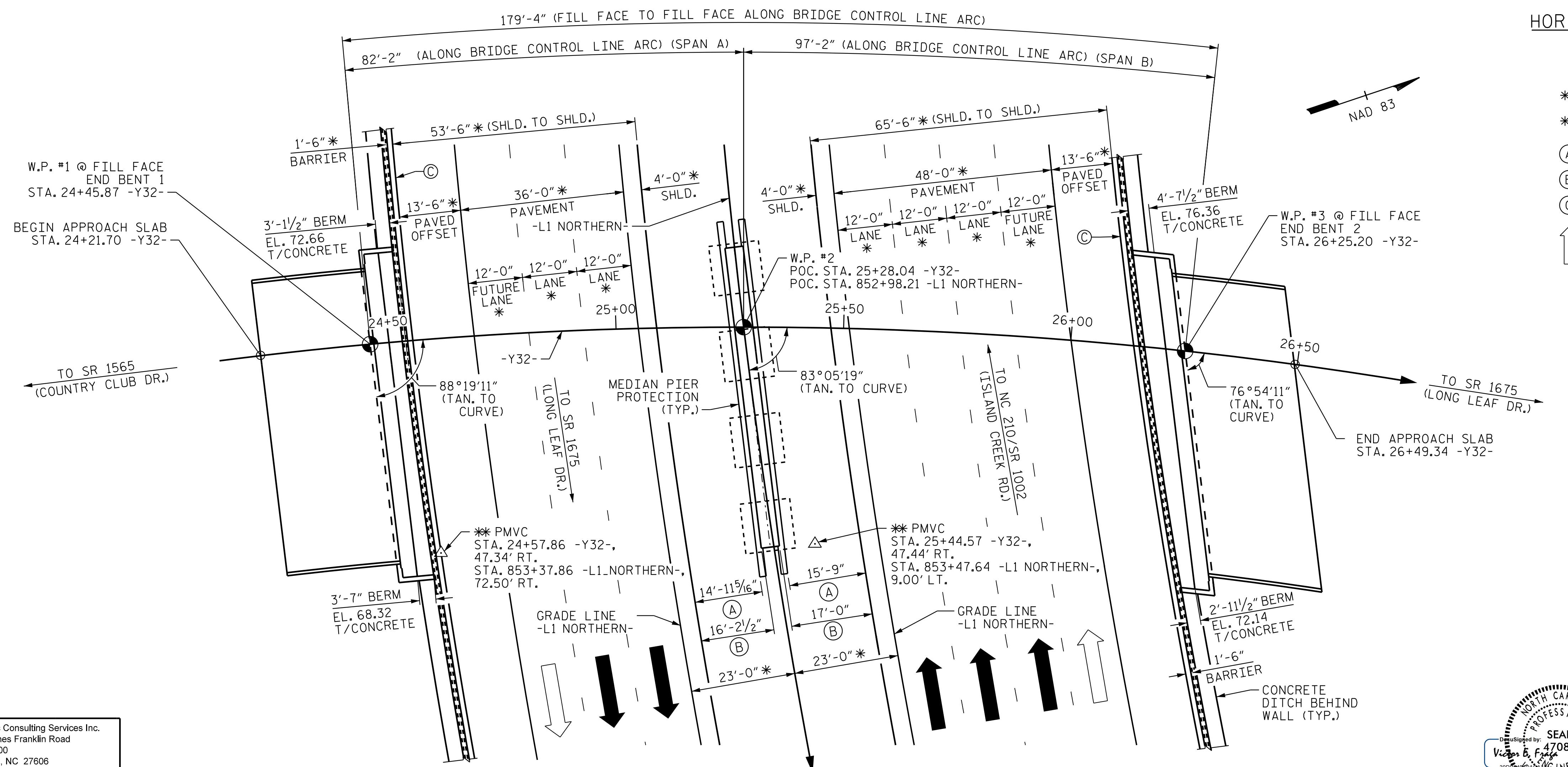
☉ SURVEY -Y32- GRADE DATA

PI STA. = 30+51.02 -Y32-  
Δ = 74°29'03.97" (RT.)  
D = 6°21'58.31"  
L = 1,170.00'  
T = 684.18'  
R = 900.00'

HORIZONTAL CURVE DATA -Y32-

PI STA. = 858+75.41 -L1 NORTHERN-  
Δ = 56°02'57.00" (LT.)  
D = 3°52'16.81"  
L = 1,447.80'  
T = 787.74'  
R = 1,480.00'

HORIZONTAL CURVE DATA -L1 NORTHERN-



- \* DENOTES RADIAL DIMENSION
- \*\* PMVC DENOTES POINT OF MINIMUM VERTICAL CLEARANCE
- (A) MIN. HORIZ. CLEARANCE TO FACE OF PIER PROTECTION
- (B) MIN. HORIZ. CLEARANCE TO FACE OF CAP
- (C) SINGLE FACE PRECAST CONCRETE BARRIER
- ↑ DENOTES FUTURE TRAVEL LANE

PROJECT NO. R-3300B

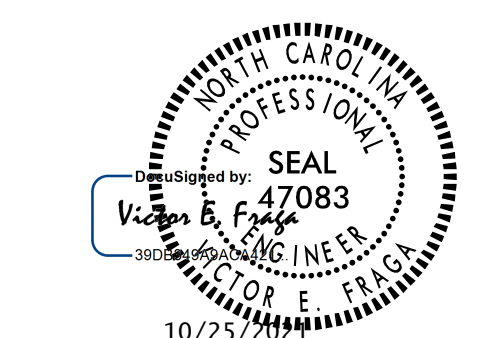
PENDER COUNTY

STATION: 852+98.21 -L1 NORTHERN-  
25+28.04 -Y32-

SHEET 1 OF 4 BRIDGE NO. 700262

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

**GENERAL DRAWING**  
FOR BRIDGE OVER NC 417  
ON SR 1593 (JENKINS RD.)  
BETWEEN DIRT ROAD & US 17 BUS.



REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S17-01	
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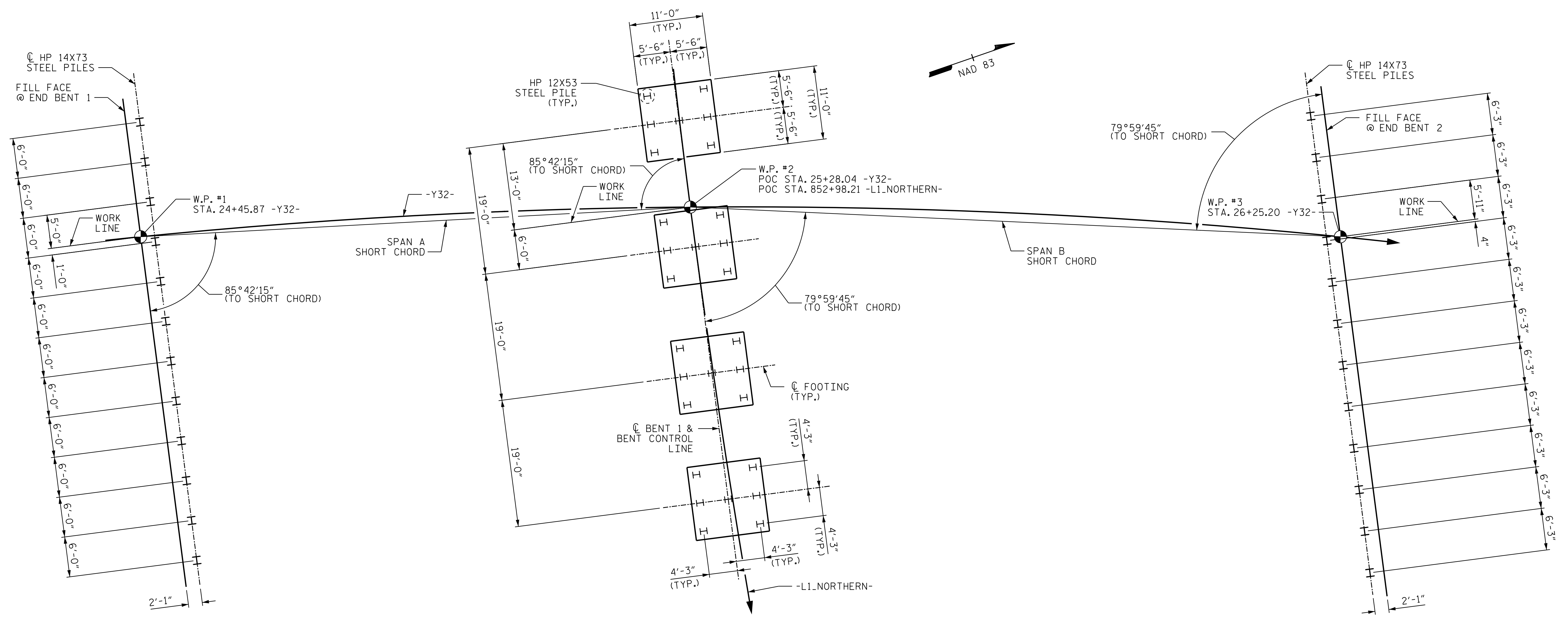
DRAWN BY : V. T. THOMPSON DATE : 09/16/19

CHECKED BY : V. E. FRAGA DATE : 02/10/20

DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

10/25/2021 jgelle U:\Structures\BIB - Y32 - Dr-off-fing\Final\R3300B.SMU.GDL.700262.dgn

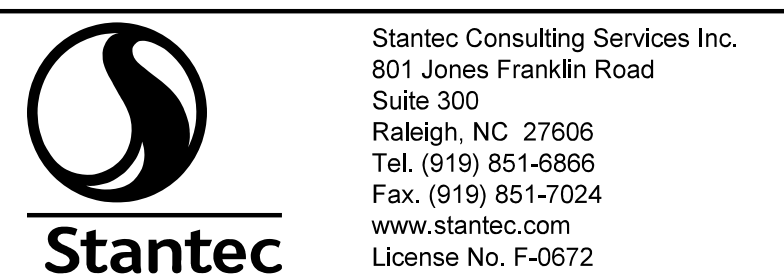
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**FOUNDATION LAYOUT**

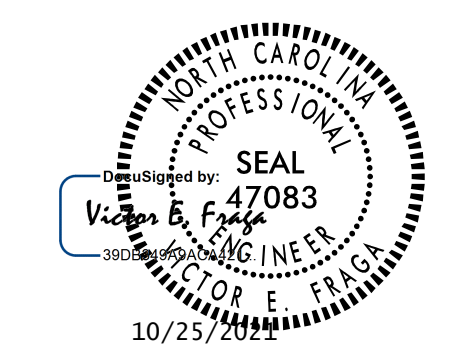
**NOTES:**

- FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 140 TONS PER PILE.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.1. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 115 TONS PER PILE.
- DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 155 TONS PER PILE.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
- DRIVE PILES AT BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 310 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT BENT NO.1. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 75 TO 125 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT NO.1. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.
- TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING, OR REDRIVING IS REQUIRED AT END BENT NO.1 OR 2 AND BENT NO.1. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- THE END BENT NO.1 AND 2 EMBANKMENTS ARE TO BE CONSTRUCTED IN STAGES. OBSERVE A 2 MONTH WAITING PERIOD FOR STAGE 1 AFTER PLACING FILL AND CONSTRUCTING THE MECHANICALLY STABILIZED EARTH (MSE) ABUTMENT WALLS TO EL. 72 FT BEFORE CONTINUING FILL PLACEMENT. OBSERVE AN ADDITIONAL 4 MONTH WAITING PERIOD FOR STAGE 2 AFTER CONSTRUCTING THE MECHANICALLY STABILIZED EARTH (MSE) ABUTMENT WALL AND THE SURCHARGE WITH TEMPORARY FABRIC WALL BEFORE BEGINNING END BENT CONSTRUCTION AT END BENTS NO.1 AND 2. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS.
- SEE ROADWAY PLANS AND SECTION 235 OF THE STANDARD SPECIFICATIONS FOR THE SETTLEMENT GAUGES, STAGED CONSTRUCTION, AND SURCHARGE REQUIRED AT END BENTS NO.1 AND 2.



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DRAWN BY : T. N. ENNIS DATE : 01/23/20  
 CHECKED BY : V. E. FRAGA DATE : 02/11/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21



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PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

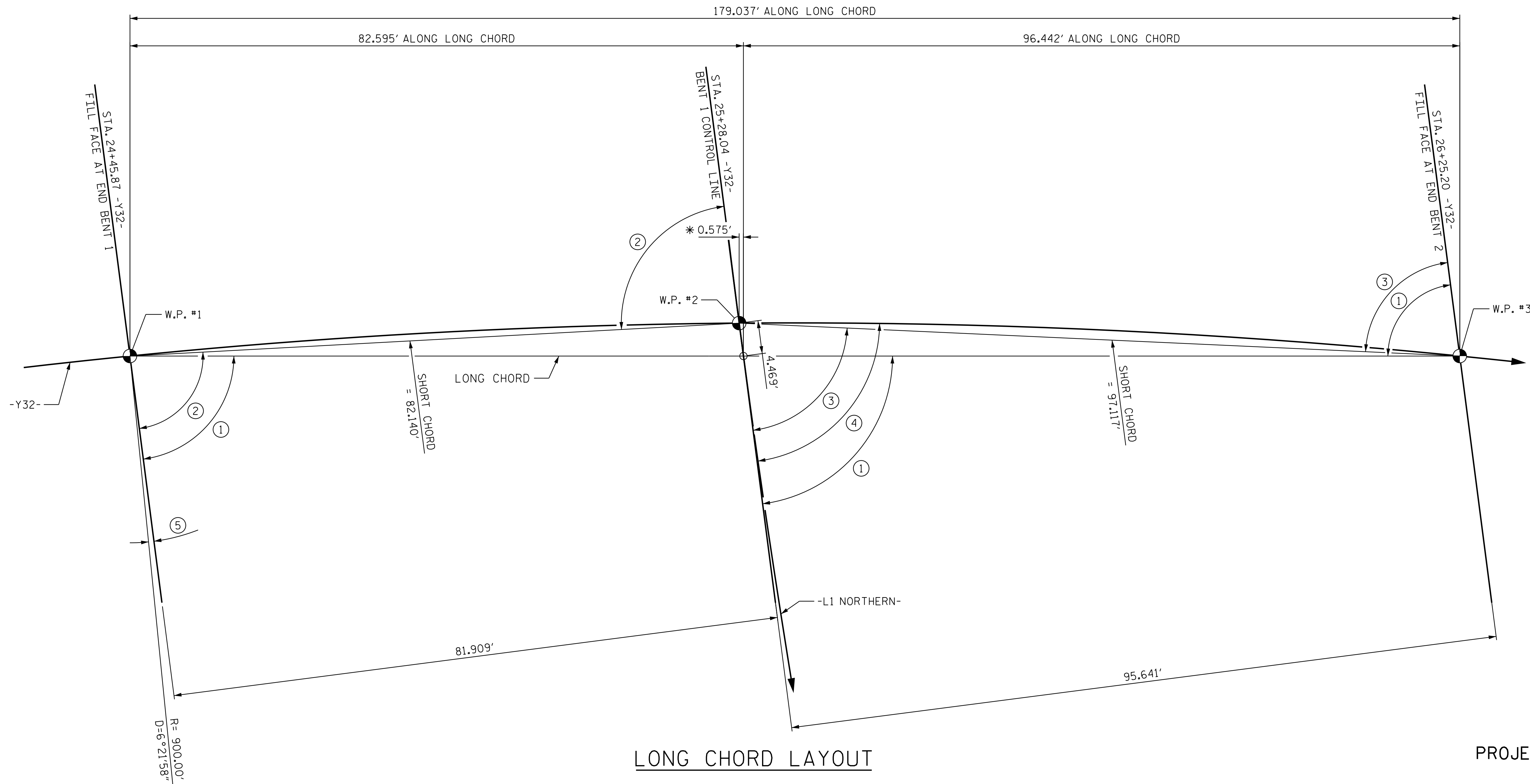
SHEET 2 OF 4

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S17-02
1			3			TOTAL SHEETS
2			4			38

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**

FOR BRIDGE OVER NC 417  
 ON SR 1593 (JENKINS RD.)  
 BETWEEN DIRT ROAD & US 17 BUS.



**LONG CHORD LAYOUT**

- ANGLES**
- ① 82°36'41"
  - ② 85°42'15"
  - ③ 79°59'45"
  - ④ 83°05'19" TANGENT TO CURVE
  - ⑤ 01°40'49"

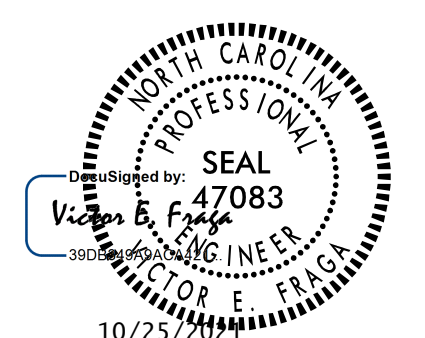
**NOTES:**  
 ALL BENTS ARE PARALLEL.  
 \* POINT ON BENT CONTROL LINE PROJECTED ON LONG CHORD.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

SHEET 3 OF 4

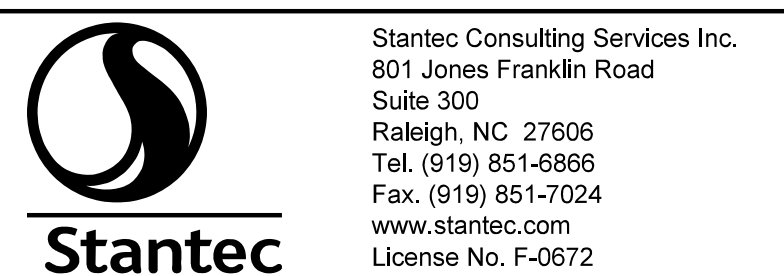
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
 FOR BRIDGE OVER NC 417  
 ON SR 1593 (JENKINS RD.)  
 BETWEEN DIRT ROAD & US 17 BUS.



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NO.	BY:	DATE:	NO.	BY:	DATE:	S17-03
1			3			TOTAL SHEETS
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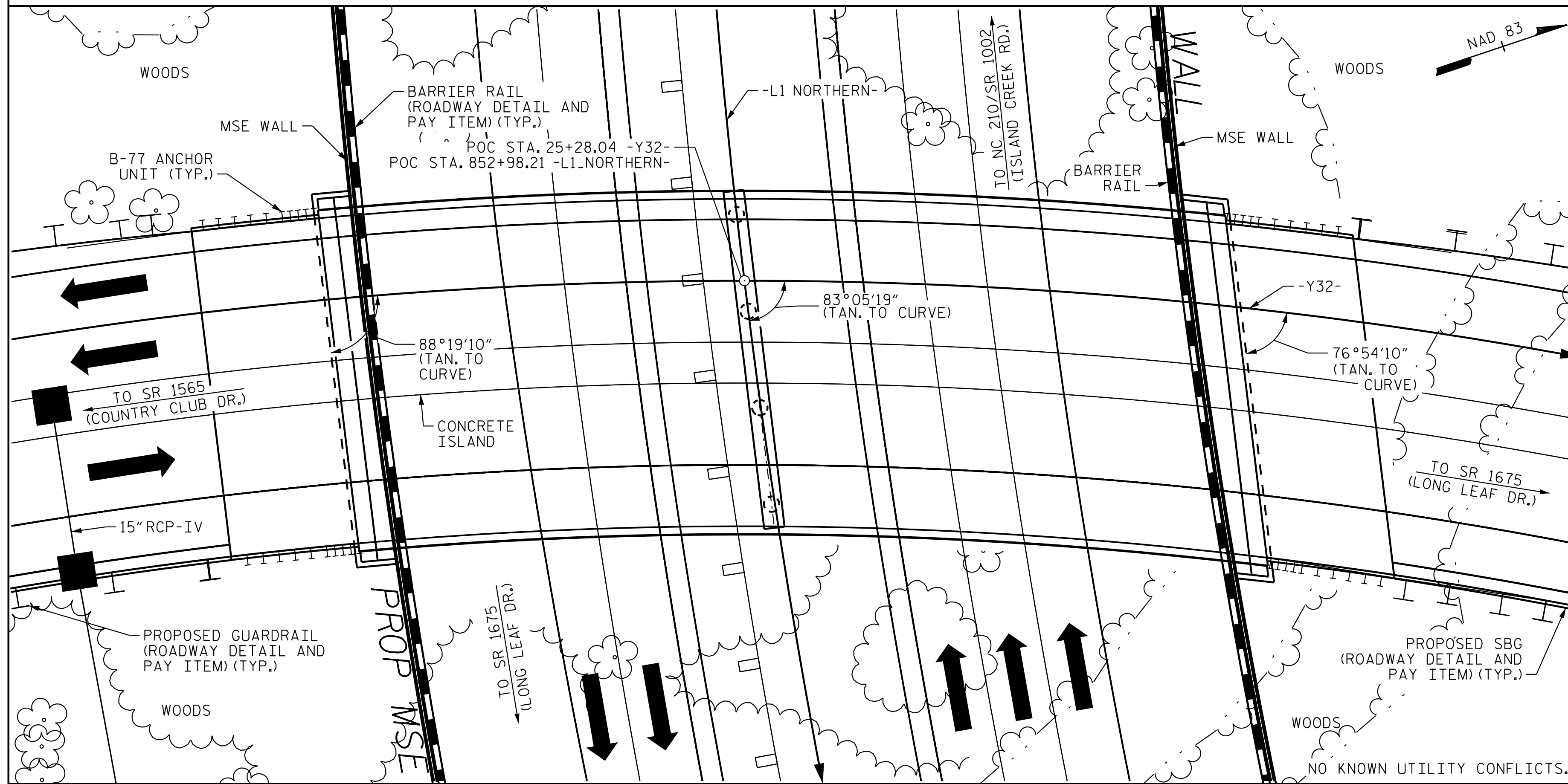


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DRAWN BY : T. N. ENNIS DATE : 01/23/20  
 CHECKED BY : V. E. FRAGA DATE : 02/11/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

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 10/25/2021  
 jgelle

BM #35: R/R SPIKE IN 12" PINE. N240112 E2399751, STA. 845+19.76 -L1-NORTH-, 858.38' LT. EL. 57.17



LOCATION SKETCH

**NOTES**

- ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.
- NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- FOR CALCIUM NITRITE CORROSION INHIBITOR, SEE STANDARD SPECIFICATIONS.
- FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- FOR MODIFIED 54" PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

**TOTAL BILL OF MATERIAL**

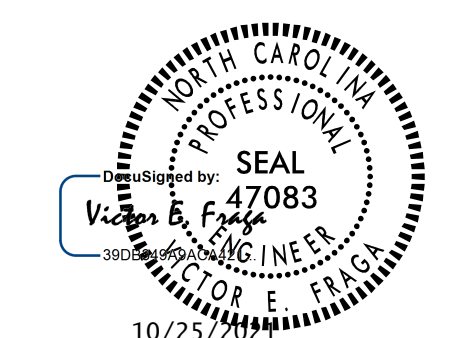
	FOUNDATION EXCAVATION FOR BENT	PDA TESTING	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR HP 14 X 73 STEEL PILES	HP 12 X 53 STEEL PILES		HP 14 X 73 STEEL PILES		STEEL PILE POINTS	PILE REDRIVES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	MODIFIED 54" PRESTRESSED CONCRETE GIRDERS		
	LUMP SUM	EA.	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	EA.	EA.	NO.	LIN. FT.	NO.	LIN. FT.	EA.	EA.	LIN. FT.	SQ. YD.	LUMP SUM	NO.	LIN. FT.	
SUPERSTRUCTURE			11,952	13,768		LUMP SUM											355.4		LUMP SUM	14	1,229.9	
END BENT 1					55.0		7,298			12			12	1,080	12	6			85			
BENT 1	LUMP SUM				137.6		28,688	2,074	28		28	2,660			28	14						
END BENT 2					56.7		7,827			12			12	1,200	12	6			123			
TOTAL	LUMP SUM	2	11,952	13,768	249.3	LUMP SUM	43,813	2,074	28	24	28	2,660	24	2,280	52	26	355.4	208	LUMP SUM	14	1,229.9	

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTH-  
25+28.04 -Y32-

SHEET 4 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
 FOR BRIDGE OVER NC 417  
 ON SR 1593 (JENKINS RD.)  
 BETWEEN DIRT ROAD & US 17 BUS.



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DRAWN BY : V. E. FRAGA DATE : 02/10/20  
 CHECKED BY : T. R. DUDECK DATE : 02/24/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

10/25/2021 10/25/2021 U:\Structures\BIB - Y32 - Dr-off-ting\Final\R3300B.SMU.G02.7002662.dgn

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	$\gamma_{DC}$	$\gamma_{DW}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.09	--	1.75	0.752	1.18	A	EL	78.90	0.796	1.13	B	I	18.30	0.80	0.817	1.09	A	I	39.50		
	HL-93 (OPERATING)	N/A		1.48	--	1.35	0.752	1.53	A	EL	78.90	0.796	1.48	B	I	18.30	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.12	40.3	1.75	0.817	1.86	A	I	39.50	0.796	1.51	B	I	18.40	0.80	0.750	1.12	B	ER	0.00		
	HS-20 (OPERATING)	36.000		1.99	71.6	1.35	0.817	2.40	A	I	39.50	0.796	1.99	B	I	18.30	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		2.16	29.2	1.40	0.817	5.29	A	I	39.50	0.796	5.07	B	I	18.40	0.80	0.752	2.16	A	ER	78.90	
		SNGARBS2	20.000		1.48	29.6	1.40	0.817	3.90	A	I	39.50	0.796	3.53	B	I	18.40	0.80	0.752	1.48	A	ER	78.90	
		SNAGRIS2	22.000		1.34	29.5	1.40	0.817	3.67	A	I	39.50	0.796	3.25	B	I	18.40	0.80	0.752	1.34	A	ER	78.90	
		SNCOTTS3	27.250		1.28	34.9	1.40	0.817	2.62	A	I	39.50	0.796	2.45	B	I	18.40	0.80	0.752	1.28	A	I	78.90	
		SNAGGRS4	34.925		1.36	47.5	1.40	0.817	2.19	A	I	39.50	0.796	1.92	B	I	18.30	0.80	0.817	1.36	A	I	39.50	
		SNS5A	35.550		1.34	47.6	1.40	0.817	2.15	A	I	39.50	0.796	1.93	B	I	18.30	0.80	0.817	1.34	A	I	39.50	
		SNS6A	39.950		1.22	48.7	1.40	0.817	1.97	A	I	39.50	0.796	1.74	B	I	18.30	0.80	0.817	1.22	A	I	39.50	
		SNS7B	42.000		1.16	48.7	1.40	0.817	1.87	A	I	39.50	0.796	1.69	B	I	18.30	0.80	0.817	1.16	A	I	39.50	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.20	39.6	1.40	0.817	2.40	A	I	39.50	0.796	2.07	B	I	18.30	0.80	0.750	1.20	B	ER	0.00	
		TNT4A	33.075		1.20	39.7	1.40	0.817	2.41	A	I	39.50	0.796	2.17	B	I	18.30	0.80	0.750	1.20	B	ER	0.00	
		TNT6A	41.600		1.22	50.8	1.40	0.817	1.96	A	I	39.50	0.796	1.78	B	I	18.30	0.80	0.817	1.22	A	I	39.50	
		TNT7A	42.000		1.22	51.2	1.40	0.817	1.97	A	I	39.50	0.796	1.75	B	I	18.30	0.80	0.817	1.22	A	I	39.50	
		TNT7B	42.000		1.26	52.9	1.40	0.817	2.02	A	I	39.50	0.796	1.67	B	I	18.30	0.80	0.817	1.26	A	I	39.50	
		TNAGRIT4	43.000		1.21	52.0	1.40	0.817	1.94	A	I	39.50	0.796	1.64	B	I	18.30	0.80	0.817	1.21	A	I	39.50	
TNAGT5A	45.000		1.14	51.3	1.40	0.817	1.84	A	I	39.50	0.796	1.56	B	I	18.30	0.80	0.817	1.14	A	I	39.50			
TNAGT5B	45.000		③	1.13	50.9	1.40	0.817	1.81	A	I	39.50	0.796	1.54	B	I	18.40	0.80	0.817	1.13	A	I	39.50		

**NOTES:**  
 MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.  
 ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.  
 FOR LOAD RATING PURPOSES, IT IS ASSUMED A TIME PERIOD OF AT LEAST 40 DAYS SHALL ELAPSE FROM THE TRANSFER OF PRESTRESSING FORCE INTO ANY GIRDER AND THE PLACEMENT OF DECK SLAB CONCRETE IN THE SPANS.

# CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

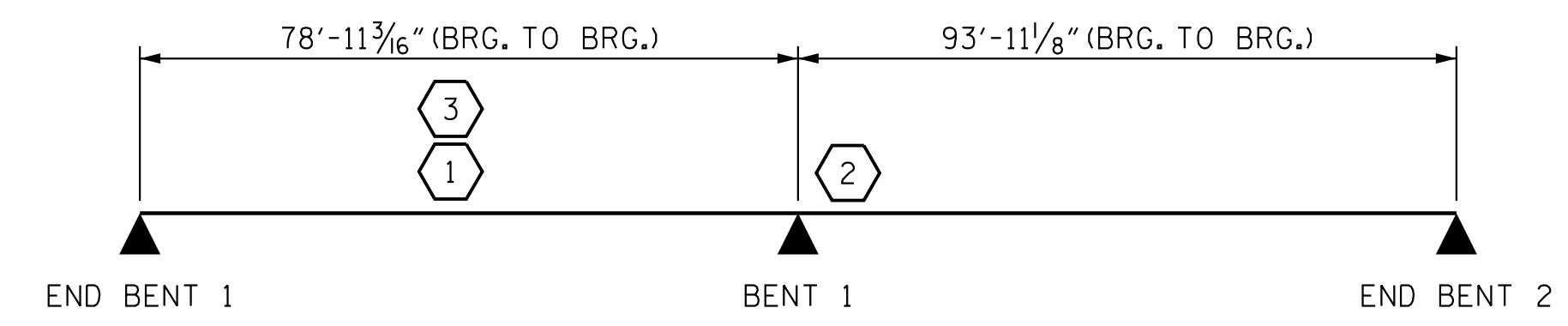
GIRDER LOCATION

I - INTERIOR GIRDER  
 EL - EXTERIOR LEFT GIRDER  
 ER - EXTERIOR RIGHT GIRDER

**GIRDER PROPERTIES**

54" MODIFIED BULB TEE

AREA = 707 SQ. IN.  
 WEIGHT = 737 LB/FT.  
 $Y_{BOTTOM} = 27.79$  IN.  
 $Y_{TOP} = 26.21$  IN.  
 $I_{xx} = 277,560$  IN.<sup>4</sup>  
 $I_{yy} = 40,051$  IN.<sup>4</sup>  
 $V/S = 3.23$  IN.



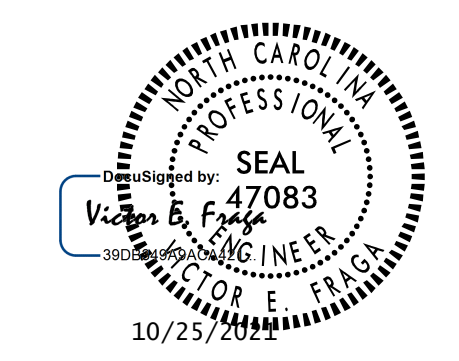
**LRFR SUMMARY**

SPAN LENGTHS REPRESENT THE AVERAGE SPAN LENGTH FROM BEARING TO BEARING, ACTUAL BEARING TO BEARING LENGTHS VARY PER GIRDER.

PROJECT NO. R-3300B

PENDER COUNTY

STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-



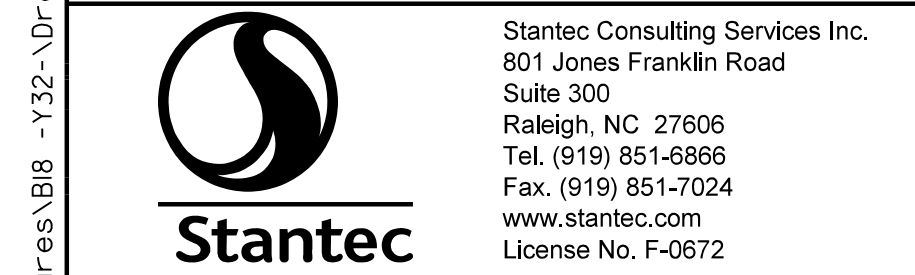
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD  
 LRFR SUMMARY FOR  
 PRESTRESSED  
 CONCRETE GIRDERS  
 (NON-INTERSTATE TRAFFIC)

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1			3			TOTAL SHEETS
2			4			38

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STD. NO. LRFR1



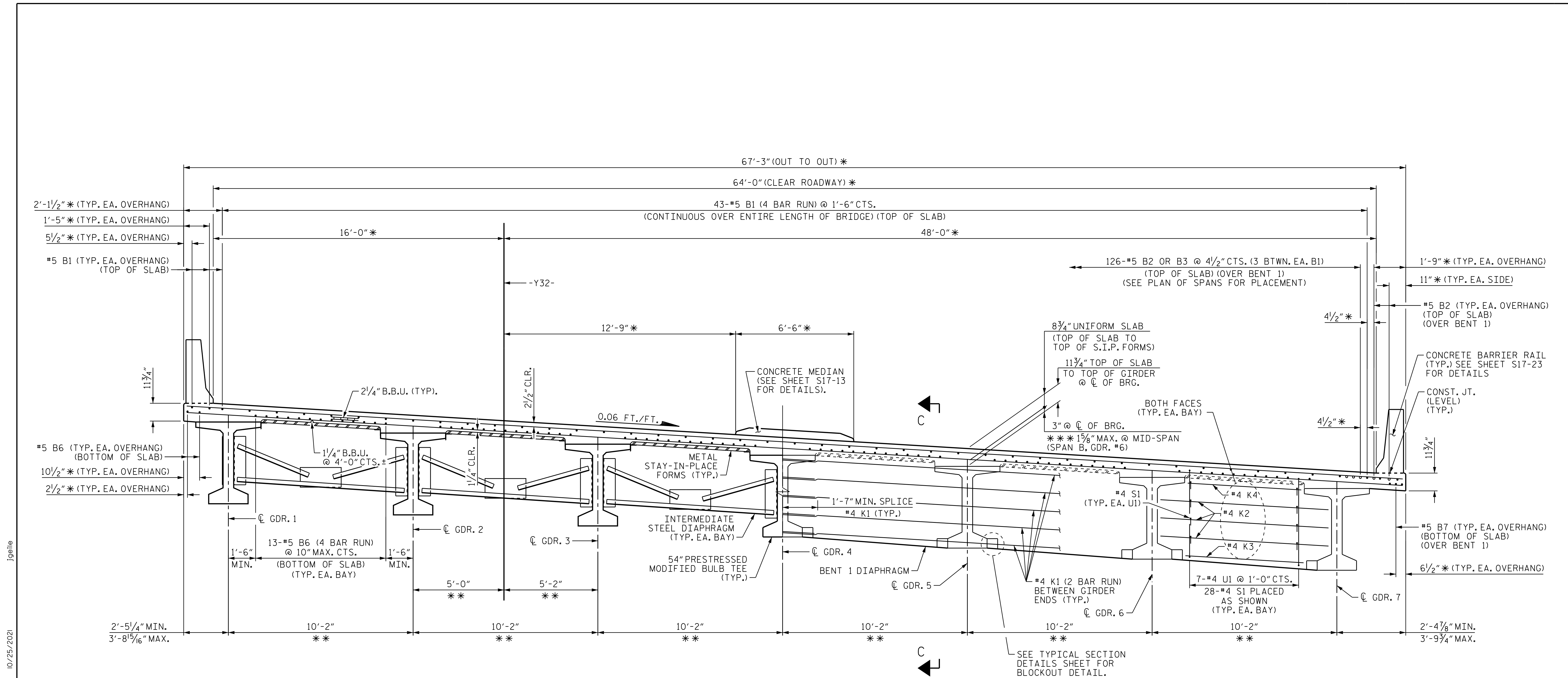
ASSEMBLED BY: V. E. FRAGA DATE: 01/30/20  
 CHECKED BY: T. N. ENNIS DATE: 02/10/20

DRAWN BY: MAA 1/08 REV. 10/1/11 MAA/GM  
 CHECKED BY: GM/DI 2/08 REV. 12/17 MAA/THC

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21

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HALF SECTION AT INTERMEDIATE DIAPHRAGM

HALF SECTION AT BENT 1 DIAPHRAGM

### TYPICAL SECTION

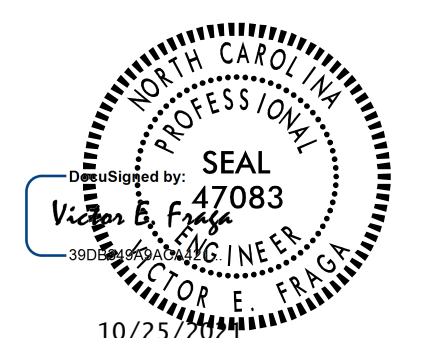
- \* DENOTES RADIAL DIMENSION.
- \*\* RADIAL DIMENSION. GIRDERS ARE PLACED ALONG CHORDS INTERSECTING CONCENTRIC ARCS AT END BENT FILL FACES AND BENT CONTROL LINE. SEE FRAMING PLAN FOR MORE DETAIL ON GIRDER PLACEMENT.
- SEE CHORD TO ARC OFFSETS FOR MORE INFORMATION ON OVERHANGS.
- \*\*\* BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE

## TYPICAL SECTION



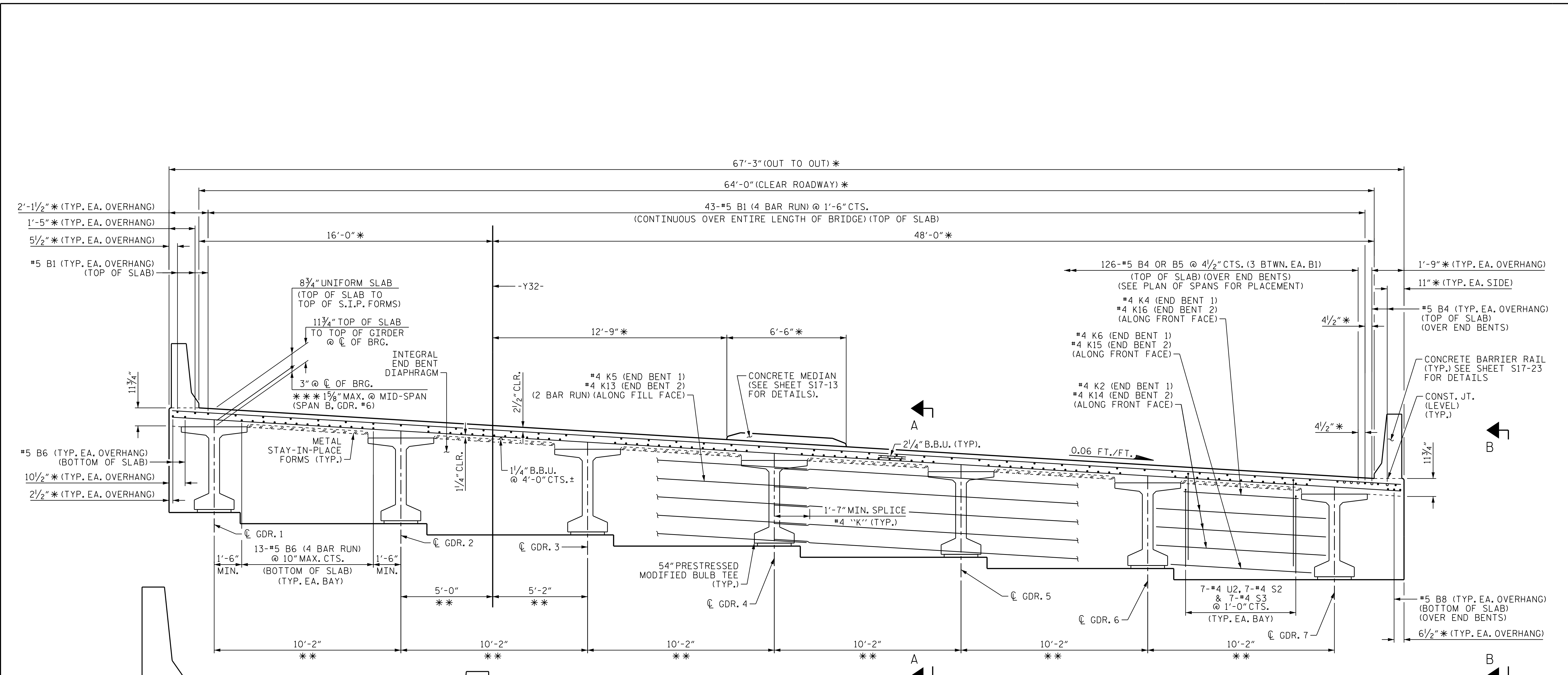
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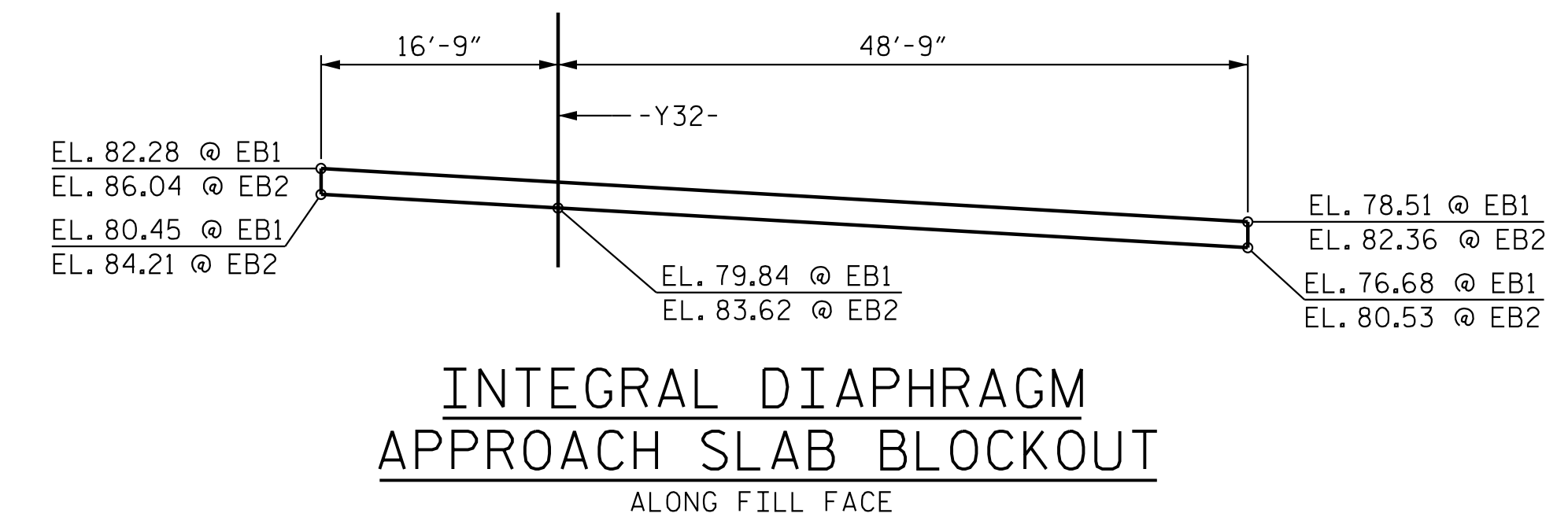
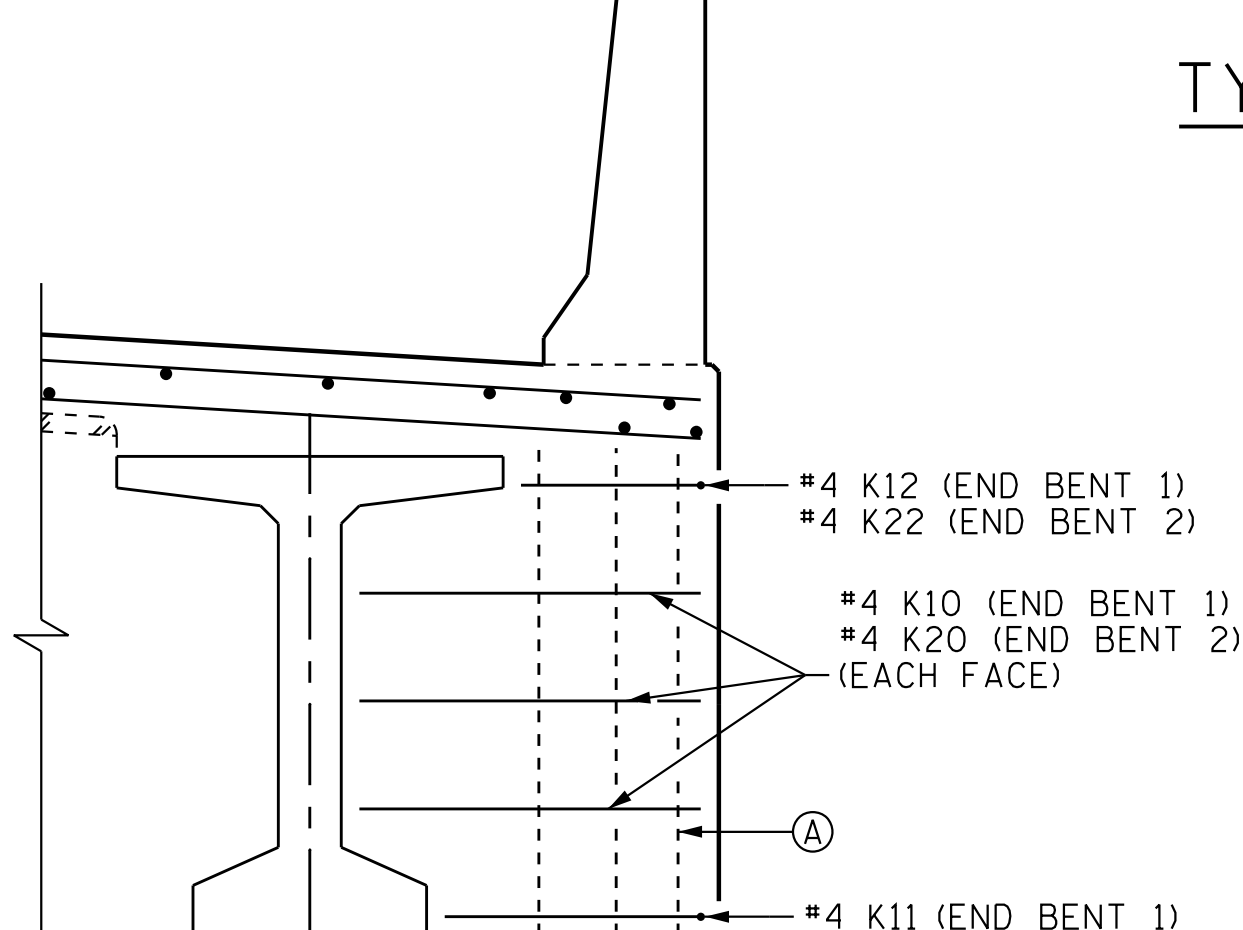
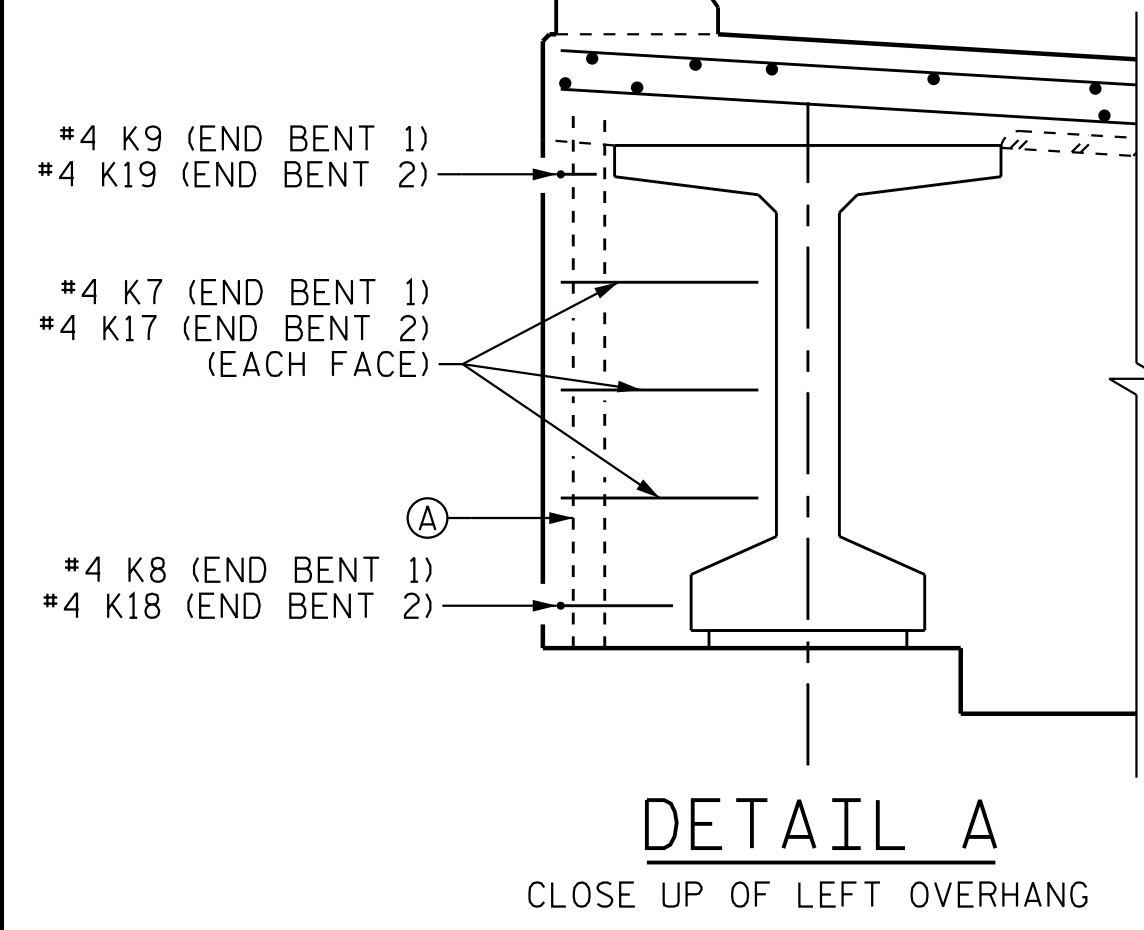
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**TYPICAL SECTION AT END BENT**

- \* DENOTES RADIAL DIMENSION.
- \*\* RADIAL DIMENSION, GIRDERS ARE PLACED ALONG CHORDS INTERSECTING CONCENTRIC ARCS AT END BENT FILL FACES AND BENT CONTROL LINE. SEE FRAMING PLAN FOR MORE DETAIL ON GIRDER PLACEMENT.
- Ⓐ #4 "V" BARS PROTRUDING FROM CAP (SEE END BENT DRAWINGS FOR DETAILS)
- \*\*\* BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS.

