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SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

# **ROADWAY** SUBSURFACE INVESTIGATION

COUNTY <u>BEAU</u>FORT

PROJECT DESCRIPTION BR. NO. 3 ON US 17 BUS. OVER NORFOLK SOUTHERN RAILROAD

INVENTORY

STATE PROJECT REFERENCE NO. B - 5302

### **CAUTION NOTICE**

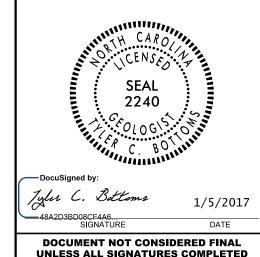
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (199) 707-6805. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORCHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE DBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DIES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS FOR BE NECOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- TES:
  THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
  OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS
  OR CONTRACT FOR THE PROJECT.
  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
  FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
  CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL
J.K. CRENSHAW
R.E. SMITH
J.M. EDMONSON
INVESTIGATED BY J.K. CRENSHAW
DRAWN BY _J.K. CRENSHAW
CHECKED BY
SUBMITTED BY D.N. ARGENBRIGHT
DATE <i>JULY</i> , 2016



PROJECT REFERENCE NO.	SHEET NO.
B-5302	2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUYIUM (ALLUY.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,  VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, <u>SUBANGULAR, SUBROUNDED</u> , OR <u>ROUNDED</u> .	WEATHERED // NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE CRYSTALLINE CRYSTALLINE CRYSTALLINE	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
LLASS. (≤ 35% PASSING "2001) (> 35% PASSING "2001)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE. <u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 B-2-6 A-2-7 A-4, A-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL COCCOGCOCCOCCOCCOCCOCCOCCOCCOCCOCCOCCOCC	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7. PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 50 MX GRANULAR SIL1- MUCK,	PERCENTAGE OF MATERIAL	CP) SHELL BEDS. ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
#40 30 MX 50 MX 51 MN PEAT SOILS PEAT SOILS SOILS PEAT	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 40 MX 41 MN	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITLE UR HIGHLY	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SOULS	GROUND WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	(SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	∇ PW     PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	PARENT MATERIAL.
AS SUBGRADE POOR POOR ONSOTTHOLE	SPRING OR SEEP	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
CONSISTENCY OR DENSENESS  RANGE OF STANDARD RANGE OF UNCONFINED	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE)  25/025  DIP & DIP DIRECTION  WITH SOIL DESCRIPTION  OF ROCK STRUCTURES	<u>IF TESTED, WOULD YIELD SPT REFUSAL</u>	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LODGE ( 4	SPT SINDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT  (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL  OPT ONT TEST BORING  INSTALLATION  SECTION OF THE PROPERTY OF THE	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.  IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.  MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THIN TOHOWAT EMBHACINETY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT         < 2         < 0.25           GENERALLY         SOFT         2 TO 4         0.25 TO 0.5	── INFERRED SOIL BOUNDARY ————————————————————————————————————	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.  RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL   STIFF   8 TO 15   1 TO 2   (COHESIVE)   VERY STIFF   15 TO 30   2 TO 4	THE ALLEMAN SOLUTION A PIEZOMETER COST N. MALLES	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTRUCTUTION	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.  SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIF	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
COARSE FINE	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILI CLAY		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
GRAIN MM 305 75 2.0 0.25 0.05 0.005	ABBRE VIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY  MOD MODERATELY  7 - UNIT WEIGHT  CPT - CONE PENETRATION TEST  NP - NON PLASTIC  7 - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION GOIDE FOR FILES HOLD TOKE	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.  STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO   SD SAND, SANDY   SS - SPLIT SPOON   F - FINE   SL SILT, SILTY   ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM COUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
LL _ LIOUID LIMIT	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
PLASTIC   SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	
(P) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK:
ON CONTINUE MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: FEET
OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:  CME-45C CLAY BITS AUTOMATIC MANUAL	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	R.E ROADWAY EMBANKMENT
ATTAIN UPTIMUM MUISTURE	CME-55   CORE SIZE:	THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY		INDURATION  FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
PLASTICITY INDEX (PI)  NON PLASTIC 0-5 VERY LOW	CME-550 HARD FACED FINGER BITS TUNGCARBIDE INSERTS	DIRRING WITH FINGED EDEER NUMEROUS CRAINS.	
SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST CASING WAY ADVANCED HAND TOOLS:	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	Post Hole Digger	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER,	
COLOR	TOUGHT AUGER	CRAING ARE DISCIPLE TO SERARATE WITH STEEL PROPE.	
		INDURATED DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT X VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
		SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

