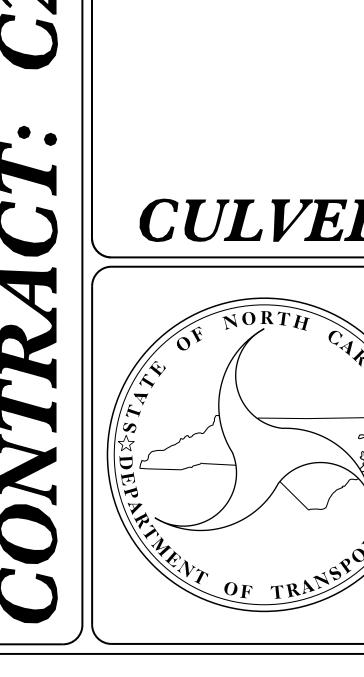
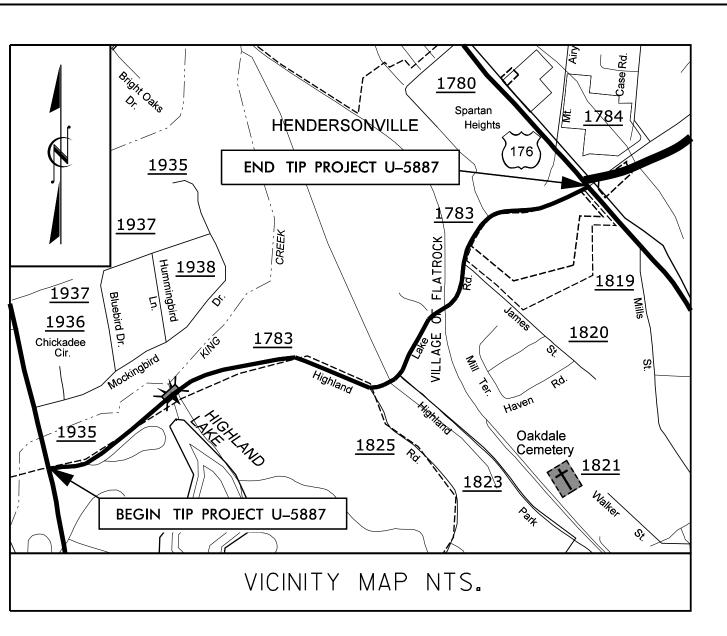
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BEGIN TIP PROJECT U-5887 -

-L-STA.10+00.00

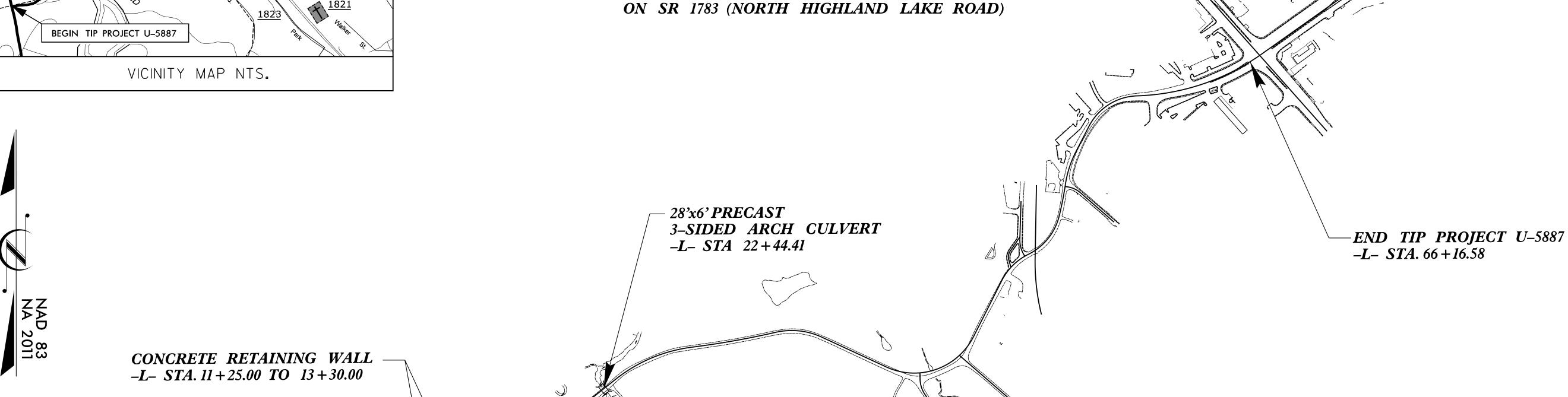
STATE OF NORTH CAROLINA

DIVISION OF HIGHWAYS

HENDERSON COUNTY

NEW STRUCTURE NO. 440394 OVER KING CREEK

STATE	STATE	SHEET NO.	TOTAL SHEETS			
N.C.	ι	J-5887		36		
STATE PROJ. NO.		F. A. PROJ. NO.	DESCRIPT	DESCRIPTION		
44634.1.1			PE			
44634.2.1			R/V	✓		
446	534.3.1		CON	ST.		





SIGNATURES COMPLETED

Consulting Engineers Asheville,

■ North Carolina

Tennessee □ Tennessee

□ North Carolina

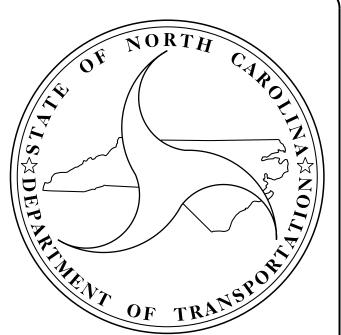
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South Carolina
864.574.4775

Middlesboro, □ Kentucky

CULVERT & RETAINING WALL



DESIGN DATA

ADT 2010 = 6700ADT 2040 = 7200

> T = 5 %V = 35 MPH

FUNC CLASS = MAJOR COLLECTOR

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT U-5887 = 1.058 MI LENGTH STRUCTURE TIP PROJECT U-5887 = 0.006 MI TOTAL LENGTH OF TIP PROJECT U-5887 = 1.064 MI

Prepared in the Office of: VAUGHN & MELTON

FOR THE NORTH CAROLINA DIVISION OF HIGHWAYS

2018 STANDARD SPECIFICATIONS

LETTING DATE:

MAY 18, 2021

HARDY WILLIS, PE PROJECT ENGINEER

CHRIS CORDELL, PE PROJECT DESIGN ENGINEER

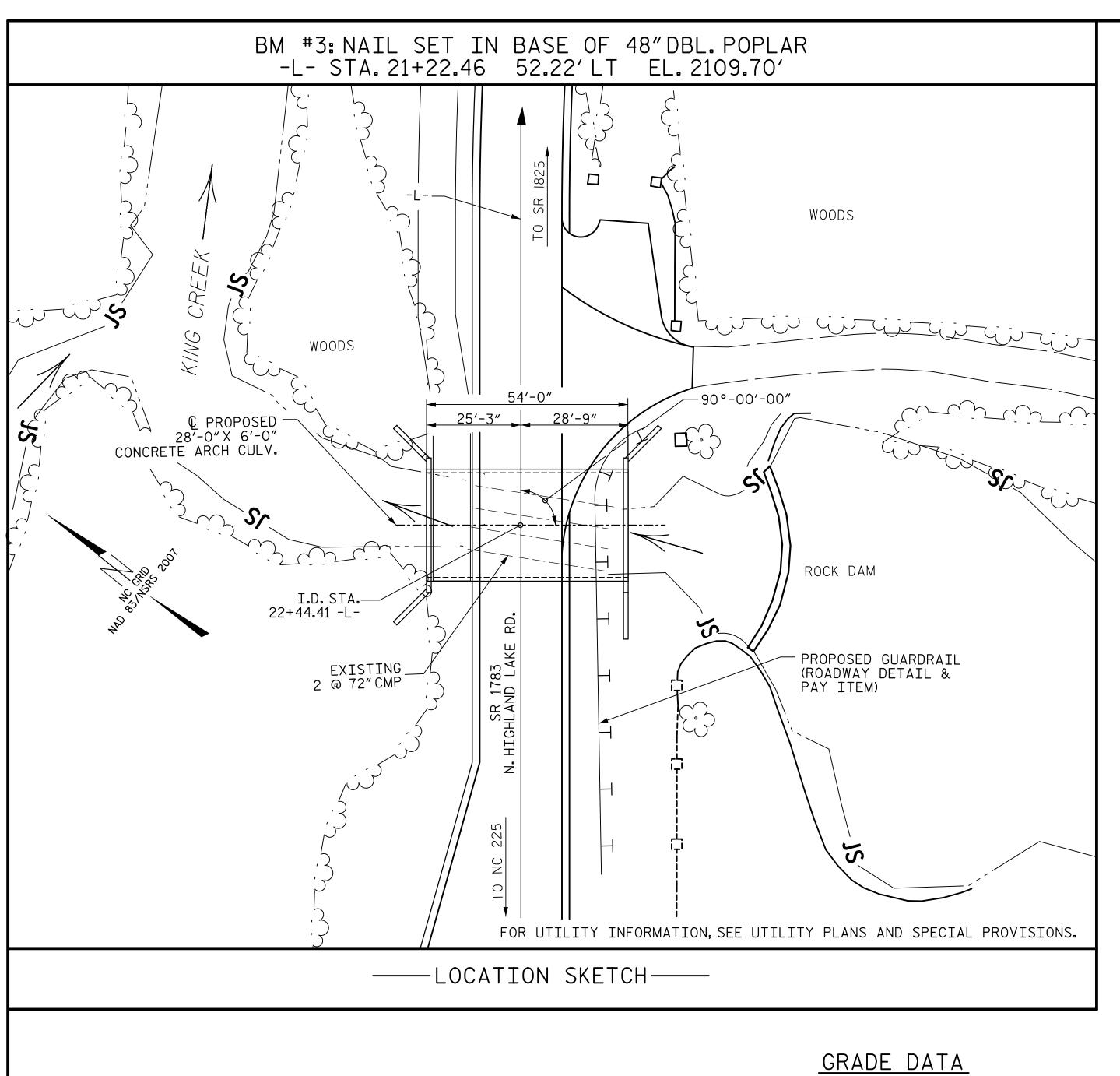
STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. **RALEIGH**, N.C. 27610

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

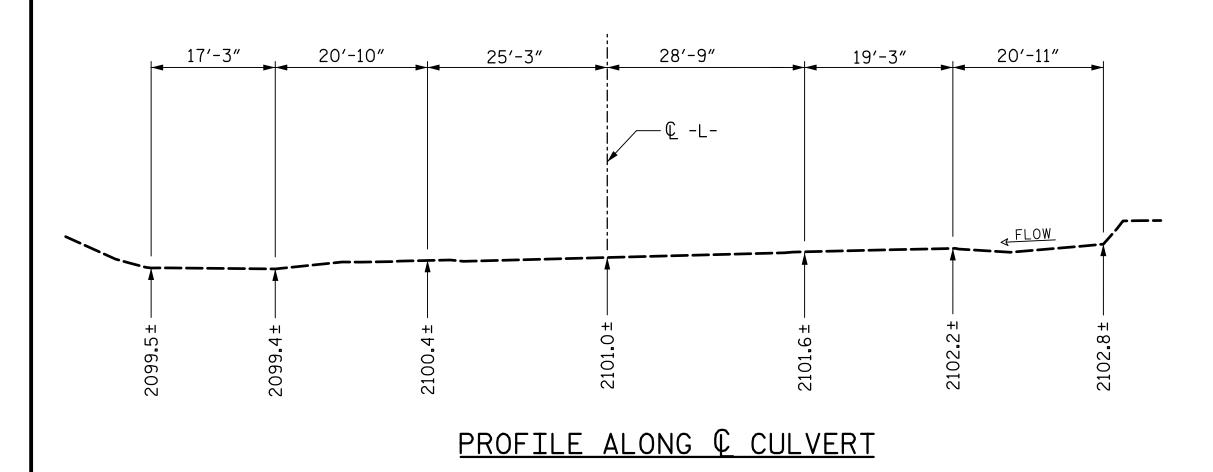
STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION FEDERAL HIGHWAY ADMINISTRATION

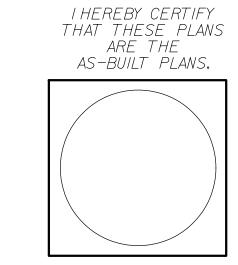
<u>APPROVED</u> **DIVISION ADMINISTRATOR**



GRADE POINT ELEV. @ STATION 22+44.41 = 2111.23 ± BED ELEV. @ STATION 22+44.41 = 2100.82 ± ROADWAY SLOPES 2:1



HYDRAULIC	DATA
DESIGN DISCHARGE	= 1000 CFS
DESIGN FREQUENCY	= 25 YRS
DESIGN HW ELEVATION	= 2108.1 FT
DRAINAGE AREA	= 3.8 SQ.MI.
BASE DISCHARGE	= 1500 CFS
BASE FREQUENCY	= 100 YRS
BASE HW ELEVATION	= 2111 . 2 FT
OVERTOPPING DISCHARGE	= 1350 CFS
OVERTOPPING FREQUENCY	= 50+ YRS
OVERTOPPING ELEVATION	= 2110.8 FT



8 EA.

NOTES

ASSUMED LIVE LOAD ---- HL-93 OR ALTERNATE LOADING.

DESIGN FILL ---- MAX.=4.83' MIN.=4.08'

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTES SHEET.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

CAST-IN-PLACE CONCRETE SHALL BE POURED IN THE FOLLOWING 1. FOOTINGS.

- 2. HEADWALL FOOTINGS, WALLS AND WINGS FULL HEIGHT, EXCEPT FOR RAIL PARAPET PORTION OF OUTLET HEADWALL.
- 3. RAIL PARAPET ON OUTLET HEADWALL.
- 4. A 3'-0"STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENDING LENGTH OF THE EXPANSION JOINT.
- FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC MANAGEMENT PLANS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

1801.01 FOR STANDARD TEMPORARY SHORING.

PILE DRIVING EQUIPMENT SETUP

FOR HP 12X53 STEEL PILES

- FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.
- FOR PILES SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. TEMPORARY SHORING MAY BE REQUIRED. SEE STANDARD DRAWING NO.
- THE EXISTING STRUCTURE, CONSISTING OF TWO LINES OF 72"CMP

ARCHITECTURAL SURFACE TREATMENT IS REQUIRED FOR THE EXPOSED FACES OF THE WINGWALLS, THE FRONT FACE OF HEADWALLS, INTERIOR AND EXTERIOR FACES OF THE CONCRETE PARAPET AND END POSTS.