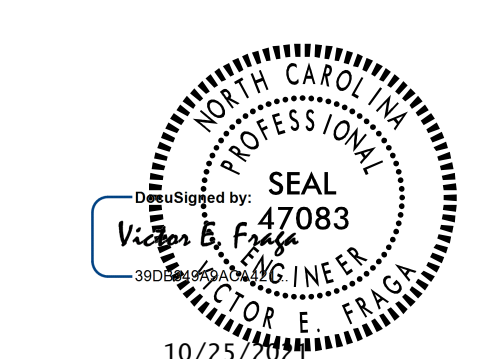


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PENDER COUNTY  
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25+28.04 -Y32-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE

**TYPICAL SECTION**



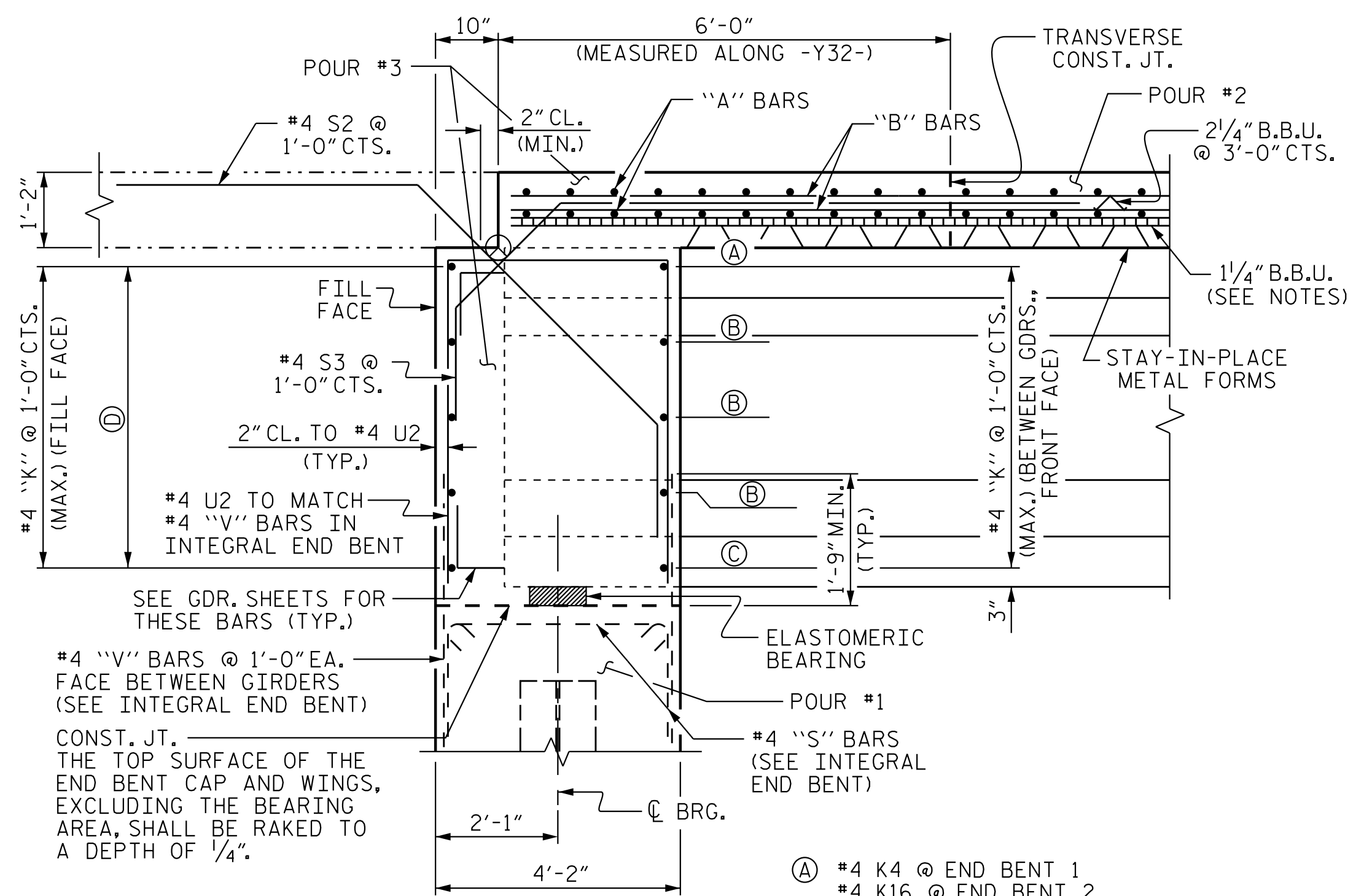
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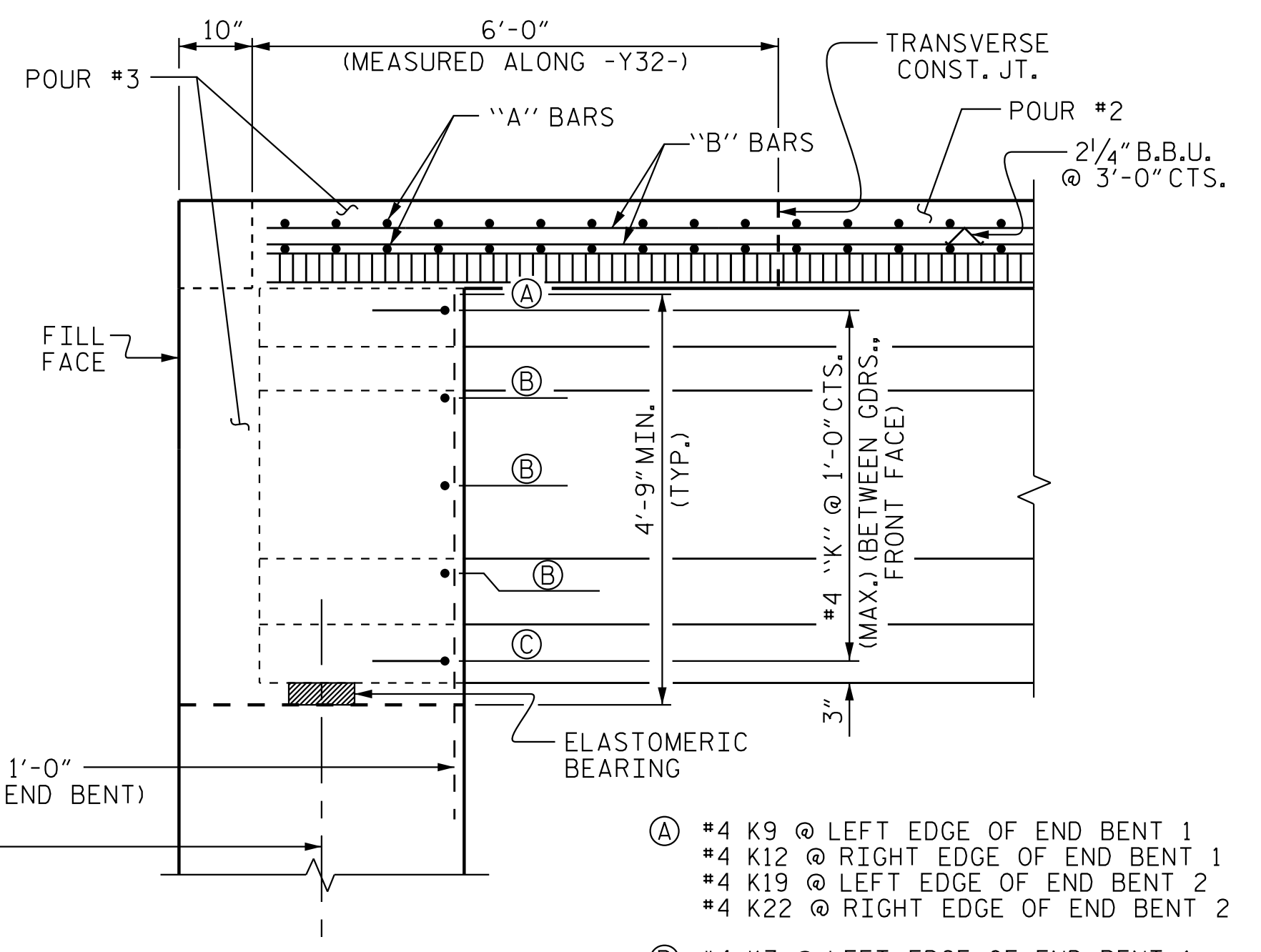
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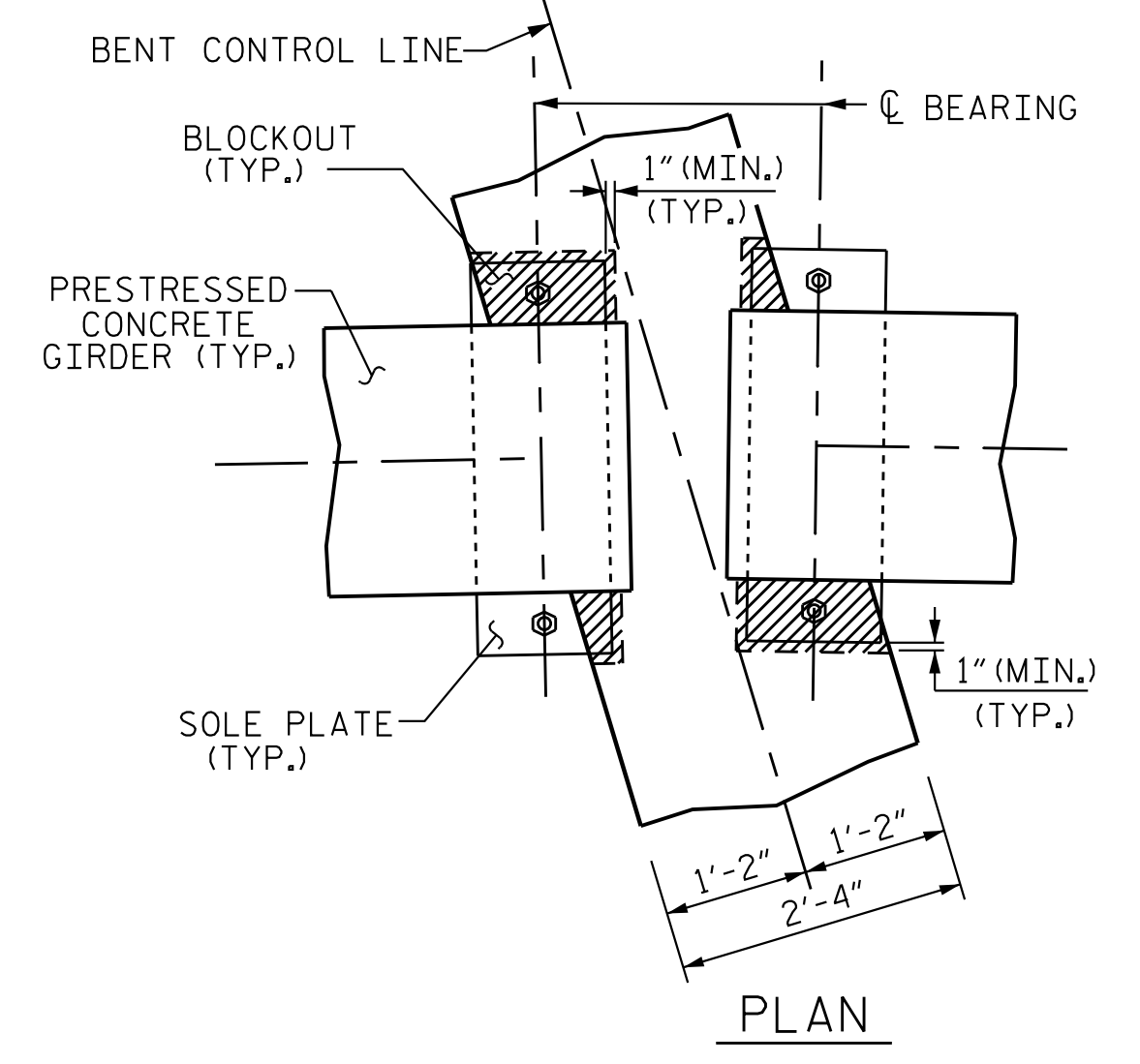
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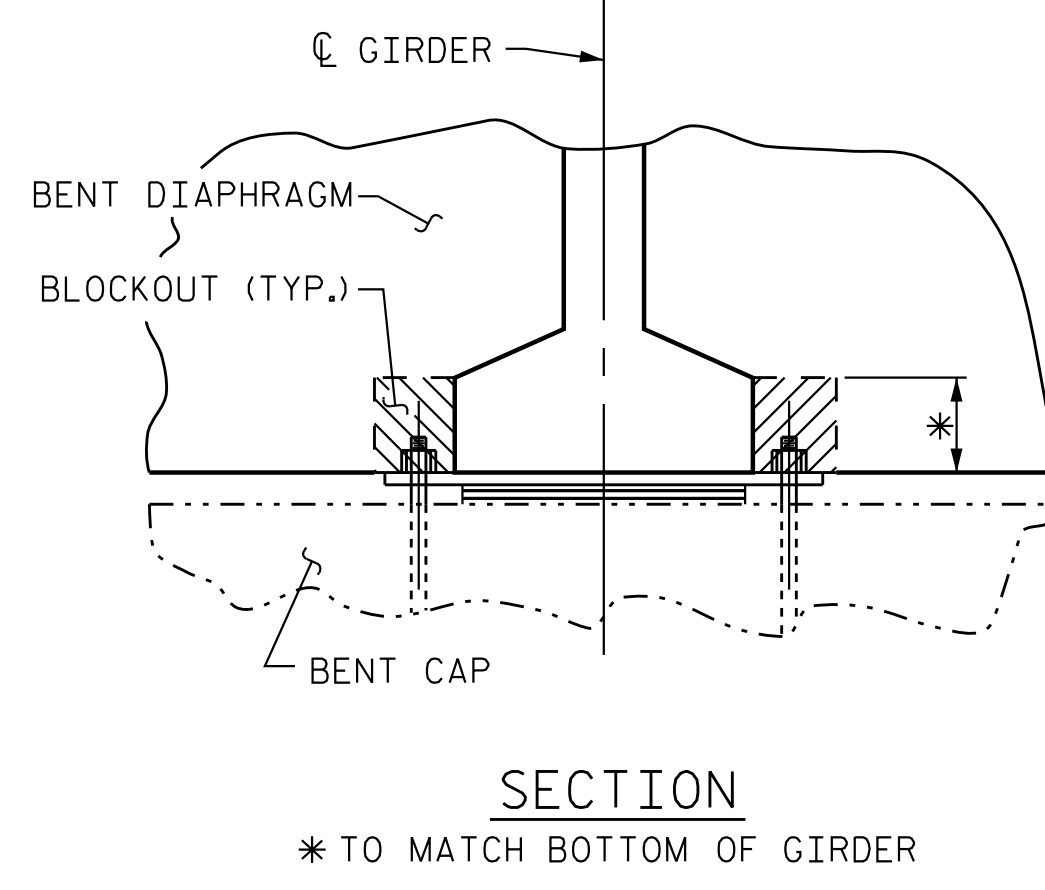
**SECTION A-A**  
END OF GIRDER DETAIL  
AT INTEGRAL END BENT



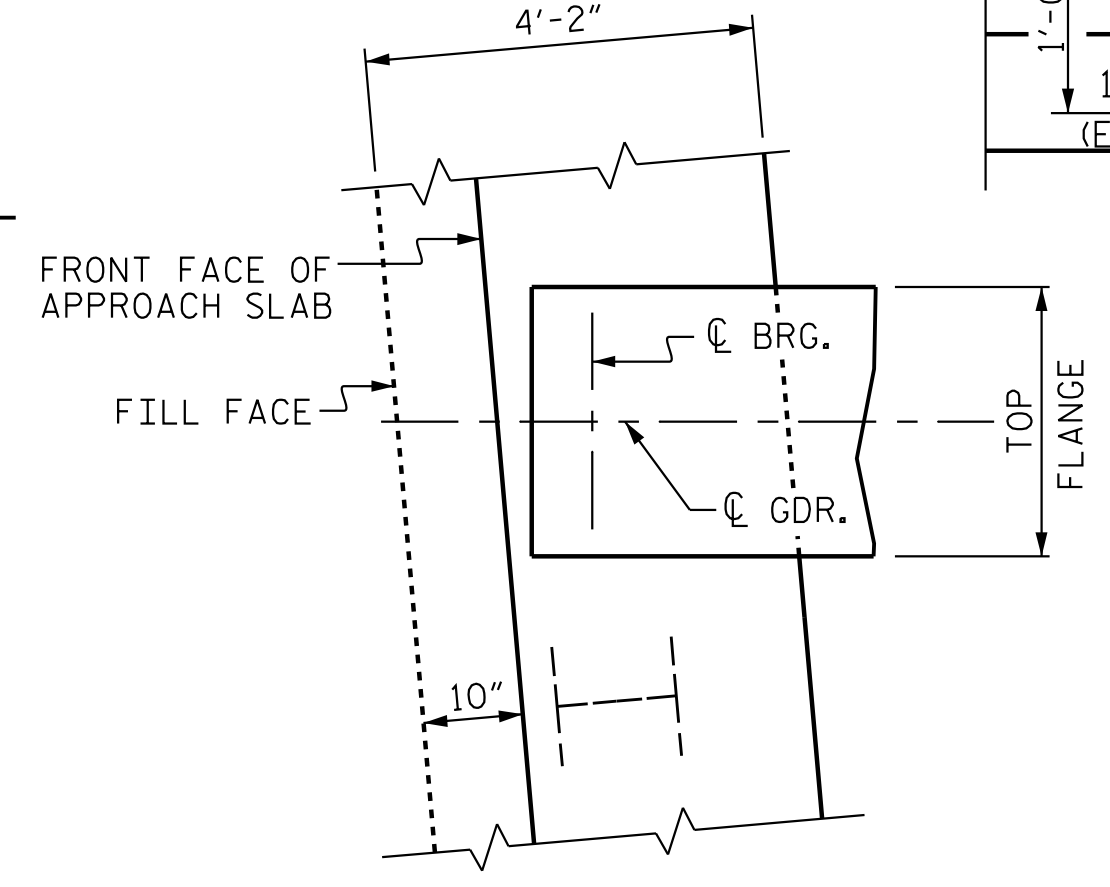
**VIEW B-B**  
INTEGRAL END BENT DIAPHRAGM  
OUTSIDE EDGE OF SUPERSTRUCTURE



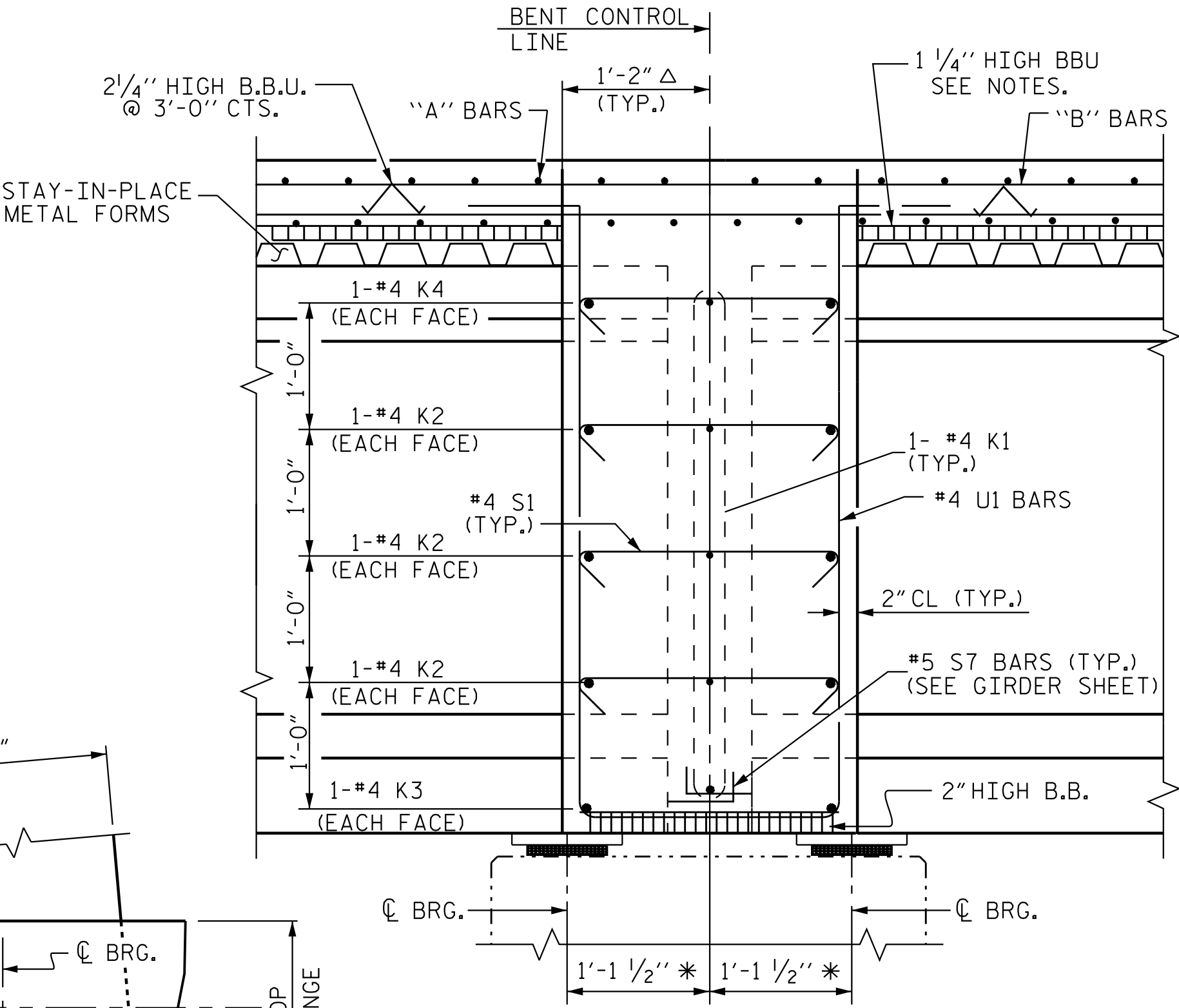
**PLAN**  
BENT DIAPHRAGM BLOCK-OUT DETAIL



**SECTION**  
\* TO MATCH BOTTOM OF GIRDER



**PLAN OF GIRDER AT  
INTEGRAL END BENT DIAPHRAGM**



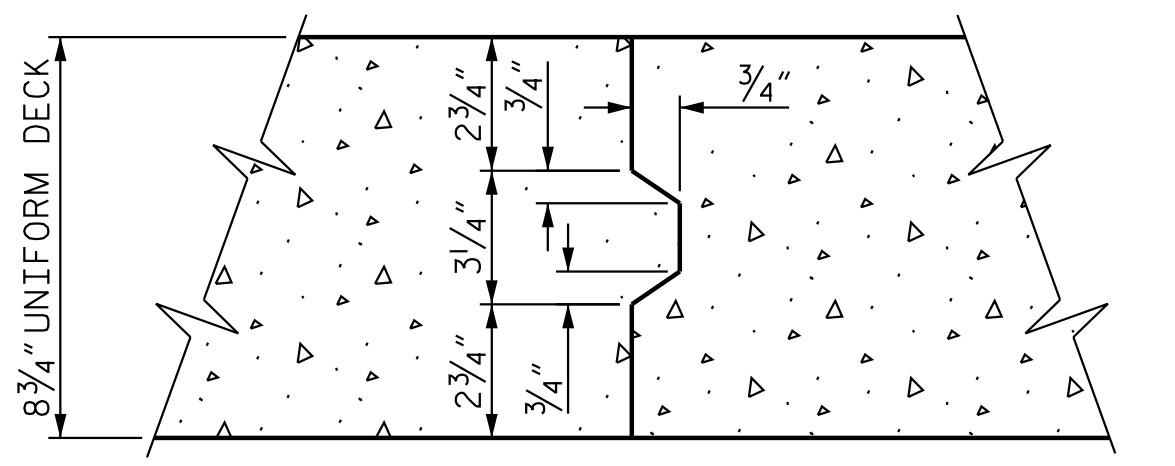
**SECTION C-C**

\* MEASURED ALONG  $\bar{C}$  GIRDER  
Δ MEASURED PERPENDICULAR TO BENT 1 CONTROL LINE

**NOTES**

- PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
- LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
- BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
- NO CHAMFER IS REQUIRED ON CORNERS OF GIRDER BUILDUPS.
- PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.
- ALL REINFORCING STEEL IN CONCRETE MEDIAN SHALL BE EPOXY COATED.
- ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.
- A TIME PERIOD OF AT LEAST 40 DAYS SHALL ELAPSE FROM THE TRANSFER OF PRESTRESSING FORCE INTO ANY GIRDER AND THE PLACEMENT OF DECK SLAB CONCRETE IN THE SPANS.

BOTTOM OF OVERHANG ELEV. @ OUTSIDE EDGE OF SUPERSTR.		
OVERHANG	END BENT	ELEV.
LEFT SIDE	1	81.10
RIGHT SIDE	1	77.33
LEFT SIDE	2	84.67
RIGHT SIDE	2	81.01



**TRANSVERSE CONSTRUCTION JOINT  
IN DECK SLAB**

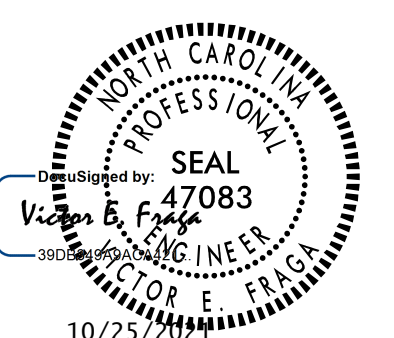
REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT.

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 852+98.21 -L1-NORTHERN-  
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SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUPERSTRUCTURE

**TYPICAL SECTION  
DETAILS**



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NOTES

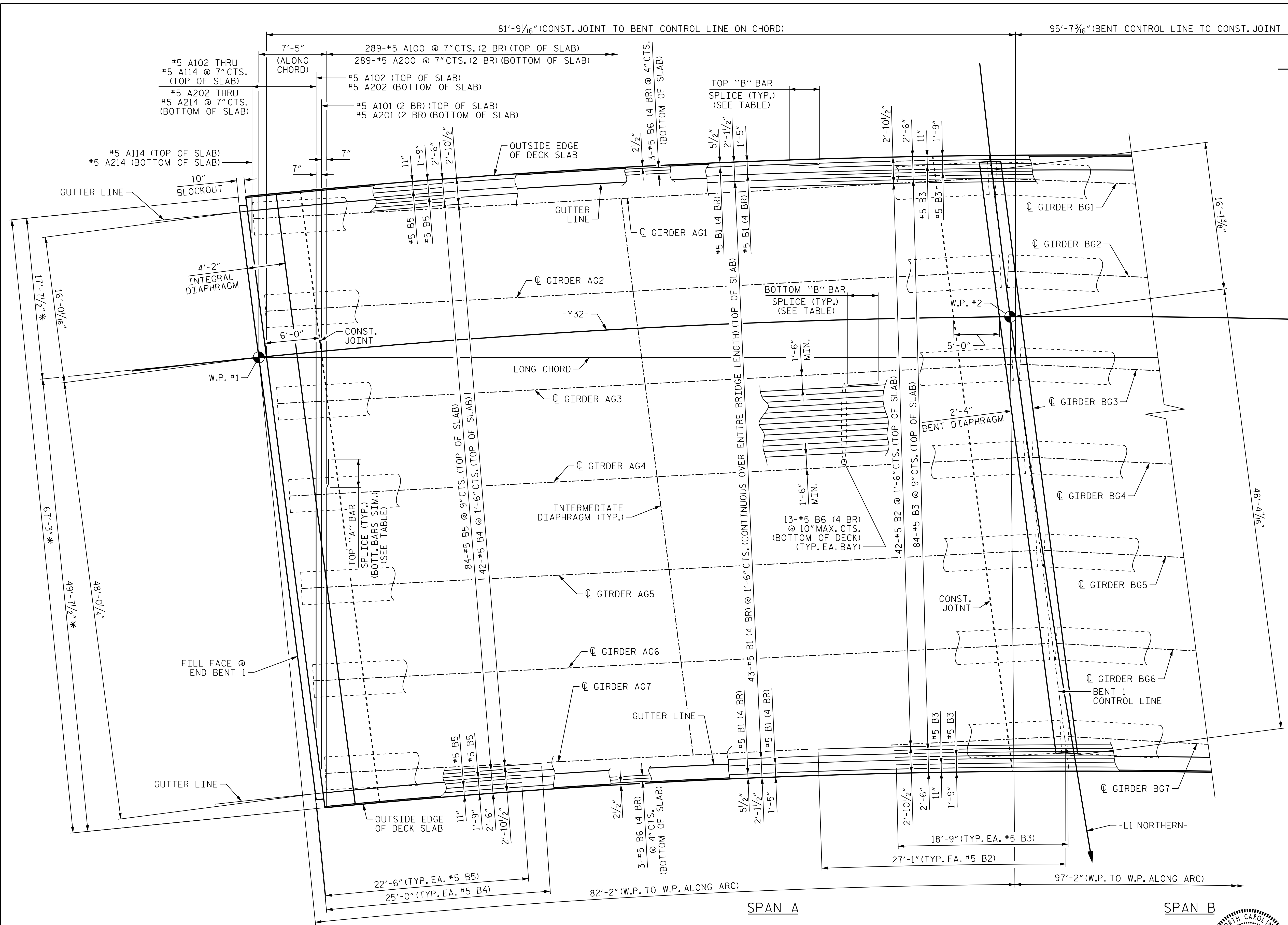
2 BR DENOTES 2 BAR RUN  
 4 BR DENOTES 4 BAR RUN

SEE PLAN OF SPANS SHEET 3 OF 3 FOR END BENT DIAPHRAGM DETAILS.

SEE PLAN OF SPANS SHEET 3 OF 3 FOR INTERIOR BENT DIAPHRAGM DETAILS.

\* DENOTES RADIAL DIMENSION.

BAR MARK	MIN. SPLICE
#5 A100, #5 A101 #5 A115, #5 A116	2'-5"
#5 A200, #5 A201 #5 A215, #5 A216	2'-0"
#5 B1	2'-5"
#5 B6	2'-0"



PLAN

PROJECT NO. R-3300B

PENDER COUNTY

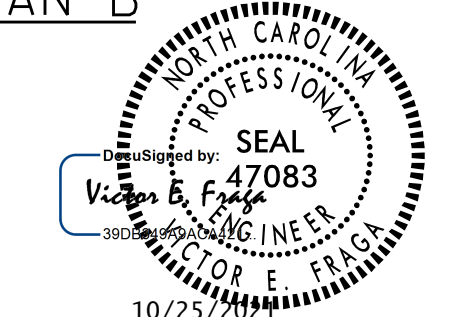
STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
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 RALEIGH

SUPERSTRUCTURE

PLAN OF SPAN (SPAN A)



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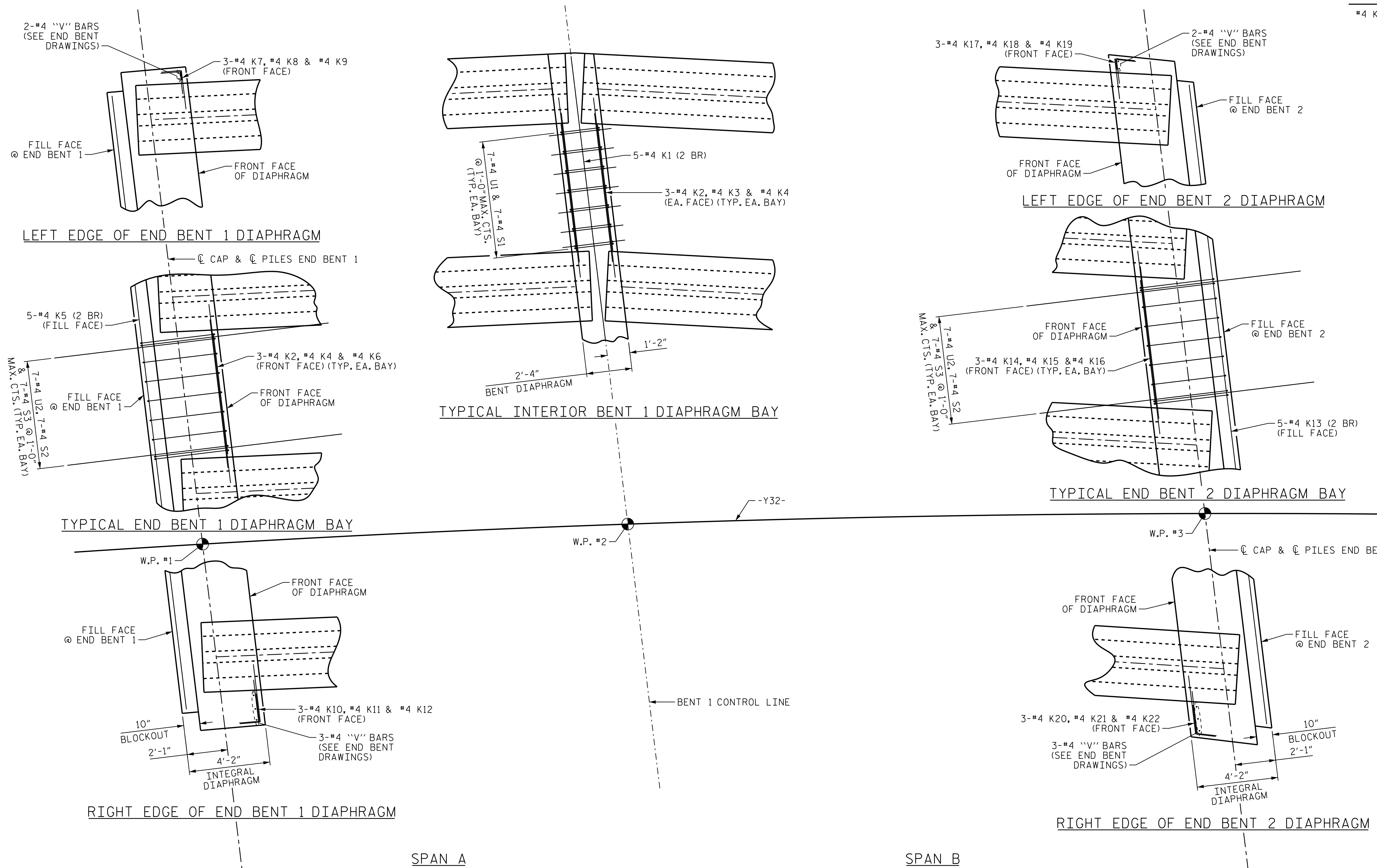
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NOTES

#4 K1 MIN. SPLICE = 1'-7"



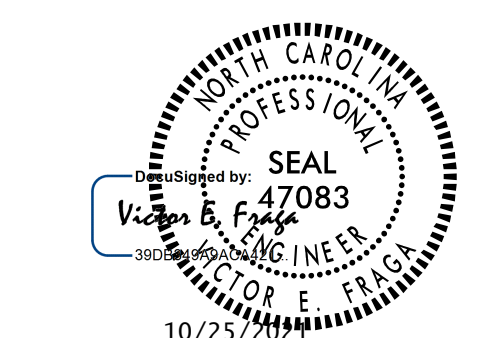
PLAN OF END BENT AND BENT DIAPHRAGMS

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

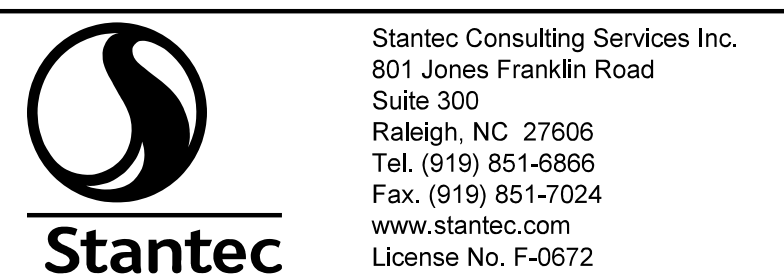
SHEET 3 OF 3

STATE OF NORTH CAROLINA  
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 RALEIGH  
 SUPERSTRUCTURE  
 PLAN OF SPANS  
 (DETAILS)

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2			4			SHEETS	



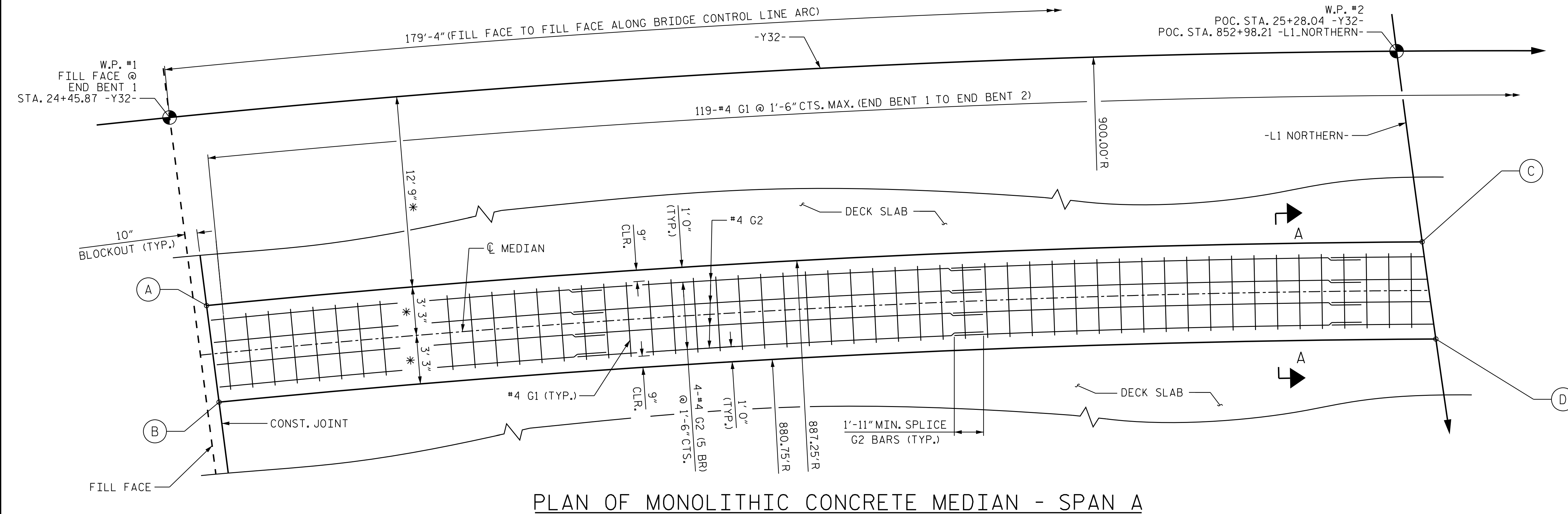
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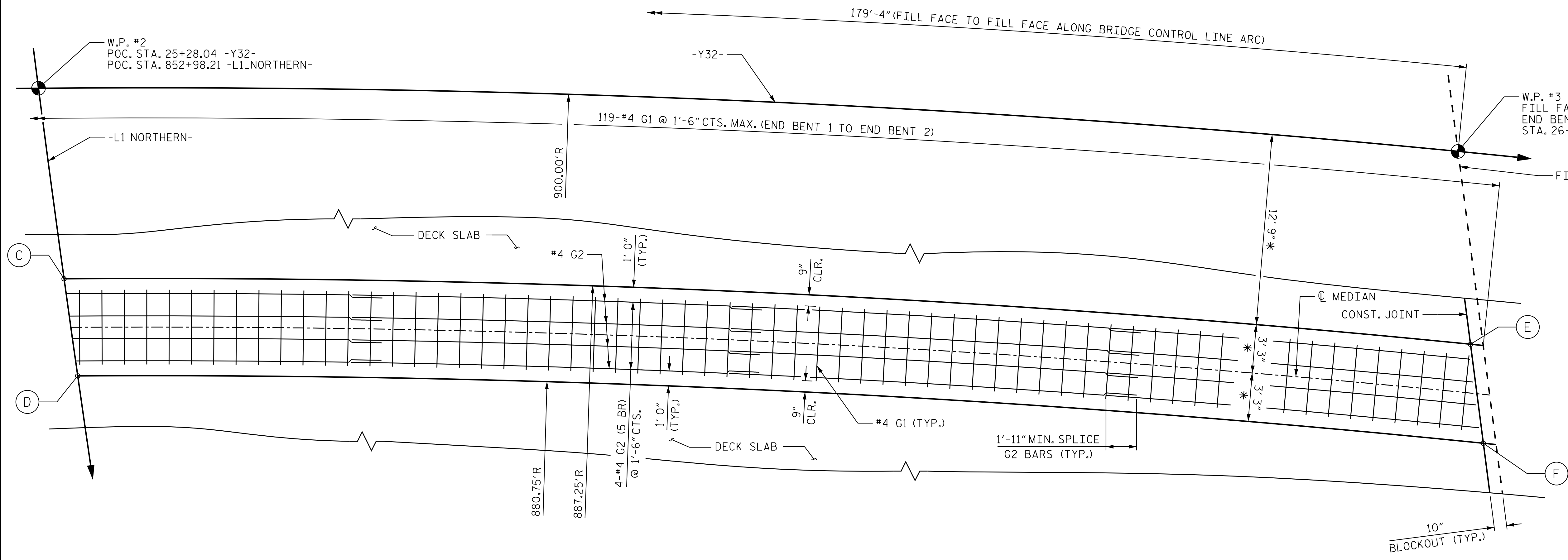
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10/25/2021 10/25/2021 10/25/2021



PLAN OF MONOLITHIC CONCRETE MEDIAN - SPAN A



PLAN OF MONOLITHIC CONCRETE MEDIAN - SPAN B

**NOTES**

5 BR DENOTES 5 BAR RUN.

SEE APPROACH SLAB DETAILS SHEET 3 OF 3 FOR MEDIAN ON APPROACH SLAB.

\* RADIAL DIMENSION.

MONOLITHIC CONCRETE MEDIAN LAYOUT		
POINT	STATION	OFFSET
(A)	24+47.09 -Y32-	12.75 RT.
(B)	24+47.30 -Y32-	19.25 RT.
(C)	25+29.66 -Y32-	12.75 RT.
(D)	25+30.55 -Y32-	19.25 RT.
(E)	26+27.34 -Y32-	12.75 RT.
(F)	26+28.90 -Y32-	19.25 RT.

PROJECT NO. R-3300B

PENDER COUNTY

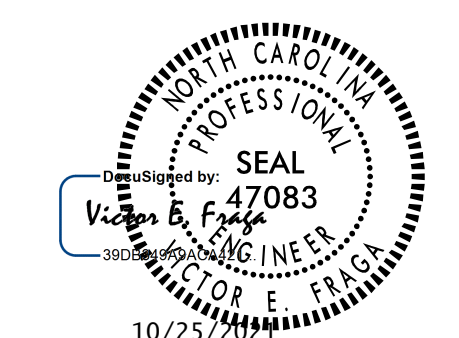
STATION: 852+98.21 -L1\_NORTHERN-  
25+28.04 -Y32-

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

**SUPERSTRUCTURE  
MONOLITHIC  
CONCRETE MEDIAN  
DETAILS**

REVISIONS						SHEET NO.
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**NOTES**

GROOVED CONTRACTION JOINTS, 1" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE MONOLITHIC CONCRETE MEDIAN IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT A SPACING OF 10 FT. BETWEEN EXPANSION JOINT, NO CONTRACTION JOINT WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FT. IN LENGTH. PLACE 1/2" EXPANSION JOINTS AT 30' INTERVALS.

ALL REINFORCING STEEL IN MONOLITHIC CONCRETE MEDIAN SHALL BE EPOXY COATED.

FOR MONOLITHIC CONCRETE MEDIAN ON APPROACH SLAB, SEE "BRIDGE APPROACH SLAB" SHEETS.

NO SEPARATE MEASUREMENT OR PAYMENT WILL BE MADE FOR MATERIALS OR LABOR REQUIRED TO CONSTRUCT THE CONCRETE MEDIAN. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR REINFORCED CONCRETE DECK SLAB.

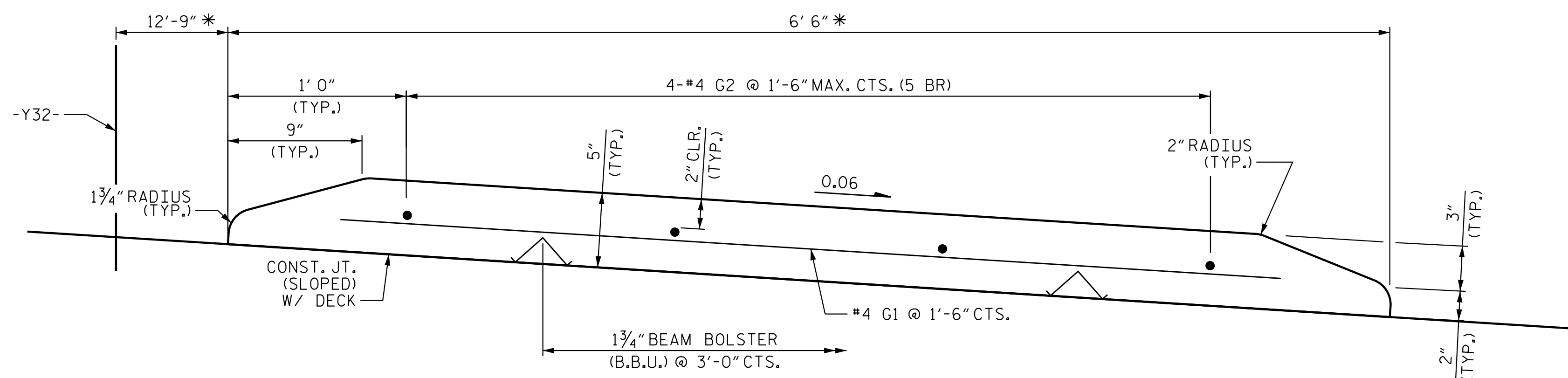
THE CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONCRETE MEDIAN IS INCLUDED IN THE "SUPERSTRUCTURE BILL OF MATERIAL" AND INCLUDED IN THE SQUARE FOOT PRICE FOR "REINFORCED CONCRETE DECK SLAB."

CONCRETE MEDIAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

2" MINIMUM CLEARANCE FROM ALL FINISHED CONCRETE SURFACES TO ALL STEEL REINFORCEMENT.

FOR ADDITIONAL INFORMATION, SEE 2018 ROADWAY STANDARD DRAWING 852.01.

\* DENOTES RADIAL DIMENSION.

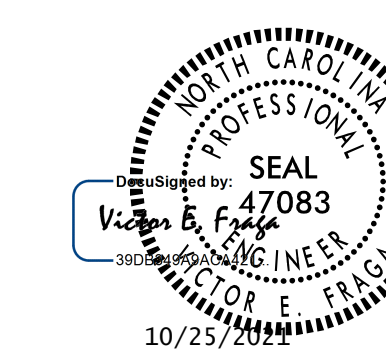


**SECTION A-A**

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

SHEET 2 OF 2

STATE OF NORTH CAROLINA  
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 SUPERSTRUCTURE  
 MONOLITHIC  
 CONCRETE MEDIAN  
 DETAILS



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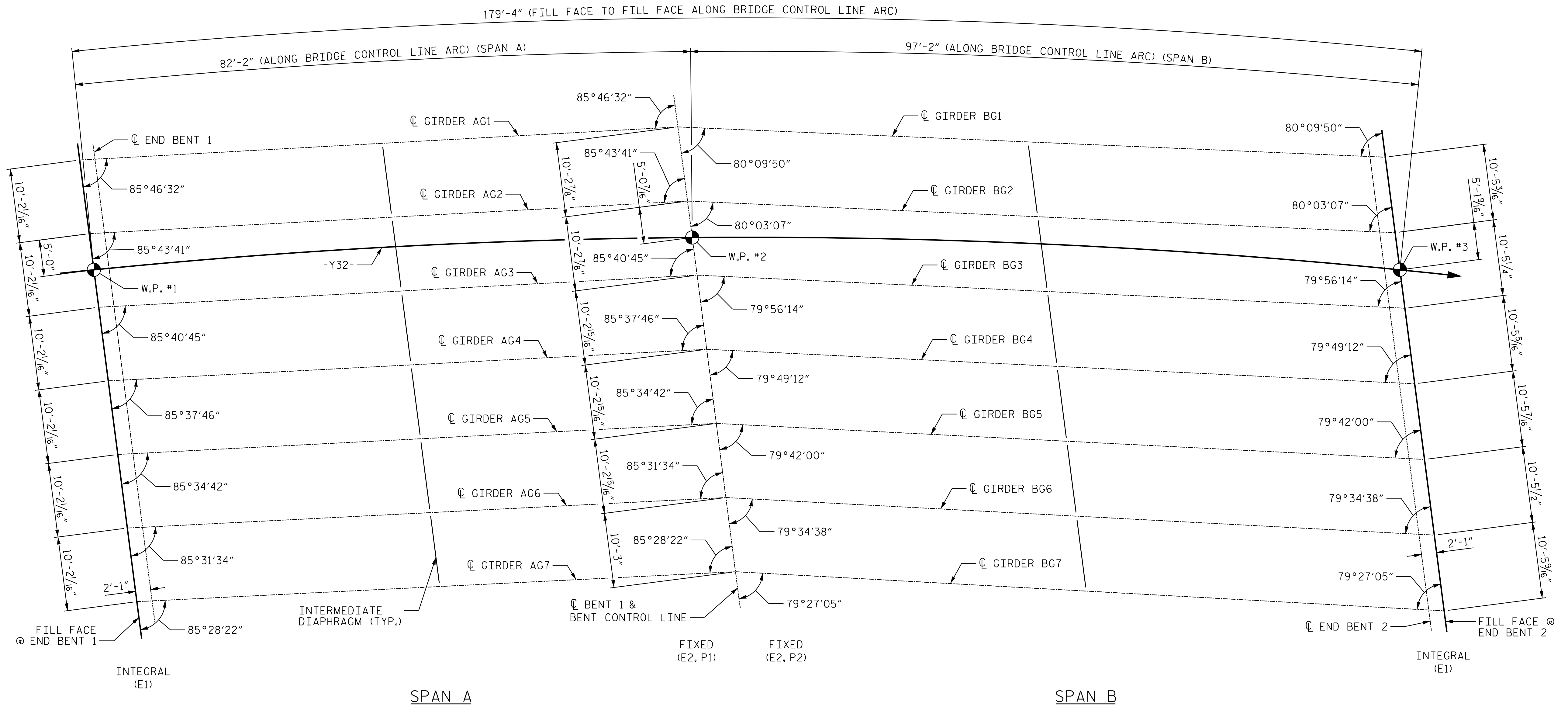
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 10/25/2021  
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### FRAMING PLAN

SEE GIRDER DETAILS FOR DIAPHRAGM LOCATIONS

PROJECT NO. R-3300B

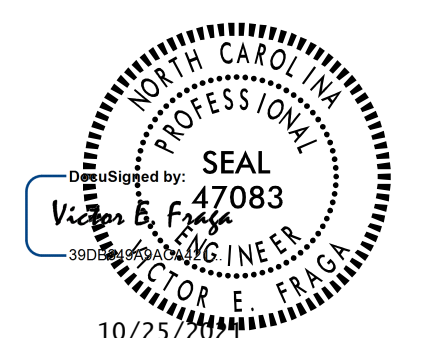
PENDER COUNTY

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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
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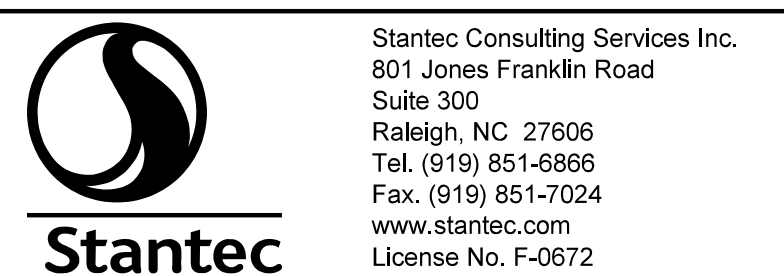
**SUPERSTRUCTURE**

**FRAMING PLAN**



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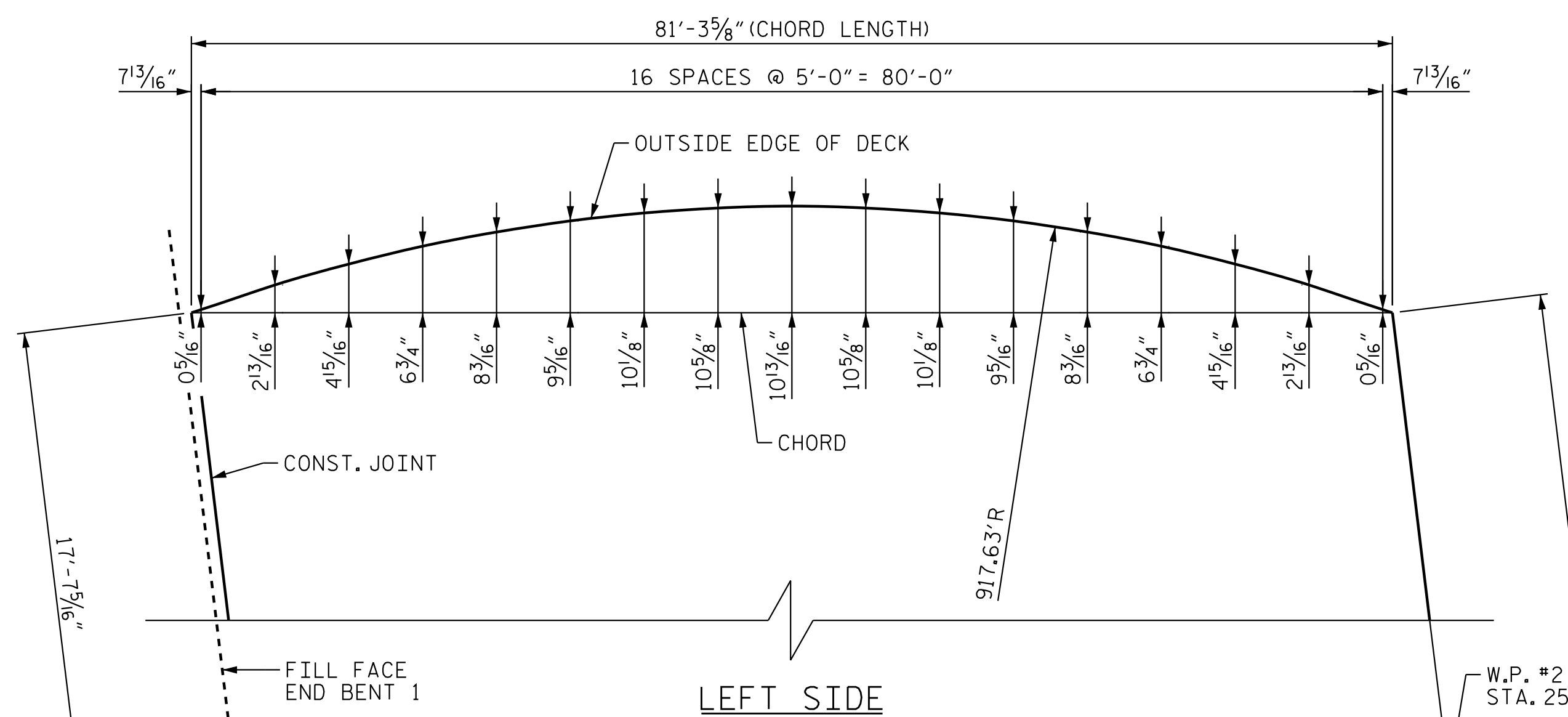


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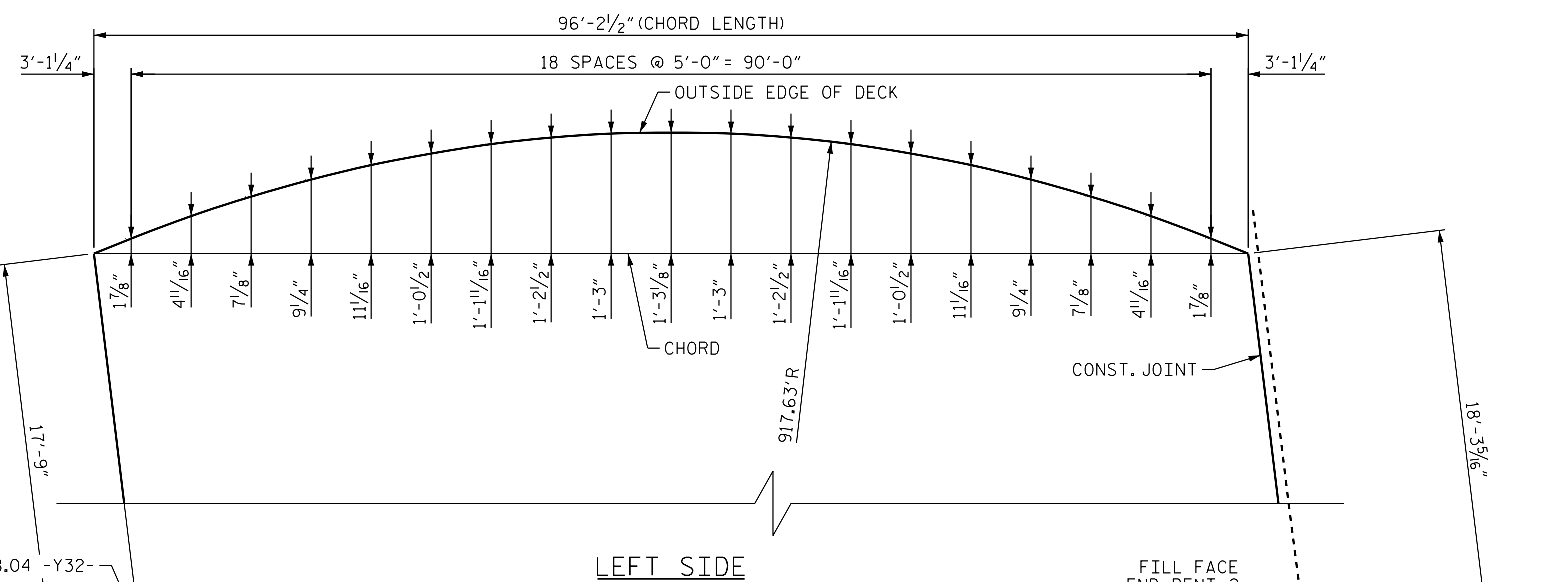
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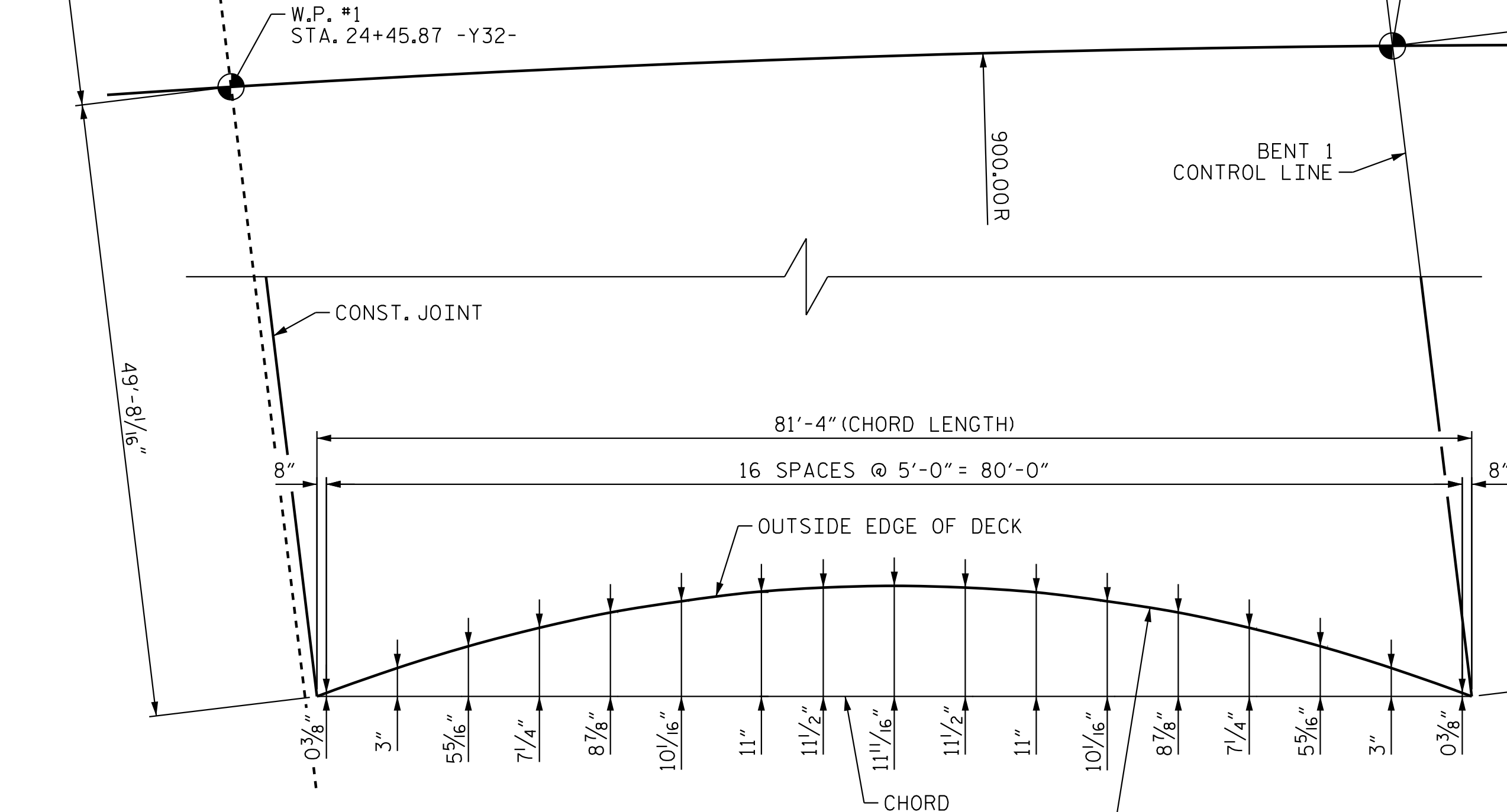
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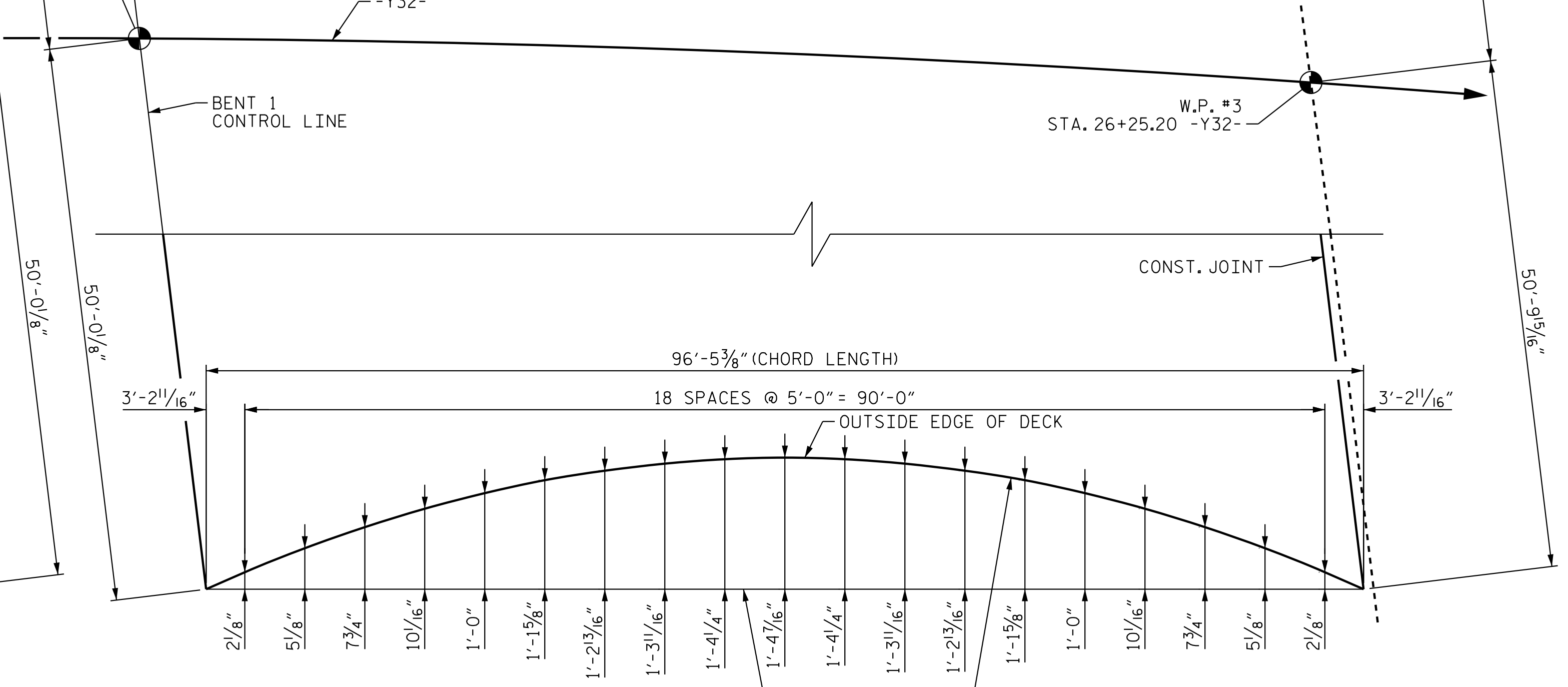
LEFT SIDE



