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REFERENCE: R-1015

PROJECT: 34360

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY CRAVEN
PROJECT DESCRIPTION US 70 (HAVELOCK BYPASS)
FROM NORTH OF CARTERET/CRAVEN COUNTY
LINE TO NORTH OF PINE GROVE ROAD
SITE DESCRIPTION SITE 3 - DUAL BRIDGES NO. 276
AND NO. 277 ON -L- (US 70 - HAVELOCK BYPASS)
OVER EAST PRONG OF SLOCUM CREEK
-L- STATION 177+67.00

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-1015	1	44

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

M. JOHNSON

P. GRAINGER

J.K. CRENSHAW

MID ATLANTIC INC.

INVESTIGATED BY J. K. CRENSHAW

DRAWN BY T. LYNN

CHECKED BY B. HOWEY

SUBMITTED BY B. D. KEANEY

DATE JULY, 2018



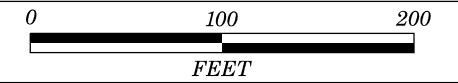
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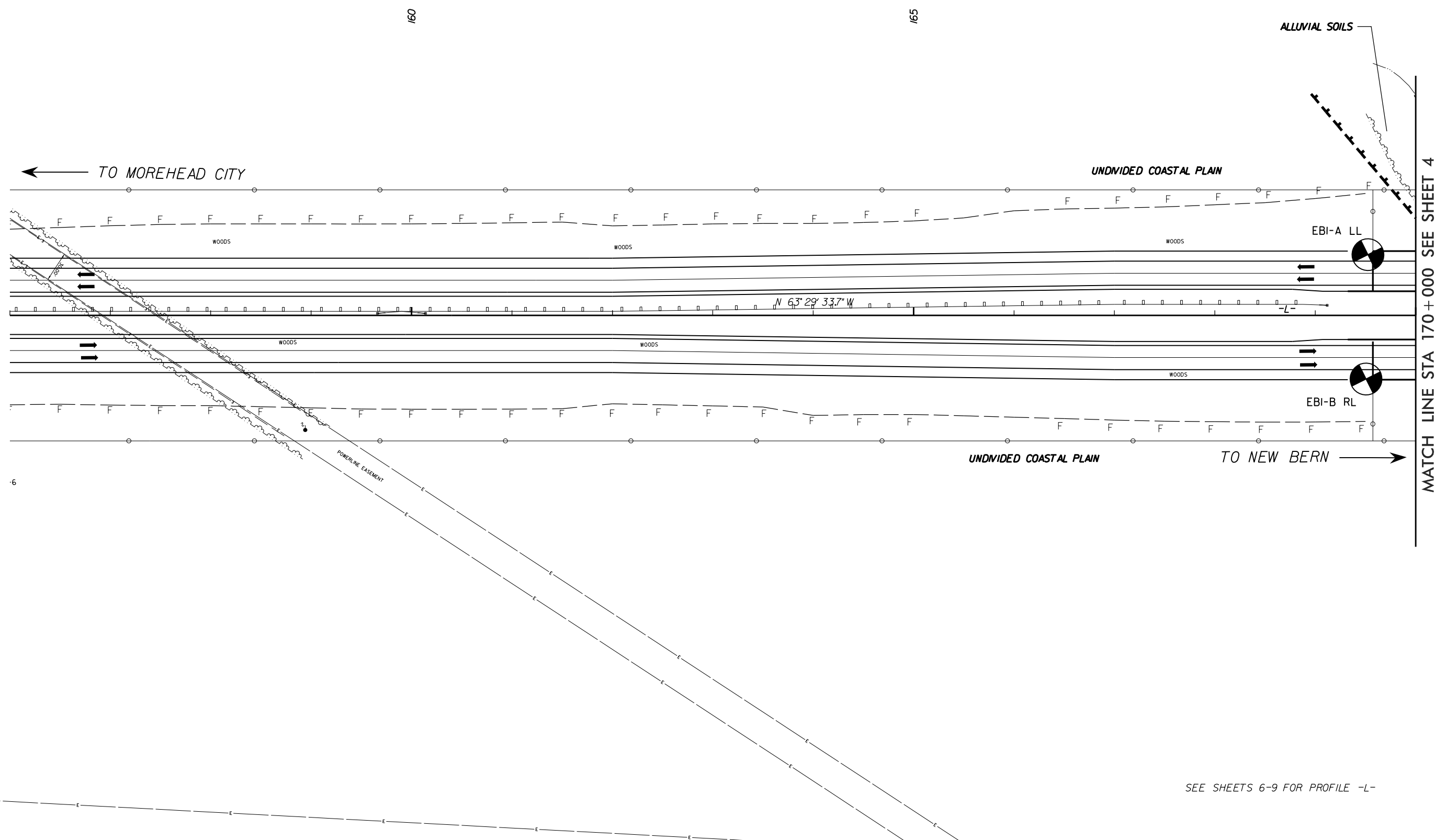
**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																													
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>										<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>										<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>										<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																													
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS										WEATHERED ROCK (WR)										CRYSTALLINE ROCK (CR)																													
<p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>										MINERALOGICAL COMPOSITION										NON-CRYSTALLINE ROCK (NCR)										COASTAL PLAIN SEDIMENTARY ROCK (CP)																													
<p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.</p>										COMPRESSION										WEATHERING										FRESH																													
<p>SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50</p>										PERCENTAGE OF MATERIAL										VERY SLIGHT (V SL.)										SLIGHT (SL.)																													
<p>ORGANIC MATERIAL TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC > 10%</p>										<p>GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL TRACE 1 - 10% LITTLE 10 - 20% SOME 20 - 35% HIGHLY 35% AND ABOVE</p>										MODERATE (MOD.)										MODERATELY SEVERE (MOD. SEV.)																													
GROUND WATER										SEVERE (SEV.)										VERY SEVERE (V SEV.)										COMPLETE																													
<p>▽ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ▽ STATIC WATER LEVEL AFTER 24 HOURS ▽ PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA ○ SPRING OR SEEP</p>										MISCELLANEOUS SYMBOLS										VERY HARD										HARD																													
<p>ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY</p>										<p>DIP & DIP DIRECTION OF ROCK STRUCTURES SPT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION</p>										MODERATELY HARD										MEDIUM HARD																													
<p>SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER HIGHLY ORGANIC SOILS</p>										RECOMMENDATION SYMBOLS										SOFT										VERY SOFT																													
<p>FAIR TO POOR POOR UNSUITABLE</p>										<p>UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK</p>										ABBREVIATIONS										FRACURE SPACING																													
CONSISTENCY OR DENSENESS										TEXTURE OR GRAIN SIZE										SOIL MOISTURE - CORRELATION OF TERMS										PLASTICITY																													
<p>PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT²)</p>										<p>U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053</p>										<p>SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION</p>										<p>NON PLASTIC SLIGHTLY PLASTIC MODERATELY PLASTIC HIGHLY PLASTIC</p>																													
<p>VERY LOOSE 4 TO 10 LOOSE 10 TO 30 MEDIUM DENSE 30 TO 50 DENSE > 50</p>										<p>< 4 4 TO 10 10 TO 30 30 TO 50 > 50</p>										<p>VERY SOFT 2 TO 4 SOFT 4 TO 8 MEDIUM STIFF 8 TO 15 STIFF 15 TO 30 VERY STIFF > 30</p>										<p>< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4</p>																													
<p>EXCELLENT TO GOOD FAIR TO POOR FAIR TO POOR POOR UNSUITABLE</p>										<p>GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3</p>										<p>LIQUID LIMIT (LL) - SATURATED - USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE</p>										<p>PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH</p>																													
EQUIPMENT USED ON SUBJECT PROJECT										FRACURE SPACING										BEDDING										INDURATION																													
<p>DRILL UNITS: CME-45C, CME-55, CME-550, VANE SHEAR TEST, PORTABLE HOIST, D-25, CME-45B</p>										<p>ADVANCING TOOLS: CLAY BITS, 6" CONTINUOUS FLIGHT AUGER, 8" HOLLOW AUGERS, HARD FACED FINGER BITS, TUNG-CARBIDE INSERTS, CASING w/ ADVANCER, TRICONE 2 1/16" STEEL TEETH, TRICONE TUNG-CARB., CORE BIT</p>										<p>HAMMER TYPE: AUTOMATIC, MANUAL CORE SIZE: B, H, N HAND TOOLS: POST HOLE DIGGER, HAND AUGER, SOUNDING ROD, VANE SHEAR TEST</p>										<p>VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY CLOSE LESS THAN 0.16 FEET</p>										<p>VERY THICKLY BEDDED 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET</p>										<p>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.</p>									
COLOR										RECOMMENDATION SYMBOLS										FRACURE SPACING										BEDDING																													
<p>DESCRIPTORS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>										<p>UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK</p>										<p>VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT</p>										<p>VERY THICKLY BEDDED 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET</p>																													
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<p>NON PLASTIC 0-5 SLIGHTLY PLASTIC 6-15 MODERATELY PLASTIC 16-25 HIGHLY PLASTIC 26 OR MORE</p>										<p>UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK</p>										<p>VERY HARD HARD MODERATELY HARD MEDIUM HARD SOFT VERY SOFT</p>										<p>VERY THICKLY BEDDED 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET</p>																													
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PLASTICITY										RECOMMENDATION SYMBOLS										FRACURE SPACING										BEDDING																													

SITE PLAN

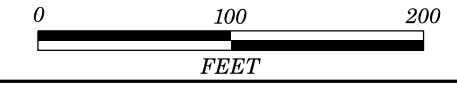


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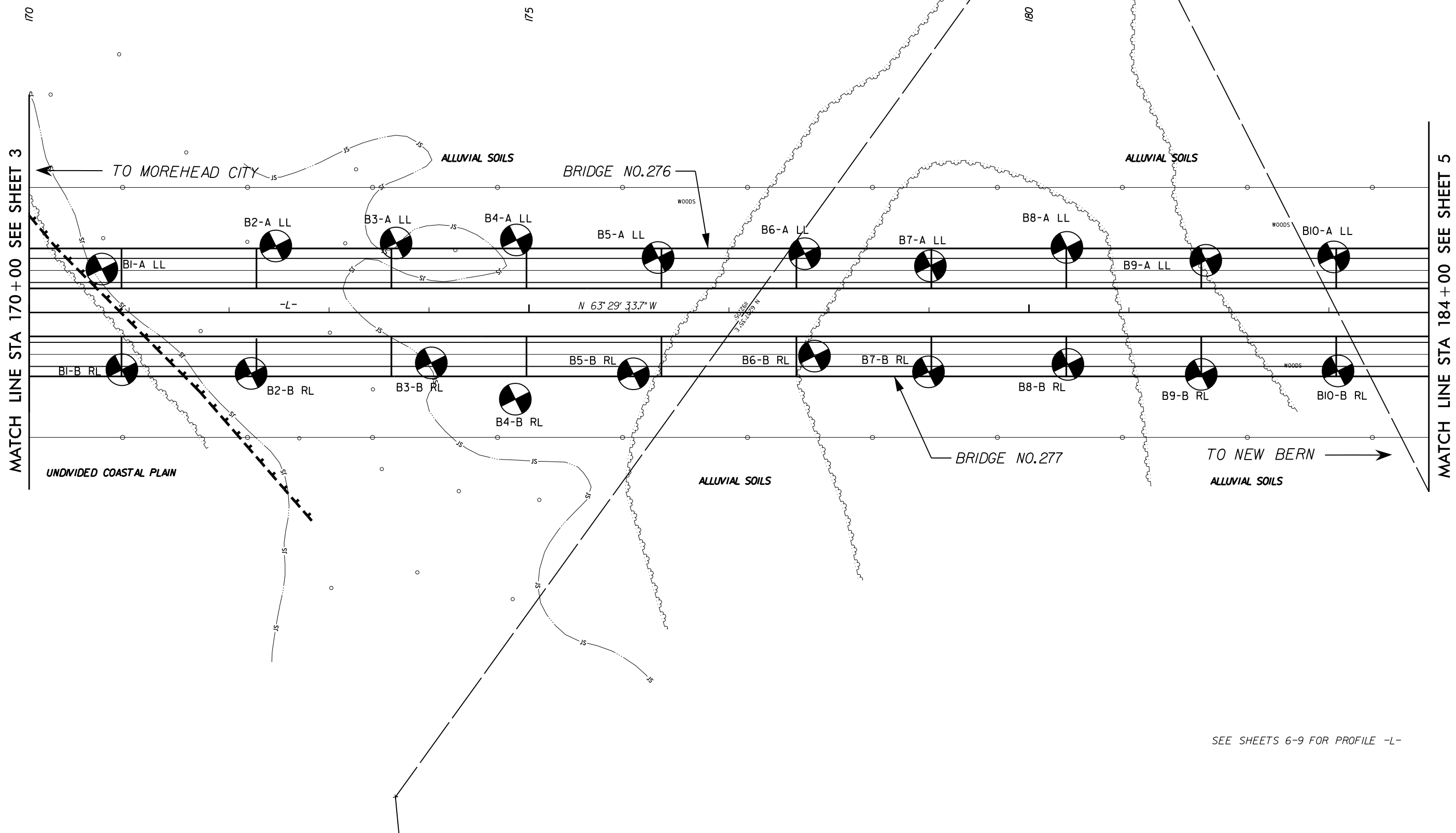
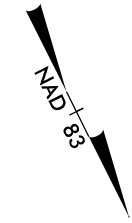


SEE SHEETS 6-9 FOR PROFILE -L-

SITE PLAN

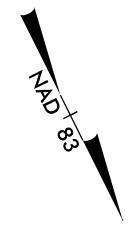


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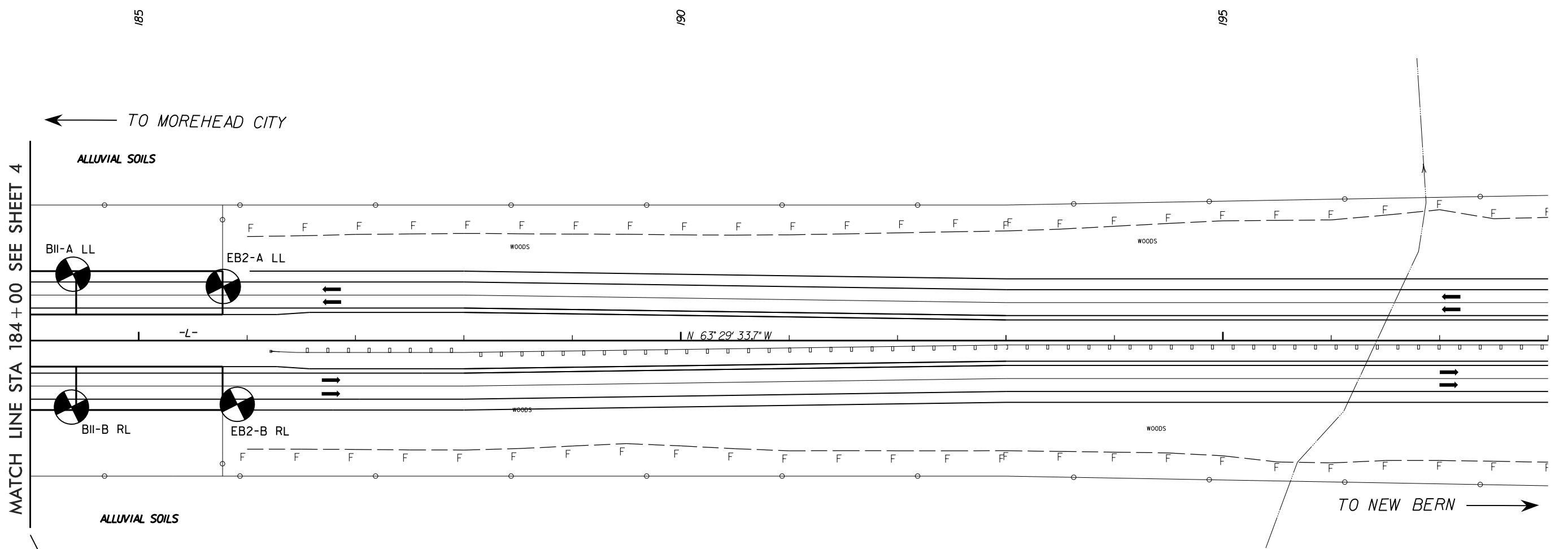


SEE SHEETS 6-9 FOR PROFILE -L-

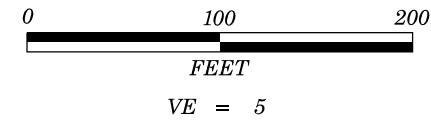
PROJECT REFERENCE NO.	SHEET NO.
R-1015	5
SITE PLAN	
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SKEW = 90°

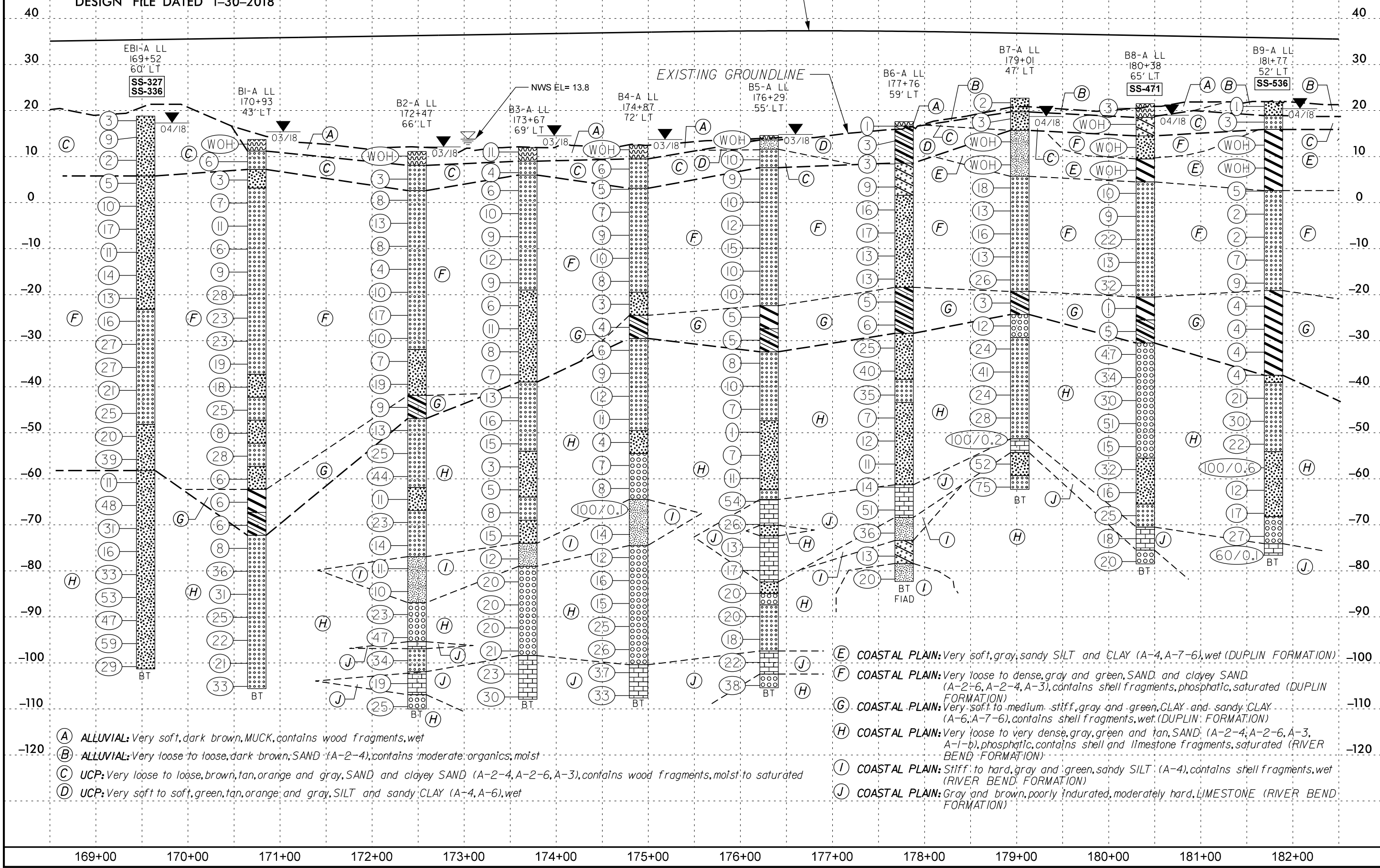


SEE SHEETS 6-9 FOR PROFILE -L-



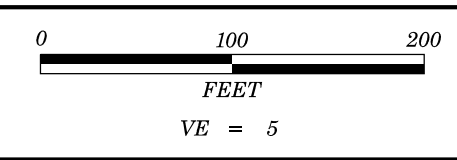
PROJECT REFERENCE NO.	SHEET NO.
R-1015	6
PROFILE - BRIDGE NO. 276 BORINGS PROJECTED ONTO -L-	

NOTES:
 1. BORINGS AND INFERRED STRATIGRAPHY ARE PROJECTED ONTO -L-
 2. GROUNDLINE TAKEN FROM ROADWAY DESIGN FILE DATED 1-30-2018



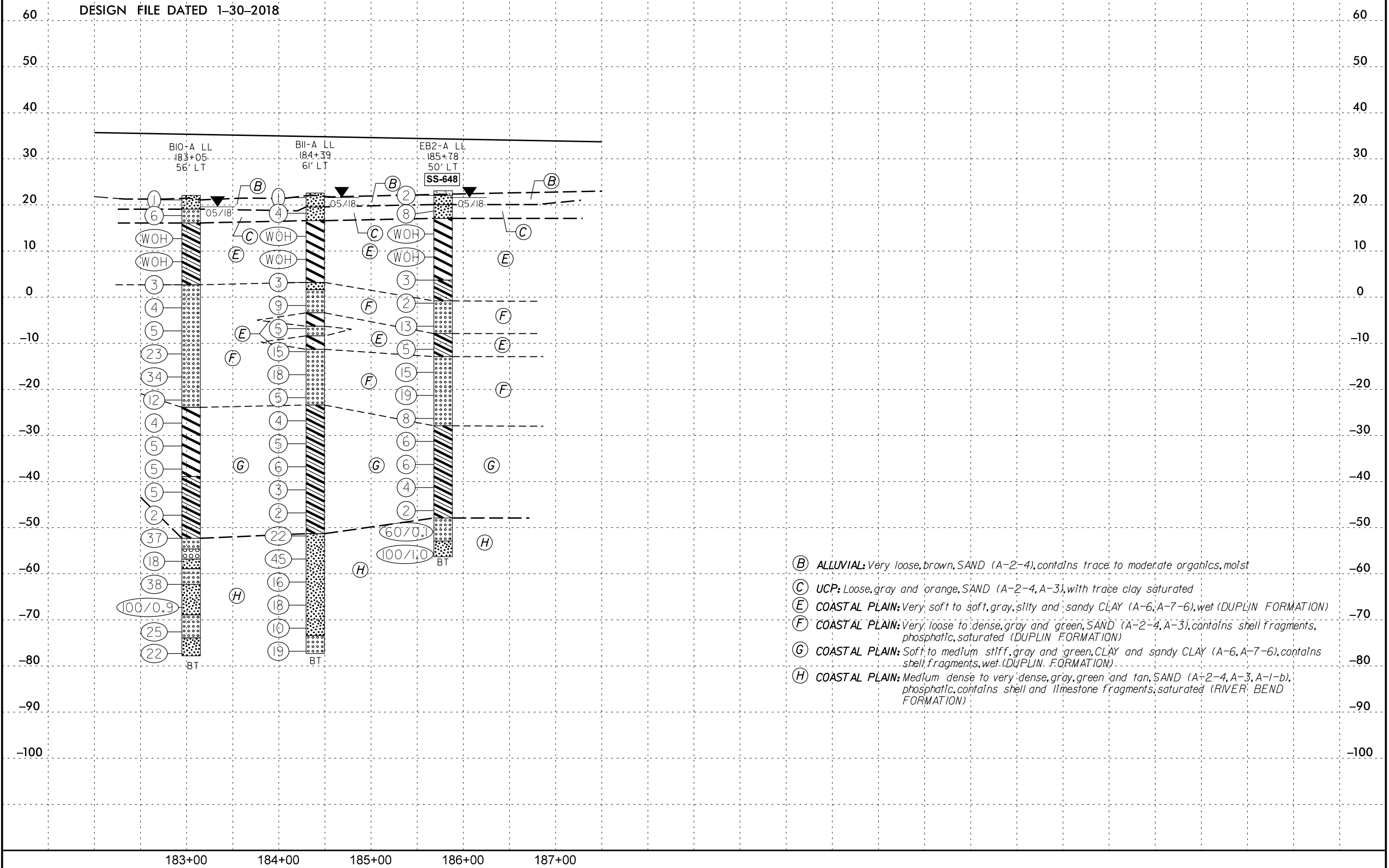
- (A) ALLUVIAL: Very soft, dark brown, MUCK, contains wood fragments, wet
- (B) ALLUVIAL: Very loose to loose, dark brown, SAND (A-2-4), contains moderate organics, moist
- (C) UCP: Very loose to loose, brown, tan, orange and gray, SAND and clayey SAND (A-2-4, A-2-6, A-3), contains wood fragments, moist to saturated
- (D) UCP: Very soft to soft, green, tan, orange and gray, SILT and sandy CLAY (A-4, A-6), wet

- (E) COASTAL PLAIN: Very soft, gray, sandy SILT and CLAY (A-4, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to dense, gray and green, SAND, and clayey SAND (A-2-6, A-2-4, A-3), contains shell fragments, phosphatic, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Very soft to medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Very loose to very dense, gray, green and tan, SAND (A-2-4, A-2-6, A-3, A-1-b), phosphatic, contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff to hard, gray and green, sandy SILT (A-4), contains shell fragments, wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray and brown, poorly indurated, moderately hard, LIMESTONE (RIVER BEND FORMATION)



PROJECT REFERENCE NO.	SHEET NO.
R-1015	7
PROFILE - BRIDGE NO. 276 BORINGS PROJECTED ONTO -L-	

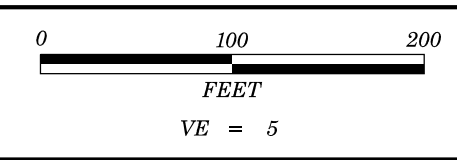
NOTES:
 1. BORINGS AND INFERRED STRATIGRAPHY ARE PROJECTED ONTO -L-
 2. GROUNDLINE TAKEN FROM ROADWAY DESIGN FILE DATED 1-30-2018



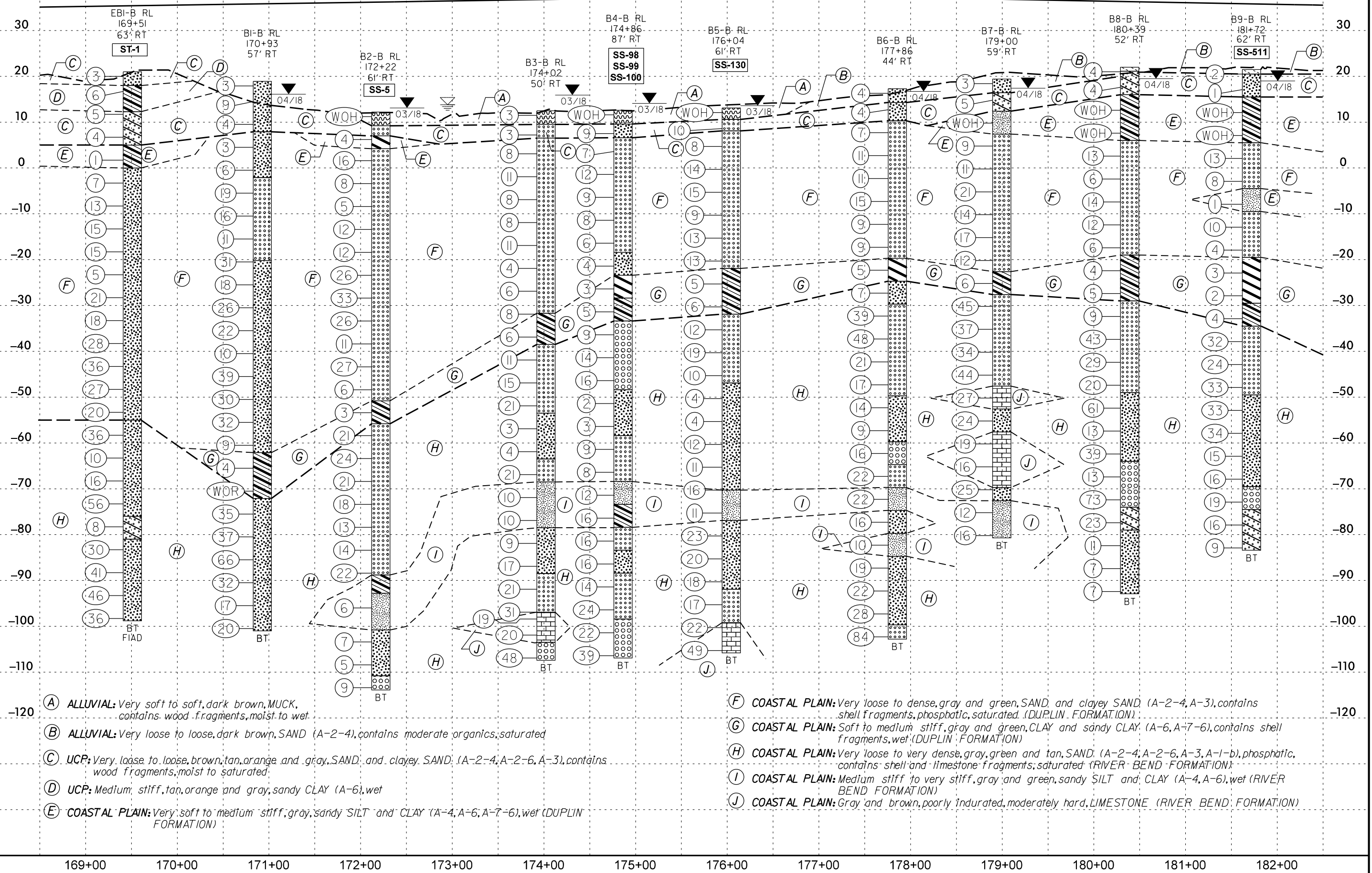
- (B) ALLUVIAL: Very loose, brown, SAND (A-2-4), contains trace to moderate organics, moist
- (C) UCP: Loose, gray and orange, SAND (A-2-4, A-3), with trace clay saturated
- (E) COASTAL PLAIN: Very soft to soft, gray, silty and sandy CLAY (A-6, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to dense, gray and green, SAND (A-2-4, A-3), contains shell fragments, phosphatic, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Medium dense to very dense, gray, green and tan, SAND (A-2-4, A-3, A-1-b), phosphatic, contains shell and limestone fragments, saturated (RIVER BEND FORMATION)

NOTES:

1. BORINGS AND INFERRED STRATIGRAPHY ARE PROJECTED ONTO -L-
2. GROUNDLINE TAKEN FROM ROADWAY DESIGN FILE DATED T-30-2018

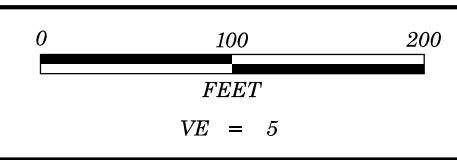


PROJECT REFERENCE NO.	SHEET NO.
R-1015	8
PROFILE - BRIDGE NO. 277	
BORINGS PROJECTED ONTO -L-	



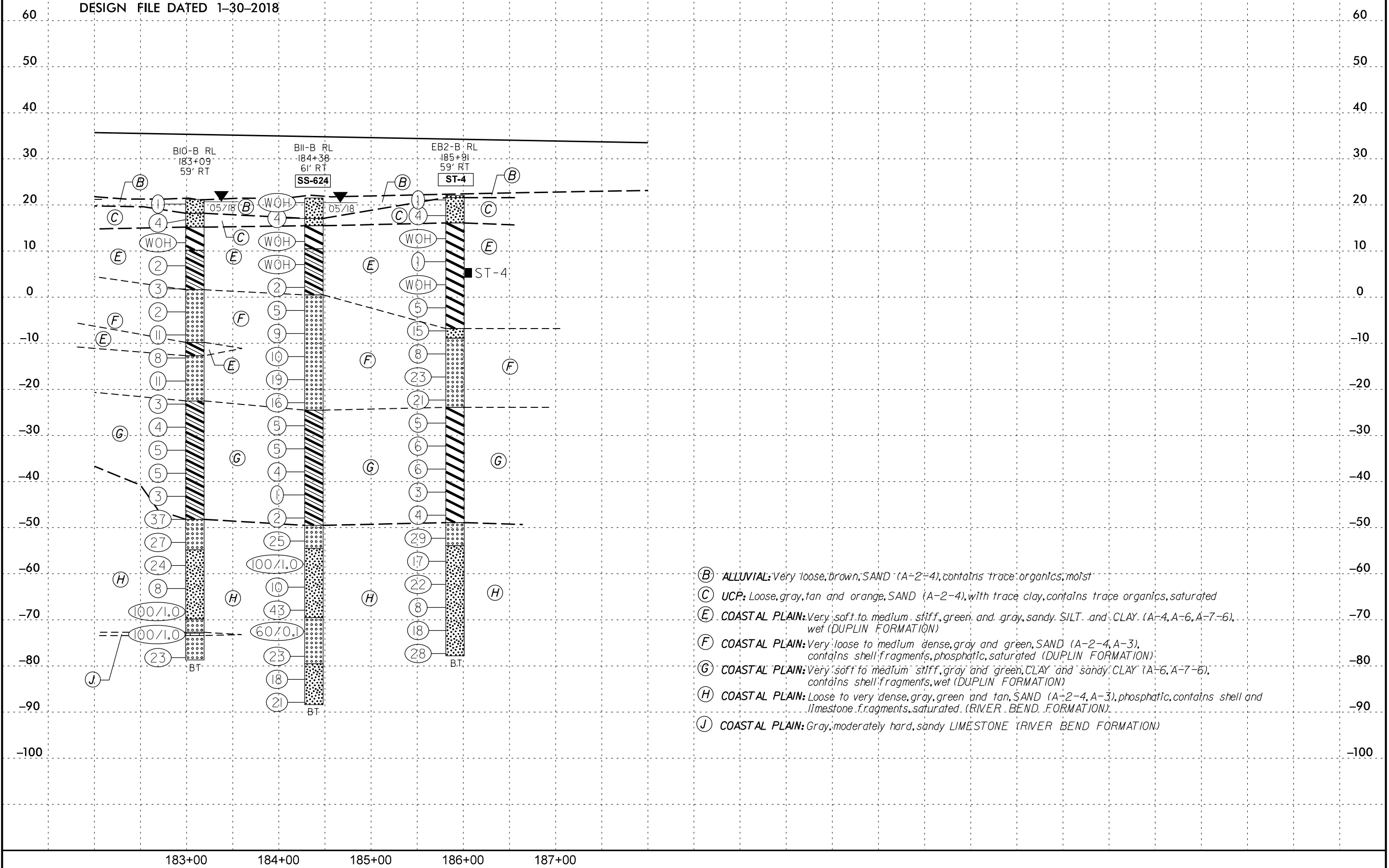
- (A) **ALLUVIAL:** Very soft to soft, dark brown, MUCK, contains wood fragments, moist to wet.
- (B) **ALLUVIAL:** Very loose to loose, dark brown, SAND (A-2-4), contains moderate organics, saturated.
- (C) **UCP:** Very loose to loose, brown, tan, orange and gray, SAND, and clayey SAND (A-2-4, A-2-6, A-3), contains wood fragments, moist to saturated.
- (D) **UCP:** Medium stiff, tan, orange and gray, sandy CLAY (A-6), wet.
- (E) **COASTAL PLAIN:** Very soft to medium stiff, gray, sandy SILT and CLAY (A-4, A-6, A-7-6), wet (DUPLIN FORMATION).

- (F) **COASTAL PLAIN:** Very loose to dense, gray and green, SAND and clayey SAND (A-2-4, A-3), contains shell fragments, phosphatic, saturated (DUPLIN FORMATION).
- (G) **COASTAL PLAIN:** Soft to medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION).
- (H) **COASTAL PLAIN:** Very loose to very dense, gray, green and tan, SAND (A-2-4, A-2-6, A-3, A-1-b), phosphatic, contains shell and limestone fragments, saturated (RIVER BEND FORMATION).
- (I) **COASTAL PLAIN:** Medium stiff to very stiff, gray and green, sandy SILT and CLAY (A-4, A-6), wet (RIVER BEND FORMATION).
- (J) **COASTAL PLAIN:** Gray and brown, poorly indurated, moderately hard, LIMESTONE (RIVER BEND FORMATION).



PROJECT REFERENCE NO.	SHEET NO.
R-1015	9
PROFILE - BRIDGE NO. 277	
BORINGS PROJECTED ONTO -L-	

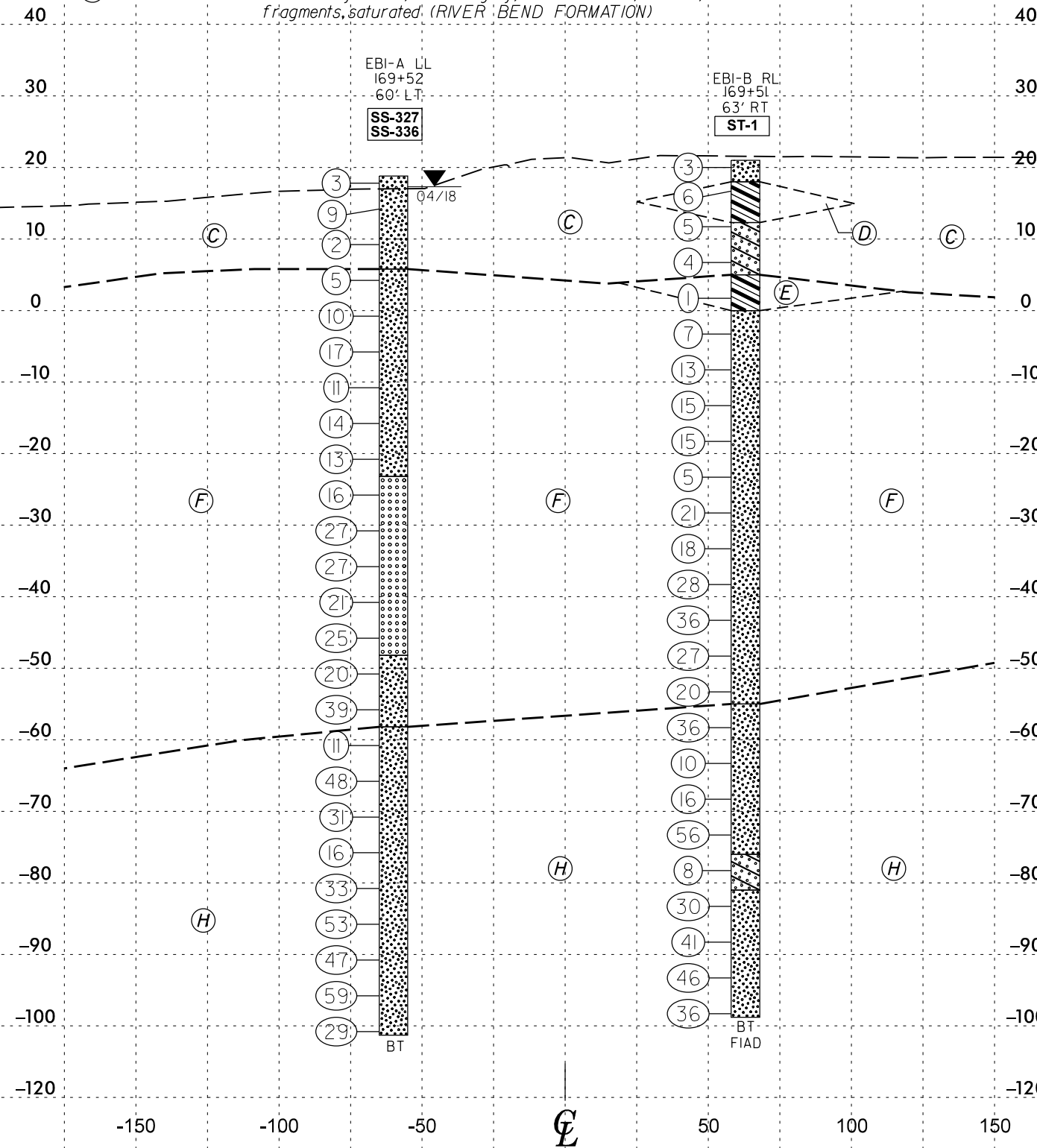
NOTES:
 1. BORINGS AND INFERRED STRATIGRAPHY ARE PROJECTED ONTO -L-
 2. GROUNDLINE TAKEN FROM ROADWAY DESIGN FILE DATED 1-30-2018



- (B) ALLUVIAL: Very loose, brown, SAND (A-2-4), contains trace organics, moist
- (C) UCP: Loose, gray, tan and orange, SAND (A-2-4), with trace clay, contains trace organics, saturated
- (E) COASTAL PLAIN: Very soft to medium stiff, green and gray, sandy SILT and CLAY (A-4, A-6, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to medium dense, gray and green, SAND (A-2-4, A-3), contains shell fragments, phosphatic, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Very soft to medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, gray, green and tan, SAND (A-2-4, A-3), phosphatic, contains shell and limestone fragments, saturated. (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, moderately hard, sandy LIMESTONE (RIVER BEND FORMATION)

NOTE: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE SECTION

- (C) UCP: Very loose to loose, tan, orange and light gray, SAND and clayey SAND (A-2-4, A-2-6), moist to saturated
- (D) UCP: Medium stiff, gray and orange, fine sandy and silty CLAY (A-6), moist to wet
- (E) COASTAL PLAIN: Very soft, gray, sandy CLAY (A-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Loose to dense, gray, brown and tan, fine to coarse SAND (A-2-4, A-3), with silt and clay interbeds, contains shell fragments, saturated (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, tan and gray, SAND (A-2-4, A-2-6), contains cemented shell fragments, saturated (RIVER BEND FORMATION)

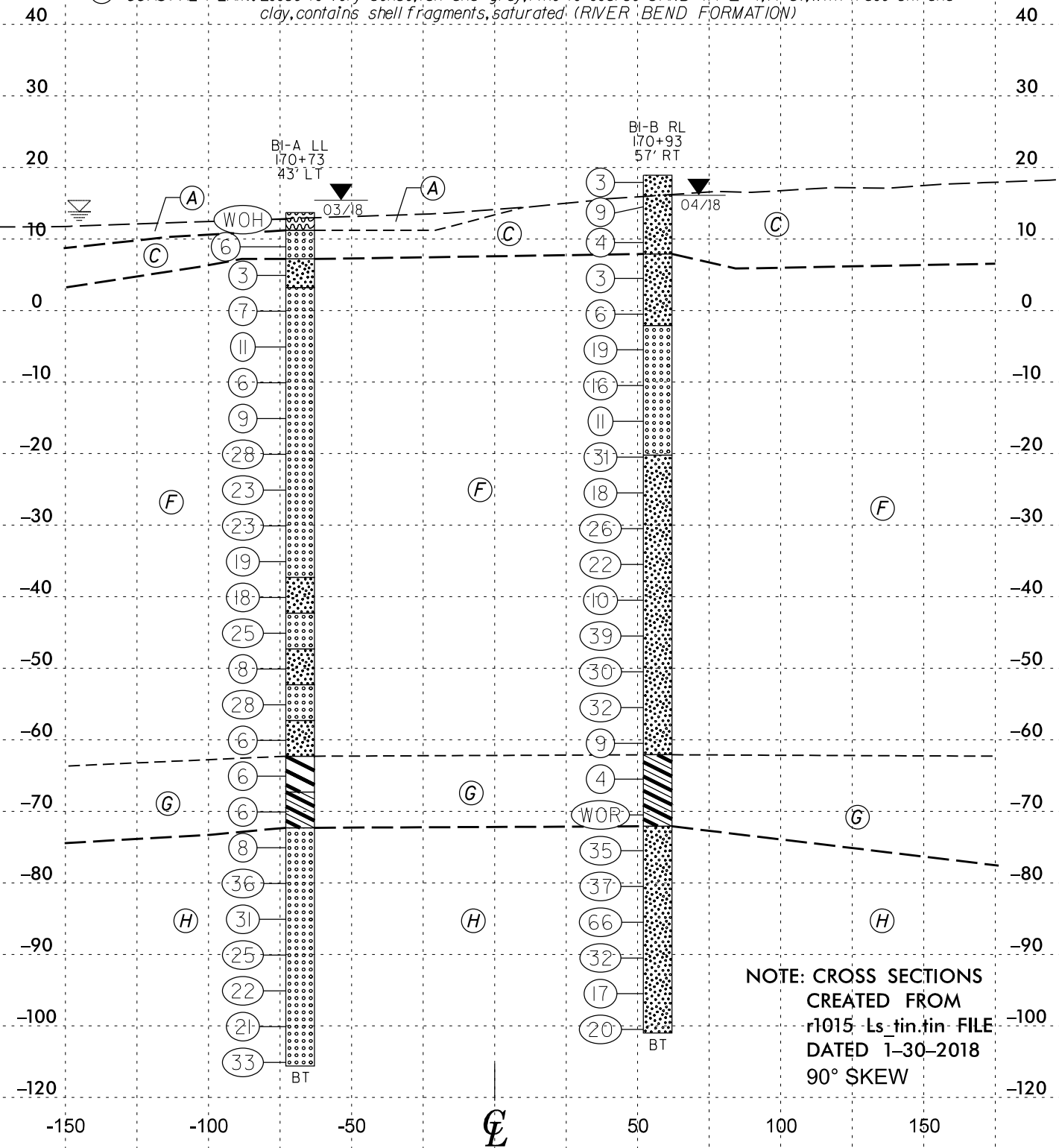


HORIZ. SCALE 0 50 100 (FEET)

VE = 2.5

CROSS SECTION - END BENT 1
-L- STA 169+55.33

- (A) ALLUVIAL: Very soft, dark brown, MUCK, moist
- (C) UCP: Very loose to loose, gray and tan, clayey and silty SAND (A-3, A-2-4), contains wood fragments, moist to saturated
- (F) COASTAL PLAIN: Very loose to dense, gray, tan and brown, SAND and clayey SAND (A-2-6, A-2-4, A-3), phosphatic, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Very soft to medium stiff, green and gray, CLAY and sandy CLAY (A-6, A-7-6), micaceous, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, tan and gray, fine to coarse SAND (A-2-4, A-3), with trace silt and clay, contains shell fragments, saturated (RIVER BEND FORMATION)



HORIZ. SCALE 0 50 100 (FEET)

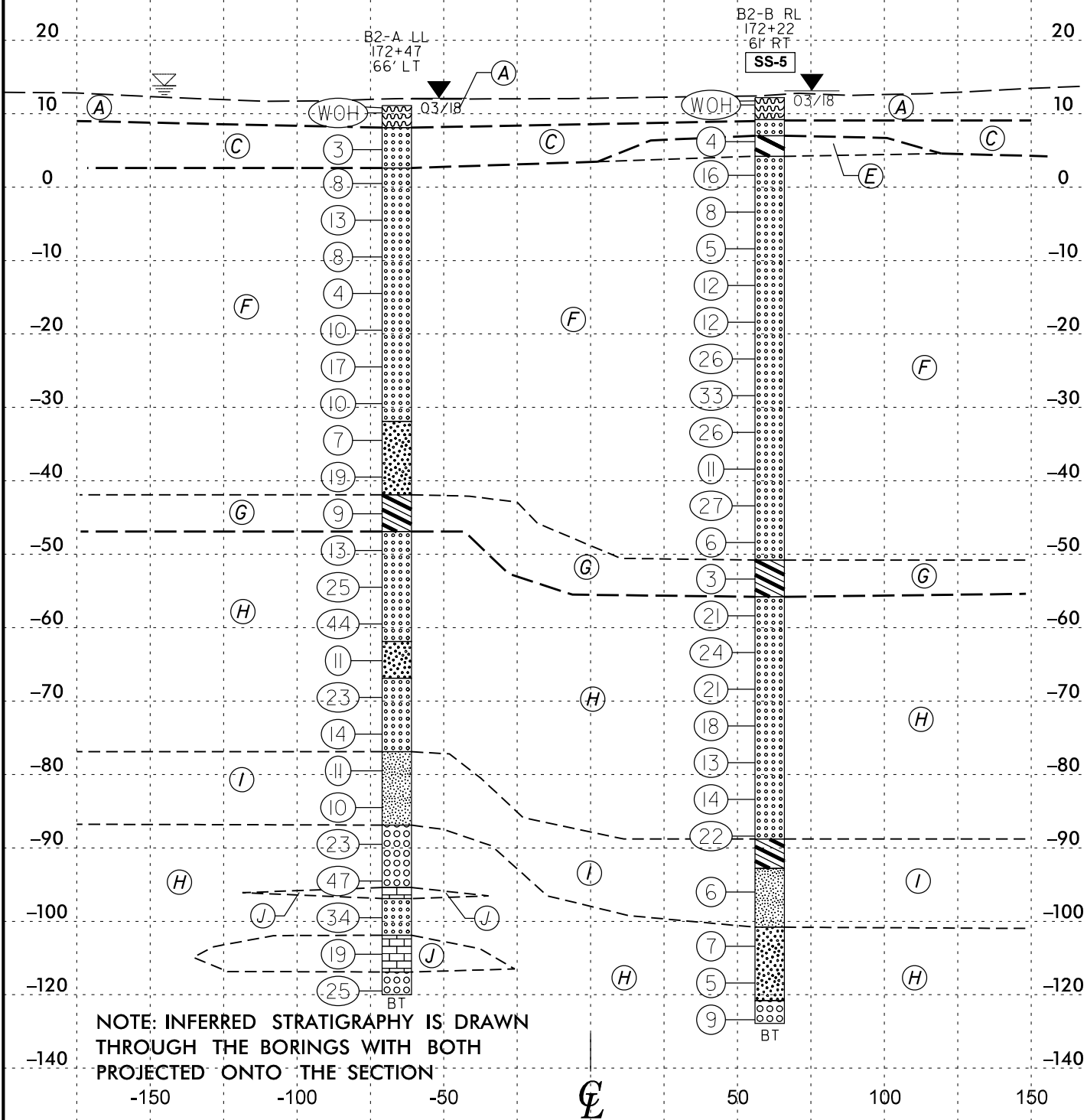
VE = 2.5

CROSS SECTION - BENT 1
-L- STA 170+91.50

NOTE: CROSS SECTIONS
CREATED FROM
r1015-Ls_tin.tin FILE
DATED 1-30-2018
90° SKEW

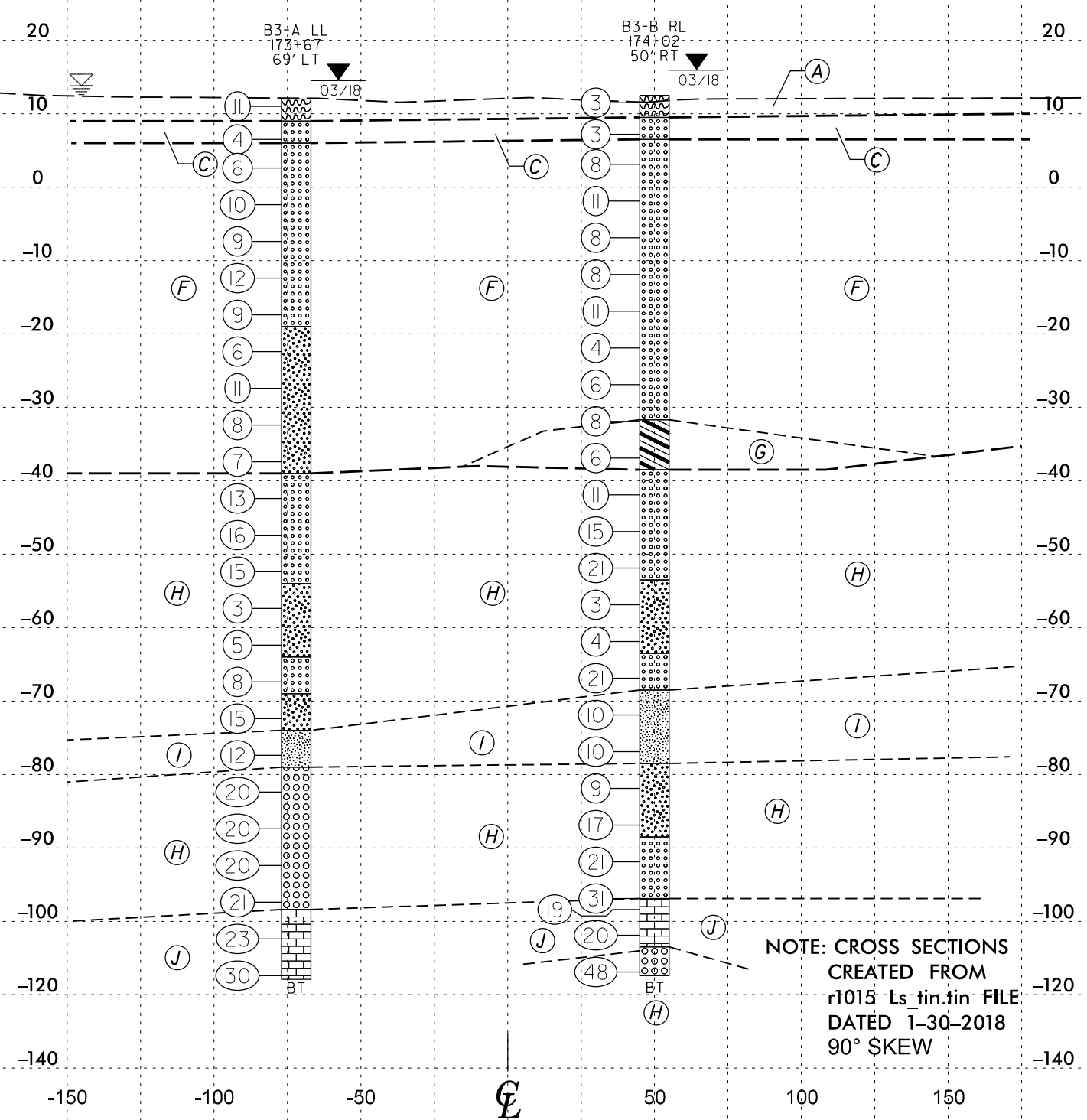
- (A) ALLUVIAL: Very soft, dark brown, MUCK, wet
- (C) UCP: Very loose, brown, gray and tan, SAND (A-3), contains wood fragments, saturated
- (E) COASTAL PLAIN: Medium stiff, gray and green, silty CLAY (A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Loose to dense, gray and brown, SAND (A-2-4, A-3), phosphatic, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to stiff, gray, sandy CLAY (A-6), wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to medium dense, green and gray, SAND (A-1-b, A-2-4, A-3), saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Medium stiff to very stiff, gray and green, SILT and sandy CLAY (A-4, A-6), wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, sandy, moderately hard, LIMESTONE (RIVER BEND FORMATION)

- (A) ALLUVIAL: Soft to stiff, dark brown, MUCK, contains wood fragments, moist to wet
- (C) UCP: Very loose, tan, SAND (A-3), contains wood fragments, saturated
- (F) COASTAL PLAIN: Loose to medium dense, gray and green, SAND (A-2-4, A-3), phosphatic, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Medium stiff, gray and green, sandy CLAY (A-6), contains shell fragments (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Very loose to dense, gray and green, SAND (A-1-b, A-2-4, A-3), contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff, gray and green, sandy SILT (A-4), contains shell fragments, wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, sandy, moderately hard, LIMESTONE (RIVER BEND FORMATION)



HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 2
-L- STA 172+26+75



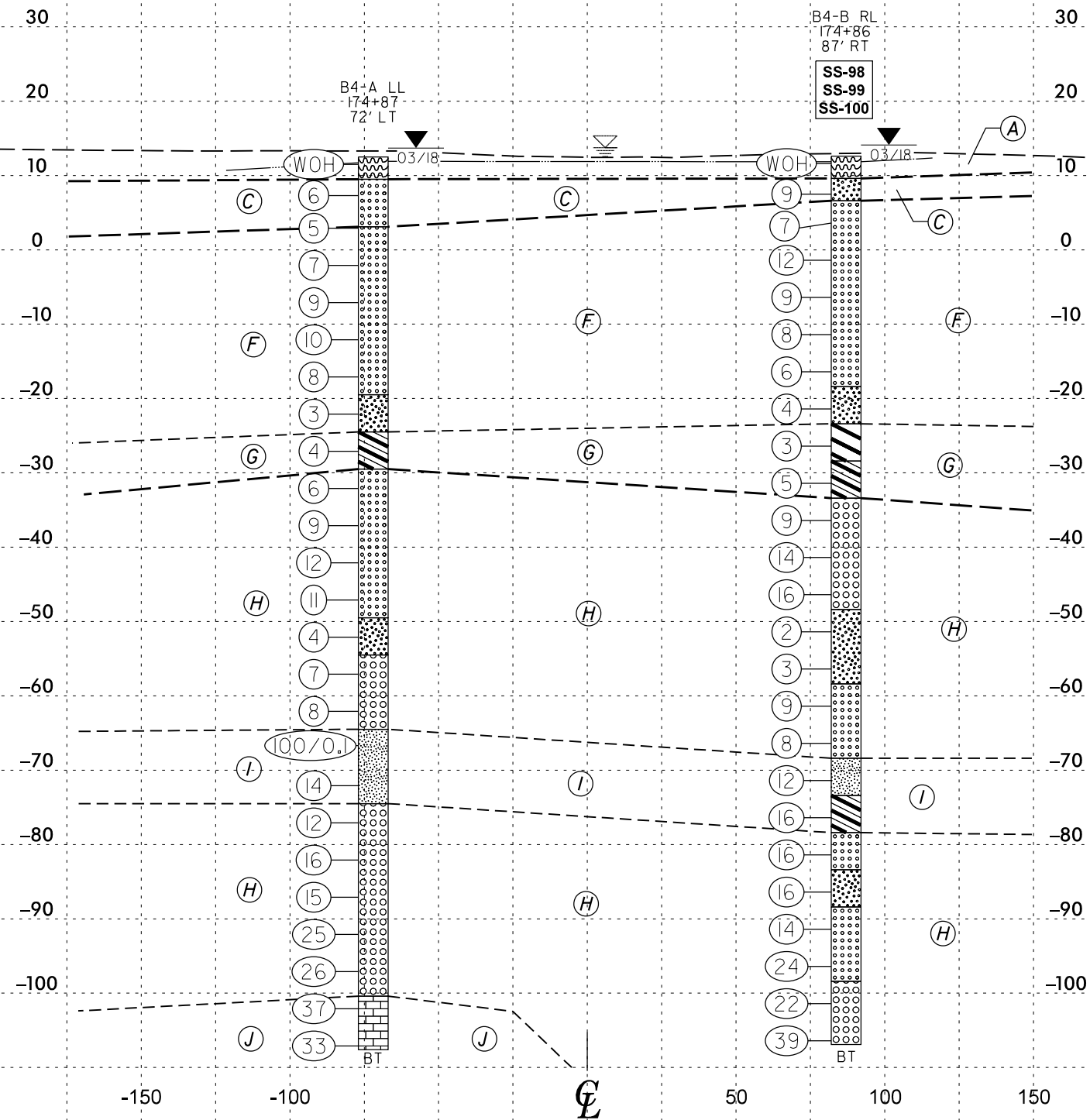
HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 3
-L- STA 173+61+75

