

REFERENCE: W-5602

PROJECT: 50139

SEE SHEET 3 FOR PLAN SHEET LAYOUT  
AT TIME OF INVESTIGATION

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<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>PROFILE</u>
-L-	27+11 TO 168+78	5-15	17-27

**CROSS SECTIONS**

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	48+50 TO 50+00	28-29
-L-	85+50 TO 86+50	30-31
-L-	91+00 TO 94+50	32-35

**STATE OF NORTH CAROLINA**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

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**ROADWAY**  
**SUBSURFACE INVESTIGATION**

COUNTY ONSLOW  
PROJECT DESCRIPTION NC 172 (SNEADS FERRY RD.)  
FROM NC 210 TO CAMP LEJEUNE GATE

**INVENTORY**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	W-5602	1	35

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
- THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  - BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

J.K. CRENSHAW

CATLIN, INC.

INVESTIGATED BY T.C. BOTTOMS

DRAWN BY T.C. BOTTOMS

CHECKED BY D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

DATE APRIL 2016

DS  
DNA



DocuSigned by:  
Tyler C. Bottoms 4/18/2016  
48A2D3BD08CF4A6  
SIGNATURE DATE

**DOCUMENT NOT CONSIDERED FINAL  
UNLESS ALL SIGNATURES COMPLETED**

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT  
SUBSURFACE INVESTIGATION  
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (ASTM T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:  WEATHERED ROCK (WR)  CRYSTALLINE ROCK (CR)  NON-CRYSTALLINE ROCK (NCR)  COASTAL PLAIN SEDIMENTARY ROCK (CP)										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.									
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>										<b>ANGULARITY OF GRAINS</b>										<b>WEATHERING</b>																			
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS										THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.  FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.																			
<b>MINERALOGICAL COMPOSITION</b>										<b>COMPRESSION</b>										<b>PERCENTAGE OF MATERIAL</b>										<b>GROUND WATER</b>									
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.										SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50										ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE										WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP									
<b>CONSISTENCY OR DENSENESS</b>										<b>MISCELLANEOUS SYMBOLS</b>										<b>ROCK HARDNESS</b>										<b>RECOMMENDATION SYMBOLS</b>									
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> )										ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY										DIP & DIP DIRECTION OF ROCK STRUCTURES SPT DMT VST PMT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD TEST BORING WITH CORE SPT N-VALUE										UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL									
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053										AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - COY PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI. - HIGHLY										MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY										VST - VANE SHEAR TEST WEA. - WEATHERED % - UNIT WEIGHT %g - DRY UNIT WEIGHT  SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO									
<b>TEXTURE OR GRAIN SIZE</b>										<b>ABBREVIATIONS</b>										<b>FRACTURE SPACING</b>										<b>BEDDING</b>									
BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CS, SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.)										DRILL UNITS: CME-45C CME-55 CME-550 VANE SHEAR TEST PORTABLE HOIST										TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FOOT VERY CLOSE LESS THAN 0.16 FEET										TERM THICKNESS VERY THICKLY BEDDED 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET									
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>										<b>EQUIPMENT USED ON SUBJECT PROJECT</b>										<b>INDURATION</b>										<b>SOIL MOISTURE SCALE (ATTERBERG LIMITS)</b>									
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION										ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING w/ ADVANCER TRICONE * STEEL TEETH TRICONE * TUNG-CARB. CORE BIT										HAMMER TYPE: AUTOMATIC MANUAL  CORE SIZE: -B -H -N  HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST										- SATURATED - (SAT.) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE									
<b>PLASTICITY</b>										<b>RECOMMENDATION SYMBOLS</b>										<b>ROCK HARDNESS</b>										<b>RECOMMENDATION SYMBOLS</b>									
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH										UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL										VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.										BENCH MARK:  ELEVATION: FEET  NOTES:									
<b>COLOR</b>										<b>RECOMMENDATION SYMBOLS</b>										<b>ROCK HARDNESS</b>										<b>RECOMMENDATION SYMBOLS</b>									
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.										UNDERCUT SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL										VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.										BENCH MARK:  ELEVATION: FEET  NOTES:									

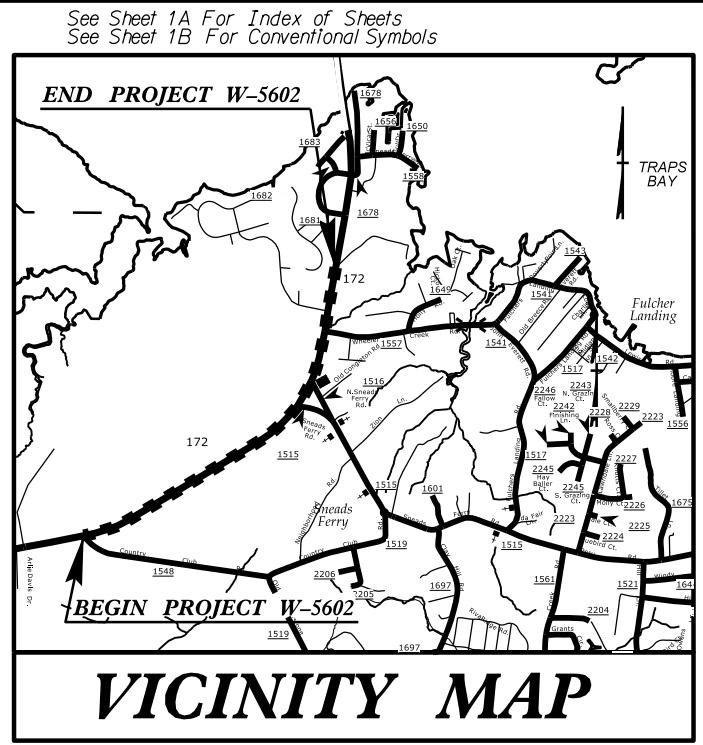
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	W-5602	3	35
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
50139.1.FR1	HSIP-0172 (13)	PE	
50139.1.FR1	HSIP-0172 (13)	R /W	

STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

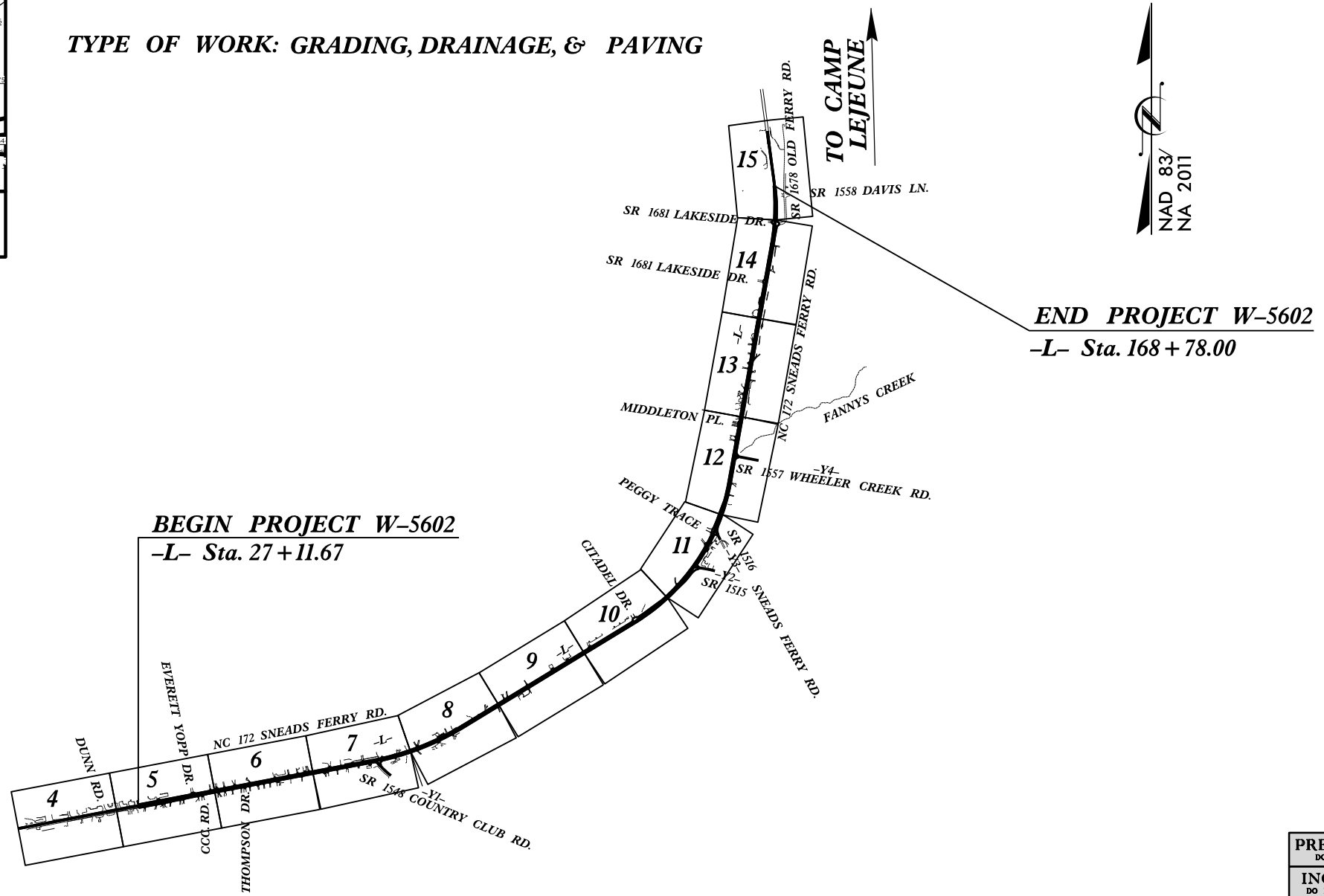
# ONSLOW COUNTY

LOCATION: NC 172 (SNEADS FERRY ROAD)  
FROM NC 210 TO CAMP LEJEUNE GATE

TYPE OF WORK: GRADING, DRAINAGE, & PAVING



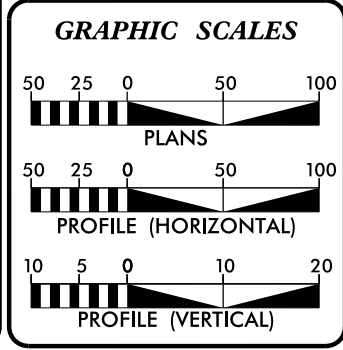
**CONTRACT: TIP PROJECT: W-5602**



CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

**PRELIMINARY PLANS**  
DO NOT USE FOR CONSTRUCTION

**INCOMPLETE PLANS**  
DO NOT USE FOR R/W ACQUISITION



**DESIGN DATA**

ADT 2013	=	20,000
DHV	=	%
D	=	%
T	=	3% *
V	=	50 MPH
* TTST = 1% DUAL = 2%		
FUNC CLASS	=	MINOR ARTERIAL REGIONAL TIER

**PROJECT LENGTH**

LENGTH ROADWAY PROJECT W-5602	=	2.683 MILES
TOTAL LENGTH PROJECT W-5602	=	2.683 MILES

NCDOT CONTACT: DAVID B. LEONARD, PE  
DIVISION DESIGN ENGINEER - DIVISION 3

2012 STANDARD SPECIFICATIONS

OCTOBER 25, 2016  
RIGHT OF WAY DATE:

AUGUST 15, 2017  
LETTING DATE:

CLAUDETTE M.K. ROQUE, PE  
PROJECT ENGINEER

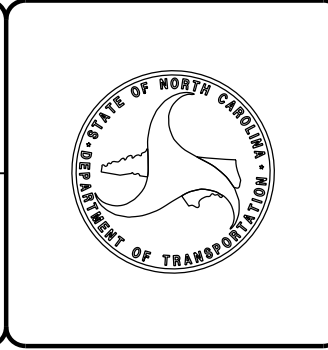
BRET PALIS  
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

SIGNATURE: \_\_\_\_\_ P.E.

ROADWAY DESIGN ENGINEER

SIGNATURE: \_\_\_\_\_ P.E.



I:\APR-2016 14:03 L:\ERO\Greenville Investigation\TIP\W5602\_GEO\_RDWY\CADD\_ORIGINAL\W-5602\_010416\W-5602\_010416\W5602\_Rdy\_tsh.dgn \$\$\$USERNAME\$\$\$



PAT McCrory  
Governor  
NICHOLAS J. TENNYSON  
Secretary

April 11, 2016

STATE PROJECT: 50139.1.FR1 (W-5602)  
 F.A. PROJECT: HSIP-0172(13)  
 COUNTY: Onslow  
 DESCRIPTION: NC 172 (Sneads Ferry Rd.) from NC 210 to Camp Lejeune Gate  
 SUBJECT: Geotechnical Inventory

**Project Description**

This project is located in Onslow County on NC 172. Proposed construction consists of widening NC 172 to four lanes from NC 210 to 500 feet north of the intersection with Old Ferry Rd. This geotechnical investigation was confined to the areas of proposed construction.

Fieldwork for this project was conducted during February and March of 2016. Hand auger borings were completed and representative soil samples were collected for visual classification in the field and submitted to the soils lab for testing.

The following alignment was investigated. The subsurface profile and selected cross sections of this alignment are included in this report.

<u>Line</u>	<u>Station(±)</u>
-L-	27+11 to 168+78

**Areas of Special Geotechnical Interest**

- 1) The entire project was found to exhibit seasonal high ground water.

- 2) The following sections contain cohesive soils which have the potential to cause embankment/subgrade and or slope stability problems during construction:

<u>Line</u>	<u>Station(±)</u>
-L-	57+63 to 58+38
-L-	151+62 to 154+38
-L-	157+63 to 160+38

- 3) The following sections contain organic soils which have the potential to cause embankment/subgrade and or slope stability problems during construction:

<u>Line</u>	<u>Station(±)</u>
-L-	48+78 to 49+80
-L-	85+64 to 86+34
-L-	91+85 to 94+30

**Physiography and Geology**

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Natural ground elevations along the proposed alignment range from 11± feet to 49± feet above sea level.

Surficial soils in this area are generally classified as alluvial.

**Ground Water**

Ground water data was collected in February and March of 2016, during a time of normal precipitation. Ground water elevations ranged from 6± to 46± feet above sea level.