LEFT SIDE



RIGHT SIDE



RIGHT SIDE

ARC OFFSETS- SPAN A

ARC OFFSETS- SPAN B

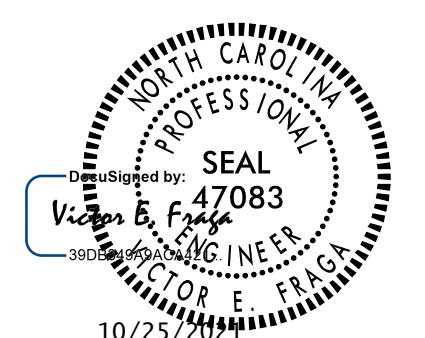
PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-



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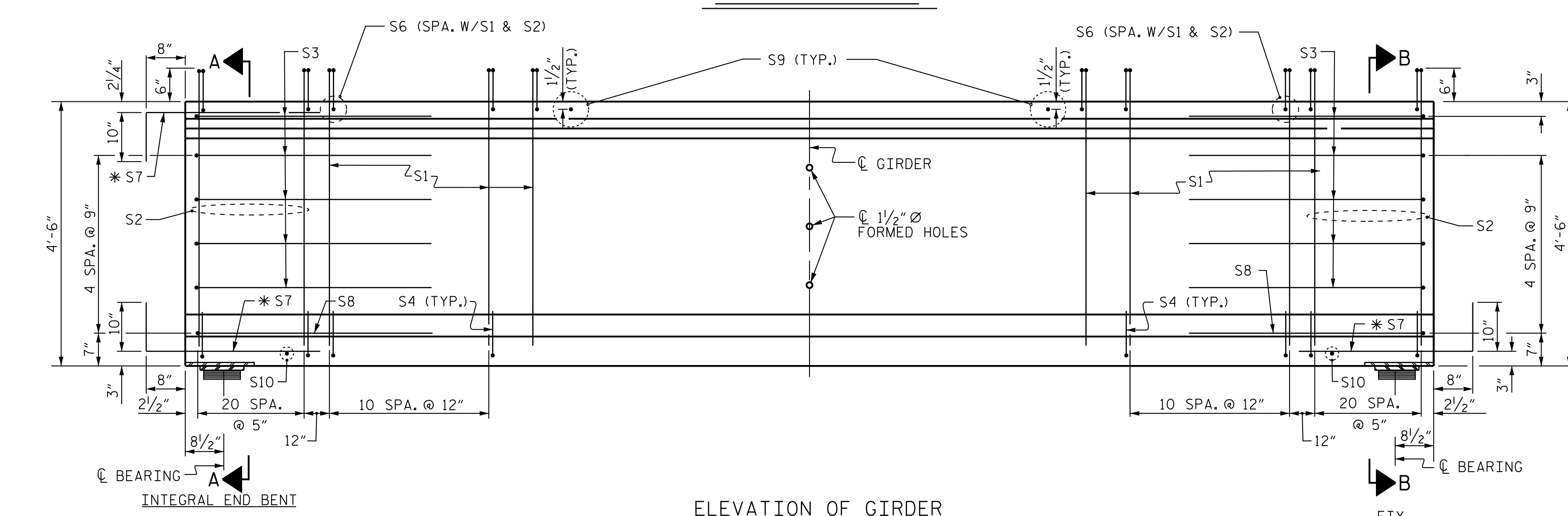
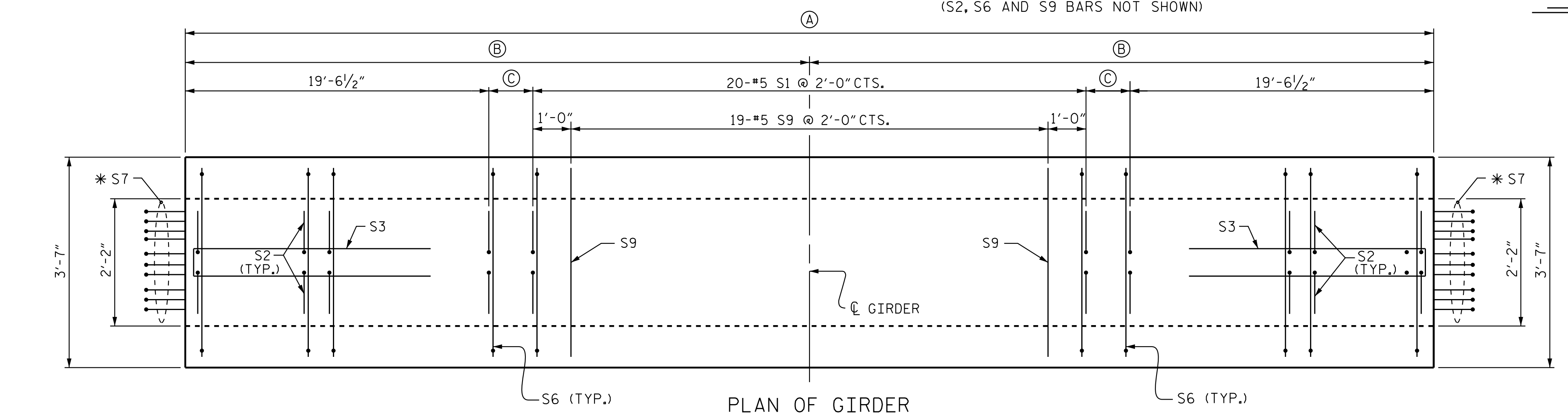
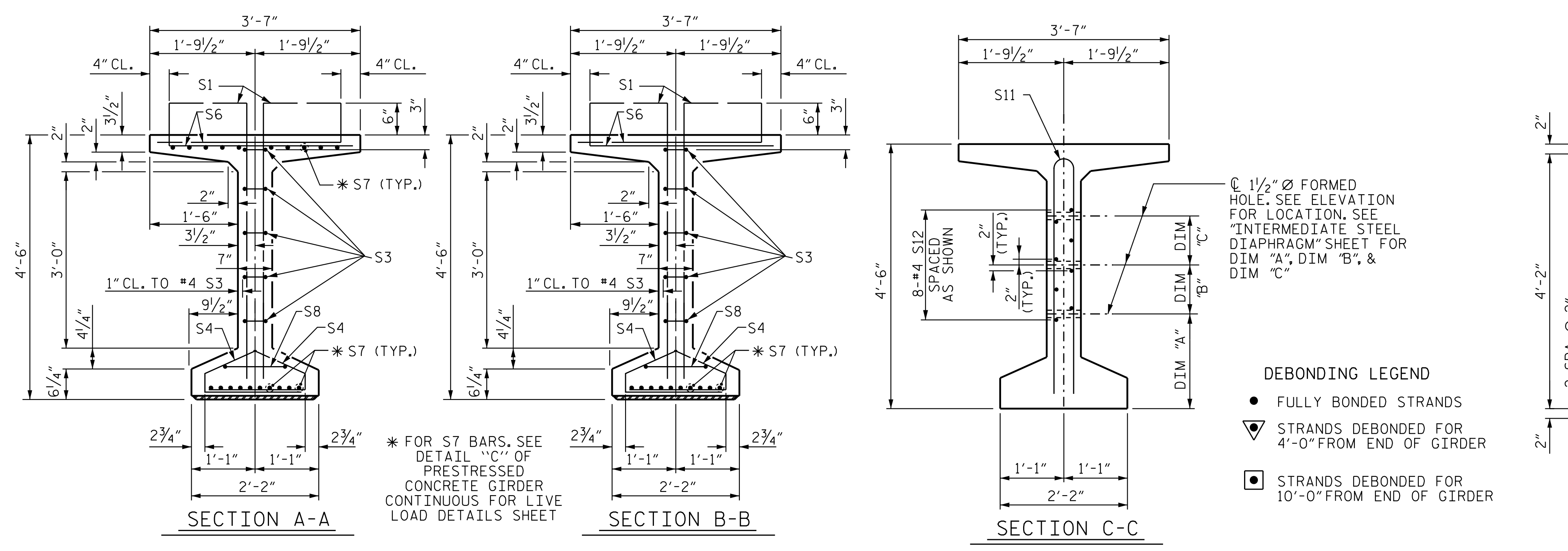
DRAWN BY : T. N. ENNIS DATE : 01/27/20  
 CHECKED BY : V. E. FRAGA DATE : 02/11/20

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE : 10/25/21



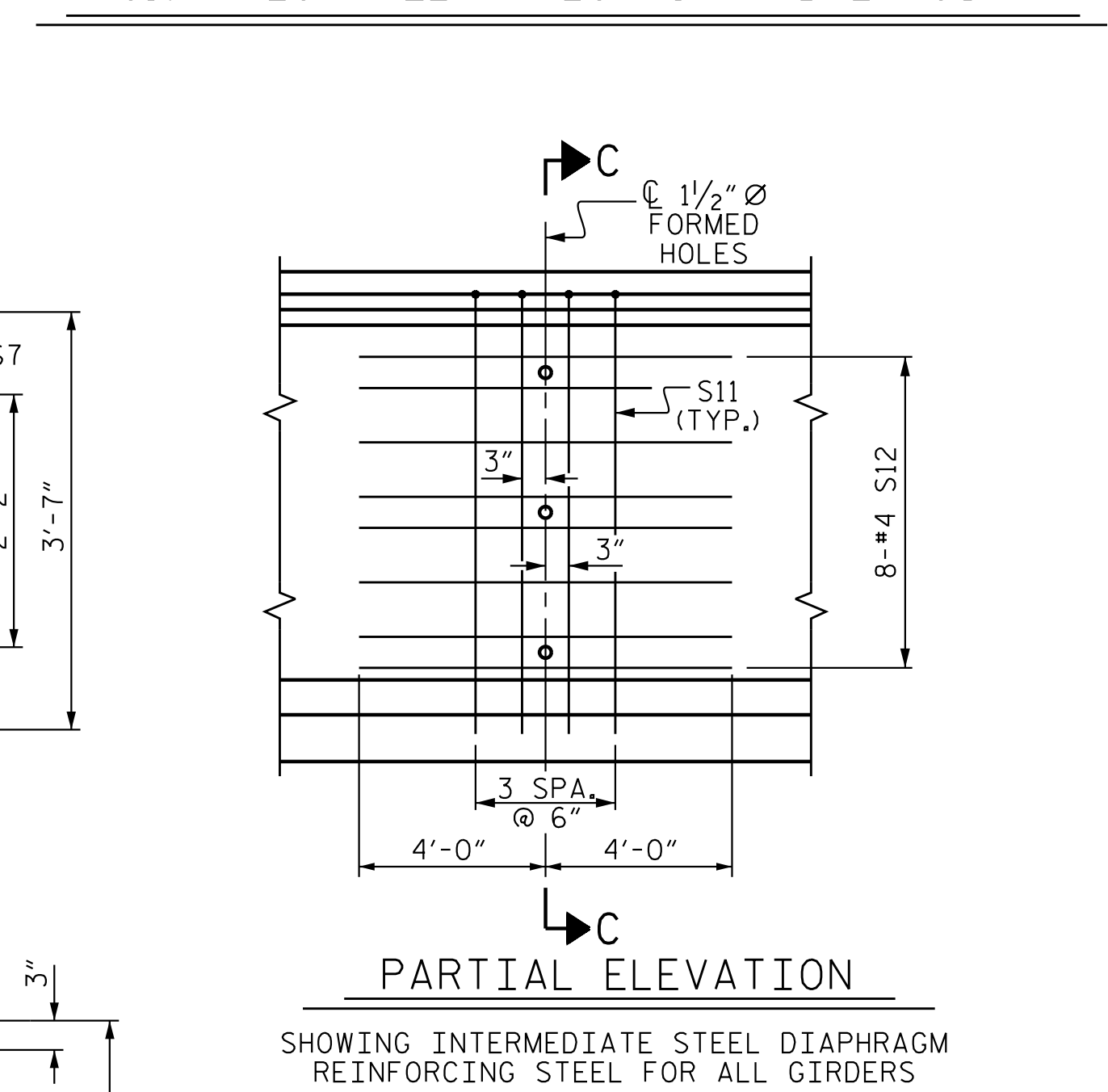
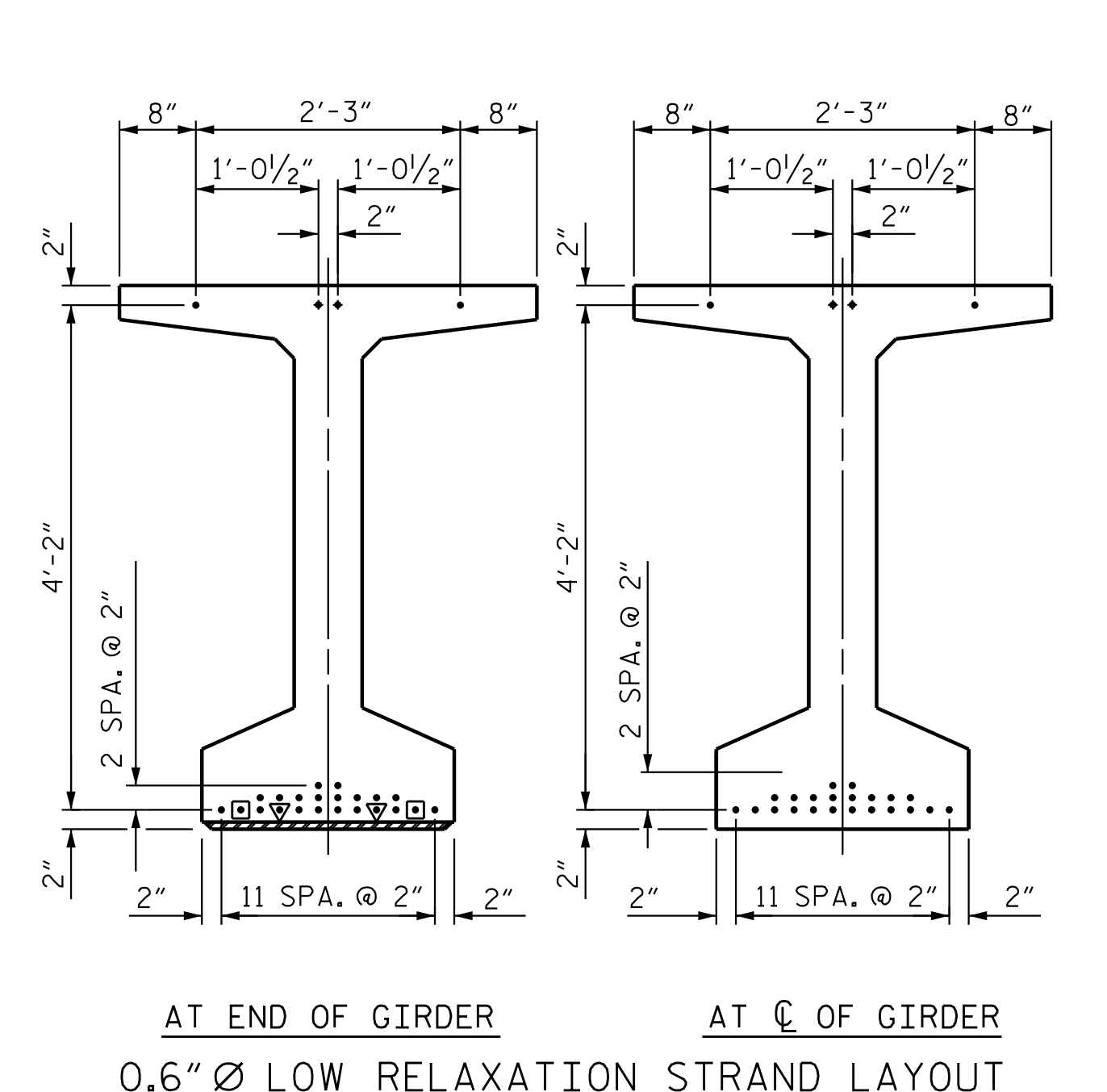
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE CHORD TO ARC OFFSETS					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
SHEET NO. S17-15					TOTAL SHEETS 38



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 DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21

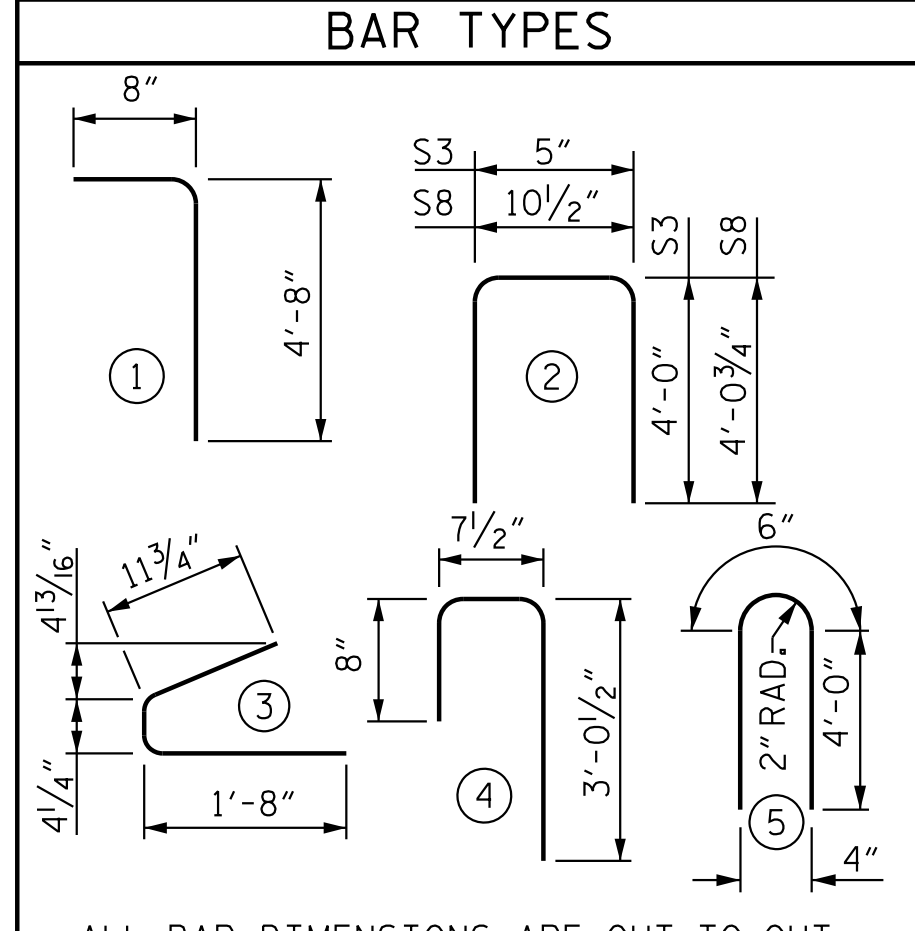


	GIRDER DIMENSIONS		
	A	B	C
AG1	80'-4"	40'-2"	1'-7 1/2"
AG2	80'-4"	40'-2"	1'-7 1/2"
AG3	80'-4 1/8"	40'-2 1/16"	1'-7 9/16"
AG4	80'-4 1/8"	40'-2 1/16"	1'-7 9/16"
AG5	80'-4 1/4"	40'-2 1/8"	1'-7 5/8"
AG6	80'-4 3/8"	40'-2 3/16"	1'-7 11/16"
AG7	80'-4 3/8"	40'-2 3/16"	1'-7 11/16"

0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	84	#5	1	5'-4"	467
S2	84	#6	1	5'-4"	673
S3	10	#4	2	8'-5"	56
S4	128	#4	3	3'-0"	257
S6	168	#5	4	4'-4"	759
* S7	30	#5	STR.	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	19	#5	STR.	3'-3"	64
S10	2	#3	STR.	1'-10"	1
S11	4	#5	5	8'-6"	35
S12	8	#4	STR.	8'-0"	43

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



ALL BAR DIMENSIONS ARE OUT-TO-OUT

	QUANTITIES FOR ONE GIRDER		
	REINFORCING STEEL LB.	6,000 PSI CONCRETE C.Y.	0.6" Ø L.R. STRANDS No.
GIRDER	2,489	14.7	26

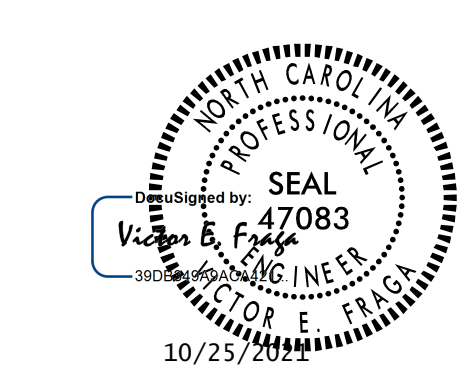
GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
7	A	562'-5 1/4"

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SHEET 1 OF 4

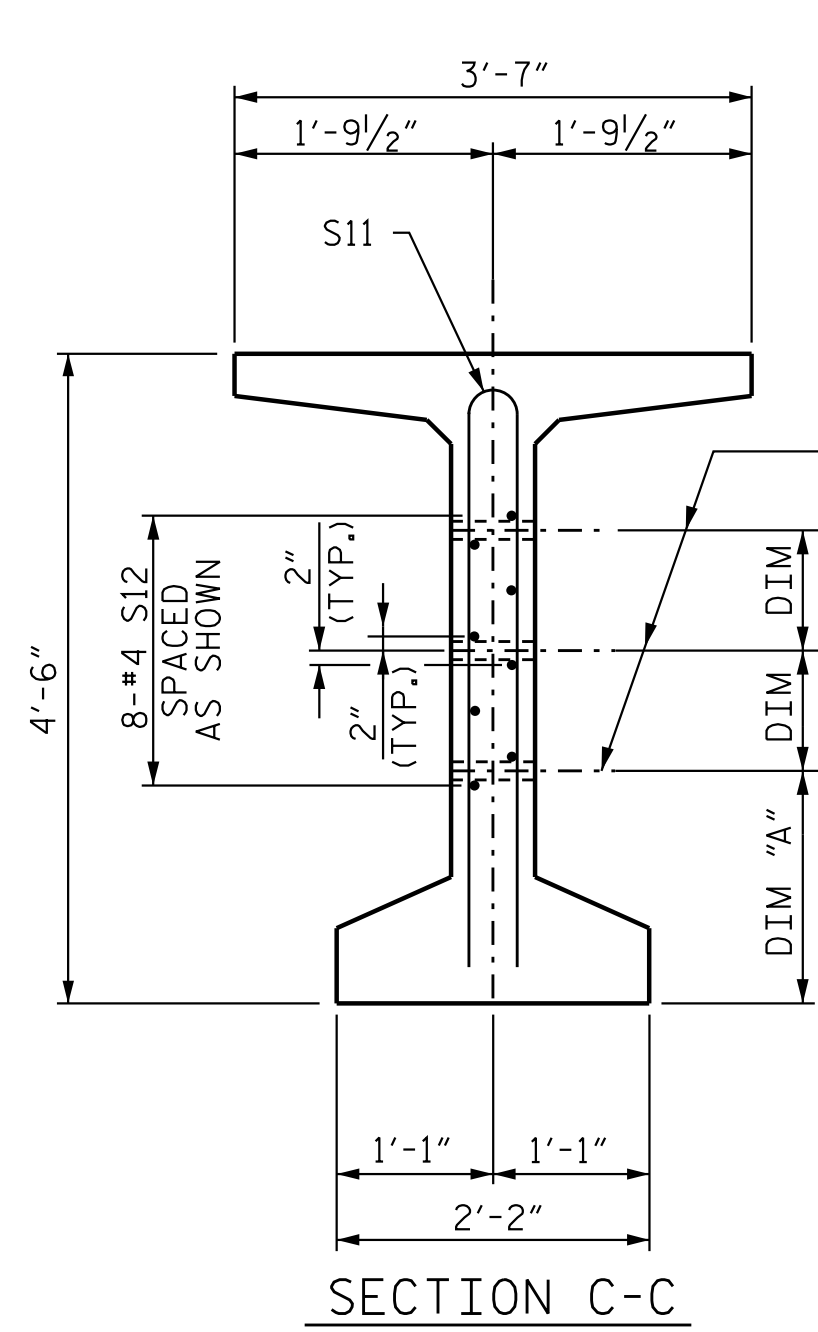
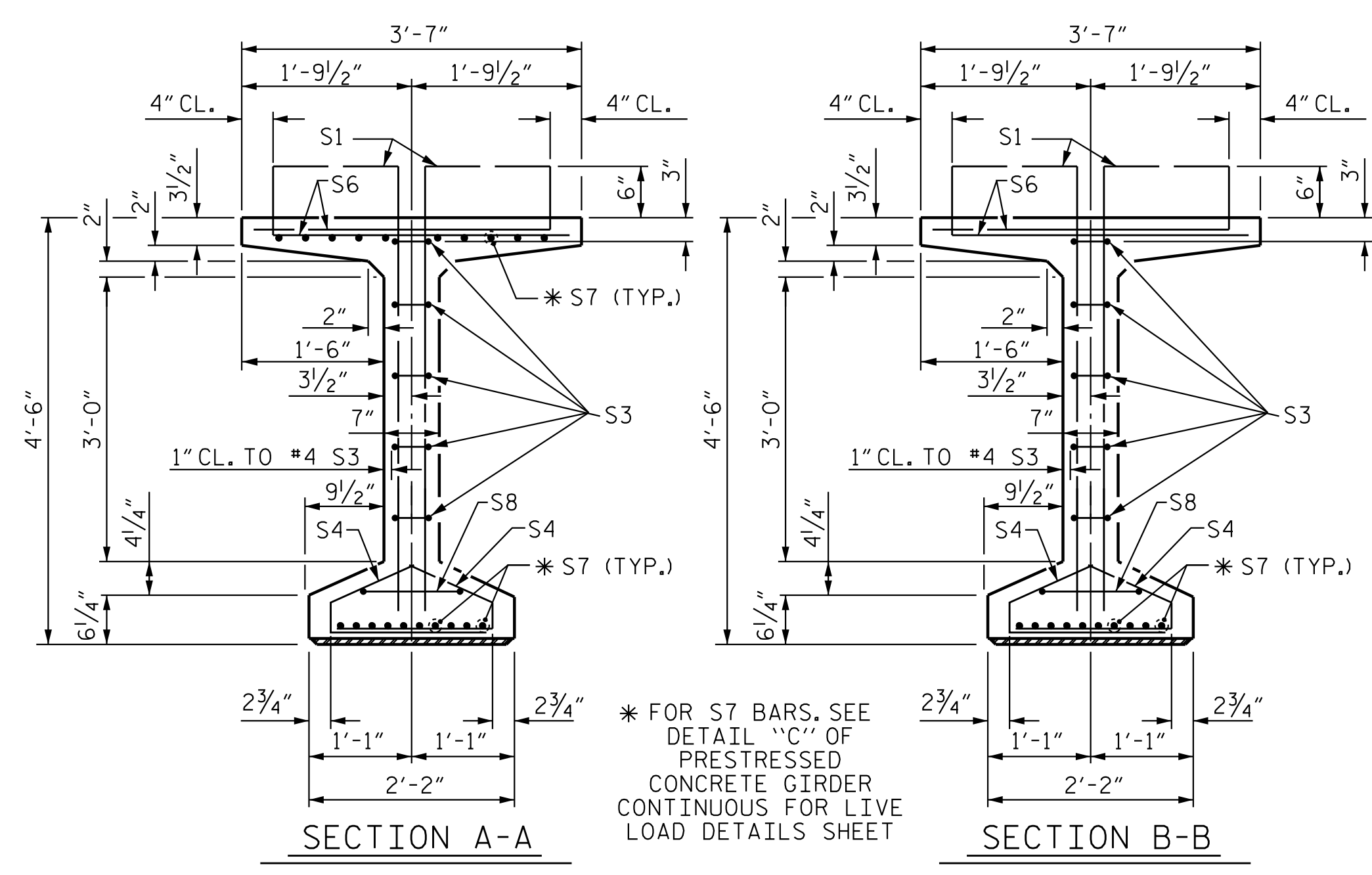
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 54" MODIFIED BULB TEE  
 CONTINUOUS FOR LIVE LOAD  
 PRESTRESSED CONCRETE GIRDER  
 (SPAN A)

REVISIONS						SHEET NO. S17-16
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 38
2			4			



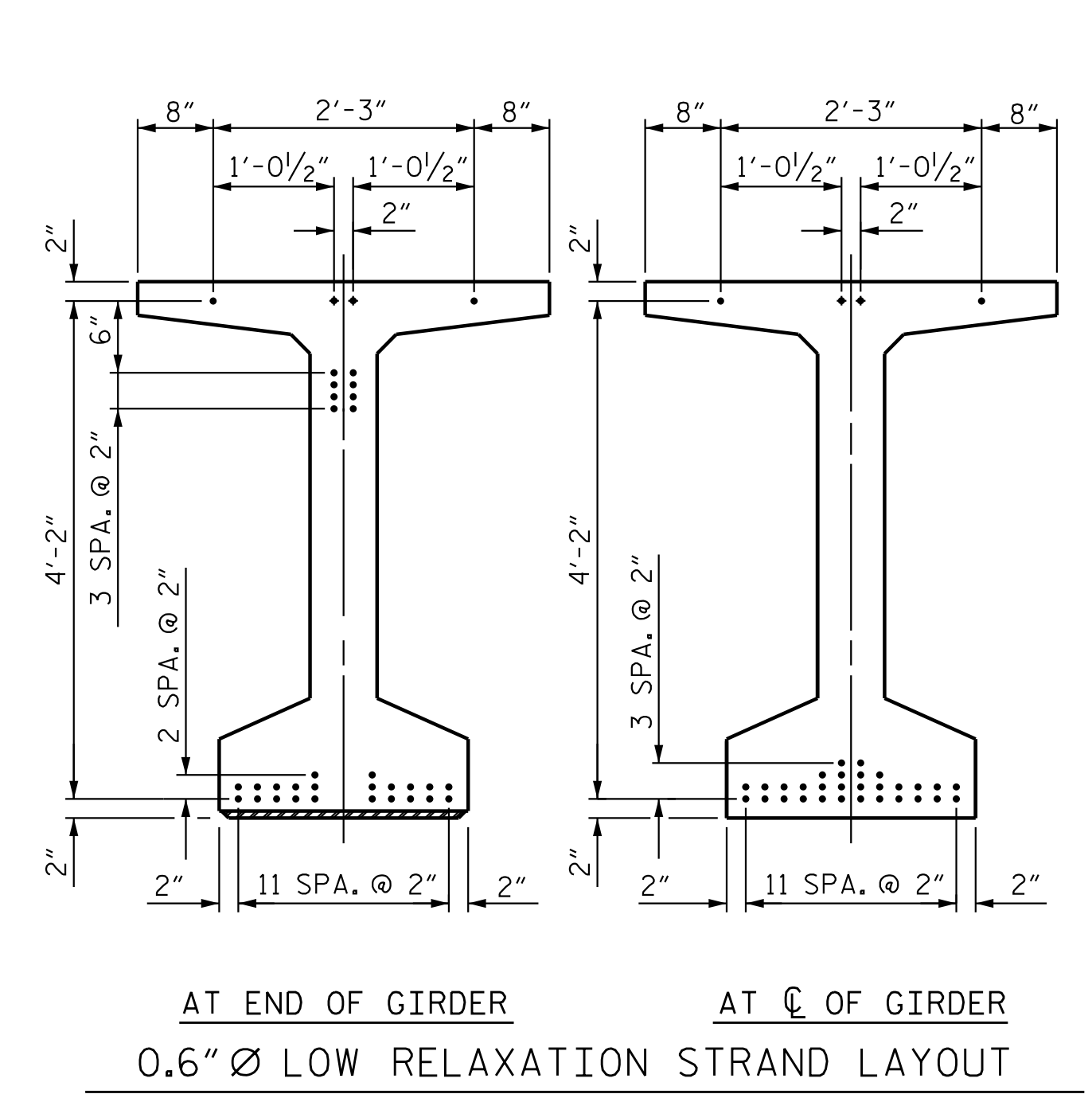
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1/2" Ø FORMED HOLE. SEE ELEVATION FOR LOCATION. SEE "INTERMEDIATE STEEL DIAPHRAGM" SHEET FOR DIM "A", DIM "B", & DIM "C"

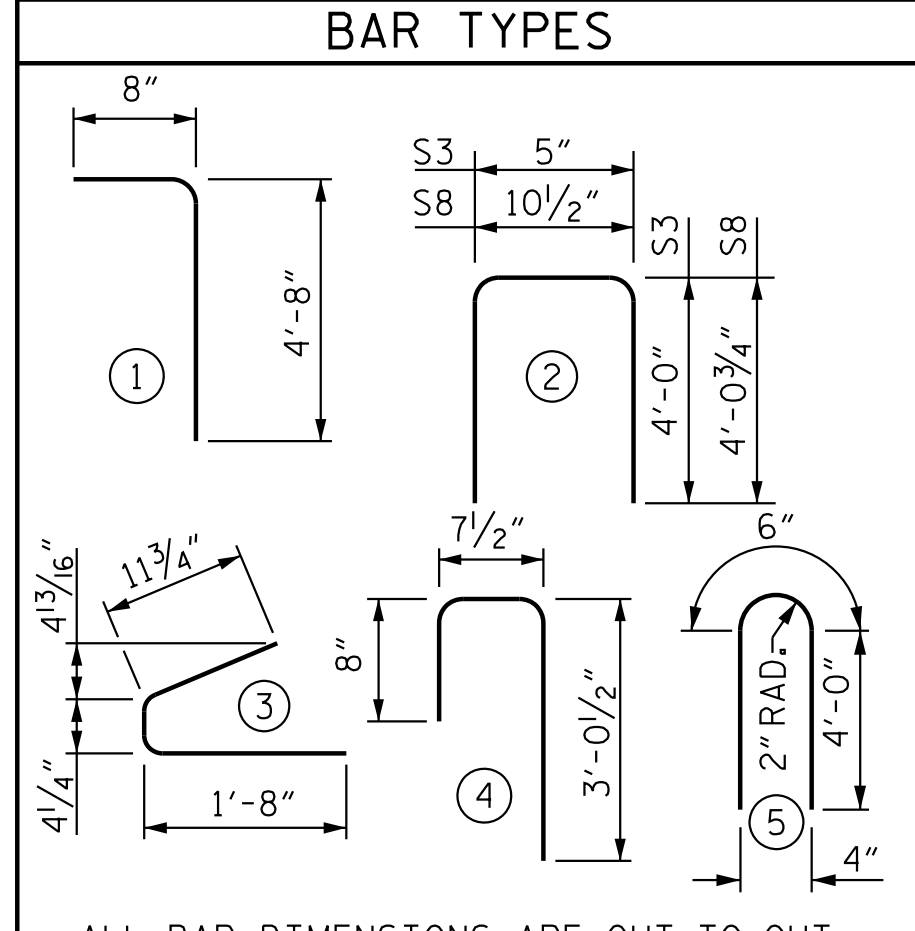
DEBONDING LEGEND  
● FULLY BONDED STRANDS



0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GIRDER					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	164	#5	1	5'-4"	912
S2	44	#6	1	5'-4"	352
S3	10	#4	2	8'-5"	56
S4	144	#4	3	3'-0"	289
S6	208	#5	4	4'-4"	940
* S7	30	#5	STR.	3'-8"	115
S8	2	#5	2	9'-0"	19
S9	31	#5	STR.	3'-3"	105
S10	2	#3	STR.	1'-10"	1
S11	4	#5	5	8'-6"	35
S12	8	#4	STR.	8'-0"	43

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

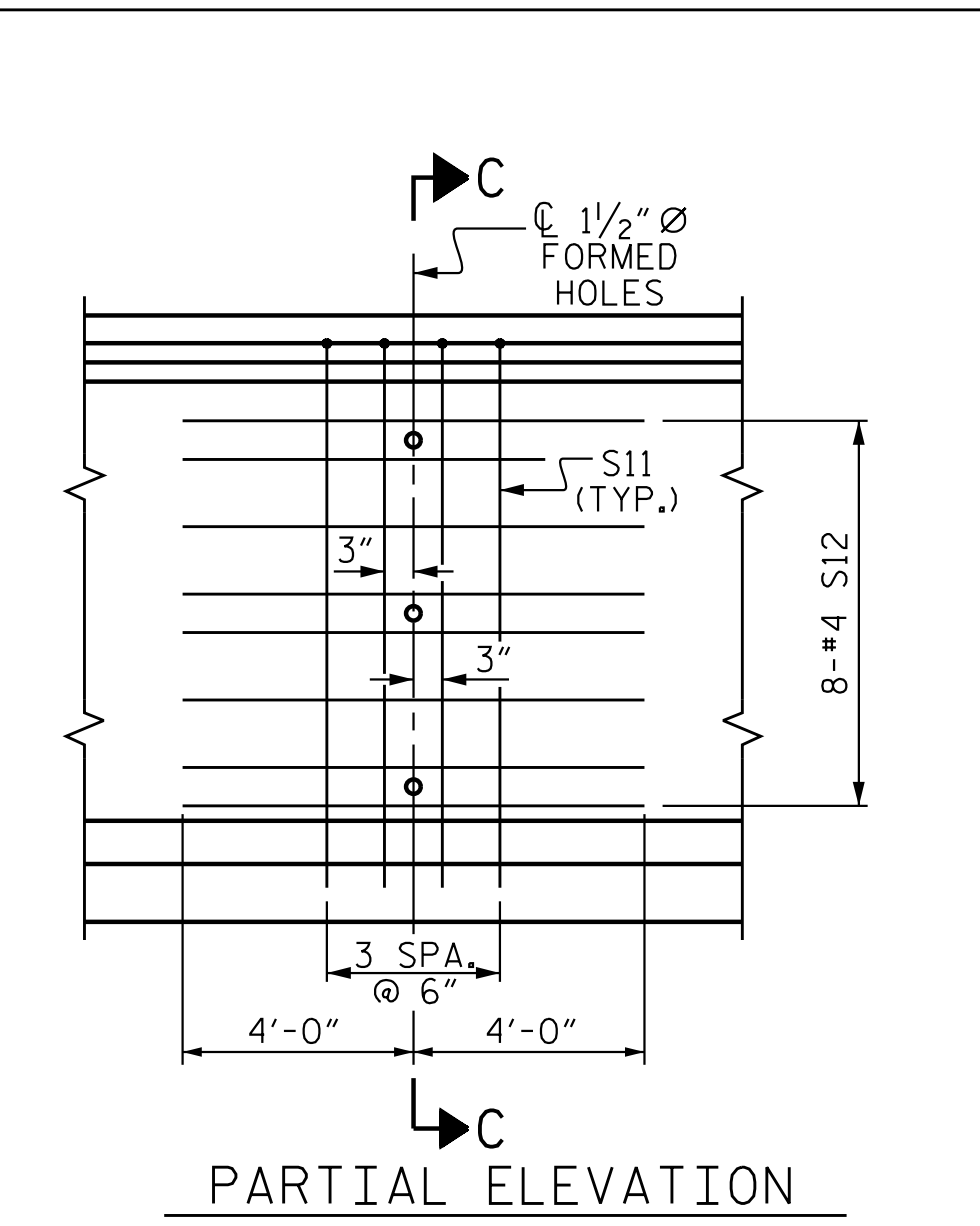
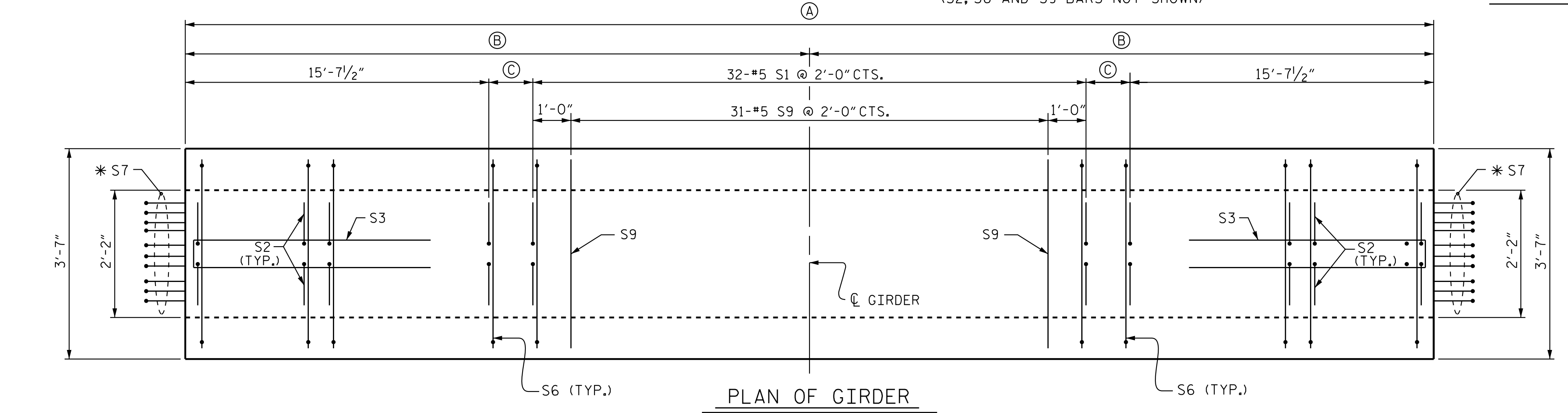


ALL BAR DIMENSIONS ARE OUT-TO-OUT

QUANTITIES FOR ONE GIRDER			
	REINFORCING STEEL	9,000 PSI CONCRETE	0.6" Ø L.R. STRANDS
	LB.	C.Y.	No.
GIRDER	2,867	17.4	34

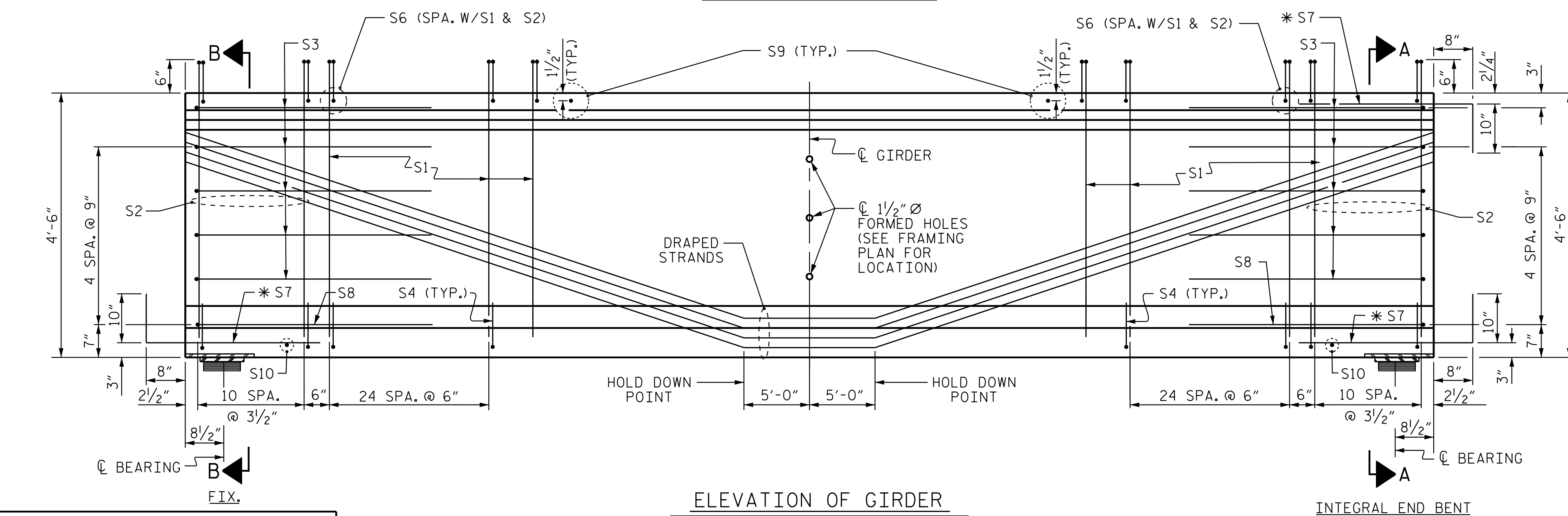
GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
7	(A)	667'-5 3/8"

THE UPLIFT FORCE DUE TO DRAPED STRANDS IS 27 KIPS.



SHOWING INTERMEDIATE STEEL DIAPHRAGM REINFORCING STEEL FOR ALL GIRDERS

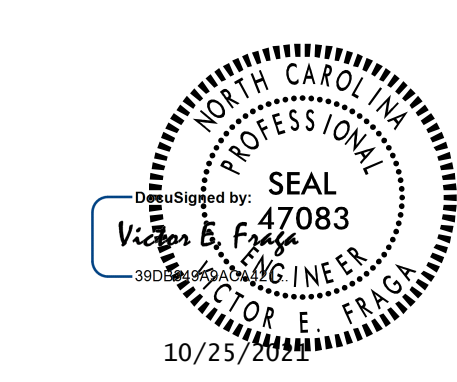
GIRDER DIMENSIONS			
	(A)	(B)	(C)
BG1	95'-3"	47'-7 1/2"	1'-0"
BG2	95'-3 3/8"	47'-7 1/16"	1'-0 3/16"
BG3	95'-3 3/4"	47'-7 7/8"	1'-0 3/8"
BG4	95'-4 1/8"	47'-8 1/16"	1'-0 9/16"
BG5	95'-4 5/8"	47'-8 5/16"	1'-0 3/16"
BG6	95'-5"	47'-8 1/2"	1'-1"
BG7	95'-5 1/2"	47'-8 3/4"	1'-1 1/4"



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### NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4,500 PSI FOR SPAN A GIRDERS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5,500 PSI FOR SPAN B GIRDERS.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

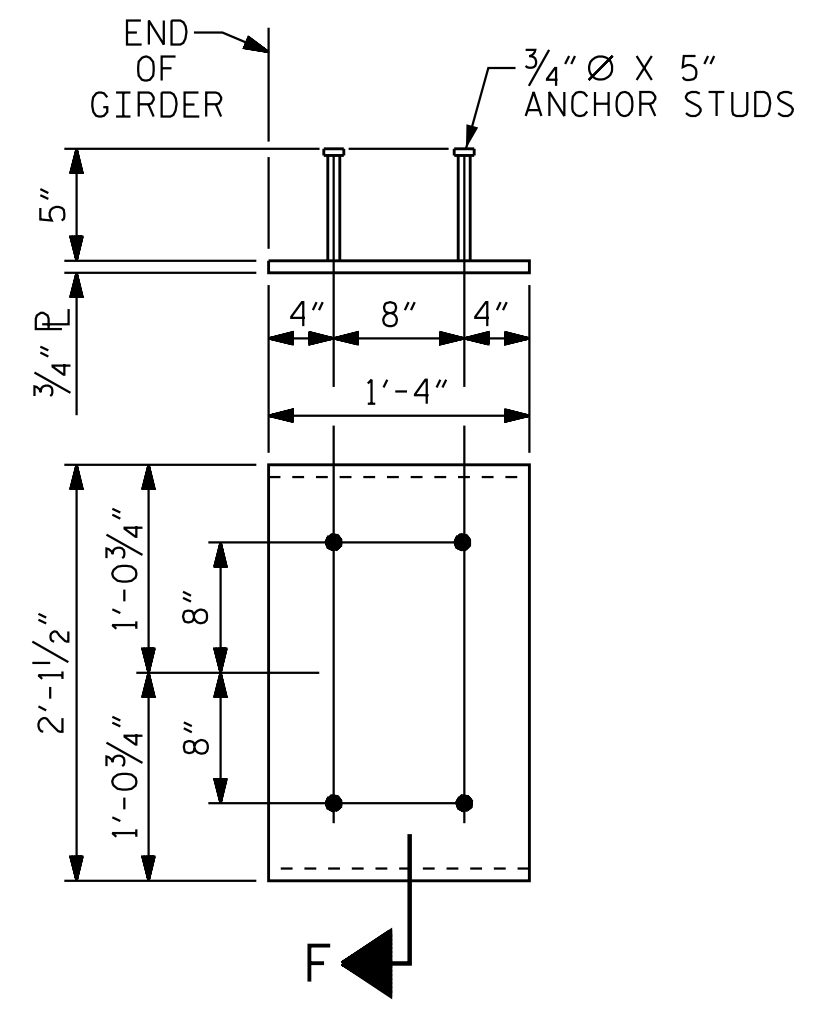
THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN 1/2" OF THE THEORETICAL LOCATION SHOWN.

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE.

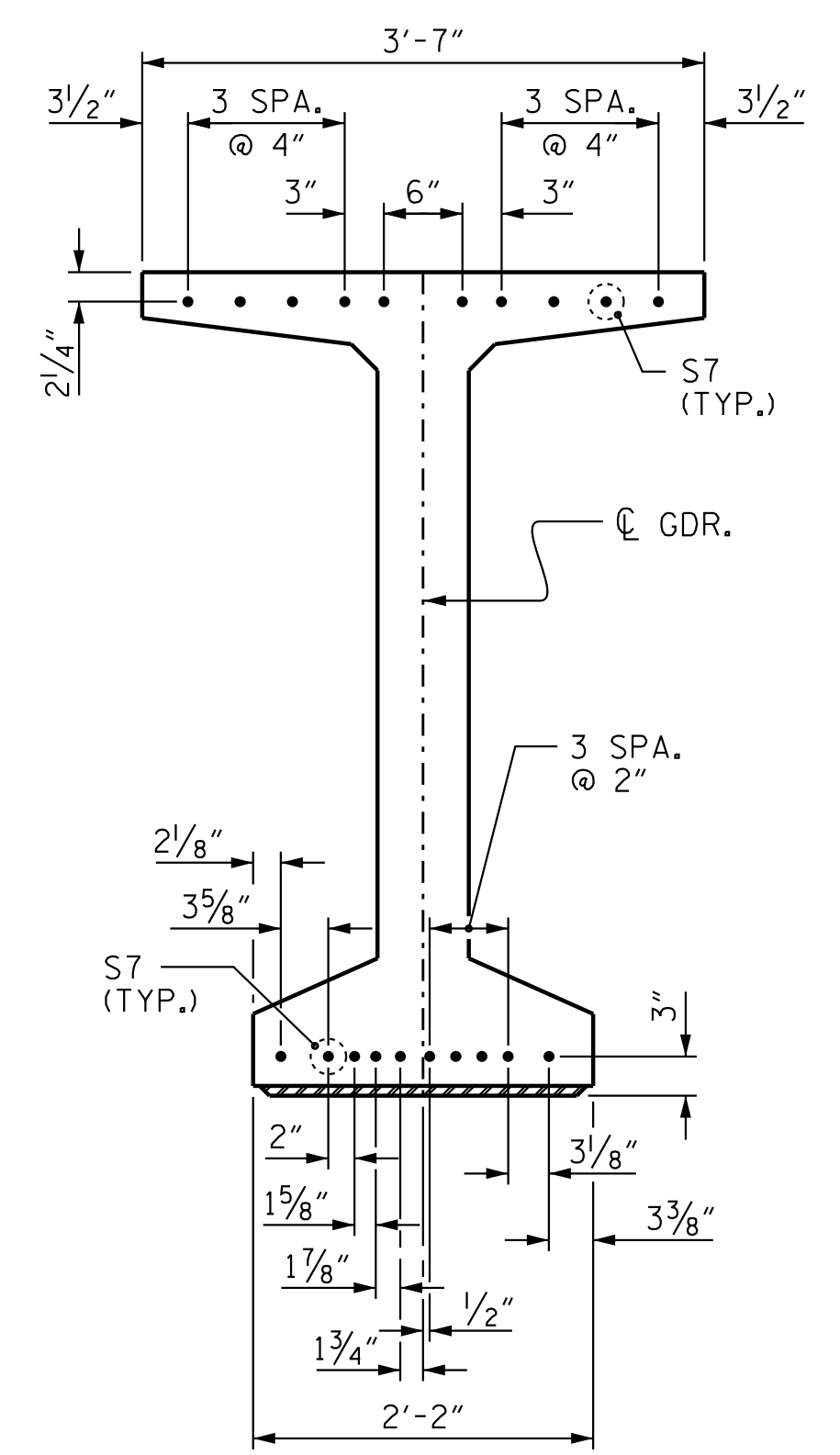
THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4500 lbs.

PRESTRESSED CONCRETE GIRDERS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR. SEE STANDARD SPECIFICATIONS FOR CALCIUM NITRITE CORROSION INHIBITOR.