NOTE: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE SECTION

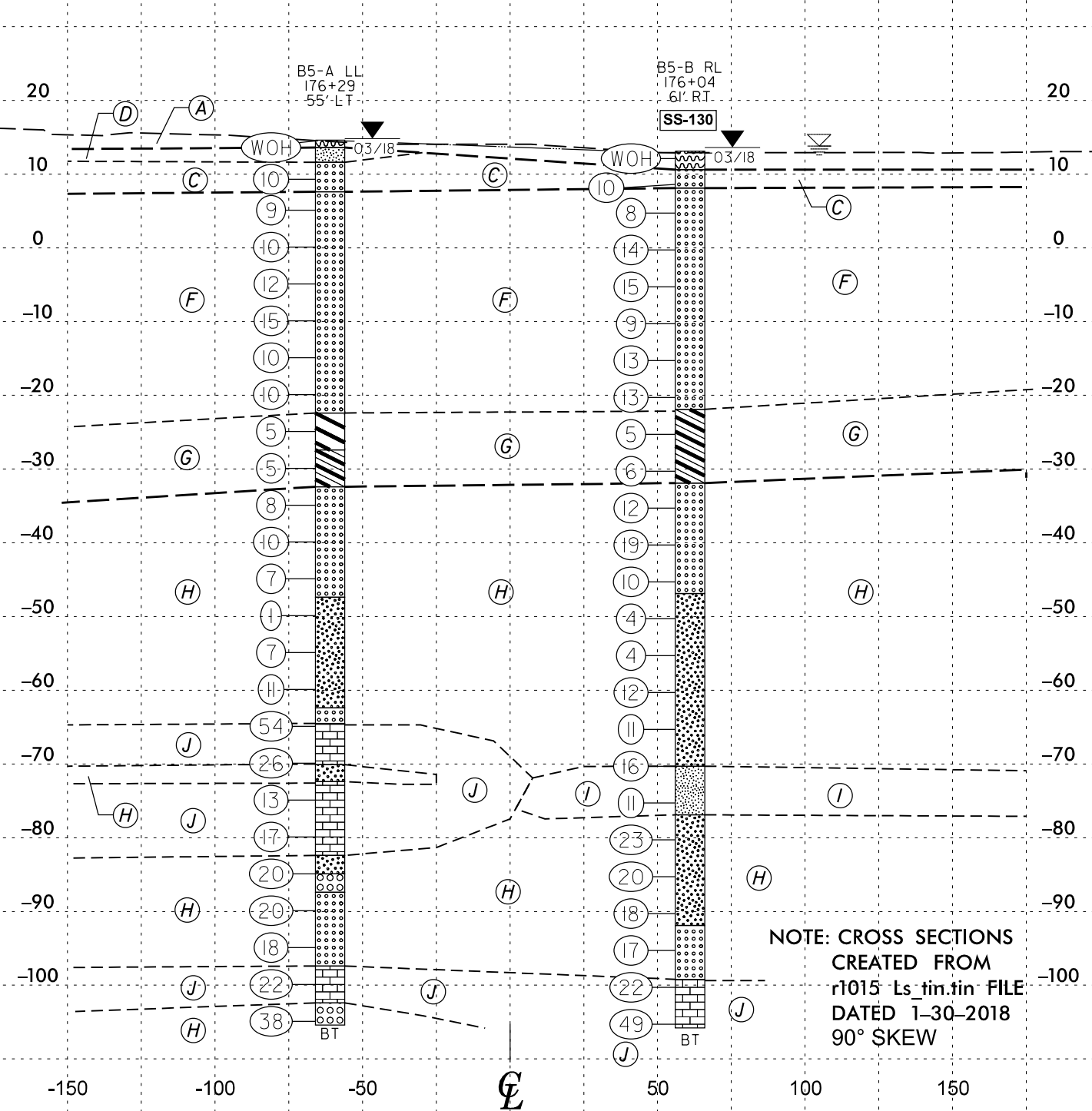
- (A) ALLUVIAL: Very soft, dark brown, MUCK, wet
- (C) UCP: Loose, tan and brown, SAND (A-2-4, A-3), contains wood fragments, saturated
- (F) COASTAL PLAIN: Very loose to medium dense, gray and green, SAND (A-2-4, A-3), contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to dense, gray and green, SAND (A-2-4, A-3, A-1-b), with trace clay, contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff to hard, green, sandy SILT and sandy CLAY (A-4, A-6), contains shell fragments, wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Brown and gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)

- (A) ALLUVIAL: Very soft, dark brown, MUCK, wet
- (C) UCP: Loose to medium dense, tan, SAND (A-3), contains wood fragments, saturated
- (D) UCP: Very soft, green, SILT (A-4), wet
- (F) COASTAL PLAIN: Loose to medium dense, gray and green, SAND (A-3), contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Medium stiff, gray and green, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, saturated (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Very loose to dense, gray and green, SAND (A-2-4, A-3, A-1-b), with trace clay, contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff, gray and green, sandy SILT (A-4), wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray and brown, moderately hard, LIMESTONE (RIVER BEND FORMATION)



HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 4
-L- STA 174+96.75



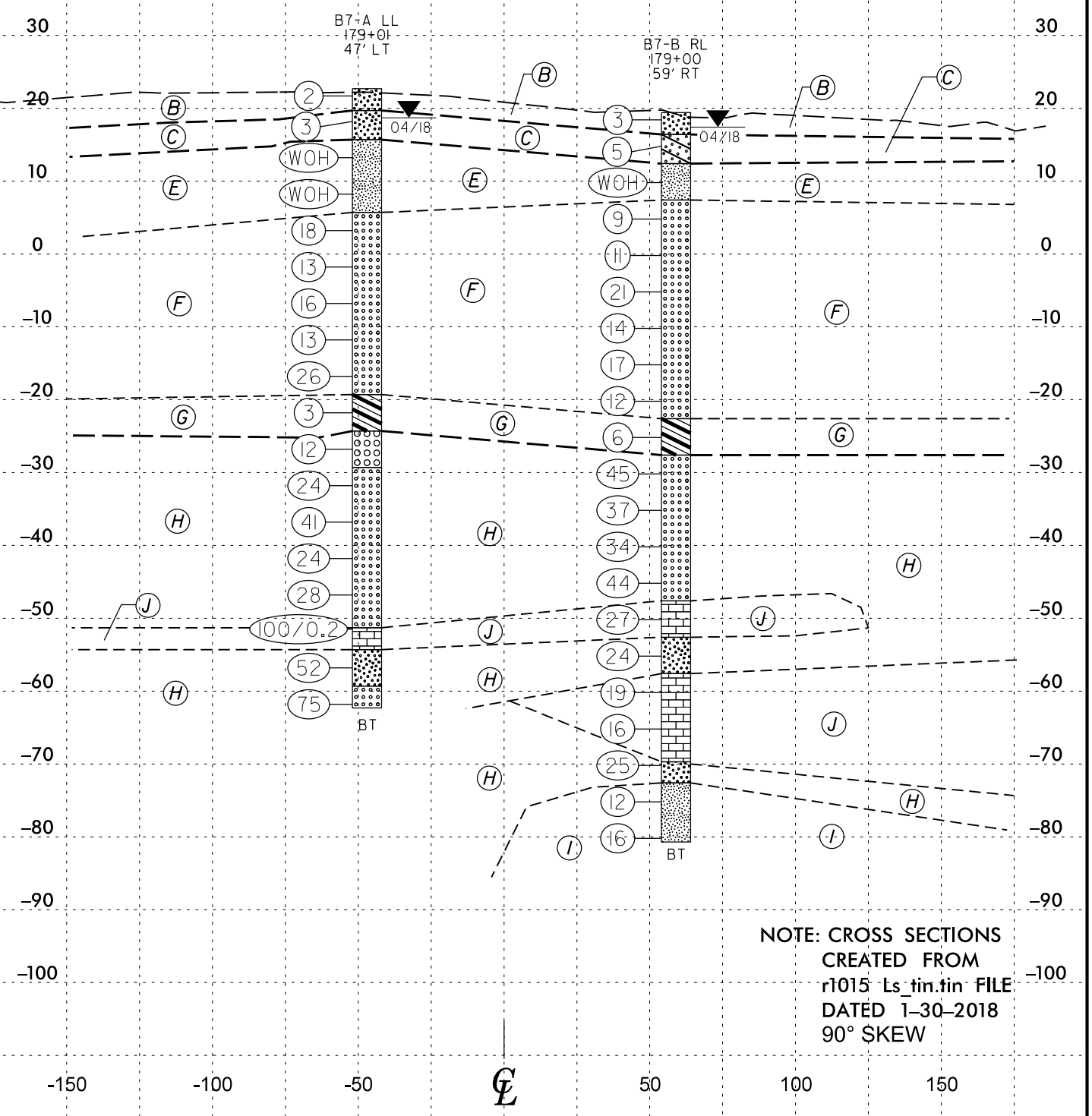
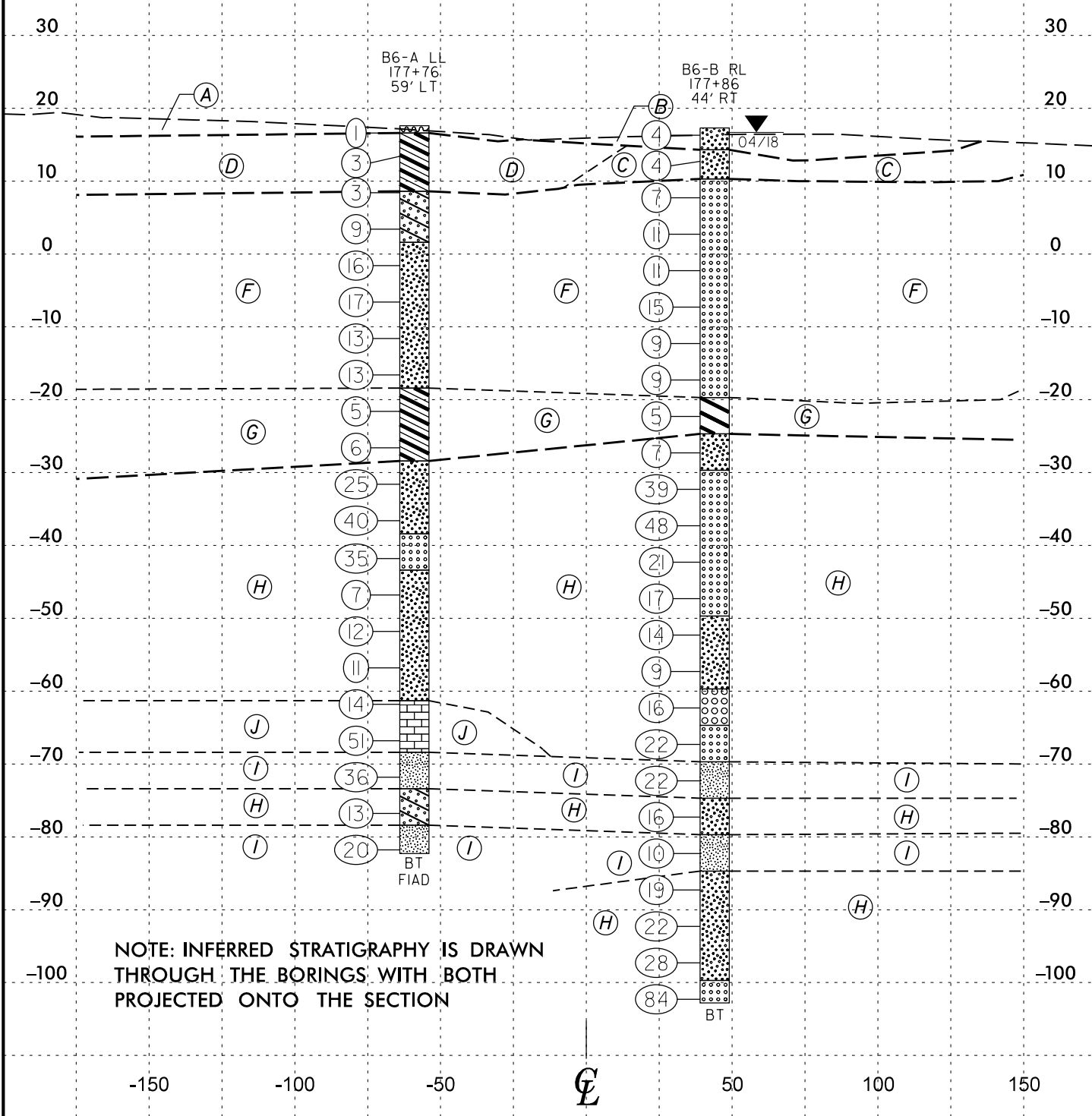
HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 5
-L- STA 176+32.00

NOTE: CROSS SECTIONS
CREATED FROM
r1015_Ls_tin.tin FILE
DATED 1-30-2018
90° SKEW

- (A) ALLUVIAL: Very soft, brown, MUCK, wet
- (B) ALLUVIAL: Loose, dark brown, SAND (A-2-4), moist to saturated
- (C) UCP: Loose, gray and tan, SAND (A-2-4), saturated
- (D) UCP: Soft, tan, orange and gray, fine sandy, silty CLAY (A-6), wet
- (F) COASTAL PLAIN: Very loose to medium dense, gray, SAND and clayey SAND (A-2-6, A-2-4, A-3), phosphatic, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Medium stiff, green and gray, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, gray, green and tan, SAND (A-2-6, A-2-4, A-3, A-1-b), contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff to hard, gray and green, sandy SILT (A-4), contains shell fragments, wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, sandy, moderately hard, LIMESTONE (RIVER BEND FORMATION)

- (B) ALLUVIAL: Very loose, brown and gray, SAND (A-2-4), moist
- (C) UCP: Very loose to loose, tan and orange, SAND and clayey SAND (A-2-4, A-2-6), saturated
- (E) COASTAL PLAIN: Very soft, gray, sandy SILT (A-4), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Loose to medium dense, gray and green, SAND (A-3), contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to medium stiff, gray, sandy CLAY (A-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Medium dense to very dense, gray, SAND (A-3, A-1-b, A-2-4), phosphatic, contains shell fragments, saturated (RIVER BEND FORMATION)
- (I) COASTAL PLAIN: Stiff to very stiff green, sandy SILT (A-4), contains shell fragments, wet (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, poorly indurated, moderately hard, LIMESTONE (RIVER BEND FORMATION)



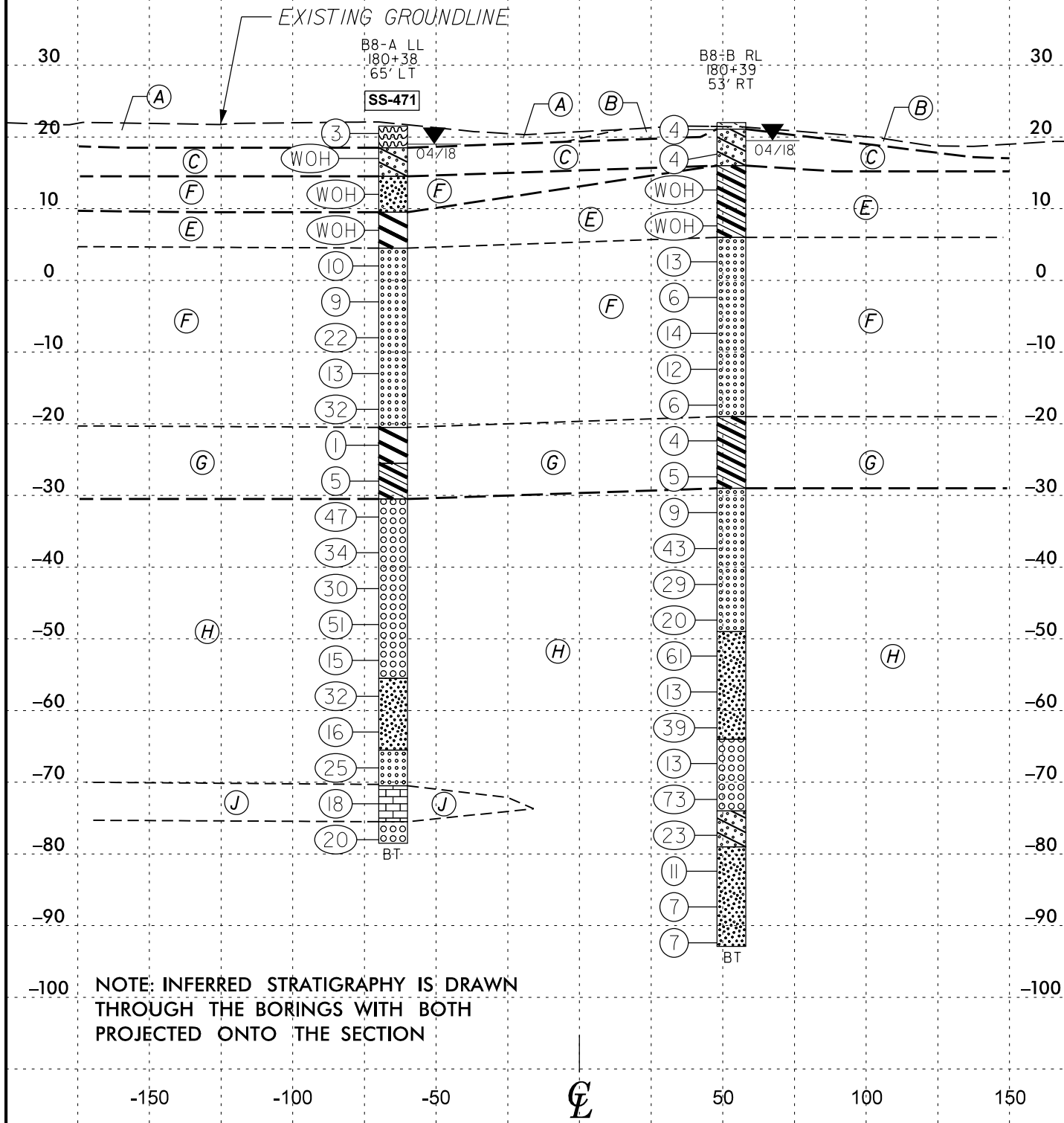
HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 6
-L- STA 177+67.00

HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 7
-L- STA 179+02.00

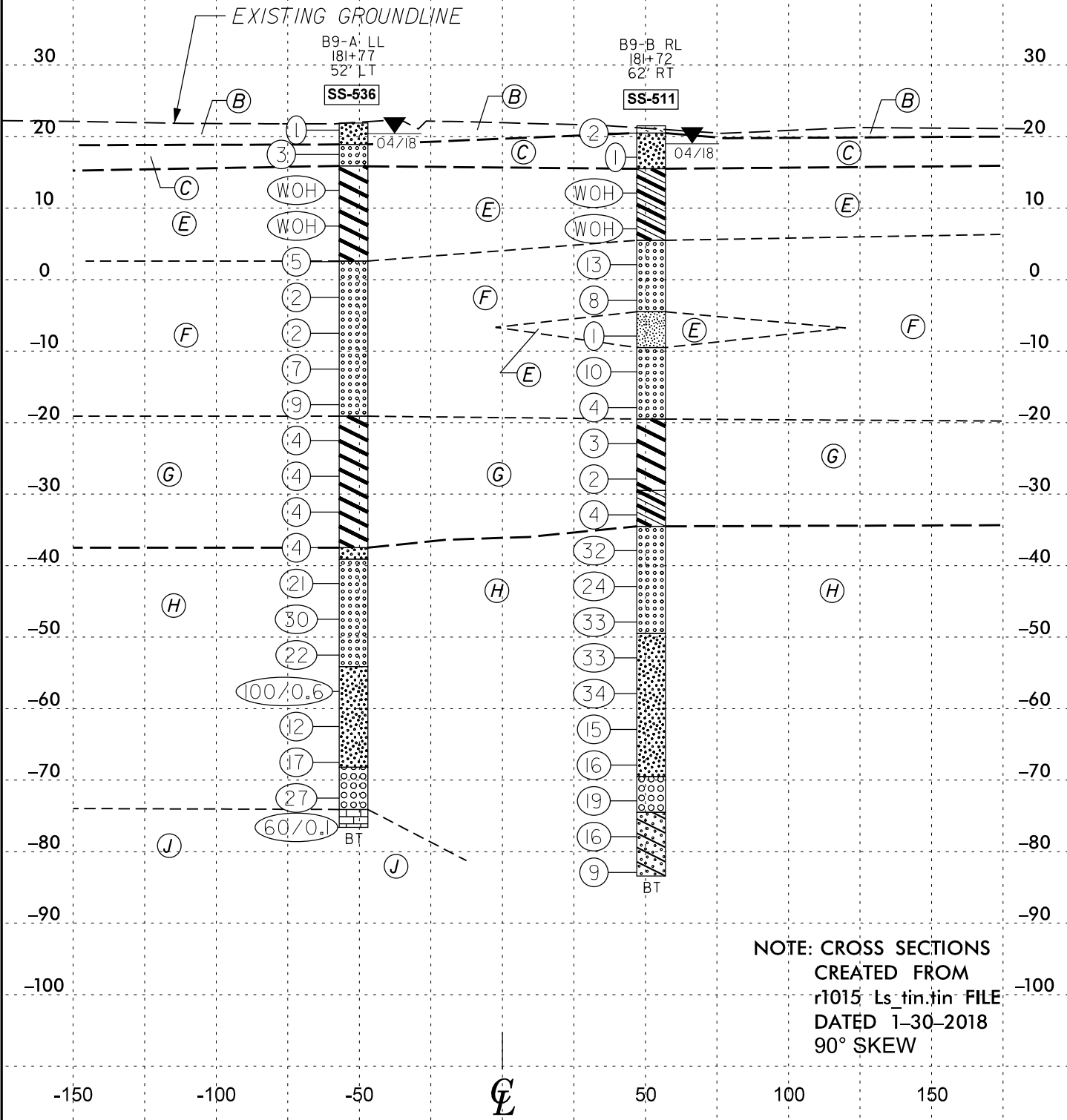
- (A) ALLUVIAL: Very soft, dark brown, MUCK, moist
- (B) ALLUVIAL: Very loose, dark brown, SAND (A-2-4), contains moderate organics, moist
- (C) UCP: Very loose, brown, tan and orange, clayey SAND (A-2-6), saturated
- (E) COASTAL PLAIN: Very soft, gray, sandy and silty CLAY (A-6, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to dense, gray, SAND (A-2-4, A-3) with clay interbeds, phosphatic, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Very soft to medium stiff, gray, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, gray and green, SAND and clayey SAND (A-1-b, A-2-6, A-2-4, A-3), phosphatic, contains shell and limestone fragments, saturated (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)



HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 8
-L- STA 180+37.25

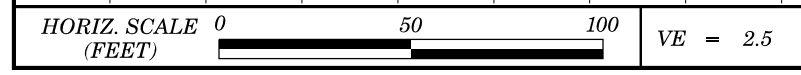
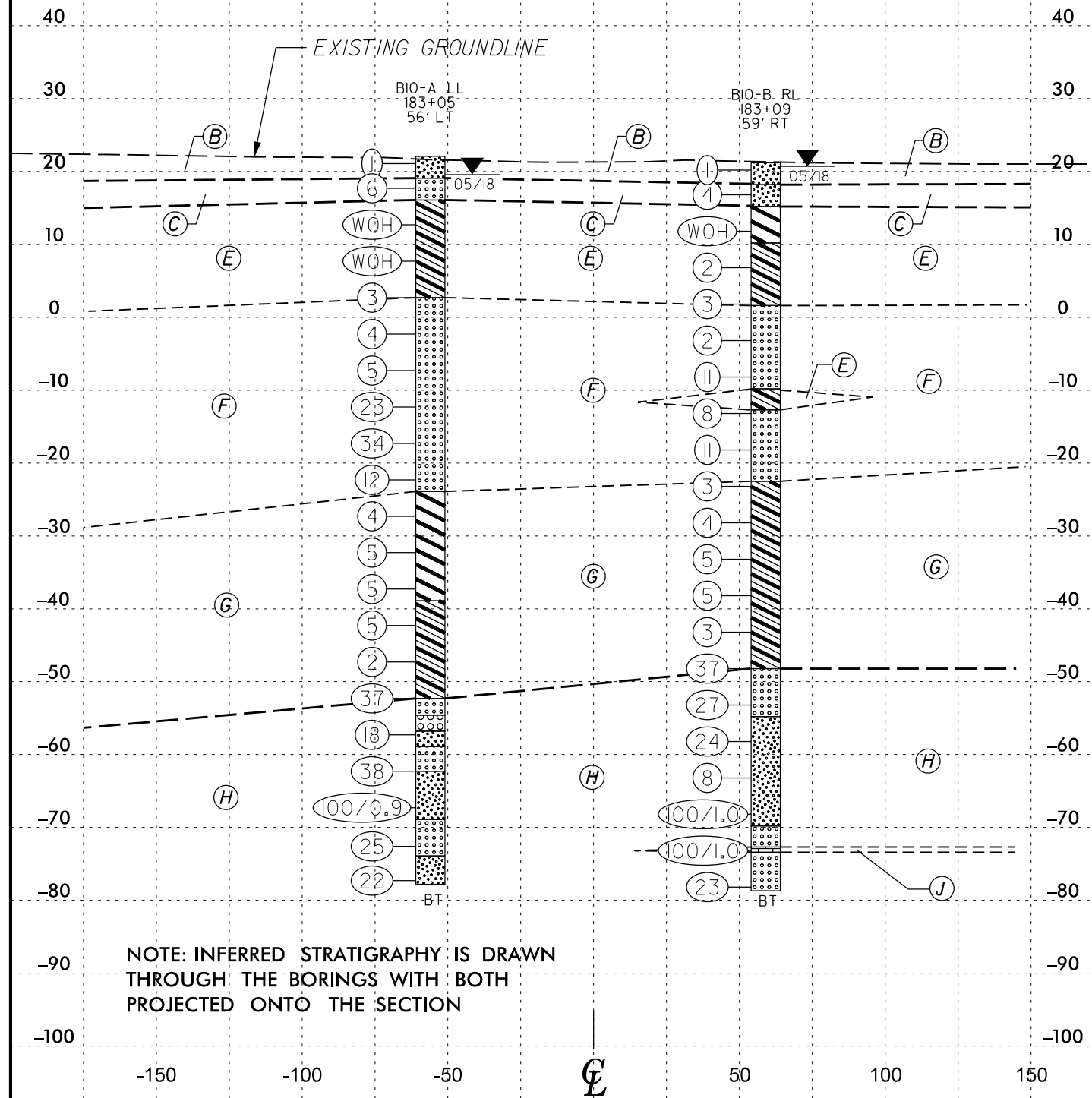
- (B) ALLUVIAL: Very loose, dark brown, SAND (A-2-4), contains moderate organics, moist
- (C) UCP: Very loose, gray, tan and orange, SAND (A-3, A-2-4), saturated
- (E) COASTAL PLAIN: Very soft, gray, SILT, CLAY and sandy CLAY (A-4, A-6, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to medium dense, gray, SAND (A-3) contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to medium stiff, gray, CLAY and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, gray and green, SAND and silty to clayey SAND (A-2-6, A-2-4, A-3), phosphatic, contains shell fragments (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)



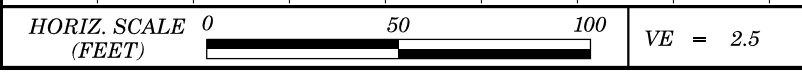
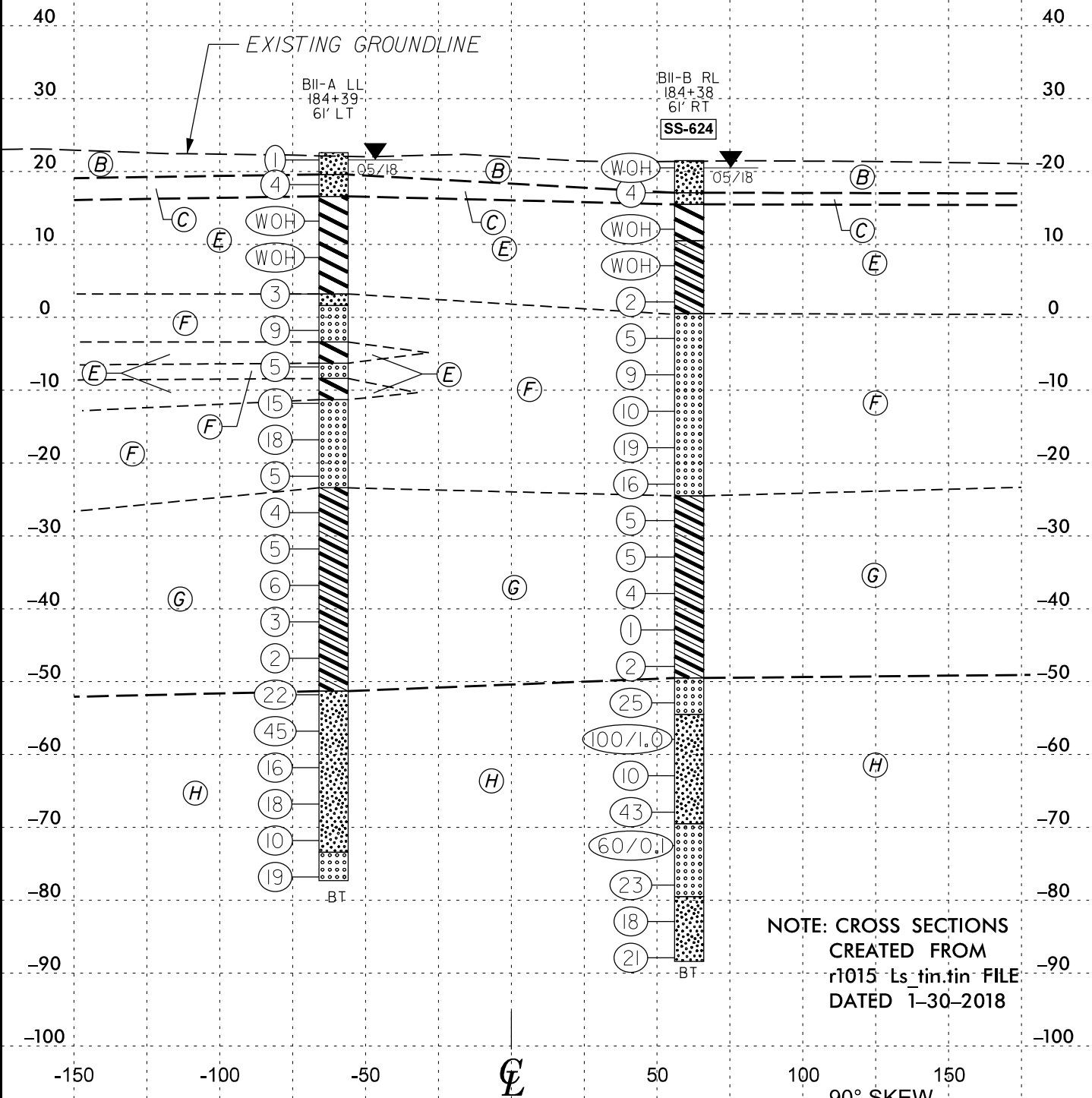
HORIZ. SCALE 0 50 100 (FEET) VE = 2.5

CROSS SECTION - BENT 9
-L- 181+72.25

- (B) ALLUVIAL: Very loose, brown, silty SAND (A-2-4), contains trace organics, moist
- (C) UCP: Loose, gray, SAND and silty SAND (A-3, A-2-4), saturated
- (E) COASTAL PLAIN: Very soft to medium stiff, gray, sandy and silty CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to dense, gray, SAND (A-3), with trace clay, contains shell fragments, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to stiff, gray, silty and sandy CLAY (A-6, A-7-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, gray and green, SAND (A-2-4, A-3, A-1-b), contains shell fragments, saturated (RIVER BEND FORMATION)
- (J) COASTAL PLAIN: Gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)

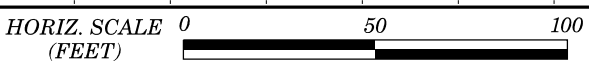
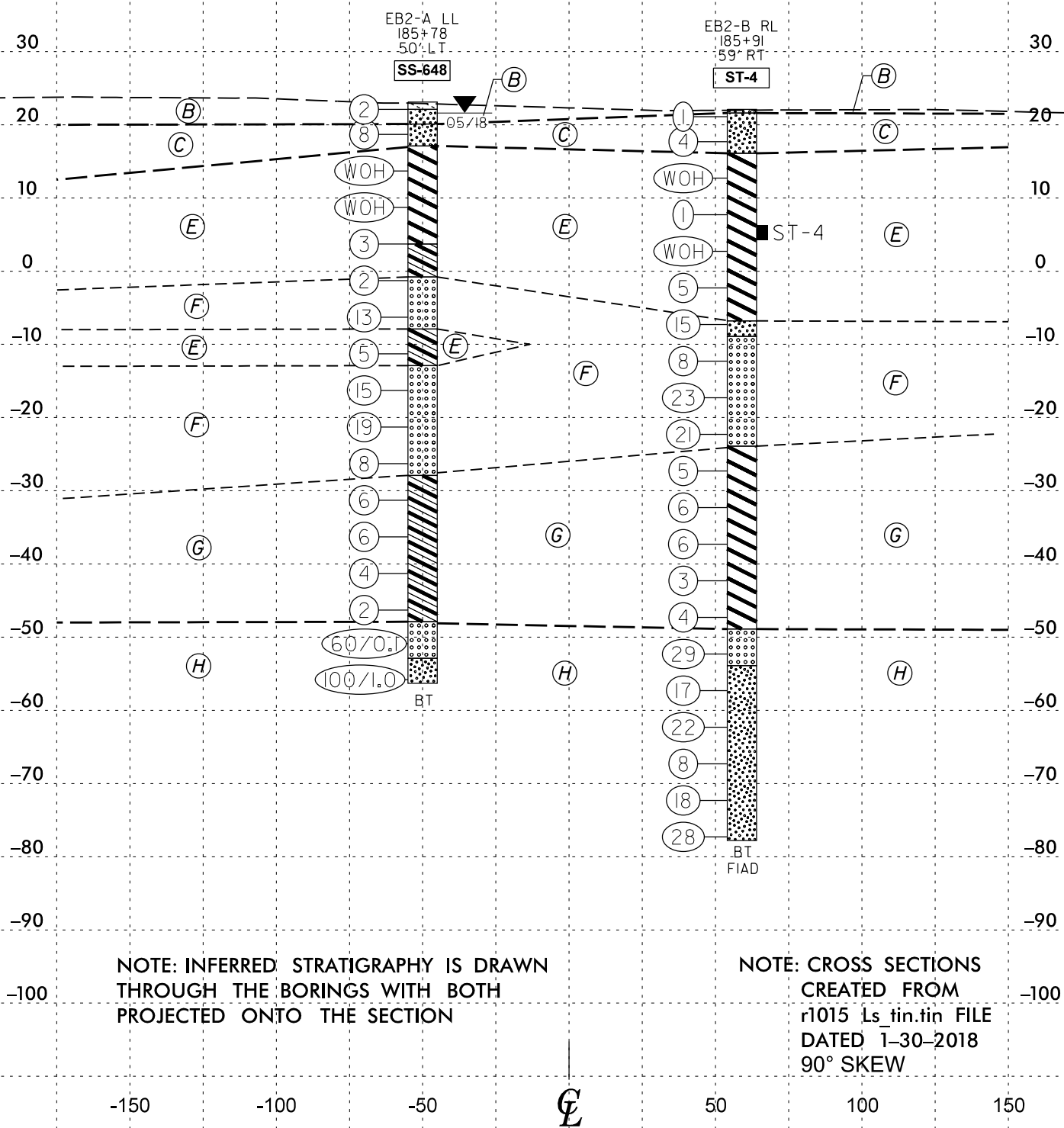


- (B) ALLUVIAL: Very loose, dark brown, silty and clayey SAND (A-2-4), moist
- (C) UCP: Loose, gray and orange, SAND (A-2-4), with trace clay, saturated
- (E) COASTAL PLAIN: Very soft to soft, gray, clay and sandy CLAY (A-6, A-7-6), wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Loose to medium dense, gray, SAND (A-2-4, A-3), with trace clay, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Very soft to medium stiff, sandy CLAY (A-6), contains shell fragments, wet (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Medium dense to very dense, gray, tan and green, SAND (A-2-4, A-3), contains shell and limestone fragments, saturated (RIVER BEND FORMATION)



NOTE: INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE SECTION

- (B) ALLUVIAL: Very loose, brown, silty SAND (A-2-4), some moderately organic, moist
- (C) UCP: Loose, gray, tan and orange, SAND (A-2-4), with silt and clay, some with trace organics, saturated
- (E) COASTAL PLAIN: Very soft to medium stiff, gray, CLAY (A-6, A-7-6), with fine sand and silt, wet (DUPLIN FORMATION)
- (F) COASTAL PLAIN: Very loose to medium dense, gray, SAND (A-2-4, A-3), with trace clay, saturated (DUPLIN FORMATION)
- (G) COASTAL PLAIN: Soft to medium stiff, gray, silty and sandy CLAY (A-6, A-7-6), contains shell fragments (DUPLIN FORMATION)
- (H) COASTAL PLAIN: Loose to very dense, green and gray, silty SAND (A-2-4, A-3), phosphatic, contains shell fragments, saturated (RIVER BEND FORMATION)



VE = 2.5

CROSS SECTION - END BENT 2
-L- STA 185+78.67

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B2-A LL		STATION 172+47		OFFSET 66 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 11.1 ft		TOTAL DEPTH 121.1 ft		NORTHING 409,882		EASTING 2,625,429	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/28/18		COMP. DATE 03/28/18		SURFACE WATER DEPTH 0.6ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
15															
10	11.1	0.0													
5	6.1	5.0	2	1	2										
0	1.5	9.6	2	2	6										
-5	-3.5	14.6	4	6	7										
-10	-8.5	19.6	3	4	4										
-15	-13.5	24.6	2	1	3										
-20	-18.5	29.6	3	5	5										
-25	-23.5	34.6	5	8	9										
-30	-28.5	39.6	4	5	5										
-35	-33.5	44.6	4	4	3										
-40	-38.5	49.6	5	7	12										
-45	-43.5	54.6	4	4	5										
-50	-48.5	59.6	4	5	8										
-55	-53.5	64.6	5	11	14										
-60	-58.5	69.6	12	19	25										
-65	-63.5	74.6	4	5	6										

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B2-A LL		STATION 172+47		OFFSET 66 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 11.1 ft		TOTAL DEPTH 121.1 ft		NORTHING 409,882		EASTING 2,625,429	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/28/18		COMP. DATE 03/28/18		SURFACE WATER DEPTH 0.6ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-65															
-70	-68.5	79.6	7	10	13										
-75	-73.5	84.6	4	6	8										
-80	-78.5	89.6	5	4	7										
-85	-83.5	94.6	4	4	6										
-90	-88.5	99.6	9	10	13										
-95	-93.5	104.6	18	28	19										
-100	-98.5	109.6	13	20	14										
-105	-103.5	114.6	9	9	10										
-110	-108.5	119.6	12	11	14										

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

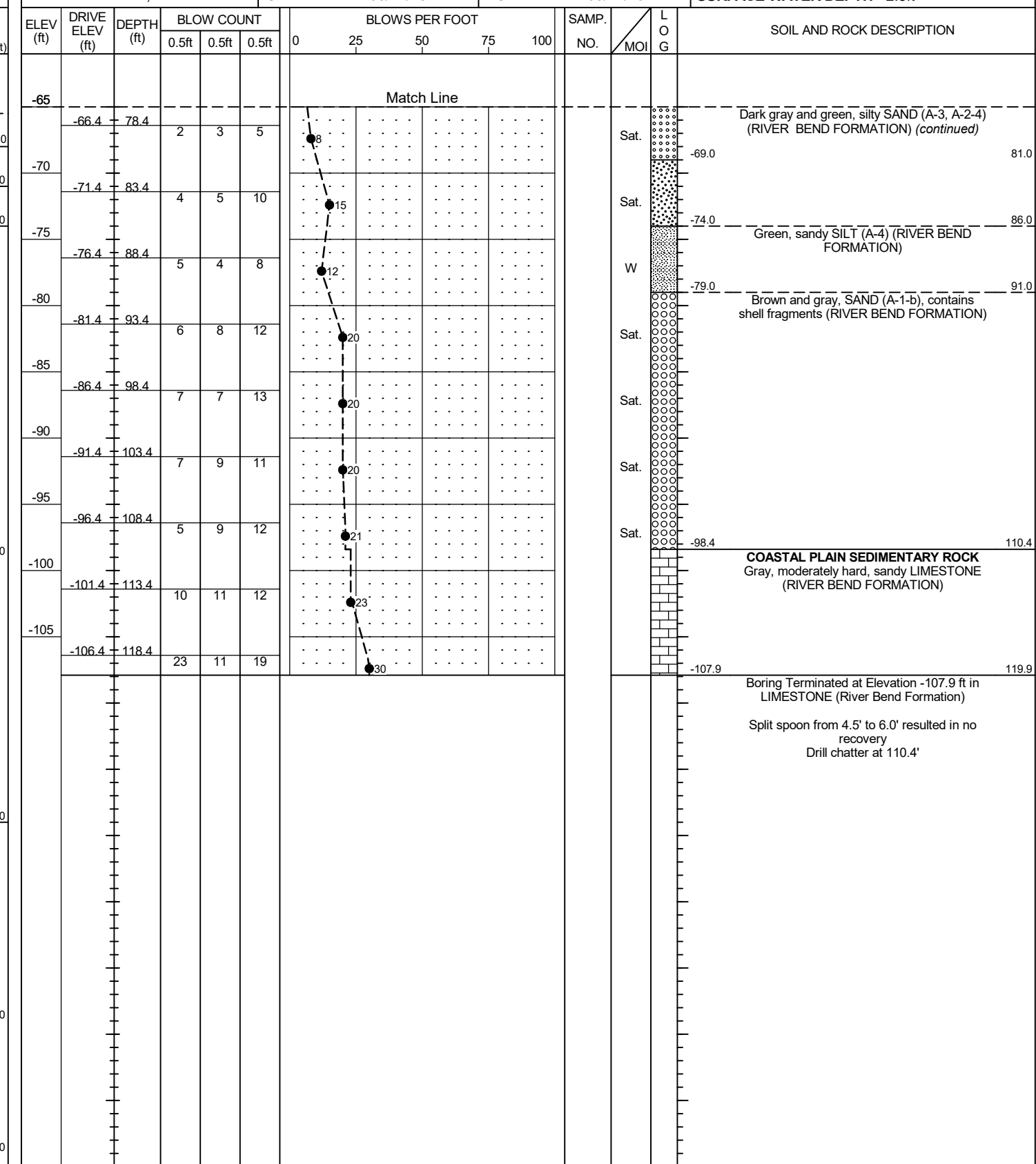
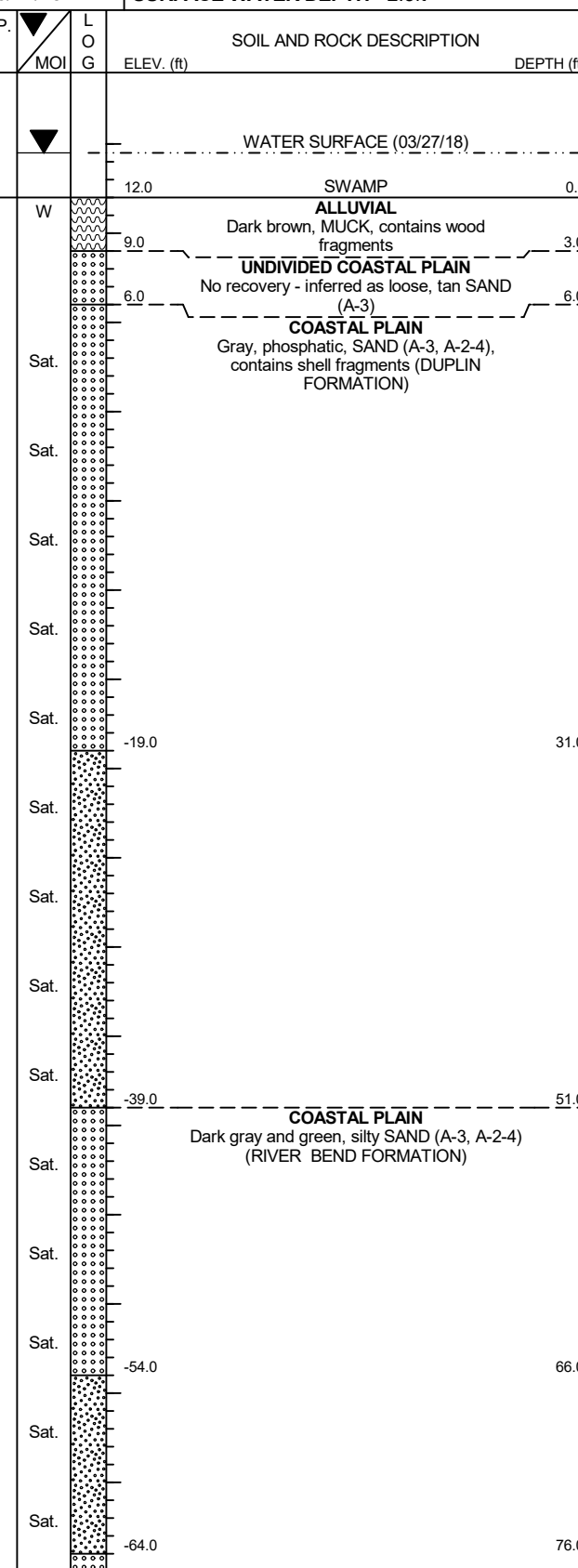
GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.										
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)									
BORING NO. B3-A LL		STATION 173+67		OFFSET 69 ft LT		ALIGNMENT -L-										
COLLAR ELEV. 12.0 ft		TOTAL DEPTH 119.9 ft		NORTHING 409,933		EASTING 2,625,320										
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER Fowler, B.		START DATE 03/27/18		COMP. DATE 03/27/18		SURFACE WATER DEPTH 2.5ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
15																
	12.0	0.0	2	7	4											
10	7.5	4.5	4	2	2											
5	3.6	8.4	WOH	2	4											
0	-1.4	13.4	5	5	5											
-5	-6.4	18.4	4	5	4											
-10	-11.4	23.4	6	5	7											
-15	-16.4	28.4	4	5	4											
-20	-21.4	33.4	3	3	3											
-25	-26.4	38.4	5	5	6											
-30	-31.4	43.4	5	4	4											
-35	-36.4	48.4	WOH	3	4											
-40	-41.4	53.4	3	5	8											
-45	-46.4	58.4	5	8	8											
-50	-51.4	63.4	5	7	8											
-55	-56.4	68.4	2	1	2											
-60	-61.4	73.4	2	2	3											
-65																

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.										
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)									
BORING NO. B3-A LL		STATION 173+67		OFFSET 69 ft LT		ALIGNMENT -L-										
COLLAR ELEV. 12.0 ft		TOTAL DEPTH 119.9 ft		NORTHING 409,933		EASTING 2,625,320										
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER Fowler, B.		START DATE 03/27/18		COMP. DATE 03/27/18		SURFACE WATER DEPTH 2.5ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
-65																
	-66.4	78.4	2	3	5											
-70	-71.4	83.4	4	5	10											
-75	-76.4	88.4	5	4	8											
-80	-81.4	93.4	6	8	12											
-85	-86.4	98.4	7	7	13											
-90	-91.4	103.4	7	9	11											
-95	-96.4	108.4	5	9	12											
-100	-101.4	113.4	10	11	12											
-105	-106.4	118.4	23	11	19											

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18



GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.										
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)									
BORING NO. B3-B RL		STATION 174+02		OFFSET 50 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 12.5 ft		TOTAL DEPTH 119.9 ft		NORTHING 410,056		EASTING 2,625,342										
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER Fowler, B.		START DATE 03/15/18		COMP. DATE 03/16/18		SURFACE WATER DEPTH 3.4ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
15																
	12.5	0.0														
10	8.2	4.3	2	2	1											
5	4.1	8.4	1	3	5											
0	-0.9	13.4	3	5	6											
-5	-5.9	18.4	5	4	4											
-10	-10.9	23.4	4	3	5											
-15	-15.9	28.4	4	5	6											
-20	-20.9	33.4	3	2	2											
-25	-25.9	38.4	3	3	3											
-30	-30.9	43.4	5	4	4											
-35	-35.9	48.4	2	2	4											
-40	-40.9	53.4	4	4	7											
-45	-45.9	58.4	5	6	9											
-50	-50.9	63.4	7	9	12											
-55	-55.9	68.4	2	1	2											
-60	-60.9	73.4	2	1	3											
-65																

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.											
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)										
BORING NO. B3-B RL		STATION 174+02		OFFSET 50 ft RT		ALIGNMENT -L-											
COLLAR ELEV. 12.5 ft		TOTAL DEPTH 119.9 ft		NORTHING 410,056		EASTING 2,625,342											
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic											
DRILLER Fowler, B.		START DATE 03/15/18		COMP. DATE 03/16/18		SURFACE WATER DEPTH 3.4ft											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	L O G	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
-65	-65.9	78.4	11	10	11												
-70	-70.9	83.4	4	4	6												
-75	-75.9	88.4	4	4	6												
-80	-80.9	93.4	6	3	6												
-85	-85.9	98.4	10	9	8												
-90	-90.9	103.4	10	12	9												
-95	-95.9	108.4	4	8	23												
	-97.4	109.9	10	11	8												
-100	-100.9	113.4	7	11	9												
-105	-105.9	118.4	6	17	31												

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

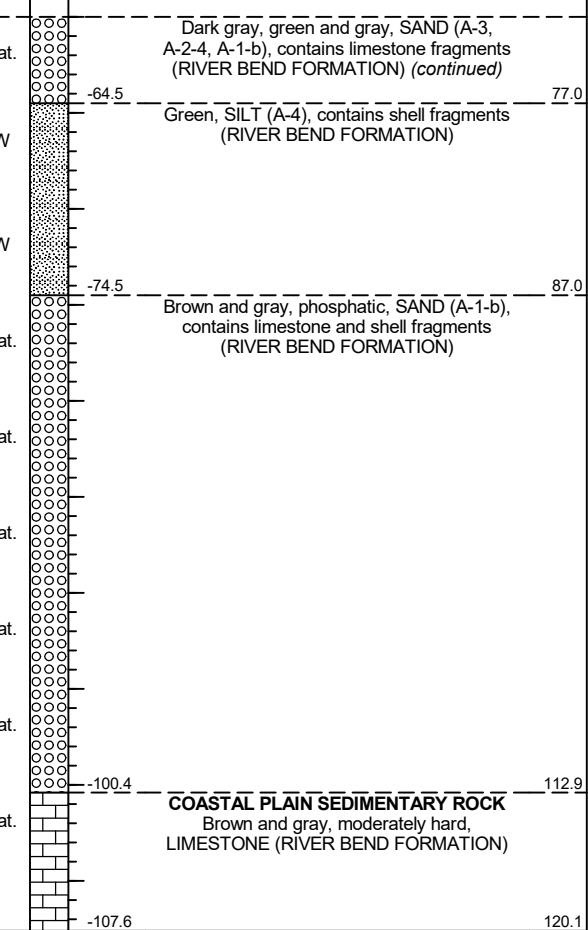
GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B4-A LL		STATION 174+87		OFFSET 72 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 12.5 ft		TOTAL DEPTH 120.1 ft		NORTHING 409,984		EASTING 2,625,211									
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 03/23/18		COMP. DATE 03/26/18		SURFACE WATER DEPTH 1.2ft									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
20															
15															
12.5	12.5	0.0													
10	8.3	4.2	6	4	2										
5	3.9	8.6	1	2	3										
0	-1.1	13.6	3	3	4										
-5	-6.1	18.6	3	4	5										
-10	-11.1	23.6	4	5	5										
-15	-16.1	28.6	5	4	4										
-20	-21.1	33.6	2	1	2										
-25	-26.1	38.6	4	2	2										
-30	-31.1	43.6	4	3	3										
-35	-36.1	48.6	3	4	5										
-40	-41.1	53.6	4	5	7										
-45	-46.1	58.6	8	7	4										
-50	-51.1	63.6	1	2	2										
-55	-56.1	68.6	4	4	3										
-60															

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B4-A LL		STATION 174+87		OFFSET 72 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 12.5 ft		TOTAL DEPTH 120.1 ft		NORTHING 409,984		EASTING 2,625,211									
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 03/23/18		COMP. DATE 03/26/18		SURFACE WATER DEPTH 1.2ft									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-60															
-65	-61.1	73.6	5	3	5										
-70	-66.1	78.6	85	100	0.1										
-75	-71.1	83.6	5	5	9										
-80	-76.1	88.6	7	5	7										
-85	-81.1	93.6	6	7	9										
-90	-86.1	98.6	7	7	8										
-95	-91.1	103.6	8	8	17										
-100	-96.1	108.6	12	11	15										
-105	-101.1	113.6	13	15	22										
-107.6	-106.1	118.6	13	14	19										

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18



GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B4-B RL		STATION 174+86		OFFSET 87 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 12.6 ft		TOTAL DEPTH 119.5 ft		NORTHING 410,126		EASTING 2,625,283	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/19/18		COMP. DATE 03/20/18		SURFACE WATER DEPTH 1.5ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
15															
	12.6	0.0													
			WOH	WOH	WOH										
10	8.5	4.1	1	4	5										
5	4.6	8.0	1	3	4										
0	-0.4	13.0	3	6	6										
-5	-5.4	18.0	3	4	5										
-10	-10.4	23.0	4	4	4										
-15	-15.4	28.0	2	2	4										
-20	-20.4	33.0	2	1	3										
-25	-25.4	38.0	WOH	1	2										
-30	-30.4	43.0	1	2	3										
-35	-35.4	48.0	2	4	5										
-40	-40.4	53.0	5	6	8										
-45	-45.4	58.0	5	6	10										
-50	-50.4	63.0	1	1	1										
-55	-55.4	68.0	2	1	2										
-60	-60.4	73.0	4	4	5										
-65															

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B4-B RL		STATION 174+86		OFFSET 87 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 12.6 ft		TOTAL DEPTH 119.5 ft		NORTHING 410,126		EASTING 2,625,283	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/19/18		COMP. DATE 03/20/18		SURFACE WATER DEPTH 1.5ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-65	-65.4	78.0	6	3	5										
-70	-70.4	83.0	4	4	8										
-75	-75.4	88.0	6	7	9										
-80	-80.4	93.0	5	7	9										
-85	-85.4	98.0	9	7	9										
-90	-90.4	103.0	7	6	8										
-95	-95.4	108.0	10	8	16										
-100	-100.4	113.0	11	9	13										
-105	-105.4	118.0	21	18	21										

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B5-A LL		STATION 176+29		OFFSET 55 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 14.6 ft		TOTAL DEPTH 120.0 ft		NORTHING 410,063		EASTING 2,625,092	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/21/18		COMP. DATE 03/22/18		SURFACE WATER DEPTH 0.2ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
20															
15	14.6	0.0													
10	10.3	4.3	6	6	4										
5	6.1	8.5	4	4	5										
0	1.1	13.5	2	4	6										
-5	-3.9	18.5	4	6	6										
-10	-8.9	23.5	4	7	8										
-15	-13.9	28.5	5	4	6										
-20	-18.9	33.5	4	6	4										
-25	-23.9	38.5	1	3	2										
-30	-28.9	43.5	3	2	3										
-35	-33.9	48.5	3	4	4										
-40	-38.9	53.5	5	5	5										
-45	-43.9	58.5	2	3	4										
-50	-48.9	63.5	2	1	0										
-55	-53.9	68.5	3	2	5										
-60	-58.9	73.5	3	4	7										

WATER SURFACE (03/21/18) 0.0
 14.6
 13.6
 11.6
 7.6
 0.0
 1.0
 3.0
 7.0
 37.0
 42.0
 47.0
 62.0

ALLUVIAL Dark brown, MUCK
 UNDIVIDED COASTAL PLAIN Green, SILT (A-4)
 Tan, SAND (A-3), contains wood fragments
 COASTAL PLAIN Gray, phosphatic, SAND (A-3), contains shell fragments (DUPLIN FORMATION)
 Gray and green, silty, sandy CLAY (A-7-6, A-6) (DUPLIN FORMATION)
 COASTAL PLAIN Dark gray, green and gray, SAND (A-3, A-2-4), contains shell fragments (RIVER BEND FORMATION)

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.	
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B5-A LL		STATION 176+29		OFFSET 55 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 14.6 ft		TOTAL DEPTH 120.0 ft		NORTHING 410,063		EASTING 2,625,092	
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Fowler, B.		START DATE 03/21/18		COMP. DATE 03/22/18		SURFACE WATER DEPTH 0.2ft	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-60															
-65	-63.9	78.5	4	14	40										
-70	-68.9	83.5	6	5	21										
-75	-73.9	88.5	10	5	8										
-80	-78.9	93.5	6	8	9										
-85	-83.9	98.5	6	9	11										
-90	-88.9	103.5	6	5	15										
-95	-93.9	108.5	8	8	10										
-100	-98.9	113.5	10	8	14										
-105	-103.9	118.5	24	17	21										

Match Line
 -62.4
 -64.5
 -70.1
 -72.4
 -82.4
 -84.9
 -87.4
 -97.4
 -102.4
 -105.4

Dark gray, green and gray, SAND (A-3, A-2-4), contains shell fragments (RIVER BEND FORMATION) (continued)
 Gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)
 Green, fine SAND (A-2-4) (RIVER BEND FORMATION)
 COASTAL PLAIN SEDIMENTARY ROCK Gray and brown, moderately hard, sandy LIMESTONE (RIVER BEND FORMATION)
 COASTAL PLAIN Brown and gray, SAND (A-2-4, A-1-b, A-3), contains shell fragments (RIVER BEND FORMATION)
 COASTAL PLAIN SEDIMENTARY ROCK Gray, moderately hard, LIMESTONE (RIVER BEND FORMATION)
 COASTAL PLAIN Gray, SAND (A-1-b), contains shell fragments (RIVER BEND FORMATION)
 Boring Terminated at Elevation -105.4 ft in SAND (River Bend Formation)
 Strata break in split spoon at depths 79.1' and 84.7'

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.										
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)									
BORING NO. B5-B RL		STATION 176+04		OFFSET 61 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 13.1 ft		TOTAL DEPTH 118.9 ft		NORTHING 410,156		EASTING 2,625,166										
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER Fowler, B.		START DATE 03/20/18		COMP. DATE 03/21/18		SURFACE WATER DEPTH 0.5ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
15	13.1	0.0	WOH	WOH	WOH											
10	9.6	3.5	6	6	4											
5	5.7	7.4	3	3	5											
0	0.7	12.4	4	6	8											
-5	-4.3	17.4	6	7	8											
-10	-9.3	22.4	3	5	4											
-15	-14.3	27.4	7	6	7											
-20	-19.3	32.4	5	7	6											
-25	-24.3	37.4	WOH	2	3											
-30	-29.3	42.4	2	2	4											
-35	-34.3	47.4	3	5	7											
-40	-39.3	52.4	5	8	11											
-45	-44.3	57.4	4	5	5											
-50	-49.3	62.4	3	2	2											
-55	-54.3	67.4	1	2	2											
-60	-59.3	72.4	1	4	8											
-65	-64.3	77.4														

