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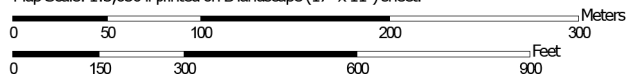
STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M04

Soil Map—Buncombe County, North Carolina



Soil Map may not be valid at this scale.

Map Scale: 1:3,650 if printed on B landscape (17" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 17N WGS84



Natural Resources Conservation Service


Web Soil Survey
National Cooperative Soil Survey

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MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils


 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole

 Slide or Slip


 Sodic Spot


 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals


Transportation

 Rails


 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

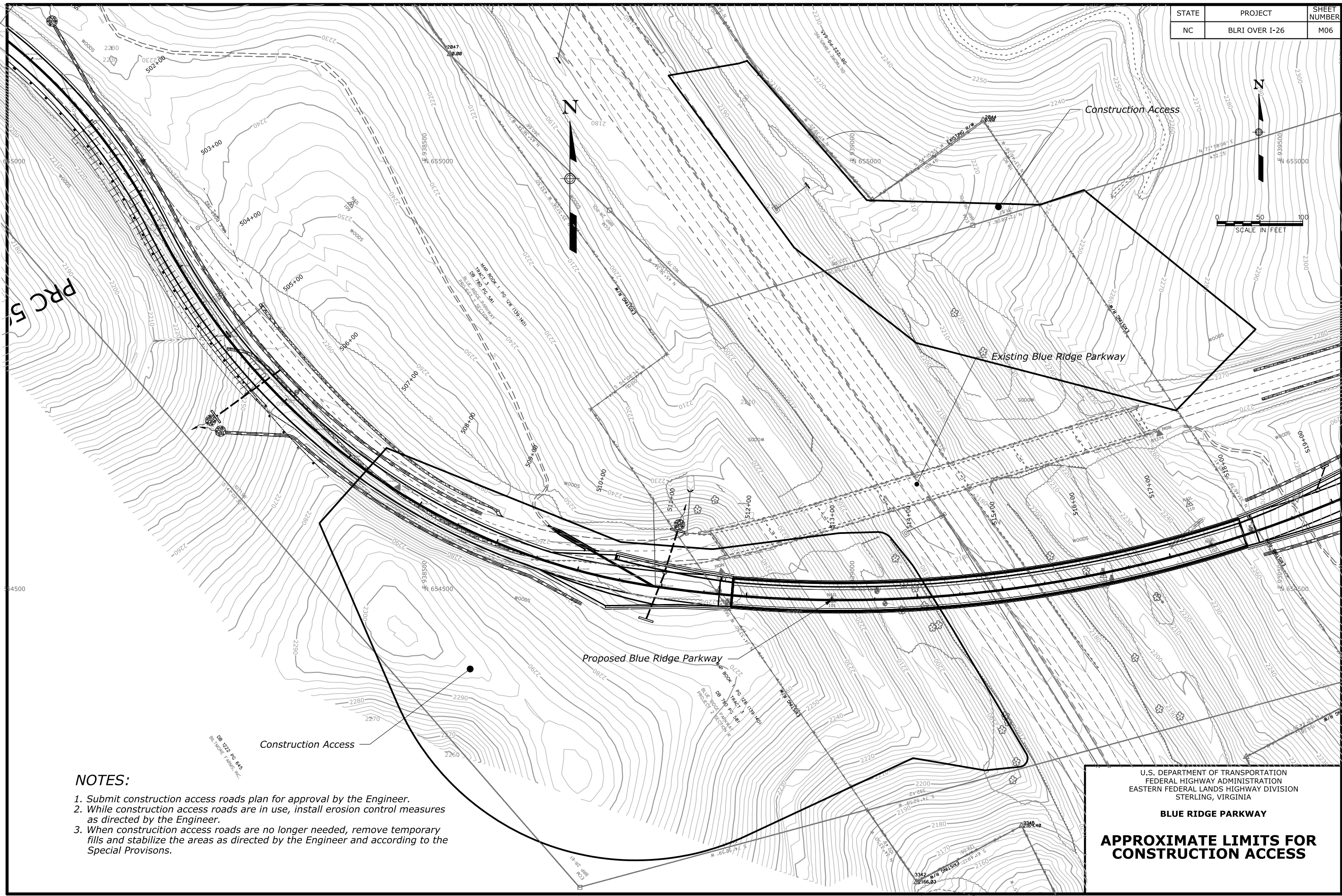
Map Unit Legend

Buncombe County, North Carolina (NC021)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CsC	Clifton sandy loam, 8 to 15 percent slopes	13.3	13.4%
CsD	Clifton sandy loam, 15 to 30 percent slopes	27.0	27.2%
CuC	Clifton-Urban land complex, 8 to 15 percent slopes	1.4	1.5%
CuD	Clifton-Urban land complex, 15 to 30 percent slopes	4.5	4.5%
EwC	Evard-Cowee complex, 8 to 15 percent slopes, stony	1.9	1.9%
EwD	Evard-Cowee complex, basin, 15 to 30 percent slopes, stony	4.6	4.6%
EwE	Evard-Cowee complex, basin, 30 to 50 percent slopes, stony	25.9	26.1%
TaC	Tate loam, 8 to 15 percent slopes	0.1	0.1%
TaD	Tate loam, 15 to 30 percent slopes	1.5	1.5%
TmC	Tate-Urban land complex, 8 to 15 percent slopes	3.4	3.4%
TmD	Tate-Urban land complex, 15 to 30 percent slopes	0.8	0.8%
UhE	Udorthents-Urban land complex, 2 to 50 percent slopes	14.8	15.0%
Totals for Area of Interest		99.1	100.0%

U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
EASTERN FEDERAL LANDS HIGHWAY DIVISION
STERLING, VIRGINIA

EROSION & SEDIMENT CONTROL NARRATIVE

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M06



NOTES:

1. Submit construction access roads plan for approval by the Engineer.
2. While construction access roads are in use, install erosion control measures as directed by the Engineer.
3. When construction access roads are no longer needed, remove temporary fills and stabilize the areas as directed by the Engineer and according to the Special Provisions.

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BLUE RIDGE PARKWAY

APPROXIMATE LIMITS FOR CONSTRUCTION ACCESS

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 BB 122 PG 6/5
 BL MOORE PARRIS, INC.

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M07

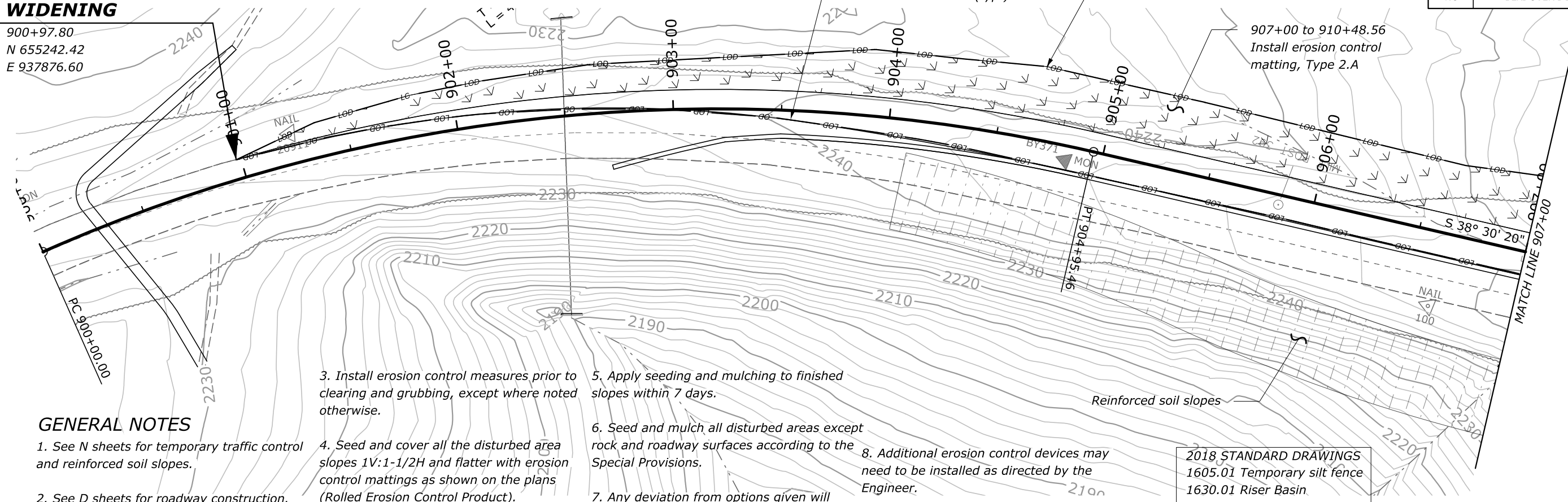
BEGIN TEMPORARY WIDENING

900+97.80
N 655242.42
E 937876.60

Limits of construction (typ.)

Limits of disturbance (typ.)

907+00 to 910+48.56
Install erosion control matting, Type 2.A



GENERAL NOTES

1. See N sheets for temporary traffic control and reinforced soil slopes.
2. See D sheets for roadway construction.

3. Install erosion control measures prior to clearing and grubbing, except where noted otherwise.

4. Seed and cover all the disturbed area slopes 1V:1-1/2H and flatter with erosion control mattings as shown on the plans (Rolled Erosion Control Product).

5. Apply seeding and mulching to finished slopes within 7 days.

6. Seed and mulch all disturbed areas except rock and roadway surfaces according to the Special Provisions.

7. Any deviation from options given will require prior approval by Engineer.

8. Additional erosion control devices may need to be installed as directed by the Engineer.

2018 STANDARD DRAWINGS
1605.01 Temporary silt fence
1630.01 Riser Basin
1631.01 Matting Installation

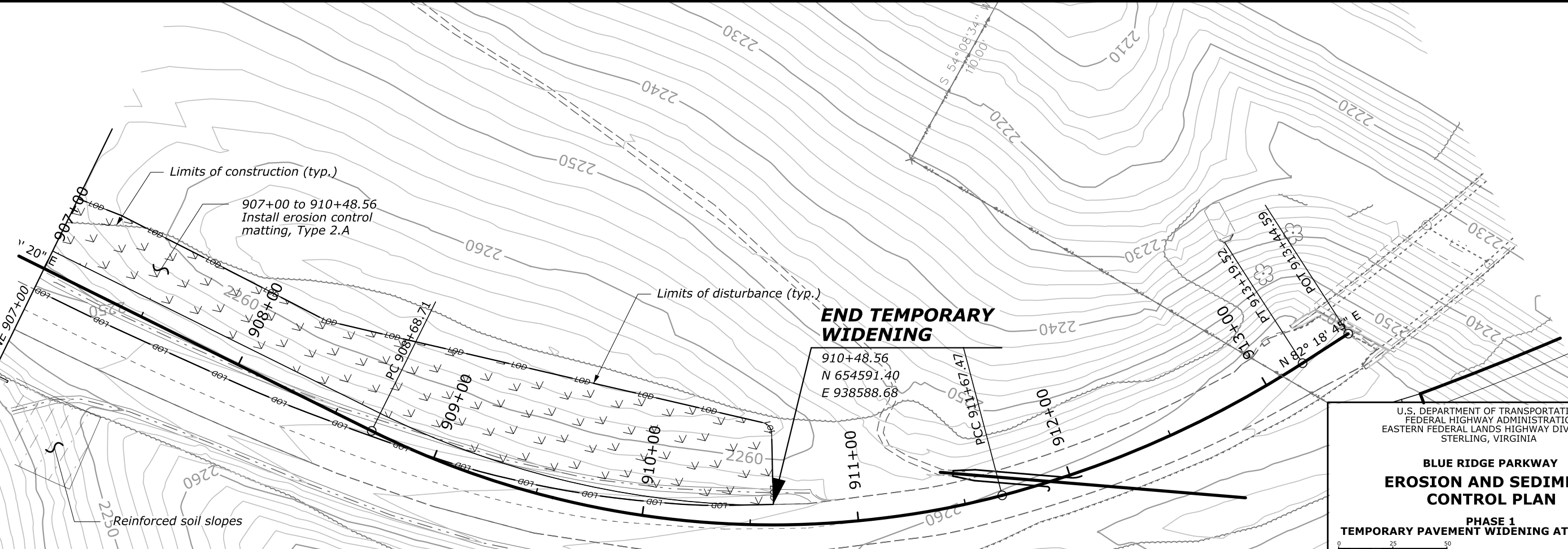
END TEMPORARY WIDENING

910+48.56
N 654591.40
E 938588.68

Limits of construction (typ.)

Limits of disturbance (typ.)

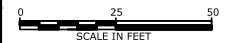
907+00 to 910+48.56
Install erosion control matting, Type 2.A



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STERLING, VIRGINIA

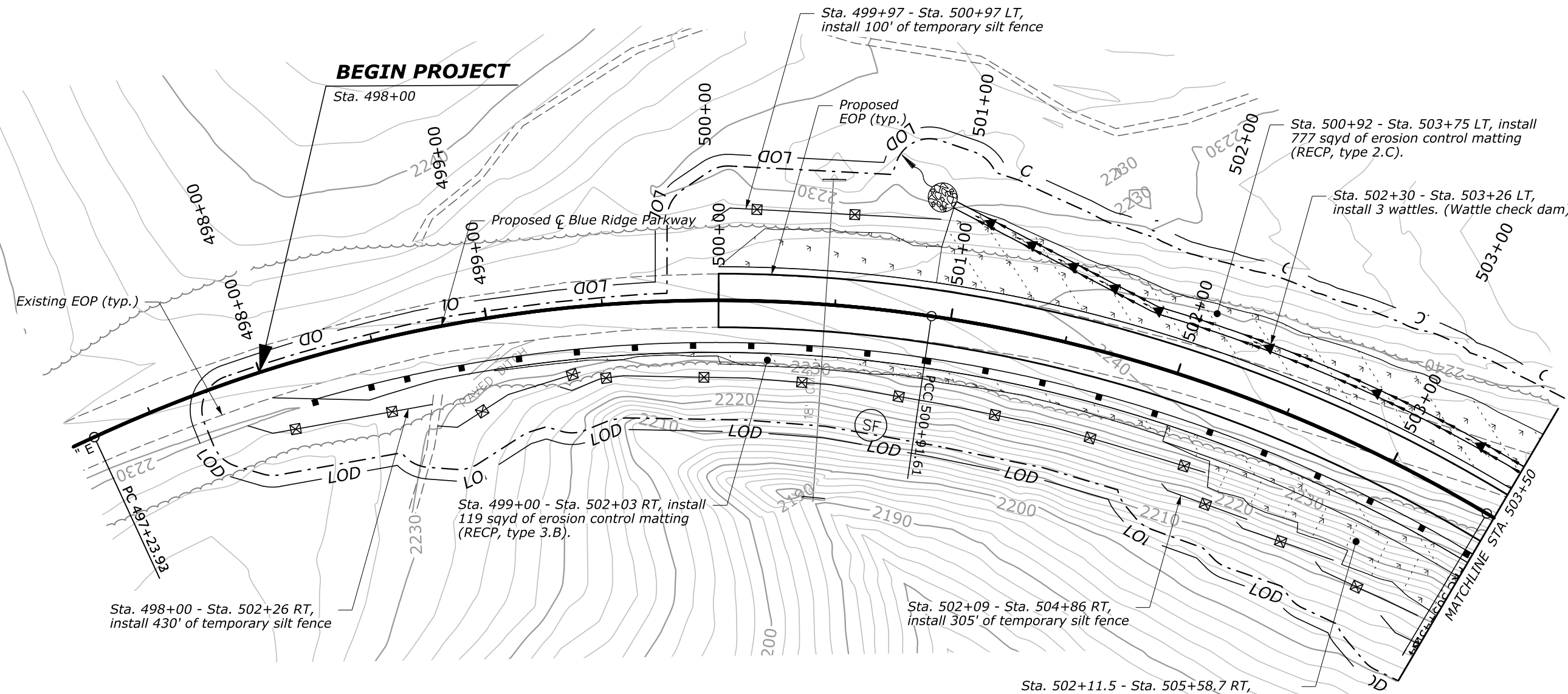
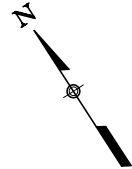
**BLUE RIDGE PARKWAY
EROSION AND SEDIMENT CONTROL PLAN**

**PHASE 1
TEMPORARY PAVEMENT WIDENING AT SOUTH END**



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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M08



Sta. 498+00 - Sta. 502+26 RT,
install 430' of temporary silt fence

Sta. 499+00 - Sta. 502+03 RT, install
119 sqyd of erosion control matting
(RECP, type 3.B).

Sta. 499+97 - Sta. 500+97 LT,
install 100' of temporary silt fence

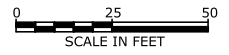
Sta. 500+92 - Sta. 503+75 LT, install
777 sqyd of erosion control matting
(RECP, type 2.C).

Sta. 502+30 - Sta. 503+26 LT,
install 3 wattles. (Wattle check dam)

Sta. 502+09 - Sta. 504+86 RT,
install 305' of temporary silt fence

Sta. 502+11.5 - Sta. 505+58.7 RT,
install 825 sqyd of erosion control matting
(RECP, type 4), on RSS slopes

- 2018 STANDARD DRAWINGS
- 1605.01 Temporary silt fence
 - 1630.01 Riser Basin
 - 1631.01 Matting Installation



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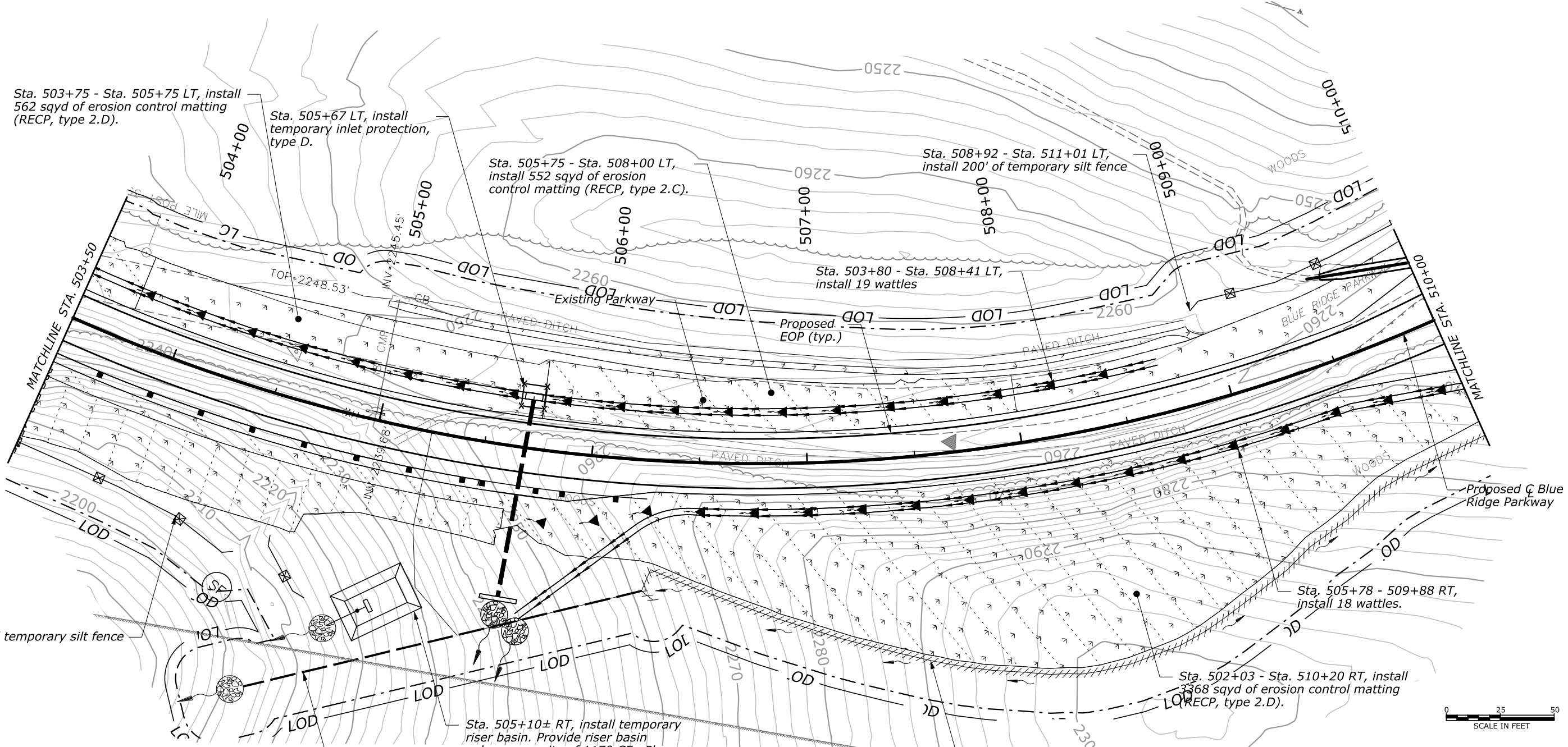
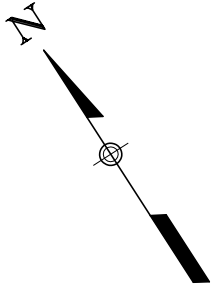
BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 2
SOUTH REINFORCED SOIL SLOPE CONSTRUCTION

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M09



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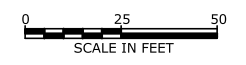
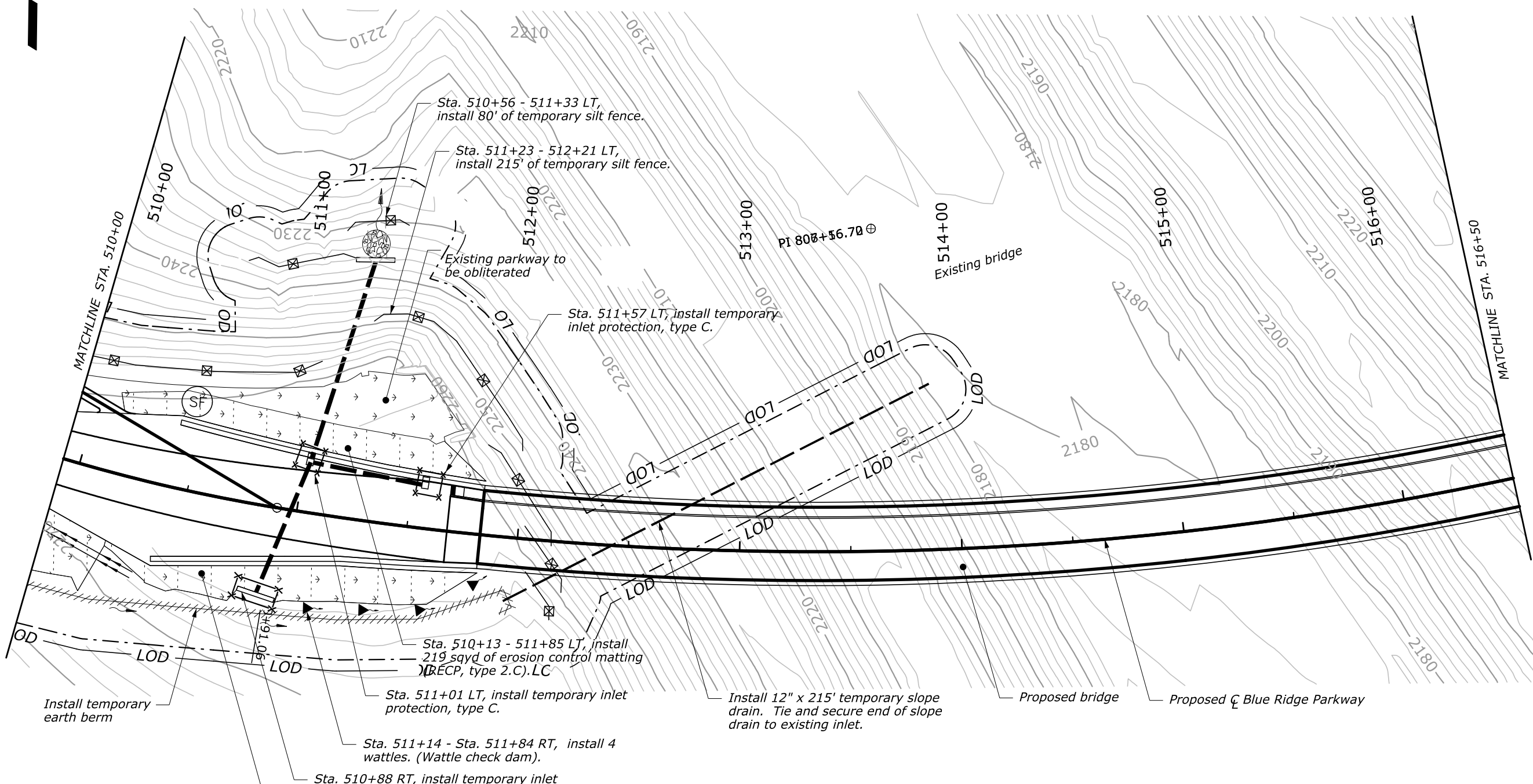
BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 2
 SOUTH REINFORCED SOIL SLOPE CONSTRUCTION

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M10



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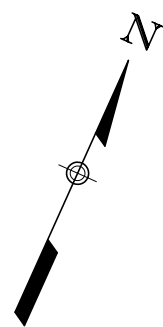
BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

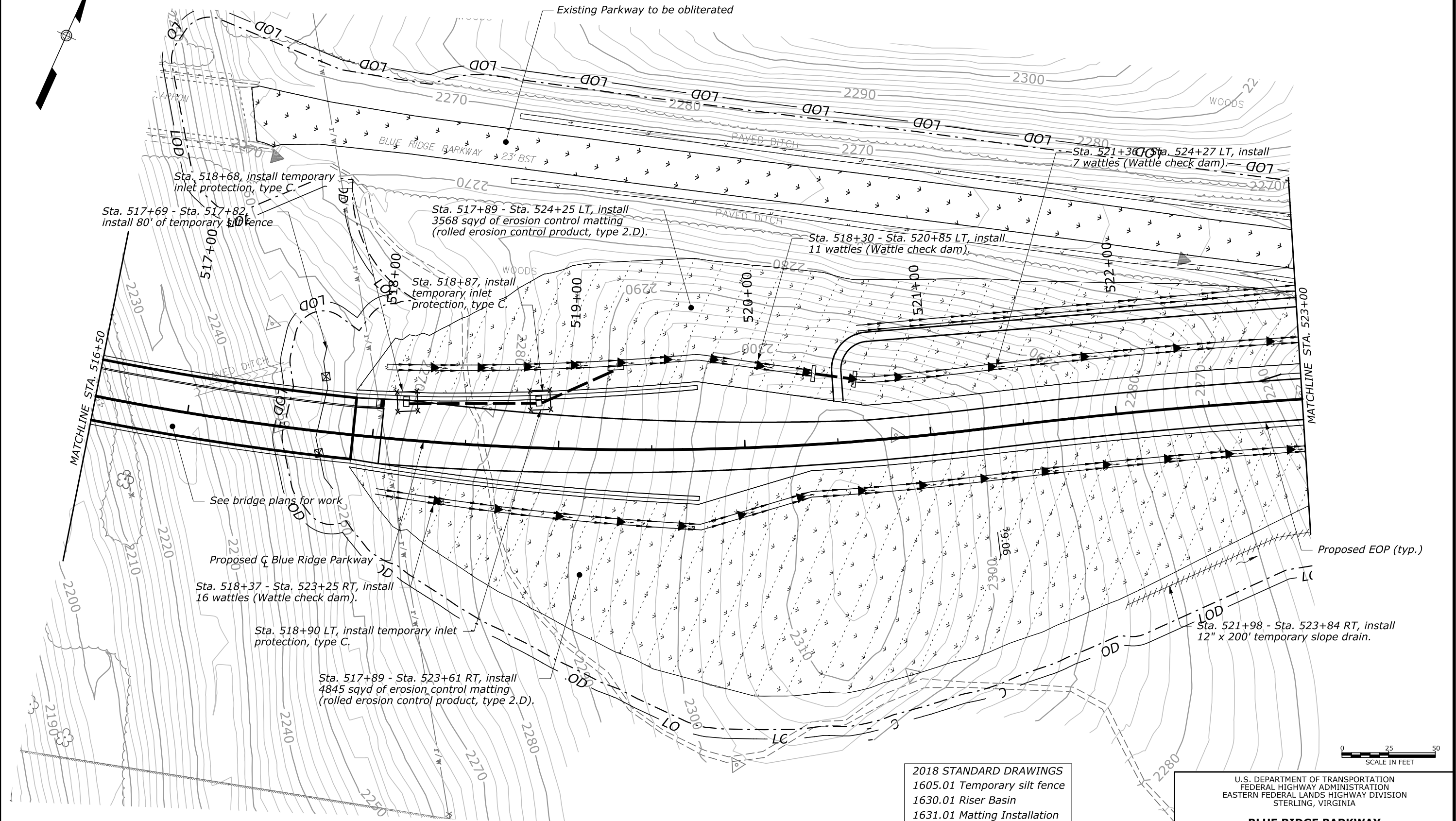
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 SOUTH REINFORCED SOIL SLOPE CONSTRUCTION

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M11



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2018 STANDARD DRAWINGS
 1605.01 Temporary silt fence
 1630.01 Riser Basin
 1631.01 Matting Installation

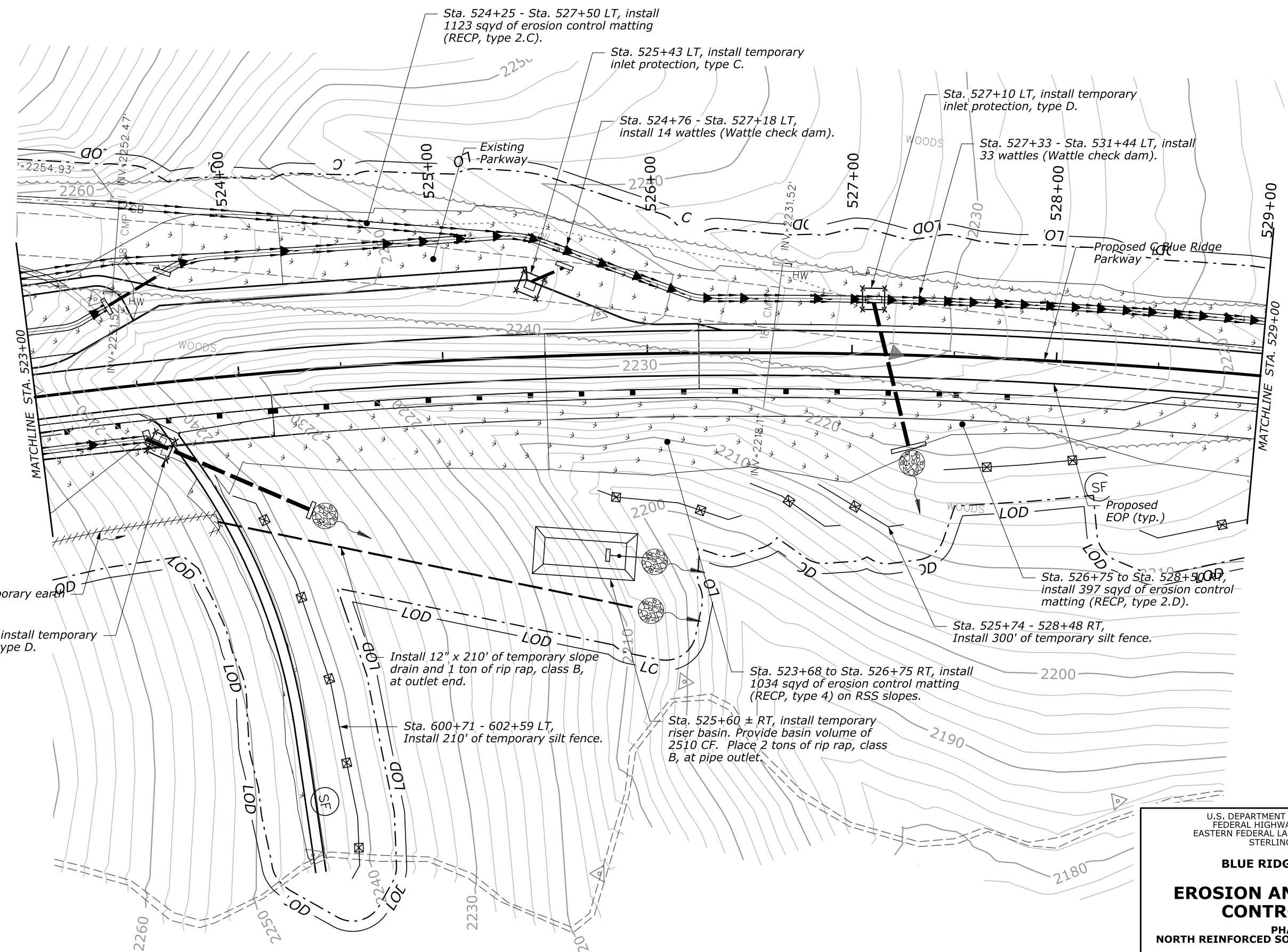
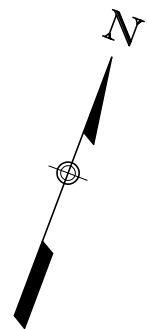
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BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 3
 NORTH REINFORCED SOIL SLOPE CONSTRUCTION

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M12



Install temporary earth berm
 Sta. 523+57 RT, install temporary inlet protection, type D.

Install 12" x 210' of temporary slope drain and 1 ton of rip rap, class B, at outlet end.

Sta. 523+68 to Sta. 526+75 RT, install 1034 sqyd of erosion control matting (RECP, type 4) on RSS slopes.

Sta. 525+60 ± RT, install temporary riser basin. Provide basin volume of 2510 CF. Place 2 tons of rip rap, class B, at pipe outlet.

Sta. 600+71 - 602+59 LT, Install 210' of temporary silt fence.

Sta. 525+74 - 528+48 RT, Install 300' of temporary silt fence.

Sta. 526+75 to Sta. 528+90 RT, install 397 sqyd of erosion control matting (RECP, type 2.D).