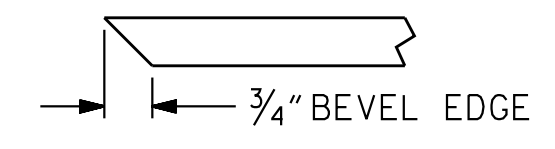


### EMBEDDED PLATE "B-1" DETAILS FOR 54" MODIFIED BULB TEES

(2 REQ'D PER GIRDER)



### DETAIL "C"



### SECTION "F"

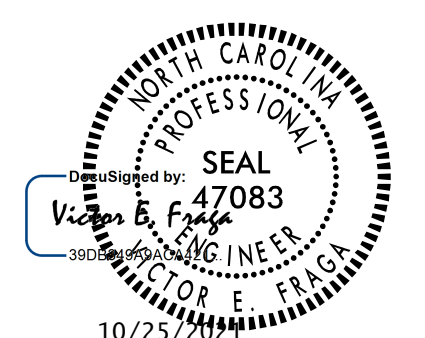
(SEE NOTES)

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

SHEET 3 OF 4

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD  
 PRESTRESSED CONCRETE GIRDER  
 CONTINUOUS FOR LIVE LOAD  
 DETAILS

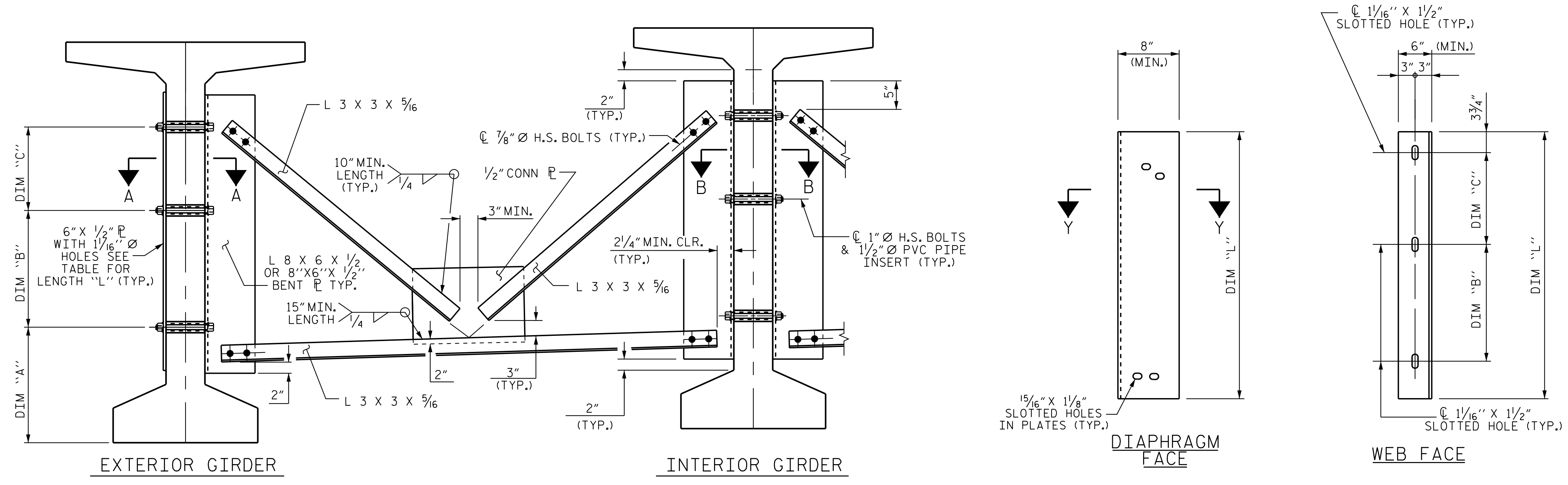


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DRAWN BY : ELR 11/91	REV. 1/15 MAA/TMG		
CHECKED BY : GRP 11/91	REV. 2/15 MAA/TMG		
	REV. 12/17 MAA/THC		



**PART SECTION AT INTERMEDIATE DIAPHRAGM**  
(54" BULB TEE GIRDER SHOWN)

**STRUCTURAL STEEL NOTES**

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

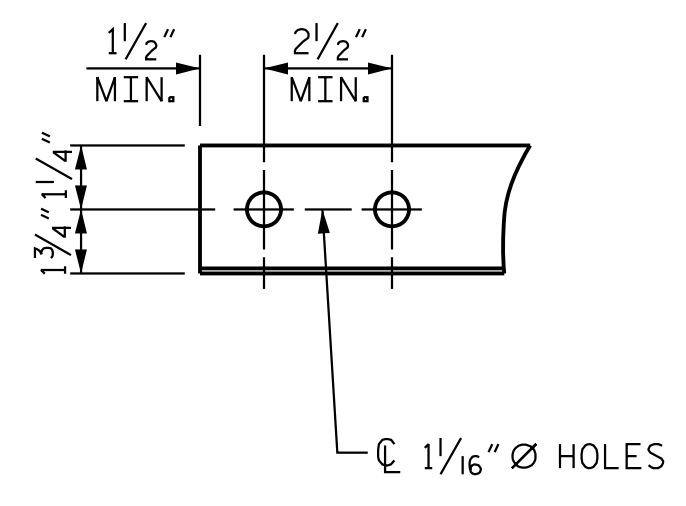
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

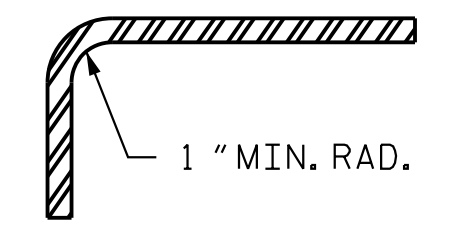
THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

**TABLE**

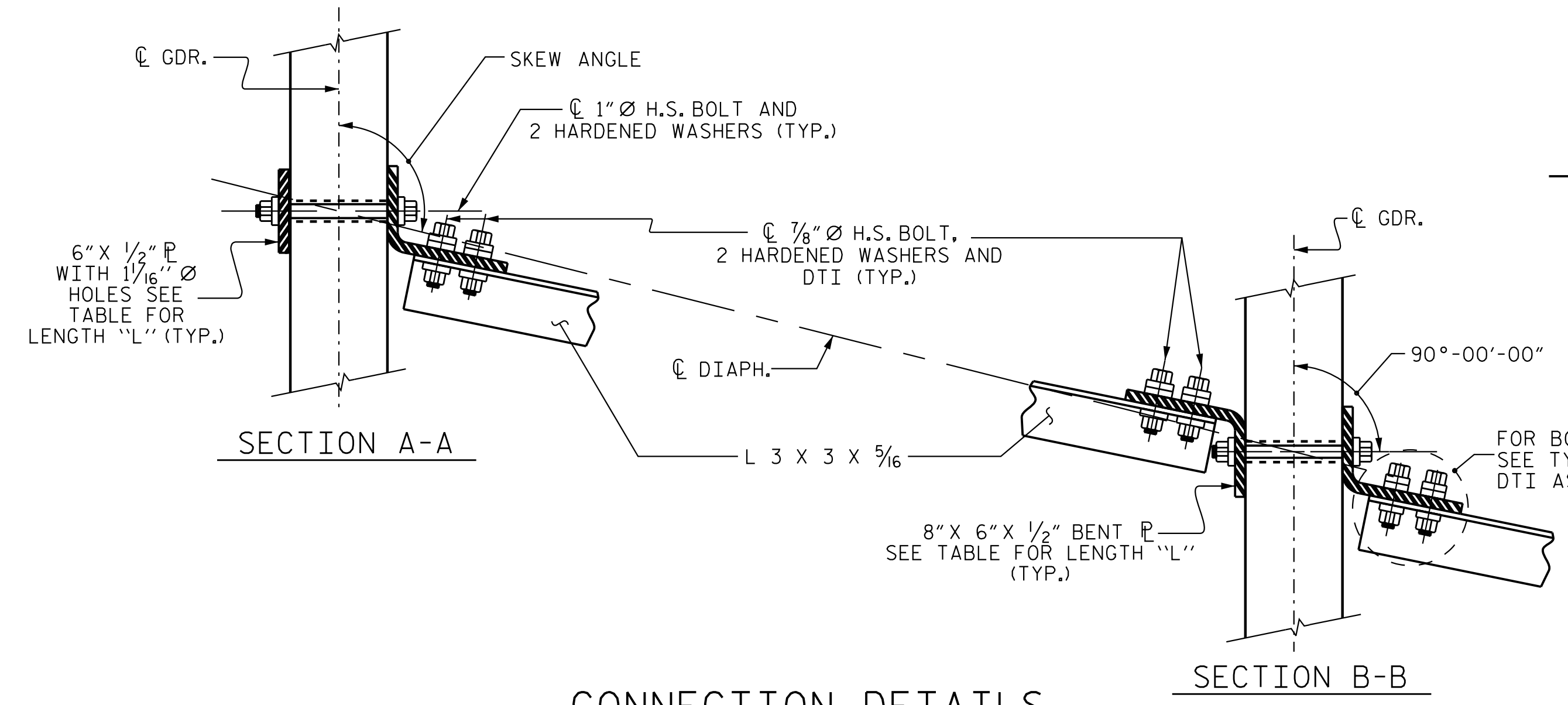
GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
54" BULB TEE	1'-4 1/2"	1'-0"	1'-0"	2'-8"



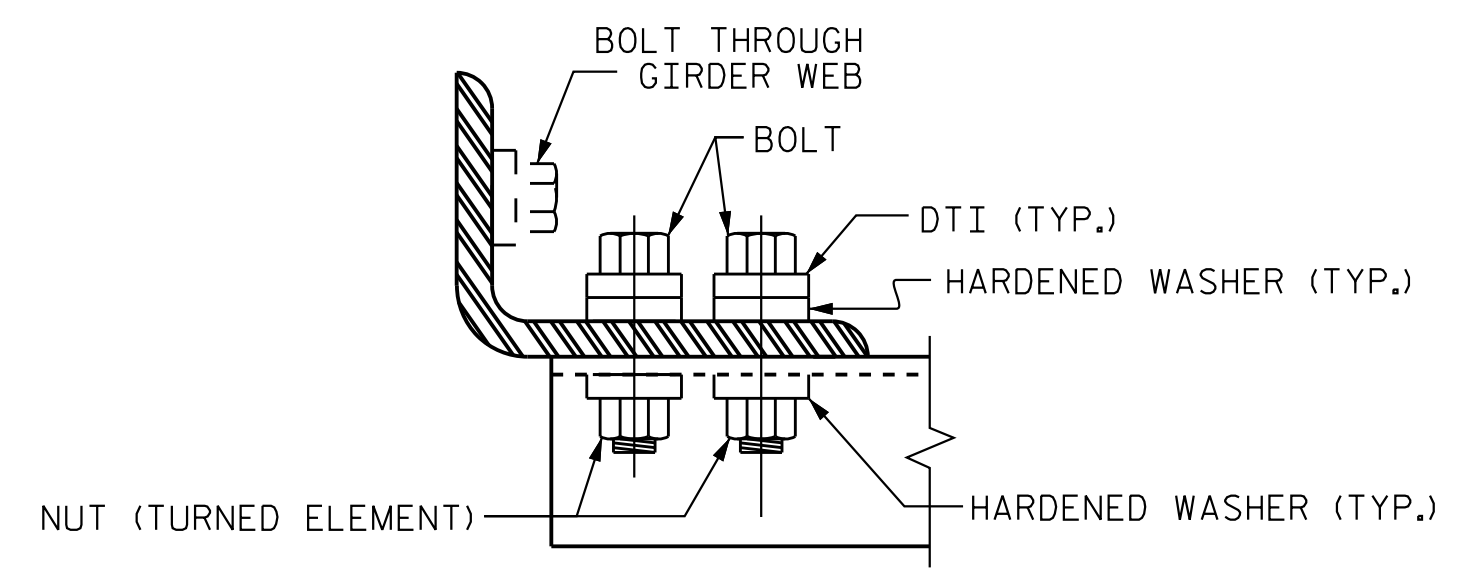
**ANGLE END**  
(L 3 X 3 X 5/16)



**CONNECTOR PLATE DETAIL**



**CONNECTION DETAILS**

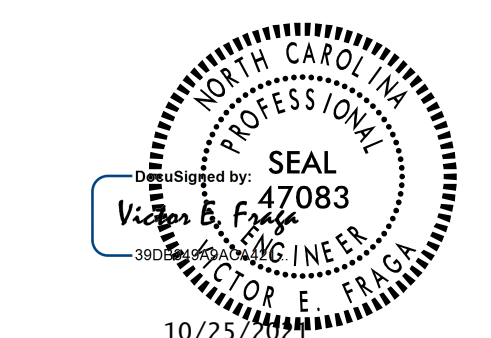


**BOLT WITH DTI ASSEMBLY DETAIL**

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SHEET 4 OF 4

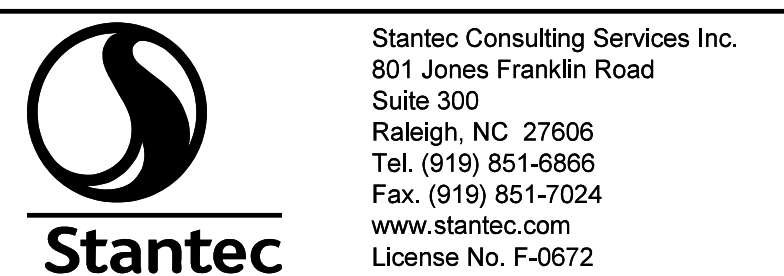
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 INTERMEDIATE  
 STEEL DIAPHRAGMS  
 FOR 54" MODIFIED  
 BULB TEE PRESTRESSED  
 CONCRETE GIRDERS



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 CHECKED BY: V. E. FRAGA DATE: 02/11/20  
 DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21

**NOTES**

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

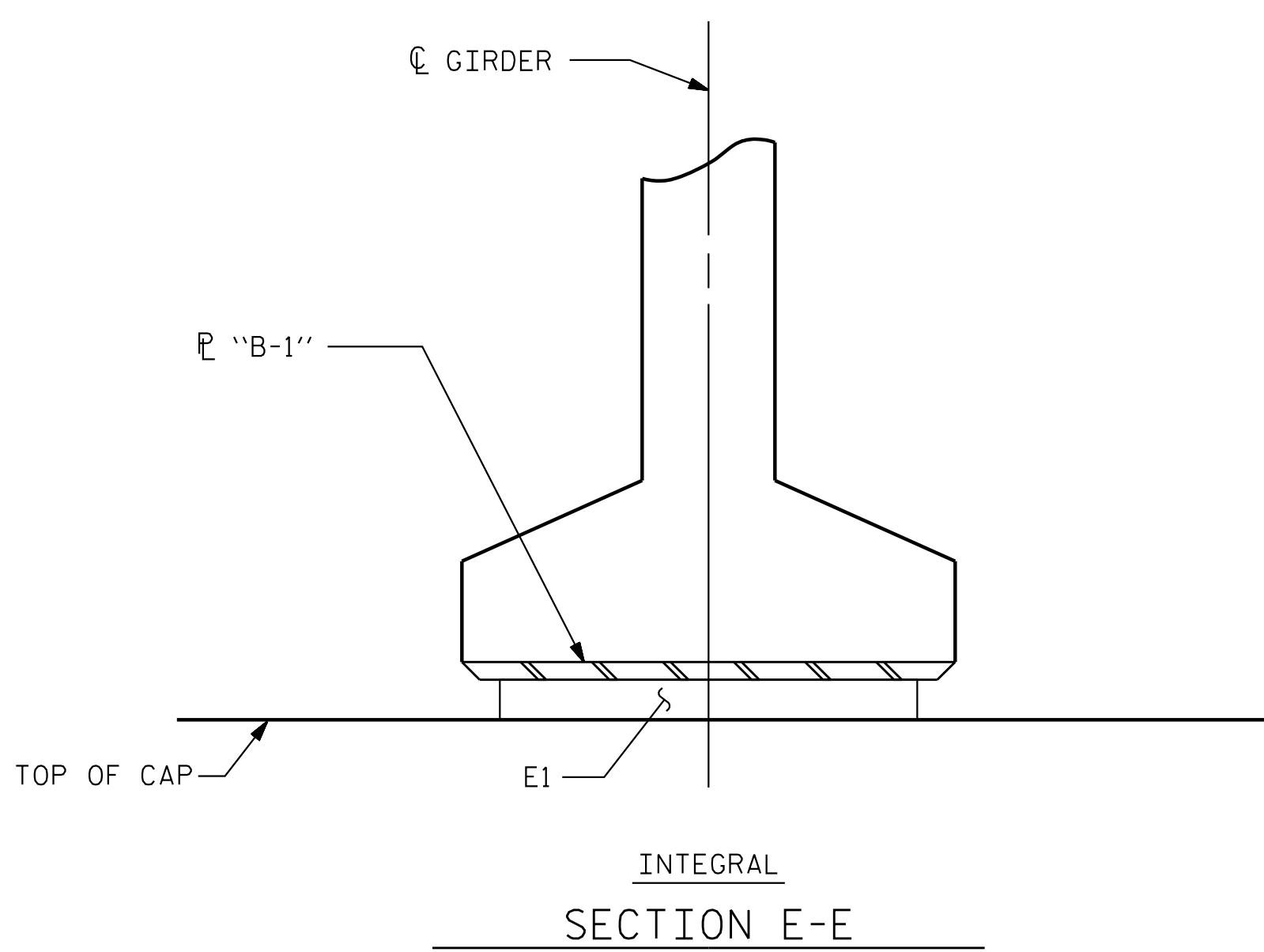
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

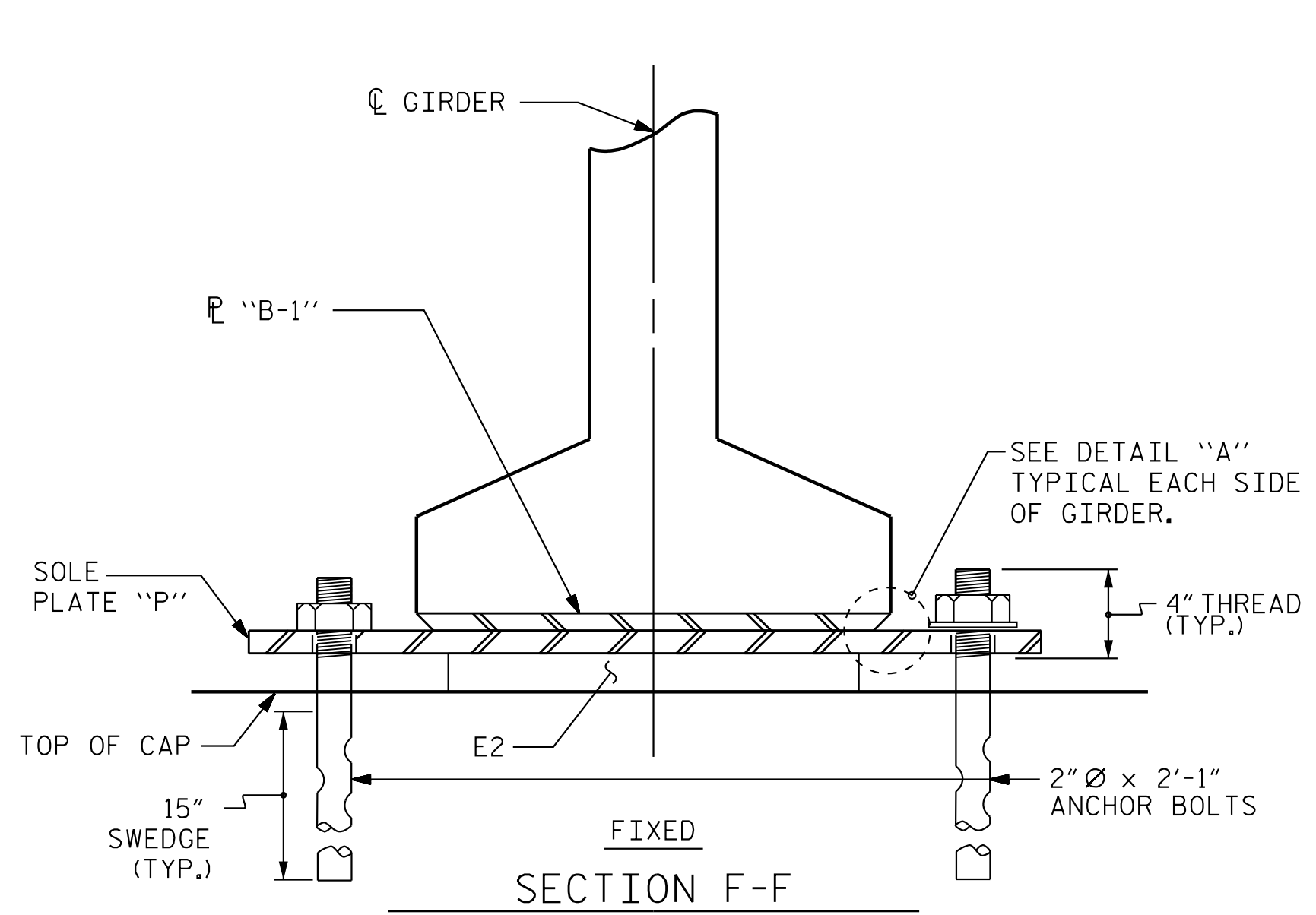
THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

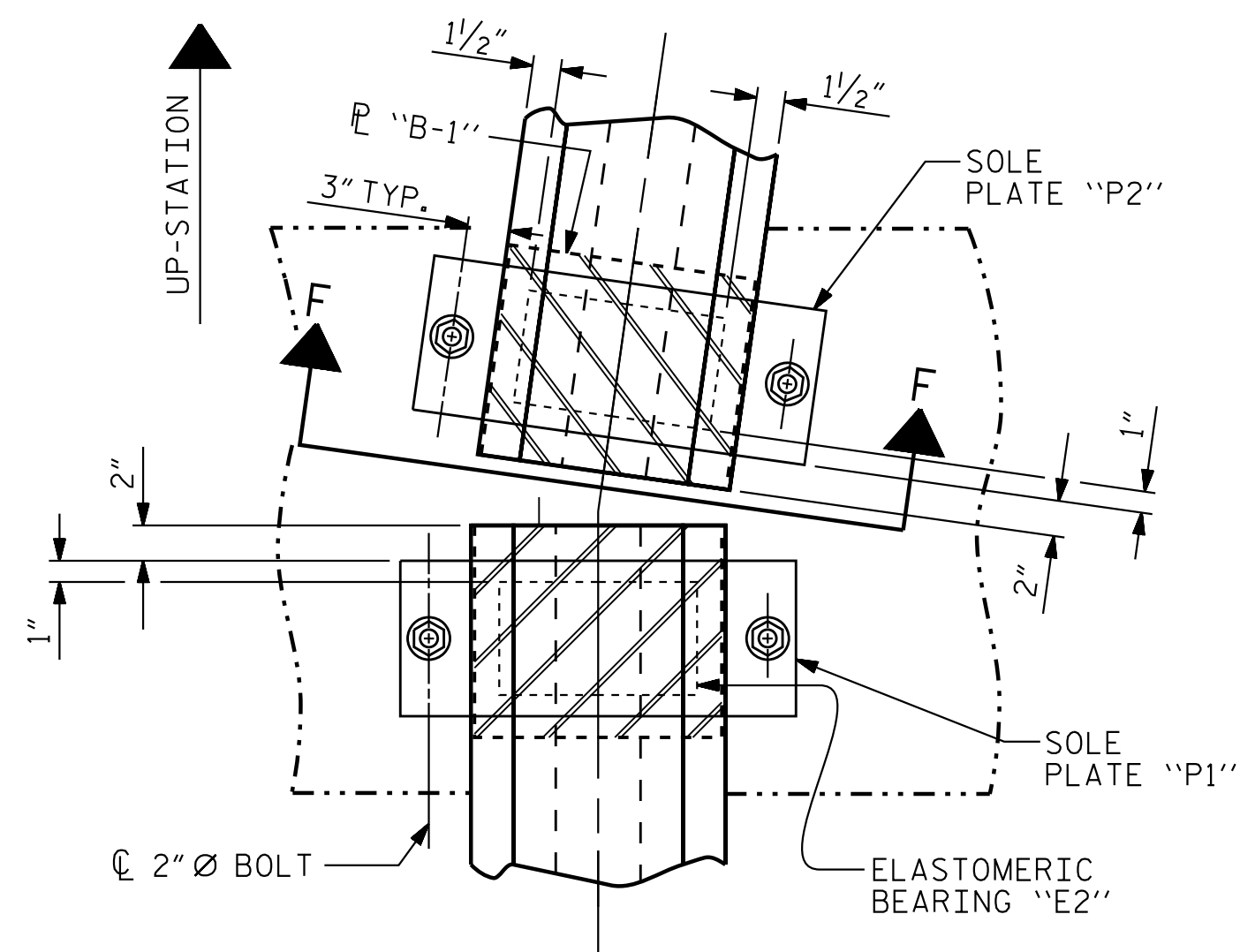
ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.



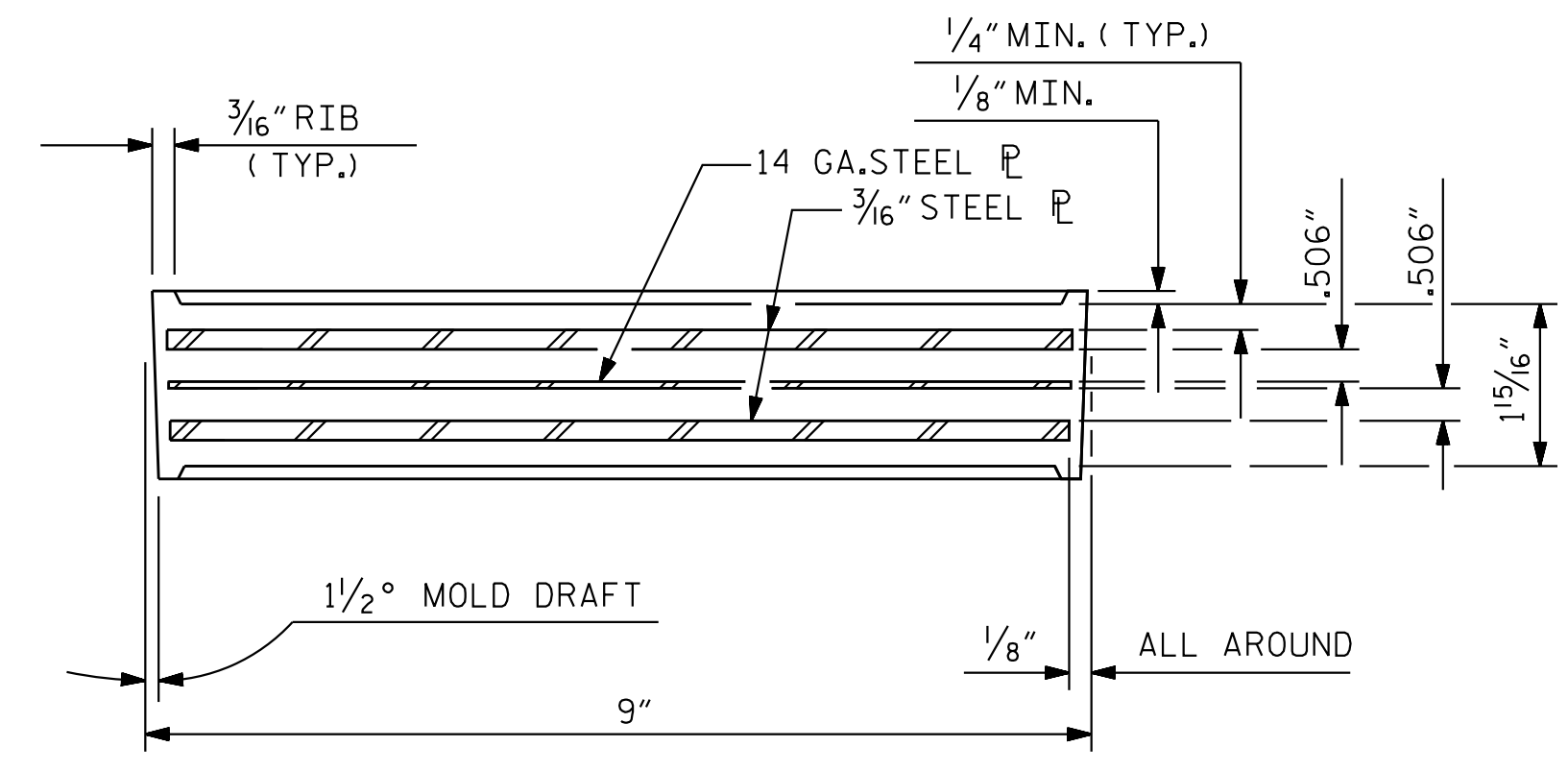
INTEGRAL SECTION E-E



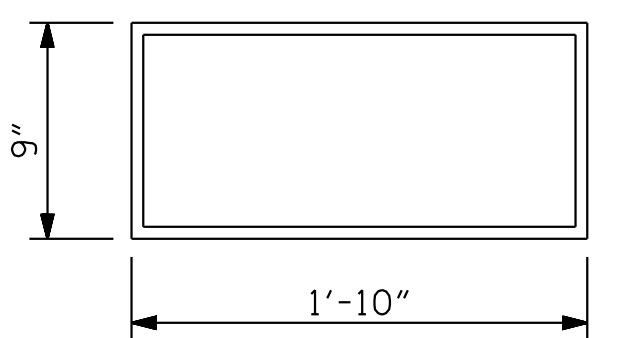
FIXED SECTION F-F



TYPICAL PLAN (SHOWING CONTINUOUS BENT)



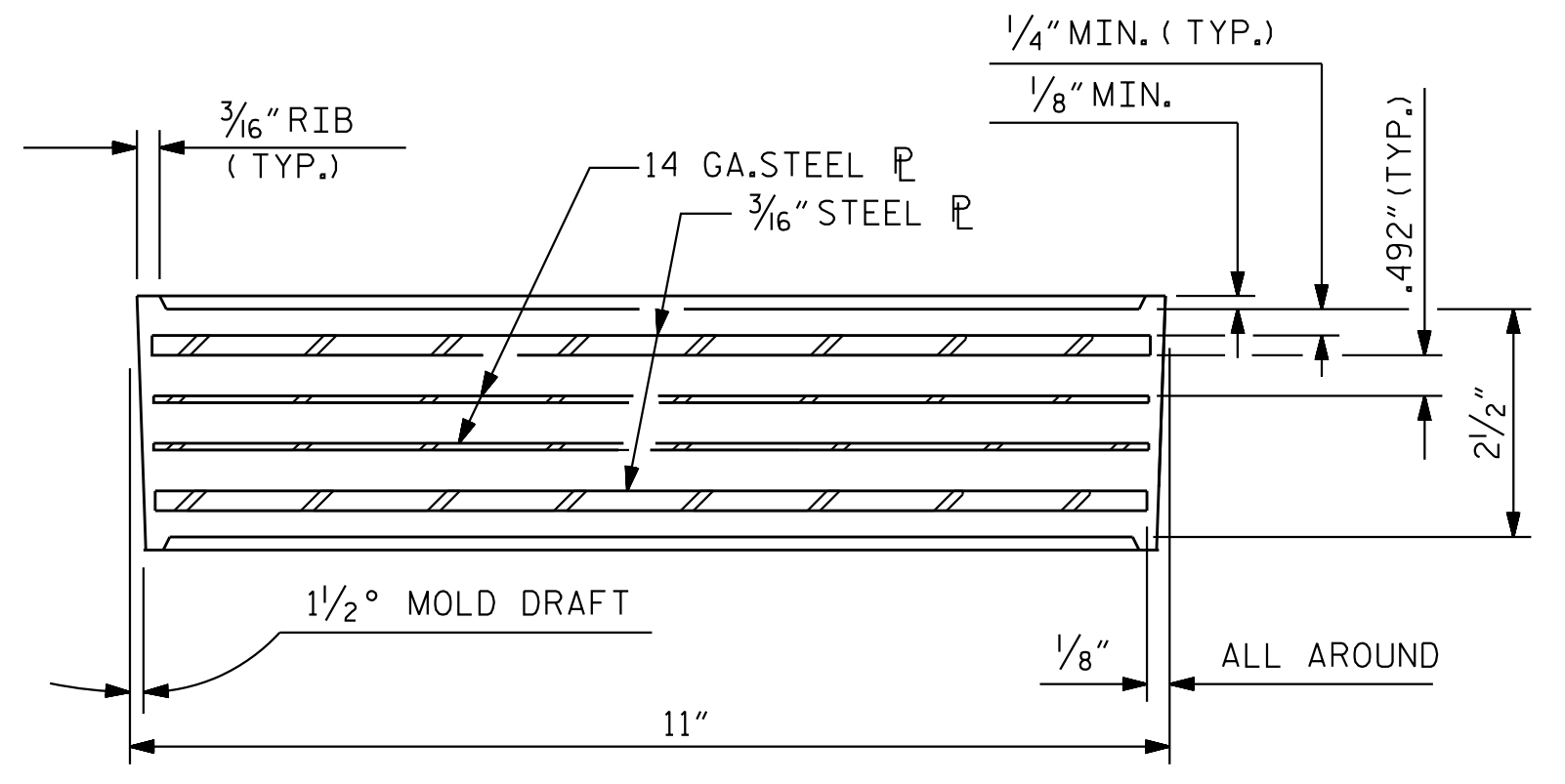
TYPICAL SECTION OF ELASTOMERIC BEARINGS



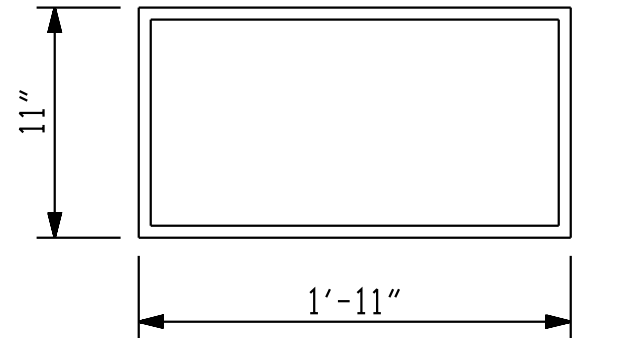
E1 (14 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

TYPE IV



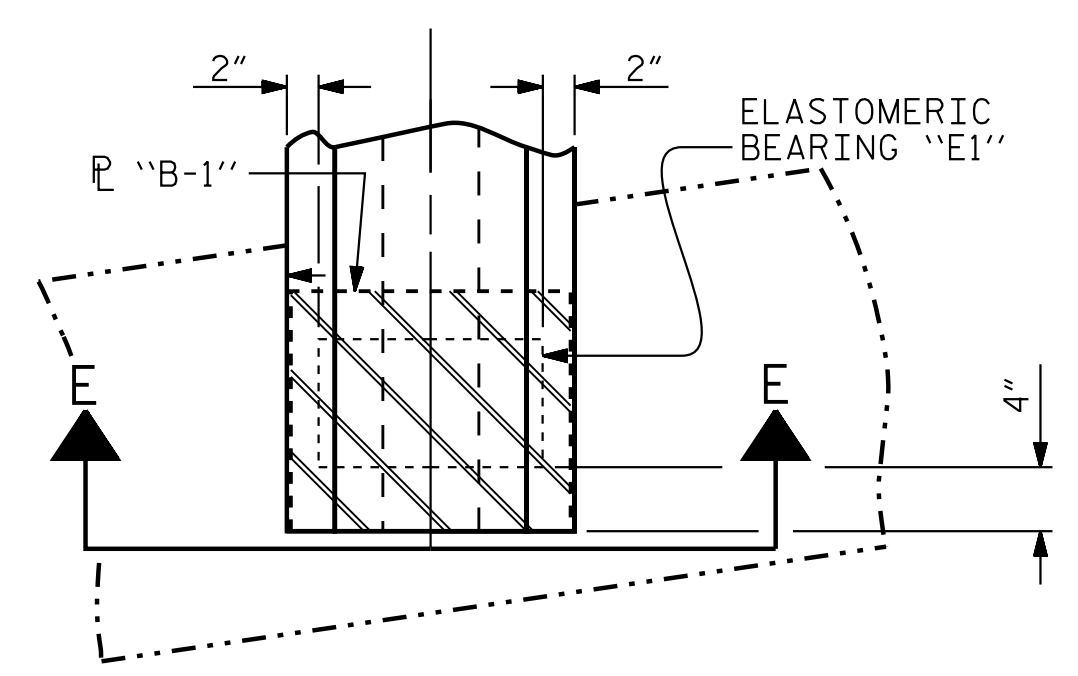
TYPICAL SECTION OF ELASTOMERIC BEARINGS



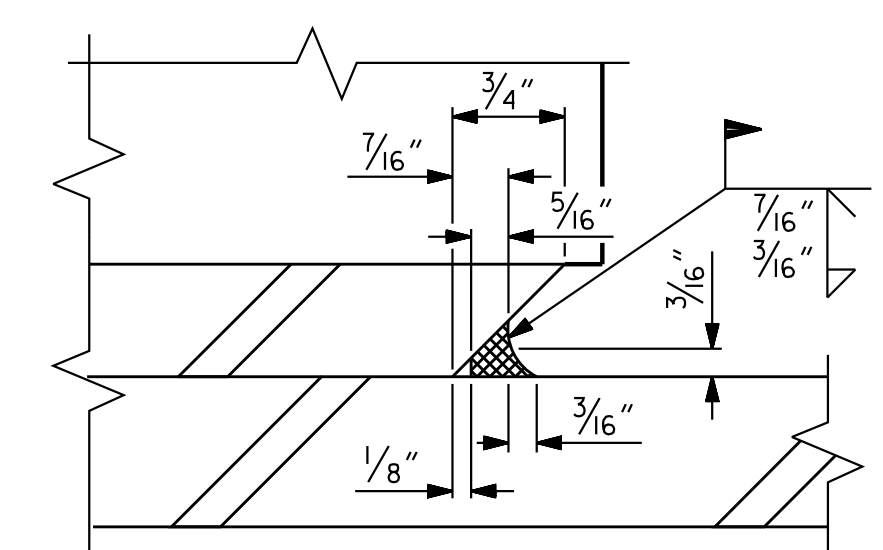
E2 (14 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

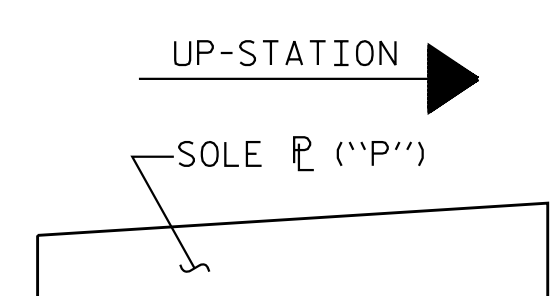
TYPE VI



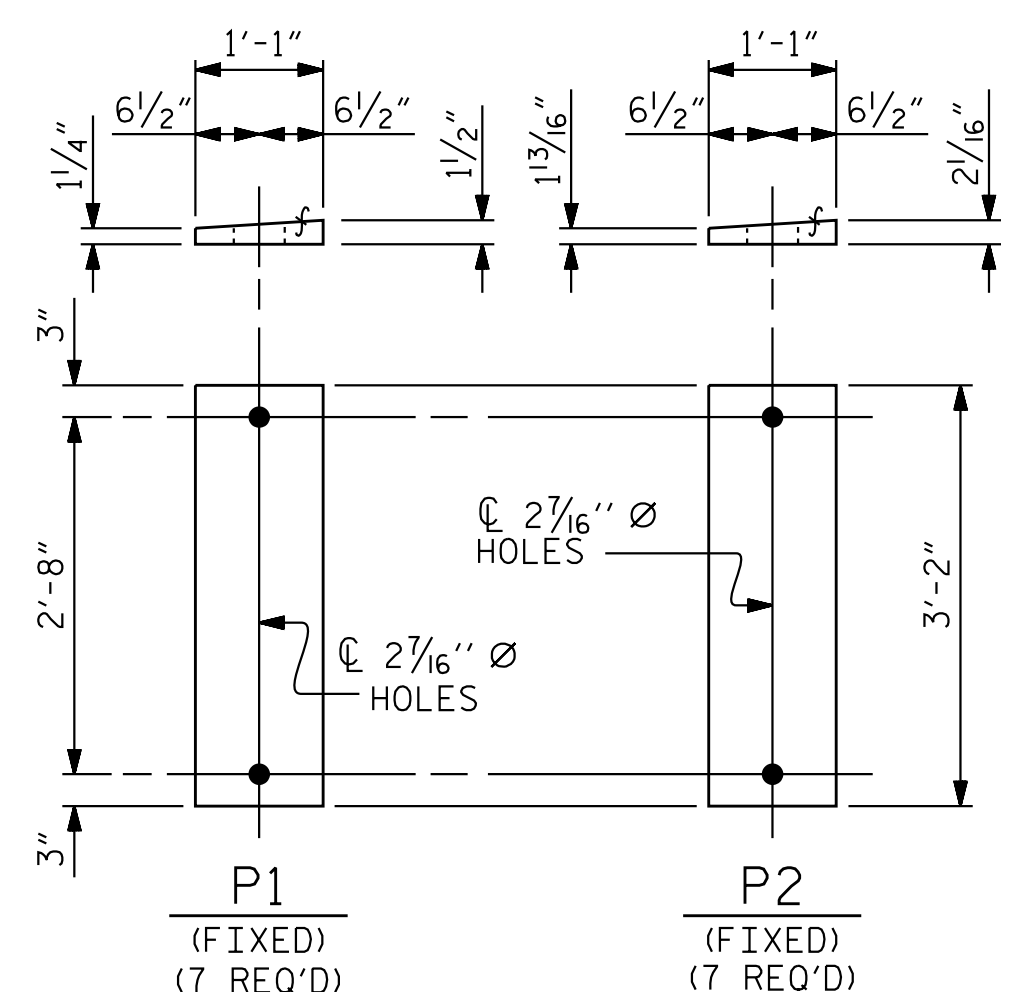
TYPICAL PLAN (SHOWING INTEGRAL END BENT)



DETAIL "A"



SOLE PLATE PLACEMENT DETAIL



SOLE PLATE DETAILS ("P")

MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k
TYPE VI	420 k

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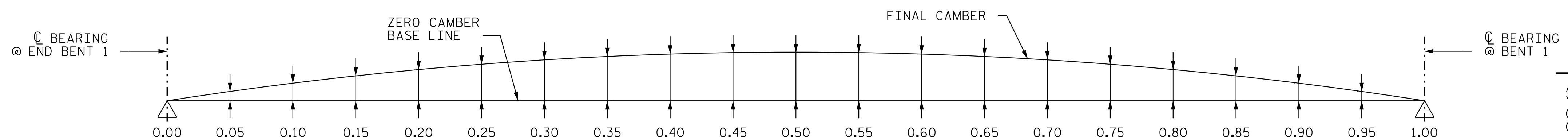
DRAWN BY : V. E. FRAGA DATE : 01/30/20  
CHECKED BY : T. N. ENNIS DATE : 02/07/20  
DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

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PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

REVISIONS						SHEET NO. S17-20
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			38

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**NOTES**

ALL VALUES ARE SHOWN IN DECIMALS OF A FOOT EXCEPT "FINAL CAMBER" WHICH IS SHOWN IN INCHES.  
 (+) FINAL CAMBER INDICATES NET UPWARD DISPLACEMENT  
 (-) FINAL CAMBER INDICATES NET DOWNWARD DISPLACEMENT

GIRDER 1																					
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.024	0.046	0.068	0.088	0.105	0.120	0.132	0.141	0.146	0.148	0.146	0.141	0.132	0.120	0.105	0.088	0.068	0.046	0.024	0.000
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.010	0.020	0.029	0.039	0.046	0.053	0.058	0.062	0.064	0.065	0.063	0.061	0.056	0.051	0.044	0.036	0.027	0.018	0.009	0.000
FINAL CAMBER ↑	+0"	+3/16"	+5/16"	+7/16"	+9/16"	+1 1/16"	+1 3/16"	+7/8"	+1 5/16"	+1"	+1"	+1"	+1 5/16"	+1 5/16"	+1 3/16"	+3/4"	+5/8"	+1/2"	+3/8"	+3/16"	+0"

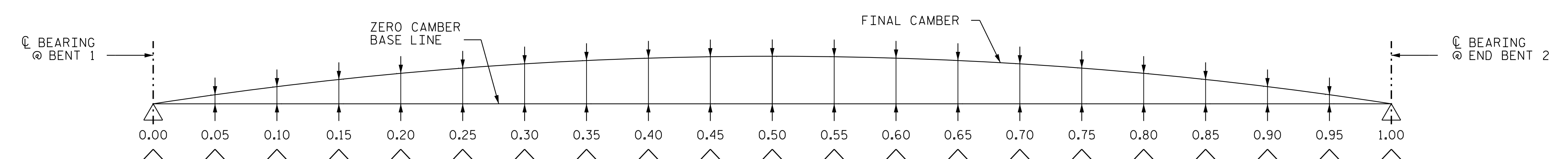
GIRDERS 2-6																					
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.024	0.046	0.068	0.088	0.105	0.120	0.132	0.141	0.146	0.148	0.146	0.141	0.132	0.120	0.105	0.088	0.068	0.046	0.024	0.000
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.011	0.023	0.034	0.045	0.054	0.062	0.068	0.073	0.075	0.076	0.074	0.072	0.066	0.060	0.051	0.043	0.032	0.021	0.011	0.000
FINAL CAMBER ↑	+0"	+1/8"	+5/16"	+7/16"	+1/2"	+5/8"	+1 1/16"	+3/4"	+1 3/16"	+7/8"	+7/8"	+7/8"	+1 3/16"	+1 3/16"	+3/4"	+5/8"	+9/16"	+7/16"	+5/16"	+1/8"	+0"

GIRDER 7																					
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.024	0.046	0.068	0.088	0.105	0.120	0.132	0.141	0.146	0.148	0.146	0.141	0.132	0.120	0.105	0.088	0.068	0.046	0.024	0.000
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.010	0.019	0.028	0.037	0.045	0.052	0.056	0.061	0.062	0.063	0.061	0.059	0.054	0.050	0.042	0.035	0.026	0.017	0.009	0.000
FINAL CAMBER ↑	+0"	+3/16"	+5/16"	+1/2"	+5/8"	+3/4"	+1 3/16"	+1 5/16"	+1 5/16"	+1"	+1"	+1"	+1"	+1 5/16"	+7/8"	+3/4"	+5/8"	+1/2"	+3/8"	+3/16"	+0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.

**SCHMATIC CAMBER ORDINATES SPAN A**



GIRDER 1																					
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.034	0.067	0.098	0.127	0.152	0.174	0.191	0.203	0.211	0.214	0.211	0.203	0.191	0.174	0.152	0.127	0.098	0.067	0.034	0.000
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.019	0.037	0.056	0.074	0.089	0.103	0.113	0.122	0.126	0.129	0.127	0.124	0.115	0.107	0.092	0.078	0.059	0.040	0.020	0.000
FINAL CAMBER ↑	+0"	+3/16"	+3/8"	+1/2"	+5/8"	+3/4"	+1 3/16"	+1 5/16"	+1"	+1"	+1"	+1"	+1 5/16"	+1 5/16"	+1 3/16"	+3/4"	+9/16"	+1/2"	+5/16"	+3/16"	+0"

GIRDERS 2-6																						
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00	
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.034	0.067	0.098	0.127	0.152	0.174	0.191	0.204	0.211	0.214	0.211	0.204	0.191	0.174	0.152	0.127	0.098	0.067	0.034	0.000	
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.021	0.042	0.063	0.084	0.100	0.117	0.127	0.138	0.141	0.145	0.142	0.139	0.129	0.120	0.103	0.087	0.066	0.045	0.022	0.000	
FINAL CAMBER ↑	+0"	+1/8"	+5/16"	+7/16"	+1/2"	+5/8"	+1 1/16"	+3/4"	+1 3/16"	+1 3/16"	+1 3/16"	+1 3/16"	+1 3/16"	+3/4"	+3/4"	+5/8"	+9/16"	+1/2"	+3/8"	+1/4"	+1/8"	+0"

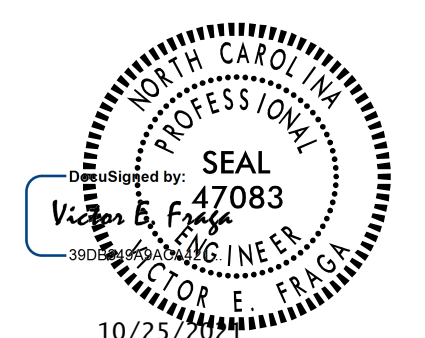
GIRDER 7																					
	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.034	0.067	0.098	0.127	0.152	0.174	0.191	0.204	0.211	0.214	0.211	0.204	0.191	0.174	0.152	0.127	0.098	0.067	0.034	0.000
DEFLEC. DUE TO SUPERIMPOSED DL * ↓	0.000	0.017	0.034	0.051	0.068	0.081	0.095	0.103	0.112	0.115	0.118	0.116	0.114	0.106	0.098	0.084	0.071	0.054	0.036	0.018	0.000
FINAL CAMBER ↑	+0"	+3/16"	+3/8"	+9/16"	+1 1/16"	+7/8"	+1 5/16"	+1 1/16"	+1 1/8"	+1 1/8"	+1 1/8"	+1 1/8"	+1 1/16"	+1"	+1 5/16"	+1 3/16"	+1 1/16"	+9/16"	+3/8"	+3/16"	+0"

\* INCLUDES FUTURE WEARING SURFACE IN SUPERIMPOSED DEAD LOAD.

**SCHMATIC CAMBER ORDINATES SPAN B**

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
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STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 DEAD LOAD DEFLECTIONS



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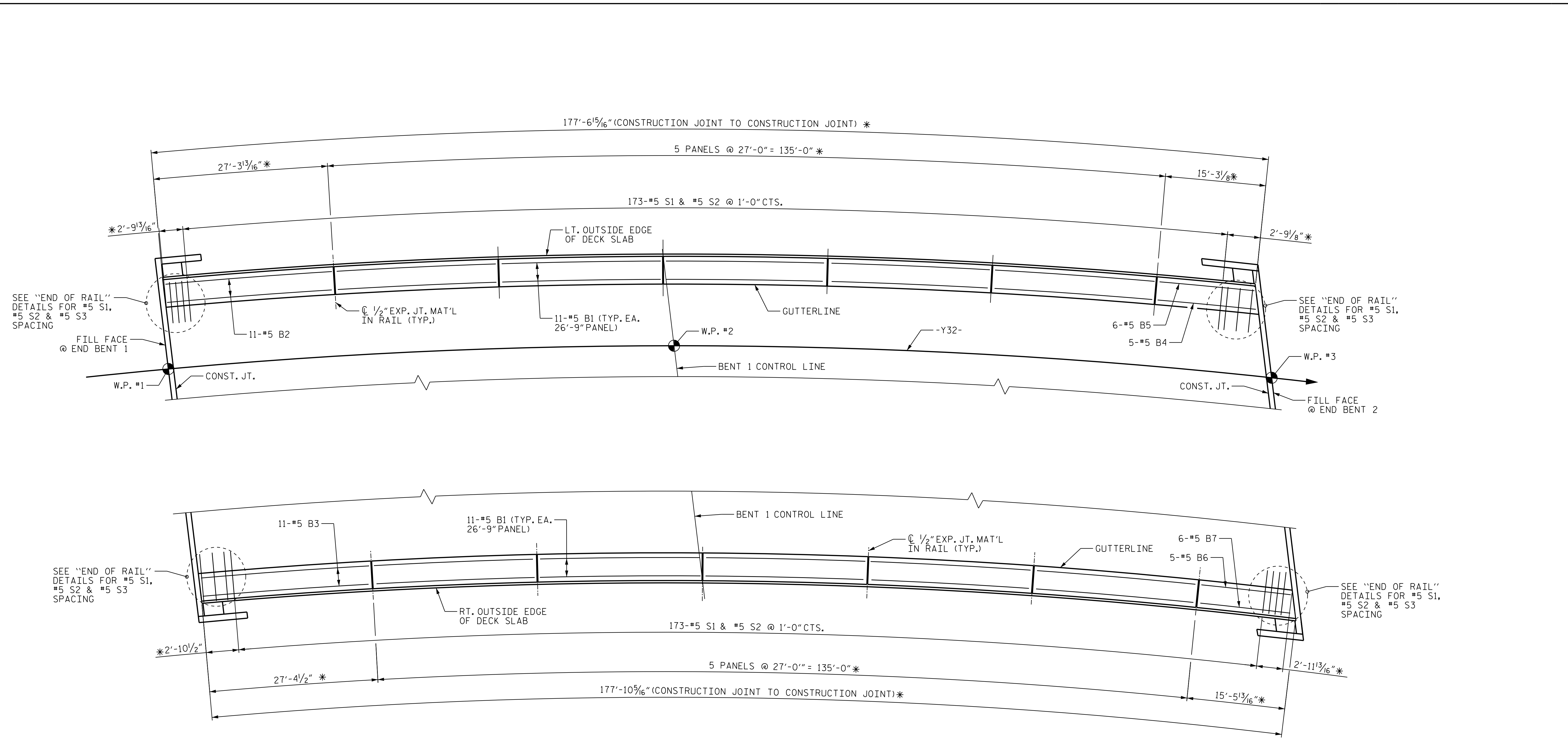
DRAWN BY : V. E. FRAGA DATE : 01/30/20  
 CHECKED BY : T. N. ENNIS DATE : 02/10/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

NO.	REVISIONS			NO.	REVISIONS			SHEET NO.
	BY:	DATE:			BY:	DATE:		
1				3				S17-21
2				4				TOTAL SHEETS 38

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### BARRIER RAIL PLAN

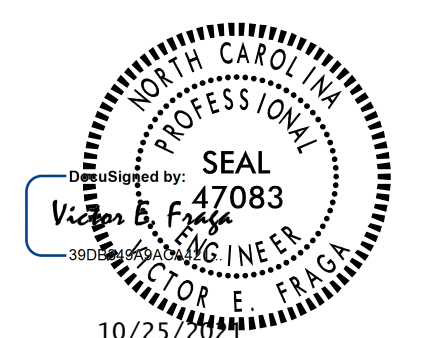
\* ALONG BACKFACE OF BARRIER RAIL

FOR ADDITIONAL BARRIER RAIL DETAILS, NOTES & BILL OF MATERIAL, SEE "CONCRETE BARRIER RAIL" SHEET 2 OF 2.

PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
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SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 CONCRETE  
 BARRIER RAIL



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DRAWN BY : N. D'AIUTO DATE : 10/02/19  
 CHECKED BY : T. N. ENNIS DATE : 02/11/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

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2			4			38

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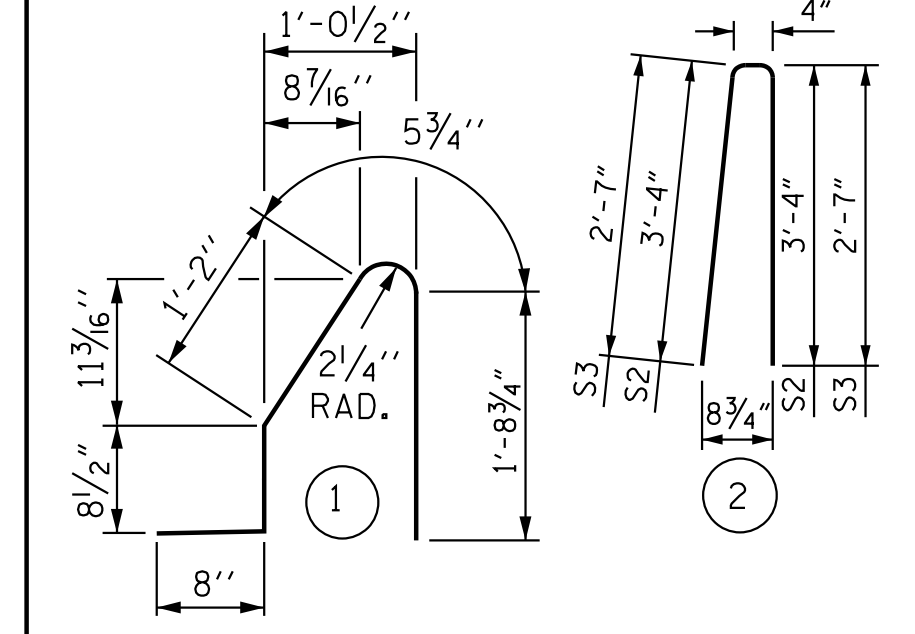
NOTES

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

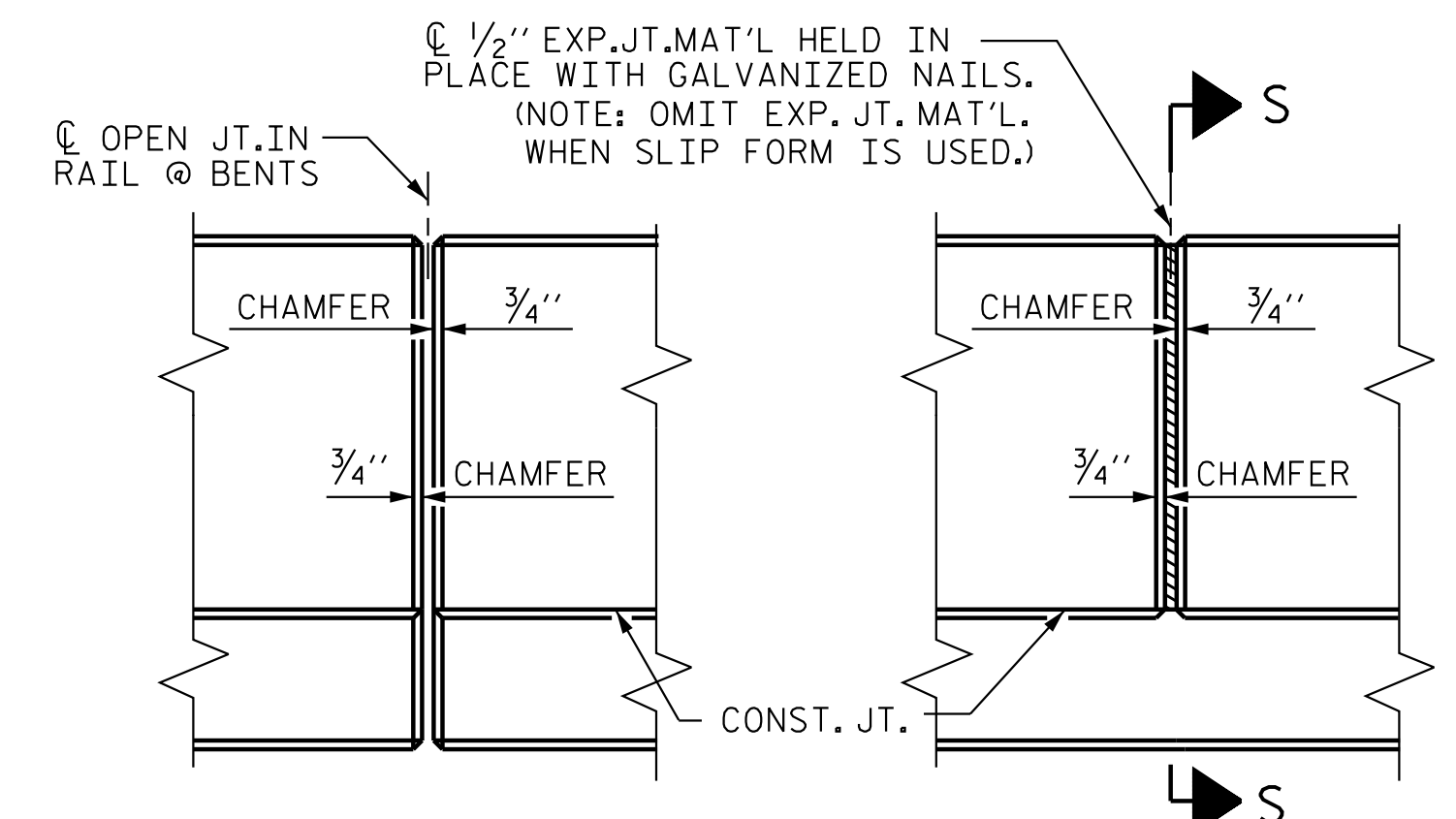
BAR TYPES



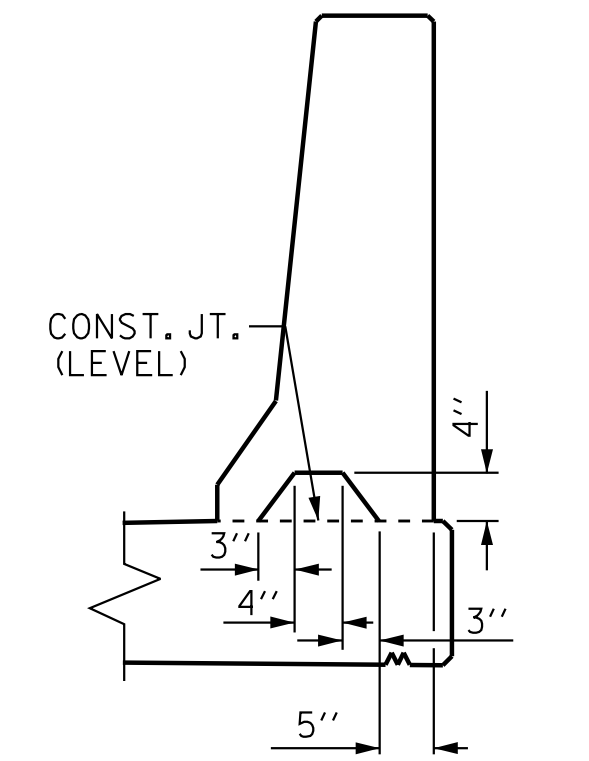
ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

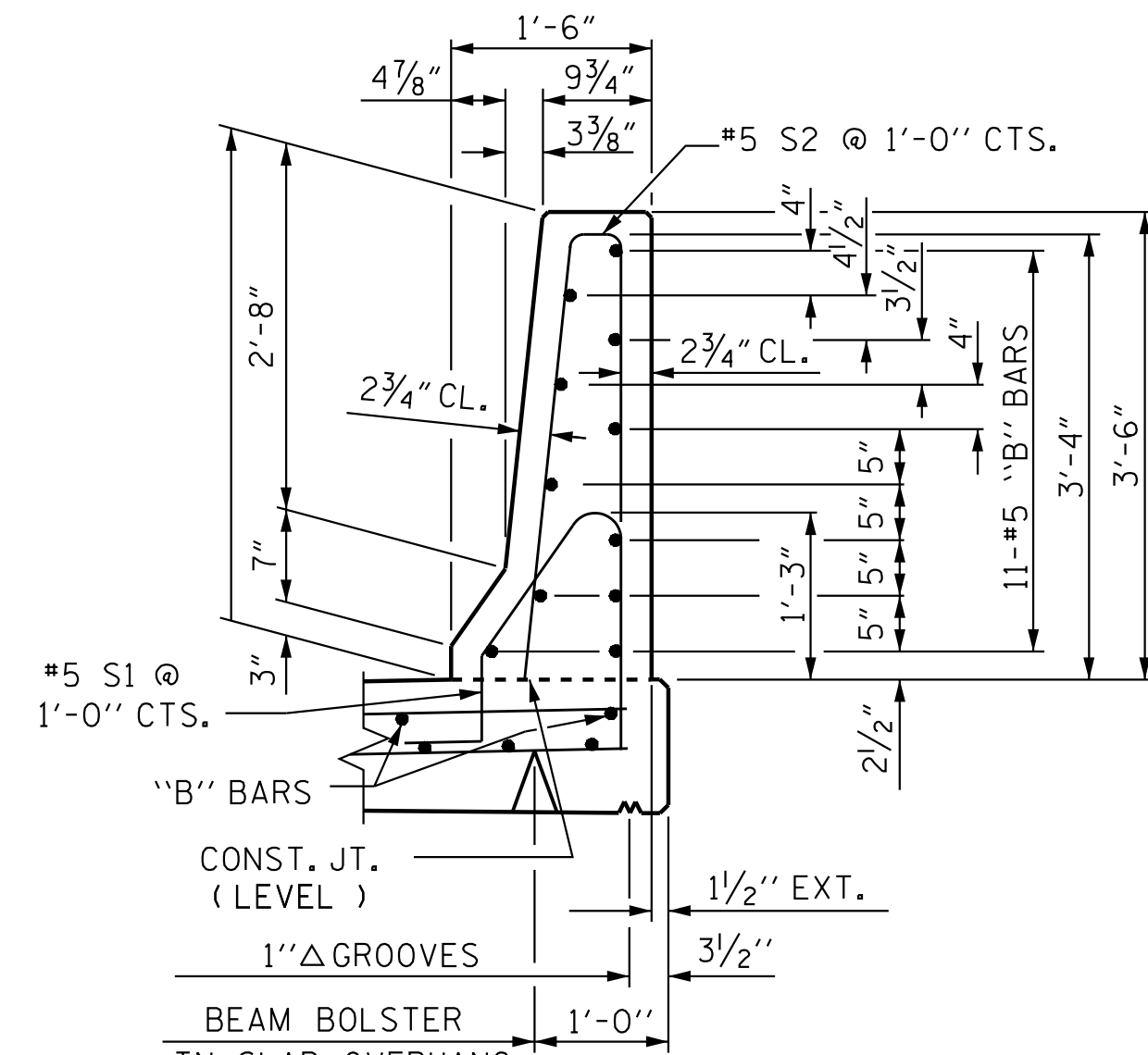
CONCRETE BARRIER RAIL (BOTH SIDES)						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
*B1	110	#5	STR	26'-6"	3041	
*B2	11	#5	STR	26'-10"	308	
*B3	11	#5	STR	27'-0"	310	
*B4	5	#5	STR	15'-2"	80	
*B5	6	#5	STR	14'-11"	94	
*B6	5	#5	STR	14'-9"	77	
*B7	6	#5	STR	15'-0"	94	
*S1	358	#5	1	4'-9"	1774	
*S2	350	#5	2	7'-0"	2556	
*S3	8	#5	2	5'-6"	46	
* EPOXY COATED REINFORCING STEEL					8,380	LBS.
CLASS AA CONCRETE					48.3	CU. YDS.
CONCRETE BARRIER RAIL					355.4	LIN. FT.



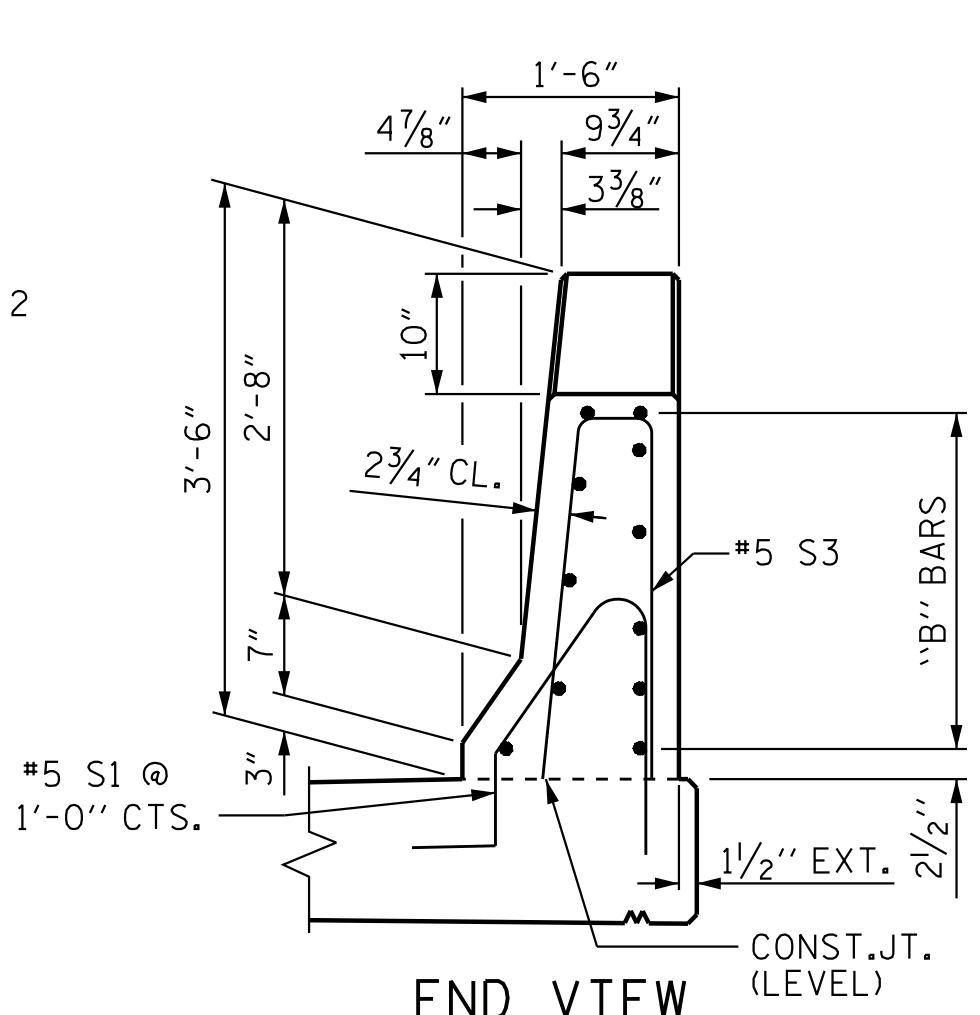
ELEVATION AT EXPANSION JOINTS  
BARRIER RAIL DETAILS



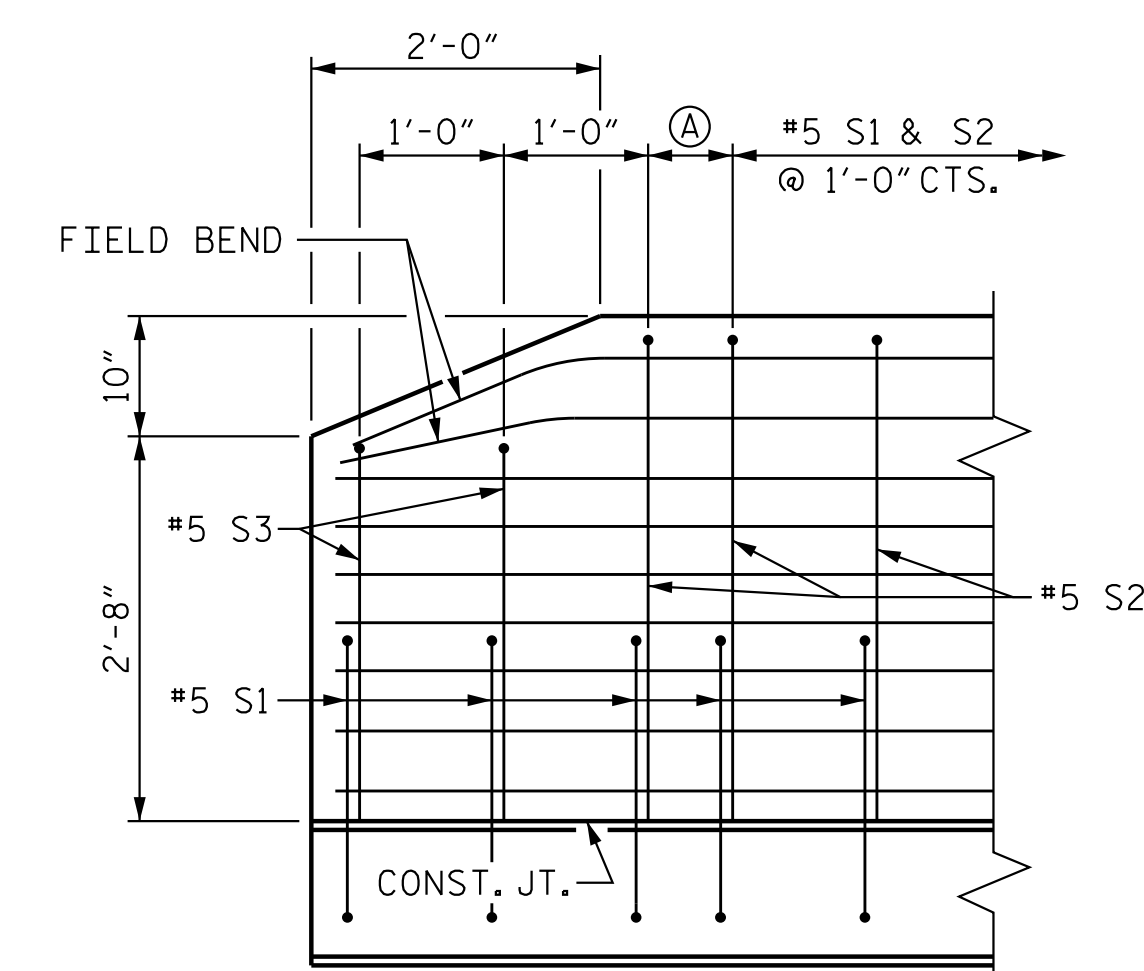
SECTION S-S  
AT DAM IN OPEN JOINT  
(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)



SECTION THRU RAIL

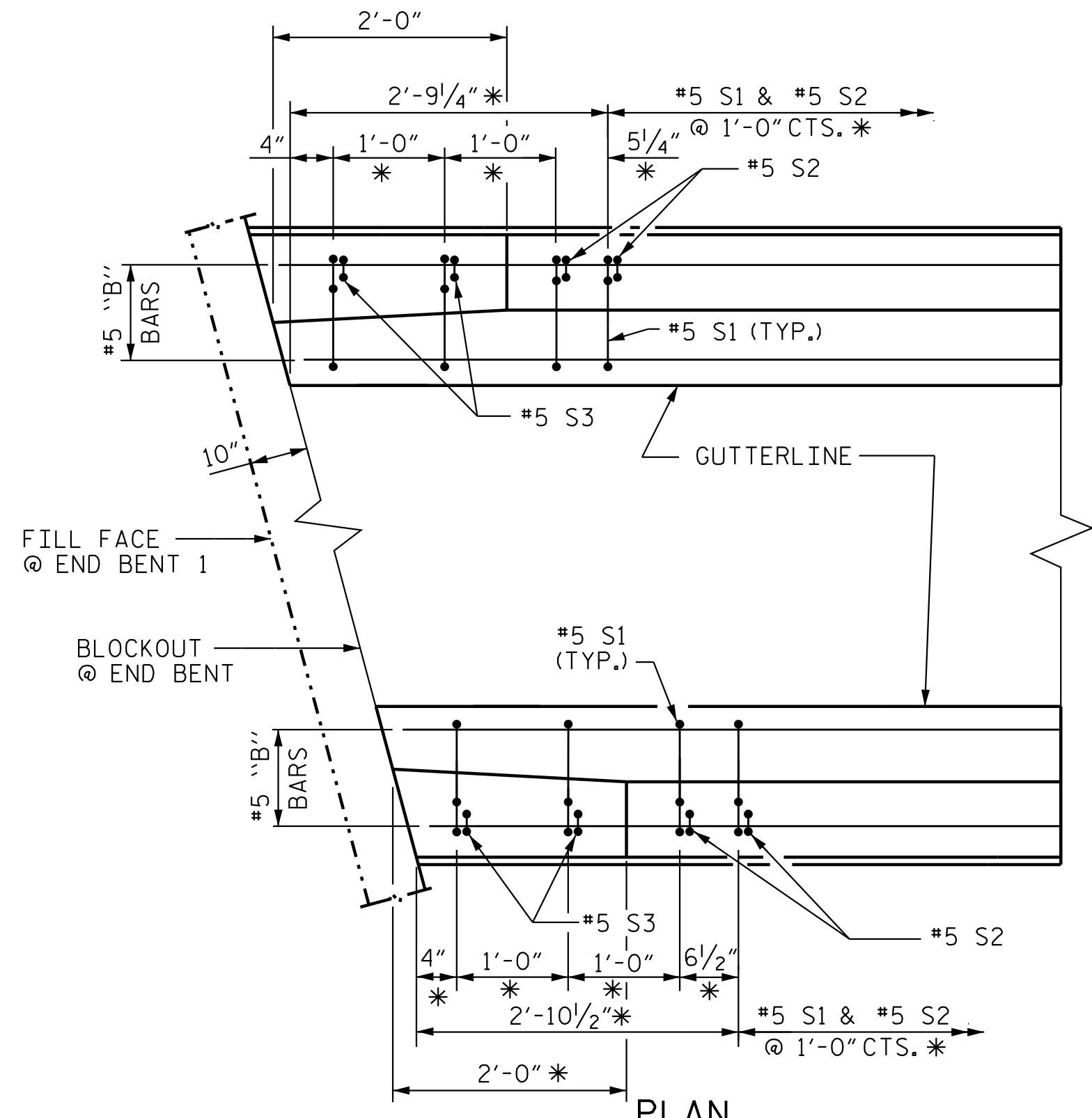


END VIEW

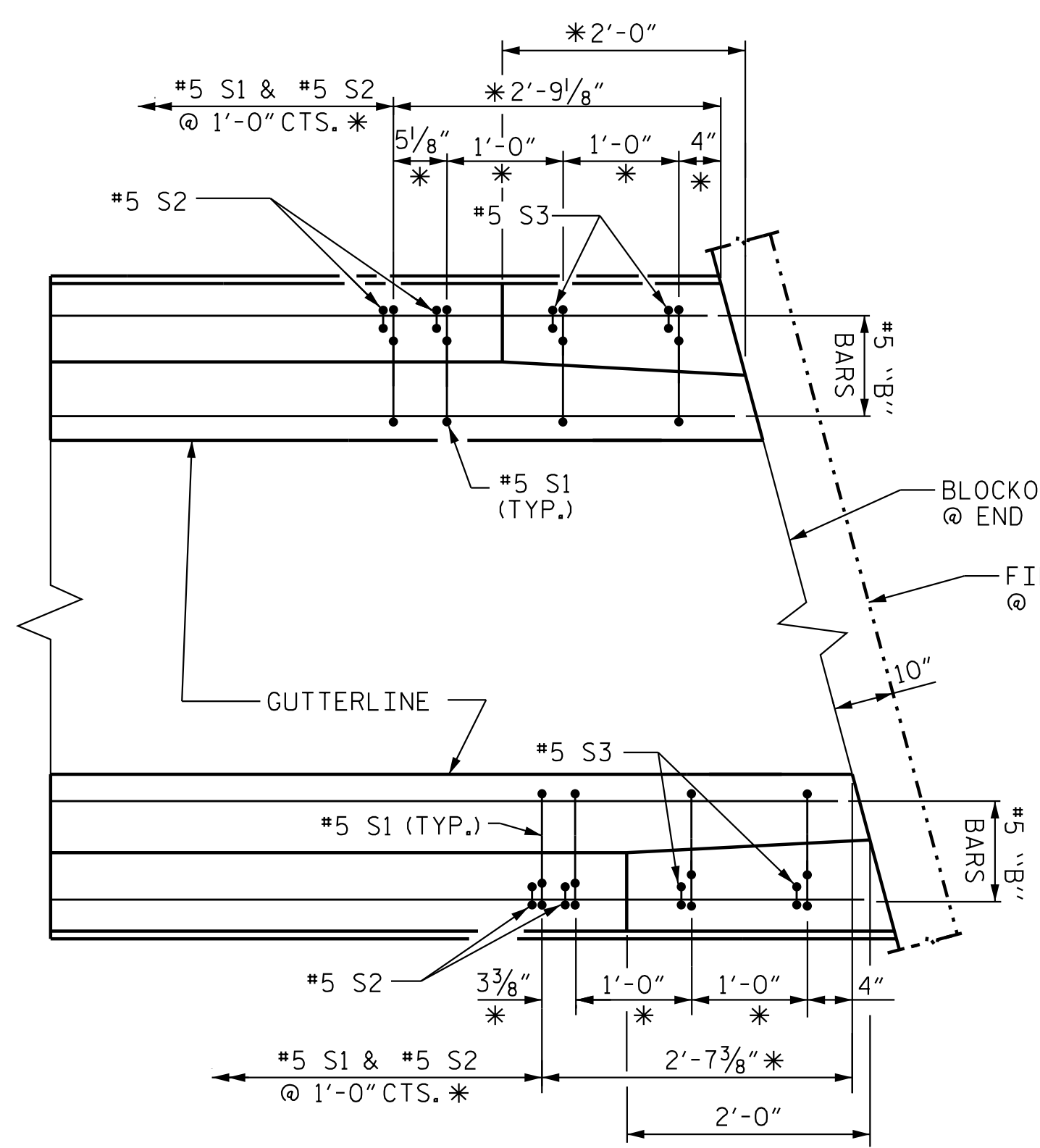


SIDE VIEW

Ⓐ VARIES, SEE SPACING IN PLAN VIEWS



PLAN  
END BENT 1 SHOWN  
\* ALONG BACKFACE OF RAIL



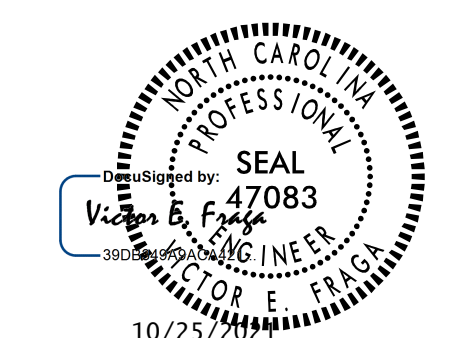
PLAN  
END BENT 2 SHOWN  
\* ALONG BACKFACE OF RAIL

END OF RAIL DETAILS

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

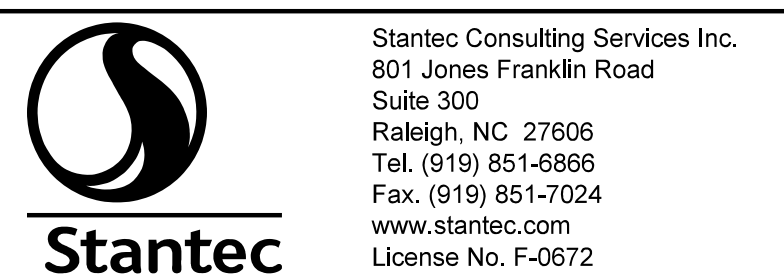
SHEET 2 OF 2

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUPERSTRUCTURE  
CONCRETE BARRIER RAIL  
DETAILS



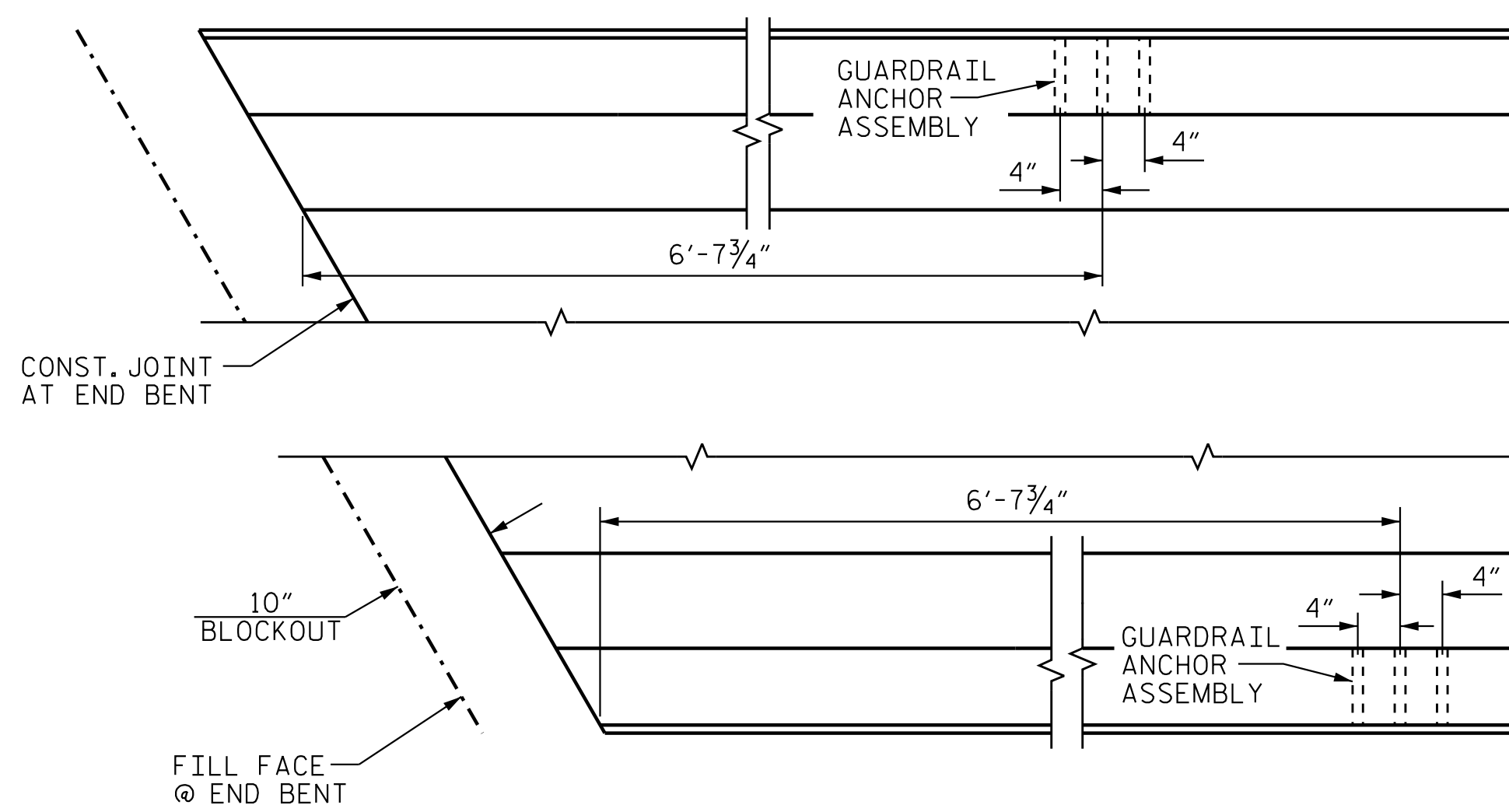
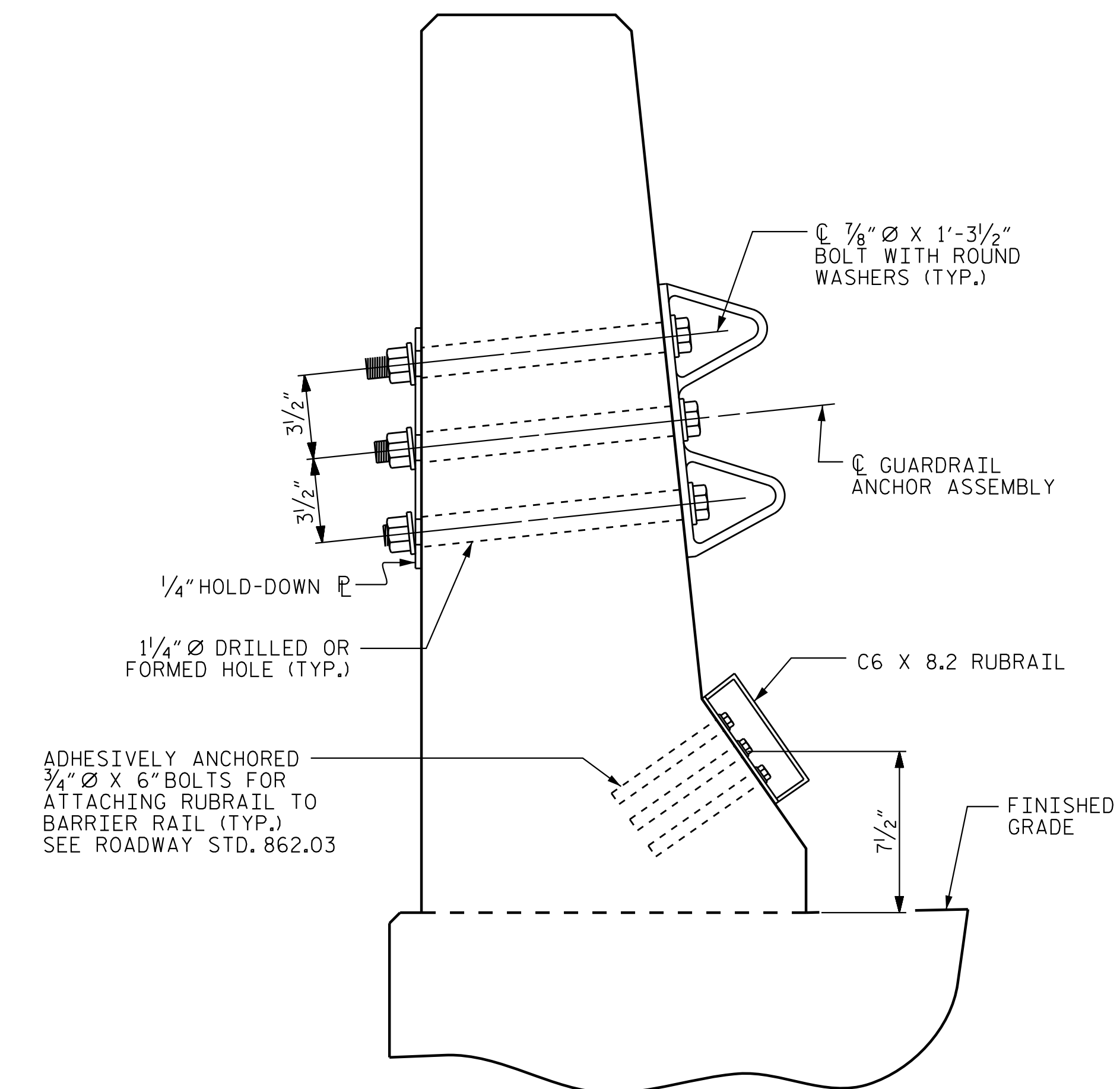
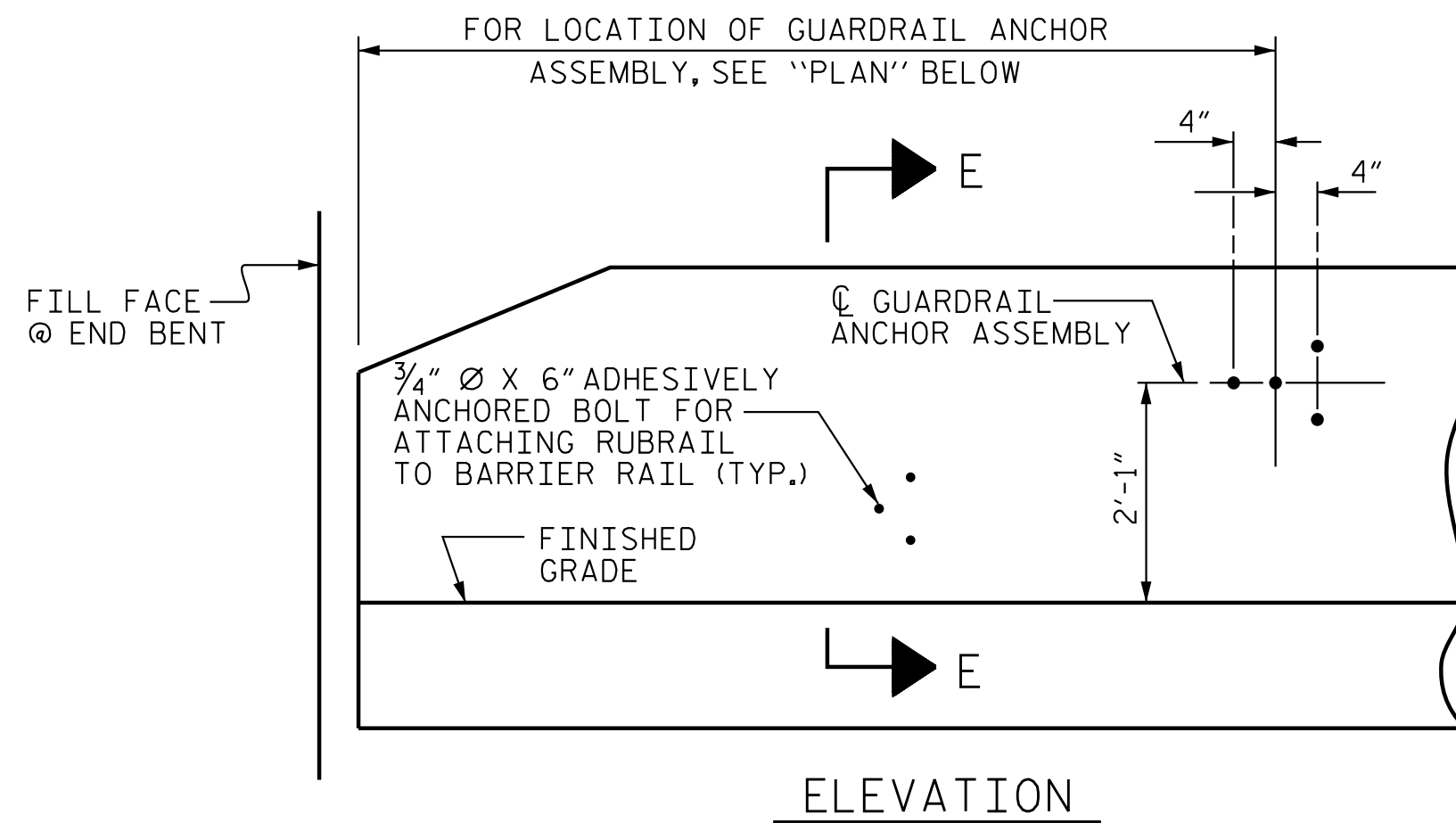
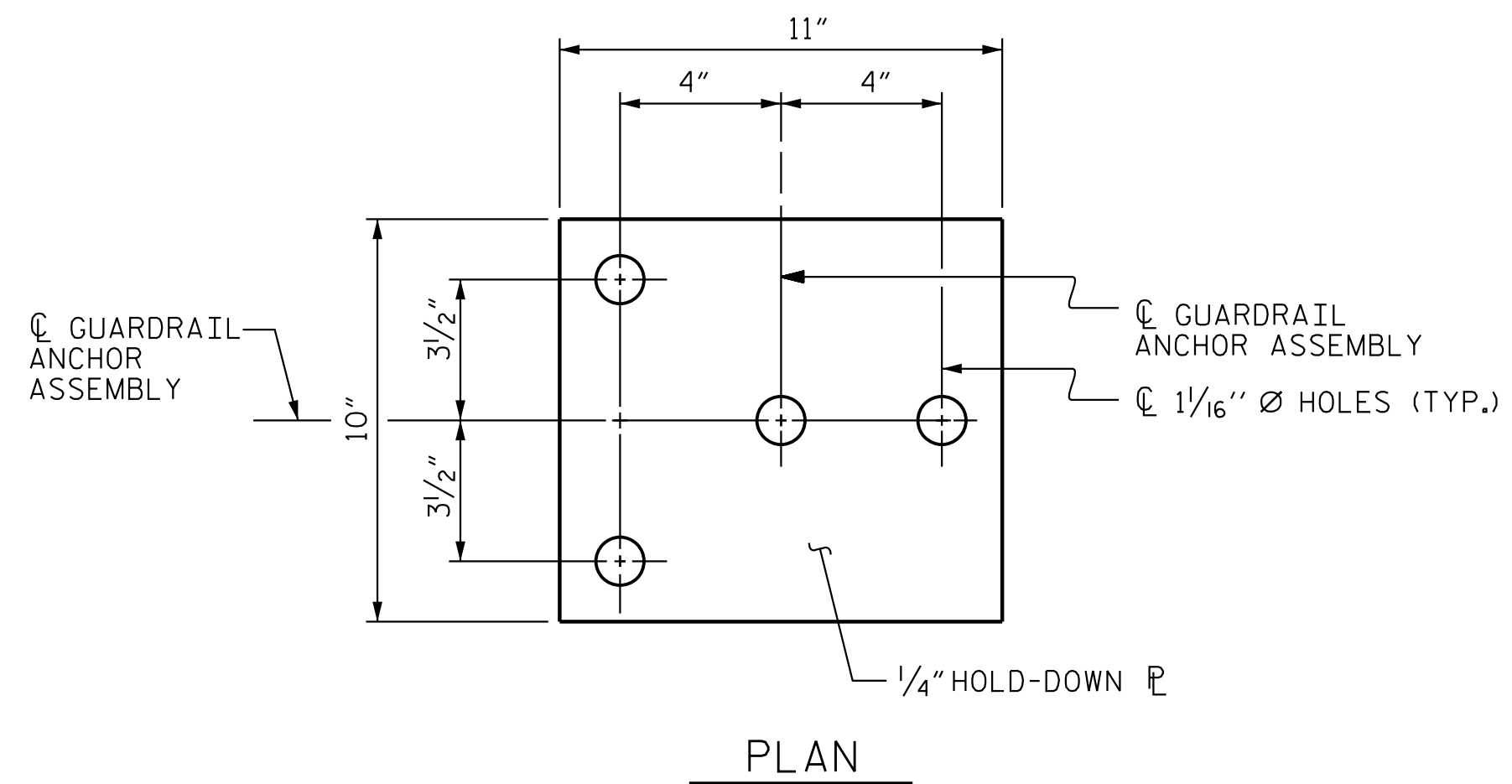
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1			3			TOTAL SHEETS	38
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jgelle  
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LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

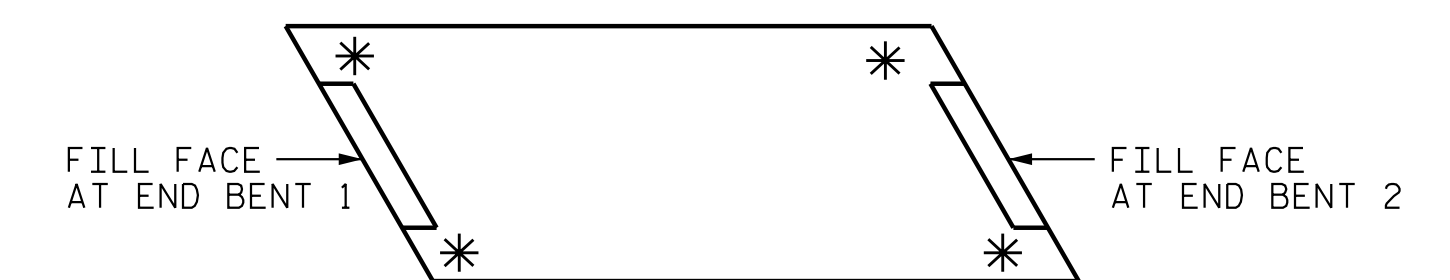
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

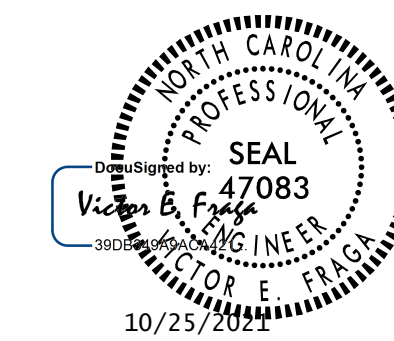
THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
STANDARD  
GUARDRAIL ANCHORAGE  
FOR BARRIER RAIL



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REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S17-24
1			3			TOTAL SHEETS
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STD. NO. GRA2

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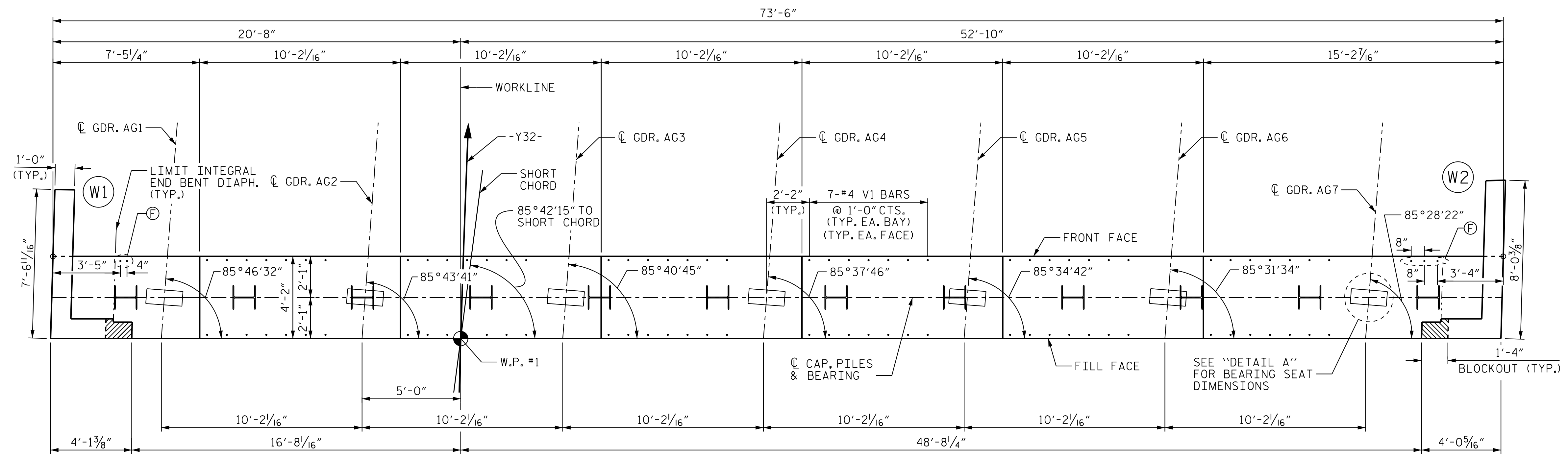
ASSEMBLED BY : J. B. GEILE DATE : 09/06/17  
CHECKED BY : N. D'AIUTO DATE : 05/14/19

DRAWN BY : TLA 5/06 MAA/GM  
CHECKED BY : GM 5/06 REV. 6/13 MAA/GM  
REV. 12/17 MAA/THC

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE : 10/25/21

10/25/2021 Jgeilie U:\Structures\BIB - Y32 - \Dr-off\ing\Final\R3300B\_SMU\_GR\_700262.dgn

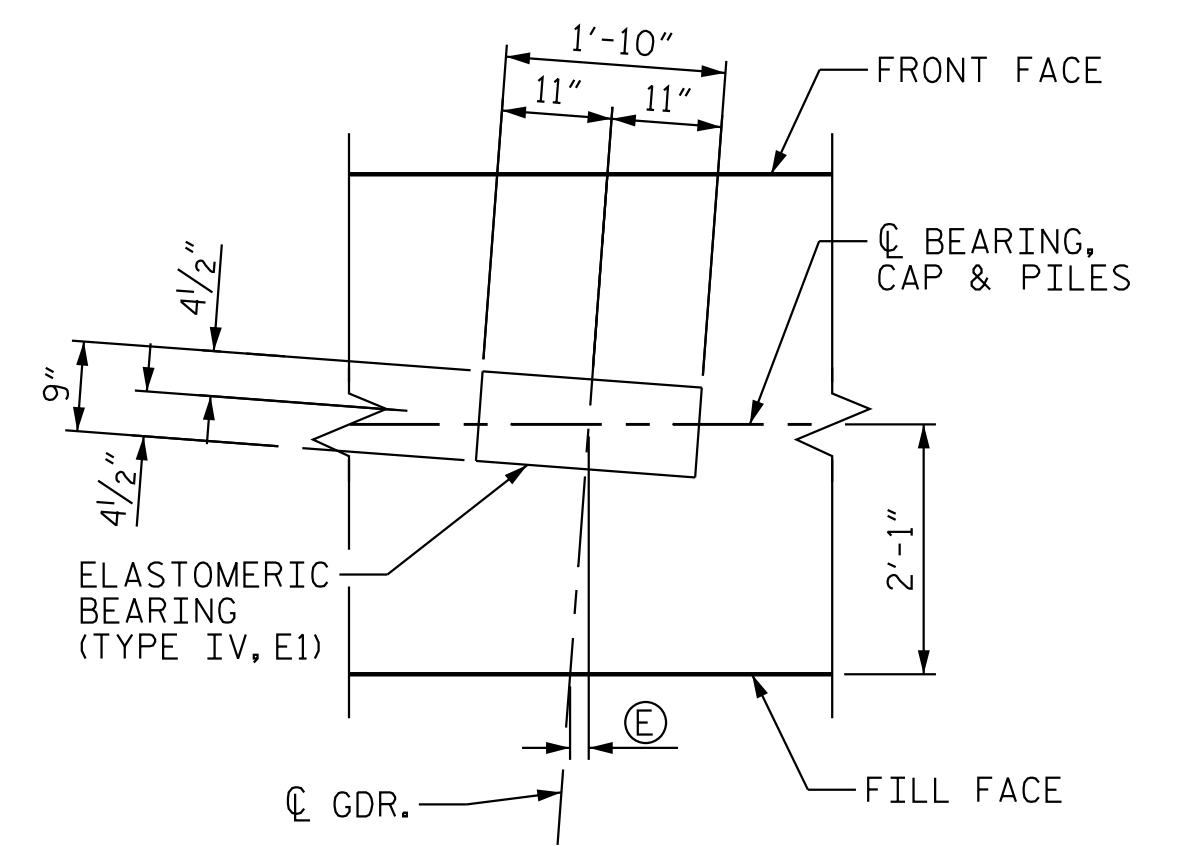




Ⓢ #4 V2 BARS PLACED AS SHOWN  
4'-9" MIN. PROJ. ABOVE BRIDGE SEAT  
NOT SHOWN IN ELEV. VIEW FOR CLARITY

**PLAN**

FOR WING WALL DETAILS, SEE "END BENT 1,  
DETAILS - WING WALLS" SHT. 2 OF 3



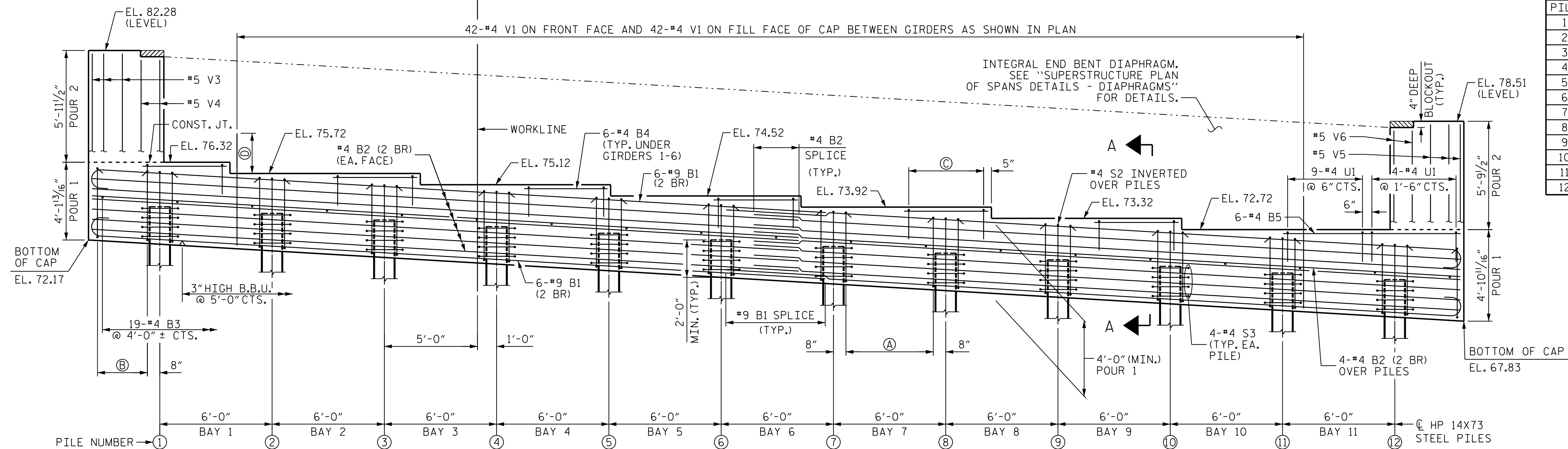
**DETAIL A**

GIRDER	Ⓢ
AG1	1 7/8"
AG2	1 7/8"
AG3	1 7/8"
AG4	1 5/16"
AG5	1 5/16"
AG6	1 5/16"
AG7	2"

**NOTES**

- STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- FOR WING DETAILS, SEE SHEET 2 OF 3.
- FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.
- (2 BR) DENOTES 2 BAR RUN.
- TIE #4 V1 AT GIRDERS TO #4 "K" BARS, SEE "TYPICAL SECTION DETAILS" SHEET 3 OF 3.
- TOP OF CAP AREAS OUTSIDE OF SUPERSTRUCTURE SHALL BE SLOPED AT A RATE OF 1/4" PER FOOT UPWARDS FROM FRONT FACE PERPENDICULAR TO FILL FACE.