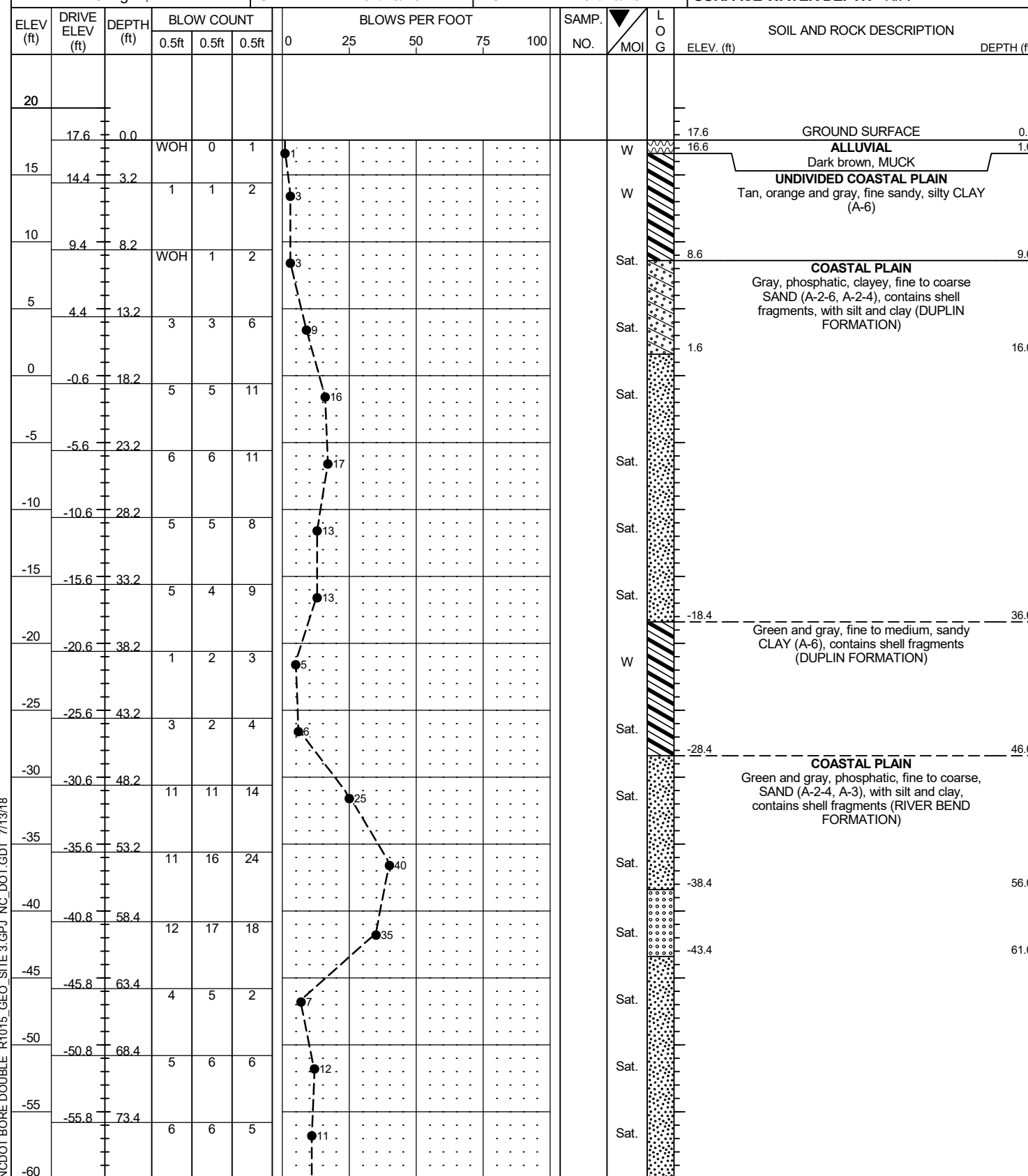
WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J. K.										
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)									
BORING NO. B5-B RL		STATION 176+04		OFFSET 61 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 13.1 ft		TOTAL DEPTH 118.9 ft		NORTHING 410,156		EASTING 2,625,166										
DRILL RIG/HAMMER EFF./DATE MD0314 D-25 84% 09/05/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER Fowler, B.		START DATE 03/20/18		COMP. DATE 03/21/18		SURFACE WATER DEPTH 0.5ft										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
-65			8	6	5											
-70	-69.3	82.4	5	7	9											
-75	-74.3	87.4	4	5	6											
-80	-79.3	92.4	8	10	13											
-85	-84.3	97.4	8	10	10											
-90	-89.3	102.4	4	6	12											
-95	-94.3	107.4	8	8	9											
-100	-99.3	112.4	7	11	11											
-105	-104.3	117.4	22	23	26											

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

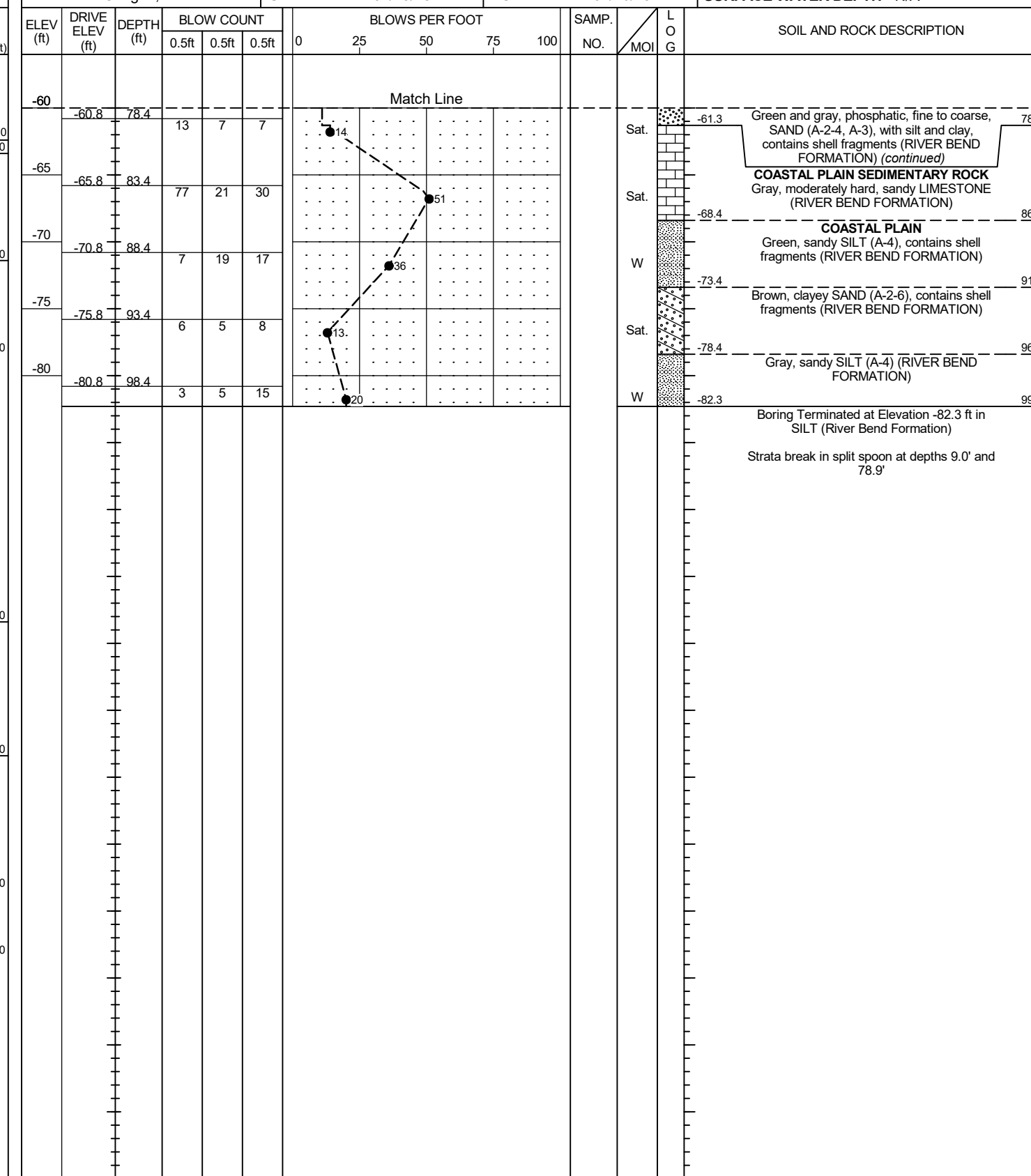
GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1	TIP R-1015	COUNTY CRAVEN	GEOLOGIST Johnson, M.
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)			GROUND WTR (ft)
BORING NO. B6-A LL	STATION 177+76	OFFSET 59 ft LT	ALIGNMENT -L-
COLLAR ELEV. 17.6 ft	TOTAL DEPTH 99.9 ft	NORTHING 410,125	EASTING 2,624,959
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Coogan, M.	START DATE 04/13/18	COMP. DATE 04/16/18	SURFACE WATER DEPTH N/A



WBS 34360.1.1	TIP R-1015	COUNTY CRAVEN	GEOLOGIST Johnson, M.
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)			GROUND WTR (ft)
BORING NO. B6-A LL	STATION 177+76	OFFSET 59 ft LT	ALIGNMENT -L-
COLLAR ELEV. 17.6 ft	TOTAL DEPTH 99.9 ft	NORTHING 410,125	EASTING 2,624,959
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Coogan, M.	START DATE 04/13/18	COMP. DATE 04/16/18	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

GEOTECHNICAL BORING REPORT

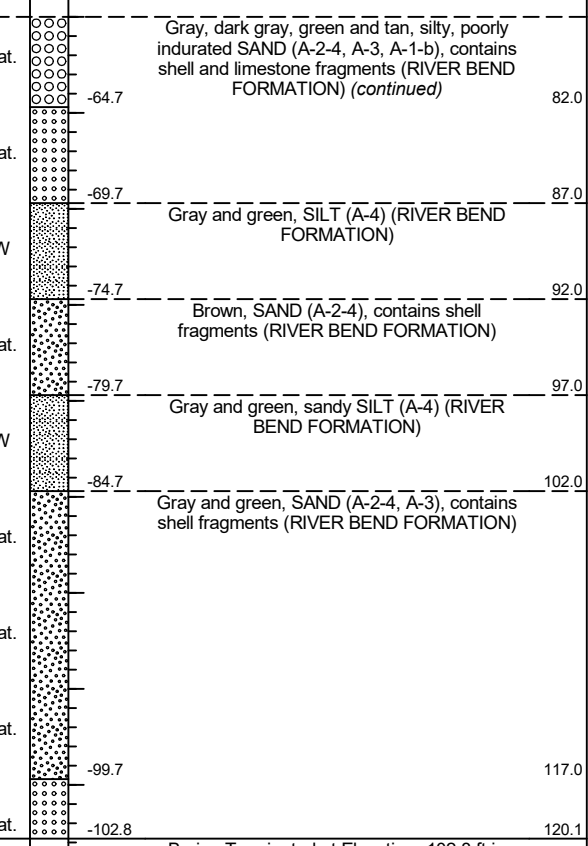
BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J.K.											
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)										
BORING NO. B6-B RL		STATION 177+86		OFFSET 44 ft RT		ALIGNMENT -L-											
COLLAR ELEV. 17.3 ft		TOTAL DEPTH 120.1 ft		NORTHING 410,221		EASTING 2,624,996											
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic											
DRILLER Wiggins, M.		START DATE 04/17/18		COMP. DATE 04/17/18		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
20	17.3	0.0													17.3	GROUND SURFACE	0.0
15	13.7	3.6	2	2											14.3	ALLUVIAL Dark brown, SAND (A-2-4)	3.0
10	8.7	8.6	4	1	3										10.3	UNDIVIDED COASTAL PLAIN Gray and tan, SAND (A-2-4)	7.0
5	3.7	13.6	2	2	5											COASTAL PLAIN Gray, phosphatic, SAND (A-3), with trace clay in part, contains shell fragments (DUPLIN FORMATION)	
0	-1.3	18.6	5	4	7												
-5	-6.3	23.6	4	5	6												
-10	-11.3	28.6	6	7	8												
-15	-16.3	33.6	3	2	7												
-20	-21.3	38.6	5	5	4												
-25	-26.3	43.6	4	5	6												
-30	-31.3	48.6	2	2	3												
-35	-36.3	53.6	11	19	20												
-40	-41.3	58.6	3	3	4												
-45	-46.3	63.6	15	20	28												
-50	-51.3	68.6	11	10	11												
-55	-56.3	73.6	5	8	9												
-60			16	8	6												
			5	4	5												

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J.K.											
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)										
BORING NO. B6-B RL		STATION 177+86		OFFSET 44 ft RT		ALIGNMENT -L-											
COLLAR ELEV. 17.3 ft		TOTAL DEPTH 120.1 ft		NORTHING 410,221		EASTING 2,624,996											
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic											
DRILLER Wiggins, M.		START DATE 04/17/18		COMP. DATE 04/17/18		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
-60	-61.3	78.6	10	8	8												
-65	-66.3	83.6	13	13	9												
-70	-71.3	88.6	8	10	12												
-75	-76.3	93.6	7	7	9												
-80	-81.3	98.6	4	4	6												
-85	-86.3	103.6	10	10	9												
-90	-91.3	108.6	13	11	11												
-95	-96.3	113.6	9	14	14												
-100	-101.3	118.6	30	53	31												

Match Line



GEOTECHNICAL BORING REPORT

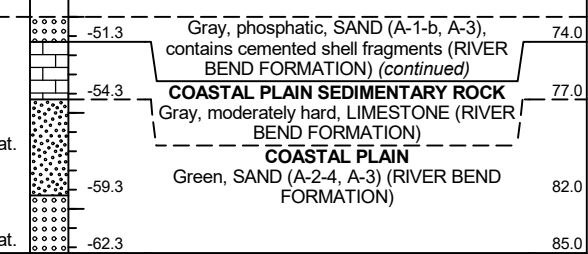
BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J.K.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B7-A LL		STATION 179+01		OFFSET 47 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 22.7 ft		TOTAL DEPTH 85.0 ft		NORTHING 410,192		EASTING 2,624,852									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Wiggins, M.		START DATE 04/18/18		COMP. DATE 04/18/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
30															
25	22.7	0.0													
20	19.2	3.5	WOH	1	1										
15	14.2	8.5	WOH	2	1										
10	9.2	13.5	WOH	WOH	WOH										
5	4.2	18.5	WOH	WOH	WOH										
0	-0.8	23.5		5	7	11									
-5	-5.8	28.5		5	6	7									
-10	-10.8	33.5		7	7	9									
-15	-15.8	38.5		5	8	5									
-20	-20.8	43.5		9	11	15									
-25	-25.8	48.5		2	1	2									
-30	-30.8	53.5		2	3	9									
-35	-35.8	58.5		2	3	9									
-40	-40.8	63.5		11	12	12									
-45	-45.8	68.5		13	16	25									
-50				9	10	14									
				10	12	16									

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Crenshaw, J.K.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B7-A LL		STATION 179+01		OFFSET 47 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 22.7 ft		TOTAL DEPTH 85.0 ft		NORTHING 410,192		EASTING 2,624,852									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Wiggins, M.		START DATE 04/18/18		COMP. DATE 04/18/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-50	-50.8	73.5													
-55	-55.8	78.5													
-60	-60.8	83.5													

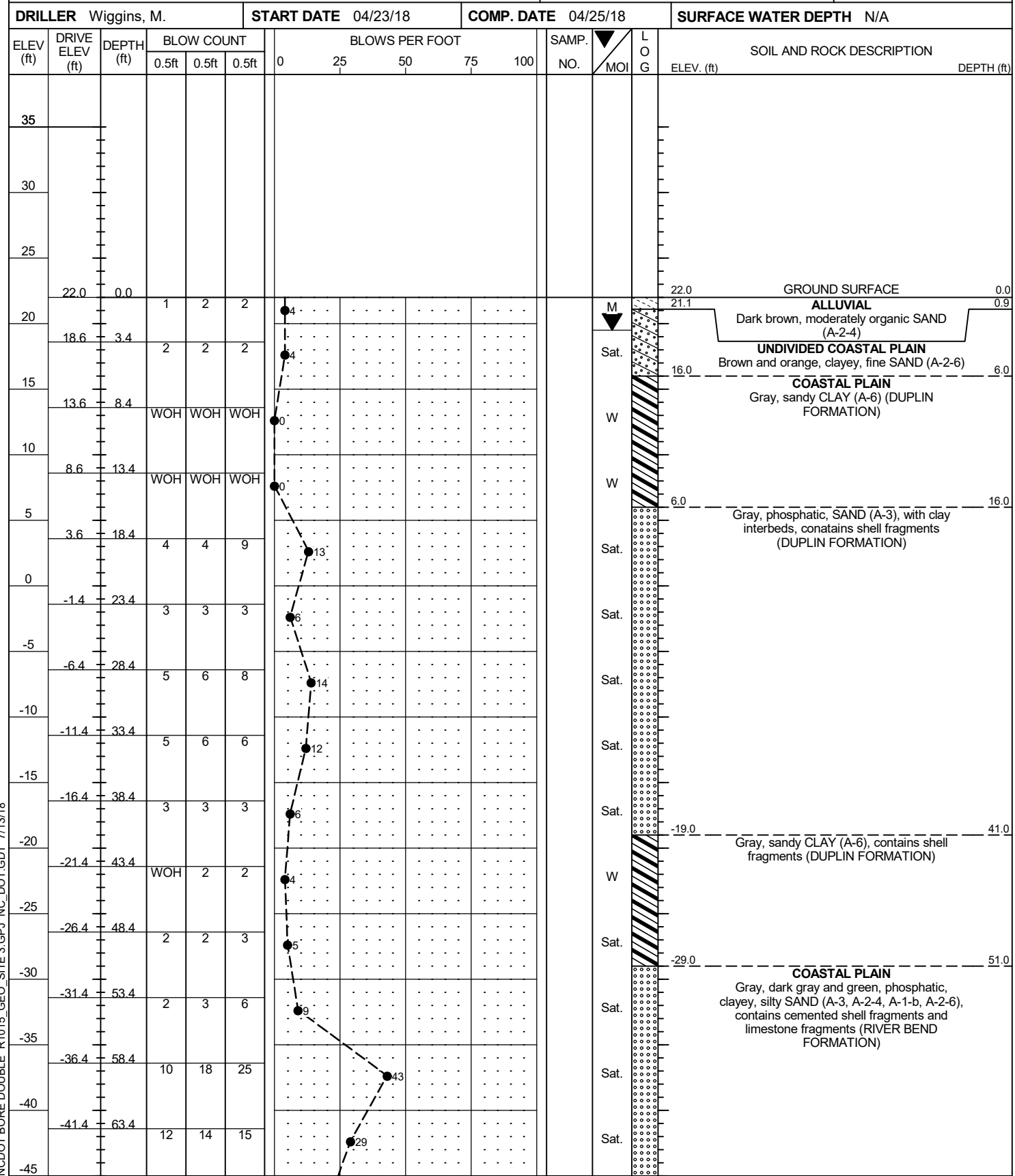
Match Line



GEOTECHNICAL BORING REPORT

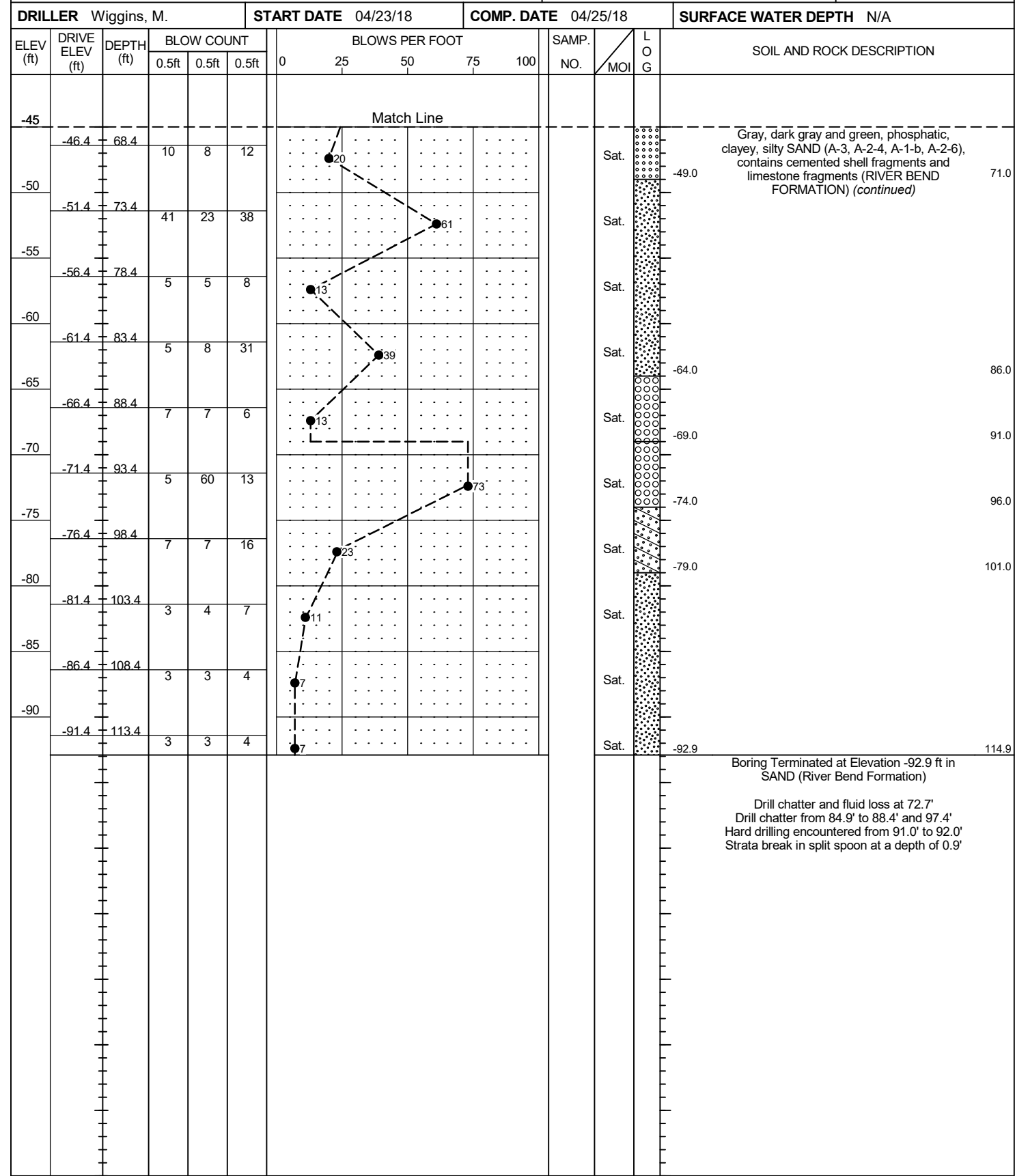
BORE LOG

WBS 34360.1.1	TIP R-1015	COUNTY CRAVEN	GEOLOGIST Grainger, P.
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)			GROUND WTR (ft)
BORING NO. B8-B RL	STATION 180+39	OFFSET 52 ft RT	ALIGNMENT -L-
COLLAR ELEV. 22.0 ft	TOTAL DEPTH 114.9 ft	NORTHING 410,248	EASTING 2,624,726
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Wiggins, M.	START DATE 04/23/18	COMP. DATE 04/25/18	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

WBS 34360.1.1	TIP R-1015	COUNTY CRAVEN	GEOLOGIST Grainger, P.
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)			GROUND WTR (ft)
BORING NO. B8-B RL	STATION 180+39	OFFSET 52 ft RT	ALIGNMENT -L-
COLLAR ELEV. 22.0 ft	TOTAL DEPTH 114.9 ft	NORTHING 410,248	EASTING 2,624,726
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER Wiggins, M.	START DATE 04/23/18	COMP. DATE 04/25/18	SURFACE WATER DEPTH N/A



GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B9-B RL		STATION 181+72		OFFSET 62 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 21.5 ft		TOTAL DEPTH 104.9 ft		NORTHING 410,410		EASTING 2,624,658	
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Wiggins, M.		START DATE 04/25/18		COMP. DATE 04/26/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
25															
	21.5	0.0	1	1	1										21.5
20	18.1	3.4	1	1	0										20.5
15	13.1	8.4	WOH	WOH	WOH										15.5
10	8.1	13.4	WOH	WOH	WOH										10.0
5	3.1	18.4	5	5	8										5.5
0	-1.9	23.4	3	4	4										0.0
-5	-6.9	28.4	1	0	1										-4.5
-10	-11.9	33.4	5	5	5										-9.5
-15	-16.9	38.4	4	2	2										-19.5
-20	-21.9	43.4	2	2	1										-29.5
-25	-26.9	48.4	1	1	1										-34.5
-30	-31.9	53.4	3	2	2										-49.5
-35	-36.9	58.4	11	14	18										-71.0
-40	-41.9	63.4	9	12	12										
-45	-46.9	68.4	8	15	18										
-50	-51.9	73.4	14	13	20										
-55															

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B9-B RL		STATION 181+72		OFFSET 62 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 21.5 ft		TOTAL DEPTH 104.9 ft		NORTHING 410,410		EASTING 2,624,658	
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Wiggins, M.		START DATE 04/25/18		COMP. DATE 04/26/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-55															
	-56.9	78.4	17	17	17										
-60	-61.9	83.4	5	4	11										
-65	-66.9	88.4	8	5	11										
-70	-71.9	93.4	23	7	12										
-75	-76.9	98.4	10	8	8										
-80	-81.9	103.4	5	5	4										

NCDOT BORE DOUBLE R1015_SITE 3.GPJ_NC_DOT_GDT 7/13/18

SS-511 42%

Match Line

Gray and green, phosphatic, silty SAND (A-1-b, A-3, A-2-4, A-2-6), contains shell and limestone fragments (RIVER BEND FORMATION) (continued)

Boring Terminated at Elevation -83.4 ft in SAND (River Bend Formation)

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B10-A LL		STATION 183+05		OFFSET 56 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 22.1 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,364		EASTING 2,624,487									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 04/27/18		COMP. DATE 04/30/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
30															
25															
20	22.1	0.0	WOH	1	0										
15	18.7	3.4		2	4										
10	13.7	8.4	WOH	WOH	WOH										
5	8.7	13.4	WOH	WOH	WOH										
0	3.7	18.4	WOH	1	2										
-5	-1.3	23.4		2	2										
-10	-6.3	28.4		2	3										
-15	-11.3	33.4		8	10										
-20	-16.3	38.4		10	18										
-25	-21.3	43.4		10	8										
-30	-26.3	48.4		2	2										
-35	-31.3	53.4		2	3										
-40	-36.3	58.4		1	3										
-45	-41.3	63.4		2	2										
-50	-46.3	68.4	WOH	WOH	2										

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 276 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B10-A LL		STATION 183+05		OFFSET 56 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 22.1 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,364		EASTING 2,624,487									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 04/27/18		COMP. DATE 04/30/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-50															
-55															
-60															
-65															
-70															
-75															

Match Line

Gray, silty, sandy CLAY (A-7-6, A-6), contains shell fragments (DUPLIN FORMATION) (continued) 74.4

COASTAL PLAIN 76.7

Gray and green, silty, medium to coarse SAND (A-3, A-1-b, A-2-4), contains cemented shell fragments (RIVER BEND FORMATION) 78.9

81.0

84.4

91.0

96.0

99.9

Boring Terminated at Elevation -77.8 ft in SAND (River Bend Formation)

Loss of circulation at 83.0' and from 84.9' to 88.4'

Caving at 30.0'

Drill chatter at 74.3' and from 89.9' to 90.9'

Strata break in split spoon at depths 19.4', 74.4', 78.9' and 84.4'

NCDOT BORE DOUBLE R1015_SITE 3.GPJ NC_DOT_GDT 7/13/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B10-B RL		STATION 183+09		OFFSET 59 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 21.2 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,468		EASTING 2,624,534	
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Powell, B.		START DATE 05/01/18		COMP. DATE 05/01/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
25															
	21.2	0.0													21.2
20	17.8	3.4	WOH	WOH	1										18.2
15	12.8	8.4													15.2
10	7.8	13.4	WOH	WOH	WOH										10.2
5	2.8	18.4	WOH	1	1										1.6
0	-2.2	23.4	1	1	1										-1.6
-5	-7.2	28.4	3	5	6										-9.8
-10	-12.2	33.4	WOH	3	5										-12.7
-15	-17.2	38.4	4	5	6										-22.5
-20	-22.2	43.4	1	1	2										-48.2
-25	-27.2	48.4	2	2	2										-54.8
-30	-32.2	53.4	2	2	3										
-35	-37.2	58.4	1	2	3										
-40	-42.2	63.4	2	1	2										
-45	-47.2	68.4	2	8	29										
-50	-52.2	73.4	11	12	15										

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.	
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)
BORING NO. B10-B RL		STATION 183+09		OFFSET 59 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 21.2 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,468		EASTING 2,624,534	
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER Powell, B.		START DATE 05/01/18		COMP. DATE 05/01/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-55															
	-57.2	78.4	8	9	15										
-60	-62.2	83.4	4	3	5										
-65	-67.2	88.4	20	54	46/0.5										
-70	-72.2	93.4	65	35/0.5											
-75	-77.2	98.4	15	14	9										

Match Line

Gray and green, SAND (A-3, A-2-4), contains cemented shell and limestone fragments (RIVER BEND FORMATION) (continued)

COASTAL PLAIN SEDIMENTARY ROCK
Gray, moderately hard, sandy LIMESTONE (RIVER BEND FORMATION)

COASTAL PLAIN
Gray, SAND (A-3), with limestone fragments (RIVER BEND FORMATION)

Boring Terminated at Elevation -78.7 ft in SAND (River Bend Formation)

Drill chatter from 72.6' to 75.1', 87.7' to 88.4' and 93.9' to 94.6'
Strata breaks in split spoon at depths 19.6', 33.9', 43.7' and 69.4'

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B11-B RL		STATION 184+38		OFFSET 61 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 21.5 ft		TOTAL DEPTH 109.9 ft		NORTHING 410,528		EASTING 2,624,420									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 05/02/18		COMP. DATE 05/02/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
35															
30															
25															
20	21.5	0.0	WOH	WOH	WOH										
15	18.1	3.4	WOH	2	2										
10	13.1	8.4	WOH	WOH	WOH										
5	8.1	13.4	WOH	WOH	WOH										
0	3.1	18.4	1	1	1										
-5	-1.9	23.4	4	3	2										
-10	-6.9	28.4	2	2	7										
-15	-11.9	33.4	3	5	5										
-20	-16.9	38.4	7	9	10										
-25	-21.9	43.4	5	7	9										
-30	-26.9	48.4	2	2	3										
-35	-31.9	53.4	2	2	3										
-40	-36.9	58.4	2	1	3										
-45	-41.9	63.4	WOH	WOH	1										

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. B11-B RL		STATION 184+38		OFFSET 61 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 21.5 ft		TOTAL DEPTH 109.9 ft		NORTHING 410,528		EASTING 2,624,420									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 05/02/18		COMP. DATE 05/02/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-45															
-50	-46.9	68.4	WOH	WOH	2										
-55	-51.9	73.4	10	11	14										
-60	-56.9	78.4	62	38/0.5											
-65	-61.9	83.4	9	5	5										
-70	-66.9	88.4	5	7	36										
-75	-71.9	93.4	19	60/0.1											
-80	-76.9	98.4	11	10	13										
-85	-81.9	103.4	6	7	11										
	-86.9	108.4	6	8	13										

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18

SS-624 42% W

Match Line

Gray, fine sandy CLAY (A-6)(8), with little silt and trace sand, contains shell fragments (DUPLIN FORMATION) (continued)

COASTAL PLAIN
Gray and green, cemented, silty, fine to medium SAND (A-3, A-2-4), contains shell and limestone fragments (RIVER BEND FORMATION)

Boring Terminated at Elevation -88.4 ft in SAND (River Bend Formation)

Drill chatter from 75.3' to 76.3', 90.3' to 90.8' and 94.0' to 94.5'
Strata break in split spoon and a depth of 4.4'

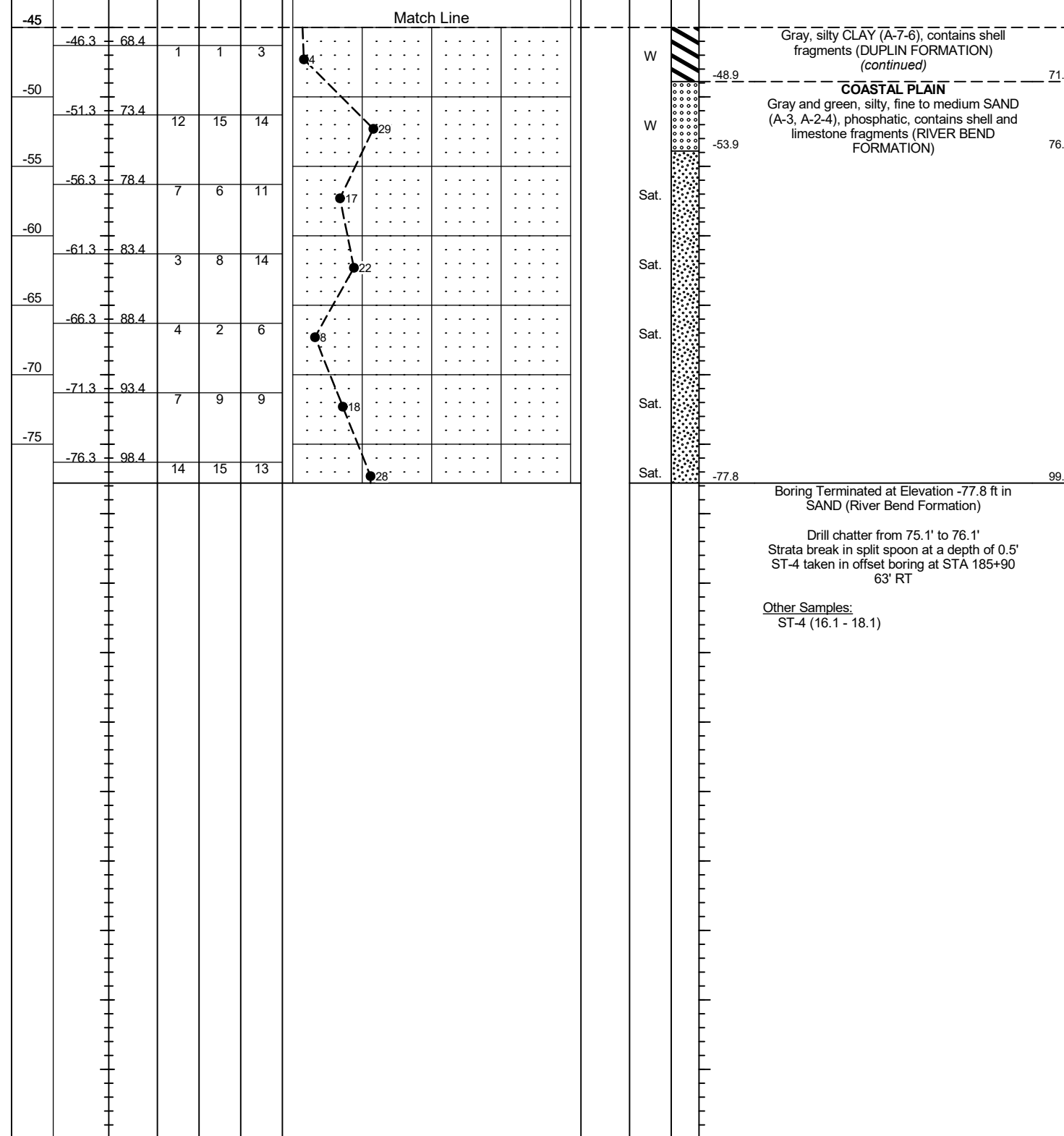
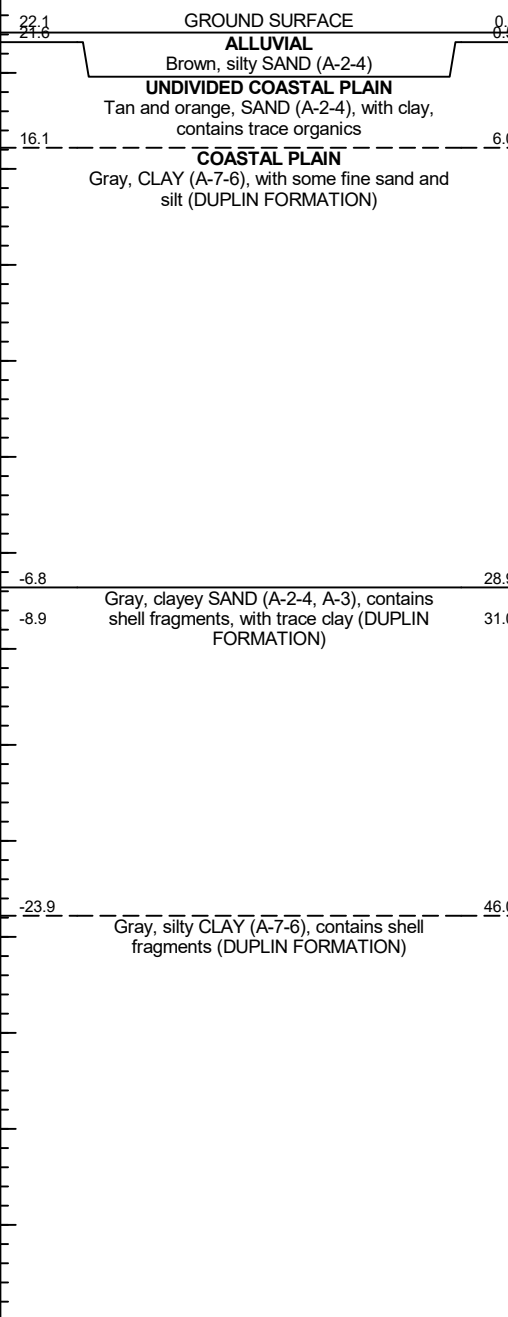
GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. EB2-B RL		STATION 185+91		OFFSET 59 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 22.1 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,594		EASTING 2,624,282									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 05/03/18		COMP. DATE 05/04/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
35															
30															
25															
20	22.1	0.0	WOH	WOH	1										
15	18.7	3.4	WOH	1	3										
10	13.7	8.4	WOH	WOH	WOH										
5	8.7	13.4	WOH	WOH	1										
0	3.7	18.4	WOH	WOH	WOH										
-5	-1.3	23.4	1	2	3										
-10	-6.3	28.4	5	7	8										
-15	-11.3	33.4	3	3	5										
-20	-16.3	38.4	8	11	12										
-25	-21.3	43.4	10	10	11										
-30	-26.3	48.4	1	2	3										
-35	-31.3	53.4	2	3	3										
-40	-36.3	58.4	2	3	3										
-45	-41.3	63.4	1	1	2										

WBS 34360.1.1		TIP R-1015		COUNTY CRAVEN		GEOLOGIST Grainger, P.									
SITE DESCRIPTION Site 3 - Bridge No. 277 Over East Prong of Slocum Creek on US 70 (Havelock Bypass)							GROUND WTR (ft)								
BORING NO. EB2-B RL		STATION 185+91		OFFSET 59 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 22.1 ft		TOTAL DEPTH 99.9 ft		NORTHING 410,594		EASTING 2,624,282									
DRILL RIG/HAMMER EFF./DATE MD1904 CME-45B 78% 08/20/2017			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER Powell, B.		START DATE 05/03/18		COMP. DATE 05/04/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-45															
-50															
-55															
-60															
-65															
-70															
-75															

NCDOT BORE DOUBLE R1015_GEO_SITE 3.GPJ NC_DOT_GDT 7/13/18



Other Samples:
ST-4 (16.1 - 18.1)

SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
SS- 327	60 LT	169+52	8.6- 10.1	A- 2- 4(0)	NP	NP	9.8	74.1	2.2	13.9	100	99	17.7	26.2	-
SS- 336	60 LT	169+52	53.6- 55.1	A- 3(0)	NP	NP	1.8	90.7	0	7.7	100	99.6	9	23.7	-
SS- 5	61 RT	172+22	19.6- 21.1	A- 3(0)	NP	NP	28.1	69.9	0.3	1.7	99.8	97.1	2.5	20.2	-
SS- 98	87 RT	174+86	38.0- 39.5	A- 7- 5(15)	50	29	14.9	31.8	19.5	33.8	92.5	81.5	59.4	37.5	-
SS- 99	87 RT	174+86	43.0- 44.5	A- 6 (1)	35	15	44.1	23.5	11.2	21.2	96.2	68.5	35.9	31.6	-
SS- 100	87 RT	174+86	48.0- 49.5	A- 1- b(0)	NP	NP	71.8	22.4	0.2	5.6	99	49	6.4	14.7	-
SS- 130	61 RT	176+04	72.4- 73.9	A- 2- 4(0)	NP	NP	40.6	38.2	9.6	11.3	88.8	64.9	20.9	16.2	-
SS- 471	65 LT	180+38	53.5- 55.0	A- 1- b(0)	NP	NP	82.3	12.6	0	5.7	98.5	43.6	6	19.4	-
SS- 536	52 LT	181+77	43.4- 44.9	A- 7- 6(26)	53	35	10.7	25	26.2	38.1	99.1	91.6	75.2	56.4	-
SS- 511	62 RT	181+72	28.4- 29.9	A- 4(0)	NP	NP	43	24.3	16.6	16.1	100	71.9	37.6	41.9	-
SS- 624	61 RT	184+38	63.4- 64.9	A- 6(8)	38	17	8.2	42.8	18	31	97.3	91.2	60.3	41.6	-
SS- 648	50 LT	185+78	68.4- 69.9	A- 6(9)	38	19	5.5	47.1	16.5	30.9	99.4	95.7	58.7	39.4	-
ST- 1	58 RT	169+54	4.5- 6.5	A- 6(3)	34	12	0.2	59.1	6	34.7	100	99.9	45.2	29.2	-
ST- 4	63 RT	185+90	16.1- 18.1	A- 7- 6(31)	61	37	0.2	22.9	22.7	54.2	100	99.9	80.5	60.7	-



Photo 1: Looking towards End Bent 2 and up station of -L-



Photo 2: Looking towards End Bent 1 and down station of -L-



Photo 3: Looking upstream at the East Prong of Slocum Creek



Photo 4: Looking downstream at the East Prong of Slocum Creek



REFERENCE: R-1015

PROJECT: 34360

CONTENTS

<u>SHEET NO.</u>	<u>DESCRIPTION</u>
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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY CRAVEN

PROJECT DESCRIPTION US 70 (Havelock Bypass) from
North of Pine Grove to North of Carteret County Line

SITE DESCRIPTION Site No. 4 - Bridge Nos. 278 and 279 on
-L- (US 70 Havelock Bypass) over -RR EY2- (Camp Lejeune Railroad)
STA 227+57.02 -L- /22+70.14 -RR EY2-

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-1015	1	21

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

GW Stalls

BR Spiro

MR English

BB Bahhur

KR Cullen

RM Bleifernich

INVESTIGATED BY GET SOLUTIONS

DRAWN BY BB Bahhur, KR Cullen

CHECKED BY GW Stalls

SUBMITTED BY GW Stalls

DATE June 2018



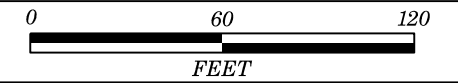
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Gerald W. Stalls, Jr. 3/6/2018
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SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED**

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION												GRADATION												ROCK DESCRIPTION												TERMS AND DEFINITIONS											
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6												WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.												HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:												ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.											
SOIL LEGEND AND AASHTO CLASSIFICATION												ANGULARITY OF GRAINS												WEATHERED ROCK (WR)																							
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS												THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.												NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.																							
MINERALOGICAL COMPOSITION												CRYSTALLINE ROCK (CR)												FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.																							
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.												NON-CRYSTALLINE ROCK (NCR)												FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.																							
COMPRESSIBILITY												COASTAL PLAIN SEDIMENTARY ROCK (CP)												COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.																							
SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50												WEATHERING																																			
PERCENTAGE OF MATERIAL												FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.																																			
ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE												VERY SLIGHT (V SLI) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.																																			
GROUND WATER												SLIGHT (SLI) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.																																			
WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP												MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.																																			
MISCELLANEOUS SYMBOLS												MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL																																			
ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY												SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF																																			
DIP & DIP DIRECTION OF ROCK STRUCTURES TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SOUNDING ROD TEST BORING WITH CORE SPT N-VALUE												VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF																																			
COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.												RECOMMENDATION SYMBOLS												ROCK HARDNESS																							
UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL												VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.																																			
ABBREVIATIONS												HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.																																			
AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE. - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY MED. - MEDIUM MICA. - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WEA. - WEATHERED WU - UNIT WEIGHT %g - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO												MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.																																			
TEXTURE OR GRAIN SIZE												MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.																																			
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053												SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.																																			
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)												VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.																																			
GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3												SOIL MOISTURE - CORRELATION OF TERMS												FRACTURE SPACING																							
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION												LIQUID LIMIT - SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE												TERM SPACING MORE THAN 10 FEET VERY THICK BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET																							
LL - LIQUID LIMIT PL - PLASTIC LIMIT OM - OPTIMUM MOISTURE SHRINKAGE LIMIT												- WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE												BEDDING																							
												- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE												FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.																							
												- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE												MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.																							
PLASTICITY												EQUIPMENT USED ON SUBJECT PROJECT												INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.																							
NON PLASTIC PLASTICITY INDEX (PI) 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH												DRILL UNITS: [X] CME-45C [] CME-55 [] CME-550 [] VANE SHEAR TEST [] PORTABLE HOIST ADVANCING TOOLS: [] CLAY BITS [] 6" CONTINUOUS FLIGHT AUGER [] 8" HOLLOW AUGERS [] HARD FACED FINGER BITS [] TUNG.-CARBIDE INSERTS [X] CASING [] W/ ADVANCER [X] TRICONE 2.88" STEEL TEETH [X] TRICONE 3.25" STEEL TEETH [] CORE BIT HAMMER TYPE: [X] AUTOMATIC [] MANUAL CORE SIZE: [] -B [] -H [] -N HAND TOOLS: [] POST HOLE DIGGER [] HAND AUGER [] SOUNDING ROD [] VANE SHEAR TEST												EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.																							
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																								INDURATION																							
																								FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.																							
																								BENCH MARK: BM2 STA 228+71.03 -L-, 242.76' LT., RR SPIKE IN TREE. ELEVATION: 28.81 FEET																							
																								NOTES:																							

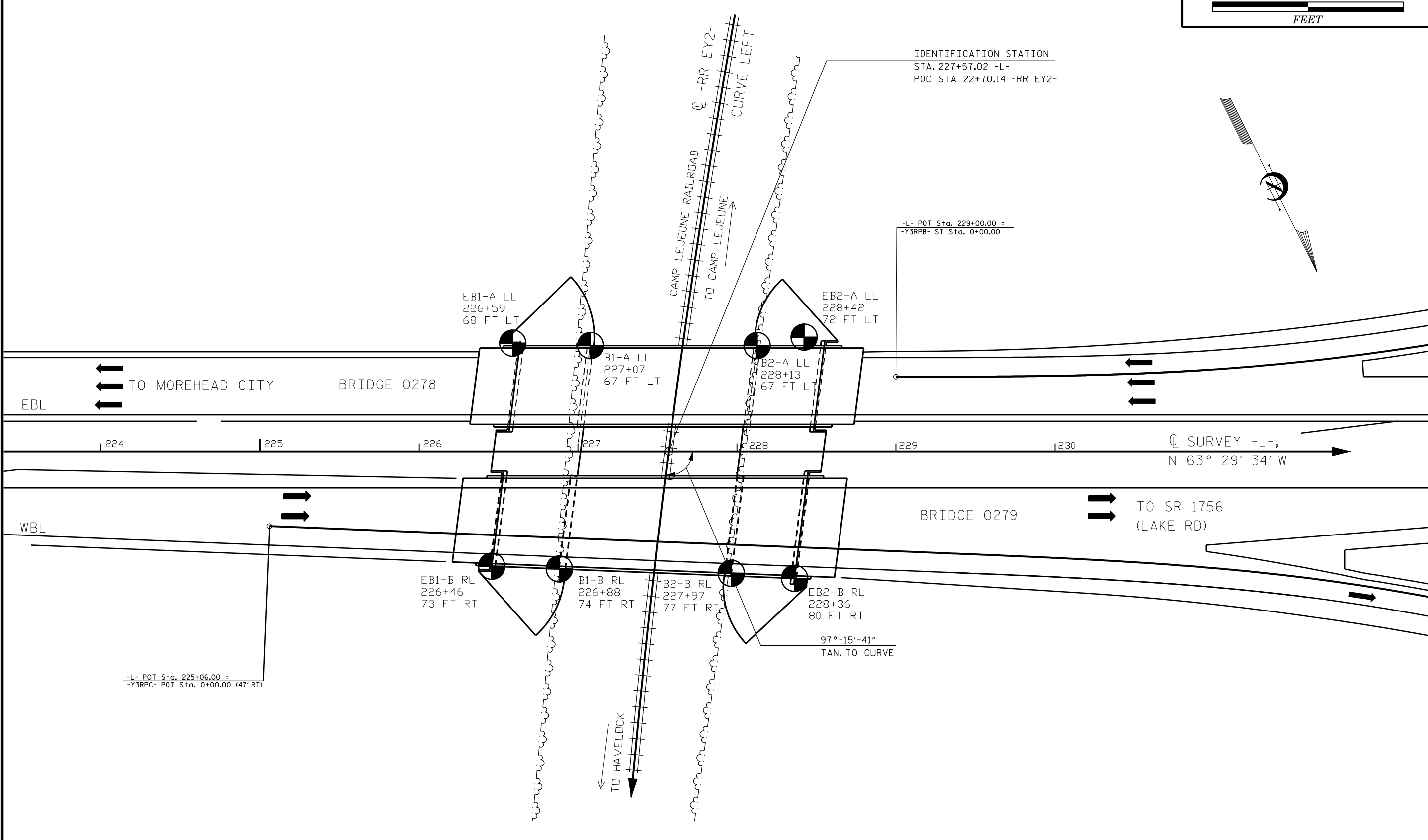
SITE PLAN



BENCH MARK: BM12 STA. 228+71.03 -L-, 242.76' LT., RR SPIKE IN TREE. EL. 28.81

IDENTIFICATION STATION
 STA. 227+57.02 -L-
 POC STA 22+70.14 -RR EY2-

-L- POT Sta. 229+00.00 =
 -Y3RPB- ST Sta. 0+00.00



EB1-A LL
 226+59
 68 FT LT

EB2-A LL
 228+42
 72 FT LT

B1-A LL
 227+07
 67 FT LT

B2-A LL
 228+13
 67 FT LT

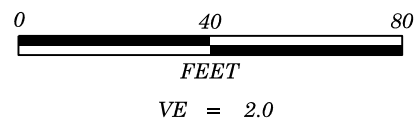
EB1-B RL
 226+46
 73 FT RT

EB2-B RL
 228+36
 80 FT RT

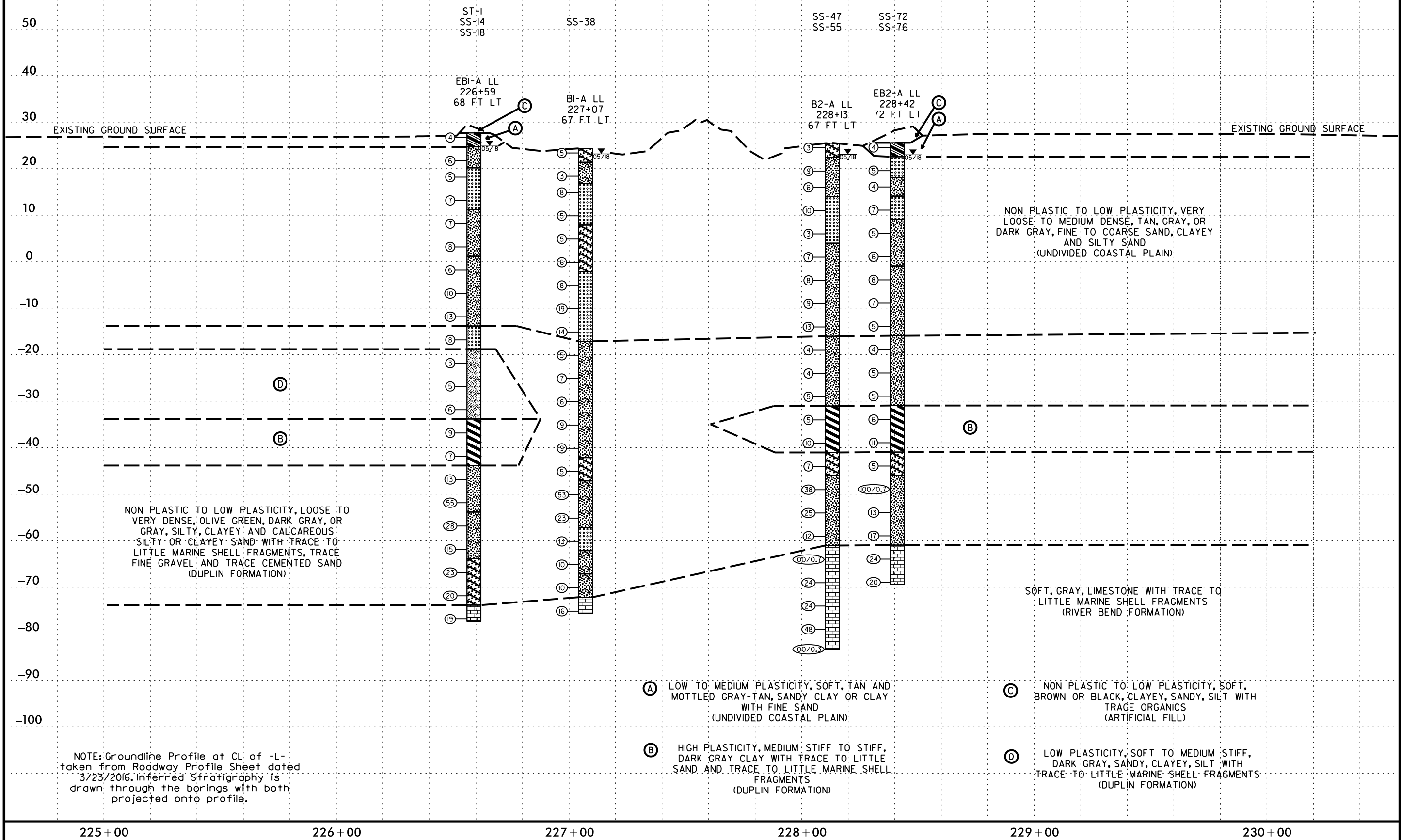
B1-B RL
 226+88
 74 FT RT

B2-B RL
 227+97
 77 FT RT

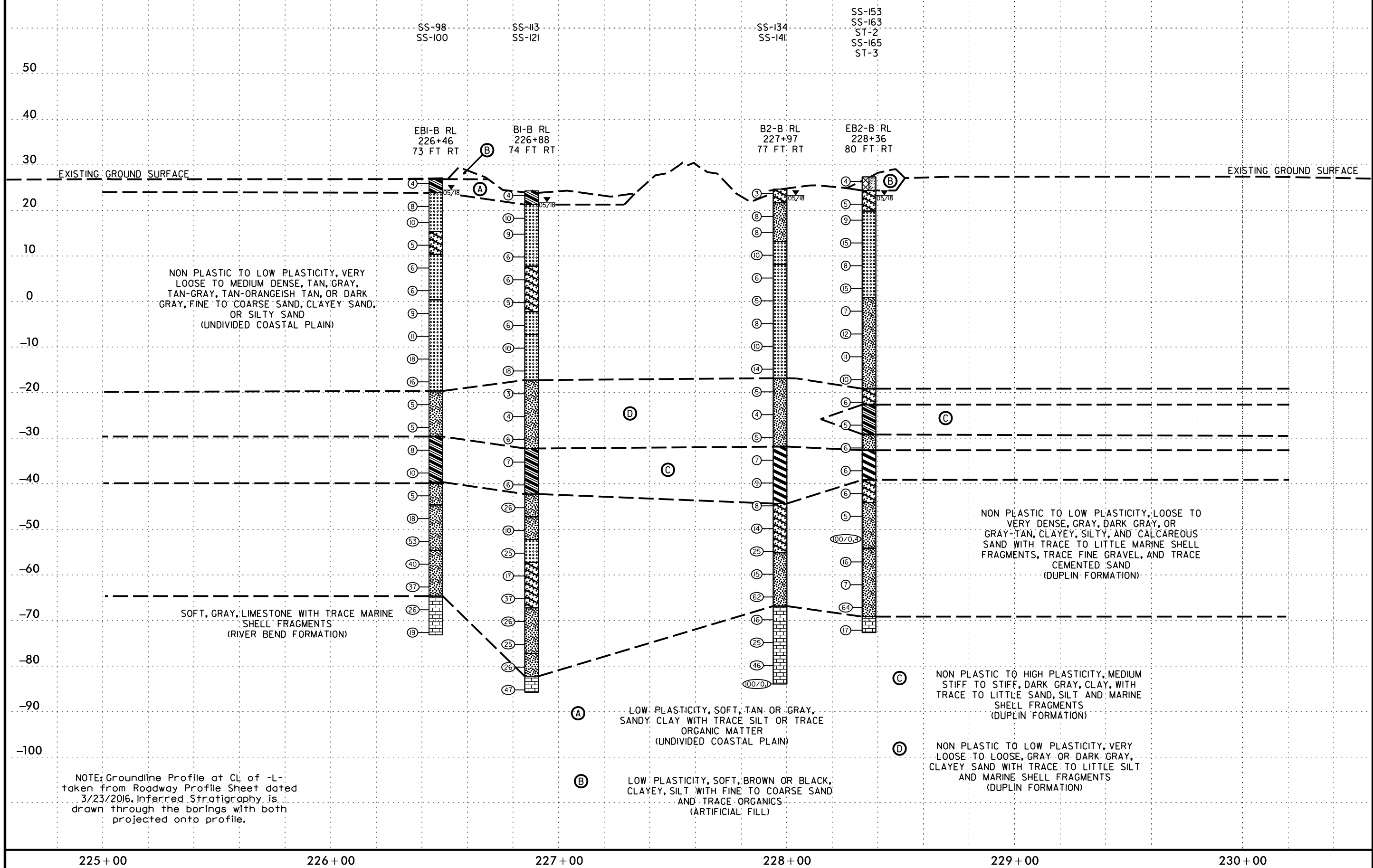
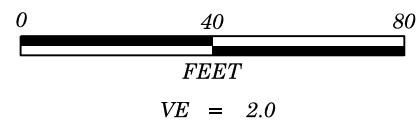
-L- POT Sta. 225+06.00 =
 -Y3RPC- POT Sta. 0+00.00 (47' RT)



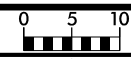
PROJECT REFERENCE NO.	SHEET NO.
R-1015	4
BRIDGE 278 SUBSURFACE PROFILE ALONG CENTERLINE -L-	



NOTE: Groundline Profile at CL of -L- taken from Roadway Profile Sheet dated 3/23/2016. Inferred Stratigraphy is drawn through the borings with both projected onto profile.