**Soils**

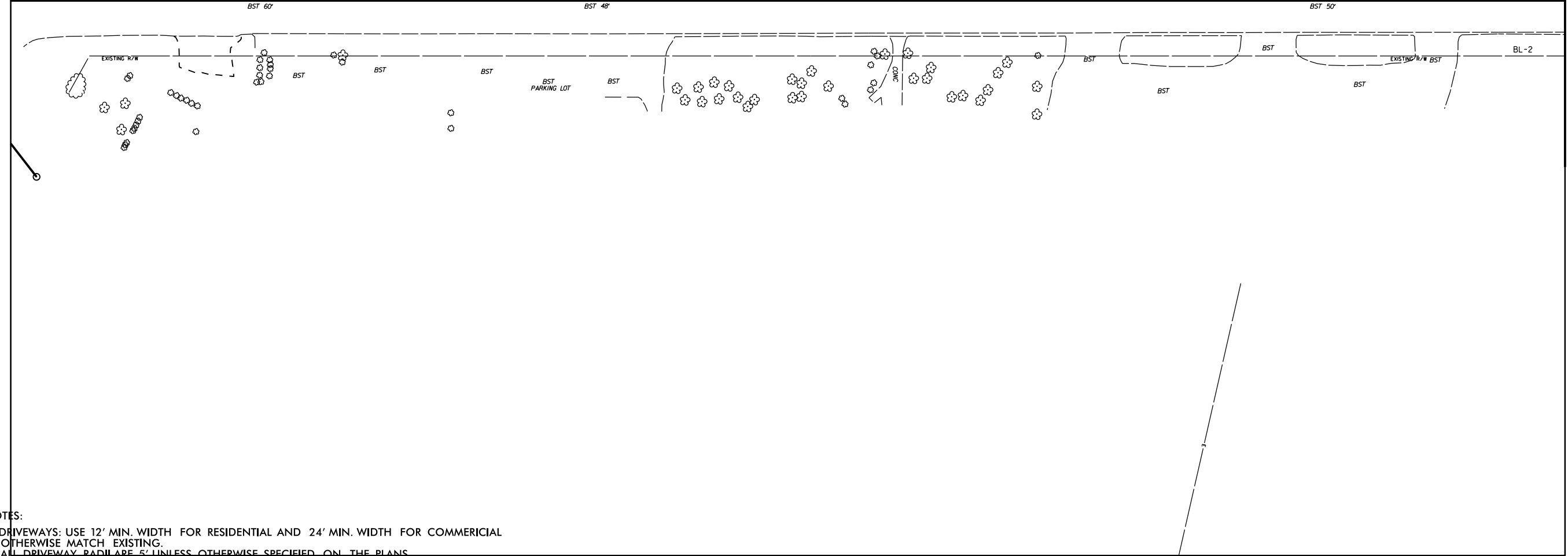
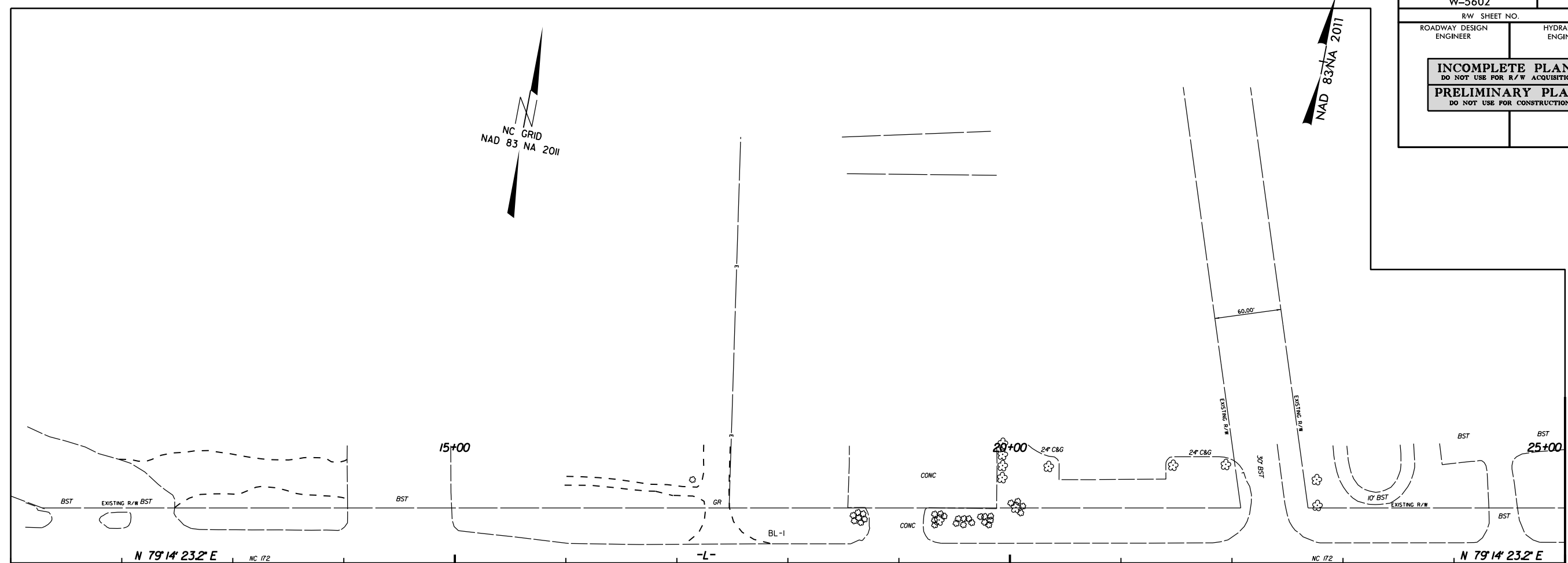
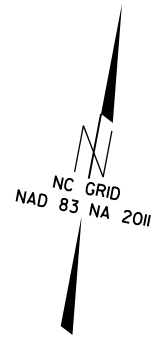
Soils within this project area have been divided into two categories: roadway embankment and alluvial.

Roadway embankment soils were encountered along existing NC 172. These soils are comprised of 1 to 4± feet of loose to medium dense sand (A-2-4, A-3).

Alluvial soils were encountered beneath the roadway embankment. They are comprised of 6± or more feet of loose to medium dense sand and soft sandy silt (A-4). Medium dense sand with trace to little organic material was also encountered with organic contents ranging from 0.8 to 4.4%



PROJECT REFERENCE NO.	SHEET NO.
W-5602	4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



REVISIONS

MATCHLINE -L- STA. 25 + 00.00  
SHEET 5

NOTES:

1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

I:\APR 2016 14:32 1. Investment\TIP\W5602.GEO\RDWY\CADD\_GEO\TECH\Site&Sub\W5602\_rdl\_psh04.dgn  
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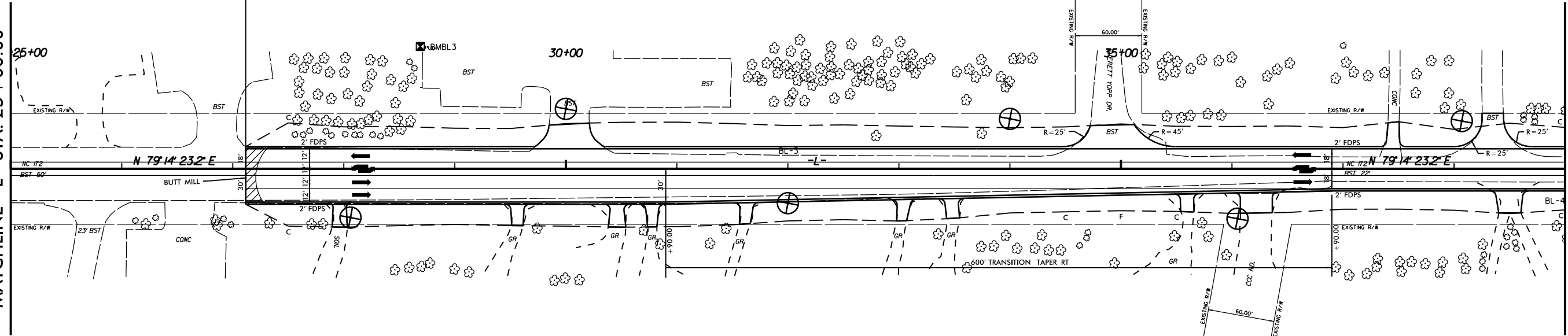
PROJECT REFERENCE NO.	SHEET NO.
W-5602	5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



BEGIN PROJECT W-5602  
 -L- STA. 27+11.67  
 BEGIN OVERLAY/WIDENING

SHEET 4  
MATCHLINE -L- STA. 25+00.00

MATCHLINE -L- STA. 39+00.00  
SHEET 6



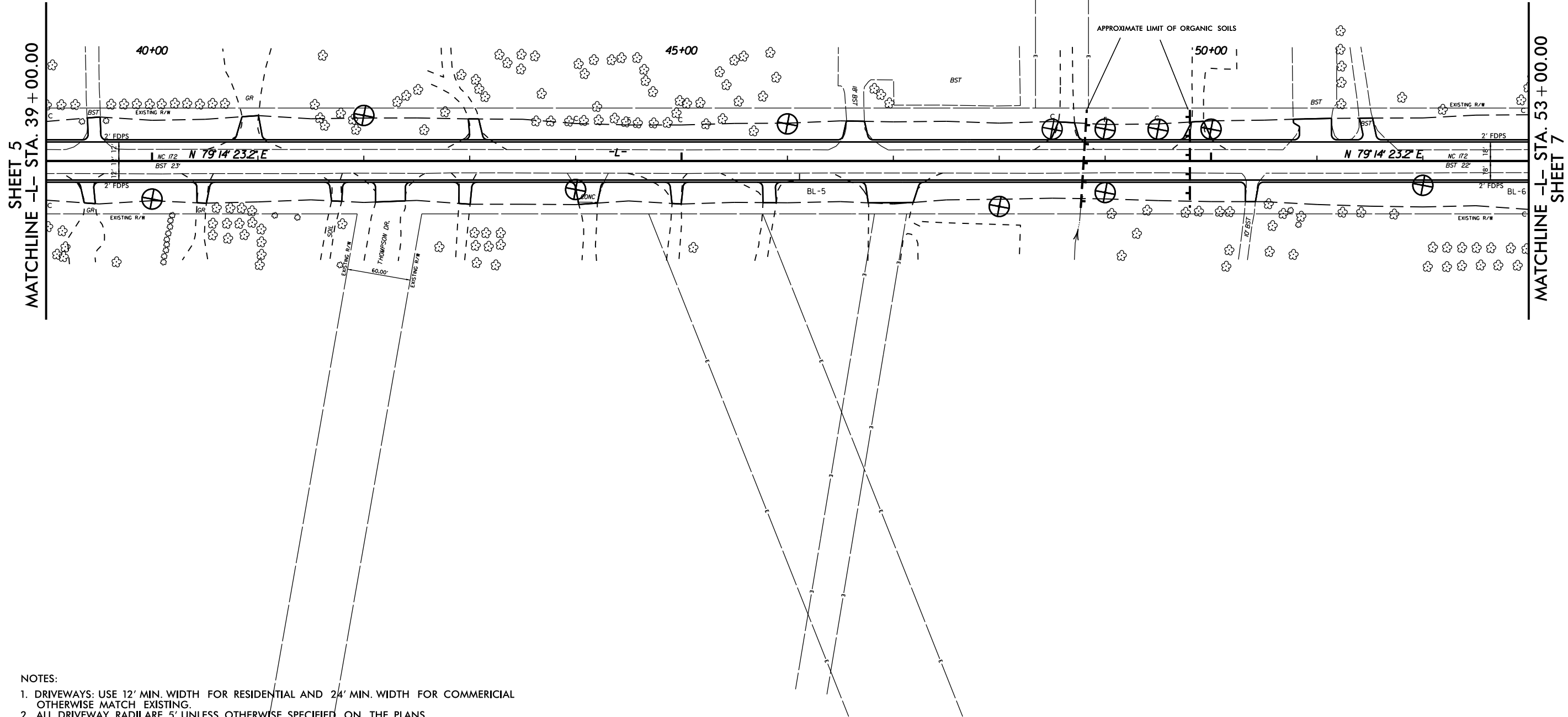
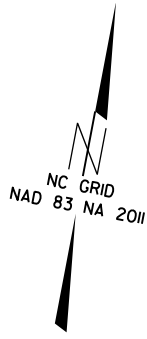
REVISIONS

NOTES:

1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

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PROJECT REFERENCE NO.	SHEET NO.
W-5602	6
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

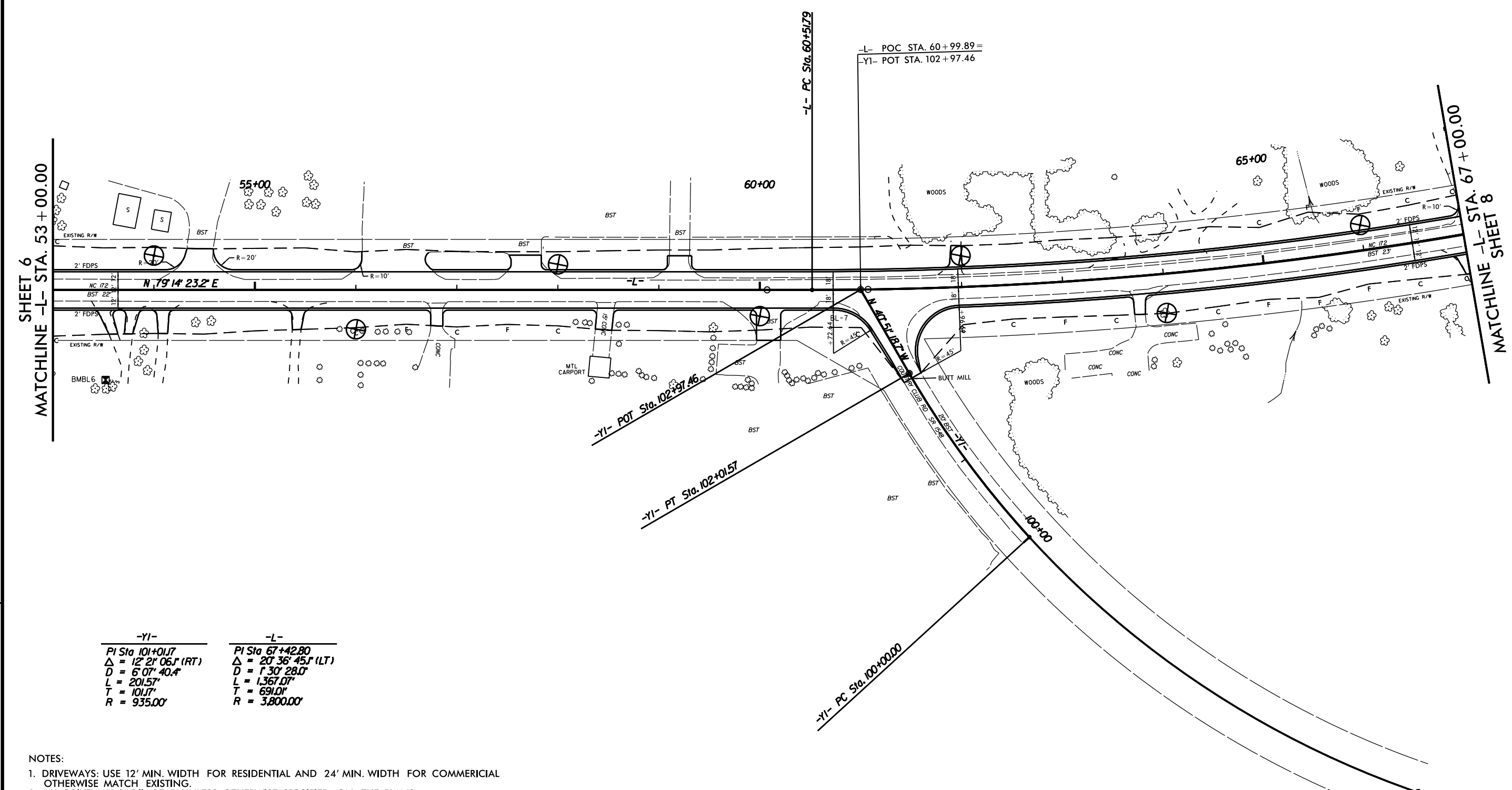
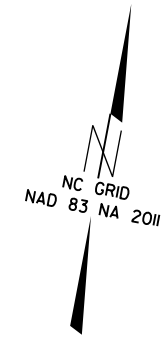


**NOTES:**

1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

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PROJECT REFERENCE NO.	SHEET NO.
W-5602	7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



-YI-	-L-
PI Sta 101+01.7	PI Sta 67+42.80
Δ = 12° 21' 06.7 (RT)	Δ = 20° 36' 45.7 (LT)
D = 6' 07" 40.4"	D = 1' 30" 28.0"
L = 201.57'	L = 1,367.07'
T = 101.17'	T = 691.01'
R = 935.00'	R = 3,800.00'