AND LOCATED AT THE PROPOSED STRUCTURE, SHALL BE REMOVED. ARE RECOMMENDED. FOR ARCHITECTURAL SURFACE TREATMENT, SEE SPECIAL PROVISIONS.

TOTAL CULVERT QUANTITIES REMOVAL OF EXISTING STRUCTURE AT STA. 22+44.41 -L-LUMP SUM PRECAST REINFORCED CONCRETE THREE SIDED CULVERT AT STA. 22+44.41 -L-LUMP SUM 64 LIN. FT. PILE EXCAVATION IN SOIL PILE EXCAVATION NOT IN SOIL 80 LIN.FT. UNCLASSIFIED STRUCTURE EXCAVATION LUMP SUM AT STATION 22+44.41 -L-CLASS 'A' CONCRETE 175.4 CU. YDS. REINFORCING STEEL 16,844 LBS. HP12x53 STEEL PILES 152 LIN.FT. ANODIZED TWO BAR METAL RAIL 28.5 LIN. FT. 18"STEEL SHEET PILES 440 SQ.FT. 629 CU. YDS. FOUNDATION EXCAVATION ARCHITECTURAL SURFACE TREATMENT 1274 SQ. FT. EPOXY COATED REINFORCING STEEL 396 LBS. 36.0 LIN.FT. 1'-3" X 2'-6" CONCRETE PARAPET

FOR PRECAST REINFORCED THREE-SIDED CULVERT, SEE SPECIAL PROVISIONS. FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

FOOTING IS DESIGNED FOR ASSUMED PRECAST CULVERT WALL THICKNESS OF 1'-O". HEADWALL IS DESIGNED FOR ASSUMED PRECAST CULVERT TOP SLAB THICKNESS OF 10" AT CROWN. ANY CHANGE IN DESIGN DIMENSIONS WILL REQUIRE ADJUSTMENT OF DETAILS AND REINFORCEMENT LENGTHS.

FOR 18"STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

(NOTES CONTINUED ON SHEET C-4)

FOUNDATION RECOMMENDATIONS:

THE SPREAD FOOTINGS ARE DESIGNED FOR A FACTORED RESISTANCE OF 4 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 9 TSF JUST BEFORE PLACING CONCRETE.

KEY SPREAD FOOTINGS AT LEAST 12" INTO WEATHERED ROCK or ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE PLANS.

THE SCOUR CRITICAL ELEVATION IS THE BOTTOM OF FOOTING. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

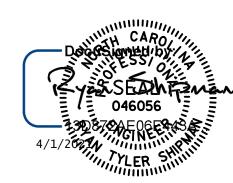
SPREAD FOOTINGS MUST BE PLACED ON NON-SCOURABLE ROCK. IF ADEQUATE MATERIAL IS NOT ENCOUNTERED AT THE PLAN BOTTOM OF FOOTING EXCAVATION, EXCAVATE DOWN AND 1-FOOT INTO NON-SCOURABLE ROCK.

IF THE TOP OF NON-SCOURABLE ROCK IS LOCATED AT A DEPTH GREATER THAN 3 FEET BELOW PLAN BOTTOM OF FOOTING ELEVATION, DRILLED-IN PILES

DRILLED-IN- PILES SHALL BE INSTALLED VERTICAL, WITH A CENTER -TO-CENTER SPACING NO GREATER THAN 5 FEET, AND WITH A PENETRATION OF AT LEAST 10 FEET INTO WEATHERED ROCK/CRYSTALLINE ROCK. FOR PILE EXCAVATION. SEE SECTION 450 OF THE STANDARD SPECIFICATION.

CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATIONS.

IF THE TOP OF NON-SCOURABLE ROCK IS LOCATED BELOW THE PLANNED BOTTOM OF FOOTING ELEVATION, PZ27 SHEETING SHALL BE USED TO PROVIDE SCOUR PROTECTION, SHEETING SHALL BE DRIVEN TO REFUSAL AND THE TOP CAST DIRECTLY INTO THE FOOTING CONCRETE, REFUSAL ELEVATIONS ARE EXPECTED TO BE VARIABLE, RANGING FROM APPROXIMATELY 2,108 FEET TO 2,088 FEET ALONG THE EAST FOOTING TO APPROXIMATELY 2,101 FEET TO 2,102 FEET ALONG THE WEST FOOTING.



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22+44.41 -L-STATION: NEW STRUCTURE 440394 SHEET 1 OF 21 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

U-5887

COUNTY

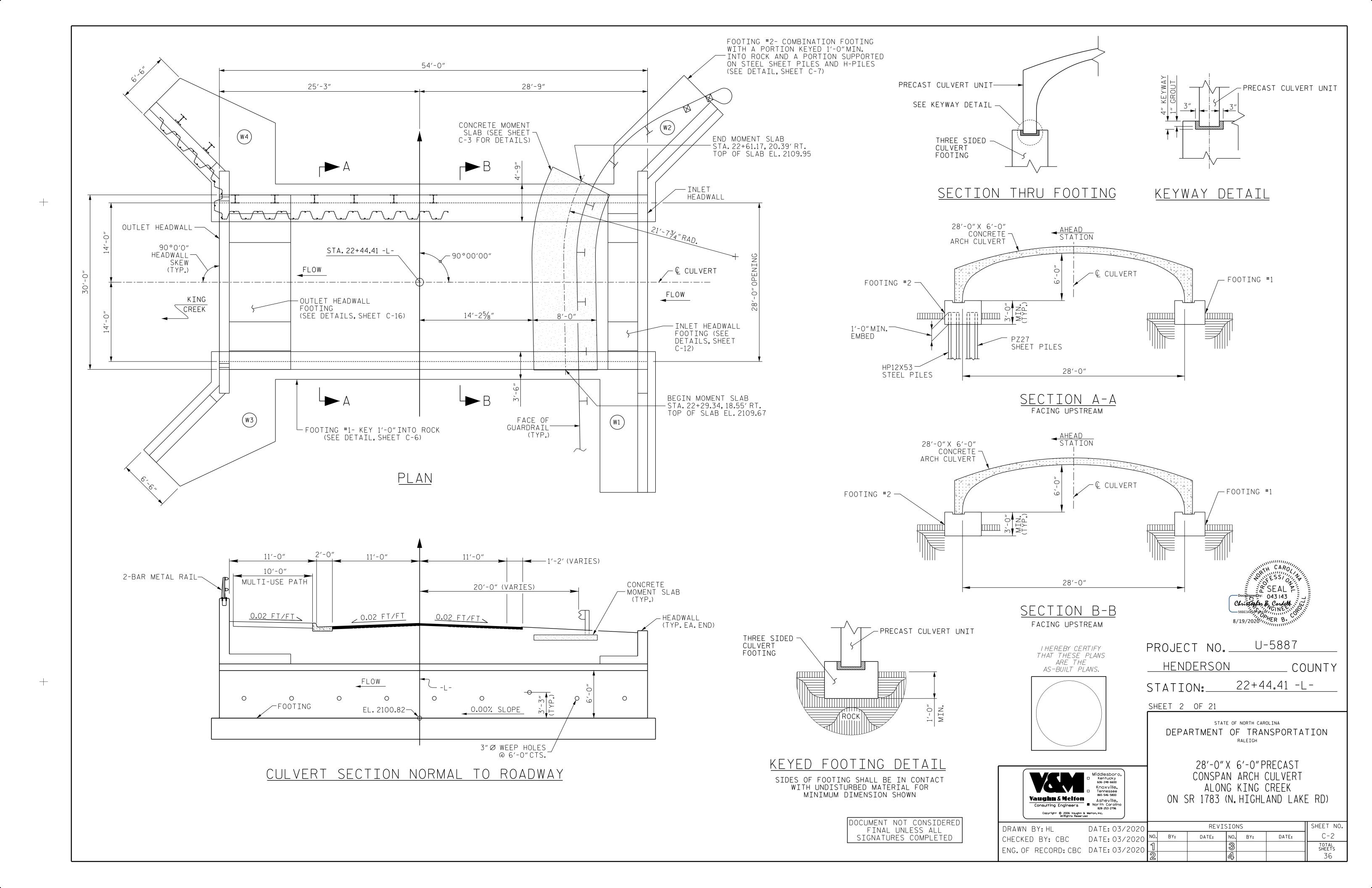
GENERAL DRAWING

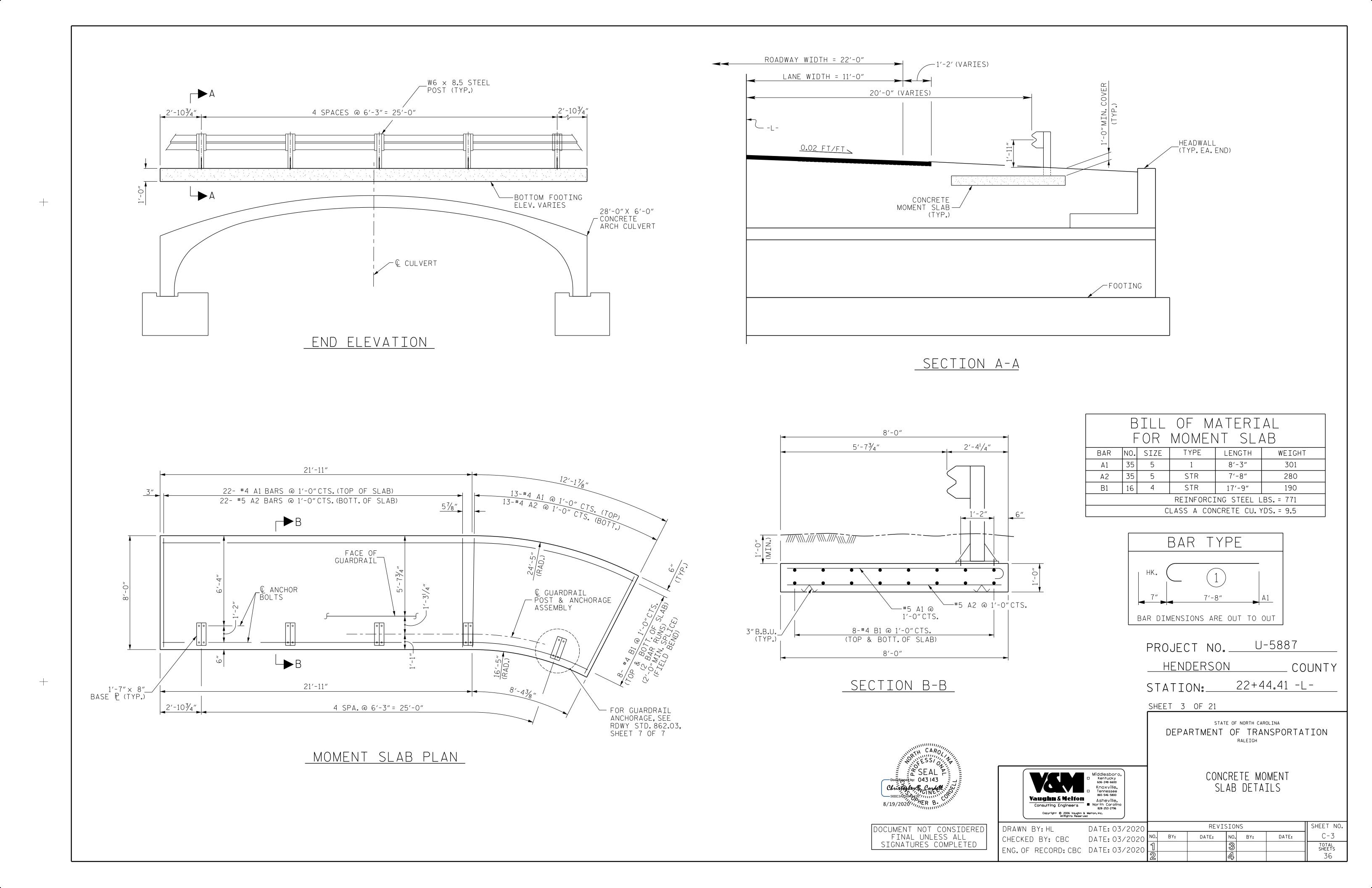
28'-0" X 6'-0" PRECAST CONCRETE ARCH CULVERT ALONG KING CREEK ON SR 1783 (N. HIGHLAND LAKE RD) 90° SKEW

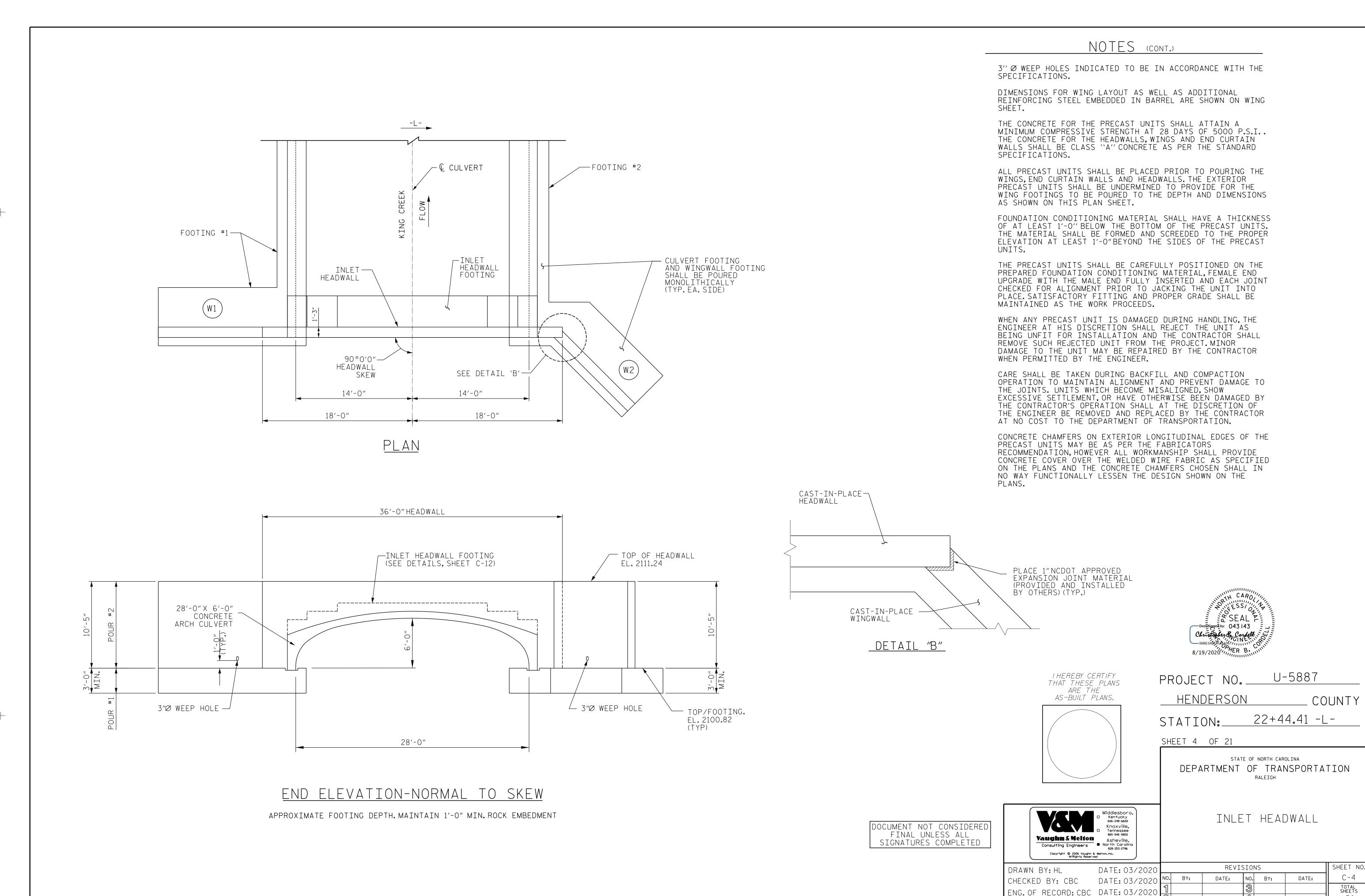
SHEET NO. REVISIONS DRAWN BY: HL DATE: 03/2020 C-1 DATE: 03/2020 DATE: BY: CHECKED BY: CBC TOTAL SHEETS ENG. OF RECORD: CBC DATE: 03/2020