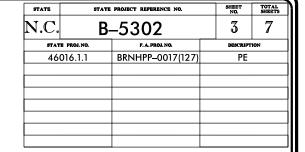
See Sheet 1-A For Index of Sheets VICINITY MAP ◆ ◆ ◆ ◆ OFFSITE DETOUR

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

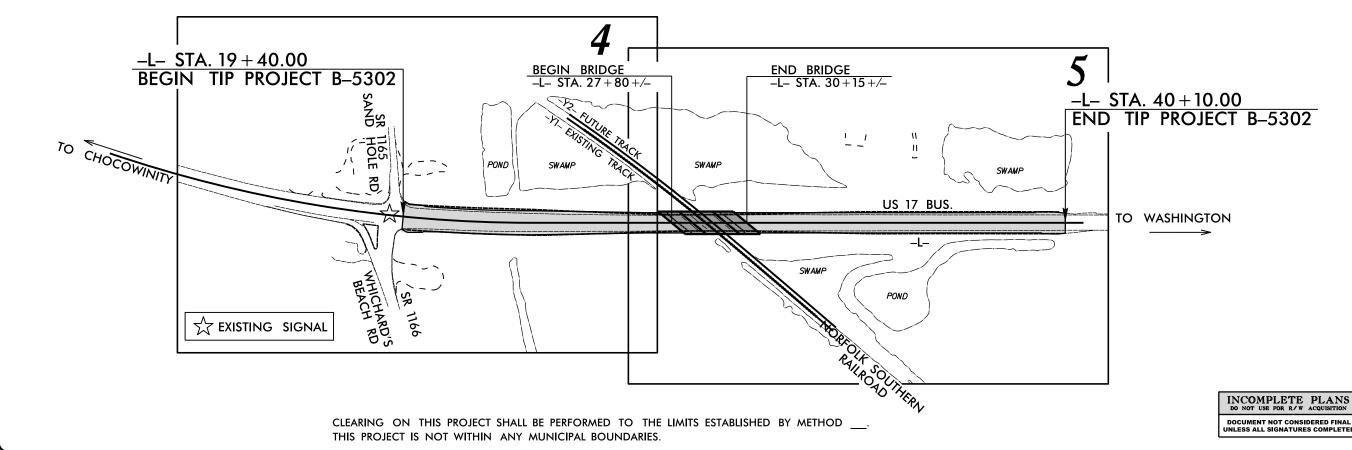
# BEAUFORT COUNTY

LOCATION: REPLACE BRIDGE NO. 3 OVER NORFOLK SOUTHERN RAILROAD ON US 17 BUS.

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE







M

IEC.

PR

**GRAPHIC SCALES** PROFILE (VERTICAL)

## **DESIGN DATA**

ADT 2019 = 14,909ADT 2039 = 20,273K = 9 %D = 60 %

T = 8 % \*V = 60 MPH\* TTST = 4% DUAL = 4% FUNC CLASS =

RURAL MINOR ARTERIAL

**REGIONAL TIER** 

### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5302 = 0.347 mile +/-LENGTH STRUCTURES TIP PROJECT B-5302 = 0.045 mile +/-TOTAL LENGTH TIP PROJECT B-5302 = 0.392 mile +/-

### Prepared For: **DIVISION OF HIGHWAYS** 1000 Birch Ridge Dr., Raleigh NC, 27610

TGS ENGINEERS 706 HILLSBOROUGH ST SUITE 200 RALEIGH, NC 27603 2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE:

MAY 19, 2017

LETTING DATE: MAY 21, 2019

V. MARCUS LOWERY, PE TRAVIS COOK, EI

GARY LOVERING, PE PROJECT ENGINEER NCDOT ROADWAY DESIGN

HYDRAULICS ENGINEER

ROADWAY DESIGN **ENGINEER** 

SIGNATURE:





PAT McCRORY

NICHOLAS J. TENNYSON
Secretary

July 12, 2016

STATE PROJECT: 46016.1.1 (B-5302) F.A. PROJECT: BRNHPP-0017(127)

COUNTY: Beaufort

DESCRIPTION: Bridge No. 3 on US 17 Bus. Over Norfolk Southern Railroad

SUBJECT: Geotechnical Inventory

### **Project Description**

This project is located in Beaufort County at the bridge on US 17 Business over Norfolk Southern Railroad. Proposed construction consists of widening the existing bridge approach. This geotechnical investigation was confined to the areas of proposed construction.