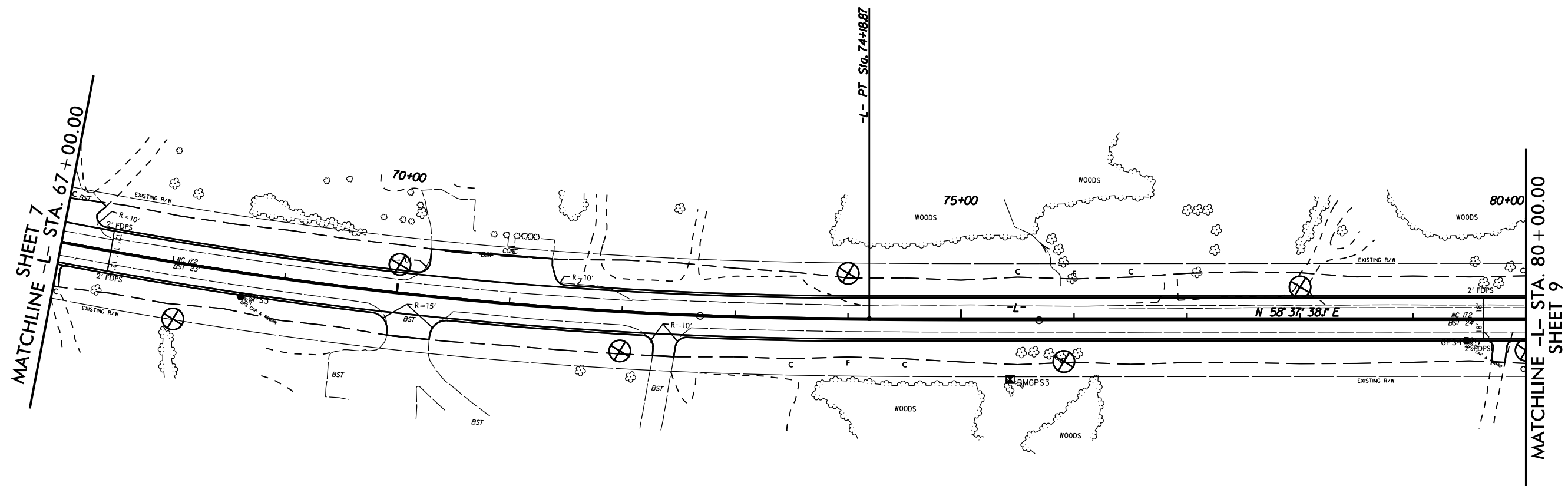
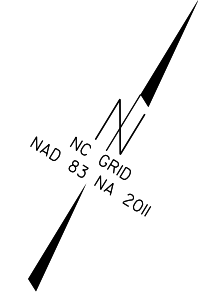
- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

REVISIONS

I:\APR 2016 14:34 [1]s Investment\TIP\W5602\_GEO\RDWY\CADD\_GEO\RDWY\CADD\_GEO\TECH\Site&Sub\W5602\_rdl\_psh07.dgn  
 8/17/99



PROJECT REFERENCE NO.	SHEET NO.
W-5602	8
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



MATCHLINE -L- STA. 67+00.00  
SHEET 7

MATCHLINE -L- STA. 80+00.00  
SHEET 9

-L-  
 PI Sta 67+42.80  
 $\Delta = 20^\circ 36' 45.1''$  (LT)  
 $D = 1^\circ 30' 28.0''$   
 $L = 1,367.07'$   
 $T = 691.01'$   
 $R = 3,800.00'$

**NOTES:**

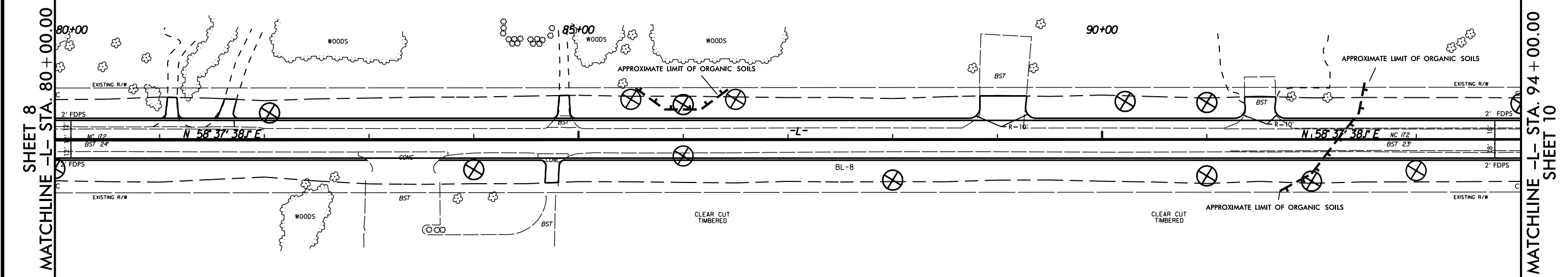
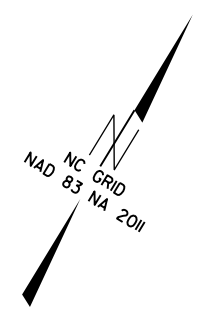
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

REVISIONS

8/17/99

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PROJECT REFERENCE NO.	SHEET NO.
W-5602	9
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



REVISIONS

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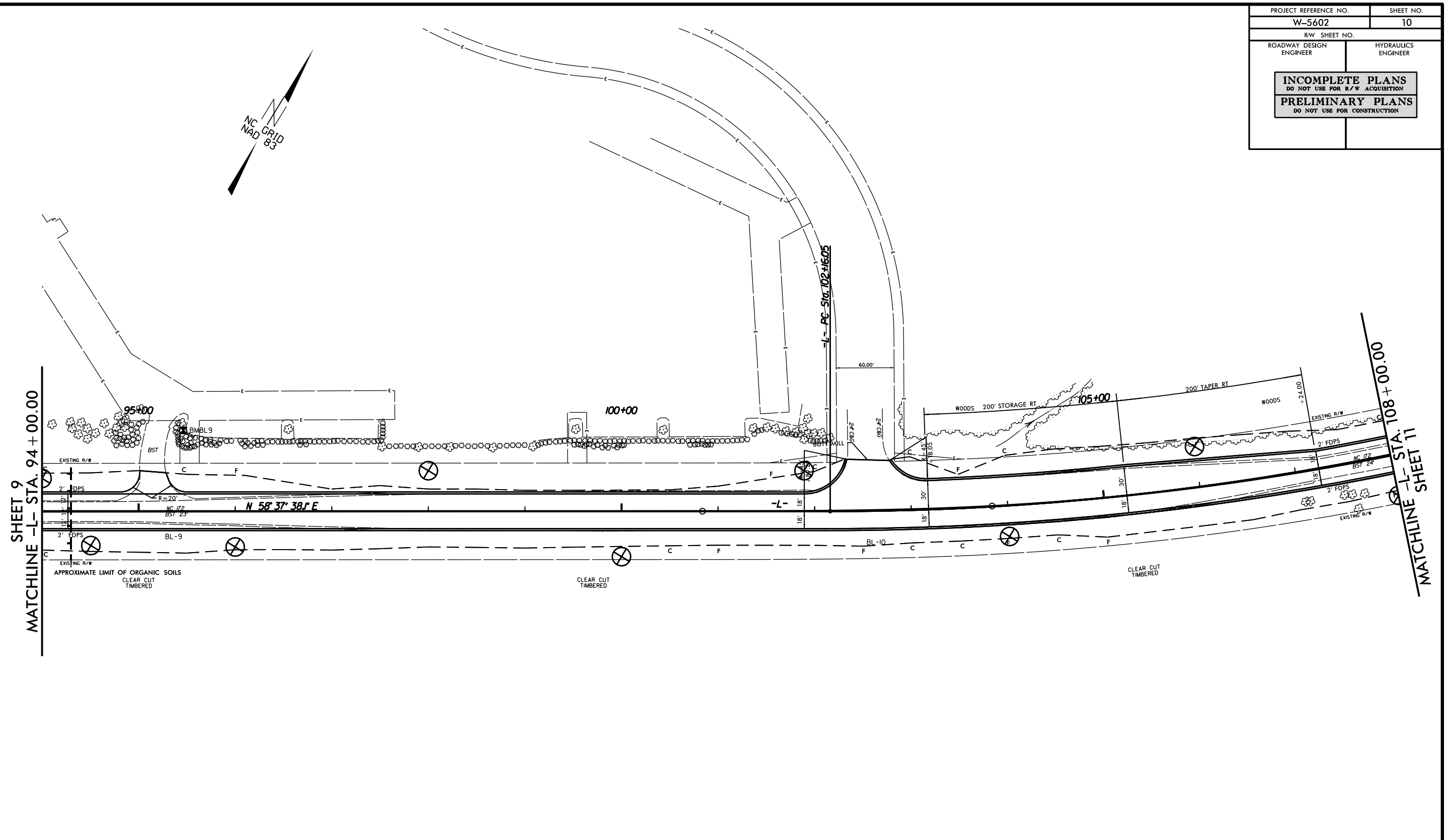
- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

PROJECT REFERENCE NO.	SHEET NO.
W-5602	10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



SHEET 9  
 MATCHLINE -L- STA. 94+00.00

MATCHLINE -L- STA. 108+00.00  
 SHEET 11



-L-

PI Sta 115+20.67  
 $\Delta = 48^{\circ} 26' 34.7''$  (LT)  
 $D = 1^{\circ} 58' 32.6''$   
 $L = 2,451.92'$   
 $T = 1,304.62'$   
 $R = 2,900.00'$

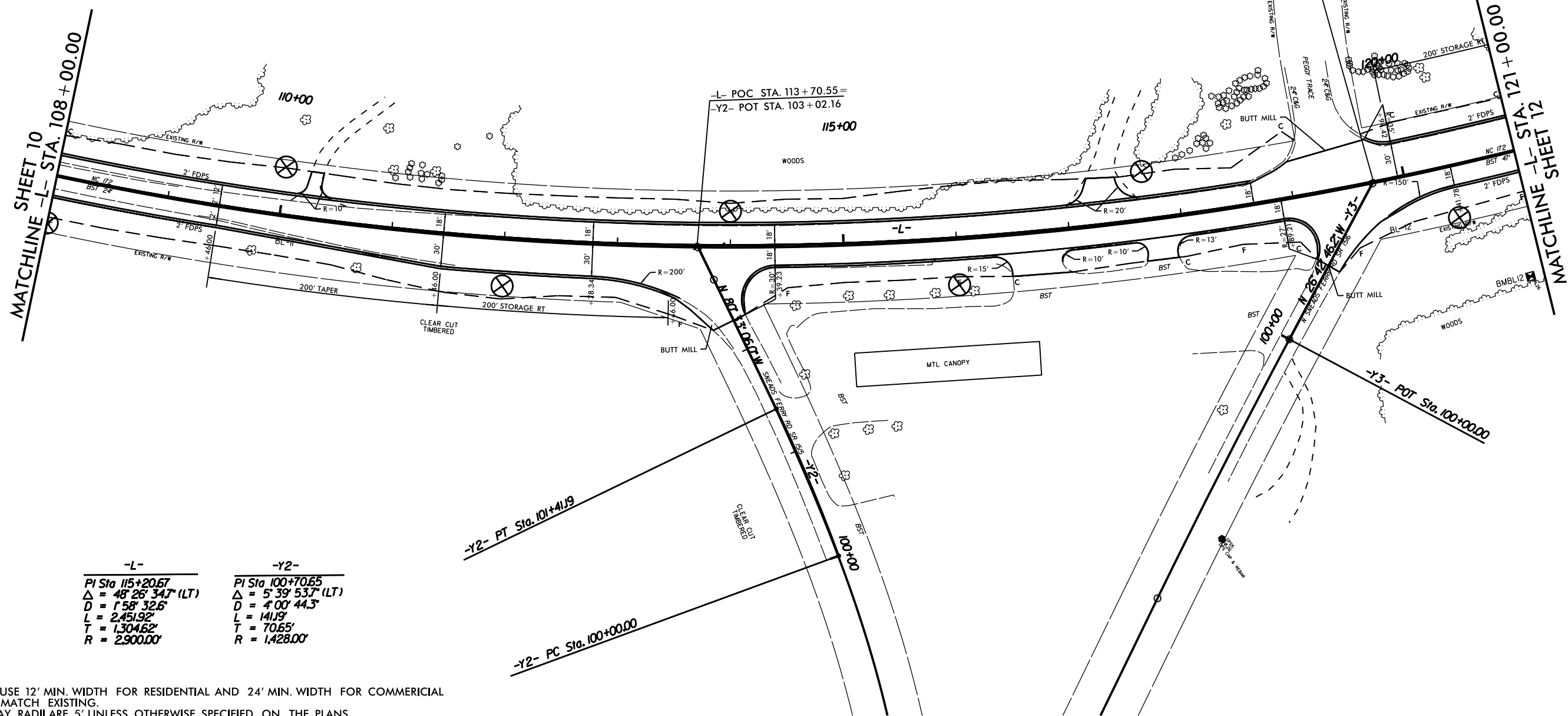
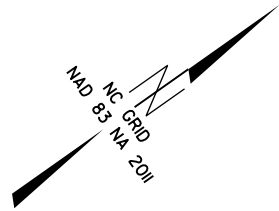
**NOTES:**

1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

REVISIONS

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PROJECT REFERENCE NO.	SHEET NO.
W-5602	11
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



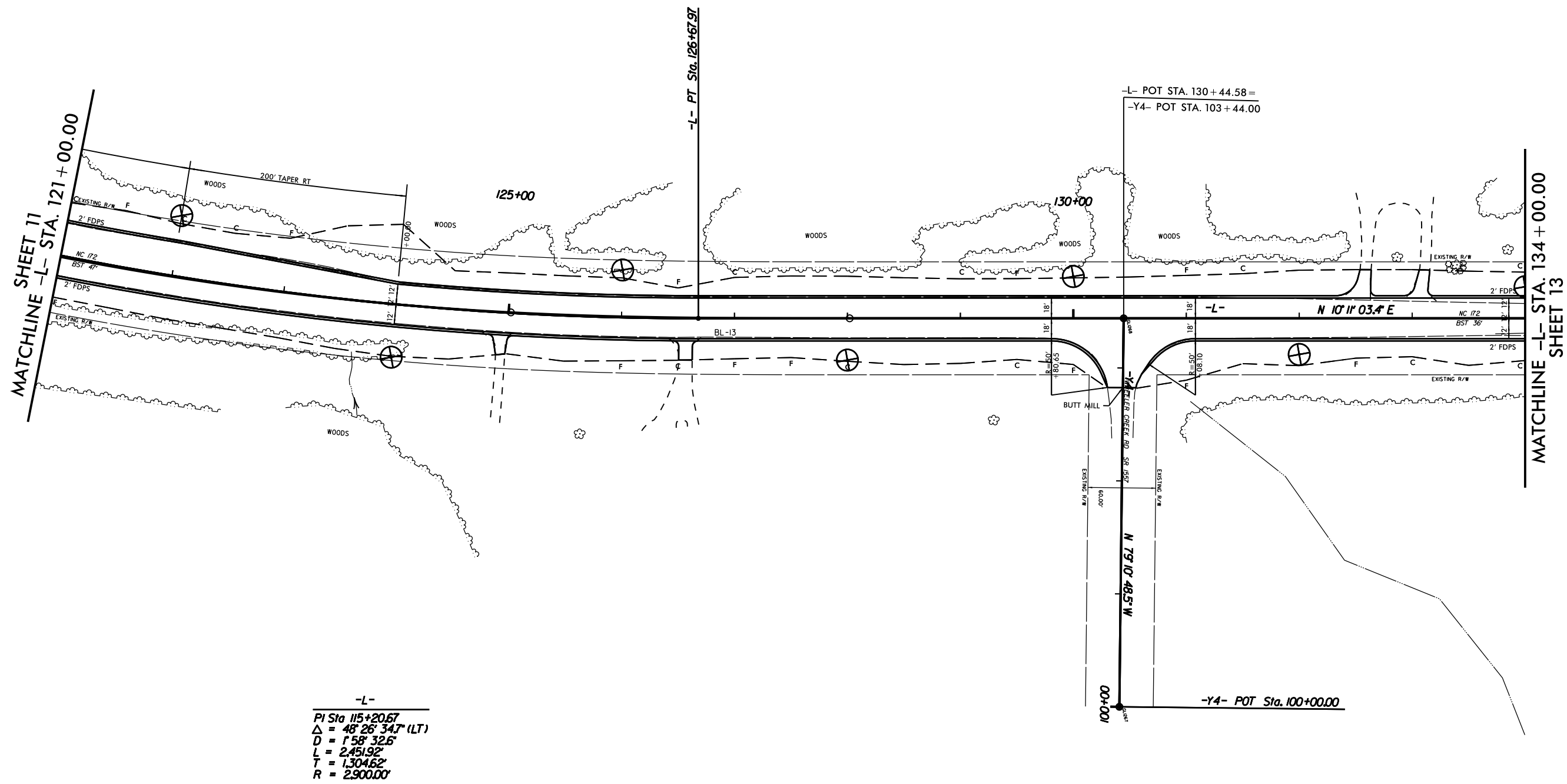
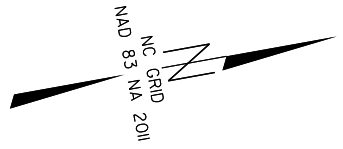
-L-	-Y2-
PI Sta 115+20.67	PI Sta 100+70.65
$\Delta = 48^{\circ} 26' 34.7" (LT)$	$\Delta = 5^{\circ} 39' 53.7" (LT)$
D = 1'58' 32.6"	D = 4'00' 44.3"
L = 2451.92'	L = 141.9'
T = 1304.62'	T = 70.65'
R = 2900.00'	R = 1428.00'

- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

REVISIONS

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 8/17/99

PROJECT REFERENCE NO.	SHEET NO.
W-5602	12
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



-L-

PI Sta 115+20.67  
 $\Delta = 48^{\circ} 26' 34.7\" (LT)$   
 $D = 158' 32.6'$   
 $L = 245.192'$   
 $T = 1304.62'$   
 $R = 2900.00'$

- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

REVISIONS

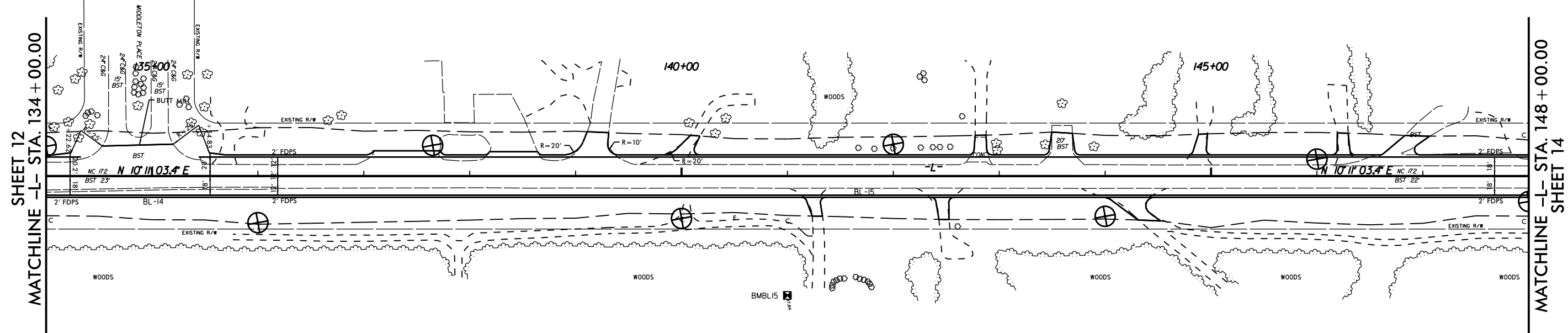
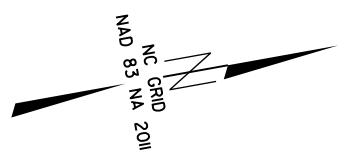
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MATCHLINE -L- STA. 134+00.00 SHEET 13

MATCHLINE -L- STA. 121+00.00 SHEET 11

PROJECT REFERENCE NO.	SHEET NO.
W-5602	13
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

8/17/99



SHEET 12  
MATCHLINE -L- STA. 134+00.00

MATCHLINE -L- STA. 148+00.00  
SHEET 14

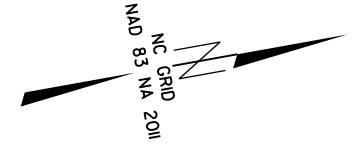
REVISIONS

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NOTES:

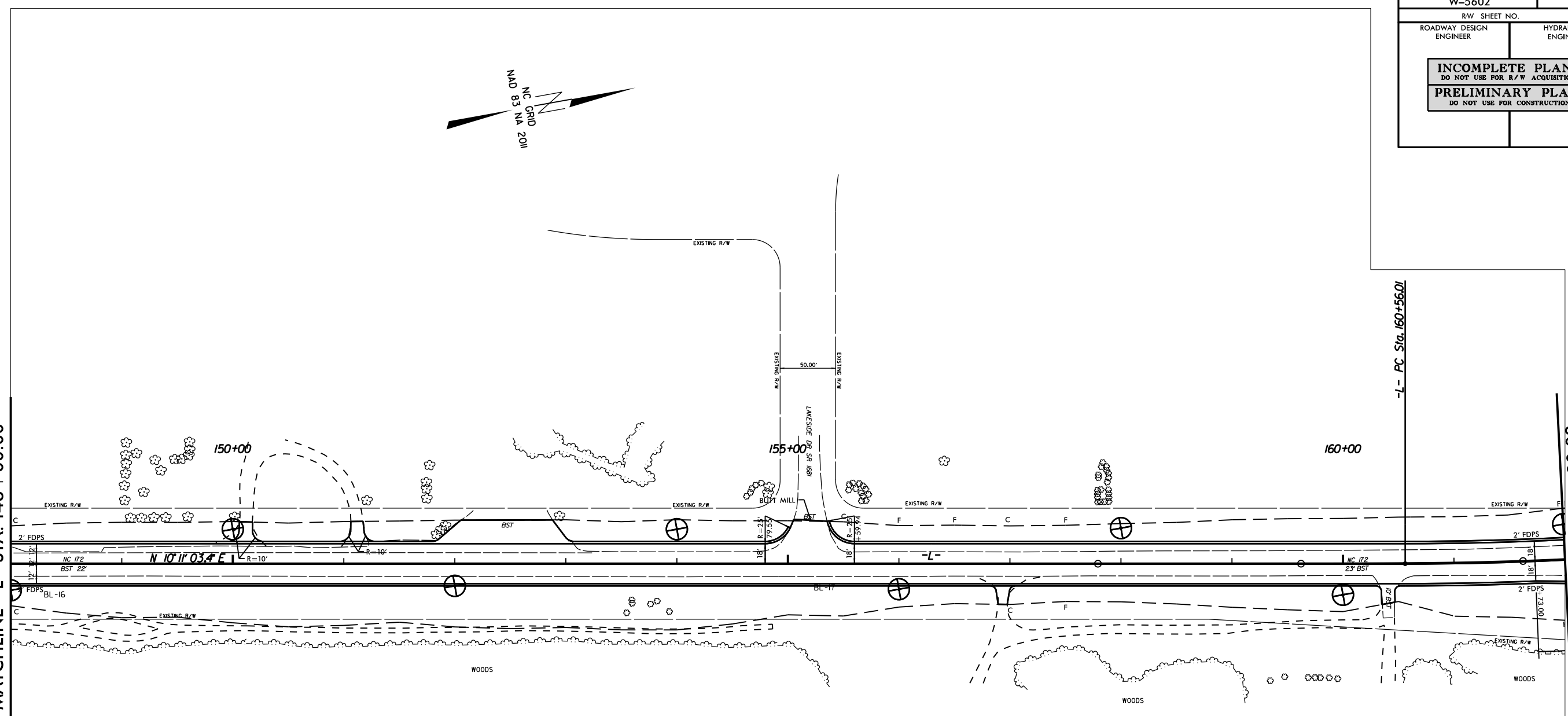
1. DRIVEWAYS USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

PROJECT REFERENCE NO.	SHEET NO.
W-5602	14
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



SHEET 13  
 MATCHLINE -L- STA. 148+00.00

MATCHLINE -L- STA. 162+00.00  
 SHEET 15



-L-

PI Sta 165+12.18  
 $\Delta = 17^{\circ} 52' 43.4\"$  (LT)  
 $D = 158' 32.6'$   
 $L = 904.92'$   
 $T = 456.17'$   
 $R = 2900.00'$

- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.

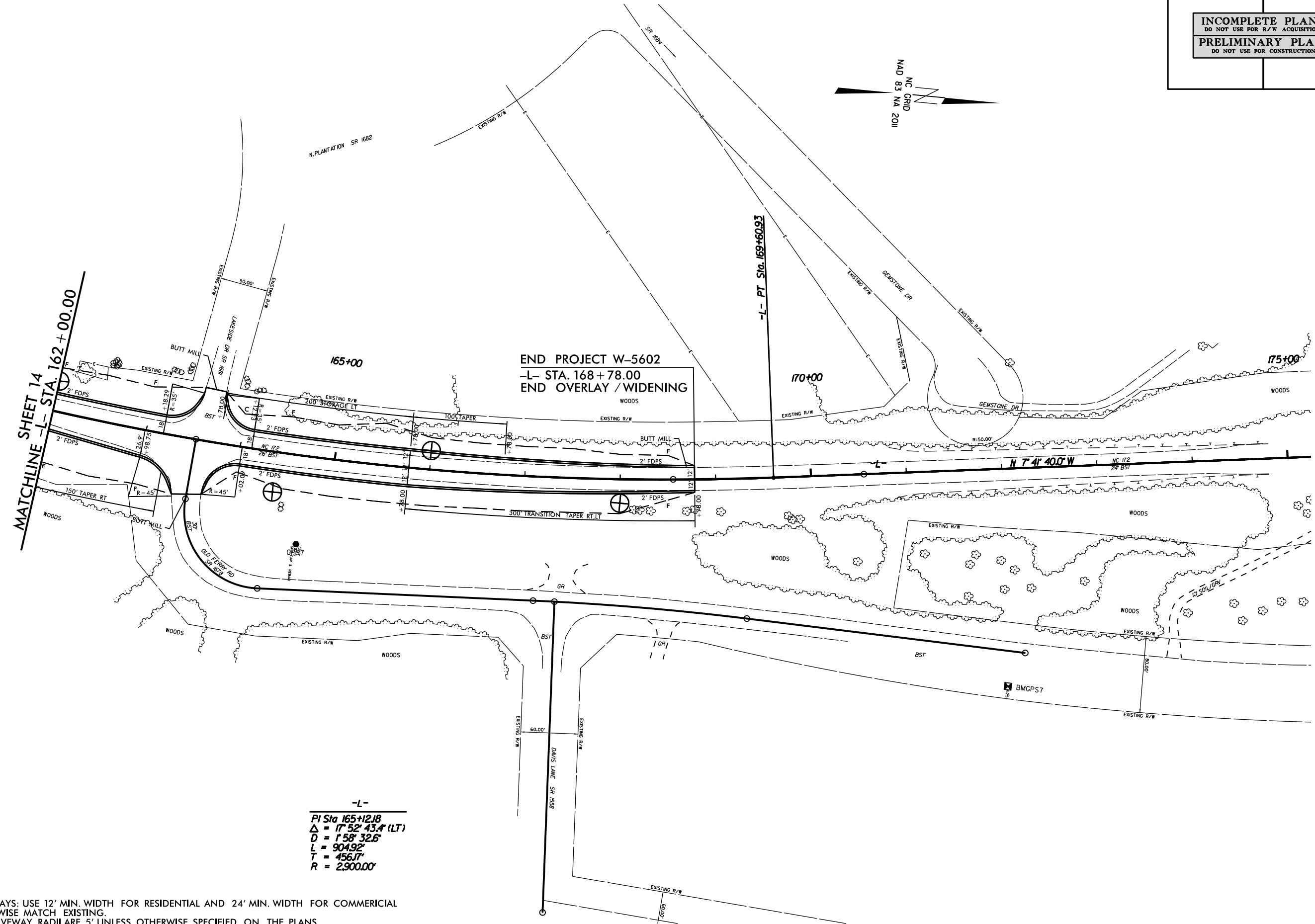
REVISIONS

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PROJECT REFERENCE NO.	SHEET NO.
W-5602	15
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

REVISIONS

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-L-

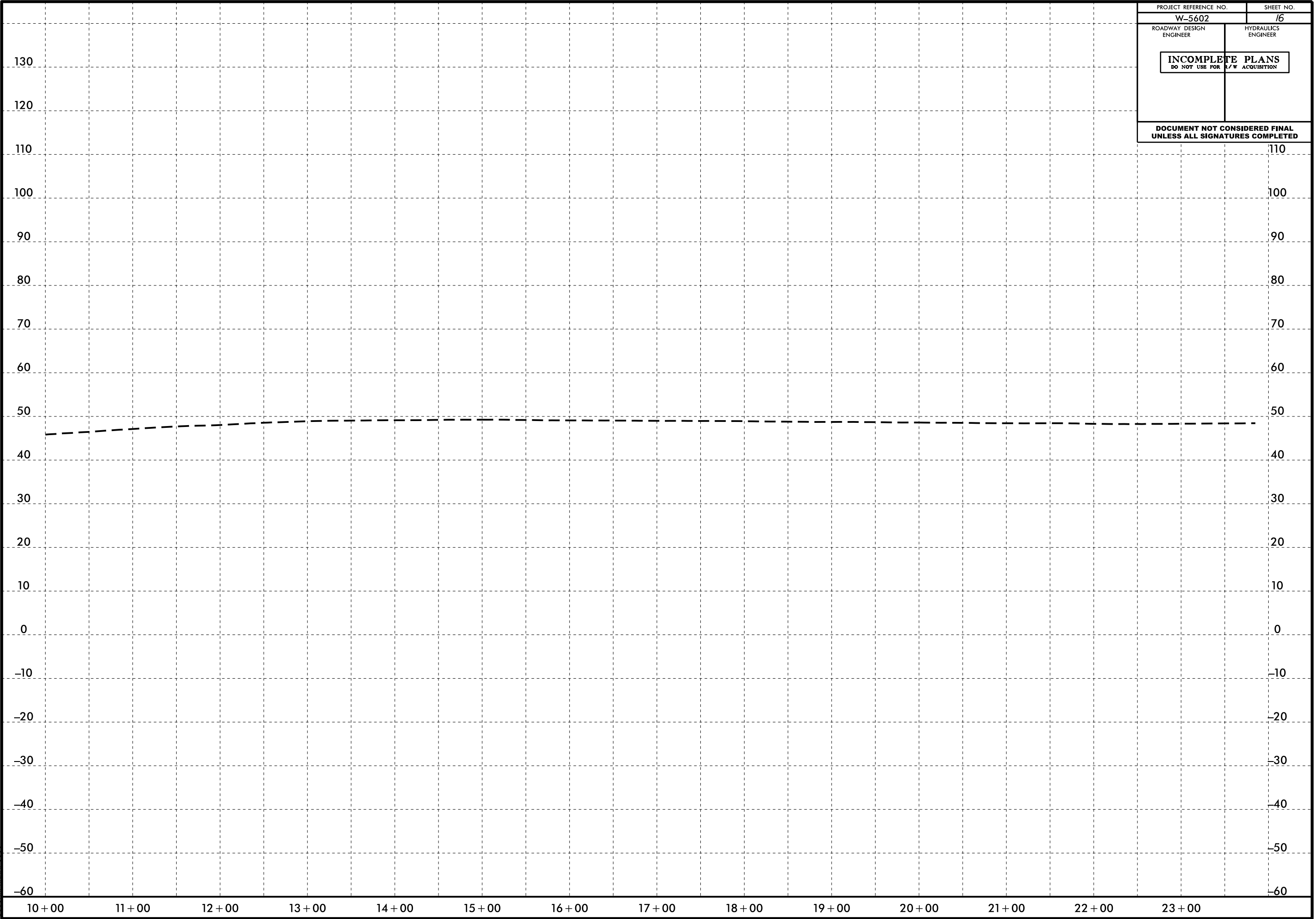
PI Sta 165+12.18  
 $\Delta = 17' 52' 43.4" (LT)$   
 $D = 158' 32.6'$   
 $L = 904.92'$   
 $T = 456.17'$   
 $R = 2900.00'$

- NOTES:
1. DRIVEWAYS: USE 12' MIN. WIDTH FOR RESIDENTIAL AND 24' MIN. WIDTH FOR COMMERCIAL OTHERWISE MATCH EXISTING.
  2. ALL DRIVEWAY RADII ARE 5' UNLESS OTHERWISE SPECIFIED ON THE PLANS.