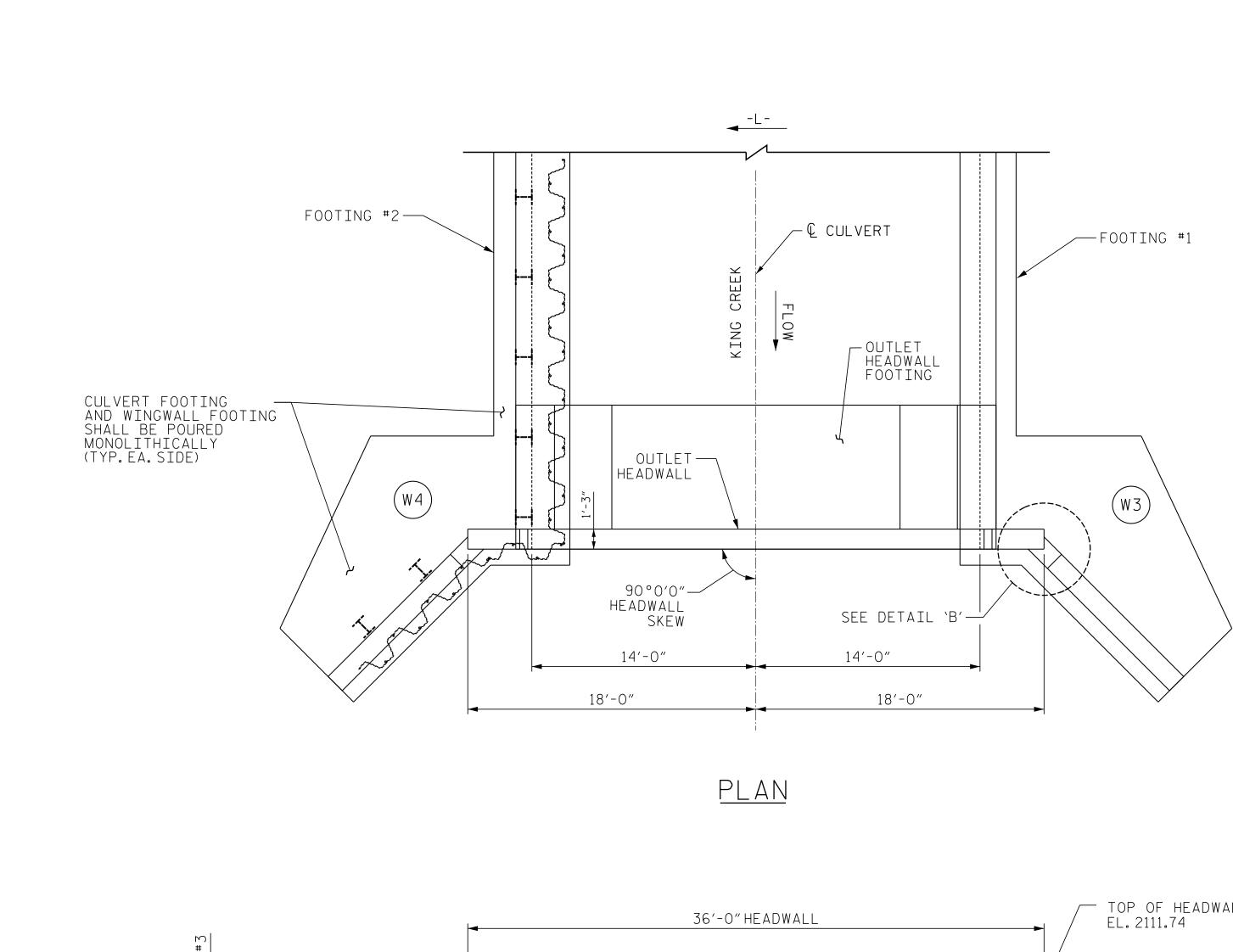
PROJECT NO. __

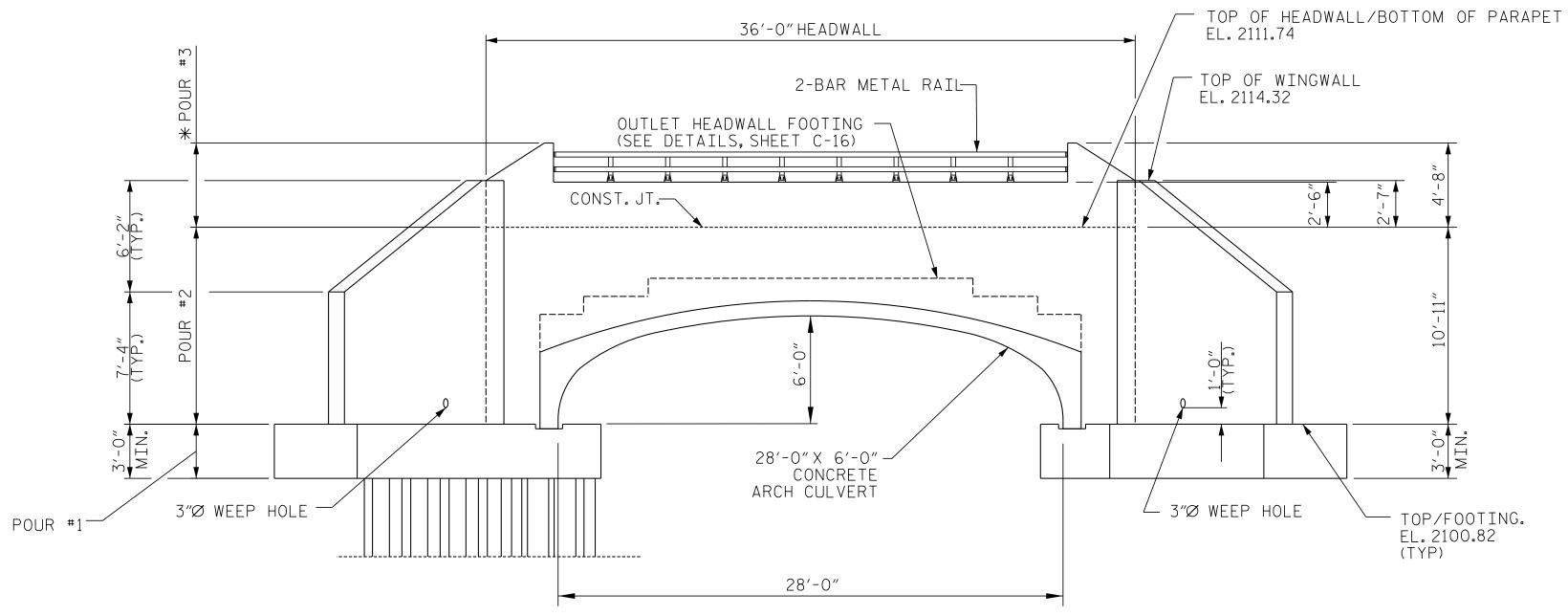
HENDERSON





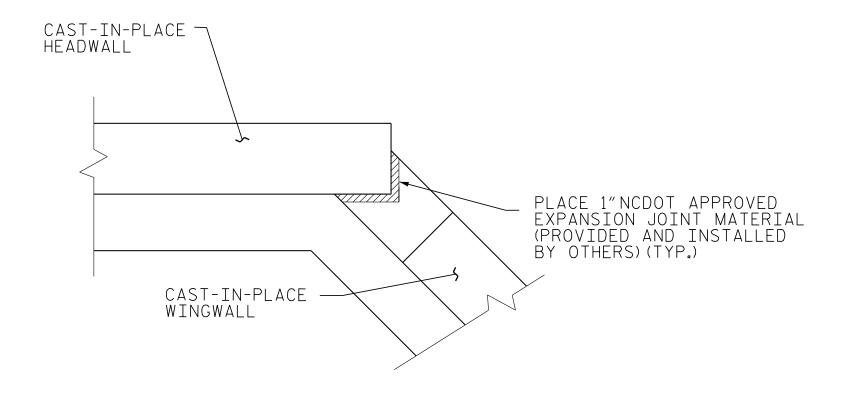






END ELEVATION-NORMAL TO SKEW

APPROXIMATE FOOTING DEPTH.MAINTAIN 1'-0" MIN.ROCK EMBEDMENT ** UPPER PORTION OF WINGS TO BE INCLUDED IN POUR 2



<u>DETAIL "B"</u>



I HEREBY CERTIFY
THAT THESE PLANS
ARE THE
AS-BUILT PLANS.

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606-248-6600
Knoxville,
Tennessee
865-546-5800
Asheville,
Consulting Engineers