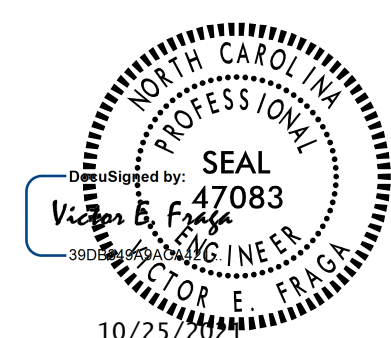
PILE	ELEVATION
1	73.97
2	73.62
3	73.27
4	72.91
5	72.56
6	72.21
7	71.85
8	71.50
9	71.14
10	70.79
11	70.44
12	70.08



**ELEVATION**

- Ⓐ 8-#4 S1 & 8-#4 S2 @ 8" CTS. (TYP. EA. BAY)
- Ⓑ 5-#4 S1 & 5-#4 S2 @ 8" CTS. (TYP. EA. OVERHANG)
- Ⓒ 9-#4 U1 @ 6" CTS. (TYP. UNDER GIRDERS 1-6)
- Ⓓ 1'-9" MIN. PROJECTION ABOVE BRIDGE SEAT

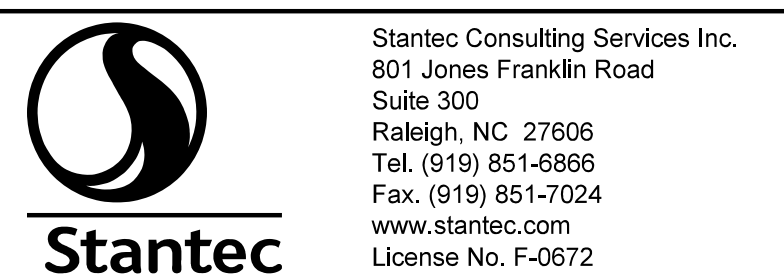
SPLICE LENGTH	
#9 B1	5'-4"
#4 B2	2'-5"



PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

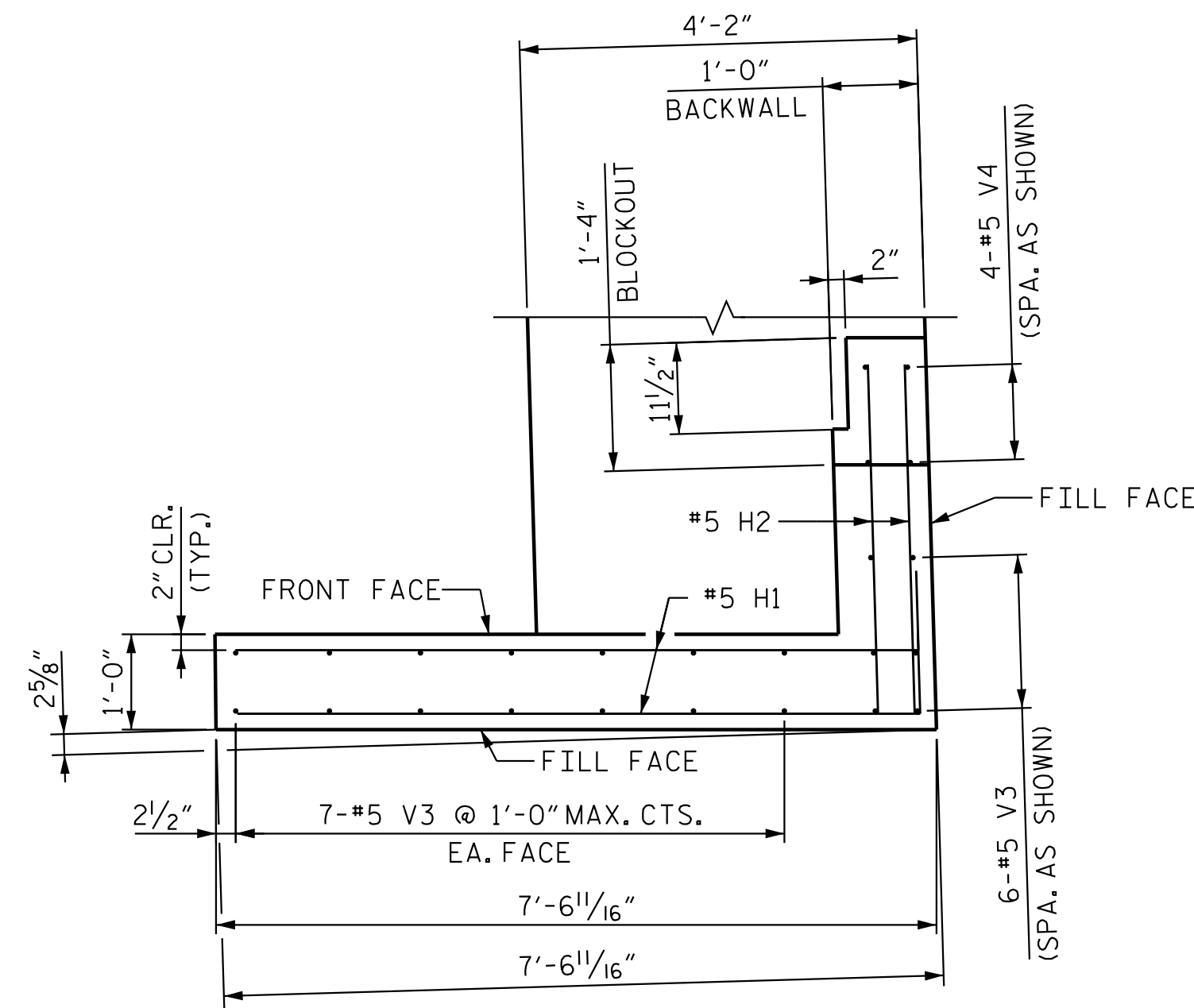
SHEET 1 OF 3

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1			3			TOTAL SHEETS 38
2			4			



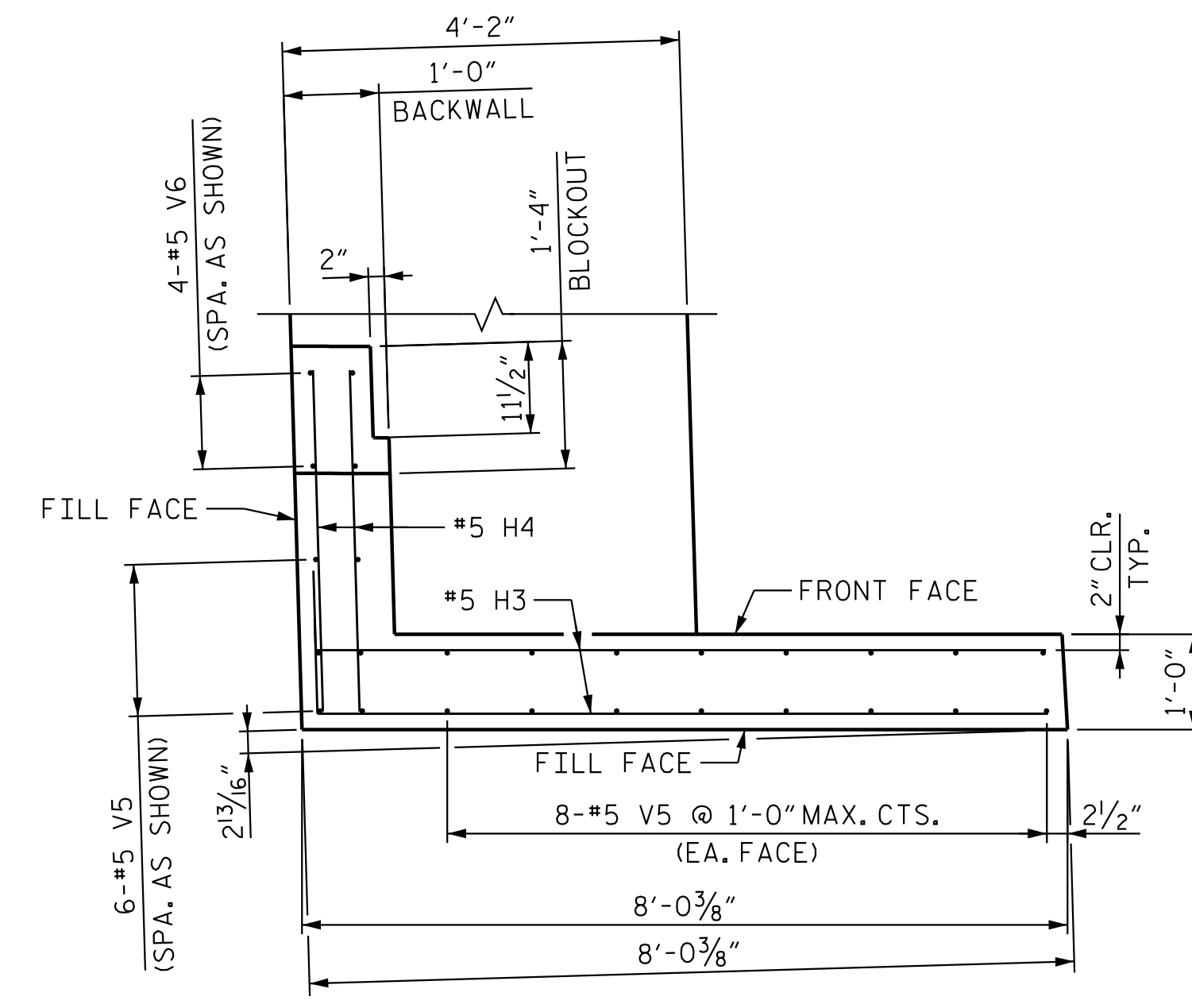
DRAWN BY: T. N. ENNIS DATE: 01/31/20  
 CHECKED BY: V. E. FRAGA DATE: 02/11/20  
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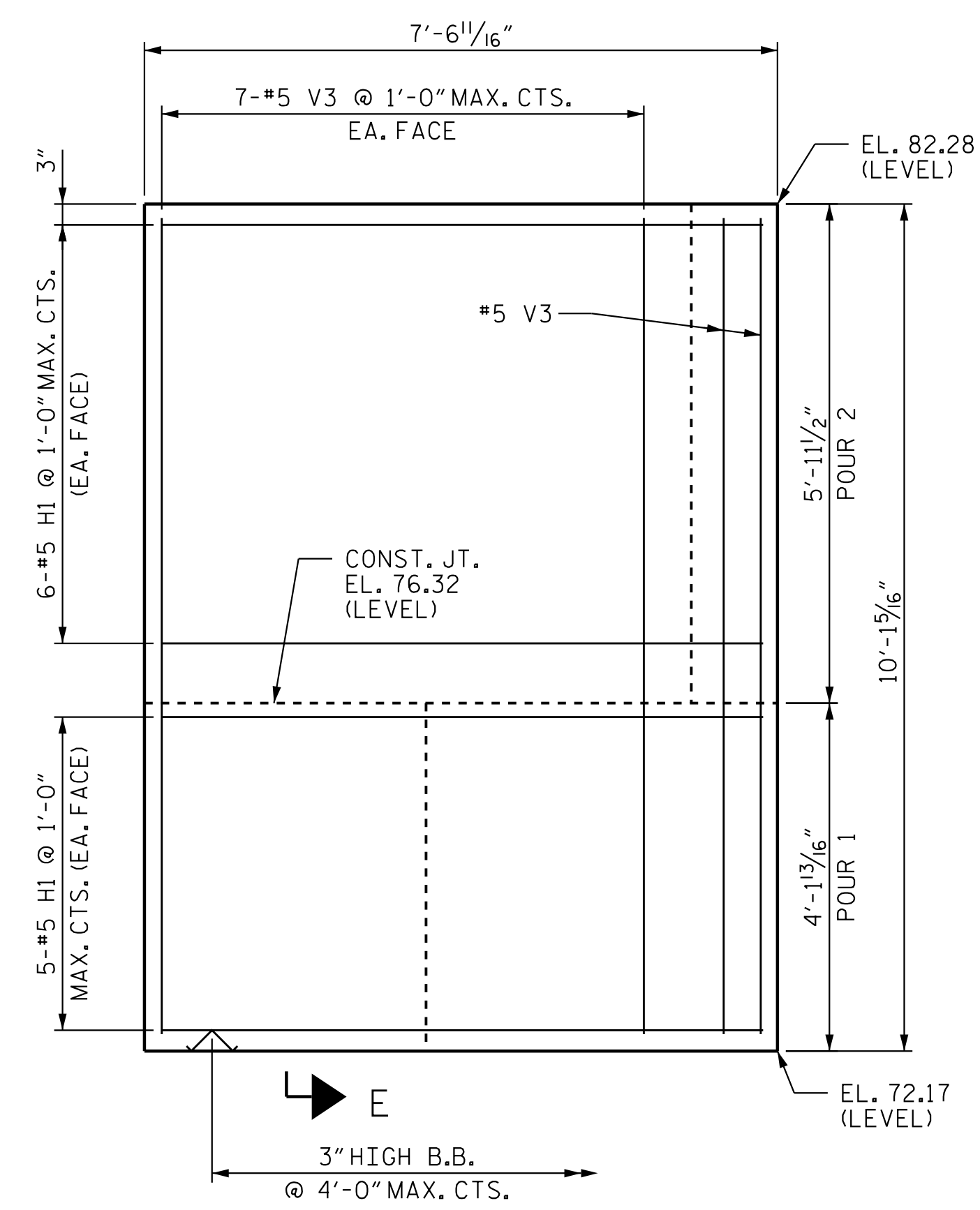
PLAN OF LEFT WING (W1)

➔ E



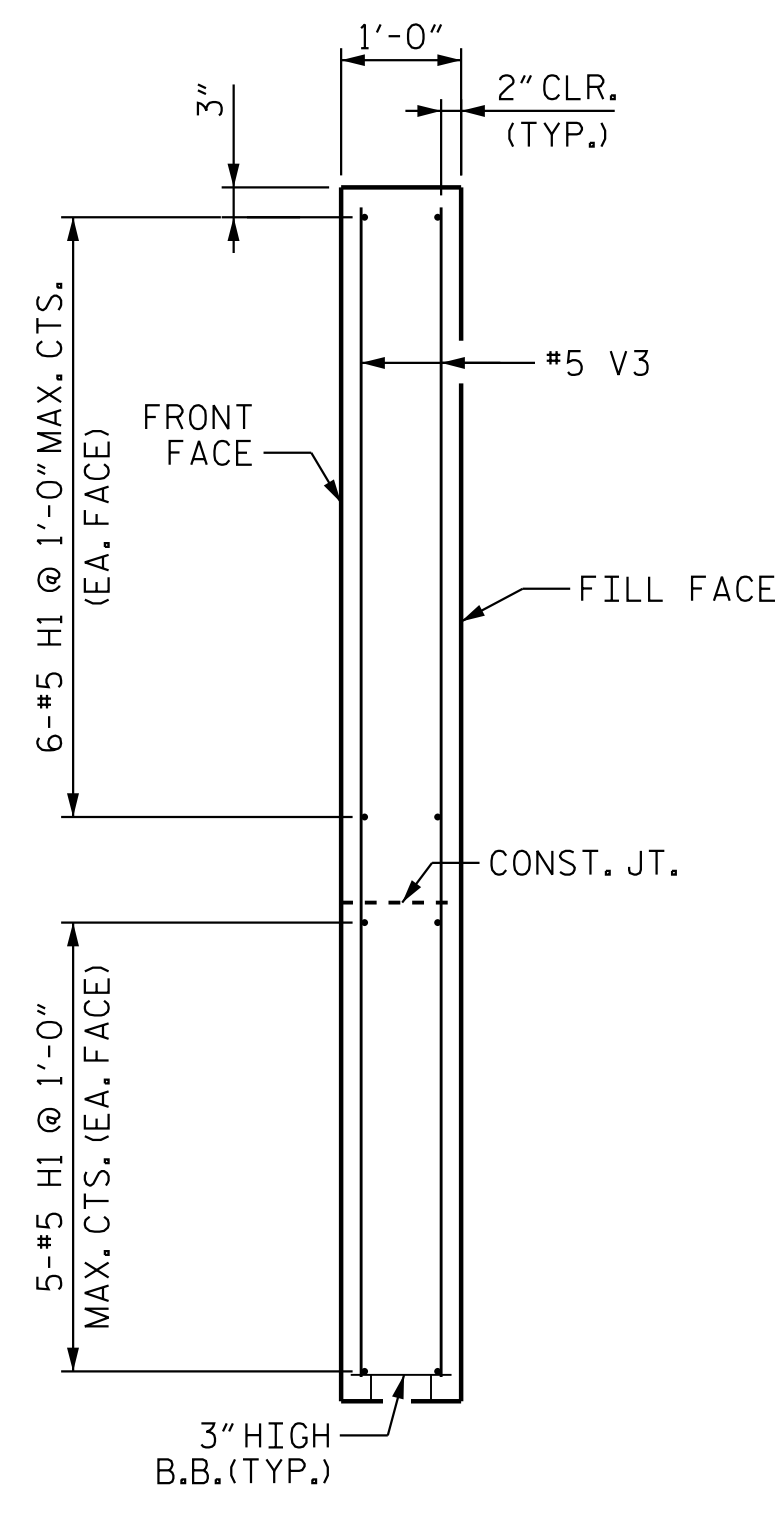
PLAN OF RIGHT WING (W2)

➔ F

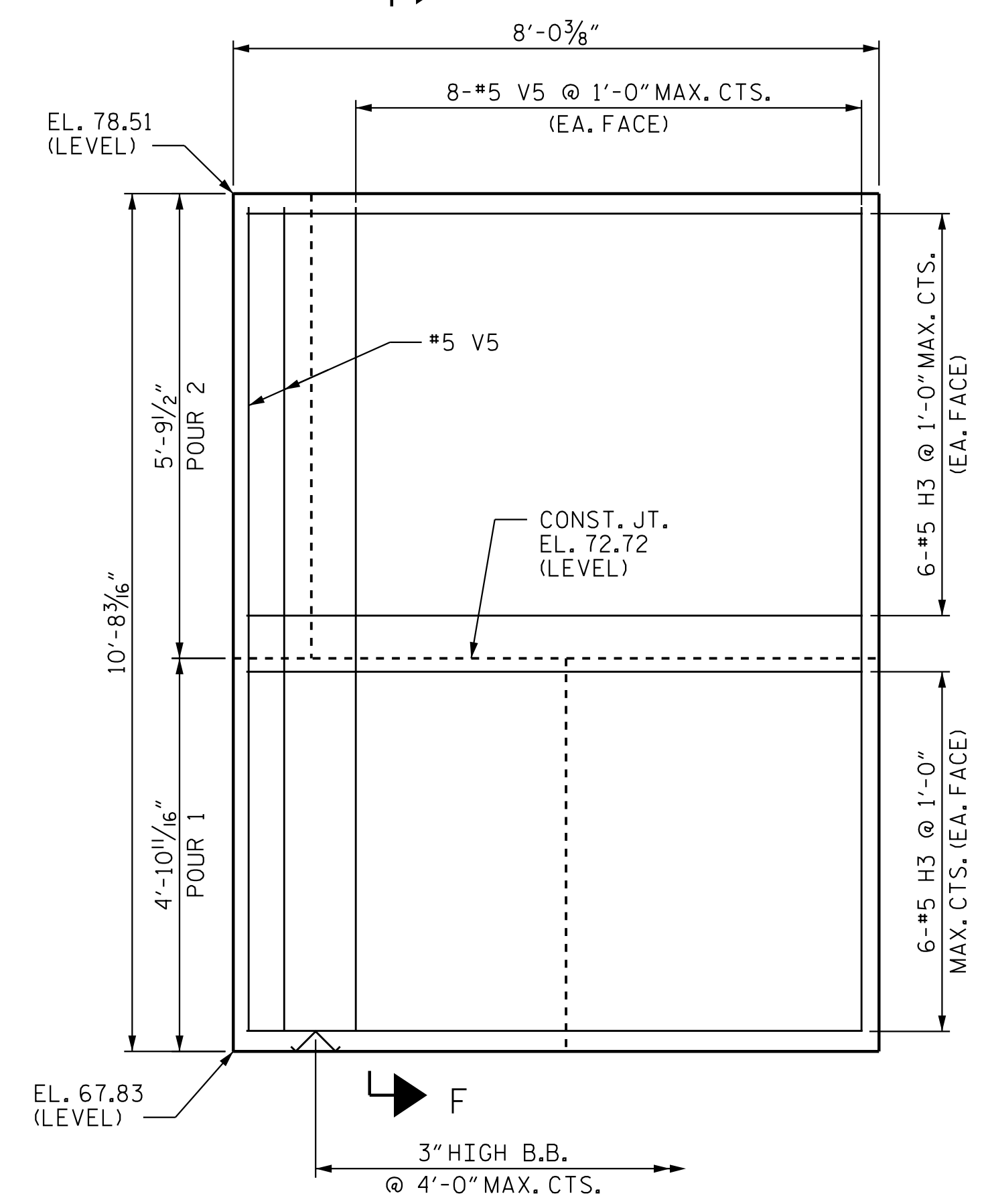


ELEVATION OF LEFT WING (W1)

➔ E

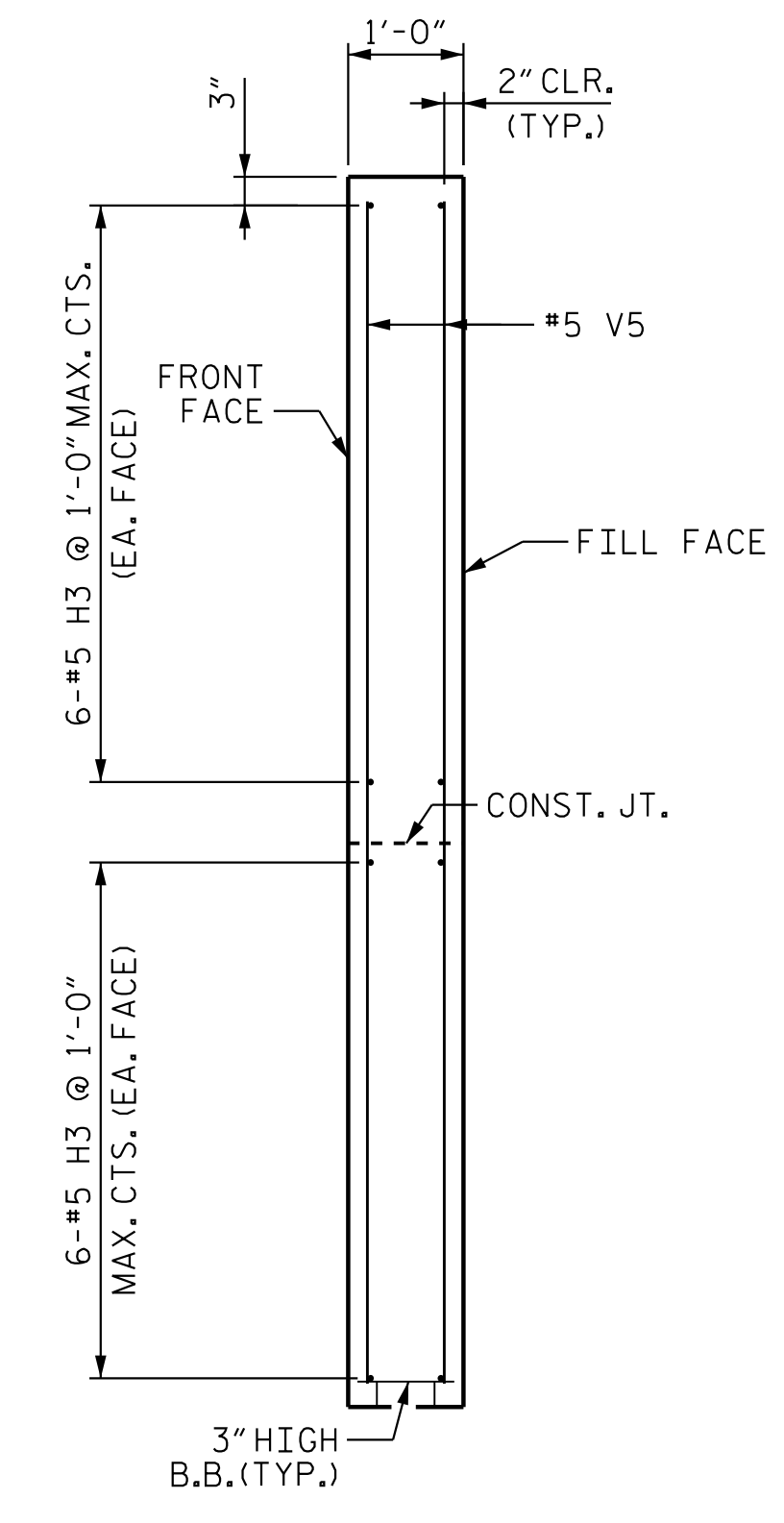


SECTION E-E



ELEVATION OF RIGHT WING (W2)

➔ F

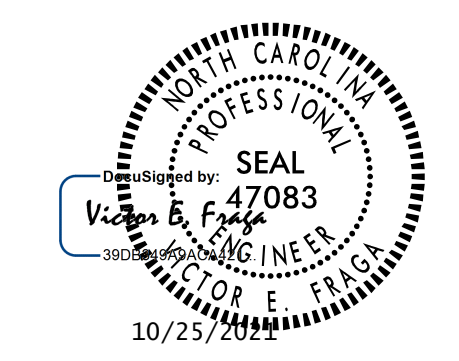


SECTION F-F

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE  
 END BENT 1  
 DETAILS-  
 WING WALLS



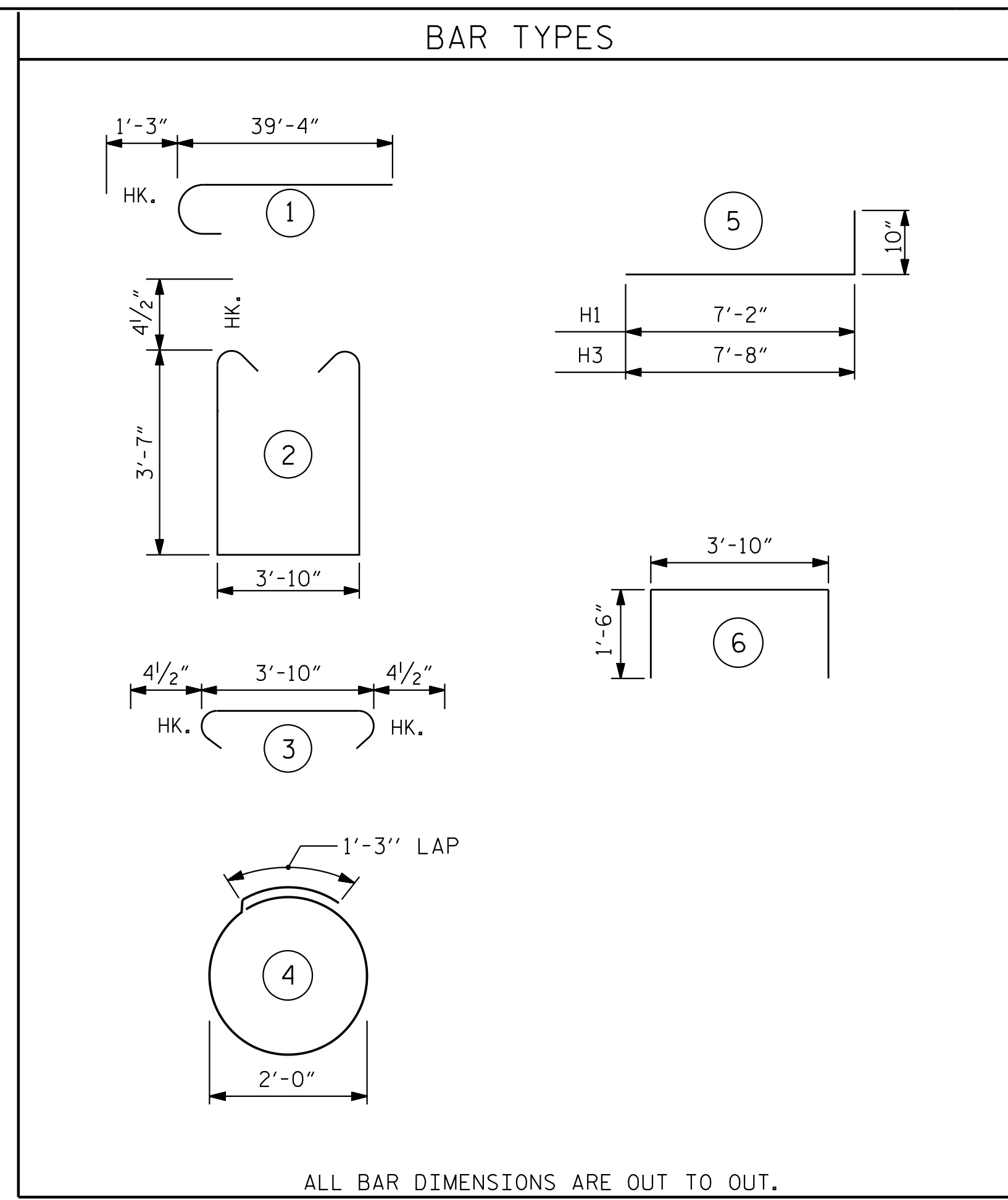
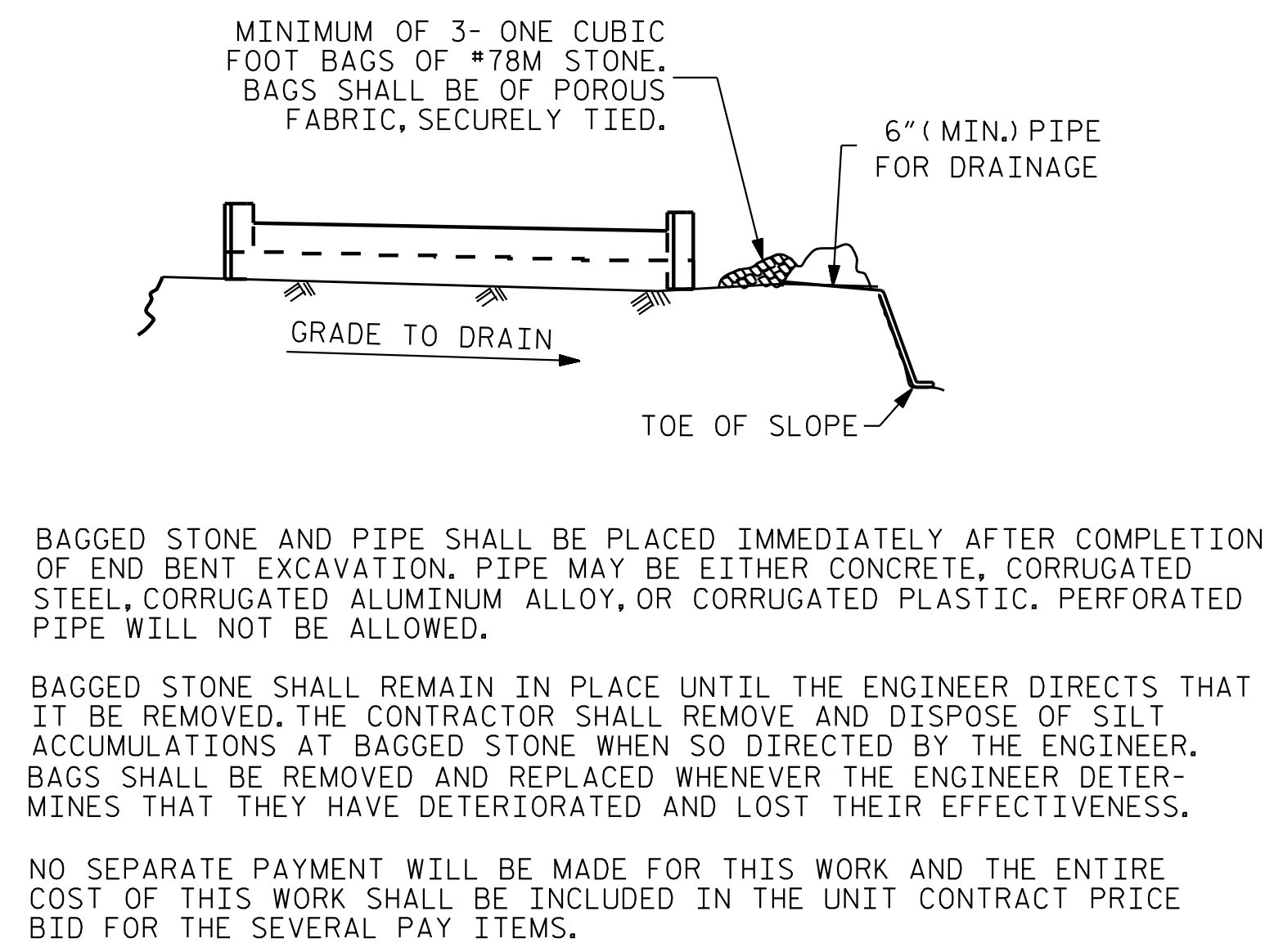
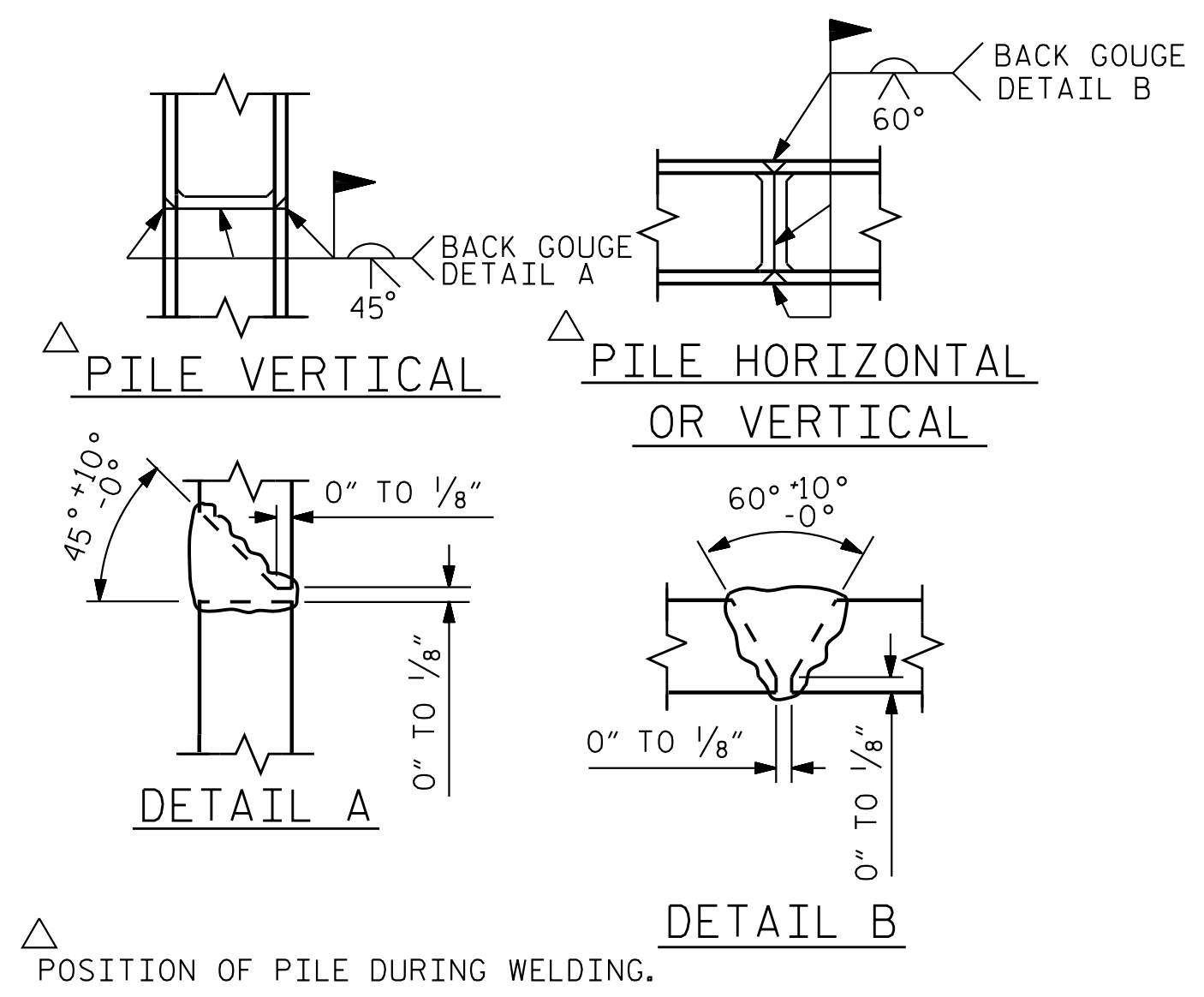
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 jgelle



**BILL OF MATERIAL**

**END BENT 1**

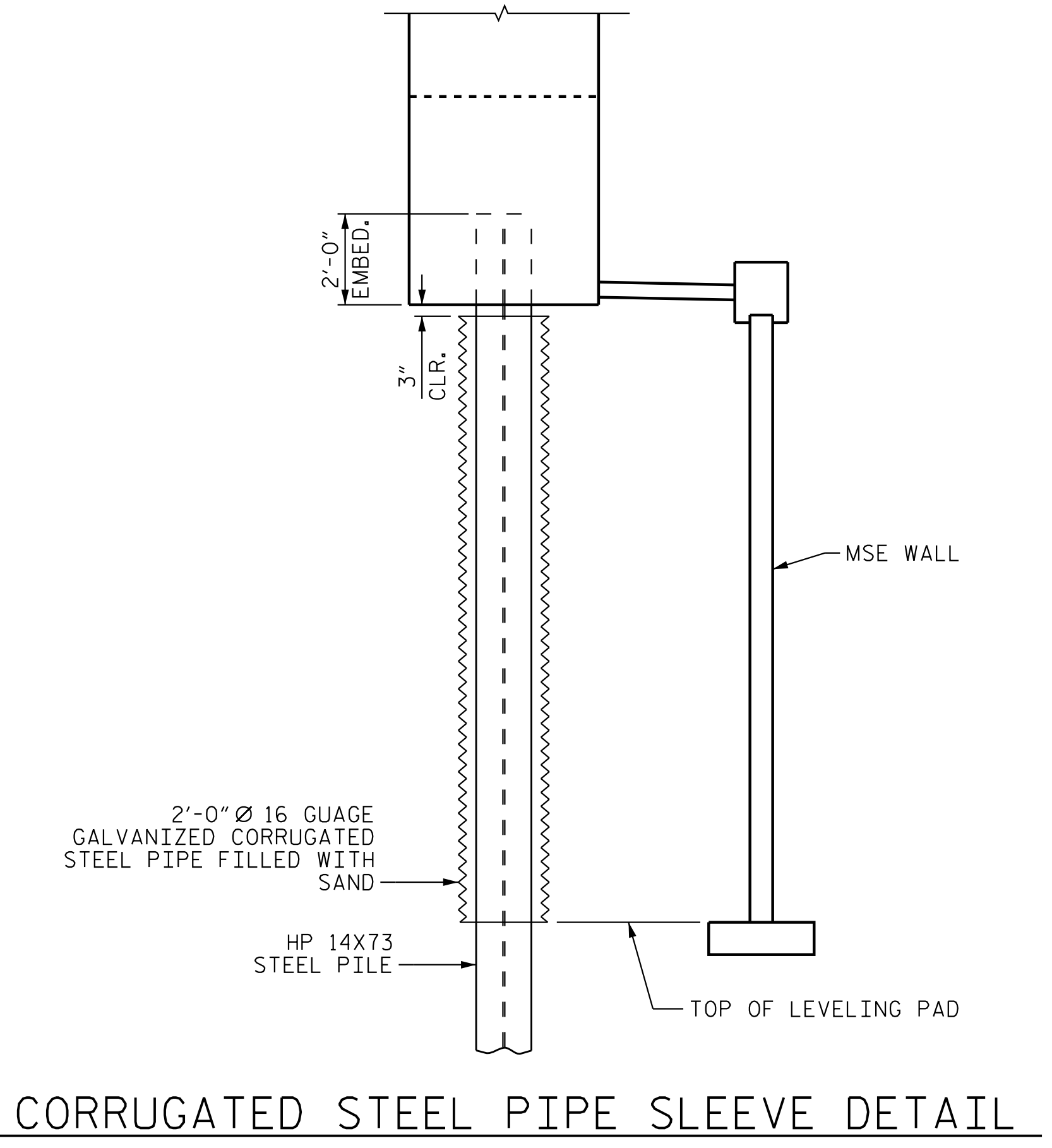
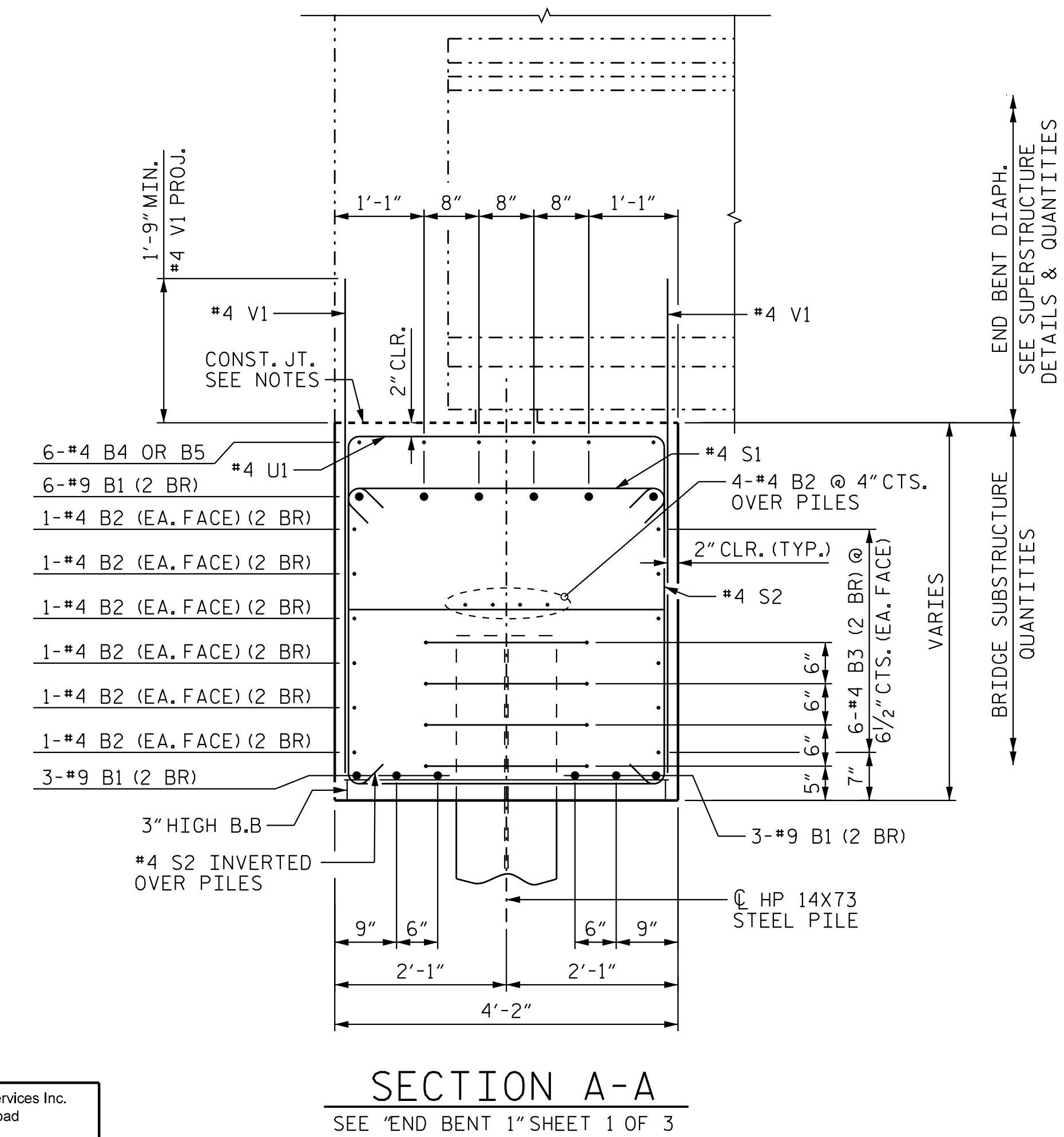
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	24	#9	1	40'-7"	3312
B2	32	#4	STR	37'-10"	809
B3	19	#4	STR	3'-10"	49
B4	36	#4	STR	4'-6"	108
B5	6	#4	STR	9'-6"	38
H1	22	#5	5	8'-0"	184
H2	22	#5	STR	3'-8"	84
H3	24	#5	5	8'-6"	213
H4	24	#5	STR	3'-7"	90
S1	98	#4	3	4'-7"	300
S2	110	#4	2	11'-9"	863
S3	48	#4	4	7'-7"	243
U1	67	#4	7	6'-10"	306
V1	84	#4	STR	6'-0"	337
V2	5	#4	STR	9'-0"	30
V3	20	#4	STR	9'-9"	130
V4	4	#4	STR	9'-5"	25
V5	22	#4	STR	10'-3"	151
V6	4	#4	STR	9'-11"	26
REINFORCING STEEL					LBS. 7,298

CLASS A CONCRETE BREAKDOWN:

POUR 1: CAP, ETC. C.Y. 50.4  
POUR 2: UPPER WINGS C.Y. 4.6  
CLASS A CONCRETE TOTAL C.Y. 55.0

HP14X73 PILES NO. 12  
LIN. FT. 1,080

PILE DRIVING EQUIPMENT SETUP FOR  
HP 14X73 STEEL PILES EA. 12  
PILE REDRIVES EA. 6  
STEEL PILE POINTS EA. 12



**NOTES**

TOP SURFACE AREAS OF THE END BENT CAP SHALL BE KEPT CLEAN AND FREE OF LAITANCE.

THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCLUDING THE BEARING AREA & TOP OF CAP OUTSIDE THE LIMITS OF THE SUPERSTRUCTURE, SHALL BE RAKED TO A DEPTH OF 1/4".

2 BR DENOTES 2 BAR RUN.

PROJECT NO. R-3300B  
PENDER COUNTY  
STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUBSTRUCTURE  
END BENT 1  
DETAILS

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S17-28  
TOTAL SHEETS 38

Seal of a Professional Engineer, State of North Carolina, License No. 47083, signed by V. E. FRAGA, dated 10/25/2021.

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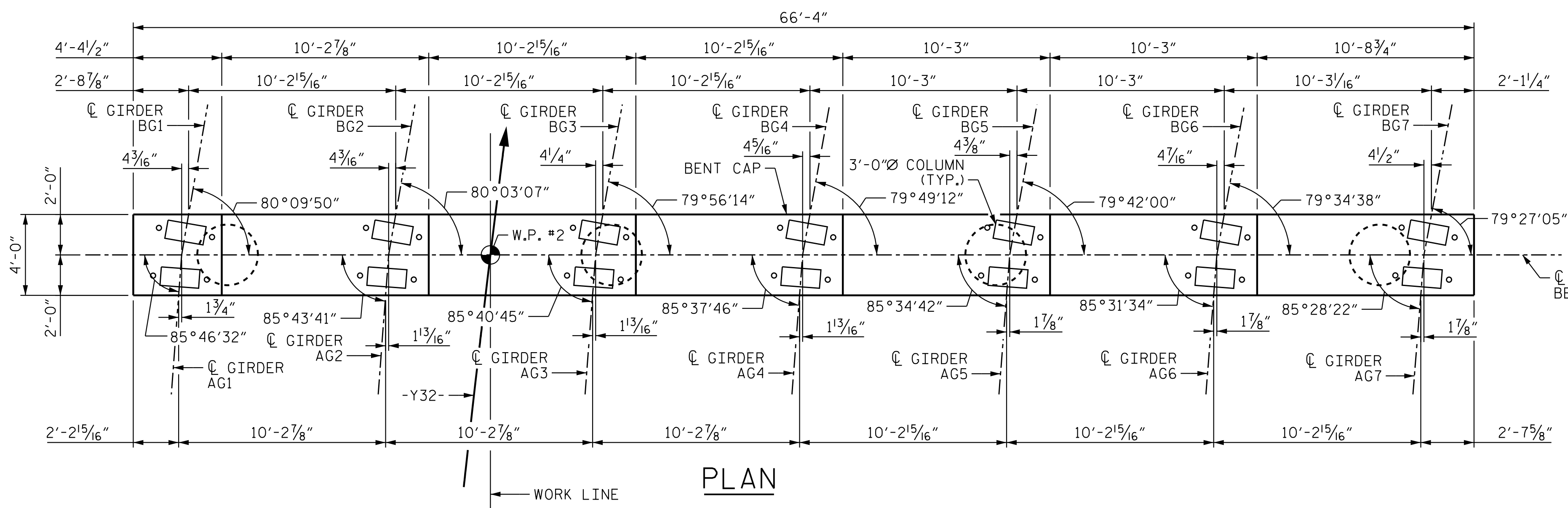
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10/25/2021 10/25/2021 10/25/2021

**NOTES**

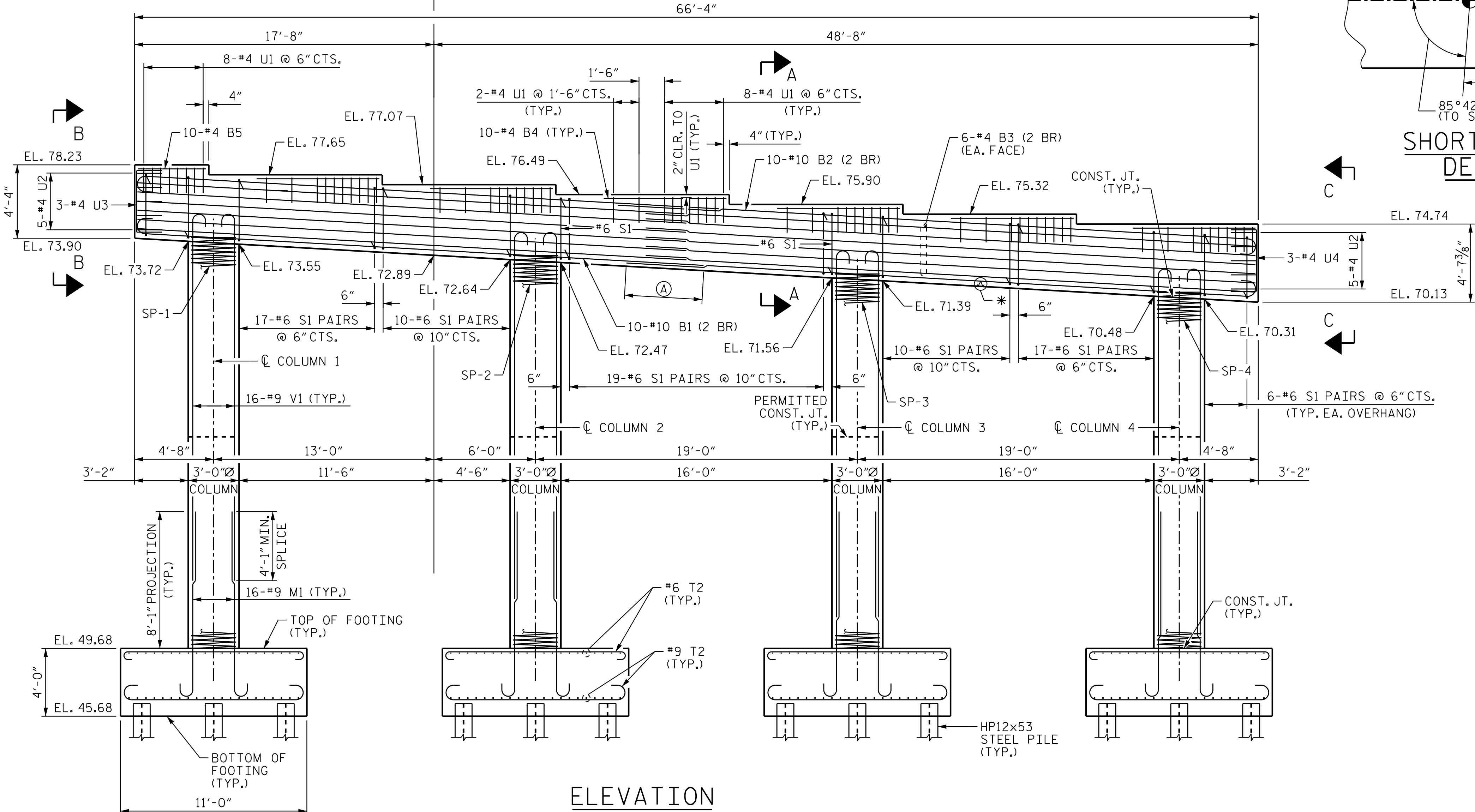
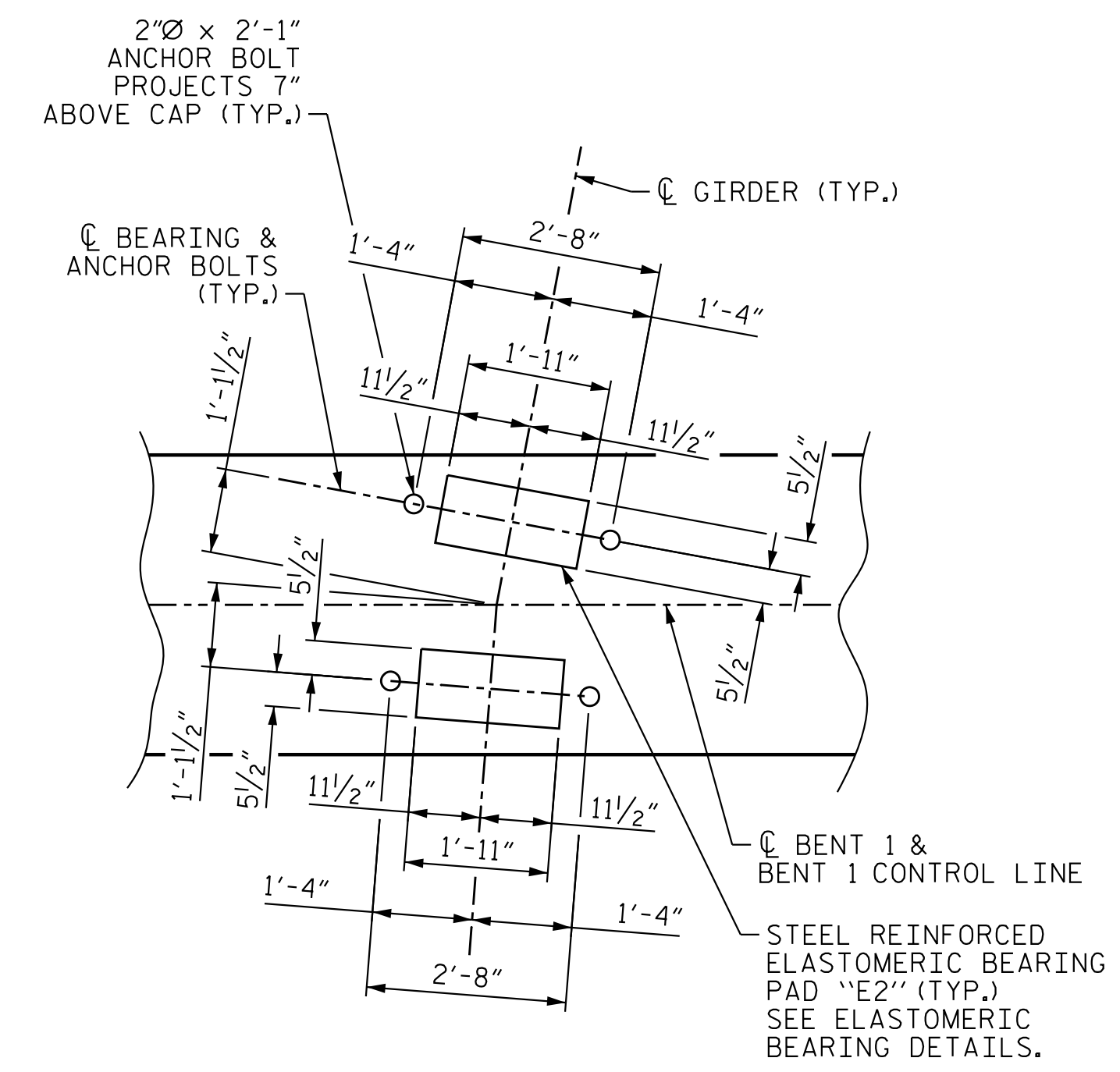
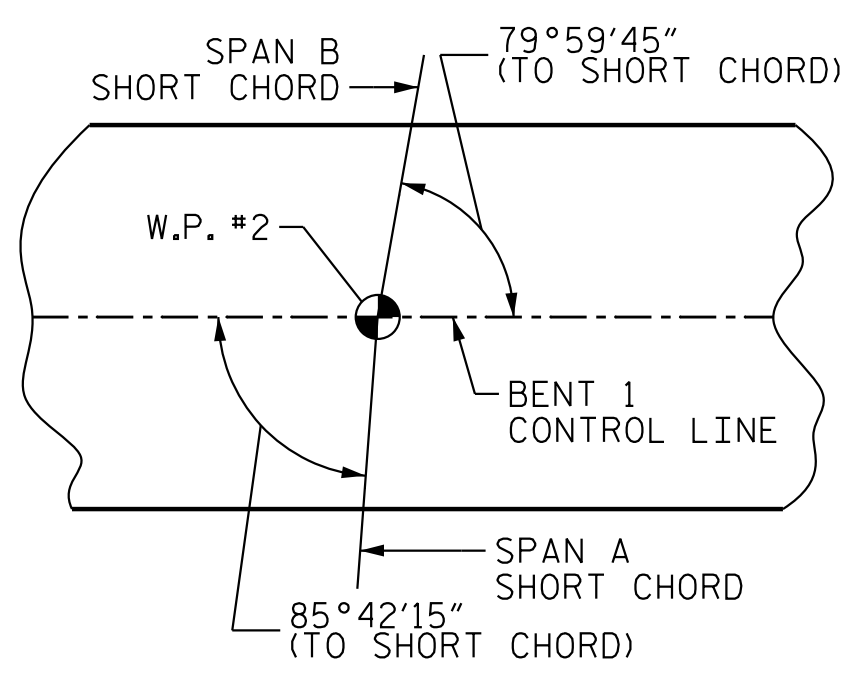
- \* 3" HIGH B.B. @ 5'-0" ± CTS.
- 2 BR DENOTES 2 BAR RUN.
- SEE BENT 1 FOUNDATION DETAILS, SHEET 2 OF 3.
- SEE BENT 1 DETAILS, SHEET 3 OF 3.
- INVERT EVERY OTHER STIRRUP PAIR.
- Ⓐ SEE TABLE FOR "B" BAR SPLICES.

