

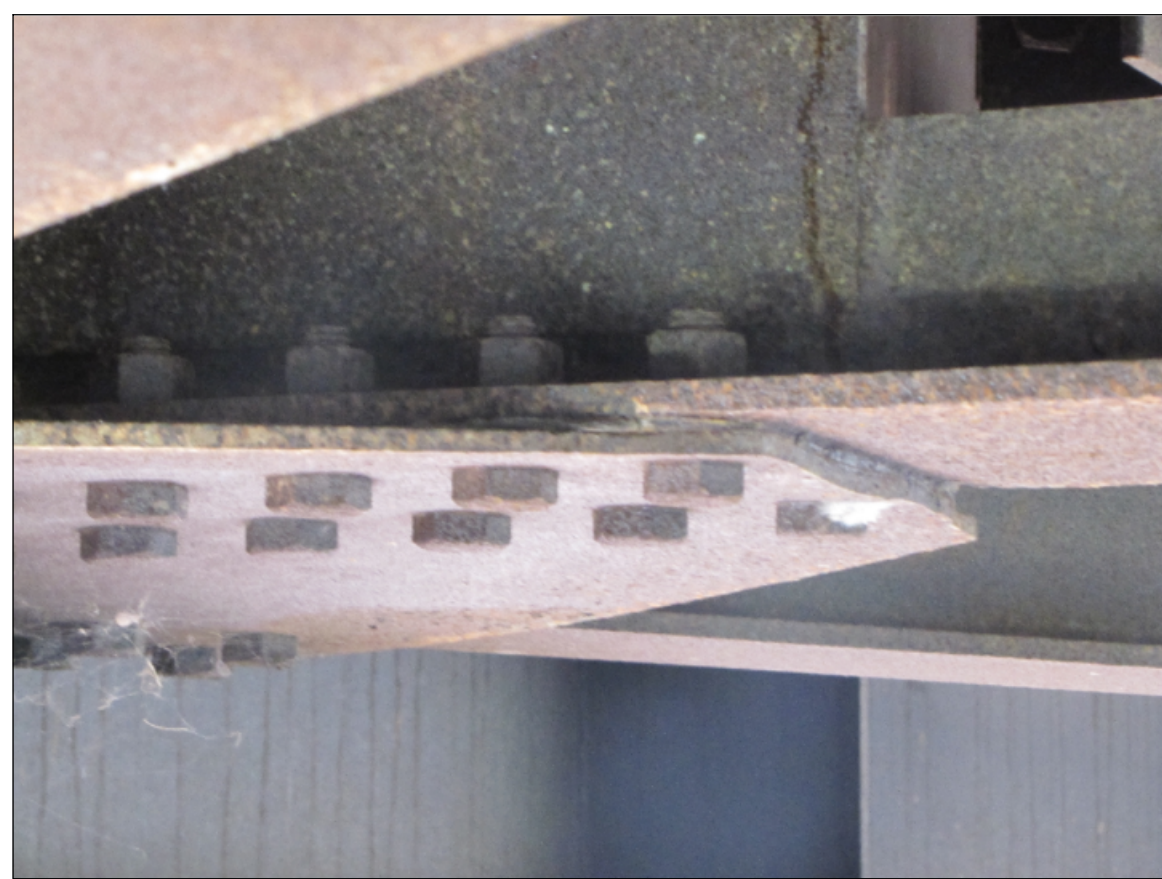
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PR-I-14



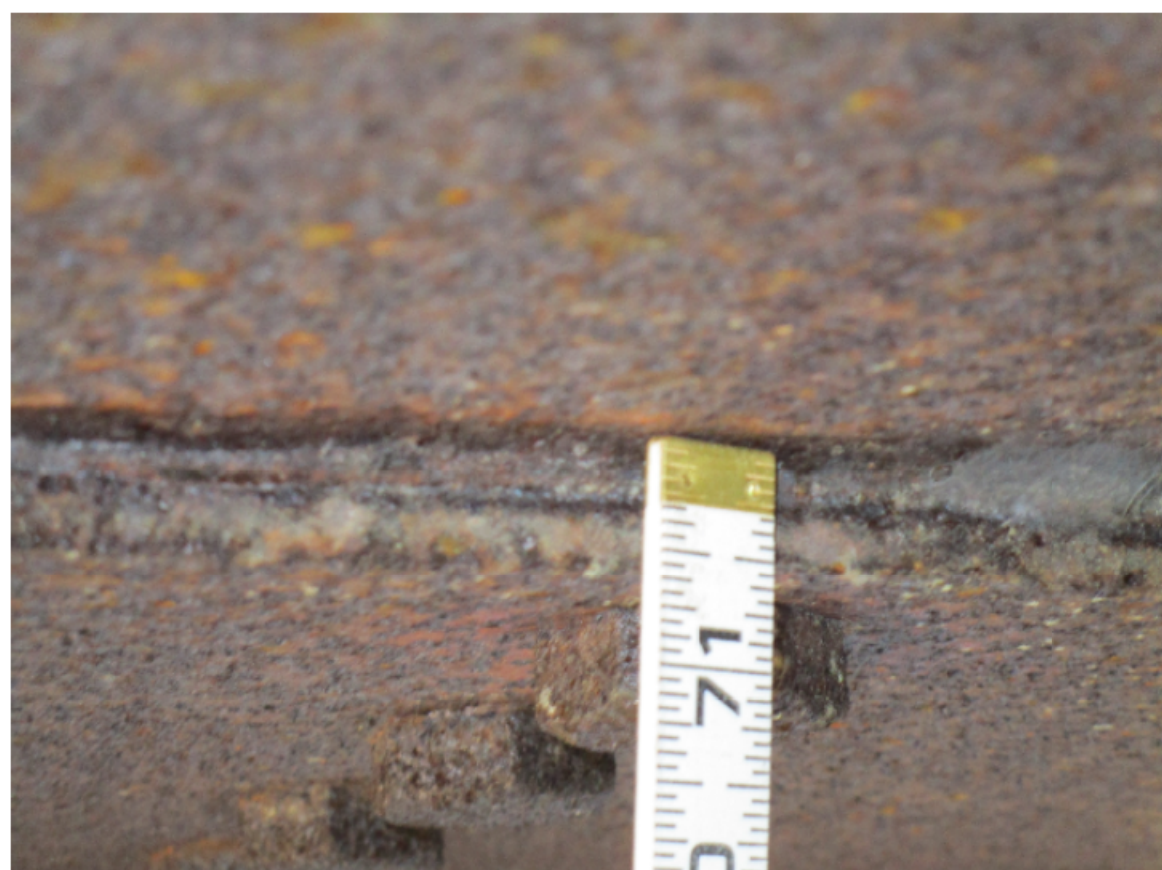
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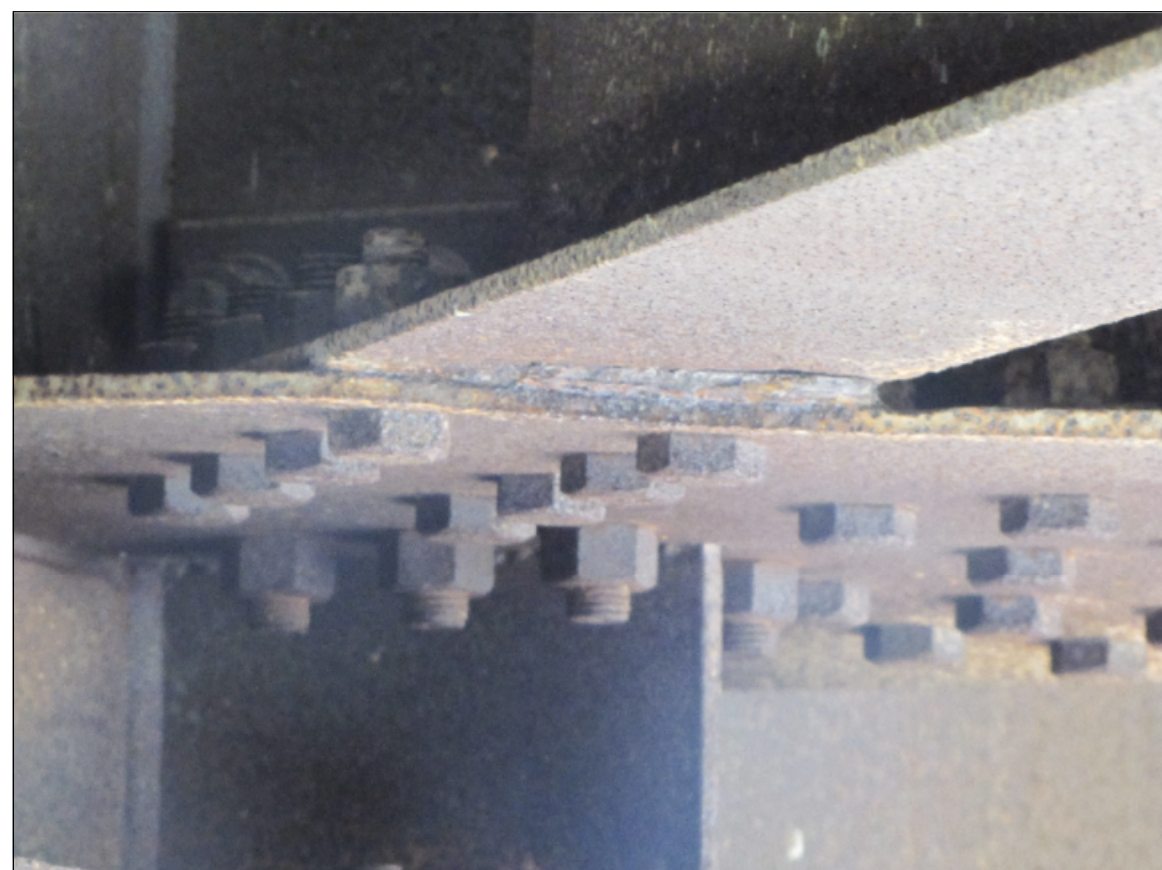
PR-I-5



PR-I-15



PR-I-2



PR-I-6



PR-I-16



PR-I-3



PR-I-17



PR-I-4



**PHOTO LOCATION LEGEND**

- PR-I-1 GIRDER 1, PANEL POINT 26, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-2 GIRDER 1, PANEL POINT 20, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND LATERAL BRACING MEMBERS.
- PR-I-3 GIRDER 1, PANEL POINT 18, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND LATERAL BRACING MEMBERS.
- PR-I-4 GIRDER 1, PANEL POINT 15, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND LATERAL BRACING MEMBERS.
- PR-I-5 GIRDER 1, PANEL POINT 15, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD.
- PR-I-6 GIRDER 2, PANEL POINT 15, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD.
- PR-I-7 GIRDER 3, PANEL POINT 12, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-8 GIRDER 3, PANEL POINT 26, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-9 GIRDER 4, PANEL POINT 12, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-10 GIRDER 3, PANEL POINT 10, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-11 GIRDER 3, PANEL POINT 15, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-12 GIRDER 3, PANEL POINT 18, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-13 GIRDER 3, PANEL POINT 23, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD.
- PR-I-14 GIRDER 3, PANEL POINT 29, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-15 GIRDER 4, PANEL POINT 6, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-16 GIRDER 4, PANEL POINT 9, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.
- PR-I-17 GIRDER 4, PANEL POINT 35, AT BOTTOM CROSSFRAME CONNECTION: PACK RUST BETWEEN GUSSET PLATE AND CROSSFRAME BOTTOM CHORD AND LATERAL BRACING MEMBERS.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 12 OF 14

**AECOM**  
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STATE OF NORTH CAROLINA  
 JOHN E. SLOAN  
 ENGINEER  
 035062  
 2/28/2020

STATE OF NORTH CAROLINA  
**DEPARTMENT OF TRANSPORTATION**  
 RALEIGH

REHABILITATION

**STRUCTURAL STEEL REPAIRS**

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-92
1			3			TOTAL SHEETS
2			4			129

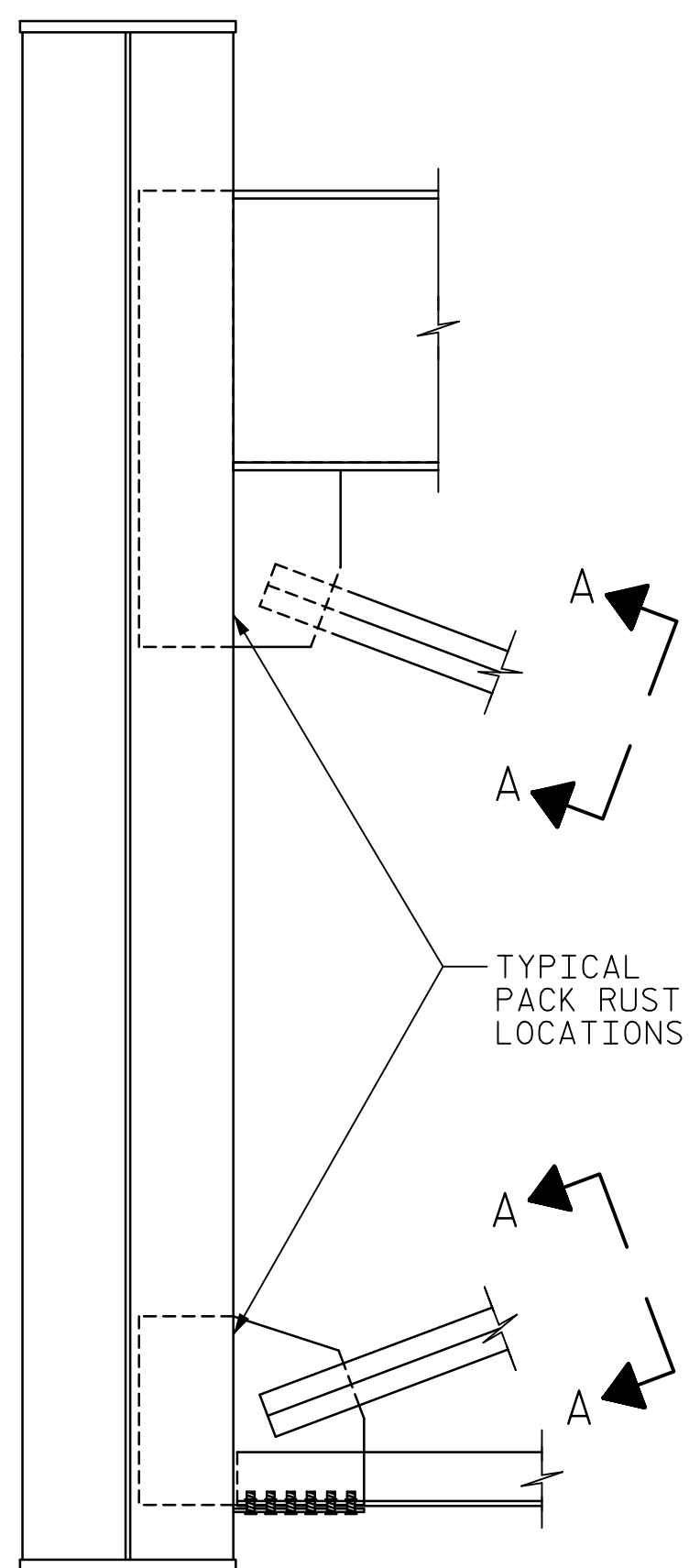
DRAWN BY : H. ROSEMOND      DATE : 2/2019  
 CHECKED BY : G. COLS      DATE : 2/2019  
 DESIGNED BY : J. SLOAN      DATE : 2/2019  
 DESIGN CHECKED BY : E. ZHOU      DATE : 2/2019

**PACK RUST REPAIRS**  
 FOR REPRESENTATION ONLY

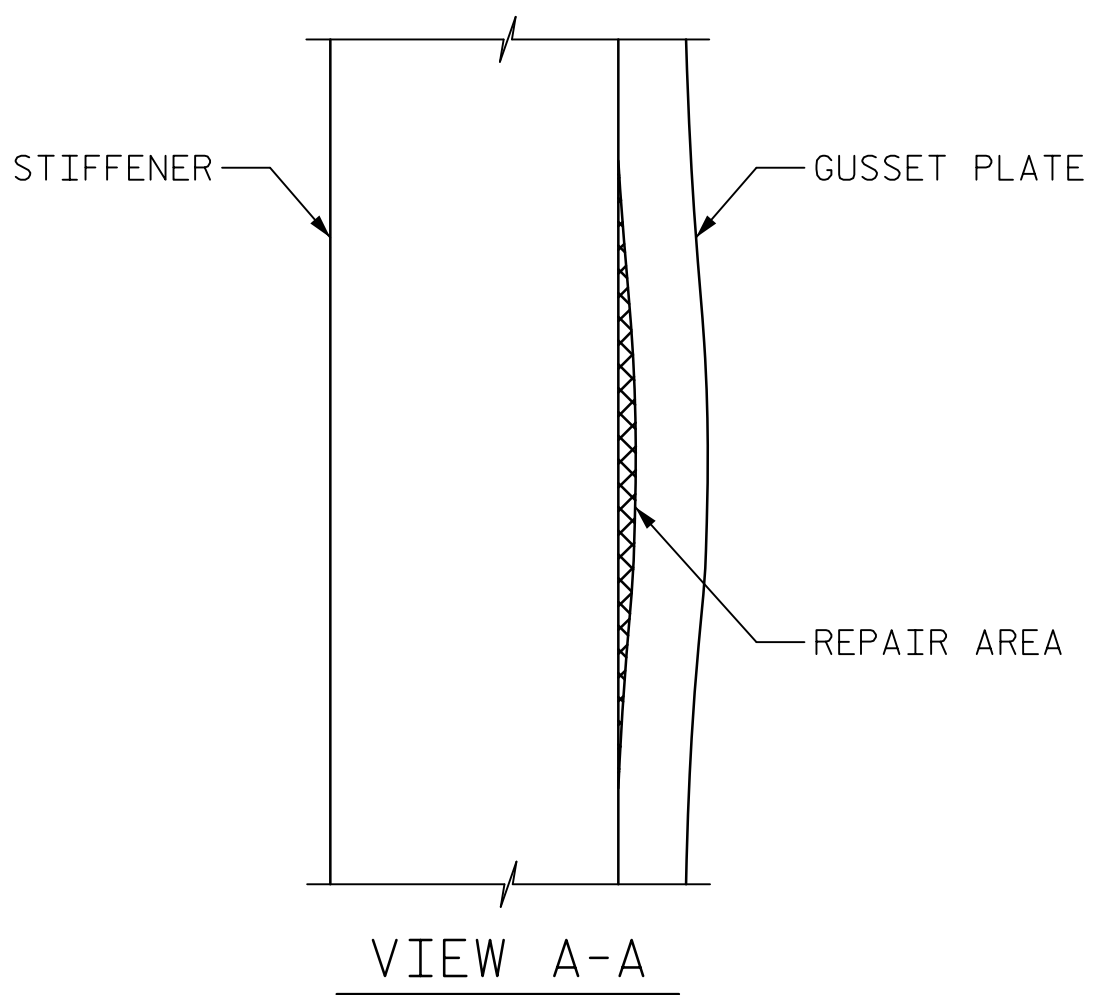
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ELEVATION OF TYPICAL PACK RUST REPAIR - TYPE II



VIEW A-A

**TYPE II PACK RUST REPAIR PROCEDURE:**

FOR PACK RUST REPAIRS, HRCSA PENETRANT, AND PAINT, SEE SPECIAL PROVISIONS.

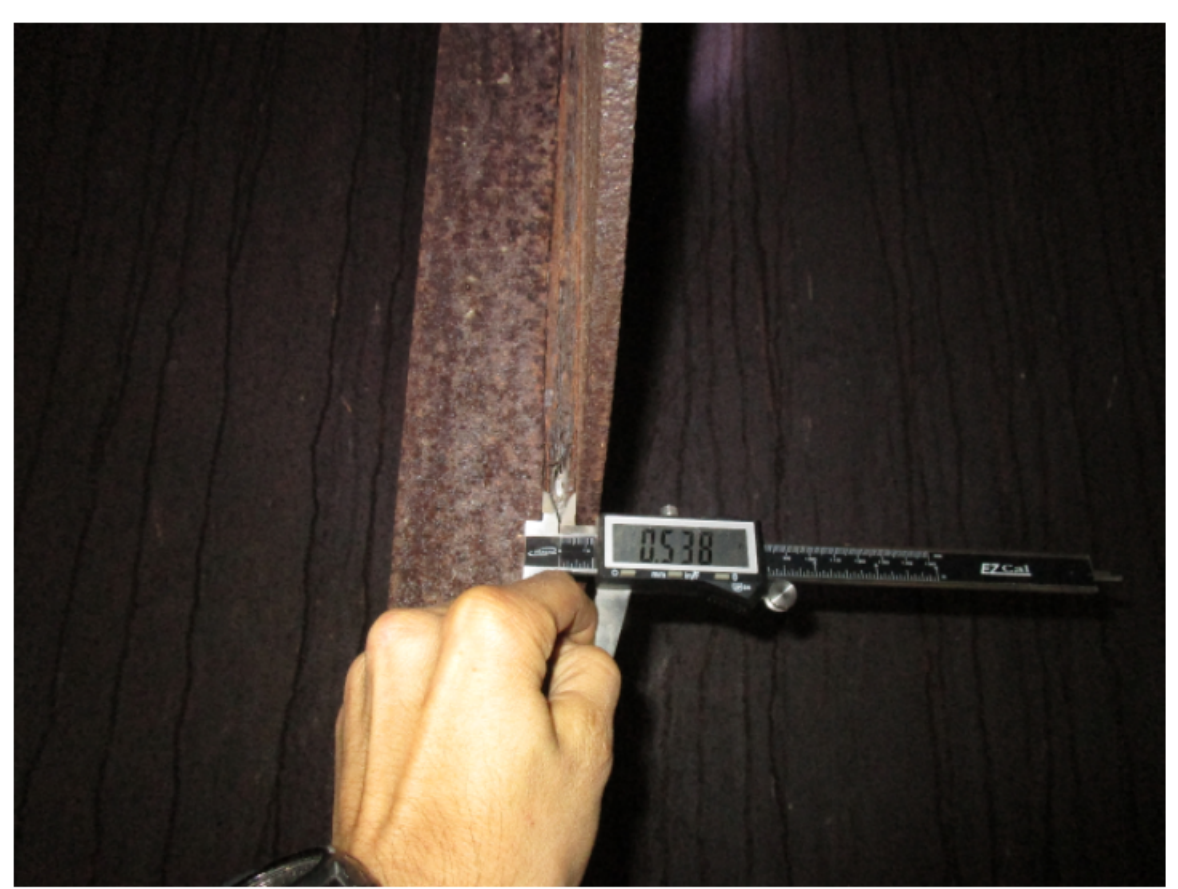
1. CLEAN AREA TO REMOVE PACK RUST. DO NOT DISCONNECT ANY MEMBERS OR LOOSEN ANY BOLTS.
2. APPLY AN HRCSA PENETRANT TO THE CLEANED AREA IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
3. APPLY COMPATIBLE ONE-STEP HRCSA PRIMER/TOP-COAT, APPLYING MULTIPLE COATS AS NECESSARY TO FILL GAP OR CREVICE. COAT THE ENTIRE GUSSET PLATE AND 3" BEYOND THE GUSSET PLATE ON EACH ADJOINING MEMBER.

THE ENGINEER SHALL INSPECT EACH REPAIR AREA BETWEEN EACH STEP OF THE REPAIR.

FOR PAY ITEMS, SEE SHEET 14.