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828-253-2796

PROJECT NO. U-5887

HENDERSON COUNTY

STATION: 22+44.41 -L-

SHEET 5 OF 21

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

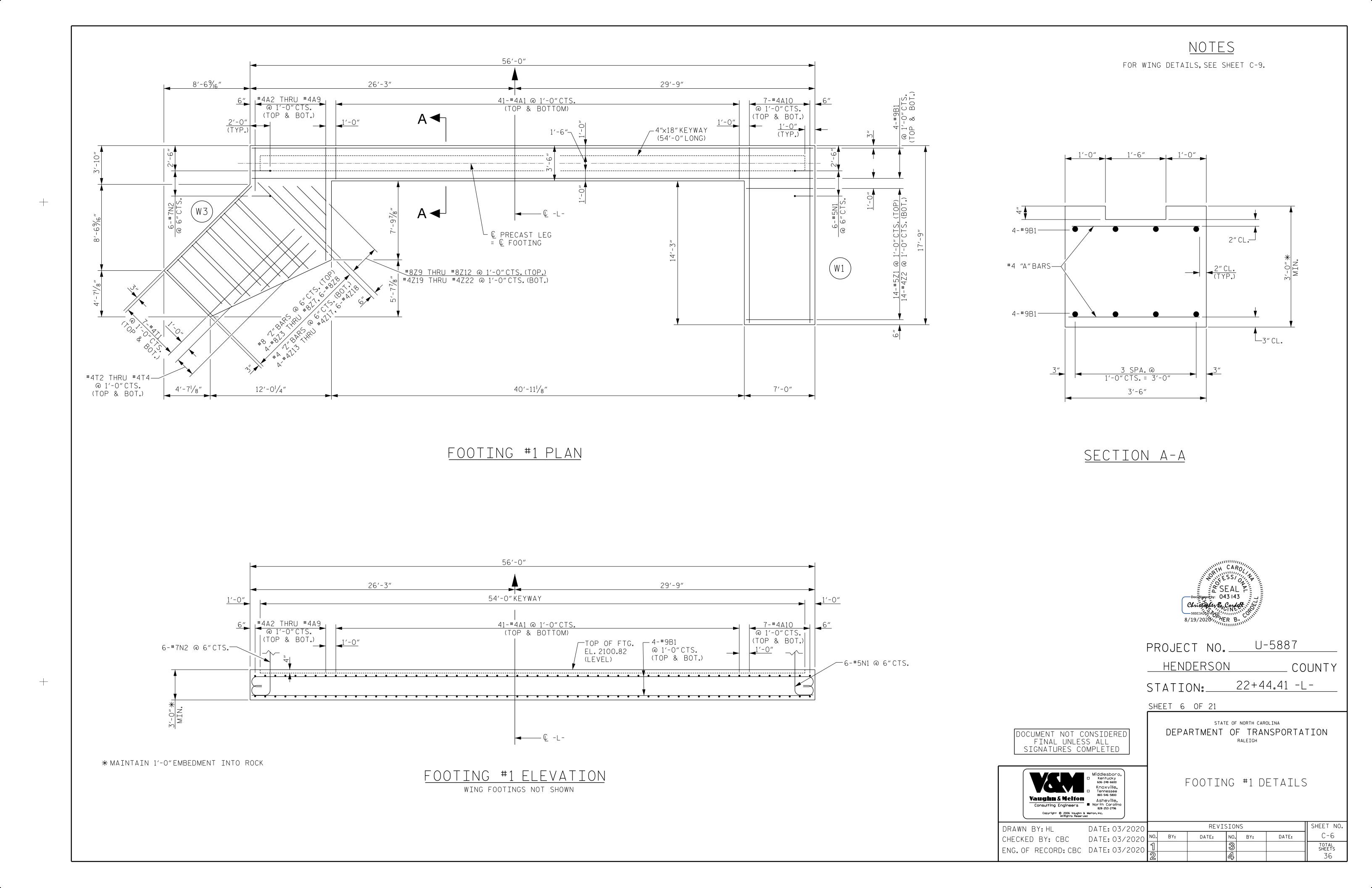
RALEIGH

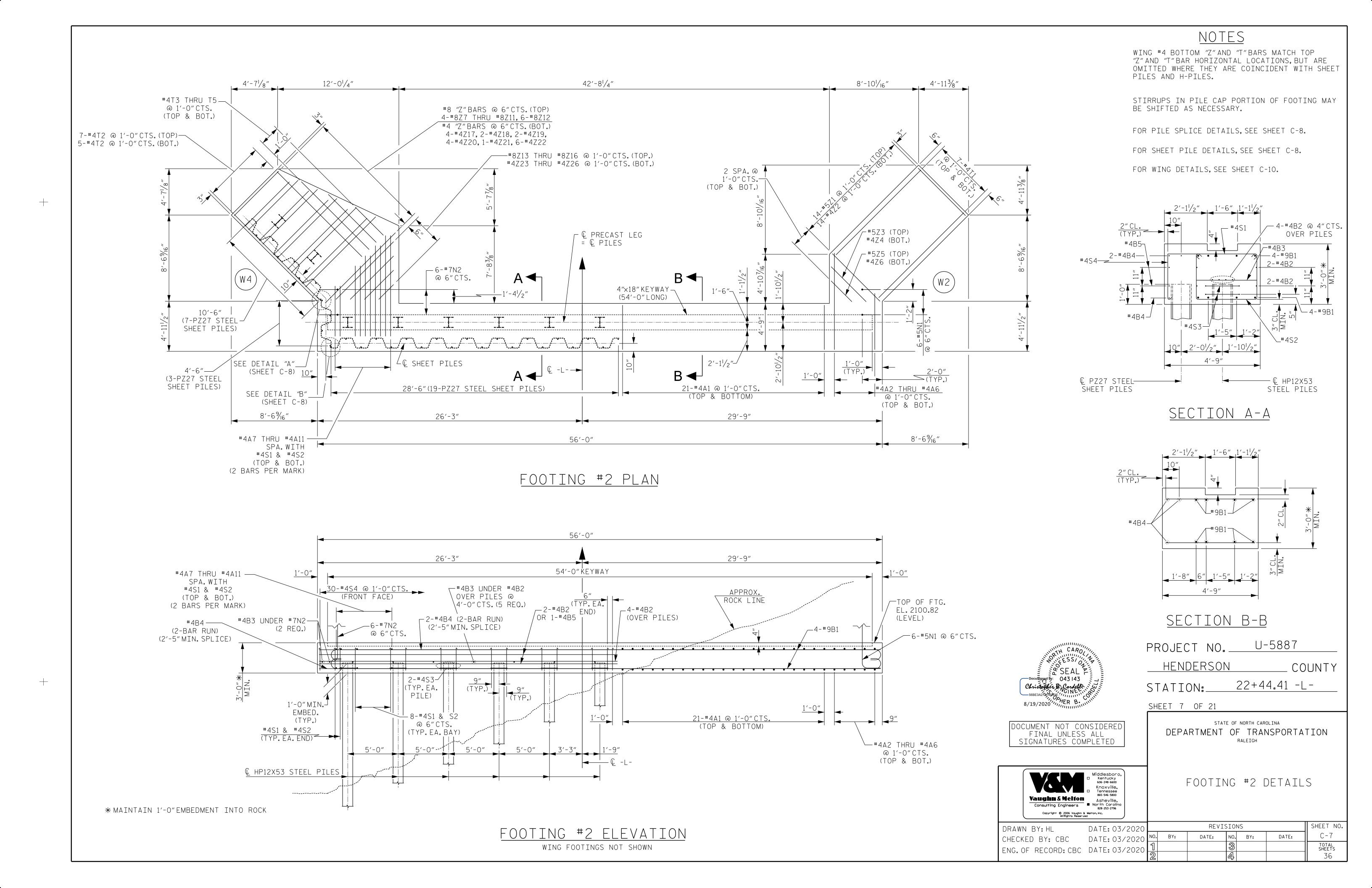
OUTLET HEADWALL

DRAWN BY: HL DATE: 03/2020 REVISIONS SHEET NO.

CHECKED BY: CBC DATE: 03/2020 NO. BY: DATE: NO. BY: DATE: C-5

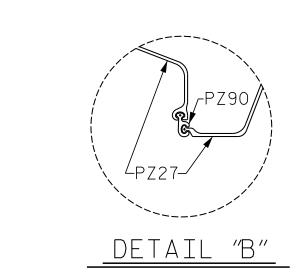
ENG. OF RECORD: CBC DATE: 03/2020 2 4 3 36

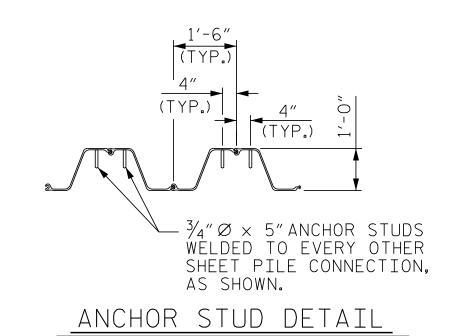




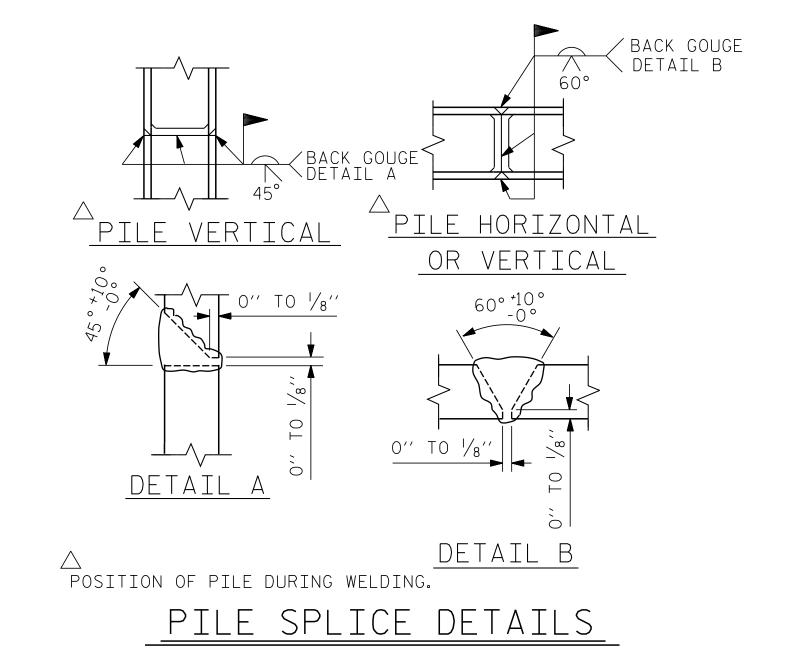


DETAIL "A"





SHEET PILE CONNECTION DETAILS





PROJECT NO. U-5887

HENDERSON COUNTY

STATION: 22+44.41 -L-

SHEET 8 OF 21

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DEPARTMENT OF TRANSPORTATION

RALEIGH

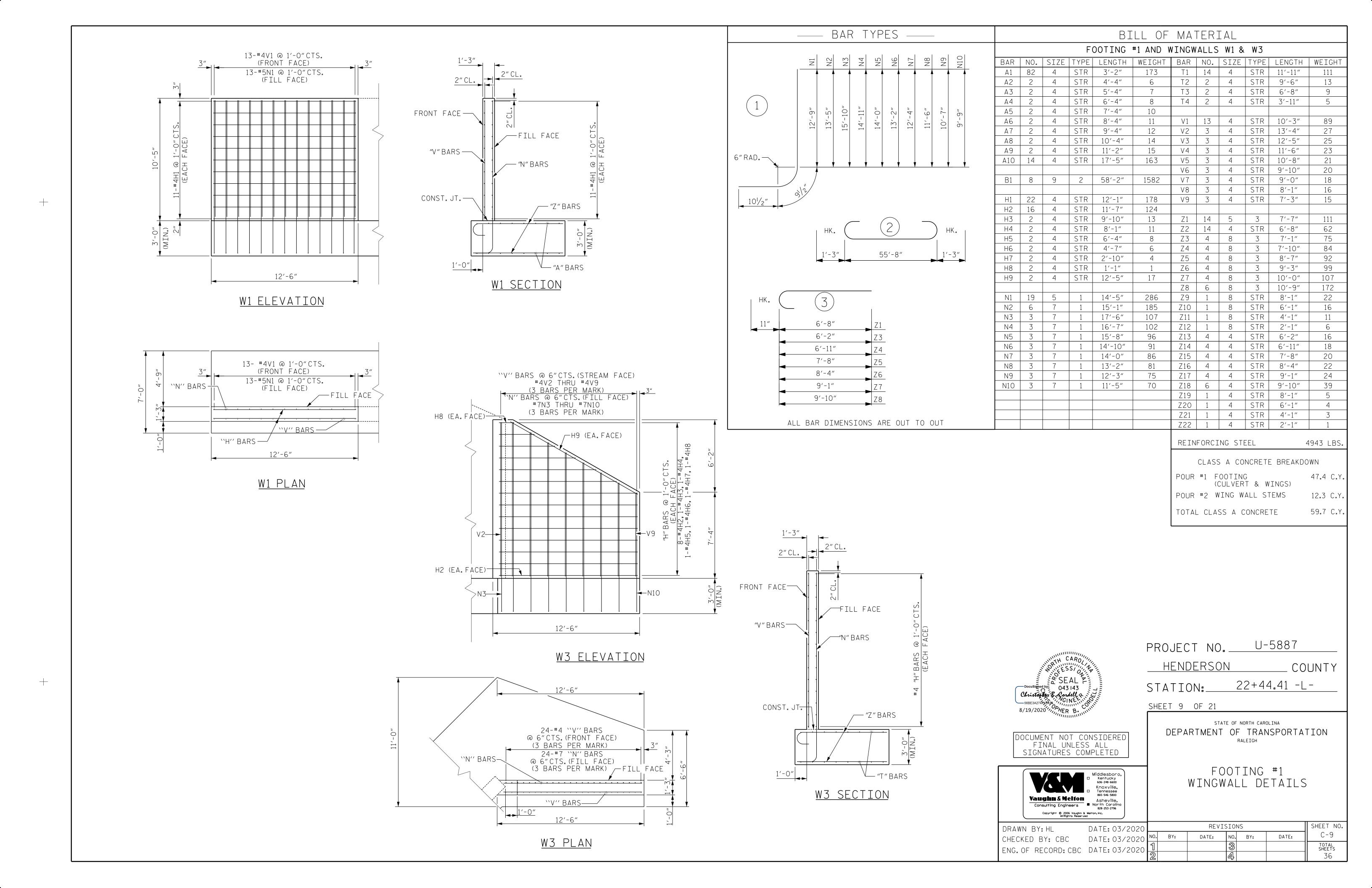
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Asheville,
North Carolina
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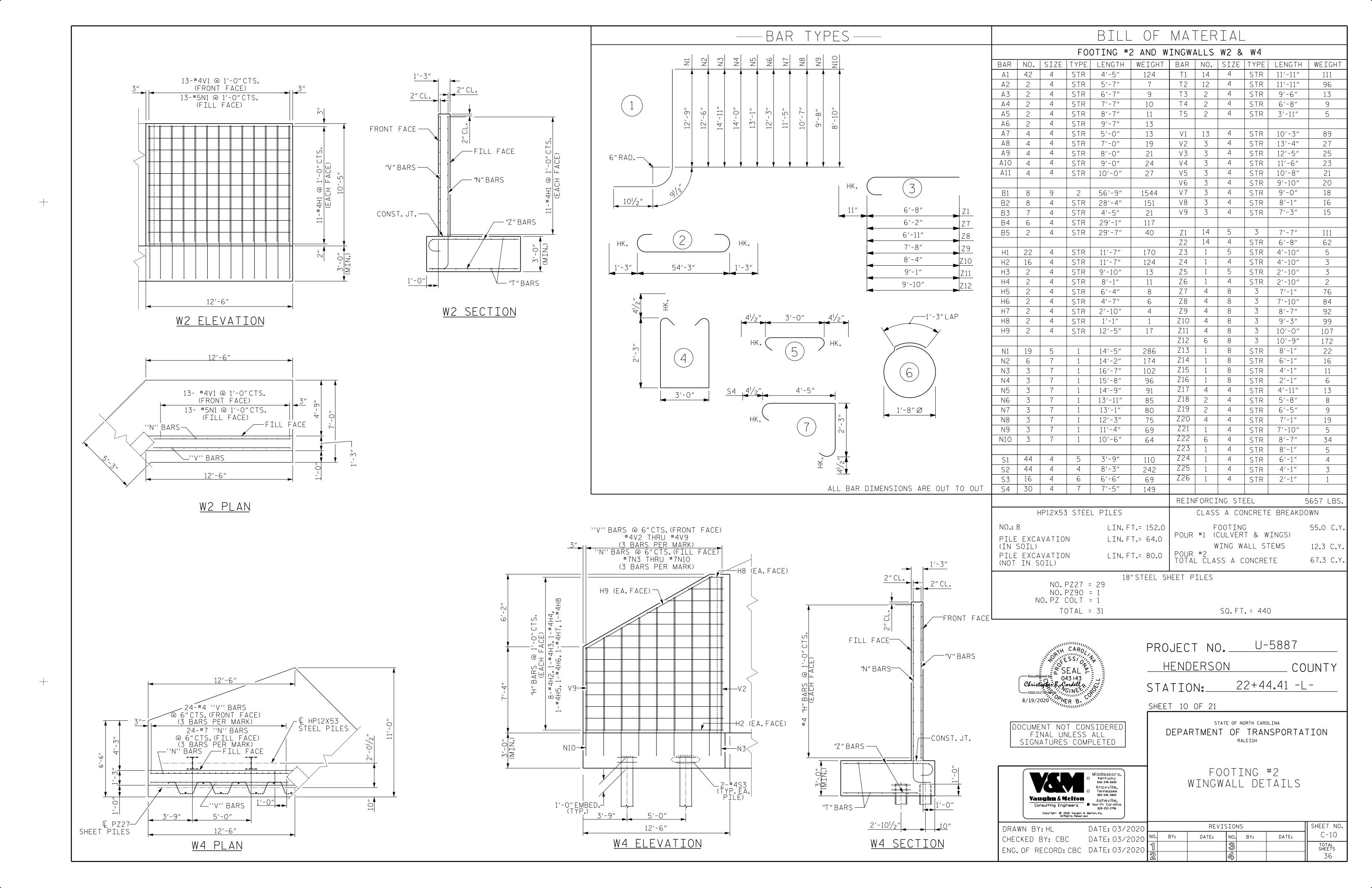
FOOTING #2 DETAILS