Fieldwork for this project was conducted during June 2016. Hand auger borings and vane shear tests were completed and representative soil samples were collected and submitted for testing.

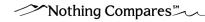
The following alignments were investigated. No plans, profiles or cross sections will be included in this report.

<u>Line</u> <u>Station(±)</u>
-L- 19+40 to 40+10

### **Areas of Special Geotechnical Interest**

- 1) The entire project was found to exhibit seasonal high ground water.
- 2) Organic sediments were encountered along the project corridor at the stations listed below.

<u>Line</u> <u>Station( $\pm$ )</u>
-L- 23+25 to 35+25



State of North Carolina | Department of Transportation | Geotechnical Engineering Unit 1020 Birch Ridge Drive | 1589 Mail Service Center | Raleigh, NC 27699-1589 919 707 6850 Sheet 4

### **Physiography and Geology**

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Natural ground elevations range from  $8\pm$  feet below sea level within the drainages adjacent to the bridge to  $30\pm$  feet above sea level along the existing US 17 Business embankment.

Surficial soils in this area are generally classified as alluvial.

### **Ground Water**

Ground water data was collected in June of 2016, during a time of normal precipitation. Ground water elevations ranged from 0± to 19± feet above sea level.

### Soils

Soils within this project area have been divided into two categories: roadway embankment and alluvial.

Roadway embankment soils were encountered along existing US 17 Bus. These soils are comprised of  $1\pm$  foot of medium dense sand (A-2-4) and  $1\pm$  or more feet of medium stiff silt and sandy clay (A-4, A-6). Moisture samples from within the cohesive units returned natural moisture contents from 17.1% to 25%.

Soils identified as alluvial are comprised of 0.5± or more feet of medium dense sand (A-3, A-2-4) and 5± to 11.5± feet of very soft to very stiff moderately organic sand and muck. Test results yielded organic percentages ranging from 9.7% to 33%. Moisture tests from the organic sediments resulted in moisture contents ranging from 36.6% to 282.3%. Vane Shear Tests indicate shear strengths between 125 and 2088 psf.

### Vane Shear Tests

Station	Offset	Depth	S (psf)
26+50	115' RT	3.5	167
26+50	115' RT	4.0	292
26+50	115' RT	4.5	459
26+50	115' RT	5.0	668
26+50	115' RT	5.5	793
26+50	115' RT	6.0	710
26+50	115' RT	6.5	710
26+50	115' RT	7.0	835
26+50	115' RT	7.5	1336
26+50	115' RT	8.0	919
26+50	115' RT	8.5	1044
26+50	115' RT	9.0	1002
26+50	115' RT	9.5	1253
26+50	115' RT	10.0	919
26+50	115' RT	10.5	1127
33+50	112' RT	3.0	152
33+50	112' RT	3.5	334
33+50	112' RT	4.0	919

Sheet 4A

Station	Offset	Depth	S (psf)
33+50	112' RT	4.5	417
33+50	112' RT	5.0	459
33+50	112' RT	5.5	731
33+50	112' RT	6.0	1253
33+50	112' RT	6.5	2088
33+50	112' RT	7.0	2088
33+50	112' RT	7.5	1879
33+50	112' RT	8.0	1545
33+50	112' RT	8.5	543
33+50	112' RT	9.0	1253