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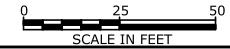
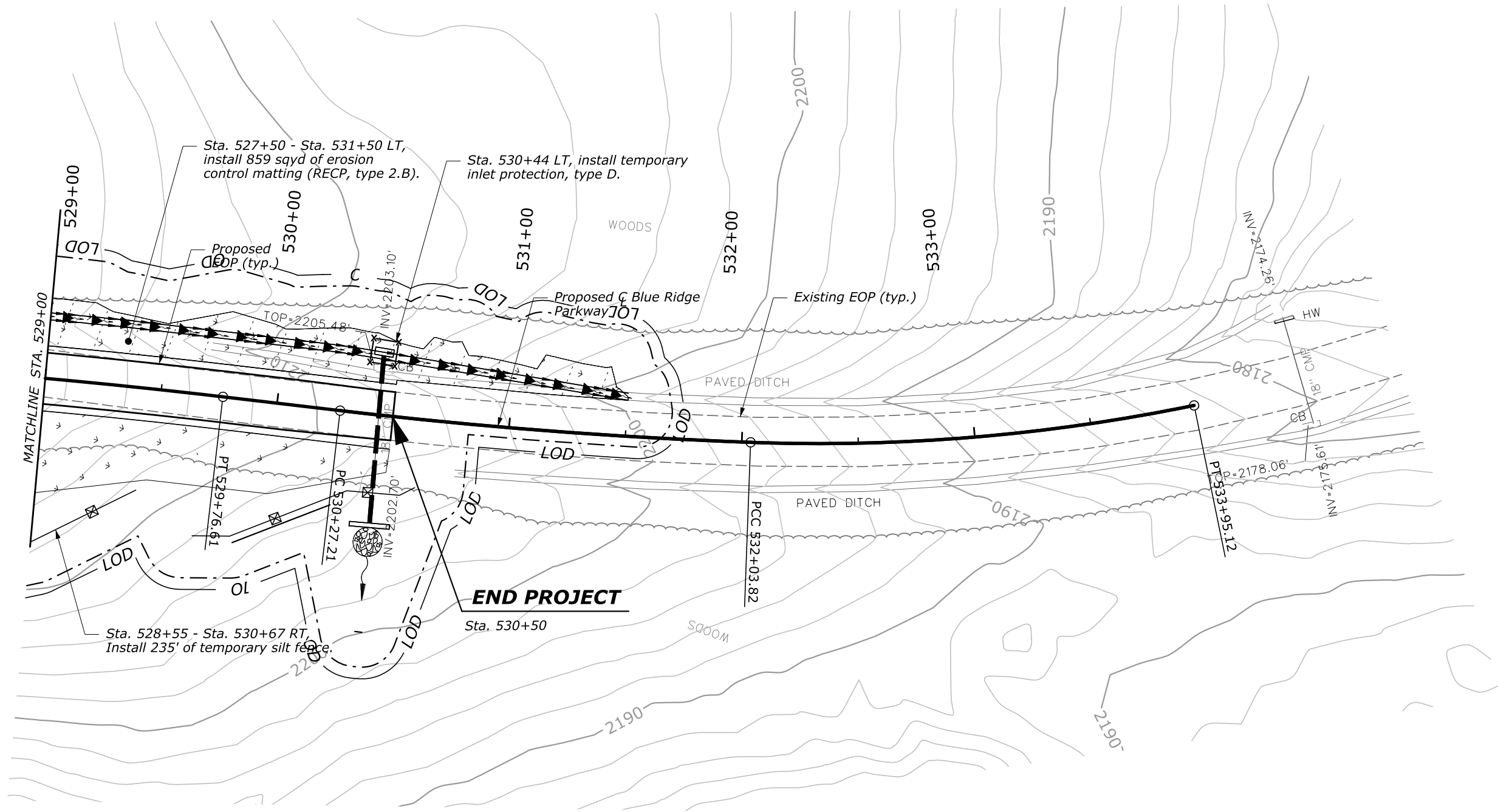
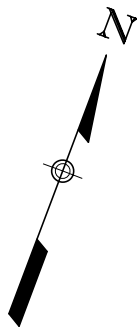
BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 3
 NORTH REINFORCED SOIL SLOPE CONSTRUCTION

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M13



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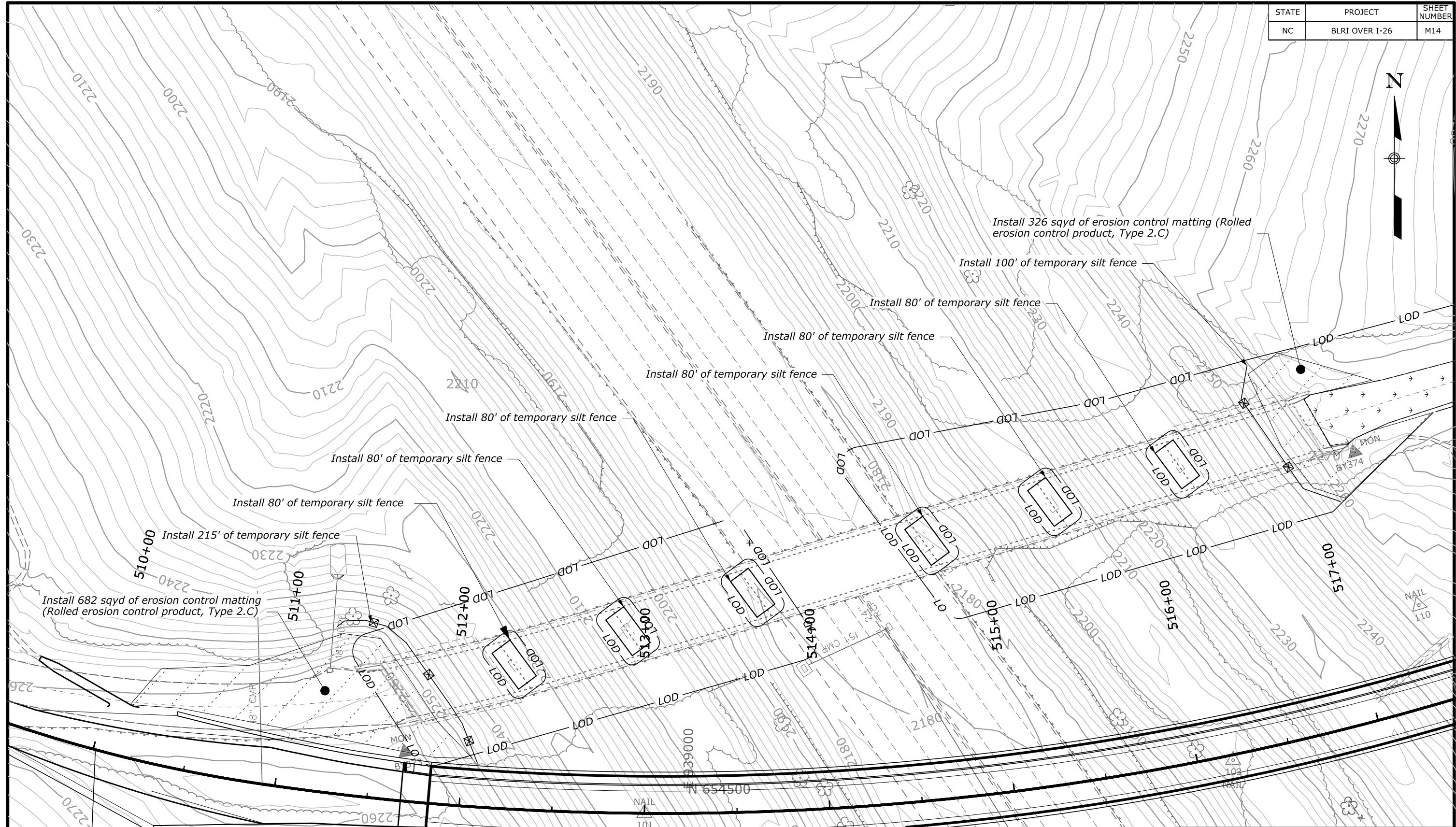
BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 3
 NORTH REINFORCED SOIL SLOPE CONSTRUCTION

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NC	BLRI OVER I-26	M14



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BLUE RIDGE PARKWAY

EROSION AND SEDIMENT CONTROL PLAN

PHASE 4
OBLITERATION OF EXISTING ROAD
BRIDGE REMOVAL

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M15

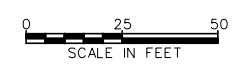
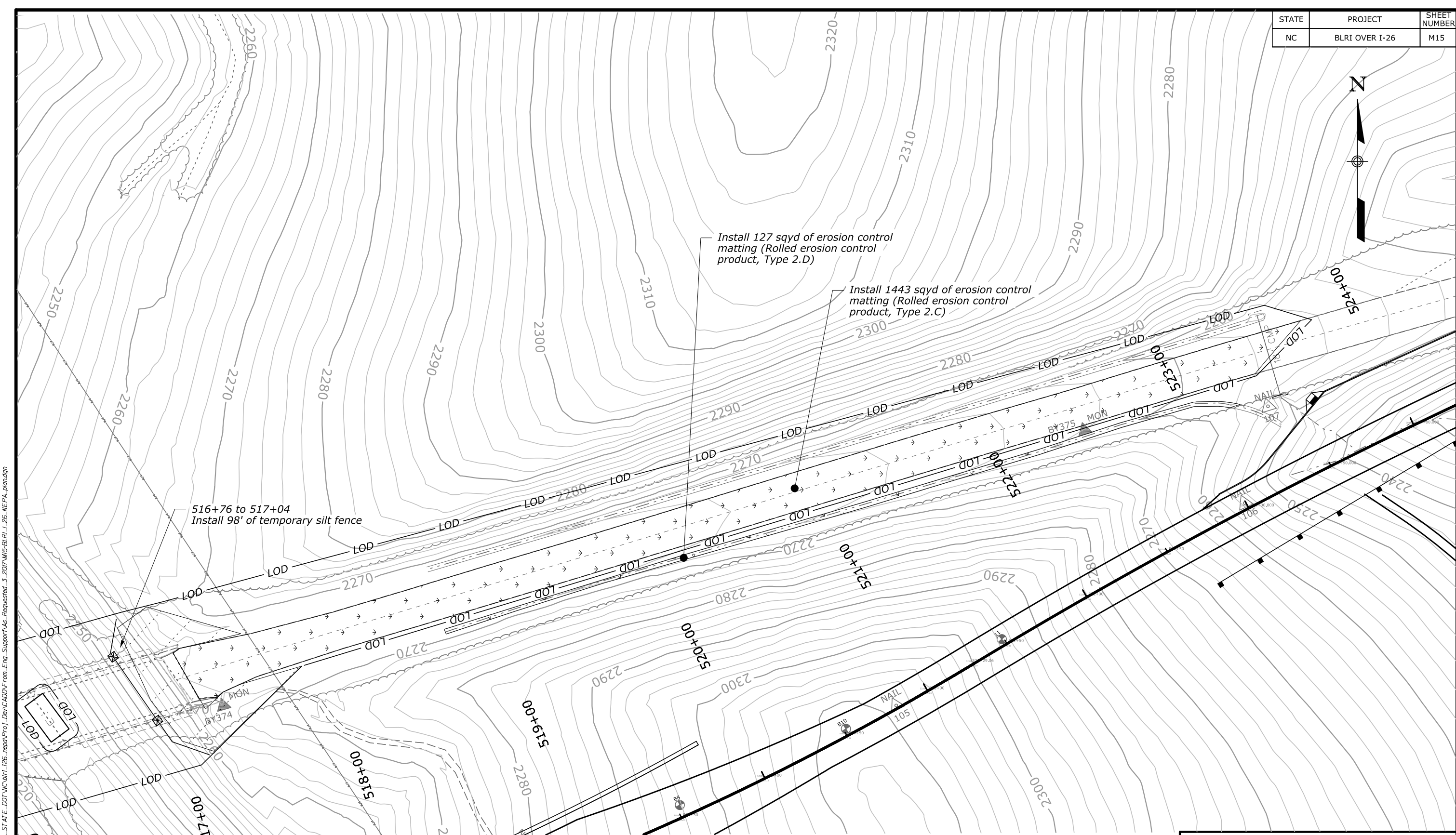


Install 127 sqyd of erosion control matting (Rolled erosion control product, Type 2.D)

Install 1443 sqyd of erosion control matting (Rolled erosion control product, Type 2.C)

516+76 to 517+04
Install 98' of temporary silt fence

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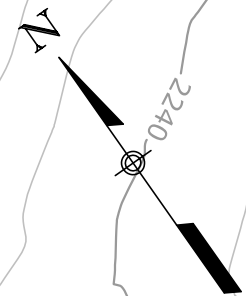
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY

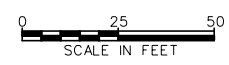
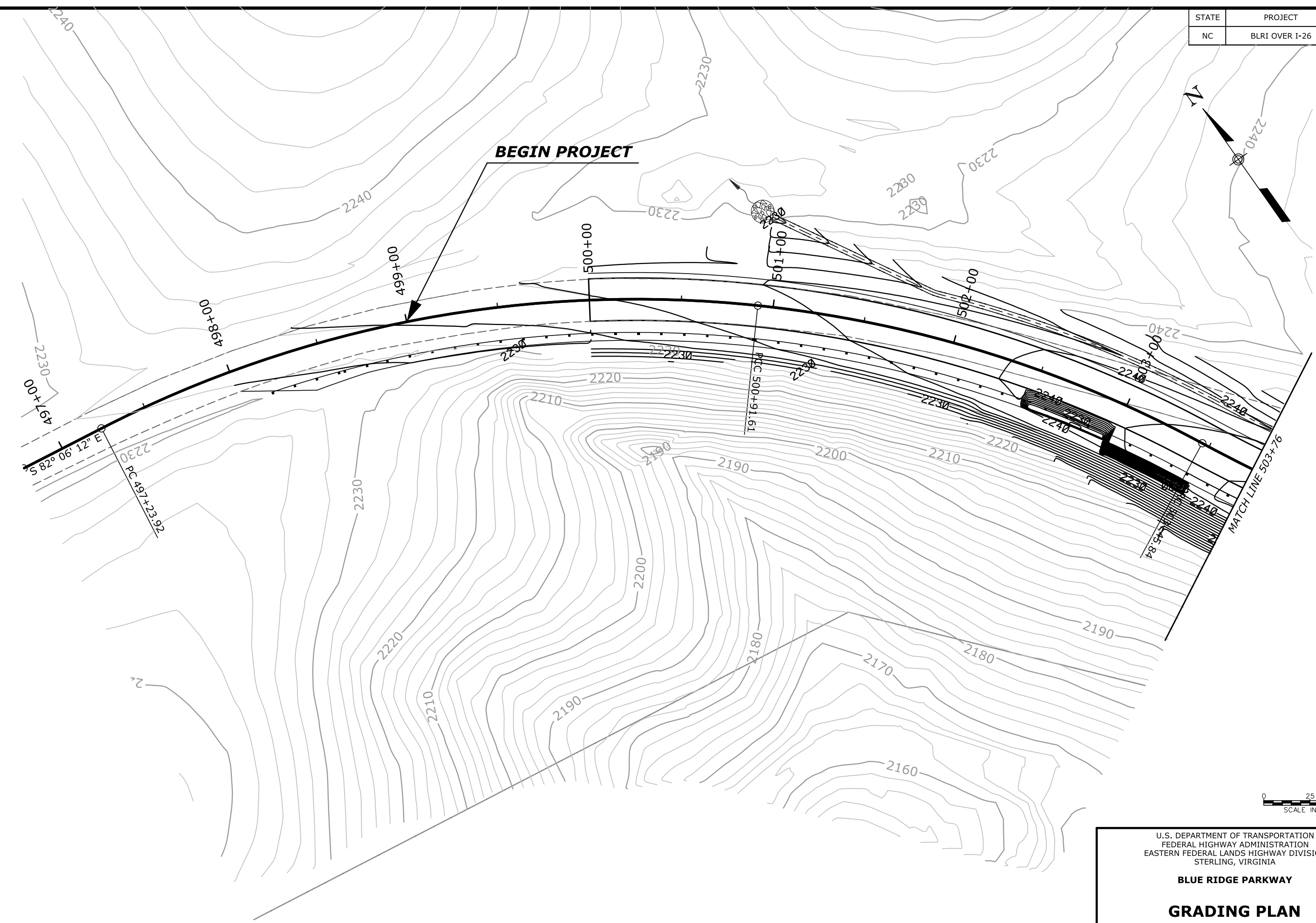
EROSION AND SEDIMENT CONTROL PLAN

PHASE 4
OBLITERATION OF EXISTING ROAD AND BRIDGE REMOVAL

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M16



BEGIN PROJECT



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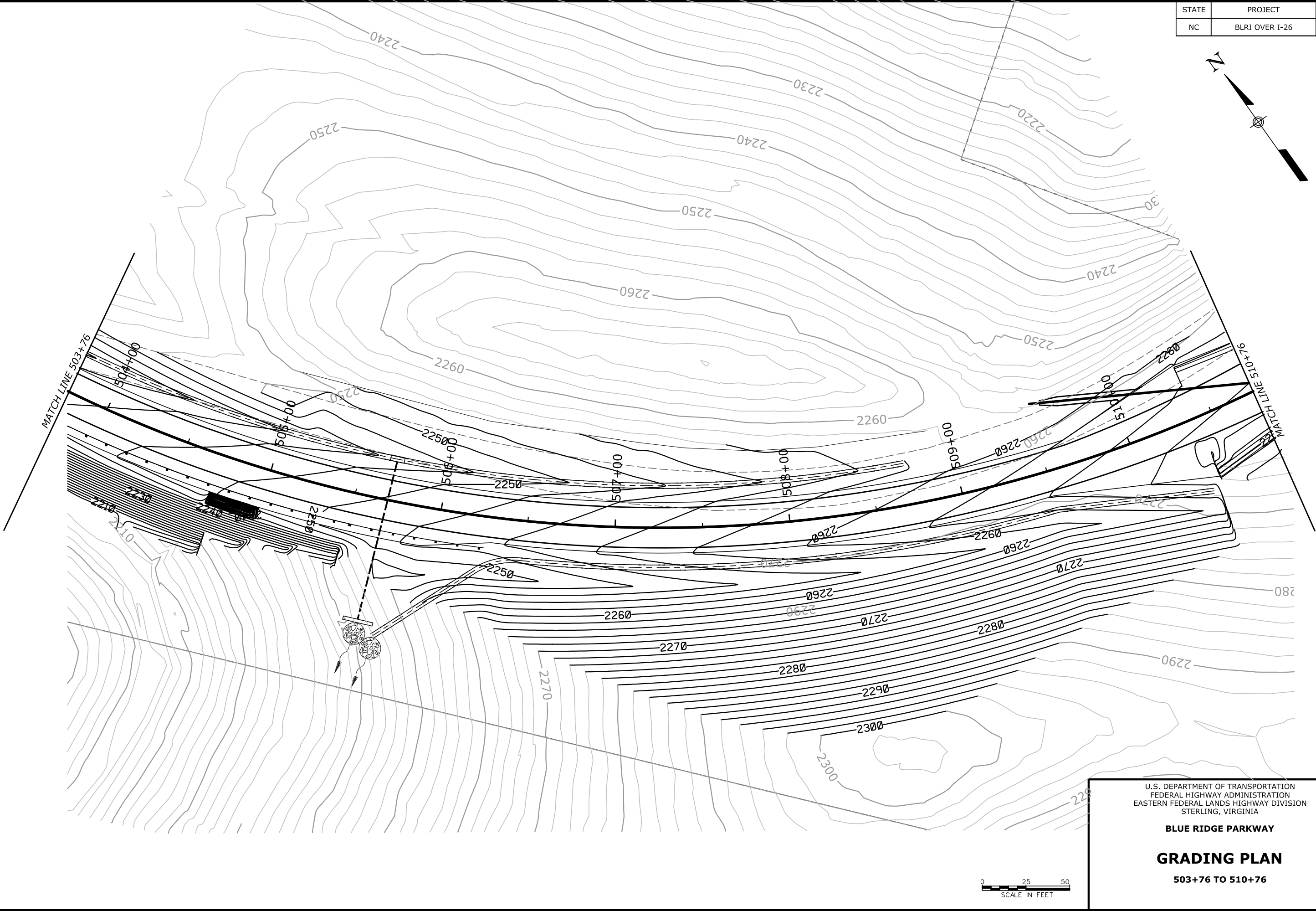
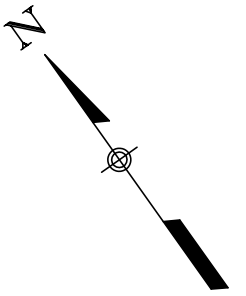
BLUE RIDGE PARKWAY

GRADING PLAN

499+00 TO 503+76

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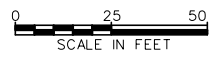


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BLUE RIDGE PARKWAY

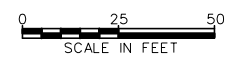
GRADING PLAN

503+76 TO 510+76



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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M18



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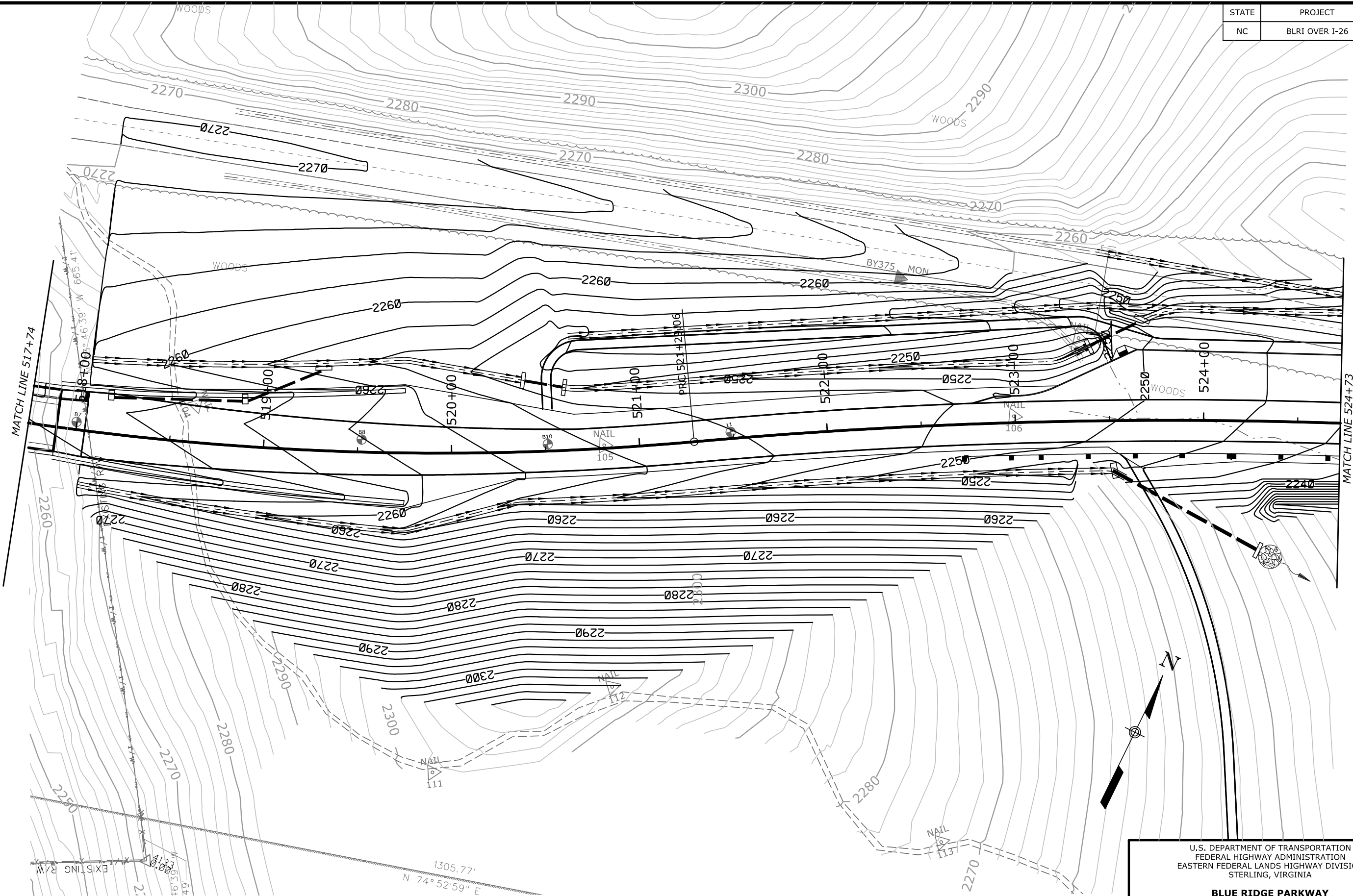
BLUE RIDGE PARKWAY

GRADING PLAN

510+76 TO 517+76

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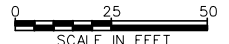
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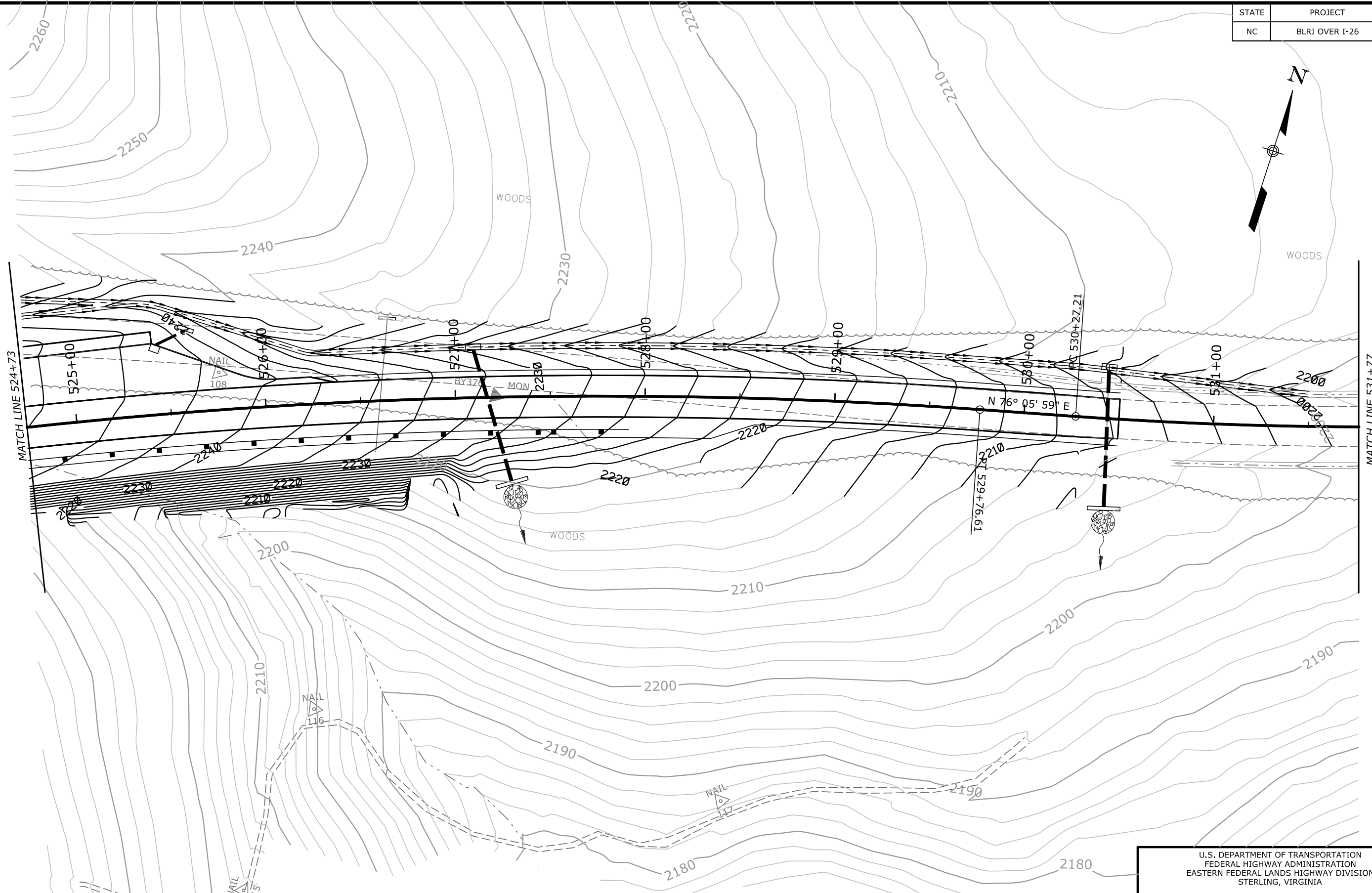
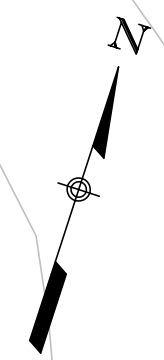
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 STERLING, VIRGINIA

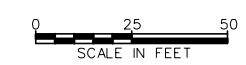
BLUE RIDGE PARKWAY
GRADING PLAN
 517+74 TO 524+43



STATE	PROJECT	SHEET NUMBER
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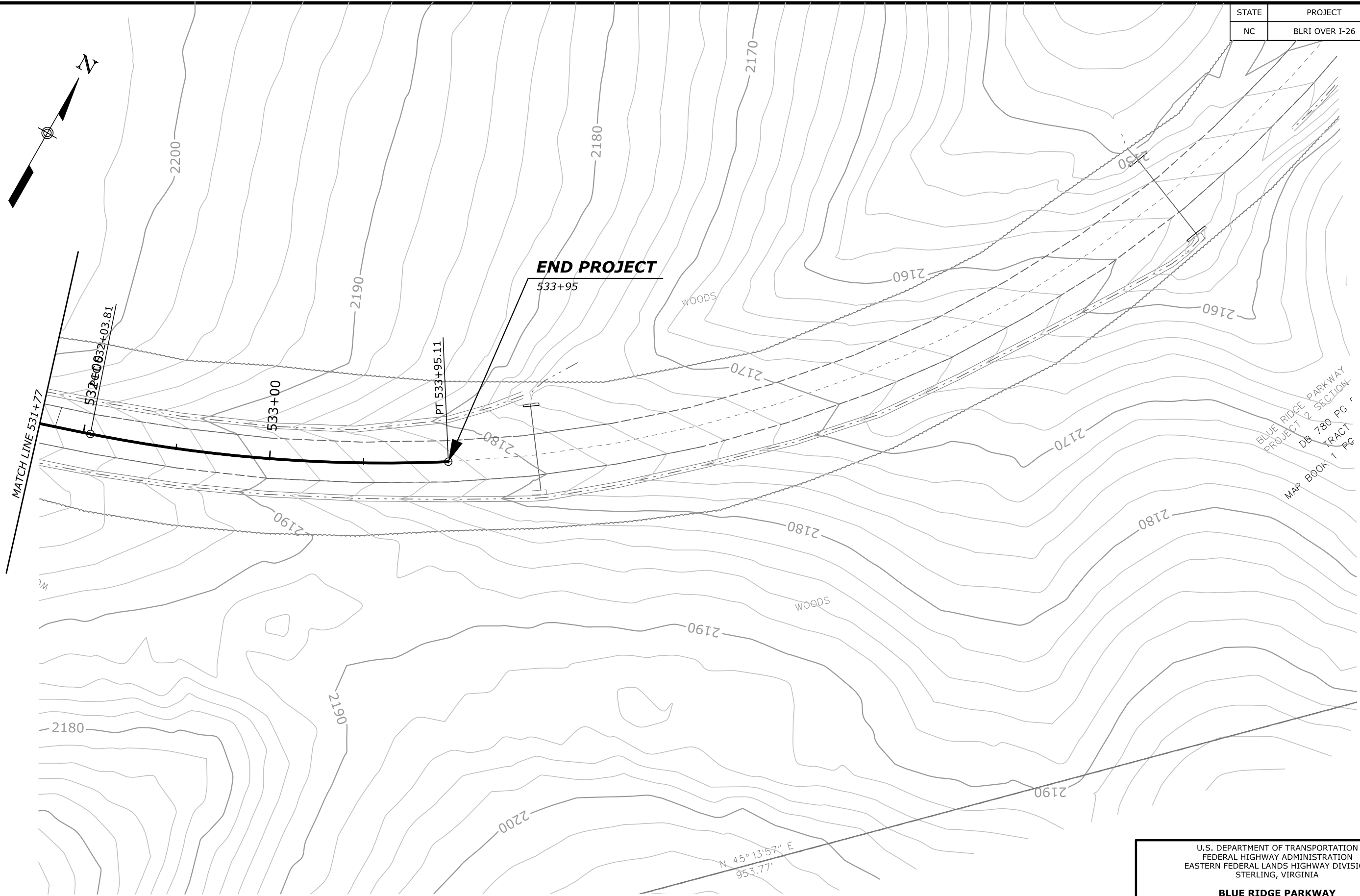
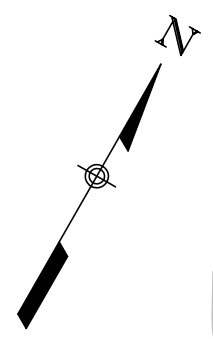
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY

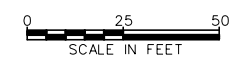
GRADING PLAN

524+73 to 531+77

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M21



BLUE RIDGE PARKWAY
 PROJECT 2 SECTION
 DB 780 PG 1
 TRACT
 MAP BOOK 1 PG



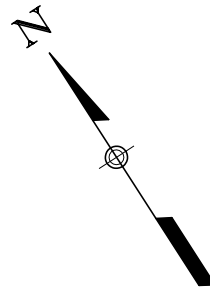
U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION
 EASTERN FEDERAL LANDS HIGHWAY DIVISION
 STERLING, VIRGINIA

BLUE RIDGE PARKWAY

GRADING PLAN

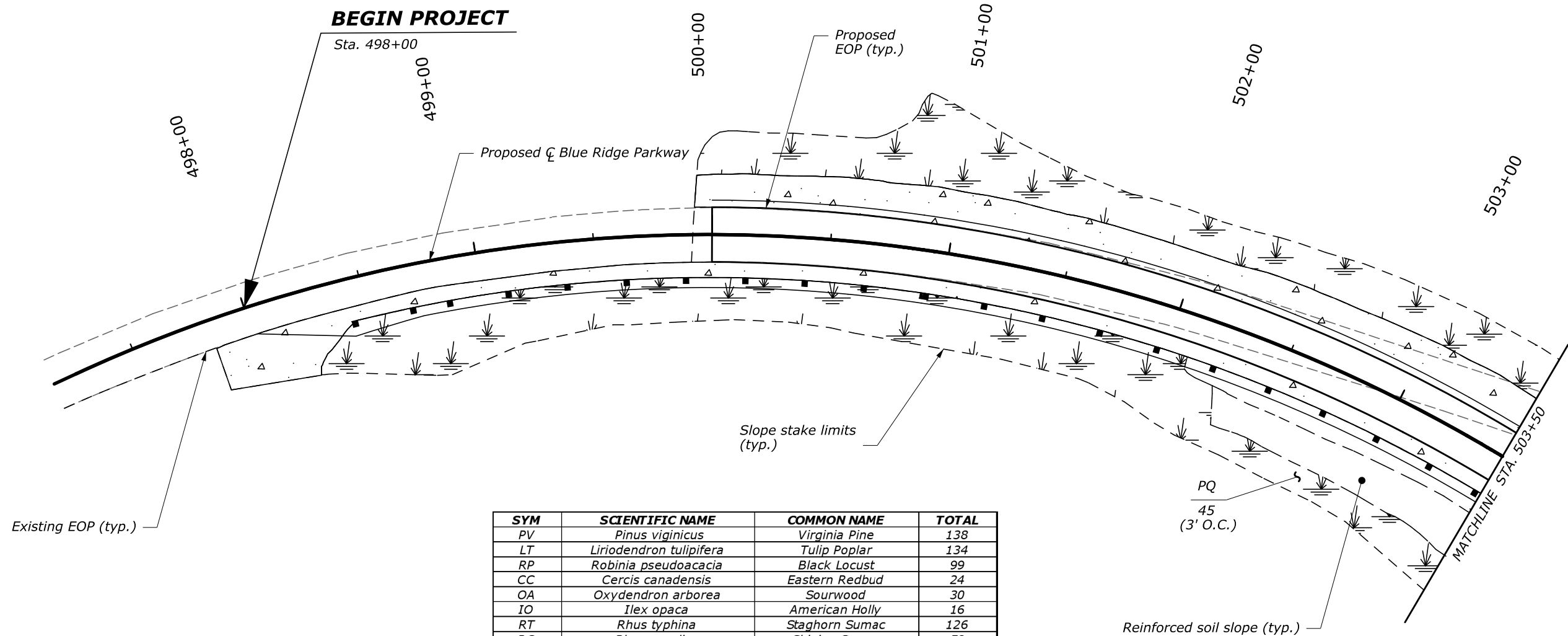
531+77 to 533+95

14-Mar-2019 10:38 AM \\NF1151\reserves\hnd\7\roadat\p\dot\PROJECTS\STATE_DOT\NC\NRI_126_nepa\Proj_Dev\CADD\From_Eng_Support\As_Requested_3_2017\M2\M21\BLRI_I_26_NEPA_plandgn



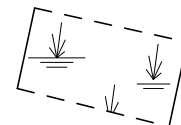
GENERAL NOTES:

1. See D sheet for construction work
2. Plant trees and shrubs according to planting methods shown on the plans
3. Any deviation from options given will require prior approval by Engineer.



SYM	SCIENTIFIC NAME	COMMON NAME	TOTAL
PV	<i>Pinus virginicus</i>	Virginia Pine	138
LT	<i>Liriodendron tulipifera</i>	Tulip Poplar	134
RP	<i>Robinia pseudoacacia</i>	Black Locust	99
CC	<i>Cercis canadensis</i>	Eastern Redbud	24
OA	<i>Oxydendron arborea</i>	Sourwood	30
IO	<i>Ilex opaca</i>	American Holly	16
RT	<i>Rhus typhina</i>	Staghorn Sumac	126
RC	<i>Rhus copalina</i>	Shining Sumac	58
RG	<i>Rhus glabra</i>	Smooth Sumac	124
RS	<i>Rubus species</i>	Rubus	135
RW	<i>Rhododendron catawbiense</i>	Catawba Rhododendron	22
LB	<i>Lindera benzoin</i>	Spicebush	32
VP	<i>Viburnum prunifolium</i>	Blackhaw Viburnum	58
HP	<i>Hypericum prolificum</i>	Shrubby St. John's Wort	50
PQ	<i>Parthenocissus quinquefolia</i>	Virginia Creeper	220
			1266

LEGEND



Permanent vegetation (Meadow seed mix)



Road shoulder turf establishment

PQ
45
(3' O.C.)

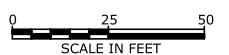
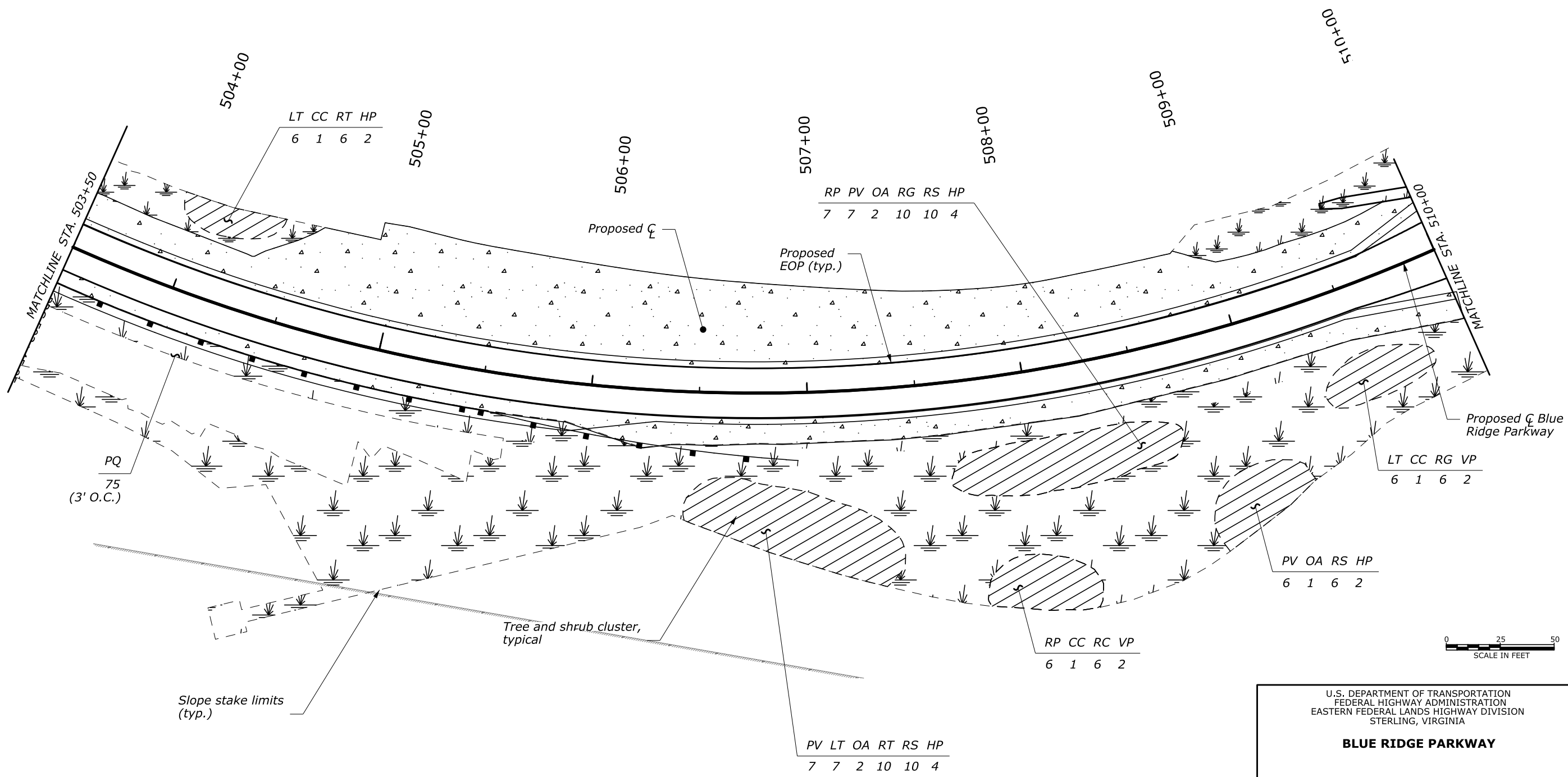
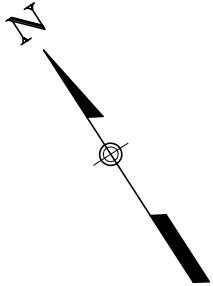
Reinforced soil slope (typ.)