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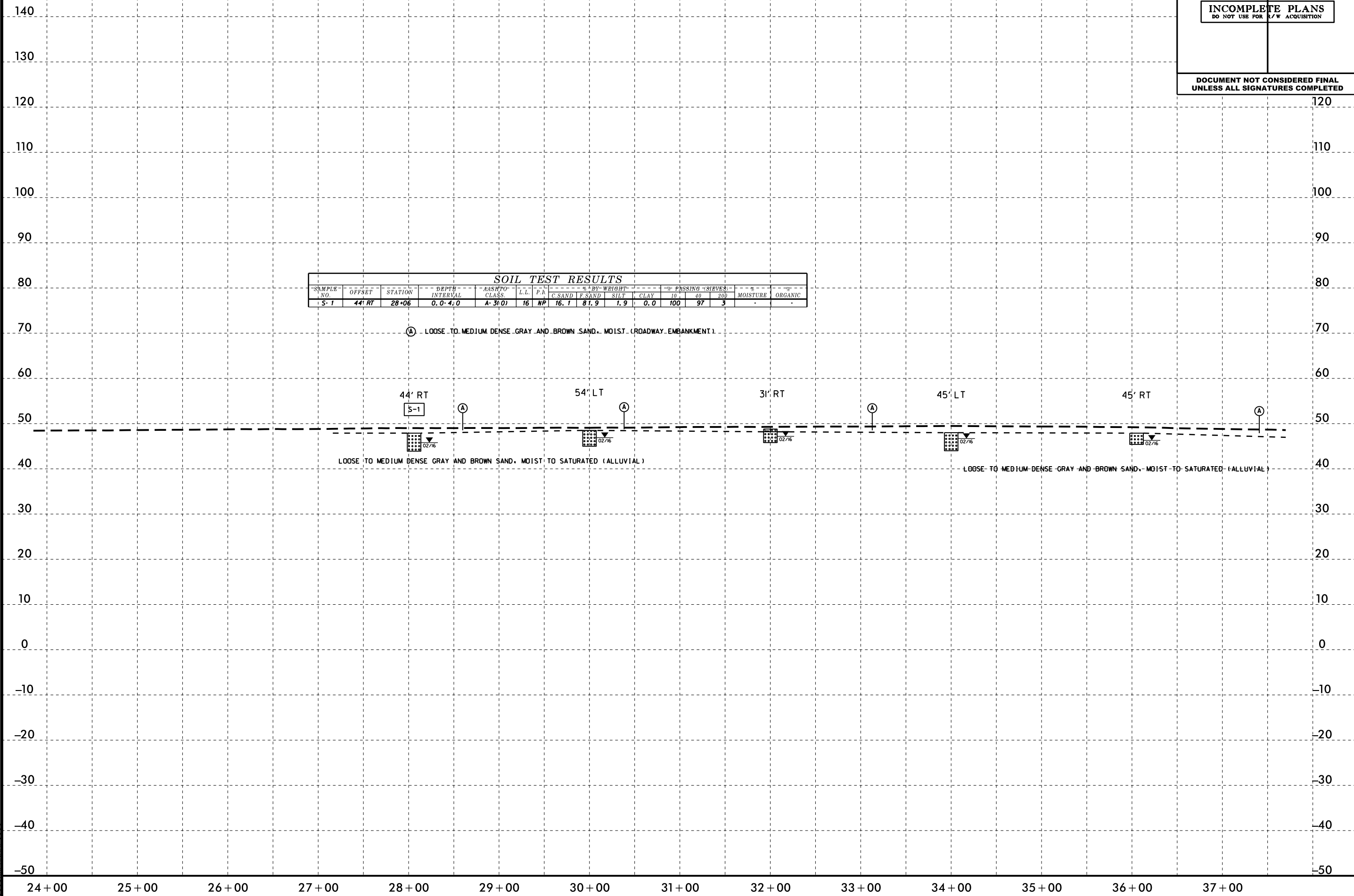
PROJECT REFERENCE NO.	SHEET NO.
W-5602	16
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



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PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>17</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				PASSING SIEVES			MOISTURE	ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-1	44' RT	28+06	0.0-4.0	A-3(0)	16	NP	16.1	81.9	1.9	0.0	100	97	3	-	-

(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)

44' RT  
S-1

(A)

54' LT

(A)

31' RT

(A)

45' LT

45' RT

(A)

LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

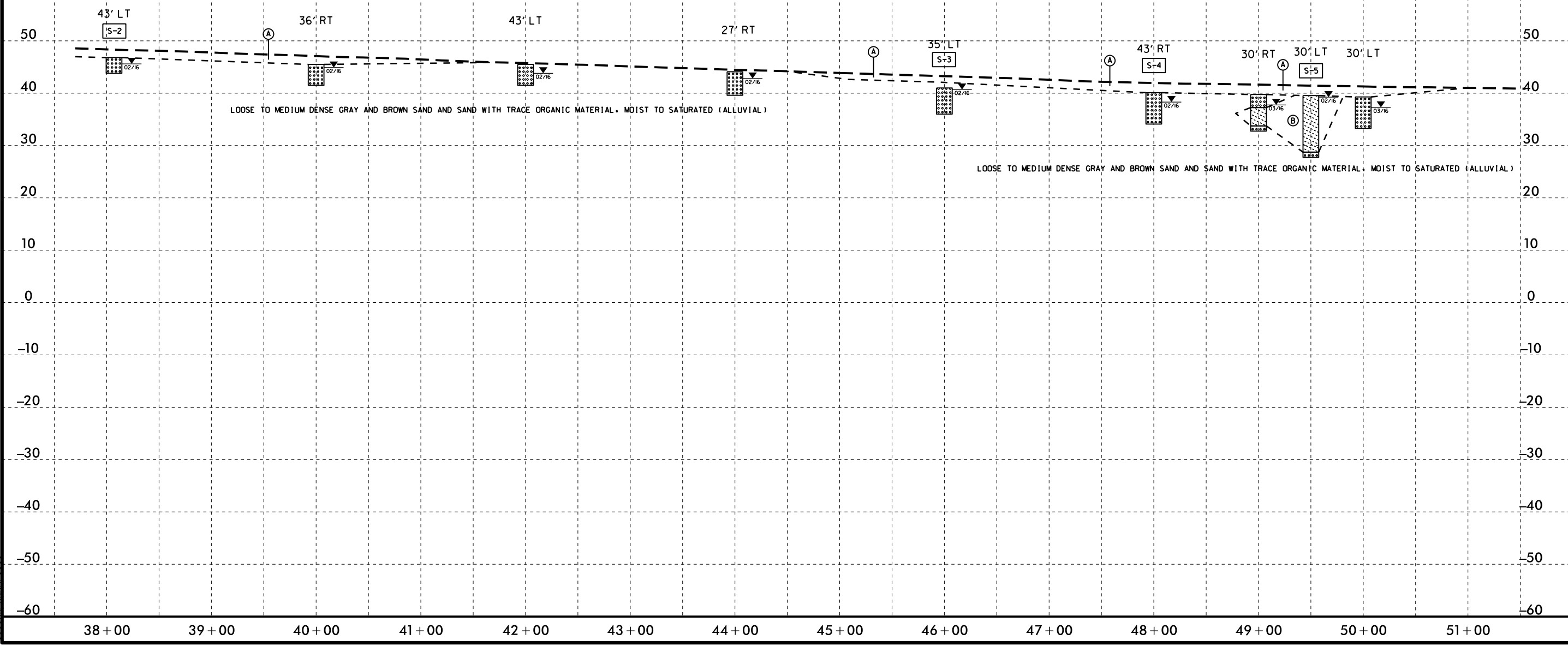
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\$\$\$\$\$ SPRING \$\$\$\$

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 \$\$\$\$ SPRING \$\$\$

PROJECT REFERENCE NO.	SHEET NO.
W-5602	18
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.C.	P.L.	% BY WEIGHT				% PASSING SIEVES		MOISTURE	ORGANIC
							SAND	F SAND	SILT	CLAY	#10	#200		
S-2	43' LT	38+07	0.0-3.0	A-3(0)	11	NP	20.6	76.5	2.9	0.0	100	96	4	-
S-3	35' LT	46+00	2.0-2.5	A-3(0)	24	NP	12.7	81.9	3.9	2.0	100	97	8	1.9
S-4	43' RT	48+00	0.0-6.0	A-3(0)	20	NP	12.7	81.1	4.1	2.0	100	98	8	2.9
S-5	30' LT	50+00	0.3-10.8	A-3(0)	25	NP	13.7	78.3	3.9	4.0	100	97	10	4.4

- (A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)
- (B) MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)



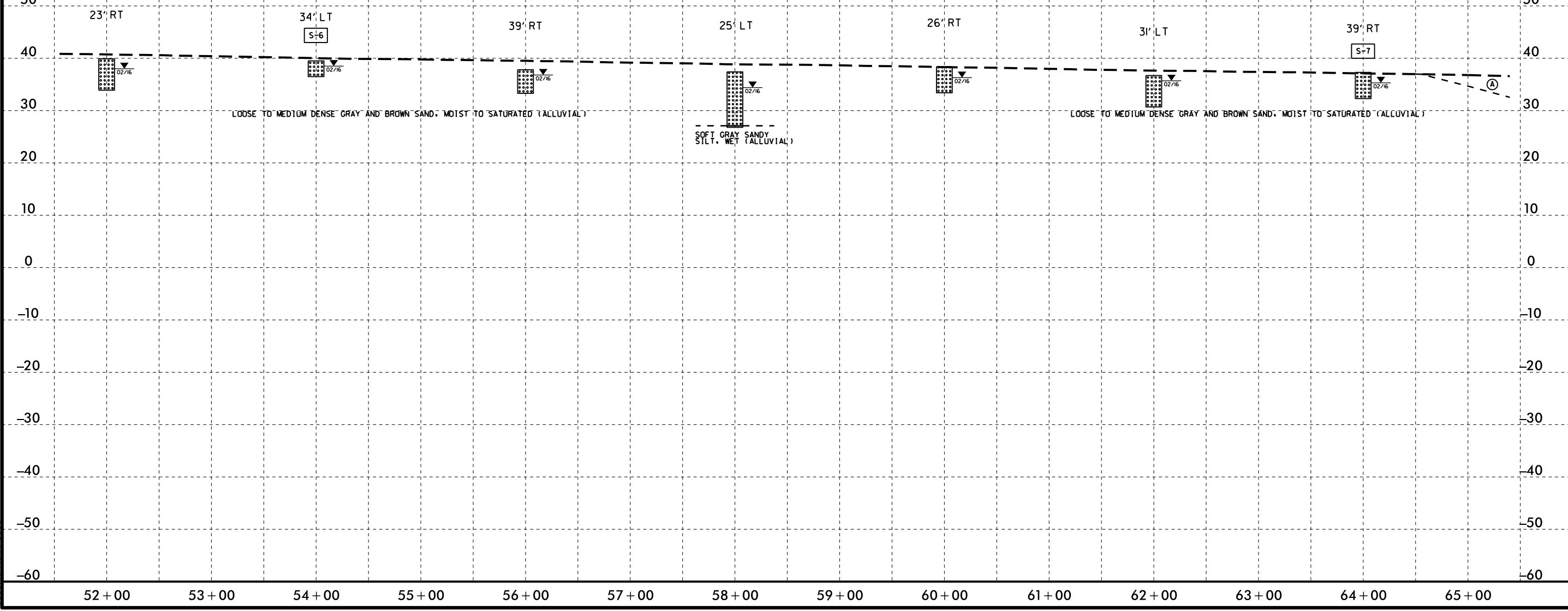
38+00      39+00      40+00      41+00      42+00      43+00      44+00      45+00      46+00      47+00      48+00      49+00      50+00      51+00

5/14/99

PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>19</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	10	40	200		
S-6	34' LT	54+00	0.0' - 3.0'	A-3(0)	18	NP	14.7	80.5	2.7	2.0	100	97	7	-	-
S-7	39' RT	64+00	0.0' - 5.0'	A-3(0)	16	NP	16.5	81.3	2.1	0.0	100	97	4	-	-

Ⓐ LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)



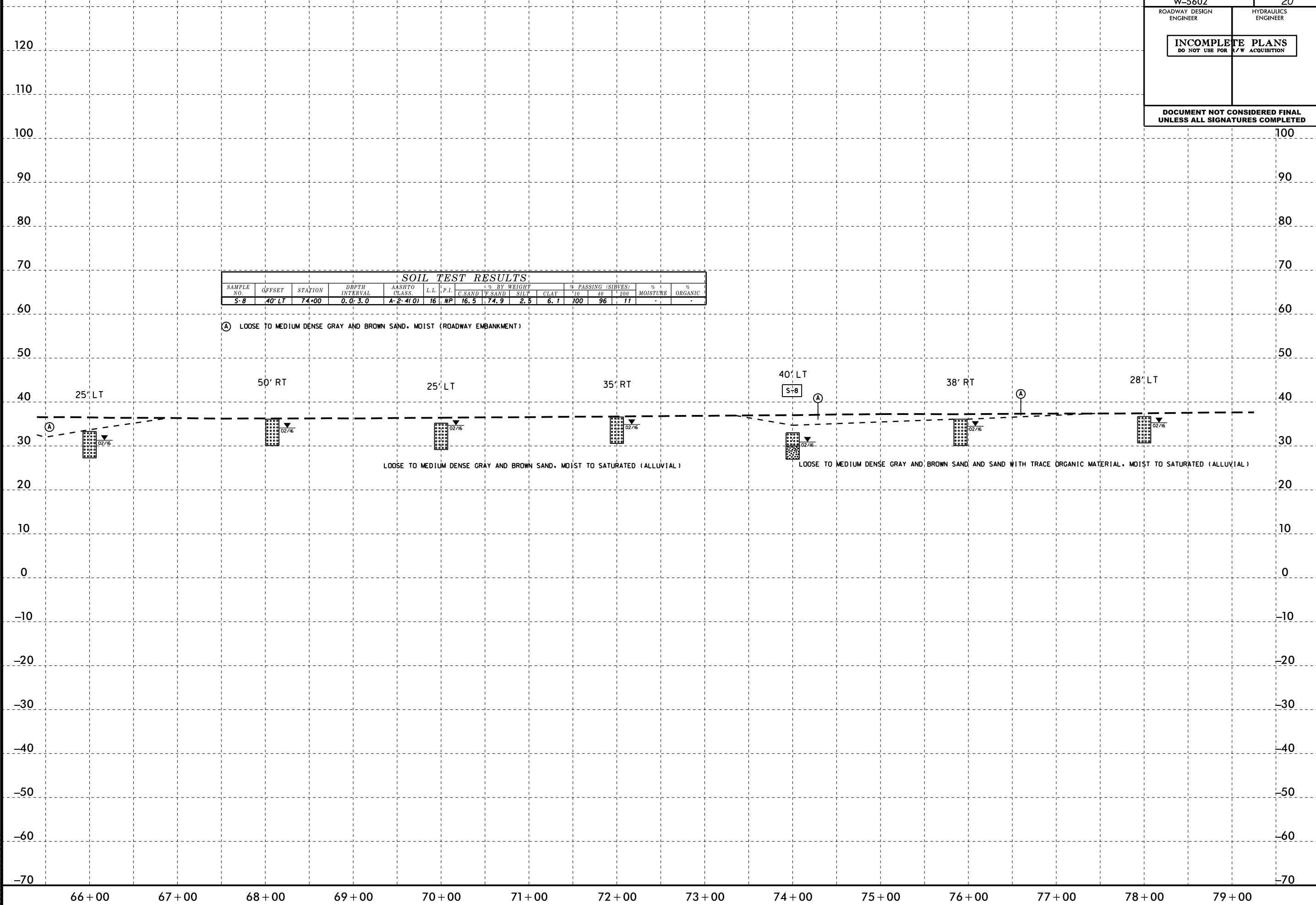
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PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>20</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIBVES)		% MOISTURE	% ORGANIC
							C SAND	F SAND	SILT	CLAY	#10	#40		
S-8	40' LT	74+00	0.0-3.0	A-2-4(0)	16	NP	16.5	74.9	2.5	6.1	100	96	11	-