DRAWN BY : M. TOM	DATE : 2/2019
CHECKED BY : G. COLS	DATE : 2/2019
DESIGNED BY : J. SLOAN	DATE : 2/2019
DESIGN CHECKED BY : E. ZHOU	DATE : 2/2019

PR-II-1



PR-II-2



PR-II-3



PR-II-4



PR-II-5



PR-II-6



**PHOTO LOCATION LEGEND**

- PR-II-1 GIRDER 3, PANEL POINT 12, AT TOP CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.
- PR-II-2 GIRDER 3, PANEL POINT 12, AT BOTTOM CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.
- PR-II-3 GIRDER 3, PANEL POINT 26, AT TOP CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.
- PR-II-4 GIRDER 3, PANEL POINT 16, AT BOTTOM CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.
- PR-II-5 GIRDER 4, PANEL POINT 12, AT TOP CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.
- PR-II-6 GIRDER 4, PANEL POINT 26, AT TOP CROSSFRAME CONNECTION; PACK RUST BETWEEN CROSSFRAME DIAGONAL GUSSET PLATE AND CONNECTOR PLATE.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 13 OF 14

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STATE OF NORTH CAROLINA  
 JOHN E. SLOAN  
 ENGINEER  
 035062  
 2/28/2020

STATE OF NORTH CAROLINA  
**DEPARTMENT OF TRANSPORTATION**  
 RALEIGH  
 REHABILITATION  
**STRUCTURAL STEEL REPAIRS**

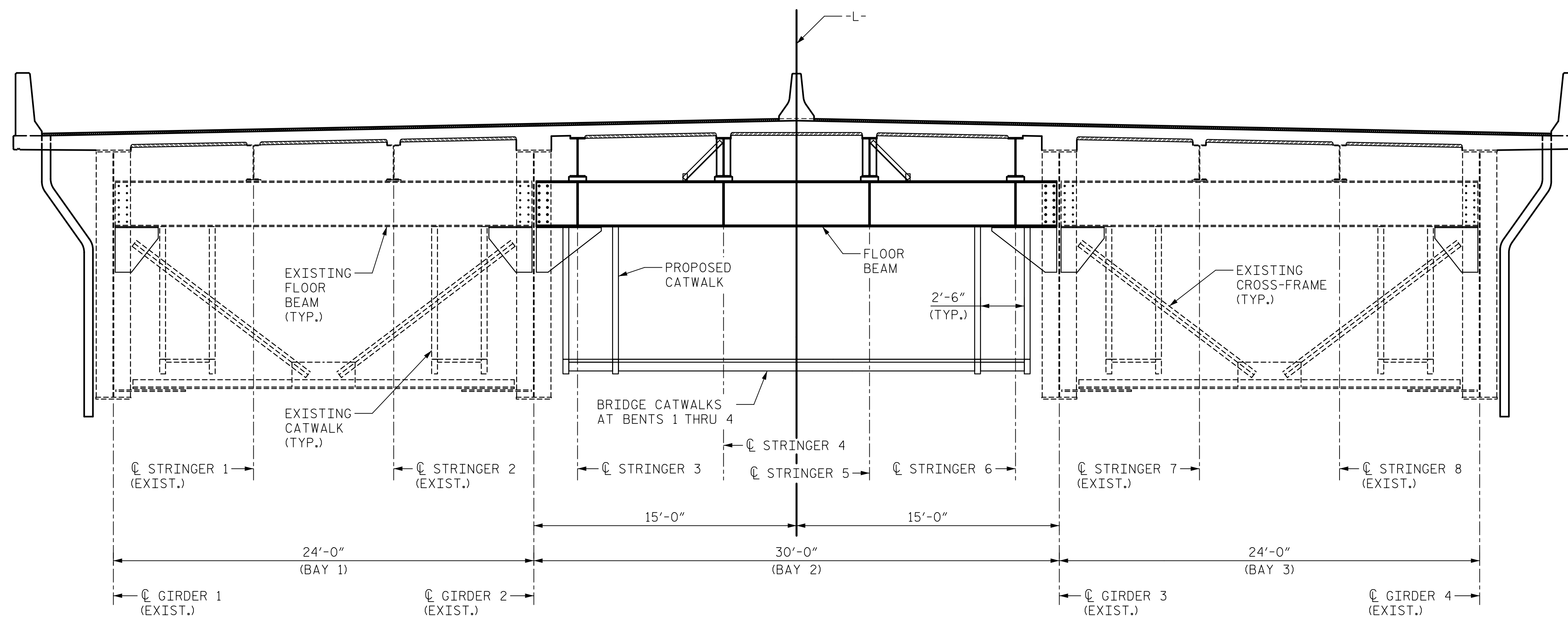
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-93
1			3			TOTAL SHEETS
2			4			129

**PACK RUST REPAIRS**  
 FOR REPRESENTATION ONLY. REPAIR SHALL BE PERFORMED AT EACH LOCATION NOTED, FOR REPAIR LOCATIONS, SEE PHOTO LOCATION LEGEND.

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TIME: 4:06:46 PM



**TYPICAL SECTION**  
(CROSS FRAMES SHOWN TYPICAL OF INTERMEDIATE CROSS FRAMES)

**NOTES:**

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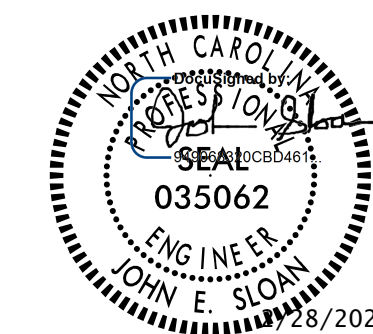
FOR FURTHER DETAILS, SEE ACCESS AND FALL PROTECTION SPECIAL PROVISION.

CONTRACTOR SHALL SUBMIT WORKING DRAWINGS IN ACCORDANCE WITH THE SPECIAL PROVISION.

PAYMENT SHALL BE MADE AS A LUMP SUM UNDER ACCESS AND FALL PROTECTION PAY ITEM.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
STATION: 35+30.22 -L-

SHEET 1 OF 3



STATE OF NORTH CAROLINA  
**DEPARTMENT OF TRANSPORTATION**  
RALEIGH

**ACCESS & FALL PROTECTION**

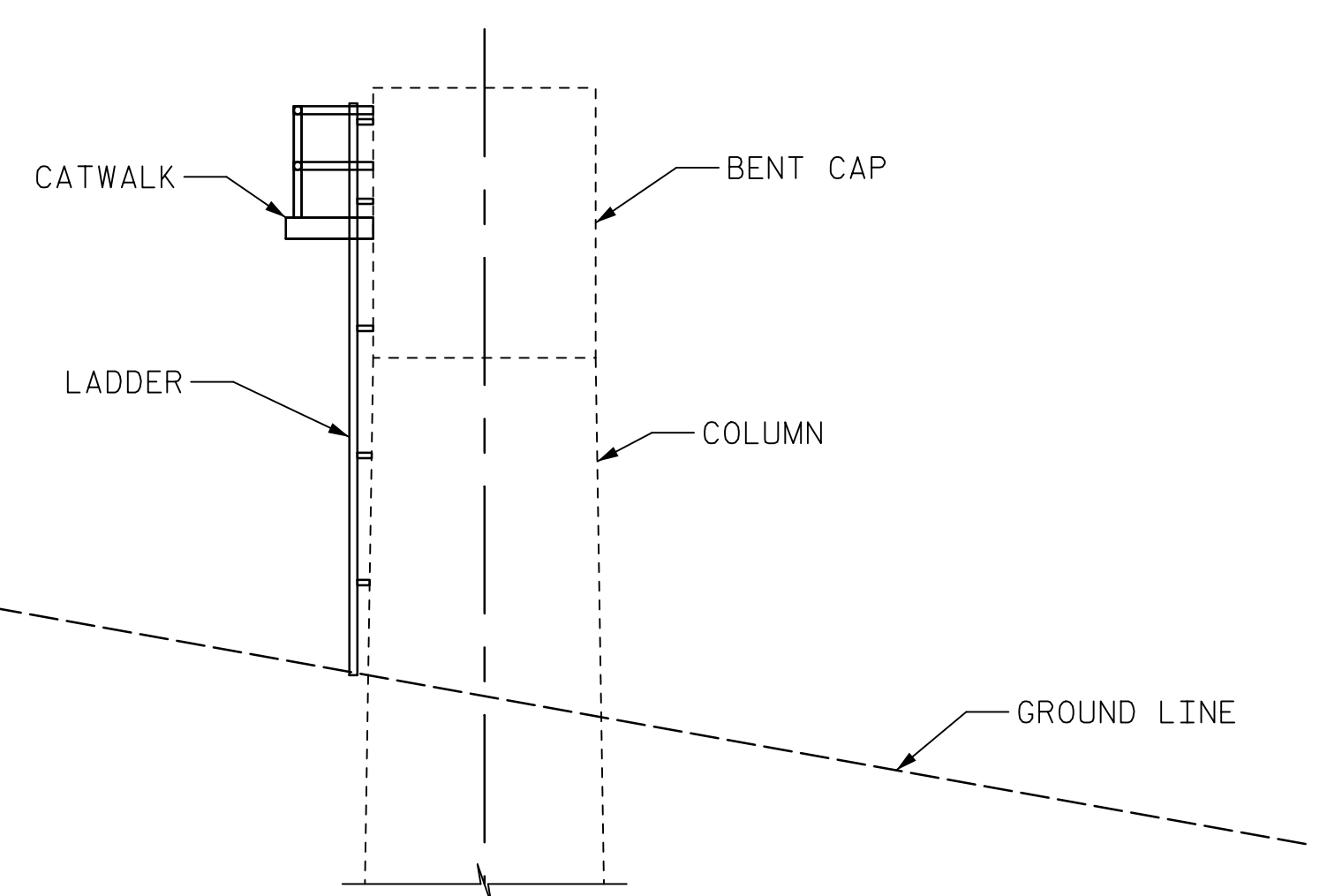
DRAWN BY : S. STREDNAK DATE : 4/2019  
CHECKED BY : J. SLOAN DATE : 4/2019  
DESIGNED BY : N. BROWN/ D. TUTTLE DATE : 4/2019  
DESIGN CHECKED BY : J. SLOAN/J. LIU DATE : 4/2019

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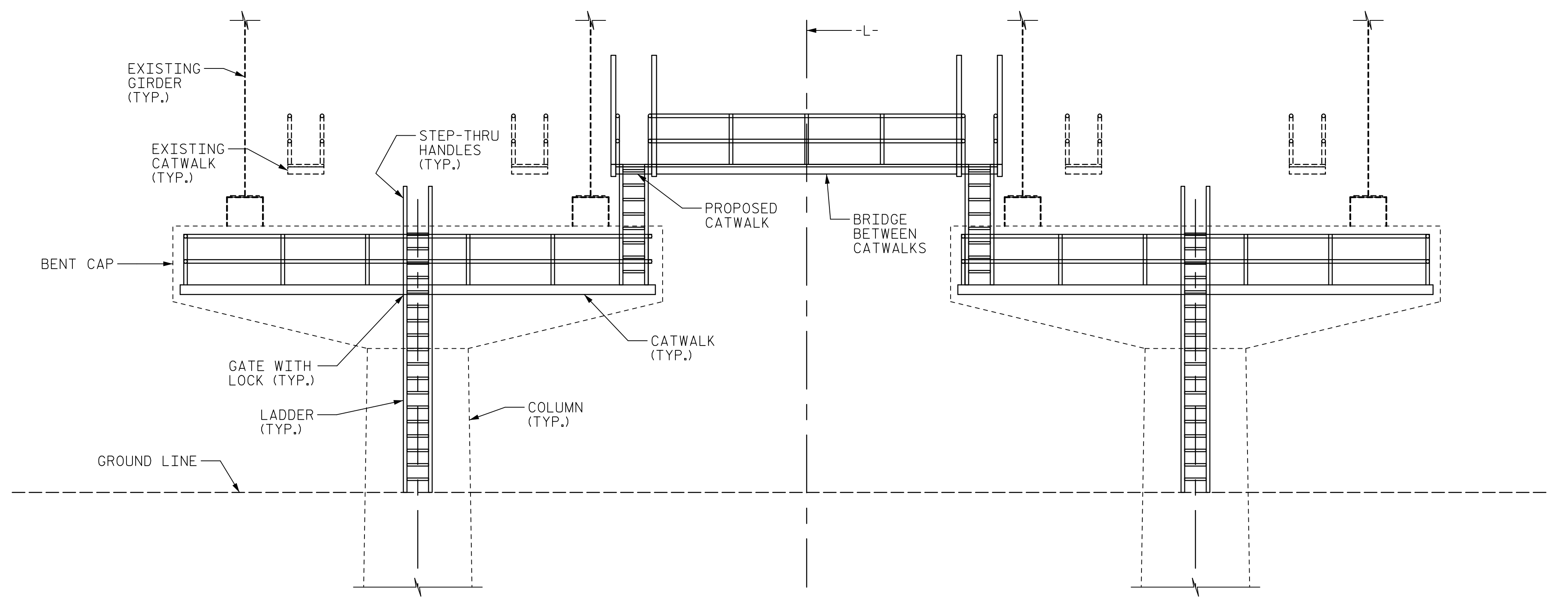
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TIME: 4:02:54 PM



**PROFILE**  
(NORTH FACE OF BENT 1 AND SOUTH FACE OF BENT 4 SHOWN)



**ELEVATION**  
(EAST FACE OF BENT 1 AND WEST FACE OF BENT 4 SHOWN)

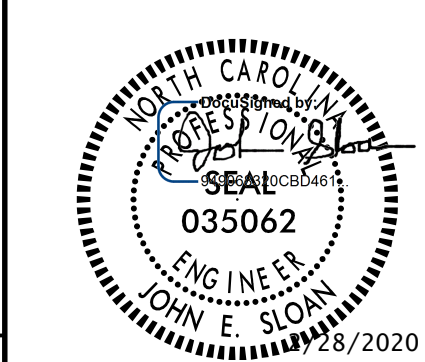
**LADDERS AND CATWALKS AT BENTS 1 AND 4**  
(SUPERSTRUCTURE CROSSFRAMES AND BENT CAP P.T. NOT SHOWN FOR CLARITY)

**NOTES:**

- THIS DRAWING IS A SCHEMATIC ONLY AND IS NOT A WORKING DRAWING.
- FOR FURTHER DETAILS, SEE ACCESS AND FALL PROTECTION SPECIAL PROVISION.
- CONTRACTOR SHALL SUBMIT WORKING DRAWINGS IN ACCORDANCE WITH THE SPECIAL PROVISION.
- PAYMENT SHALL BE MADE AS A LUMP SUM UNDER ACCESS AND FALL PROTECTION PAY ITEM.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 2 OF 3



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
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**ACCESS & FALL PROTECTION**

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CHECKED BY :	J. SLOAN	DATE :	4/2019
DESIGNED BY :	N. BROWN/D. TUTTLE	DATE :	4/2019
DESIGN CHECKED BY :	J. SLOAN	DATE :	4/2019

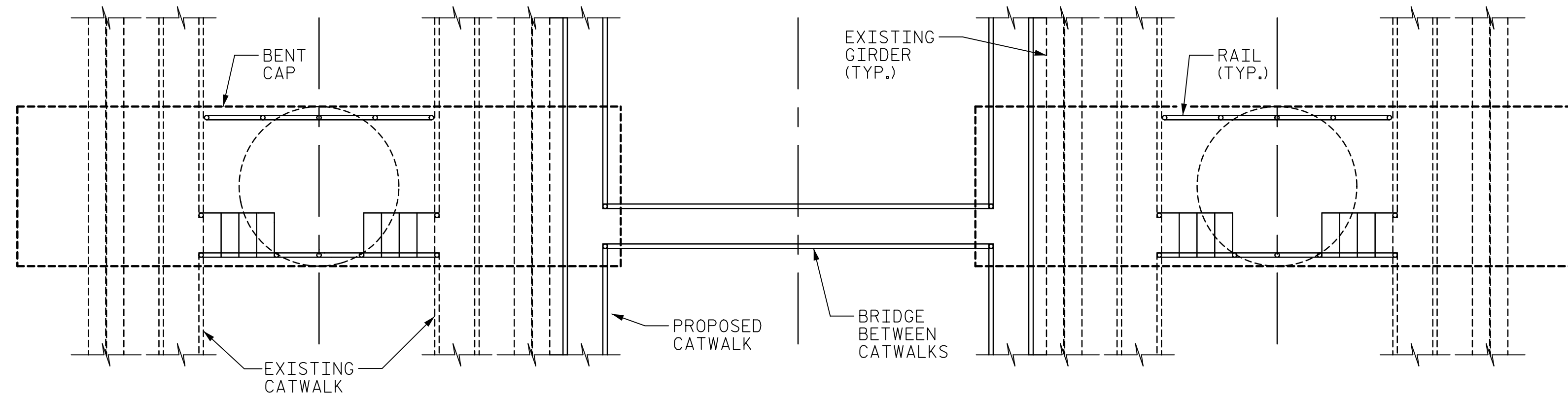
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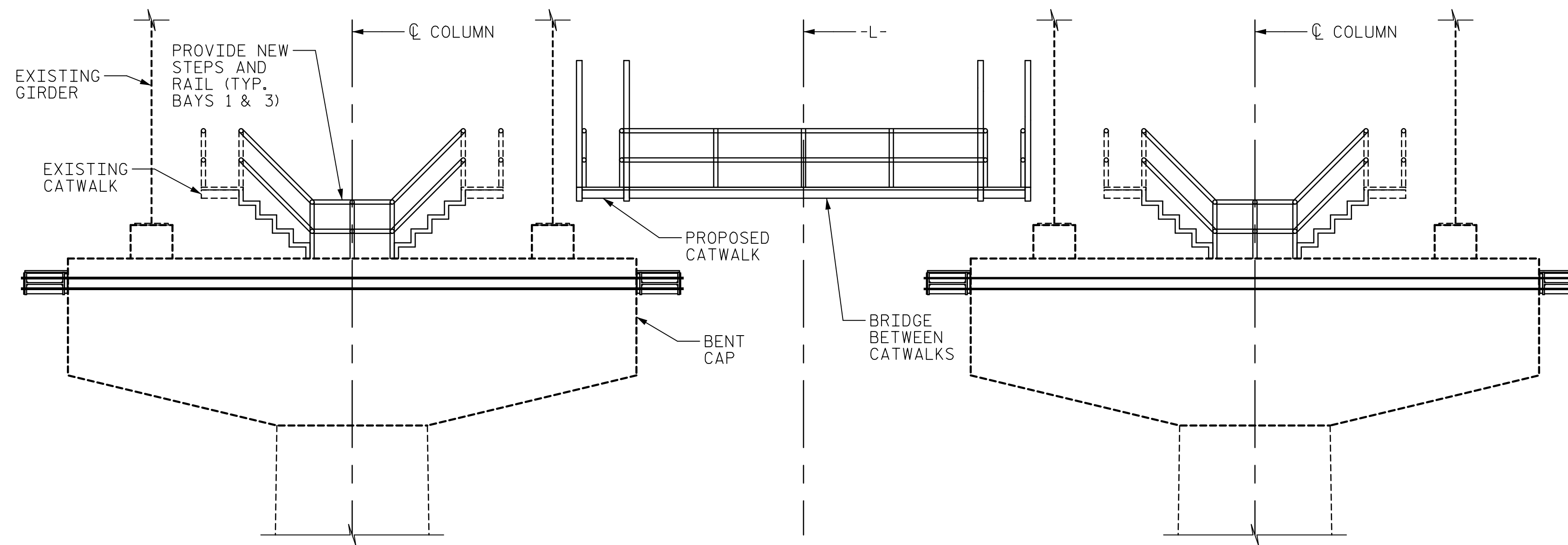


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TIME: 4:03:32 PM



**PLAN**

(P.T. ANCHORAGES AND BARS NOT SHOWN IN PLAN VIEW FOR CLARITY)



**ELEVATION**

**LADDERS AND CATWALKS AT BENTS 2 AND 3**

(SUPERSTRUCTURE CROSSFRAMES NOT SHOWN FOR CLARITY)

**NOTES:**

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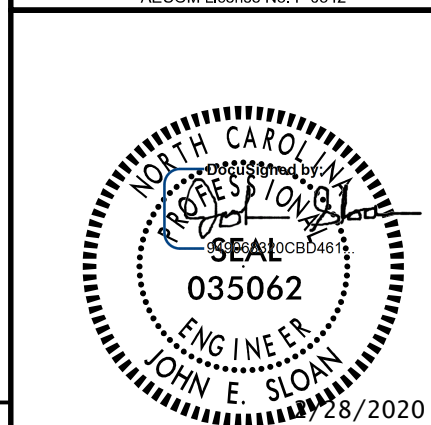
FOR FURTHER DETAILS, SEE ACCESS AND FALL PROTECTION SPECIAL PROVISION.

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PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 3 OF 3



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**ACCESS & FALL PROTECTION**

REVISIONS						SHEET NO.
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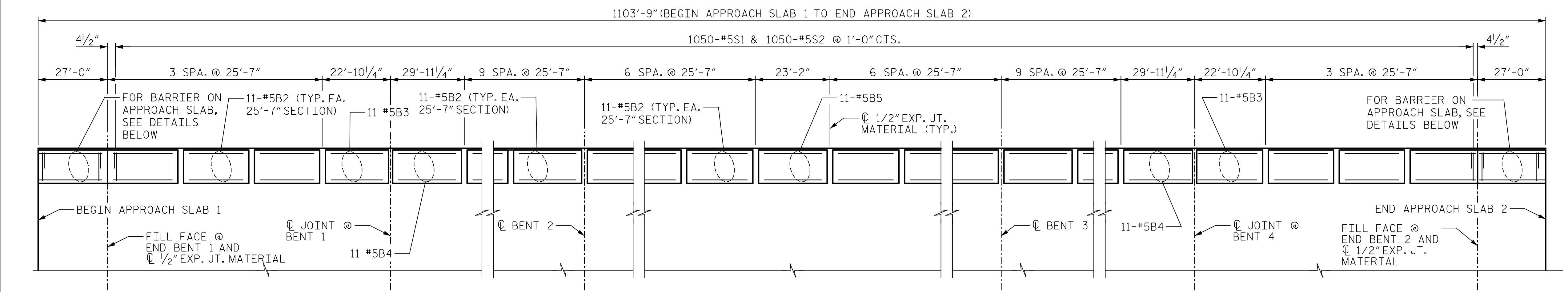
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 CHECKED BY : J. SLOAN DATE : 4/2019  
 DESIGNED BY : N. BROWN/D. TUTTLE DATE : 4/2019  
 DESIGN CHECKED BY : J. SLOAN DATE : 4/2019