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STERLING, VIRGINIA

**BLUE RIDGE PARKWAY
LANDSCAPING PLAN**

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M23



12-Dec-2018 11:23 AM M:\PROJECTS\STATE DOT\NC\Virr\126_nepa\Pro\Dev\CADD\From_Eng_Support\As_Requested_3_2017\M23-M27-BLRI\1_26_NEPA_plandgn

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 EASTERN FEDERAL LANDS HIGHWAY DIVISION
 STERLING, VIRGINIA

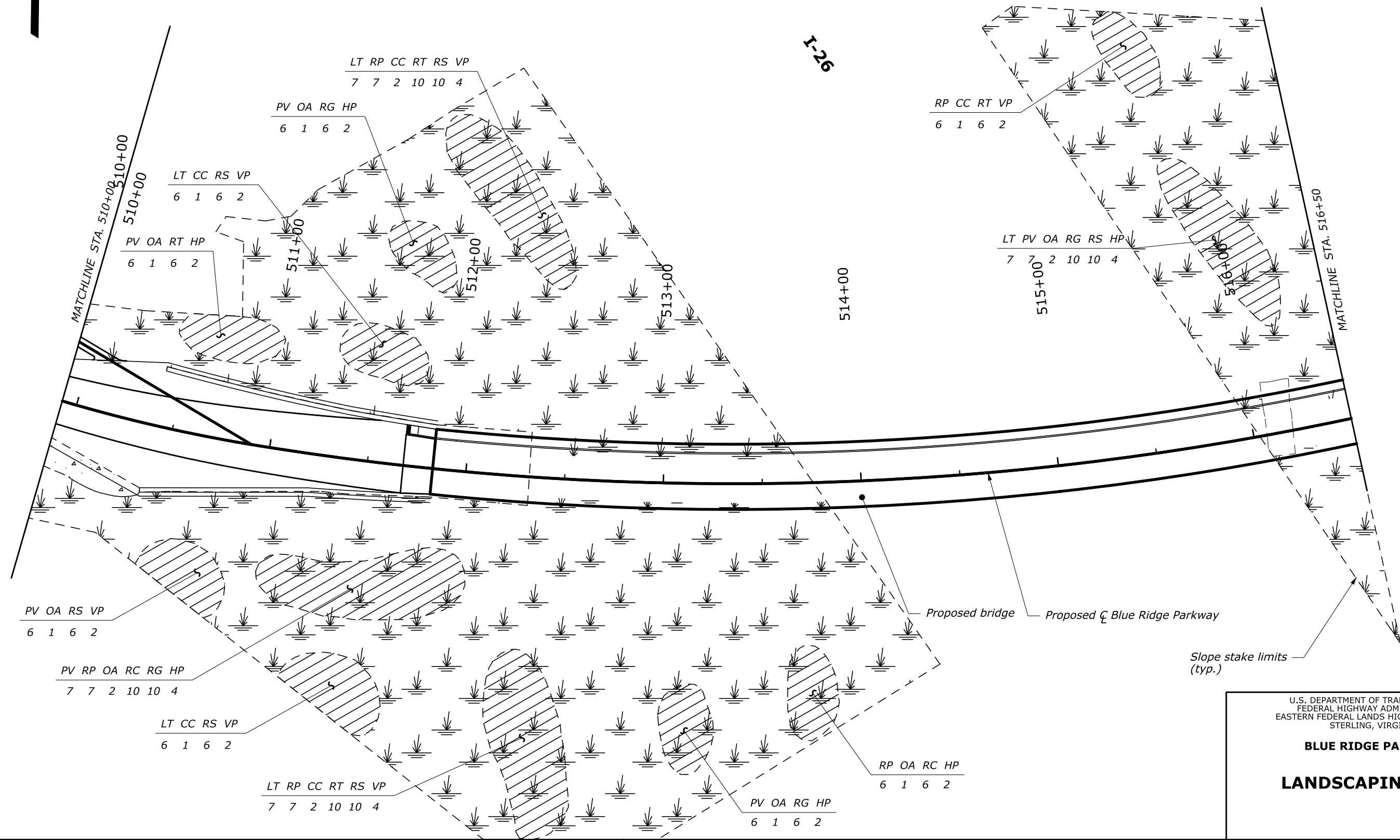
BLUE RIDGE PARKWAY

LANDSCAPING PLAN

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M24



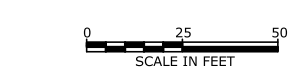
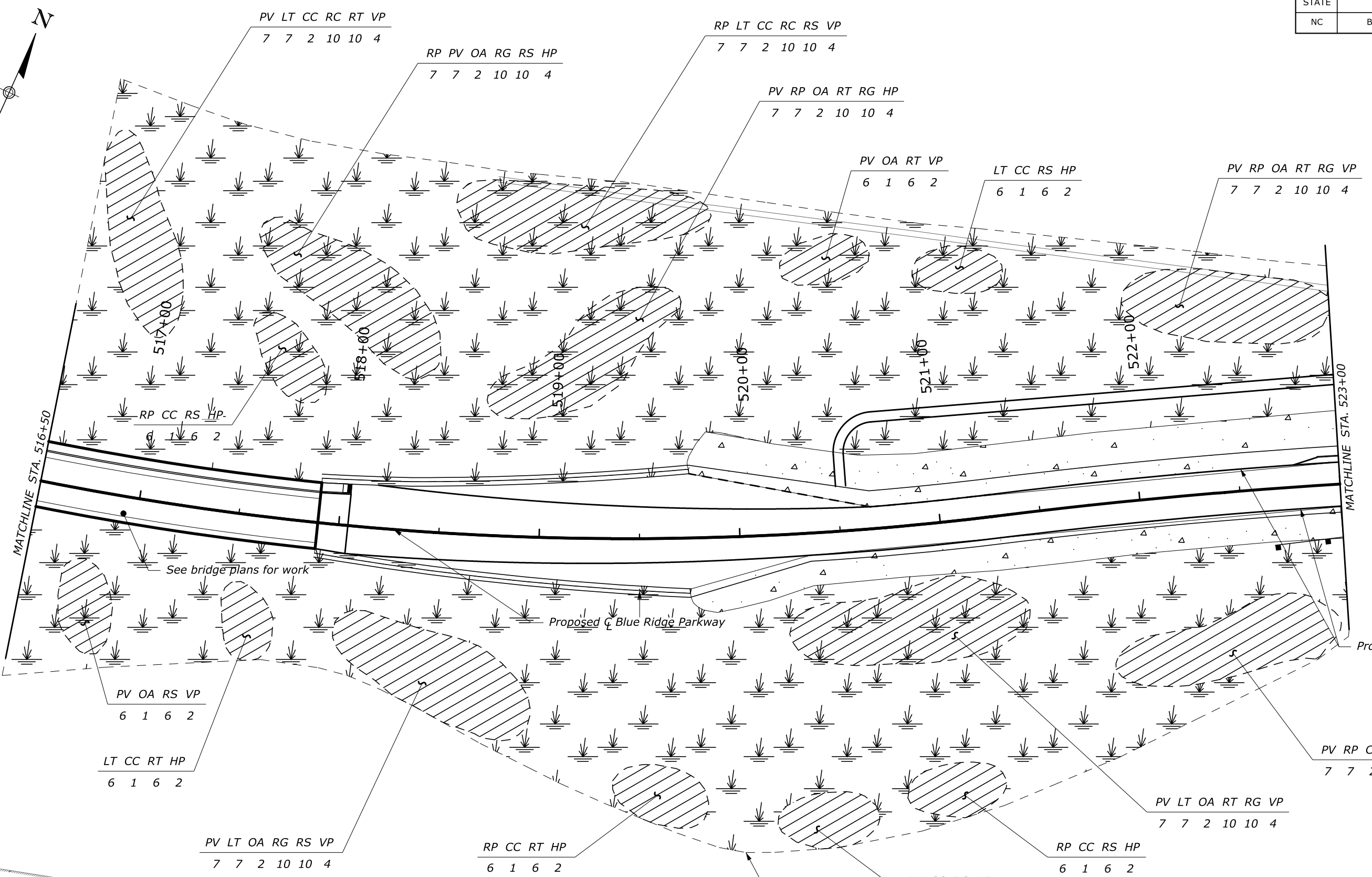
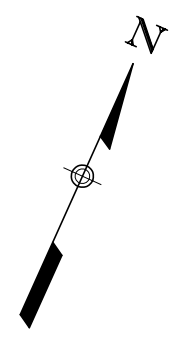
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 STERLING, VIRGINIA

**BLUE RIDGE PARKWAY
 LANDSCAPING PLAN**

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M25

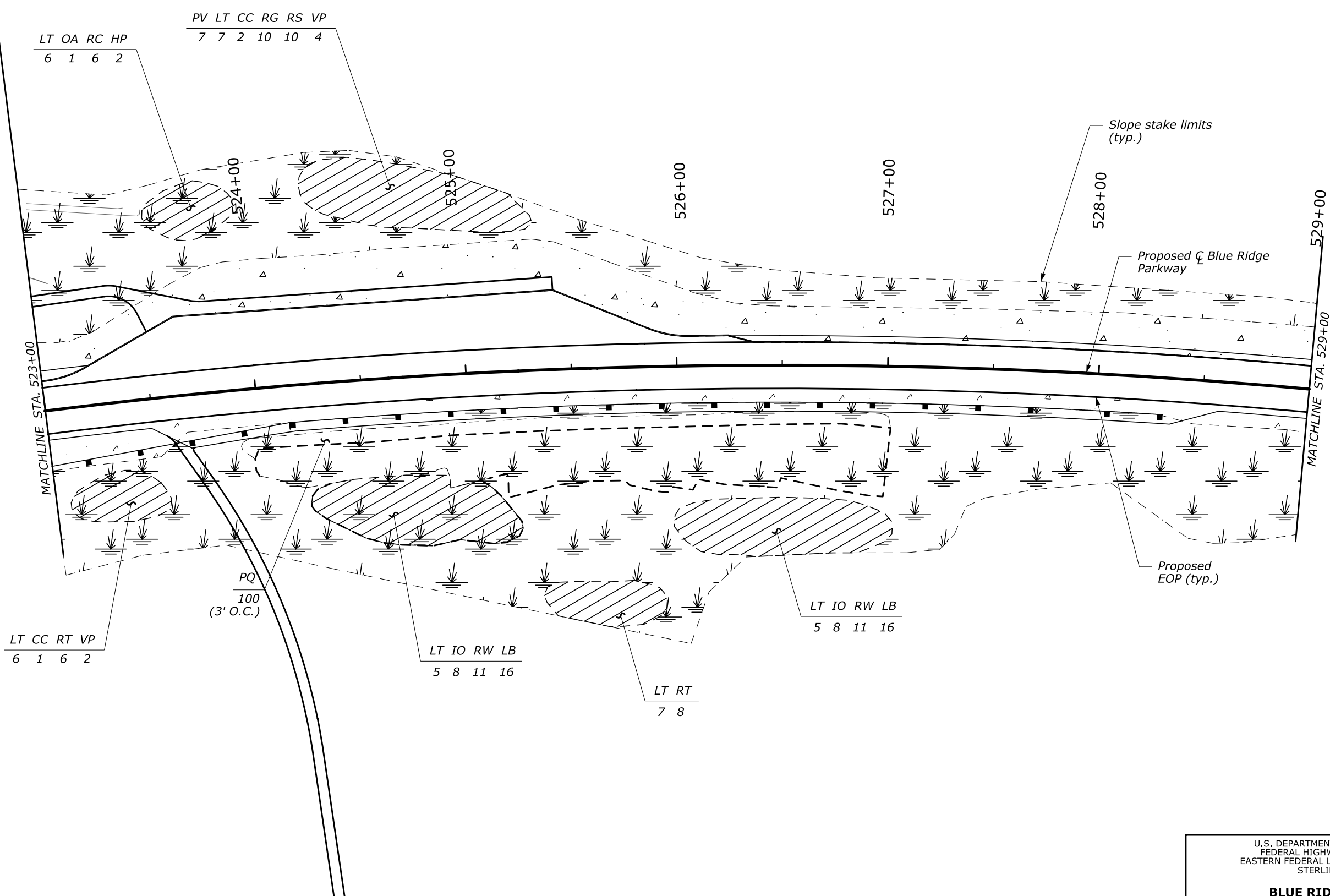
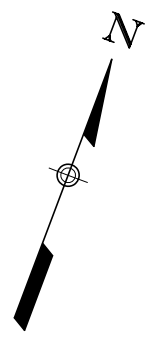


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 EASTERN FEDERAL LANDS HIGHWAY DIVISION
 STERLING, VIRGINIA

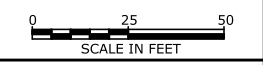
BLUE RIDGE PARKWAY
LANDSCAPING PLAN

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M26



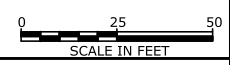
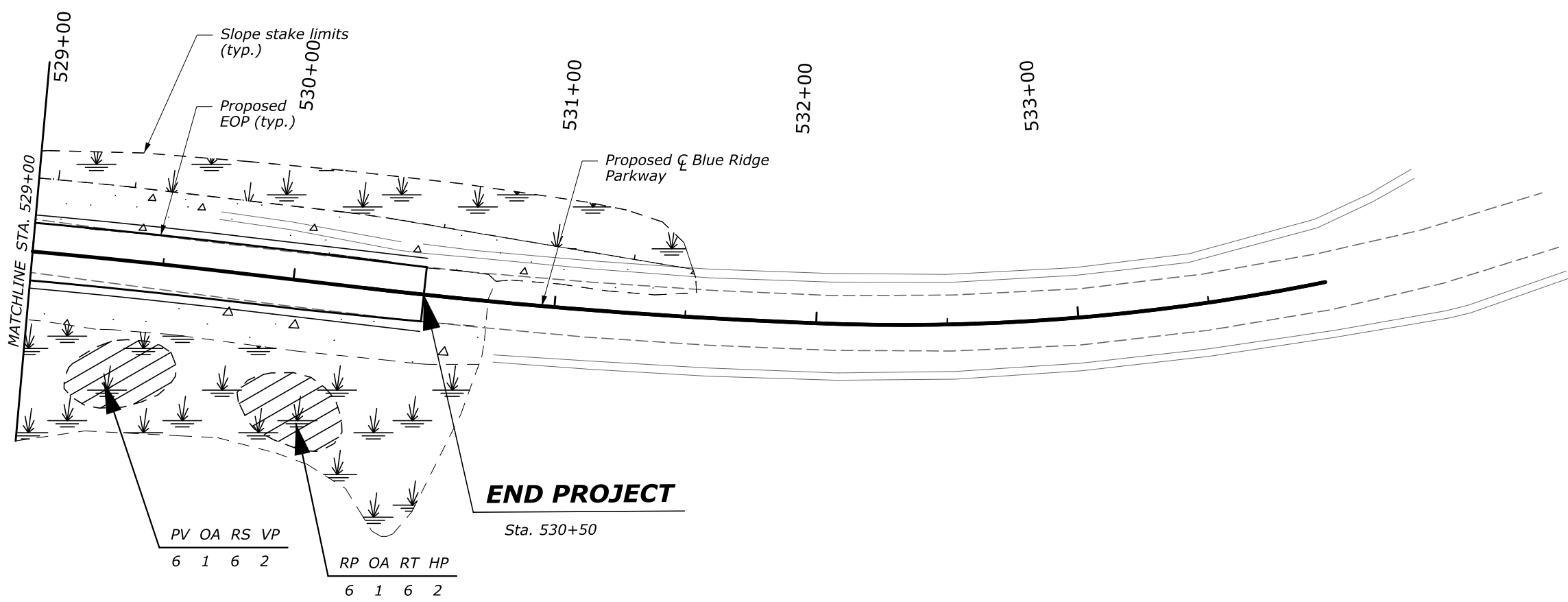
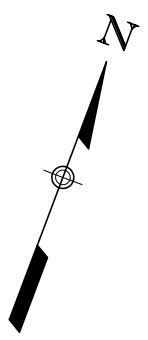
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY
LANDSCAPING PLAN

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M27



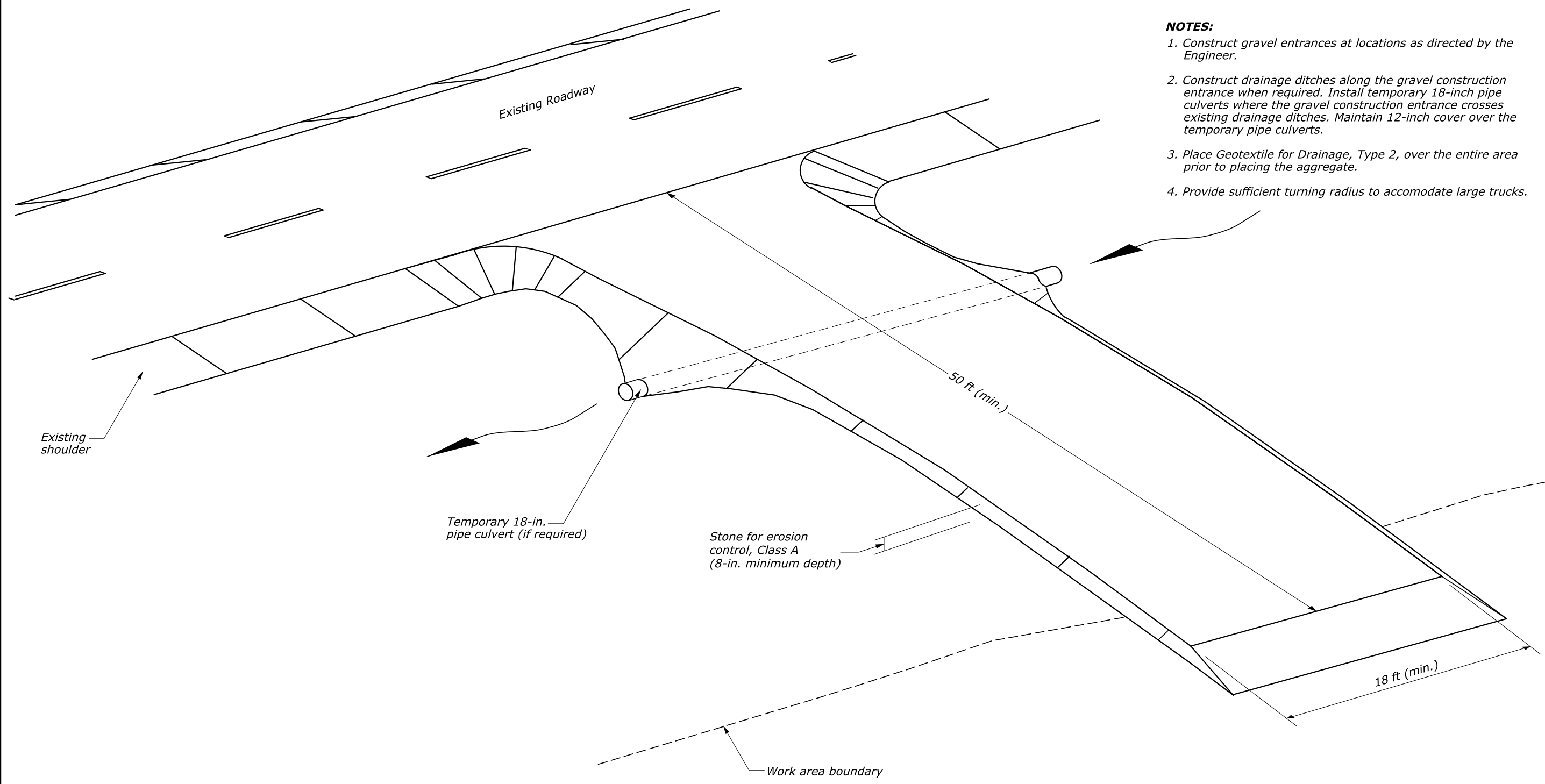
U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
EASTERN FEDERAL LANDS HIGHWAY DIVISION
STERLING, VIRGINIA

BLUE RIDGE PARKWAY

LANDSCAPING PLAN

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	M28



NOTES:

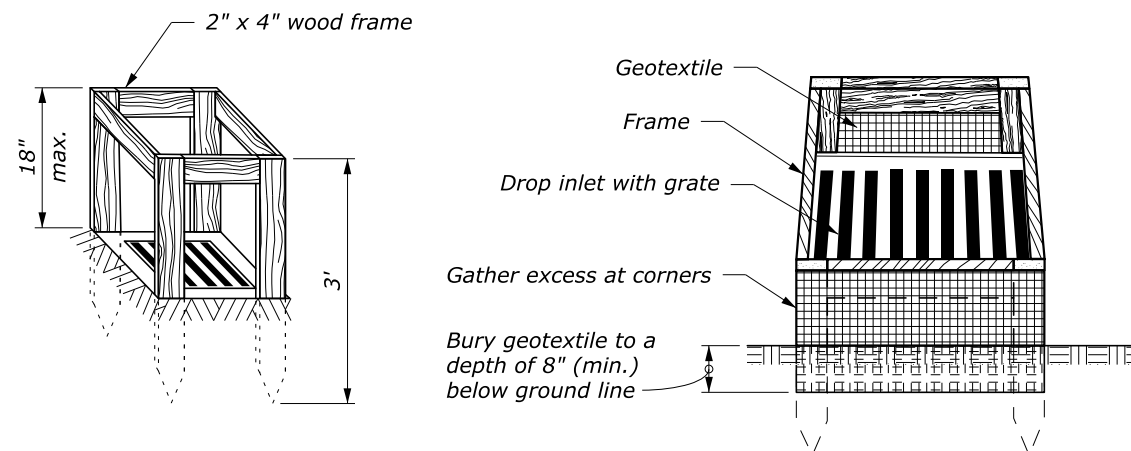
1. Construct gravel entrances at locations as directed by the Engineer.
2. Construct drainage ditches along the gravel construction entrance when required. Install temporary 18-inch pipe culverts where the gravel construction entrance crosses existing drainage ditches. Maintain 12-inch cover over the temporary pipe culverts.
3. Place Geotextile for Drainage, Type 2, over the entire area prior to placing the aggregate.
4. Provide sufficient turning radius to accommodate large trucks.

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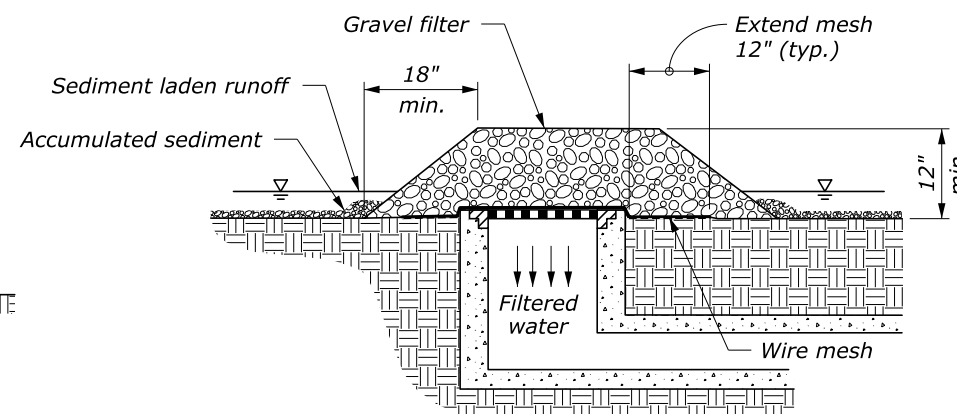
U.S. DEPARTMENT OF TRANSPORTATION
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY
GRAVEL CONSTRUCTION
ENTRANCE

NO SCALE



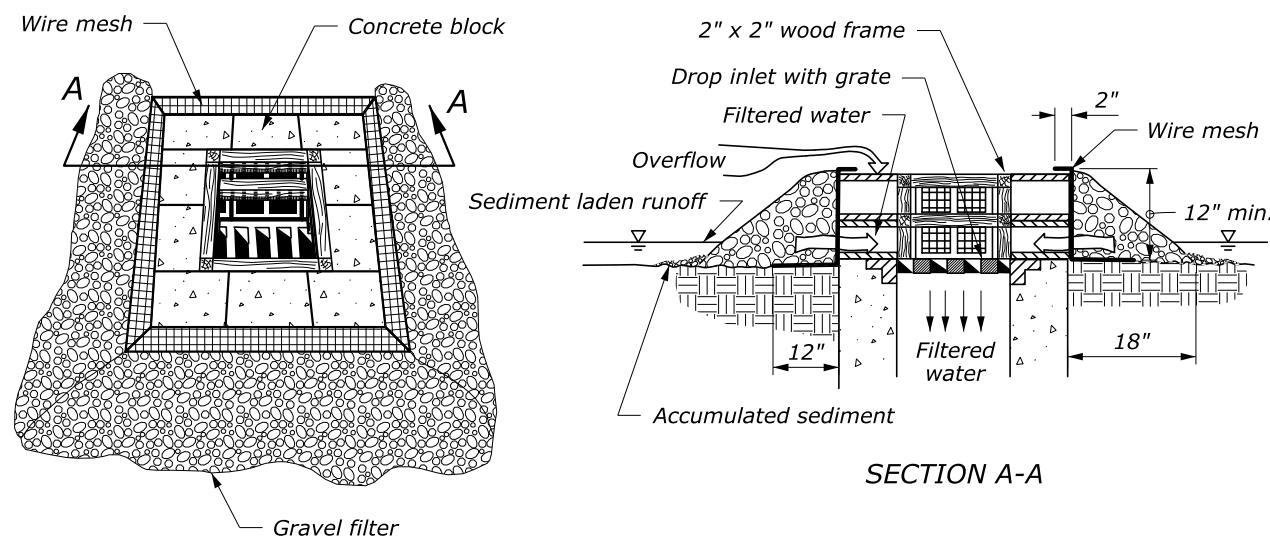
SILT FENCE DROP INLET PROTECTION (TYPE A)



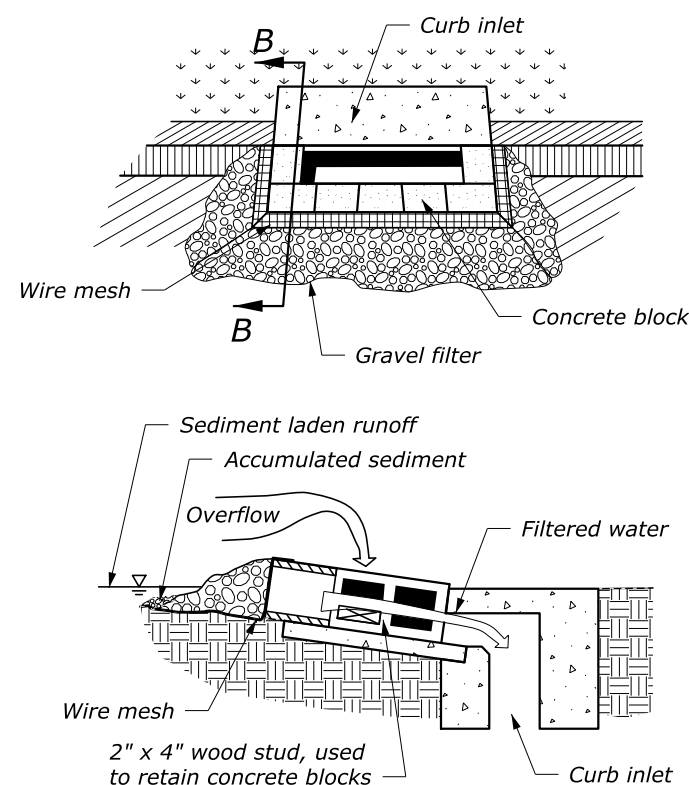
GRAVEL AND WIRE MESH DROP INLET PROTECTION (TYPE B)

NOTES:

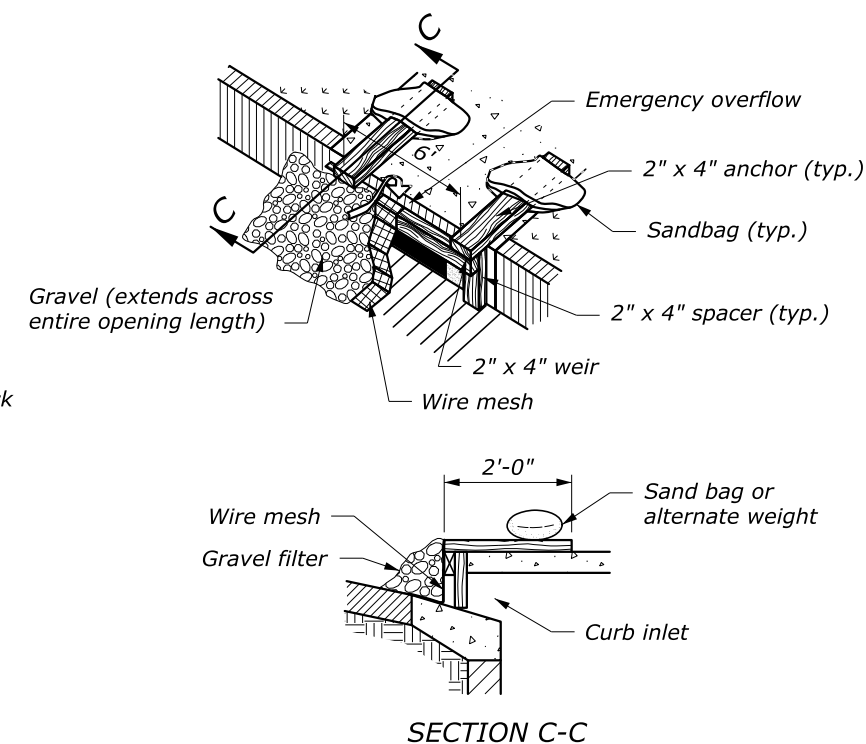
1. For gravel filters use 2"- 3" diameter coarse aggregate.
2. Use wire mesh with 1/2" x 1/2" openings.
3. Use type A inlet protection in sump locations only.
4. Use type B inlet protection only in sump locations where heavy concentrated flows are not expected. Do not use where ponding around the structure might cause inconvenience or damage.



BLOCK AND GRAVEL DROP INLET PROTECTION (TYPE C)



CURB INLET PROTECTION, BLOCK AND GRAVEL (TYPE D)



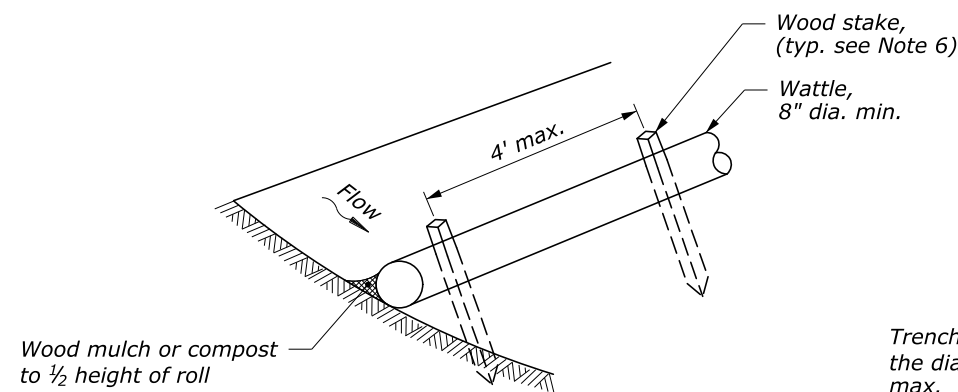
CURB INLET PROTECTION, WOODEN WEIR (TYPE E)

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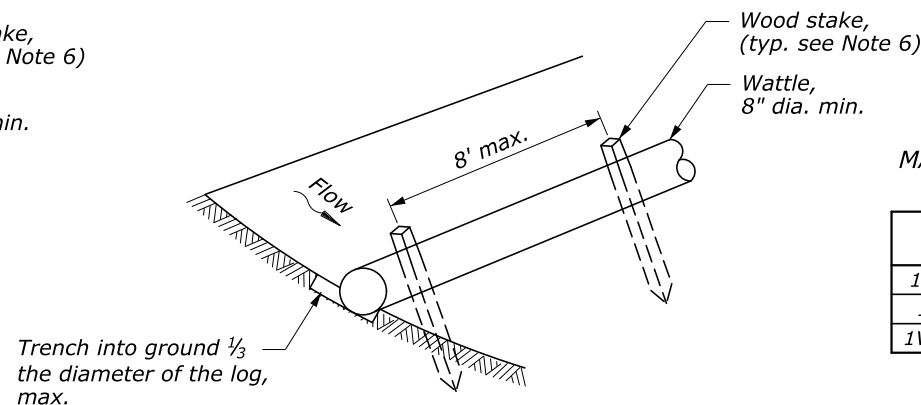
U.S. DEPARTMENT OF TRANSPORTATION
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 EASTERN FEDERAL LANDS HIGHWAY DIVISION
 STERLING, VIRGINIA

**BLUE RIDGE PARKWAY
 TEMPORARY
 INLET PROTECTION
 SUPPLEMENTAL TITLE**

NO SCALE



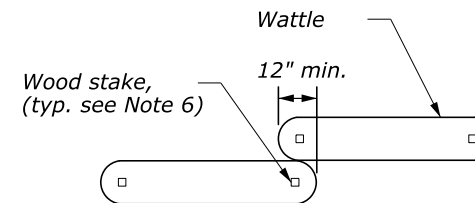
UNTRENCHED INSTALLATION ISOMETRIC VIEW



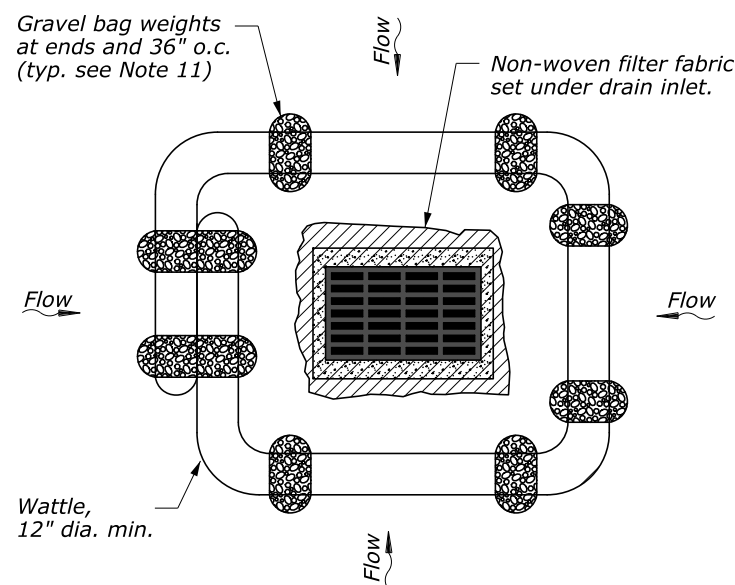
ENTRENCHED INSTALLATION ISOMETRIC VIEW

MAXIMUM ALLOWABLE SLOPE LENGTH ABOVE WATTLES

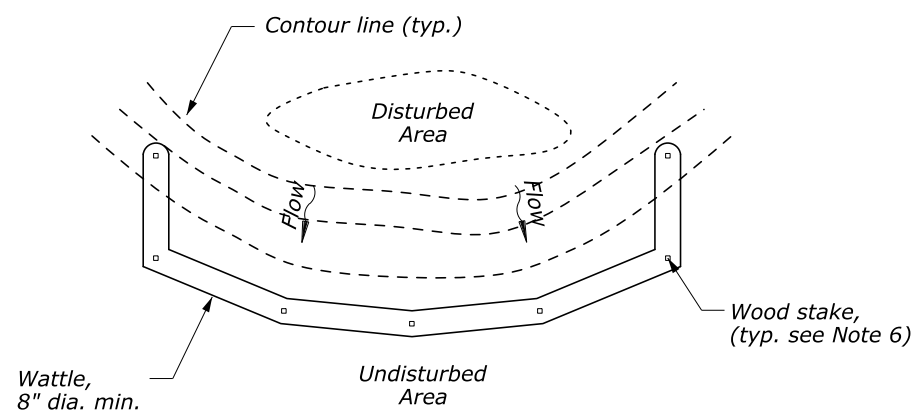
SLOPE	MAX INTERVAL
1V:4H or Flatter	20 ft
1V:4H - 1V:2H	15 ft
1V:2H or Steeper	10 ft



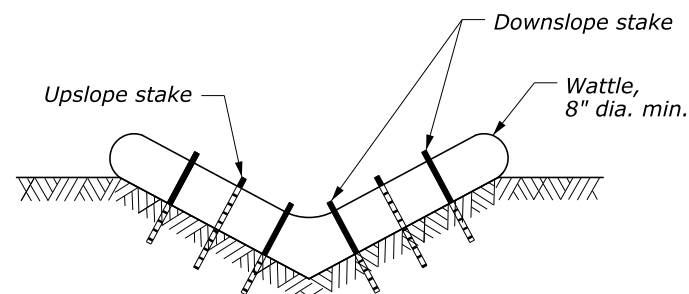
ROLL OVERLAP



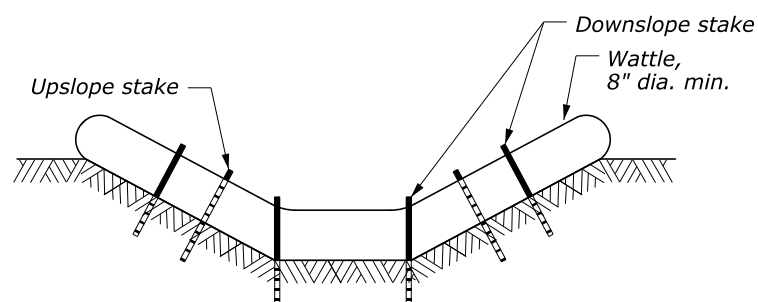
INLET PROTECTION



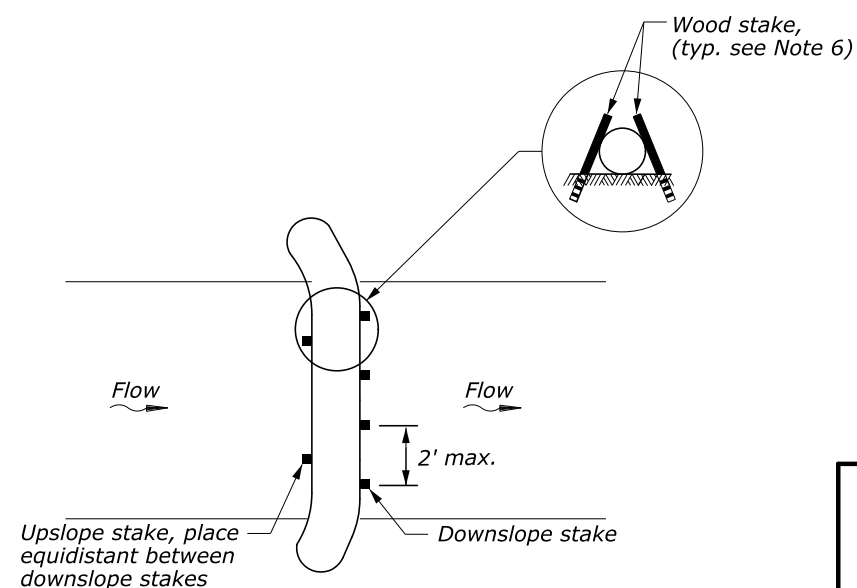
PLAN VIEW



V-DITCH WATTLE CHECK DAM CROSS SECTION



TRAPEZOIDAL DITCH WATTLE CHECK DAM CROSS SECTION



WATTLE CHECK DAM PLAN VIEW

NO SCALE

NOTES:

1. Provide wattles meeting the requirements of Special Provisions SP08-R1671, Wattles.
2. Use wattles with a minimum 8-inch diameter. For drain inlet protection, use wattles with a minimum 12-inch diameter.
3. Prior to installation, clear all obstructions including rocks, clods, and debris greater than 1-inch that may interfere with proper function of the wattle.
4. For untrenched installation, blow or hand place mulch or compost on uphill side of the slope along the wattle.
5. Place wattles on level grade and parallel to contours. Extend both ends of the wattle at least 8 feet upslope at 45 degrees to the main alignment.
6. Use wood stakes with a minimum nominal cross section of 2-inch x 2-inch and of sufficient length to attain a minimum of 12 inches into the ground and 3 inches protruding above the roll. Furnish wood stakes meeting the requirements of NCDOT 2018 Standard Specifications 1060-12.
7. When more than one wattle is needed, overlap ends 12 inches minimum and stake.
8. Remove sediment deposits when accumulation is one-half the height of the exposed wattle.
9. Replace biodegradable wattles 6 months after installation and photodegradable wattles after 12 months after installation.
10. When wattles are required on paved surfaces, use gravel bags to support them as shown on the inlet protection detail.
11. Provide gravel bag weights meeting the requirements of Special Provisions SP08-R1671, Wattles.