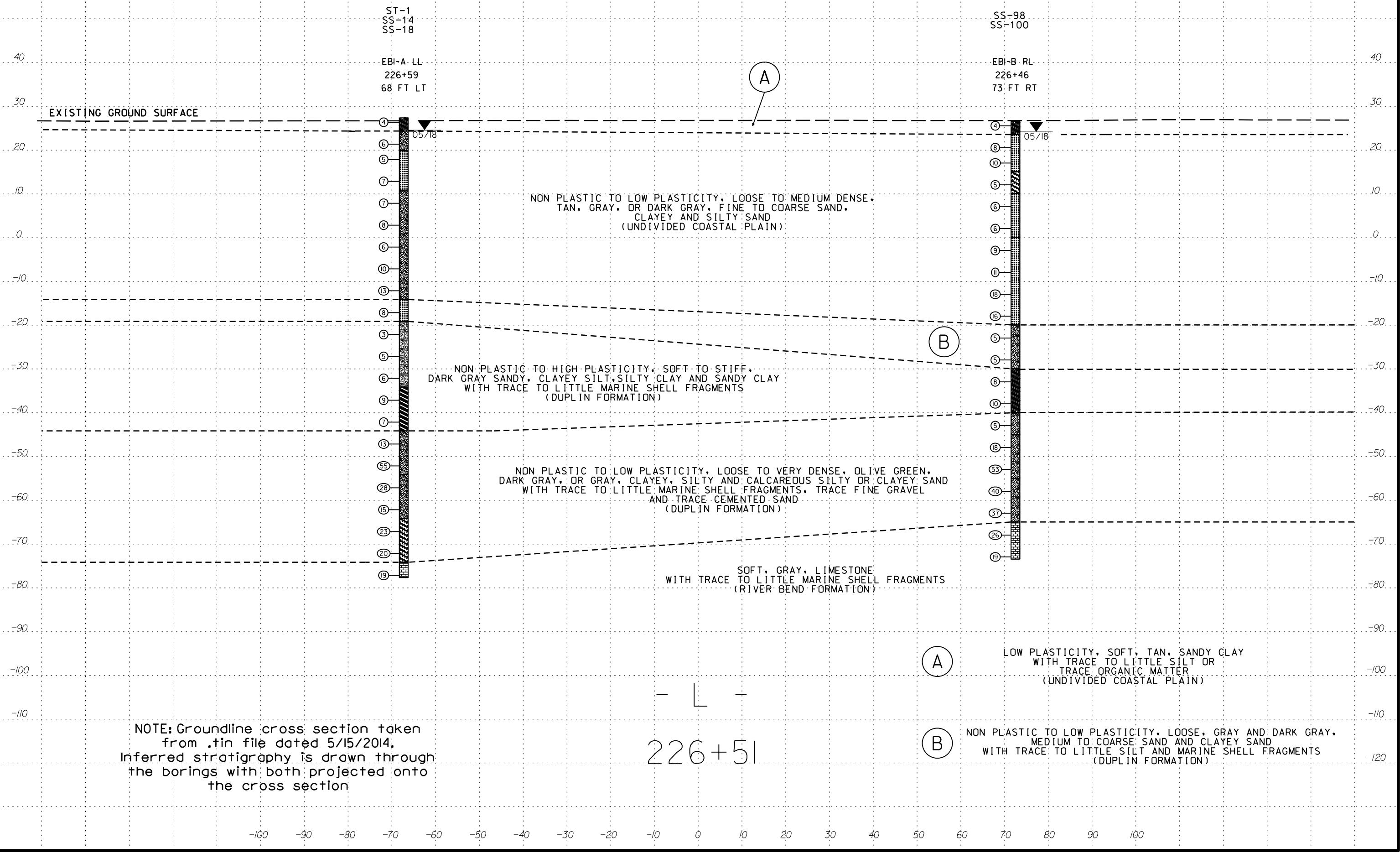


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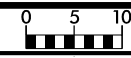


CROSS SECTION THROUGH END BENT 1

VE = 1
SKEW = 97° -15' -41"

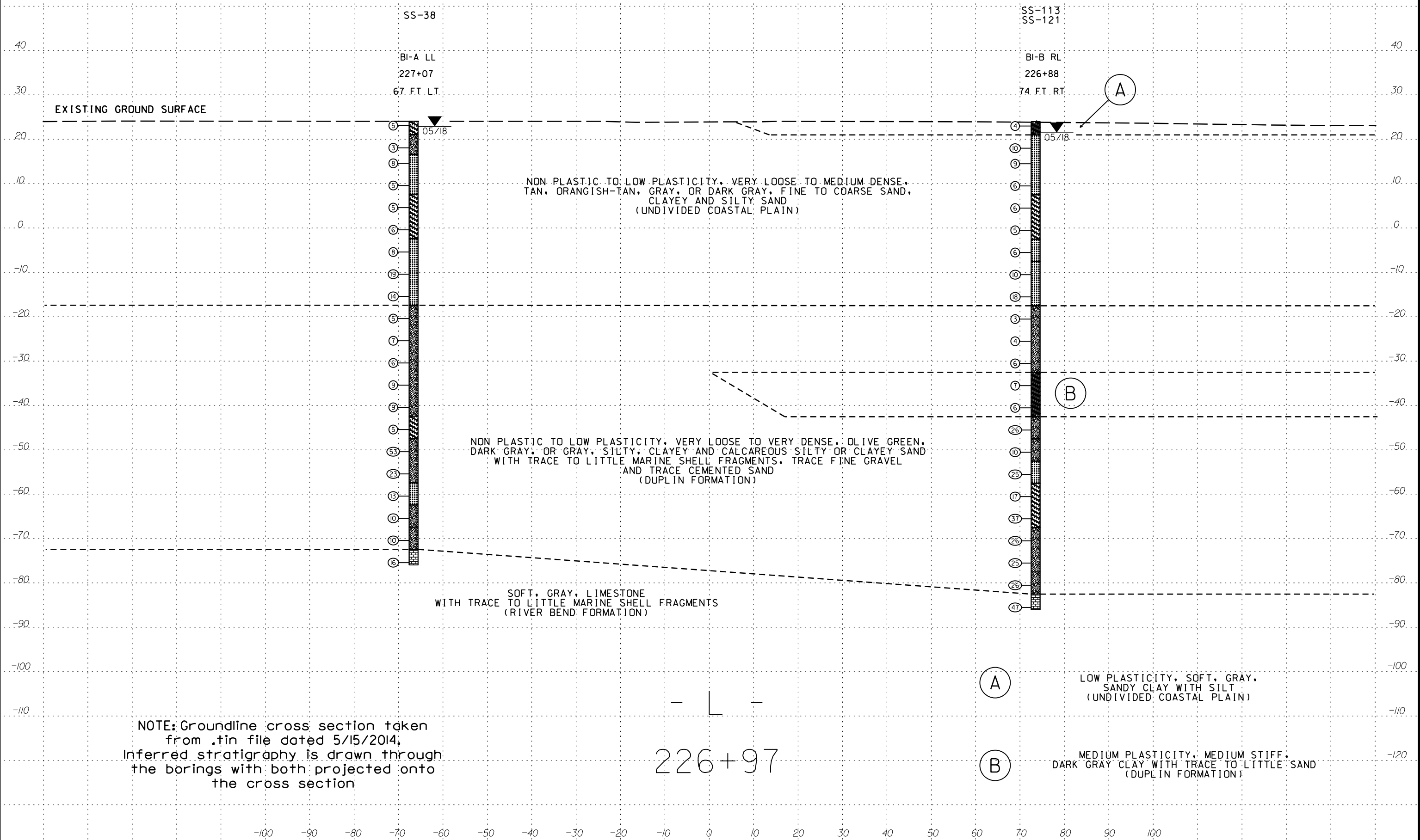


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CROSS SECTION THROUGH BENT 1

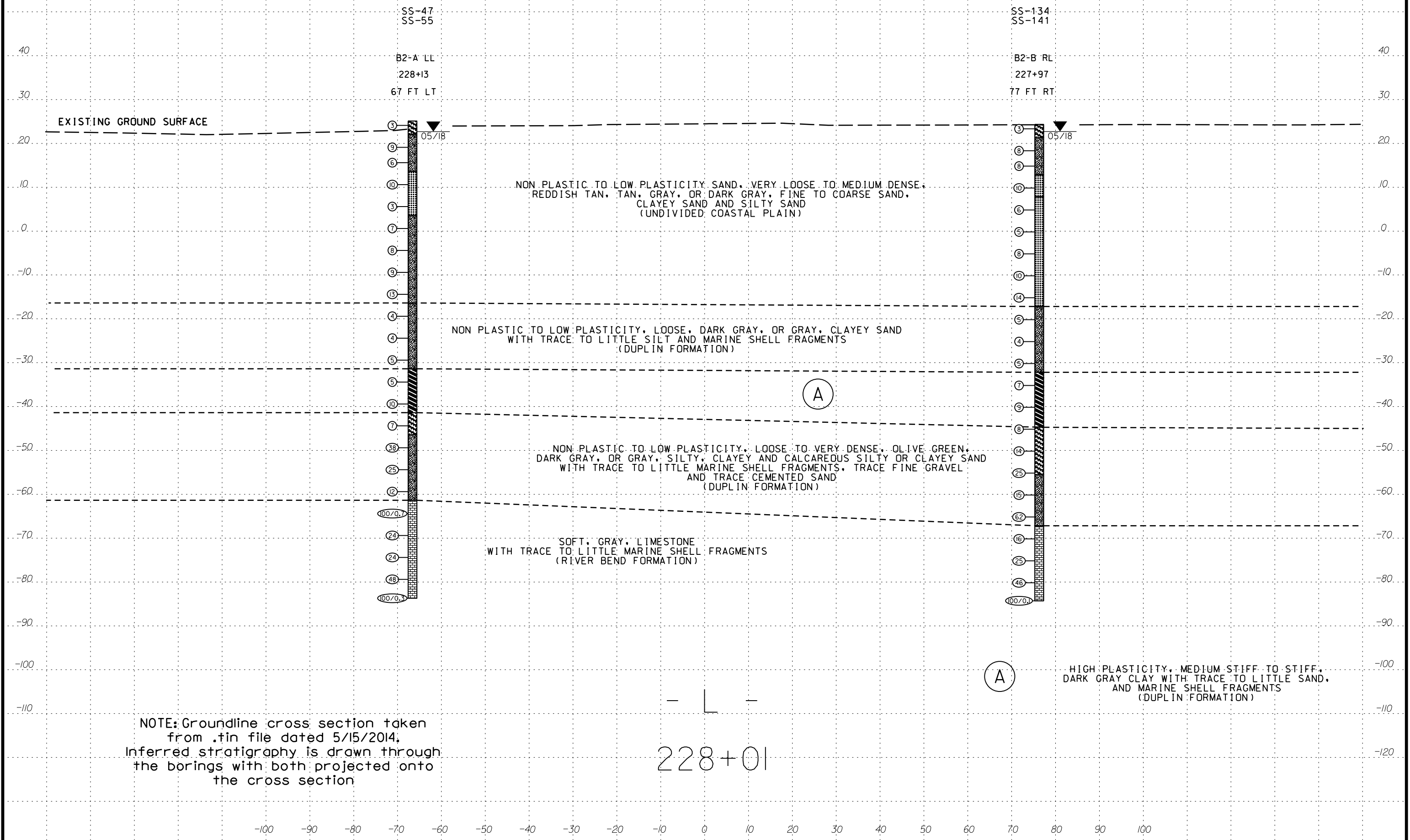
VE = 1
SKEW = 97° -15' -41"



B/23/99

CROSS SECTION THROUGH BENT 2

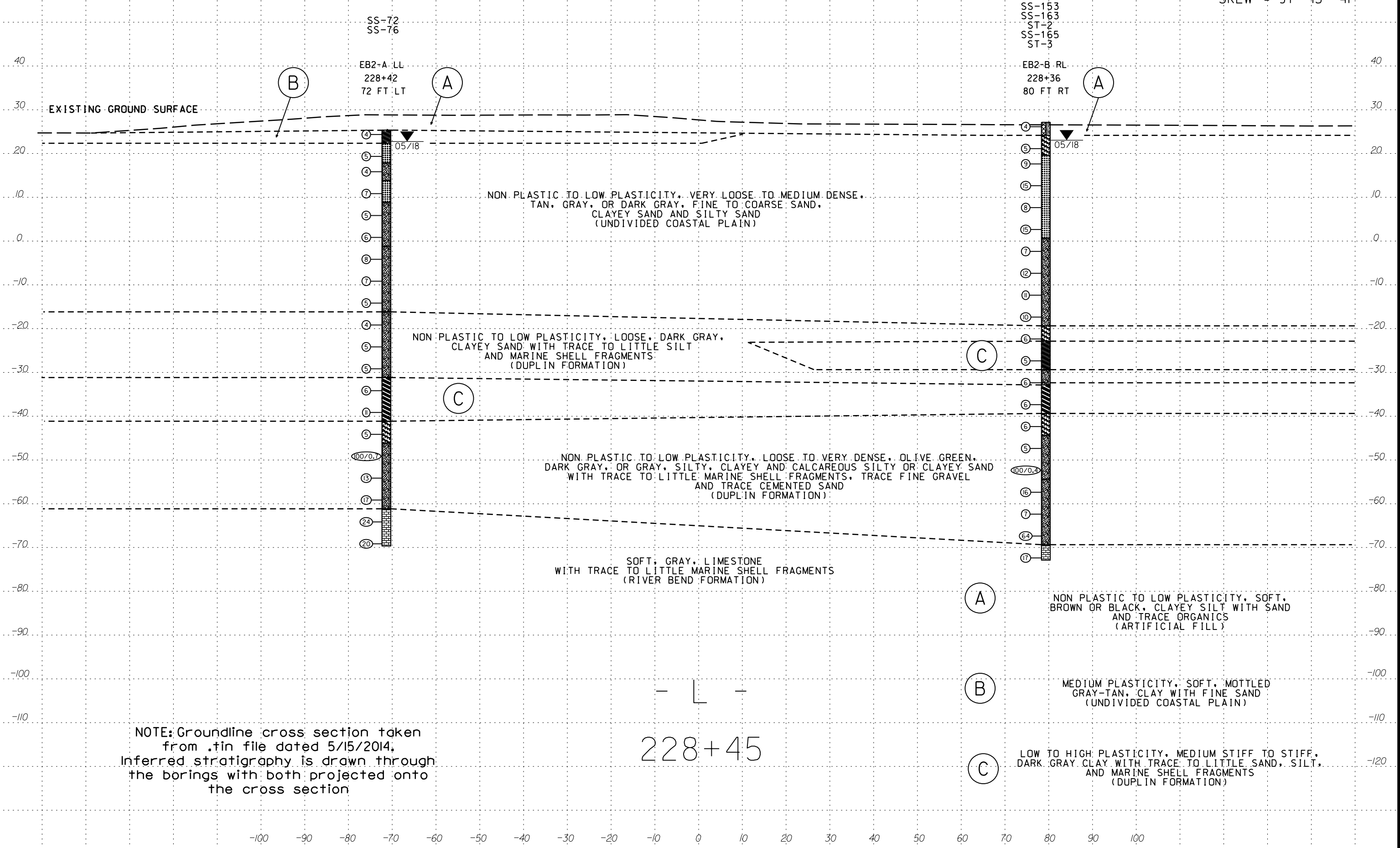
VE = 1
SKEW = 97° -15' -41"



8/23/99

CROSS SECTION THROUGH END BENT 2

SKEW = $97^{\circ} - 15' - 41''$



SS-72
SS-76

SS-153
SS-163
ST-2
SS-165
ST-3

EB2-A LL
228+42
72 FT LT

EB2-B RL
228+36
80 FT RT

- 4
- 5
- 4
- 7
- 5
- 6
- 8
- 7
- 5
- 4
- 5
- 5
- 6
- 11
- 5
- 100/0.7
- 15
- 17
- 24
- 20

- 4
- 5
- 9
- 15
- 8
- 15
- 7
- 12
- 11
- 10
- 6
- 5
- 6
- 6
- 5
- 100/0.4
- 16
- 7
- 6.4
- 17

(B)

(A)

(A)

(C)

(C)

(A)

(B)

(C)

- L -

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefemich	
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)
BORING NO. EB1-A LL		STATION 226+59		OFFSET 68 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 27.4 ft		TOTAL DEPTH 105.0 ft		NORTHING 412,296		EASTING 2,620,585	
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary		HAMMER TYPE Automatic		
DRILLER T. Donahue		START DATE 05/18/18		COMP. DATE 05/22/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
30																
	27.4	0.0	1	2	2									27.4	GROUND SURFACE	0.0
														24.4	UNDIVIDED COASTAL PLAIN Low Plasticity, Soft, Tan, Sandy, CLAY with Trace to Little Silt	3.0
	22.4	5.0	2	2	4									19.9	UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Gray and Tan, Silty, Fine to Coarse SAND with Trace Clay	7.5
	18.9	8.5	2	2	3									10.9	UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Gray, Fine to Medium SAND with Trace Silt	16.5
	13.9	13.5	3	3	4									0.9	UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Gray or Tan, Silty, Fine to Medium SAND with Trace Clay	26.5
	8.9	18.5	3	3	4											
	3.9	23.5	3	5	3											
	-1.1	28.5	2	2	4											
	-6.1	33.5	4	4	6											
	-11.1	38.5	4	6	7											
	-16.1	43.5	4	4	4											
	-21.1	48.5	1	1	2											
	-26.1	53.5	2	2	3											
	-31.1	58.5	2	3	3											
	-36.1	63.5	2	4	5											
	-41.1	68.5	2	3	4											
	-46.1	73.5	3	6	7											

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefemich	
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)
BORING NO. EB1-A LL		STATION 226+59		OFFSET 68 ft LT		ALIGNMENT -L-	
COLLAR ELEV. 27.4 ft		TOTAL DEPTH 105.0 ft		NORTHING 412,296		EASTING 2,620,585	
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary		HAMMER TYPE Automatic		
DRILLER T. Donahue		START DATE 05/18/18		COMP. DATE 05/22/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-50															
	-51.1	78.5	30	26	29										
	-56.1	83.5	14	10	18										
	-61.1	88.5	9	7	8										
	-66.1	93.5	9	11	12										
	-71.1	98.5	11	9	11										
	-76.1	103.5	9	11	8										

NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich									
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)								
BORING NO. B1-A LL		STATION 227+07		OFFSET 67 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 24.1 ft		TOTAL DEPTH 100.0 ft		NORTHING 412,319		EASTING 2,620,542									
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018				DRILL METHOD Mud Rotary		HAMMER TYPE Automatic									
DRILLER T. Donahue		START DATE 05/16/18		COMP. DATE 05/17/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
25	24.1	0.0	1	2	3								24.1	GROUND SURFACE	0.0
													21.1	UNDIVIDED COASTAL PLAIN Low Plasticity, Loose, Mottled Gray-Tan, Clayey, Fine to Medium SAND with Trace Silt	3.0
20	19.1	5.0	2	2	1								16.6	UNDIVIDED COASTAL PLAIN Non Plastic, Very Loose, Tan or Gray, Silty, Fine to Medium SAND with Trace Clay	7.5
15	15.6	8.5	5	4	4								7.6	UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Tan-Reddish Tan or Gray, Fine to Medium SAND with Trace Silt	16.5
10	10.6	13.5	3	2	3									UNDIVIDED COASTAL PLAIN Low Plasticity, Loose, Tan or Reddish Tan, Clayey, Silty, Fine to Coarse SAND	
5	5.6	18.5	1	2	3										
0	0.6	23.5	2	3	3										
-5	-4.4	28.5	3	4	4										
-10	-9.4	33.5	3	6	13										
-15	-14.4	38.5	7	7	7										
-20	-19.4	43.5	2	2	3										
-25	-24.4	48.5	3	3	4										
-30	-29.4	53.5	3	3	3										
-35	-34.4	58.5	3	4	5										
-40	-39.4	63.5	2	4	5										
-45	-44.4	68.5	2	2	3										
-50	-49.4	73.5	4	5	48										
-55	-54.4	78.5													

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich									
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)								
BORING NO. B1-A LL		STATION 227+07		OFFSET 67 ft LT		ALIGNMENT -L-									
COLLAR ELEV. 24.1 ft		TOTAL DEPTH 100.0 ft		NORTHING 412,319		EASTING 2,620,542									
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018				DRILL METHOD Mud Rotary		HAMMER TYPE Automatic									
DRILLER T. Donahue		START DATE 05/16/18		COMP. DATE 05/17/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-55			18	13	10										
-60	-59.4	83.5	8	7	6										
-65	-64.4	88.5	5	4	6										
-70	-69.4	93.5	9	4	6										
-75	-74.4	98.5	7	8	8										

Match Line

-57.4 --- 81.5
COASTAL PLAIN
Non Plastic, Medium Dense, Gray, Fine to Medium SAND with Trace Silt ("Duplin Formation")

-62.4 --- 86.5
COASTAL PLAIN
Low Plasticity, Medium Dense, Dark Gray, Silty, Fine to Medium SAND with Trace Clay ("Duplin Formation")

-67.4 --- 91.5
COASTAL PLAIN
Non Plastic, Medium Dense, Olive Green or Dark Gray, Calcareous, Silty, Fine to Medium SAND with Trace to Little Marine Shell Fragments, Trace Clay, Trace Fine Gravel, and Trace Cemented Sand Fragments ("Duplin Formation")

-72.4 --- 96.5
COASTAL PLAIN SEDIMENTARY ROCK
Soft, Gray, LIMESTONE with Trace to Little Marine Shell Fragments ("River Bend Formation")

-75.9 --- 100.0
Boring Terminated at Elevation -75.9 ft in Soft, LIMESTONE with Trace to Little Marine Shell Fragments ("River Bend Formation")

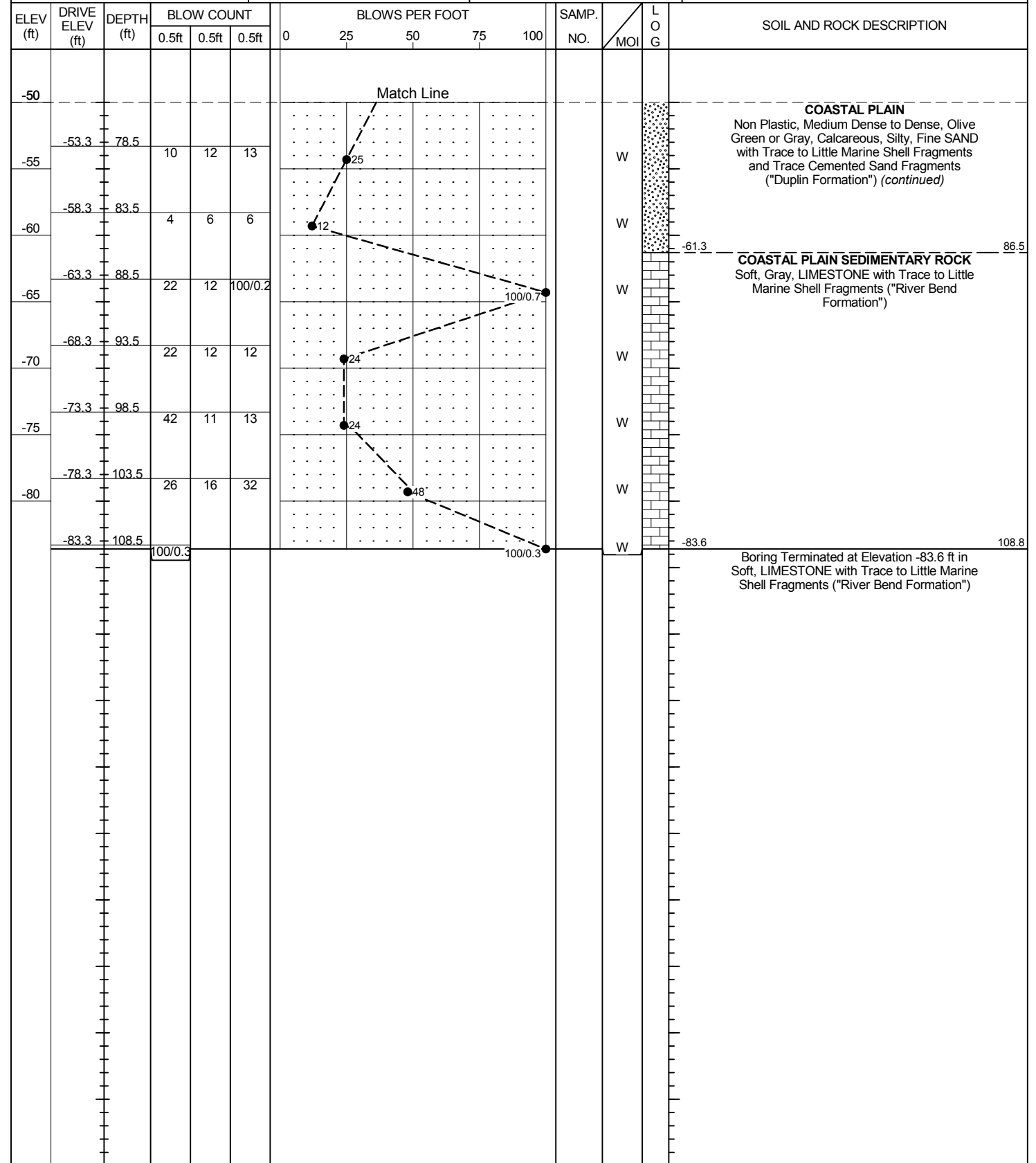
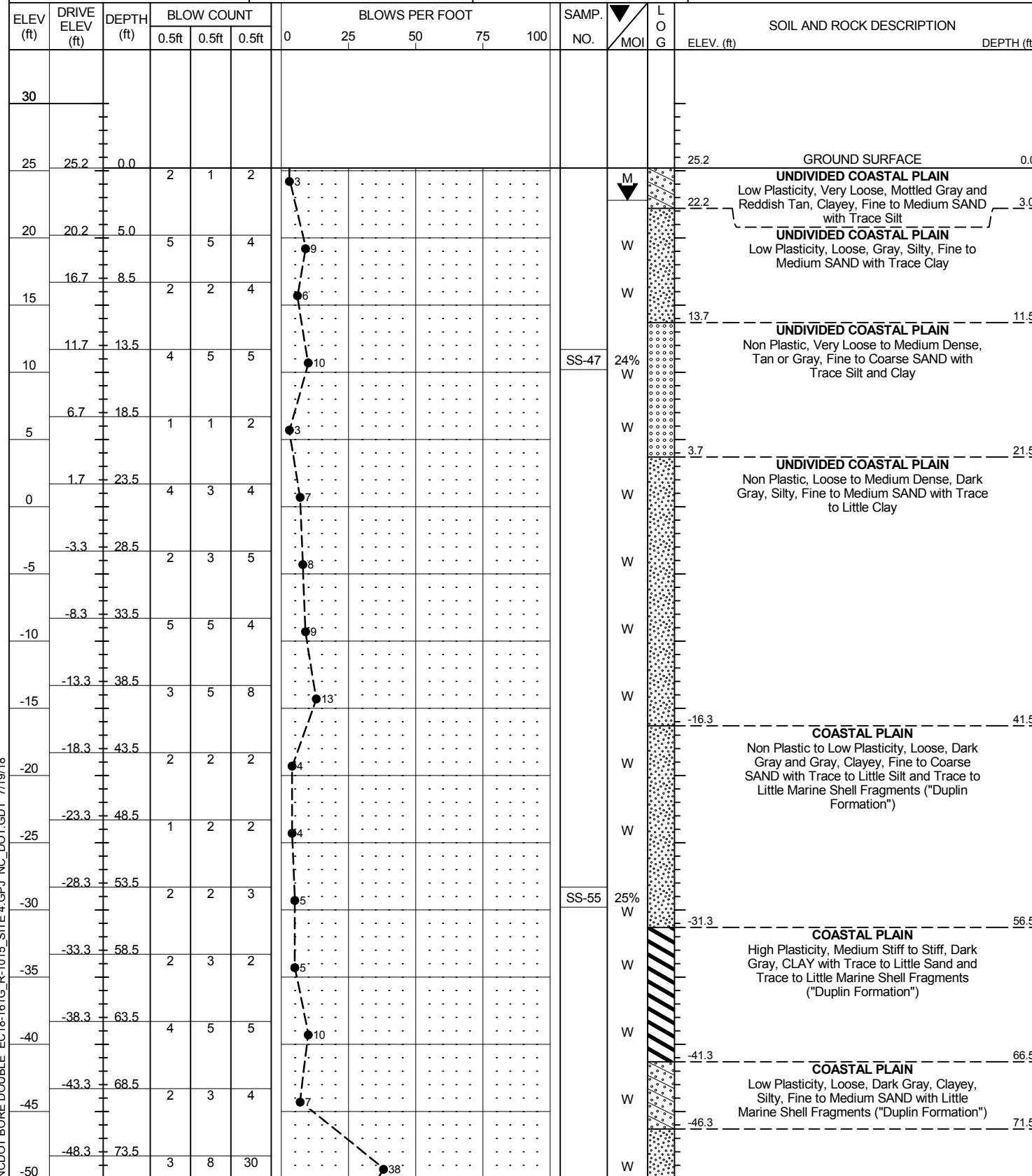
NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2	TIP R-1015	COUNTY CRAVEN	GEOLOGIST R. Bliefemich
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)			GROUND WTR (ft)
BORING NO. B2-A LL	STATION 228+13	OFFSET 67 ft LT	ALIGNMENT -L-
COLLAR ELEV. 25.2 ft	TOTAL DEPTH 108.8 ft	NORTHING 412,366	EASTING 2,620,448
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER T. Donahue	START DATE 05/21/18	COMP. DATE 05/22/18	SURFACE WATER DEPTH N/A

WBS 34360.1.2	TIP R-1015	COUNTY CRAVEN	GEOLOGIST R. Bliefemich
SITE DESCRIPTION Site No. 4 - Bridge No. 278 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)			GROUND WTR (ft)
BORING NO. B2-A LL	STATION 228+13	OFFSET 67 ft LT	ALIGNMENT -L-
COLLAR ELEV. 25.2 ft	TOTAL DEPTH 108.8 ft	NORTHING 412,366	EASTING 2,620,448
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018		DRILL METHOD Mud Rotary	HAMMER TYPE Automatic
DRILLER T. Donahue	START DATE 05/21/18	COMP. DATE 05/22/18	SURFACE WATER DEPTH N/A



NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST M. English										
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)									
BORING NO. EB1-B RL		STATION 226+46		OFFSET 73 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 26.6 ft		TOTAL DEPTH 100.0 ft		NORTHING 412,416		EASTING 2,620,659										
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER T. Donahue		START DATE 05/17/18		COMP. DATE 05/18/18		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
30																
	26.6	0.0	2	2	2									26.6	GROUND SURFACE	0.0
25														23.6	UNDIVIDED COASTAL PLAIN Low Plasticity, Soft, Tan, Sandy, CLAY with Trace Organic Matter	3.0
	21.6	5.0	2	3	5										UNDIVIDED COASTAL PLAIN Non Plastic, Loose to Medium Dense, Tan and Gray, Fine to Medium SAND with Trace Silt and Trace Clay	
20																
	18.1	8.5	3	5	5											
15																
	13.1	13.5	1	2	3									15.1	UNDIVIDED COASTAL PLAIN Low Plasticity, Loose, Tan, Clayey, SAND with Trace Silt	11.5
10														10.1	UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Tan, Fine to Medium SAND with Trace Silt	16.5
	8.1	18.5	3	3	3											
5																
	3.1	23.5	2	4	2											
0														0.1	UNDIVIDED COASTAL PLAIN Non Plastic, Loose to Medium Dense, Gray, Fine to Medium SAND with Trace Silt and Trace Clay	26.5
	-1.9	28.5	4	4	5											
-5																
	-6.9	33.5	3	5	6											
-10																
	-11.9	38.5	6	9	9											
-15																
	-16.9	43.5	6	8	8											
-20																
	-21.9	48.5	1	2	3											
-25																
	-26.9	53.5	1	2	3											
-30																
	-31.9	58.5	2	4	4											
-35																
	-36.9	63.5	3	5	5											
-40																
	-41.9	68.5	2	2	3											
-45																
	-46.9	73.5	7	7	11											
-50																

NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST M. English									
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)								
BORING NO. EB1-B RL		STATION 226+46		OFFSET 73 ft RT		ALIGNMENT -L-									
COLLAR ELEV. 26.6 ft		TOTAL DEPTH 100.0 ft		NORTHING 412,416		EASTING 2,620,659									
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic									
DRILLER T. Donahue		START DATE 05/17/18		COMP. DATE 05/18/18		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
-50															
	-51.9	78.5	7	8	45										
-55															
	-56.9	83.5	44	22	18										
-60															
	-61.9	88.5	27	15	22										
-65															
	-66.9	93.5	4	16	10										
-70															
	-71.9	98.5	8	8	11										

Match Line

COASTAL PLAIN
Non Plastic, Medium Dense to Very Dense, Gray, Silty Fine to Medium SAND with Trace Fine Gravel ("Duplin Formation") (continued)

COASTAL PLAIN
Non Plastic, Dense, Gray, Calcareous, Silty, Fine to Medium SAND with Trace to Little Marine Shell Fragments, Trace Cemented Sand Fragments, and Trace Fine Gravel ("Duplin Formation")

COASTAL PLAIN SEDIMENTARY ROCK
Soft, Gray, LIMESTONE with Trace Marine Shell Fragments ("River Bend Formation")

Boring Terminated at Elevation -73.4 ft in Soft, LIMESTONE with Trace Marine Shell Fragments ("River Bend Formation")

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich										
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)									
BORING NO. B1-B RL		STATION 226+88		OFFSET 74 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 24.0 ft		TOTAL DEPTH 110.0 ft		NORTHING 412,436		EASTING 2,620,622										
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER T. Donahue		START DATE 05/16/18		COMP. DATE 05/17/18		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
25	24.0	0.0	2	2	2									24.0	GROUND SURFACE	0.0
														21.0	UNDIVIDED COASTAL PLAIN Low Plasticity, Soft, Gray, Sandy, CLAY with Silt	3.0
20	19.0	5.0	3	5	5											
15	15.5	8.5	4	5	4											
10	10.5	13.5	3	3	3											
5	5.5	18.5	1	2	4											
0	0.5	23.5	1	2	3											
-5	-4.5	28.5	5	3	3											
-10	-9.5	33.5	4	4	6											
-15	-14.5	38.5	5	8	10											
-20	-19.5	43.5	1	1	2											
-25	-24.5	48.5	2	2	2											
-30	-29.5	53.5	2	3	3											
-35	-34.5	58.5	4	3	4											
-40	-39.5	63.5	2	3	3											
-45	-44.5	68.5	5	11	15											
-50	-49.5	73.5	4	4	6											
-55	-54.5	78.5														

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich										
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)									
BORING NO. B1-B RL		STATION 226+88		OFFSET 74 ft RT		ALIGNMENT -L-										
COLLAR ELEV. 24.0 ft		TOTAL DEPTH 110.0 ft		NORTHING 412,436		EASTING 2,620,622										
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic										
DRILLER T. Donahue		START DATE 05/16/18		COMP. DATE 05/17/18		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
-55			17	11	14											
-60	-59.5	83.5	5	5	12											
-65	-64.5	88.5	20	18	19											
-70	-69.5	93.5	10	8	18											
-75	-74.5	98.5	10	11	14											
-80	-79.5	103.5	11	14	12											
-85	-84.5	108.5	17	23	24											

NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

GEOTECHNICAL BORING REPORT

BORE LOG

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich	
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)
BORING NO. B2-B RL		STATION 227+97		OFFSET 77 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 24.4 ft		TOTAL DEPTH 108.6 ft		NORTHING 412,487		EASTING 2,620,527	
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER T. Donahue		START DATE 05/22/18		COMP. DATE 05/23/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
25	24.4	0.0												24.4	GROUND SURFACE	0.0
			1	2	1									21.4	UNDIVIDED COASTAL PLAIN Low Plasticity, Very Loose, Mottled Gray-Tan, Clayey, Fine SAND with Silt	3.0
	19.4	5.0	2	4	4										UNDIVIDED COASTAL PLAIN Non Plastic, Loose, Gray, Silty, Fine to Coarse SAND	
	15.9	8.5	2	3	5									12.9	UNDIVIDED COASTAL PLAIN Non Plastic, Medium Dense, Gray, Fine to Medium SAND	11.5
	10.9	13.5	3	4	6									7.9	UNDIVIDED COASTAL PLAIN Non Plastic, Loose to Medium Dense, Dark Gray, Fine to Coarse SAND with Trace Silt and Trace Clay	16.5
	5.9	18.5	3	3	3											
	0.9	23.5	2	2	3											
	-4.1	28.5	2	3	5											
	-9.1	33.5	2	5	5											
	-14.1	38.5	5	6	8											
	-19.1	43.5	1	2	3											
	-24.1	48.5	1	2	2											
	-29.1	53.5	2	2	3											
	-34.1	58.5	2	3	4											
	-39.1	63.5	4	4	5											
	-44.1	68.5	2	3	5											
	-49.1	73.5	5	7	7											
	-54.1	78.5	10	12	13											

WBS 34360.1.2		TIP R-1015		COUNTY CRAVEN		GEOLOGIST R. Bliefertich	
SITE DESCRIPTION Site No. 4 - Bridge No. 279 On -L- (US 70 Havelock Bypass) Over -RR EY2- (Camp Lejeune Railroad)							GROUND WTR (ft)
BORING NO. B2-B RL		STATION 227+97		OFFSET 77 ft RT		ALIGNMENT -L-	
COLLAR ELEV. 24.4 ft		TOTAL DEPTH 108.6 ft		NORTHING 412,487		EASTING 2,620,527	
DRILL RIG/HAMMER EFF./DATE GET0674 CME-45C 93% 03/22/2018			DRILL METHOD Mud Rotary			HAMMER TYPE Automatic	
DRILLER T. Donahue		START DATE 05/22/18		COMP. DATE 05/23/18		SURFACE WATER DEPTH N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
-55																
	-59.1	83.5	7	6	9											
	-64.1	88.5	12	22	40											
	-69.1	93.5	8	5	11											
	-74.1	98.5	11	11	14											
	-79.1	103.5	22	10	36											
	-84.1	108.5	100/0.1													

NCDOT BORE DOUBLE EC18-161G_R-1015_SITE 4.GPJ_NC_DOT.GDT 7/19/18

BRIDGE 0278

SOIL TEST RESULTS EB1-A LL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-1	68 FT LT	226+59	50.0 - 52.0	A-4(0)	29	3	3.8	51.5	16.1	28.7	99	97	57	35	==
SS-14	68 FT LT	226+59	63.5 - 65.0	A-7-5(69)	101	71	2.8	5.8	29.3	62.1	91	89	86	61	==
SS-18	68 FT LT	226+59	83.5 - 85.0	A-2-4(0)	NV	NP	13.4	69.0	11.7	6.0	91	90	18	25	==

SOIL TEST RESULTS B1-A LL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-38	67 FT LT	227+07	73.5 - 75.0	A-2-4	NV	NP	--	--	--	--	--	--	13	15	--

SOIL TEST RESULTS B2-A LL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-47	67 FT LT	228+13	13.5 - 15.0	A-3	NV	NP	8.6	88.5	0.9	2.0	100	99	3	24	--
SS-55	67 FT LT	228+13	53.5 - 55.0	A-2-4(0)	NV	NP	41.0	33.8	9.2	15.9	86	59	25	25	--

SOIL TEST RESULTS EB2-A LL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-72	72 FT LT	228+42	28.5 - 30.0	A-2-4(0)	NV	NP	7.7	76.4	4.9	10.9	100	100	17	29	--
SS-76	72 FT LT	228+42	48.5 - 50.0	A-2-4(0)	NV	NP	9.1	66.6	10.3	14.0	95	89	34	28	--

BRIDGE 0279

SOIL TEST RESULTS EB1-B RL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-98	73 FT RT	226+46	58.5 - 60.0	A-6(1)	34	14	--	--	--	--	--	--	38	28	--
SS-100	73 FT RT	226+46	68.5 - 70.0	A-2-4(0)	26	9	27.5	44.3	10.1	18.1	86	77	32	25	--

SOIL TEST RESULTS B1-B RL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-113	74 FT RT	226+88	28.5 - 30.0	A-3	NV	NP	4.4	86.6	2.0	7.0	100	100	10	25	--
SS-121	74 FT RT	226+88	68.5 - 70.0	A-2-4(0)	NV	NP	42.2	45.0	7.1	5.8	98	83	13	22	--

SOIL TEST RESULTS B2-B RL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-134	77 FT RT	227+97	18.5 - 20.0	A-3	NV	NP	68.6	24.7	1.7	5.0	100	68	8	24	--
SS-141	77 FT RT	227+97	53.5 - 55.0	A-2-4(0)	29	8	25.9	42.1	11.9	20.1	69	57	27	29	--

SOIL TEST RESULTS EB2-B RL

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
SS-153	80 FT RT	228+36	0.0 - 1.5	A-4(0)	NV	NP	11.3	55.9	15.3	17.6	100	98	36	23	3.9
SS-163	80 FT RT	228+36	48.5 - 50.0	A-2-7(0)	47	29	--	--	--	--	--	--	21	31	--
ST-2	80 FT RT	228+36	50.0 - 52.0	A-6(2)	39	16	3.6	67.5	8.5	20.3	99	98	39	34	--
SS-165	80 FT RT	228+36	58.5 - 60.0	A-2-4(0)	29	7	--	--	--	--	--	--	25	30	--
ST-3	80 FT RT	228+36	60.0 - 62.0	A-7-6(53)	75	48	1.6	7.5	25.8	65.1	99	98	95	57	--

SITE PHOTOGRAPHS

Bridge 0278 LEFT BRIDGE



VIEW AT EB1-A LL ALONG -L-
FACING UP-STATIONING TOWARDS B1-A LL
AND -RR EY2-
(CAMP LEJEUNE RAILROAD)



VIEW AT EB2-A LL ALONG -L-
FACING DOWN-STATIONING TOWARDS B2-A LL
AND -RR EY2-
(CAMP LEJEUNE RAILROAD)

SITE PHOTOGRAPHS

Bridge 0279 RIGHT BRIDGE



VIEW AT EB1-B RL ALONG -L-
FACING UP-STATIONING TOWARDS B1-B RL
AND -RR EY2-
(CAMP LEJEUNE RAILROAD)



VIEW AT EB2-B RL ALONG -L-
FACING DOWN-STATIONING TOWARDS B2-B RL
AND -RR EY2-
(CAMP LEJEUNE RAILROAD)

REFERENCE: R-1015

PROJECT: 34360

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

COUNTY CRAVEN

PROJECT DESCRIPTION US 70 (Havelock Bypass) from North
of Pine Grove to North of Carteret County Line

SITE DESCRIPTION MSE Retaining Walls 3 & 4: Site No. 9
- Dual Bridges on US 70 over 70 Business between SR 1747
and SR 1176 - Station 516+87.37 -L- / 69+02.79 -RP2AC-

CONTENTS

SHEET NO.	DESCRIPTION
1	TITLE SHEET
2	LEGEND
3	SITE PLAN
4-5	PROFILES
6	SOIL TEST RESULTS

APPENDICES

APPENDIX	TITLE	SHEETS
A	CONSOLIDATION & TRIAXIAL TESTS RESULTS	7-35

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-1015	1	35

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF PREPARING THE SCOPE OF WORK TO BE INCLUDED IN THE REQUEST FOR PROPOSAL. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

SOIL AND ROCK BOUNDARIES WITHIN A BOREHOLE ARE BASED ON GEOTECHNICAL INTERPRETATION UNLESS ENCOUNTERED IN A SAMPLE. INTERPRETED BOUNDARIES MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN SAMPLED STRATA AND BOREHOLE INFORMATION MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
 - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

P.M. WEAVER

C.R. PASTRANA

GET Solutions

INVESTIGATED BY ESP Associates, INC.

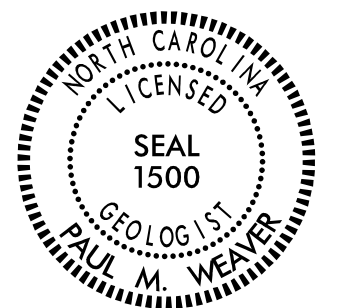
DRAWN BY C.R. PASTRANA

CHECKED BY P.M. WEAVER

SUBMITTED BY ESP Associates, INC.

DATE August 2018

 **ESP**
 ESP ASSOCIATES, INC.
 7011 ALBERT PICK RD
 SUITE E
 GREENSBORO, NC 27409
 FIRM # C-0587
 WWW.ESPASSOCIATES.COM



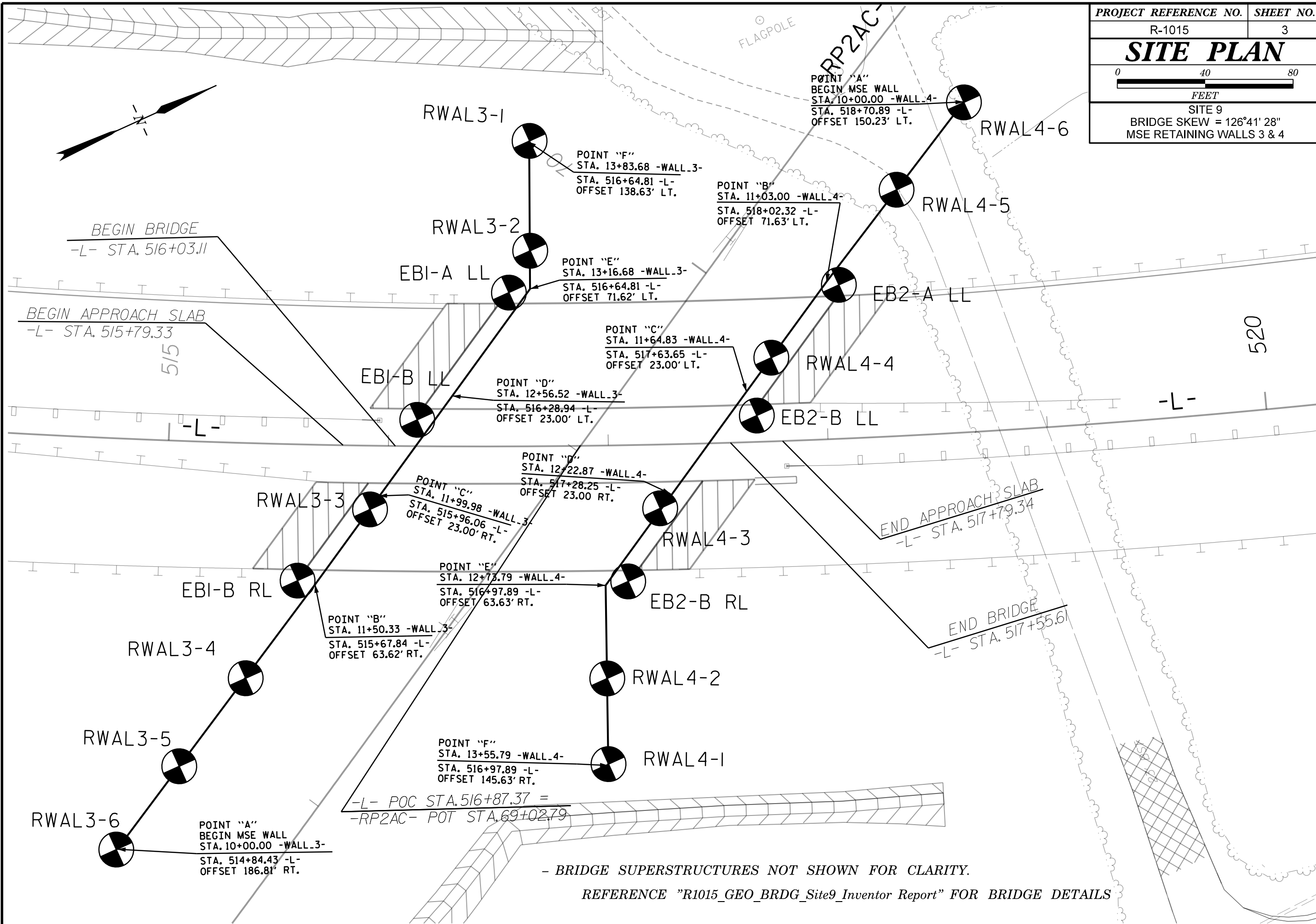
DocuSigned by:
Paul M. Weaver 8/28/2018
 01847D3739804040 DATE

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UNLESS ALL SIGNATURES COMPLETED

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

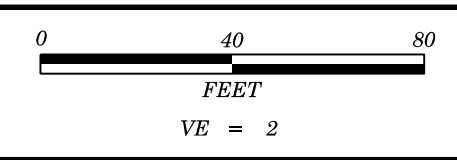
SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																																																											
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>										<p>WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>										<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>										<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																											
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS										WEATHERED ROCK (WR)										NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.																																																											
<p>GENERAL CLASS.</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th colspan="5">GRANULAR MATERIALS (≤ 35% PASSING #200)</th> <th colspan="5">SILT-CLAY MATERIALS (> 35% PASSING #200)</th> <th colspan="5">ORGANIC MATERIALS</th> </tr> <tr> <th>GROUP CLASS.</th> <th>A-1</th> <th>A-3</th> <th>A-2</th> <th>A-4</th> <th>A-5</th> <th>A-6</th> <th>A-7</th> <th>A-1, A-2</th> <th>A-3</th> <th>A-4, A-5</th> <th>A-6, A-7</th> <th colspan="5"></th> </tr> <tr> <th>SYMBOL</th> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td colspan="5"></td> </tr> </table>										GRANULAR MATERIALS (≤ 35% PASSING #200)					SILT-CLAY MATERIALS (> 35% PASSING #200)					ORGANIC MATERIALS					GROUP CLASS.	A-1	A-3	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7						SYMBOL																	<p>THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.</p>										<p>CRISTALLINE ROCK (CR)</p> <p>NON-CRYSTALLINE ROCK (NCR)</p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP)</p>										<p>CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</p> <p>NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</p> <p>COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</p>										
GRANULAR MATERIALS (≤ 35% PASSING #200)					SILT-CLAY MATERIALS (> 35% PASSING #200)					ORGANIC MATERIALS																																																																															
GROUP CLASS.	A-1	A-3	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7																																																																														
SYMBOL																																																																																									
<p>MINERALOGICAL COMPOSITION</p> <p>MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.</p>										<p>COMPRESSION</p> <p>SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50</p>										<p>WEATHERING</p> <p>FRESH - ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.</p> <p>VERY SLIGHT (IV SLI) - ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.</p> <p>SLIGHT (SLI) - ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.</p> <p>MODERATE (MOD) - SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.</p> <p>MODERATELY SEVERE (MOD. SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i></p> <p>SEVERE (SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF</i></p> <p>VERY SEVERE (IV SEV.) - ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</i></p> <p>COMPLETE - ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</p>										<p>PERCENTAGE OF MATERIAL</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th>ORGANIC MATERIAL</th> <th>GRANULAR SOILS</th> <th>SILT - CLAY SOILS</th> <th>OTHER MATERIAL</th> </tr> <tr> <td>TRACE OF ORGANIC MATTER</td> <td>2 - 3%</td> <td>3 - 5%</td> <td>TRACE 1 - 10%</td> </tr> <tr> <td>LITTLE ORGANIC MATTER</td> <td>3 - 5%</td> <td>5 - 12%</td> <td>LITTLE 10 - 20%</td> </tr> <tr> <td>MODERATELY ORGANIC</td> <td>5 - 10%</td> <td>12 - 20%</td> <td>SOME 20 - 35%</td> </tr> <tr> <td>HIGHLY ORGANIC</td> <td>> 10%</td> <td>> 20%</td> <td>HIGHLY 35% AND ABOVE</td> </tr> </table>										ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE 1 - 10%	LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE 10 - 20%	MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME 20 - 35%	HIGHLY ORGANIC	> 10%	> 20%	HIGHLY 35% AND ABOVE																														
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<p>GROUND WATER</p> <p> WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING</p> <p> STATIC WATER LEVEL AFTER 24 HOURS</p> <p> PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA</p> <p> SPRING OR SEEP</p>										<p>MISCELLANEOUS SYMBOLS</p> <p> ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION</p> <p> SOIL SYMBOL</p> <p> ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT</p> <p> INFERRED SOIL BOUNDARY</p> <p> INFERRED ROCK LINE</p> <p> ALLUVIAL SOIL BOUNDARY</p> <p> DIP & DIP DIRECTION OF ROCK STRUCTURES</p> <p> SPT TEST BORING</p> <p> AUGER BORING</p> <p> CORE BORING</p> <p> MONITORING WELL</p> <p> PIEZOMETER INSTALLATION</p> <p> SLOPE INDICATOR INSTALLATION</p> <p> CONE PENETROMETER TEST</p> <p> SOUNDING ROD</p> <p> TEST BORING WITH CORE</p> <p> SPT N-VALUE</p>										<p>RECOMMENDATION SYMBOLS</p> <p> UNDERCUT</p> <p> SHALLOW UNDERCUT</p> <p> UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE</p> <p> UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK</p> <p> UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL</p>										<p>ABBREVIATIONS</p> <p>AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CORE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY</p> <p>MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY</p> <p>VST - VANE SHEAR TEST WEA. - WEATHERED W - UNIT WEIGHT W_d - DRY UNIT WEIGHT</p> <p>SAMPLE ABBREVIATIONS</p> <p>S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO</p>																																																											
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<p>COLOR</p> <p>DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.</p>										<p>INDURATION</p> <p>FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.</p> <p>FRIABLE - RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.</p> <p>MODERATELY INDURATED - GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.</p> <p>INDURATED - GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.</p> <p>EXTREMELY INDURATED - SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.</p>										<p>NOTES:</p> <p>F.J.A.D. = FILLED IMMEDIATELY AFTER DRILLING</p> <p>STATIONS AND OFFSETS FOR -WALL_3- AND -WALL_4- WERE MEASURED FROM "R-1015 MSE WALL_END BENT 1.Site 9.dgn" AND "R-1015 MSE WALL_END BENT 2.Site 9.dgn" FILES. NO GPK FILE WAS PROVIDED WITH WALL ALIGNMENTS. ALL BORINGS WERE REFERENCED TO -L- TO OBTAIN BORING ELEVATIONS. THE -L- STATION AND OFFSET EQUIVALENT FOR THE BORING LOCATIONS ARE PROVIDED ON THE BORING LOGS.</p>																																																																					
<p>BENCH MARK: BM-26; RR SPIKE IN TREE STA. 520+57.00 -L- 308' LEFT AND "r1015.is_tin.tin" WAS USED TO DETERMINE GROUND ELEVATIONS AT BORINGS</p> <p style="text-align: right;">ELEVATION: 26.38 FEET</p>																																																																																									

PROJECT REFERENCE NO.	SHEET NO.
R-1015	3
SITE PLAN	
SITE 9 BRIDGE SKEW = 126°41' 28" MSE RETAINING WALLS 3 & 4	

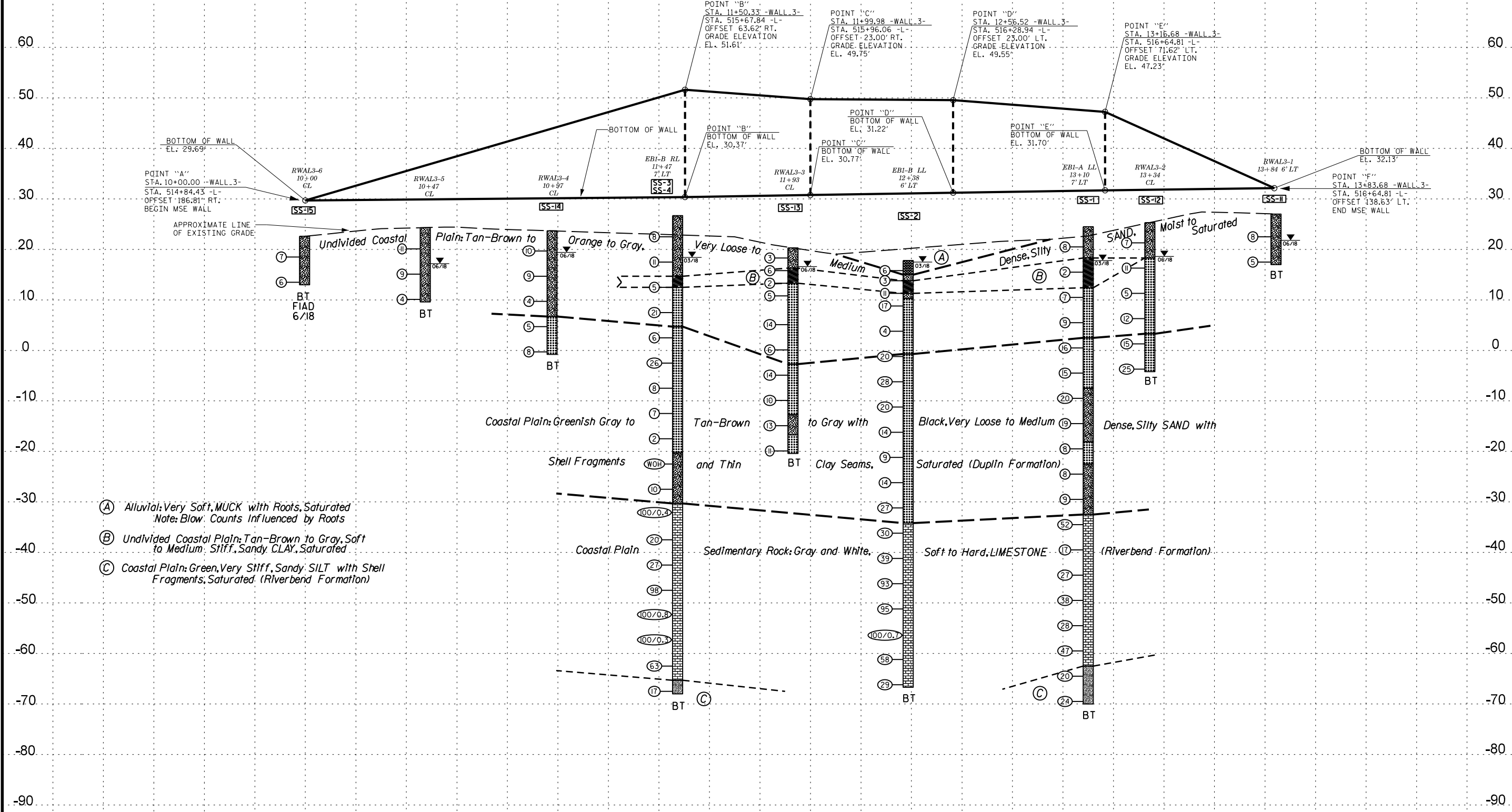


- BRIDGE SUPERSTRUCTURES NOT SHOWN FOR CLARITY.