ROADWAY NOTES SHEET 5

LINE	PROJECT	<u>B-5302</u>	<b>DATE</b> 6/28/16 - 6/29/16	
-L-	COUNTY	BEAUFORT		
	NOTES BY	J. Crenshaw		
	NOTEO BI	o. Orchshaw		EST.
STATION	DEPTH	SAMP	DESCRIPTION	CLASS
26+00	0.0-4.0	S-5	MED. STIFF ORANGE TAN SILT, MOIST TO WET R.E.	A-4
115' RT	4.0-11.0	S-6	SOFT DARK BROWN MUCK, SAT. ALLUVIAL	MUCK
	11.0-11.5		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
1.2 25+00	0.0-1.0		MED. DENSE BROWN SAND, MOIST R.E.	A-2-4
41' RT	1.0-6.0		MED. STIFF TAN SILT, MOIST TO WET, R.E.	A-2-4 A-4
41 101	1.0 0.0		MEB. CTILT TAR GILT, MOIOT TO WET, THE	71.4
PERCHED H20	)			
5.5				
24+50	0.0-5.5		MED. STIFF ORANGE TAN SILT, MOIST R.E.	A-4
95' RT	5.5-12.0		SOFT DARK BROWN MUCK, SAT. ALLUVIAL	MUCK
24 HR H2O	12.0-12.5	+	MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-2-4
FIAD				
24+50	0.0-6.5		MED. STIFF ORANGE TAN SILT, MOIST R.E.	A-4
95' LT	6.5-7.5		MED. DENSE ORANGE BROWN SAND, SAT.R.E.	A-2-4
	7.5-15.0		SOFT DARK BROWN MUCK, SAT. ALLUVIAL	MUCK
24 HR H2O	15.0-15.5		MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-2-4
FIAD				
24+00	0.0-1.0	_	MED. STIFF ORANGE TAN SILT, MOIST R.E.	A-4
110' LT	1.0-7.0	S-4	SOFT DARK BROWN MOD. ORG. SILT, WET ALLUVIAL	A-4
24 HR H2O	7.0-7.5		MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-2-4
1.0				
23+50	0.0-1.0		MED. STIFF ORANGE TAN SILT, MOIST TO WET R.E.	A-4
105' LT	1.0-10.0		SOFT DARK BROWN MUCK, SAT. ALLUVIAL	MUCK
	10.0-10.5		MED. DENSE ORANGE BROWN SAND, ALLUVIAL	A-3
24 HR H2O				
FIAD				
23+50	0.0-1.0		MED. STIFF ORANGE TAN SILT, MOIST R.E.	A-4
100'RT	1.0-6.5 6.5-7.0	+	SOFT DARK BROWN MUCK, WET ALLUVIAL MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	MUCK A-3
24 HR H2O	0.5-7.0		WILD. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
1.2				
23+00	0.0-6.0		MED. DENSE ORANGE BROWN SAND, MOIST TO SAT. ALLUVIAL	A-3
100' RT				
24 HR H2O				
FIAD	0005	0.0	MED DENICE TAN EDOMAN CAND MOJETTO CAT DE	A 0 4
22+50 95' RT	0.0-2.5 2.5-4.0	S-2 S-3	MED. DENSE TAN BROWN SAND, MOIST TO SAT. R.E. MED. STIFF ORANGE BROWN SANDY CLAY, WET R.E.	A-2-4 A-6
33 KI	4.0-6.0	3-3	MED. STIFF GRANGE BROWN SANDT CLAT, WET R.E.  MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-6 A-3
24 HR H2O	5			
0.9				
20+00	0.0-5.5	S-1	MED. STIFF BROWN GRAY SILT, MOIST TO WET R.E.	A-4
43' RT	5.5-6.0		MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-3
041151100				
24 HR H2O				
4.9				