WATTLE CHECK DAM SPACING TABLE

DITCH GRADE *	CHECK DAM SPACING(S) **	
	12" HIGH	18" HIGH
2%	50 ft	75 ft
3%	33 ft	50 ft
4%	25 ft	40 ft
5%	20 ft	30 ft
6%	16 ft	25 ft
7%	14 ft	21 ft
8%	12 ft	18 ft
9%	11 ft	16 ft
10%	10 ft	15 ft

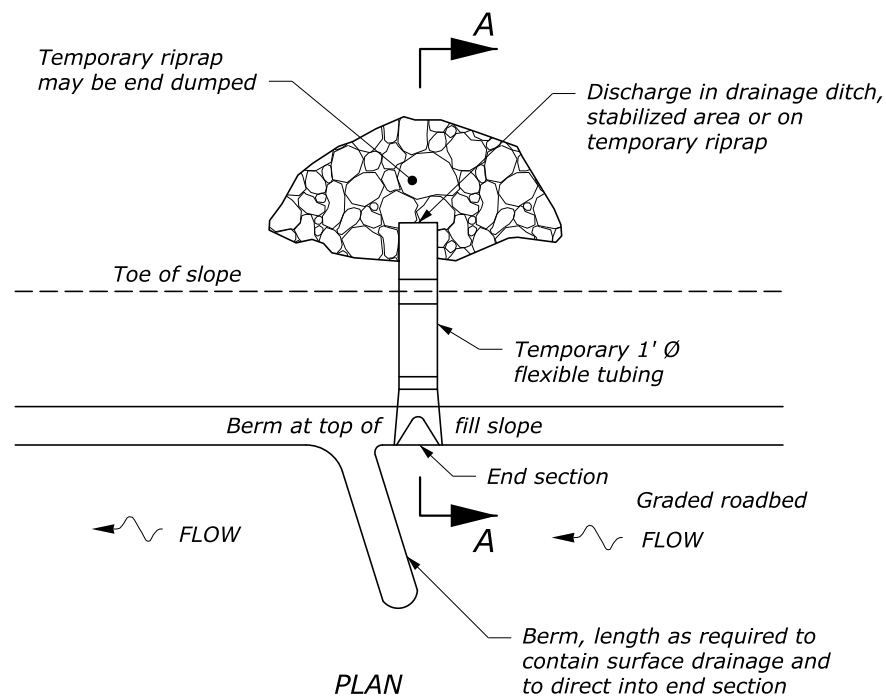
* Do not install check dams on grades below 2%
 ** Adjust spacing as approved based on site conditions

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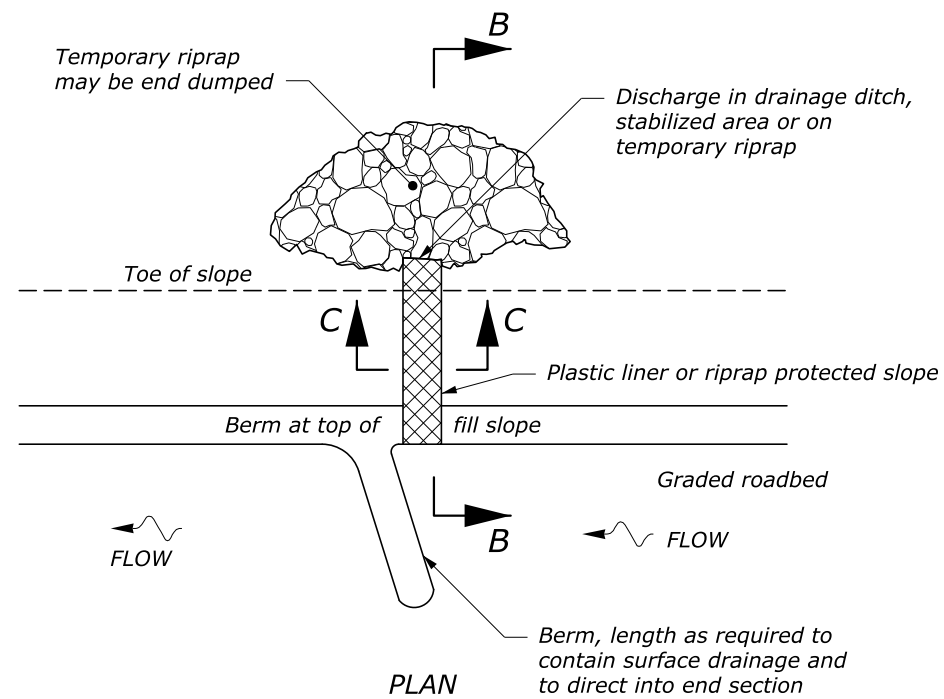
BLUE RIDGE PARKWAY

WATTLES

SUPPLEMENTAL TITLE



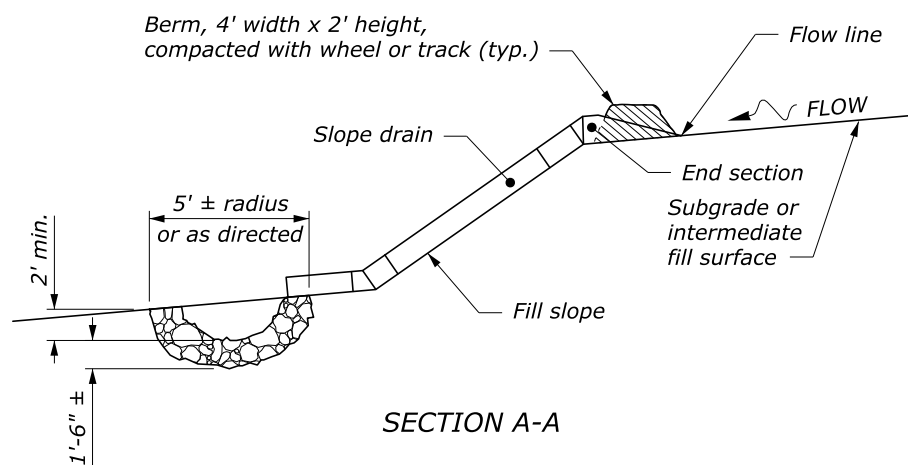
SLOPE DRAINS



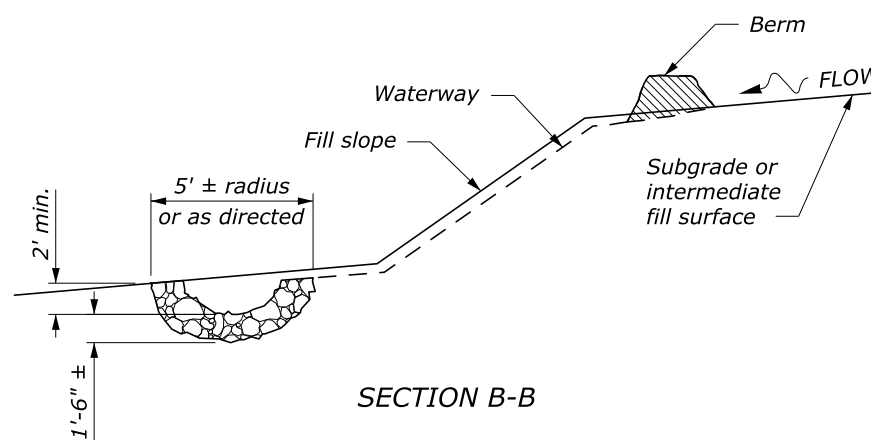
PLASTIC LINED WATERWAY

NOTES:

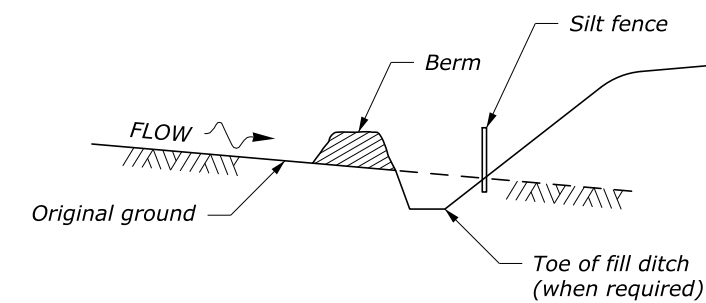
1. Use temporary slope drains (berms, drains, and riprap) as the embankment is constructed. Use spacings as shown on the Erosion and Sediment Control Plans or as designated by the Engineer. Place all slope drains at the end of each work shift. Use slope drains until the slopes are permanently stabilized.
2. Construct temporary berms at the top of all erodible cut slopes as shown on the Erosion and Sediment Control Plans or as designated by the Engineer. Use check dams to reduce the runoff velocity when existing grades are steep.
3. Do not use transverse or longitudinal joints in plastic liner. Plastic liner is not required for rock embankments.
4. Use toe-of-fill slope berms to divert offsite runoff away from disturbed areas.
5. Seed and mulch all cut slope berms and toe-of-fill berms immediately after berm construction.
6. Place rip rap as shown on NCDOT Standard Drawing 876.02.



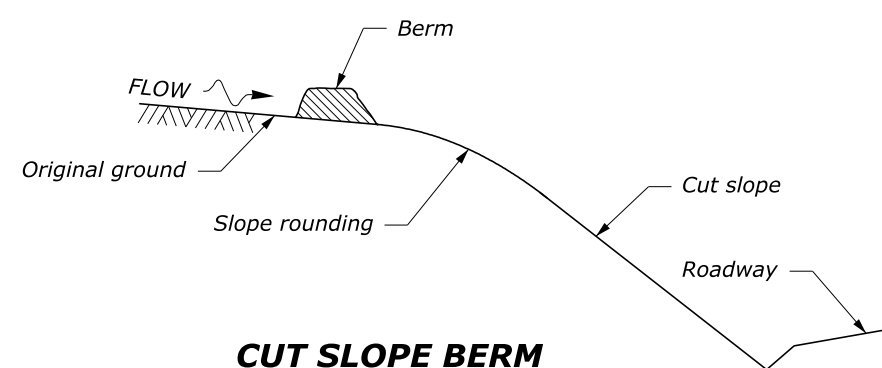
SECTION A-A



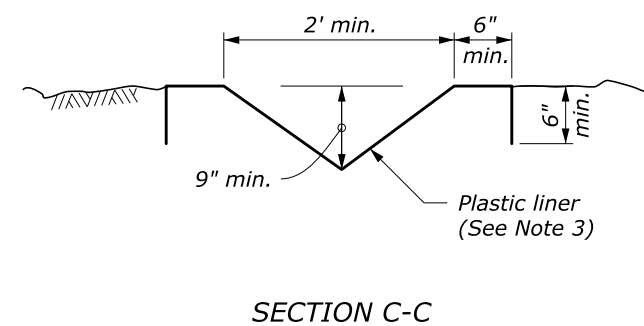
SECTION B-B



TOE-OF-FILL SLOPE BERM



CUT SLOPE BERM



SECTION C-C

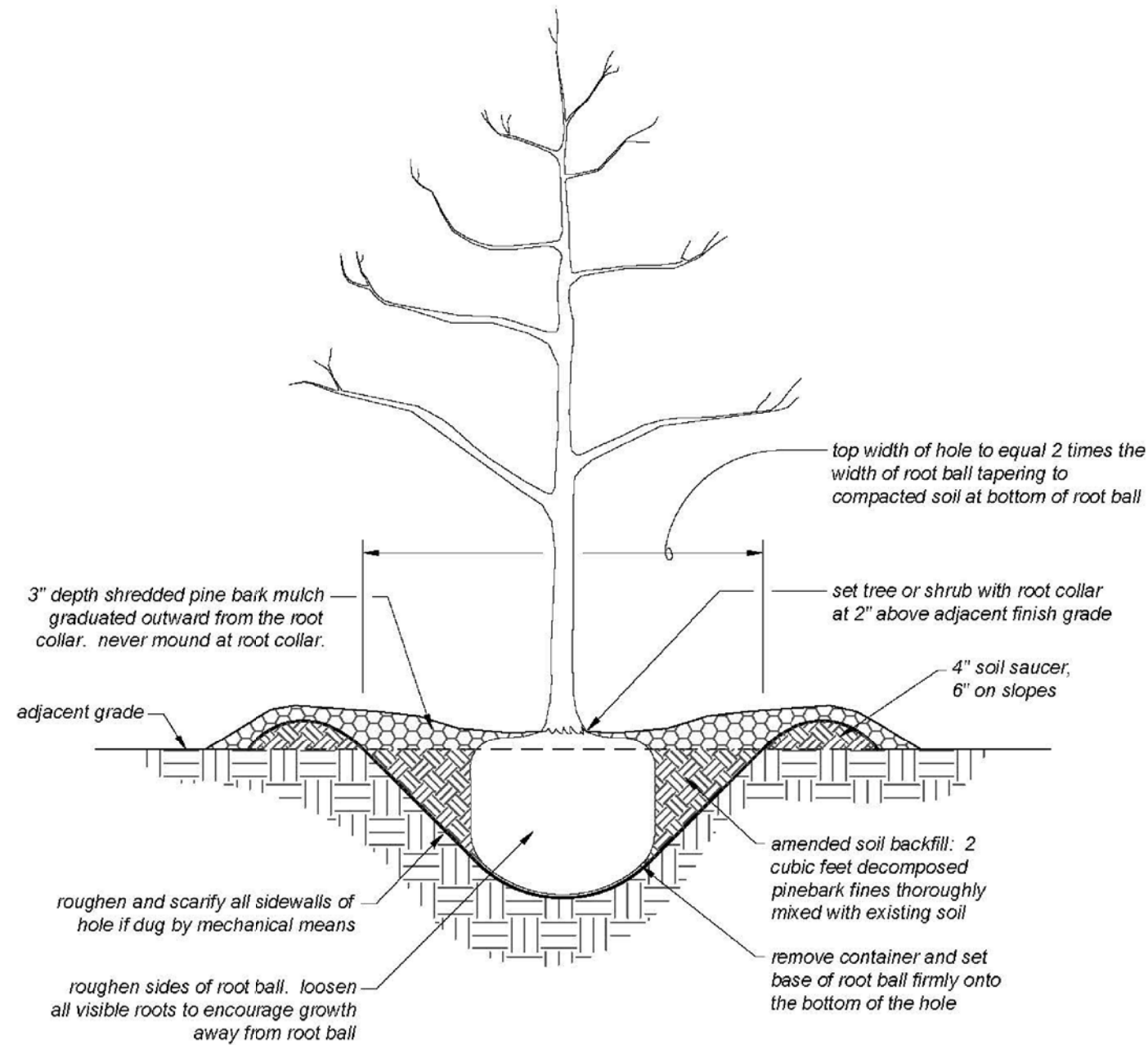
U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION
 EASTERN FEDERAL LANDS HIGHWAY DIVISION
 STERLING, VIRGINIA

**BLUE RIDGE PARKWAY
 TEMPORARY EROSION CONTROL
 BERMS, SLOPE DRAINS,
 AND LINED WATERWAYS**

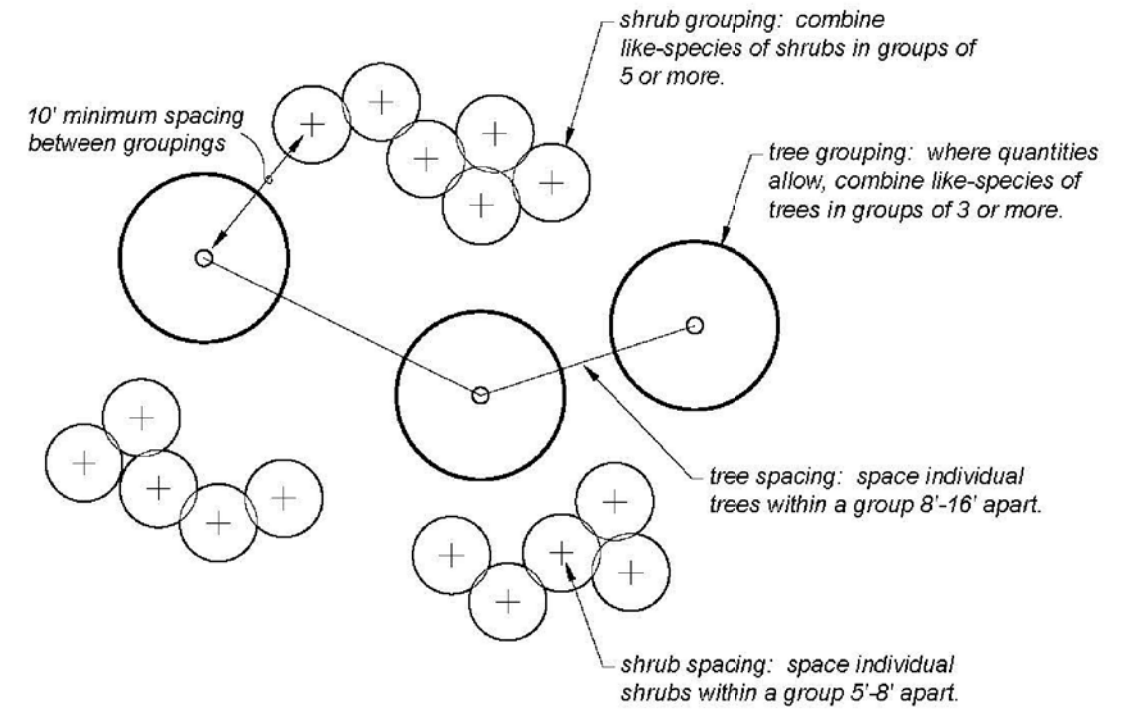
NO SCALE

NOTES:

1. Furnish pine bark mulch in accordance to Special Provisions
2. Furnish trees & shrubs as specified in plans & in accordance to Special Provisions



TYPICAL TREE AND SHRUB PLANTING



TYPICAL TREE AND SHRUB CLUSTER LAYOUT

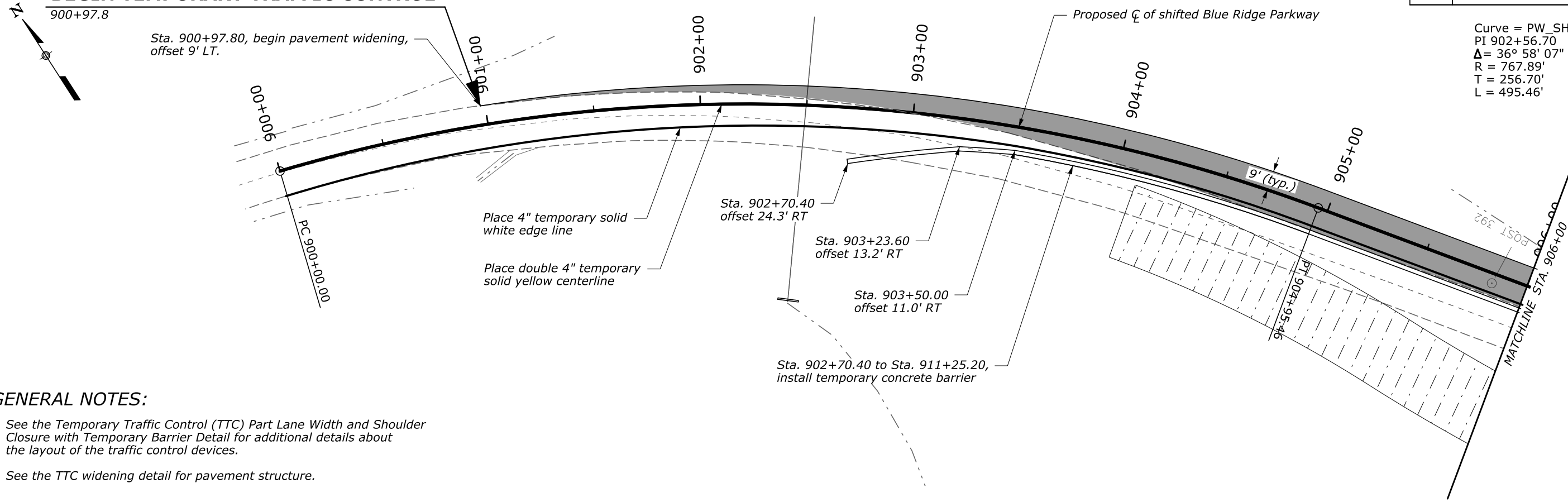
U.S. DEPARTMENT OF TRANSPORTATION
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 STERLING, VIRGINIA

**BLUE RIDGE PARKWAY
 TREES AND SHRUBS
 PLANTING METHODS**

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	N01

BEGIN TEMPORARY TRAFFIC CONTROL

Curve = PW_SHIFT-1
 PI 902+56.70
 $\Delta = 36^\circ 58' 07''$ (RT)
 R = 767.89'
 T = 256.70'
 L = 495.46'



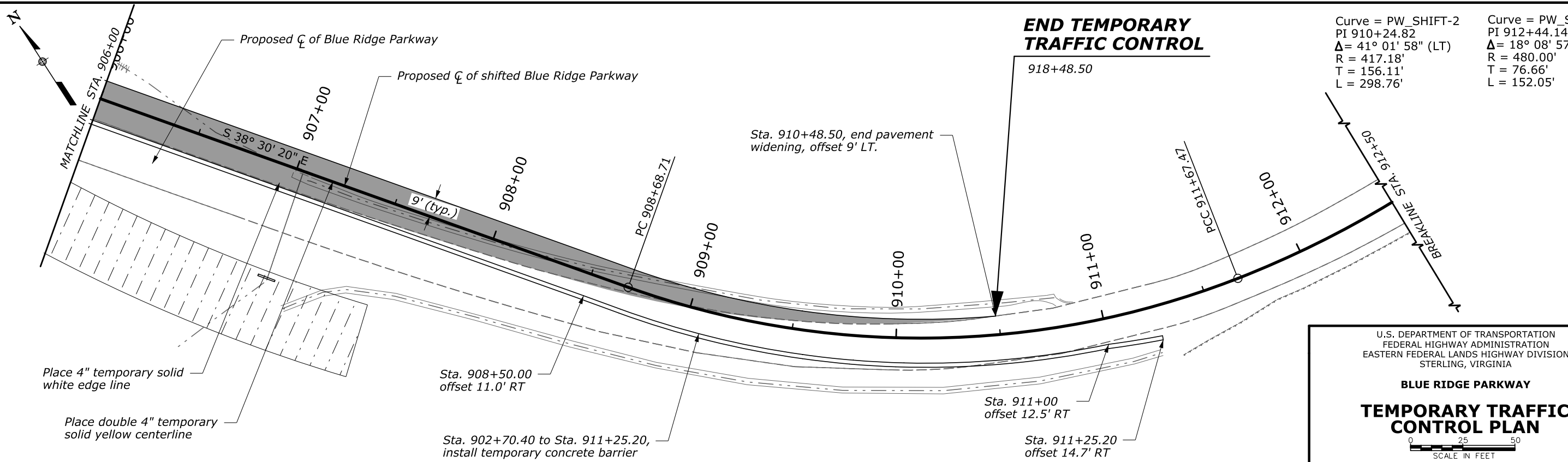
GENERAL NOTES:

1. See the Temporary Traffic Control (TTC) Part Lane Width and Shoulder Closure with Temporary Barrier Detail for additional details about the layout of the traffic control devices.
2. See the TTC widening detail for pavement structure.

END TEMPORARY TRAFFIC CONTROL

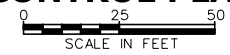
Curve = PW_SHIFT-2
 PI 910+24.82
 $\Delta = 41^\circ 01' 58''$ (LT)
 R = 417.18'
 T = 156.11'
 L = 298.76'

Curve = PW_SHIFT-3
 PI 912+44.14
 $\Delta = 18^\circ 08' 57''$ (LT)
 R = 480.00'
 T = 76.66'
 L = 152.05'



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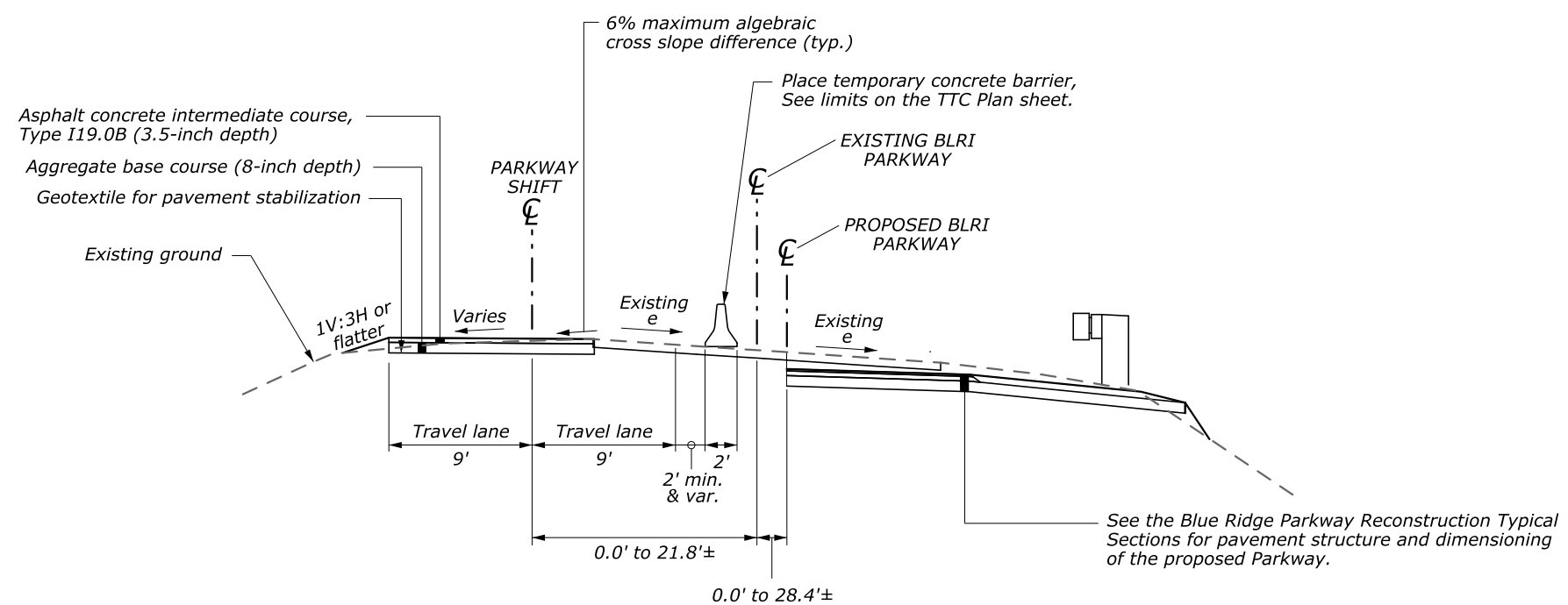
**BLUE RIDGE PARKWAY
 TEMPORARY TRAFFIC CONTROL PLAN**



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NOTES:

1. Prepare the surface on which the aggregate course is placed according to Articles 225 and 500 of the NCDOT 2018 Standard Specifications.
2. After traffic is shifted to the reconstructed Parkway, remove temporary pavement and base course. Restore surface to the existing contours prior to constructing the temporary widening and establish turf on all disturbed areas in accordance with Articles 1060 and 1660 of the NCDOT 2018 Standard Specifications.



PARKWAY SHIFT WIDENING

Sta. 901+00.00 to Sta. 904+09.50
 Sta. 908+64.30 to Sta. 910+76.70

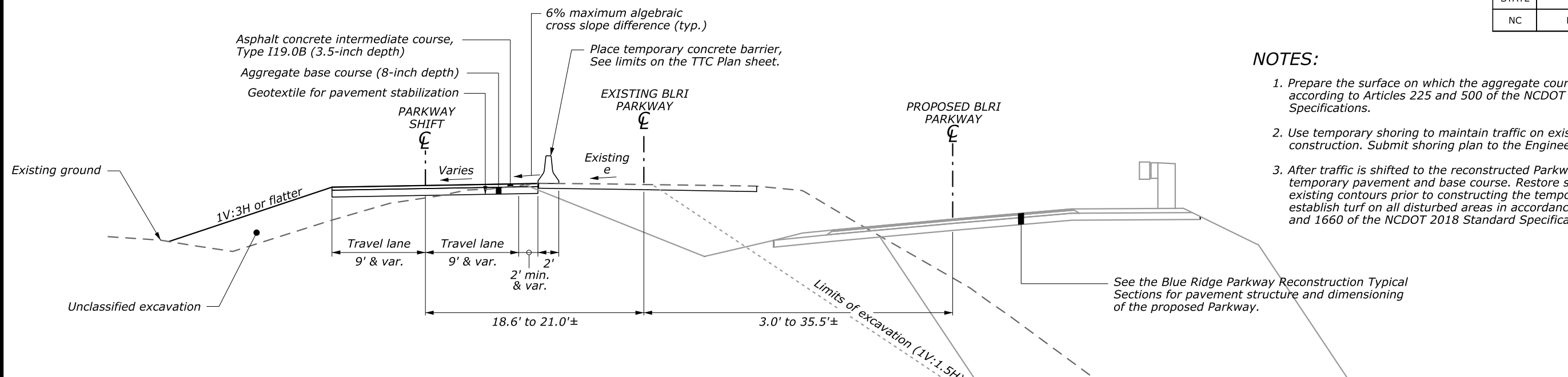
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY

**TEMPORARY TRAFFIC CONTROL
 WIDENING DETAIL**

NOT TO SCALE

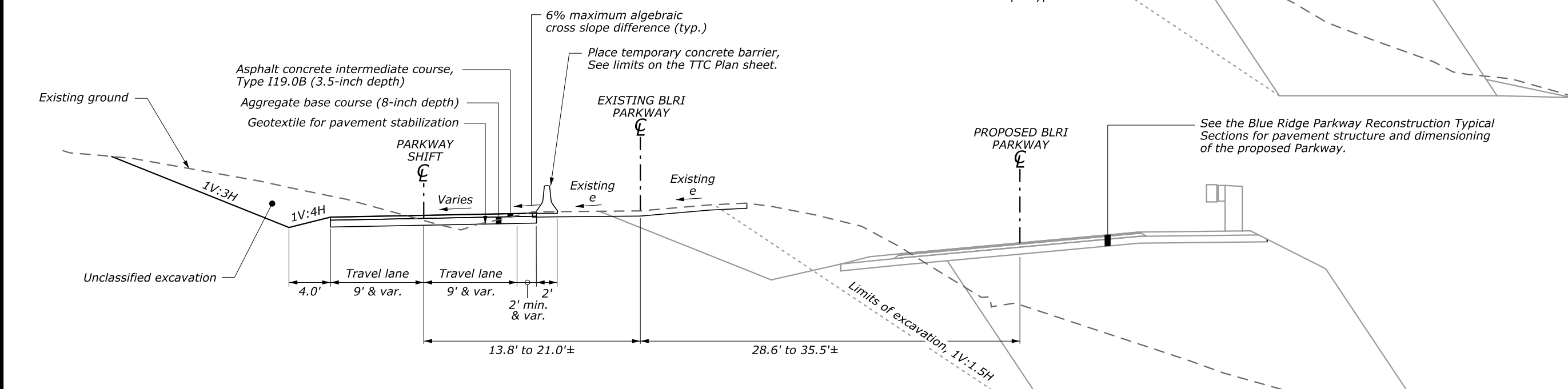


PARKWAY SHIFT WIDENING

Sta. 904+09.50 to Sta. 906+77.80

NOTES:

1. Prepare the surface on which the aggregate course is placed according to Articles 225 and 500 of the NCDOT 2018 Standard Specifications.
2. Use temporary shoring to maintain traffic on existing Parkway during construction. Submit shoring plan to the Engineer for approval.
3. After traffic is shifted to the reconstructed Parkway, remove temporary pavement and base course. Restore surface to the existing contours prior to constructing the temporary widening and establish turf on all disturbed areas in accordance with Articles 1060 and 1660 of the NCDOT 2018 Standard Specifications.



PARKWAY SHIFT WIDENING

Sta. 906+77.80 to Sta. 908+64.30

Construct Reinforced Soil Slopes. See Reinforced Soil Slope Typical Section.

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BLUE RIDGE PARKWAY

**TEMPORARY TRAFFIC CONTROL
WIDENING DETAIL**

NOT TO SCALE

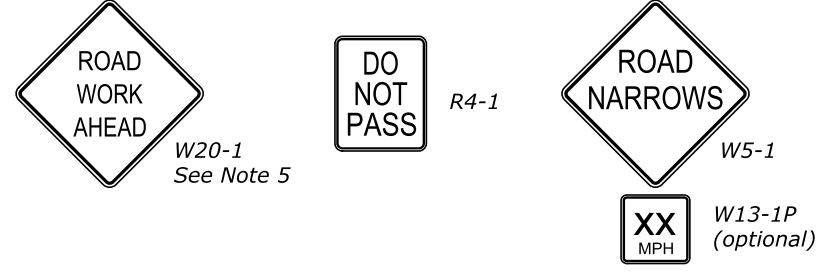
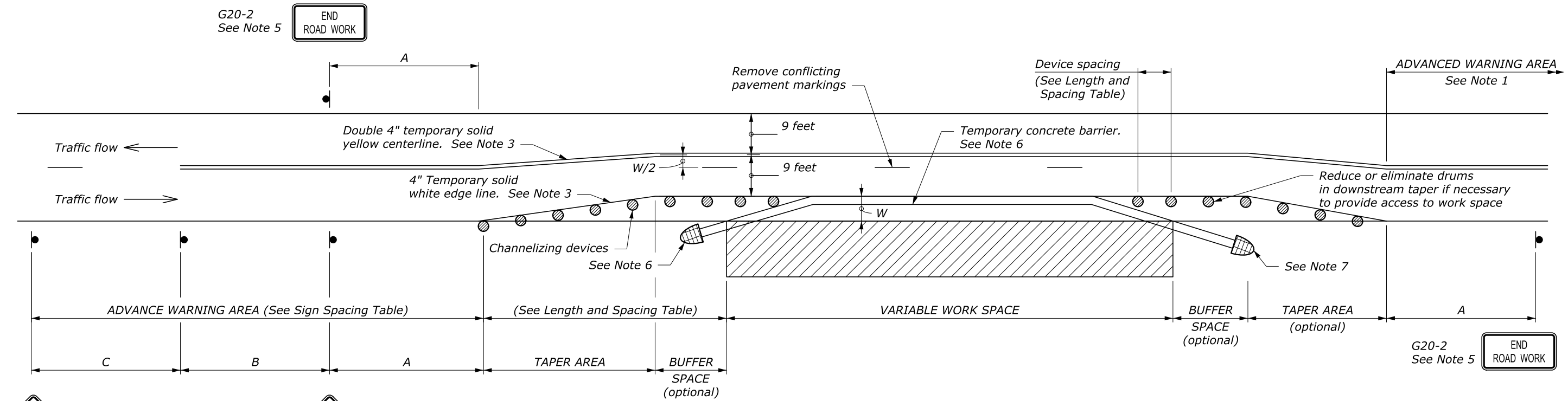
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APPROACH SPEED* MPH	MINIMUM TAPER LENGTH FEET	BUFFER SPACE LENGTH FEET	CHANNELIZING DEVICE			WORK ZONE CLEAR ZONE WIDTH FEET
			TAPER AREA	BUFFER SPACE	WORK SPACE	
20	Shifting taper formula: $L = \frac{WS^2}{120}$ for $S \leq 40$ MPH	115	20	40	40	10
25		155	25	50	50	10
30	$L = \frac{WS}{2}$ for $S \geq 45$ MPH	200	30	60	60	10
35		250	35	70	70	10
40	Where: L = Minimum length of taper W = Width of offset in feet S = Numerical value of posted speed limit or 85 percentile speed prior to work in miles per hour	305	40	80	80	15
45		360	45	90	90	20
50	425	50	100	100	20	
55	495	55	110	110	20	
60	570	60	120	120	30	
65	645	65	130	130	30	
70	730	70	140	140	30	

ROAD TYPE	DISTANCE BETWEEN SIGNS IN FEET		
	A	B	C
Urban and Rural 30 MPH and less	100	100	100
Urban and Rural 35 MPH to 50 MPH	350	350	350
Rural greater than 50 MPH	500	500	500
Expressway / Freeway	1000	1500	2640

- NOTES:**
- Signs are shown for one direction of travel only. Place devices similar to those depicted for the opposite direction of travel.
 - Final location and spacing of signs and devices may be changed to fit field conditions as approved by the Engineer.
 - If the roadway surface is paved, install temporary pavement extend markings to connect zones.
 - If closure is completely within the project limits, eliminate the "ROAD WORK AHEAD" (W20-1) and "END ROAD WORK" (G20-2) signs.
 - Install "PASS WITH CARE" sign (R4-2) at ends of no-passing zone if directed by the Engineer.
 - Place the barrier according to the AASHTO Roadside Design Guide. Terminate barrier ends outside the work zone clear zone or protect the barrier ends with a crash cushion.
 - Do not allow equipment, materials, or vehicles to be parked or stored in the buffer space.

* Approach speed based on the regulatory posted speed, not the advisory speed.