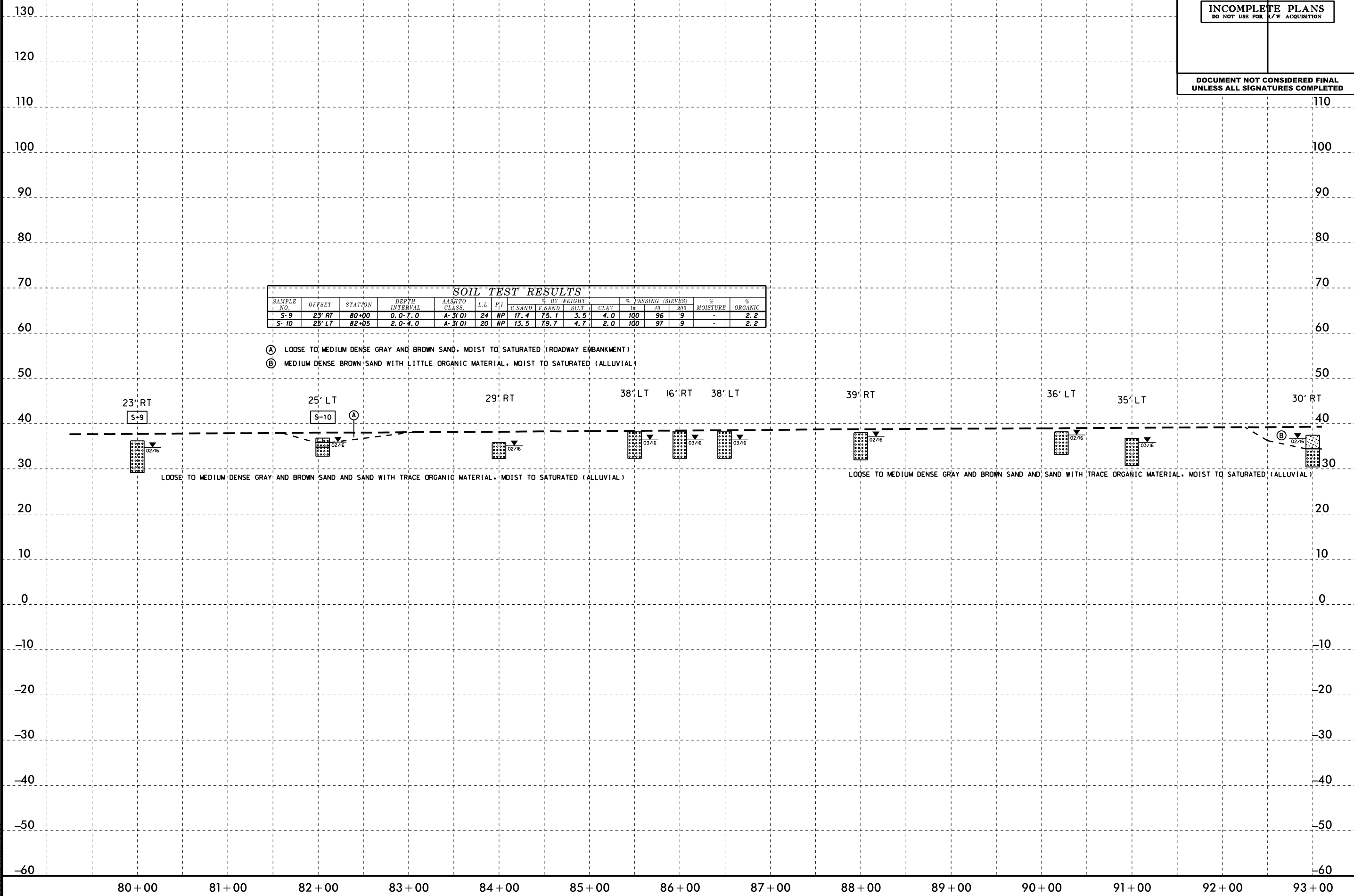
(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)



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PROJECT REFERENCE NO. W-5602	SHEET NO. 21
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



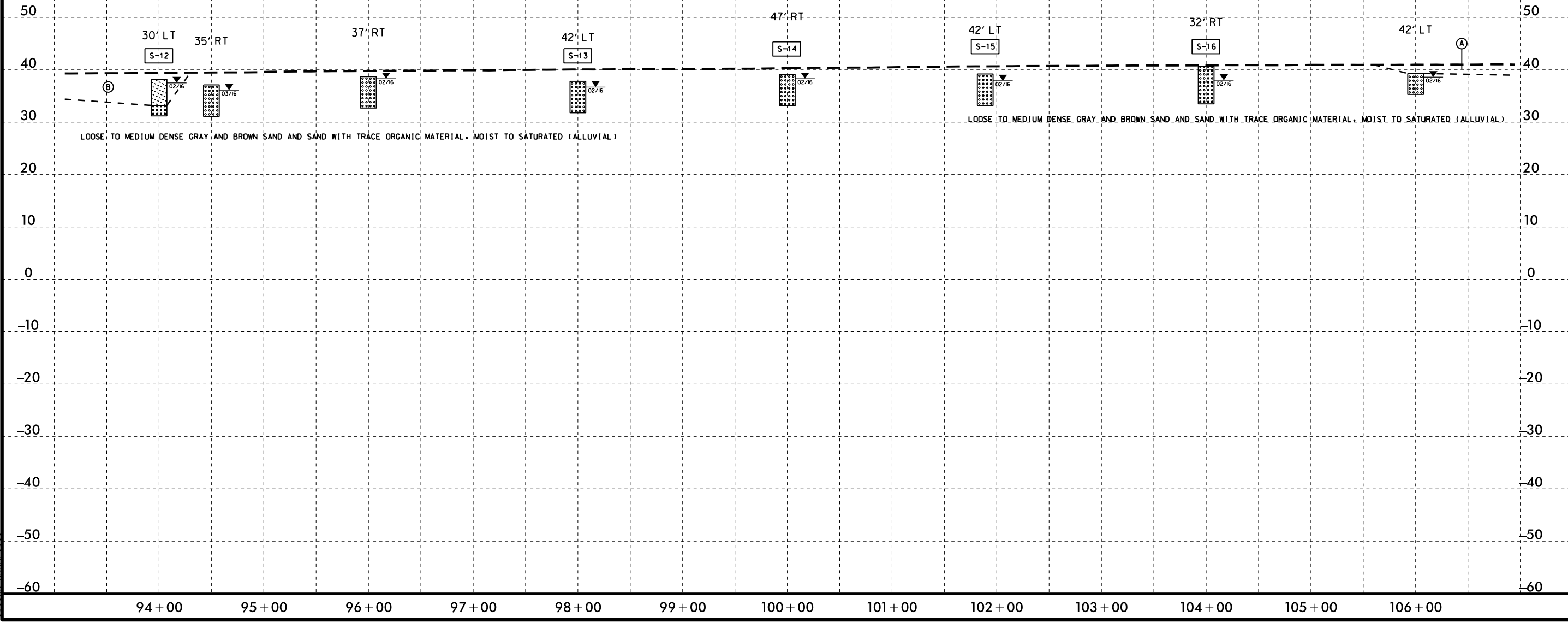
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PROJECT REFERENCE NO. W-5602	SHEET NO. 22
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIBVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	#10	#40	#200		
S-12	34' LT	94+00	0.0-5.0	A-2-4(0)	23	NP	16.8	70.2	7.0	6.1	100	96	15	-	4.5
S-13	42' LT	98+00	1.0-6.0	A-3(0)	24	NP	21.8	75.1	3.1	0.0	100	94	5	-	2.3
S-14	47' RT	100+00	2.0-6.0	A-3(0)	16	NP	16.5	75.3	6.2	2.0	100	97	10	-	1.8
S-15	42' LT	101+89	1.0-6.0	A-3(0)	27	NP	17.4	75.7	4.9	2.0	100	96	9	-	2
S-16	32' RT	104+00	0.5-7.2	A-3(0)	20	NP	20.2	77.9	1.9	0.0	100	94	4	-	0.8

- (A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)
- (B) MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

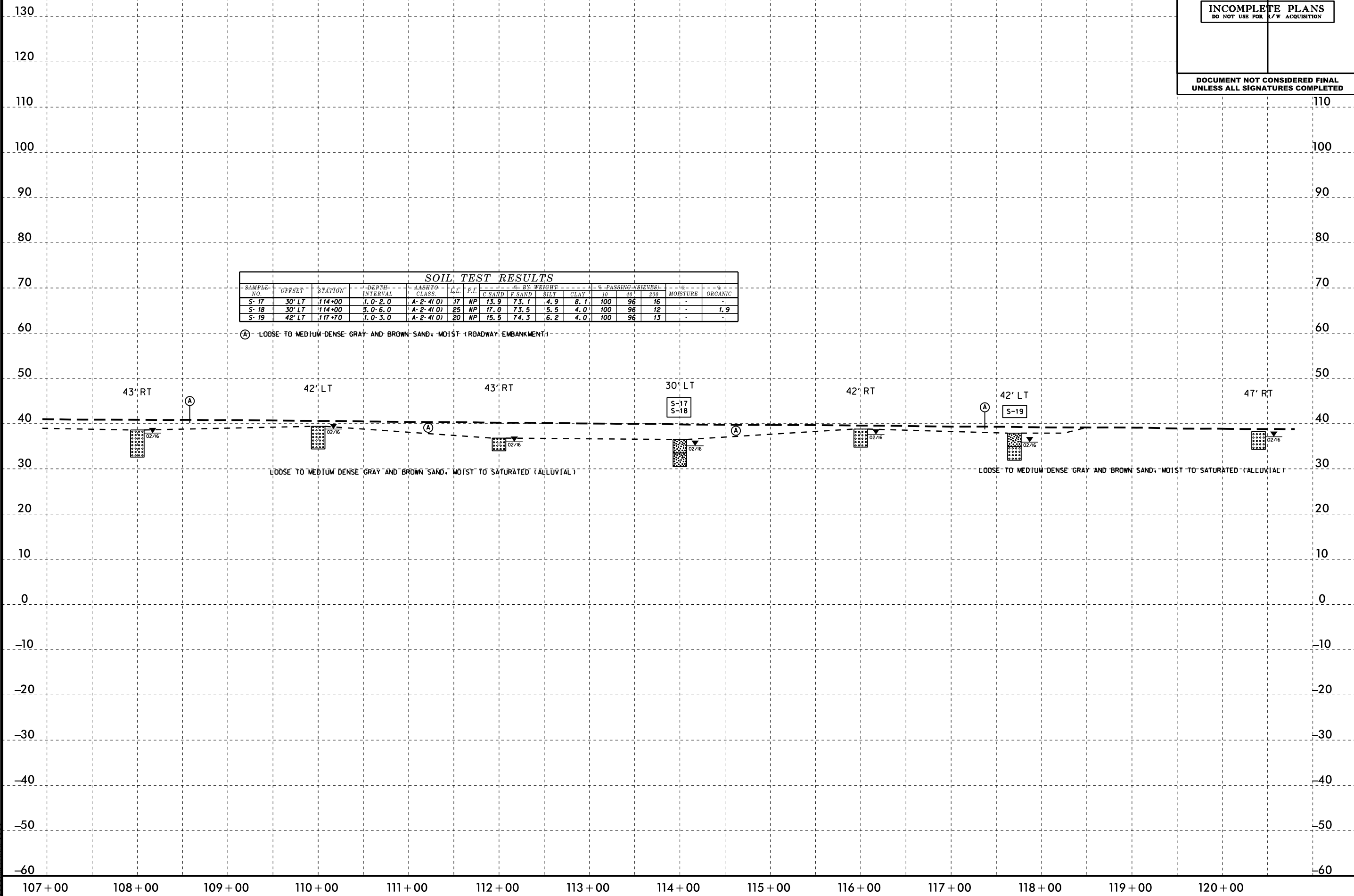


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5/14/99

PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>23</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-17	30' LT	114+00	1.0-2.0	A-2-4(0)	17	NP	13.9	73.7	4.9	8.1	100	96	16	-	-
S-18	30' LT	114+00	3.0-6.0	A-2-4(0)	25	NP	17.0	73.5	5.5	4.0	100	96	12	-	1.9
S-19	42' LT	117+70	1.0-3.0	A-2-4(0)	20	NP	15.5	74.3	6.2	4.0	100	96	13	-	-

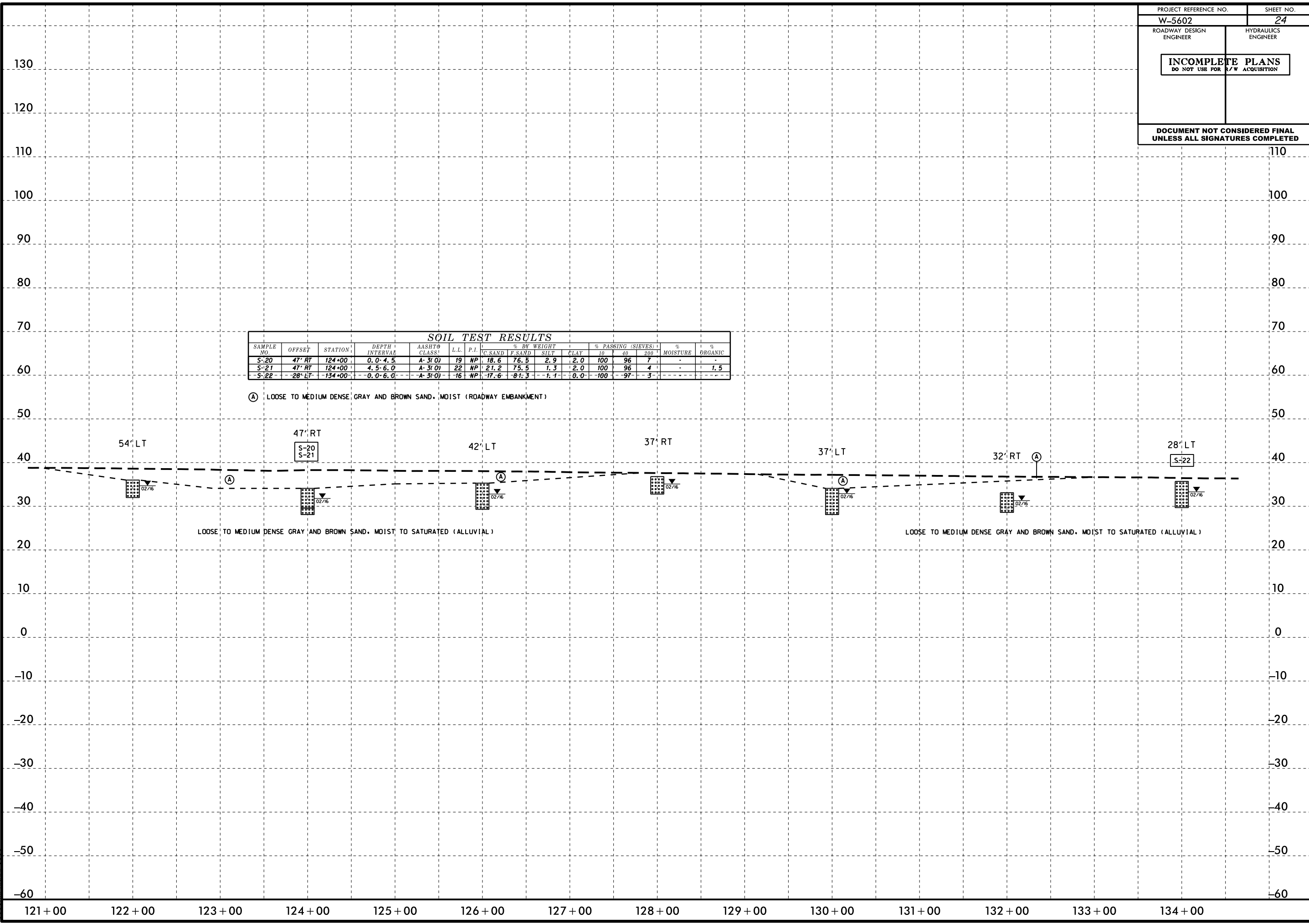


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PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>24</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-20	47' RT	124+00	0.0-4.5	A-3(0)	19	NP	18.6	76.5	2.9	2.0	100	96	7	-	-
S-21	47' RT	124+00	4.5-6.0	A-3(0)	22	NP	21.2	75.5	1.3	2.0	100	96	4	-	1.5
S-22	28' LT	134+00	0.0-6.0	A-3(0)	16	NP	17.6	81.3	1.1	0.0	100	97	3	-	-