BAR MARK	MIN. SPLICE
#10 B1	4'-7"
#10 B2	6'-0"
#4 B3	2'-5"



SPAN B

SPAN A



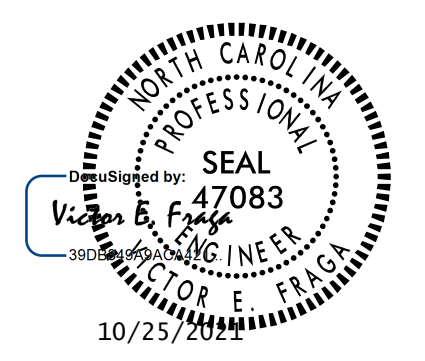
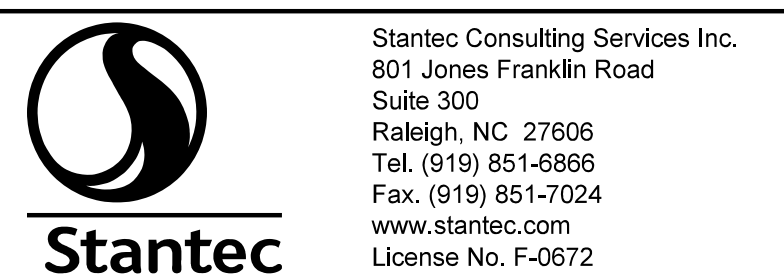
PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-25+28.04 -Y32-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE

**BENT 1**

REVISIONS						SHEET NO.	
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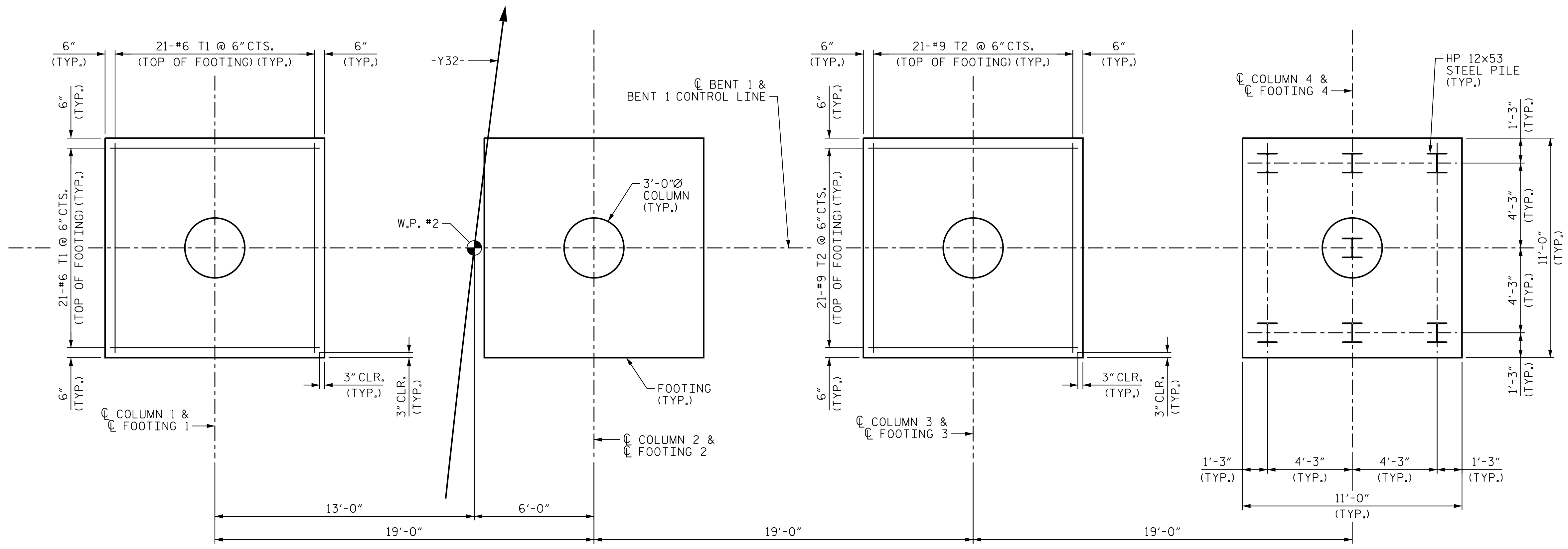


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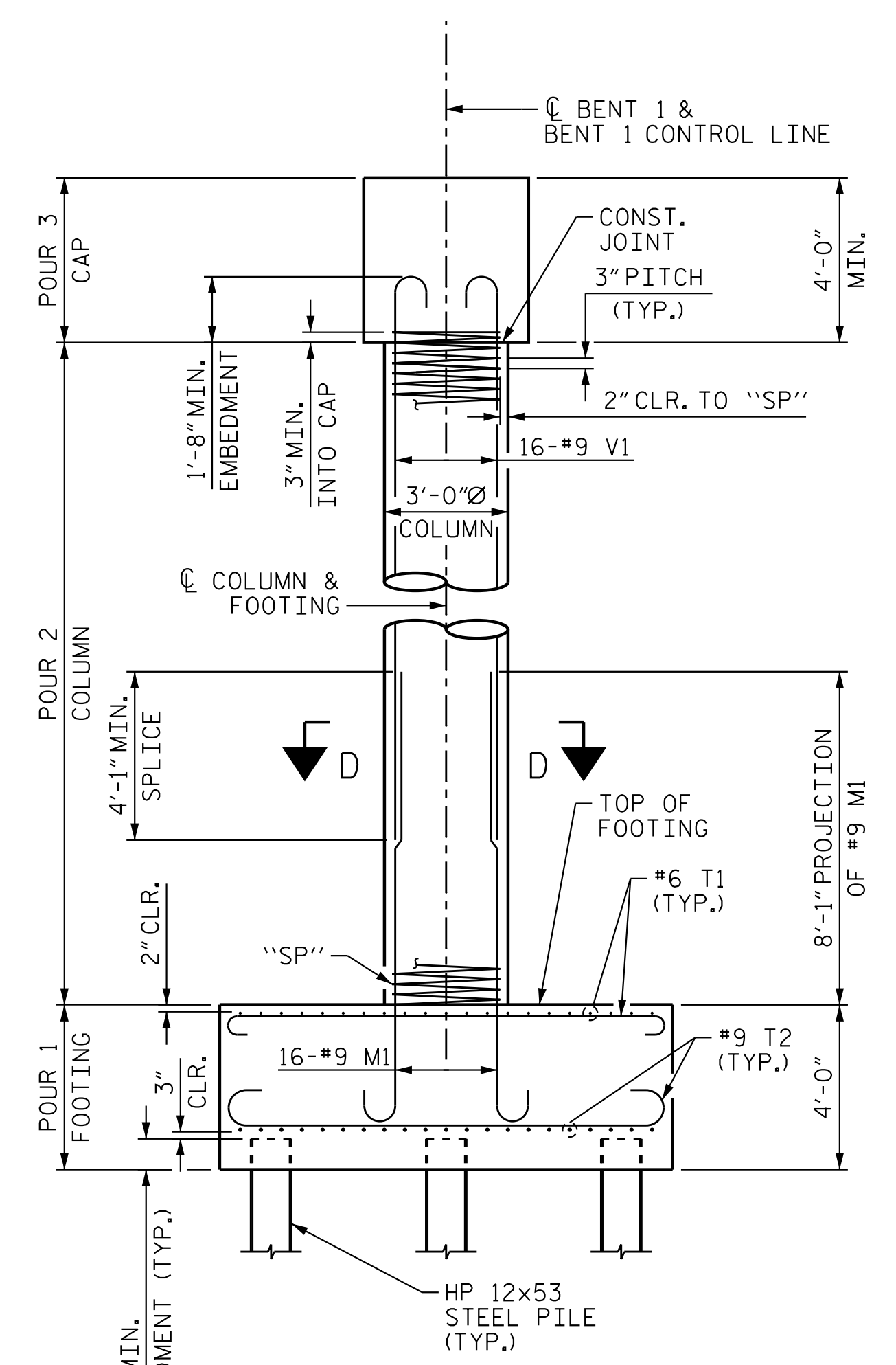
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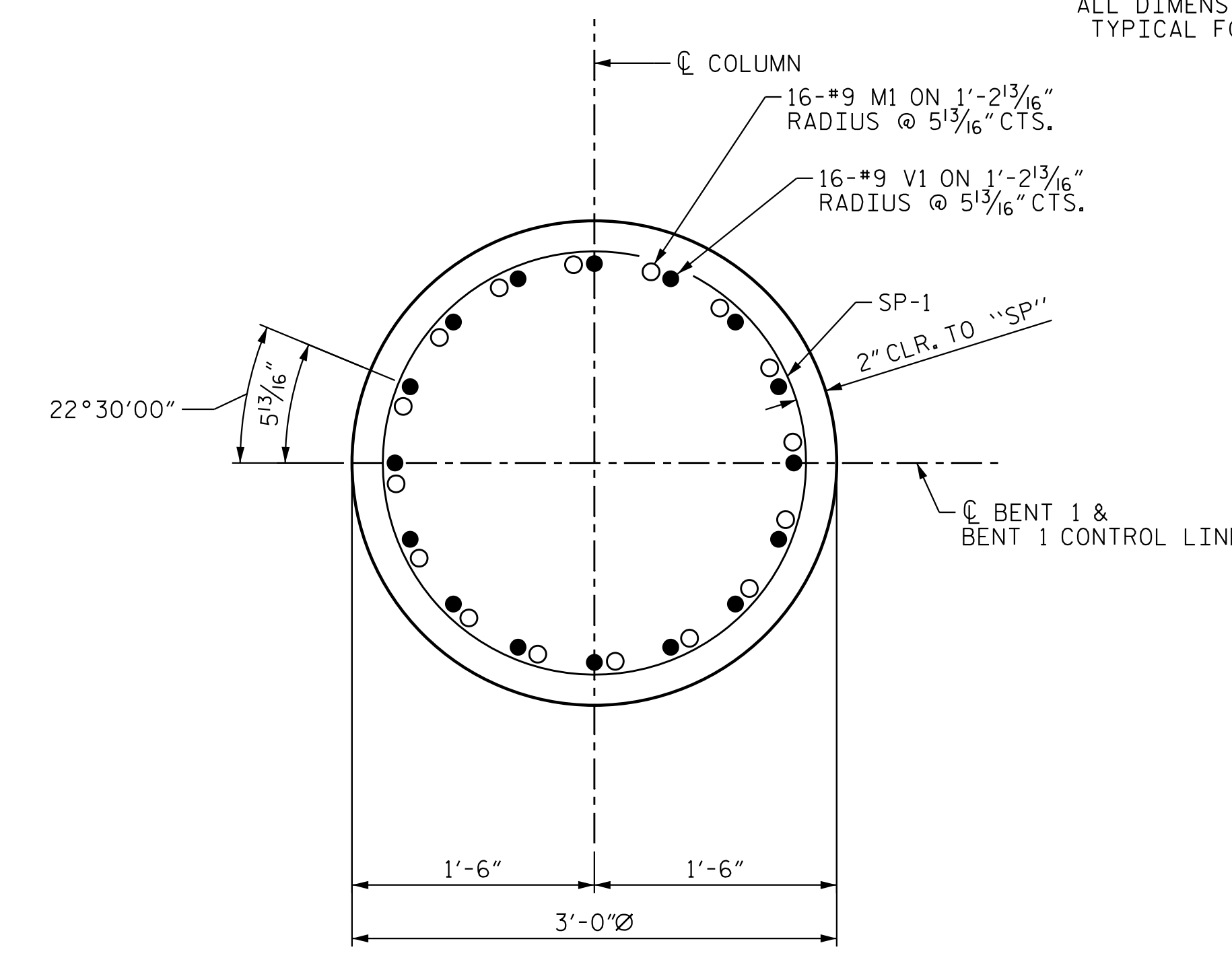


### PLAN OF FOOTINGS

ALL DIMENSIONS AND DETAILS SHOWN FOR FOOTINGS ARE TYPICAL FOR EACH FOOTING UNLESS NOTED OTHERWISE.



### END ELEVATION



### SECTION D-D

COLUMN	CL HEIGHT	LT. HEIGHT	RT. HEIGHT
1	23'-11 7/16"	24'-0 7/16"	23'-10 3/8"
2	22'-10 7/16"	22'-11 1/2"	22'-9 7/16"
3	21'-9 1/2"	21'-10 1/2"	21'-8 7/16"
4	20'-8 1/2"	20'-9 9/16"	20'-7 1/2"

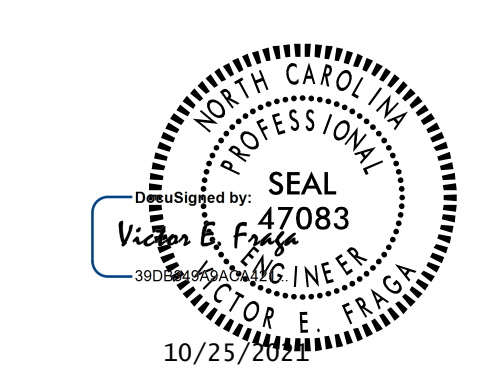
NOTE: LT. AND RT. HEIGHTS TAKEN FROM TOP LEFT CORNER ELEVATION OR TOP RIGHT CORNER ELEVATION TO TOP OF FOOTING ELEVATION FOR EACH COLUMN.

PROJECT NO. R-3300B  
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 STATION: 852+98.21 -L1-NORTHERN-  
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SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE

## BENT 1 FOUNDATION DETAILS



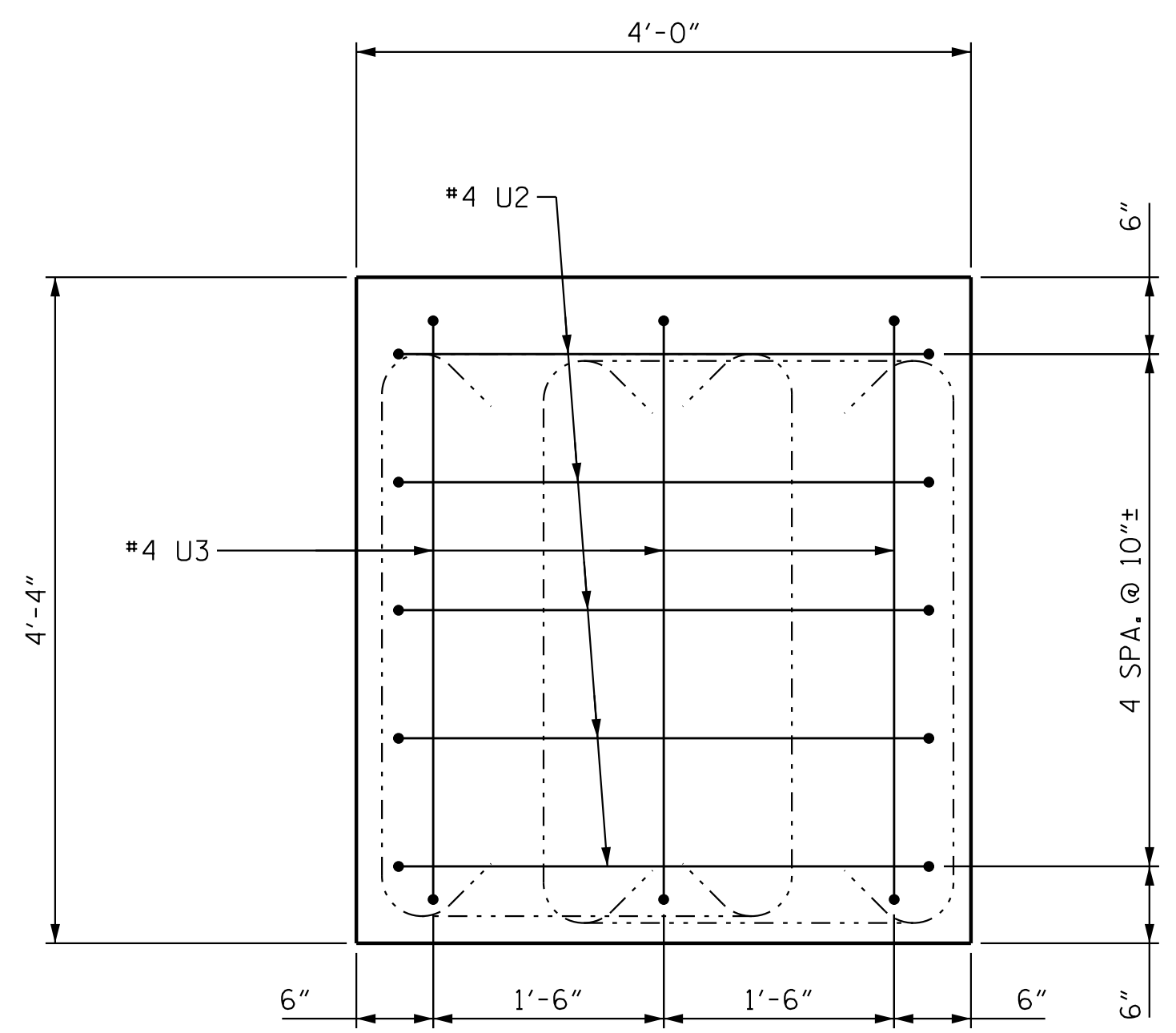
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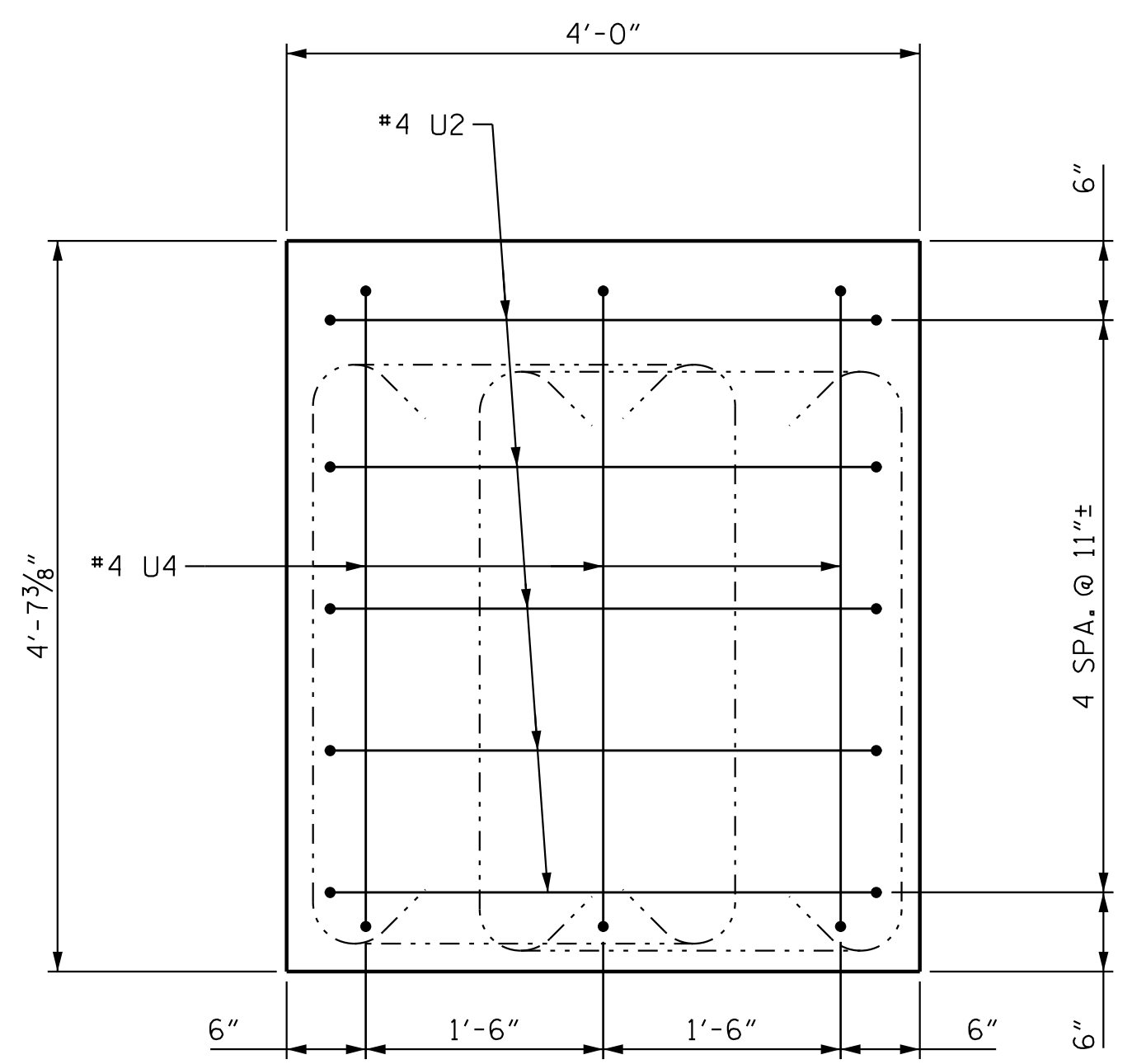
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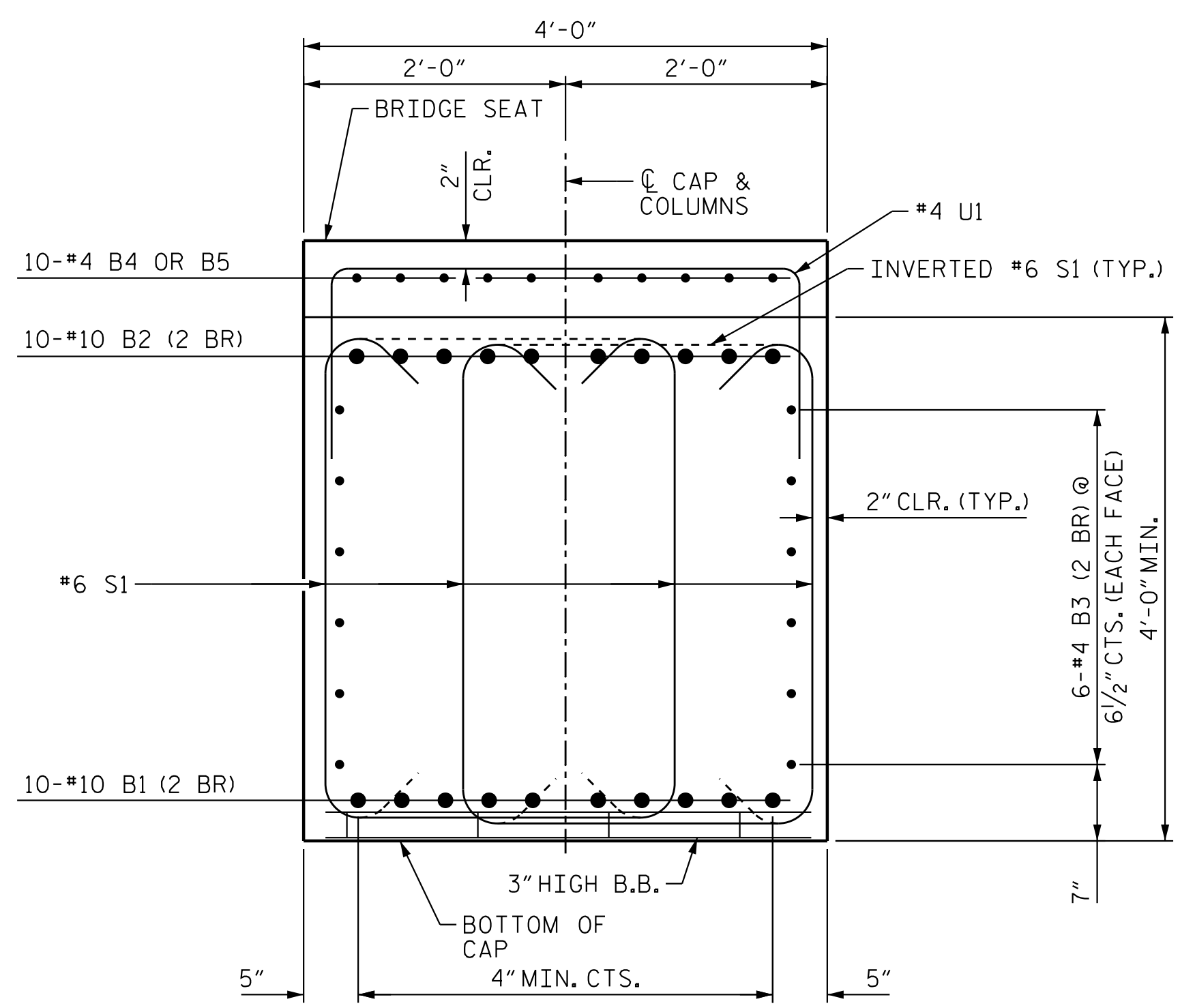
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SECTION B-B

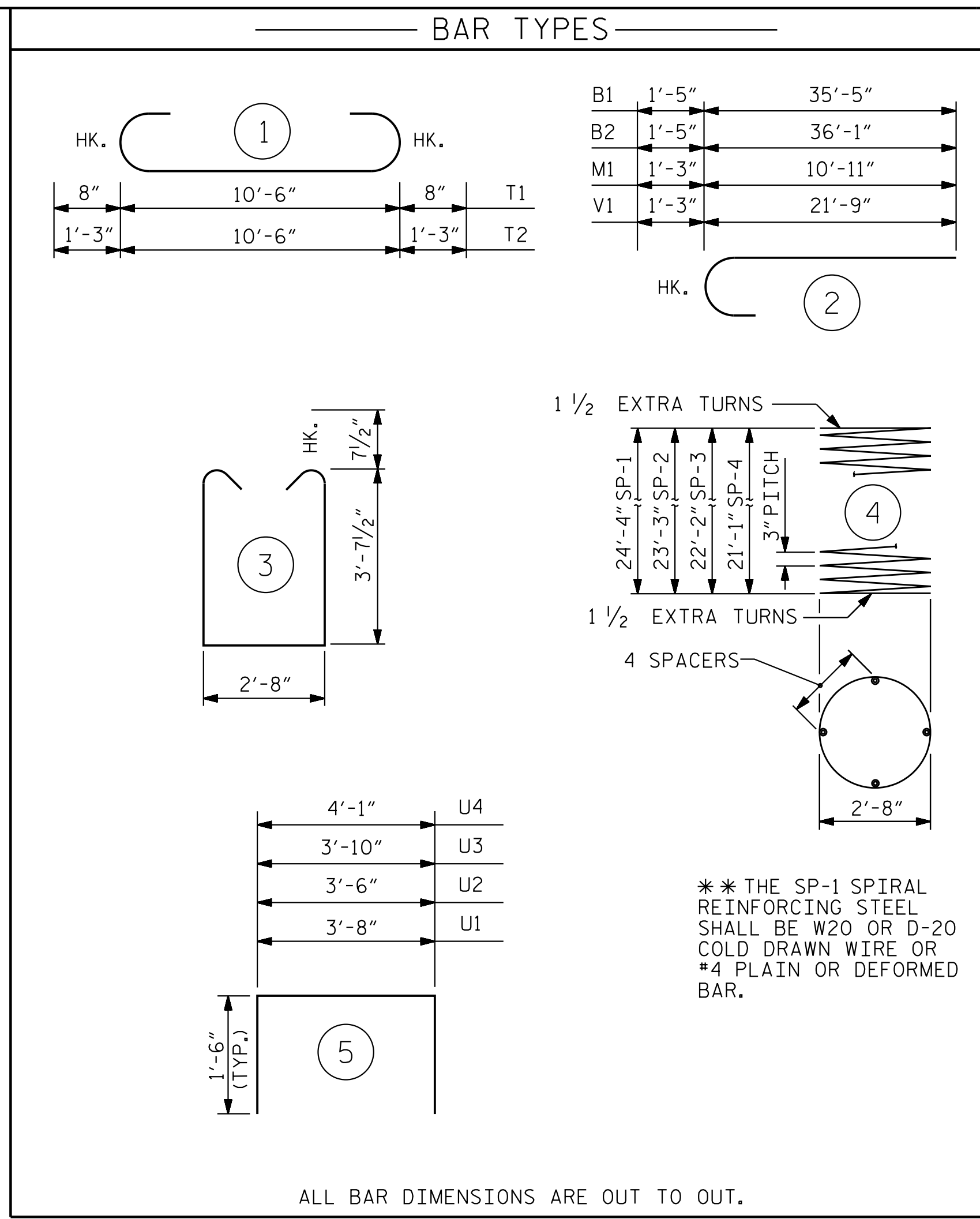


SECTION C-C



SECTION A-A

SHIFT "B" BARS AS NECESSARY TO CLEAR ANCHOR BOLTS

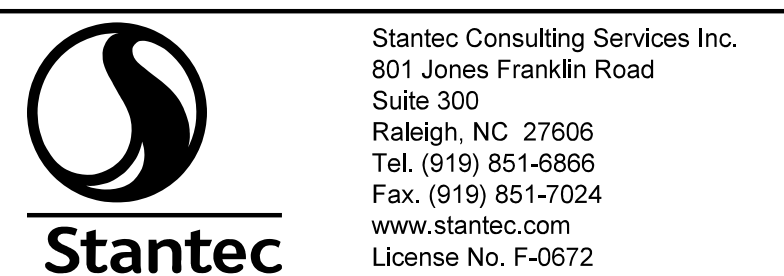


BILL OF MATERIAL					
BENT 1					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	20	#10	2	36'-10"	3,170
B2	20	#10	2	37'-6"	3,227
B3	24	#4	STR	34'-4"	550
B4	60	#4	STR	7'-3"	291
B5	10	#4	STR	4'-0"	27
M1	64	#9	2	12'-2"	2,647
S1	178	#6	3	11'-2"	2,985
T1	168	#6	1	11'-10"	2,986
T2	168	#9	1	13'-0"	7,426
U1	68	#4	5	6'-8"	303
U2	10	#4	5	6'-6"	43
U3	3	#4	5	6'-10"	14
U4	3	#4	5	7'-1"	14
V1	64	#9	2	23'-0"	5,005
REINFORCING STEEL				LBS.	28,688
SP-1	1	**	4	831'-8"	556
SP-2	1	**	4	790'-6"	528
SP-3	1	**	4	757'-7"	506
SP-4	1	**	4	724'-8"	484
SPIRAL COLUMN REINFORCING STEEL				LBS.	2,074
CLASS A CONCRETE BREAKDOWN					
POUR #1 FOOTINGS				C. Y.	71.8
POUR #2 COLUMNS				C. Y.	23.4
POUR #3 CAP				C. Y.	42.4
TOTAL CLASS A CONC.				C. Y.	137.6
HP 12X53 STEEL PILES				NO.	28
				LIN. FT.	2,660
PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES				EA.	28
PILE REDRIVES				EA.	14
STEEL PILE POINTS				EA.	28
FOUNDATION EXCAVATION				LUMP SUM	

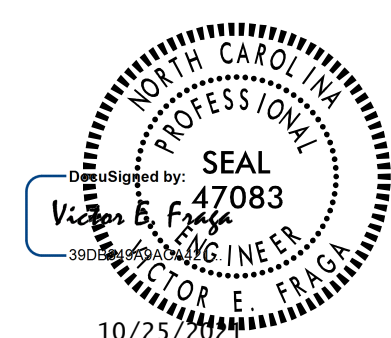
\*\* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

ALL BAR DIMENSIONS ARE OUT TO OUT.

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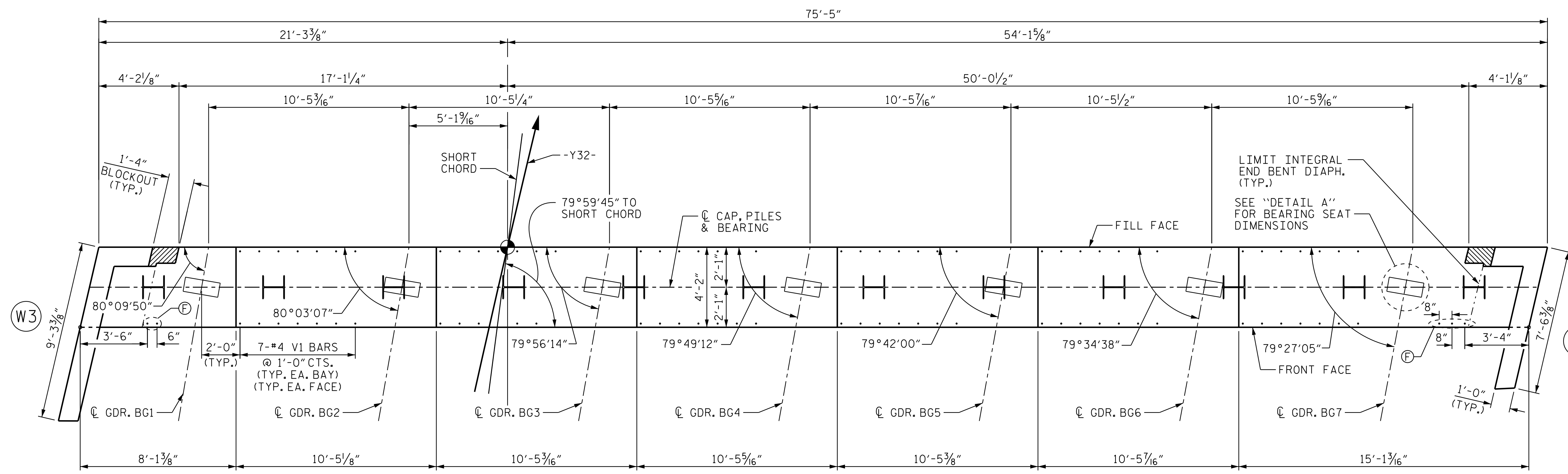
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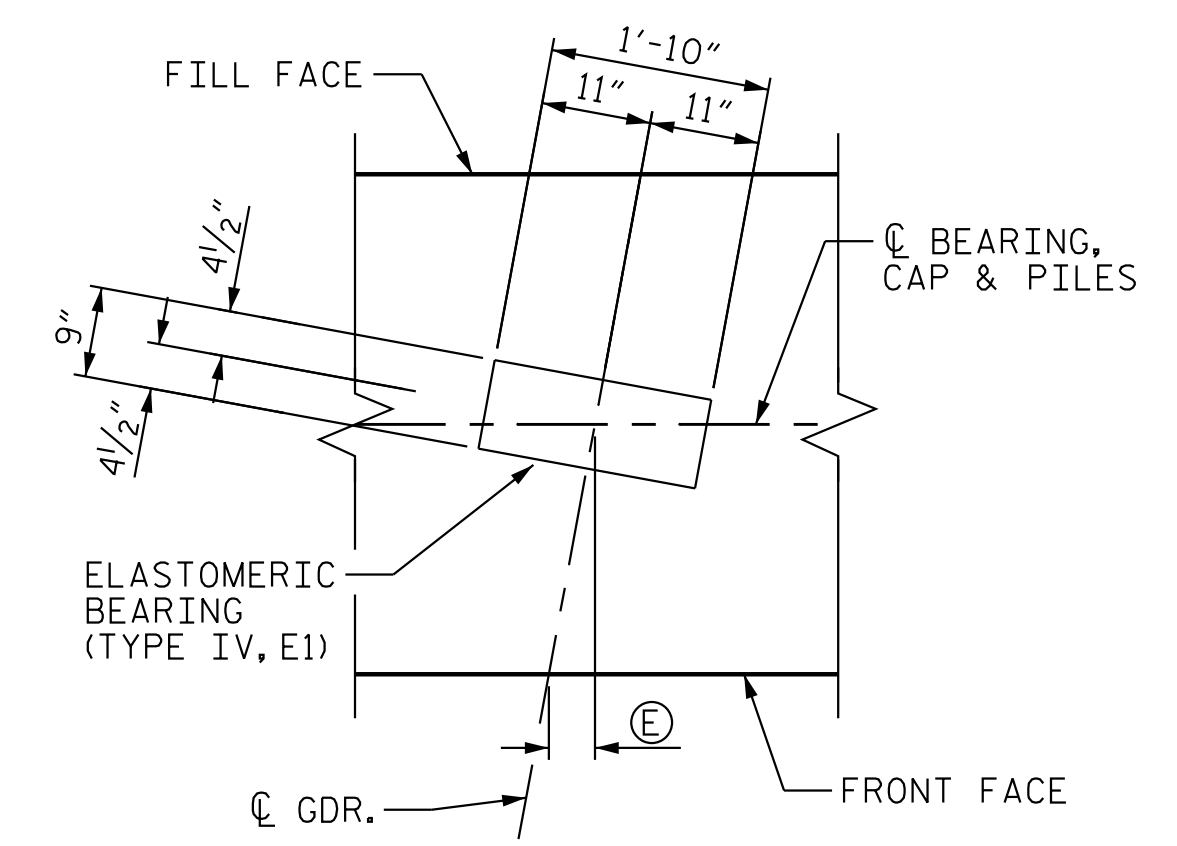
SHEET 3 OF 3

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STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUBSTRUCTURE  
 BENT 1  
 DETAILS



**PLAN**  
FOR WING WALL DETAILS, SEE "END BENT 2, DETAILS - WING WALLS" SHT. 2 OF 3



**DETAIL A**

PILE CUTOFF ELEVATIONS	
PILE	ELEVATION
1	77.67
2	77.32
3	76.97
4	76.63
5	76.28
6	75.93
7	75.58
8	75.23
9	74.88
10	74.53
11	74.18
12	73.83

GIRDER	(E)
BG1	4 5/16"
BG2	4 7/8"
BG3	4 7/16"
BG4	4 1/2"
BG5	4 9/16"
BG6	4 7/8"
BG7	4 5/8"

**NOTES**

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

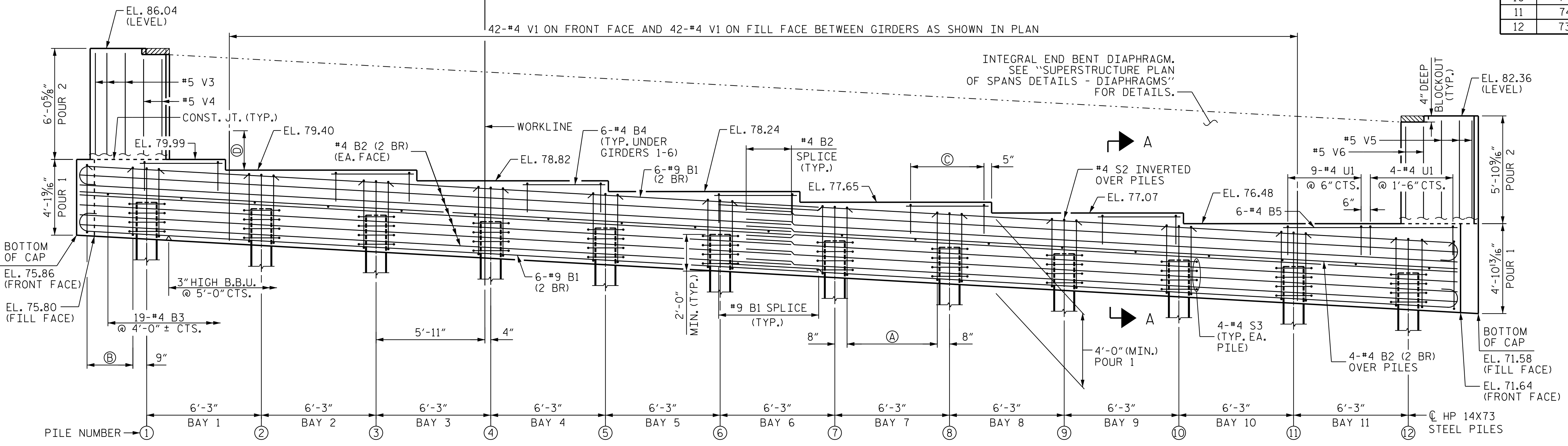
FOR WING DETAILS, SEE SHEET 2 OF 3.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

(2 BR) DENOTES 2 BAR RUN

TIE #4 V1 AT GIRDERS TO #4 "K" BARS, SEE "TYPICAL SECTION DETAILS" SHEET 3 OF 3.

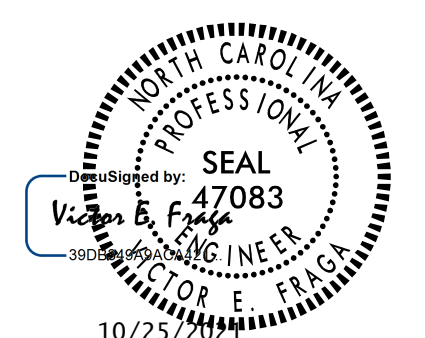
TOP OF CAP AREAS OUTSIDE OF SUPERSTRUCTURE SHALL BE SLOPED AT A RATE OF 1/4" PER FOOT UPWARDS FROM FRONT FACE PERPENDICULAR TO FILL FACE.



**ELEVATION**  
WING WALLS NOT SHOWN FOR CLARITY

- (A) 11-#4 S1 & 11-#4 S2 @ 6" MAX. CTS. (TYP. EA. BAY)
- (B) 6-#4 S1 & 6-#4 S2 @ 6" CTS. (TYP. EA. OVERHANG)
- (C) 9-#4 U1 @ 6" CTS. (TYP. UNDER GIRDERS 1-6)
- (D) 1'-9" MIN. PROJECTION ABOVE BRIDGE SEAT

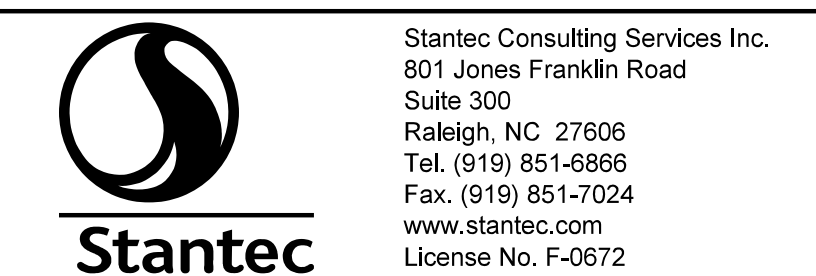
SPLICE LENGTH	
#9 B1	5'-4"
#4 B2	2'-5"



PROJECT NO. R-3300B  
PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
25+28.04 -Y32-

SHEET 1 OF 3

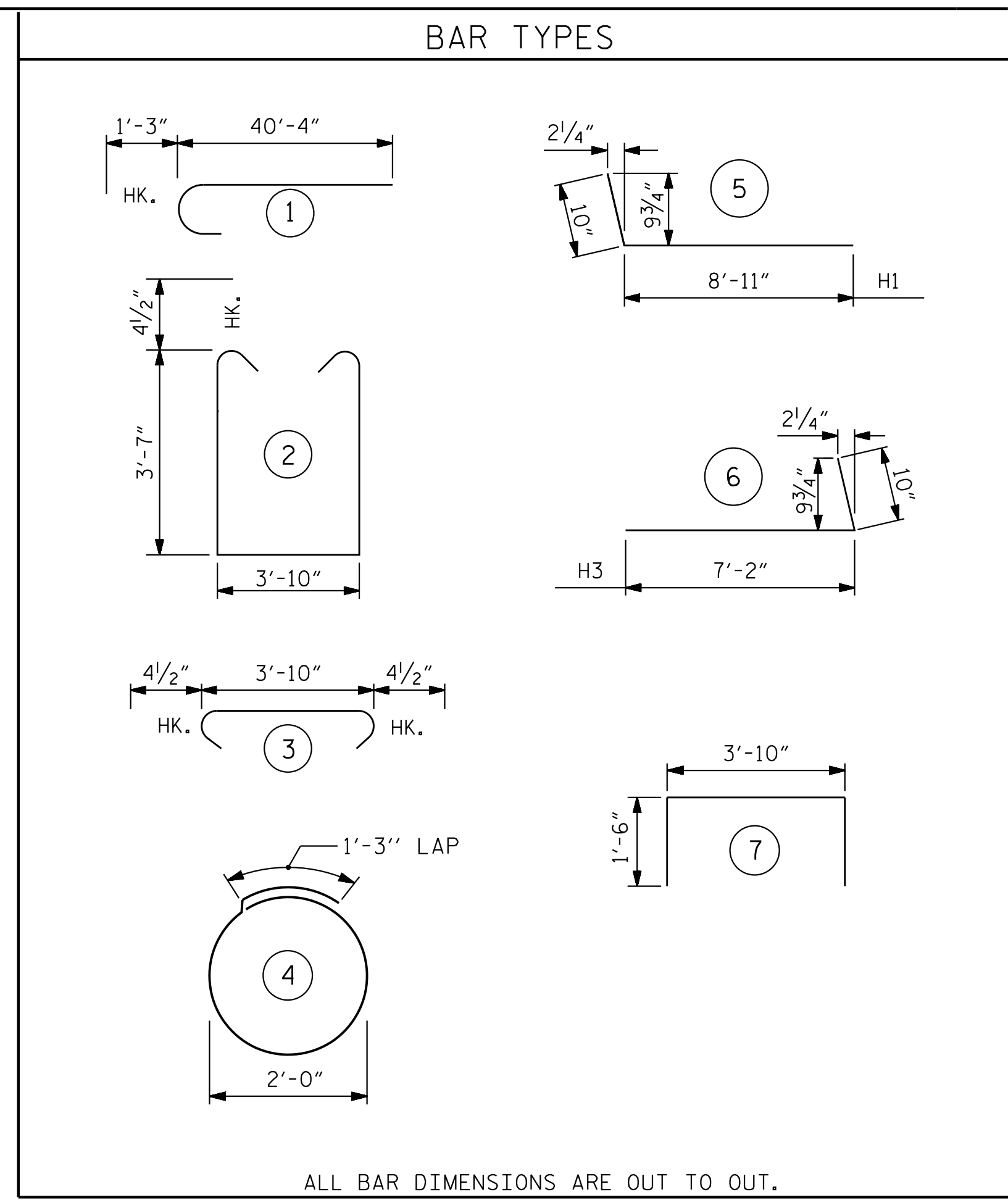
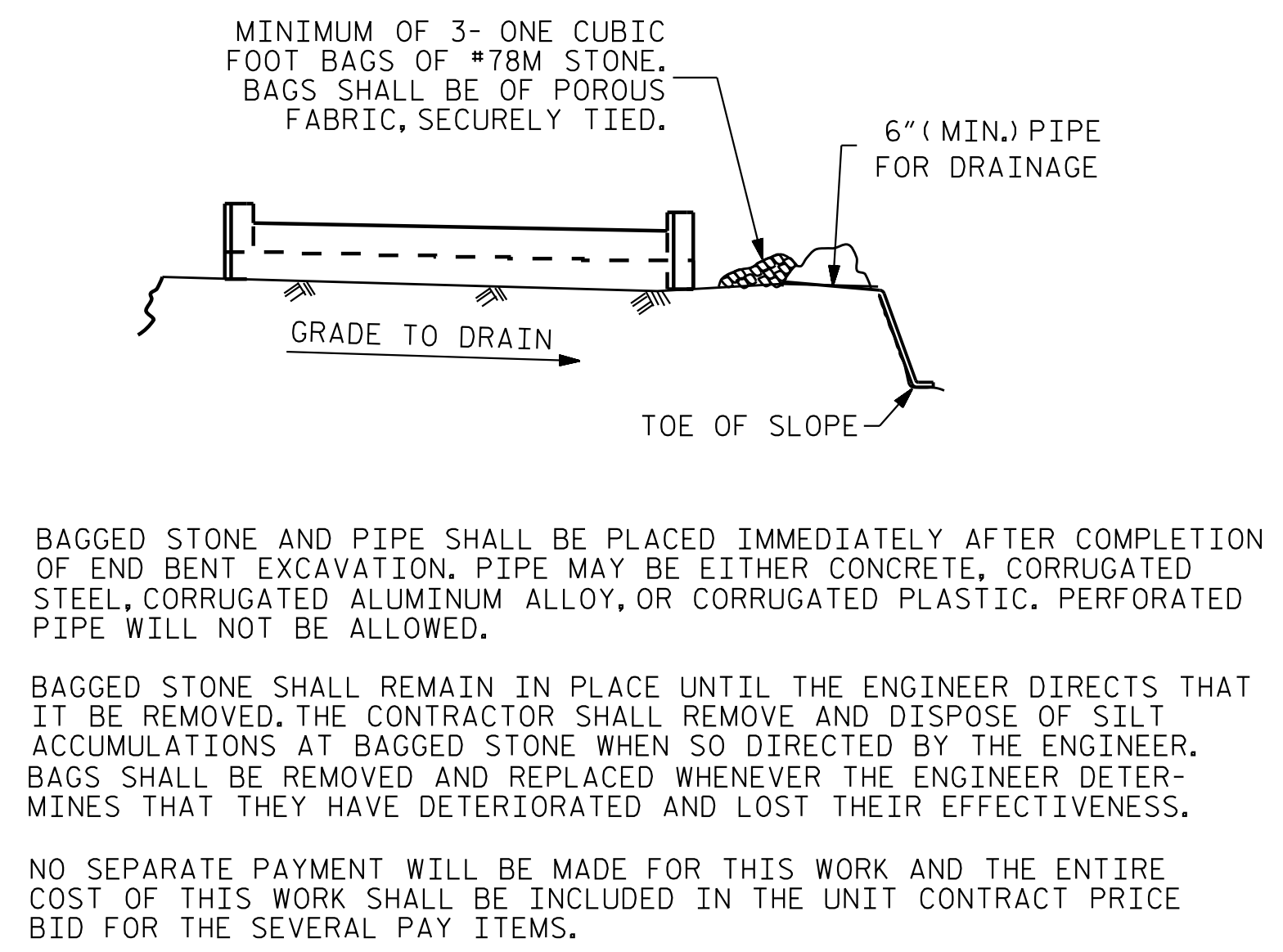
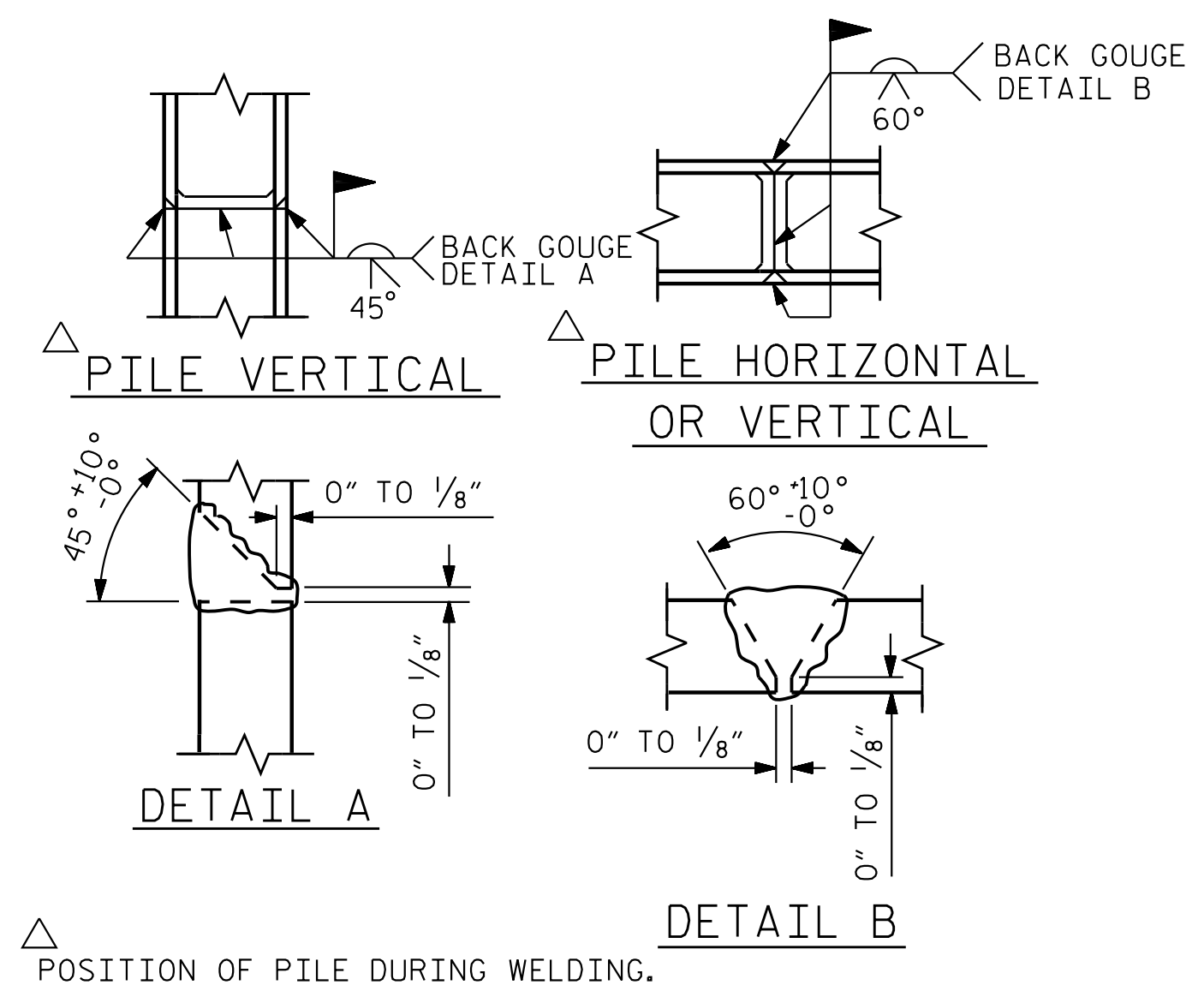
REVISIONS						SHEET NO. S17-32
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS 38
2			4			



DRAWN BY: T. N. ENNIS DATE: 02/05/20  
 CHECKED BY: V. E. FRAGA DATE: 02/11/20  
 DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21

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**BILL OF MATERIAL**

**END BENT 1**

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	24	#9	1	41'-7"	3393
B2	32	#4	STR	38'-10"	830
B3	19	#4	STR	3'-10"	49
B4	36	#4	STR	4'-6"	108
B5	6	#4	STR	9'-6"	38
H1	22	#5	5	9'-9"	224
H2	22	#5	STR	3'-9"	86
H3	24	#5	6	8'-0"	200
H4	24	#5	STR	3'-8"	92
S1	133	#4	3	4'-7"	407
S2	145	#4	2	11'-9"	1138
S3	48	#4	4	7'-7"	243
U1	67	#4	7	6'-10"	306
V1	84	#4	STR	6'-0"	337
V2	5	#4	STR	9'-0"	30
V3	24	#4	STR	9'-9"	156
V4	4	#4	STR	9'-5"	25
V5	20	#4	STR	10'-4"	138
V6	4	#4	STR	10'-0"	27
REINFORCING STEEL					LBS. 7,827

CLASS A CONCRETE BREAKDOWN:

POUR 1: CAP, ETC. C.Y. 51.7

POUR 2: UPPER WINGS C.Y. 5.0

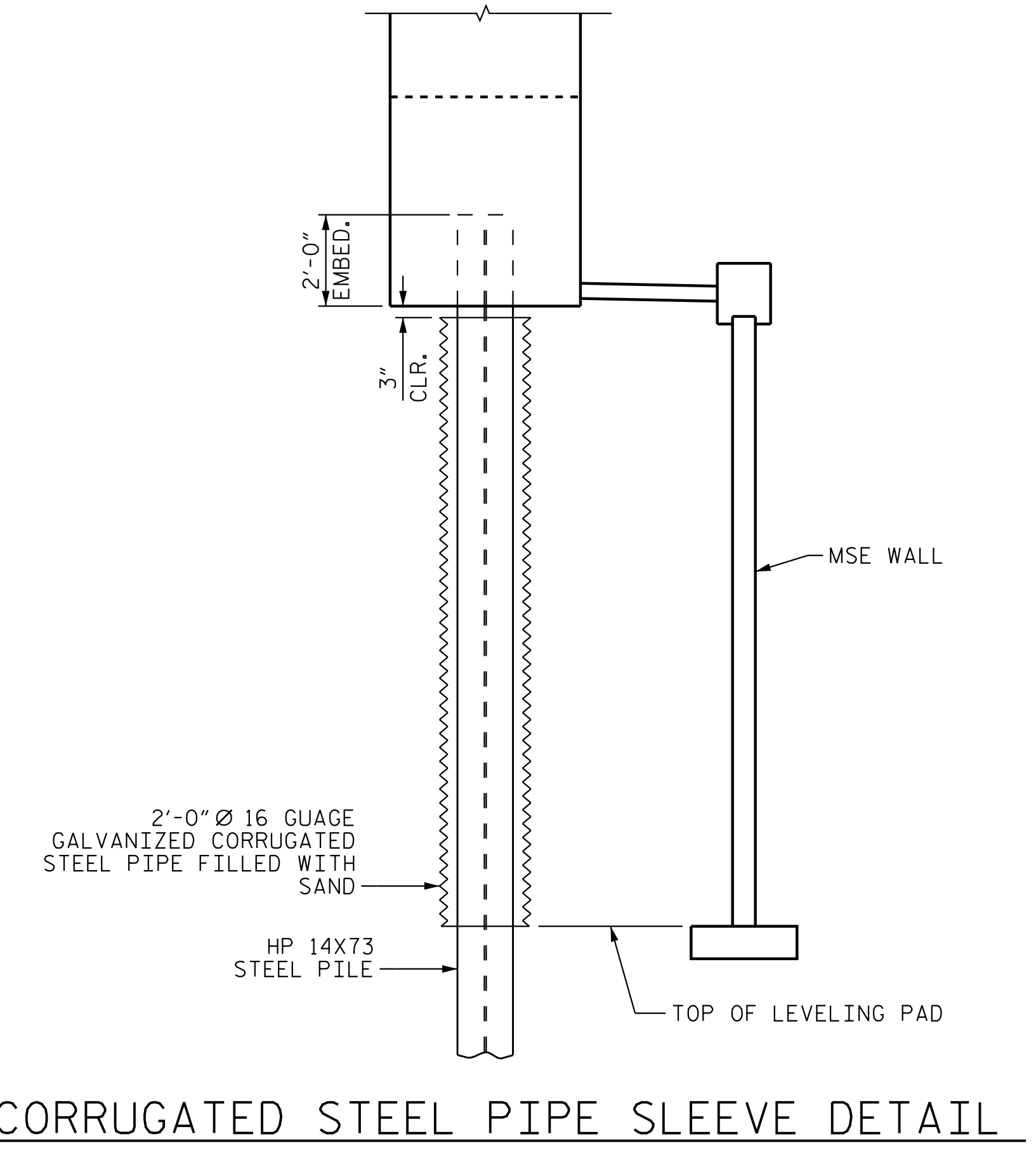
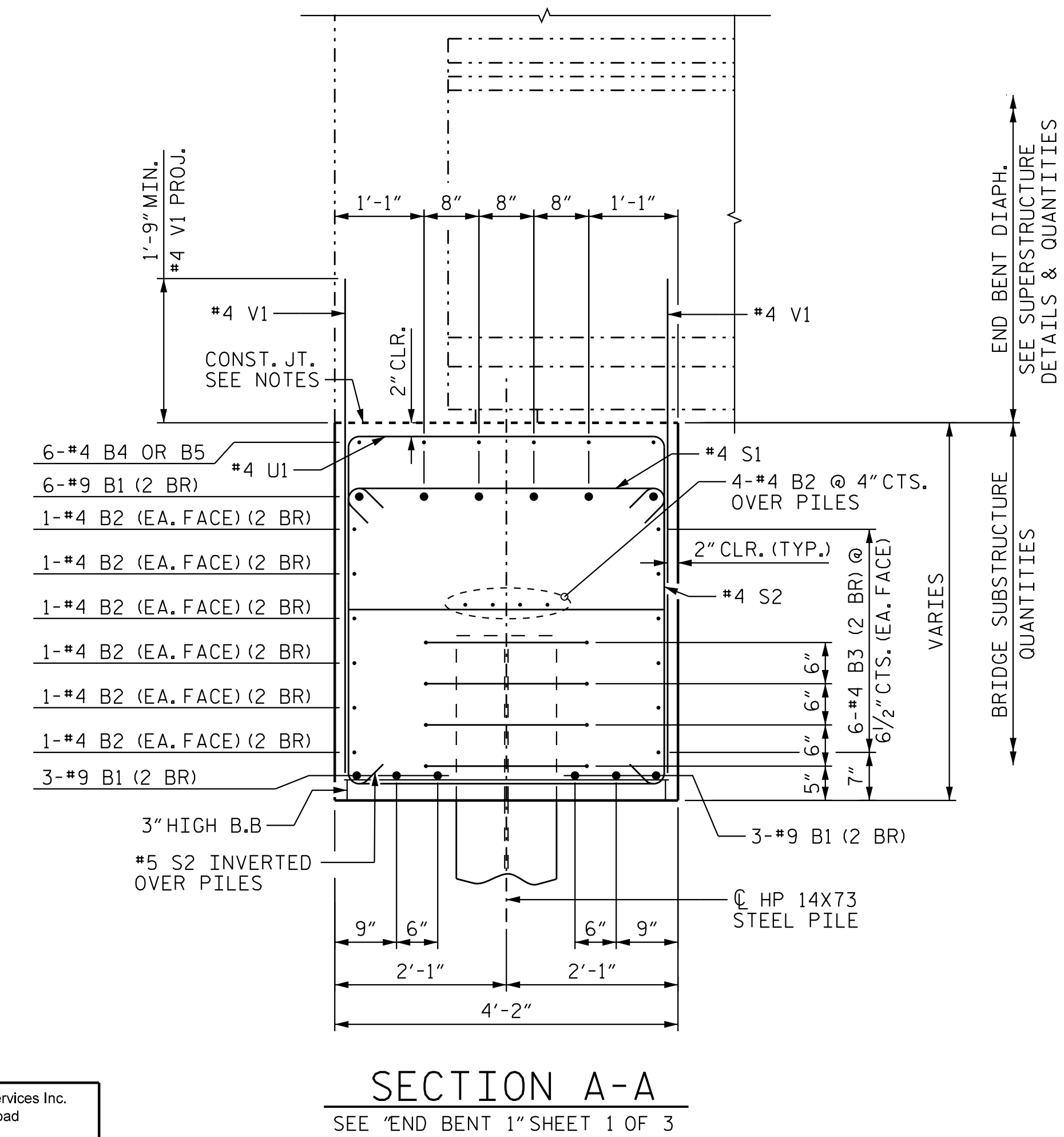
CLASS A CONCRETE TOTAL C.Y. 56.7

HP14X73 PILES NO. 12  
LIN. FT. 1,200

PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 STEEL PILES EA. 12

PILE REDRIVES EA. 6

STEEL PILE POINTS EA. 12



**NOTES**

TOP SURFACE AREAS OF THE END BENT CAP SHALL BE KEPT CLEAN AND FREE OF LAITANCE.

THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCLUDING THE BEARING AREA & TOP OF CAP OUTSIDE THE LIMITS OF THE SUPERSTRUCTURE, SHALL BE RAKED TO A DEPTH OF 1/4".

2 BR DENOTES 2 BAR RUN.

PROJECT NO. R-3300B

PENDER COUNTY

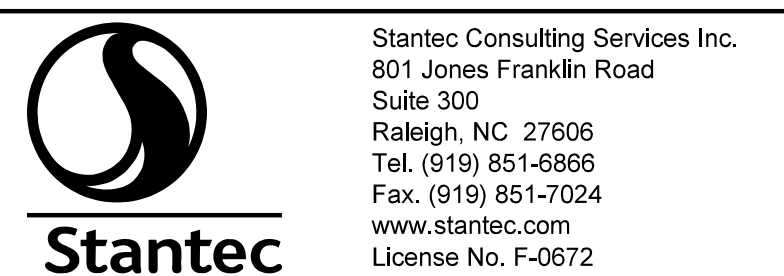
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SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

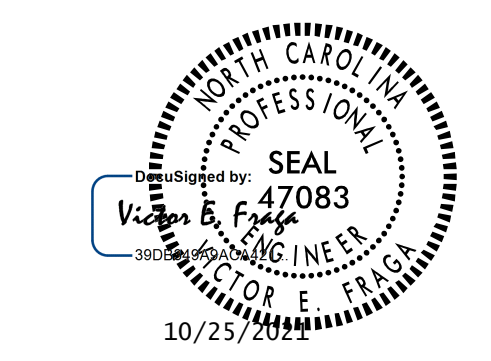
**SUBSTRUCTURE  
END BENT 2  
DETAILS**

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1			3			TOTAL SHEETS 38
2			4			



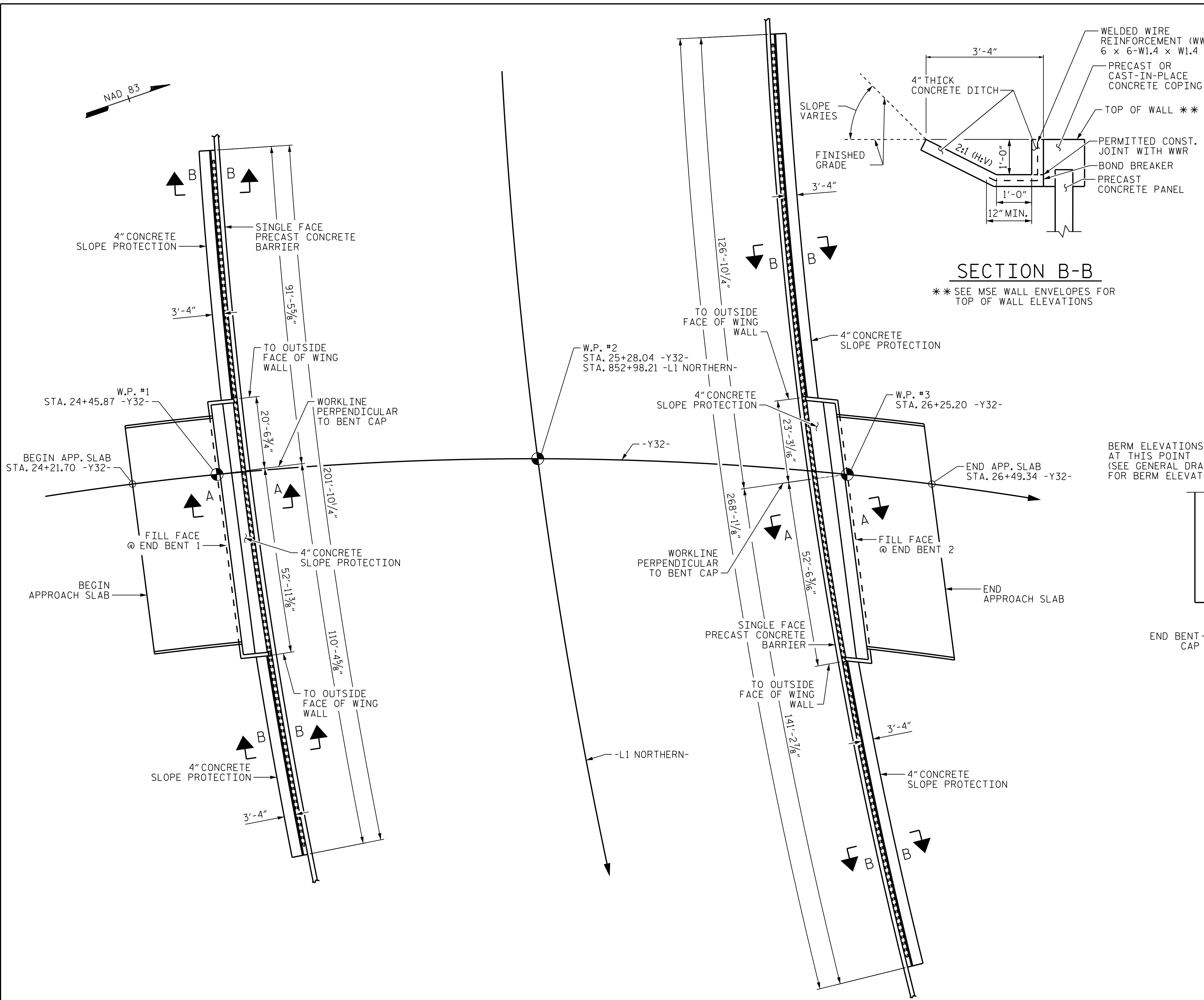
DRAWN BY: T. N. ENNIS DATE: 02/06/20  
CHECKED BY: V. E. FRAGA DATE: 02/11/20

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21



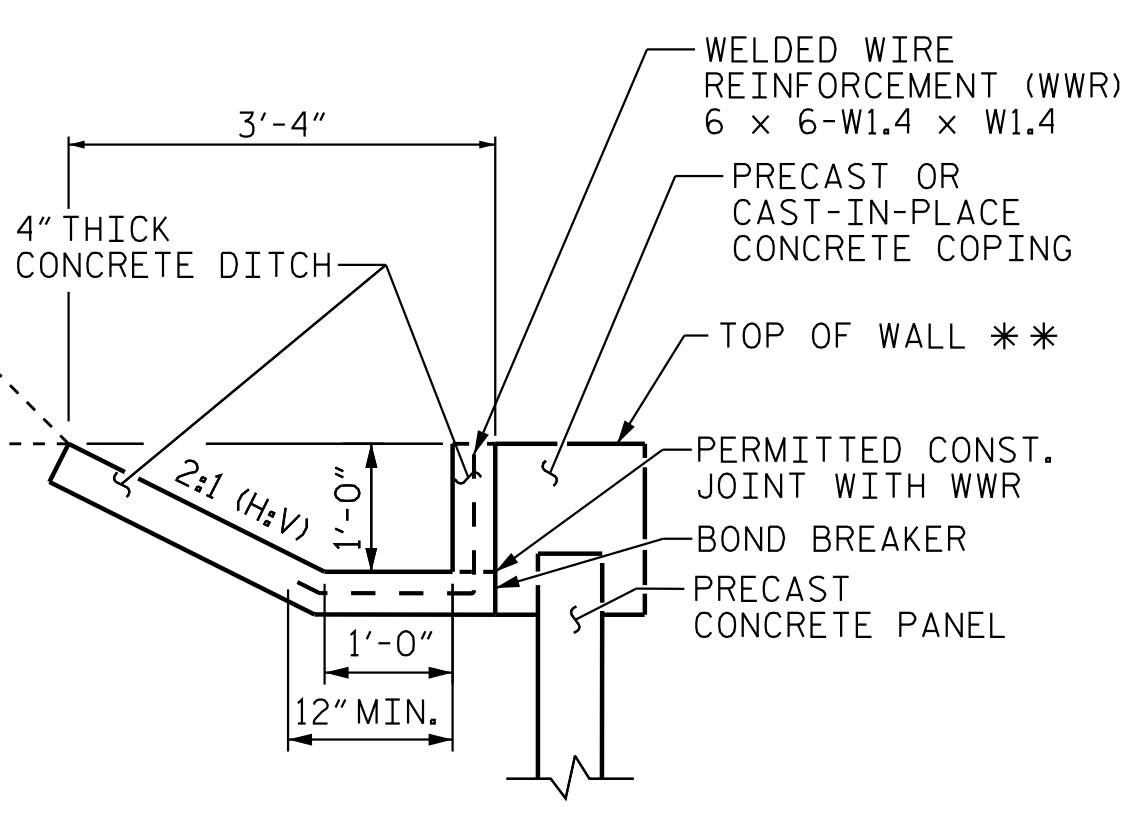
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SECTION B-B

\*\* SEE MSE WALL ENVELOPES FOR TOP OF WALL ELEVATIONS



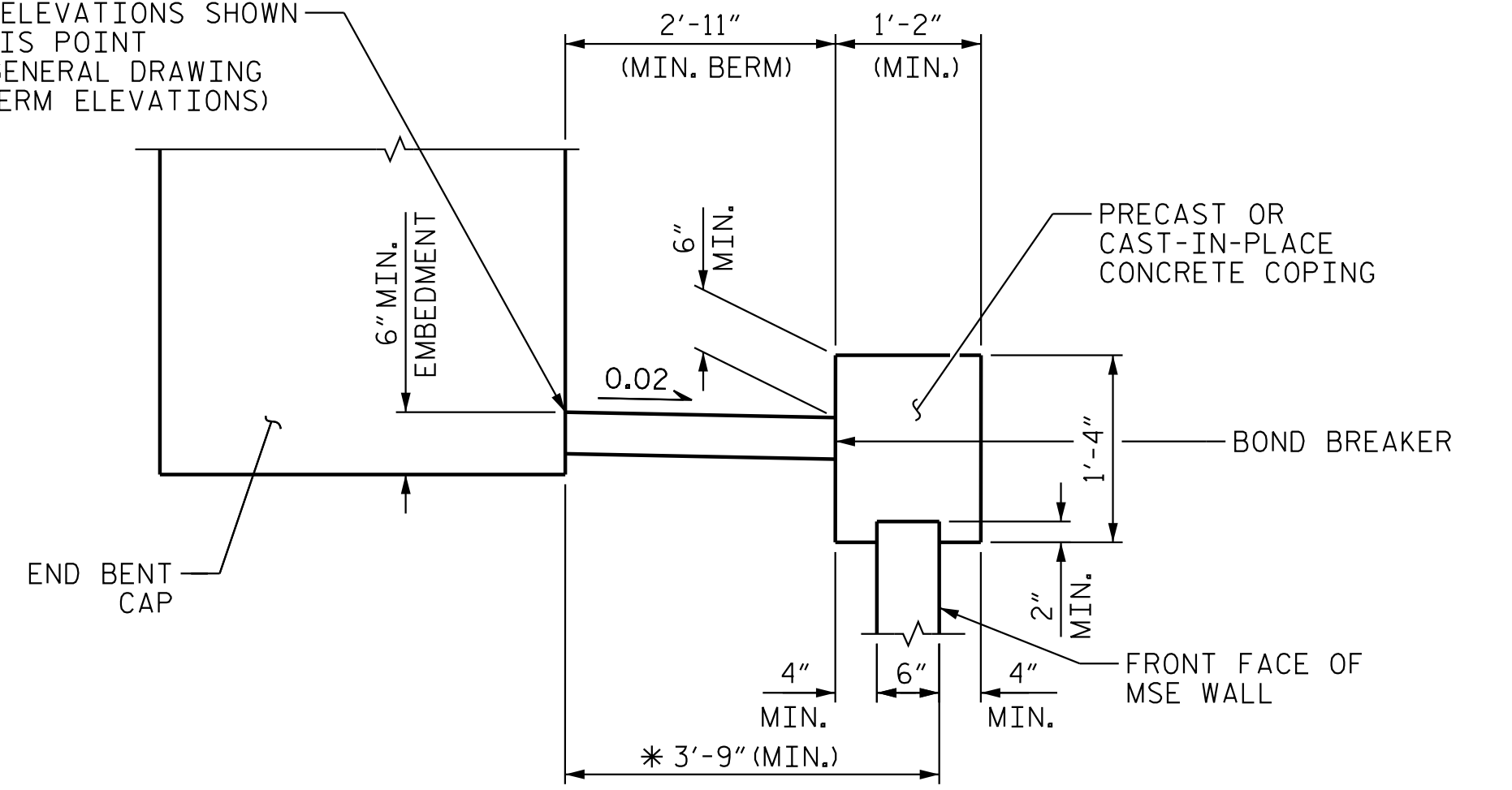
GENERAL NOTES

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING. SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. PROVIDE AT LEAST A 6" LAP TO SPLICE MATS OF WELDED WIRE FABRIC. THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

ALL ITEMS AND WORK NECESSARY TO FURNISH AND CONSTRUCT THE VERTICAL PORTION OF SLOPE PAVING ARE INCIDENTAL AND SHOULD BE INCLUDED IN THE UNIT CONTRACT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA. 852+98.21 -L1-NORTHERN-	4" INCH SLOPE PROTECTION	WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX. L.F.
END BENT 1	85	153
END BENT 2	123	221
TOTAL	208	374

BERM ELEVATIONS SHOWN AT THIS POINT (SEE GENERAL DRAWING FOR BERM ELEVATIONS)



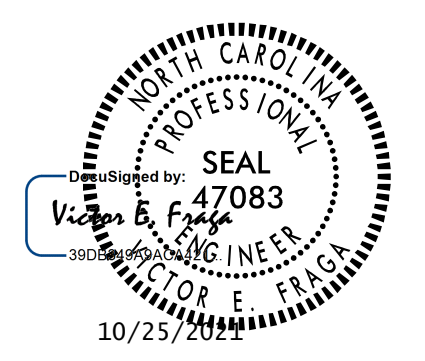
SECTION A-A

PLAN

PROJECT NO. R-3300B  
 PENDER COUNTY  
 STATION: 852+98.21 -L1-NORTHERN-  
 25+28.04 -Y32-

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SLOPE PROTECTION DETAILS



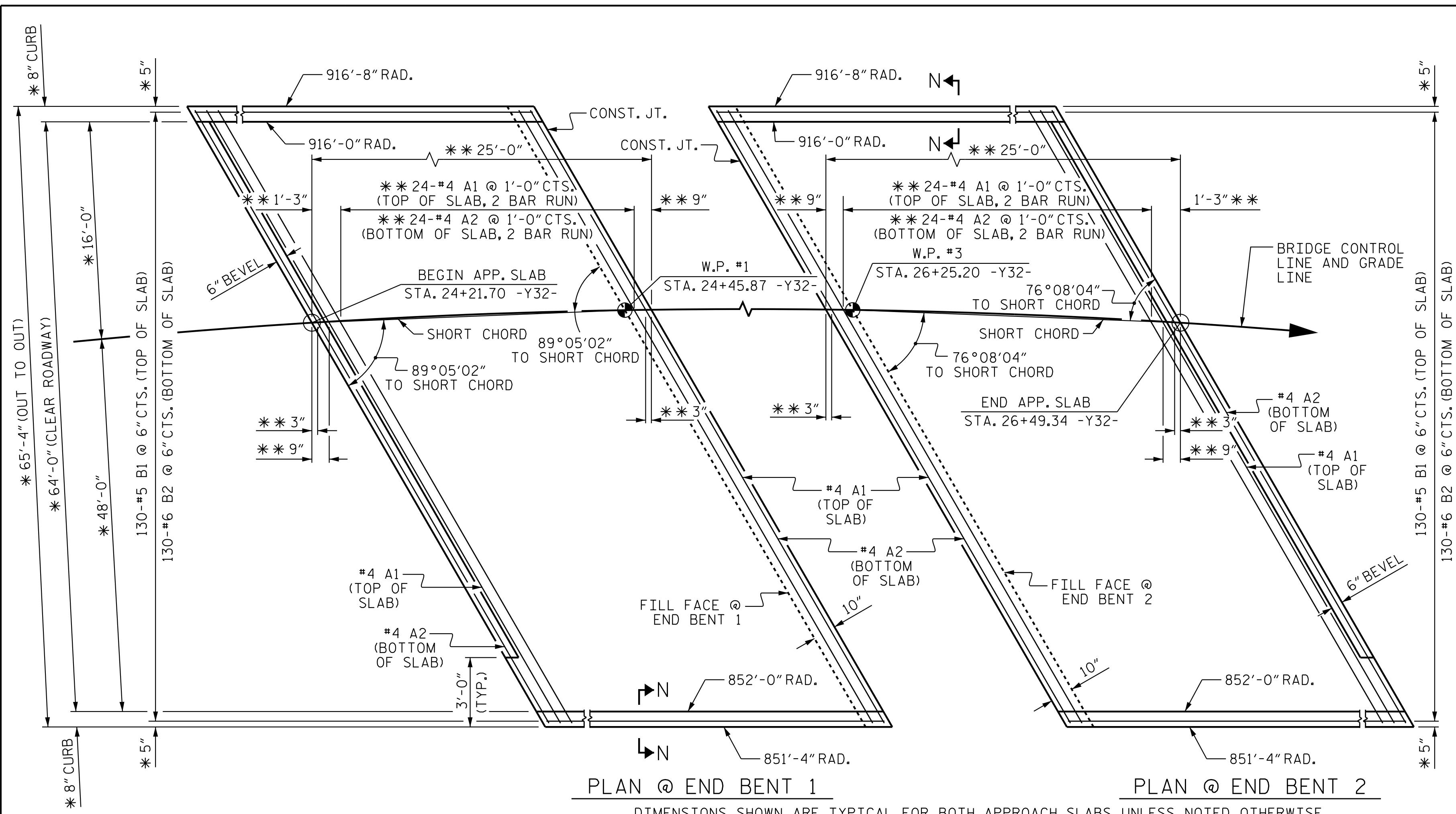
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DRAWN BY : V. E. FRAGA DATE : 06/26/20  
 CHECKED BY : J. T. KELVINGTON DATE : 06/29/20  
 DESIGN ENGINEER OF RECORD : V. E. FRAGA DATE : 10/25/21

10/25/2021 10:25/2021



PLAN @ END BENT 1  
 PLAN @ END BENT 2  
 DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS, UNLESS NOTED OTHERWISE.

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 6" Ø DRAINAGE PIPE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 6" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

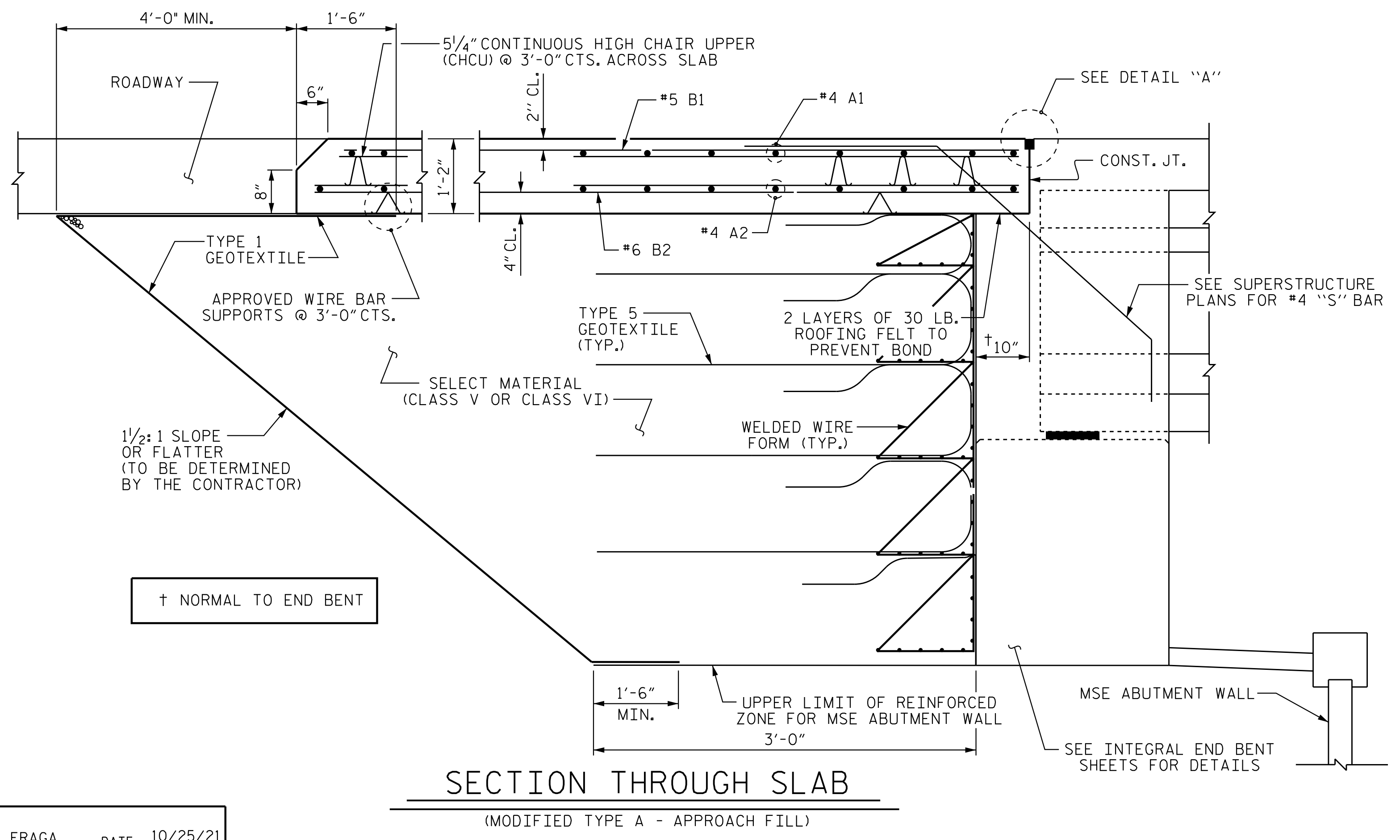
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

\* RADIAL DIMENSION  
 \*\* ALONG ARC

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

BILL OF MATERIAL					
FOR ONE APPROACH SLAB (2 REQ'D)					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	54	#4	STR	34'-8"	1,250
A2	54	#4	STR	34'-6"	1,244
* B1	130	#5	STR	24'-2"	3,277
B2	130	#6	STR	24'-8"	4,816
* G1	17	#4	STR	5'-3"	60
* G2	4	#4	STR	24'-8"	66
REINFORCING STEEL				LBS.	6,060
* EPOXY COATED REINFORCING STEEL				LBS.	4,653
CLASS AA CONCRETE				C. Y.	72.9



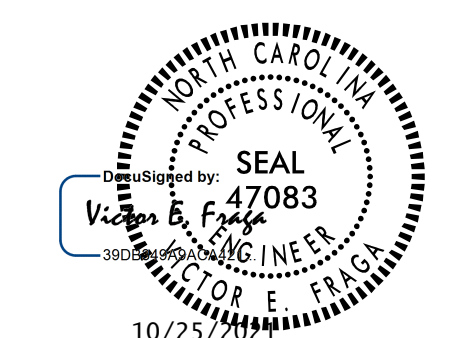
SECTION THROUGH SLAB  
 (MODIFIED TYPE A - APPROACH FILL)

PROJECT NO. R-3300B  
 PENDER COUNTY  
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SHEET 1 OF 3

STATE OF NORTH CAROLINA  
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 RALEIGH  
 STANDARD

BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT WITH FLEXIBLE PAVEMENT



REVISIONS						SHEET NO. S17-36
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1			3			TOTAL SHEETS 38
2			4			

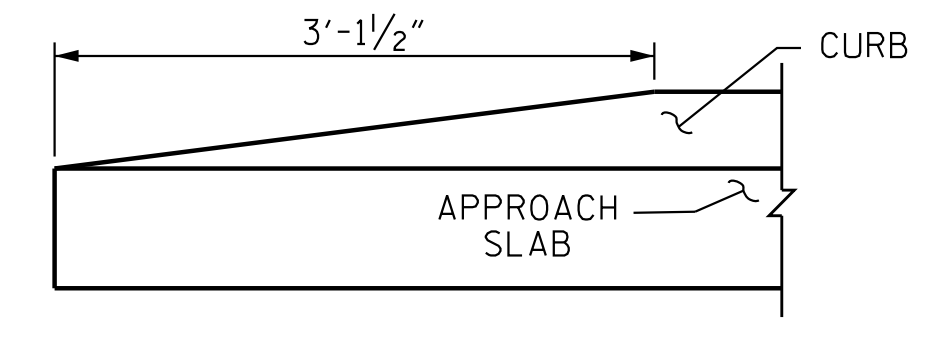
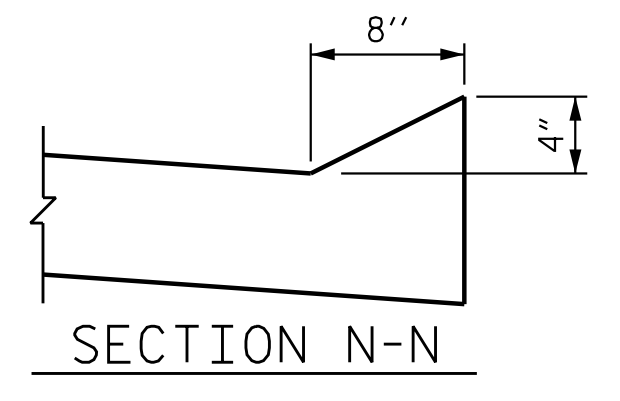
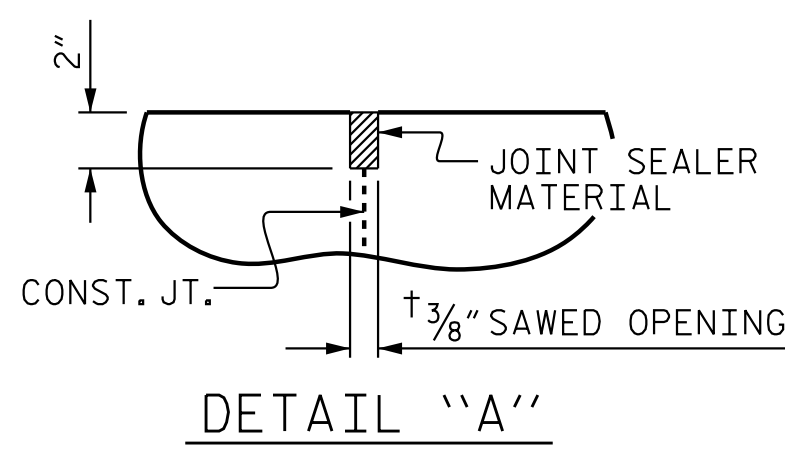
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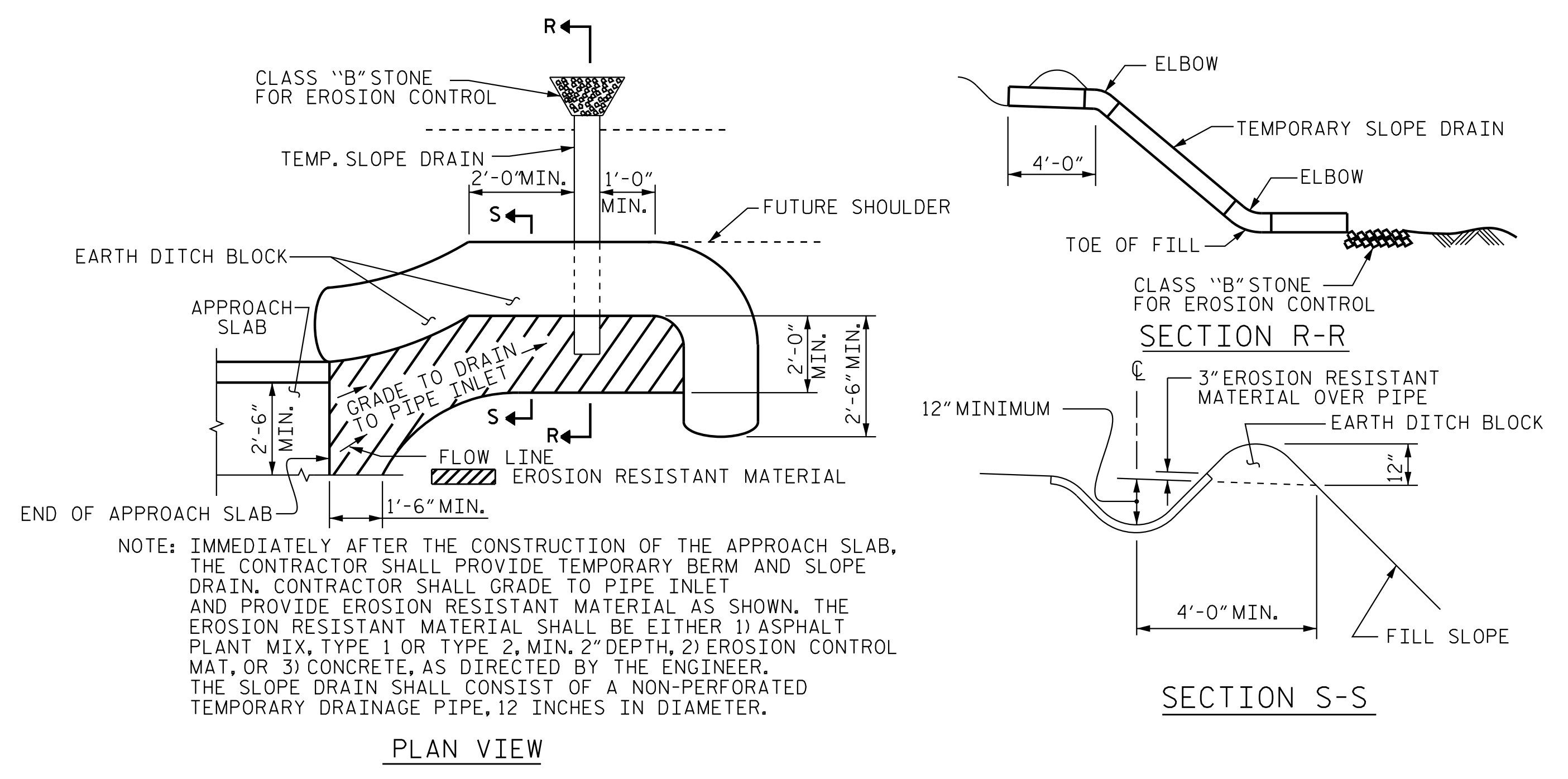
ASSEMBLED BY: J. F. KENNEDY DATE: 09/23/19  
 CHECKED BY: T. N. ENNIS DATE: 02/10/20

DRAWN BY: TLA 10/05 REV. 6/13 MAA/GM  
 CHECKED BY: GM 5/06 REV. 12/17 MAA/THC  
 REV. 06/19 BNB/THC

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE: 10/25/21

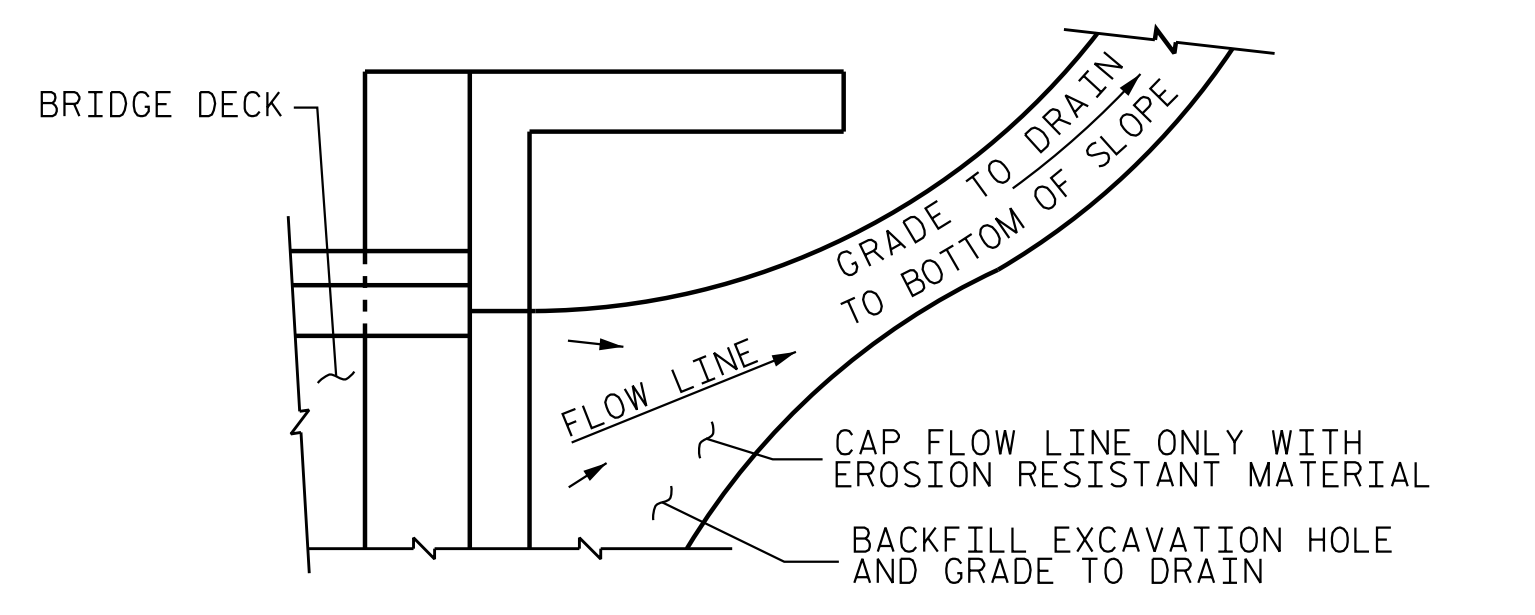


END OF CURB WITHOUT SHOULDER BERM GUTTER



NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

**TEMPORARY BERM AND SLOPE DRAIN DETAILS**  
(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



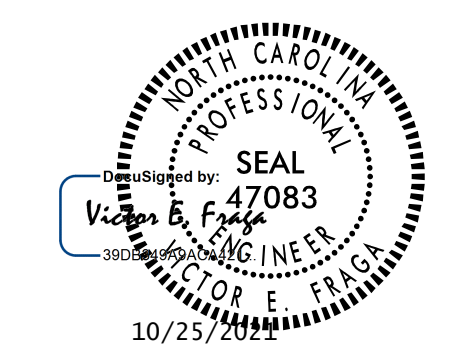
NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

**TEMPORARY DRAINAGE DETAIL**

PROJECT NO. R-3300B  
PENDER COUNTY  
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SHEET 2 OF 3

STATE OF NORTH CAROLINA  
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 STANDARD  
**BRIDGE APPROACH  
 SLAB DETAILS**



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1			3			TOTAL SHEETS	38
2			4				

STD. NO. BAS5

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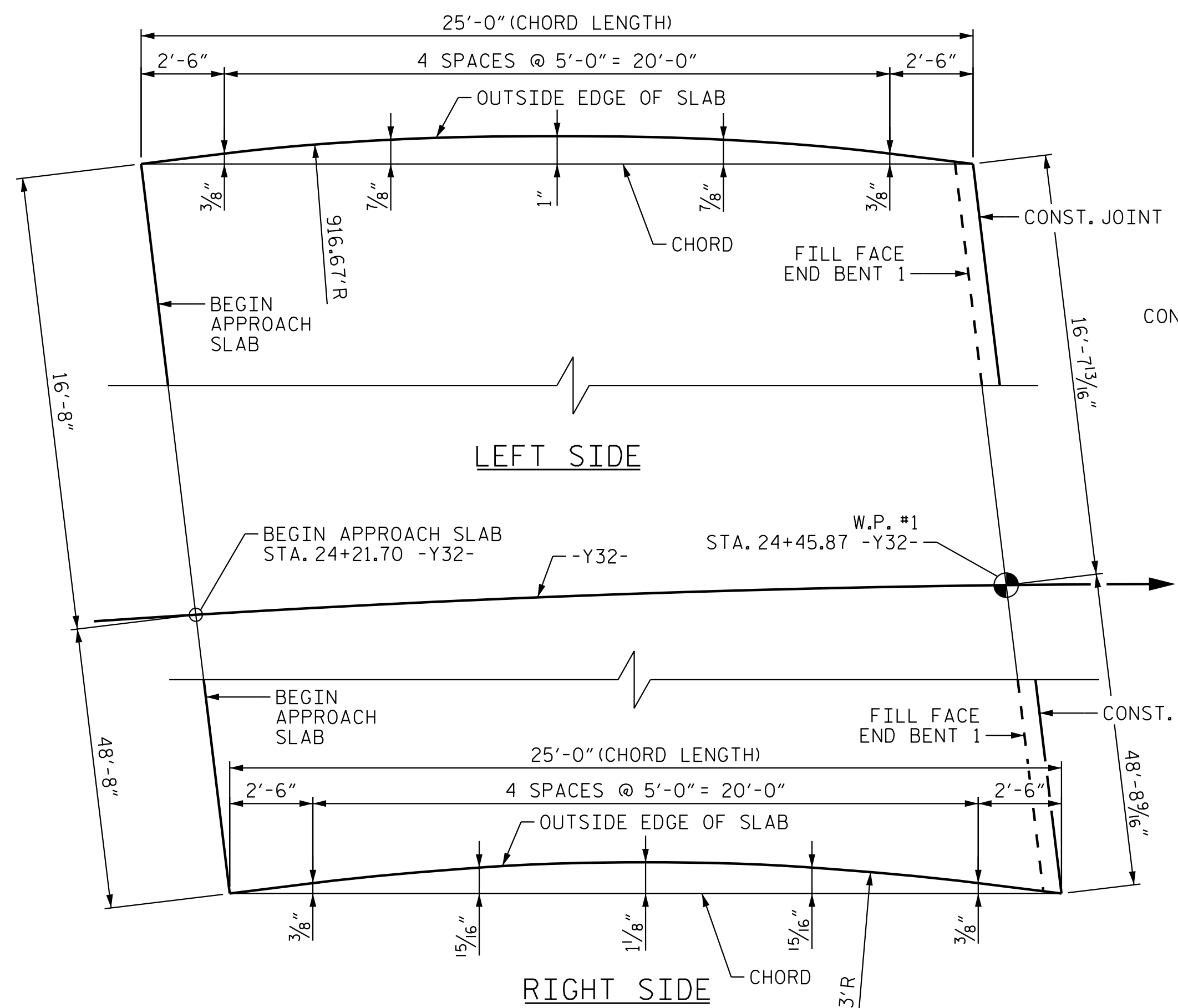
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 CHECKED BY : T. N. ENNIS DATE : 02/10/20

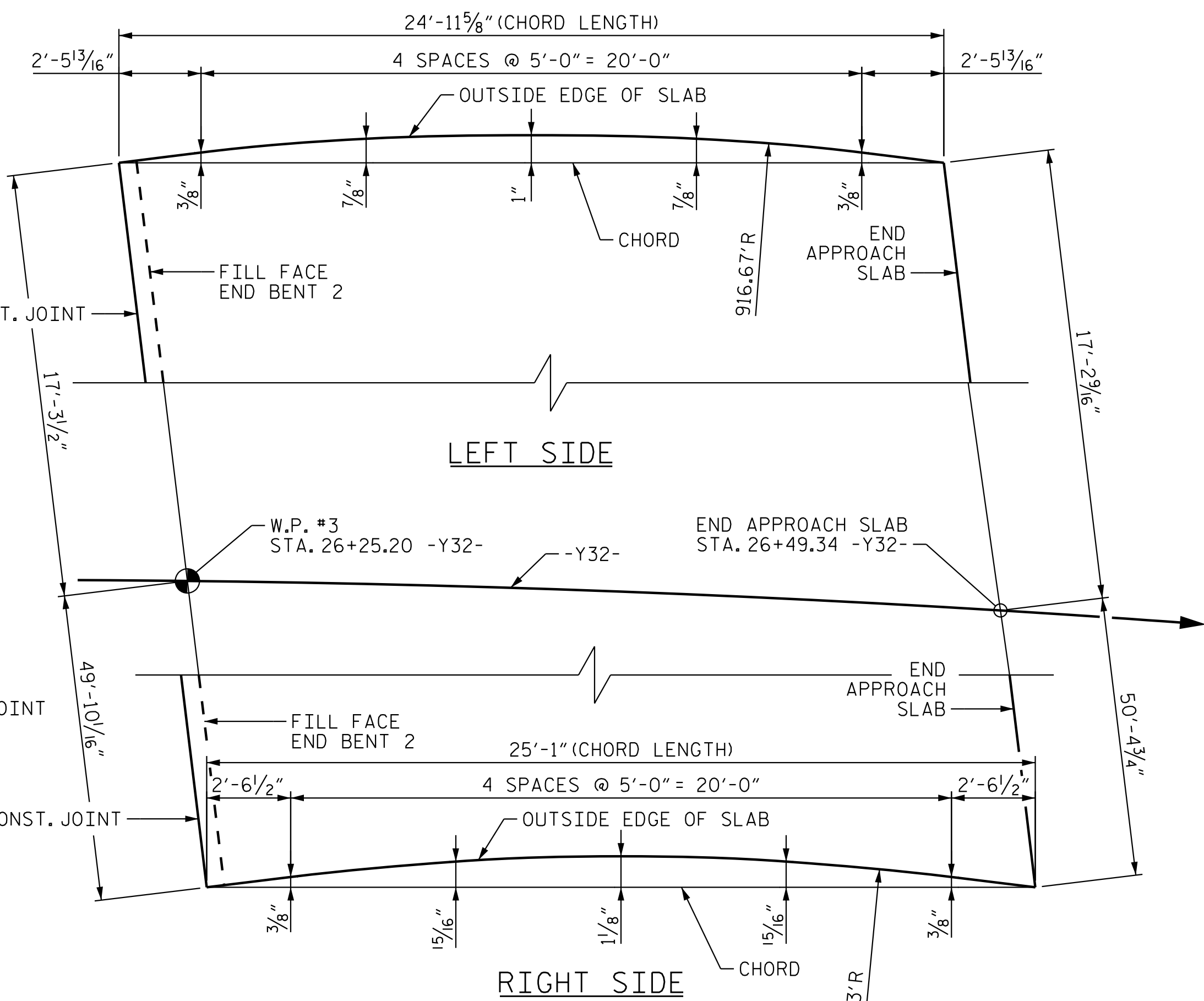
DRAWN BY : TLA 10/05 REV. 12/21/11 MAA/GM  
 CHECKED BY : GM 5/06 REV. 6/13 MAA/GM  
 REV. 12/17 MAA/THC

DESIGN ENGINEER OF RECORD: V. E. FRAGA DATE : 10/25/21





ARC OFFSETS -  
BEGIN APPROACH SLAB



ARC OFFSETS -  
END APPROACH SLAB

**NOTES**

GROOVED CONTRACTION JOINTS, 1" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE MONOLITHIC CONCRETE MEDIAN IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINTS SHALL BE LOCATED AT A SPACING OF 10 FT. BETWEEN EXPANSION JOINTS, NO CONTRACTION JOINT WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FT. IN LENGTH. PLACE 1/2" EXPANSION JOINTS AT 30' INTERVALS.

ALL REINFORCING STEEL IN MONOLITHIC CONCRETE MEDIAN SHALL BE EPOXY COATED.

FOR MONOLITHIC CONCRETE MEDIAN ON BRIDGE DECK SLAB, SEE "MONOLITHIC CONCRETE MEDIAN DETAILS" SHEETS.

NO SEPARATE MEASUREMENT OR PAYMENT WILL BE MADE FOR MATERIALS OR LABOR REQUIRED TO CONSTRUCT THE CONCRETE MEDIAN. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE APPROACH SLAB.

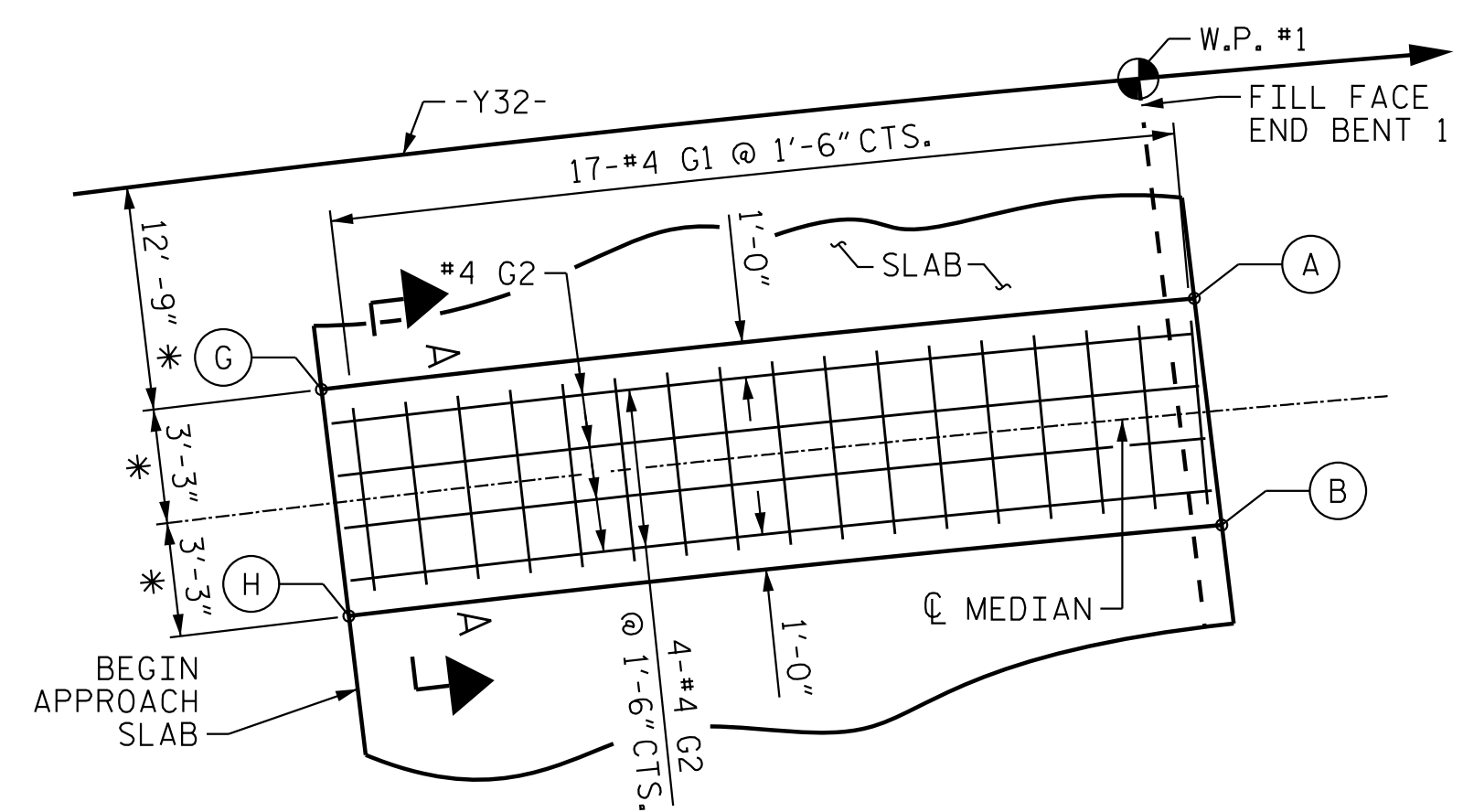
THE CONCRETE AND REINFORCING STEEL REQUIRED FOR THE CONCRETE MEDIAN IS INCLUDED IN THE "SUPERSTRUCTURE BILL OF MATERIAL" AND INCLUDED IN THE SQUARE FOOT PRICE FOR "REINFORCED CONCRETE DECK SLAB."

CONCRETE MEDIAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

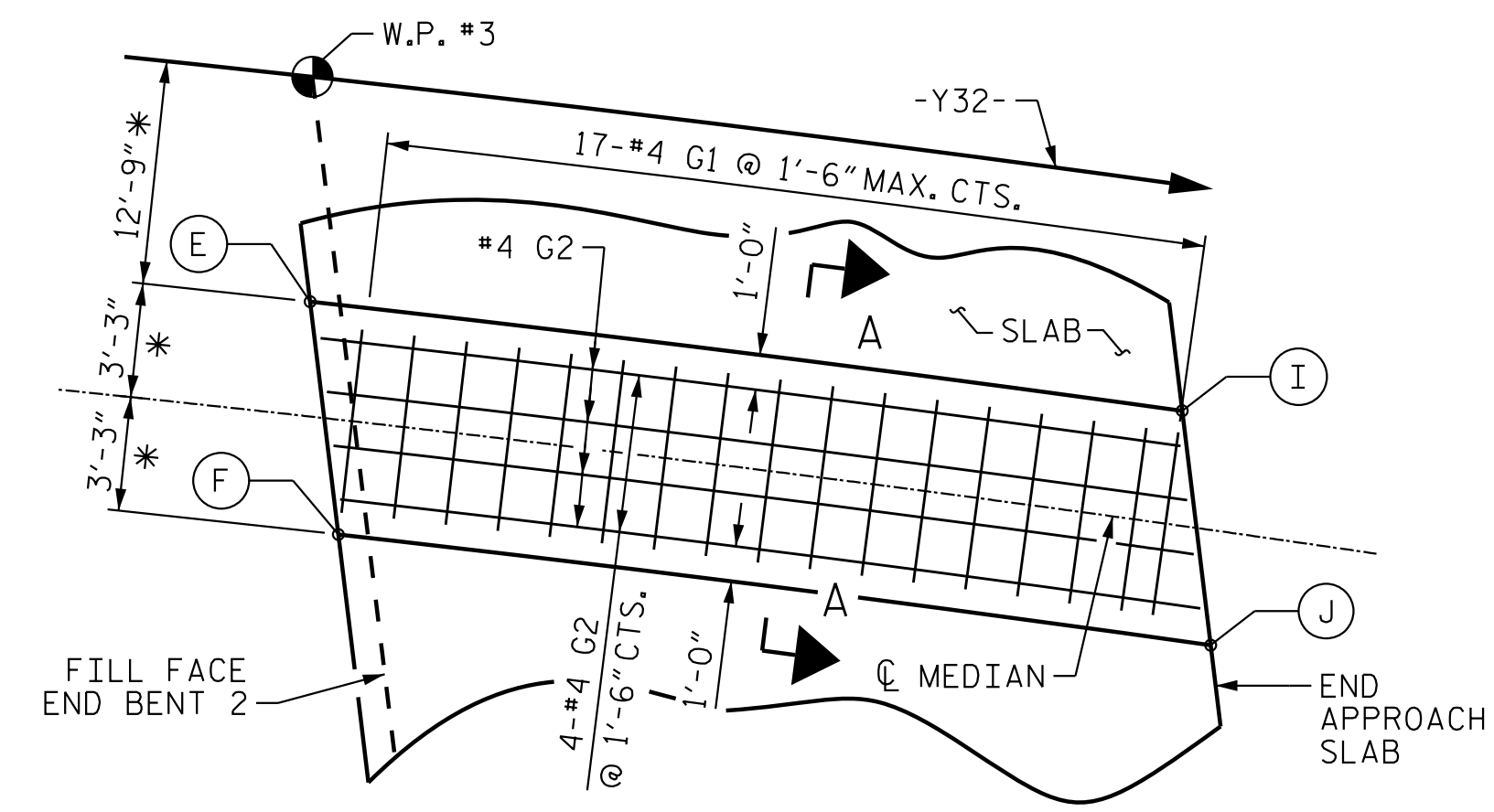
2" MINIMUM CLEARANCE FROM ALL FINISHED CONCRETE SURFACES TO ALL STEEL REINFORCEMENT.

FOR ADDITIONAL INFORMATION, SEE 2018 ROADWAY STANDARD DRAWING 852.01.

\* DENOTES RADIAL DIMENSION.



BEGIN APPROACH SLAB MEDIAN



END APPROACH SLAB MEDIAN

MONOLITHIC CONCRETE MEDIAN LAYOUT		
POINT	STATION	OFFSET
(A)	SEE SHEET S17-12.	
(B)	SEE SHEET S17-12.	
(C)	SEE SHEET S17-12.	
(D)	SEE SHEET S17-12.	
(G)	24+21.73 -Y32-	12.75 RT.
(H)	24+21.75 -Y32-	19.25 RT.
(I)	26+52.72 -Y32-	12.75 RT.
(J)	26+54.48 -Y32-	19.25 RT.

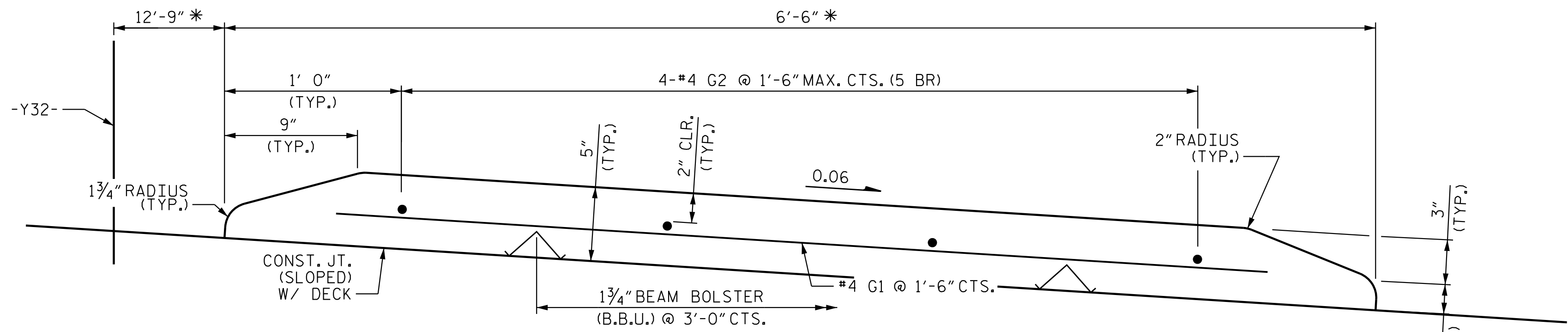
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SHEET 3 OF 3

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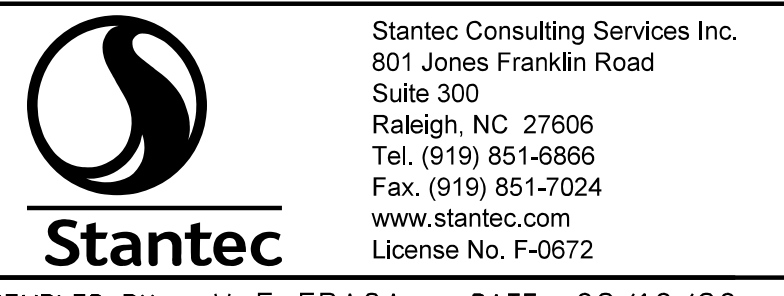
**BRIDGE APPROACH SLAB DETAILS**

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2			4			



SECTION A-A

\* RADIAL DIMENSION.

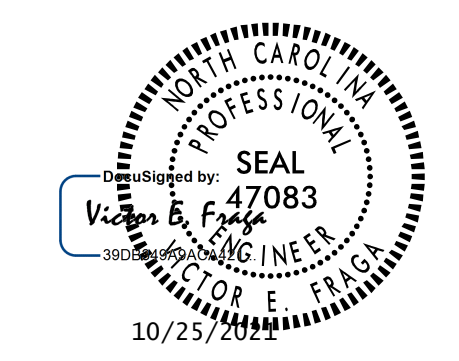


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ASSEMBLED BY : V. E. FRAGA DATE : 02/10/20  
 CHECKED BY : T. N. ENNIS DATE : 02/11/20

DRAWN BY : TLA 10/05 REV. 12/21/11 MAA/GM  
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