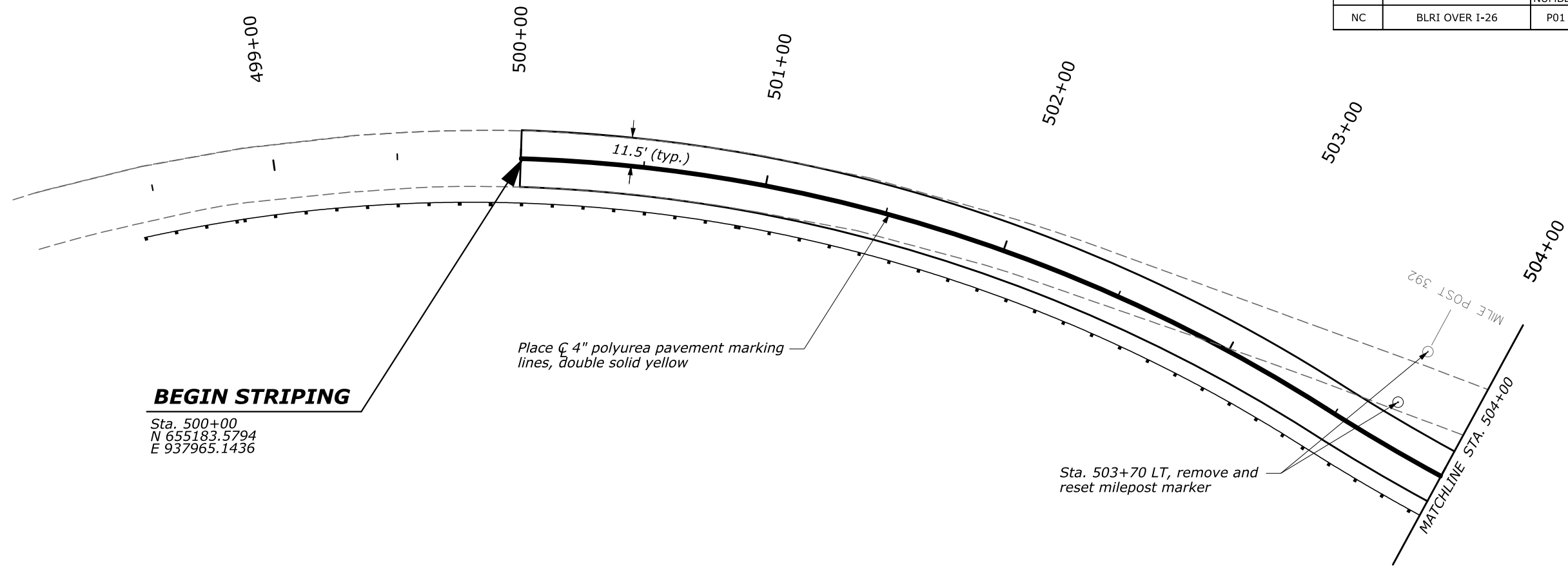
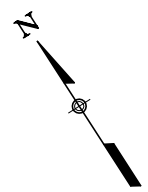
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BLUE RIDGE PARKWAY
TEMPORARY TRAFFIC CONTROL
PART LANE WIDTH AND
SHOULDER CLOSURE LAYOUT
WITH TEMPORARY BARRIER

NO SCALE

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	P01



BEGIN STRIPING

Sta. 500+00
 N 655183.5794
 E 937965.1436

Sta. 503+70 LT, remove and reset milepost marker

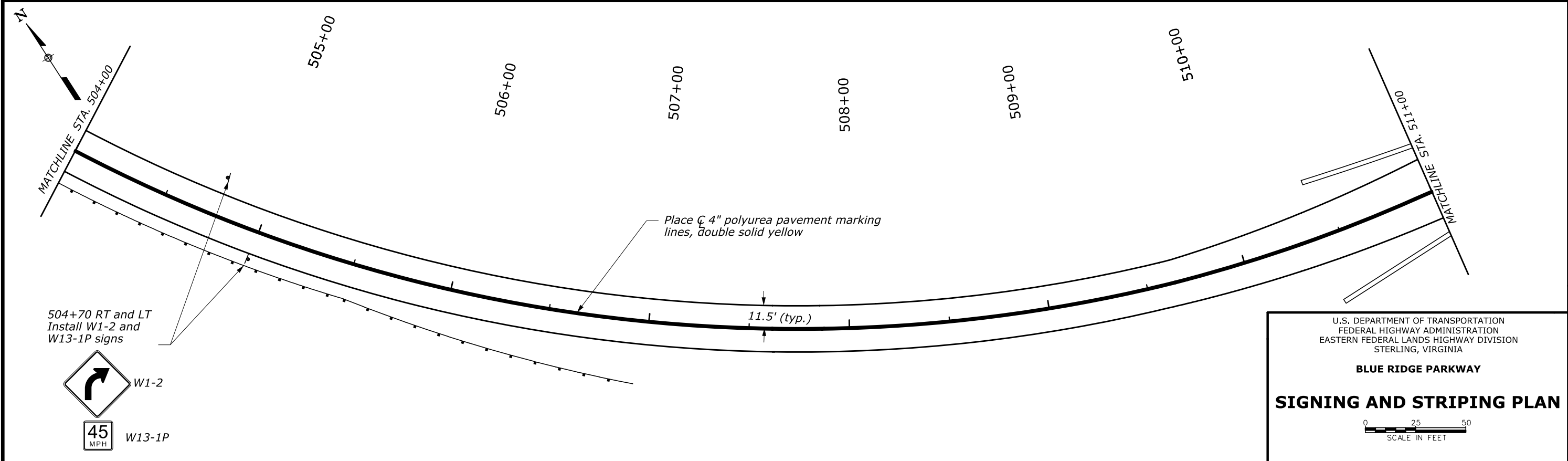
MILE POST 392

MATCHLINE STA. 504+00

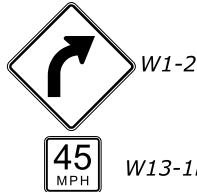
GENERAL NOTE:

Tie the proposed pavement marking lines to the existing pavement marking lines.

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504+70 RT and LT
 Install W1-2 and
 W13-1P signs



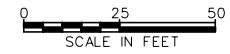
Place 4" polyurea pavement marking lines, double solid yellow

11.5' (typ.)

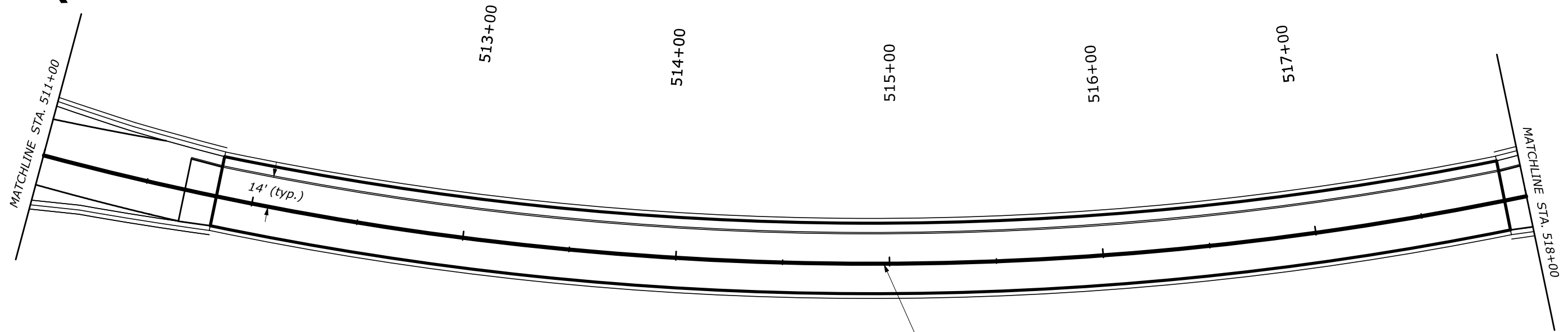
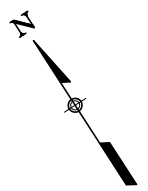
U.S. DEPARTMENT OF TRANSPORTATION
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 STERLING, VIRGINIA

BLUE RIDGE PARKWAY

SIGNING AND STRIPING PLAN

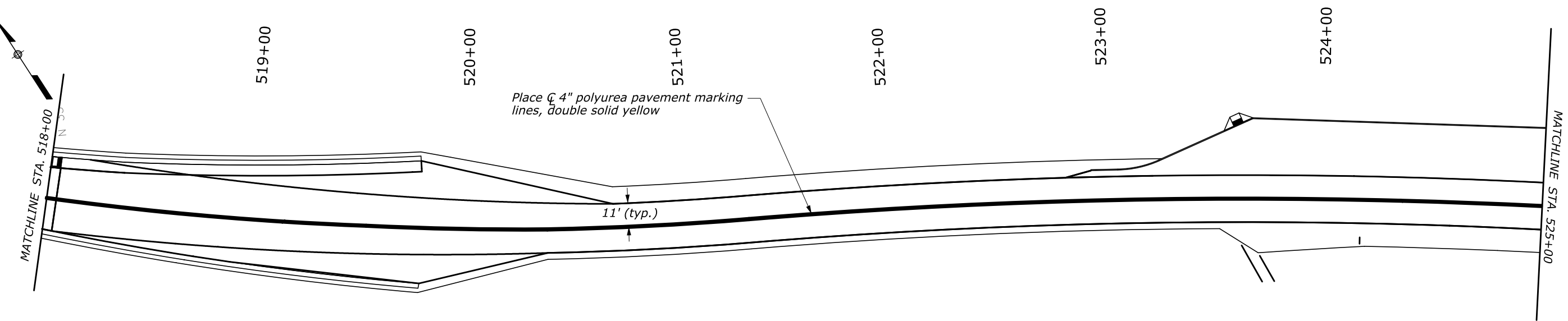
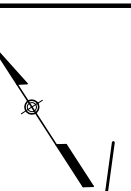


STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	P02



Place 4" polyurea pavement marking lines, double solid yellow

513+00
514+00
515+00
516+00
517+00



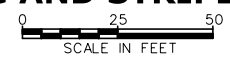
Place 4" polyurea pavement marking lines, double solid yellow

519+00
520+00
521+00
522+00
523+00
524+00

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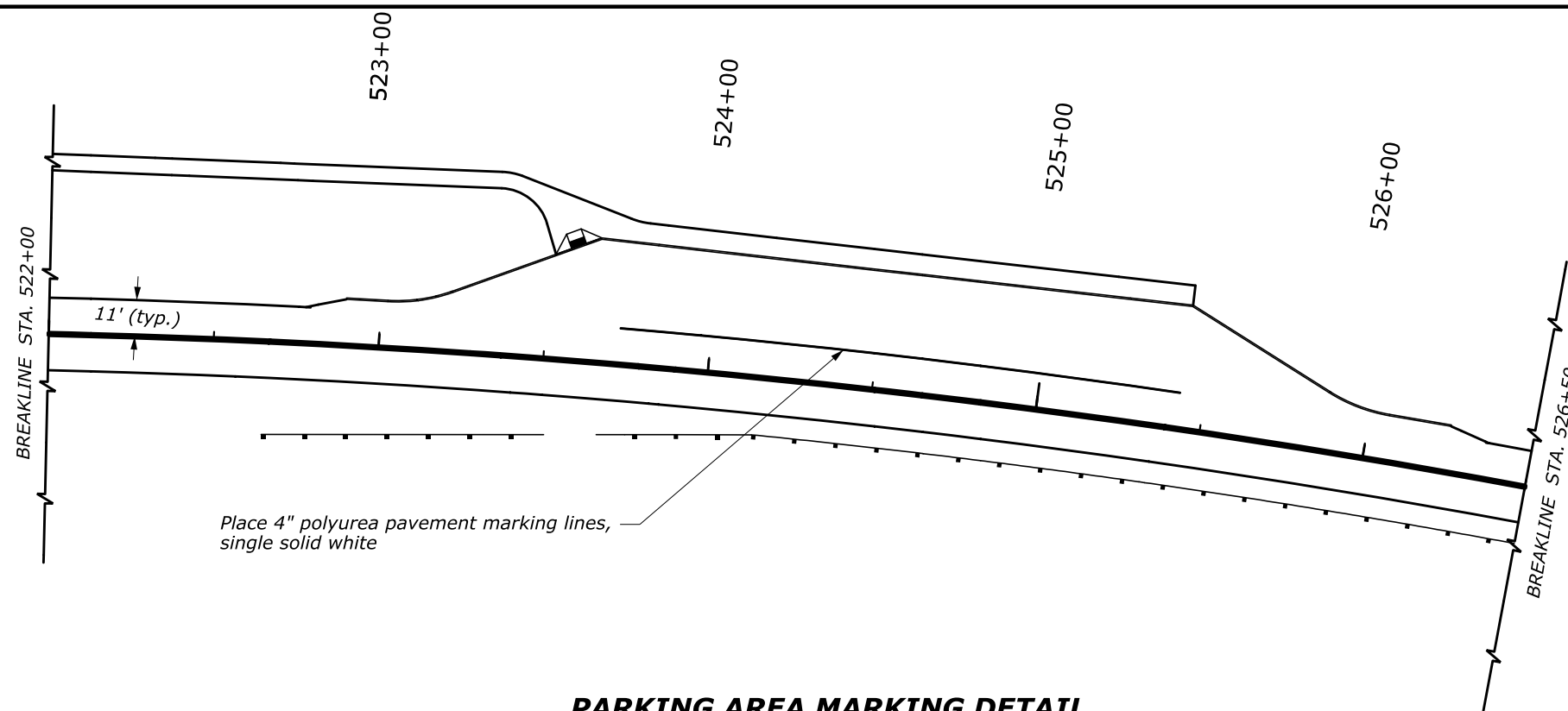
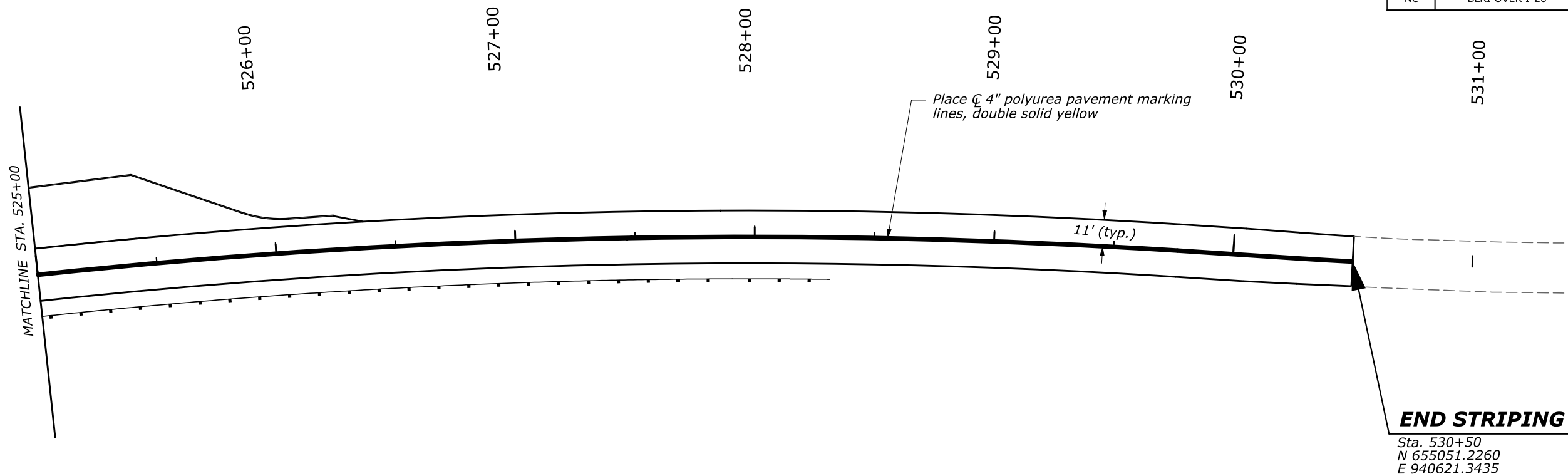
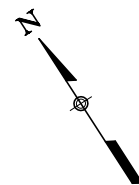
BLUE RIDGE PARKWAY

SIGNING AND STRIPING PLAN



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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	P03

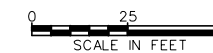


PARKING AREA MARKING DETAIL

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EASTERN FEDERAL LANDS HIGHWAY DIVISION
STERLING, VIRGINIA

BLUE RIDGE PARKWAY

SIGNING AND STRIPING PLAN



STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R01

I. General:

"NCDOT" = North Carolina Department of Transportation

All elevations are in feet.

Furnish Class AA concrete in cast-in-place substructures according to the NCDOT Standard Specifications, unless noted otherwise in the plans.

Furnish Class Special concrete in the bridge railing, sidewalk, and approach slabs according to the Special Provisions, unless noted otherwise in the plans.

Furnish non-ferrous reinforcing steel bar supports in cast-in-place concrete.

For Maintenance and Protection of Traffic beneath Proposed Structure at Station 514+86.30, see Special Provision.

For Thermal Sprayed Coatings (Metallization), see Special Provision.

For Post Tensioning, see Special Provision.

For Precast Segmental Bridge Construction, see Special Provision.

For High Strength Concrete, see Special Provision.

For Post-Tensioning Grout, see Special Provision.

For Disc Bearings, see Special Provision.

For Modular Expansion Joint Seals, see Special Provision.

For Falsework and Formwork, see Special Provision.

For Submittal of Working Drawings, see Special Provision.

For Epoxy Jointing of Precast Segments, see Special Provision.

For Crane Safety, see Special Provision.

For Mass Concrete, see Special Provision.

For Demolition of Existing Bridge, see Special Provision.

For Bridge Deck and Approach Slab Rideability (IRI), see Special Provision.

For Latex Modified Concrete (LMC) Overlay, see Special Provision.

For Caltrans Barrier Rail, see Special Provision.

II. Specifications:

1. Construction:

NCDOT Standard Specifications for Roads and Structures, January 2018.

AASHTO LRFD Bridge Construction Specifications, 3rd Edition, 2010, with interim revisions through 2016.

Project Special Provisions BLRI Over I-26

2. Design

NCDOT Structure Design Manual, with revisions and updates through December 2016.

AASHTO LRFD Bridge Design Specifications, 7th Edition, 2014, with interim revisions and errata through 2016.

CEB-FIP Model Code, 1990.

III. Design Loads:

1. Load factors and load combinations:

Generally follow AASHTO LRFD Section 1 and 3, with the exceptions, additions, and/or modifications as noted in these design criteria.

Load modifiers:

AASHTO LRFD LOAD MODIFIERS		
Ductility η_b	Redundancy η_r	Operational Importance η_i
1.0	1.0	1.0

III. Design Loads (Cont.):

2. Dead loads:

Unit weight of concrete: Per AASHTO LRFD Bridge Design Specifications

Traffic Barrier Rail: Caltrans Type 80SW, 580 LB/LF, plus additional 880 LB/LF for lightweight concrete sidewalk (Unit weight: 110 LB/CF).

Allowance for future wearing surface: 25 LB/SF

3. Earth pressure loads:

Per AASHTO LRFD Bridge Design Specifications

4. Creep, shrinkage, secondary forces from post-tensioning, and locked in forces due to the construction process (including jacking of cantilevers in segmental construction):

Per AASHTO LRFD Bridge Design Specifications and the CEP-FIP Model Code

Design relative humidity = 70%

The average age at initial loading has been assumed to be 90 days for determination of creep and shrinkage parameters.

5. Live load, dynamic load allowance (impact), and pedestrian live load:

Per AASHTO LRFD Bridge Design Specifications

Vehicular design live load: HL-93

6. Braking force:

5% of design truck plus lane load or 5% of design tandem plus lane load per the NCDOT Structure Design Manual

7. Centrifugal force:

Per AASHTO LRFD Bridge Design Specifications. Design speed = 45 MPH.

8. Wind loads:

Per AASHTO LRFD Bridge Design Specifications

Base wind velocity = 115 MPH at 33' height (open country)

9. Earthquake effects:

Per AASHTO LRFD Bridge Design Specifications

10. Thermal effects:

Per AASHTO LRFD Bridge Design Specification and the NCDOT Structure Design Manual

Range of temperatures for stresses resulting from variations in temperature:

Concrete structures: 10°F to 110°F

Assumed normal fabrication and erection temperature: 60°F

Temperature gradient: Per AASHTO LRFD Bridge Specifications for Solar Radiation Zone 3.

Range of temperatures for sizing of expansion joints (post-tensioned concrete):

Concrete structures: 10°F to 110°F

Total movement, M

= 1.25 X long-term creep/shrinkage/post-tensioning + 1.2 X temperature

11. Construction loads:

Formwork and falsework: AASHTO Guide Design Specification for Bridge Temporary Works, 1st Edition, 1995, with interim revisions through 2008

Segmental superstructure construction: AASHTO LRFD Bridge Design Specifications

Segment lifting device: 100 kips centered along box girder at 2 feet from tip of previously erected segment and 400 kips centered along box girder at \bar{C} pier. Weight of device is assumed evenly distributed to each web. Support points and anchor points for device are assumed to be within 3 feet of face of web.

Work platform: 5 kips

Closure joint form and strong back: 10 kips

IV. Materials:

1. Concrete:

Concrete strengths will be as follows:

Location	Class	28-day strength	Release strength
Cast-in-place columns, bent and end bent caps, parapets, pedestals, bent cap steps, footings, moment slab	AA	4.5 KSI	N/A
Precast, prestressed segmental piers, cast-in-place pier footings	HIGH STRENGTH	6.0 KSI	4.8 KSI
Precast, prestressed segmental box girders (including cast-in-place joints), bridge rails, approach slabs	HIGH STRENGTH	6.0 KSI	4.8 KSI

Superstructure concrete will contain a minimum of 25% fly ash Class F or a minimum of 40% ground granulated blast furnace slag.

Substructure concrete will contain a minimum of 25% fly ash Class F or a minimum of 40% ground granulated blast furnace slag. In addition, silica fume at a minimum of 5% shall be used in footings, columns, and piles.

For high strength concrete, see Special Provisions.

Calcium nitrite will be used in the superstructure and substructure precast concrete (including piles) and cast-in-place concrete barrier rail parapets at a rate of 4 gallons per cubic yard.

Chamfer all exposed corners of concrete structures $\frac{3}{4}$ ", unless otherwise noted in the plans.

2. Post-tensioning steel for precast segmental box girders, columns, & caps

See post-tensioning Special Provision.

Prestressing parameters (strand):

Post-tensioning strand will conform to ASTM A416, Grade 270, low relaxation, 0.6" diameter 7-wire strand

Ultimate strength: 270 KSI

Modulus of elasticity (for design): 28,500 KSI

Friction coefficient (polyethylene duct): 0.23

Wobble coefficient (internal tendons): 0.0002 RAD/FT

Anchor set: 0.375"

Maximum jacking stress prior to seating: 216 KSI

Maximum anchor stress after anchor set: 189 KSI

Maximum stress after anchor set away from anchor: 200 KSI

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EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

GENERAL NOTES - 1

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
								CC/CWN	CC/CWN	HC	No Scale	George Choubah	1 of 228	December 2018	BRP-1265

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NC	BLRI OVER I-26	R02

IV. Materials (Cont.):

- Post-tensioning steel for precast segmental box girders, columns, & caps (Cont.):

Prestressing parameters (bar):

Post-tensioning bar will conform to ASTM A722, Grade 150 Type II
 Modulus of elasticity (for design): 30,000 KSI
 Maximum and required jacking stress: 108 KSI (unless noted otherwise)
 Maximum anchor stress at anchor: 105 KSI (unless noted otherwise)
 Wobble coefficient: 0.0002 RAD/FT
 Anchor set: 0.0625"

- Reinforcing steel:

Reinforcing steel and epoxy coated reinforcing steel will conform to ASTM A615, Grade 60 deformed.
 Provide 2-inch cover for reinforcing steel unless noted otherwise.

- Bearings:

Disc bearings will be per Special Provisions.
 Bearings are designed for future replacement.

- Structural steel, steel pipe, and related steel items:

When called for in the plans, provide galvanizing in accordance with the NCDOT Standard Specifications (Section 1076).
 Steel H-Piles will conform to ASTM A572, Grade 50.

V. Allowable Stresses:

- Reinforced concrete elements:

For pier footings, minimum concrete strength : 6.0 ksi

For all other reinforced concrete elements, per AASHTO LRFD Bridge Design Specifications and NCDOT Structure Design Manual.

- Precast segmental concrete box girder superstructures:

Per AASHTO LRFD Bridge Design Specifications, as listed below:

During construction:

Temporary concrete compressive stress limits before losses: 60% of concrete strength at age of check (per AASHTO LRFD Article 5.9.4.1.1)

Temporary concrete tensile stress limits before losses (permanent loads only): no tension (per AASHTO LRFD Table 5.9.4.1.2-1)

Temporary concrete tensile stress limits before losses (with construction loadings applied): based on applicable loading condition of AASHTO LRFD Table 5.14.2.3.3-1

After construction:

Concrete compressive stress limits at Service limit state after losses (sum of effective prestress and permanent loads): 2.7 KSI (per AASHTO LRFD Table 5.9.4.2.1-1)

Concrete compressive stress limits at Service limit state after losses: 3.6 KSI (per AASHTO LRFD Table 5.9.4.2.1-1 and adjusted where required by the section slenderness factor)

Concrete tensile stress limits at Service limit state after losses (longitudinal): no tension (per AASHTO LRFD Table 5.9.4.2.2-1)

Concrete tensile stress limits at Service limit state after losses (transverse): 0.230 KSI (per AASHTO LRFD Table 5.9.4.2.2-1)

Principal concrete tensile stress in web at neutral axis at service limit state after losses: 0.270 KSI (per AASHTO LRFD Table 5.9.4.2.2-1)

Shear and torsion design at the Strength limit state will be per AASHTO LRFD Article 5.8.6.

V. Allowable Stresses (Cont.):

- Superstructure segment casting and erection:

The following minimum concrete strength limits shall be observed:

Prior to lifting segments or lowering support forms: 2.5 KSI
 Prior to full transverse post-tensioning: 6.0 KSI
 Prior to longitudinal post-tensioning of precast segments: 6.0 KSI
 Prior to stressing longitudinal post-tensioning (cast-in-place closure joints only): 2.5 KSI

- Precast segmental concrete column segments:

Per AASHTO LRFD Bridge Design Specifications, as listed below:

During construction:

Temporary concrete compressive stress limits before losses: 60% of concrete strength at age of check (per AASHTO LRFD Article 5.9.4.1.1)

Temporary concrete tensile stress limits before losses (permanent loads only): no tension (per AASHTO LRFD Table 5.9.4.1.2-1)

Temporary concrete tensile stress limits before losses (with construction loadings applied): based on applicable loading condition of AASHTO LRFD Table 5.14.2.3.3-1

After construction:

Concrete compressive stress limits at Service limit state after losses (sum of effective prestress and permanent loads): 2.7 KSI (per AASHTO LRFD Table 5.9.4.2.1-1)

Concrete compressive stress limits at Service limit state after losses: 3.6 KSI (per AASHTO LRFD Table 5.9.4.2.1-1 and adjusted where required by the section slenderness factor)

Concrete tensile stress limits at Service limit state after losses: no tension (per AASHTO LRFD Table 5.9.4.2.2-1)

Shear and torsion design at the Strength limit state will be per AASHTO LRFD Article 5.8.6.

- Column segment casting and erection:

The following minimum concrete strength limits will be observed:

Minimum concrete strength prior to lifting segments: 2.5 KSI
 Minimum concrete strength prior to stressing post-tensioning: 6.0 KSI

VII. Segmental Construction:

Superstructure:

For proposed erection sequence, see construction sequence drawings.

The contractor will submit any deviation from the proposed erection sequence to the engineer for approval.

The contractor will ensure that the structure remains safe and stable under all anticipated construction stages and loading conditions.

The design is based on a continuous construction season with no stoppage period.

For all stages during erection, construction loads will be submitted to the engineer for approval.

The contractor will submit precast segment shop drawings and erection procedure manuals in accordance with the Special Provisions. The contractor's specialty engineer will detail all reinforcing steel and all tendons with the proper vertical and horizontal alignment for each precast segment and closure joint.

Anchorage systems and blockouts will be determined by the post-tensioning system used. The contractor will determine post-tensioning dimensions and will adjust blockouts as required for stressing equipment. Approved local anchorage zone reinforcing is required at the ends of all post-tensioning tendons, unless otherwise noted. The contractor will adjust local anchorage zone reinforcing and other reinforcing as necessary to clear tendons.

If stacking of the segments is desired, submit a segment stacking plan and a calculation demonstrating adequate performance of the stacked design to the engineer for approval prior to start of segment stacking. The performance of stacking and condition of segments after stacking remains the responsibility of the contractor.

Segment joints will be epoxy bonded per the epoxy jointing of precast segments Special Provision.

The following activities are not allowed over traffic: placement of segments and beams, placement and removal of formwork, placement and removal of concrete.

Substructure:

The contractor will submit precast segment and bent cap shop drawings based on the details provided on the plans and in accordance with the Special Provisions.

Anchorage systems will be designed by the post-tensioning bar/tendon supplier for the system used in accordance with the Special Provisions. Dimensions of anchor plates and blockouts will be designed by the supplier. Local zone reinforcing will be designed by the supplier and incorporated into the shop drawings.

Segment joints shall be epoxy bonded per the epoxy jointing of precast segment Special Provisions.

VIII Foundations:

Abutments:

Resistance factor for verification of driven piles established by dynamic testing only: 0.75

Piers:

Verification of competent rock should be performed in the field and approved by the CO prior to placing the concrete.

Rock damaged during excavation should be removed and replaced with concrete with minimum compressive strength of 4 ksi.

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 EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

GENERAL NOTES - 2

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R03

Sheet #	Sheet Description	Sheet #	Sheet Description	Sheet #	Sheet Description
1	General Notes - 1	81	Segment 1-5D Reinforcement	161	Segment 2-3U Bar List
2	General Notes - 2	82	Segment 1-5D Bar List	162	Segment 2-4U Reinforcement
3	List of Sheets	83	Segment 1-4D Reinforcement	163	Segment 2-4U Bar List
4	Site Plan	84	Segment 1-4D Bar List	164	Segment 2-5U Reinforcement
5	Plan and Elevation	85	Segment 1-3D Reinforcement	165	Segment 2-5U Bar List
6	Foundation Layout	86	Segment 1-3D Bar List	166	Segment 2-6U Reinforcement
7	Abutment 1 Layout	87	Segment 1-2D Reinforcement	167	Segment 2-6U Bar List
8	Abutment 1 Reinforcement	88	Segment 1-2D Bar List	168	Segment 2-7U Reinforcement
9	Wingwall A Layout	89	Segment 1-1D Reinforcement	169	Segment 2-7U Bar List
10	Wingwall A Reinforcement	90	Segment 1-1D Bar List	170	Segment 2-8U Reinforcement
11	Wingwall B Layout	91	Pier Segment Dimensions	171	Segment 2-8U Bar List
12	Wingwall B Reinforcement	92	Segment P1-1 Reinforcement - 1	172	Segment 2-9U Reinforcement
13	Abutment 2 Layout	93	Segment P1-1 Reinforcement - 2	173	Segment 2-9U Bar List
14	Abutment 2 Reinforcement	94	Segment P1-1 Bar List	174	Segment 2-10U Reinforcement
15	Wingwall C Layout	95	Segment P1-2 Reinforcement - 1	175	Segment 2-10U Bar List
16	Wingwall C Reinforcement	96	Segment P1-2 Reinforcement - 2	176	Segment 2-11U Reinforcement
17	Wingwall D Layout	97	Segment P1-2 Bar List	177	Segment 2-11U Bar List - 1
18	Wingwall D Reinforcement	98	Segment 1-1U Reinforcement	178	Segment 2-11U Bar List - 2
19	Pier Layout	99	Segment 1-1U Bar List	179	Segment 2-12U Reinforcement
20	Pier 1 Footing	100	Segment 1-2U Reinforcement	180	Segment 2-12U Bar List
21	Pier 2 Footing	101	Segment 1-2U Bar List	181	Segment 2-13U Reinforcement
22	Pier Segment PC-1 Layout	102	Segment 1-3U Reinforcement	182	Segment 2-13U Bar List
23	Pier Segment PC-1 Reinforcement - 1	103	Segment 1-3U Bar List	183	Segment A2-2D Reinforcement
24	Pier Segment PC-1 Reinforcement - 2	104	Segment 1-4U Reinforcement	184	Segment A2-2D Bar List
25	Pier Segment PC-1 Reinforcement - 3	105	Segment 1-4U Bar List	185	Segment A2-1D Reinforcement
26	Pier Segment PC-1 Bar List - 1	106	Segment 1-5U Reinforcement	186	Segment A2-1D Bar List
27	Pier Segment PC-1 Bar List - 2	107	Segment 1-5U Bar List	187	Segment A2 Reinforcement - 1
28	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Layout	108	Segment 1-6U Reinforcement	188	Segment A2 Reinforcement - 2
29	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Reinforcement	109	Segment 1-6U Bar List	189	Segment A2 Reinforcement - 3
30	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 1	110	Segment 1-7U Reinforcement	190	Segment A2 Bar List
31	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 2	111	Segment 1-7U Bar List	191	Closure Segments Reinforcement
32	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 3	112	Segment 1-8U Reinforcement	192	Closure Segments Bar List
33	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 4	113	Segment 1-8U Bar List	193	Bulkhead Details
34	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 5	114	Segment 1-9U Reinforcement	194	Longitudinal Post-Tensioning Layout - 1
35	Pier Segments PC-2, PC-3, PC-4, PC-5, & PC-6-2 Bar List - 6	115	Segment 1-9U Bar List	195	Longitudinal Post-Tensioning Layout - 2
36	Pier Segment PC-6-1 Layout	116	Segment 1-10U Reinforcement	196	Longitudinal Post-Tensioning Schedule
37	Pier Segment PC-6-1 Reinforcement	117	Segment 1-10U Bar List	197	Longitudinal Tendon Curvature Details
38	Pier Segment PC-6-1 Bar List - 1	118	Segment 1-11U Reinforcement	198	External Tendon Curvature Details
39	Pier Segment PC-6-1 Bar List - 2	119	Segment 1-11U Bar List	199	Transverse Post-Tensioning Details
40	Pier Segment PC-7 Layout	120	Segment 1-12U Reinforcement	200	Type 1 Deviator
41	Pier Segment PC-7 Reinforcement - 1	121	Segment 1-12U Bar List	201	Type 2 Deviator
42	Pier Segment PC-7 Reinforcement - 2	122	Segment 1-13U Reinforcement	202	Bottom Slab Blister
43	Pier Segment PC-7 Bar List - 1	123	Segment 1-13U Bar List	203	Top Slab Blister
44	Pier Segment PC-7 Bar List - 2	124	Segment 2-13D Reinforcement	204	Erection Anchor Block Details
45	Pier Segment PC-8 Layout	125	Segment 2-13D Bar List	205	Post-Tensioning Details
46	Pier Segment PC-8 Reinforcement	126	Segment 2-12D Reinforcement	206	Top Erection PT Bar Anchor Block Details
47	Pier Segment PC-8 Bar List	127	Segment 2-12D Bar List	207	Bottom Erection PT Bar Anchor Block Details
48	Pier Miscellaneous Details - 1	128	Segment 2-11D Reinforcement	208	Bottom Slab Access Opening Details
49	Pier Miscellaneous Details - 2	129	Segment 2-11D Bar List	209	Abutment Access Opening Details
50	Typical Section	130	Segment 2-10D Reinforcement	210	Bridge Railing - 1
51	Segment Layout	131	Segment 2-10D Bar List	211	Bridge Railing - 2
52	Top of Deck Elevations - 1	132	Segment 2-9D Reinforcement	212	Bridge Railing - 3
53	Top of Deck Elevations - 2	133	Segment 2-9D Bar List	213	Bridge Railing - 4
54	Joint Coordinates	134	Segment 2-8D Reinforcement	214	Expansion Joint Details - 1
55	Abutment Segment Dimensions	135	Segment 2-8D Bar List	215	Expansion Joint Details - 2
56	Segment A1 Reinforcement - 1	136	Segment 2-7D Reinforcement	216	Bearing Details
57	Segment A1 Reinforcement - 2	137	Segment 2-7D Bar List	217	Bearing Replacement Details
58	Segment A1 Reinforcement - 3	138	Segment 2-6D Reinforcement	218	Approach Slab
59	Segment A1 Bar List	139	Segment 2-6D Bar List	219	Camber Diagram
60	Segment A1-1U Reinforcement	140	Segment 2-5D Reinforcement	220	Construction Sequence - 1
61	Segment A1-1U Bar List	141	Segment 2-5D Bar List	221	Construction Sequence - 2
62	Segment A1-2U Reinforcement	142	Segment 2-4D Reinforcement	222	Cantilever Construction Sequence
63	Segment A1-2U Bar List	143	Segment 2-4D Bar List	223	Abutment Bar List
64	Segment 1-13D Reinforcement	144	Segment 2-3D Reinforcement	224	Wingwall Bar List
65	Segment 1-13D Bar List	145	Segment 2-3D Bar List	225	Bridge Railing & Sidewalk Bar List
66	Segment 1-12D Reinforcement	146	Segment 2-2D Reinforcement	226	Pier Footing Bar List
67	Segment 1-12D Bar List	147	Segment 2-2D Bar List	227	Bearing Pedestal Bar List
68	Segment 1-11D Reinforcement	148	Segment 2-1D Reinforcement	228	Approach Sidewalk Bar List
69	Segment 1-11D Bar List - 1	149	Segment 2-1D Bar List		
70	Segment 1-11D Bar List - 2	150	Segment P2-1 Reinforcement - 1		
71	Segment 1-10D Reinforcement	151	Segment P2-1 Reinforcement - 2		
72	Segment 1-10D Bar List	152	Segment P2-1 Bar List		
73	Segment 1-9D Reinforcement	153	Segment P2-2 Reinforcement - 1		
74	Segment 1-9D Bar List	154	Segment P2-2 Reinforcement - 2		
75	Segment 1-8D Reinforcement	155	Segment P2-2 Bar List		
76	Segment 1-8D Bar List	156	Segment 2-1U Reinforcement		
77	Segment 1-7D Reinforcement	157	Segment 2-1U Bar List		
78	Segment 1-7D Bar List	158	Segment 2-2U Reinforcement		
79	Segment 1-6D Reinforcement	159	Segment 2-2U Bar List		
80	Segment 1-6D Bar List	160	Segment 2-3U Reinforcement		

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 FEDERAL HIGHWAY ADMINISTRATION
 EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