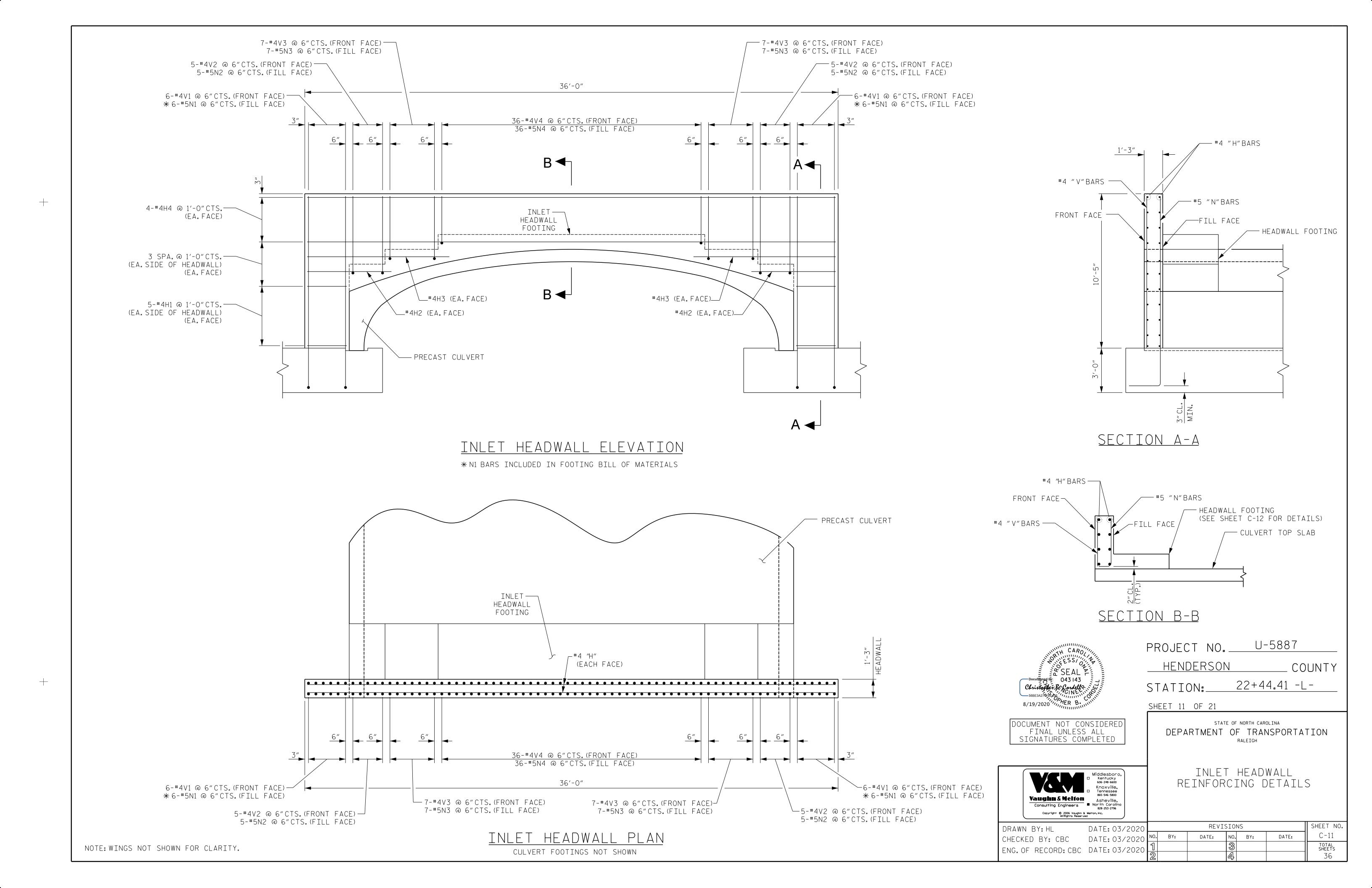
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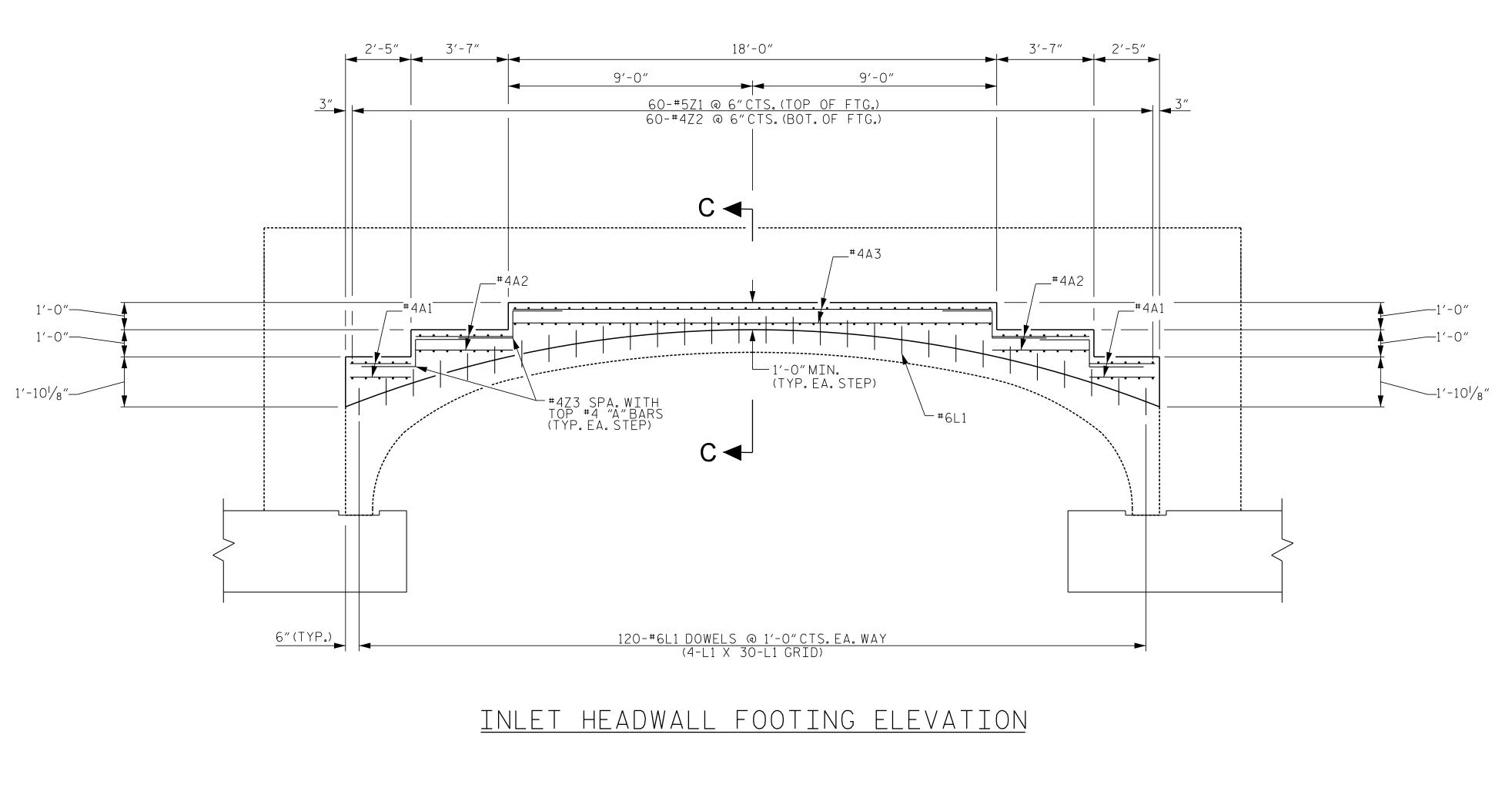
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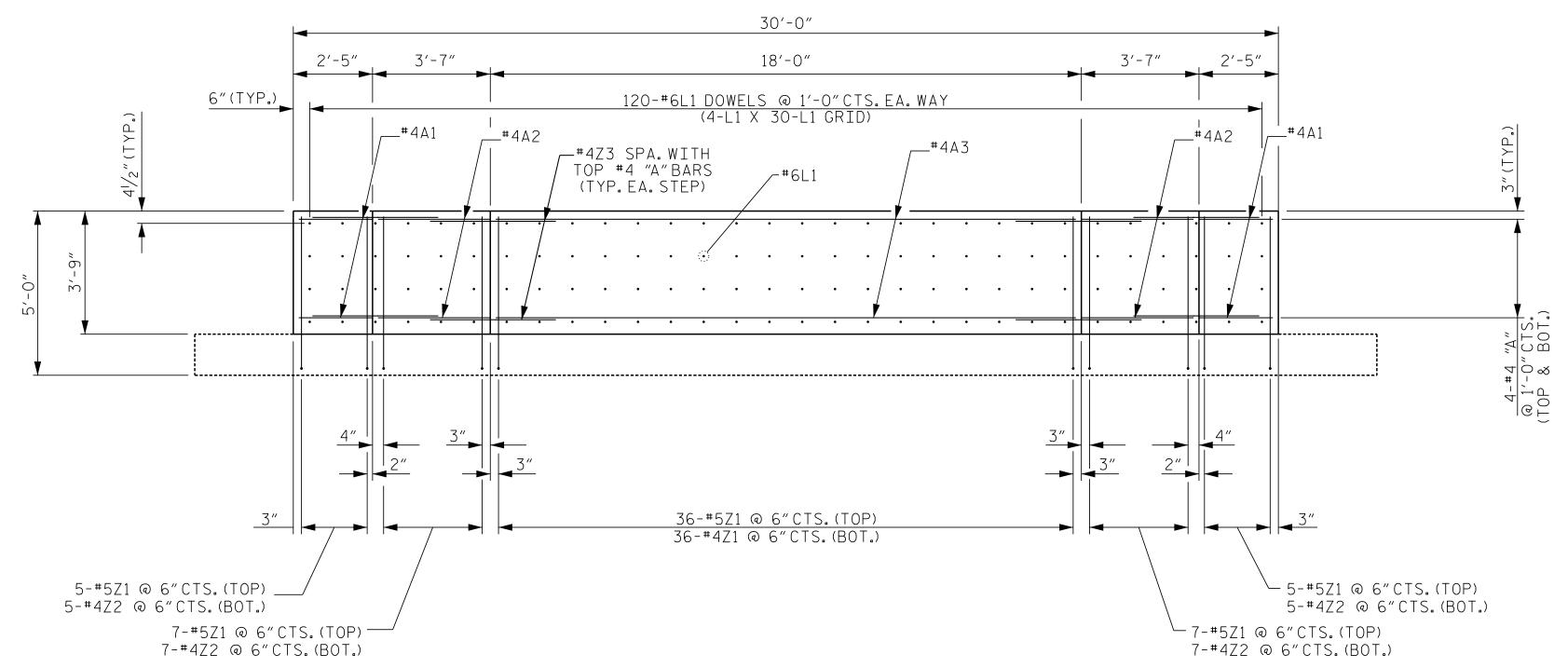
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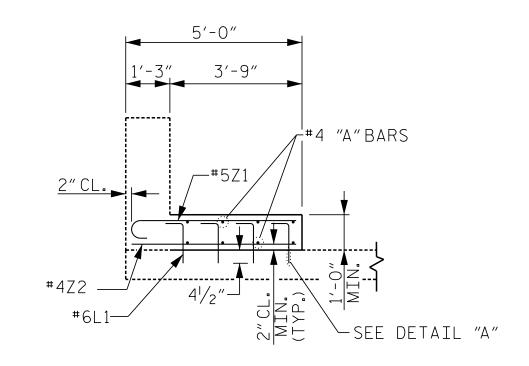




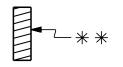




INLET HEADWALL FOOTING PLAN



<u>SECTION C-C</u>



* * STRUCTURAL CONNECTION INSERTS 2 STRUT OR EQUAL; LENGTH = $4 \frac{1}{2}$ ", INSERT WIDTH = 2", DIA. = $\frac{3}{4}$ ". 120 INSERTS REQ'D.

DETAIL "A"



U-5887 PROJECT NO.___ <u>HENDERSON</u> COUNTY

22+44.41 -L-STATION:_

SHEET 12 OF 21

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> INLET HEADWALL FOOTING DETAILS

Consulting Engineers North Carolina 828-253-2796

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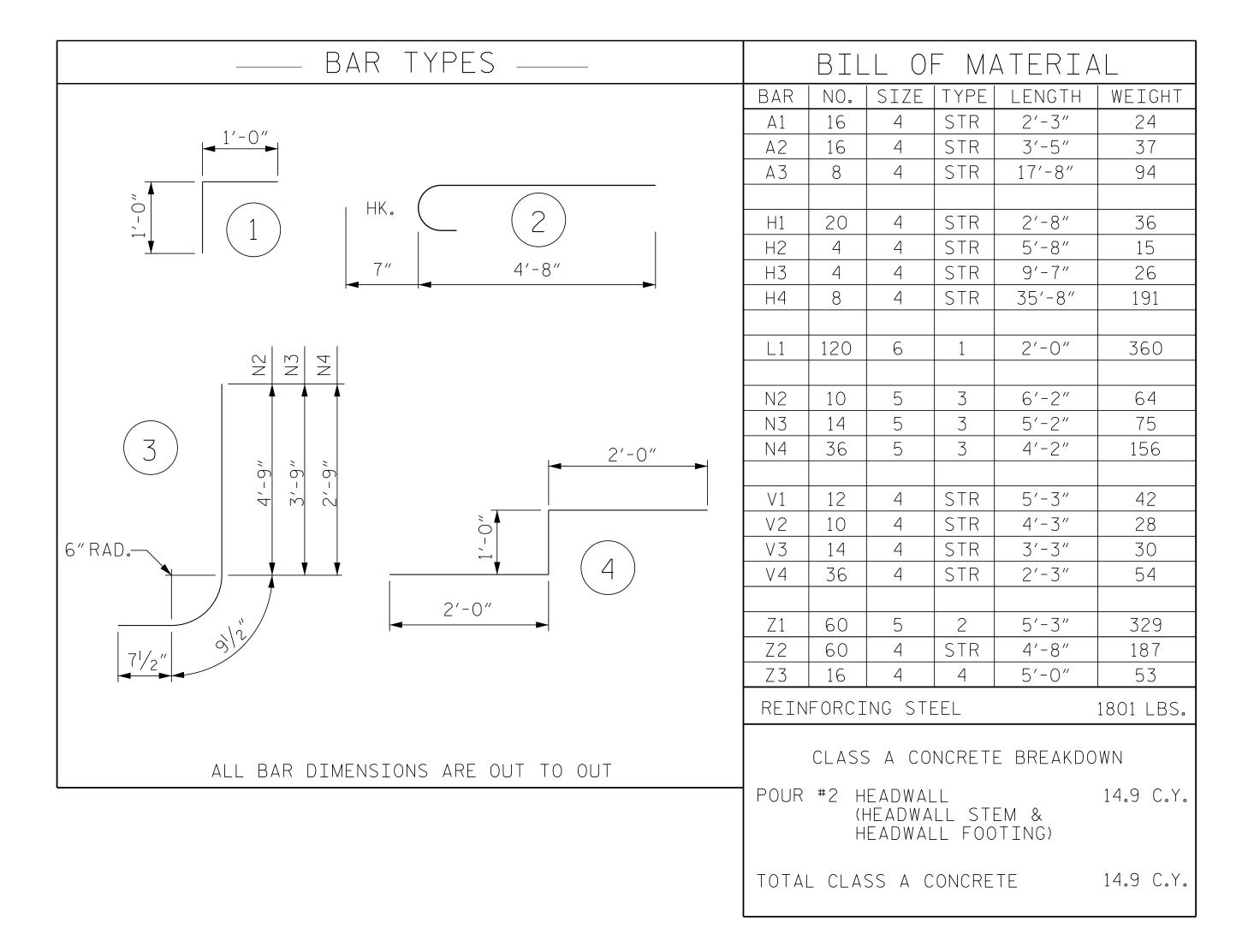
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REVISIONS DATE:

SHEET NO. C-12 DATE: BY: CHECKED BY: CBC DATE: 03/2020 TOTAL SHEETS ENG. OF RECORD: CBC DATE: 03/2020

NOTE: WINGS NOT SHOWN FOR CLARITY.