REFERENCE "R1015_GEO_BRDG_Site9_Inventor Report" FOR BRIDGE DETAILS



PROJECT REFERENCE NO.	SHEET NO.
R-1015	4
SITE 9: END BENT 1 PROFILE ALONG MSE RETAINING WALL 3	

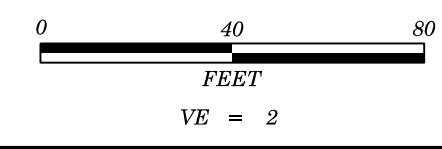


- (A) Alluvial: Very Soft, MUCK with Roots, Saturated
Note: Blow Counts Influenced by Roots
- (B) Undivided Coastal Plain: Tan-Brown to Gray, Soft to Medium Stiff, Sandy CLAY, Saturated
- (C) Coastal Plain: Green, Very Stiff, Sandy SILT with Shell Fragments, Saturated (Riverbend Formation)

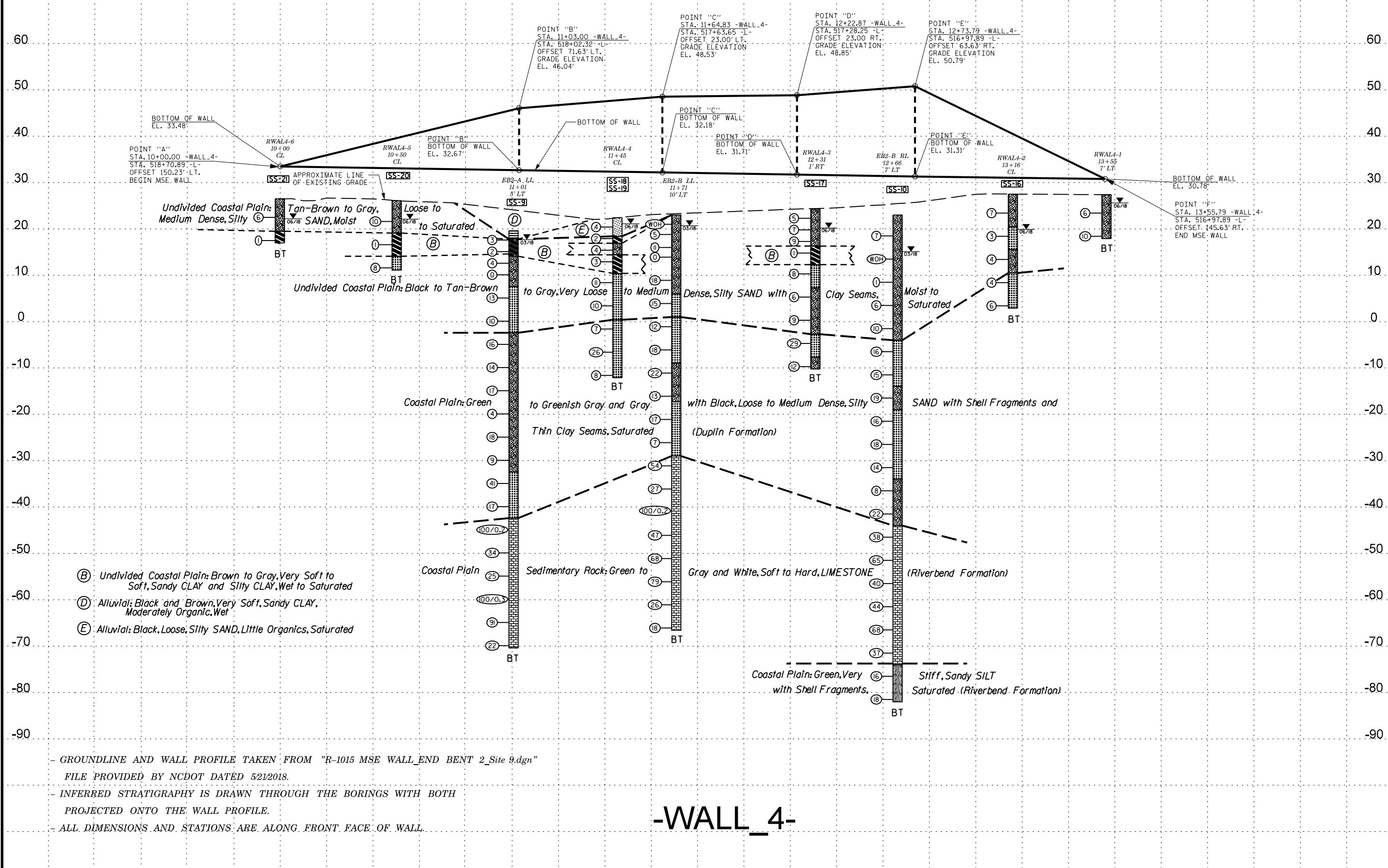
- GROUNDLINE AND WALL PROFILE TAKEN FROM "R-1015 MSE WALL_END BENT 1_Site 9.dgn"
FILE PROVIDED BY NCDOT DATED 5/21/2018.
- INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
PROJECTED ONTO THE WALL PROFILE.
- ALL DIMENSIONS AND STATIONS ARE ALONG FRONT FACE OF WALL.

-WALL_3-

10+00 11+00 12+00 13+00 14+00



PROJECT REFERENCE NO.	SHEET NO.
R-1015	5
SITE 9: END BENT 2 PROFILE ALONG MSE RETAINING WALL 4	



GROUNDLINE AND WALL PROFILE TAKEN FROM "R-1015 MSE WALL_END BENT 2_Site 9.dgn"
 FILE PROVIDED BY NCDOT DATED 5/21/2018.
 INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH
 PROJECTED ONTO THE WALL PROFILE.
 ALL DIMENSIONS AND STATIONS ARE ALONG FRONT FACE OF WALL.

-WALL_4-

SOILS LABORATORY TESTS RESULTS

WBS NO.: 34360.1.1

TIP NO.: R-1015

COUNTY: Craven

SITE DESCRIPTION: Site #9 - Dual Bridges on US 70 (Havelock Bypass) over US 70 Business - MSE Wall Abutments

BORING NO.	SAMPLE NO.	Boring Location			DEPTH INTERVAL (FT)	AASHTO	N	L.L	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
		Alignment	Station	Offset						CSE. SAND	F. SAND	SILT	CLAY	10	40	200		
EB1-A LL	SS-1	-WALL_3-	13+10	7' LT	53.0-54.5	A-2-4 (0)	9	NP	NP	29	50	6	15	80	67	18	30.8	-
EB1-B LL	SS-2	-WALL_4-	12+38	6' LT	13.0-14.5	A-3 (1)	4	NP	NP	27	66	2	5	100	98	8	-	-
EB1-B RL	SS-3	-WALL_3-	11+47	7' LT	3.2-4.7	A-2-4 (0)	8	NP	NP	36	50	2	12	100	79	15	-	-
EB1-B RL	SS-4	-WALL_3-	11+47	7' LT	48.2-49.7	A-2-4 (0)	0	NP	NP	68	21	3	8	99	64	11	26.1	0.5
EB2-A LL	SS-9	-WALL_4-	11+01	5' LT	48.5-50.0	A-2-4 (0)	9	NP	NP	12	67	7	14	100	93	27	31.0	-
EB2-B RL	SS-10	-WALL_4-	12+66	7' LT	18.5-20.0	A-2-4 (0)	6	NP	NP	7	79	5	9	100	97	17	31.1	-
RWAL3-1	SS-11	-WALL_3-	13+84	CL	3.5-5.0	A-2-4 (0)	8	NP	NP	13	65	11	11	100	98	23	14.2	-
RWAL3-2	SS-12	-WALL_3-	13+34	CL	3.0-4.5	A-2-4 (0)	7	NP	NP	11	72	10	7	100	98	18	13.1	-
RWAL3-3	SS-13	-WALL_3-	11+93	CL	14.2-15.7	A-3 (1)	14	NP	NP	29	68	0	3	100	89	5	21.8	-
RWAL3-4	SS-14	-WALL_3-	10+97	CL	13.0-14.5	A-2-4 (0)	4	24	10	18	48	22	12	99	92	35	29.8	-
RWAL3-6	SS-15	-WALL_3-	10+00	CL	3.1-4.6	A-2-4 (0)	7	NP	NP	7	70	2	21	100	99	24	19.4	-
RWAL4-2	SS-16	-WALL_4-	13+16	CL	3.1-4.6	A-2-4 (0)	7	NP	NP	9	69	3	19	100	98	23	16.8	-
RWAL4-3	SS-17	-WALL_4-	12+31	1' LT	1.0-2.5	A-2-4 (0)	5	24	8	6	66	16	12	100	99	29	20.4	-
RWAL4-4	SS-18	-WALL_4-	11+45	CL	1.0-2.5	A-2-4 (0)	4	NP	NP	7	71	8	14	100	99	24	25.7	3.9
RWAL4-4	SS-19	-WALL_4-	11+45	CL	28.0-29.5	A-3 (1)	26	NP	NP	62	32	2	4	100	65	8	17.9	-
RWAL4-5	SS-20	-WALL_4-	10+50	CL	8.5-10.0	A-7-6 (17)	1	41	21	2	17	61	20	100	99	84	68.6	-
RWAL4-6	SS-21	-WALL_4-	10+00	CL	3.0-4.5	A-2-4 (0)	6	NP	NP	8	74	2	16	100	98	19	21.6	-
-	ST-1	-L-	516+16	16' LT	4.0-6.0	A-6 (7)	N/A	31	18	6	39	20	35	100	98	57	34.6	-
-	ST-2	-L-	517+11	59' RT	11.0-13.0	A-2-4 (0)	N/A	NP	NP	6	84	1	9	100	98	11	23.6	-
-	ST-3	-L-	517+15	23' RT	9.8-11.8	A-7-6 (36)	N/A	65	44	2	20	27	51	100	99	79	64.2	-

Jorge Dumoncia
 Certification No. 121-01-1108

For all test samples but ST-3

Signed:

Sam F. Jarr
 NCDOT Certification No. 129-04-0411

For test sample ST-3

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SUBSURFACE INVESTIGATION
APPENDIX A
CONSOLIDATION & TRIAXIAL TESTS RESULTS

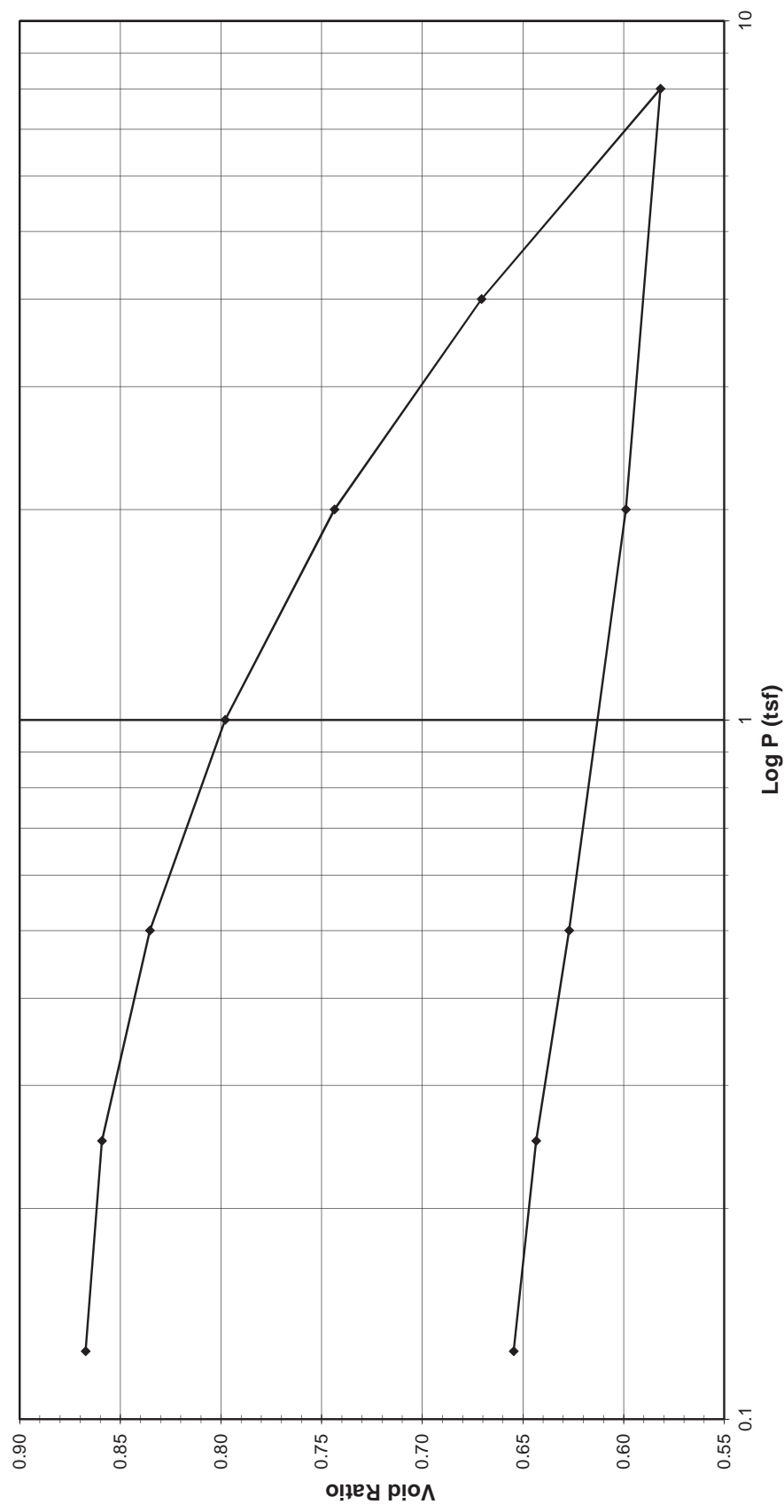
REFERENCE: R-1015

PROJECT: 34360

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 516+16, 16'LT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	4.0-6.0
Project No.	R-2018-095-001	Sample No.	ST-1
Lab ID	R-2018-095-001-002	Visual Description	LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



page 1 of 2
 DCN: CT-24E Date: 5/3/12 Revision: 6
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 Tested By 129-04-0411 Date 4/10/18 Approved By MPS Date 5/15/18
 Z:\2018 PROJECTS\ESP Associates\2018-095 ESP - R-1015 SITE 9\2018-095-001-002 GEOJAC-16TSF1 Cv.xlsm\FINAL PLOT

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 516+16, 16'LT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	4.0-6.0
Project No.	R-2018-095-001	Sample No.	ST-1
Lab ID	R-2018-095-001-002	Visual Description	LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R409
1 Division = 0.0001 (in.)

Sample Properties

	Initial	Final
<i>Water Content</i>		
Tare Number	SS-6	800
Wt. Tare & WS (g)	313.18	232.09
Wt. Tare & DS (g)	267.85	206.50
Wt. Water (g)	45.33	25.59
Wt. Tare (g)	100.77	103.09
Wt. DS (g)	167.08	103.41
Water Content (%)	27.13	24.75
<i>Sample Parameters</i>		
Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.0000	0.8822
Sample Volume (cc)	80.44	70.96
Wt. Wet Sample + Ring (g)	247.89	245.20
Wt. of Ring (g)	104.49	104.49
Wt. of Wet Sample (g)	143.40	140.71
Wet Density (pcf)	111.24	123.73
Wet Density (g/cc)	1.78	1.98
Water Content (%)	27.13	24.75
Wt. of Dry Sample (g)	112.80	112.80
Dry Density (pcf)	87.50	99.19
Dry Density (g/cc)	1.40	1.59
Void Ratio	0.8755	0.6546
Saturation (%)	81.50	99.42
Specific Gravity	2.63	Measured

Test Data Summary

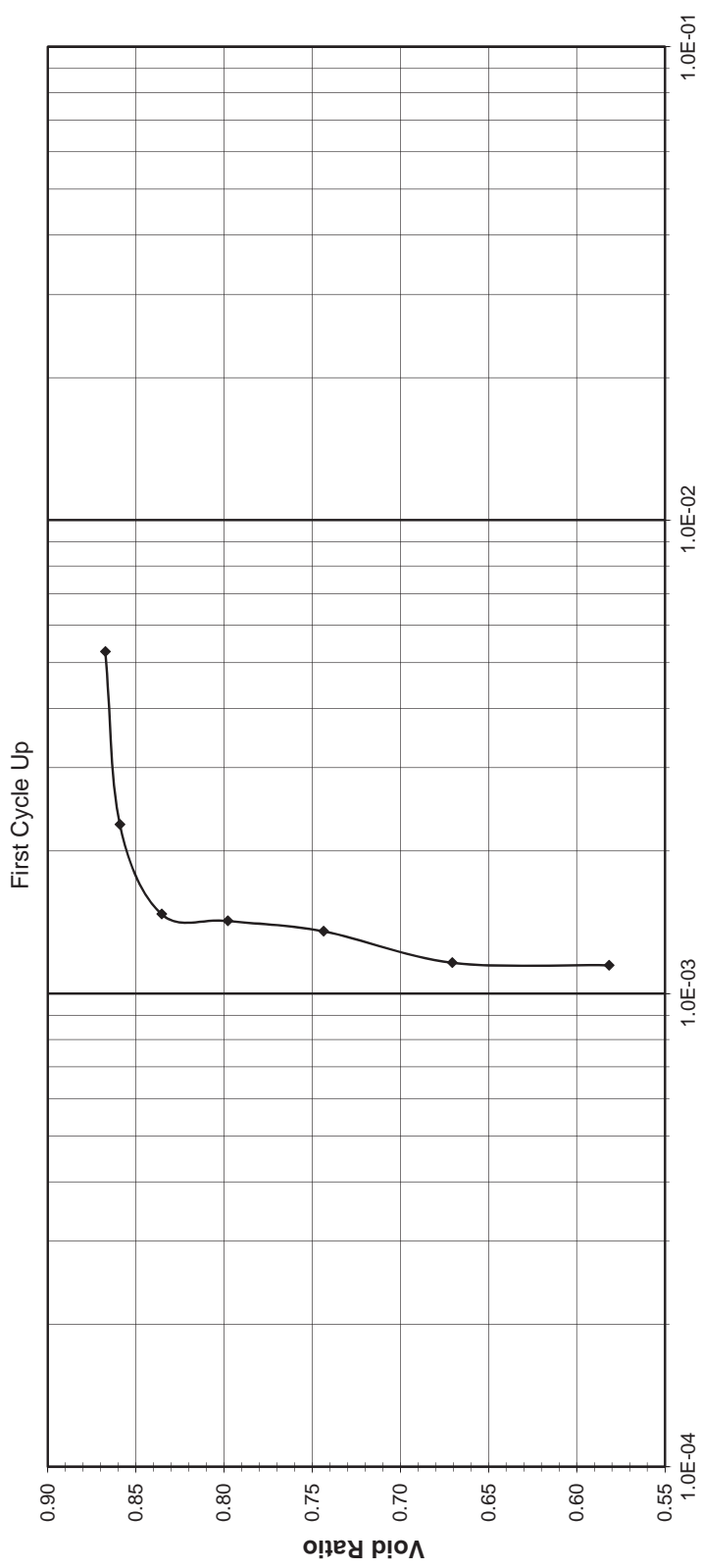
Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio
Seating	0	0	0	25.400	80.440	1.40226	0.87555
0.125	51.5	7.5	44.1	25.288	80.085	1.40846	0.86728
0.25	106.2	18.6	87.6	25.178	79.735	1.41465	0.85912
0.5	247.8	33.9	214.0	24.856	78.719	1.43292	0.83541
1	469.4	55.5	413.9	24.349	77.111	1.46280	0.79793
2	789.6	86.4	703.2	23.614	74.783	1.50832	0.74366
4	1218.3	125.8	1092.4	22.625	71.652	1.57423	0.67066
8	1734.8	168.6	1566.2	21.422	67.841	1.66267	0.58179
2	1584.7	109.7	1475.0	21.654	68.575	1.64487	0.59891
0.5	1391.9	67.4	1324.5	22.036	69.785	1.61634	0.62713
0.25	1297.5	60.3	1237.2	22.258	70.488	1.60024	0.64351
0.125	1238.4	60.3	1178.1	22.408	70.963	1.58951	0.65459

page 2 of 2
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 516+16, 16'LT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	4.0-6.0
Project No.	R-2018-095-001	Sample No.	ST-1
Lab ID	R-2018-095-001-002	Visual Description	LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Coefficient of Consolidation (cm²/sec)

◆ First Cycle Up

page 3 of 4 DCN: CT-24E Date: 5/3/12 Revision: 6 Tested By 129-04-0411 Date 4/10/18 Input Checked By GEM Date 5/15/18

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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 516+16, 16'LT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	4.0-6.0
Project No.	R-2018-095-001	Sample No.	ST-1
Lab ID	R-2018-095-001-002	Visual Description	LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R409

1 Division = 0.0001 (in.)

Sample Properties

Water Content			
Tare Number	SS-6	Initial	Final
Wt. Tare & WS (g)	313.18	800	
Wt. Tare & DS (g)	267.85	232.09	
Wt. Water (g)	45.33	25.59	
Wt. Tare (g)	100.77	103.09	
Wt. DS (g)	167.08	103.41	
Water Content (%)	27.13	24.75	

Sample Parameters

Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.000	0.882
Sample Volume (cc)	80.44	70.96
Wt. of Wet Sample + Ring (g)	247.89	245.20
Wt. of Ring (g)	104.49	104.49
Wt. of Wet Sample (g)	143.40	140.71
Wet Density (pcf)	111.24	123.73
Wet Density (g/cc)	1.78	1.98
Water Content (%)	27.13	24.75
Wt. of Dry Sample (g)	112.80	112.80
Dry Density (pcf)	87.50	99.19
Dry Density (g/cc)	1.40	1.59
Void Ratio	0.8755	0.6546
Saturation (%)	81.50	99.42
Specific Gravity	2.63	Measured

Load Increment (tsf)	Dial Reading @ t ₅₀ (div)	Machine Deflection (div)	C _v Test Data Summary	
			Corrected Dial Reading @ t ₅₀ (div)	Sample Height @ t ₅₀ (cm)
0.0 - 0.125	25.0	7.5	17.5	2.536
0.125 - 0.25	75.4	18.6	56.8	2.526
0.25 - 0.5	173.7	33.9	139.8	2.504
0.5 - 1	358.5	55.5	303.0	2.463
1 - 2	627.9	86.4	541.4	2.402
2 - 4	995.3	125.8	869.4	2.319
4 - 8	1462.1	168.6	1293.5	2.211
8 - 2	NA	109.7	NA	NA
2 - 0.5	NA	67.4	NA	NA
0.5 - 0.25	NA	60.3	NA	NA
0.25 - 0.125	NA	60.3	NA	NA

Time t ₅₀ (min.)	C _v (cm ² /sec)
1.00	0.00528
2.30	0.00228
3.50	0.00147
3.50	0.00142
3.50	0.00135
3.80	0.00116
3.50	0.00115
NA	NA
NA	NA
NA	NA
NA	NA

Tested By 129-04-0411 Date 4/10/18 Input Checked By GEM Date 5/15/18

DCN: CT-24E Date: 5/3/12 Revision: 6

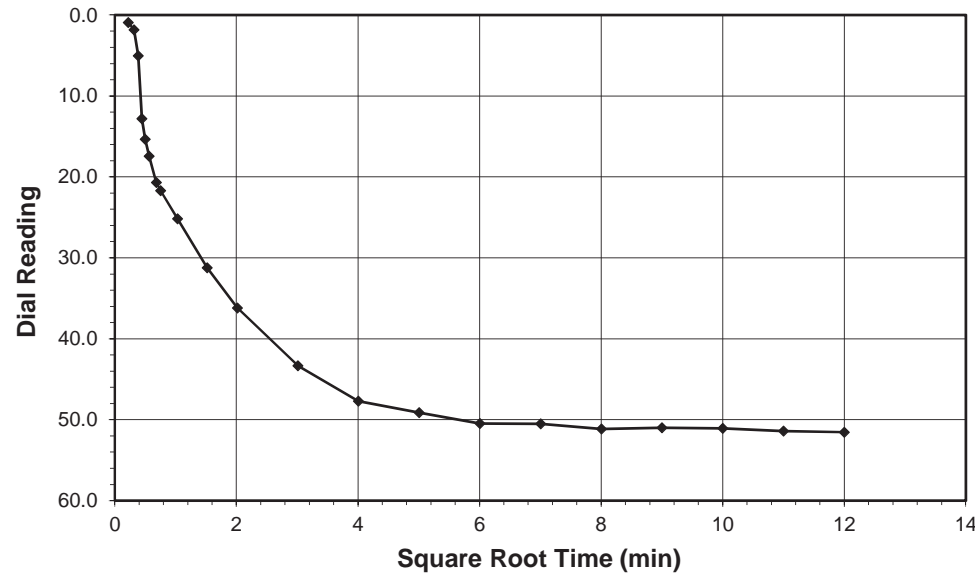
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

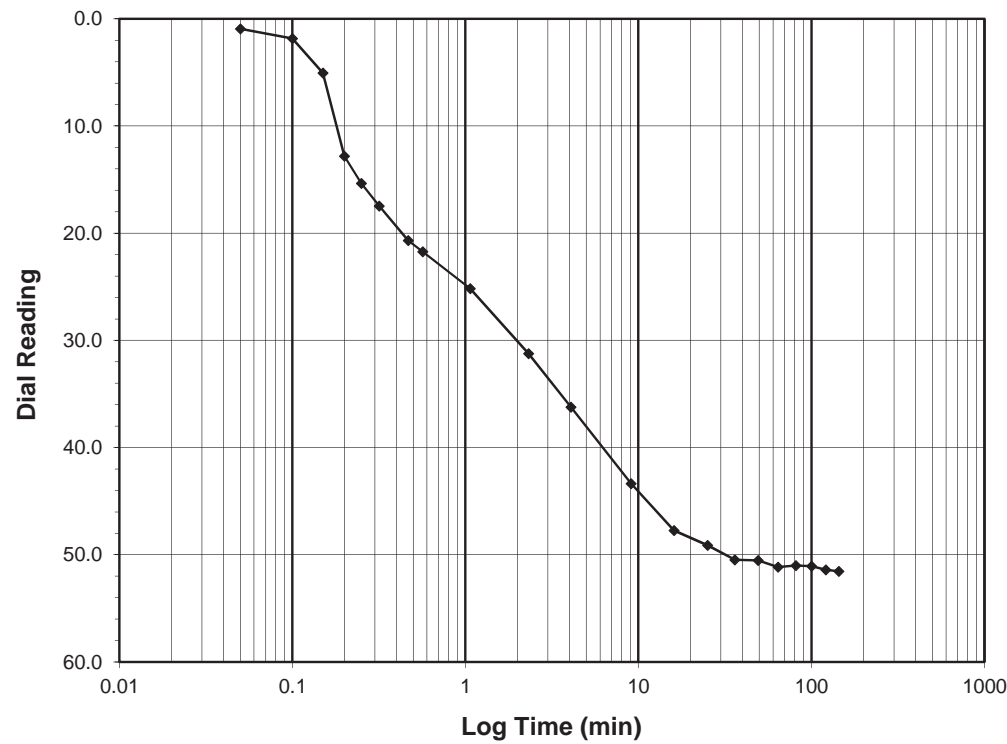
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.0-0.125
Final Reading (div) 51.5
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 4/10/18
 Start Time 17:10:38

Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	0.9
0.10	1.8
0.15	5.1
0.20	12.8
0.25	15.3
0.32	17.5
0.47	20.7
0.57	21.7
1.07	25.2
2.32	31.2
4.07	36.2
9.07	43.4
16.07	47.7
25.07	49.1
36.07	50.5
49.07	50.5
64.07	51.2
81.07	51.0
100.07	51.1
121.07	51.4
144.07	51.5

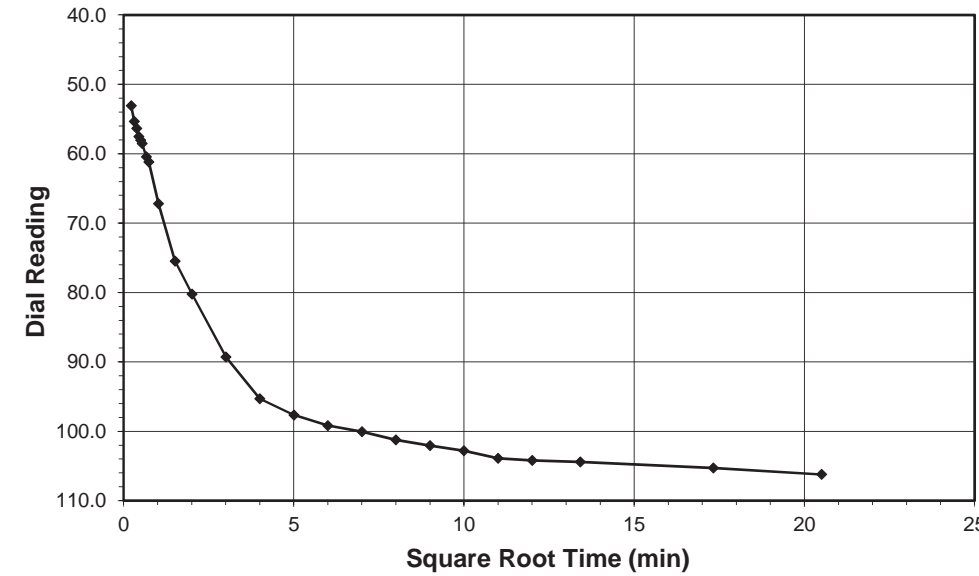


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

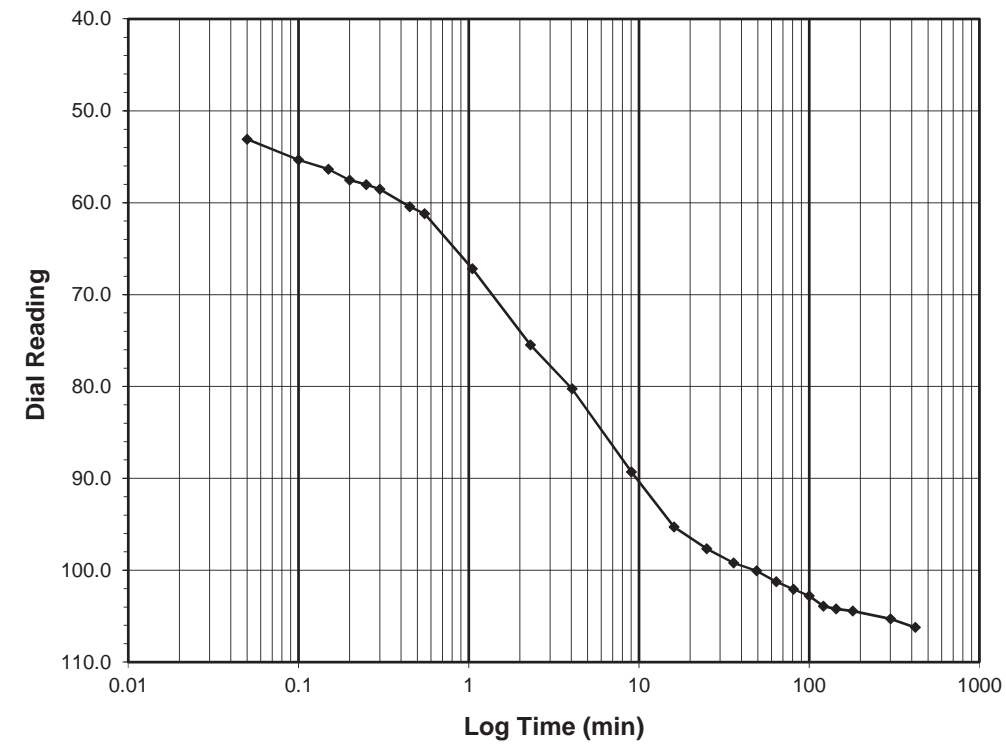
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.125-0.25
Final Reading (div) 106.2
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 4/11/18
 Start Time 0:11:01

Elapsed Time (min)	Dial Reading (div)
Initial	51.5
0.05	53.1
0.10	55.3
0.15	56.3
0.20	57.5
0.25	58.0
0.30	58.5
0.45	60.4
0.55	61.2
1.05	67.2
2.30	75.5
4.05	80.2
9.05	89.3
16.05	95.3
25.05	97.7
36.05	99.2
49.05	100.1
64.05	101.3
81.05	102.1
100.05	102.8
121.07	103.9
144.07	104.2
180.07	104.4
300.07	105.3
420.33	106.2



Tested By 129-04-0411 Date 4/10/18 Checked By GEM Date 5/15/18

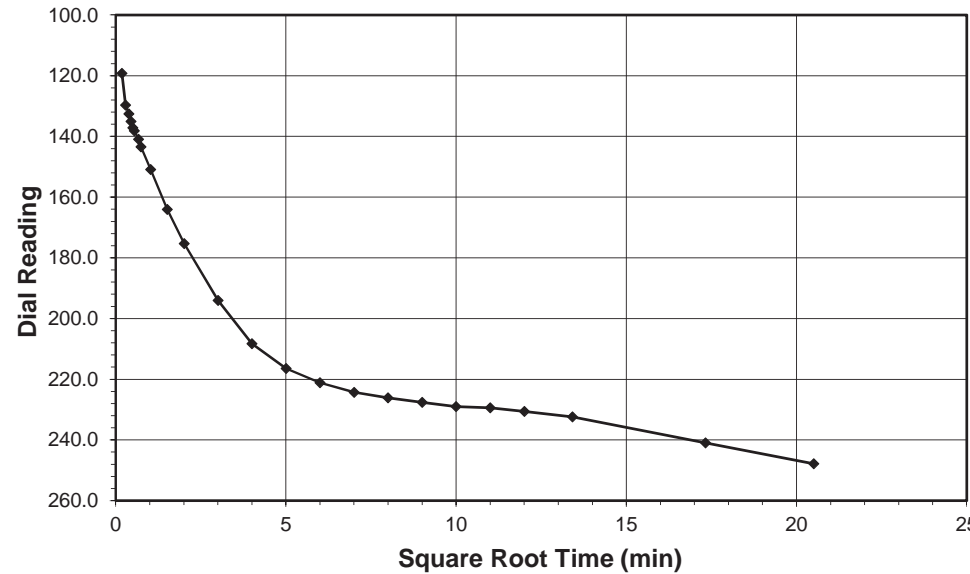
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



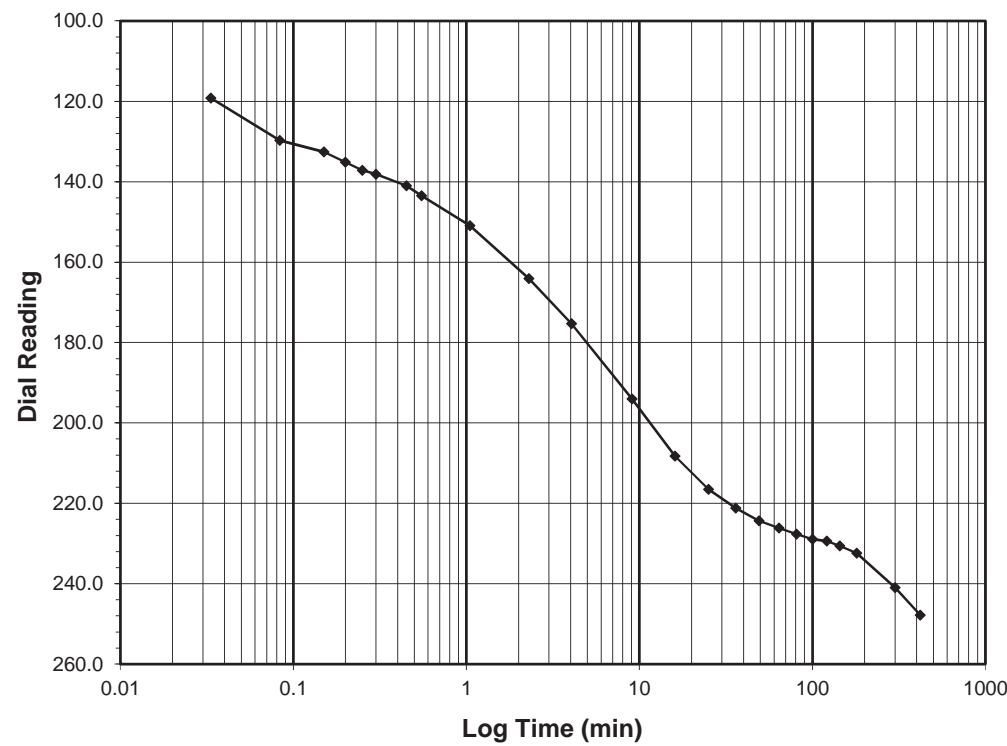
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 247.8
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/11/18
 Start Time 7:11:21

Elapsed Time (min)	Dial Reading (div)
Initial	106.2
0.03	119.2
0.08	129.7
0.15	132.6
0.20	135.1
0.25	137.1
0.30	138.1
0.45	141.0
0.55	143.5
1.05	150.9
2.30	164.1
4.05	175.3
9.05	194.1
16.05	208.3
25.05	216.5
36.05	221.2
49.05	224.3
64.05	226.1
81.05	227.7
100.05	229.0
121.05	229.4
144.05	230.6
180.05	232.4
300.05	241.0
420.38	247.8

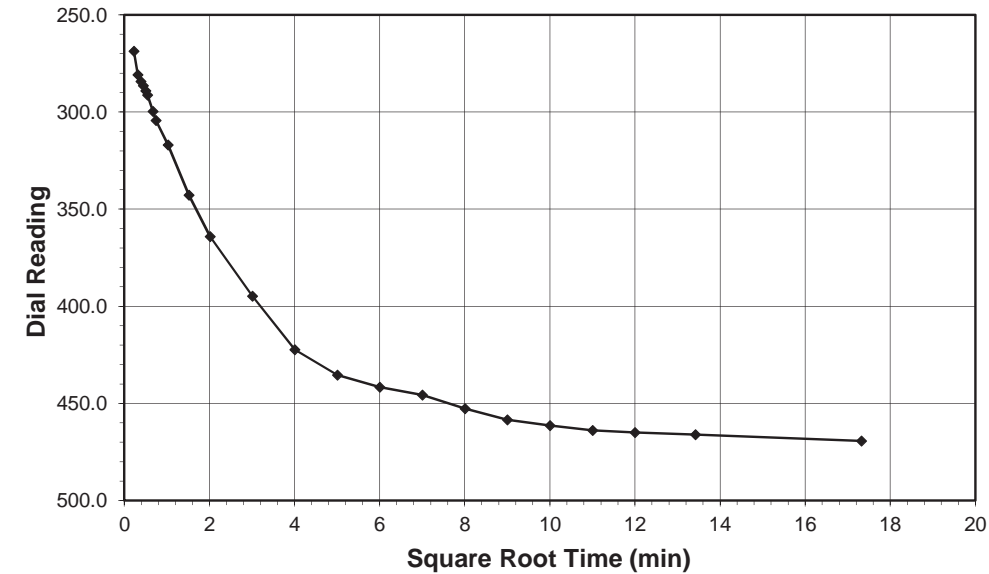


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



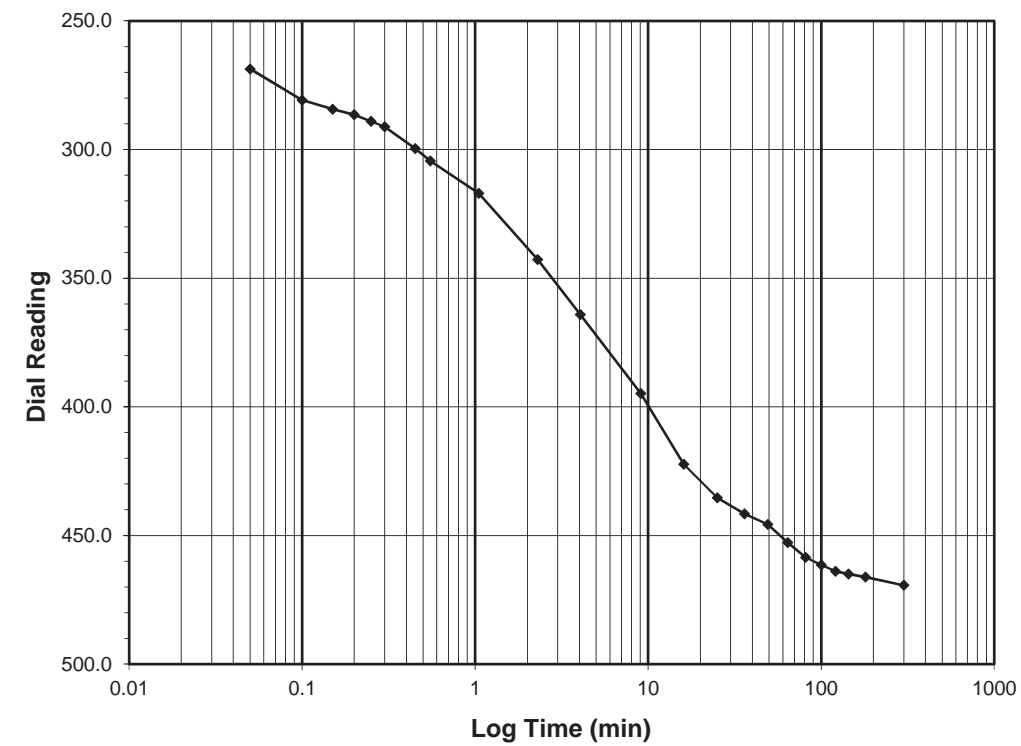
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
 Final Reading (div) 469.4
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/11/18
 Start Time 14:11:46

Elapsed Time (min)	Dial Reading (div)
Initial	247.8
0.05	268.6
0.10	280.8
0.15	284.3
0.20	286.4
0.25	289.0
0.30	291.2
0.45	299.6
0.55	304.4
1.05	317.0
2.30	342.7
4.05	364.2
9.05	394.8
16.05	422.3
25.05	435.3
36.05	441.5
49.05	445.7
64.07	452.7
81.07	458.5
100.07	461.5
121.07	463.9
144.07	465.0
180.07	466.1
300.07	469.4



Tested By 129-04-0411 Date 4/11/18 Checked By GEM Date 5/15/18

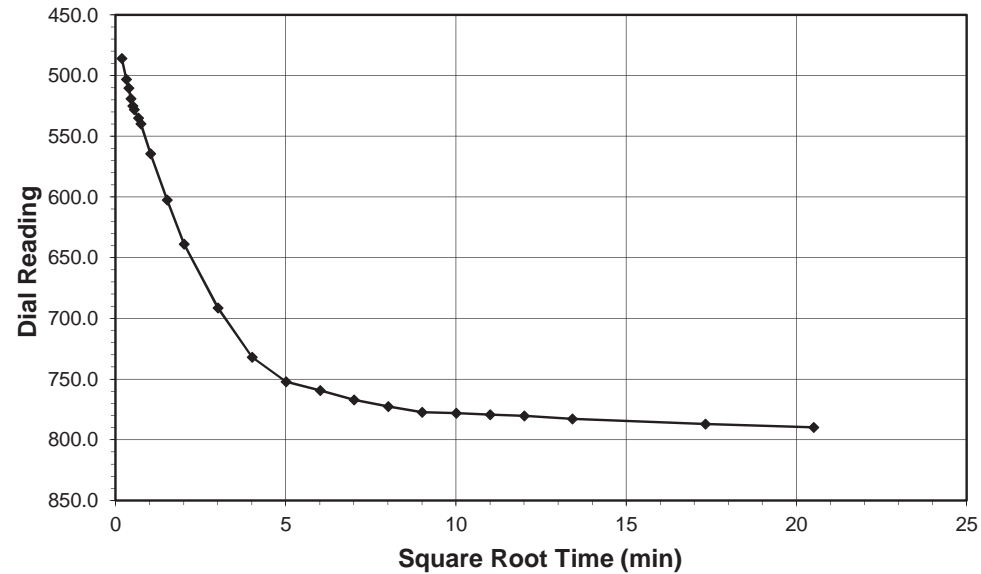
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



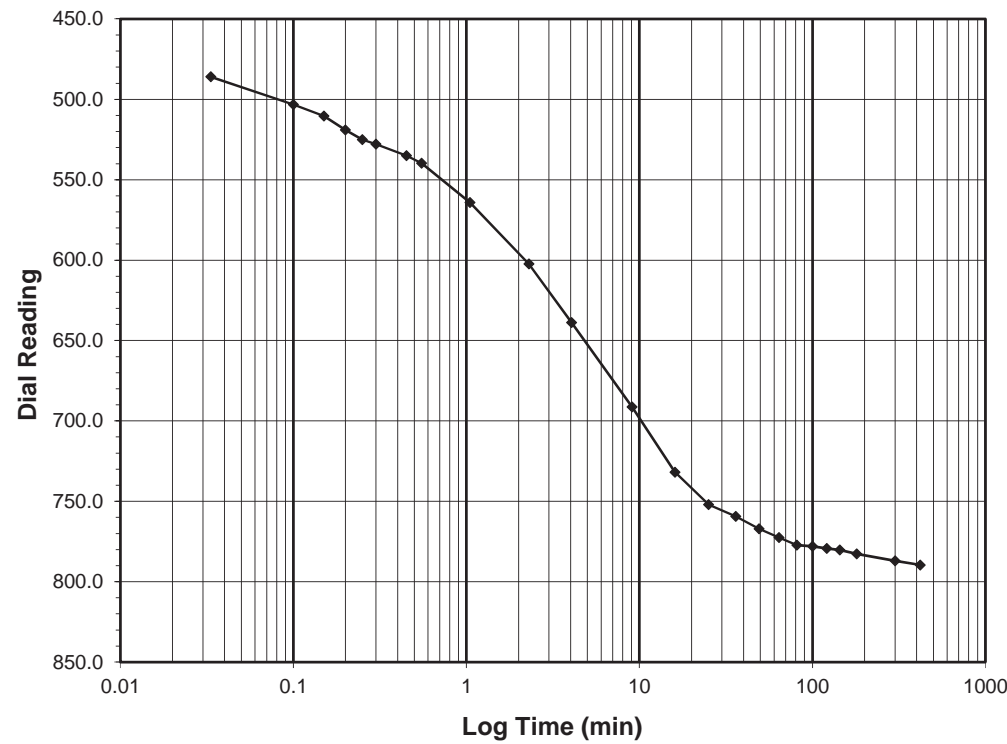
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
 Final Reading (div) 789.6
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/11/18
 Start Time 21:12:10

Elapsed Time (min)	Dial Reading (div)
Initial	469.4
0.03	486.0
0.10	503.2
0.15	510.3
0.20	519.1
0.25	525.0
0.30	527.9
0.45	535.0
0.55	539.6
1.05	564.3
2.30	602.3
4.05	638.8
9.07	691.3
16.07	732.0
25.07	752.0
36.07	759.4
49.07	767.0
64.07	772.5
81.07	777.2
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121.07	779.3
144.07	780.3
180.07	782.7
300.07	786.9
420.35	789.6

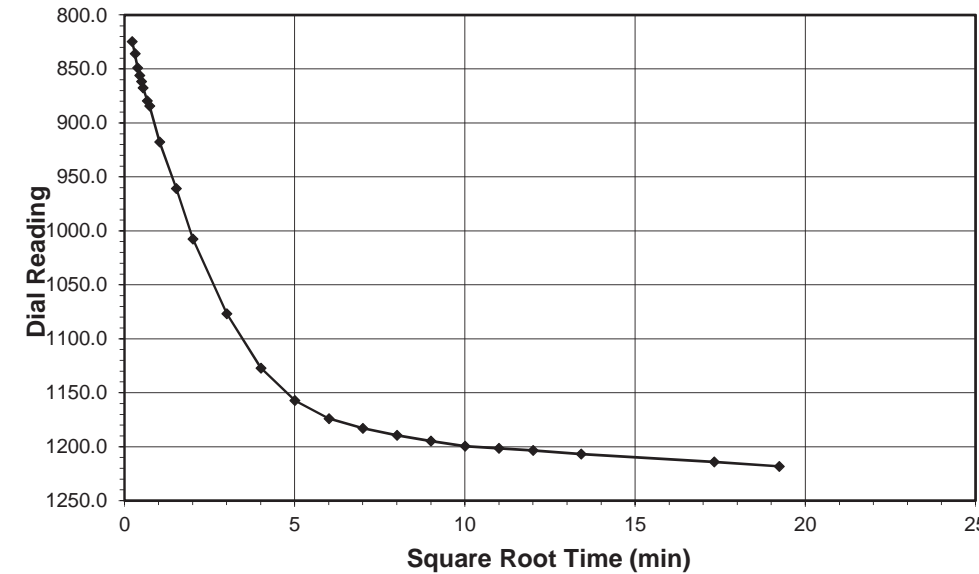


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



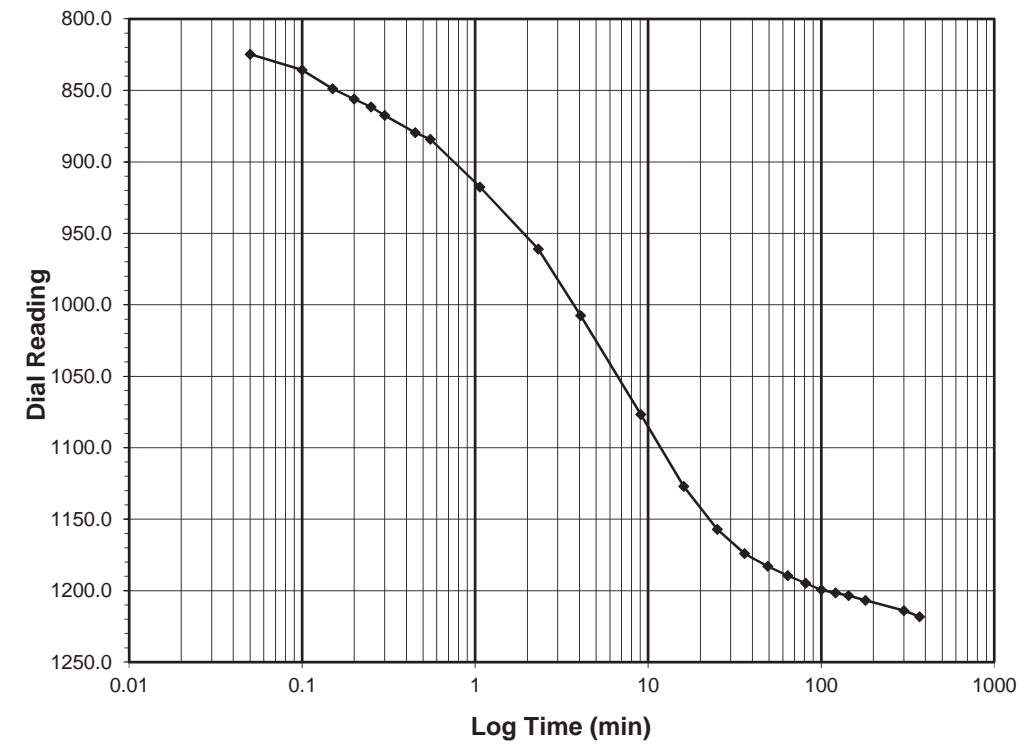
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
 Final Reading (div) 1218.3
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/12/18
 Start Time 4:12:32

Elapsed Time (min)	Dial Reading (div)
Initial	789.6
0.05	824.7
0.10	835.9
0.15	849.0
0.20	856.1
0.25	861.6
0.30	867.5
0.45	879.6
0.55	884.3
1.07	917.7
2.32	960.9
4.07	1007.6
9.07	1076.8
16.07	1127.2
25.07	1157.1
36.07	1174.0
49.07	1183.0
64.07	1189.5
81.07	1194.8
100.07	1199.5
121.07	1201.6
144.07	1203.5
180.07	1206.8
300.07	1214.1
370.07	1218.3



Tested By 129-04-0411 Date 4/11/18 Checked By GEM Date 5/15/18

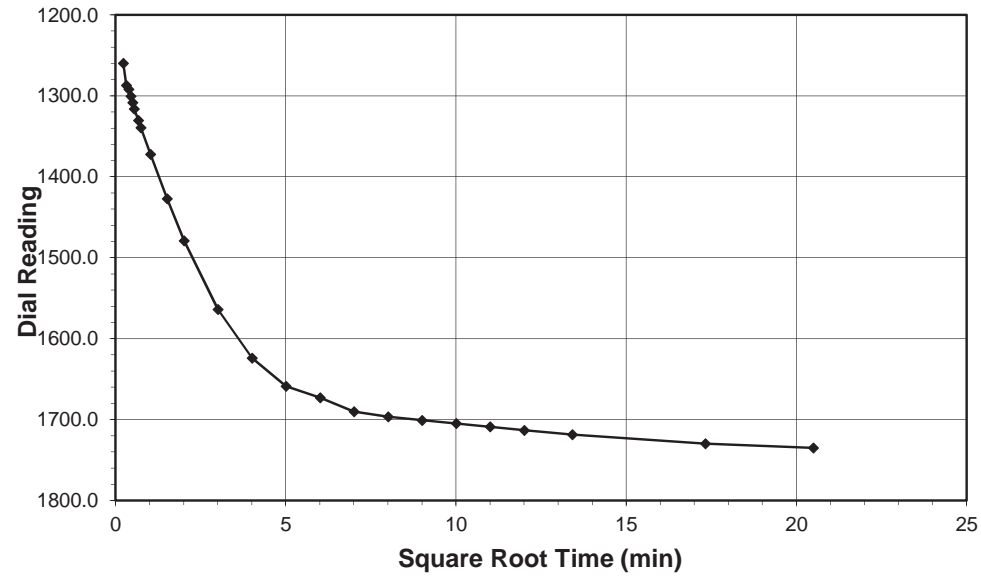
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

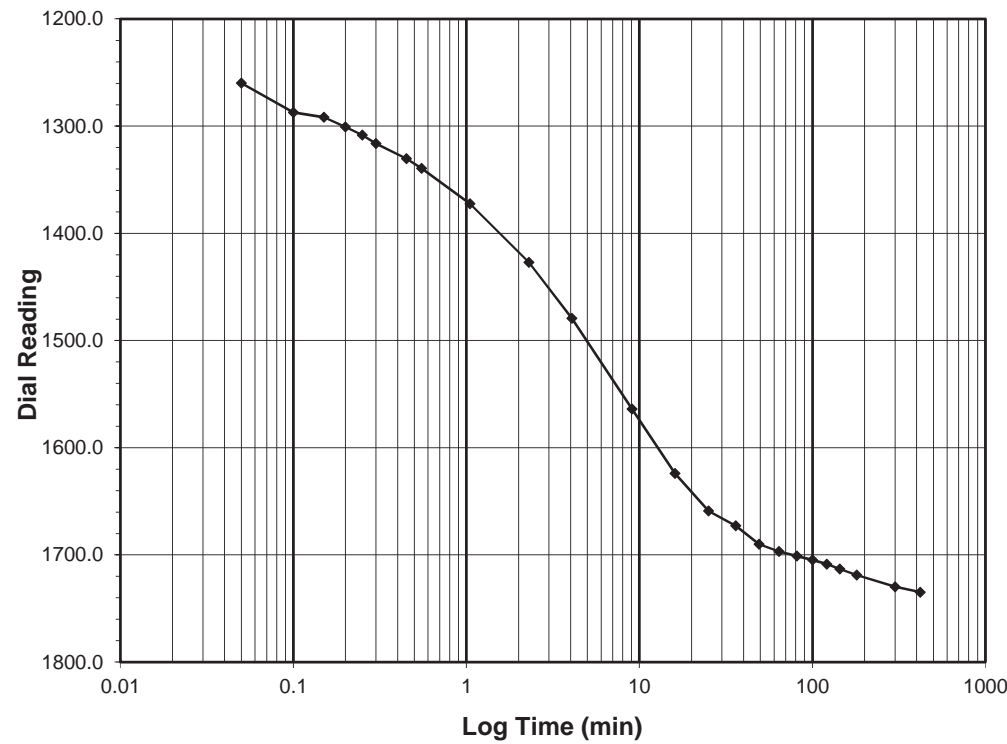
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-8.0
Final Reading (div) 1734.8
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 4/12/18
 Start Time 10:22:37

Elapsed Time (min)	Dial Reading (div)
Initial	1218.3
0.05	1259.8
0.10	1287.1
0.15	1291.8
0.20	1300.6
0.25	1308.3
0.30	1316.3
0.45	1330.3
0.55	1339.3
1.05	1372.4
2.30	1427.1
4.07	1479.2
9.07	1564.0
16.07	1624.1
25.07	1658.9
36.07	1672.8
49.07	1690.1
64.07	1696.7
81.07	1700.9
100.07	1704.6
121.07	1708.6
144.07	1713.2
180.07	1718.7
300.07	1729.9
420.22	1734.8

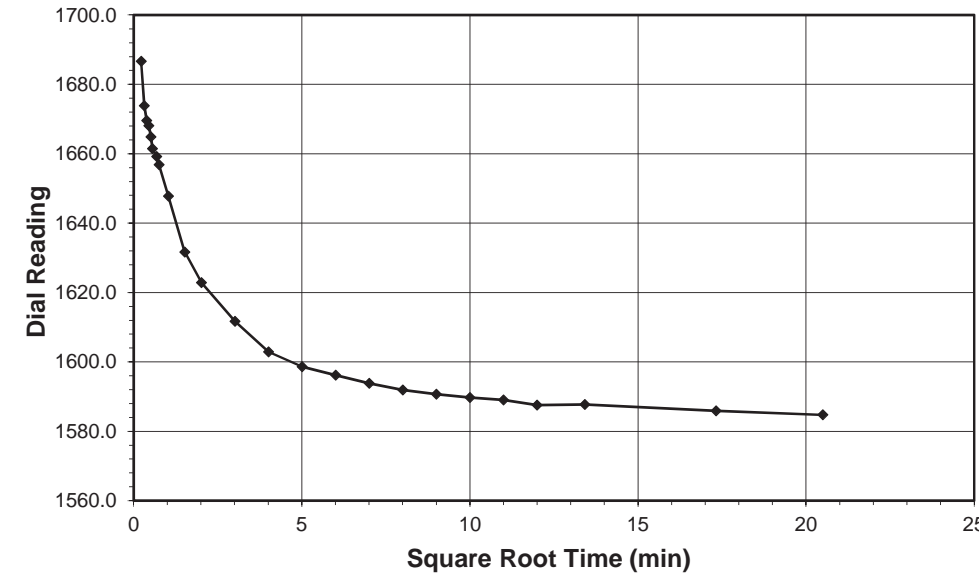


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

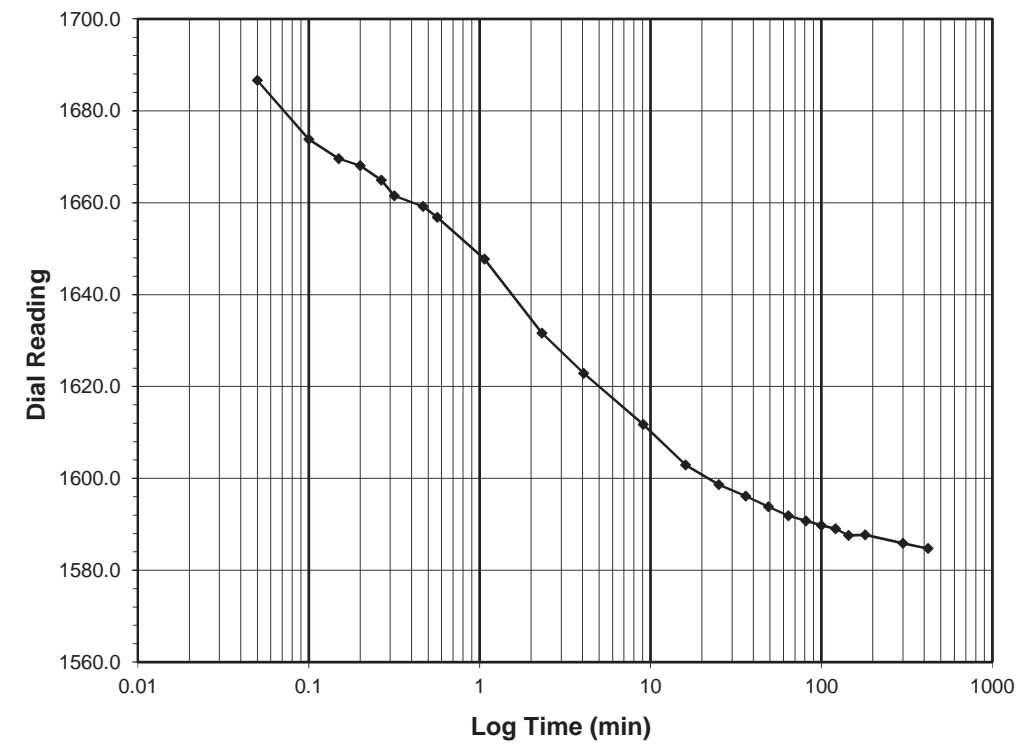
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 8.0-2.0
Final Reading (div) 1584.7
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 4/12/18
 Start Time 17:22:50