LIST OF SHEETS

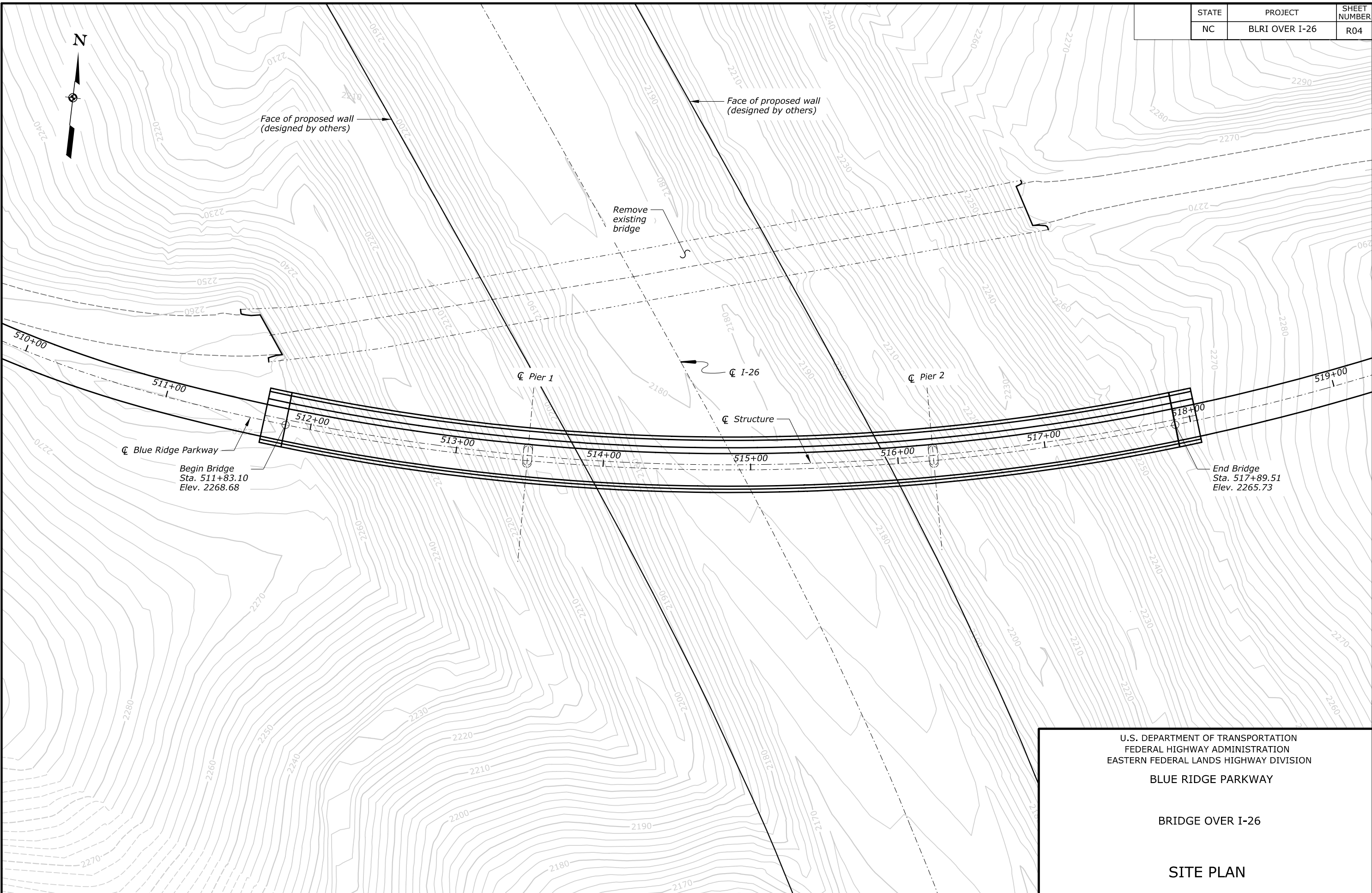
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								CC	CC	CWN	No Scale	George Choubah	3 of 228	December 2018	BRP-1265

ACTUAL FILE: R04_BLR1_I26_SITE PLAN.DGN

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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R04



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 FEDERAL HIGHWAY ADMINISTRATION
 EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

SITE PLAN

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
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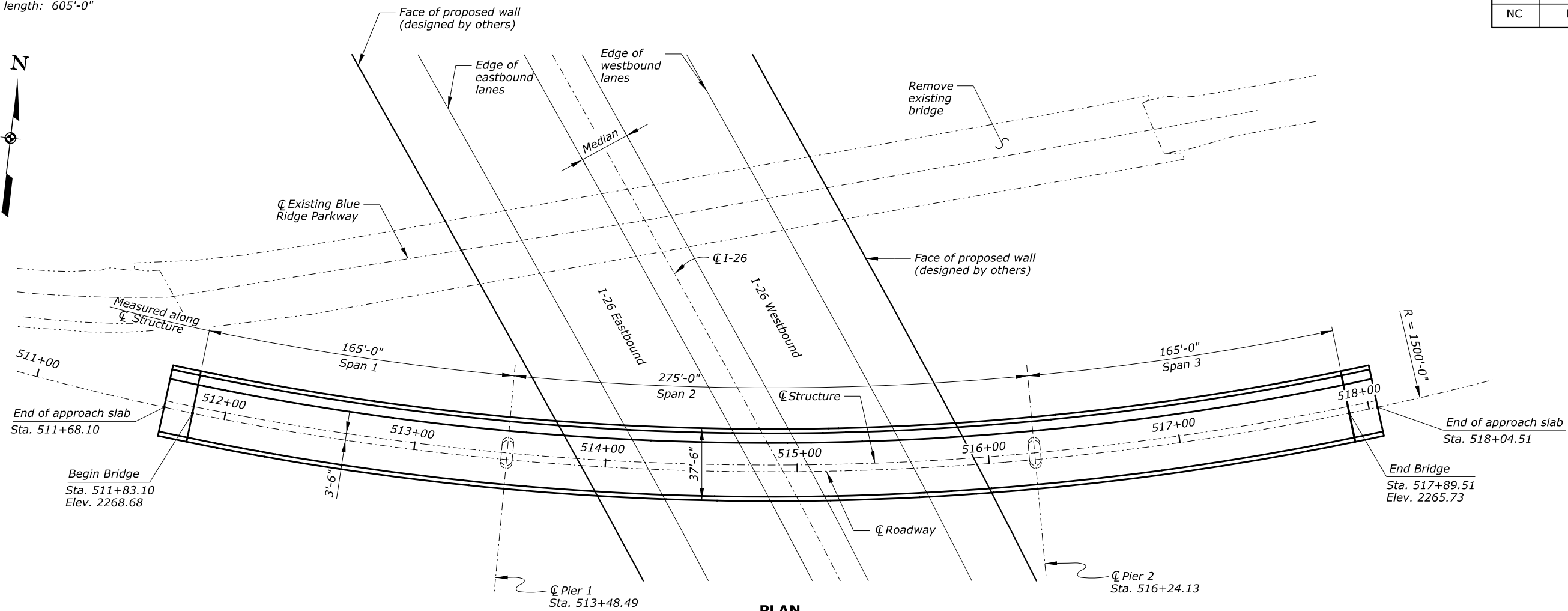
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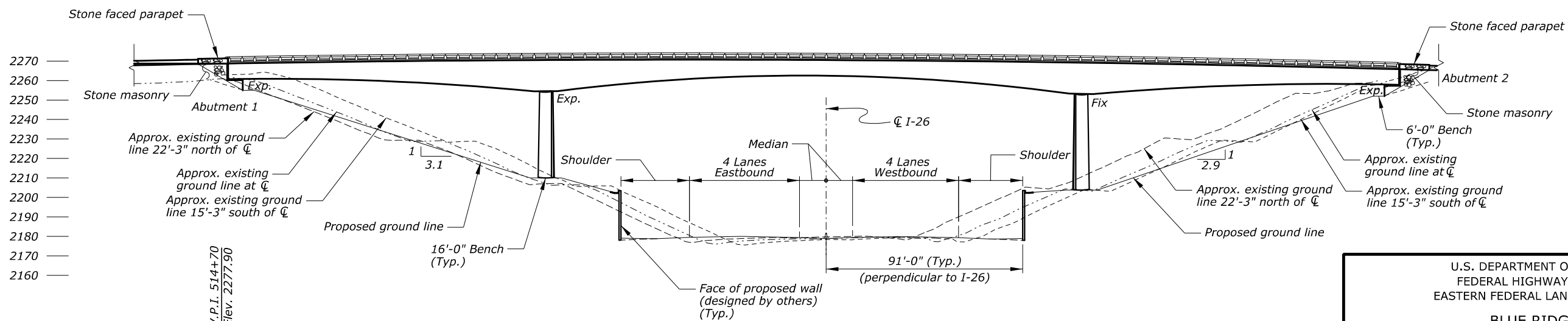
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R05

Bridge length: 605'-0"



PLAN



ELEVATION

PROFILE GRADE
NO SCALE

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EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

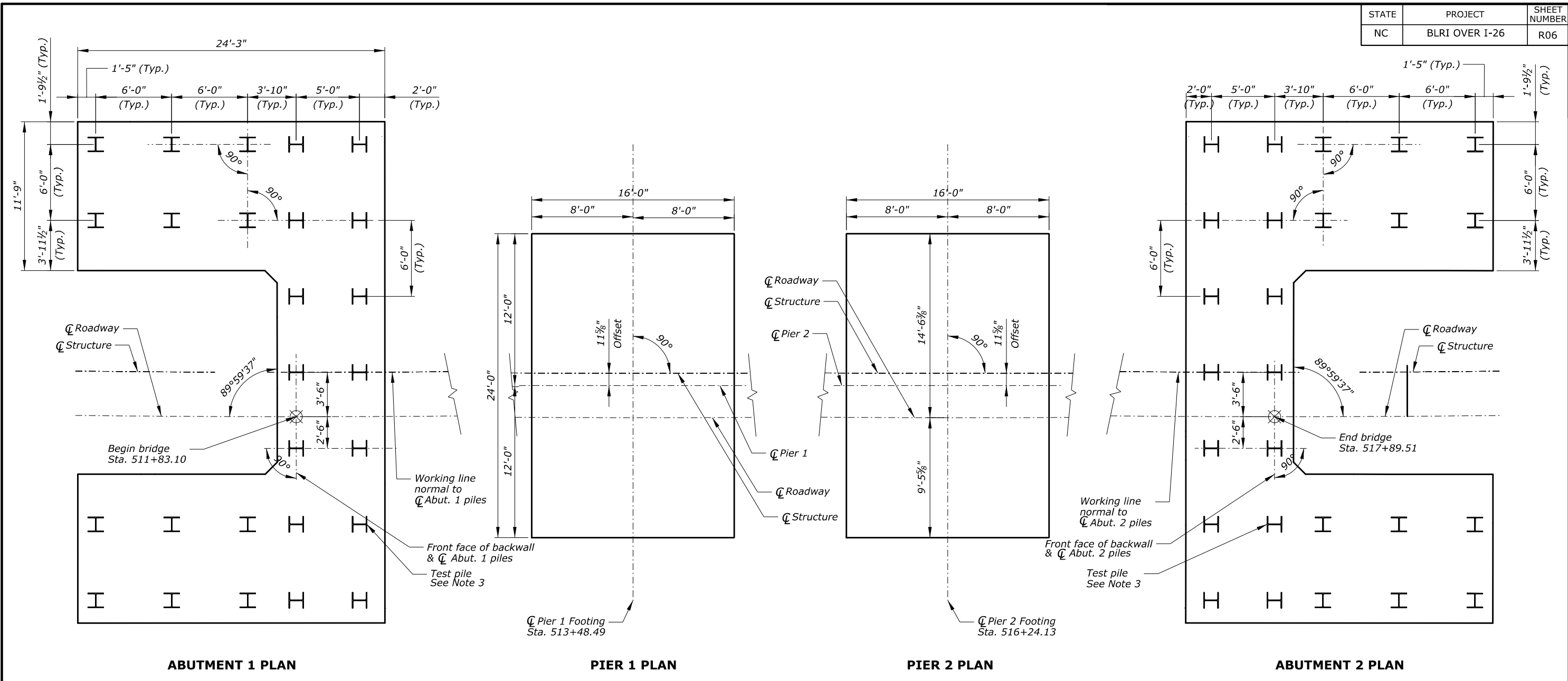
PLAN AND ELEVATION

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								AR	AR	CWN	1" = 30'-0"	George Choubah	5 of 228	December 2018	BRP-1265

ACTUAL FILE: R06_BLR1_126_FOUNDATION_LAYOUT.dgn

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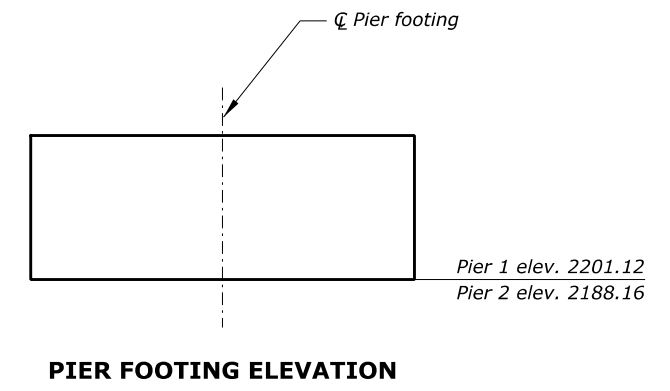
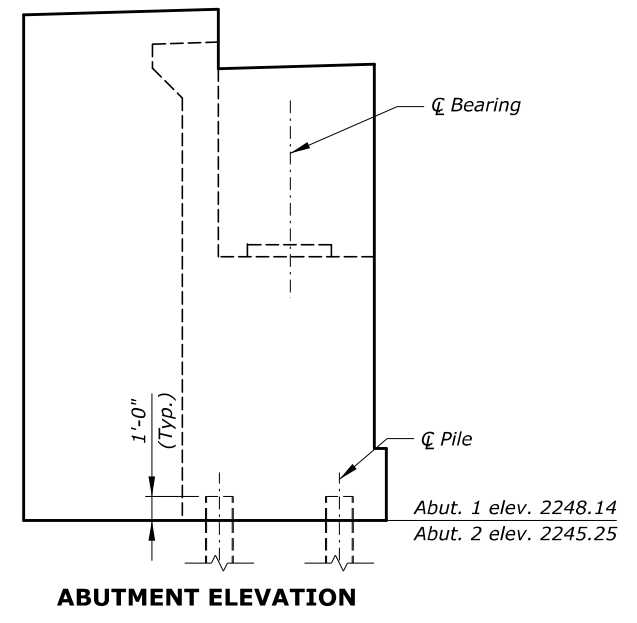


ABUTMENT 1 PLAN

PIER 1 PLAN

PIER 2 PLAN

ABUTMENT 2 PLAN



PILE DATA TABLE			
LOCATION	ESTIMATED PILE TIP ELEVATION (ft)	ESTIMATED TOTAL LENGTH (ft)	FACTORED GEOTECHNICAL RESISTANCE (Kips/Pile)
ABUTMENT 1	2188.10	1587.08	349
ABUTMENT 2	2195.15	1328.60	438

- Note:
- Pile locations are dimensioned parallel and perpendicular to working lines.
 - Furnish HP 14x89 steel piles.
 - Perform one PDA load test per abutment. Load test to be performed using production pile.

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 BLUE RIDGE PARKWAY
 BRIDGE OVER I-26
 FOUNDATION LAYOUT

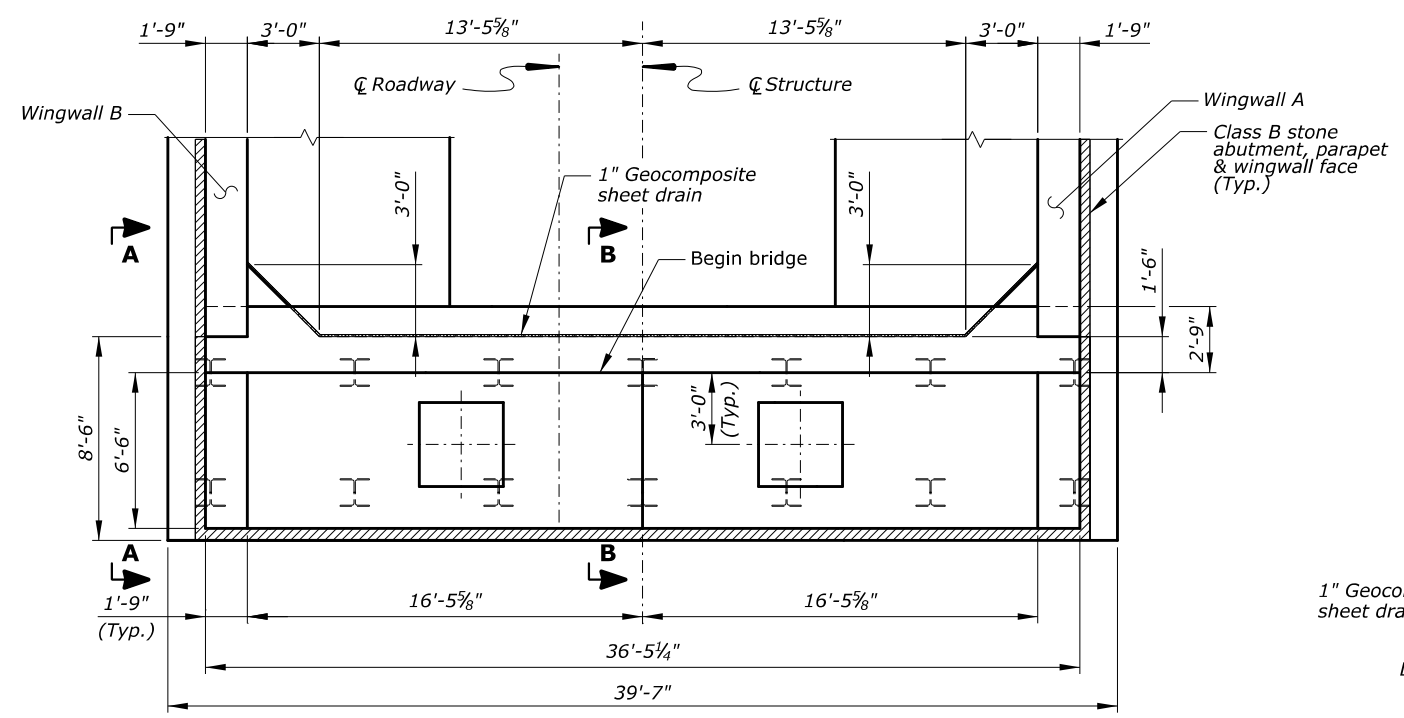
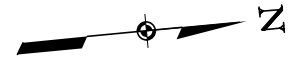
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								LE	LE	CWN	1/4" = 1'-0"	George Choubah	6 of 228	December 2018	BRP-1265

STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R07

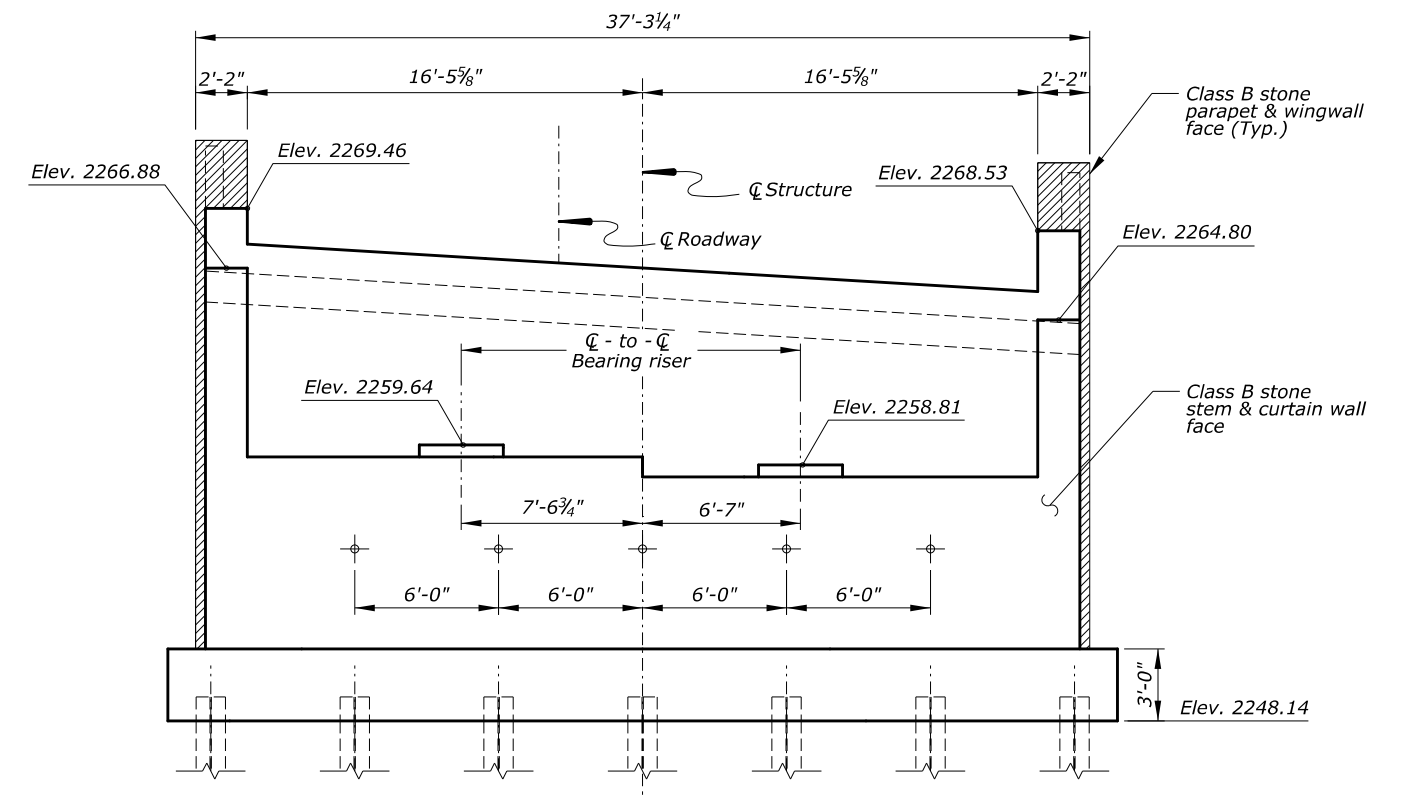
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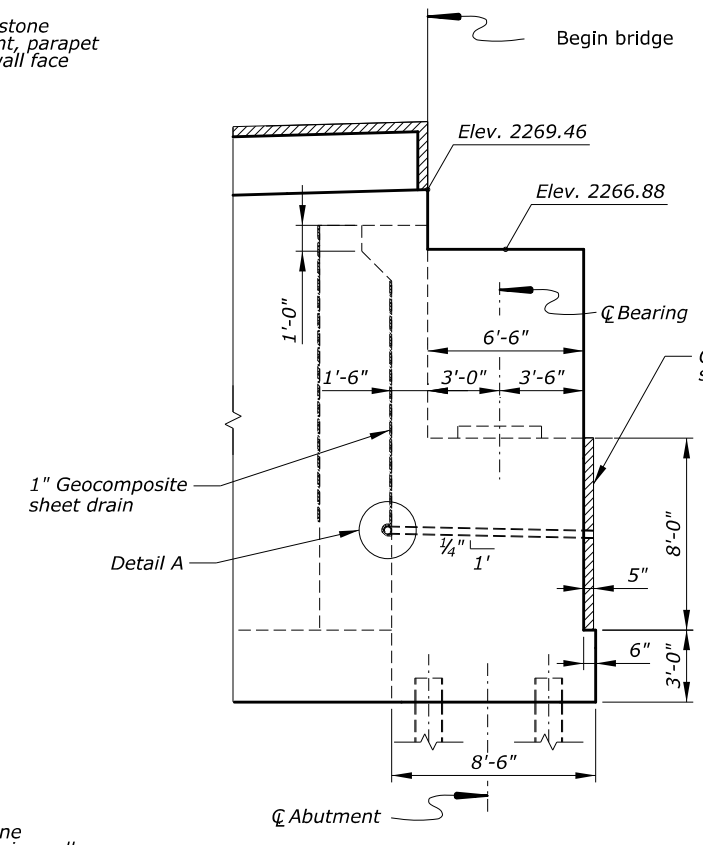


ABUTMENT 1 PLAN
Scale: 1/4" = 1'-0"

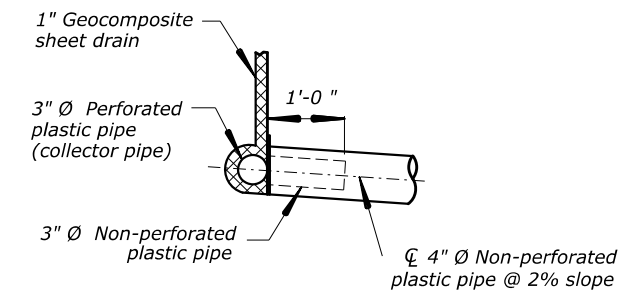


ABUTMENT 1 ELEVATION
Scale: 1/4" = 1'-0"

Key:
n.f. = near face
f.f. = far face
e.f. = each face

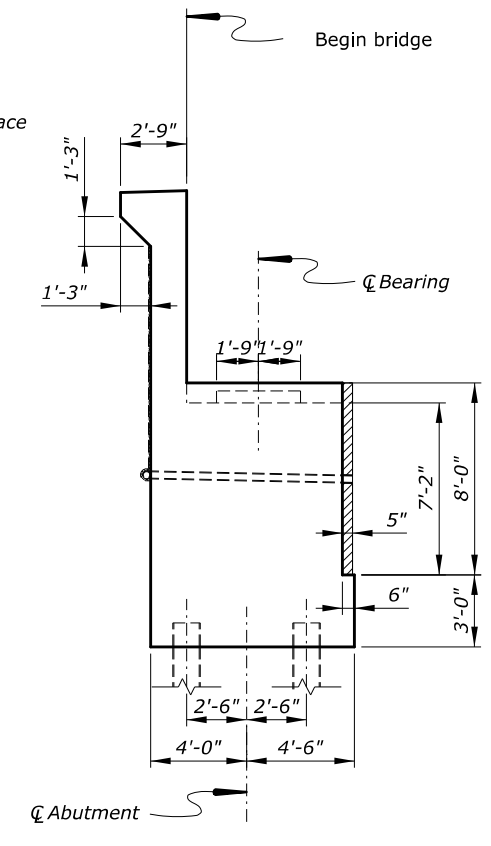


VIEW A-A
Scale: 1/4" = 1'-0"



DETAIL A
Scale: 1" = 1'-0"

- Notes:**
1. See "FOUNDATION LAYOUT" sheet for additional information.
 2. Anchor masonry to concrete per anchor manufacturer's instructions utilizing stainless steel dovetail anchor and slots, spaced at 24 inch max.
 3. Adjust elevation of weepholes to outlet 1'-0" above finish grade.
 4. See BRIDGE RAIL and "APPROACH SLAB" sheets for additional information.



SECTION B-B
Scale: 1/4" = 1'-0"

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EASTERN FEDERAL LANDS HIGHWAY DIVISION

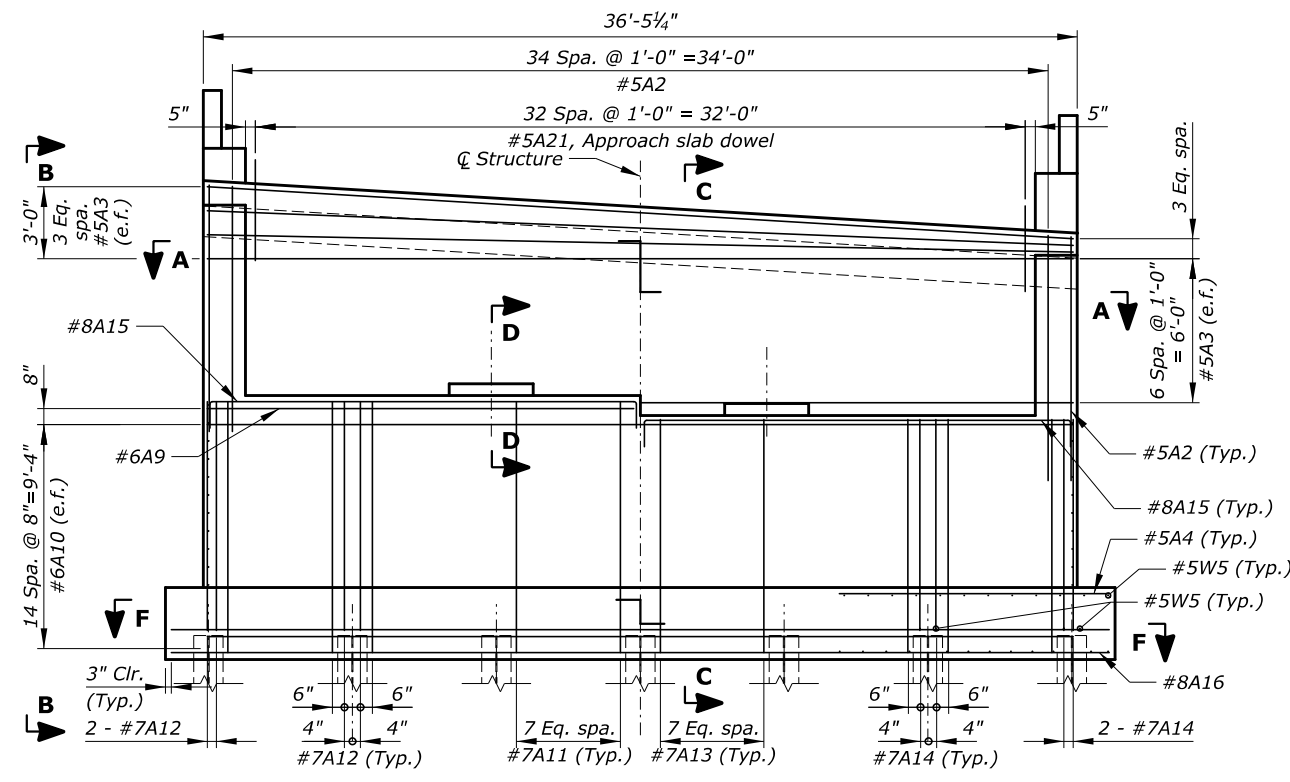
BLUE RIDGE PARKWAY

BRIDGE OVER I-26

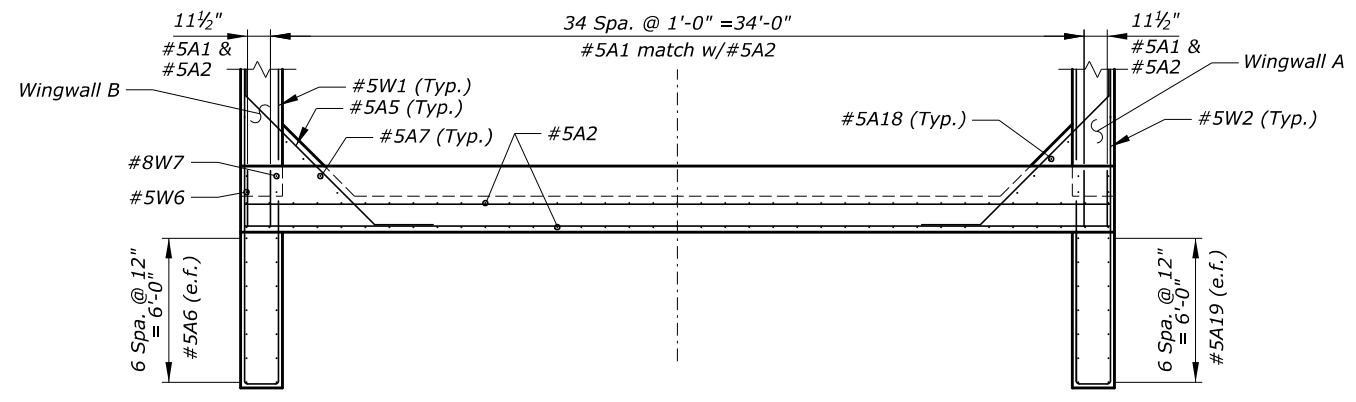
ABUTMENT 1 LAYOUT

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
								LE	LE	CWN	As Shown	George Choubah	7 of 228	December 2018	BRP-1265

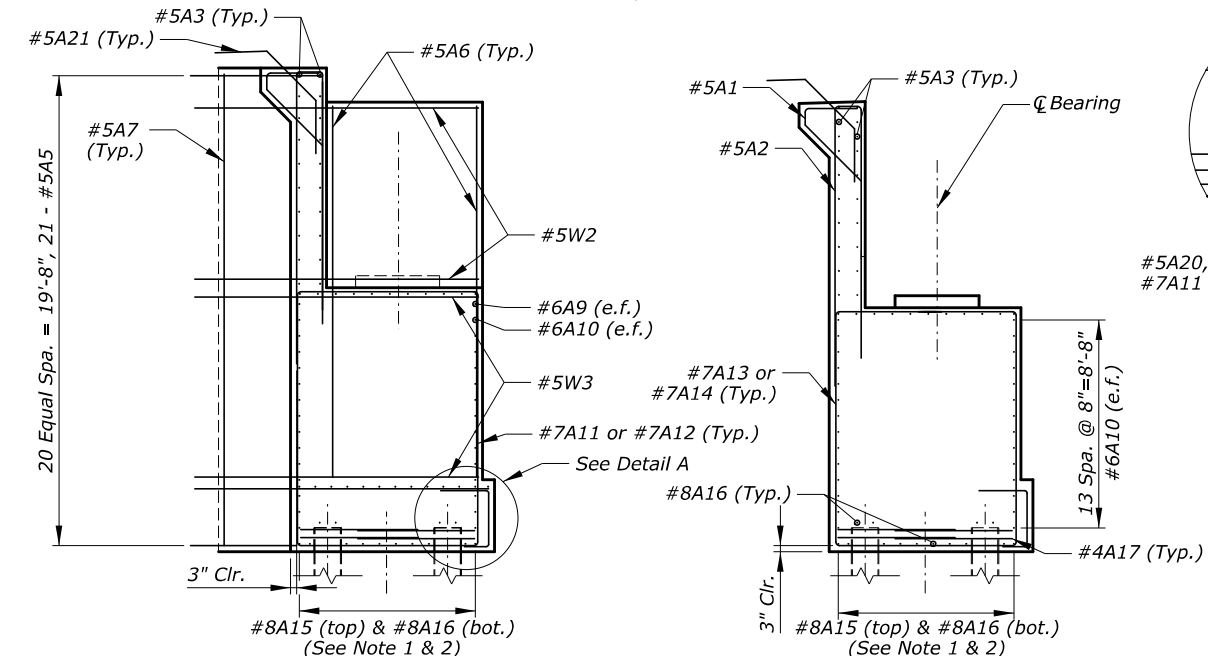
STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R08



ABUTMENT 1 ELEVATION VIEW
Scale: 1/4" = 1'-0"

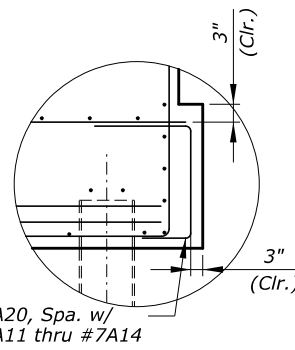


SECTION A-A
Scale: 1/4" = 1'-0"

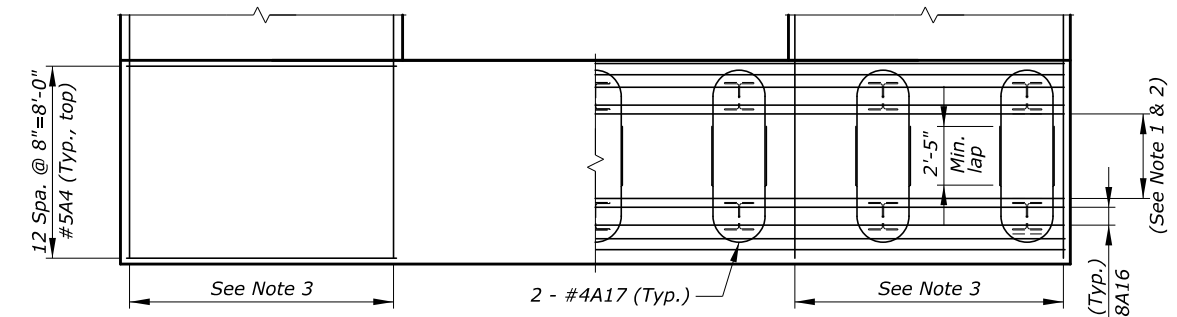


VIEW B-B
Scale: 1/4" = 1'-0"

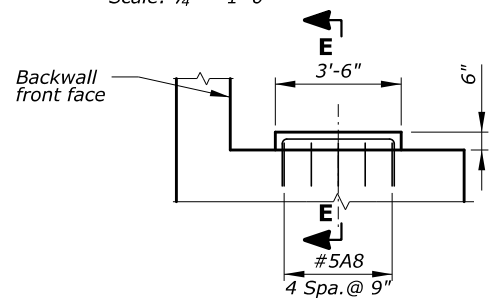
SECTION C-C
Scale: 1/4" = 1'-0"



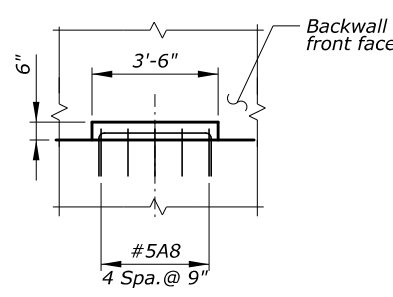
DETAIL A



SECTION F-F
Scale: 1/4" = 1'-0"



SECTION D-D
Scale: 3/8" = 1'-0"



SECTION E-E
Scale: 3/8" = 1'-0"

Notes:

1. Provide 9-inch maximum spacing for top and bottom abutment stem reinforcement.
2. Space and cut all reinforcement to maintain 2-inch clearance around H-piles.
3. See "WINGWALL A REINFORCEMENT" and "WINGWALL B REINFORCEMENT" sheets for additional information.
4. Provide a minimum embedment of 1'-6" for #5 reinforcement. Provide a minimum lap of 2'-0" for #5 reinforcement.