PROJECT NO. U-5887

<u>HENDERSON</u> ____ COUNTY

STATION: 22+44.41 -L-

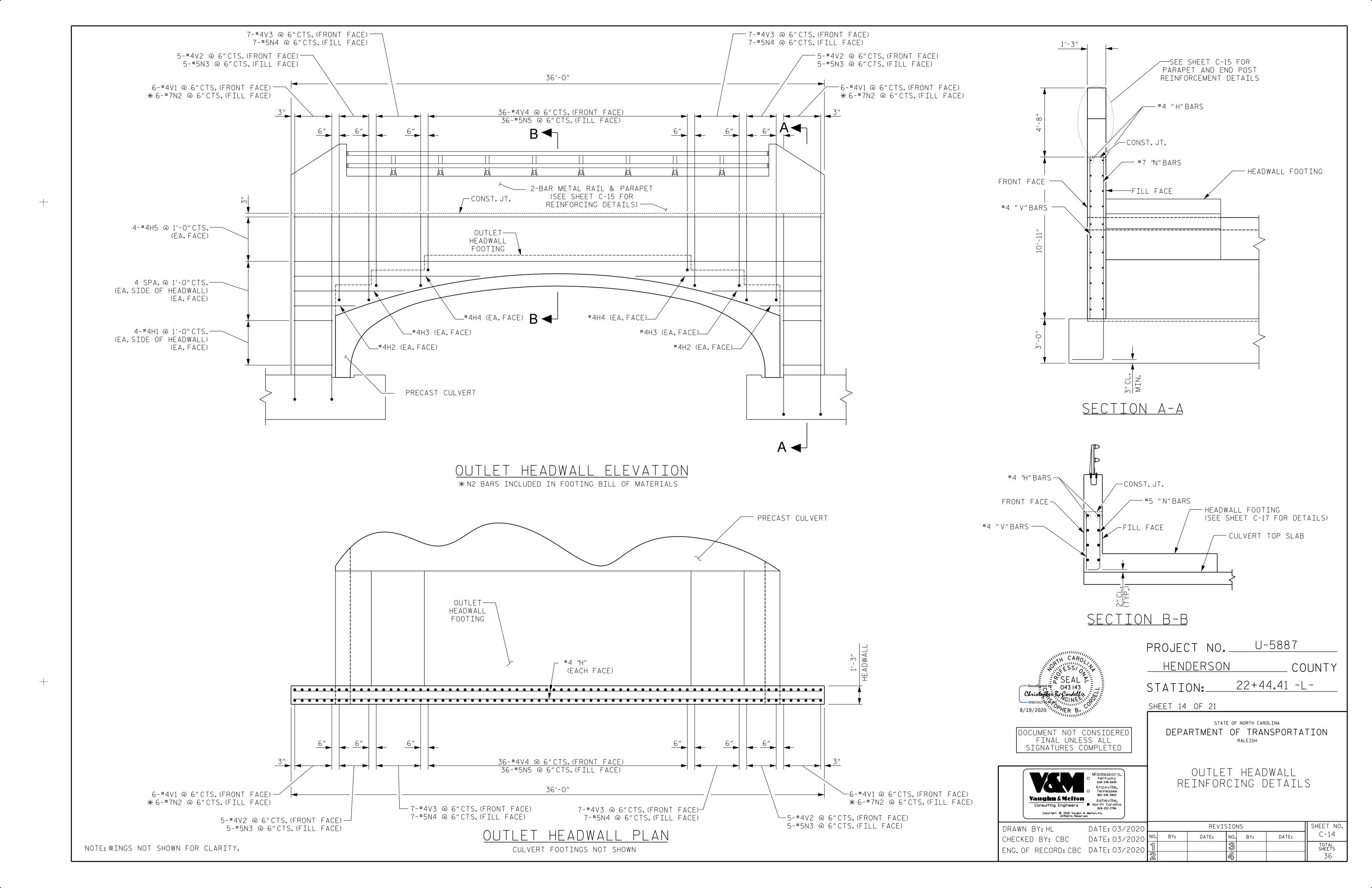
SHEET 13 OF 21

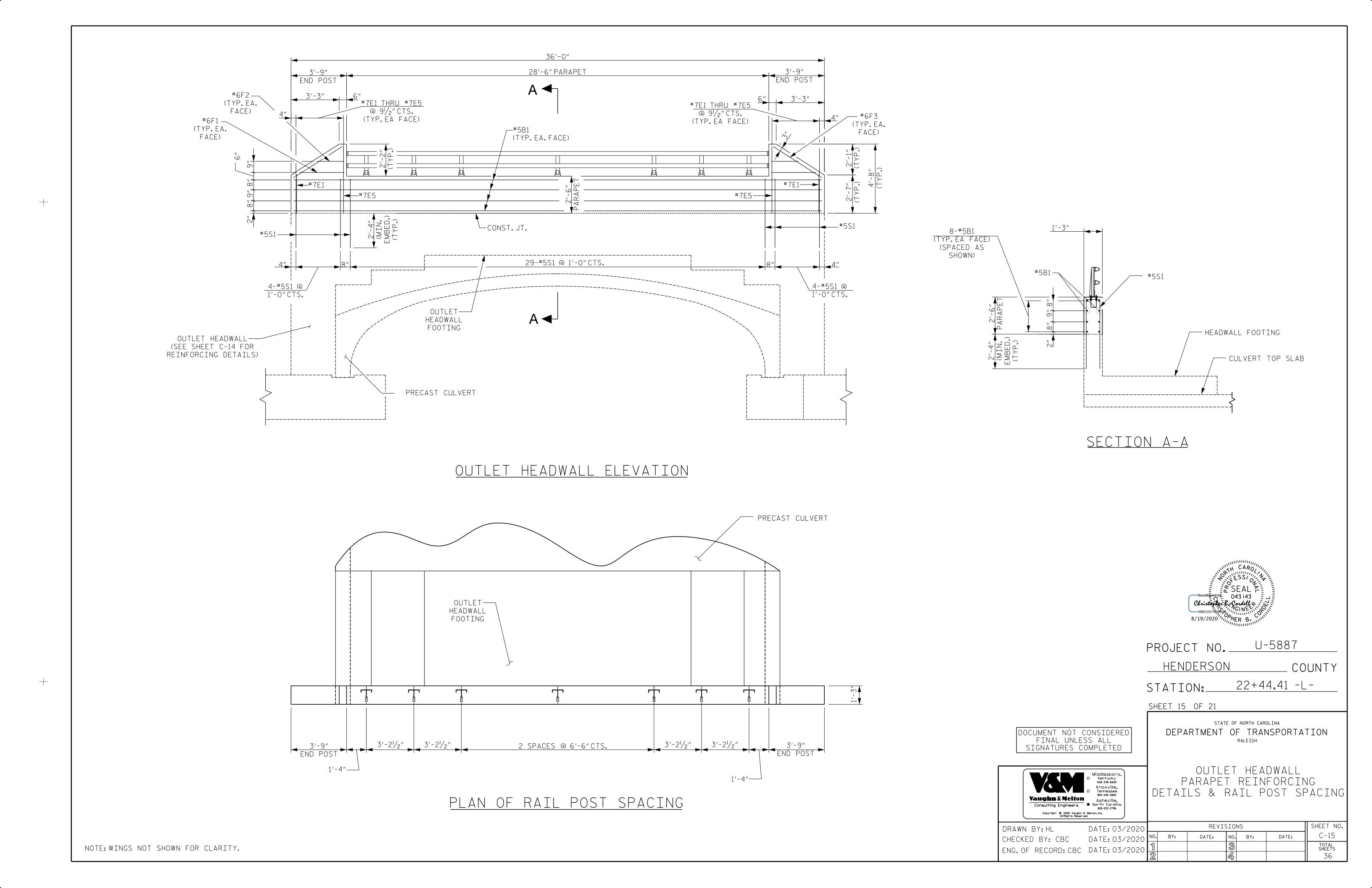
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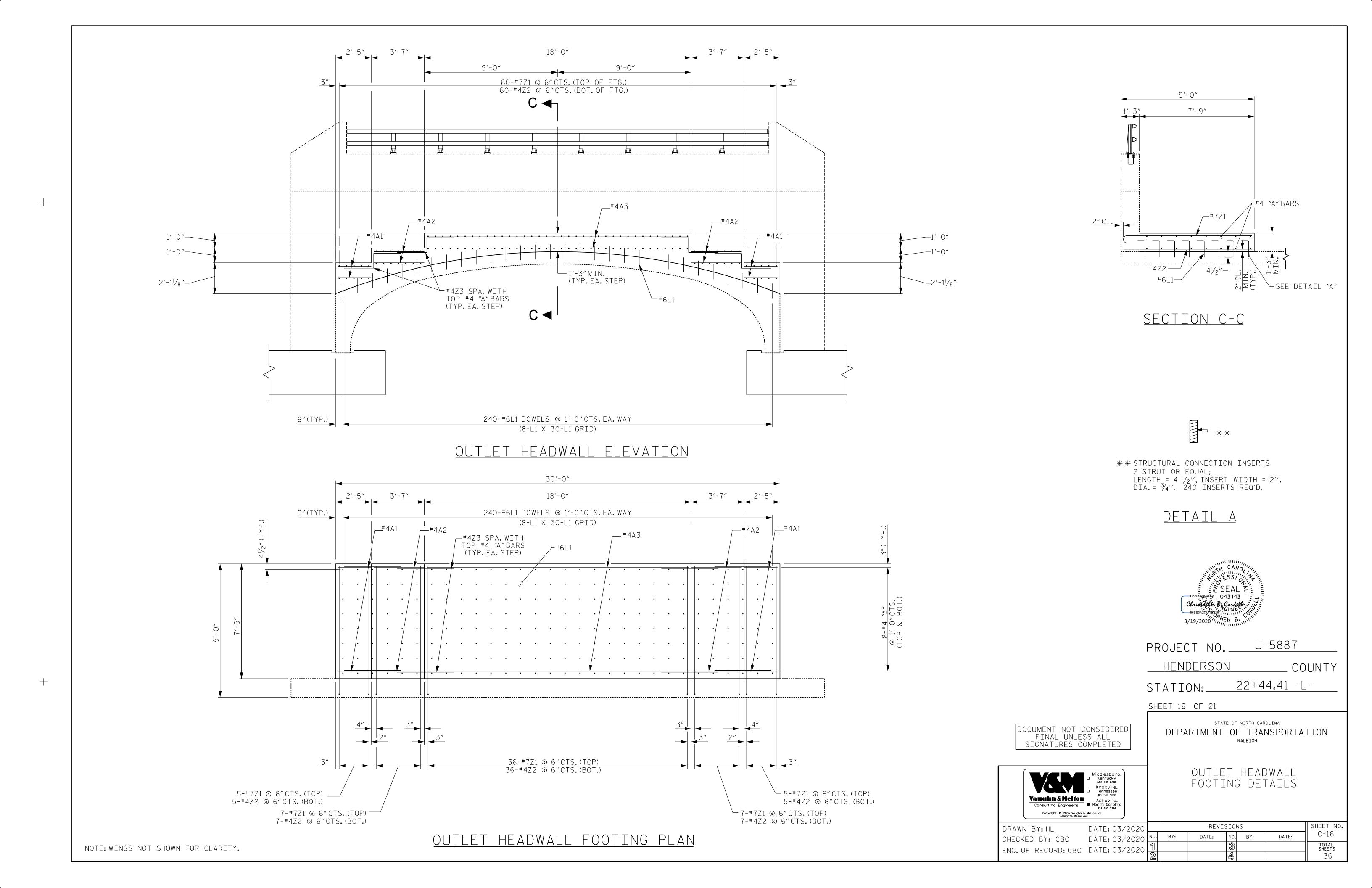
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

INLET HEADWALL AND HEADWALL FOOTING BILL OF MATERIAL

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DRAWN BY:HL	DATE: 03/2020			REVIS	10I2	NS		SHEET NO.
CHECKED BY: CBC		NO.	BY:	DATE:	NO.	BY:	DATE:	C-13
ENG. OF RECORD: CBC		1			3			TOTAL SHEETS
		2			<u> </u> 4],			II 36







NOTES

STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{1}{2}$ ".
- B. 1 $\frac{3}{4}$ '' \varnothing x 1 $\frac{5}{8}$ '' bolt with washer. Bolt shall conform to the requirements of astm a307. Bolt AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $34^{\prime\prime}$ Ø X $158^{\prime\prime}$ GALVANIZED BOLT AND WASHER.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\sqrt{6}$ WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