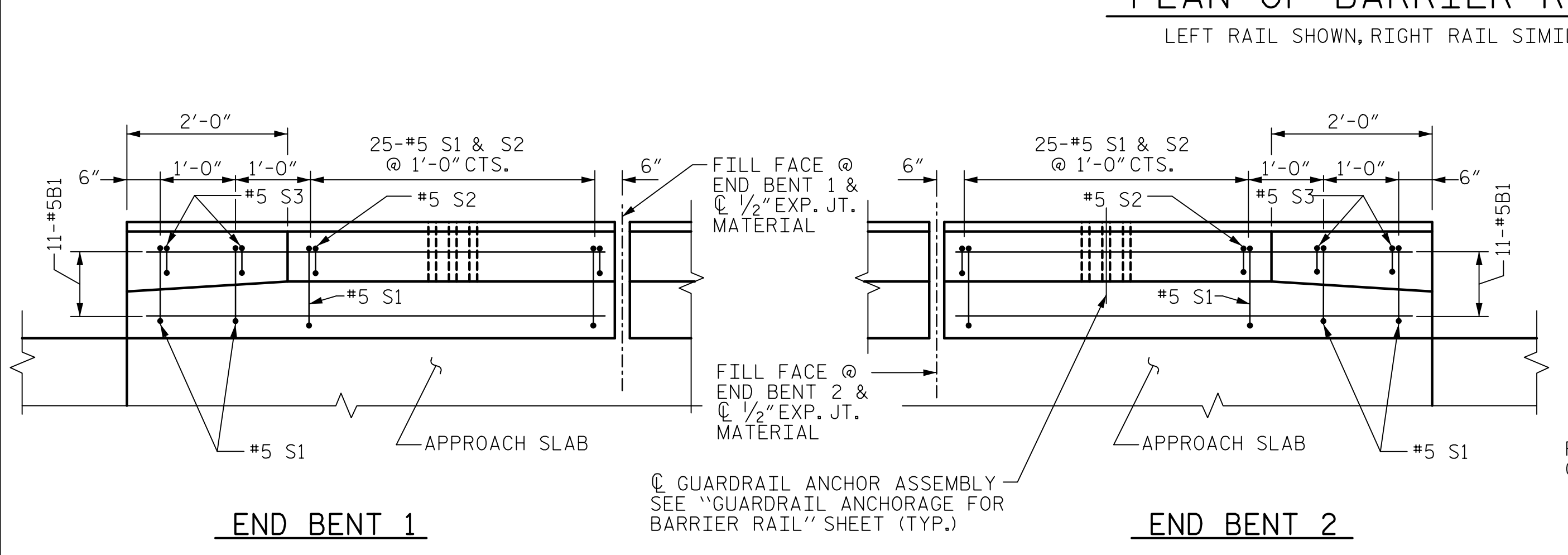
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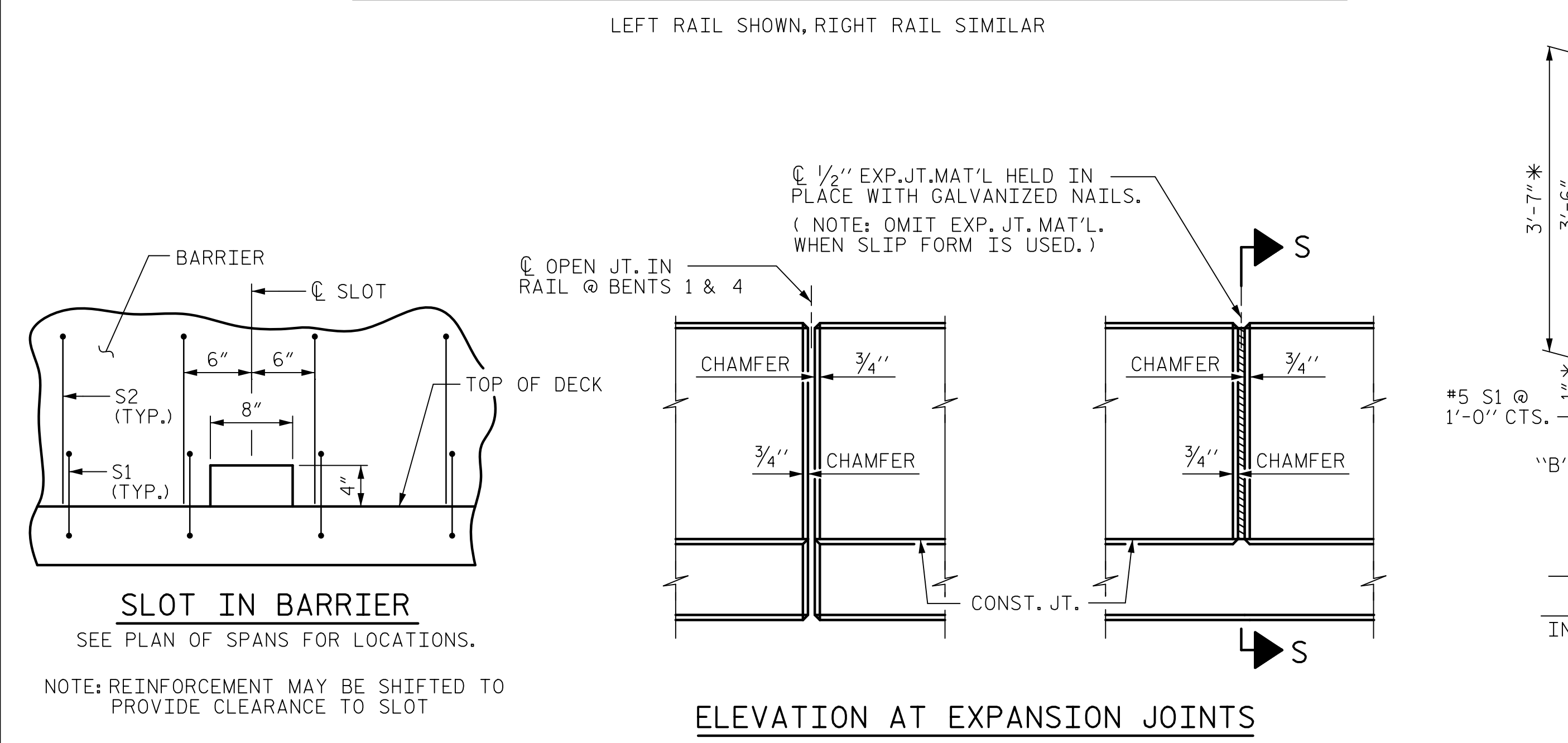
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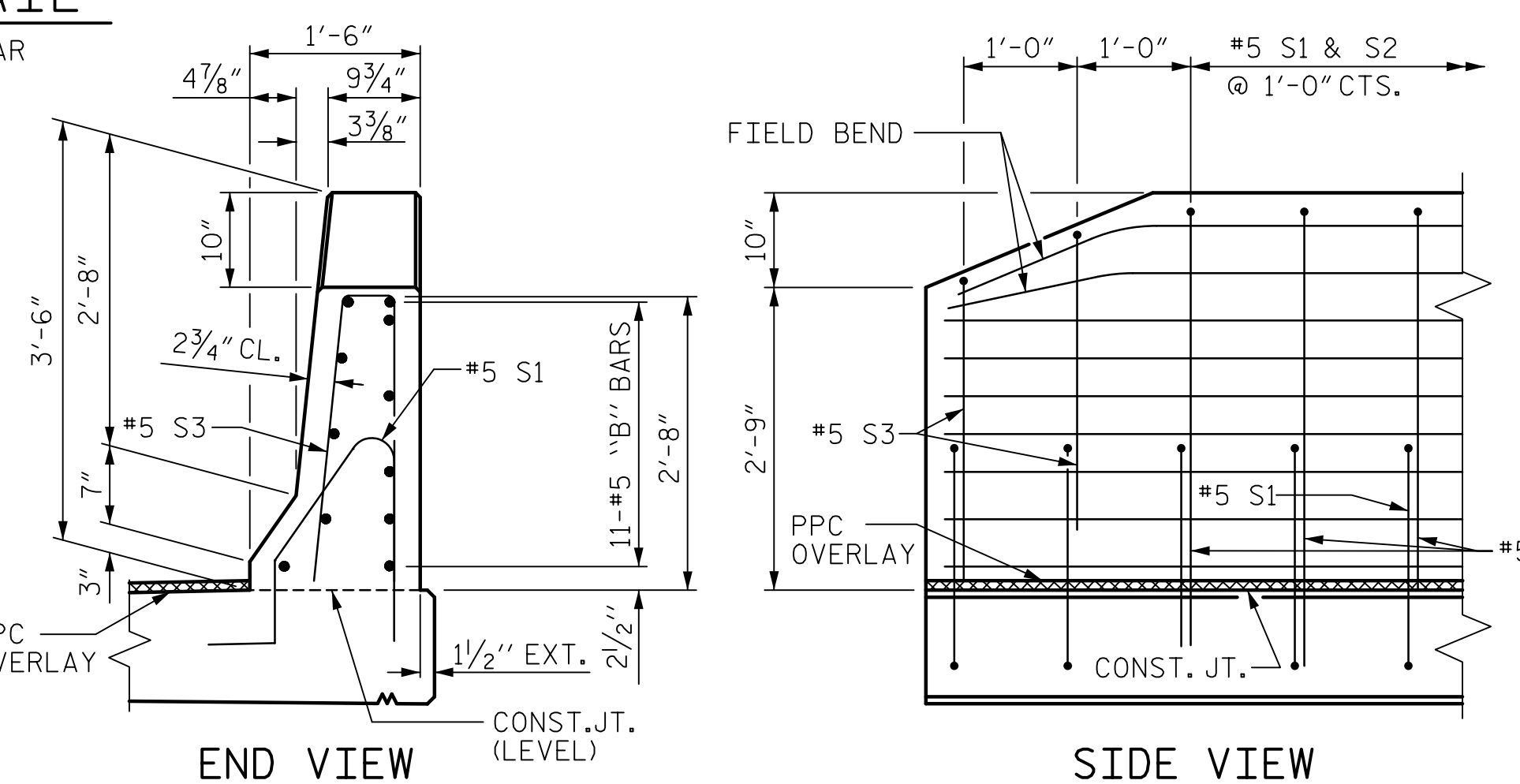
**PLAN OF BARRIER RAIL**  
LEFT RAIL SHOWN, RIGHT RAIL SIMILAR



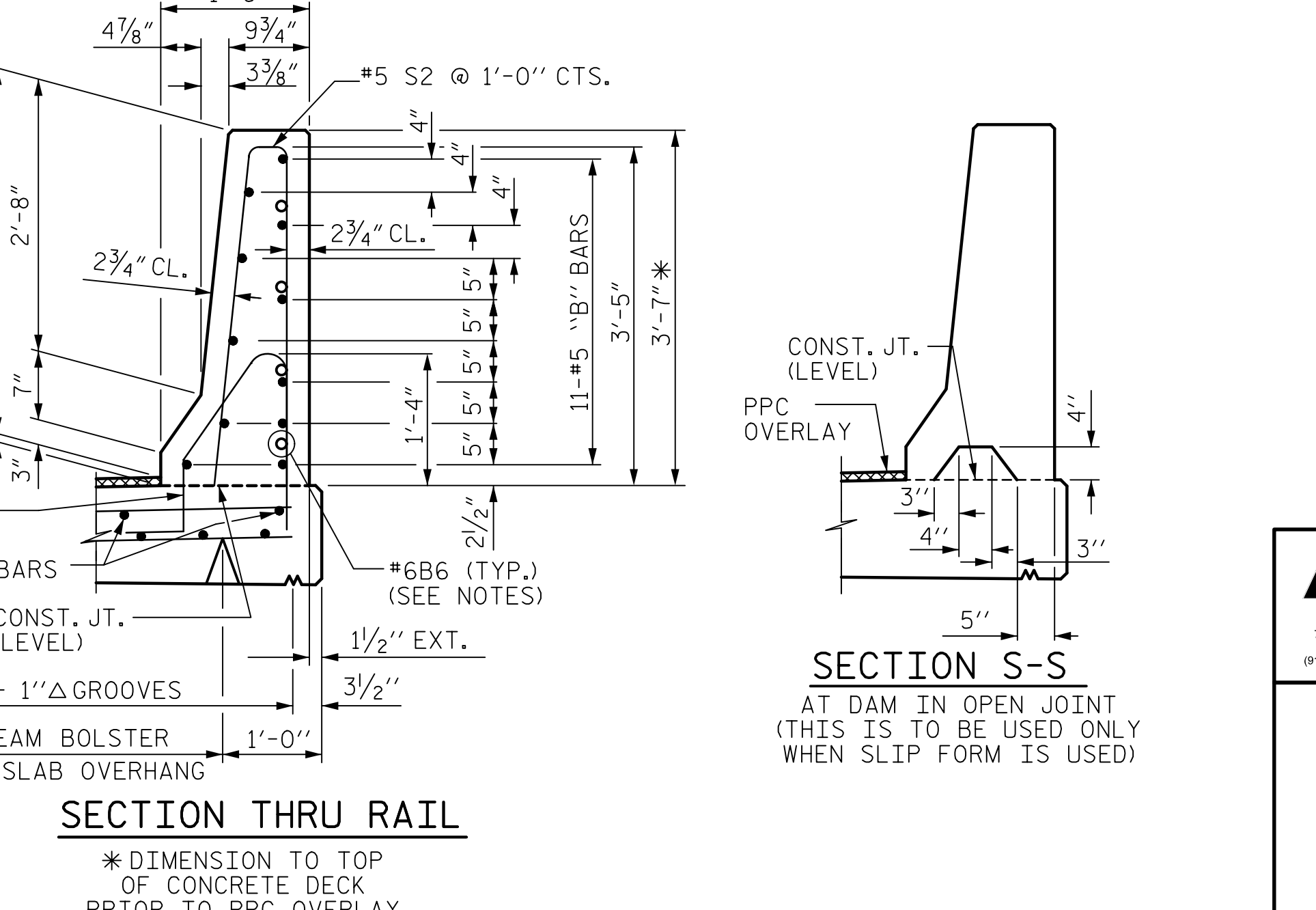
**PLAN OF BARRIER RAIL ON APPROACH SLAB**  
LEFT RAIL SHOWN, RIGHT RAIL SIMILAR



**ELEVATION AT EXPANSION JOINTS**



**END OF RAIL DETAILS**



**SECTION THRU RAIL**  
\* DIMENSION TO TOP OF CONCRETE DECK PRIOR TO PPC OVERLAY

**BAR TYPES**

ALL BAR DIMENSIONS ARE OUT TO OUT

**BILL OF MATERIAL**  
FOR CONCRETE BARRIER RAIL ONLY

BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT
* B1	44	5	STR	26'-7"	1,220
* B2	792	5	STR	25'-2"	20,789
* B3	44	5	STR	22'-5"	1,029
* B4	44	5	STR	29'-6"	1,354
* B5	22	5	STR	22'-9"	522
* B6	16	6	STR	12'-0"	288
* S1	2208	5	1	4'-10"	11,131
* S2	2208	5	2	7'-2"	15,504
* S3	8	5	2	5'-8"	47

\* EPOXY COATED REINFORCING STEEL 52,884 LBS.  
ALL-LIGHTWEIGHT CONCRETE 310.5 CU. YDS.  
CONCRETE BARRIER RAIL (ALL-LIGHTWEIGHT CONC.) 2,207.5 LIN. FT.

**NOTES**

THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

S1 AND S2 BARS IN THE BARRIER SHALL BE SHIFTED SLIGHTLY AS NECESSARY TO PROVIDE 2" MIN CLEARANCE TO ALL OPEN JOINTS IN THE BARRIER.

AT LOCATIONS OF INLET ON APPROACH SLAB, INSTALL #6 B7 BARS AS SHOWN IN THE "SECTION THRU RAIL." BARS SHALL BE CENTERED AROUND INLET.

FOR ALL-LIGHTWEIGHT CONCRETE, SEE SPECIAL PROVISIONS.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

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STATE OF NORTH CAROLINA  
**DEPARTMENT OF TRANSPORTATION**  
 RALEIGH

SUPERSTRUCTURE

**CONCRETE BARRIER RAIL**

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S-100  
 TOTAL SHEETS 129

DATE: 2/28/2020

DRAWN BY: M. K. TOM DATE: 1/2019  
 CHECKED BY: G. COLS DATE: 1/2019  
 DESIGNED BY: N. BROWN DATE: 1/2019  
 DESIGN CHECKED BY: J. SLOAN DATE: 1/2019

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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

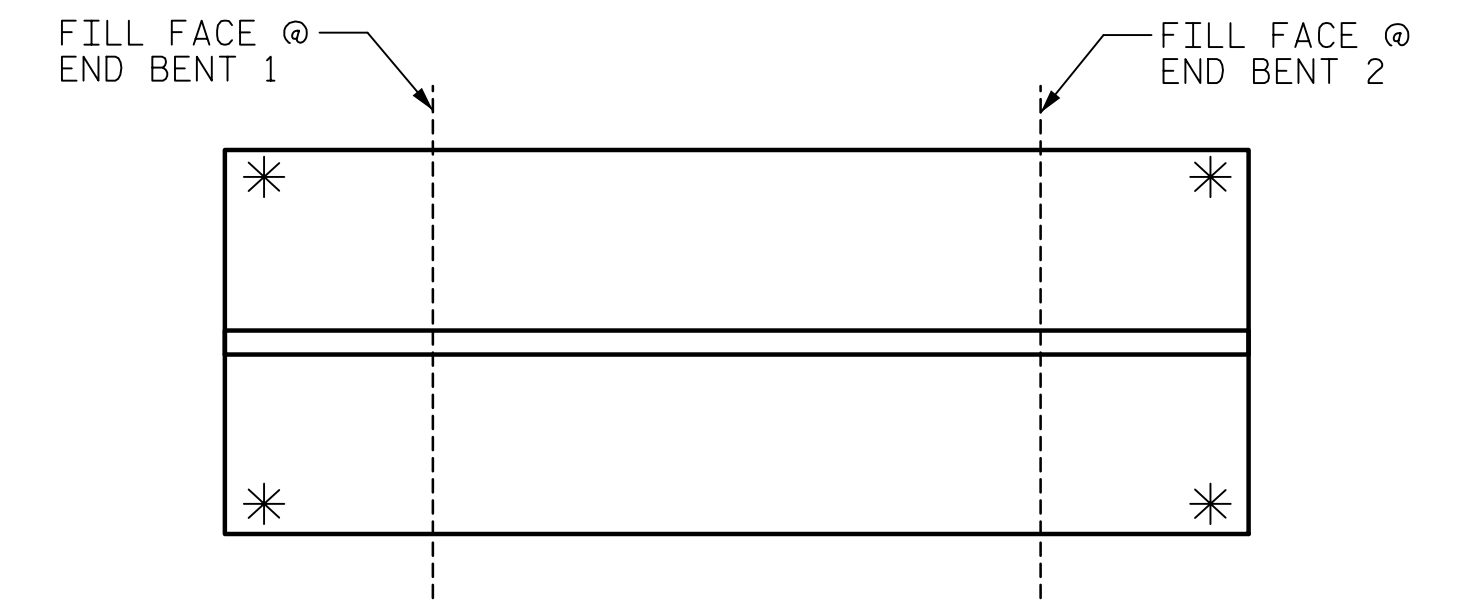
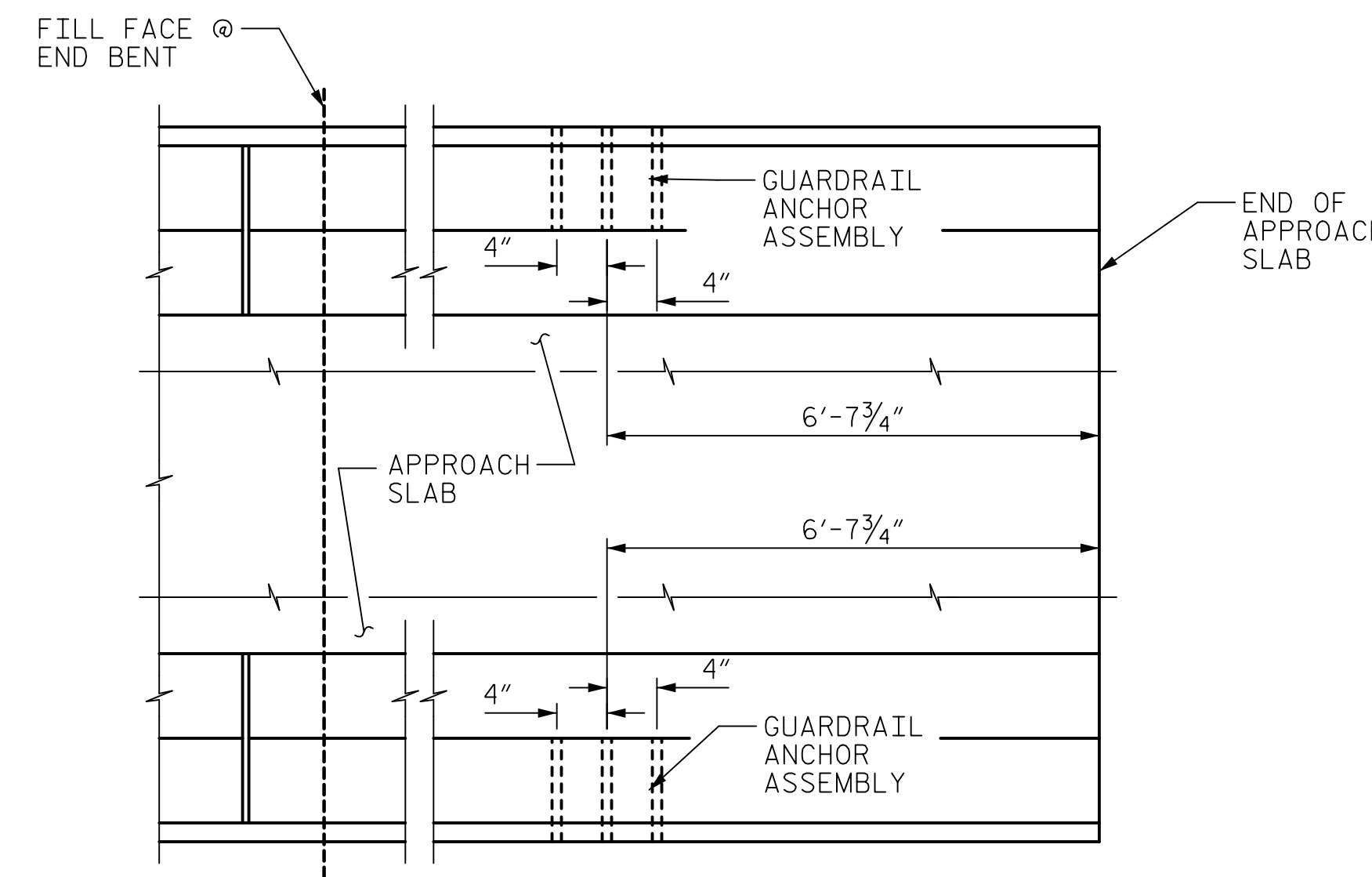
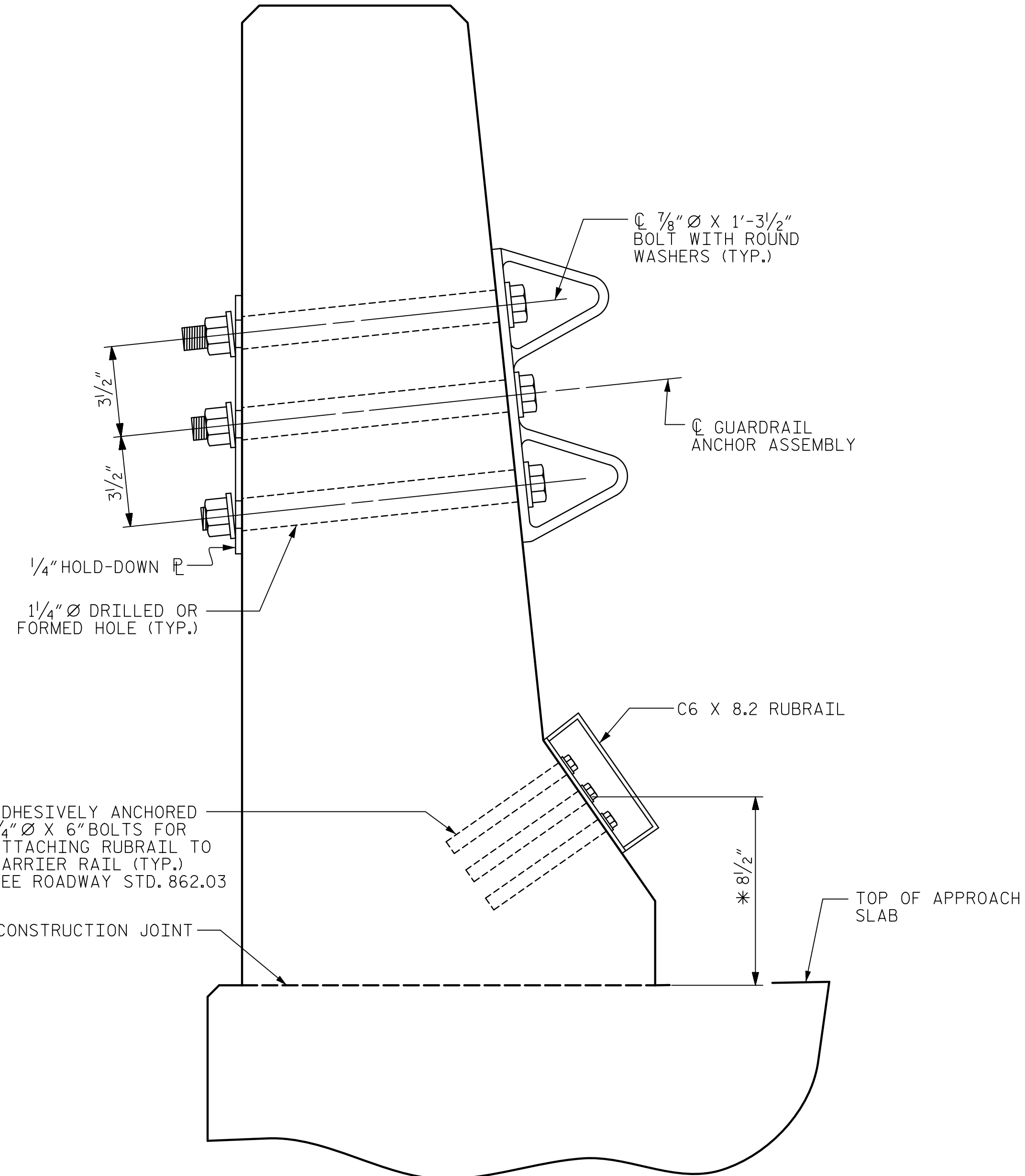
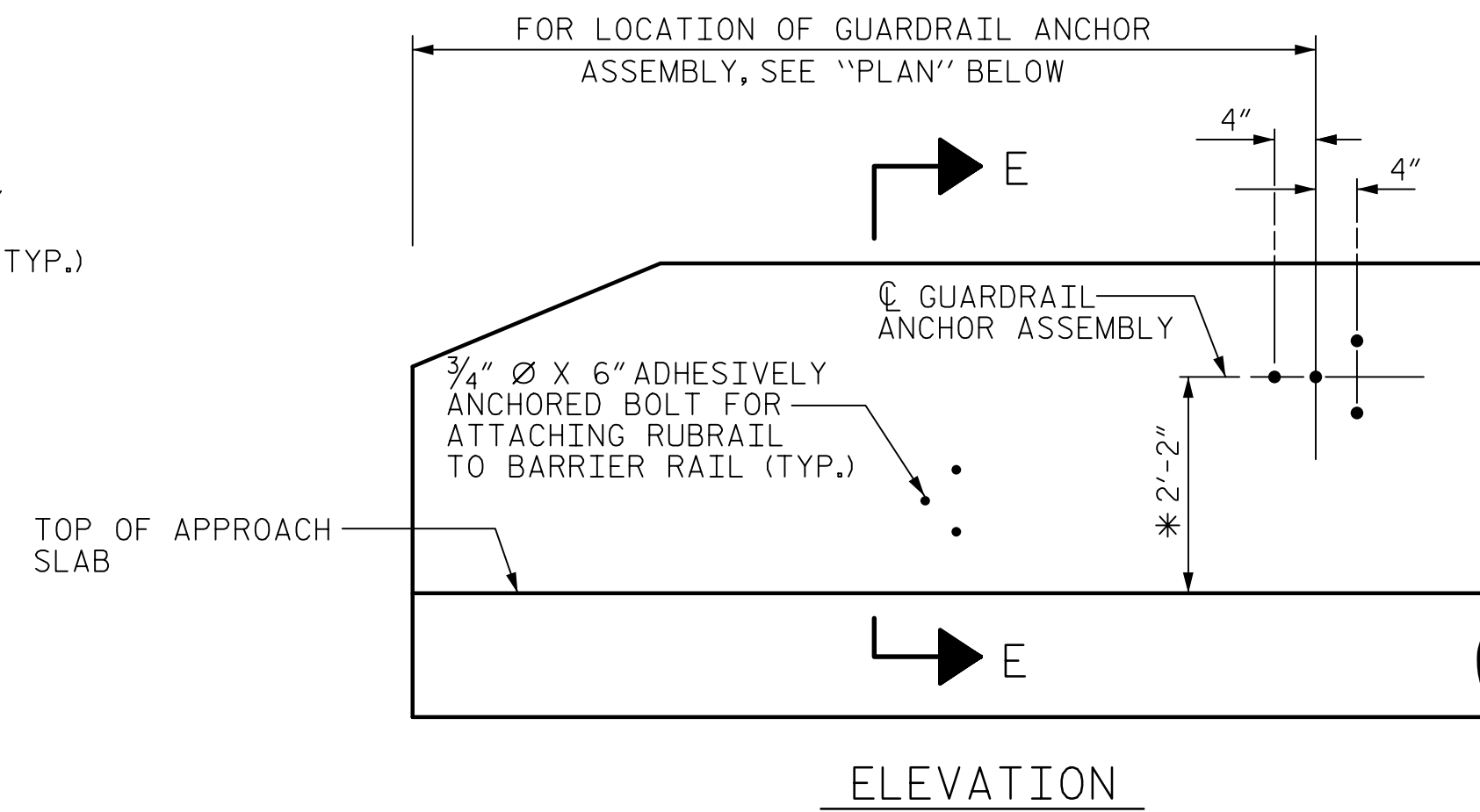
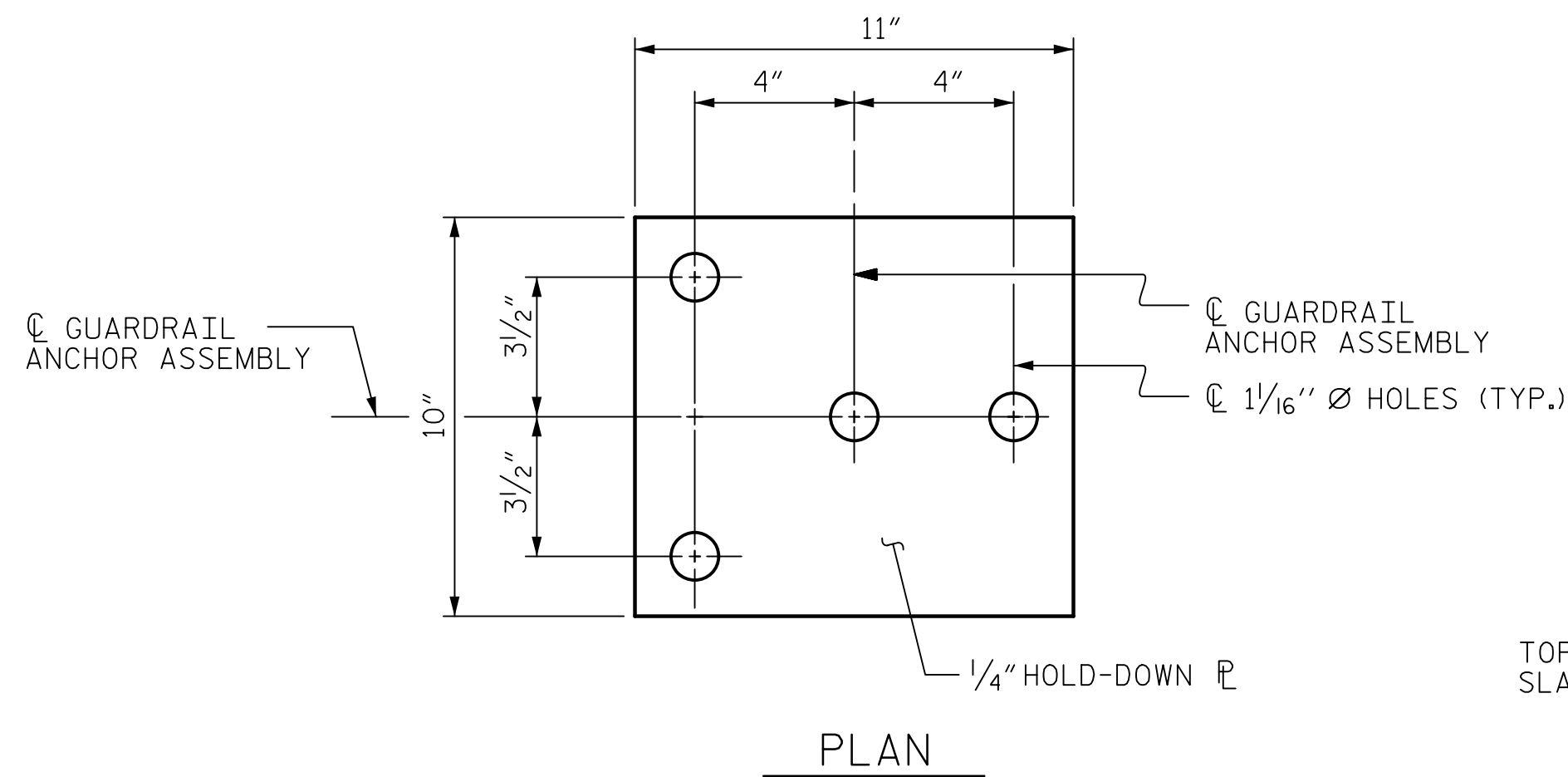
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #2 SHOWN, END BENT #1 SIMILAR.