Key:

- n.f. = near face
- f.f. = far face
- e.f. = each face

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EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

ABUTMENT 1 REINFORCEMENT

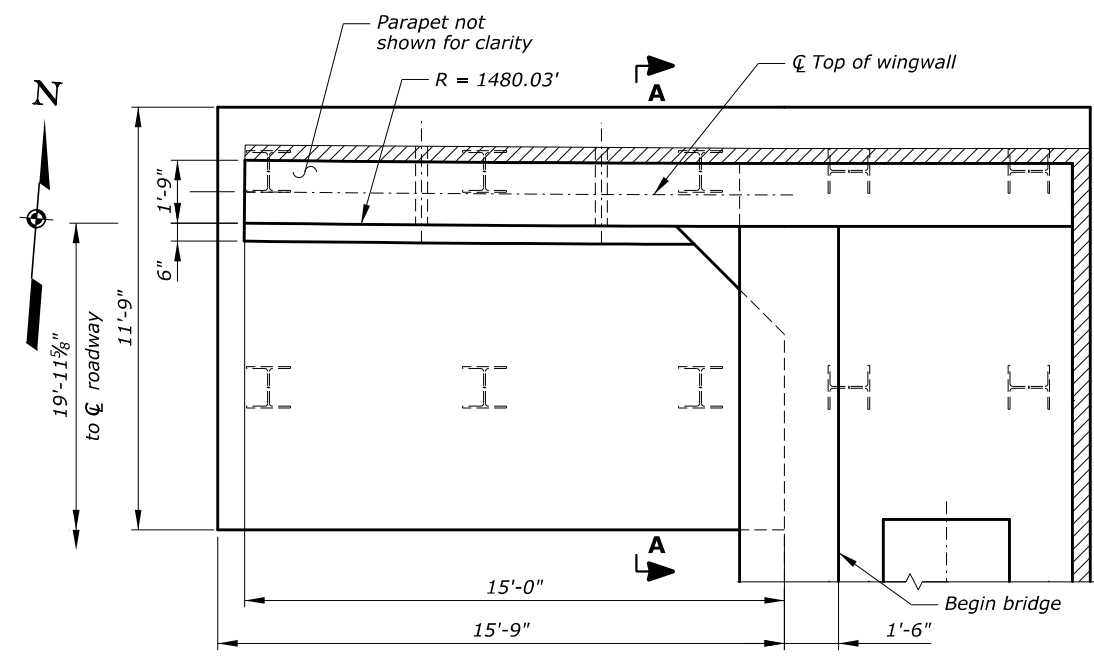
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ACTUAL FILE: R09_BLR1_I26_WINGWALL LAYOUT A.dgn

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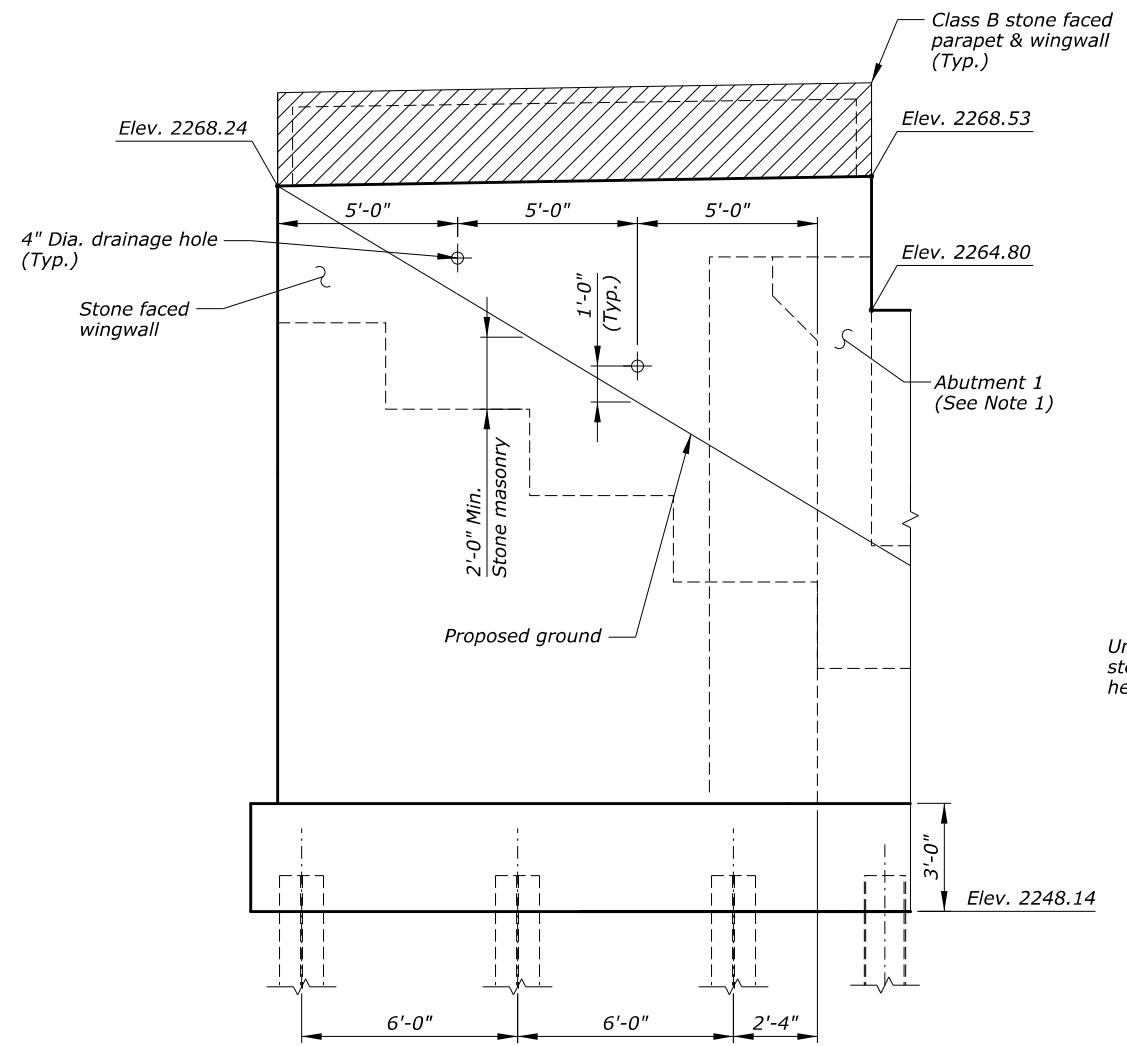
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NC	BLRI OVER I-26	R09



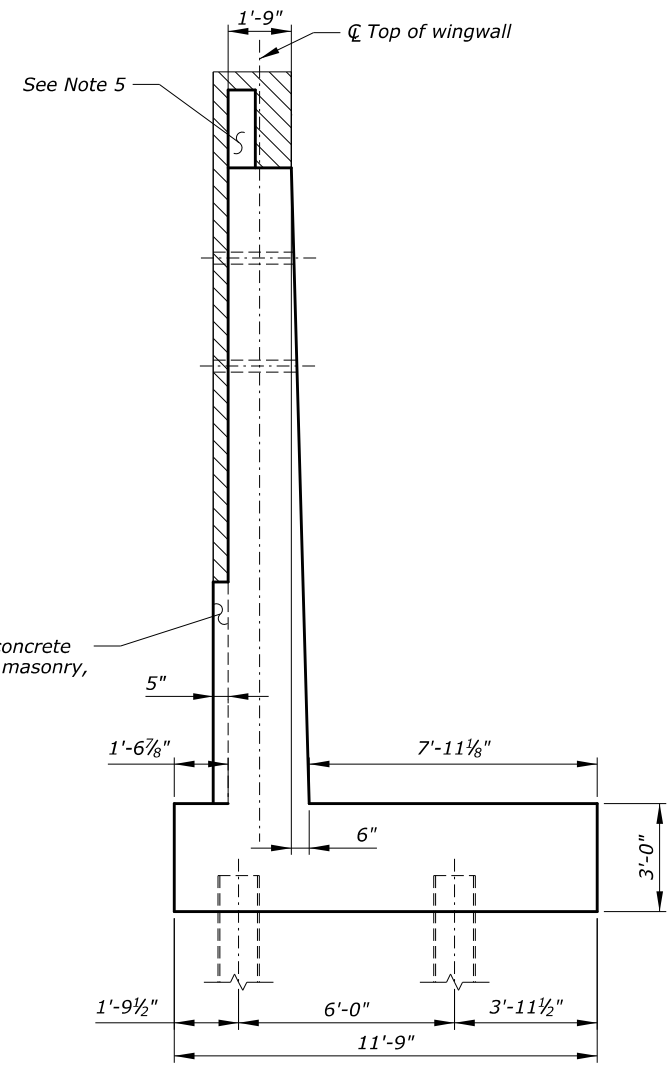
WINGWALL A PLAN

Notes:

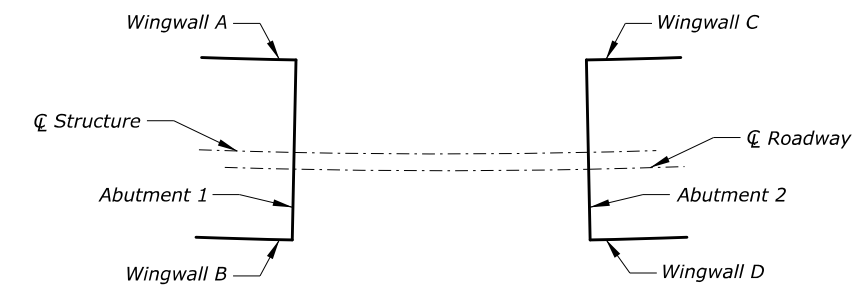
1. See "ABUTMENT 1 REINFORCEMENT" and "ABUTMENT 1 LAYOUT" sheets for additional information.
2. See "WINGWALL A REINFORCEMENT" sheet for additional information.
3. See "FOUNDATION LAYOUT" sheet for additional information.
4. Anchor masonry to concrete per anchor manufacturer's instructions utilizing stainless steel dovetail anchor and slots, spaced at 24 inch max.
5. See "WINGWALL B LAYOUT" sheet for stone masonry details.
6. Adjust elevation of weepholes to outlet 1-ft above finish grade.
7. See BRIDGE RAIL sheets and "APPROACH SLAB" sheet for additional information.



WINGWALL A ELEVATION



SECTION A-A



Key:
n.f. = near face
f.f. = far face
e.f. = each face

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 BLUE RIDGE PARKWAY

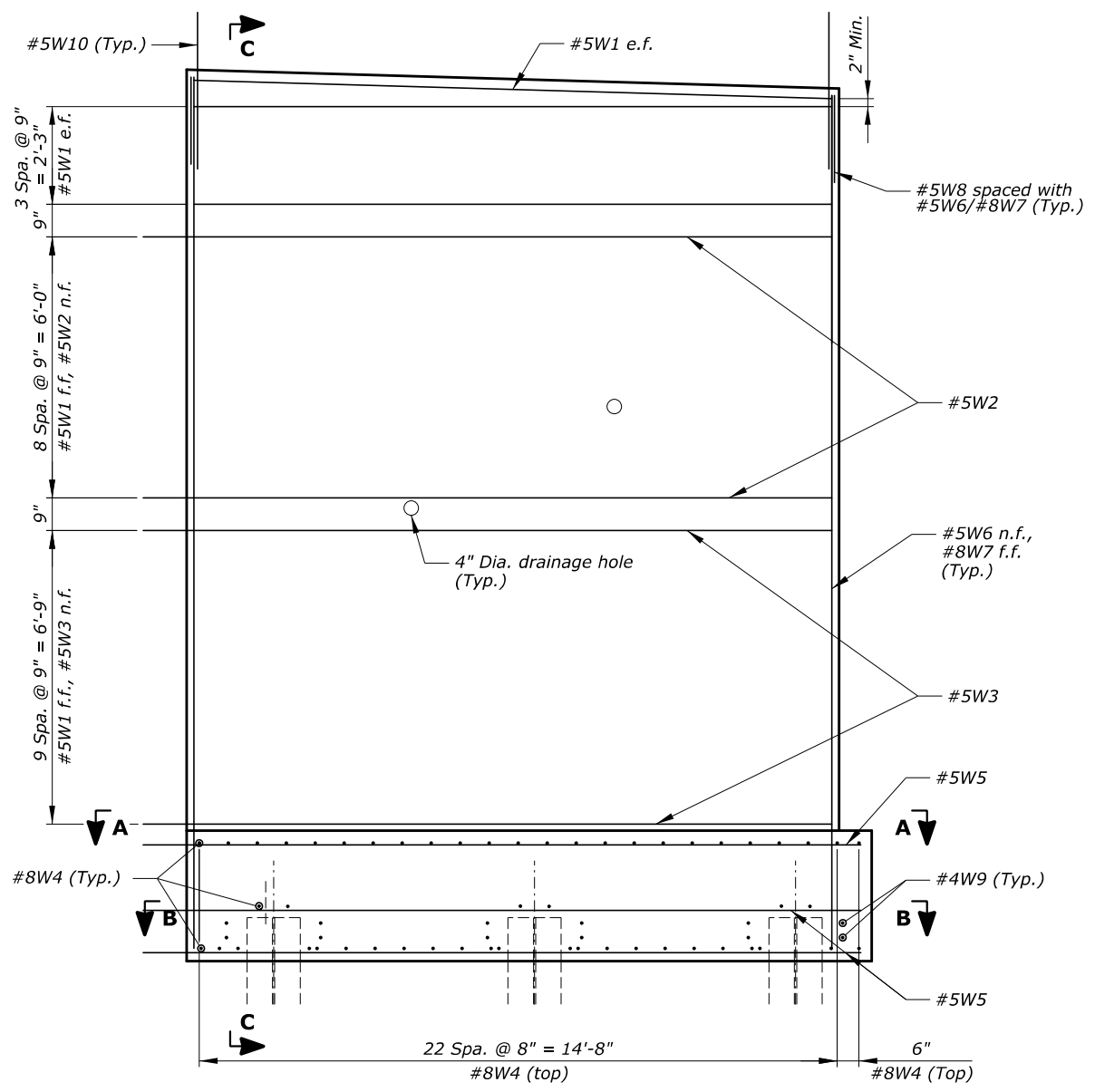
 BRIDGE OVER I-26

WINGWALL A LAYOUT

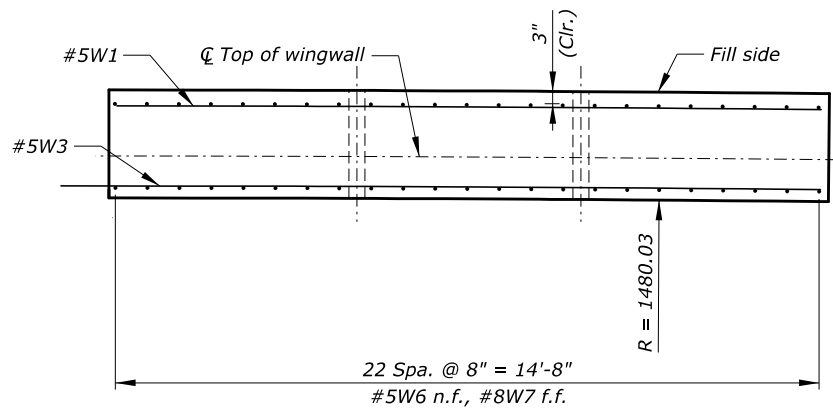
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ACTUAL FILE: R10_BLR1_I26_WINGWALL A REINFORCEMENT.dgn
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R10

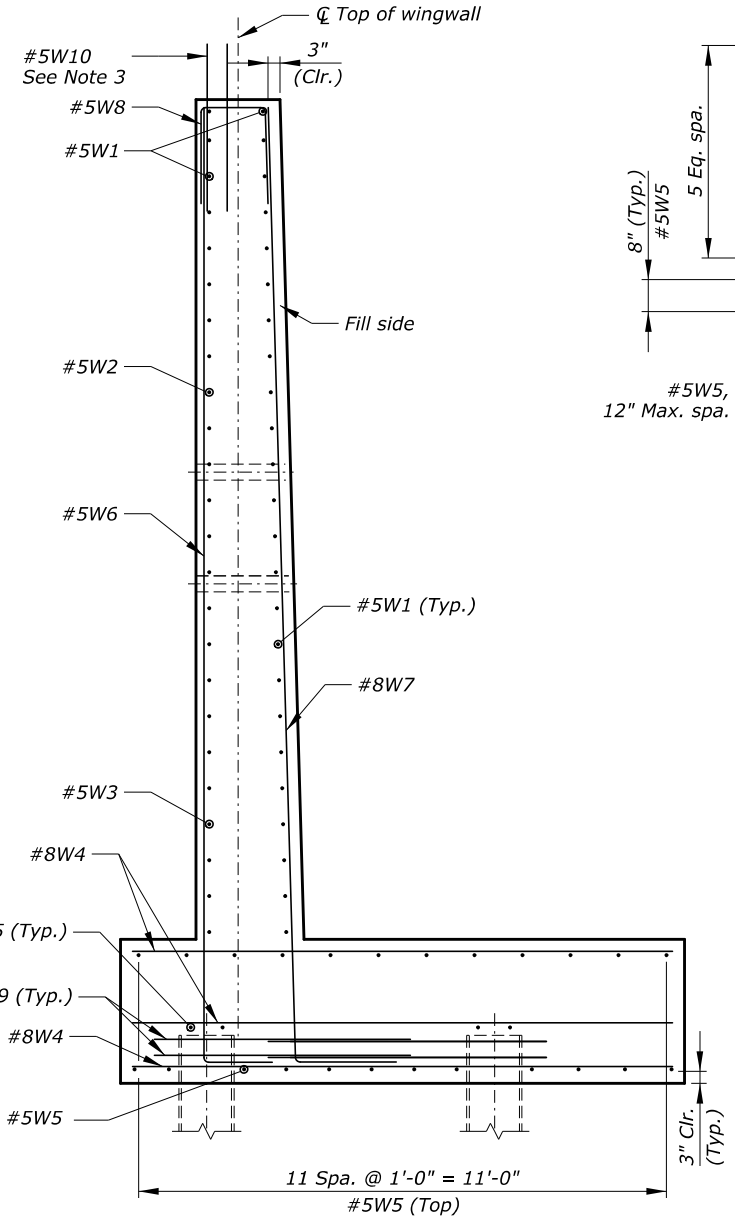


WINGWALL A ELEVATION VIEW

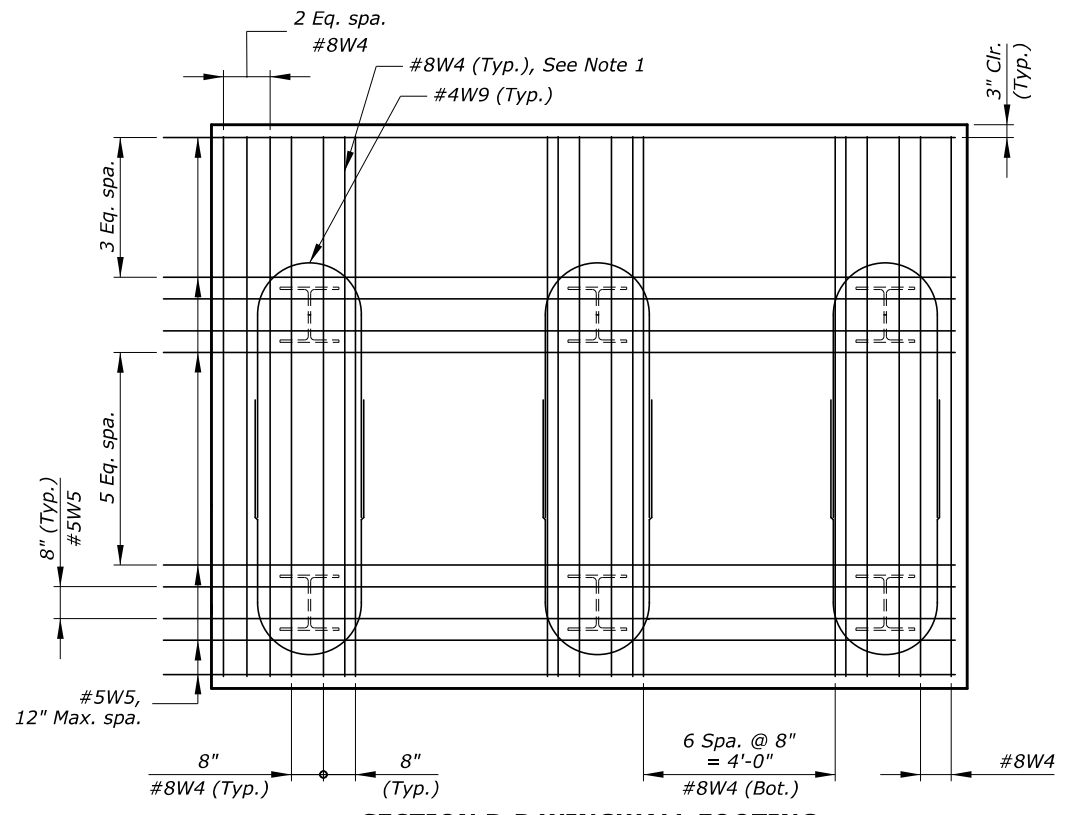


SECTION A-A

- Notes:
1. Space and cut all reinforcement to maintain 2-inch clearance around H-piles and adjacent reinforcement. Maximum spacing of 8 inches.
 2. See "WINGWALL A LAYOUT" sheet for additional information.
 3. See "WINGWALL C REINFORCEMENT" sheet for additional information.
 4. Provide a minimum of 2'-0" lap splices for #5 bars.



SECTION C-C



SECTION B-B WINGWALL FOOTING

Key:
 n.f. = near face
 f.f. = far face
 e.f. = each face

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 FEDERAL HIGHWAY ADMINISTRATION
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 BLUE RIDGE PARKWAY
 BRIDGE OVER I-26
WINGWALL A REINFORCEMENT

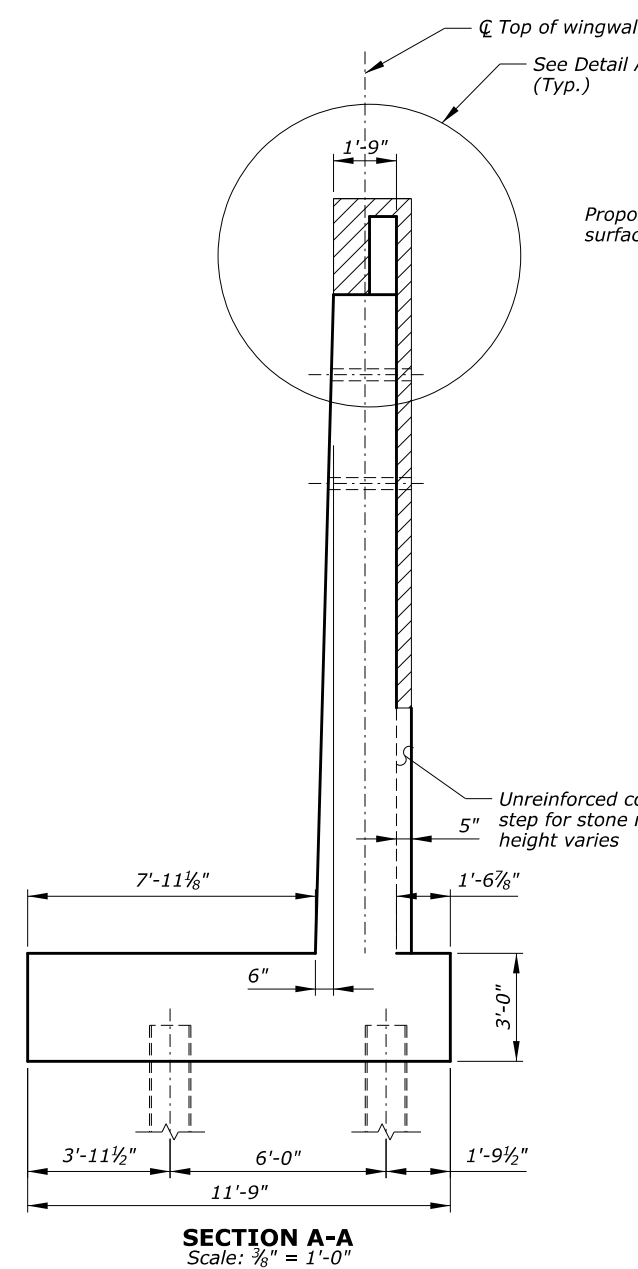
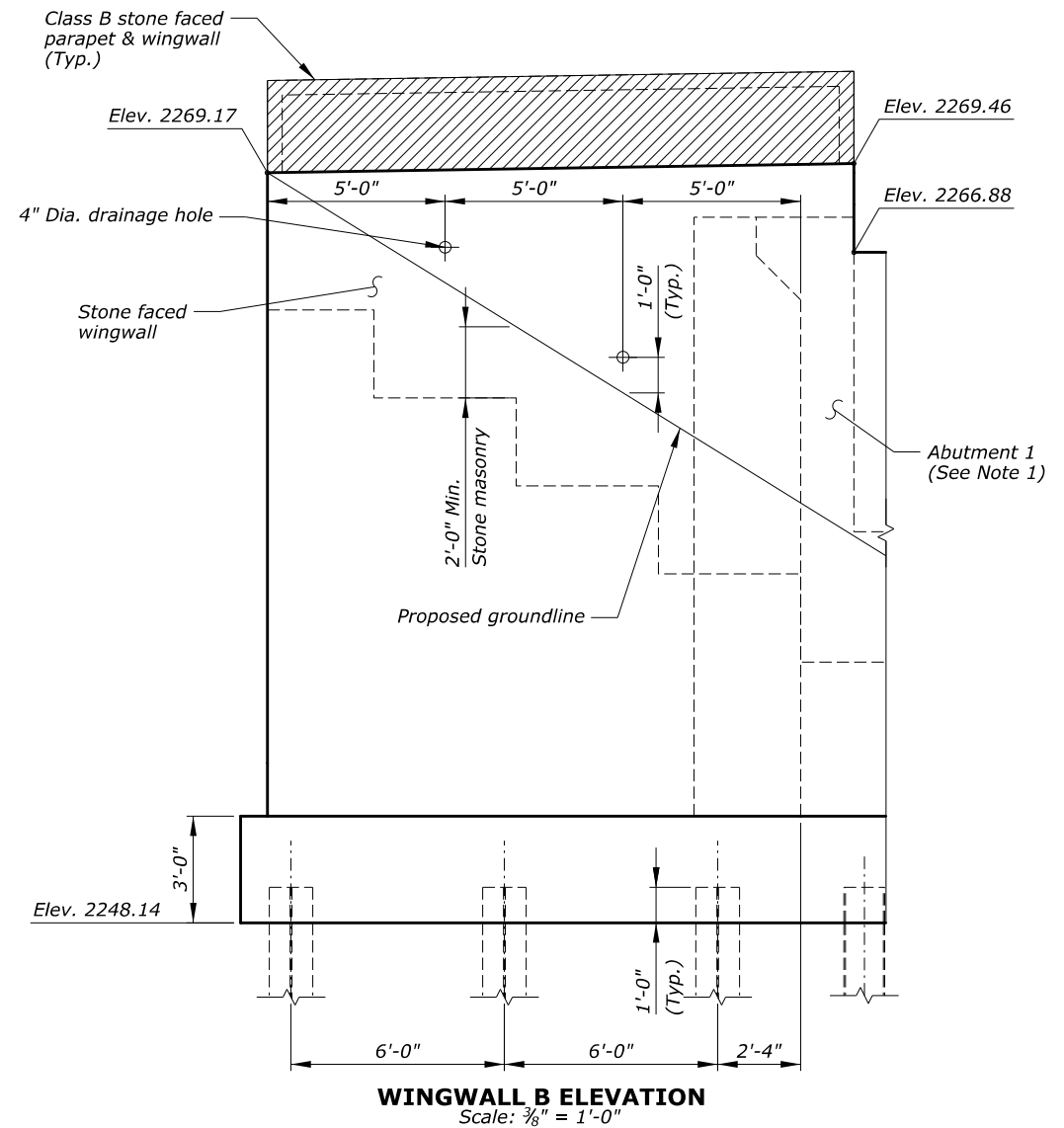
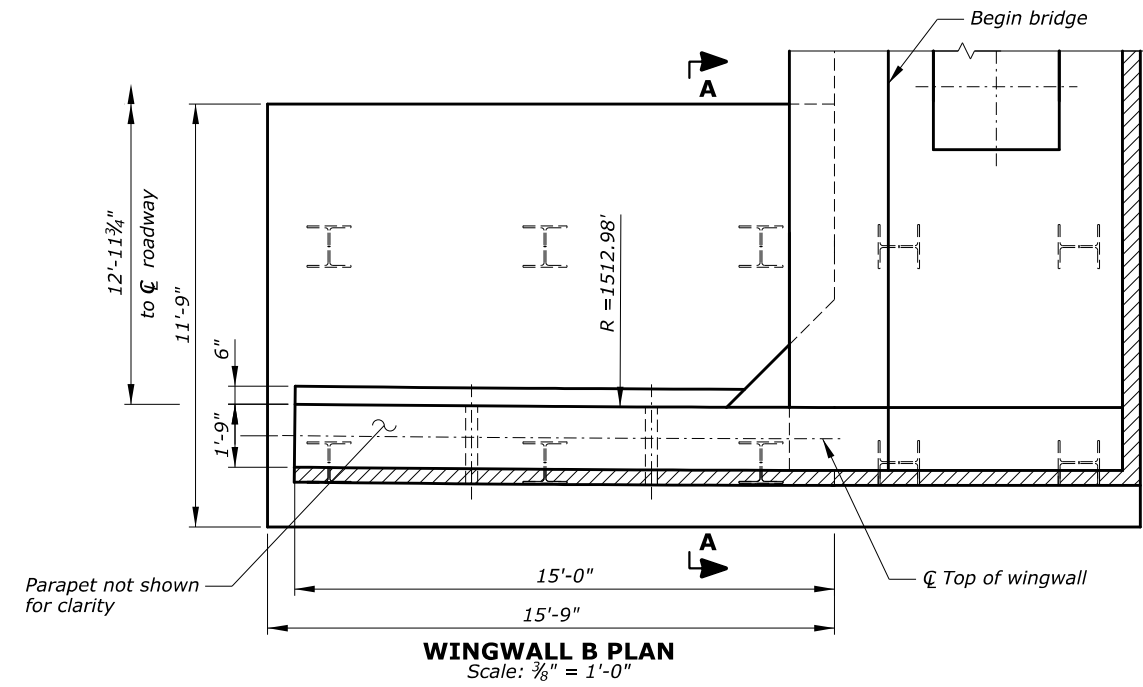
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ACTUAL FILE: R11_BLR1_126_WINGWALL LAYOUT B.dgn

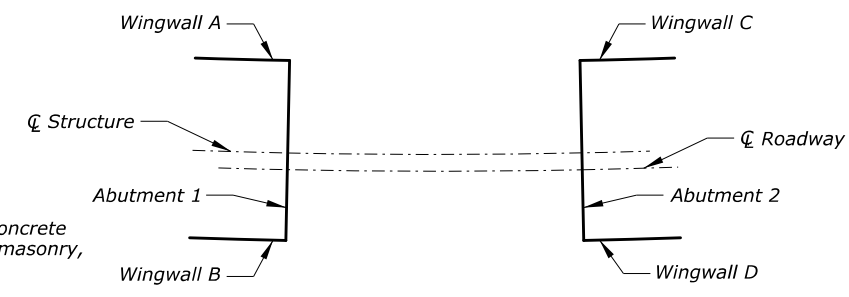
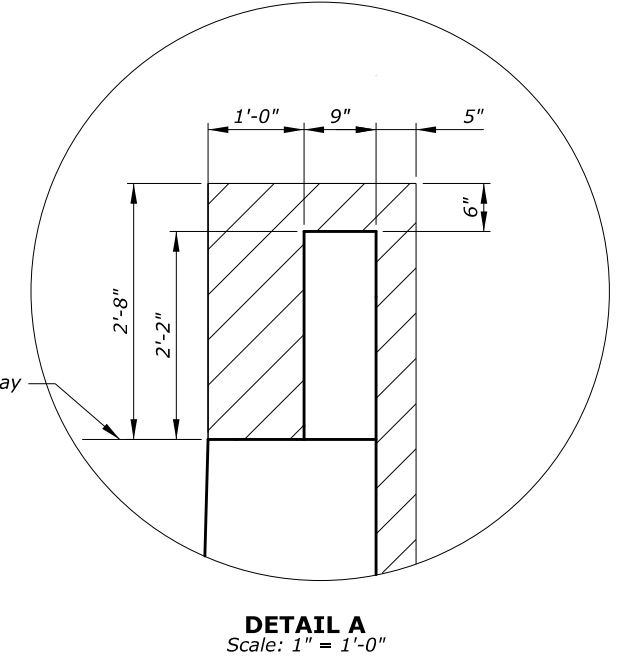
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R11



- Notes:
1. See "ABUTMENT 1 REINFORCEMENT" and "ABUTMENT 1 LAYOUT" sheets for additional information.
 2. See "FOUNDATION LAYOUT" sheet for additional information.
 3. Anchor masonry to concrete per anchor manufacturer's instructions utilizing stainless steel dovetail anchor and slots, spaced at 24 inch max.
 4. Adjust elevation of weepholes to outlet 1-ft above finish grade.
 5. See BRIDGE RAIL and "APPROACH SLAB" sheets for additional information.
 6. See "WINGWALL B REINFORCEMENT" sheet for additional information.



Key:
n.f. = near face
f.f. = far face
e.f. = each face

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BLUE RIDGE PARKWAY

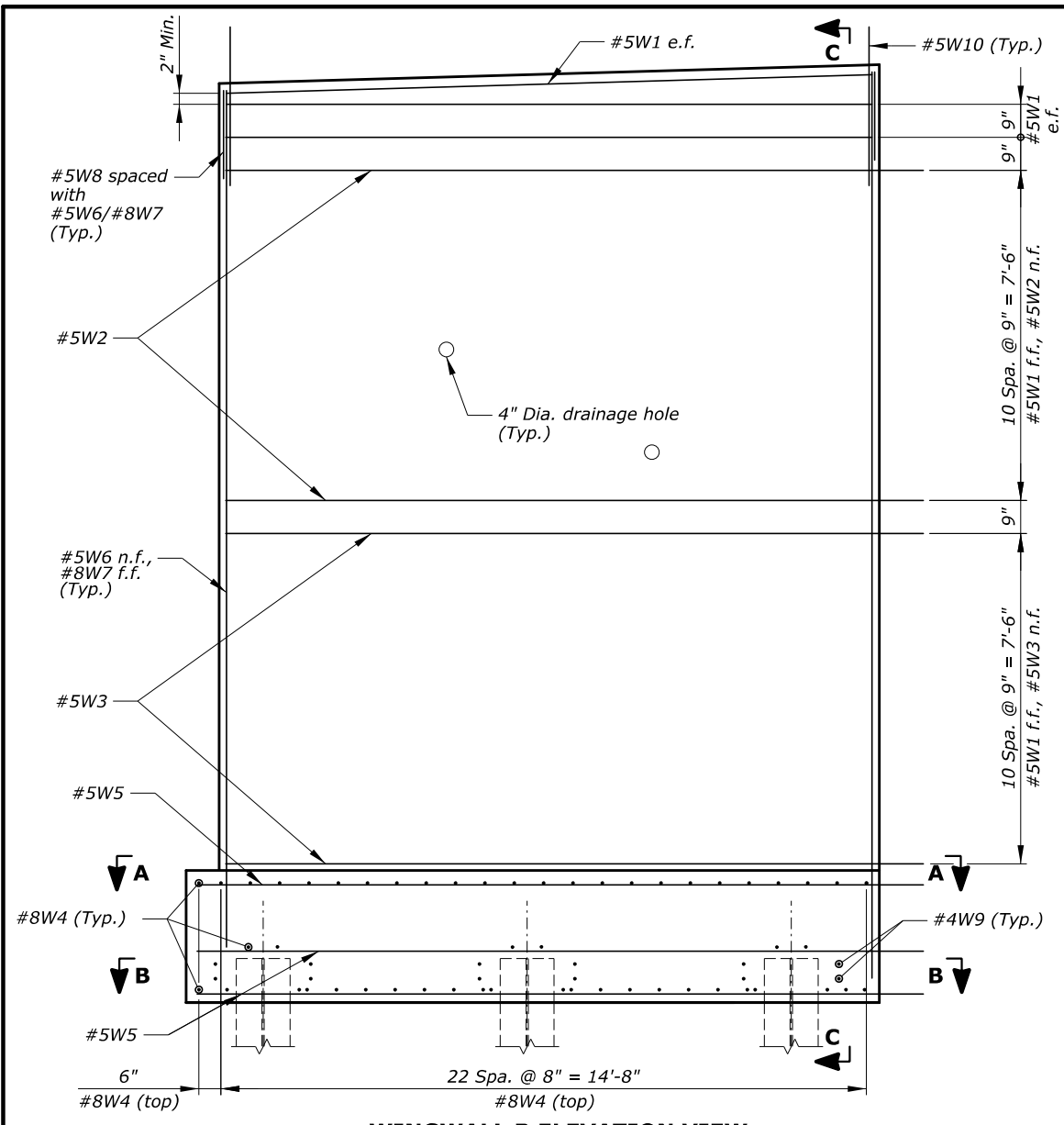
BRIDGE OVER I-26

WINGWALL B LAYOUT

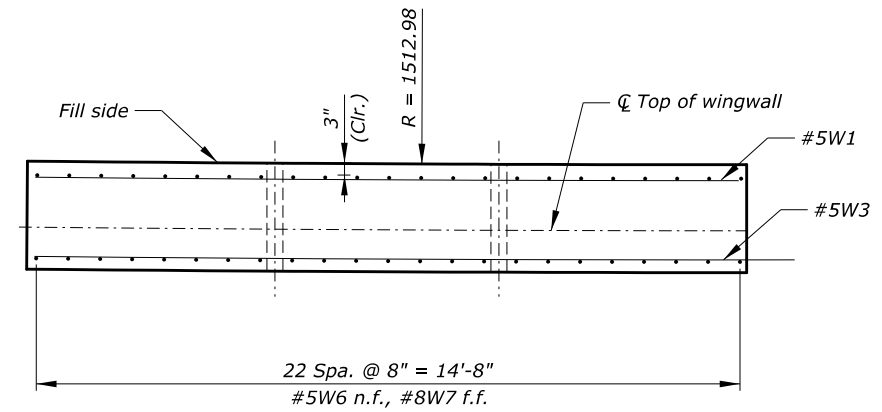
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								LE	LE	CWN	As Shown	George Choubah	11 of 228	December 2018	BRP-1265

ACTUAL FILE: R12_BLR1_I26_WINGWALL B REINFORCEMENT.dgn
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R12

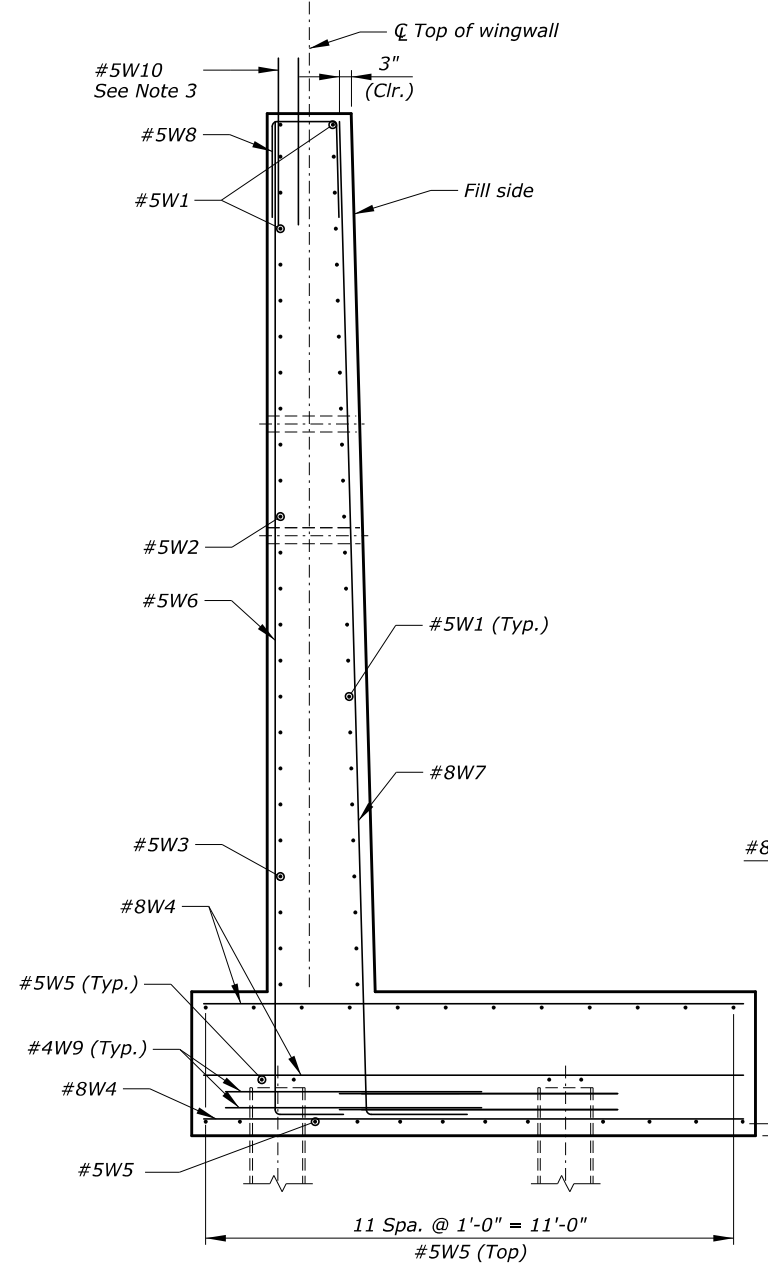


WINGWALL B ELEVATION VIEW

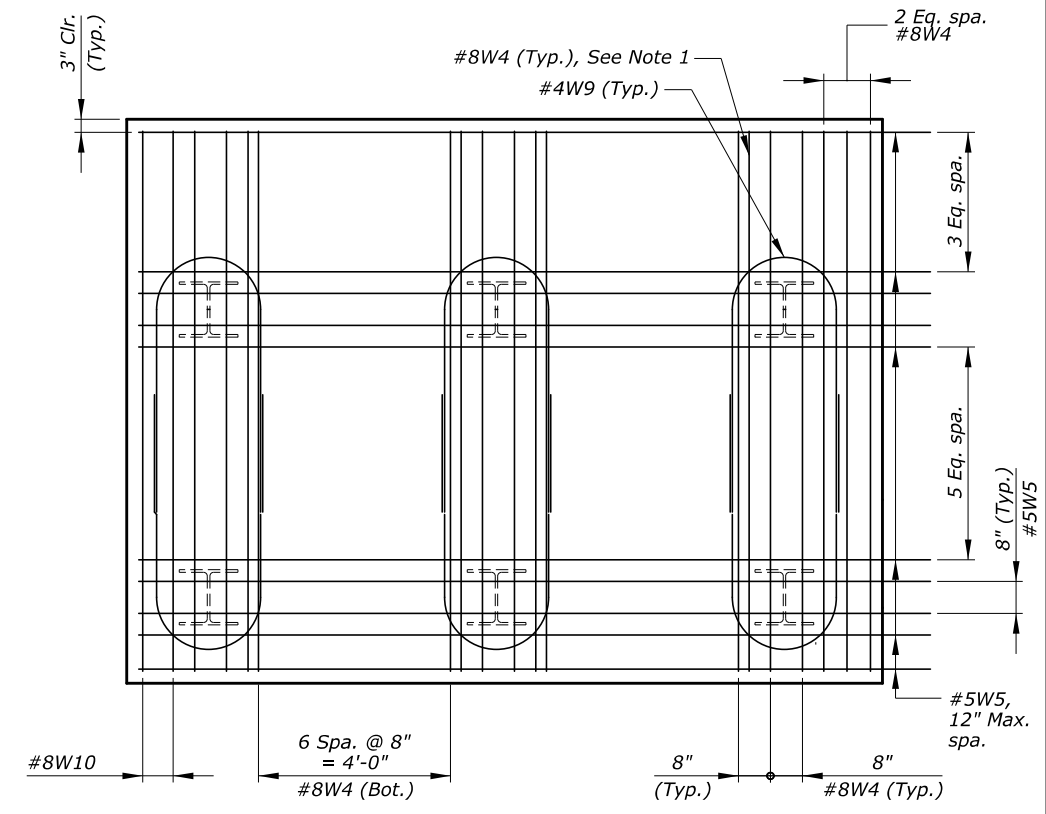


SECTION A-A

- Notes:
1. Space and cut all reinforcement to maintain 2-inch clearance around H-piles and adjacent reinforcement. Maximum spacing of 8 inches.
 2. See "WINGWALL B LAYOUT" sheet for additional information.
 3. See "WINGWALL C REINFORCEMENT" sheet for additional information.
 4. Provide a minimum of 2'-0" lap splices for #5 bars.