(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)

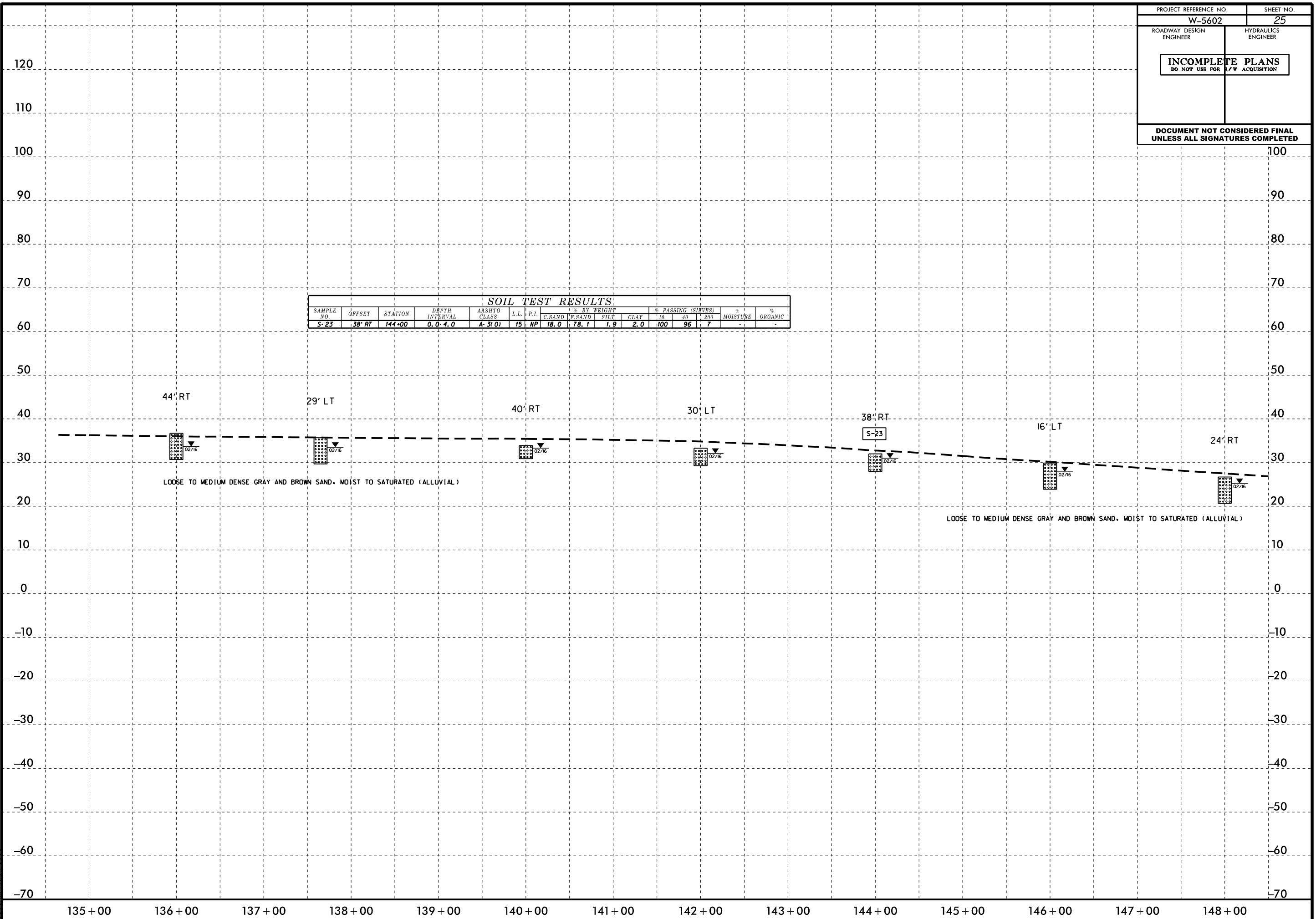
LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

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PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>25</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS															
SAMPLE NO	OFFSET	STATION	DEPTH INTERVAL	ASHFTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-23	38' RT	144+00	0.0-4.0	A-3(0)	15	NP	18.0	78.1	1.9	2.0	100	96	7	-	-



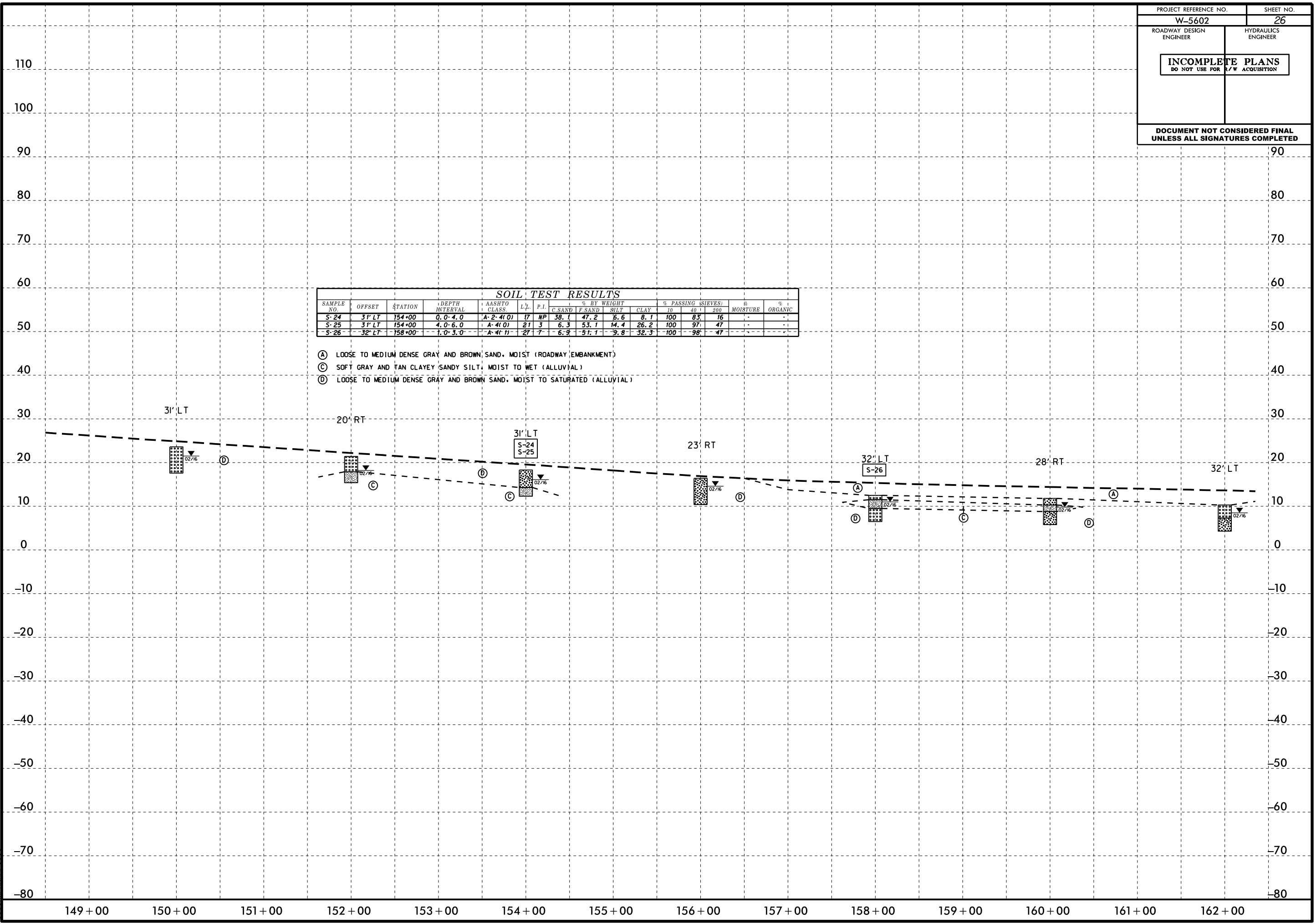
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PROJECT REFERENCE NO. W-5602	SHEET NO. 26
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		MOISTURE	% ORGANIC
							C SAND	F SAND	SILT	CLAY	10	200		
S-24	3' LT	154+00	0.0-4.0	A-2-4(0)	17	NP	38.1	47.2	5.6	8.1	100	83	16	-
S-25	3' LT	154+00	4.0-6.0	A-4(0)	21	3	6.3	53.1	14.4	26.2	100	97	47	-
S-26	32' LT	158+00	1.0-3.0	A-4(1)	27	7	6.9	51.1	9.8	32.3	100	98	47	-

- (A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)
- (C) SOFT GRAY AND TAN CLAYEY SANDY SILT, MOIST TO WET (ALLUVIAL)
- (D) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