Elapsed Time (min)	Dial Reading (div)
Initial	1734.8
0.05	1686.6
0.10	1673.8
0.15	1669.6
0.20	1668.1
0.27	1664.9
0.32	1661.5
0.47	1659.2
0.57	1656.8
1.07	1647.8
2.32	1631.6
4.07	1622.9
9.07	1611.7
16.07	1602.9
25.07	1598.7
36.07	1596.2
49.07	1593.8
64.07	1591.9
81.07	1590.7
100.07	1589.7
121.07	1589.0
144.08	1587.6
180.08	1587.7
300.08	1585.9
420.20	1584.7



Tested By 129-04-0411 Date 4/12/18 Checked By GEM Date 5/15/18

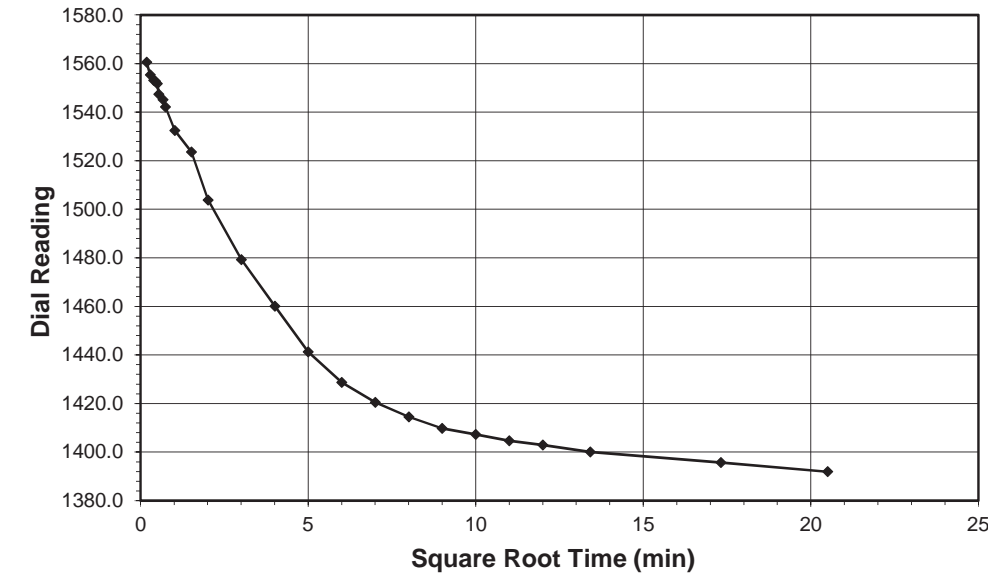
Tested By 129-04-0411 Date 4/12/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



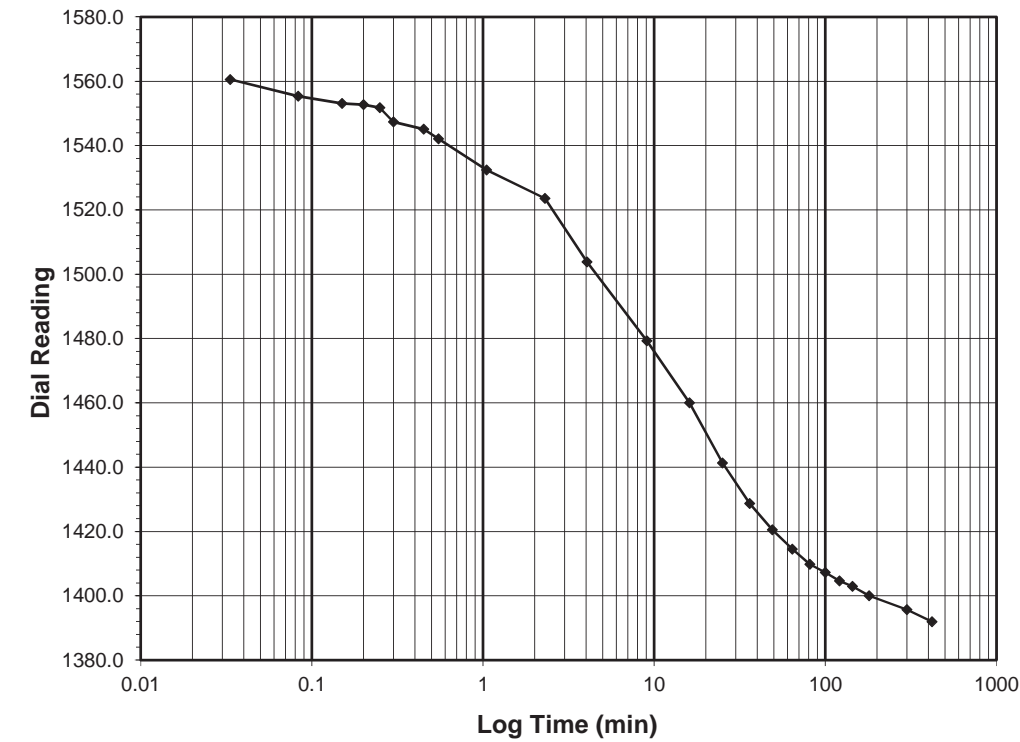
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-0.5
 Final Reading (div) 1391.9
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/13/18
 Start Time 0:23:02

Elapsed Time (min)	Dial Reading (div)
Initial	1584.7
0.03	1560.5
0.08	1555.4
0.15	1553.1
0.20	1552.7
0.25	1551.8
0.30	1547.3
0.45	1545.1
0.55	1542.1
1.05	1532.5
2.30	1523.5
4.05	1503.8
9.05	1479.3
16.05	1460.0
25.05	1441.3
36.05	1428.7
49.05	1420.5
64.05	1414.5
81.05	1409.8
100.05	1407.3
121.05	1404.6
144.05	1402.9
180.05	1400.0
300.05	1395.7
420.33	1391.9

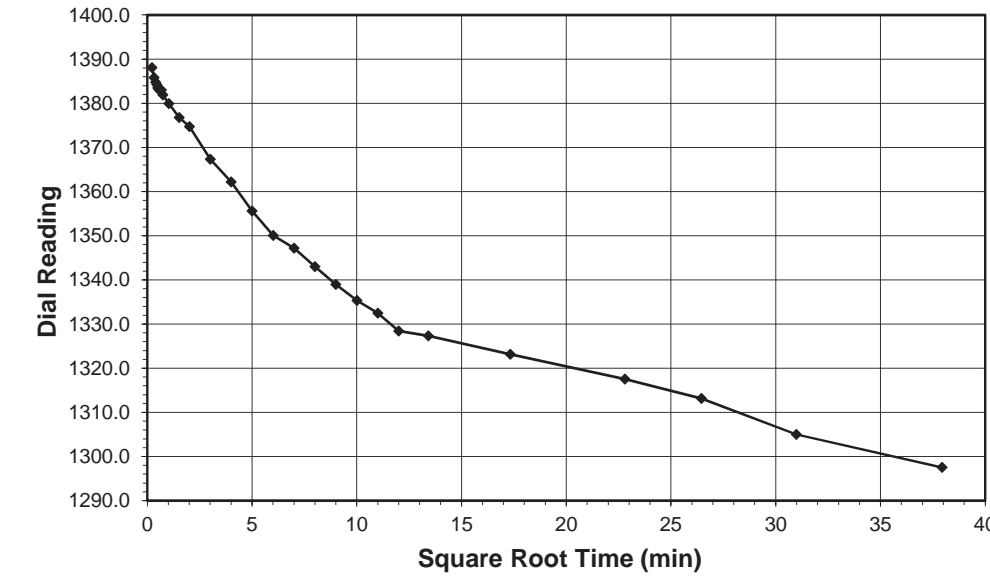


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



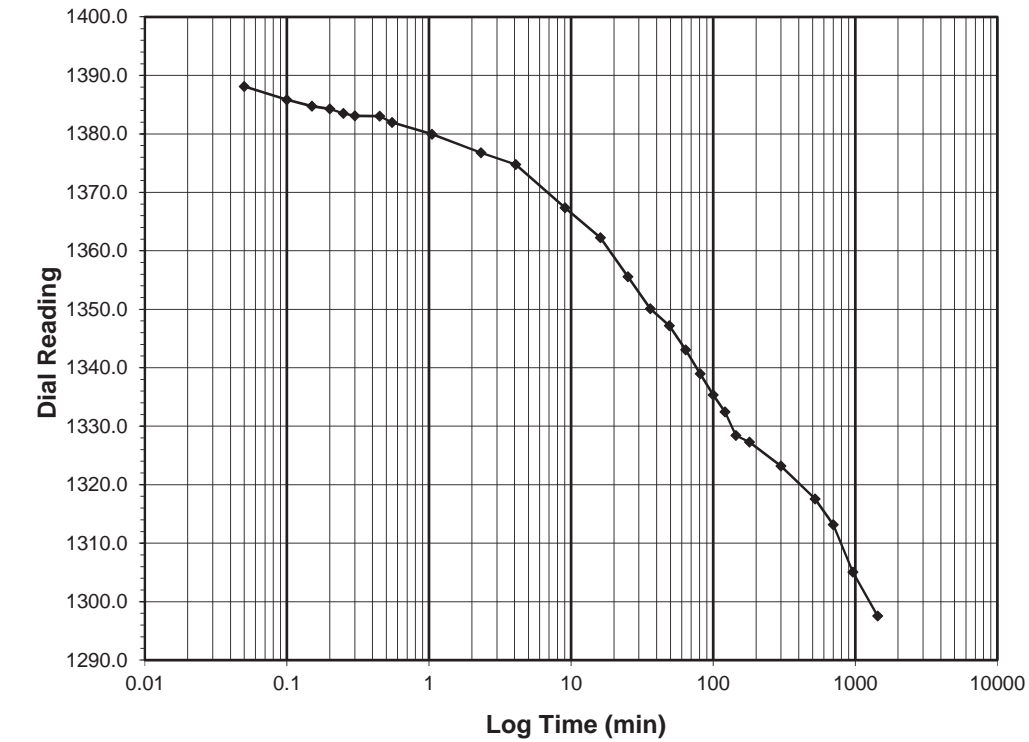
Client ESP Associates Boring No. -L- STA. 516+16, 16'LT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 4.0-6.0
 Project No. R-2018-095-001 Sample No. ST-1
 Lab ID R-2018-095-001-002 Visual Description LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-0.25
 Final Reading (div) 1297.5
 Consolidometer No. R409
 1 Division (in) 0.0001
 Start Date 4/13/18
 Start Time 7:23:23

Elapsed Time (min)	Dial Reading (div)
Initial	1391.9
0.05	1388.1
0.10	1385.8
0.15	1384.8
0.20	1384.3
0.25	1383.5
0.30	1383.1
0.45	1383.0
0.55	1381.9
1.05	1379.9
2.32	1376.8
4.07	1374.7
9.07	1367.3
16.07	1362.2
25.07	1355.6
36.07	1350.1
49.07	1347.2
64.07	1343.0
81.07	1339.0
100.07	1335.3
121.07	1332.4
144.07	1328.4
180.07	1327.3
300.07	1323.2
520.07	1317.6
700.08	1313.2
960.08	1305.0
1440.02	1297.5



Tested By 129-04-0411 Date 4/13/18 Checked By GEM Date 5/15/18

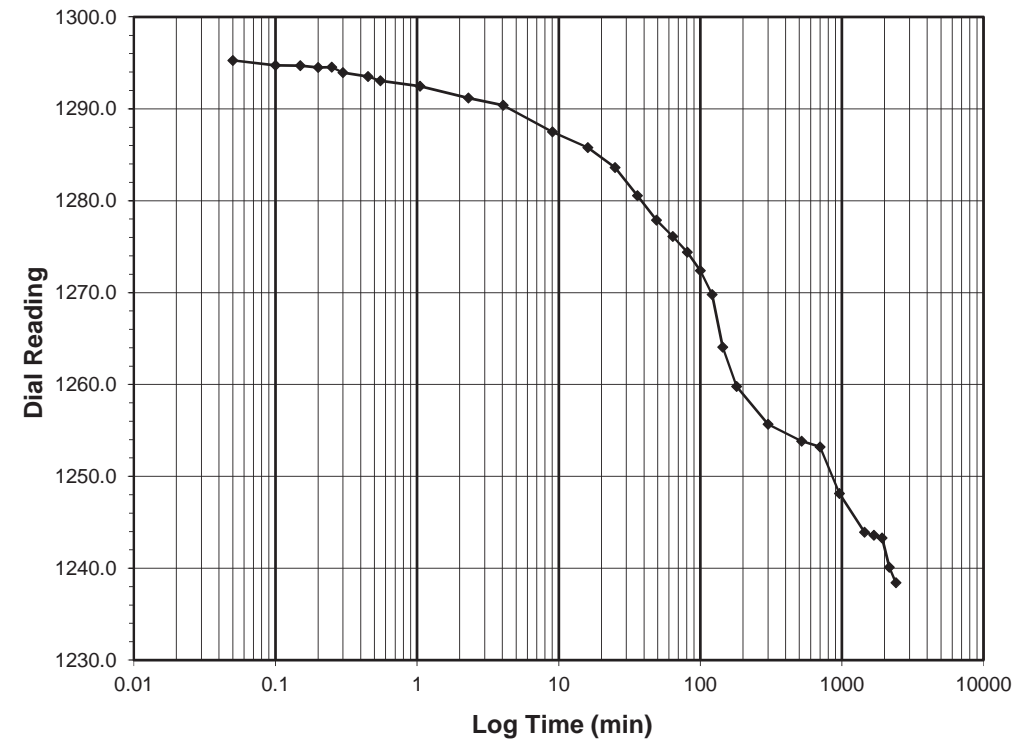
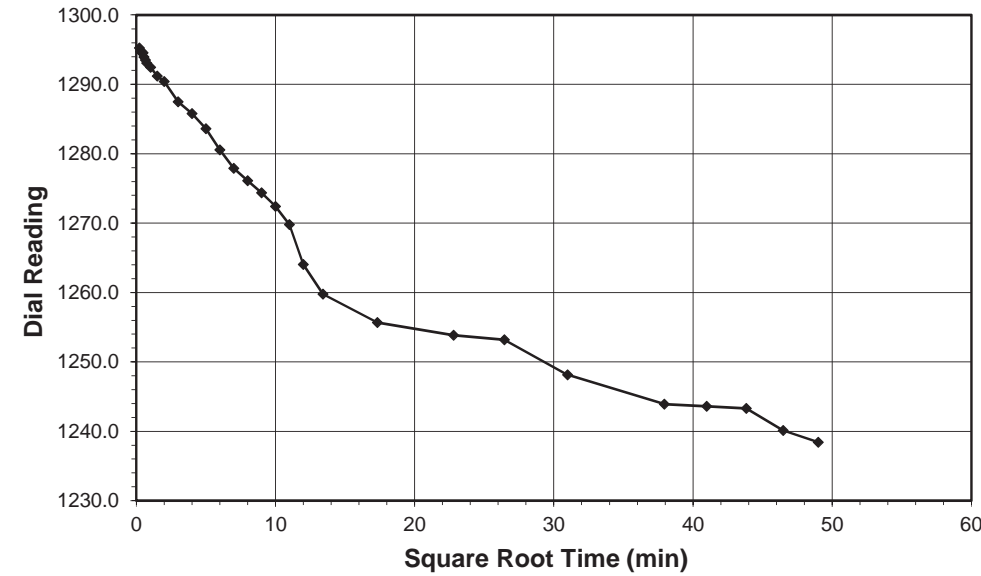
Tested By 129-04-0411 Date 4/13/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client	ESP Associates	Boring No.	-L- STA. 516+16, 16'LT
Client Project	R-1015 Site 9 - CS34.327.00	Depth (ft)	4.0-6.0
Project No.	R-2018-095-001	Sample No.	ST-1
Lab ID	R-2018-095-001-002	Visual Description	LIGHT BROWN / GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.125
Final Reading (div) 1238.4
 Consolidometer No. **R409**
 1 Division (in) 0.0001

Start Date 4/14/18
 Start Time 7:23:25

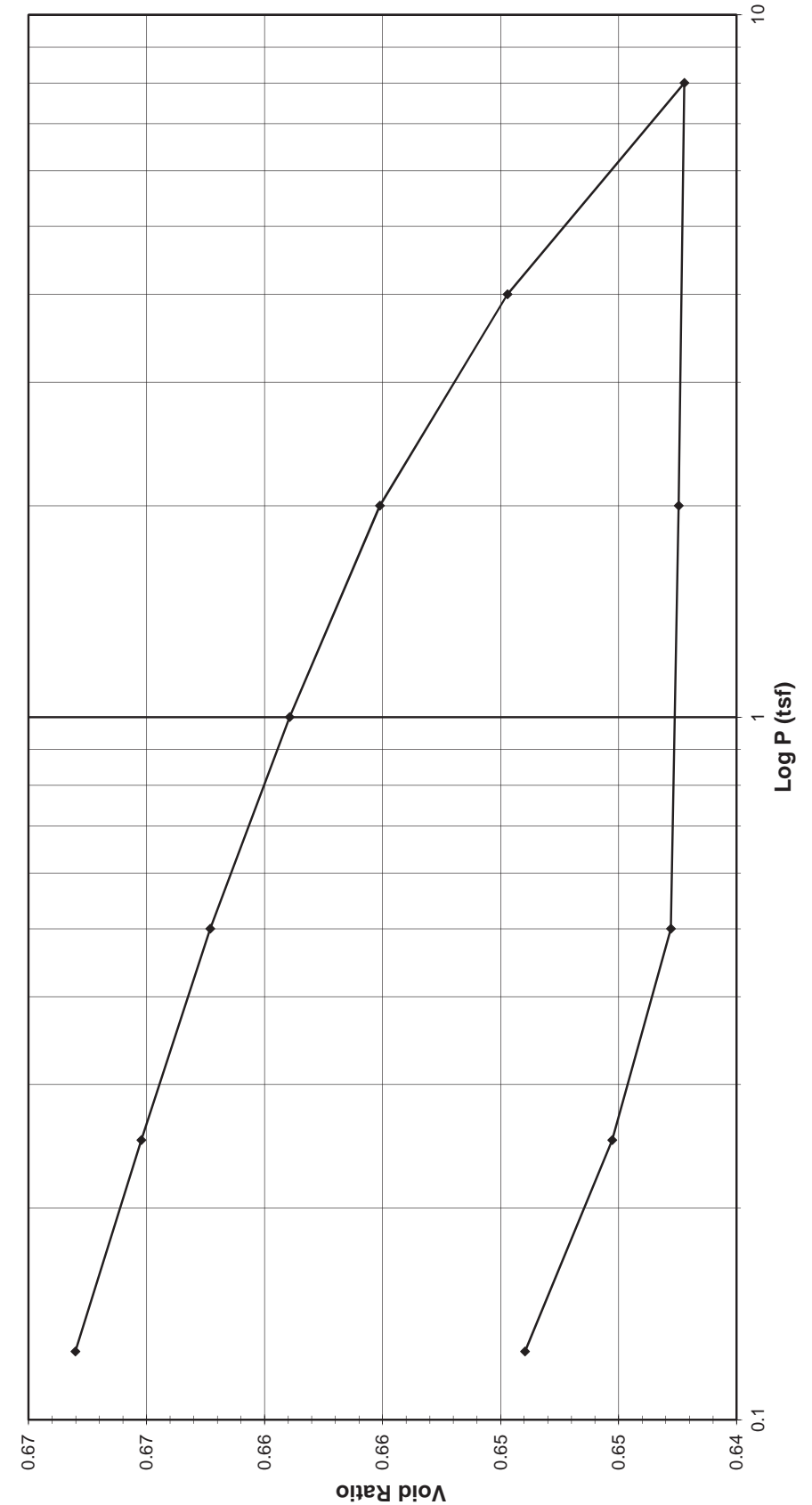
Elapsed Time (min)	Dial Reading (div)
Initial	1297.5
0.05	1295.3
0.10	1294.7
0.15	1294.7
0.20	1294.5
0.25	1294.6
0.30	1294.0
0.45	1293.5
0.55	1293.1
1.05	1292.5
2.30	1291.2
4.05	1290.4
9.05	1287.5
16.05	1285.8
25.05	1283.6
36.05	1280.6
49.05	1277.9
64.05	1276.1
81.05	1274.4
100.05	1272.4
121.05	1269.8
144.05	1264.1
180.05	1259.8
300.05	1255.7
520.05	1253.8
700.05	1253.2
960.05	1248.1
1440.05	1243.9
1680.07	1243.6
1920.05	1243.3
2160.07	1240.1
2400.07	1238.4



ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 517+11, 59'RT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	11.0-13.0
Project No.	R-2018-095-001	Sample No.	ST-2
Lab ID	R-2018-095-001-011	Visual Description	GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 4/14/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 517+11, 59RT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	11.0-13.0
Project No.	R-2018-095-001	Sample No.	ST-2
Lab ID	R-2018-095-001-011	Visual Description	GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R470
1 Division = 0.0001 (in.)

Sample Properties

	Initial	Final
Tare Number	TB-08	815
Wt. Tare & WS (g)	485.43	287.95
Wt. Tare & DS (g)	418.64	259.52
Wt. Water (g)	66.79	28.43
Wt. Tare (g)	135.33	135.83
Wt. DS (g)	283.31	123.69
Water Content (%)	23.57	22.98

Test Data Summary

Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio
Seating	0	0	0	25.400	80.440	1.57952	0.67139
0.125	24.4	4.2	20.2	25.349	80.278	1.58272	0.66802
0.25	49.9	13.1	36.9	25.306	80.143	1.58537	0.66523
0.5	81.7	27.4	54.3	25.262	80.003	1.58815	0.66231
1	125.6	51.0	74.5	25.211	79.840	1.59138	0.65893
2	184.5	87.1	97.4	25.153	79.656	1.59506	0.65511
4	254.4	124.7	129.7	25.071	79.396	1.60028	0.64971
8	337.6	163.1	174.5	24.957	79.036	1.60758	0.64222
2	276.1	103.0	173.1	24.960	79.047	1.60734	0.64246
0.5	227.0	55.9	171.1	24.965	79.063	1.60703	0.64279
0.25	204.7	48.4	156.3	25.003	79.183	1.60460	0.64527
0.125	182.6	48.4	134.2	25.059	79.361	1.60100	0.64897

Water Content

Tare Number	TB-08	815
Wt. Tare & WS (g)	485.43	287.95
Wt. Tare & DS (g)	418.64	259.52
Wt. Water (g)	66.79	28.43
Wt. Tare (g)	135.33	135.83
Wt. DS (g)	283.31	123.69
Water Content (%)	23.57	22.98

Sample Parameters

Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.0000	0.9866
Sample Volume (cc)	80.44	79.36
Wt. Wet Sample + Ring (g)	371.31	370.56
Wt. of Ring (g)	214.30	214.30
Wt. of Wet Sample (g)	157.01	156.26
Wet Density (pcf)	121.80	122.87
Wet Density (g/cc)	1.95	1.97
Water Content (%)	23.57	22.98
Wt. of Dry Sample (g)	127.06	127.06
Dry Density (pcf)	98.56	99.90
Dry Density (g/cc)	1.58	1.60
Void Ratio	0.6714	0.6490
Saturation (%)	92.70	93.50
Specific Gravity	2.64	Measured

page 2 of 2

DCN: CT-24E Date: 5/3/12 Revision: 6

Tested By 129-04-0411 Date 4/17/18 Input Checked By GEM Date 5/15/18

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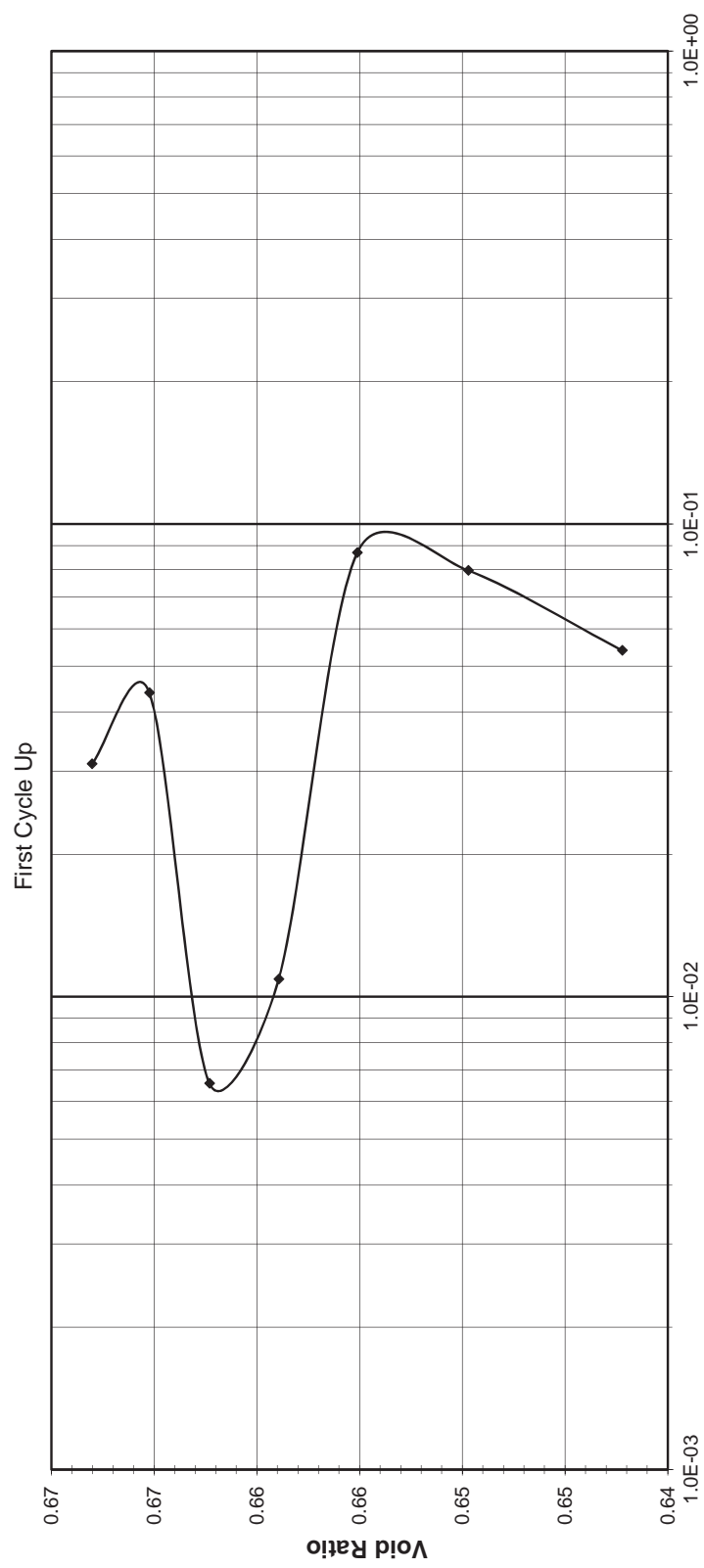
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ONE DIMENSIONAL CONSOLIDATION

AASHTO T-216

Client	ESP Associates	Boring No.	-L- STA. 517+11, 59RT
Client Reference	R-1015 Site 9 - CS34.327.00	Depth (ft)	11.0-13.0
Project No.	R-2018-095-001	Sample No.	ST-2
Lab ID	R-2018-095-001-011	Visual Description	GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Coefficient of Consolidation (cm²/sec)

— First Cycle Up

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client: ESP Associates
 Client Reference: R-1015 Site 9 - CS34.327.00
 Project No.: R-2018-095-001
 Lab ID: R-2018-095-001-011
 Boring No.: -L- STA. 517+11, 59'RT
 Depth (ft): 11.0-13.0
 Sample No.: ST-2
 Visual Description: GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R470
 1 Division = 0.0001 (in.)

Sample Properties	Initial	Final
Water Content		
Tare Number	TB-08	815
Wt. Tare & WS (g)	485.43	287.95
Wt. Tare & DS (g)	418.64	259.52
Wt. Water (g)	66.79	28.43
Wt. Tare (g)	135.33	135.83
Wt. DS (g)	283.31	123.69
Water Content (%)	23.57	22.98
Sample Parameters		
Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.000	0.987
Sample Volume (cc)	80.44	79.36
Wt. Wet Sample + Ring (g)	371.31	370.56
Wt. of Ring (g)	214.30	214.30
Wt. of Wet Sample (g)	157.01	156.26
Wet Density (pcf)	121.80	122.87
Wet Density (g/cc)	1.95	1.97
Water Content (%)	23.57	22.98
Wt. of Dry Sample (g)	127.06	127.06
Dry Density (pcf)	98.56	99.90
Dry Density (g/cc)	1.58	1.60
Void Ratio	0.6714	0.6490
Saturation (%)	92.70	93.50
Specific Gravity	2.64	Measured

Load Increment (tsf)	Dial Reading @ t ₅₀ (div)	Machine Deflection (div)	Corrected Dial Reading @ t ₅₀ (div)	Sample Height @ t ₅₀ (cm)	Time t ₅₀ (min.)	C _v (cm ² /sec)
0.0 - 0.125	12.2	4.2	8.0	2.538	0.17	0.03110
0.125 - 0.25	38.7	13.1	25.6	2.534	0.12	0.04391
0.25 - 0.5	74.2	27.4	46.8	2.528	0.80	0.00656
0.5 - 1	117.3	51.0	66.2	2.523	0.48	0.01089
1 - 2	164.8	87.1	77.6	2.520	0.06	0.08690
2 - 4	235.6	124.7	110.9	2.512	0.07	0.07968
4 - 8	321.7	163.1	158.6	2.500	0.10	0.05399
8 - 2	NA	103.0	NA	NA	NA	NA
2 - 0.5	NA	55.9	NA	NA	NA	NA
0.5 - 0.25	NA	48.4	NA	NA	NA	NA
0.25 - 0.125	NA	48.4	NA	NA	NA	NA

Tested By: 129-04-0411 Date: 4/17/18 Input Checked By: GEM Date: 5/15/18
 Measured By: 129-04-0411 Date: 4/17/18

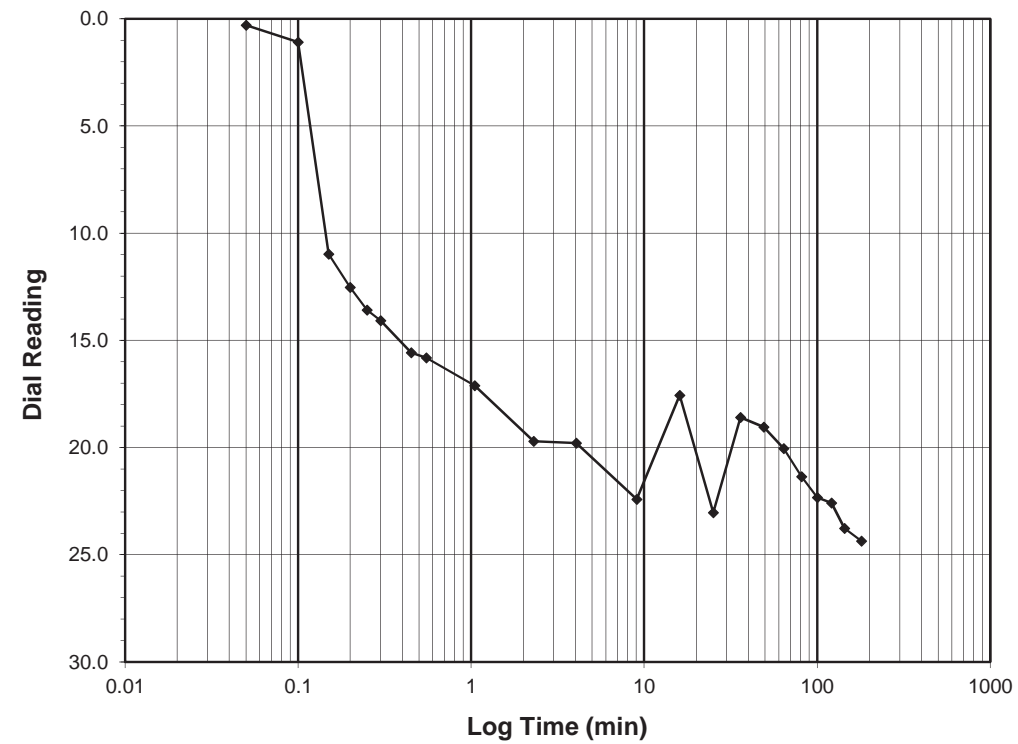
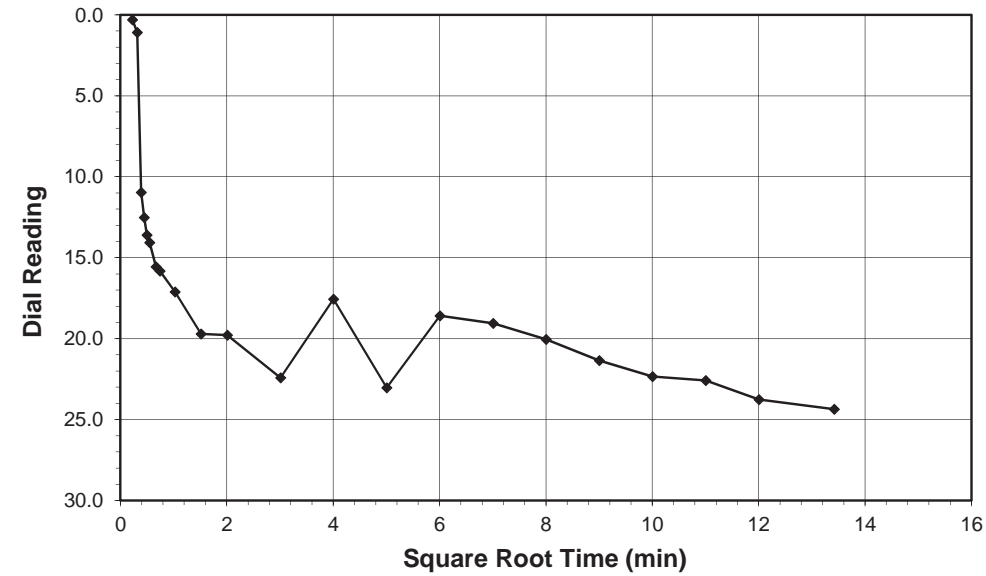
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client: ESP Associates
 Client Project: R-1015 Site 9 - CS34.327.00
 Project No.: R-2018-095-001
 Lab ID: R-2018-095-001-011
 Boring No.: -L- STA. 517+11, 59'RT
 Depth (ft): 11.0-13.0
 Sample No.: ST-2
 Visual Description: GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf): 0.0-0.125
 Final Reading (div): 24.4
 Consolidometer No.: R470
 1 Division (in): 0.0001
 Start Date: 4/17/18
 Start Time: 10:32:22

Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	0.3
0.10	1.1
0.15	11.0
0.20	12.5
0.25	13.6
0.30	14.1
0.35	15.6
0.40	15.8
0.45	15.8
0.50	15.8
0.55	15.8
1.05	17.1
2.30	19.7
4.05	19.8
9.05	22.4
16.05	17.6
25.05	23.0
36.05	18.6
49.07	19.0
64.07	20.0
81.07	21.4
100.07	22.3
121.07	22.6
144.07	23.8
180.07	24.4

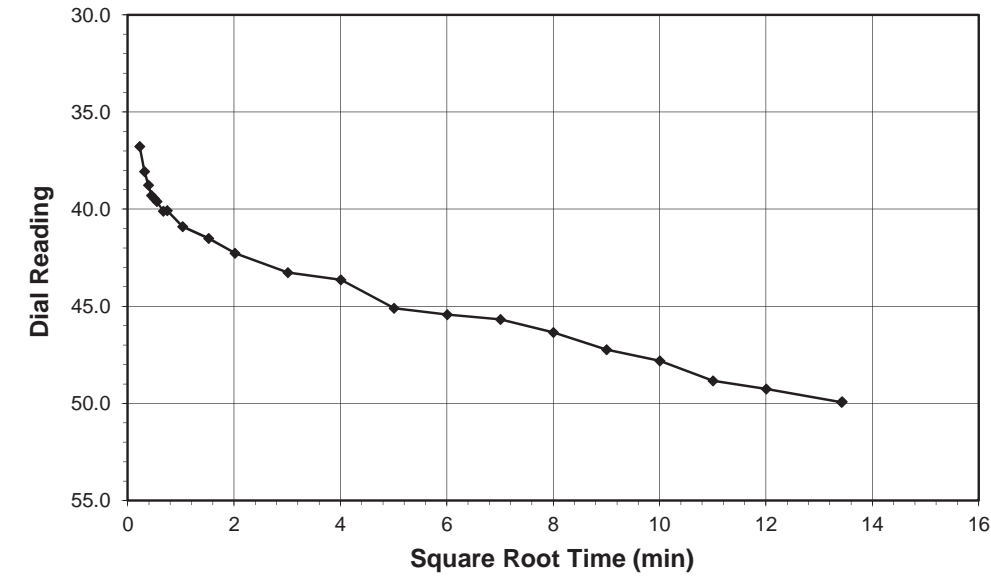
Tested By: 129-04-0411 Date: 4/17/18 Checked By: GEM Date: 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



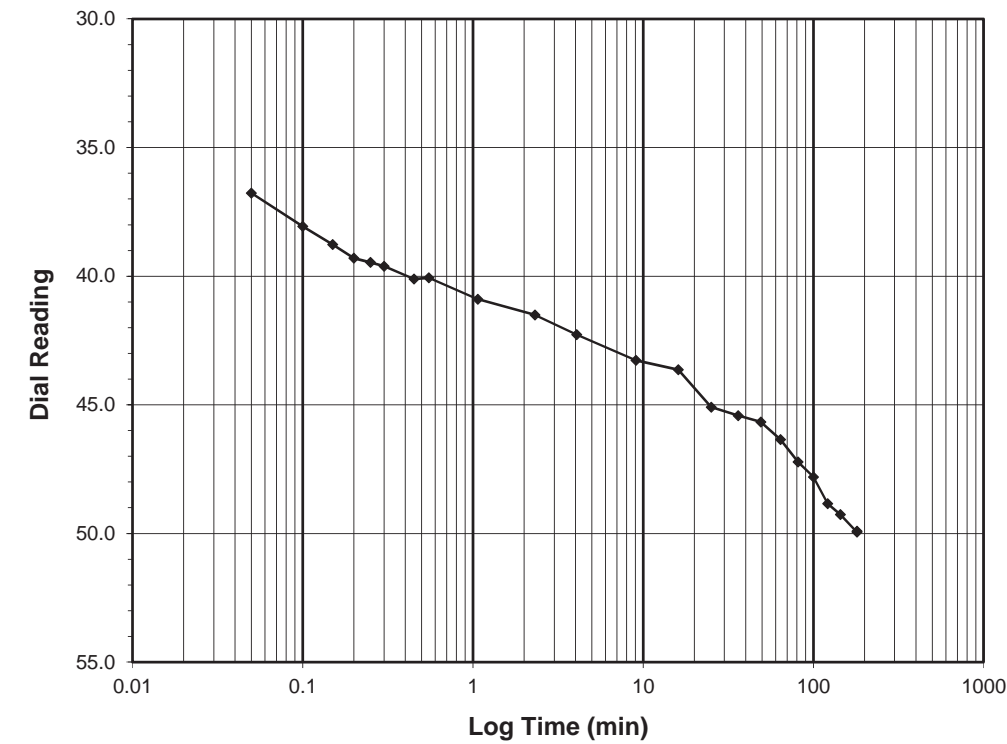
Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.125-0.25
 Final Reading (div) 49.9
 Consolidometer No. R470
 1 Division (in) 0.0001
 Start Date 4/17/18
 Start Time 13:32:46

Elapsed Time (min)	Dial Reading (div)
Initial	24.4
0.05	36.8
0.10	38.1
0.15	38.8
0.20	39.3
0.25	39.5
0.30	39.6
0.45	40.1
0.55	40.1
1.07	40.9
2.32	41.5
4.07	42.3
9.07	43.3
16.07	43.6
25.07	45.1
36.07	45.4
49.07	45.7
64.07	46.4
81.07	47.2
100.07	47.8
121.07	48.8
144.07	49.3
180.07	49.9
180.42	49.9

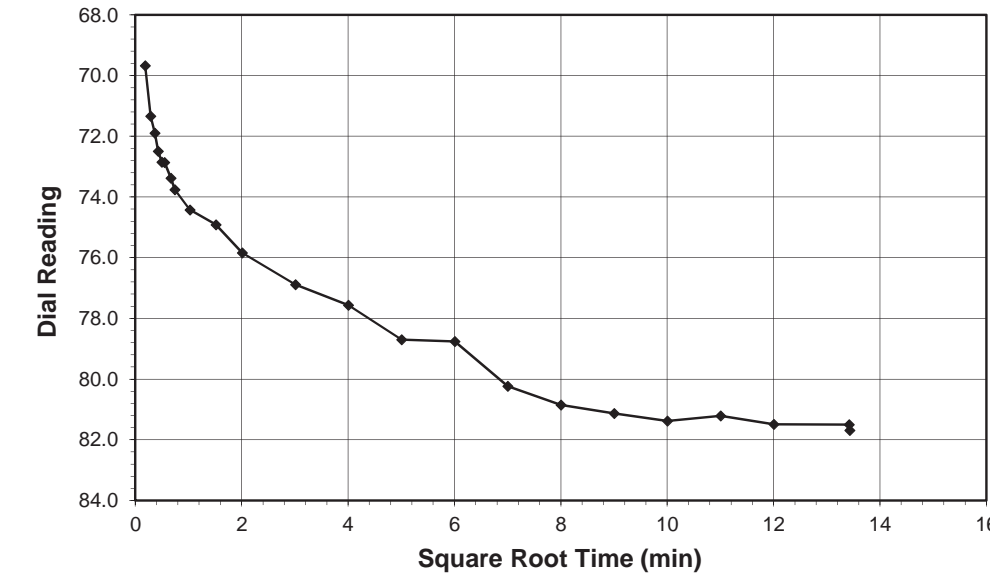


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



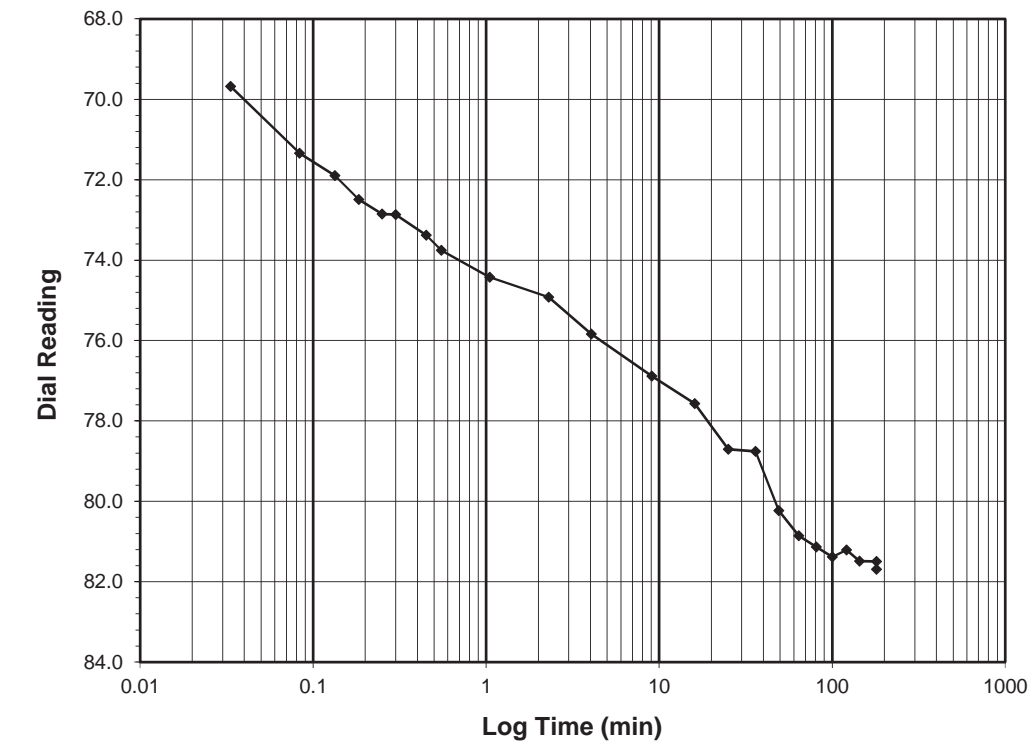
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 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 81.7
 Consolidometer No. R470
 1 Division (in) 0.0001
 Start Date 4/17/18
 Start Time 16:33:11

Elapsed Time (min)	Dial Reading (div)
Initial	49.9
0.03	69.7
0.08	71.3
0.13	71.9
0.18	72.5
0.25	72.9
0.30	72.9
0.45	73.4
0.55	73.8
1.05	74.4
2.30	74.9
4.05	75.8
9.05	76.9
16.05	77.6
25.05	78.7
36.05	78.8
49.05	80.2
64.05	80.9
81.05	81.1
100.05	81.4
121.05	81.2
144.05	81.5
180.05	81.5
180.32	81.7



Tested By 129-04-0411 Date 4/17/18 Checked By GEM Date 5/15/18

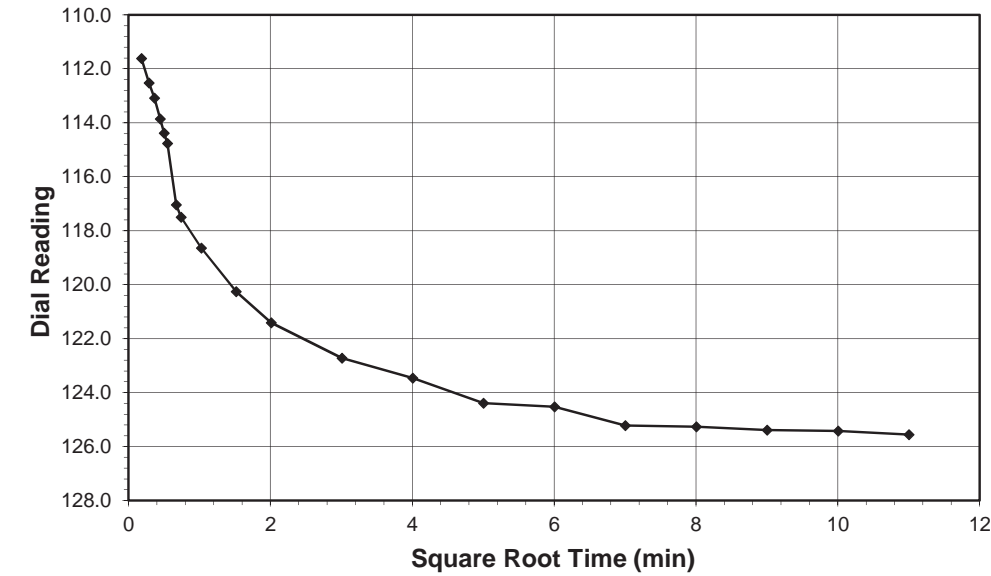
Tested By 129-04-0411 Date 4/17/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



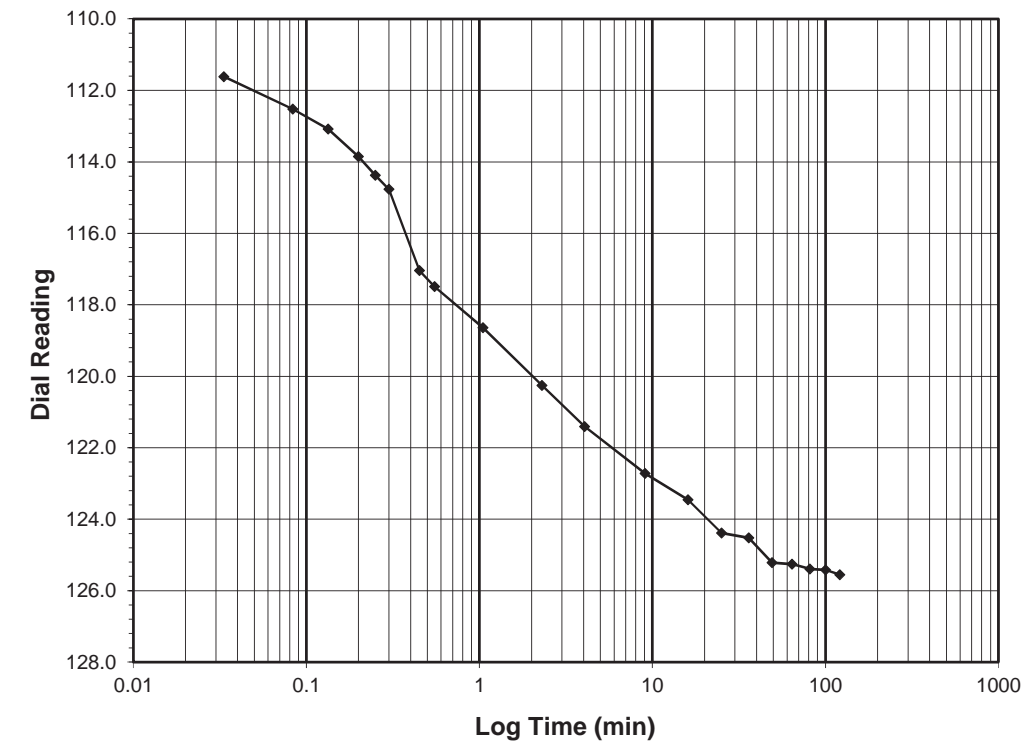
Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
Final Reading (div) 125.6
 Consolidometer No. **R470**
 1 Division (in) 0.0001
 Start Date 4/17/18
 Start Time 19:33:31

Elapsed Time (min)	Dial Reading (div)
Initial	81.7
0.03	111.6
0.08	112.5
0.13	113.1
0.20	113.9
0.25	114.4
0.30	114.8
0.45	117.0
0.55	117.5
1.05	118.6
2.30	120.3
4.05	121.4
9.05	122.7
16.05	123.5
25.05	124.4
36.05	124.5
49.07	125.2
64.07	125.3
81.07	125.4
100.07	125.4
121.07	125.6

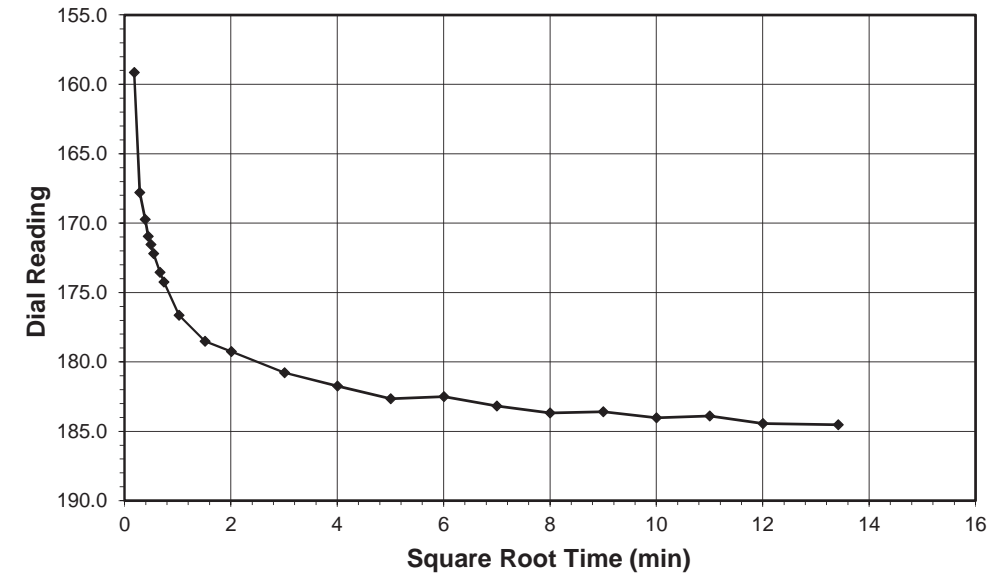


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



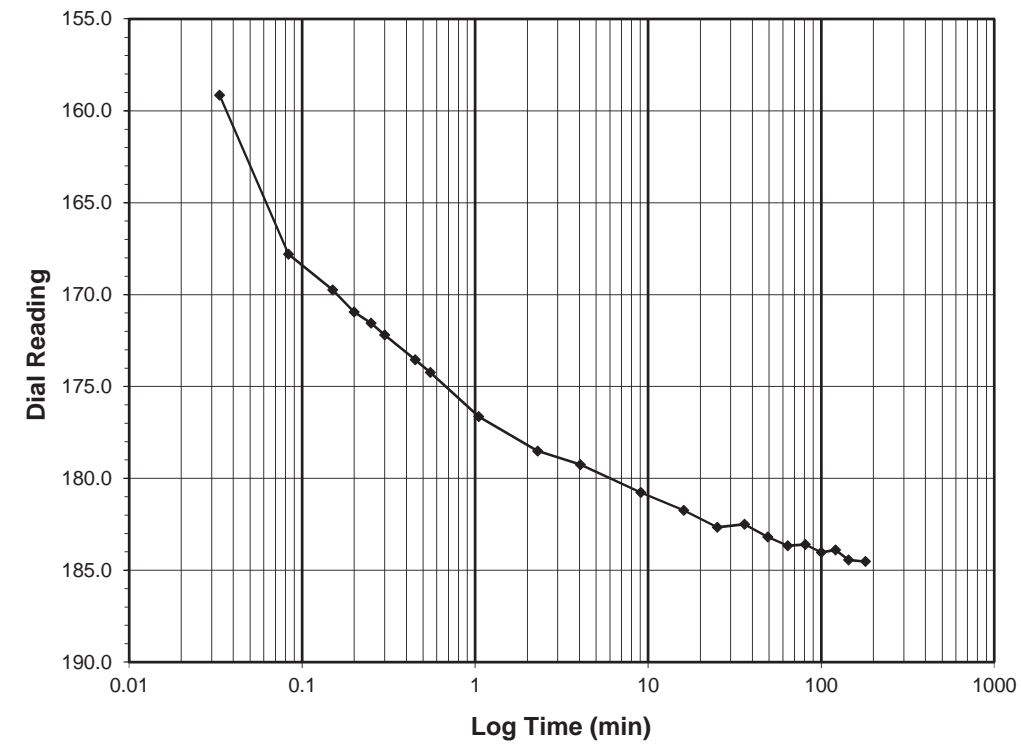
Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
Final Reading (div) 184.5
 Consolidometer No. **R470**
 1 Division (in) 0.0001
 Start Date 4/17/18
 Start Time 22:33:55

Elapsed Time (min)	Dial Reading (div)
Initial	125.6
0.03	159.1
0.08	167.8
0.15	169.7
0.20	170.9
0.25	171.5
0.30	172.2
0.45	173.5
0.55	174.2
1.05	176.6
2.30	178.5
4.05	179.3
9.05	180.8
16.05	181.7
25.05	182.7
36.05	182.5
49.05	183.2
64.05	183.7
81.05	183.6
100.07	184.0
121.07	183.9
144.07	184.4
180.07	184.5



Tested By 129-04-0411 Date 4/17/18 Checked By GEM Date 5/15/18

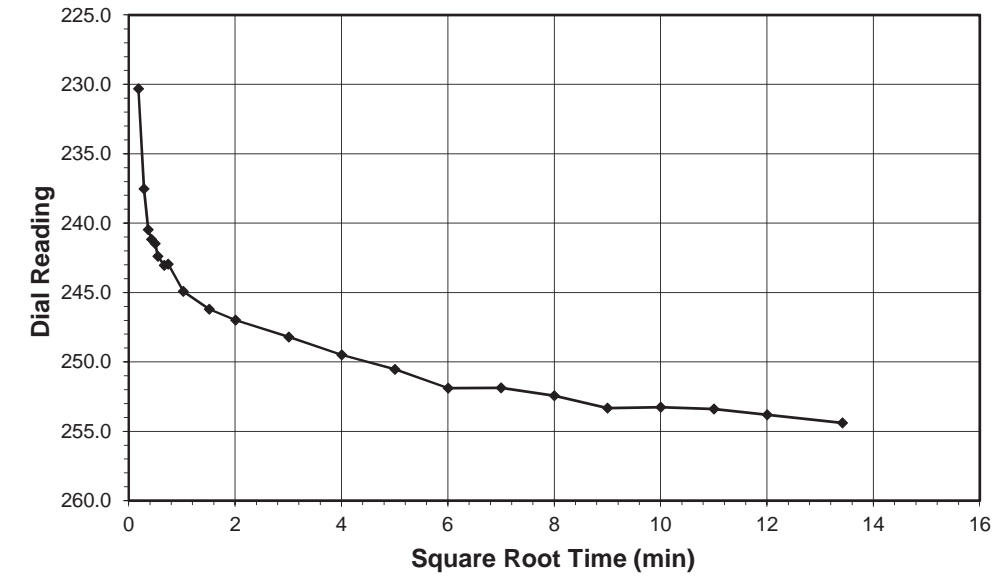
Tested By 129-04-0411 Date 4/17/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

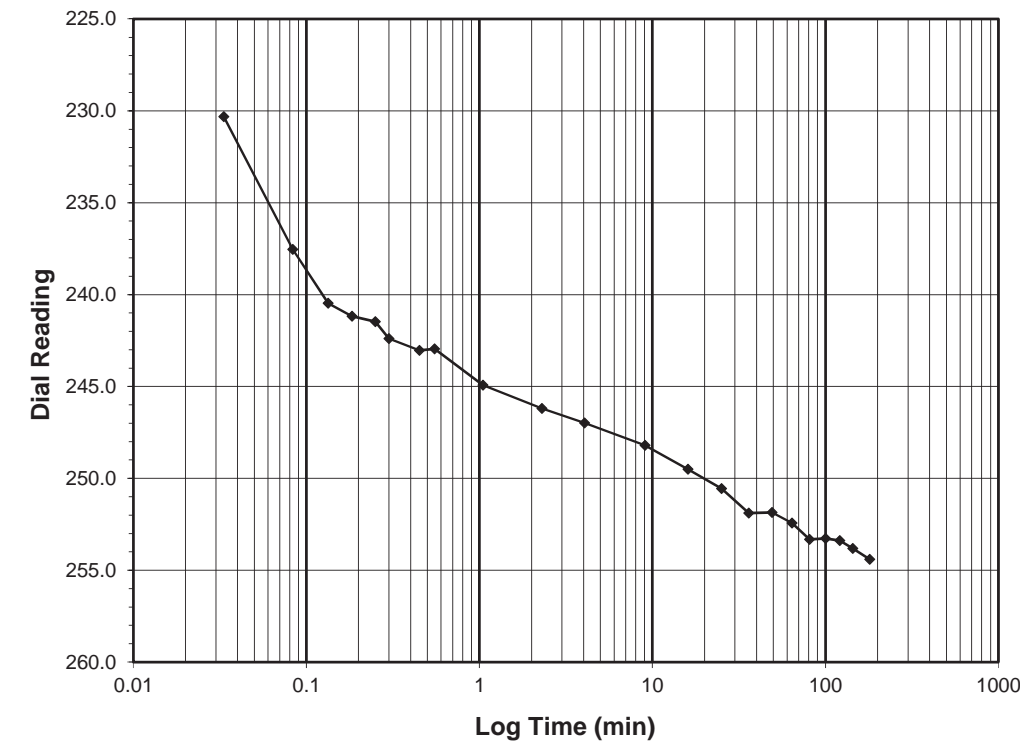
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-4.0
Final Reading (div) 254.4
 Consolidometer No. **R470**
 1 Division (in) 0.0001