SECTION C-C



SECTION B-B WINGWALL FOOTING

Key:
 n.f. = near face
 f.f. = far face
 e.f. = each face

U.S. DEPARTMENT OF TRANSPORTATION
 FEDERAL HIGHWAY ADMINISTRATION
 EASTERN FEDERAL LANDS HIGHWAY DIVISION

 BLUE RIDGE PARKWAY

 BRIDGE OVER I-26

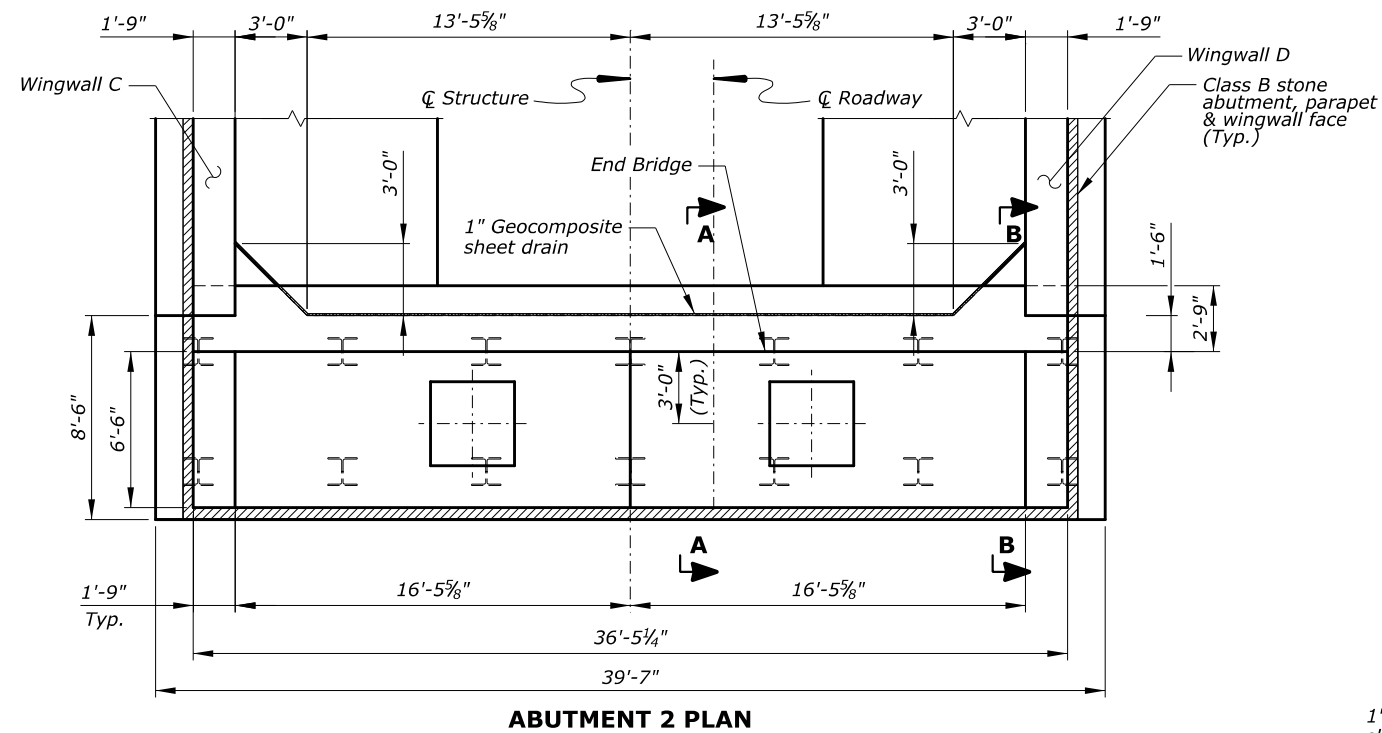
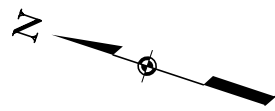
WINGWALL B REINFORCEMENT

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
								LE	LE	CWN	1/2" = 1'-0"	George Choubah	12 of 228	December 2018	BRP-1265

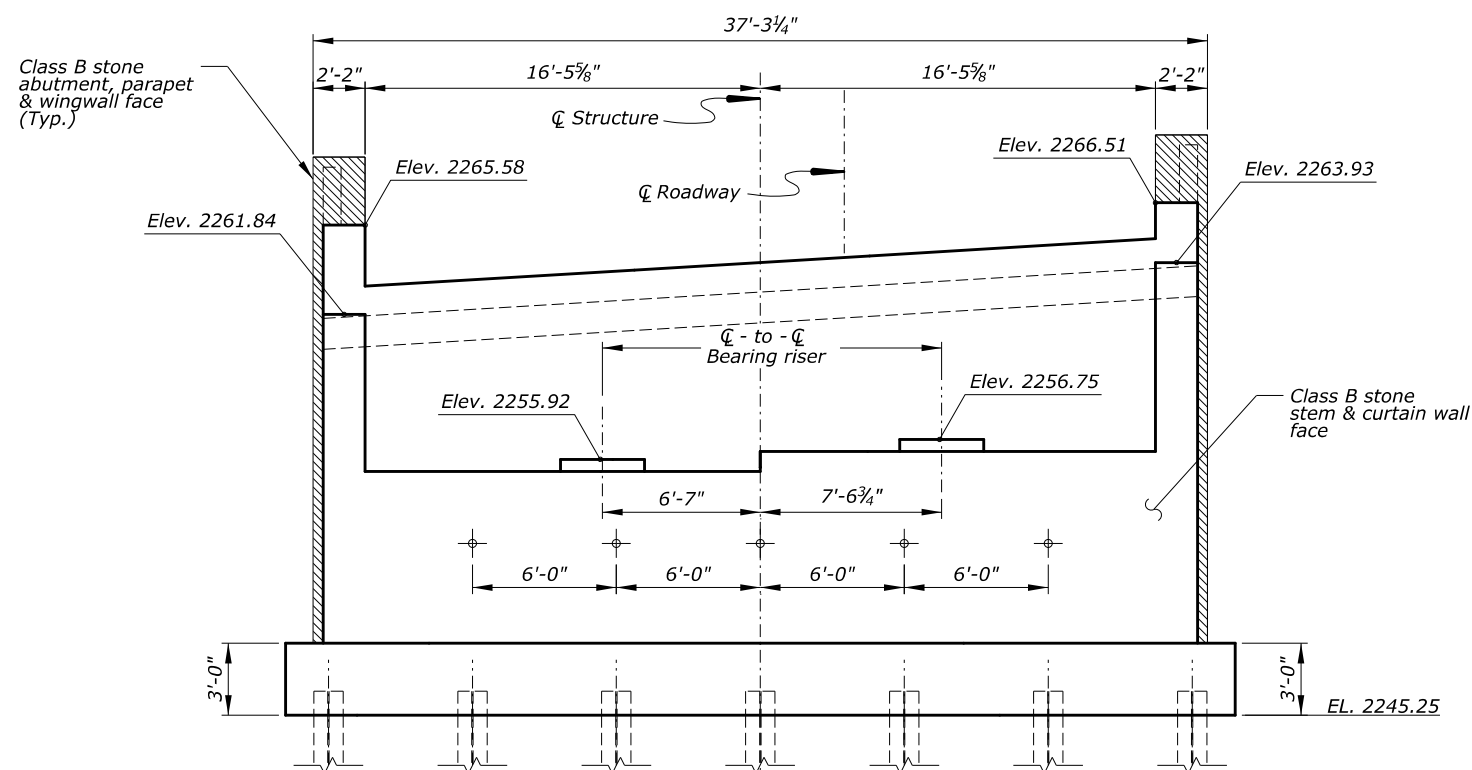
STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R13

Note:

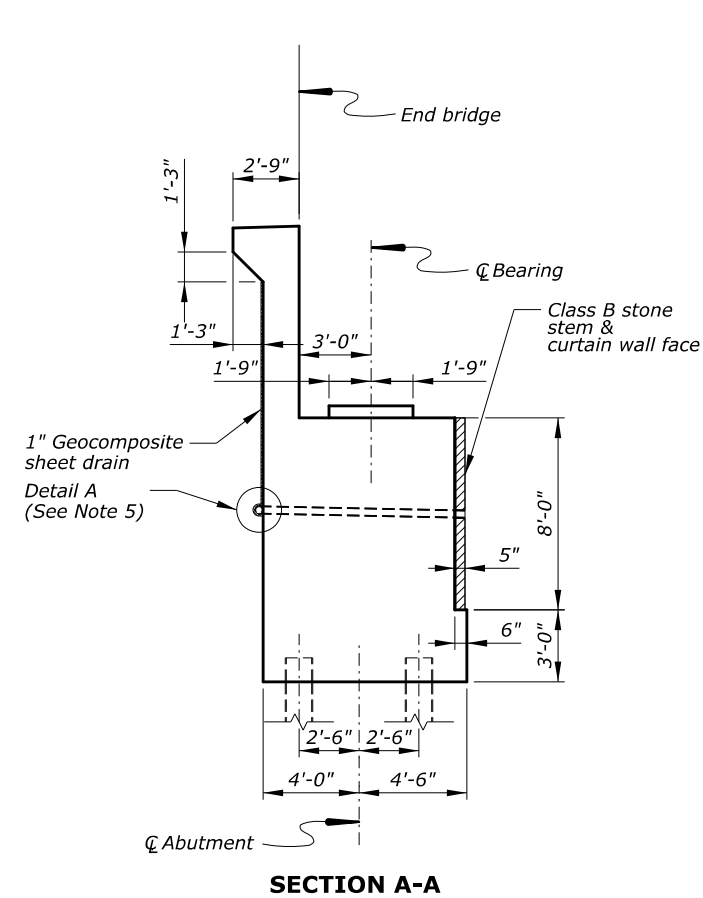
1. See "FOUNDATION LAYOUT" sheet for additional information.
2. Anchor masonry to concrete per anchor manufacturer's instructions utilizing stainless steel dovetail anchor and slots, spaced at 24 inch max.
3. Adjust elevation of weepholes to outlet 1'-0" above finish grade.
4. See BRIDGE RAIL and "APPROACH SLAB" sheets for additional information.
5. See "ABUTMENT 1 LAYOUT" sheet for additional information.



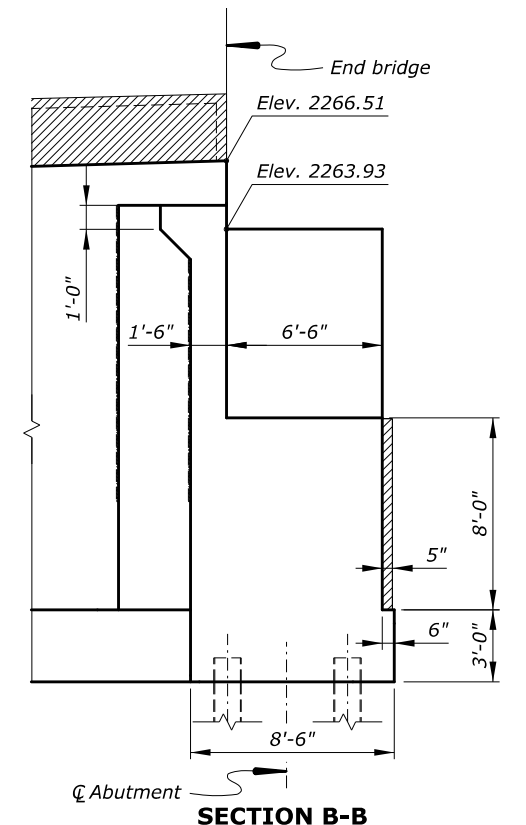
ABUTMENT 2 PLAN



ABUTMENT 2 ELEVATION



SECTION A-A



SECTION B-B

Key:
 n.f. = near face
 f.f. = far face
 e.f. = each face

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
								LE	LE	CWN	1/4" = 1'-0"	George Choubah	13 of 228	December 2018	BRP-1265

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 EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

ABUTMENT 2 LAYOUT

ACTUAL FILE: R13_BLR1_I26_ABUTMENT 2 LAYOUT.DGN

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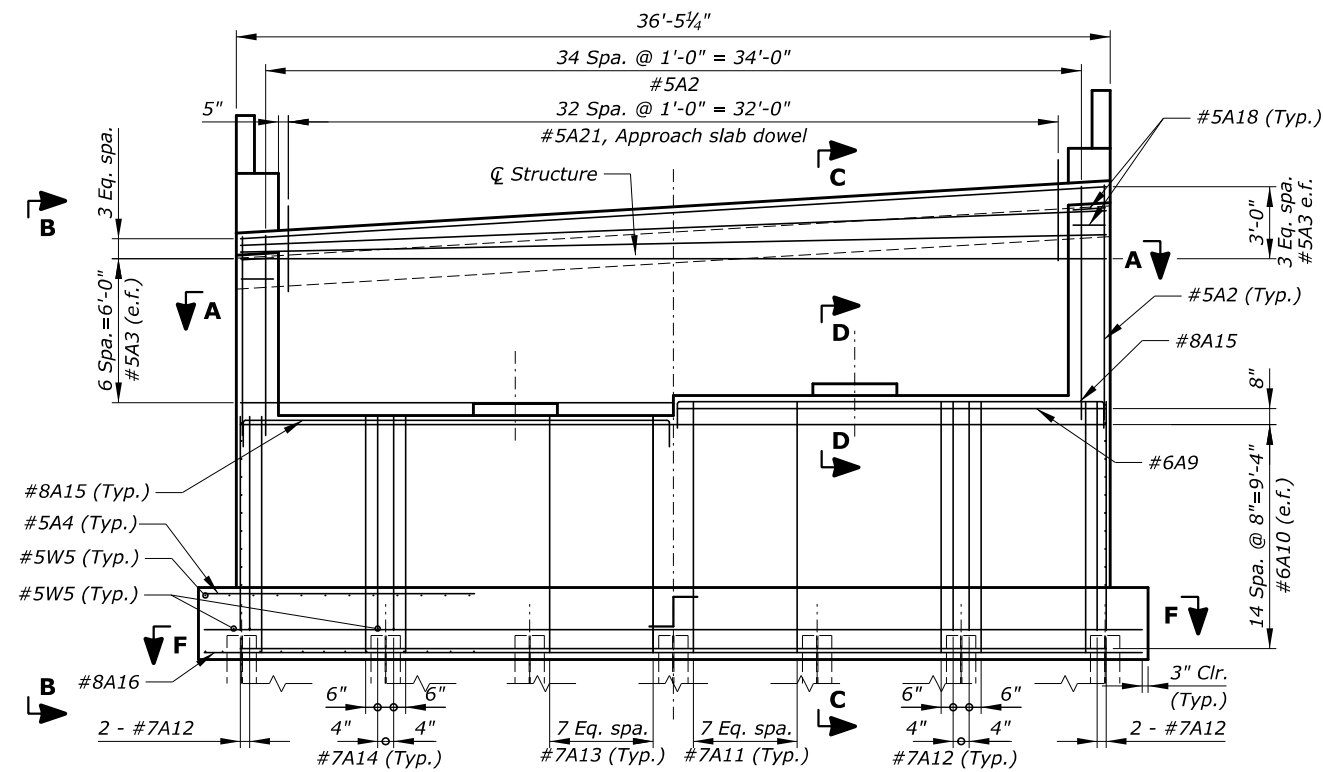
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STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R14

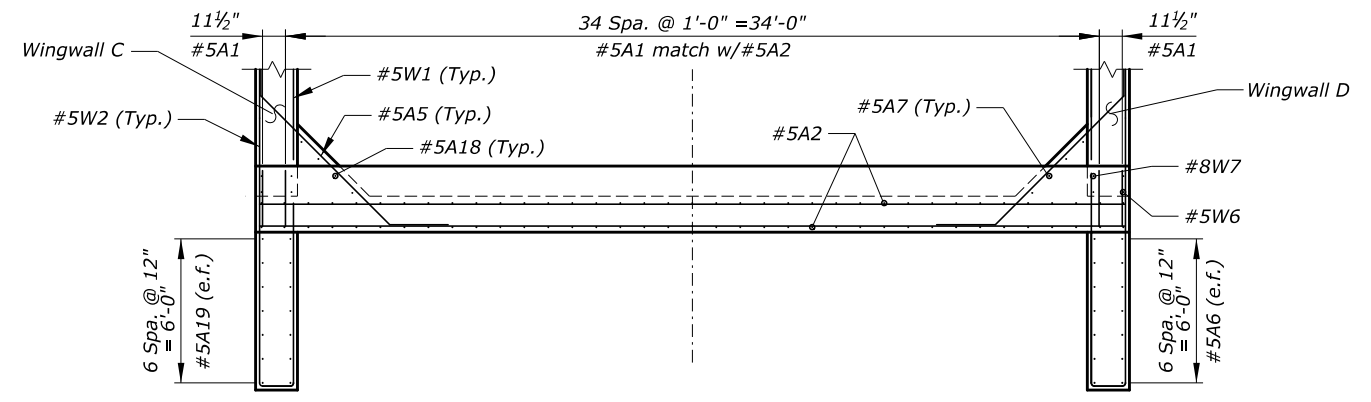
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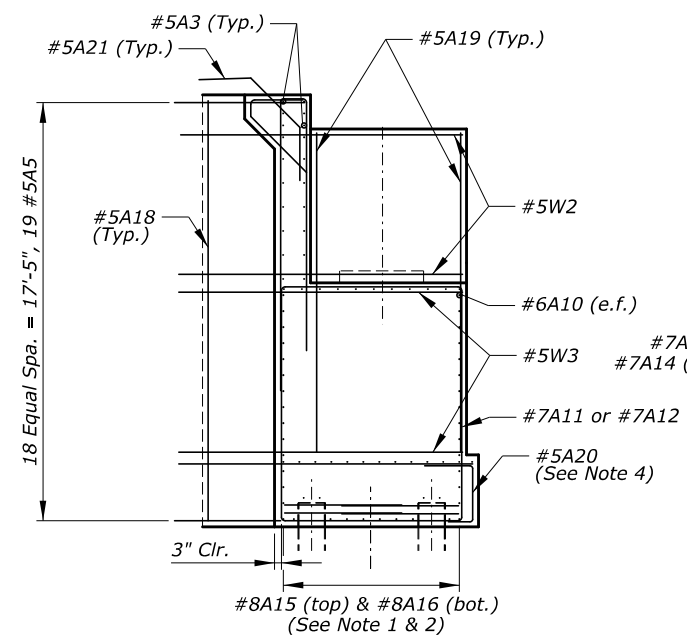
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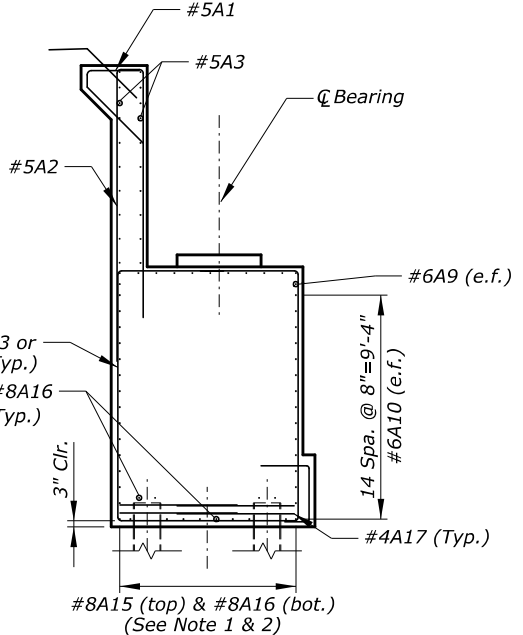
ABUTMENT 2 ELEVATION VIEW
Scale: 1/4" = 1'-0"



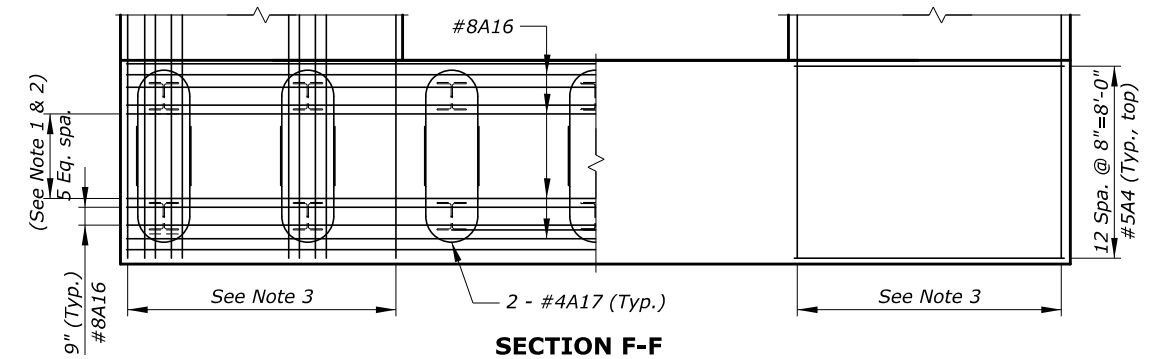
SECTION A-A
Scale: 1/4" = 1'-0"



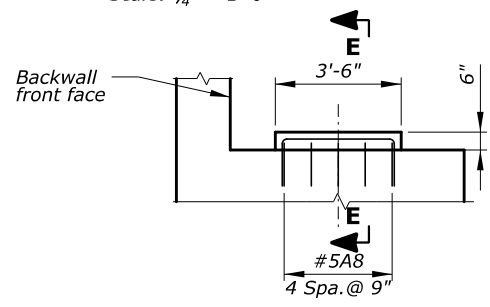
VIEW B-B
Scale: 1/4" = 1'-0"



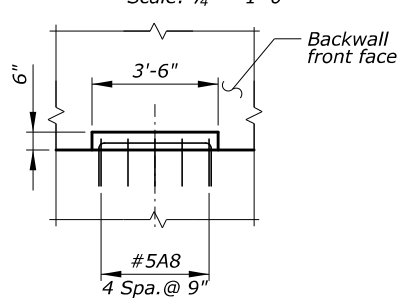
SECTION C-C
Scale: 1/4" = 1'-0"



SECTION F-F
Scale 1/4" = 1'-0"



SECTION D-D
Scale: 3/8" = 1'-0"



SECTION E-E
Scale: 3/8" = 1'-0"

Notes:

1. Provide 9 inch maximum spacing for top and bottom abutment stem reinforcement.
2. Space and cut all reinforcement to maintain 2 inch clearance around H-piles.
3. See "WINGWALL C REINFORCEMENT" and "WINGWALL D REINFORCEMENT" sheets for additional information.
4. See "ABUTMENT 1 REINFORCEMENT" sheet for additional information.
5. Provide a minimum embedment of 1'-6" for #5 reinforcement. Provide a minimum lap of 2'-0" for #5 reinforcement.

Key:

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- e.f. = each face

U.S. DEPARTMENT OF TRANSPORTATION
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EASTERN FEDERAL LANDS HIGHWAY DIVISION

BLUE RIDGE PARKWAY

BRIDGE OVER I-26

ABUTMENT 2 REINFORCEMENT

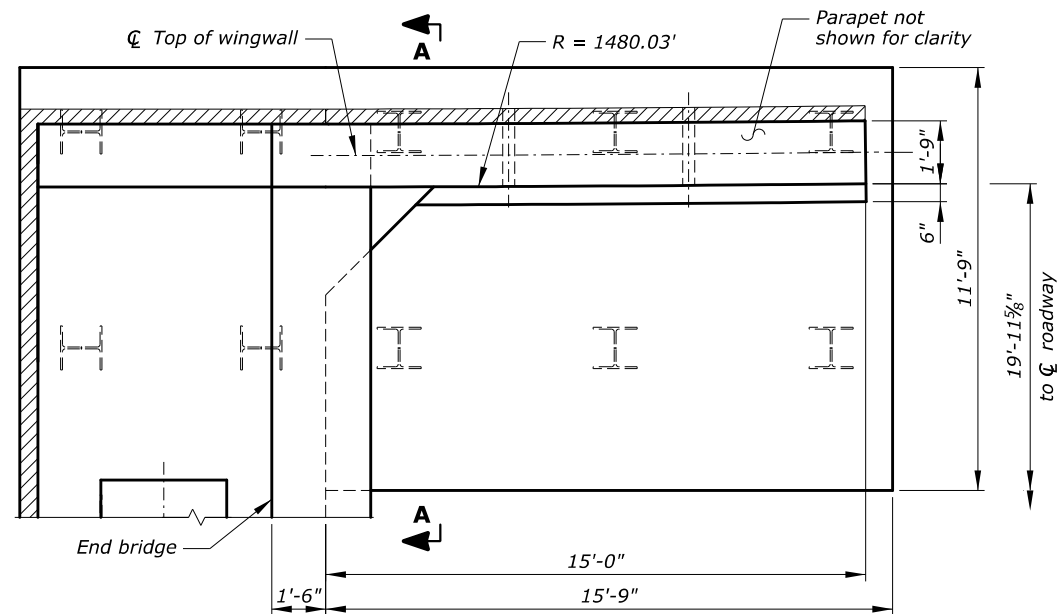
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								LE	LE	CWN	As Shown	George Choubah	14 of 228	December 2018	BRP-1265

ACTUAL FILE: R15_BLR1_126_WINGWALL LAYOUT C.dgn

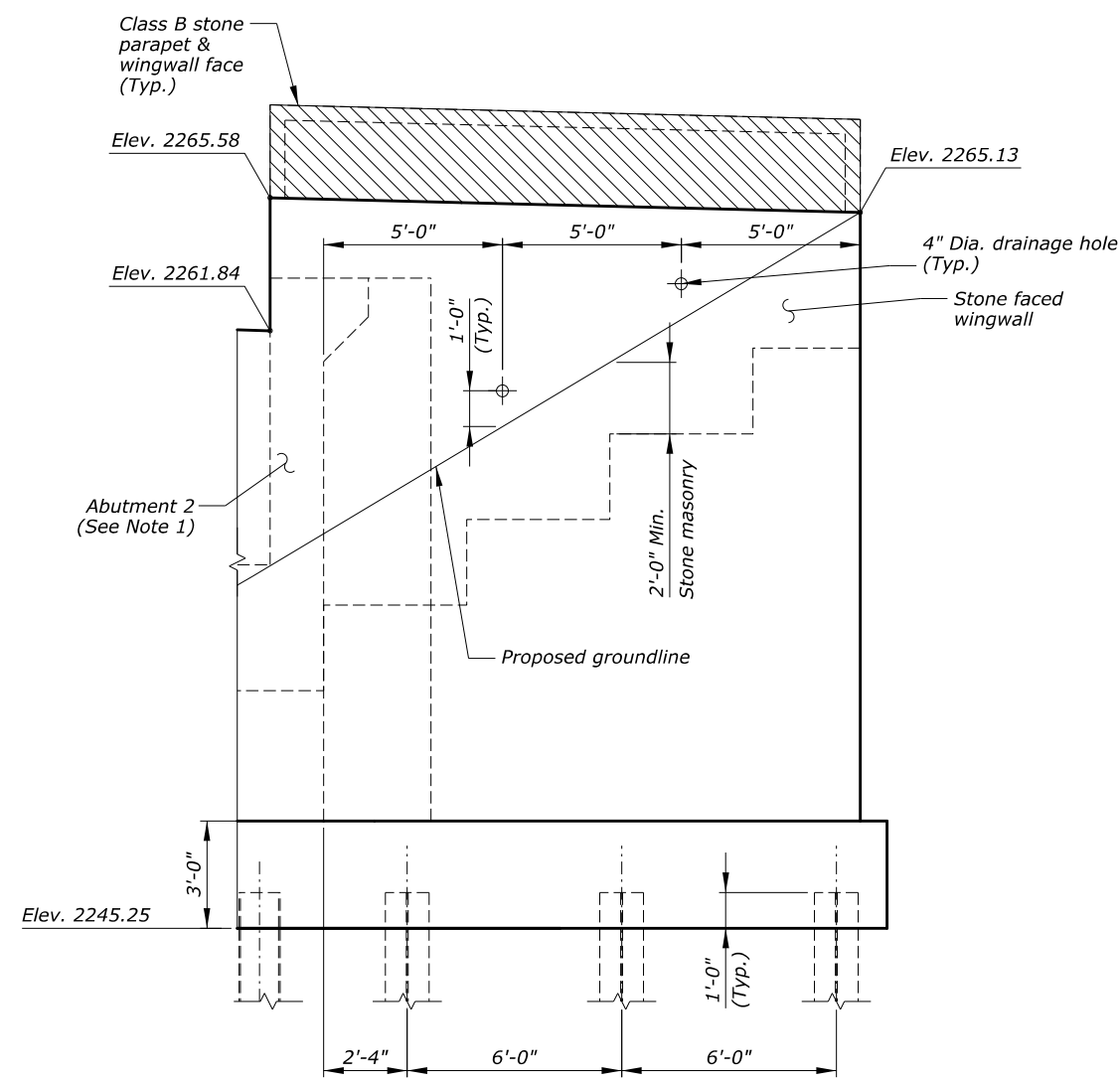
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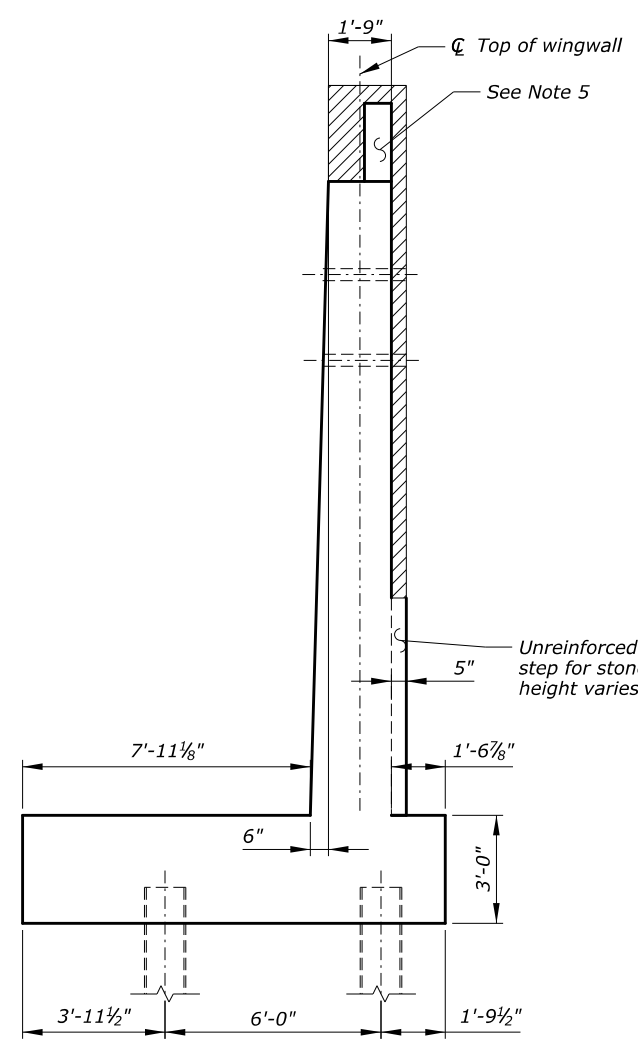
STATE	PROJECT	SHEET NUMBER
NC	BLRI OVER I-26	R15



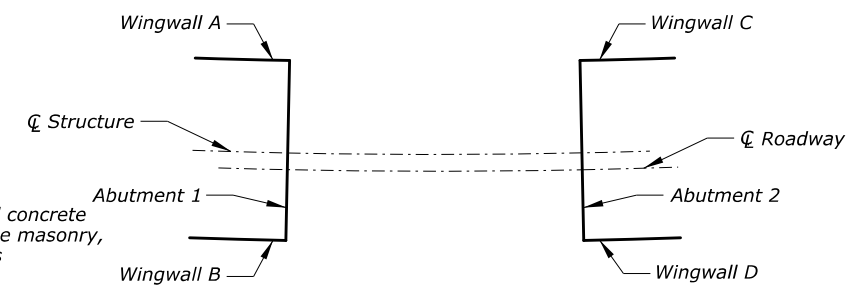
WINGWALL C PLAN



WINGWALL C ELEVATION



SECTION A-A



- Notes:
1. See "ABUTMENT 2 REINFORCEMENT" and "ABUTMENT 2 LAYOUT" sheets for additional information.
 2. See "WINGWALL C REINFORCEMENT" sheet for additional information.
 3. See "FOUNDATION LAYOUT" sheet for additional information.
 4. Anchor masonry to concrete per anchor manufacturer's instructions utilizing stainless steel dovetail anchor and slots, spaced at 24 inch max.
 5. See "WINGWALL B LAYOUT" sheet for stone masonry details.
 6. Adjust elevation of weepholes to outlet 1-ft above finish grade.
 7. See BRIDGE RAIL and "APPROACH SLAB" sheets for additional information.

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BLUE RIDGE PARKWAY

BRIDGE OVER I-26

WINGWALL C LAYOUT

Key:
 n.f. = near face
 f.f. = far face
 e.f. = each face

NO.	DATE	BY	REVISIONS	NO.	DATE	BY	REVISIONS	DESIGNED BY	DRAWN BY	CHECKED BY	SCALE	PROJECT TEAM LEADER	BRIDGE PLAN SHEET	DATE	BRP NO.
								LE	LE	CWN	3/8" = 1'-0"	George Choubah	15 of 228	December 2018	BRP-1265