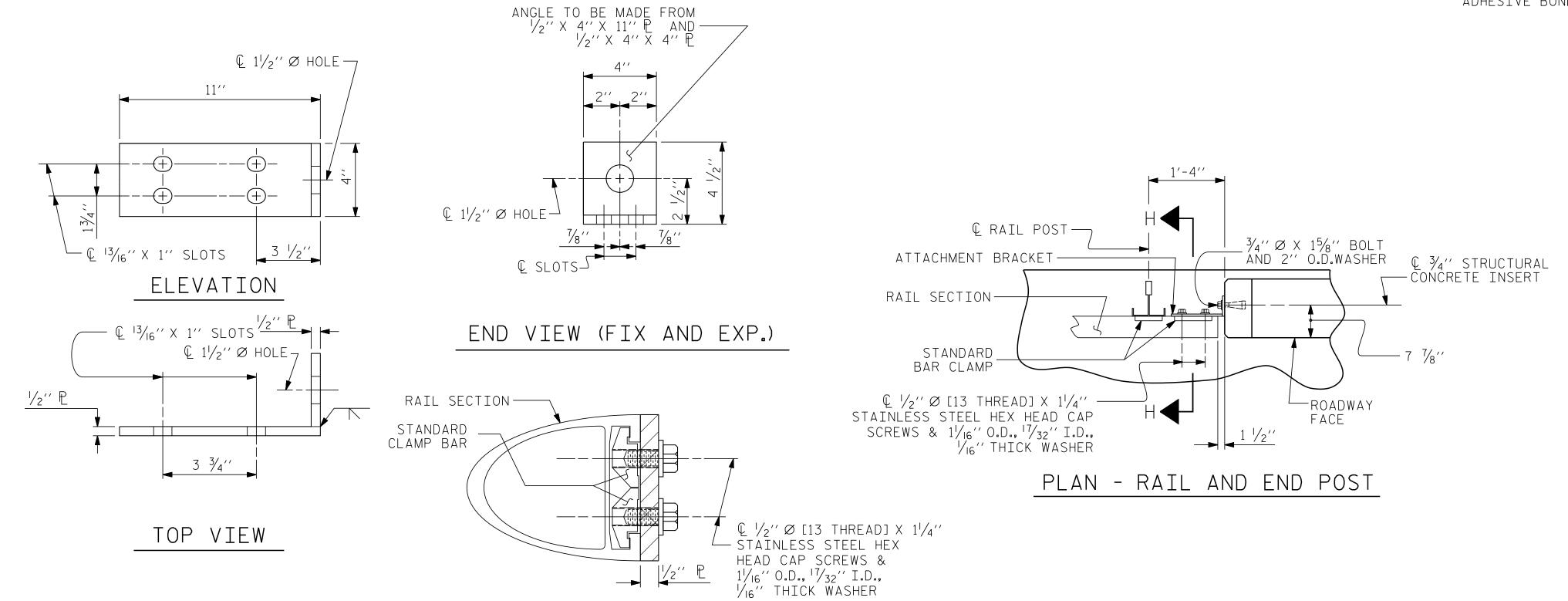
- A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. $\frac{3}{4}$ " structural concrete insert shall have a working load shear capacity of 4800 lbs. The FERRULES SHALL ENGAGE A $\frac{3}{4}$ ''Ø X 1 $\frac{5}{8}$ '' BOLT WITH 2''O.D.WASHER IN PLACE.THE $\frac{3}{4}$ ''Ø X 1 $\frac{5}{8}$ '' BOLT SHALL HAVE N.C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. $\frac{1}{2}$ " \varnothing PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " \emptyset X $1\frac{5}{8}$ " BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " $\frac{3}{4}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " \varnothing x 1 $\frac{5}{8}$ " bolt shall apply to the $\frac{3}{4}$ " \varnothing x 6 $\frac{1}{2}$ " bolt. Field testing of the ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



DETAILS FOR ATTACHING METAL RAIL TO END POST

SECTION H-H (FIX)

TLA/GM MAA/GM MAA/THC

DRAWN BY: FCJ 1/88 CHECKED BY: CRK 3/89

SEAL P

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HENDERSON COUNTY 22+44.41 -L-STATION: SHEET 17 OF 21 STATE OF NORTH CAROLINA

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

STANDARD RAIL POST SPACINGS

DEPARTMENT OF TRANSPORTATION

== AND ====== END OF RAIL DETAILS FOR ONE OR TWO BAR METAL RAILS

SHEET NO. REVISIONS DRAWN BY: HL DATE: 03/2020 C-17 DATE: DATE: 03/2020 DATE: CHECKED BY: CBC TOTAL SHEETS ENG. OF RECORD: CBC DATE: 03/2020

R.P.W.(TYP.ALL \

CONTACT POINTS)/

+

PLAN

WIRE STRUT

PROJECT NO.___

FERRULE-

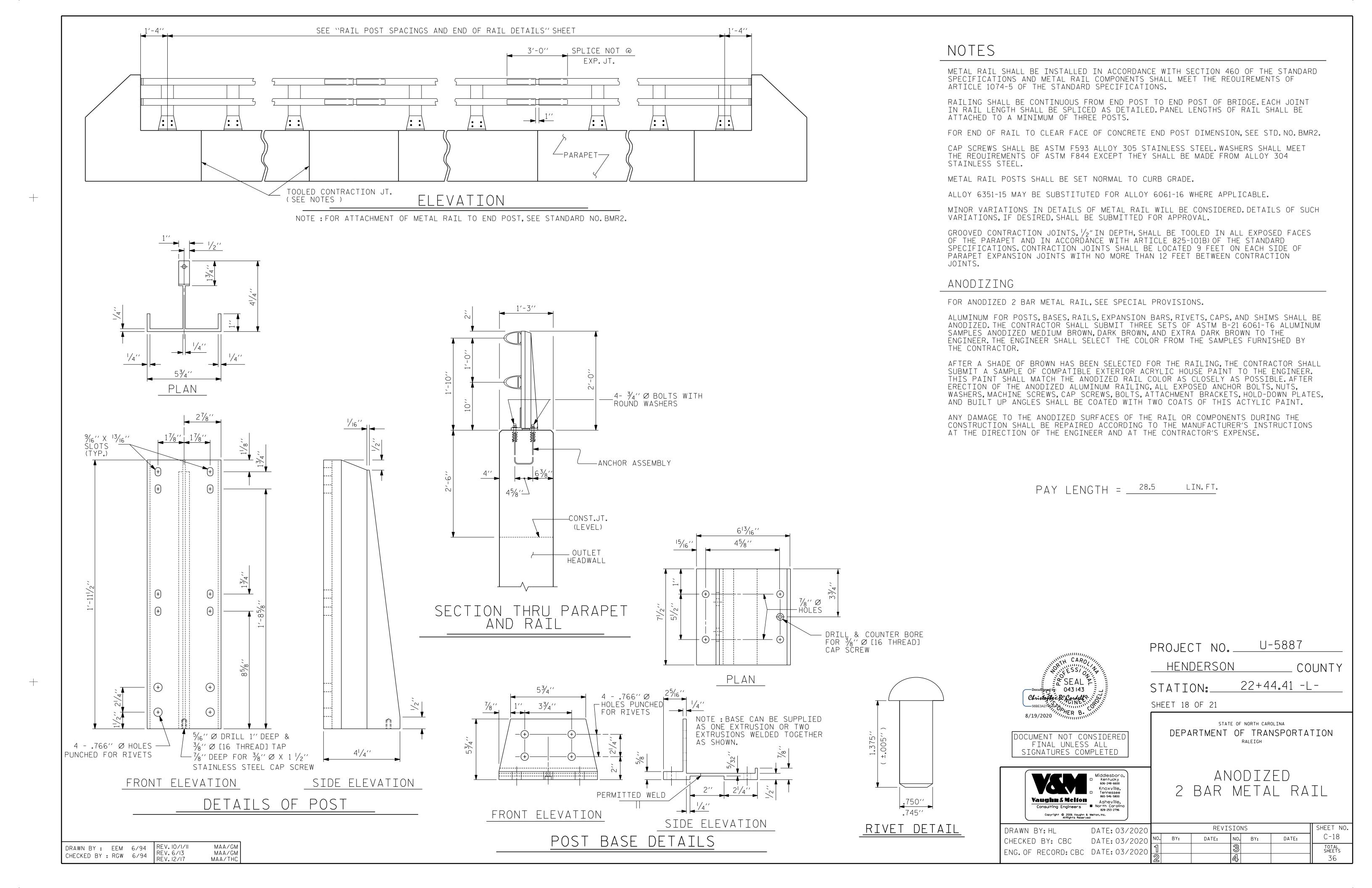
STD. NO. BMR2

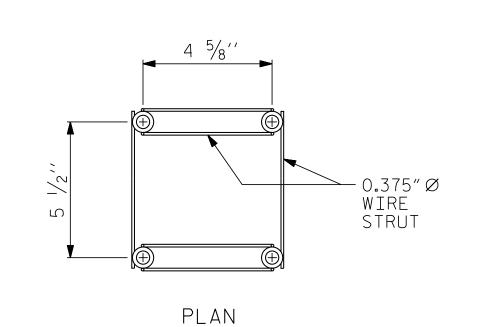
CLOSED-END

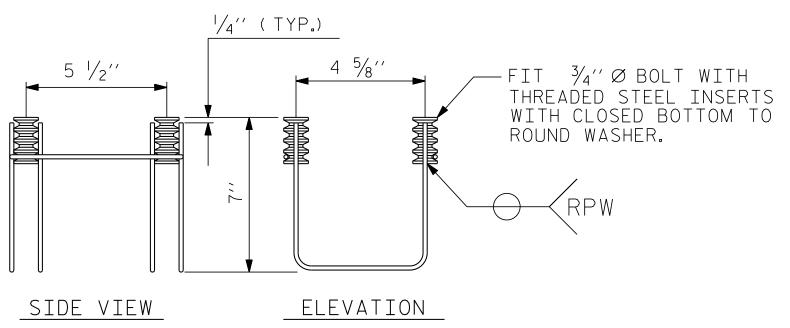
FERRULE

ELEVATION

U-5887







4-BOLT METAL RAIL ANCHOR ASSEMBLY

(7 ASSEMBLIES REQUIRED)

NOTES

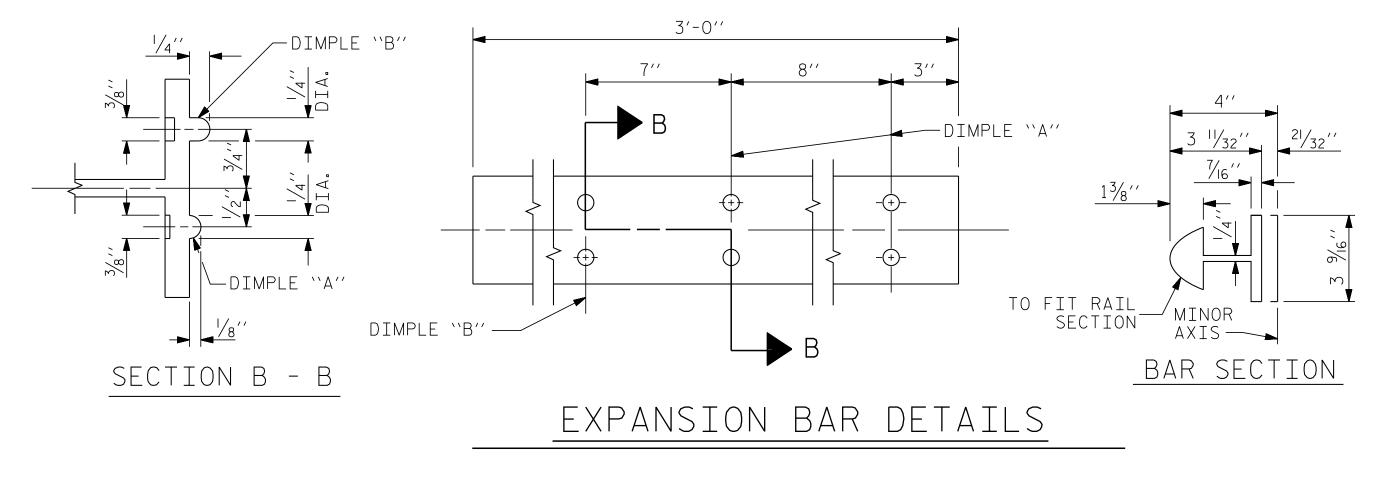
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

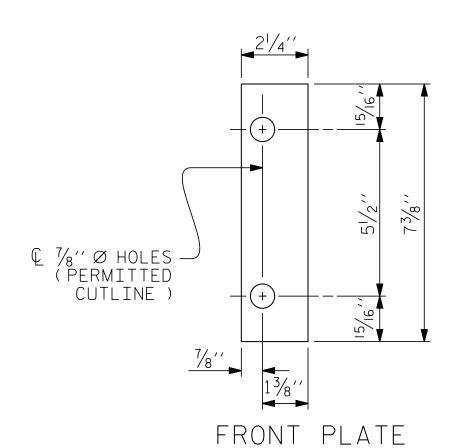
THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

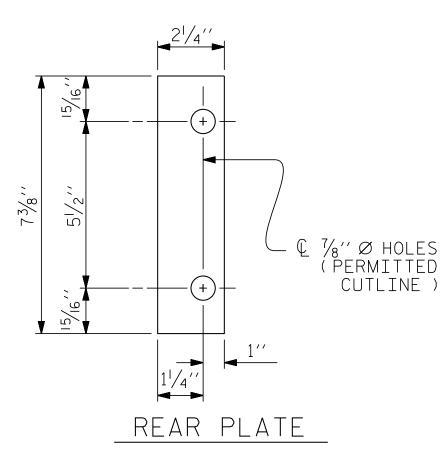
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 $\frac{3}{4}$ " \varnothing X 2 $\frac{1}{2}$ " BOLTS WITH WASHERS.BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " \varnothing X $2\frac{1}{2}$ " GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " \varnothing wire strut with a minimum tensile STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " \varnothing BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

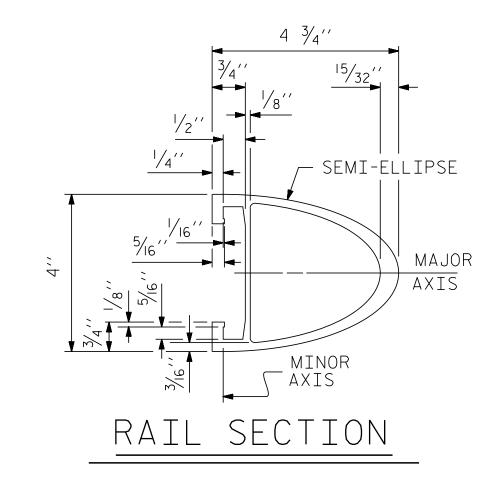
WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