Start Date 4/18/18
 Start Time 1:34:20

Elapsed Time (min)	Dial Reading (div)
Initial	184.5
0.03	230.3
0.08	237.5
0.13	240.5
0.18	241.2
0.25	241.5
0.30	242.4
0.45	243.0
0.55	242.9
1.05	244.9
2.30	246.2
4.05	247.0
9.05	248.2
16.05	249.5
25.05	250.5
36.05	251.9
49.05	251.9
64.05	252.4
81.05	253.3
100.05	253.3
121.05	253.4
144.05	253.8
180.05	254.4

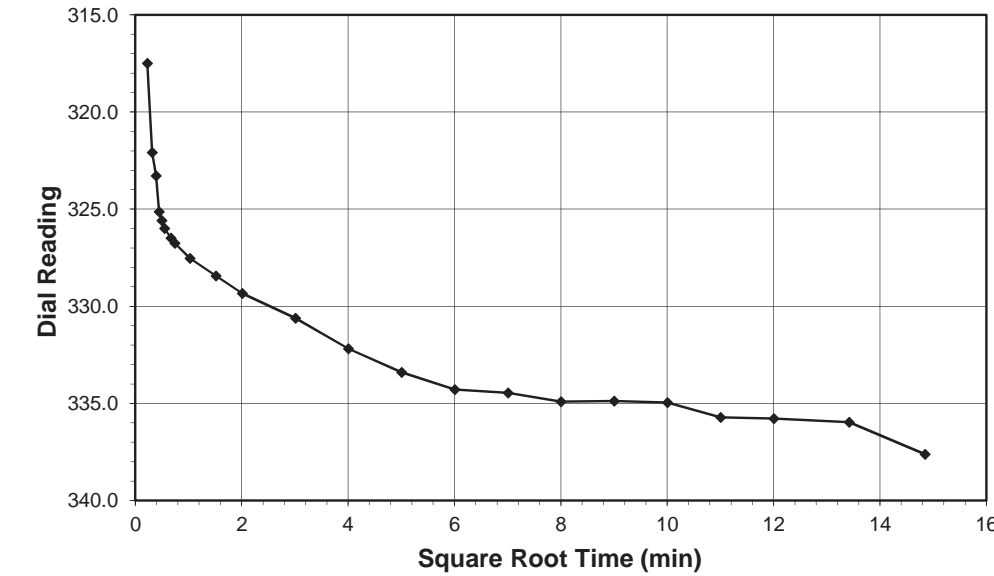


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

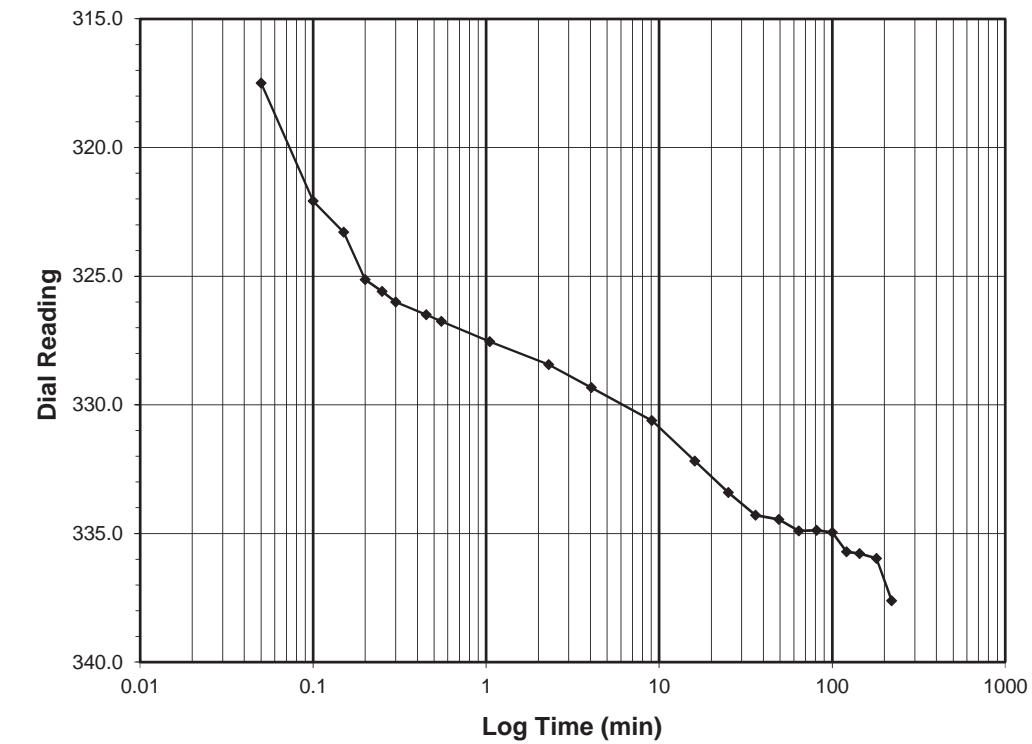
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 4.0-8.0
Final Reading (div) 337.6
 Consolidometer No. **R470**
 1 Division (in) 0.0001

Start Date 4/18/18
 Start Time 4:34:44

Elapsed Time (min)	Dial Reading (div)
Initial	254.4
0.05	317.5
0.10	322.1
0.15	323.3
0.20	325.1
0.25	325.6
0.30	326.0
0.45	326.5
0.55	326.8
1.05	327.5
2.30	328.4
4.05	329.3
9.07	330.6
16.07	332.2
25.07	333.4
36.07	334.3
49.07	334.5
64.07	334.9
81.07	334.9
100.07	335.0
121.07	335.7
144.07	335.8
180.07	336.0
220.47	337.6



Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18

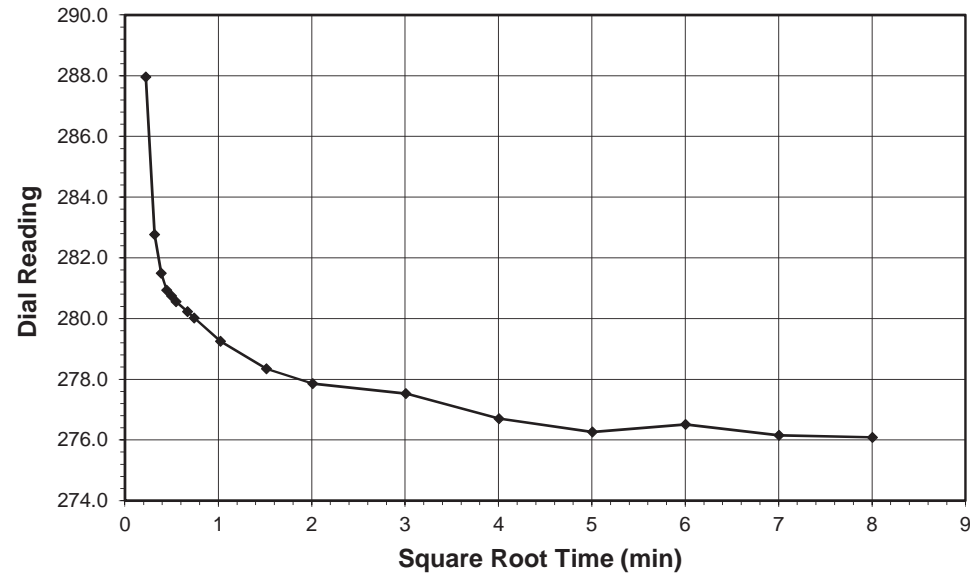
Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



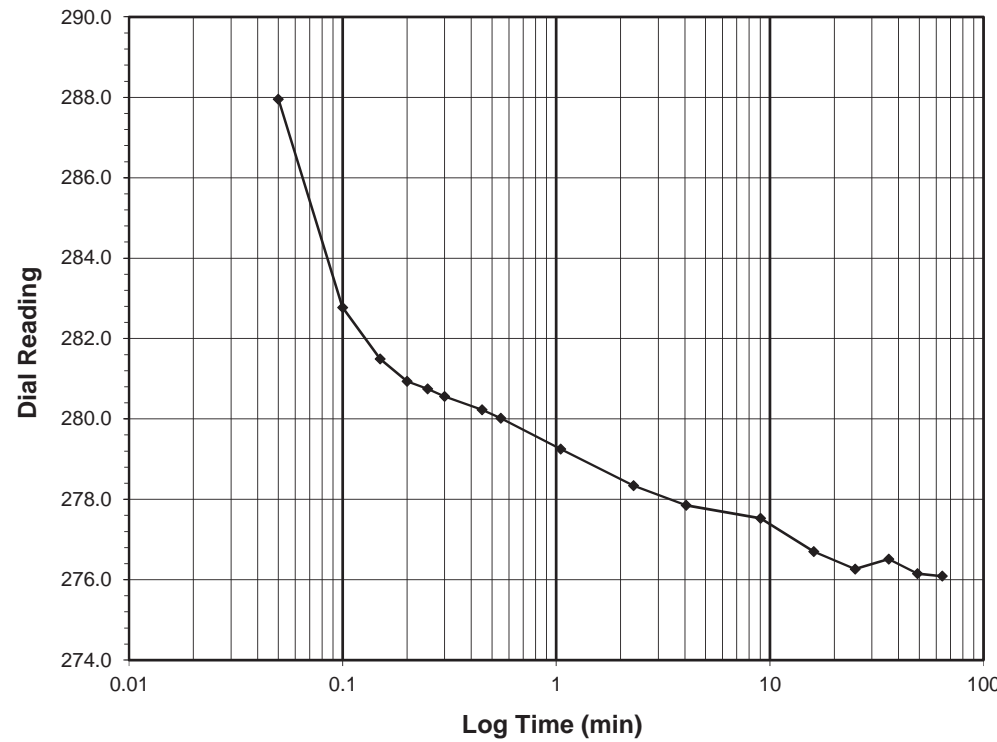
Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf)	8.0-2.0
Final Reading (div)	276.1
Consolidometer No.	R470
1 Division (in)	0.0001
Start Date	4/18/18
Start Time	8:15:13

Elapsed Time (min)	Dial Reading (div)
Initial	337.6
0.05	288.0
0.10	282.8
0.15	281.5
0.20	280.9
0.25	280.7
0.30	280.6
0.45	280.2
0.55	280.0
1.05	279.3
2.30	278.3
4.05	277.9
9.05	277.5
16.05	276.7
25.05	276.3
36.05	276.5
49.05	276.1
64.05	276.1

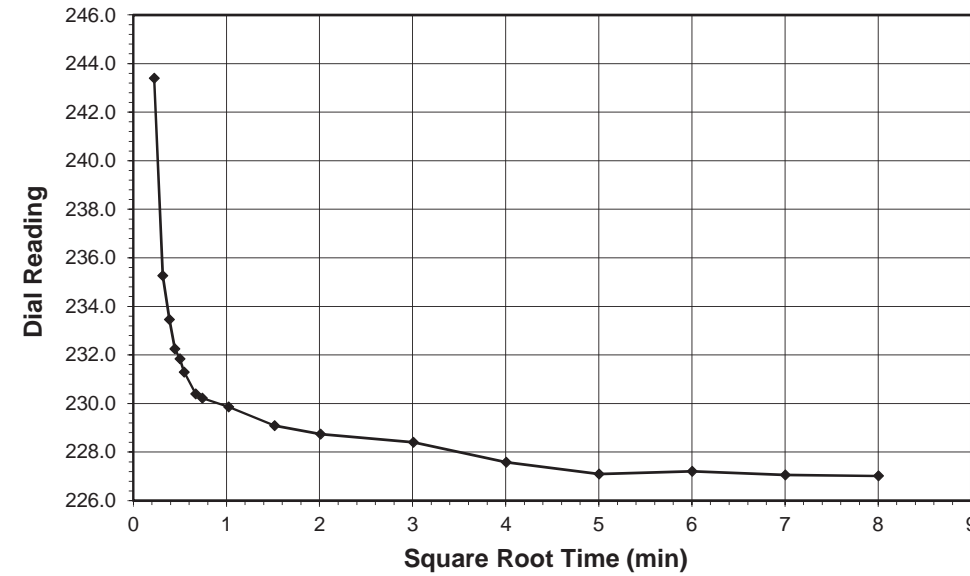


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



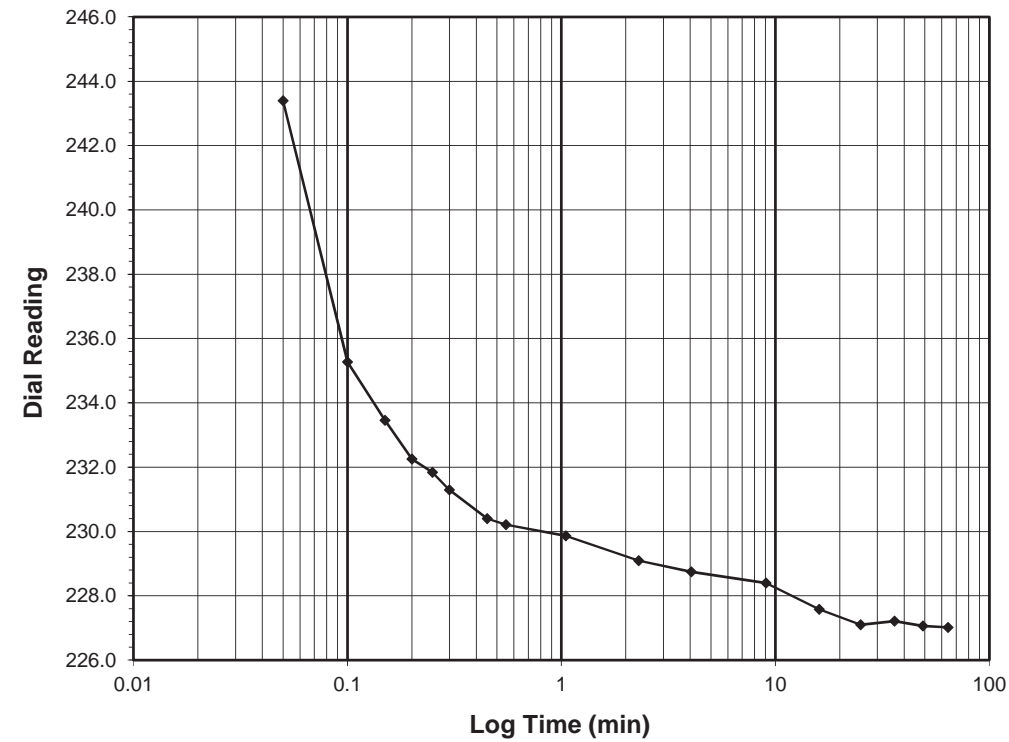
Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf)	2.0-0.5
Final Reading (div)	227.0
Consolidometer No.	R470
1 Division (in)	0.0001
Start Date	4/18/18
Start Time	11:15:34

Elapsed Time (min)	Dial Reading (div)
Initial	276.1
0.05	243.4
0.10	235.3
0.15	233.5
0.20	232.3
0.25	231.8
0.30	231.3
0.45	230.4
0.55	230.2
1.05	229.9
2.30	229.1
4.05	228.7
9.05	228.4
16.05	227.6
25.05	227.1
36.05	227.2
49.05	227.1
64.07	227.0



Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18

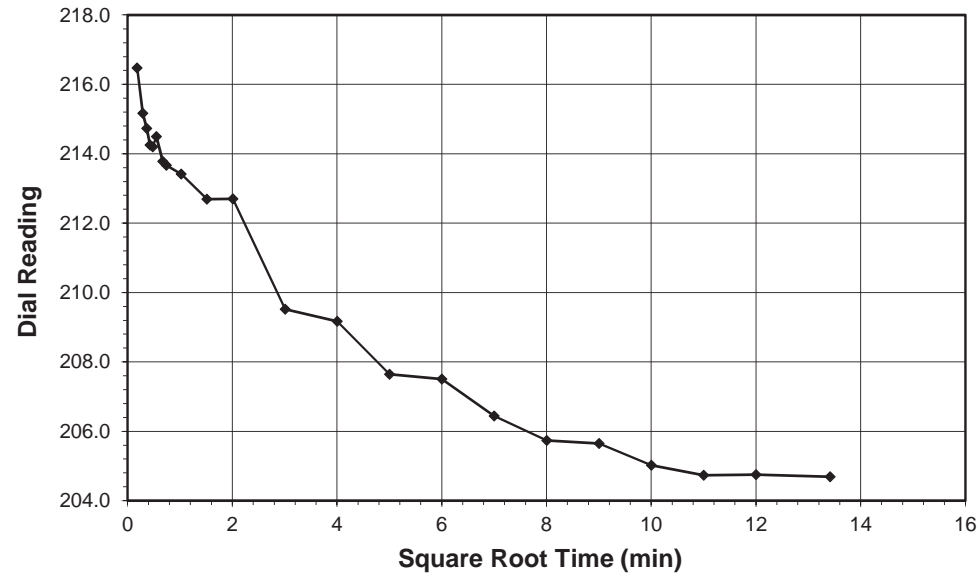
Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

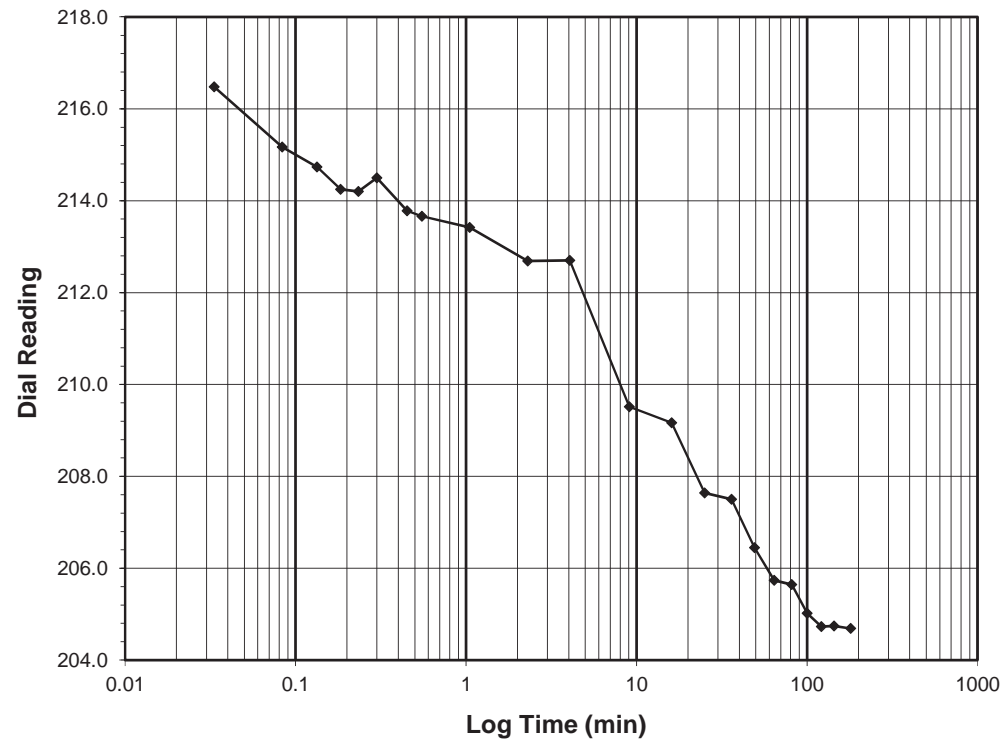
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-0.25
Final Reading (div) 204.7
 Consolidometer No. **R470**
 1 Division (in) 0.0001

Start Date 4/18/18
 Start Time 14:15:59

Elapsed Time (min)	Dial Reading (div)
Initial	227.0
0.03	216.5
0.08	215.2
0.13	214.7
0.18	214.3
0.23	214.2
0.30	214.5
0.45	213.8
0.55	213.7
1.05	213.4
2.30	212.7
4.05	212.7
9.05	209.5
16.05	209.2
25.05	207.6
36.05	207.5
49.05	206.4
64.05	205.7
81.05	205.6
100.05	205.0
121.05	204.7
144.07	204.7
180.07	204.7

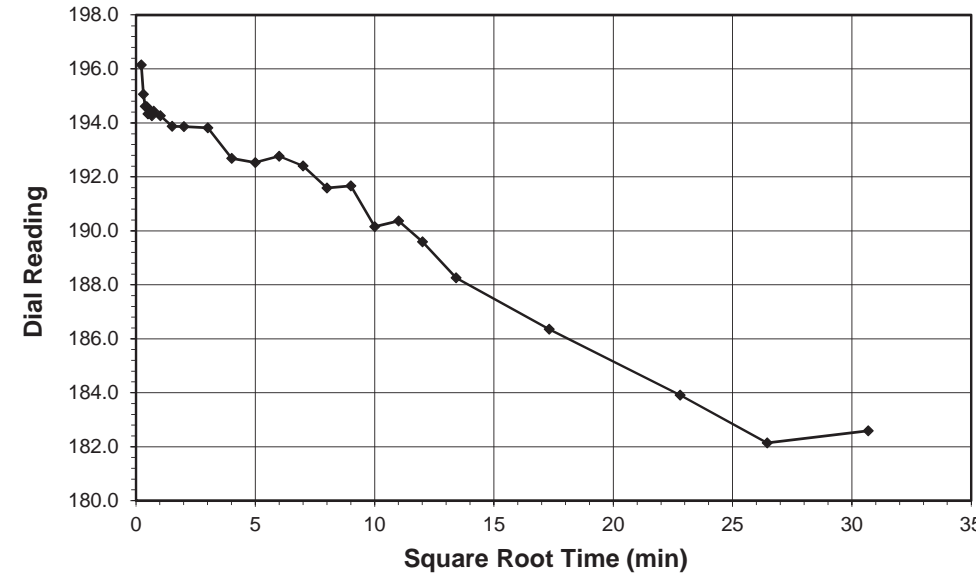


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client ESP Associates Boring No. -L- STA. 517+11, 59'RT
 Client Project R-1015 Site 9 - CS34.327.00 Depth (ft) 11.0-13.0
 Project No. R-2018-095-001 Sample No. ST-2
 Lab ID R-2018-095-001-011 Visual Description GRAY SAND

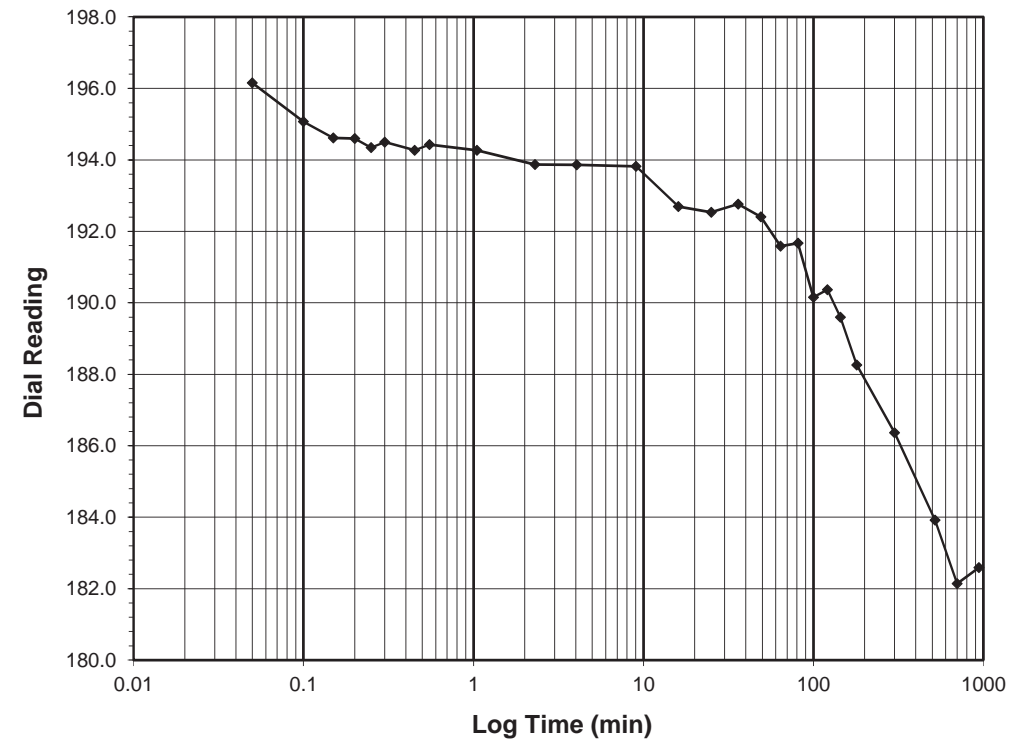
Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.125
Final Reading (div) 182.6
 Consolidometer No. **R470**
 1 Division (in) 0.0001

Start Date 4/18/18
 Start Time 17:16:22

Elapsed Time (min)	Dial Reading (div)
Initial	204.7
0.05	196.2
0.10	195.1
0.15	194.6
0.20	194.6
0.25	194.3
0.30	194.5
0.45	194.3
0.55	194.4
1.05	194.3
2.30	193.9
4.05	193.9
9.05	193.8
16.05	192.7
25.05	192.5
36.05	192.8
49.05	192.4
64.07	191.6
81.07	191.7
100.07	190.2
121.07	190.4
144.07	189.6
180.07	188.3
300.07	186.4
520.07	183.9
700.07	182.1
941.73	182.6



Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18

Tested By 129-04-0411 Date 4/18/18 Checked By GEM Date 5/15/18



SPECIFIC GRAVITY
AASHTO T-100-15

Client	ESP Associates	Location	-L-STA.217+15,23'RT
Client Reference	R-2015 Site 9 MSE Wall CS34.368	Depth (ft)	9.8-11.8
Project No.	R-2018-181-001	Sample No.	ST-3
Lab ID	R-2018-181-001-001	Visual Description	GRAY (Minus No.4 sieve material, airdried)

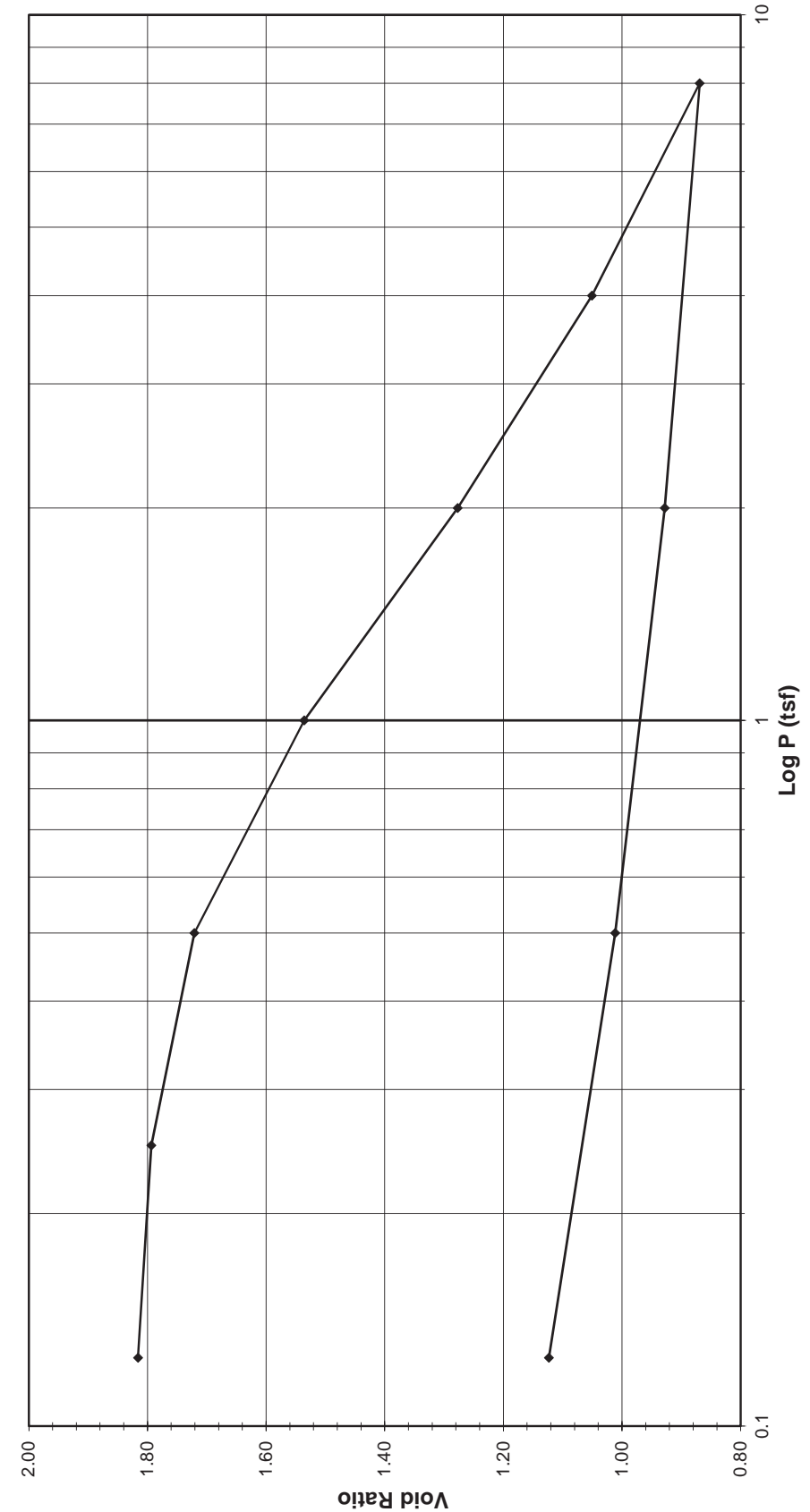
Replicate Number	1	2
Pycnometer ID	R 280	R 347
Weight of Pycnometer + Soil + Water (gm)	718.53	711.58
Temperature, T (°Celsius)	28.1	27.8
Weight of Pycnometer + Water (gm)	685.04	668.72
Tare Number	280	347
Weight of Tare + Dry Soil (gm)	241.14	239.34
Weight of Tare (gm)	187.33	170.75
Weight of Dry Soil (gm)	53.81	68.59
Specific Gravity of Soil @ T	2.649	2.666
Specific Gravity of Water @ T	0.9962	0.9963
Conversion Factor for Temperature T	0.9980	0.9981
Specific Gravity @ 20° Celsius	2.654	2.671

Average Specific Gravity @ 20° Celsius	2.66
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Location	-L-STA.217+15,23'RT
Client Reference	R-1015 Site 9 MSE Wall CS34.368	Depth (ft)	9.8-11.8
Project No.	R-2018-181-001	Sample No.	ST-3
Lab ID	R-2018-181-001-001	Visual Description	GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Tested By 129-04-0411 Date 7/3/18 Checked By GEM Date 7/3/18

DCN: CT-S5 Date: 03/24/05 Revision: 10 R:\2018 PROJECTS\ESP Associates\2018-181 ESP - R-1015 SITE 9 MSE WALL\2018-181-001-001 rename DOTSp. Gravity.xlsm\Sheet1

Tested By 129-04-0411 Date 6/26/18 Approved By MPS Date 7/6/18

DCN: CT-24E Date: 5/3/12 Revision: 6

page 1 of 2

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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Location	-L-STA.217+15,23RT
Client Reference	R-1015 Site 9 MSE Wall CS34.368	Depth (ft)	9.8-11.8
Project No.	R-2018-181-001	Sample No.	ST-3
Lab ID	R-2018-181-001-001	Visual Description	GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R487
1 Division = 0.0001 (in.)

Sample Properties

	Initial	Final
<i>Water Content</i>		
Tare Number	912	826
Wt. Tare & WS (g)	335.79	237.23
Wt. Tare & DS (g)	244.03	205.02
Wt. Water (g)	91.76	32.21
Wt. Tare (g)	101.01	133.76
Wt. DS (g)	143.02	71.26
Water Content (%)	64.16	45.20
<i>Sample Parameters</i>		
Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.0000	1.0000
Sample Volume (cc)	80.44	80.44
Wt. Wet Sample + Ring (g)	337.93	323.70
Wt. of Ring (g)	214.69	214.69
Wt. of Wet Sample (g)	123.24	109.01
Wet Density (pcf)	95.60	84.56
Wet Density (g/cc)	1.53	1.36
Water Content (%)	64.16	45.20
Wt. of Dry Sample (g)	75.07	75.07
Dry Density (pcf)	58.24	58.24
Dry Density (g/cc)	0.93	0.93
Void Ratio	1.8501	1.8501
Saturation (%)	92.24	64.99
Specific Gravity	2.66	Measured

Test Data Summary

Applied Pressure (tsf)	Final Dial Reading (div)	Machine Deflection (div)	Corrected Reading (div)	Height of Sample (mm)	Volume (cc)	Dry Density (g/cc)	Void Ratio
Seating	0	0	0	25.400	80.440	0.93329	1.85013
0.125	123.7	3.6	120.1	25.095	79.474	0.94463	1.81591
0.25	207.3	9.0	198.3	24.896	78.844	0.95218	1.79360
0.5	474.4	21.0	453.4	24.248	76.793	0.97762	1.72091
1	1143.0	40.0	1103.0	22.598	71.567	1.04900	1.53575
2	2071.0	59.6	2011.4	20.291	64.260	1.16828	1.27685
4	2889.6	85.0	2804.6	18.276	57.880	1.29707	1.05078
8	3565.8	123.4	3442.5	16.656	52.749	1.42323	0.86898
2	3350.2	114.6	3235.6	17.182	54.413	1.37970	0.92795
0.5	3027.2	84.0	2943.2	17.924	56.765	1.32254	1.01129
0.125	2626.5	76.4	2550.1	18.923	59.927	1.25276	1.12332

DCN: CT-24E Date: 5/3/12 Revision: 6
 Tested By 129-04-0411 Date 6/26/18 Input Checked By GEM Date 7/6/18

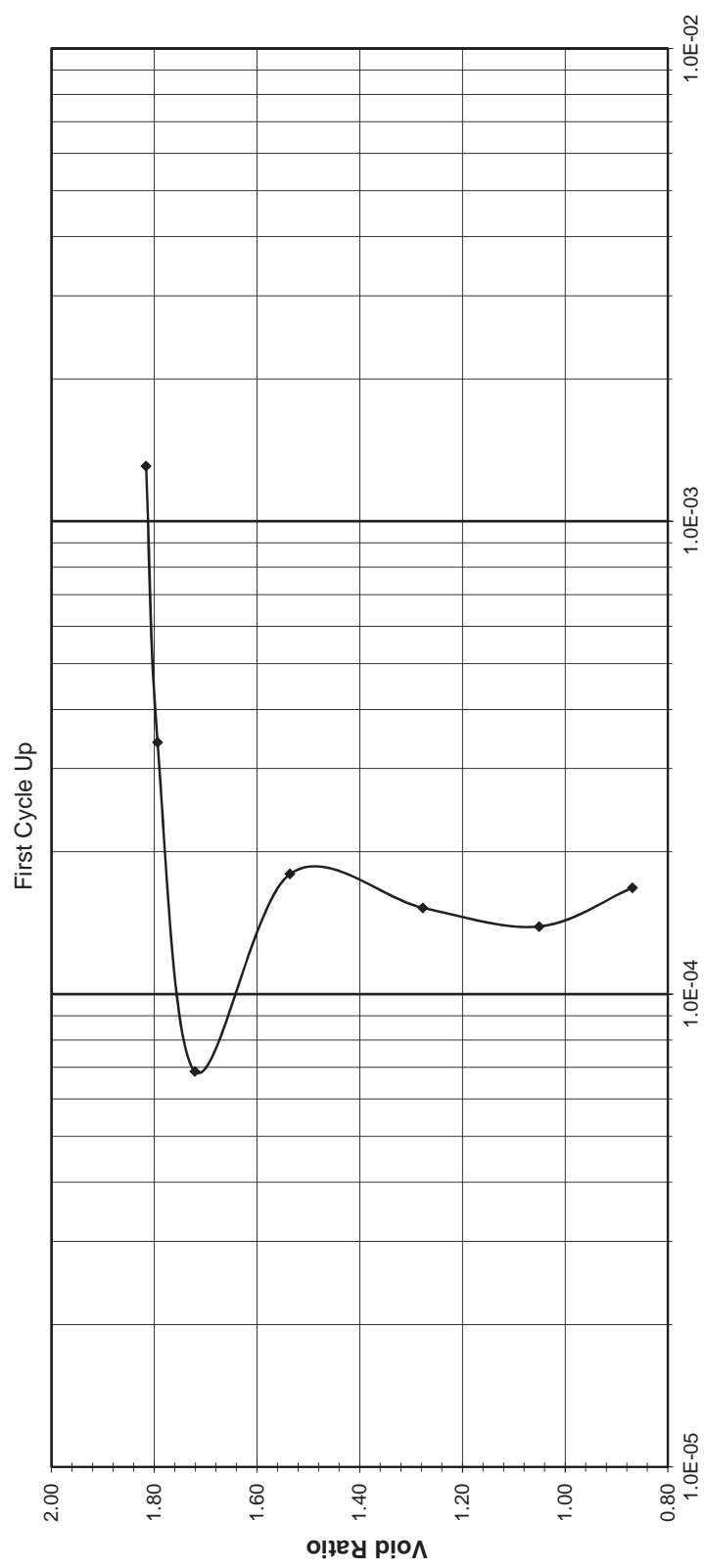
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ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client	ESP Associates	Location	-L-STA.217+15,23RT
Client Reference	R-1015 Site 9 MSE Wall CS34.368	Depth (ft) Sample	9.8-11.8
Project No.	R-2018-181-001	No. Visual	ST-3
Lab ID	R-2018-181-001-001	Description	GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Coefficient of Consolidation (cm²/sec)

— First Cycle Up

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client ESP Associates Location -L-STA.217+15,23'RT
 Client Reference R-1015 Site 9 MSE Wall CS34.368 Depth (ft) Sample 9.8-11.8
 Project No. R-2018-181-001 No. Visual ST-3
 Lab ID R-2018-181-001-001 Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED

Consolidometer No. R487
 1 Division = 0.0001 (in.)

Sample Properties

	Initial	Final
Water Content		
Tare Number	912	826
Wt. Tare & WS (g)	335.79	237.23
Wt. Tare & DS (g)	244.03	205.02
Wt. Water (g)	91.76	32.21
Wt. Tare (g)	101.01	133.76
Wt. DS (g)	143.02	71.26
Water Content (%)	64.16	45.20
Sample Parameters		
Sample Diameter (in)	2.5	2.5
Sample Height (in)	1.000	1.000
Sample Volume (cc)	80.44	80.44
Wt. Wet Sample + Ring (g)	337.93	323.70
Wt. of Ring (g)	214.69	214.69
Wt. of Wet Sample (g)	123.24	109.01
Wet Density (pcf)	95.60	84.56
Wet Density (g/cc)	1.53	1.36
Water Content (%)	64.16	45.20
Wt. of Dry Sample (g)	75.07	75.07
Dry Density (pcf)	58.24	58.24
Dry Density (g/cc)	0.93	0.93
Void Ratio	1.8501	1.8501
Saturation (%)	92.24	64.99
Specific Gravity	2.66	Measured

Load Increment (tsf)	Dial Reading @ t ₅₀ (div)	Machine Deflection (div)	Corrected Dial Reading @ t ₅₀ (div)	Sample Height @ t ₅₀ (cm)	Time t ₅₀ (min.)	C _v (cm ² /sec)
0.0 - 0.125	60.3	3.6	56.7	2.526	4.00	0.00131
0.125 - 0.25	184.2	9.0	175.2	2.495	15.00	0.00034
0.25 - 0.5	360.8	21.0	339.8	2.454	72.00	0.00007
0.5 - 1	817.6	40.0	777.6	2.342	25.10	0.00018
1 - 2	1565.5	59.6	1505.9	2.157	25.10	0.00015
2 - 4	2486.4	85.0	2401.4	1.930	22.00	0.00014
4 - 8	3228.8	123.4	3105.4	1.751	15.00	0.00017
8 - 2	NA	114.6	NA	NA	NA	NA
2 - 0.5	NA	84.0	NA	NA	NA	NA
0.5 - 0.125	NA	76.4	NA	NA	NA	NA

Tested By 129-04-0411 Date 6/26/18 Input Checked By GEM Date 7/6/18

DCN: CT-24E Date: 5/3/12 Revision: 6 Z:\2018 PROJECTS\ESP Associates\2018-181 ESP - R-1015 SITE 9 MSE WALL\2018-181-001-001 DOT GEOJAC-16TSF1 Cv.xlsm\FINAL PLOT

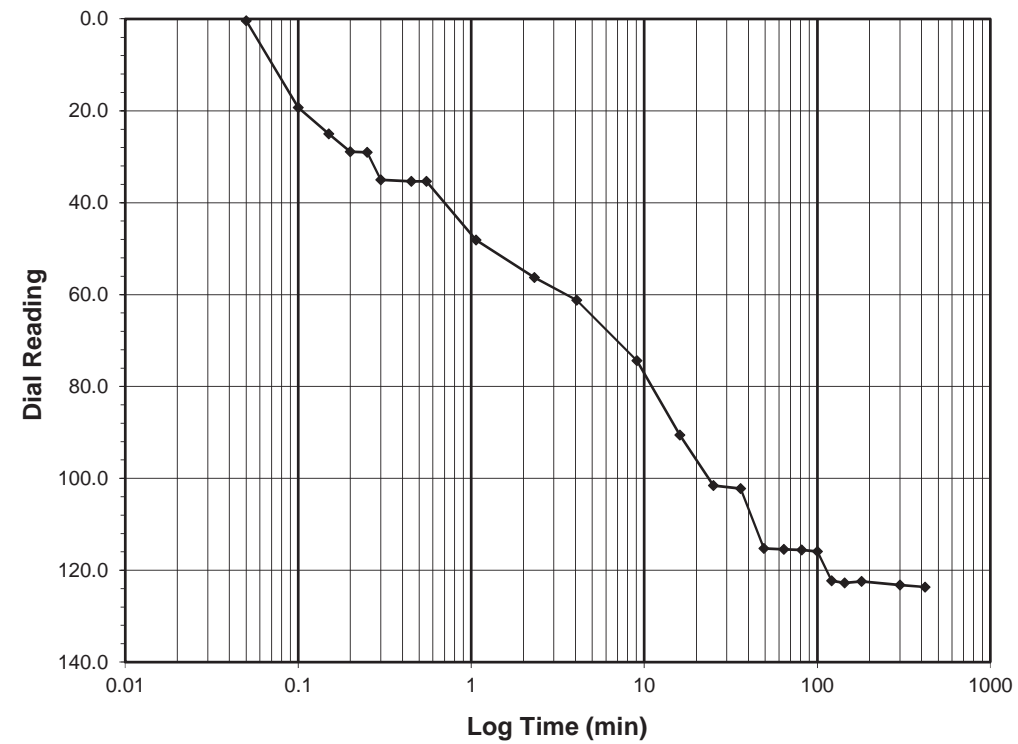
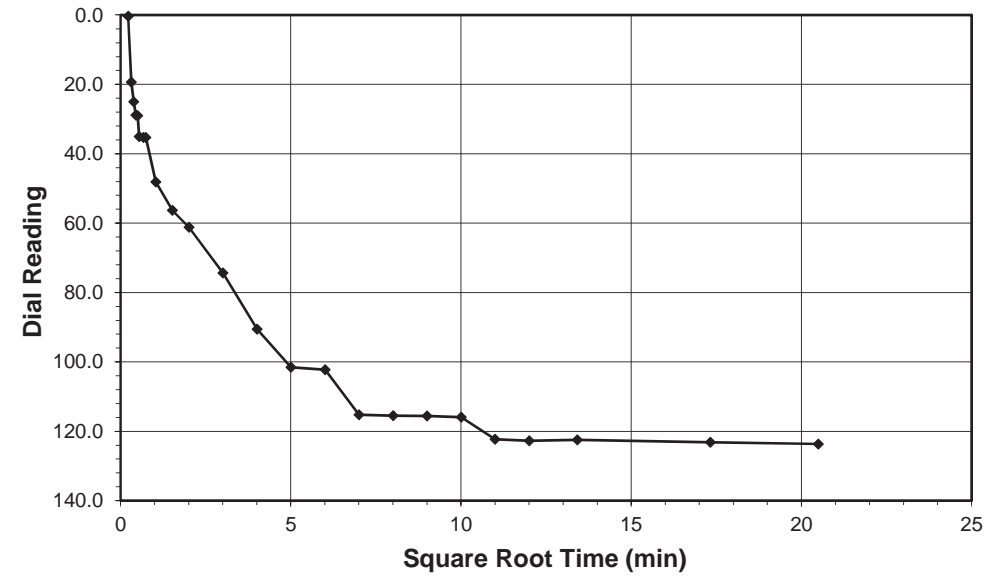
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page 4 of 4

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216

Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf)	0.0-0.125
Final Reading (div)	123.7
Consolidometer No.	R487
1 Division (in)	0.0001
Start Date	6/26/18
Start Time	17:27:35

Elapsed Time (min)	Dial Reading (div)
Initial	0.0
0.05	0.4
0.10	19.3
0.15	25.0
0.20	28.9
0.25	29.0
0.30	35.0
0.45	35.3
0.55	35.3
1.07	48.1
2.32	56.3
4.07	61.2
9.07	74.4
16.07	90.5
25.07	101.6
36.07	102.2
49.07	115.2
64.07	115.5
81.07	115.6
100.07	115.9
121.07	122.3
144.07	122.8
180.07	122.4
300.07	123.2
420.05	123.7

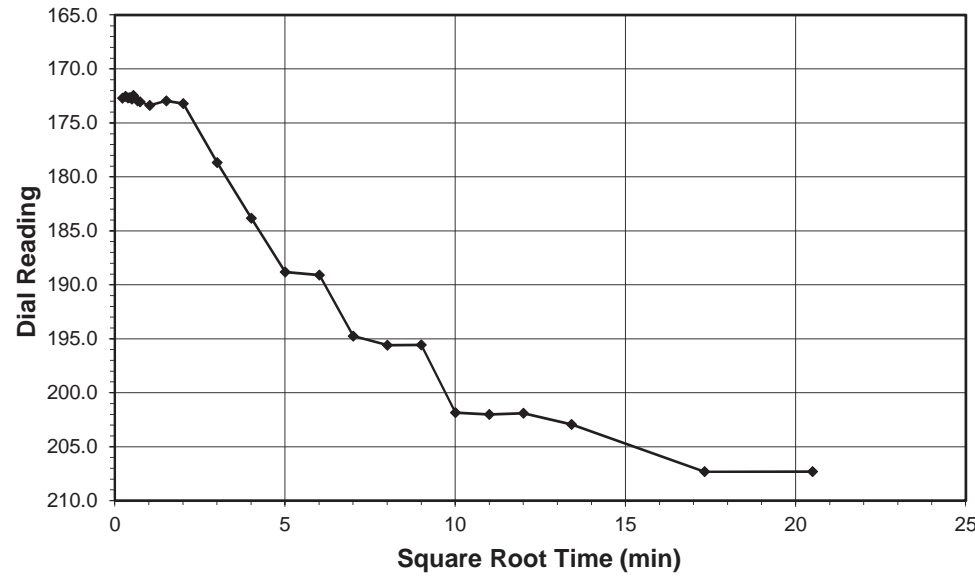
Tested By 129-04-0411 Date 6/26/18 Checked By GEM Date 7/6/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



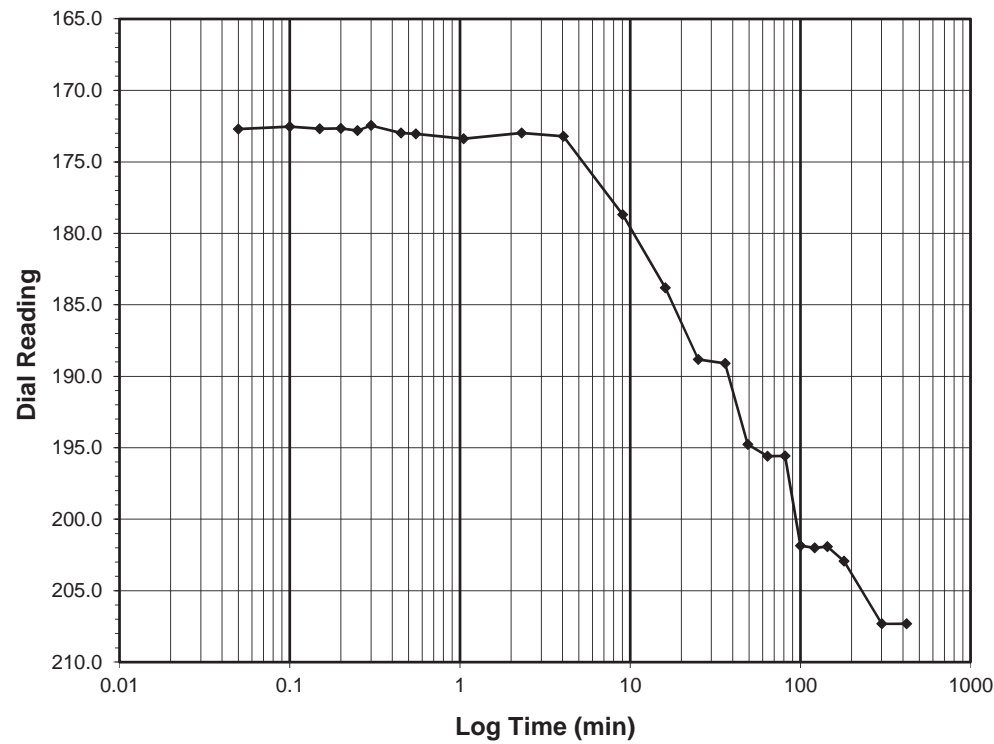
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.125-0.25
 Final Reading (div) 207.3
 Consolidometer No. R487
 1 Division (in) 0.0001
 Start Date 6/27/18
 Start Time 0:27:38

Elapsed Time (min)	Dial Reading (div)
Initial	123.7
0.05	172.7
0.10	172.5
0.15	172.7
0.20	172.7
0.25	172.8
0.30	172.5
0.45	173.0
0.55	173.0
1.05	173.4
2.30	173.0
4.05	173.2
9.05	178.7
16.07	183.8
25.07	188.8
36.07	189.1
49.07	194.8
64.07	195.6
81.07	195.6
100.07	201.9
121.07	202.0
144.07	201.9
180.07	202.9
300.07	207.3
420.25	207.3

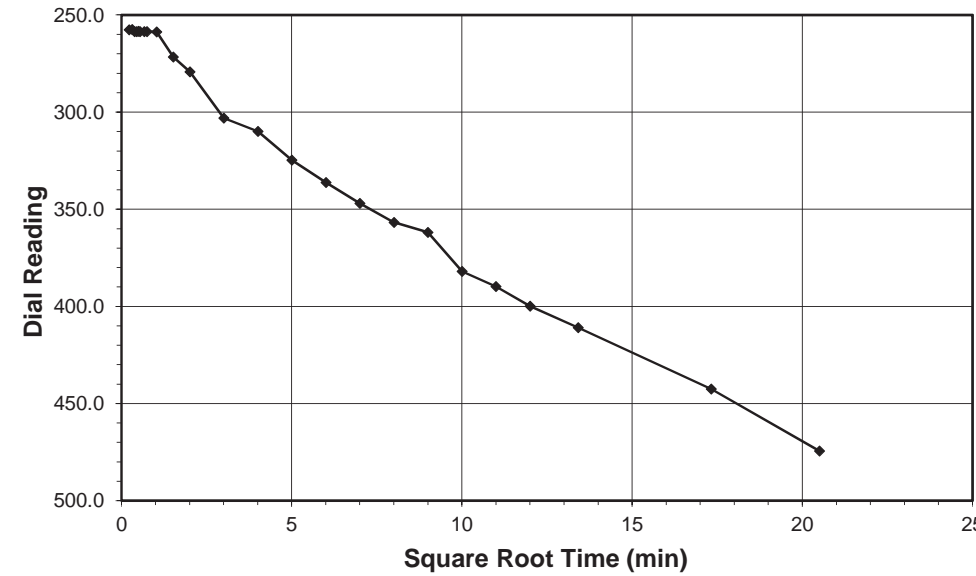


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



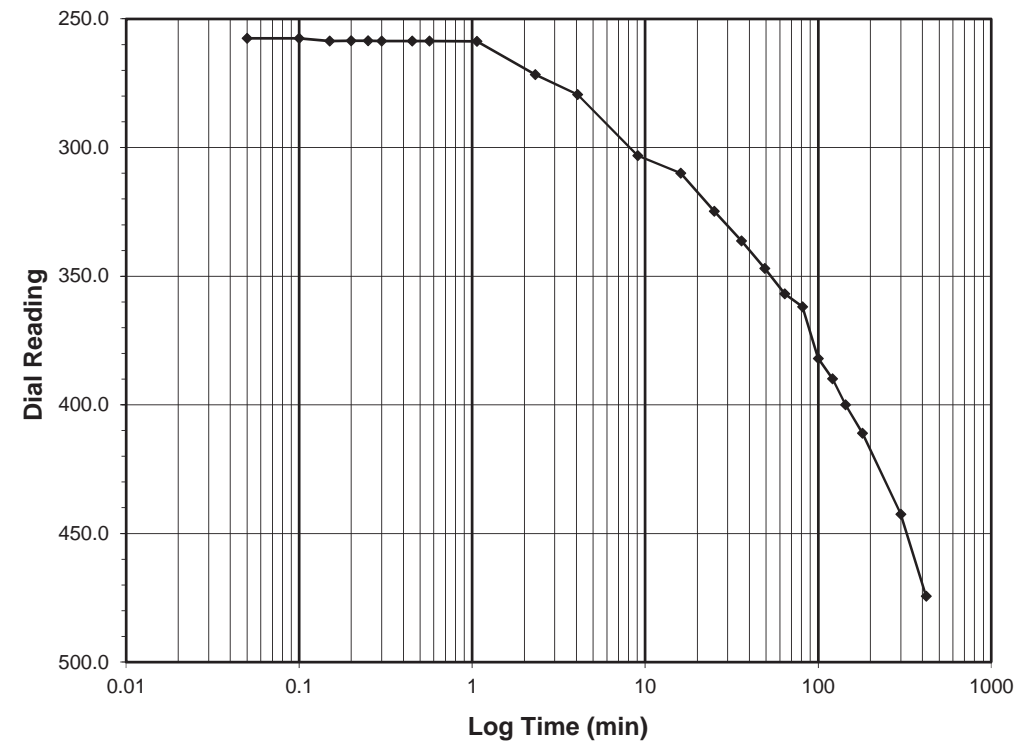
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.25-0.5
 Final Reading (div) 474.4
 Consolidometer No. R487
 1 Division (in) 0.0001
 Start Date 6/27/18
 Start Time 7:27:53

Elapsed Time (min)	Dial Reading (div)
Initial	207.3
0.05	257.6
0.10	257.5
0.15	258.6
0.20	258.5
0.25	258.5
0.30	258.6
0.45	258.5
0.57	258.5
1.07	258.7
2.32	271.7
4.07	279.3
9.07	303.1
16.07	310.0
25.07	324.7
36.07	336.3
49.07	347.0
64.07	356.8
81.07	361.9
100.07	382.0
121.07	389.9
144.07	400.0
180.08	411.0
300.08	442.6
420.50	474.4



Tested By 129-04-0411 Date 6/27/18 Checked By GEM Date 7/6/18

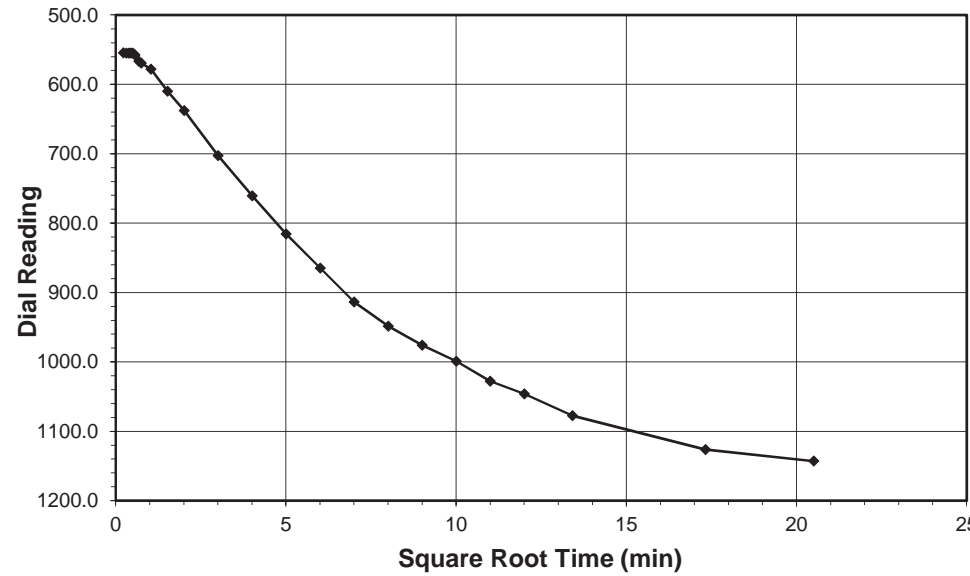
Tested By 129-04-0411 Date 6/27/18 Checked By GEM Date 7/6/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



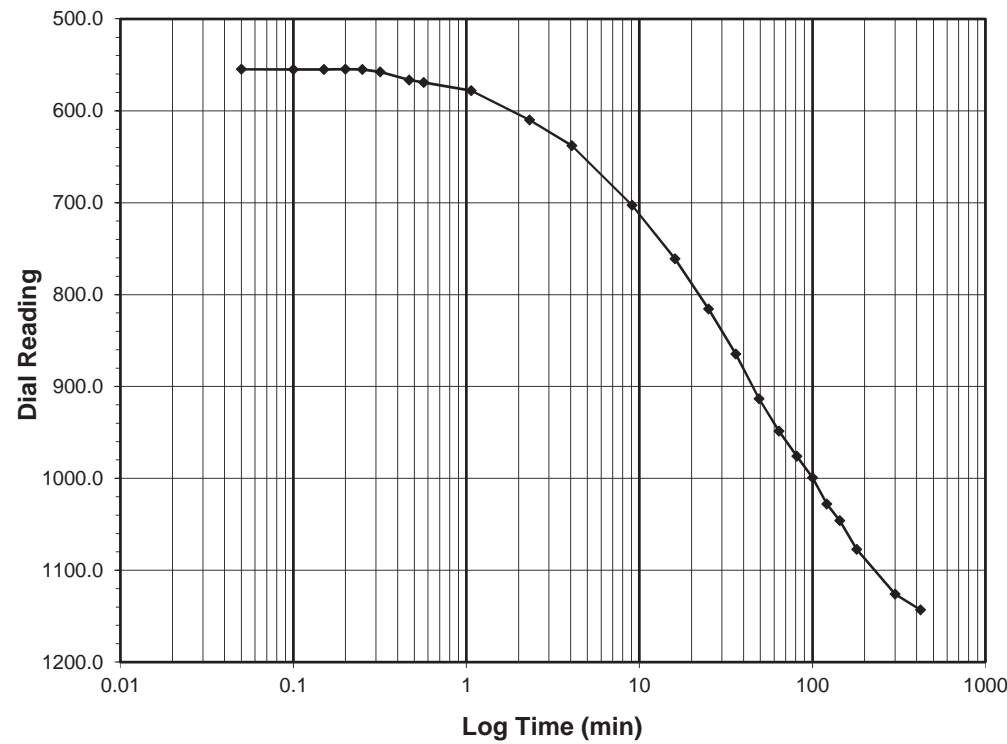
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 0.5-1.0
 Final Reading (div) 1143.0
 Consolidometer No. R487
 1 Division (in) 0.0001
 Start Date 6/27/18
 Start Time 14:28:23

Elapsed Time (min)	Dial Reading (div)
Initial	474.4
0.05	554.7
0.10	554.8
0.15	555.0
0.20	554.8
0.25	554.9
0.32	557.6
0.47	566.6
0.57	569.3
1.07	578.0
2.32	609.9
4.07	638.0
9.07	702.9
16.07	760.9
25.07	815.7
36.07	864.8
49.07	913.5
64.07	948.5
81.07	976.1
100.07	999.1
121.07	1027.9
144.07	1046.0
180.07	1077.3
300.08	1126.1
420.50	1143.0

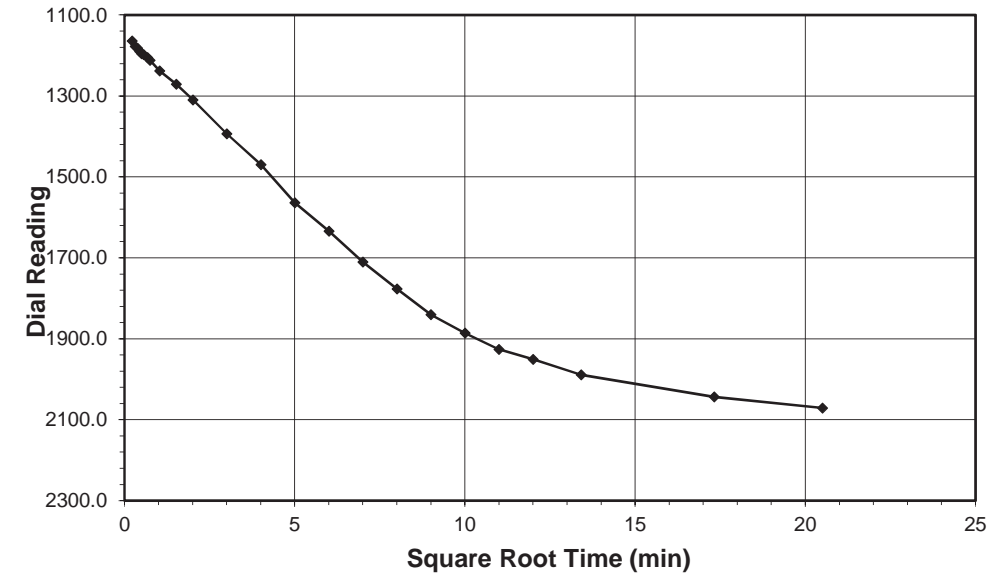


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



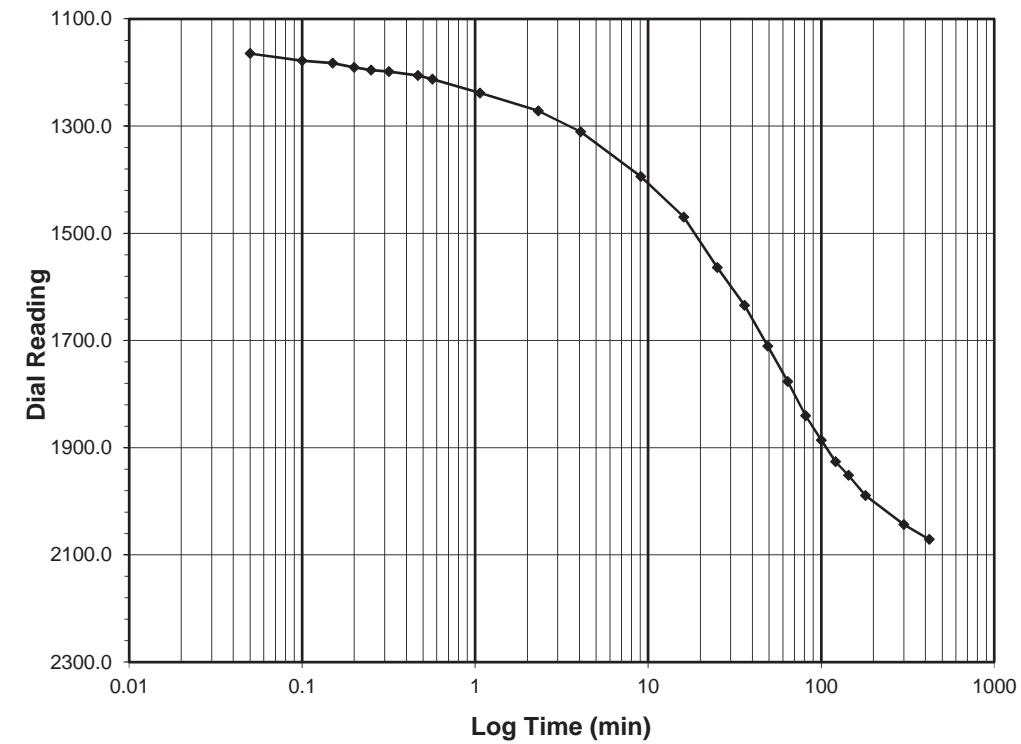
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 1.0-2.0
 Final Reading (div) 2071.0
 Consolidometer No. R487
 1 Division (in) 0.0001
 Start Date 6/27/18
 Start Time 21:28:53

Elapsed Time (min)	Dial Reading (div)
Initial	1143.0
0.05	1164.5
0.10	1177.9
0.15	1182.6
0.20	1190.2
0.25	1195.3
0.32	1198.2
0.47	1205.6
0.57	1212.2
1.07	1238.0
2.32	1271.4
4.07	1310.2
9.07	1393.9
16.07	1469.5
25.07	1563.9
36.07	1634.3
49.07	1710.4
64.07	1776.7
81.07	1840.5
100.07	1885.8
121.07	1926.1
144.07	1951.3
180.08	1989.4
300.08	2043.6
420.47	2071.0



Tested By 129-04-0411 Date 6/27/18 Checked By GEM Date 7/6/18

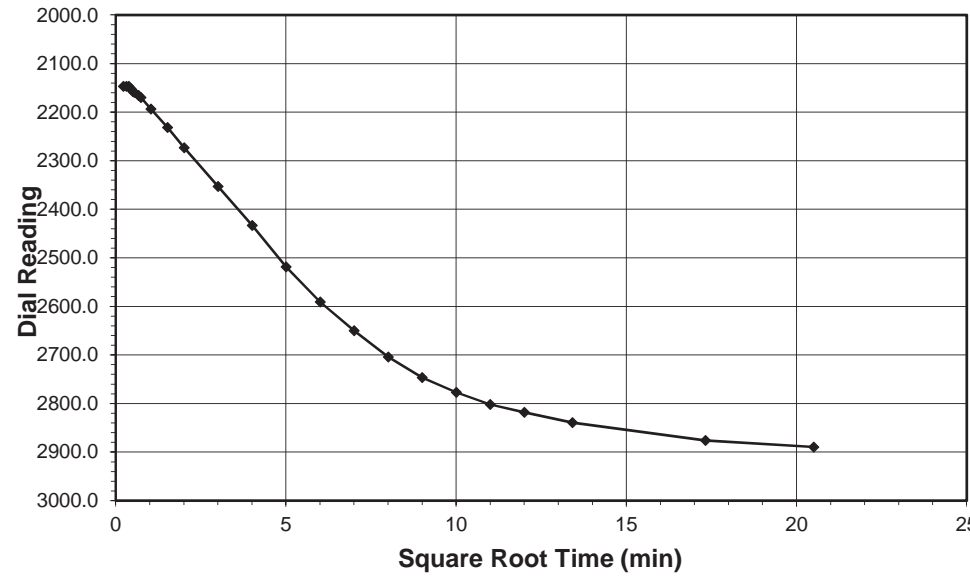
Tested By 129-04-0411 Date 6/27/18 Checked By GEM Date 7/6/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



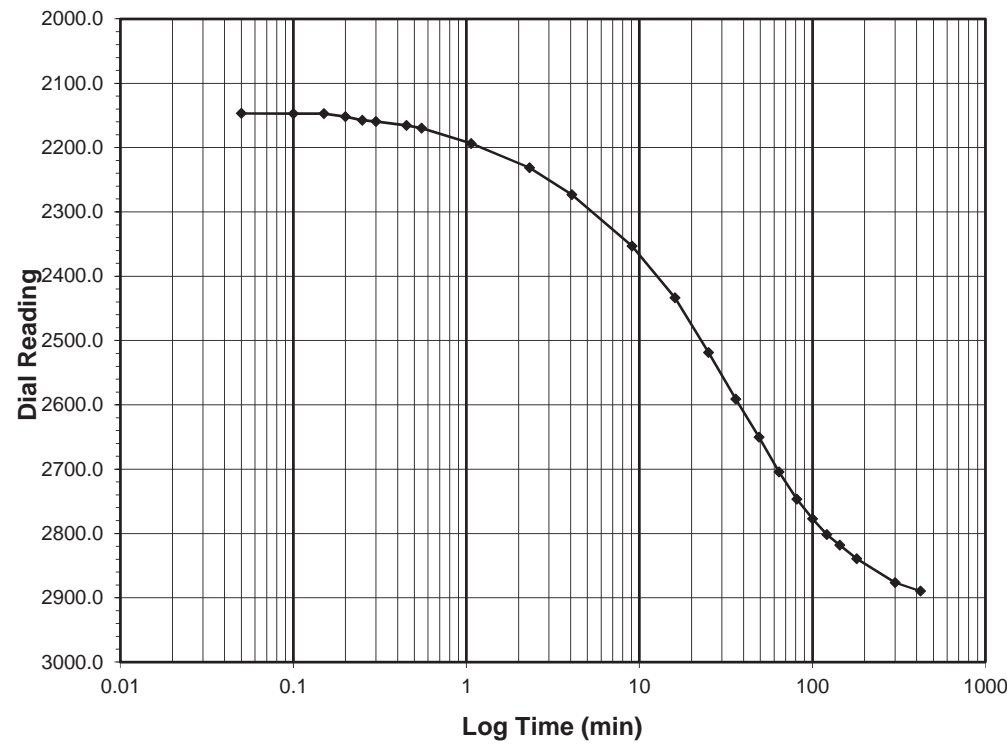
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) **2.0-4.0**
 Final Reading (div) **2889.6**
 Consolidometer No. **R487**
 1 Division (in) 0.0001
 Start Date 6/28/18
 Start Time 4:29:21

Elapsed Time (min)	Dial Reading (div)
Initial	2071.0
0.05	2146.9
0.10	2147.0
0.15	2147.1
0.20	2152.0
0.25	2157.7
0.30	2159.6
0.45	2165.7
0.55	2169.8
1.07	2193.9
2.32	2231.5
4.07	2273.1
9.07	2353.0
16.07	2433.4
25.07	2518.7
36.07	2590.8
49.07	2650.2
64.07	2704.3
81.08	2746.5
100.08	2777.2
121.08	2801.8
144.08	2817.9
180.08	2839.1
300.08	2876.3
420.48	2889.6

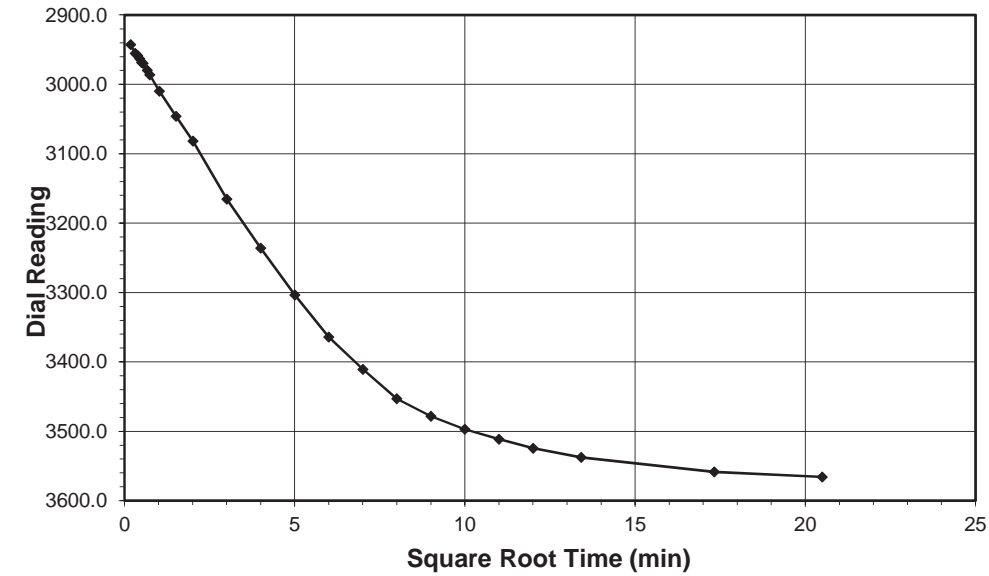


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



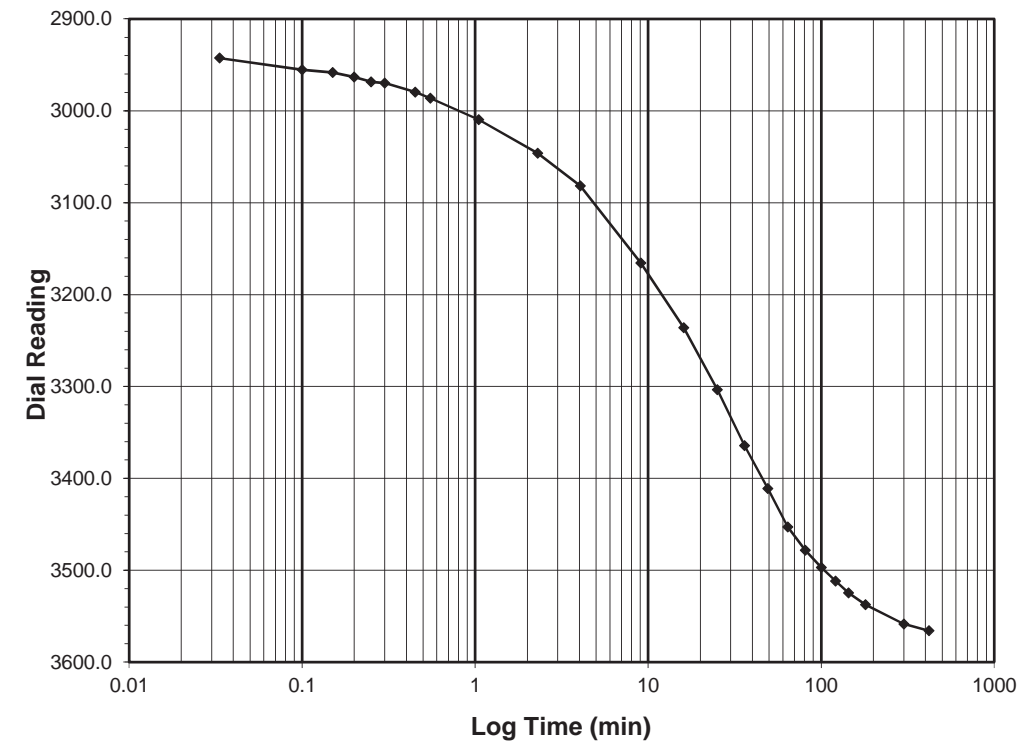
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) **4.0-8.0**
 Final Reading (div) **3565.8**
 Consolidometer No. **R487**
 1 Division (in) 0.0001
 Start Date 6/28/18
 Start Time 11:29:50

Elapsed Time (min)	Dial Reading (div)
Initial	2889.6
0.03	2942.6
0.10	2955.3
0.15	2958.2
0.20	2963.4
0.25	2968.4
0.30	2969.8
0.45	2979.7
0.55	2986.8
1.05	3009.7
2.30	3046.0
4.05	3081.7
9.05	3165.5
16.05	3235.9
25.05	3303.6
36.05	3364.4
49.05	3411.0
64.05	3452.9
81.05	3478.3
100.05	3496.9
121.05	3511.7
144.05	3524.5
180.05	3537.5
300.05	3558.4
420.05	3565.8



Tested By 129-04-0411 Date 6/28/18 Checked By GEM Date 7/6/18

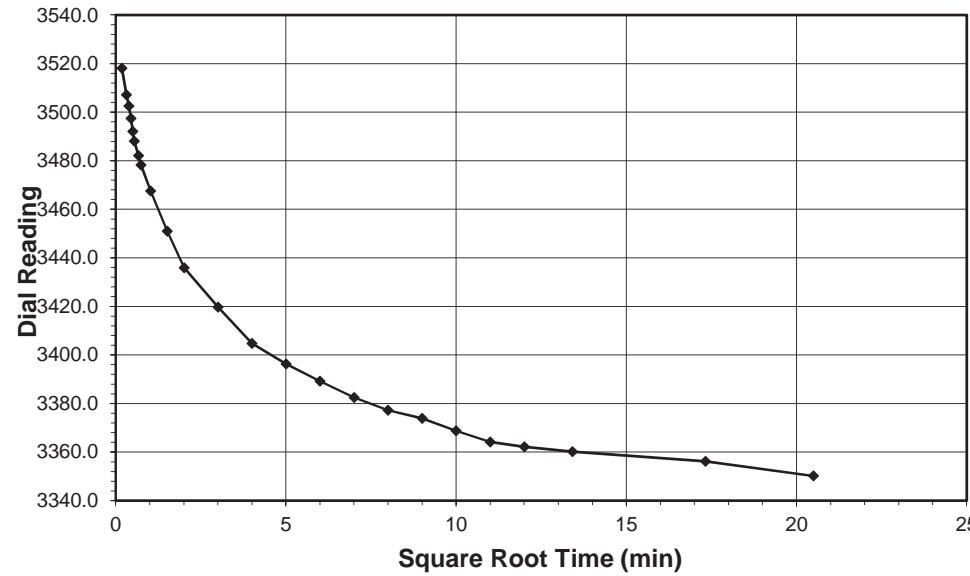
Tested By 129-04-0411 Date 6/28/18 Checked By GEM Date 7/6/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



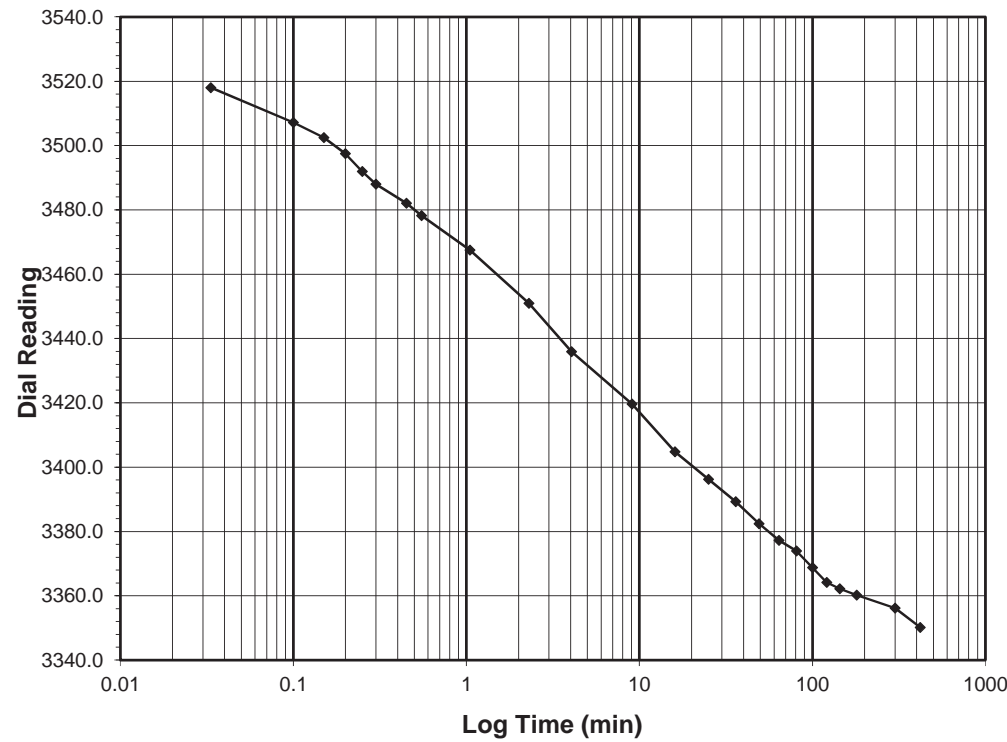
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 8.0-2.0
Final Reading (div) 3350.2
 Consolidometer No. **R487**
 1 Division (in) 0.0001
 Start Date 6/28/18
 Start Time 18:29:54

Elapsed Time (min)	Dial Reading (div)
Initial	3565.8
0.03	3518.0
0.10	3507.1
0.15	3502.5
0.20	3497.5
0.25	3492.0
0.30	3488.0
0.45	3482.0
0.55	3478.2
1.05	3467.5
2.30	3451.0
4.05	3435.9
9.05	3419.6
16.05	3404.8
25.05	3396.2
36.05	3389.2
49.05	3382.4
64.05	3377.2
81.05	3373.9
100.05	3368.8
121.05	3364.2
144.05	3362.2
180.07	3360.2
300.07	3356.1
420.08	3350.2

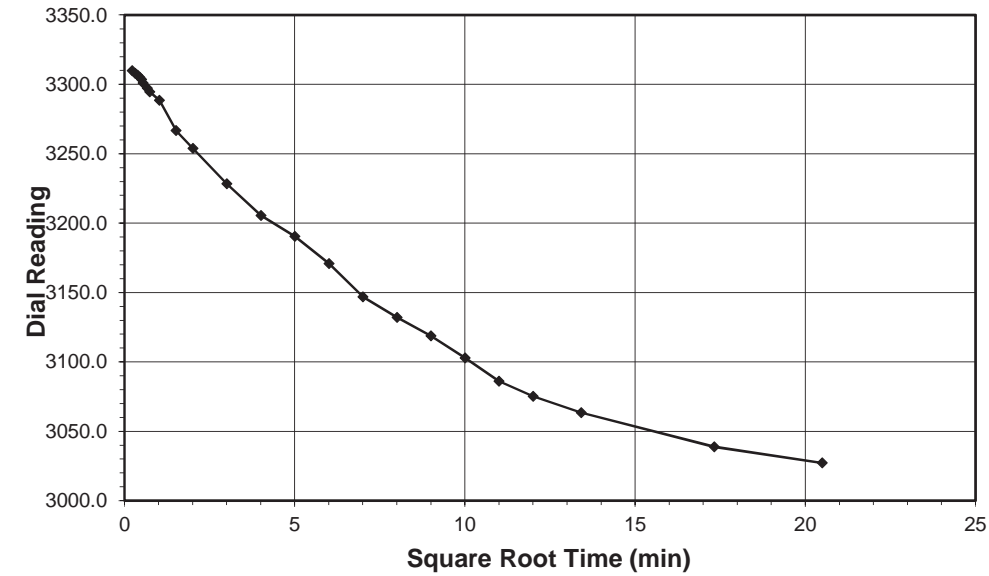


ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



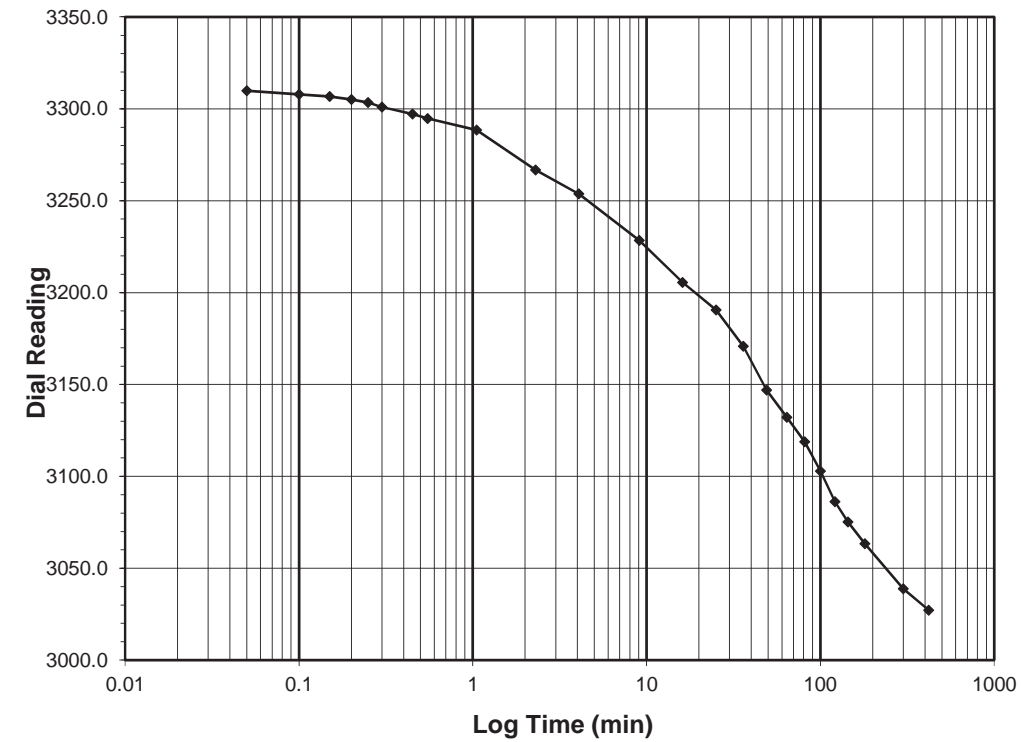
Client ESP Associates Location -L-STA.217+15,23'RT
 Client Project R-1015 Site 9 MSE Wall CS34.368 Depth (ft) 9.8-11.8
 Project No. R-2018-181-001 Sample No. ST-3
 Lab ID R-2018-181-001-001 Visual Description GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf) 2.0-0.5
Final Reading (div) 3027.2
 Consolidometer No. **R487**
 1 Division (in) 0.0001
 Start Date 6/29/18
 Start Time 1:30:00

Elapsed Time (min)	Dial Reading (div)
Initial	3350.2
0.05	3309.8
0.10	3307.9
0.15	3306.7
0.20	3305.0
0.25	3303.5
0.30	3300.9
0.45	3297.2
0.55	3294.7
1.05	3288.4
2.30	3266.7
4.07	3253.8
9.07	3228.3
16.07	3205.5
25.07	3190.5
36.07	3170.8
49.07	3147.0
64.07	3132.1
81.07	3118.8
100.07	3102.8
121.07	3086.2
144.07	3075.2
180.07	3063.4
300.07	3038.8
420.10	3027.2



Tested By 129-04-0411 Date 6/28/18 Checked By GEM Date 7/6/18

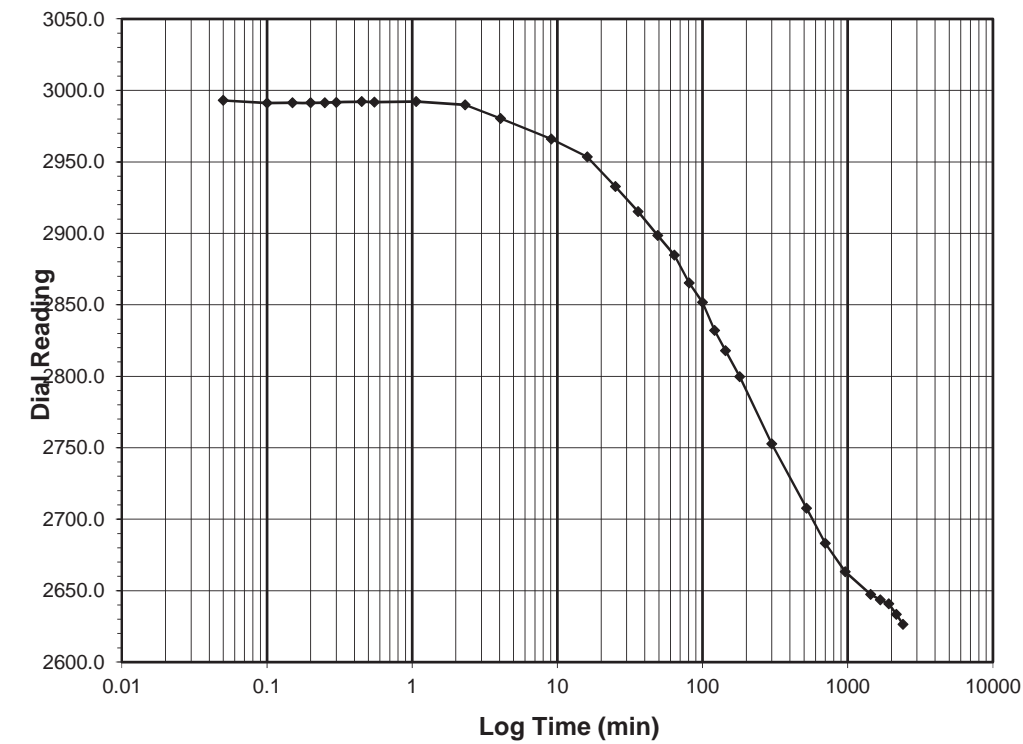
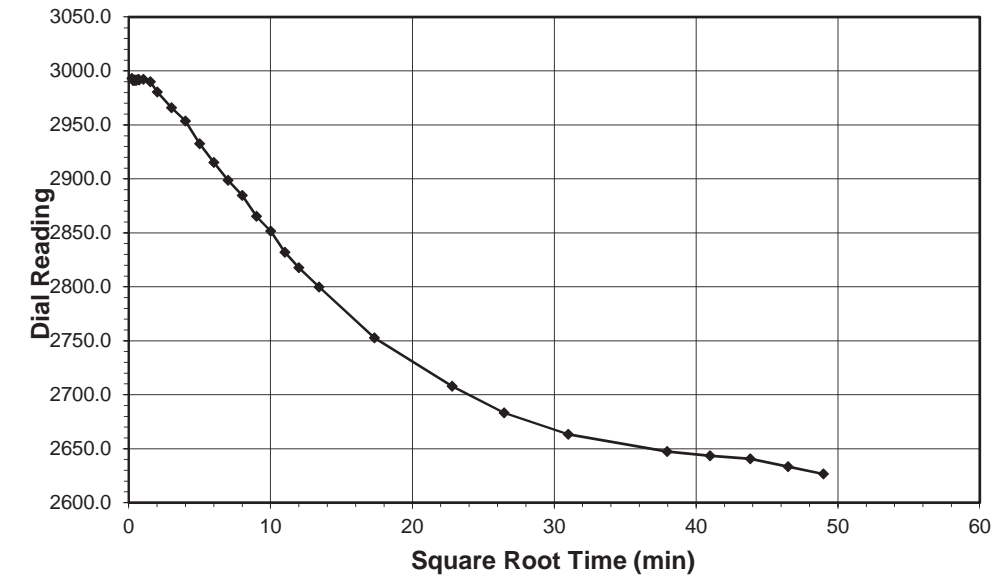
Tested By 129-04-0411 Date 6/29/18 Checked By GEM Date 7/6/18

ONE DIMENSIONAL CONSOLIDATION
AASHTO T-216



Client: ESP Associates Location: -L-STA.217+15,23'RT
 Client Project: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001 Visual Description: GRAY CLAY

Sample Conditions: UNDISTURBED, INUNDATED AND DOUBLE DRAINED



Test Load (tsf): 0.5-0.125
Final Reading (div): 2626.5
Consolidometer No.: R487
1 Division (in): 0.0001

Start Date: 6/29/18
Start Time: 8:30:06

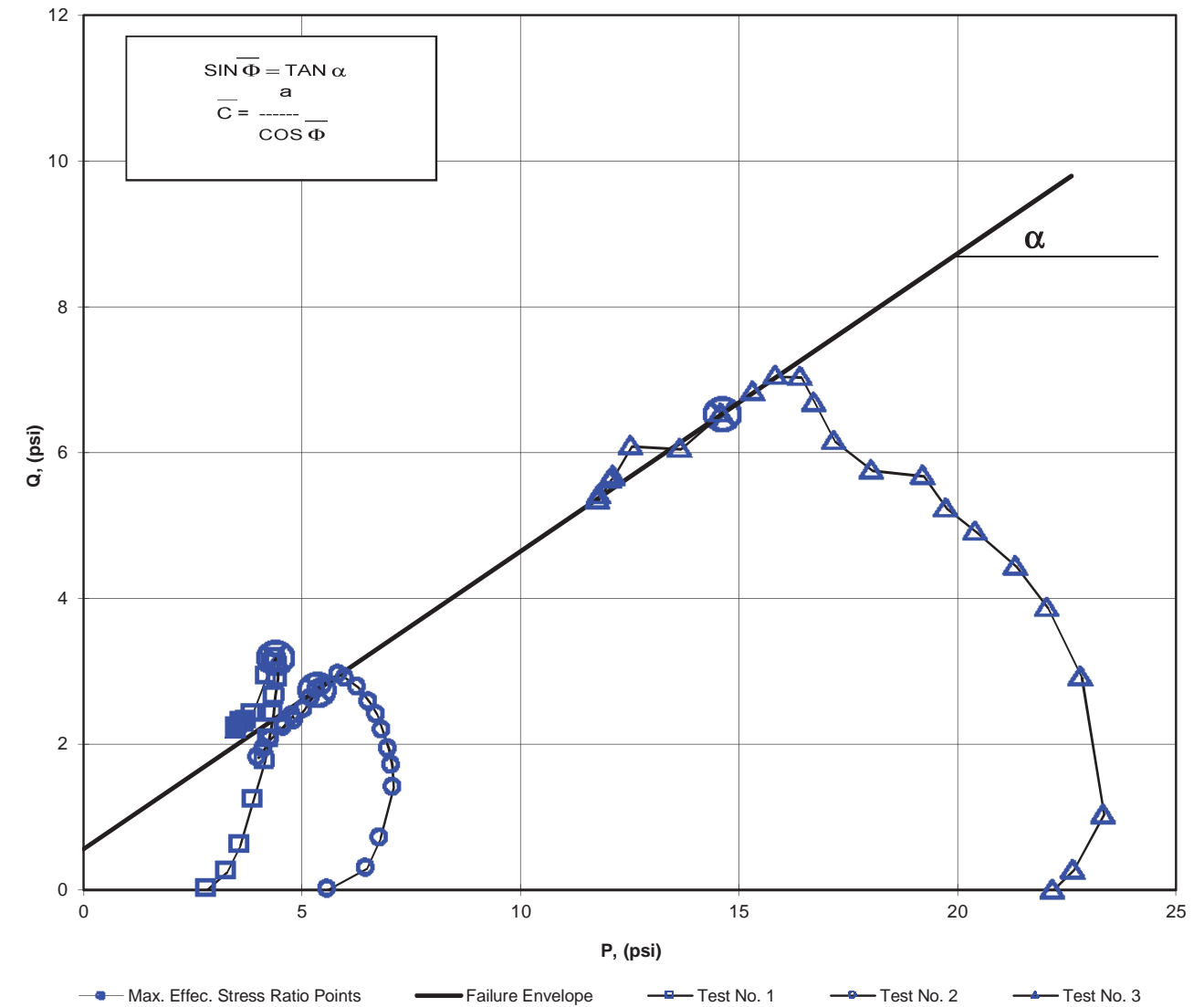
Elapsed Time (min)	Dial Reading (div)
Initial	3027.2
0.05	2993.0
0.10	2991.3
0.15	2991.4
0.20	2991.4
0.25	2991.4
0.30	2991.5
0.45	2992.2
0.55	2991.7
1.07	2992.3
2.32	2989.9
4.07	2980.5
9.07	2965.9
16.07	2953.5
25.07	2932.7
36.07	2915.3
49.07	2898.6
64.07	2884.7
81.07	2865.3
100.07	2851.7
121.07	2832.1
144.07	2817.8
180.07	2799.7
300.07	2752.8
520.08	2707.8
700.08	2683.2
960.08	2663.3
1440.08	2647.4
1680.08	2643.5
1920.08	2640.8
2160.08	2633.4
2400.08	2626.5

CONSOLIDATED UNDRAINED TRIAXIAL TEST WITH PORE PRESSURE READINGS
AASHTO T-297



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Consolidated Undrained Triaxial Test with Pore Pressure



a = 0.56	C = 0.61
alpha = 22.2	Phi = 24.10

Tested By: 129-04-0411 Date: 6/29/18 Checked By: GEM Date: 7/6/18

Tested By: 129-04-0411 Date: 7/2/18 Approved By: MPS Date: 7/16/18

MOHR TOTAL STRENGTH ENVELOPE
AASHTO T-297

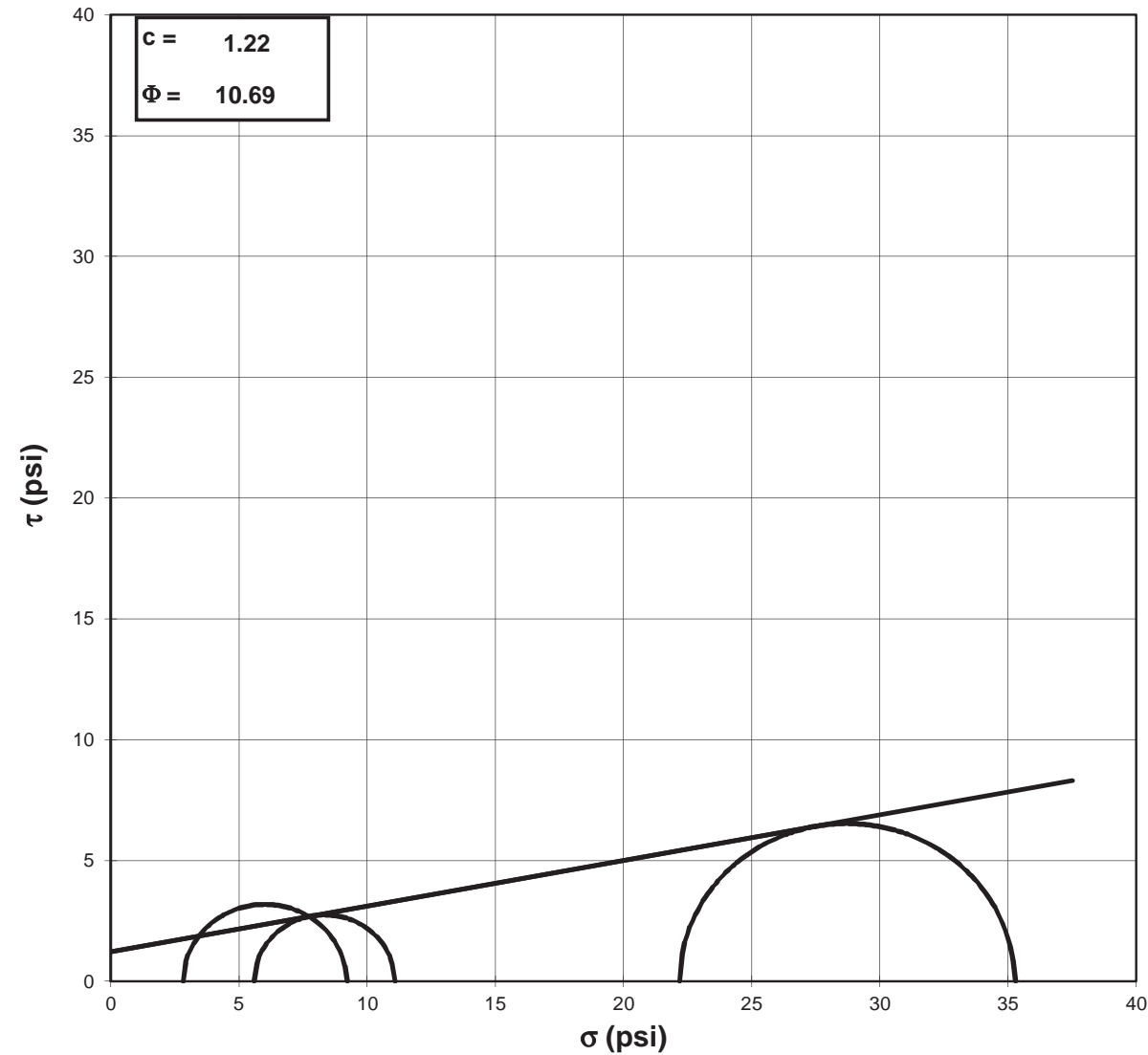


Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001
 Visual Description: TAN CLAYEY SAND (UNDISTURBED)

CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001
 Visual Description: TAN CLAYEY SAND (UNDISTURBED)



Stage No.	1
Test No.	1

INITIAL SAMPLE DIMENSIONS (in)

Length 1:	6.097	Diameter 1:	2.838
Length 2:	6.097	Diameter 2:	2.869
Length 3:	6.071	Diameter 3:	2.852
Length 4:	6.087	Diameter 4:	2.874
Avg. Length:	6.088	Avg. Diam.:	2.858

PRESSURES (psi)

Cell Pressure (psi)	42.8
Back Pressure (psi)	40.0
Eff. Conf. Pressure (psi)	2.8
Pore Pressure Response (%)	100

VOLUME CHANGE

Initial Burette Reading (ml)	24.0
Final Burette Reading (ml)	20.1
Final Change (ml)	3.9

MAXIMUM OBLIQUITY POINTS

P	=	4.40
Q	=	3.18

Initial Dial Reading (mil)	55
Dial Reading After Saturation (mil)	99
Dial Reading After Consolidation (mil)	115

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
10.7	0.000	40.0
13.8	0.001	39.8
18.3	0.003	39.8
26.2	0.009	40.2
32.9	0.015	40.4
36.8	0.021	40.6
41.2	0.030	40.9
44.4	0.039	41.1
47.5	0.052	41.3
50.4	0.073	41.5
51.5	0.105	41.6
50.8	0.141	41.6
48.7	0.177	41.6
42.2	0.220	41.4
41.1	0.251	41.4
40.9	0.293	41.4
41.1	0.351	41.5
43.8	0.754	41.5
43.5	0.768	41.5
43.8	0.782	41.5
44.0	0.796	41.5
42.8	0.818	41.5
43.5	0.872	41.5
43.0	0.924	41.5

Failure Based on Maximum Effective Principal Stress Ratio

NOTE: GRAPH NOT TO SCALE

Tested By: 129-04-0411 Date: 7/2/18 Approved By: MPS Date: 7/16/18

page 2 of 11 DCN: CT-S28 DATE: 4/12/13 REVISION: 3

Tested By: 129-04-0411 Date: 7/2/18 Input Checked By: GEM Date: 7/16/18

page 3 of 11 DCN: CT-S28 DATE: 4/12/13 REVISION: 3

Sigmatrax.xls

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

Effective Confining Pressure (psi)	2.8	Stage No.	1
		Test No.	1

Stage No.	1
Test No.	2

INITIAL SAMPLE DIMENSIONS (in)			
Length 1:	6.265	Diameter 1:	2.869
Length 2:	6.235	Diameter 2:	2.865
Length 3:	6.234	Diameter 3:	2.860
Length 4:	6.200	Diameter 4:	2.848
Avg. Length:	6.234	Avg. Diam.:	2.861

INITIAL DIMENSIONS		VOLUME CHANGE	
Initial Sample Length (in)	6.09	Volume After Consolidation (in ³)	37.98
Initial Sample Diameter (in)	2.86	Length After Consolidation (in)	6.03
Initial Sample Area (in ²)	6.42	Area After Consolidation (in ²)	6.300
Initial Sample Volume (in ³)	39.06		

PRESSURES (psi)	
Cell Pressure (psi)	45.6
Back Pressure (psi)	40.0
Eff. Conf. Pressure (psi)	5.6
Pore Pressure	
Response (%)	100

VOLUME CHANGE	
Initial Burette Reading (ml)	48.0
Final Burette Reading (ml)	25.6
Final Change (ml)	22.4

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
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MAXIMUM OBLIQUITY POINTS

\bar{P}	=	5.35
Q	=	2.75

Initial Dial Reading (mil)	8
Dial Reading After Saturation (mil)	31
Dial Reading After Consolidation (mil)	77

0.02	0.48	-0.21	3.53	3.0	1.159	-0.43	3.28	0.24
0.05	1.21	-0.16	4.20	3.0	1.403	-0.13	3.59	0.60
0.15	2.46	0.17	5.13	2.7	1.922	0.07	3.90	1.23
0.25	3.51	0.43	5.92	2.4	2.459	0.12	4.16	1.76
0.35	4.13	0.64	6.32	2.2	2.887	0.16	4.26	2.07
0.51	4.81	0.89	6.75	1.9	3.481	0.19	4.35	2.41
0.65	5.31	1.10	7.05	1.7	4.057	0.21	4.39	2.66
0.86	5.80	1.28	7.35	1.6	4.737	0.22	4.45	2.90
1.22	6.22	1.51	7.54	1.3	5.688	0.24	4.44	3.11
1.74	6.36	1.61	7.59	1.2	6.199	0.25	4.40	3.18
2.33	6.21	1.61	7.43	1.2	6.094	0.26	4.32	3.11
2.94	5.85	1.55	7.13	1.3	5.578	0.27	4.20	2.92
3.66	4.82	1.38	6.28	1.5	4.309	0.29	3.87	2.41
4.17	4.63	1.40	6.06	1.4	4.237	0.30	3.75	2.32
4.86	4.56	1.42	5.97	1.4	4.226	0.31	3.69	2.28
5.83	4.55	1.45	5.93	1.4	4.299	0.32	3.65	2.27
12.51	4.60	1.50	5.93	1.3	4.450	0.33	3.63	2.30
12.75	4.55	1.50	5.87	1.3	4.420	0.33	3.60	2.27
12.97	4.57	1.50	5.91	1.3	4.425	0.33	3.62	2.29
13.20	4.58	1.50	5.91	1.3	4.449	0.33	3.62	2.29
13.57	4.40	1.52	5.71	1.3	4.364	0.35	3.51	2.20
14.47	4.45	1.52	5.76	1.3	4.400	0.34	3.54	2.23
15.33	4.34	1.49	5.68	1.3	4.235	0.34	3.51	2.17

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
19.2	0.000	40.0
22.8	0.001	39.4
28.0	0.003	39.5
36.7	0.008	39.9
40.4	0.015	40.2
43.3	0.022	40.5
46.6	0.032	40.9
49.2	0.042	41.3
51.5	0.056	41.6
54.2	0.081	42.1
55.9	0.117	42.5
56.9	0.161	42.7
55.0	0.202	42.9
54.8	0.252	43.0
53.5	0.288	43.0
51.7	0.339	43.0
50.1	0.424	43.1
51.6	0.519	43.2
49.8	0.590	43.2
50.9	0.684	43.2
48.5	0.757	43.4
46.8	0.826	43.4
47.5	0.912	43.4
45.7	0.932	43.4

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

Effective Confining Pressure (psi)	5.6	Stage No.	1
		Test No.	2

INITIAL DIMENSIONS

Initial Sample Length (in)	6.23
Initial Sample Diameter (in)	2.86
Initial Sample Area (in ²)	6.43
Initial Sample Volume (in ³)	40.06

VOLUME CHANGE

Volume After Consolidation (in ³)	38.25
Length After Consolidation (in)	6.16
Area After Consolidation (in ²)	6.205

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
0.02	0.57	-0.60	6.77	6.2	1.092	-1.05	6.48	0.29
0.05	1.40	-0.50	7.50	6.1	1.230	-0.35	6.80	0.70
0.12	2.80	-0.10	8.50	5.7	1.492	-0.04	7.10	1.40
0.24	3.41	0.23	8.78	5.4	1.635	0.07	7.07	1.70
0.35	3.87	0.54	8.92	5.1	1.765	0.14	6.99	1.93
0.52	4.38	0.94	9.04	4.7	1.941	0.21	6.85	2.19
0.68	4.80	1.27	9.12	4.3	2.108	0.27	6.73	2.40
0.91	5.16	1.63	9.13	4.0	2.298	0.32	6.55	2.58
1.32	5.56	2.09	9.07	3.5	2.586	0.38	6.29	2.78
1.90	5.80	2.50	8.90	3.1	2.871	0.43	6.00	2.90
2.61	5.91	2.70	8.80	2.9	3.039	0.46	5.85	2.95
3.28	5.58	2.89	8.28	2.7	3.060	0.52	5.50	2.79
4.08	5.49	2.99	8.10	2.6	3.105	0.54	5.35	2.75
4.68	5.26	3.00	7.86	2.6	3.028	0.57	5.23	2.63
5.49	4.94	3.01	7.53	2.6	2.911	0.61	5.06	2.47
6.87	4.64	3.07	7.17	2.5	2.836	0.66	4.85	2.32
8.41	4.78	3.20	7.18	2.4	2.991	0.67	4.79	2.39
9.58	4.45	3.24	6.81	2.4	2.887	0.73	4.58	2.23
11.10	4.53	3.24	6.89	2.4	2.926	0.72	4.62	2.27
12.28	4.14	3.38	6.36	2.2	2.868	0.82	4.29	2.07
13.40	3.84	3.38	6.06	2.2	2.732	0.88	4.14	1.92
14.80	3.87	3.39	6.09	2.2	2.753	0.87	4.15	1.94
15.12	3.61	3.40	5.81	2.2	2.647	0.94	4.00	1.81

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

Stage No.	1
Test No.	3

INITIAL SAMPLE DIMENSIONS (in)

Length 1:	6.114	Diameter 1:	2.853
Length 2:	6.192	Diameter 2:	2.857
Length 3:	6.167	Diameter 3:	2.883
Length 4:	6.134	Diameter 4:	2.844
Avg. Length:	6.152	Avg. Diam.:	2.859

PRESSURES (psi)

Cell Pressure (psi)	62.2
Back Pressure (psi)	40.0
Eff. Conf. Pressure (psi)	22.2
Pore Pressure Response (%)	99

VOLUME CHANGE

Initial Burette Reading (ml)	72.0
Final Burette Reading (ml)	14.5
Final Change (ml)	57.5

MAXIMUM OBLIQUITY POINTS

\bar{P}	=	14.61
Q	=	6.53

Initial Dial Reading (mil)	31
Dial Reading After Saturation (mil)	88
Dial Reading After Consolidation (mil)	429

LOAD (LB)	DEFORMATION (IN)	PORE PRESSURE (PSI)
16.3	0.000	40.0
19.6	0.001	39.8
28.8	0.002	39.9
51.7	0.008	42.3
63.3	0.014	44.0
70.2	0.019	45.3
76.2	0.028	46.7
80.1	0.036	47.7
85.7	0.049	48.6
86.9	0.070	49.9
92.3	0.100	51.1
99.2	0.138	52.1
104.3	0.175	52.8
105.1	0.218	53.4
102.9	0.249	53.7
99.7	0.293	54.1
94.4	0.352	54.6
101.2	0.756	55.7
95.7	0.769	55.7
95.8	0.783	55.7
95.5	0.797	55.8
92.0	0.823	55.8
93.7	0.878	55.8

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

Effective Confining Pressure (psi)	22.2	Stage No.	1
		Test No	3

INITIAL DIMENSIONS

Initial Sample Length (in)	6.15
Initial Sample Diameter (in)	2.86
Initial Sample Area (in ²)	6.42
Initial Sample Volume (in ³)	39.50

VOLUME CHANGE

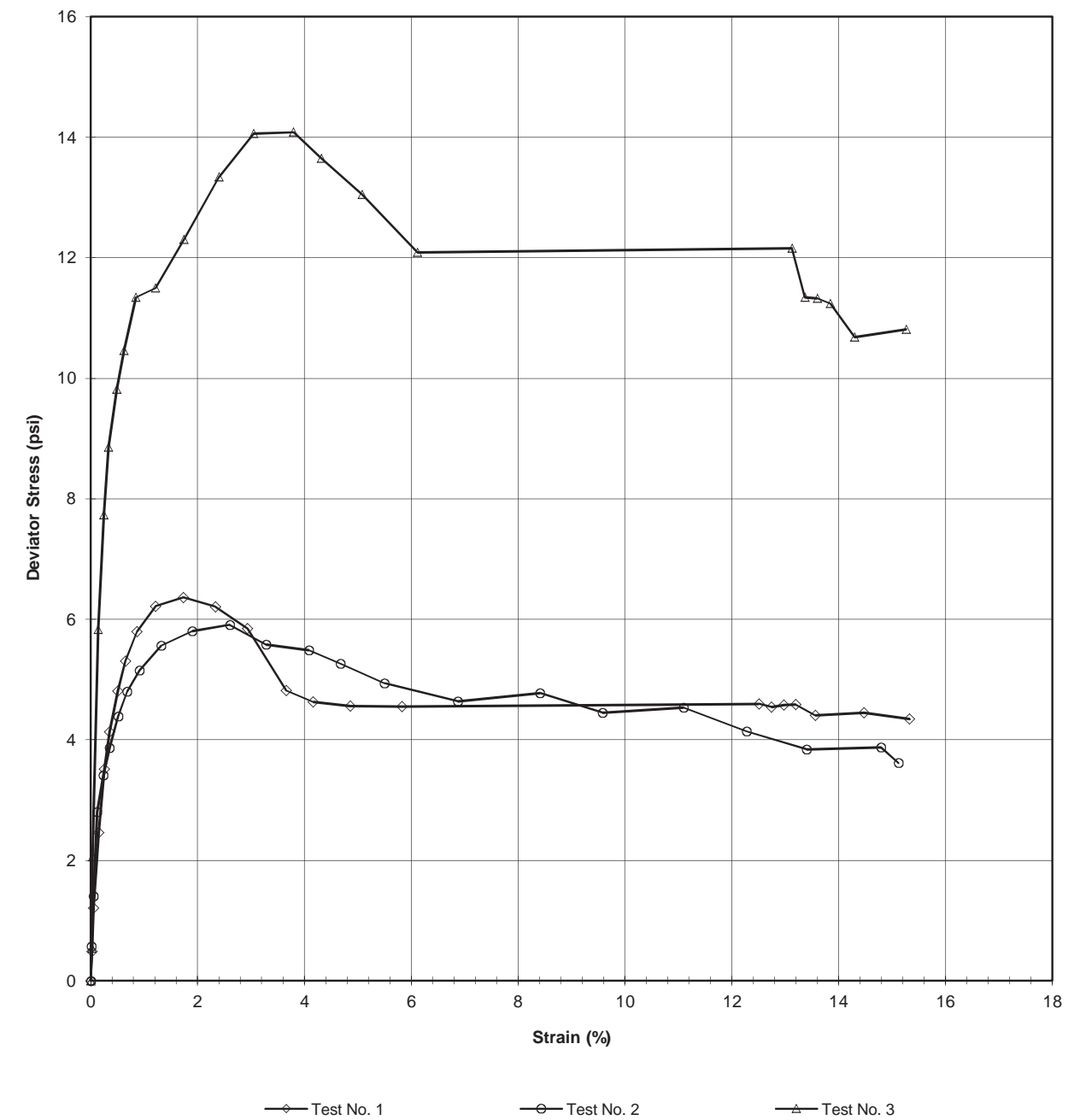
Volume After Consolidation (in ³)	34.89
Length After Consolidation (in)	5.75
Area After Consolidation (in ²)	6.064

Strain (%)	Deviation Stress	ΔU	$\bar{\sigma}_1$	$\bar{\sigma}_3$	Effective Principle Stress Ratio	\bar{A}	\bar{P}	Q
0.02	0.54	-0.20	22.94	22.4	1.024	-0.37	22.67	0.27
0.04	2.06	-0.13	24.39	22.3	1.092	-0.07	23.36	1.03
0.14	5.83	2.26	25.76	19.9	1.292	0.39	22.85	2.91
0.24	7.73	3.99	25.94	18.2	1.425	0.52	22.08	3.87
0.33	8.85	5.28	25.78	16.9	1.523	0.60	21.35	4.43
0.49	9.82	6.68	25.35	15.5	1.633	0.69	20.43	4.91
0.63	10.46	7.67	24.99	14.5	1.720	0.74	19.76	5.23
0.84	11.34	8.63	24.91	13.6	1.836	0.77	19.24	5.67
1.21	11.50	9.89	23.81	12.3	1.934	0.87	18.06	5.75
1.74	12.30	11.15	23.36	11.1	2.113	0.92	17.20	6.15
2.40	13.34	12.13	23.41	10.1	2.325	0.92	16.74	6.67
3.05	14.06	12.81	23.46	9.4	2.497	0.92	16.42	7.03
3.80	14.08	13.38	22.90	8.8	2.597	0.96	15.86	7.04
4.32	13.65	13.68	22.17	8.5	2.603	1.01	15.34	6.83
5.09	13.05	14.12	21.13	8.1	2.614	1.09	14.61	6.53
6.11	12.09	14.57	19.72	7.6	2.584	1.22	13.67	6.04
13.13	12.16	15.73	18.63	6.5	2.880	1.31	12.55	6.08
13.37	11.34	15.73	17.81	6.5	2.753	1.40	12.14	5.67
13.60	11.32	15.72	17.80	6.5	2.748	1.40	12.14	5.66
13.85	11.24	15.76	17.69	6.4	2.745	1.42	12.06	5.62
14.30	10.69	15.76	17.13	6.4	2.659	1.49	11.79	5.34
15.26	10.81	15.79	17.22	6.4	2.687	1.48	11.82	5.41

**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS**
AASHTO T-297



Client: ESP Associates Location: -L-STA 217+15, 23' RT
 Client Reference: R-1015 Site 9 MSE Wall CS34.368 Depth (ft): 9.8-11.8
 Project No.: R-2018-181-001 Sample No.: ST-3
 Lab ID: R-2018-181-001-001
 Visual Description: TAN CLAYEY SAND (UNDISTURBED)



Tested By: 129-04-0411 Date: 7/2/18 Approved By: MPS Date: 7/16/18



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**



**CONSOLIDATED UNDRAINED TRIAXIAL TEST
WITH PORE PRESSURE READINGS
AASHTO T-297**

Client: ESP Associates
 Client Reference: R-1015 Site 9 MSE Wall CS34.368
 Project No.: R-2018-181-001
 Lab ID: R-2018-181-001-001

Location: -L-STA 217+15, 23'
 Depth (ft): 9.8-11.8
 Sample No.: ST-3

Client: ESP Associates
 Client Reference: R-1015 Site 9 MSE Wall CS34.368
 Project No.: R-2018-181-001
 Lab ID: R-2018-181-001-001 Specific Gravity (Measured) 2.66

Visual Description: TAN CLAYEY SAND (UNDISTURBED)

SAMPLE CONDITION SUMMARY

	-L-STA 217+15, 23' RT	-L-STA 217+15, 23' RT	-L-STA 217+15, 23' RT
Location:	-L-STA 217+15, 23' RT	-L-STA 217+15, 23' RT	-L-STA 217+15, 23' RT
Depth (ft):	9.8-11.8	9.8-11.8	9.8-11.8
Sample No.:	ST-3	ST-3	ST-3
Test No.	T1	T2	T3
Deformation Rate (in/min)	0.0006	0.0006	0.0006
Back Pressure (psi)	40.0	40.0	40.0
Consolidation Time (days)	2	2	2
Moisture Content (%) (INITIAL)	64.2	64.2	64.2
Total Unit Weight (pcf)	99.7	99.3	100.8
Dry Unit Weight (pcf)	60.7	60.5	61.4
Moisture Content (%) (FINAL)	67.1	63.1	52.4
Initial State Void Ratio, e	1.735	1.745	1.705
Void Ratio at Shear, e	1.659	1.621	1.390

TEST 1 INITIAL



TEST 1 FINAL



TEST 2 INITIAL



TEST 2 FINAL



TEST 3 INITIAL



TEST 3 FINAL



Tested By: 129-04-0411 Date: 7/2/18 Input Checked By: GEM Date: 7/16/18

Tested By 129-04-0411 Date 7/2/18

Approved By MPS Date 7/16/18