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STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 GUARDRAIL ANCHORAGE  
 FOR BARRIER RAIL

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE

GUARDRAIL ANCHORAGE  
 FOR BARRIER RAIL

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NO.	BY:	DATE:	NO.	BY:	DATE:	S-101
1			3			TOTAL SHEETS
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DATE: 2/27/2020  
 TIME: 4:05:52 PM

USER: Merch/Resourced  
 DGN: R:\Structures\04 Drawings\01\15BPR.20\_SML\GRA.dgn

ASSEMBLED BY : K.M. DONALD	DATE : 1/2019
CHECKED BY : G.R. COLS	DATE : 1/2019
DRAWN BY : TLA 5/06	REV. 7/12 MAA/GM
CHECKED BY : GM 5/06	REV. 6/13 MAA/GM
	REV. 12/17 MAA/THC

\* DIMENSION SHOW TO TOP OF THE APPROACH SLAB PRIOR TO INSTALLATION OF PPC OVERLAY.



**NOTES:**

THE CONCRETE MEDIAN BARRIER IN EACH CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE CONCRETE MEDIAN BARRIER AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN CONCRETE MEDIAN BARRIER EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF CONCRETE BARRIER SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

ALL REINFORCING STEEL IN THE CONCRETE MEDIAN BARRIER SHALL BE EPOXY COATED.

CONCRETE MEDIAN BARRIER RAIL SHALL BE CONSTRUCTED DURING STAGE III.

\*5D1 AND \*5D2 SHALL BE MACHINE THREADED TO FIT TO COUPLER AND DELIVERED CONNECTED. THREADS SHALL BE GREASED TO PERMIT EASY SEPARATION AFTER INSTALLATION. COUPLERS ARE CONSIDERED INCIDENTAL TO THE COST OF CONCRETE MEDIAN BARRIER.

THE \*5D1 COUPLED TO \*5D2 SHALL BE PUSHED INTO GREEN CONCRETE AFTER POURING AND SCREEDING THE DECK. THE COUPLER SHALL BE INSTALLED TO BE FLUSH WITH THE TOP OF THE FINISHED CONCRETE DECK. ONCE DECK HAS CURED, \*5D2 SHALL BE UNTHREADED AND SET ASIDE. A TEMPORARY BOLT SHALL BE INSTALLED IN THE COUPLER AND SEALED TO PREVENT DEBRIS INTRUSION AND PROTECT THE THREADS. SEE DETAIL "D". WHEN CONSTRUCTING MEDIAN BARRIER, REMOVE SEALER AND TEMPORARY BOLT AND THREAD \*5D2 IN TO COUPLER.

THE CONTRACTOR MAY SUBMIT ALTERNATIVE DETAILS FOR ANCHORING THE MEDIAN BARRIER INTO THE DECK TO THE ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION OF THE DECK. ANY ALTERNATIVE METHODS APPROVED FOR CONSTRUCTION SHALL BE AT NO ADD'L COST TO THE DEPARTMENT.

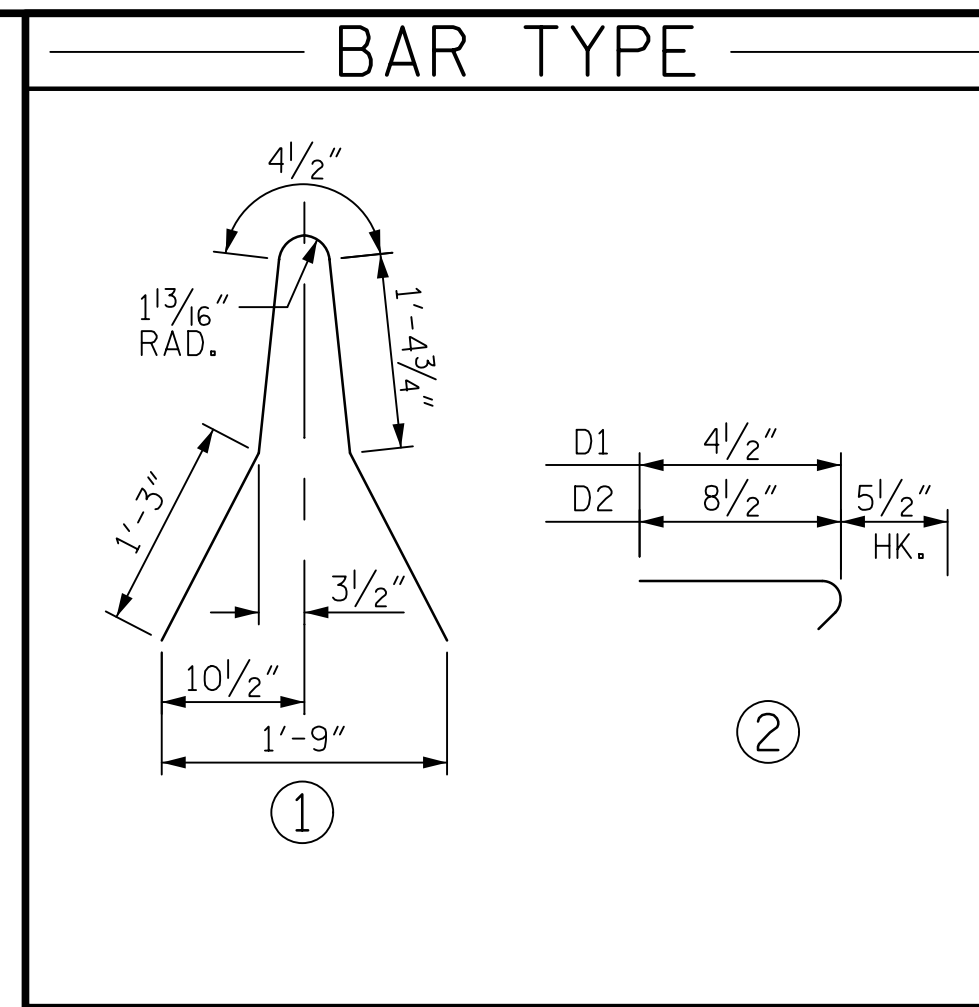
FOR MECHANICAL COUPLERS, SEE SPECIAL PROVISIONS.

S1, D1, AND D2 BARS IN THE BARRIER AND DECK SHALL BE SHIFTED SLIGHTLY, AS NECESSARY, TO PROVIDE 2" MIN. CLEARANCE TO ALL OPEN JOINTS IN THE BARRIER.

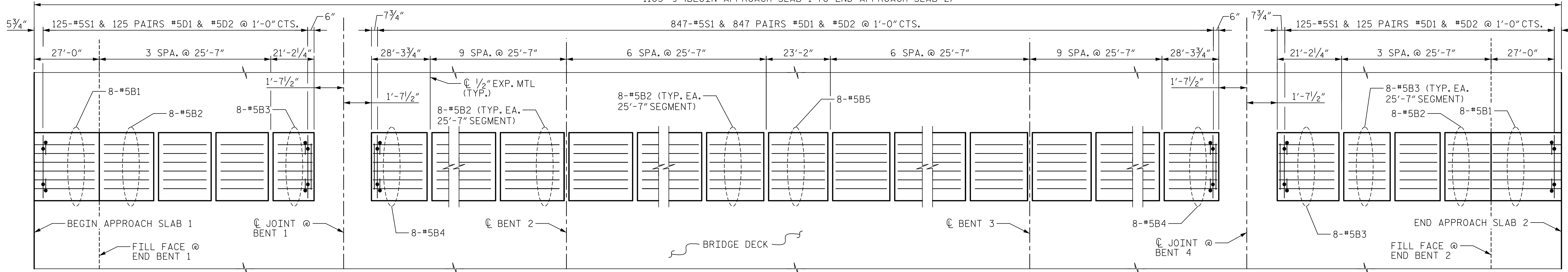
THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS FOR THE D1, D2, AND COUPLER PRIOR TO FABRICATION TO ENSURE PROPER FIT.

FOR ALL-LIGHTWEIGHT CONCRETE, SEE SPECIAL PROVISIONS.

BAR TYPE		BILL OF MATERIAL				
CONCRETE MEDIAN BARRIER						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
*B1	16	5	STR	26'-8"	445	
*B2	288	5	STR	25'-3"	7,585	
*B3	16	5	STR	20'-10"	348	
*B4	16	5	STR	27'-11"	466	
*B5	8	5	STR	22'-10"	191	
*S1	1097	5	1	5'-8"	6,484	
*D1	2194	5	2	10"	1,907	
*D2	2194	5	2	1'-2"	2,670	
*EPOXY COATED REINF. STEEL					20,096 LBS.	
ALL-LIGHTWEIGHT CONCRETE					118 CU. YDS.	
CONCRETE MEDIAN BARRIER (ALL-LIGHTWEIGHT CONCRETE)					1,097.25 LIN. FT.	

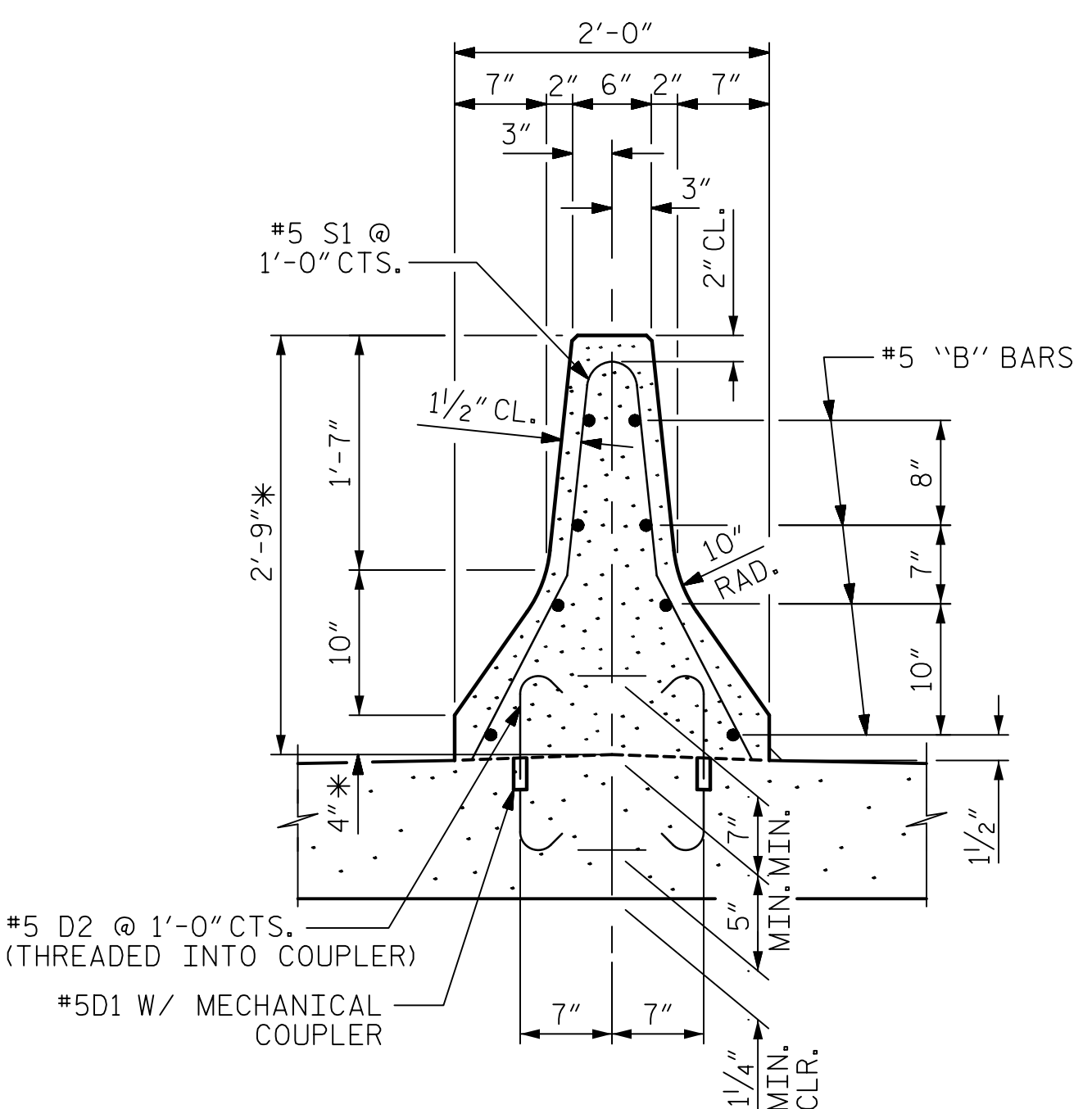


1103'-9" (BEGIN APPROACH SLAB 1 TO END APPROACH SLAB 2)



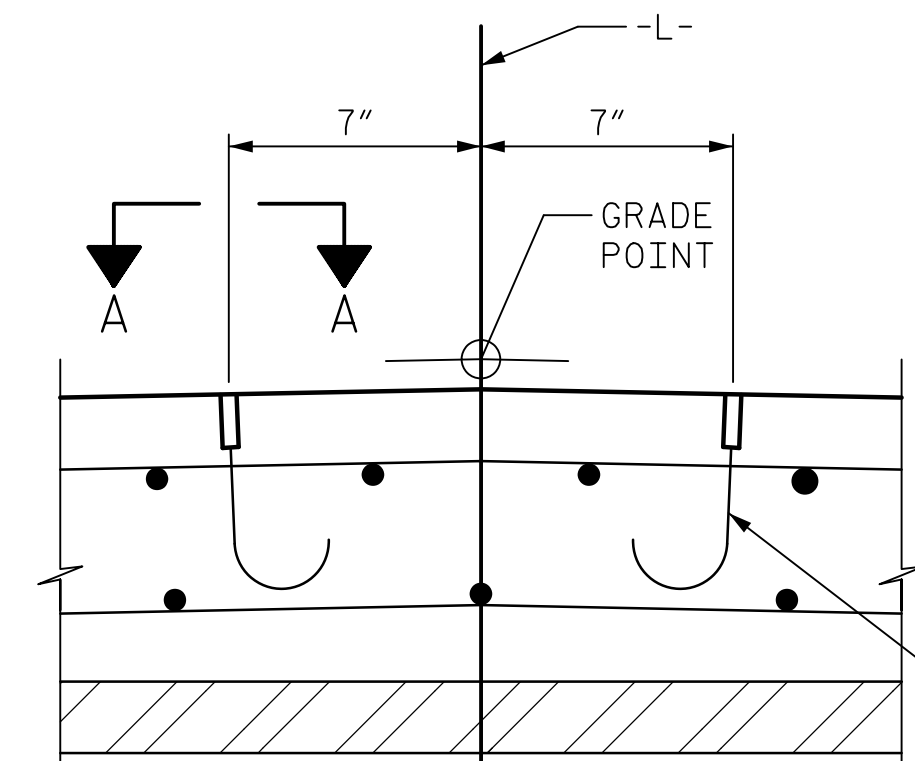
APPROACH SLAB 1      SPAN A      SPAN B      SPAN C      SPAN D      SPAN E      APPROACH SLAB 2

**PLAN OF CONCRETE MEDIAN BARRIER**



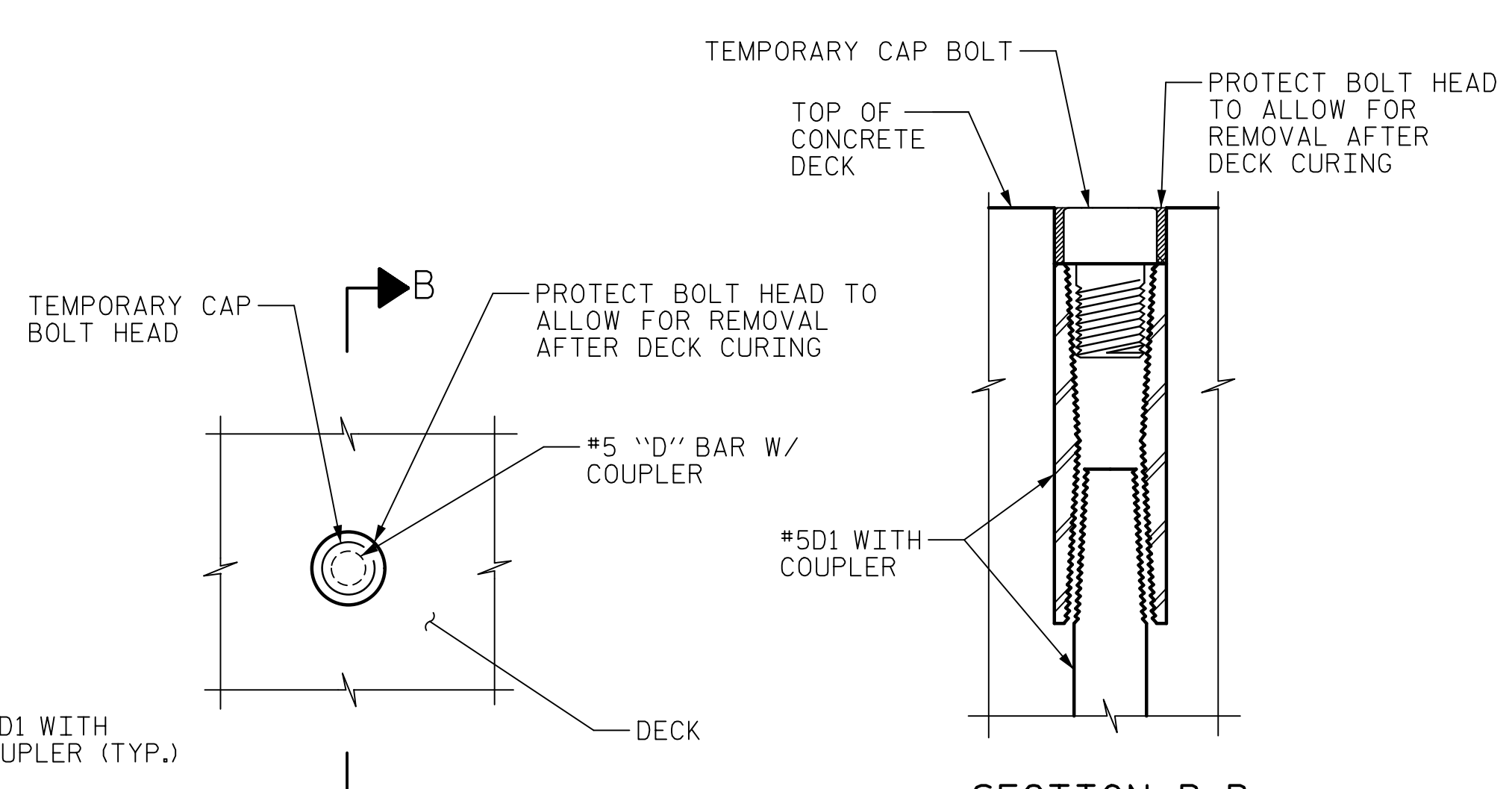
**SECTION THRU 2'-9" MEDIAN BARRIER**

\* DIMENSION TO TOP OF CONCRETE DECK PRIOR TO PPC OVERLAY



**SECTION THRU DECK**

SHOWN AFTER BARS PUSHED INTO GREEN CONCRETE AND \*5D2 UNTHREADED.



**VIEW A-A**

**SECTION B-B**

**DETAIL "D"**

**MEDIAN BARRIER COUPLER NOTES;**

- INSTALL #5 "D" BARS (WITH FEMALE COUPLER END) IN DECK AT LOCATIONS SHOWN ON "CONCRETE MEDIAN BARRIER" SHEET.
- INSTALL TEMPORARY BOLT INTO COUPLER. BOLT SHALL NOT PROJECT ABOVE THE TOP OF THE DECK.
- BOLT HEAD SHALL BE PROTECTED TO ALLOW FOR REMOVAL AFTER CURING OF DECK. ENGINEER SHALL APPROVE METHOD.
- POUR AND SCREED DECK.
- AT FINAL STAGE, REMOVE BOLT TO INSTALL #5 "D" BAR (WITH THREADED END) FOR MEDIAN BARRIER.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

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 (919) 854-6200 www.aecom.com  
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**SEAL**  
 JOHN E. SLOAN  
 ENGINEER  
 035062  
 2/28/2020

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-102
1			3			TOTAL SHEETS 129
2			4			

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DATE: 2/27/2020 TIME: 4:02:52 PM

USER: Meryl.Rasmussen\01\_189\_15BPR.20\_SML\_CBR.2.dgn

DRAWN BY: M.K. TOM	DATE: 1/2019
CHECKED BY: G. COLS	DATE: 2/2019
DESIGNED BY: G. COLS	DATE: 2/2019
DESIGN CHECKED BY: J. SLOAN	DATE: 2/2019



NOTES

PROPOSED JOINT SHALL BE WATSON BOWMAN TRANSFLEX REINFORCED ELASTOMERIC MOLDED RUBBER EXPANSION JOINT SYSTEM MODEL 1300 OR APPROVED EQUIVALENT.

ALL BLOCKOUT AND FORMED OPENING DIMENSIONS BASED ON MANUFACTURER-PROVIDED JOINT DETAILS AND SHALL BE VERIFIED PRIOR TO CONSTRUCTION.

JOINT MATERIALS SHALL BE ORDERED AND SHOP DRAWINGS APPROVED PRIOR TO CONSTRUCTION OF CONCRETE DECK AT THE JOINT.

FINAL JOINT SHALL NOT BE INSTALLED UNTIL PPC OVERLAY IS COMPLETE.

DIFFERENCES IN ACTUAL JOINT OPENINGS AND INSTALLATION TEMPERATURES SHALL BE REPORTED TO THE ENGINEER, CONTRACTOR SHALL FOLLOW MANUFACTURER'S INSTALLATION GUIDELINES AND MAKE ANY NECESSARY ADJUSTMENTS TO ACCOUNT FOR SUCH DIFFERENCES.

CONTRACTOR SHALL HAVE A REPRESENTATIVE FROM THE JOINT MANUFACTURER PRESENT DURING INSTALLATION OF THE PROPOSED MOLDED RUBBER SEGMENTAL EXPANSION JOINT.

ADHESIVE ANCHOR BOLTS AND HARDWARE FROM THE PROPOSED EXPANSION JOINT SHALL BE STAINLESS STEEL OR GALVANIZED PER ASTM A153 AND INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.

THE CONTRACTORS ATTENTION IS BROUGHT TO THE FACT THAT THE  $\text{C}$  JOINT DOES NOT COINCIDE WITH  $\text{C}$  BENT OR BENT CONTROL LINE.

FOR MOLDED RUBBER SEGMENTAL EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.

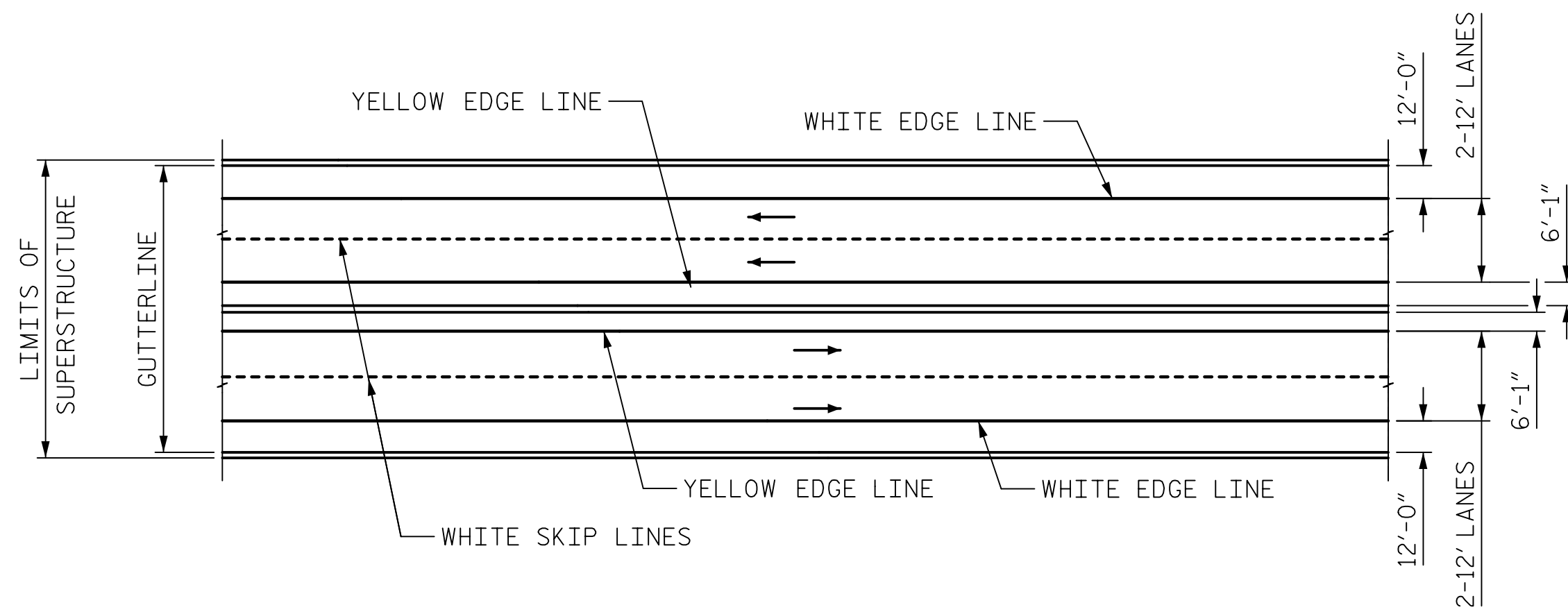
THE STEEL PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 OR APPROVED EQUAL AND BE PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. AT THE CONTRACTOR'S OPTION, THE PLATES MAY BE METALLIZED AFTER FABRICATION. SEE SPECIAL PROVISIONS FOR THERMAL SPRAYED COATINGS (METALLIZATION).

THE  $\frac{3}{4}$ "  $\text{O}$  HEX HEAD BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL.

THE  $\frac{3}{4}$ " CONCRETE INSERTS SHALL BE CLOSED-END FERRULES WITH LOOPED WIRE STRUTS ATTACHED TO THEM. THE INSERTS SHALL CONFORM TO AASHTO M169, GRADE 12L14 AND SHALL HAVE A TENSILE WORKING LOAD CAPACITY OF 3000 LBS.

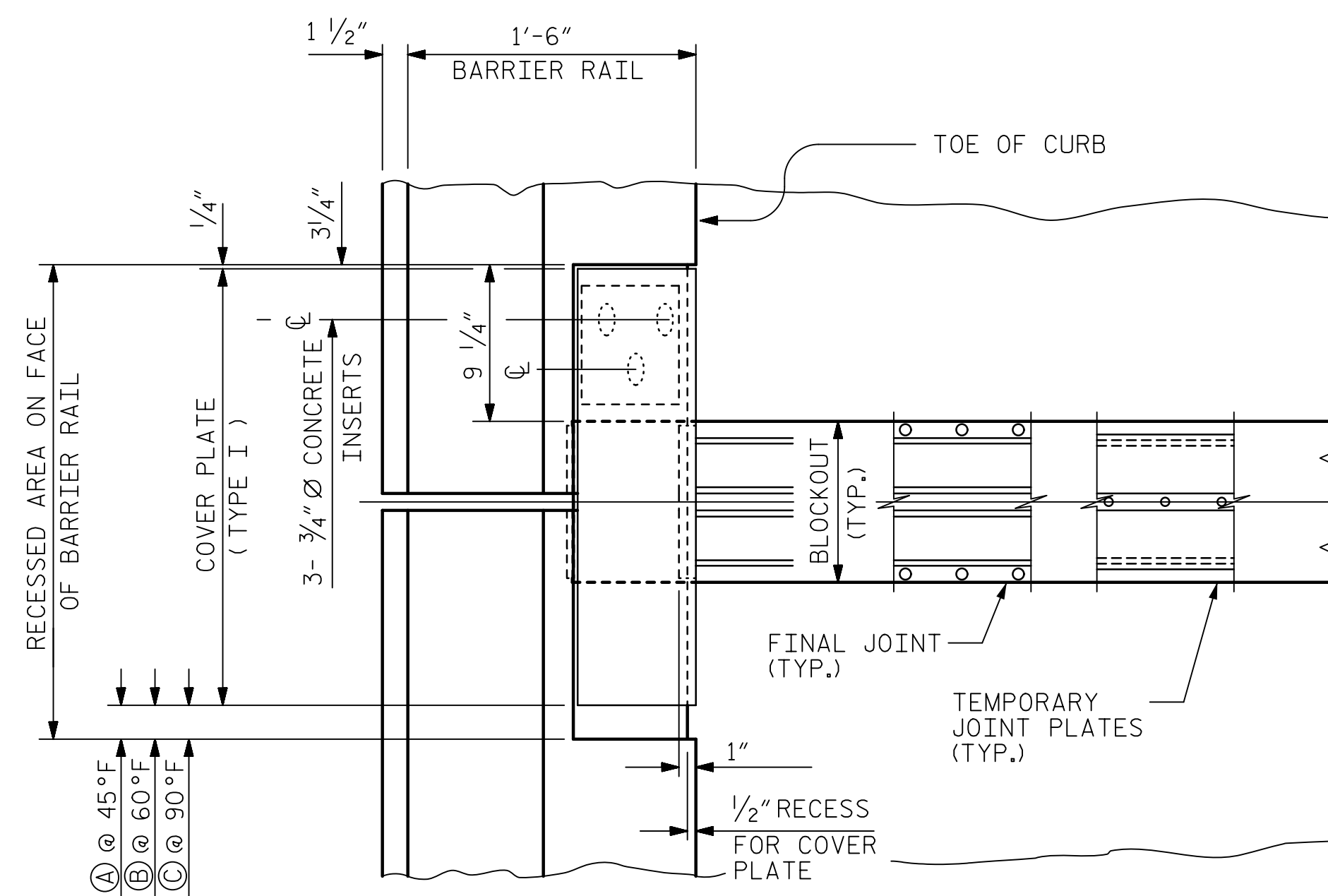
NO SEPARATE PAYMENT WILL BE MADE FOR FURNISHING AND INSTALLING THE COVER PLATES. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE LUMP SUM PRICE FOR "MOLDED RUBBER SEGMENTAL EXPANSION JOINT SEALS".

BARRIER COVER PLATES SHALL BE INSTALLED PRIOR TO TRAFFIC ADJACENT TO THAT FACE OF BARRIER.



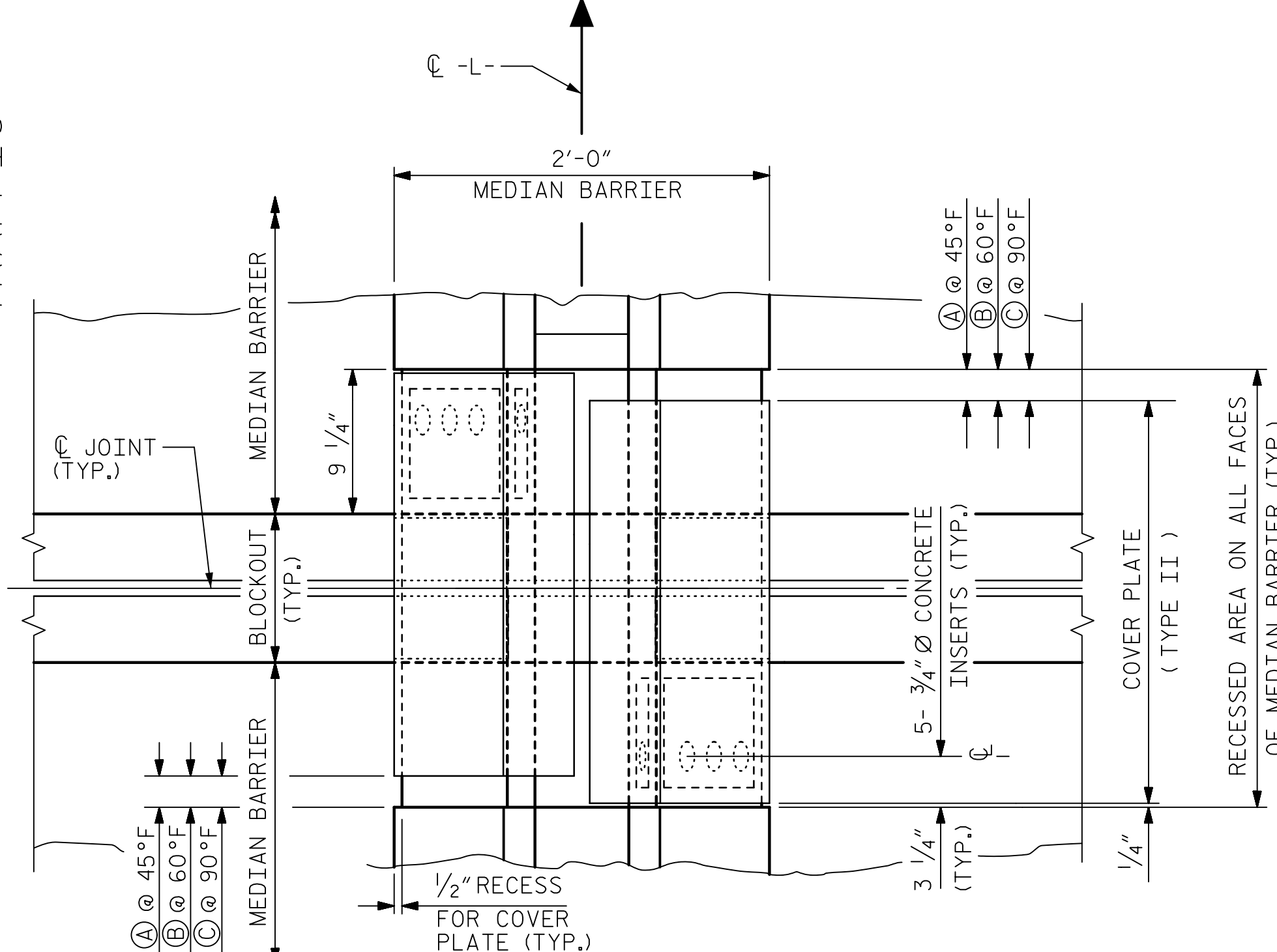
PAVEMENT MARKING ALIGNMENT

SHOWN AT FINAL STAGE. SEE TRANSPORTATION MANAGEMENT PLANS FOR PAVEMENT MARKINGS AT INTERMEDIATE STAGES



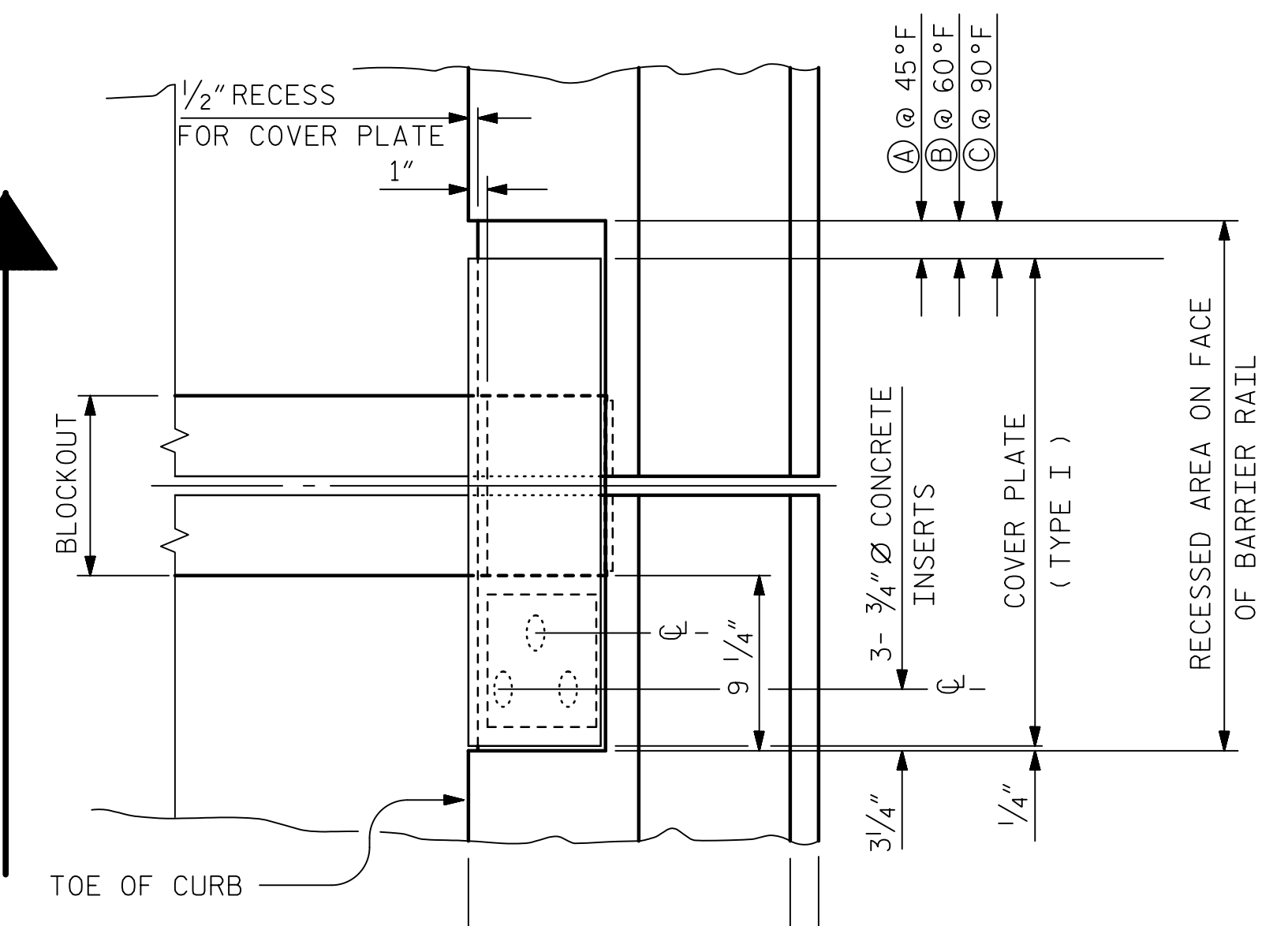
AT LEFT SIDE

FLOW OF TRAFFIC



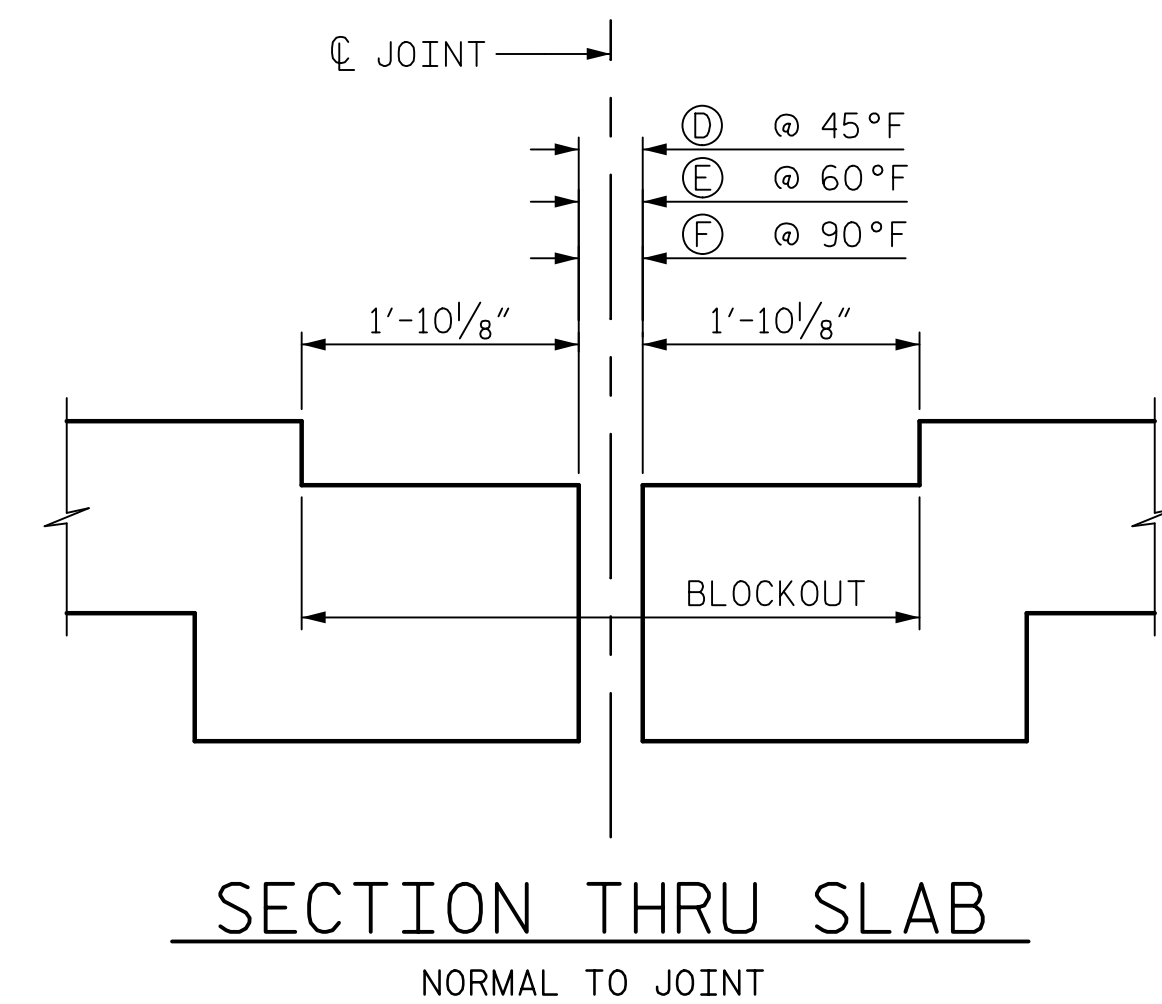
AT MEDIAN

FLOW OF TRAFFIC



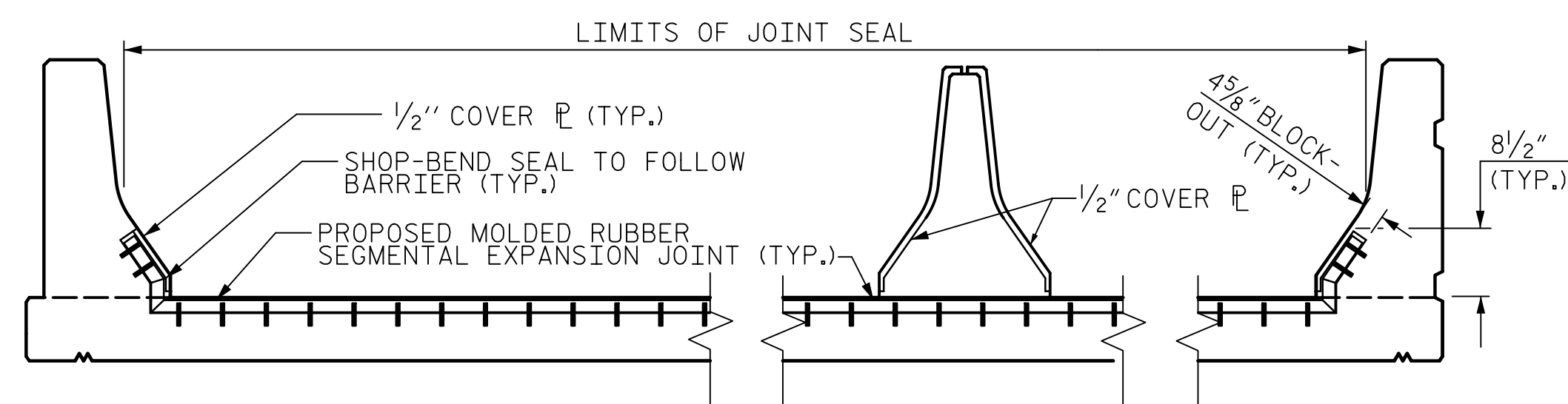
AT RIGHT SIDE

PLAN OF EXPANSION JOINT AT BENTS 1 AND 4



SECTION THRU SLAB

NORMAL TO JOINT

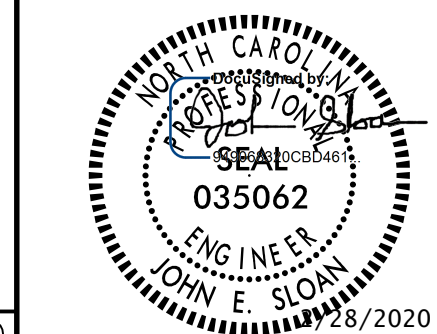


SKETCH SHOWING LIMITS OF JOINT SEAL AT BARRIER RAIL

MOVEMENT AND SETTING AT JOINT								
BENT NO.	SKIEW ANGLE	TOTAL MOVEMENT (ALONG $\text{C}$ RDWY)	A	B	C	D	E	F
1	90°-00'-00"	3 1/2"	3/4"	2 3/4"	1 3/4"	3 3/4"	3/4"	2 1/4"
4	90°-00'-00"	3 3/16"	3/8"	2 5/8"	1 1/16"	3 5/8"	3/8"	2 3/16"

PROJECT NO. 15BPR.20  
 HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUPERSTRUCTURE  
 MOLDED RUBBER  
 SEGMENTAL EXPANSION  
 JOINT SEAL DETAILS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-103
1			3			TOTAL SHEETS
2			4			129

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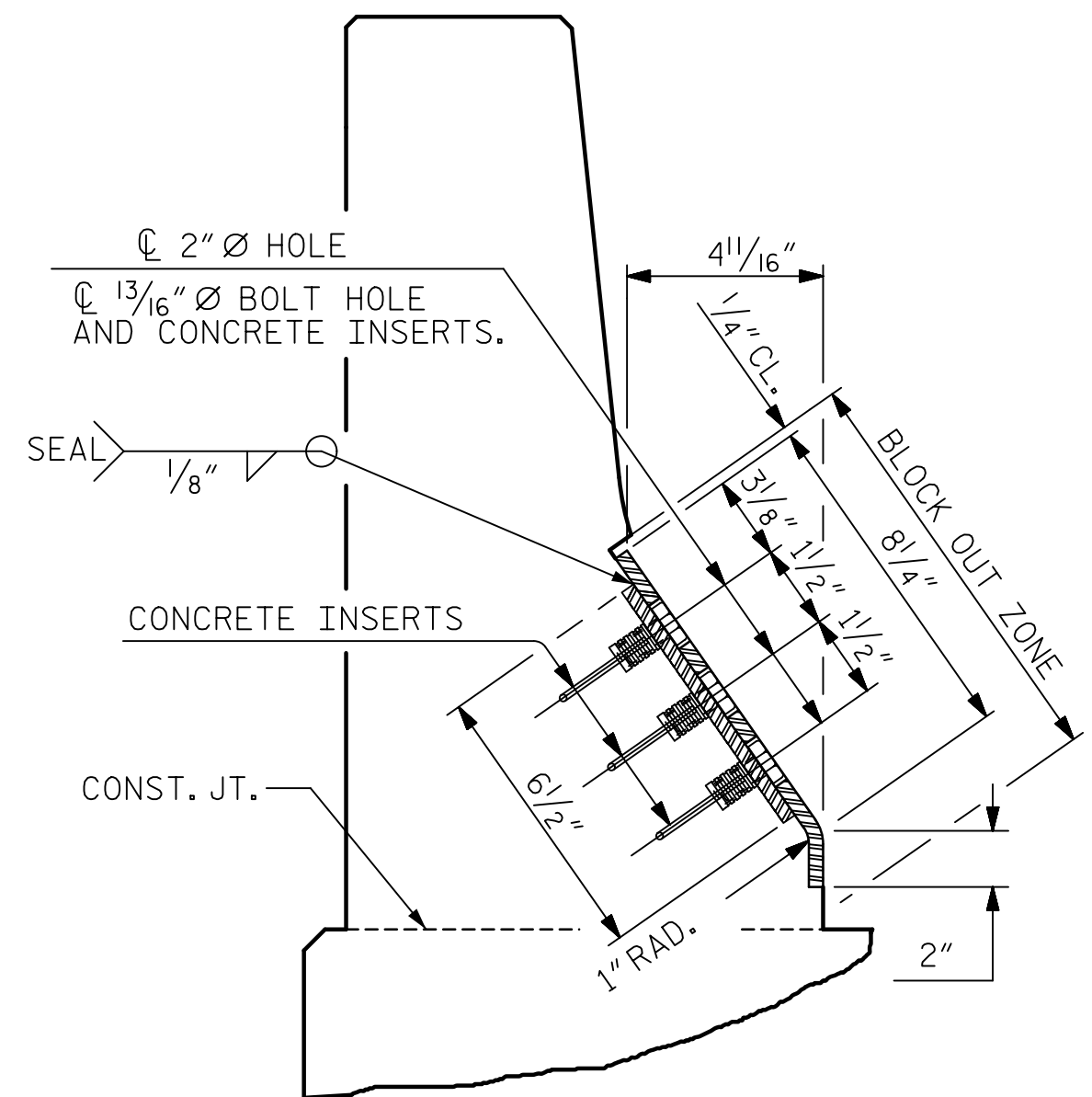
DRAWN BY: K.M. DONALD  
 CHECKED BY: G.R. COLS  
 DESIGNED BY: G.R. COLS  
 DESIGN CHECKED BY: K.M. DONALD

DATE: 01/2019  
 DATE: 02/2019  
 DATE: 02/2019  
 DATE: 02/2019

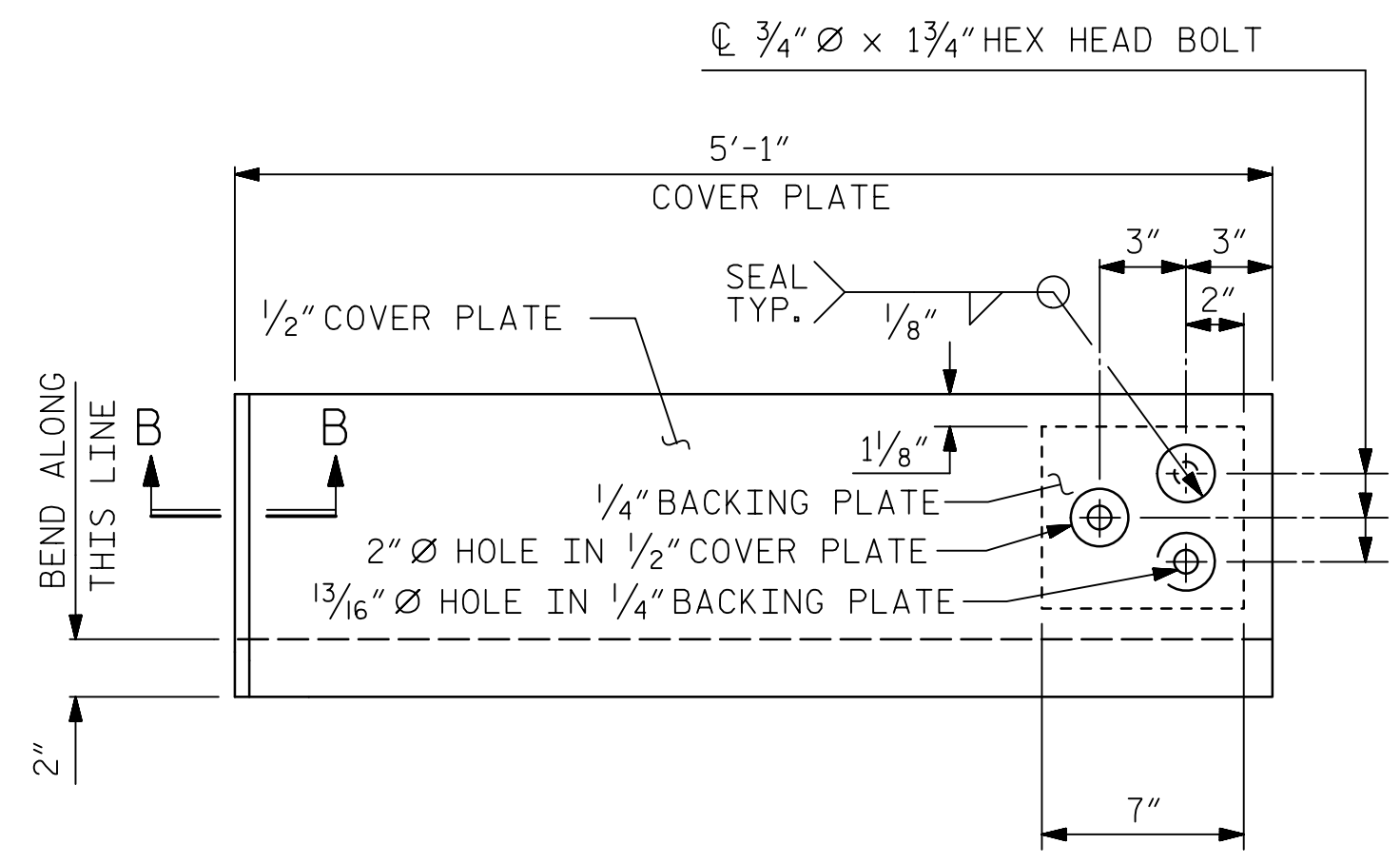


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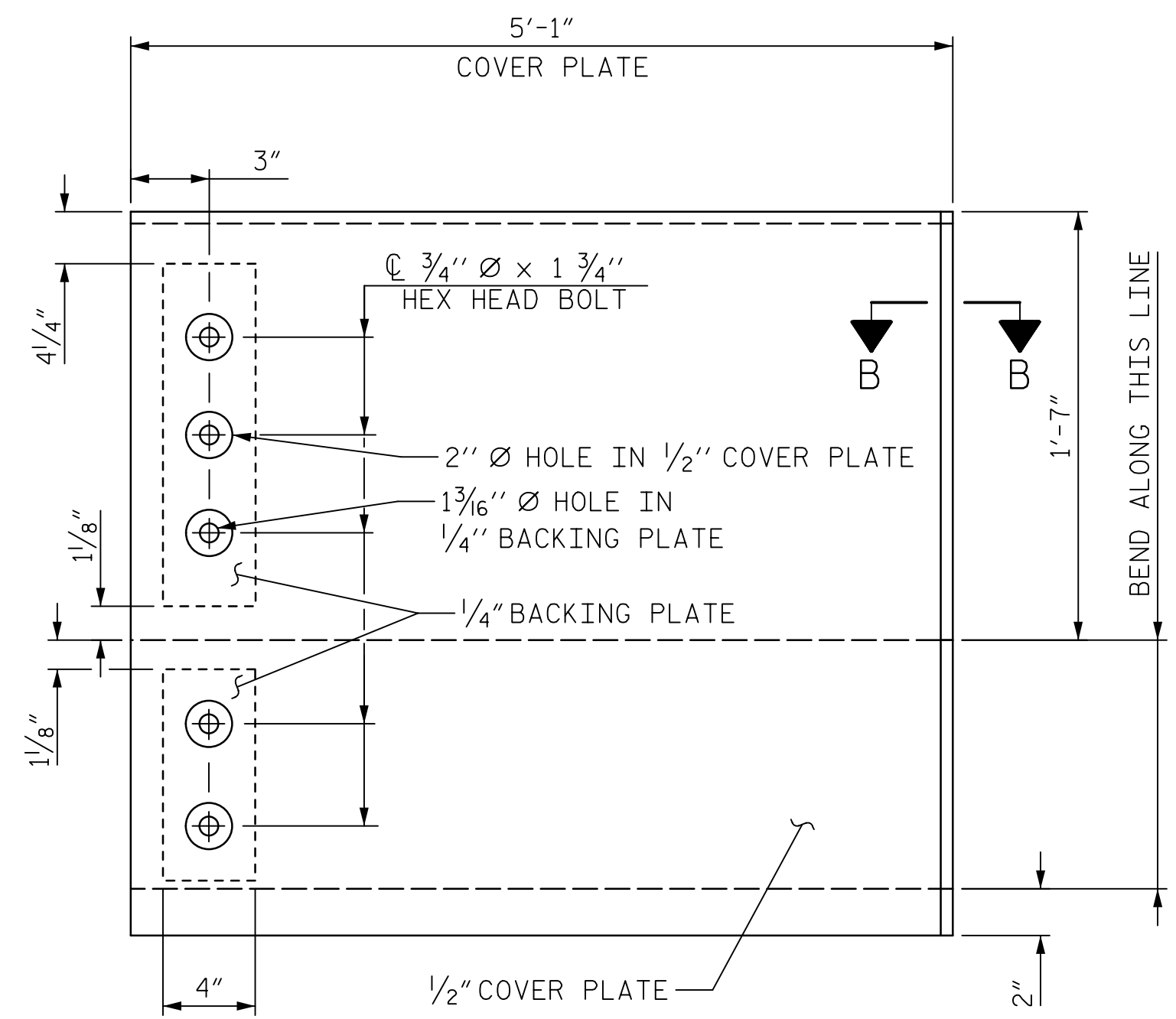
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SECTION THRU BARRIER RAIL NORMAL TO JOINT



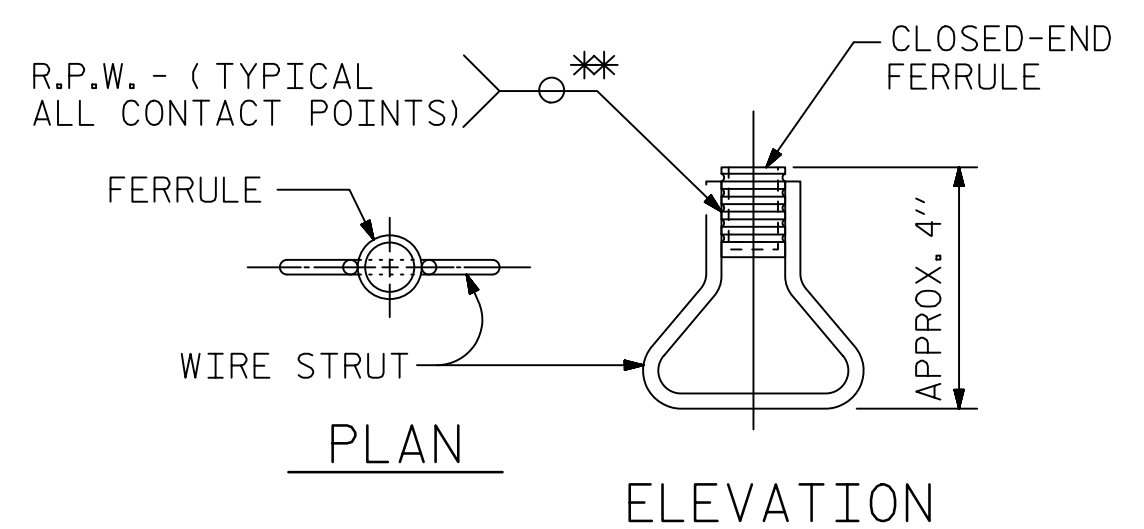
TYPE I - ELEVATION VIEW



TYPE II - ELEVATION VIEW

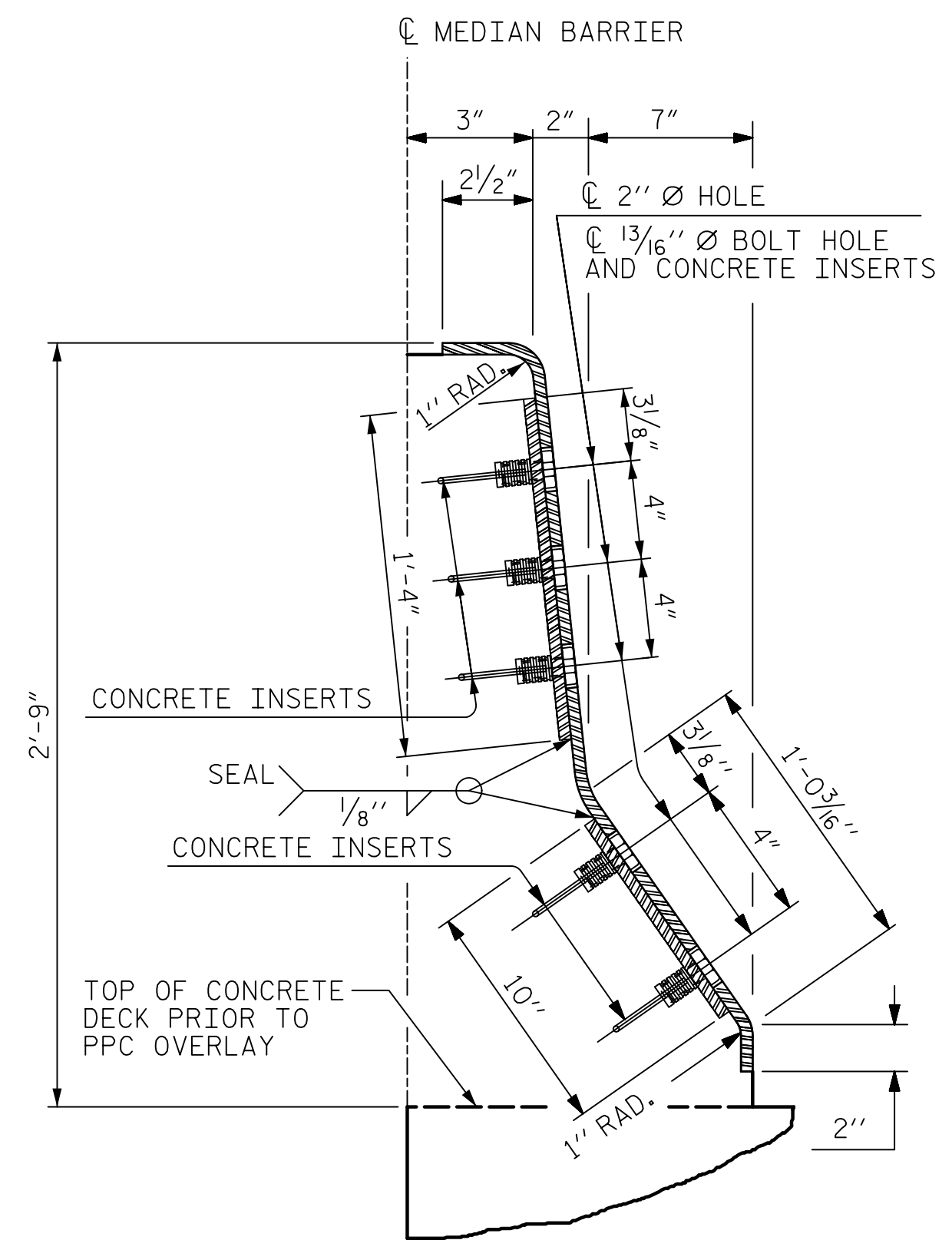
COVER PLATE DETAILS

NOTES:  
FOR NOTES, SEE SHEET 1.



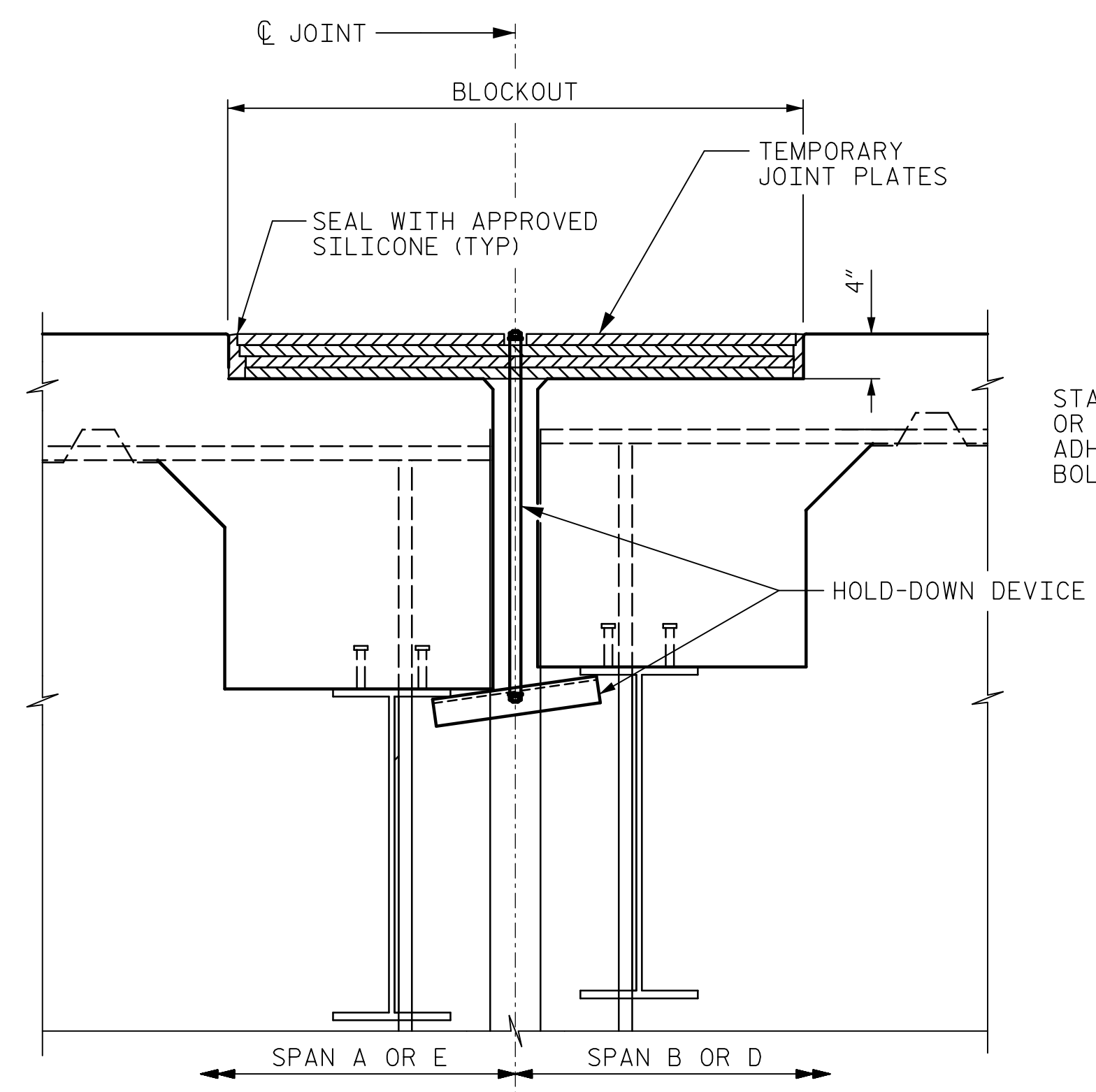
CONCRETE INSERT

\* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

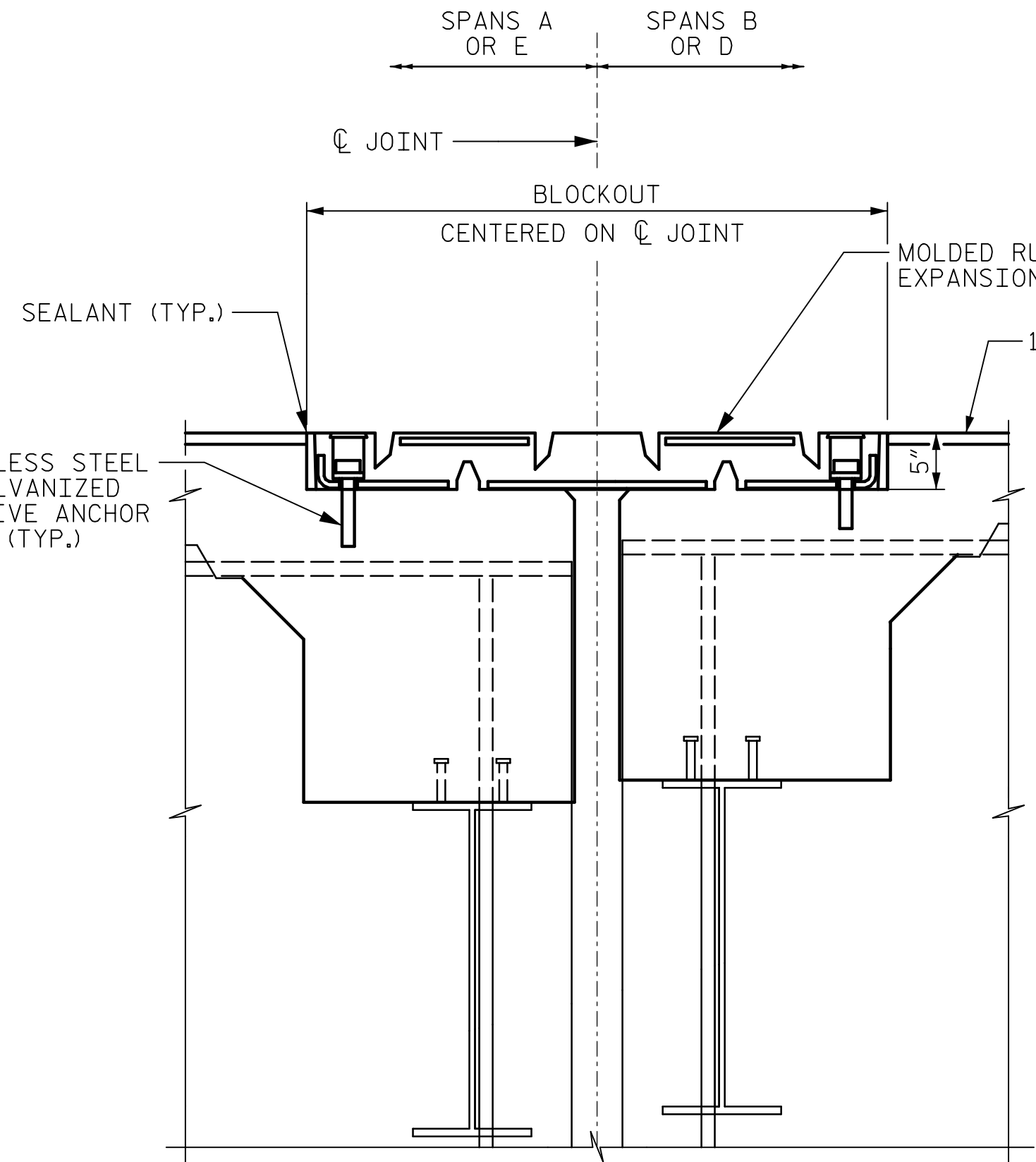


SECTION THRU MEDIAN BARRIER NORMAL TO JOINT

TRAFFIC FACE OF COVER PLATE SHALL GENERALLY FOLLOW MEDIAN BARRIER DIMENSIONS.



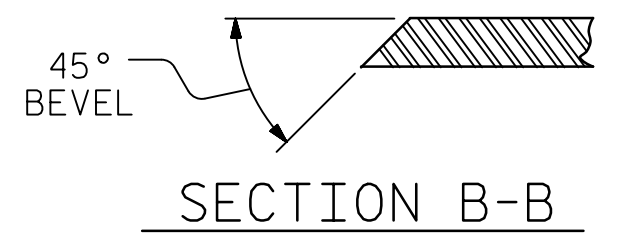
SECTION THRU TEMPORARY JOINT  
SCHEMATIC FOR REPRESENTATION ONLY. CONTRACTOR SHALL DESIGN JOINT PLATES AND HOLD DOWN DEVICE.



SECTION THRU FINAL JOINT

JOINT CONSTRUCTION SEQUENCE

1. CONSTRUCT DECK AND END DIAPHRAGM PER PLANS OR AS ADJUSTED FOR APPROVED JOINT TYPE.
2. INSTALL TEMPORARY JOINT AND BARRIER COVER PLATES. SEE SPECIAL PROVISIONS.
3. AFTER PPC OVERLAY IS COMPLETE IN THAT STAGE, REMOVE TEMPORARY JOINT PLATES. INSTALL MOLDED RUBBER SEGMENTAL EXPANSION JOINT PER MANUFACTURER'S SPECIFICATION. REINSTALL BARRIER COVER PLATES PRIOR TO TRAFFIC IN ADJACENT LANE.



PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
STATION: 35+30.22 -L-

SHEET 2 OF 2

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AECOM TECHNICAL SERVICES OF NC, INC.  
701 CORPORATE CENTER DRIVE, SUITE 475  
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STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

SUPERSTRUCTURE  
MOLDED RUBBER SEGMENTAL EXPANSION JOINT SEAL DETAILS

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1			3		
2			4		

SHEET NO. S-104  
TOTAL SHEETS 129

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DRAWN BY : K.M. DONALD	DATE : 01/2019
CHECKED BY : G.R. COLS	DATE : 01/2019
DESIGNED BY : G.R. COLS	DATE : 01/2019
DESIGN CHECKED BY : K.M. DONALD	DATE : 01/2019



### DEAD LOAD DEFLECTION TABLE FOR GIRDERS

	GIRDER #1 AND #4																		
	SPAN A & SPAN E																		
	(A1)	(E1)	0.08L	0.17L	(A2)	(E2)	0.33L	0.42L	(A3)	(E3)	0.58L	0.67L	(A4)	(E4)	0.83L	0.92L	(A5)	(E5)	
TENTH POINTS																			
ORIGINAL CAMBER	↑	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
DEFLECTION DUE TO WEIGHT OF TOTAL STEEL	↓	0.000	0.021	0.040	0.056	0.068	0.076	0.078	0.076	0.068	0.056	0.040	0.021	0.000					
DEFLECTION DUE TO WEIGHT OF SLAB AND RAIL	↓	0.000	0.060	0.116	0.163	0.198	0.221	0.230	0.221	0.198	0.163	0.116	0.060	0.000					
DEFLECTION DUE TO PPC	↓	0.000	0.004	0.014	0.024	0.030	0.034	0.036	0.035	0.033	0.028	0.021	0.011	0.000					
TOTAL DEAD LOAD DEFLECTION	↓	0.00	0.08	0.17	0.24	0.30	0.33	0.34	0.33	0.30	0.25	0.18	0.09	0.00					

	GIRDER #2 AND #3																		
	SPAN A & SPAN E																		
	(A1)	(E1)	0.08L	0.17L	(A2)	(E2)	0.33L	0.42L	(A3)	(E3)	0.58L	0.67L	(A4)	(E4)	0.83L	0.92L	(A5)	(E5)	
TENTH POINTS																			
ORIGINAL CAMBER	↑	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
DEFLECTION DUE TO WEIGHT OF TOTAL STEEL	↓	0.000	0.027	0.051	0.072	0.088	0.098	0.102	0.098	0.088	0.072	0.051	0.027	0.000					
DEFLECTION DUE TO WEIGHT OF SLAB AND RAIL	↓	0.000	0.083	0.157	0.220	0.268	0.300	0.312	0.300	0.268	0.219	0.156	0.082	0.000					
DEFLECTION DUE TO PPC	↓	0.000	0.004	0.014	0.022	0.028	0.032	0.033	0.033	0.031	0.025	0.019	0.010	0.000					
TOTAL DEAD LOAD DEFLECTION	↓	0.00	0.11	0.22	0.31	0.38	0.43	0.45	0.43	0.39	0.32	0.23	0.12	0.00					

DATE: 2/27/2020  
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**NOTES:**


- ## REPRESENTS CROSSFRAME POINT. SEE FRAMING PLAN SHEETS.
- ALL VALUES ARE SHOWN IN INCHES (DECIMAL FORM).

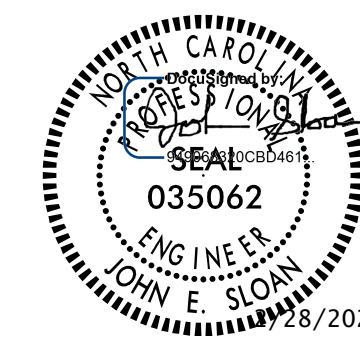
PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-

SHEET 1 OF 5

DRAWN BY : S. STREDNAK      DATE : 02/2019  
 CHECKED BY : J. SLOAN      DATE : 02/2019  
 DESIGNED BY : N. BROWN/D. RITACCO      DATE : 02/2019  
 DESIGN CHECKED BY : J. LIU      DATE : 02/2019

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	REVISIONS						
	NO.	BY:	DATE:	NO.	BY:	DATE:	
1				3			
2				4			





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DEAD LOAD DEFLECTION TABLE FOR GIRDERS																																				
GIRDER #1																																				
SPAN B																																				
	①	0.03L	0.06L	②	0.12L	0.15L	③	0.21L	0.24L	④	0.30L	0.33L	⑤	0.39L	0.42L	⑥	0.48L	0.52L	⑦	0.58L	0.61L	⑧	0.67L	0.70L	⑨	0.76L	0.79L	⑩	0.85L	0.88L	⑪	0.94L	0.97L	⑫		
A	DEFLECTION DUE TO EXISTING COVERPLATES	↓	0.004	0.026	0.048	0.070	0.091	0.111	0.129	0.145	0.159	0.170	0.180	0.187	0.192	0.195	0.195	0.194	0.189	0.183	0.174	0.164	0.152	0.138	0.124	0.109	0.093	0.078	0.063	0.049	0.037	0.026	0.018	0.011	0.007	0.006
B	DEFLECTION DUE TO NEW STEEL	↓	-0.031	-0.022	-0.014	-0.005	0.002	0.010	0.017	0.023	0.029	0.034	0.038	0.041	0.044	0.046	0.047	0.047	0.047	0.046	0.044	0.042	0.039	0.035	0.032	0.028	0.024	0.020	0.016	0.013	0.010	0.007	0.005	0.004	0.003	0.003
C	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 2	↓	-0.010	0.023	0.055	0.087	0.116	0.144	0.170	0.193	0.213	0.231	0.244	0.254	0.260	0.263	0.262	0.258	0.249	0.238	0.225	0.208	0.189	0.170	0.148	0.126	0.105	0.084	0.065	0.049	0.035	0.024	0.016	0.012	0.010	0.012
D	DEFLECTION DUE TO BAY 3 DECK REMOVAL	↓	0.011	0.016	0.023	0.029	0.036	0.042	0.049	0.054	0.060	0.065	0.069	0.072	0.075	0.077	0.078	0.078	0.077	0.076	0.074	0.070	0.066	0.062	0.057	0.051	0.045	0.039	0.032	0.027	0.021	0.016	0.011	0.007	0.004	0.002
E	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 3	↓	-0.020	-0.032	-0.045	-0.058	-0.070	-0.082	-0.093	-0.103	-0.112	-0.120	-0.126	-0.131	-0.135	-0.136	-0.137	-0.136	-0.133	-0.129	-0.123	-0.116	-0.108	-0.099	-0.089	-0.078	-0.068	-0.057	-0.047	-0.037	-0.028	-0.020	-0.014	-0.009	-0.005	-0.003
F	DEFLECTION DUE TO BAY 1 DECK REMOVAL	↓	-0.031	-0.253	-0.473	-0.686	-0.887	-1.076	-1.249	-1.403	-1.537	-1.650	-1.740	-1.809	-1.855	-1.876	-1.875	-1.853	-1.808	-1.743	-1.660	-1.558	-1.441	-1.312	-1.173	-1.029	-0.883	-0.738	-0.601	-0.474	-0.359	-0.259	-0.178	-0.115	-0.072	-0.057
G	DEFLECTION DUE TO SLAB IN BAY 1	↓	0.035	0.262	0.486	0.703	0.910	1.104	1.282	1.441	1.582	1.703	1.800	1.876	1.930	1.958	1.964	1.948	1.908	1.848	1.768	1.668	1.550	1.417	1.274	1.121	0.966	0.816	0.672	0.538	0.416	0.307	0.215	0.140	0.082	0.049
H	DEFLECTION DUE TO FINAL BARRIER RAILS AND PPC	↓	0.017	0.072	0.128	0.181	0.232	0.280	0.324	0.364	0.399	0.428	0.453	0.472	0.485	0.492	0.493	0.489	0.479	0.465	0.445	0.421	0.393	0.361	0.327	0.290	0.254	0.217	0.181	0.147	0.116	0.087	0.062	0.043	0.029	0.022
I	TOTAL SDL ( I = A+B+C+D+E+F+G+H )	↓	-0.026	0.092	0.209	0.321	0.431	0.534	0.628	0.715	0.793	0.861	0.917	0.963	0.997	1.018	1.027	1.025	1.010	0.983	0.947	0.899	0.840	0.772	0.698	0.618	0.536	0.458	0.383	0.312	0.247	0.188	0.136	0.093	0.058	0.034

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																																													
GIRDER #1																																													
SPAN C																																													
	⑫	0.02L	0.05L	⑬	0.10L	0.12L	⑭	0.17L	0.19L	⑮	0.24L	0.26L	⑯	0.31L	0.33L	⑰	0.38L	0.40L	⑱	0.45L	0.48L	⑲	0.52L	0.55L	⑳	0.60L	0.62L	㉑	0.67L	0.69L	㉒	0.74L	0.76L	㉓	0.81L	0.83L	㉔	0.88L	0.90L	㉕	0.95L	0.98L	㉖		
A	DEFLECTION DUE TO EXISTING COVERPLATES	↓	0.006	0.010	0.017	0.027	0.038	0.052	0.068	0.085	0.103	0.122	0.141	0.160	0.178	0.195	0.211	0.225	0.238	0.249	0.257	0.263	0.267	0.268	0.267	0.263	0.257	0.249	0.238	0.225	0.211	0.195	0.178	0.160	0.141	0.122	0.103	0.085	0.068	0.052	0.038	0.027	0.017	0.010	0.006
B	DEFLECTION DUE TO NEW STEEL	↓	0.003	0.004	0.006	0.009	0.013	0.017	0.022	0.027	0.032	0.038	0.044	0.050	0.056	0.061	0.066	0.071	0.075	0.078	0.081	0.083	0.084	0.085	0.084	0.083	0.081	0.078	0.075	0.071	0.067	0.062	0.056	0.050	0.045	0.039	0.033	0.027	0.022	0.017	0.013	0.010	0.007	0.004	0.003
C	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 2	↓	0.012	0.020	0.031	0.046	0.063	0.083	0.107	0.132	0.160	0.190	0.220	0.250	0.280	0.308	0.334	0.359	0.379	0.397	0.412	0.422	0.429	0.432	0.429	0.423	0.414	0.400	0.382	0.362	0.338	0.311	0.284	0.254	0.223	0.193	0.163	0.135	0.109	0.085	0.064	0.046	0.031	0.020	0.011
D	DEFLECTION DUE TO BAY 3 DECK REMOVAL	↓	0.002	0.003	0.005	0.009	0.013	0.018	0.024	0.030	0.037	0.045	0.052	0.059	0.066	0.073	0.079	0.084	0.089	0.092	0.096	0.098	0.099	0.100	0.099	0.098	0.096	0.093	0.089	0.084	0.079	0.073	0.067	0.059	0.052	0.045	0.037	0.030	0.024	0.018	0.013	0.009	0.005	0.003	0.002
E	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 3	↓	-0.003	-0.005	-0.009	-0.014	-0.020	-0.027	-0.036	-0.045	-0.056	-0.067	-0.078	-0.089	-0.100	-0.110	-0.120	-0.129	-0.136	-0.143	-0.148	-0.151	-0.153	-0.155	-0.153	-0.151	-0.148	-0.142	-0.136	-0.129	-0.120	-0.110	-0.100	-0.089	-0.077	-0.066	-0.055	-0.045	-0.035	-0.027	-0.019	-0.013	-0.008	-0.005	-0.003
F	DEFLECTION DUE TO BAY 1 DECK REMOVAL	↓	-0.057	-0.094	-0.158	-0.243	-0.344	-0.463	-0.598	-0.745	-0.904	-1.069	-1.237	-1.403	-1.565	-1.717	-1.859	-1.987	-2.098	-2.191	-2.267	-2.320	-2.353	-2.364	-2.353	-2.320	-2.267	-2.192	-2.098	-1.987	-1.859	-1.717	-1.565	-1.404	-1.237	-1.070	-0.904	-0.745	-0.598	-0.463	-0.344	-0.242	-0.158	-0.093	-0.055
G	DEFLECTION DUE TO SLAB IN BAY 1	↓	0.049	0.064	0.102	0.159	0.231	0.319	0.421	0.535	0.659	0.788	0.921	1.052	1.179	1.301	1.413	1.514	1.601	1.675	1.733	1.774	1.798	1.805	1.793	1.764	1.718	1.655	1.579	1.489	1.387	1.274	1.153	1.027	0.899	0.771	0.645	0.523	0.411	0.312	0.225	0.155	0.099	0.061	0.047
H	DEFLECTION DUE TO FINAL BARRIER RAILS AND PPC	↓	0.022	0.036	0.057	0.084	0.115	0.151	0.191	0.235	0.281	0.329	0.377	0.423	0.467	0.509	0.547	0.581	0.610	0.634	0.652	0.664	0.671	0.672	0.668	0.659	0.644	0.624	0.600	0.570	0.537	0.499	0.458	0.415	0.370	0.324	0.278	0.233	0.190	0.151	0.115	0.084	0.057	0.035	0.022
I	TOTAL SDL ( I = A+B+C+D+E+F+G+H )	↓	0.034	0.038	0.053	0.077	0.110	0.151	0.199	0.253	0.314	0.375	0.439	0.502	0.561	0.619	0.671	0.718	0.758	0.791	0.817	0.833	0.841	0.842	0.834	0.818	0.796	0.765	0.728	0.686	0.639	0.587	0.531	0.474	0.416	0.357	0.300	0.243	0.190	0.145	0.106	0.074	0.050	0.036	0.033

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																																				
GIRDER #1																																				
SPAN D																																				
	㉖	0.03L	0.06L	㉗	0.12L	0.15L	㉘	0.21L	0.24L	㉙	0.30L	0.33L	⑳	0.39L	0.42L	㉑	0.48L	0.52L	㉒	0.58L	0.61L	㉓	0.67L	0.70L	㉔	0.76L	0.79L	㉕	0.85L	0.88L	㉖	0.94L	0.97L	㉗		
A	DEFLECTION DUE TO EXISTING COVERPLATES	↓	0.006	0.007	0.011	0.017	0.026	0.036	0.049	0.062	0.077	0.093	0.108	0.123	0.138	0.151	0.163	0.174	0.182	0.189	0.193	0.195	0.194	0.192	0.187	0.179	0.170	0.158	0.144	0.128	0.110	0.091	0.070	0.048	0.026	0.003
B	DEFLECTION DUE TO NEW STEEL	↓	0.003	0.003	0.004	0.005	0.007	0.010	0.013	0.016	0.020	0.024	0.028	0.032	0.036	0.040	0.043	0.046	0.048	0.049	0.050	0.050	0.049	0.047	0.045	0.042	0.038	0.033	0.027	0.021	0.015	0.007	0.000	-0.008	-0.017	-0.025
C	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 2	↓	0.011	0.009	0.011	0.015	0.023	0.034	0.048	0.064	0.082	0.103	0.124	0.146	0.168	0.187	0.206	0.223	0.236	0.247	0.256	0.260	0.261	0.259	0.252	0.242	0.229	0.212	0.192	0.169	0.143	0.115	0.086	0.054	0.022	-0.012
D	DEFLECTION DUE TO BAY 3 DECK REMOVAL	↓	0.002	0.004	0.007	0.011	0.016	0.021	0.027	0.032	0.039	0.045	0.051	0.057	0.062	0.066	0.070	0.074	0.076	0.077	0.078	0.077	0.076	0.075	0.072	0.068	0.064	0.059	0.054	0.048	0.042	0.036	0.029	0.023	0.016	0.011
E	DEFLECTION DUE TO SLAB AND BARRIER RAIL IN BAY 3	↓	-0.003	-0.005	-0.009	-0.014	-0.021	-0.028	-0.037	-0.047	-0.057	-0.068	-0.079	-0.089	-0.099	-0.108	-0.116	-0.123	-0.129	-0.133	-0.136	-0.137	-0.136	-0.135	-0.131	-0.126	-0.120	-0.111	-0.102	-0.093	-0.081	-0.070	-0.057	-0.045	-0.032	-0.020
F	DEFLECTION DUE TO BAY 1 DECK REMOVAL	↓	-0.055	-0.070	-0.113	-0.175	-0.255	-0.355	-0.470	-0.596	-0.733	-0.877	-1.023	-1.167	-1.305	-1.433	-1.550	-1.651	-1.734	-1.799	-1.844	-1.866	-1.867	-1.845	-1.799	-1.731	-1.641	-1.528	-1.395	-1.242	-1.069	-0.881	-0.681	-0.469	-0.249	-0.030
G	DEFLECTION DUE TO SLAB IN BAY 1	↓	0.047	0.082	0.140	0.216	0.308	0.417	0.540	0.674	0.818	0.968	1.123	1.275	1.419	1.551	1.668	1.768	1.847	1.907	1.947	1.962	1.956	1.927	1.873	1.797	1.700	1.578	1.437	1.278	1.100	0.907	0.700	0.484	0.260	0.034
H	DEFLECTION DUE TO FINAL BARRIER RAILS AND PPC	↓	0.022	0.026	0.039	0.057	0.081	0.108	0.139	0.172	0.207	0.243	0.279	0.314	0.347	0.377	0.405	0.427	0.446	0.460	0.469	0.473	0.472	0.465	0.453	0.436	0.412	0.384	0.350	0.312						



DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with 28 columns (span points 1-12) and 9 rows (A-I). Title: DEAD LOAD DEFLECTION TABLE FOR GIRDERS, GIRDER #2, SPAN B.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with 28 columns (span points 12-26) and 9 rows (A-I). Title: DEAD LOAD DEFLECTION TABLE FOR GIRDERS, GIRDER #2, SPAN C.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with 28 columns (span points 26-37) and 9 rows (A-I). Title: DEAD LOAD DEFLECTION TABLE FOR GIRDERS, GIRDER #2, SPAN D.

NOTES:

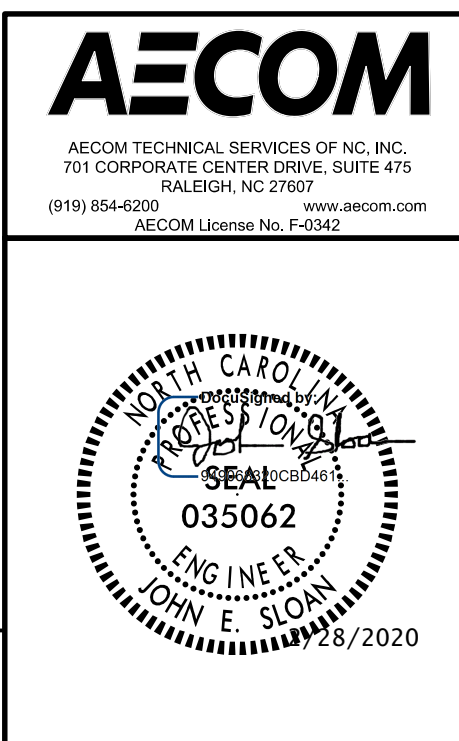
- ## REPRESENTS CROSSFRAME POINT. SEE FRAMING PLAN SHEETS. ALL VALUES ARE SHOWN IN INCHES (DECIMAL FORM).

PROJECT NO. 15BPR.20
HENDERSON COUNTY
STATION: 35+30.22 -L-

SHEET 3 OF 5

DRAWN BY : S. STREDNAK DATE : 02/2019
CHECKED BY : J. SLOAN DATE : 02/2019
DESIGNED BY : N. BROWN/D. RITACCO DATE : 02/2019
DESIGN CHECKED BY : J. LIU DATE : 02/2019

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



AECOM logo and project information: STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE DEAD LOAD DEFLECTION TABLES. Includes a table for REVISIONS and SHEET NO. S-107.

DATE: 2/27/2020 TIME: 4:02:25 PM
USER: Meryl Ressemond Path: R:\Structures\04 Drawings\401\_09\_15BPR.20\_SML.DWG



DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with columns for GIRDERS (A-I), SPAN B (1-12), and various deflection values. Includes rows for existing coverplates, new steel, slab and barrier rail, deck removal, and total SDL.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with columns for GIRDERS (A-I), SPAN C (12-26), and various deflection values. Includes rows for existing coverplates, new steel, slab and barrier rail, deck removal, and total SDL.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

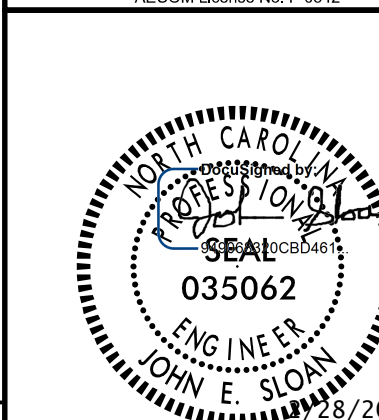
Table with columns for GIRDERS (A-I), SPAN D (26-37), and various deflection values. Includes rows for existing coverplates, new steel, slab and barrier rail, deck removal, and total SDL.

NOTES:

## REPRESENTS CROSSFRAME POINT. SEE FRAMING PLAN SHEETS. ALL VALUES ARE SHOWN IN INCHES (DECIMAL FORM).

PROJECT NO. 15BPR.20
HENDERSON COUNTY
STATION: 35+30.22 -L-

SHEET 4 OF 5



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

DEAD LOAD DEFLECTION TABLES

Revisions table with columns for NO., BY, DATE, NO., BY, DATE, and SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: S. STREDNAK
CHECKED BY: J. SLOAN
DESIGNED BY: N. BROWN/D. RITACCO
DESIGN CHECKED BY: J. LIU

DATE: 02/02/2020
TIME: 4:05:05 PM

USER: Merit Research
DOB: R:\Structures\04 Drawings\01\_2015\BPR20\_SML-DL4.dgn



DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with 28 columns (span points 1-12) and 9 rows (A-I) showing deflection values for GIRDERS SPAN B.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

Table with 28 columns (span points 12-26) and 9 rows (A-I) showing deflection values for GIRDERS SPAN C.

DEAD LOAD DEFLECTION TABLE FOR GIRDERS

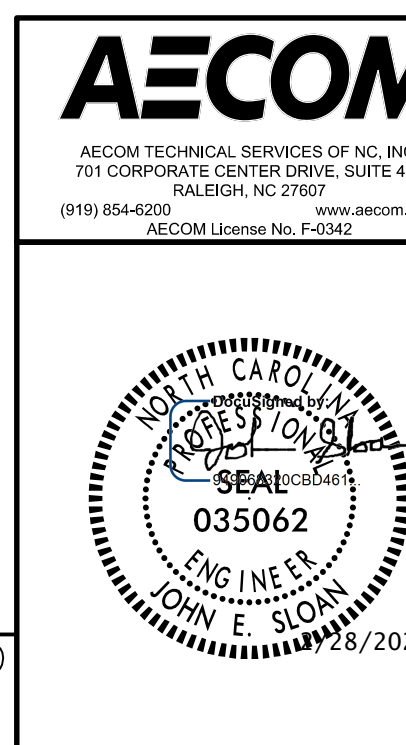
Table with 28 columns (span points 26-37) and 9 rows (A-I) showing deflection values for GIRDERS SPAN D.

NOTES:

## REPRESENTS CROSSFRAME POINT. SEE FRAMING PLAN SHEETS. ALL VALUES ARE SHOWN IN INCHES (DECIMAL FORM).

PROJECT NO. 15BPR.20
HENDERSON COUNTY
STATION: 35+30.22 -L-

SHEET 5 OF 5



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE DEAD LOAD DEFLECTION TABLES REVISIONS SHEET NO. S-109

DRAWN BY: S. STREDNAK DATE: 02/2019
CHECKED BY: J. SLOAN DATE: 02/2019
DESIGNED BY: N. BROWN/D. RITACCO DATE: 02/2019
DESIGN CHECKED BY: J. LIU DATE: 02/2019

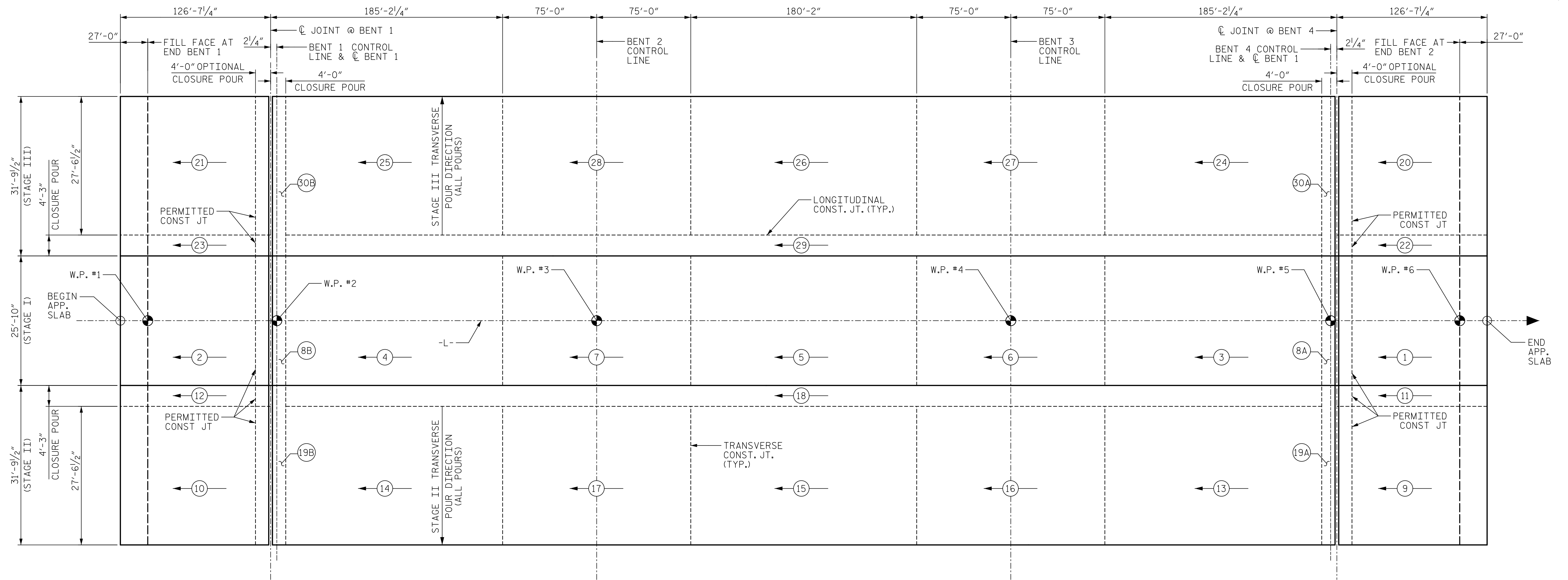
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 2/27/2020 TIME: 4:03:45 PM

USER: Cheryl.Ressenden DOB: R:\Structures\04 Drawings\01\_203\_15BPR\20\_SMU\_DL.dgn



DATE: 02/09/2020  
TIME: 10:56:05 AM



CONCRETE BREAKDOWN			
NORMAL WEIGHT (CUBIC YDS.)		ALL LIGHTWEIGHT (CUBIC YDS.)	
POUR 1	115.9	POUR 3	112.1
POUR 2	115.9	POUR 4	112.1
POUR 8A	6.9	POUR 5	111.2
POUR 8B	6.9	POUR 6	91.5
POUR 9	123.9	POUR 7	91.5
POUR 10	123.9	POUR 13	126.6
POUR 11	25.3	POUR 14	126.6
POUR 12	25.3	POUR 15	125.2
POUR 19A	7.1	POUR 16	101.6
POUR 19B	7.1	POUR 17	101.6
POUR 20	123.9	POUR 18	142.5
POUR 21	123.9	POUR 24	126.6
POUR 22	25.3	POUR 25	126.6
POUR 23	25.3	POUR 26	125.2
POUR 30A	7.1	POUR 27	101.6
POUR 30B	7.1	POUR 28	101.6
		POUR 29	142.5

### PLAN - POURING SEQUENCE

⊕ INDICATES POUR NUMBER AND POUR DIRECTION

THE FOLLOWING NUMBERED DECK POURS MAY BE POURED SIMULTANEOUSLY:

- 1 AND 2
- 6 AND 7
- 9 AND 10
- 11 AND 12
- 13 AND 14
- 16 AND 17
- 20 AND 21
- 22 AND 23
- 24 AND 25
- 27 AND 28
- 8A AND 8B
- 19A AND 19B
- 30A AND 30B

THE CONTRACTOR'S ATTENTION IS BROUGHT TO THE FACT THAT THE BRIDGE VIBRATES AND DEFLECTS UNDER LIVE LOAD. THE CONTRACTOR SHALL PLACE AND CURE THE DECK IN A MANNER THAT PREVENTS EXCESSIVE CRACKS OR DEFECTS FROM OCCURRING IN THE DECK FOR ANY REASON AS DETERMINED BY THE ENGINEER.

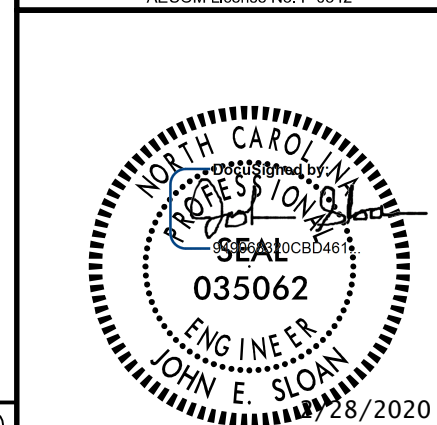
### NOTES:

STAGE I DECK POUR: CONTRACTOR SHALL PLACE ALL FORMWORK AND REINFORCEMENT FOR POURS 3-7 PRIOR TO PLACING ANY CONCRETE FOR POUR 2.

STAGE II DECK POUR: CONTRACTOR SHALL PLACE ALL FORMWORK AND REINFORCEMENT FOR POURS 13-17 PRIOR TO PLACING ANY CONCRETE FOR POUR 13.

STAGE III DECK POUR: CONTRACTOR SHALL PLACE ALL FORMWORK AND REINFORCEMENT FOR POURS 24-28 PRIOR TO PLACING ANY CONCRETE FOR POUR 24.

PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
 STATION: 35+30.22 -L-



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUPERSTRUCTURE

### DECK POUR SEQUENCE

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-110
1			3			TOTAL SHEETS
2			4			129

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY : H. ROSEMOND DATE : 02/2020  
 CHECKED BY : J. SLOAN DATE : 02/2020  
 DESIGNED BY : G. COLS DATE : 02/2020  
 DESIGN CHECKED BY : J. SLOAN DATE : 02/2020

USER: Merch\_Rosemond  
 DN: R:\Structures\04 Drawings\01\_205\_15BPR.20\_SNU\_BM1.dgn



**SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS**

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

**CONCRETE BREAKDOWN**

	STAGE I (CU. YDS.)	STAGE II (CU. YDS.)	STAGE III (CU. YDS.)
NORMAL WT. CONCRETE	245.7	312.6	312.6
FIBER REINFORCED ALL LIGHT WT. CONCRETE	518.4	724.1	724.1

**\*GROOVING BRIDGE FLOORS**

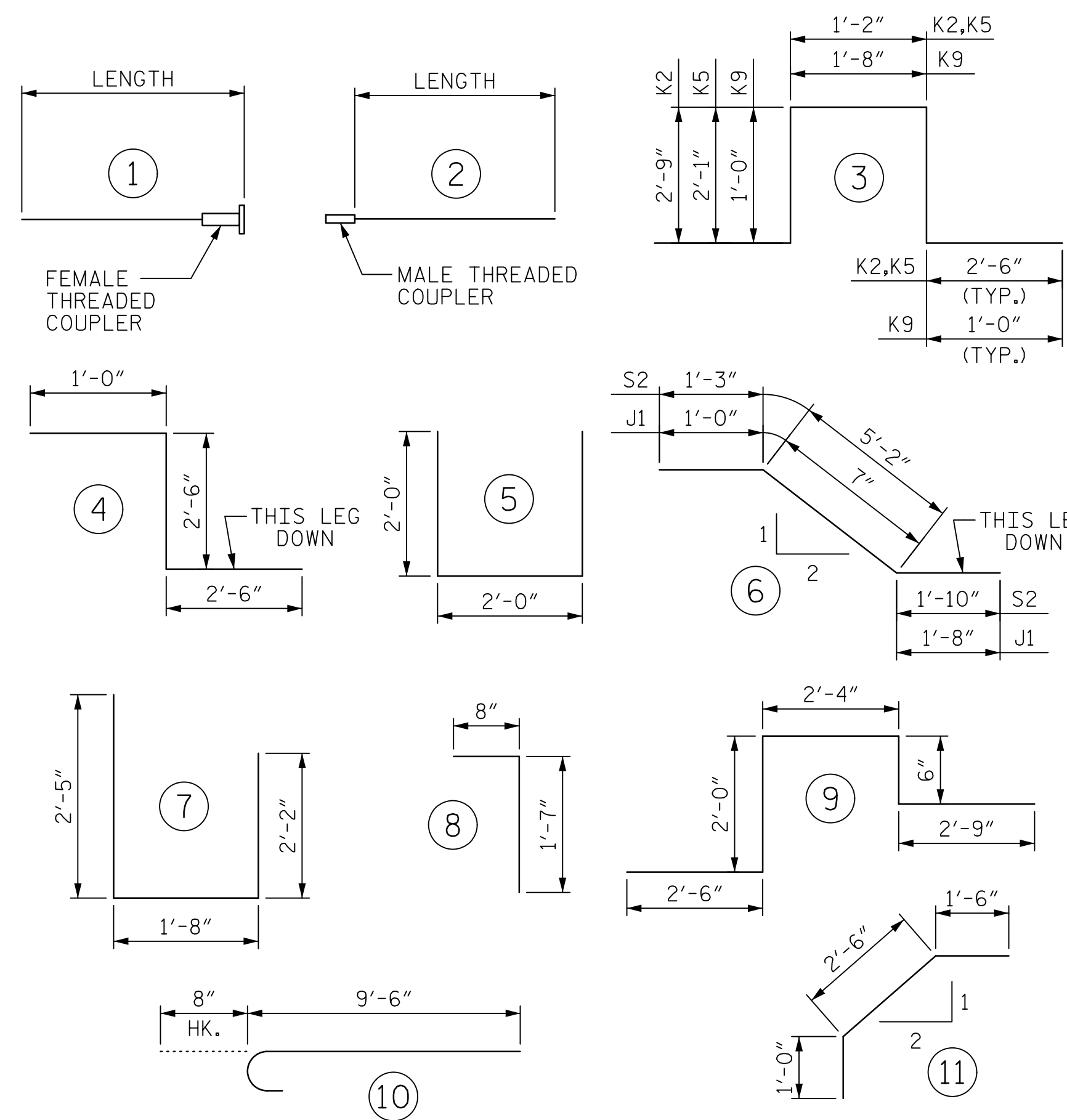
TOTAL	85,495 S.F
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\* PPC OVERLAY

**PPC OVERLAY**

SHOTBLASTING BRIDGE DECK	10,250 S.Y.
PPC MATERIALS	284.7 C.Y.
PLACING & FINISHING PPC OVERLAY	10,250 S.Y.

**BAR TYPES**

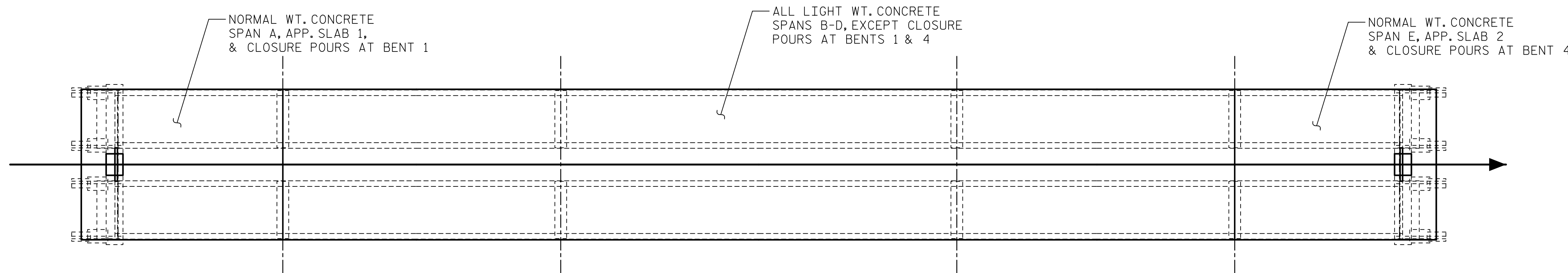


ALL BAR DIMENSIONS ARE OUT TO OUT

**BILL OF MATERIAL**

STAGE I						STAGE II OR STAGE III					
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*A1	2079	5	STR	25'-6"	55294	*A3	126	5	STR	26'-11"	3537
A2	2091	5	STR	25'-6"	55613	A4	126	6	STR	28'-6"	5394
*A3	102	5	STR	26'-11"	2864	*A5	2079	5	STR	31'-6"	68305
A4	102	6	STR	28'-6"	4366	A6	2091	5	STR	31'-6"	68699
						*A7	2091	6	10	10'-2"	31930
						A8	8	6	STR	11'-4"	136
*B1	114	4	STR	25'-11"	1974						
*B2	64	5	STR	25'-10"	1724	*B1	138	4	STR	25'-11"	2389
*B3	38	5	STR	27'-10"	1103	*B2	84	5	STR	25'-10"	2263
*B4	56	4	STR	25'-6"	954	B5	68	4	STR	27'-10"	1335
B5	68	4	STR	25'-6"	1158	*B6	176	5	STR	50'-9"	9316
B6	132	5	STR	50'-9"	6987	*B7	276	4	STR	27'-8"	5101
*B7	228	4	STR	27'-8"	4214	*B8	184	6	STR	49'-7"	13703
*B8	152	6	STR	49'-7"	11320	*B9	252	6	STR	45'-0"	17033
*B9	192	6	STR	45'-0"	12977	*B10	138	4	STR	29'-9"	2742
*B10	114	4	STR	29'-9"	2266	B11	660	5	STR	58'-8"	40385
B11	495	5	STR	58'-8"	30289	*B12	112	4	STR	16'-9"	1253
						B13	136	4	STR	16'-8"	1514
*D1	4158	5	1	2'-8"	11565	*B14	24	5	STR	4'-6"	113
D2	4182	5	1	2'-4"	10178						
*D3	112	4	1	2'-2"	162	*D5	2079	5	2	2'-8"	5782
D4	136	4	1	1'-11"	174	D6	2091	5	2	2'-4"	5089
						*D7	56	4	2	2'-2"	81
*J1	48	4	6	3'-3"	104	D8	68	4	2	1'-11"	87
K1	60	5	STR	7'-4"	459	*J1	74	4	6	3'-3"	161
*K2	16	5	3	11'-8"	195						
*K3	16	5	4	6'-0"	100	K4	60	5	STR	7'-7"	475
						*K5	16	5	3	10'-4"	172
*S1	96	4	5	6'-0"	385	*K6	8	5	9	10'-1"	84
*S2	48	4	6	8'-3"	265	K7	16	5	STR	5'-4"	89
*S3	96	4	7	6'-3"	401	K8	20	5	STR	1'-10"	38
*S4	96	4	11	5'-0"	321	*K9	1045	5	3	5'-8"	6176
*S5	96	4	8	2'-3"	144						
						*S1	108	4	5	6'-0"	433
REINFORCING STEEL					= 109,224	*S2	54	4	6	8'-3"	298
*EPOXY COATED REINF. STEEL					= 108,332	*S3	88	4	7	6'-3"	367
						*S4	88	4	11	5'-0"	294
						*S5	88	4	8	2'-3"	132
						REINFORCING STEEL				= 131,222	
						*EPOXY COATED REINF. STEEL				= 163,684	

DATE: 2/27/2020  
TIME: 4:24:54 PM



**LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB**  
( LIGHT WT. SQ. FT. = 75,343 )  
( NORMAL WT. SQ. FT. = 23,357 )

ALL LIGHT WEIGHT CONCRETE SHALL INCLUDE 4.0 LBS/CUBIC YARD OF POLYOLEFIN FIBERS. FOR FURTHER DETAILS, SEE SPECIAL PROVISIONS.

DRAWN BY : M. K. TOM	DATE : 1/2019
CHECKED BY : G. COLS	DATE : 2/2019
DESIGNED BY : G. COLS	DATE : 1/2019
DESIGN CHECKED BY : J. E. SLOAN	DATE : 2/2019

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



PROJECT NO. 15BPR.20  
HENDERSON COUNTY  
STATION: 35+30.22 -L-

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
SUPERSTRUCTURE  
BILL OF MATERIAL

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			TOTAL SHEETS
2			4			129

USER: Cheryl.Rasmussen  
DN: R:\Structures\04 Drawings\01\_2019\15BPR.20\_SLU\_BM6.dgn