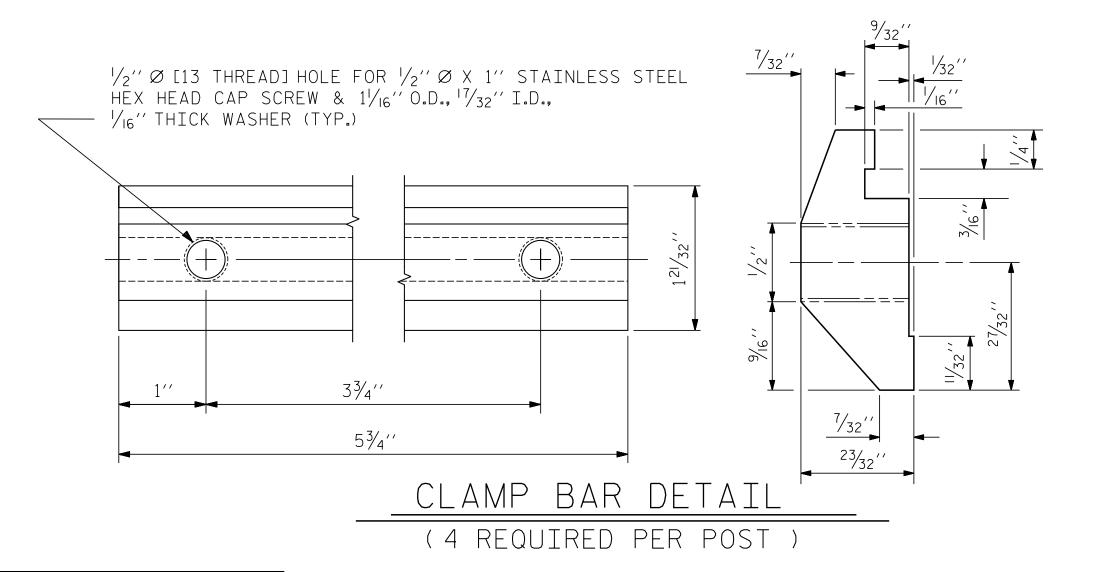


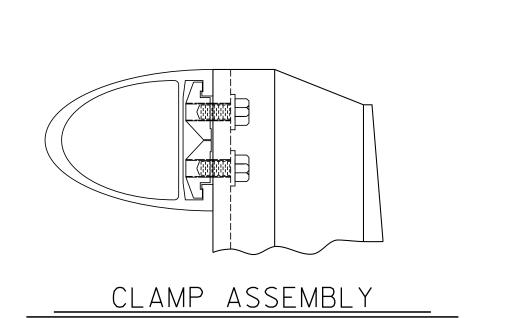


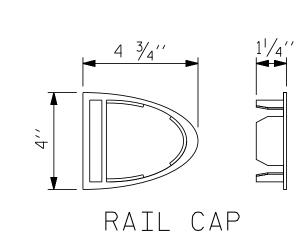


SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.









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Asheville,
North Caroline

043 143

U-5887 PROJECT NO._ HENDERSON COUNTY 22+44.41 -L-STATION:_ SHEET 19 OF 21

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

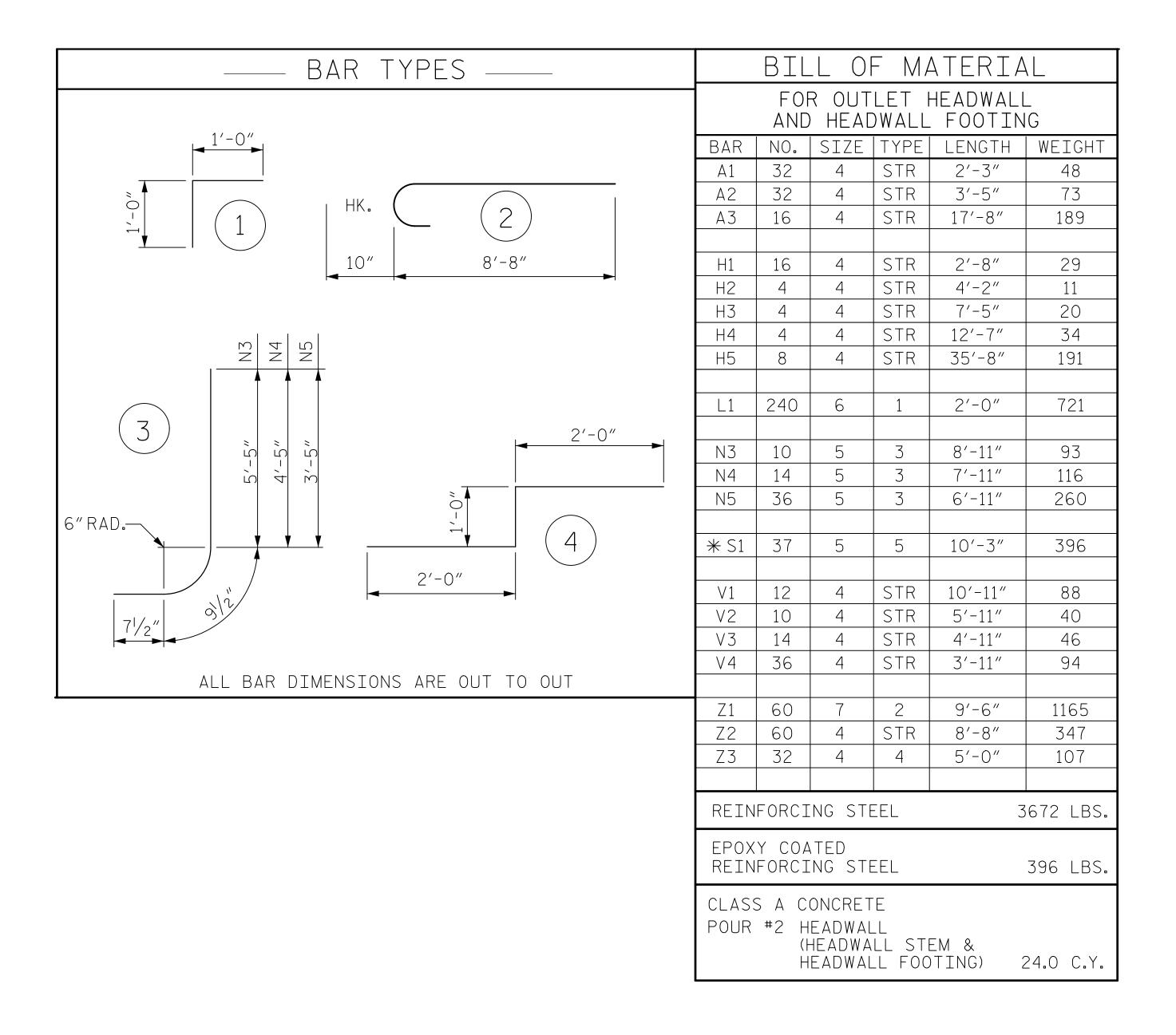
2 BAR METAL RAIL

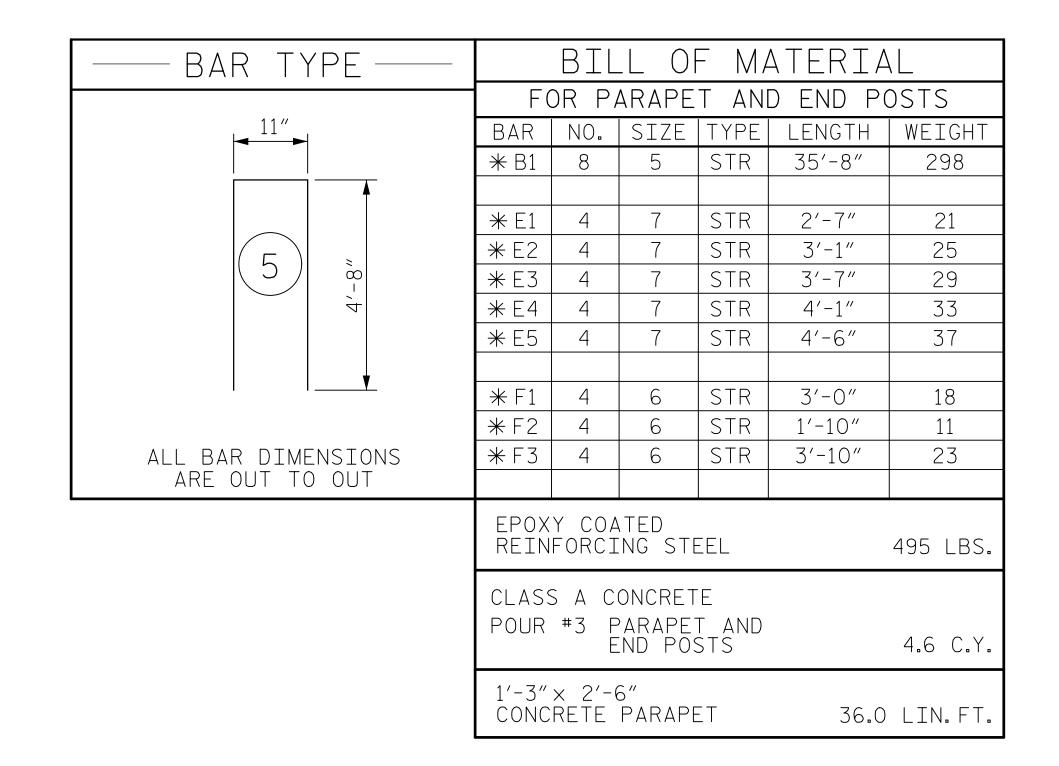
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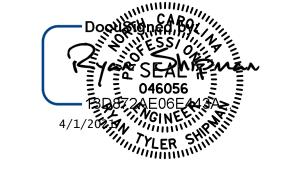
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PROJECT NO. U-5887

HENDERSON COUNTY

STATION: 22+44.41 -L-

SHEET 20 OF 21

DEPARTMENT OF TRANSPORTATION
RALEIGH

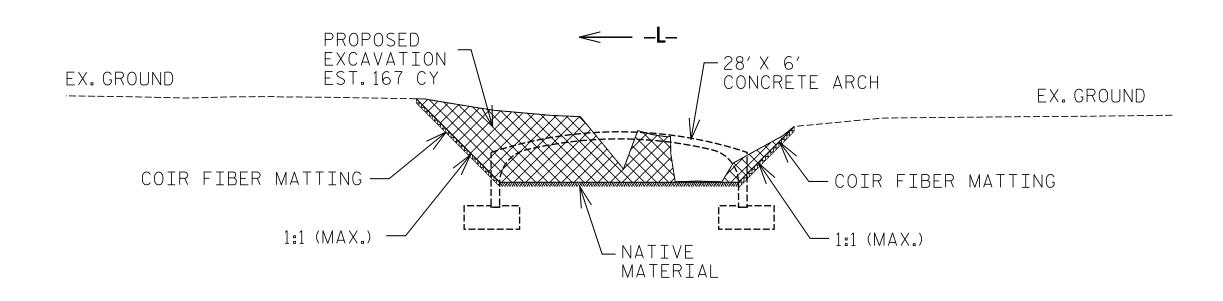
OUTLET HEADWALL AND HEADWALL FOOTING BILL OF MATERIAL

DRAWN BY: HL DATE: 03/2020 REVISIONS SHEET NO. C-20
CHECKED BY: CBC DATE: 03/2020 No. BY: DATE: No. BY: DATE: C-20
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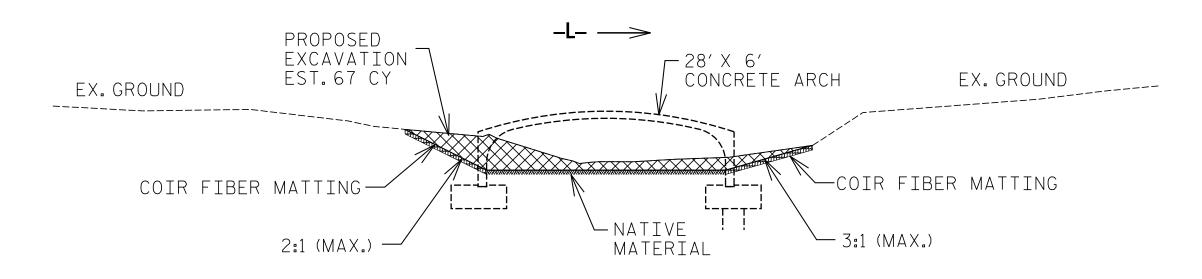
NOTES:

NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED OR FLOODPLAIN AT THE PROJECT SITE DURING CULVERT CONSTRUCTION. RIP RAP MAY BE USED TO SUPPLEMENT THE NATIVE MATERIAL. IF RIP RAP IS USED, NATIVE MATERIAL SHOULD BE PLACED ON TOP TO FILL VOIDS AND PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

THE ENTIRE COST OF WORK REQUIRED TO PLACE EXCAVATED MATERIAL OR SUPPLEMENTAL MATERIAL, SHALL BE INCLUDED IN THE LUMP SUM PRICE BID FOR "UNCLASSIFIED STRUCTURE EXCAVATION".

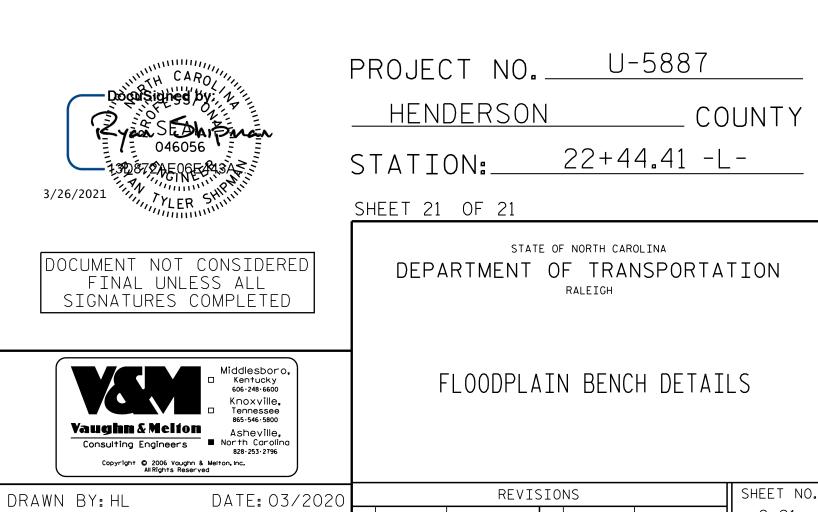


INLET CHANNEL & FLOODPLAIN BENCH PROFILE (LOOKING UPSTREAM)



OUTLET CHANNEL & FLOODPLAIN BENCH PROFILE (LOOKING DOWNSTREAM)

CHECKED BY: CBC DATE: 03/2020 ENG. OF RECORD: CBC DATE: 03/2020



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C-21

DATE:

