-0124 BR. REFERENCE

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

CONTENTS

SHEET NO.	DESCRIPTION
SHEET NO.	
l l	TITLE SHEET
2	LEGEND (SOIL)
3	SITE PLAN
4	PROFILE
5-8	CROSS SECTIONS
9-14	BORE & CORE LO
15-18	CORE PHOTOS

STRUCTURE SUBSURFACE INVESTIGATION

	WILKES
DESCRIPTION	REPLACE BRDG #0166 ON
	SR-1745 (SHUMATE MTN RD) over
	W. PRONG ROARING RIVER
	DESCRIPTION

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
v.C.	BR-0124	_	18

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNDS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU INH-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS,

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS FOR ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

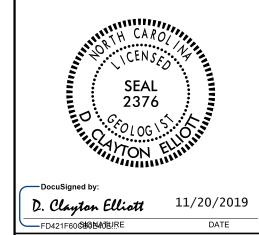
- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PRILEX.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

-NCDOT-DC CHEEK __CJ COFFEY CD JOHNSON INVESTIGATED BY NCDOT GEU /DCE DC ELLIOTT JC KUHNE CHECKED BY ____ SUBMITTED BY JC KUHNE

PERSONNEL



DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED**

PROJECT REPERENCE NO. SHEET NO.

BR-0124

2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETATED WITH A CONTINUOUS FIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHIO T 206, ASTM D1586, SOIL CLASSIFICIATION	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN Ø.1 FOOT PER 60	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER, AQUIFER - A WATER BEARING FORMATION OR STRATA,
IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE GROW OF THE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	NON-CRYSTALLINE ROCK (NCR) ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 000000000000000000000000000000000000	STEIGHTET COMPRESSIBLE LL 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR CLAY DEAT	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF JONEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
1-80 JS MA JS MA JS MX JS MN J	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	HORIZONTAL. <u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX Ø Ø Ø 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOILS	GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. OF MAINE GRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GHAVEL AND SAND SUILS SUILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS <u>▼Pw</u> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR FAIR TO POOR POOR UNSUITABLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30 CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAQLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
COMPACTNESS OF RANGE OF STANDARD RANGE OF UNCONFINED	ETT 25/425	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (TONS/FT ²)	ROADWAY EMBANKMENT (RE) Procedure of ROCK STRUCTURES ROADWAY EMBANKMENT (RE) OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4 CONTROL CONT	SOIL SYMBOL SOIL SYMBOL SPIT DAT TEST BORING INSTALLATION SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MATERIAL DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50 VERY SOFT < 2	INFERRED SOIL BOUNDARY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	<u>PERCHED WATER</u> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	MONITORING WELL TEST BORING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u> COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4	TTT+++ ALLUVIAL SOIL BOUNDARY APIEZOMETER INSTALLATION SPI N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM)	SHALLOW UNDERCUT SIFIED EXCAVATION - UNDERCUT SEED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED MITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRUCTED SUBFACE THAT BESULTS FROM EDICTION ALONG A FAULT.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY γ - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC γ_d - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE LL LIQUID LIMIT	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO 00 RCREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID; REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE < - WET - (W) ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS ω - MOISTURE CONTENT CBR - CALIFORNIA BEARING HI HIGHLY V - VERY RATIO	FRACTURE SPACING BEDDING TERM SPACING TERM THICKNESS	BENCH MARK: -BM-I- : N932585 E1381916 BL STA. 8+52.80, 20.74' RT, R.R. SPIKE IN 18" DIA, TULIP POPLAR
- MOIST - (M) COLID, AT OD NEAD ODTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE	& @ ~ -L- STA 13+53, 31' RT ELEVATION: = 1088,50 FEET
OM _ OPTIMUM MOISTURE SL_ SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: X CME-45C	MODERATELY CLOSE	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	CME-55 G* CONTINUOUS FLIGHT AUGER CORE SIZE:	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY	8* HOLLOW AUGERS	INDURATION	
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS X-N NXWL	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS;	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST	GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNG,-CARB, SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT WANE SHEAR TEST	DIFFICULT TO BREAK WITH HAMMER. EVIDENCE V INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
JENG SOON AS EIGHT, DANK, STILLANDS, ETC. MIC USED TO DESCRIBE MERCHANICE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1

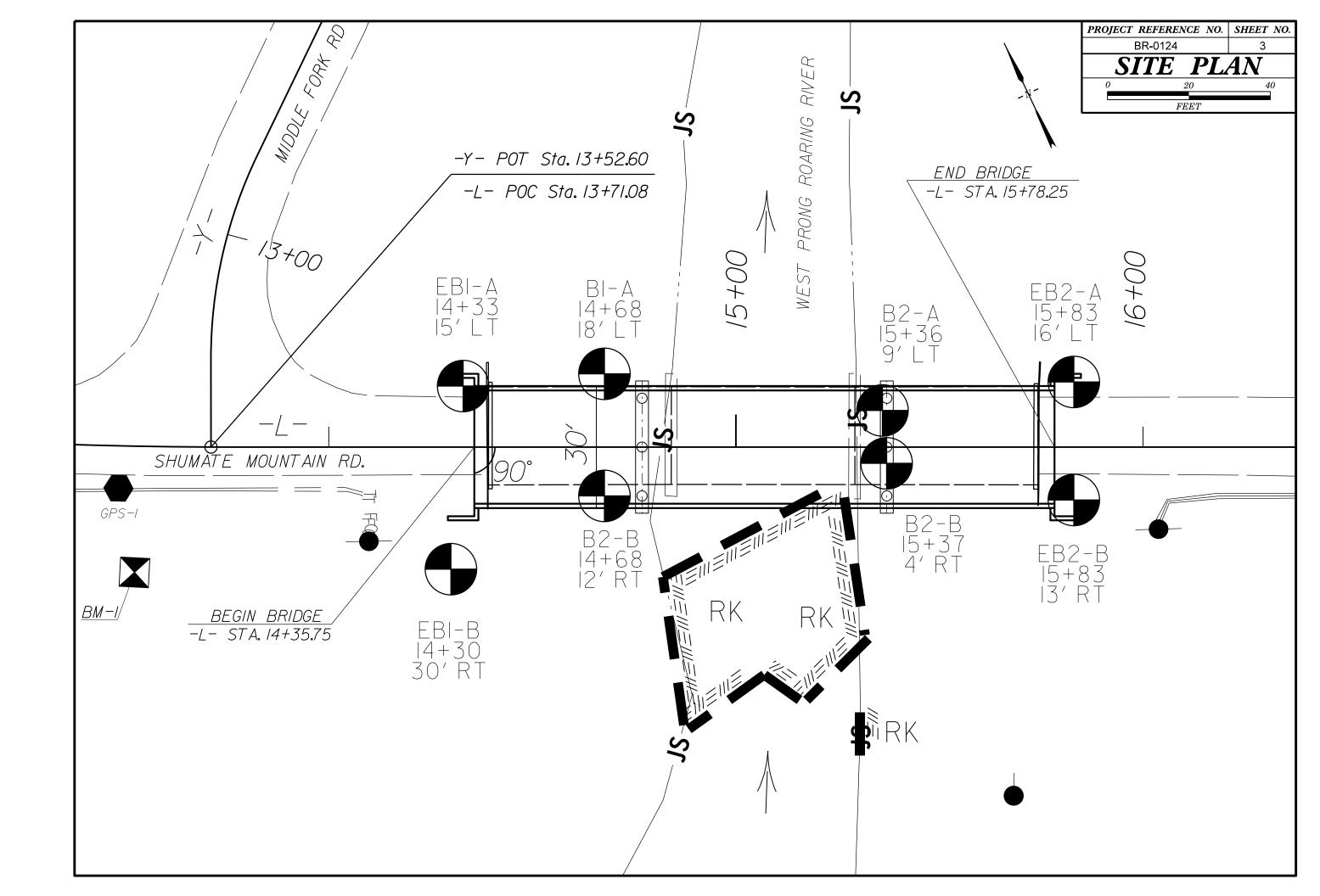
PROJECT REFERENCE NO.	SHEET NO.
BR-0124	2A

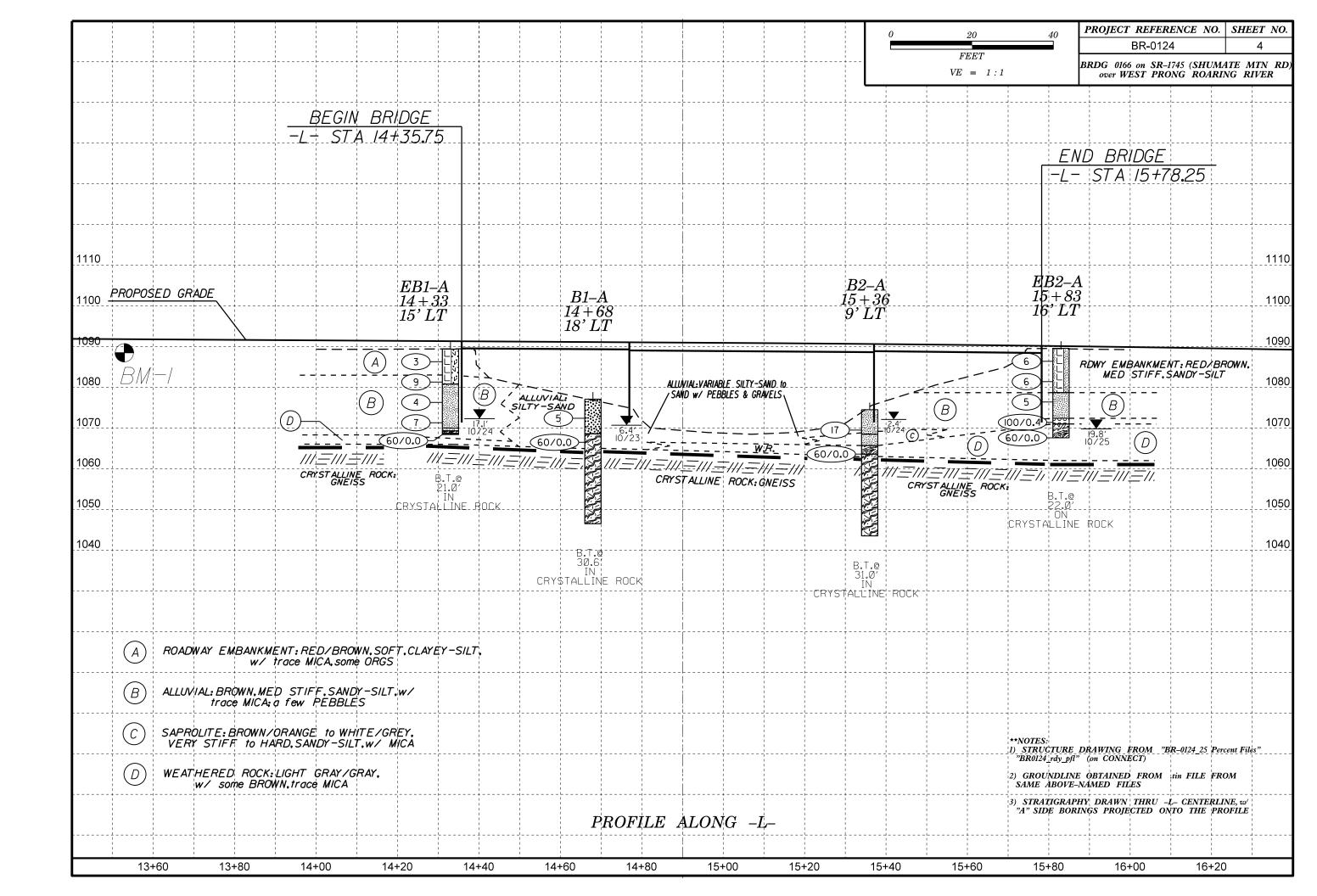
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

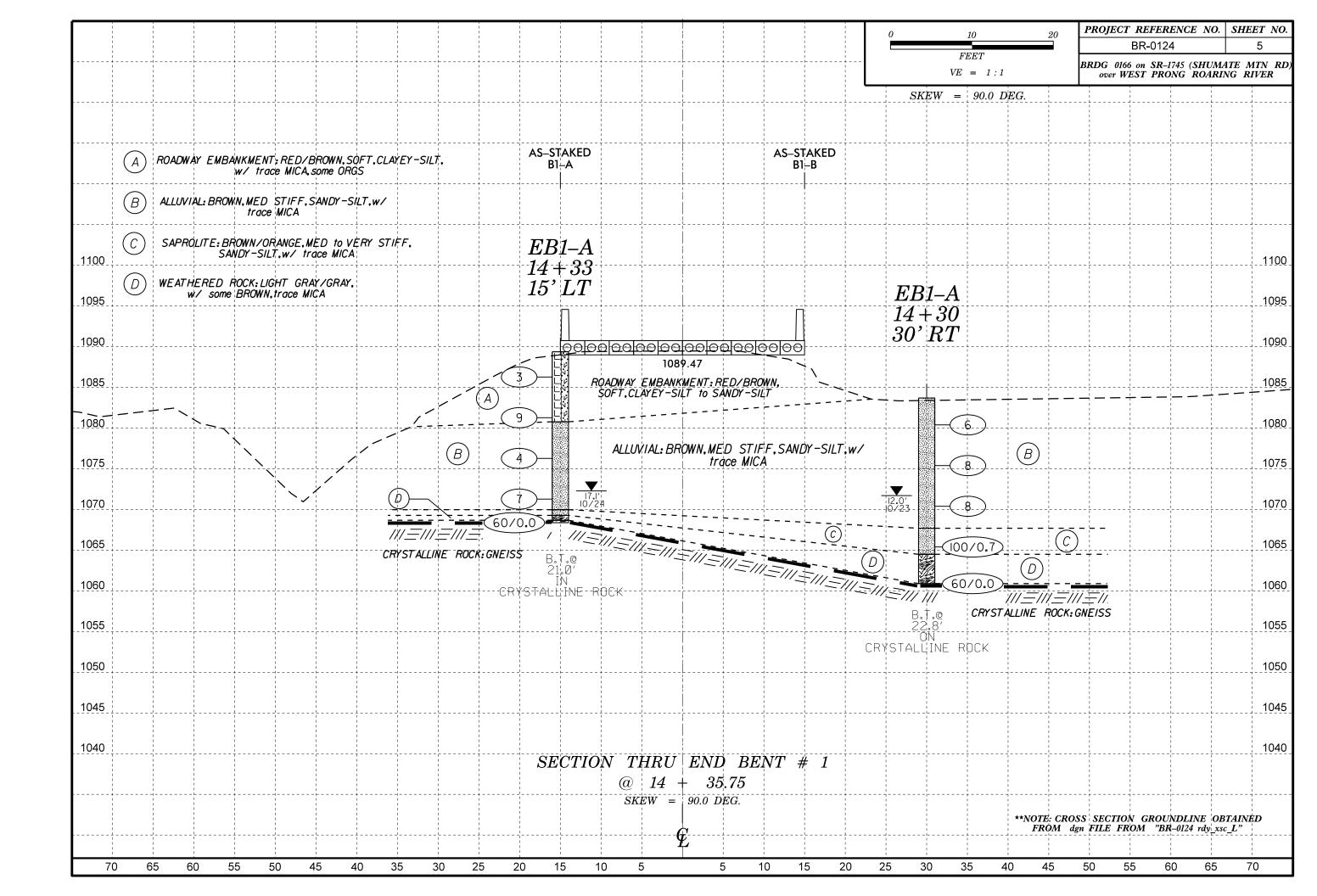
SUBSURFACE INVESTIGATION

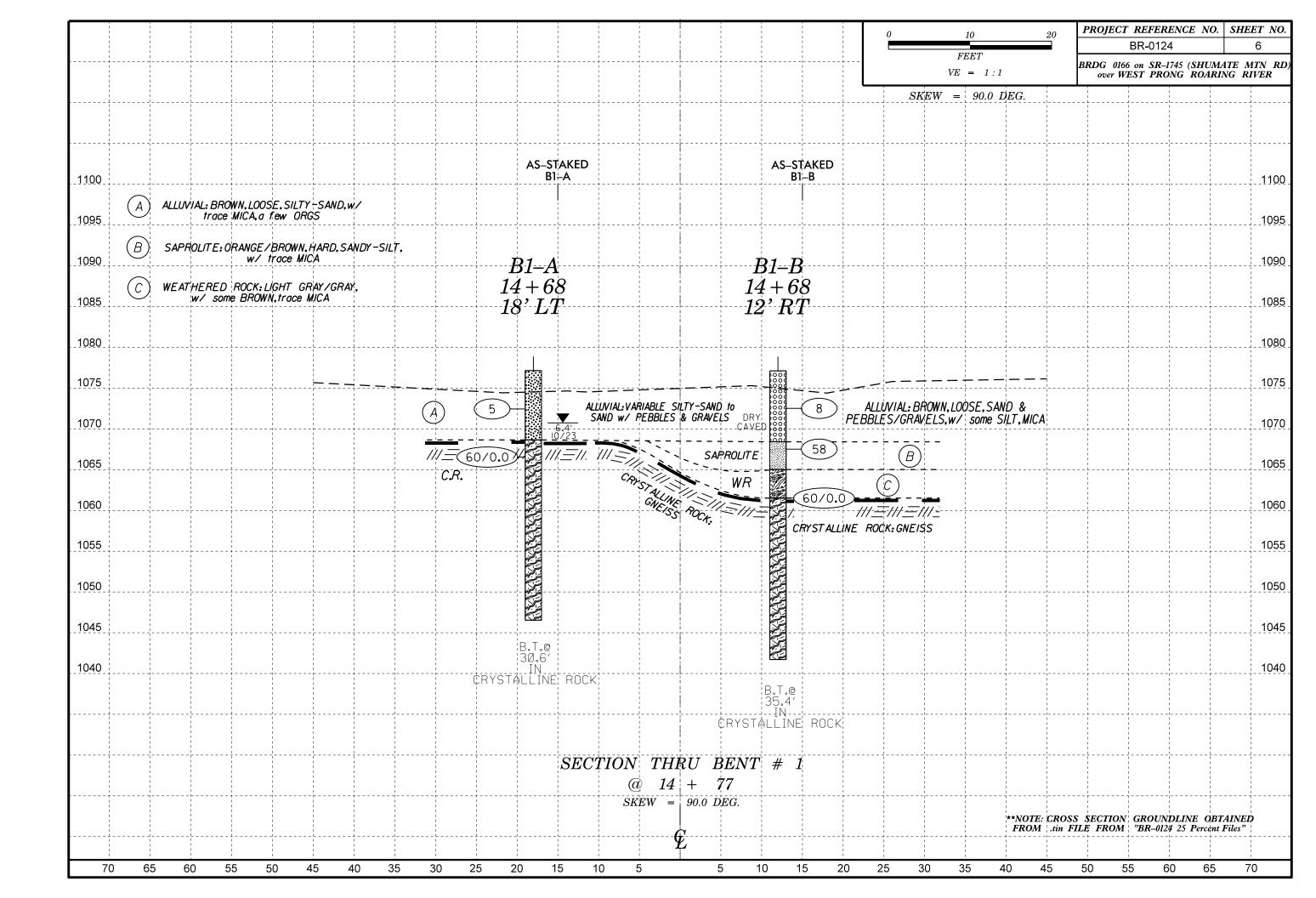
SUPPLEMENTAL LEGEND GEOLOGICAL STRENGTH INDEX (GSI) TABLES

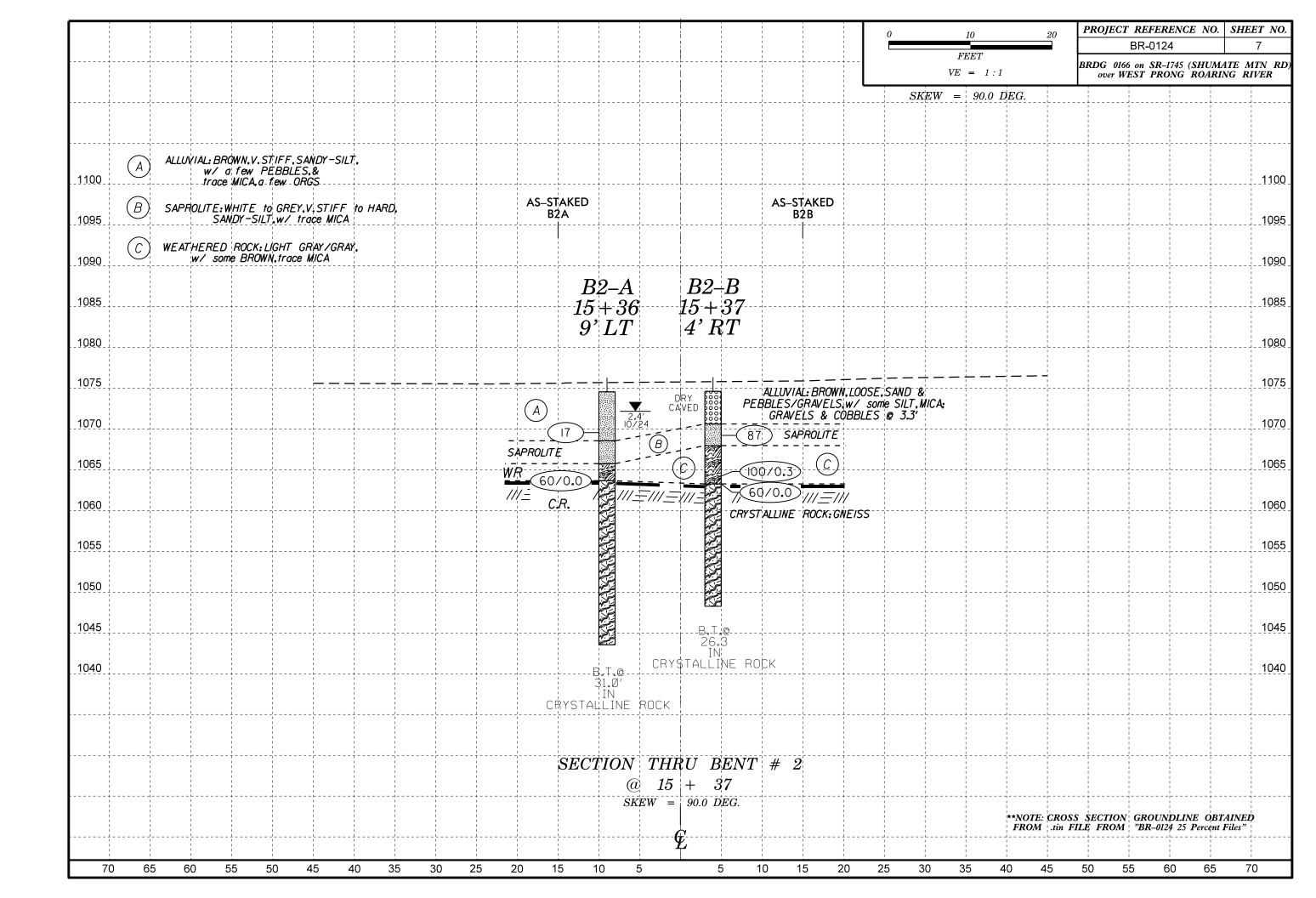
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	ted Rock Mass (Ma	-inos and Hoek, 2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI)FOR JOINTED ROCKS (Hoek and Marınos, 2000)	sec	0		S O	s e o	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS VERY GOOD Very rough, fresh unweathered surfaces	GOOD Rough, slightly weathered, iron staine surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfac with compact coatings or fillings or angular fragments	L S	Erom a description of the lithology, structure and surface conditions (barticularly of the pedding planes), choose a box in the chart. Tocate the position in the pox that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Ground a range from 33 to 37 is more realistic than giving CSI = 35. Note that the Hoek-Brown criterion does not abbly to structurally controlled failures. Where unfavourably oriented continuous weak blanar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very board surfaces with compacts. VERY GOOD - Rough, slightly weather the peading provided and the position of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor the rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very provided and the rock masses in reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair and the provided and the rock masses in reduced by the provided and the provided
STRUCTURE	DE	CREASING SI		1	1	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities	90 80			N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets	ROCK	70 60				B. Sand- stone with thin inter- siltstone with sand- wi
VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	OCKING OF	5	50			layers of siltstone in similar amounts stone layers shale with sandstone layers
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL		40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H. F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECRE			20		G. Undisturbed silty or clayey shale with or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone layers H. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed into small cock pieces.
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	N/A	N/A			10	sandstone are transformed into small rock pieces. Means deformation after tectonic disturbance

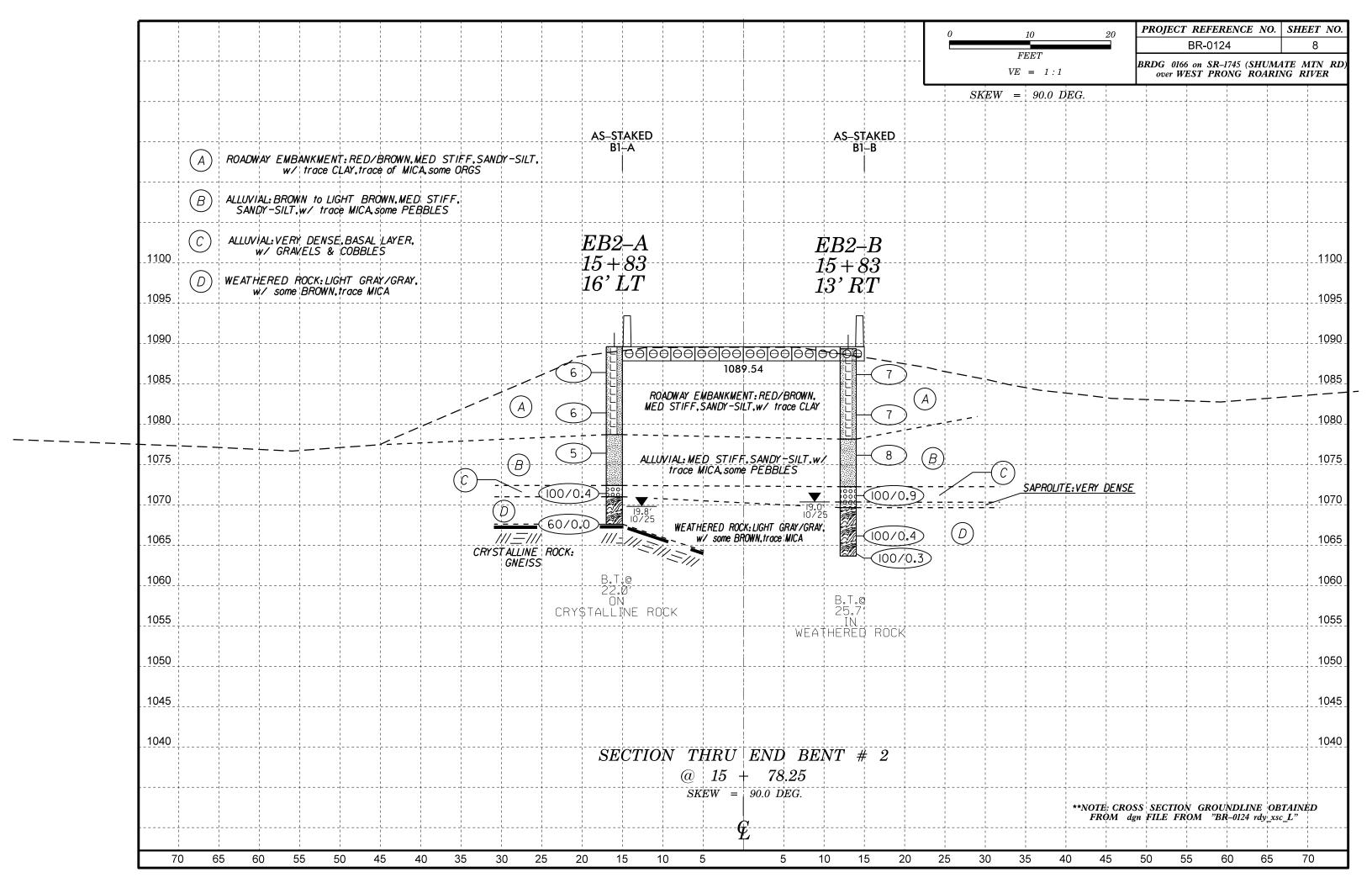




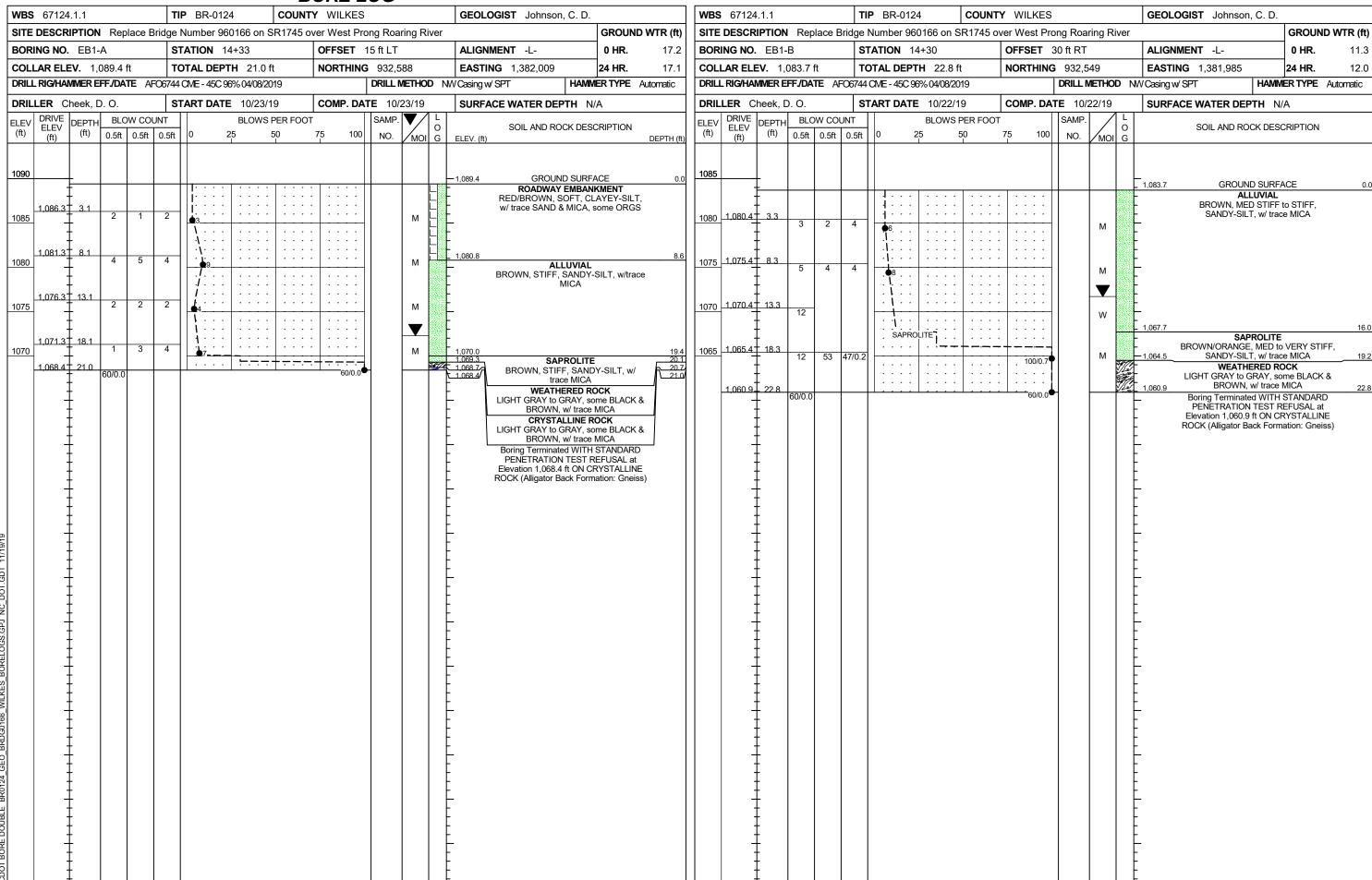








GEOTECHNICAL BORING REPORT BORE LOG



GROUND WTR (ft)

N/A

DEPTH (ft)

0 HR.

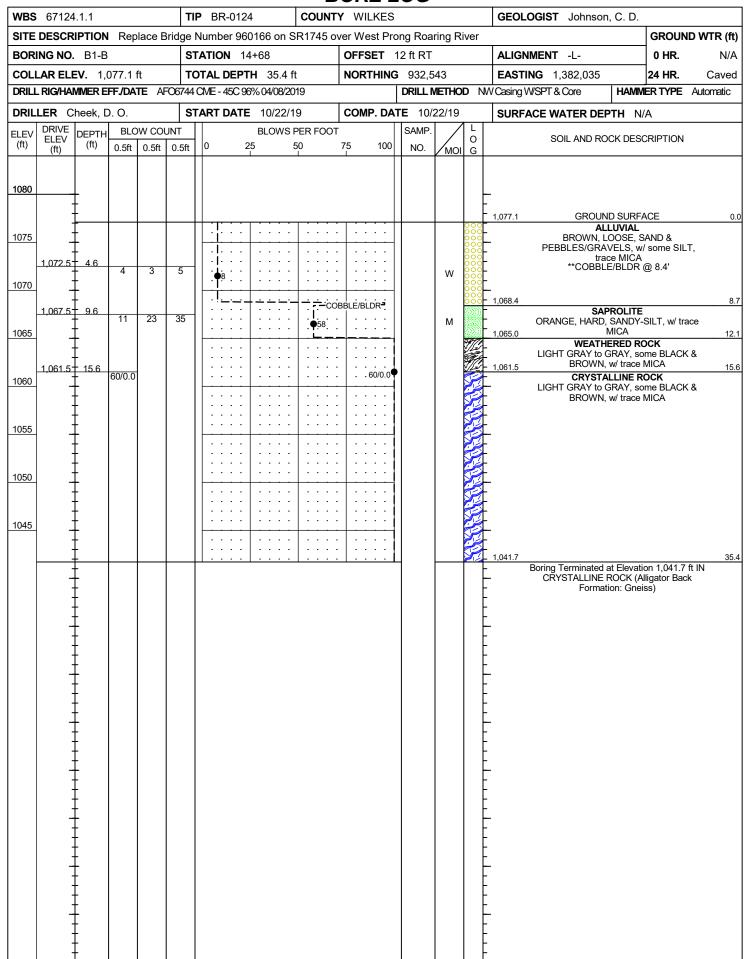
24 HR.

HAMMER TYPE Automatic

GEOTECHNICAL BORING REPORT

GEOTECHNICAL BORING REPORT **BORE LOG**

CORE LOG **WBS** 67124.1.1 **TIP** BR-0124 COUNTY WILKES GEOLOGIST Johnson, C. D. **WBS** 67124.1.1 **TIP** BR-0124 **COUNTY** WILKES GEOLOGIST Johnson, C. D. **GROUND WTR (ft)** SITE DESCRIPTION Replace Bridge Number 960166 on SR1745 over West Prong Roaring River SITE DESCRIPTION Replace Bridge Number 960166 on SR1745 over West Prong Roaring River BORING NO. B1-A **STATION** 14+68 OFFSET 18 ft LT ALIGNMENT -L-**STATION** 14+68 OFFSET 18 ft LT ALIGNMENT -L-**BORING NO.** B1-A 0 HR. N/A TOTAL DEPTH 30.6 ft **NORTHING** 932,570 **EASTING** 1,382,049 COLLAR ELEV. 1,077.1 ft TOTAL DEPTH 30.6 ft **NORTHING** 932,570 **EASTING** 1,382,049 **COLLAR ELEV.** 1,077.1 ft 24 HR. 6.4 **DRILL RIG/HAMMER EFF./DATE** AFO6744 CME - 45C 96% 04/08/2019 **DRILL METHOD** NW Casing W/SPT & Core **DRILL RIG/HAMMER EFF./DATE** AFO6744 CME - 45C 96% 04/08/2019 DRILL METHOD NW Casing W/SPT & Core **HAMMER TYPE** Automatic DRILLER Cheek, D. O. **START DATE** 10/22/19 COMP. DATE 10/22/19 DRILLER Cheek, D. O. **START DATE** 10/22/19 **COMP. DATE** 10/22/19 SURFACE WATER DEPTH N/A SURFACE WATER DEPTH N/A ELEV DRIVE DEPTH BLOW COUNT SAMP. CORE SIZE NXWL TOTAL RUN 20.0 ft **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION ELEV (ft) (ft) 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G (ft) ELEV. (ft) ELEV DEPTH RUN SAMP RATE **ELEV** DESCRIPTION AND REMARKS (ft) (ft) NO. (ft) G (ft) ELEV. (ft) Continued from previous page CRYSTALLINE ROCK (continued) 1066. 1080 (4.8) (4.0) 96% 80% 1065 GROUND SURFACE 1.077.1 ALLUVIAL BROWN, LOOSE, SILTY-SAND, w/ trace 1,061.5 + 15.6 1075 MICA, a few ORGS 5.0 0:55/1.0 (5.0) (4.7) 1060 1:07/1.0 100% 94% 1,072.4 1:11/1.0 ¶r ▼ 1:11/1.0 1070 1,056.5 + 20.6 (4.8) (3.1) 1055 1:33/1.0 96% CRYSTALLINE ROCK GSI: 10.6' - 22.3': 55 - 65 0:58/1.0 LIGHT GRAY to GRAY, some BLACK & 1,066.5 + 10.6 22.3' - 26.3' : 30 - 40 1:32/1. - 60/0.0¶ BROWN, w/ trace MICA 1,051.5 - 25.6 1065 26.3' - 30.6' : 60 - 70 (4.3) (4.2) 86% 84% 2.02/1.0 1050 1:13/1.0 1:07/1.0 1.04/1 (1,046.5 + 30.6 1060 1:30/1.0 Boring Terminated at Elevation 1,046.5 ft IN CRYSTALLINE ROCK (Alligator 1055 1050 Boring Terminated at Elevation 1,046.5 ft IN CRYSTALLINE ROCK (Alligator Back



GEOTECHNICAL BORING REPORT CORE LOG

					_						L LOO				
	67124				l	BR-01					/ILKES	GEOLOGIST Johnson,	, C. D.		
SITE	DESCR	IPTION	l Rep	lace Brid	ge Nu	mber 9	60166 o	n SR1	745 o	ver V	/est Prong Roaring River				D WTR (ft)
BOR	NG NO.	B1-B	1		STA	ΓΙΟΝ	14+68			OF	FSET 12 ft RT	ALIGNMENT -L-	0 HR.	N/A	
	AR ELE										RTHING 932,543	EASTING 1,382,035		24 HR.	Caved
DRILL	. RIG/HAI	VIMER E	FF./DA	TE AFO6	744 CIV	E - 45C	96% 04/0	3/2019			DRILL METHOD NW	Casing W/SPT & Core	HAMME	ER TYPE	Automatic
DRIL	LER C	heek, [D. O.		STAF	RT DA	ΓΕ 10/2	2/19		СО	MP. DATE 10/22/19	SURFACE WATER DEP	TH N/	4	
COR	E SIZE	NXWL	-		TOTA	AL RUN	1 19.8 f								
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	ATA RQD (ft) %	L O G	D	ESCRIPTION AND REMARKS	3		
1061.5											Со	ntinued from previous pa	ge		
1060	1,061.5-	- -	4.8	N=60/0.0	(4.7) 98%	(2.4) 50%					· 1,061.5 	CRYSTALLINE ROCK			15.6
1055	1,056.7_	- 20.4 -	5.0		(4.8)	(3.3)					•				
1000	1,051.7	- - - 25.4			96%	66%					- · ·				
1050	-	_	5.0		(5.0) 100%	(2.0) 40%					-	001 1501 0001 05			
	1,046.7	- - - 30.4										GSI : 15.6' - 29.3' : 35 - 4 29.3' - 35.4' : 45 - 5			
1045	_	-	5.0		(5.0) 100%	(3.9) 78%					-				
	1,041.7	35.4									. 1,041.7				35.4
	-1,011.7	-									Boring Terminated at E	Elevation 1,041.7 ft IN CRYST	ALLINE F	ROCK (Alli	gator
	-	-									- ·	Back Formation: Gneiss)			
	-	-									•				
	_	-									· -				
	-										•				
	-	-									•				
	_	_									-				
	-	-													
	-	_									-				
	-	-													
	-	_									•				
	_										<u>-</u>				
	-	_									•				
	-	-									•				
	-	-									- ·				
	-	-									•				
	_	-									-				
	-	-									•				
	-	-									•				
	-	_									-				
	-	_									• •				
	-										-				
	-	-													
	-	_													
	_	-									_				
	_	_													
	-	_									_				
	-	_													
	-	_									· •				
	_										· - ·				
	-	_									•				
	-	-		l							•				

WBS 67124.1.1 COUNTY WILKES **TIP** BR-0124 GEOLOGIST Johnson, C. D. GROUND WTR (ft) SITE DESCRIPTION Replace Bridge Number 960166 on SR1745 over West Prong Roaring River STATION 15+36 OFFSET 9 ft LT ALIGNMENT -L-BORING NO. B2-A 0 HR. N/A COLLAR ELEV. 1,074.5 ft TOTAL DEPTH 31.0 ft **NORTHING** 932,534 **EASTING** 1,382,097 24 HR. 2.4 DRILL RIG/HAMMER EFF./DATE AFO6744 CNE - 45C 96% 04/08/2019 HAMMER TYPE Automatic **DRILL METHOD** NW Casing W/SPT & Core DRILLER Cheek, D. O. **START DATE** 10/23/19 **COMP. DATE** 10/23/19 SURFACE WATER DEPTH N/A **BLOWS PER FOOT** SAMP. SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft MOI G 75 100 NO. ELEV. (ft) DEPTH (ft 1075 **GROUND SURFACE** ALLUVIAL \blacksquare BROWN, VERY STIFF, SANDY-SILT, w/ a few PEBBLES, w/ trace of MICA 1070 1,069.5 5.0 10 8 М 1,068.5 SAPROLITE WHITE to GREY, VERY STIFF to HARD, SANDY-SILT. w/ trace MICA 1065 WEATHERED ROCK 1,063.5 + 11.0 LIGHT GRAY to GRAY, some BLACK & BROWN, w/ trace MICA . 60/0.0 60/0.0 CRYSTALLINE ROCK . . . 1060 LIGHT GRAY to GRAY, some BLACK & BROWN, w/ trace MICA 1055 1050 1045 Boring Terminated at Elevation 1,043.5 ft IN CRYSTALLINE ROCK (Alligator Back Formation: Gneiss)

GEOTECHNICAL BORING REPORT CORE LOG

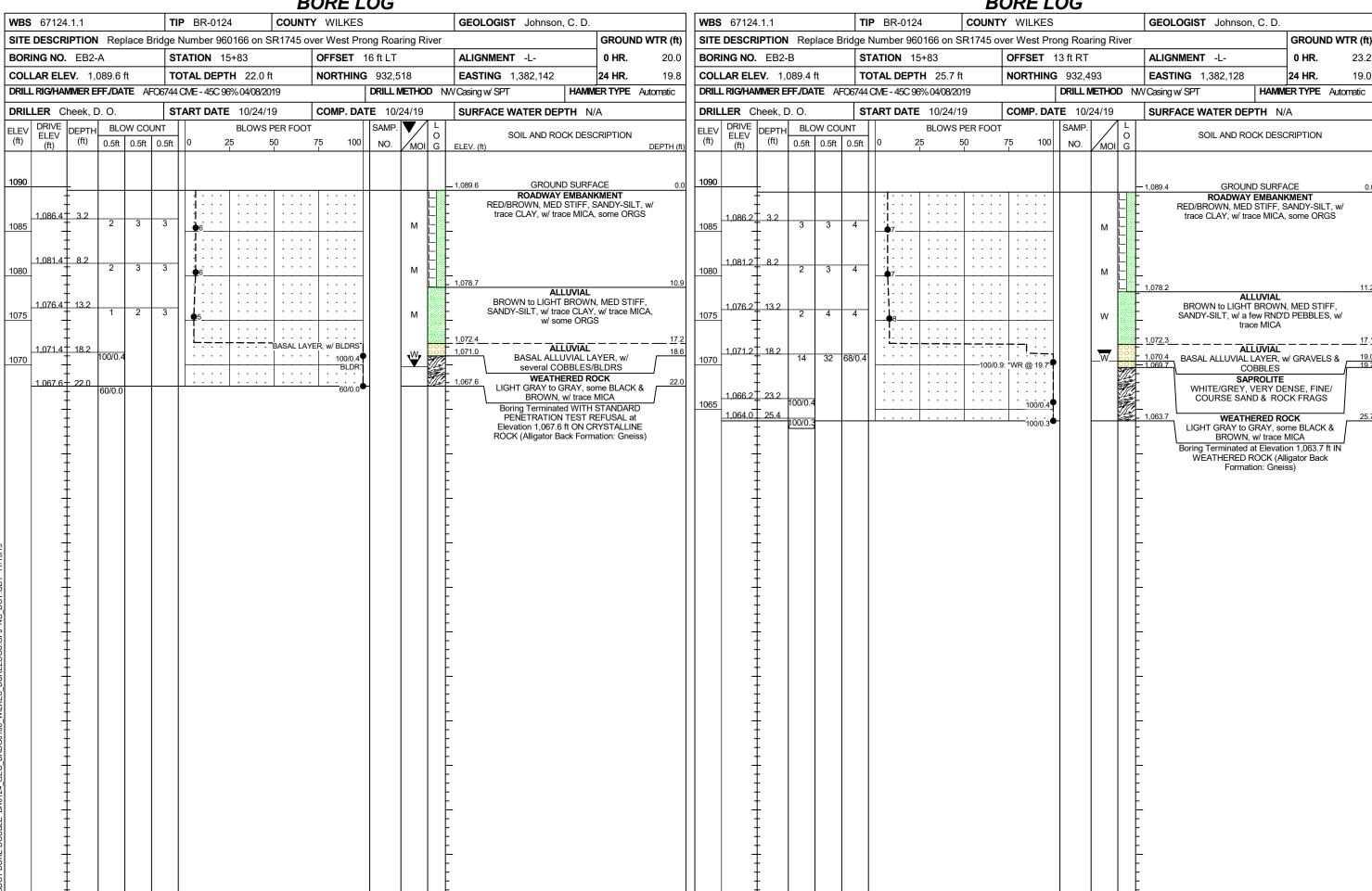
									C	Ol	RE L	OG	}										
WBS 6	67124	.1.1			TIP	BR-0	124	С	OUNT	ΥV	VILKES					GEOLOG	IST Johi	nson, (C. D.				
SITE DE	ESCRI	PTION	l Rep	olace Brid	ge Nu	mber 9	960166 o	n SR1	745 o	ver West Prong Roaring River					r	G				GROU	GROUND WTR (ft)		
BORING	B2-A			STAT	TION	15+36		OF	FSET 9	ft LT	•			ALIGNMENT -L-				0 HR . N/A					
COLLA	R ELE	V. 1,0	074.5	ft	TOTAL DEPTH 31.0 ft						RTHING	932	2,534			EASTING	1,382,0)97		24 HR.	2.4		
DRILL R	IG/HAN	/IMER E	FF./DA	TE AFO6	744 CIV	/E - 45C	96% 04/0	8/2019				DRIL	L MET	HOD	NW	Casing W/SI	PT & Core		HAMME	R TYPE	Automatic		
DRILLE	ER Ch	neek, [). O.		STAI	RT DA	TE 10/2	3/19		CC	MP. DAT	E 1	0/23/	19		SURFACE	WATER	DEPT	H N//	A			
CORE S	CODE SIZE NIVIVI TOTAL PLIN 20.0 ft																						
FLEV E	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	REC. (ft)	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RATA RQD (ft) %	L O G	ELEV. (ft	:)			DE	SCRIPTION	N AND REM	MARKS			DEPTH (
1063.5															Cor	ntinued fro	m previou	ıs pag	e				
1060	,063.5	- - -	5.0	N=60/0.0	(4.5) 90% (4.4) 88%	(4.1) 82% (4.1) 82%									CF	RYSTALLIN	E ROCK (c	continue	d)				
1050	,053.5	- - -	5.0		(4.3) 86% (4.8)	(1.0) 20% (2.0)					- - - - - -				G	SI : 11.0' - 21.0' -	- 21.0' : 55 · 31.0' : 20						
1045	,043.5	- - - 31 N			96%	40%					1,043.5										31.		
																Back Fori	mation: Gne	eiss)					

	<i>D</i>	ORE LUG		
VBS 67124.1.1	TIP BR-0124 COUNT	Y WILKES	GEOLOGIST Johnson, C. D.	
SITE DESCRIPTION Replace B	ridge Number 960166 on SR1745 o	er West Prong Roaring River		GROUND WTR (ft)
BORING NO. B2-B	STATION 15+37	OFFSET 4 ft RT	ALIGNMENT -L-	0 HR. N/A
COLLAR ELEV. 1,074.6 ft	TOTAL DEPTH 26.3 ft	NORTHING 932,522	EASTING 1,382,092	24 HR. Caved
DRILL RIG/HAMMER EFF./DATE AF	06744 CME - 45C 96% 04/08/2019	DRILL METHOD N	V Casing W/SPT & Core HAMM	IER TYPE Automatic
DRILLER Cheek, D. O.	START DATE 10/23/19	COMP. DATE 10/23/19	SURFACE WATER DEPTH N	/A
LEV CHIP CHIP COUNTY COUNTY CHIP CHIP CHIP CHIP CHIP CHIP CHIP CHIP	NT BLOWS PER FOOT 0.5ft 0 25 50	75 100 SAMP. O NO. MOI G	SOIL AND ROCK DESC	CRIPTION
075			-1,074.6 GROUND SURFA	
070			BROWN, LOOSE, SILTY GRAVELS & COBBLE 1,070.6 - SAPROLITE 1,068.0 WHITE/GREY, HARD, SAI	ES @ 3.3'
065	45	. •87— M	1,068.0 WHITE/GREY, HARD, SAI trace MICA WEATHERED RO GREY/BROWN, W/	OCK
1,064.2 10.4 1,063.3 11.3 100/0.3 60/0.0		100/0.3	CRYSTALLINE R LIGHT GRAY to GRAY, so BROWN, w/ trace	me BLACK &
055				
			-	
050			- 1,048.3 Boring Terminated at Elevati CRYSTALLINE ROCK (A Formation: Gnei	lligator Back

GEOTECHNICAL BORING REPORT CORE LOG

																	
WBS	67124	.1.1			TIP	BR-01	24	C	DUNT	ΥV	/ILKES			GEOLOGIST Johnso	n, C. D.		
SITE	DESCR	IPTION	I Rep	lace Brid	ge Nu	mber 9	960166 o	n SR1	745 o	ver V	est Pror	ng Roaring	River			GROUN	D WTR (ft)
BOR	ING NO.	B2-B			STAT	TION	15+37			OFF	SET 4	ft RT		ALIGNMENT -L-		0 HR.	N/A
COL	LAR ELE	V. 1,0	074.6	ft	TOTA	AL DEI	PTH 26	.3 ft		NO	RTHING	932,522		EASTING 1,382,092		24 HR.	Caved
DRIL	RIG/HAI	/IMER E	FF./DA	TE AFO6	744 CIV	E - 45C	96% 04/08	3/2019				DRILL MET	HOD NW	Casing W/SPT & Core	HAMM	ER TYPE	Automatic
DRIL	LER C	heek C	0 0		STAF	RT DA	ΓΕ 10/2	3/19		co	MP. DAT	E 10/23/	19	SURFACE WATER DE	PTH N/	Δ	
	E SIZE									1				001117102 11711211 02		•	
	D. I.			DRILL	REC. (ft) %	JN	0445	STR REC. (ft) %	ATA								
ELEV (ft)	ELEV (ft)	DEPTH (ft)	(ft)	RATE (Min/ft)	REC.	RQD (ft)	SAMP. NO.	REC.	RQD (ft)	L O G			DI	ESCRIPTION AND REMAR	KS		
				(IVIIII/IL)	%	%		%	- %				0				
1063.3	1,063.3-	- 11.3	5.0	N=60/0.0	(4.9)	(4.7)					1,063.3		Col	ntinued from previous p CRYSTALLINE ROCK	oage		11.3
1060	_	-			98%	94%					•						
1000	1,058.3	- 16.3									-						
	-1,000.0	-	5.0		(4.9)	(3.6)											
1055	_	_			98%	72%					=						
	1,053.3	21.3												GSI: 11.3' - 26.3': 55	- 65		
	_	-	5.0		(4.8) 96%	(3.9) 78%					-						
1050		-									-						
	1,048.3	26.3									1,048.3	Boring Torn	ninated at E	Elevation 1,048.3 ft IN CRYS	STALLINE F	SUCK (VIII	26.3
	_											Doming Telli	miateu at E	Back Formation: Gneiss)	CON (All)	gatoi
	-	-								-	-						
] -	-															
	-	-									•						
	_	-									-						
	_	_								1 - 1							
	-	-									-						
	_	-									-						
	_	-									•						
	_	-									-						
	-	_								1 -	•						
	-	-															
		-									-						
	_	-															
	_	_															
	_	-									_						
] -	-									•						
	_	-									•						
	_	_								1 1	-						
	-	-															
	_	-									•						
	_	-									-						
	_	_								1 1	•						
	-	-									-						
	_	-									-						
	-	-									•						
	_	_									-						
	-	_								<u> </u>							
	-	-									=						
	-	-									- -						
		-															
	-	_															
	-	-									-						
	-	-															
	-	_															
	-	-									-						
	_										-						

GEOTECHNICAL BORING REPORT BORE LOG



B1-ABOX 1 of 1: 10.6 - 20.1 FEET



GEOLOGICAL STRENGTH INDEX: GSI 10.6' - 20.1' : 55 - 65

B1-ABOX 2 of 2: 20.1 - 30.6 FEET



GEOLOGICAL STRENGTH INDEX: GSI

20.1' - 22.3' : 55 - 65 22.3' - 26.3' : 30 - 40 26.3' - 30.6' : 60 - 70

B1-BBOX 1 of 2: 15.6 - 25.4 FEET



GEOLOGICAL STRENGTH INDEX: GSI 15.6' - 25.4' : 35 - 45



B1-B

BOX 2 of 2: 25.4 - 35.4 FEET

GEOLOGICAL STRENGTH INDEX: GSI

25.4' - 29.3' : 35 - 45 29.3' - 35.4' : 45 - 55

B2-ABOX 1 of 2: 11.0 - 21.0 FEET



GEOLOGICAL STRENGTH INDEX: GSI 11.0' - 21.0' : 55 - 65



B2-A

BOX 2 of 2: 21.0 - 31.0 FEET

GEOLOGICAL STRENGTH INDEX: GSI 21.0' - 31.0' : 20 - 30

B2-BBOX 1 of 2: 11.3 - 20.7 FEET



GEOLOGICAL STRENGTH INDEX: GSI 11.3' - 20.7' : 55 - 65

FEET



B2-B

BOX 2 of 2: 20.7 - 26.3 FEET

GEOLOGICAL STRENGTH INDEX: GSI 20.7' - 26.3' : 55 - 65

FEET