LINE	PROJECT	B-5302	DATE 6/28/16 - 6/29/16	
-L-	COUNTY	BEAUFORT		
	NOTES BY	J. Crenshaw		ГОТ
STATION	DEPTH	SAMP	DESCRIPTION	EST. CLASS
35+00	0.0-3.5	SAME	MED. STIFF ORANGE TAN SILT, MOIST TO WET R.E.	A-4
92'RT	3.5-6.0		LOOSE DARK BROWN MOD. ORGANIC SAND, SAT. ALLUVIAL	A-2-5
	6.0-6.5		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
8.0				
34+50	0.0-6.0		MED. STIFF BROWN GRAY SILT, MOIST TO WET R.E.	A-4
40' RT				
PERCHED H2O				
5.6				
34+00	0.0-2.0		MED. STIFF ORANGE TAN SILT, MOIST R.E.	A-4
100' LT	2.0-7.0		LOOSE DARK BROWN MOD. ORGANIC SAND, SAT. ALLUVIAL	A-2-5
	7.0-7.5		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
2.1	0005		MED OTIFE TANIOUT DE	A 4
33+50 110' RT	0.0-2.5 2.5-9.0		MED. STIFF TAN SILT, MOIST R.E. V. SOFT TO V. STIFF DARK BROWN MUCK, SAT. ALLUVIAL	A-4 MUCK
TIUKI	9.0-9.5		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O	3.0-3.3		MES. SERVE SIVINGE BROWN SAME, SAME ALLES VIALE	Α-3
FIAD				
32+50	0.0-3.5		MED. STIFF ORANGE TAN SILT, MOIST TO WET R.E.	A-4
110' RT	3.5-10.0		LOOSE DARK BROWN MOD. ORGANIC SAND, SAT. ALLUVIAL	A-2-5
	10.0-10.5		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
0.5 32+00	0.0-7.5		MED. STIFF TAN SILT, MOIST TO WET R.E.	A-4
100' LT	7.5-10.5		LOOSE DARK BROWN MOD. ORGANIC SAND, SAT. ALLUVIAL	A-4 A-2-5
100 L1	10.5-11.0		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O			·	
FIAD				
32+00	0.0-4.5		MED. DENSE BROWN SAND, MOIST TO SAT. R.E.	A-2-4
90' RT	4.5-9.0		MED. STIFF TAN SILT, WET R.E.	A-4
24 HR H2O	9.0-17.5 17.5-18.0		LOOSE DARK BROWN MOD. ORGANIC SAND, SAT. ALLUVIAL MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-2-5
FIAD	17.5-16.0		WILD. DENGE CHANGE BROWN SAID, SAI. ALLOVIAL	A-3
30+50	0.0-7.5	S-7	LOOSE DARK BROWN MOD. ORGANIC SAND, MOIST TO SAT. ALLUVIAL	A-2-5
130' LT	7.5-8.0	1 .	MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
0.7				
27+00	0.0-11.5		SOFT DARK BROWN MUCK, MOIST TO SAT. ALLUVIAL	MUCK
120' LT	11.5-12.0		MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
0.8			<del> </del>	
26+50	0.0-3.5		MED. STIFF ORANGE TAN SILT, MOIST TO WET R.E.	A-4
113' RT	3.5-10.5		V. SOFT TO STIFF DARK BROWN MUCK, SAT. ALLUVIAL	MUCK
	10.5-11.0		MED. DENSE ORANGE BROWN SAND, SAT. ALLUVIAL	A-3
24 HR H2O				
FIAD				

ROADWAY NOTES SHEET 6

LINE -L-	PROJECT COUNTY NOTES BY	B-5302 BEAUFORT J. Crenshaw	<b>DATE</b> 6/28/16 - 6/29/16		LINE -L-	PROJECT COUNTY NOTES BY	B-5302 BEAUFORT J. Crenshaw	DATE 6/28/16 - 6/29/16	
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39+00	0.0-5.0		MED. STIFF TAN SILT, MOIST TO WET R.E.	A-4					
40' RT	5.0-6.0		MED. DENSE BROWN SAND, SAT. ALLUVIAL	A-3					
PERCHED H20	<u> </u>								<del>                                     </del>
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PROJECT REFERENCE NO. SHEET NO.

B-5302 7 OF 7

ROADWAY DESIGN HYDRAULGS ENGINEER

ENGINEER ENGINEER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

# $46016.1.1 \ (B-5302)$ $BR.\ NO.\ 3 \ ON \ US \ 17 \ BUS.\ OVER \ NORFOLK \ SOUTHERN \ RAILROAD$

	SOIL TEST RESULTS														
SAMPLE	OFFSET	STATION	DEPTH	AASHTO	1 1	P.I.		% BY	WEIGHT		% PAS	SSING (S	IEVES)	%	%
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	1.1.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
S- 1	43' RT	20+00	0.0-5.5	A- 4( 0)	23	6	4.8	5 <i>3</i> . 9	17.0	24. 2	100	99	50	17.1	-
S- 2	93' RT	22+50	0.0-2.5	A-2-4(0)	21	NP	<i>53. 3</i>	31.9	6.7	8. 1	85	61	15	-	-
S- 3	93' RT	22+50	2. 5- 4. 0	A-6(6)	37	20	<i>32. 3</i>	22. 2	9. 1	36.4	100	85	49	20.8	-
S- 4	110' RT	24+00	1. 0-7.0	A- 4( 0)	37	NP	31.7	<i>34</i> . 5	21.6	12. 1	97	81	36	16 1. 4	16.7
S- 5	115′ RT	26 +00	0.0-4.0	A- 4( 0)	20	2	9.9	<i>53. 7</i>	14. 1	22. 2	100	98	44	25.0	-
S- 6	115' RT	26 +00	4.0-11.0	A- 2- 5( 0)	57	NP	34.7	<i>35. 4</i>	15.8	14. 1	95	73	33	282. 3	<i>33. 0</i>
S- 7	130' RT	30 +50	0.0-7.5	A-2-5(0)	45	NP	63.4	18.6	9.9	8.1	98	57	20	36.6	9.7

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