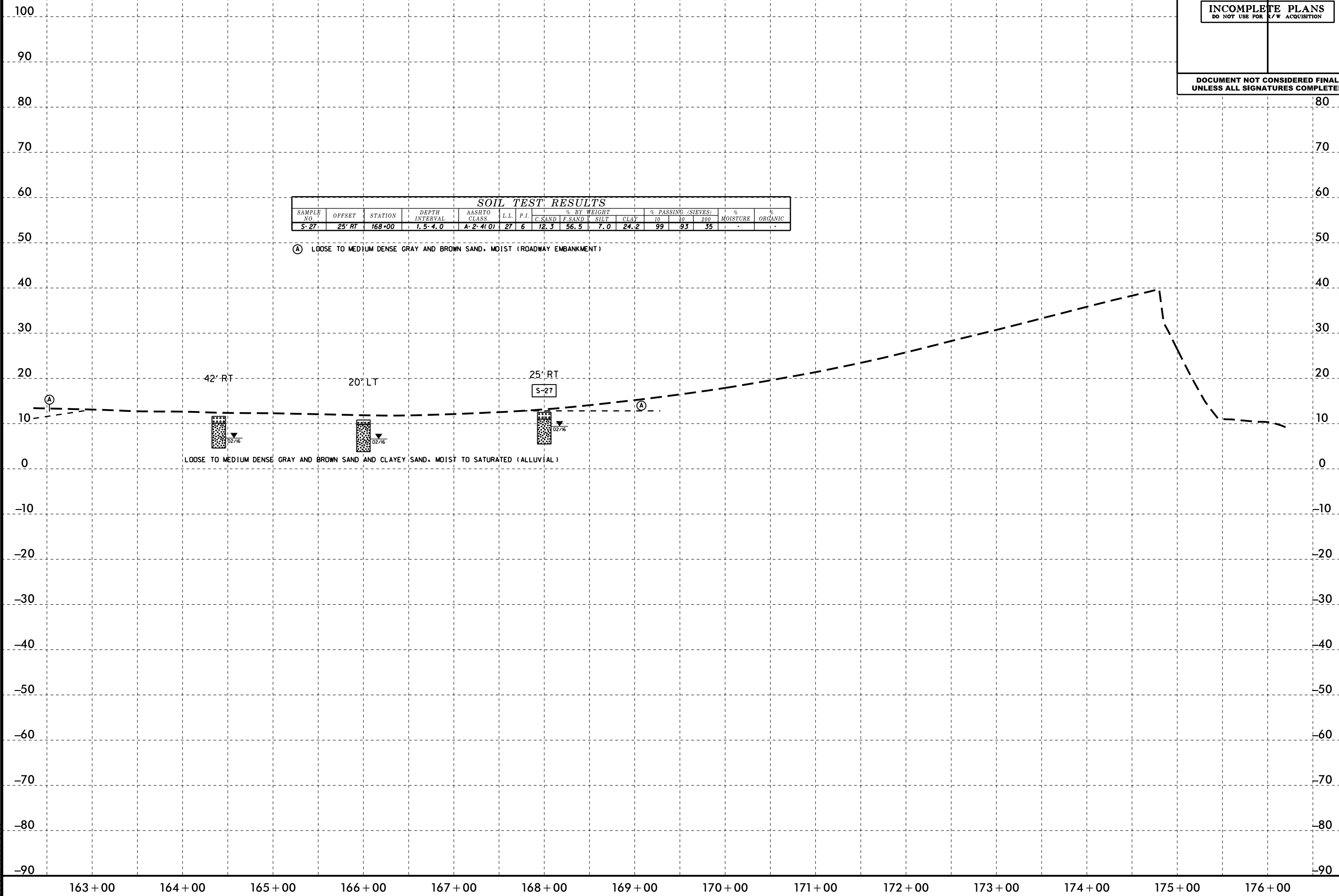


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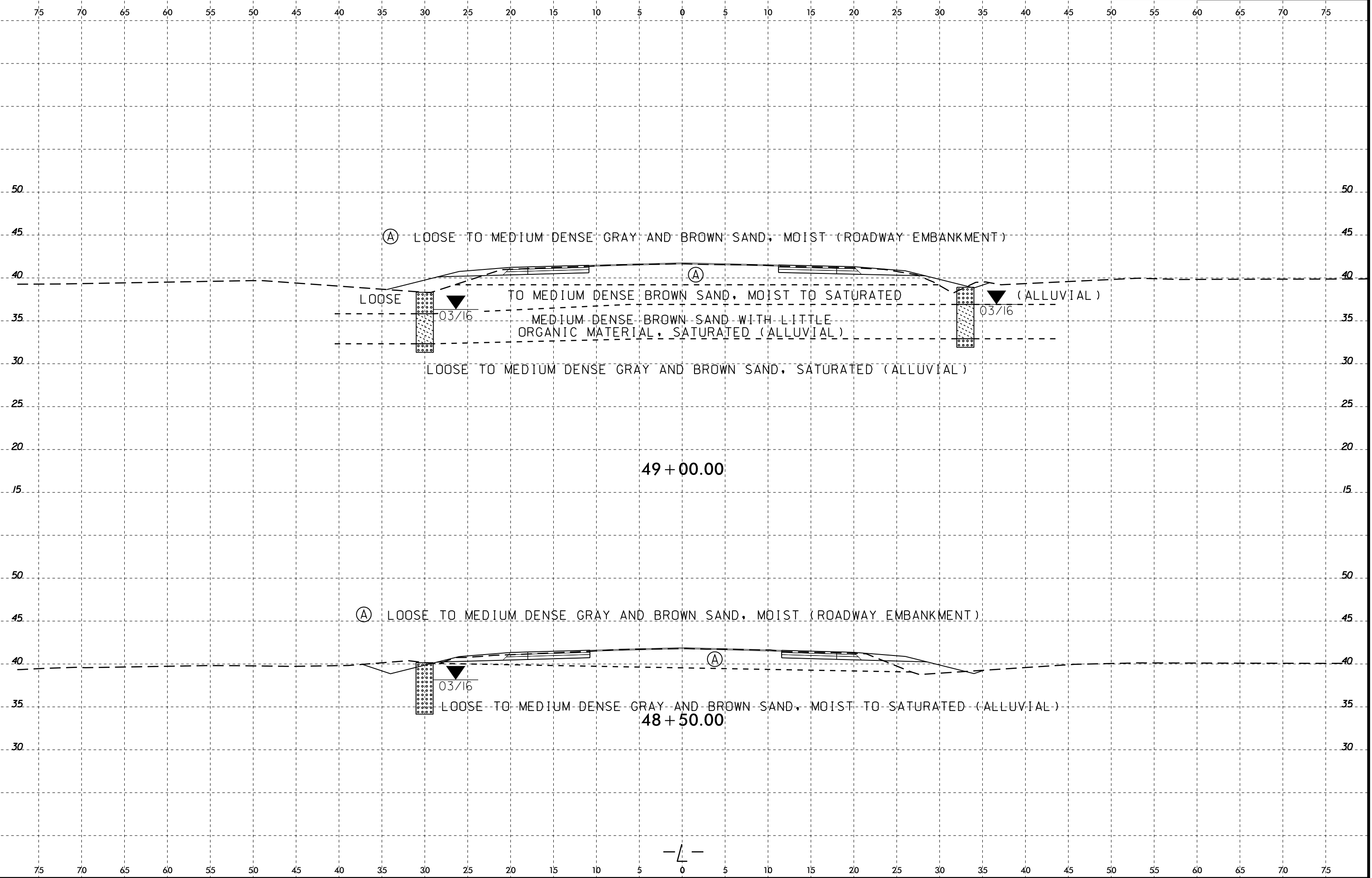
PROJECT REFERENCE NO. <b>W-5602</b>	SHEET NO. <b>27</b>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION	
<b>DOCUMENT NOT CONSIDERED FINAL</b> UNLESS ALL SIGNATURES COMPLETED	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-27	25' RT	168+00	1.5'-4.0'	A-2-A(1)	27	6	12.3	56.5	7.0	24.2	99	93	35	-	-

(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)



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(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)

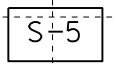


03/16  
LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST TO SATURATED (ALLUVIAL)  
50 + 00.00

### SOIL TEST RESULTS

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-5	30' LT	49+50	0.3- 10.8	A-3(0)	25	NP	13.7	78.3	3.9	4.0	100	97	10	-	4.4

(A) LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, MOIST (ROADWAY EMBANKMENT)



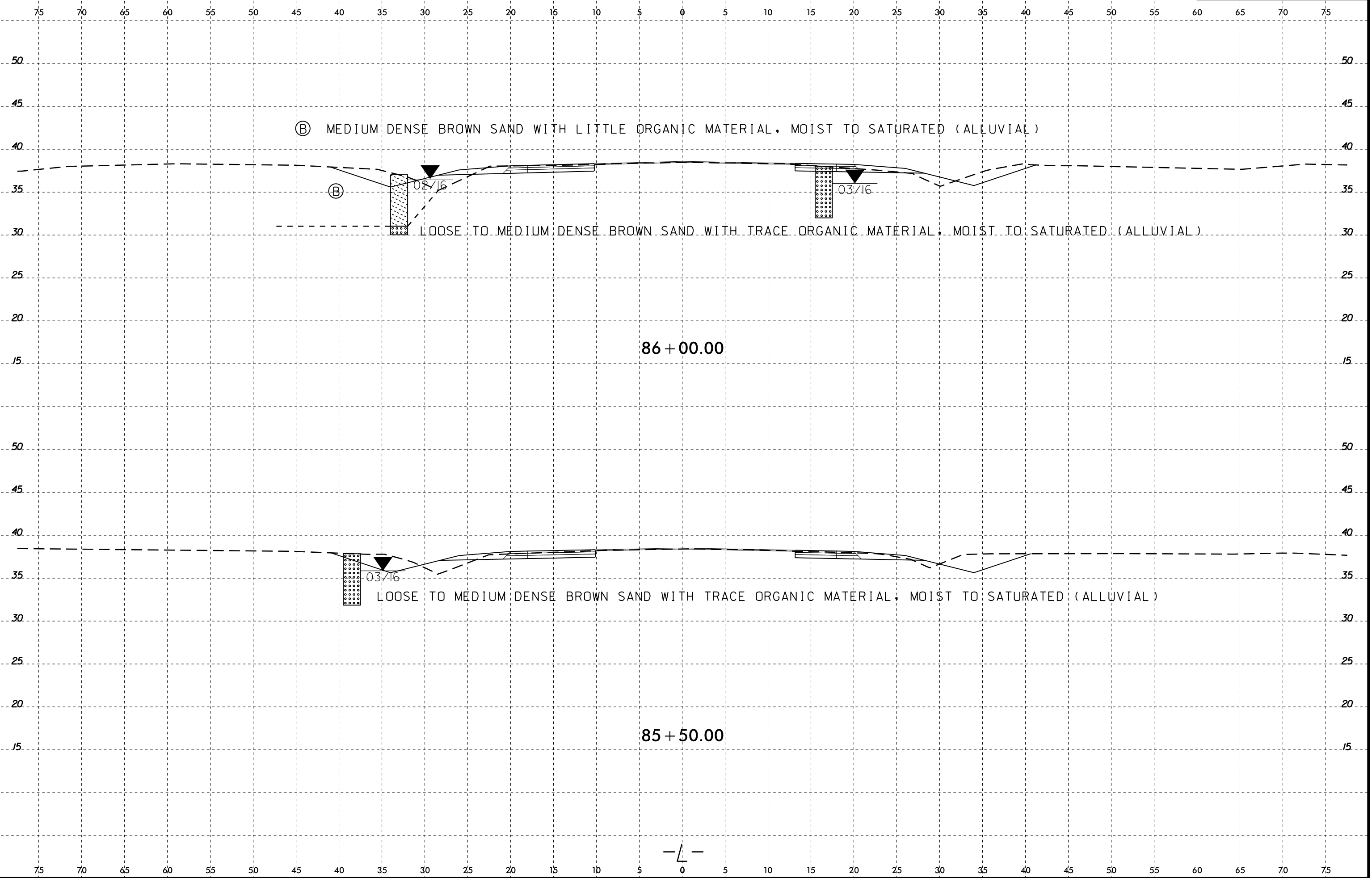
02/16

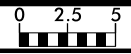
MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

LOOSE TO MEDIUM DENSE GRAY AND BROWN SAND, SATURATED (ALLUVIAL)

49 + 50.00

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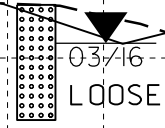
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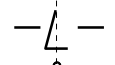
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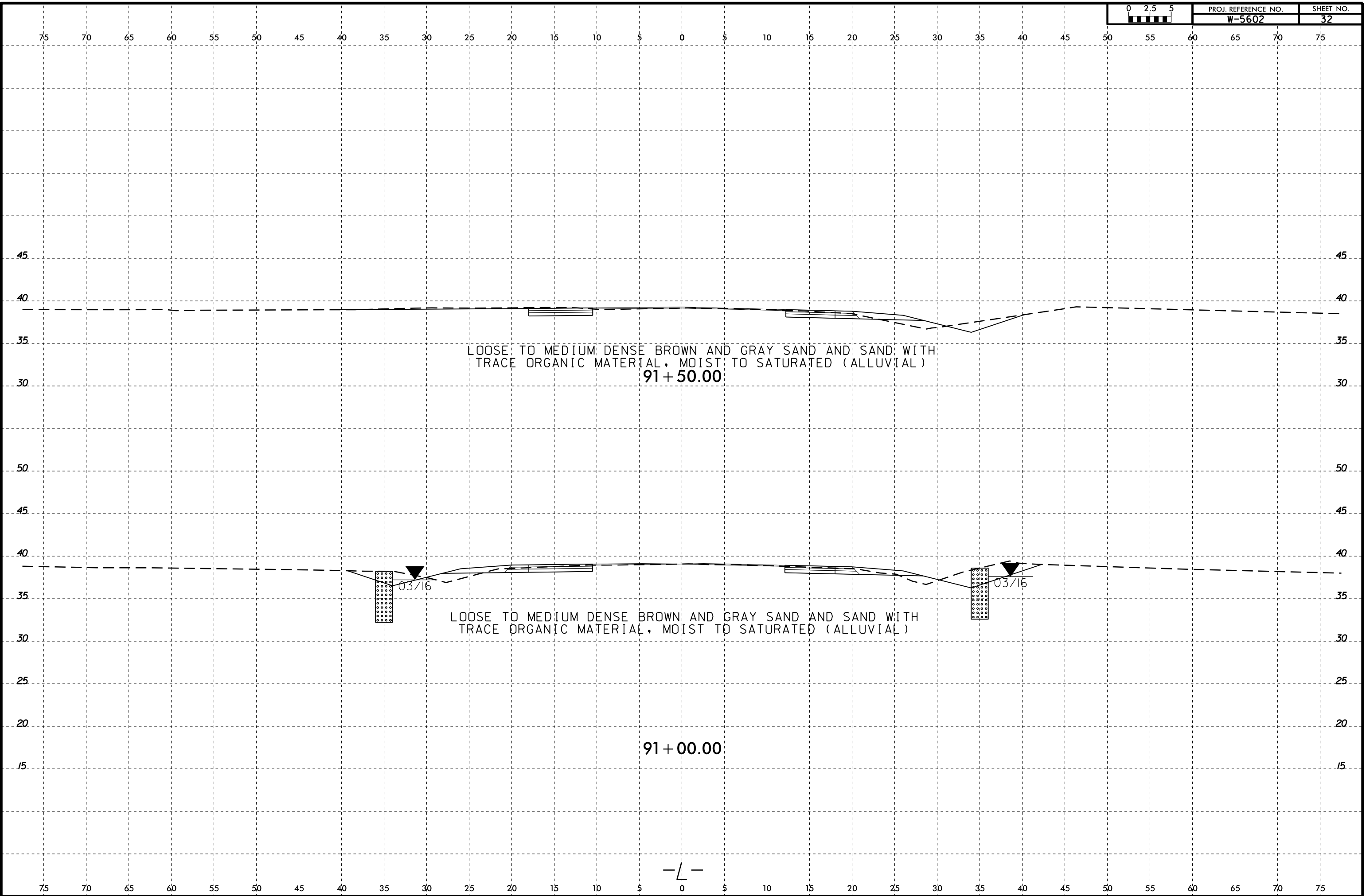
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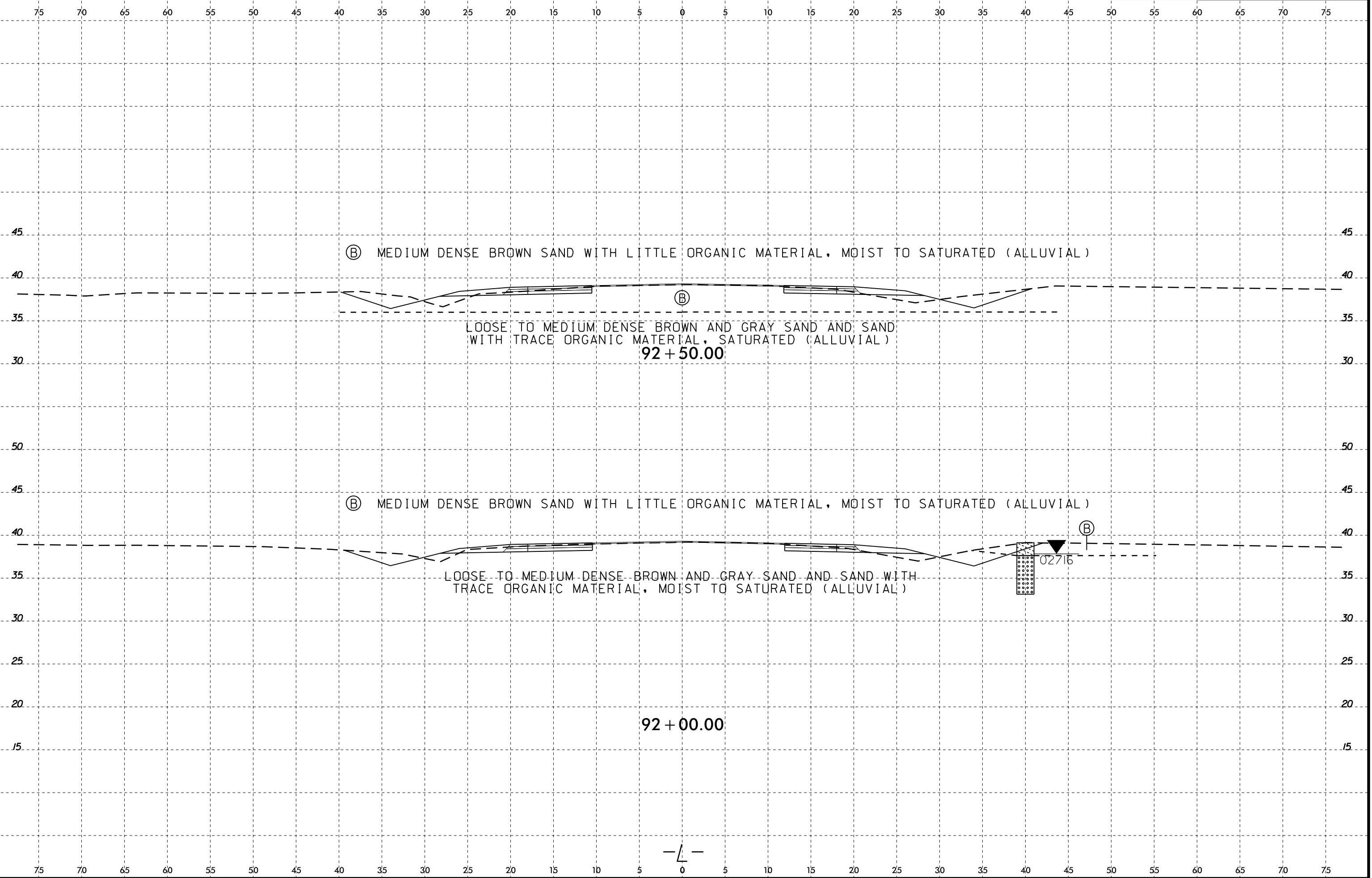
LOOSE TO MEDIUM DENSE BROWN SAND WITH TRACE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

86 + 50.00









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MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

LOOSE TO MEDIUM DENSE BROWN AND GRAY SAND AND SAND WITH TRACE ORGANIC MATERIAL, SATURATED (ALLUVIAL)

93+50.00

30 30

50 50

45 45

ⓑ MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

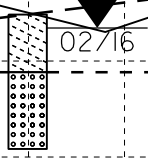
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ⓑ

LOOSE TO MEDIUM DENSE BROWN AND GRAY SAND AND SAND WITH TRACE ORGANIC MATERIAL, SATURATED (ALLUVIAL)

93+00.00

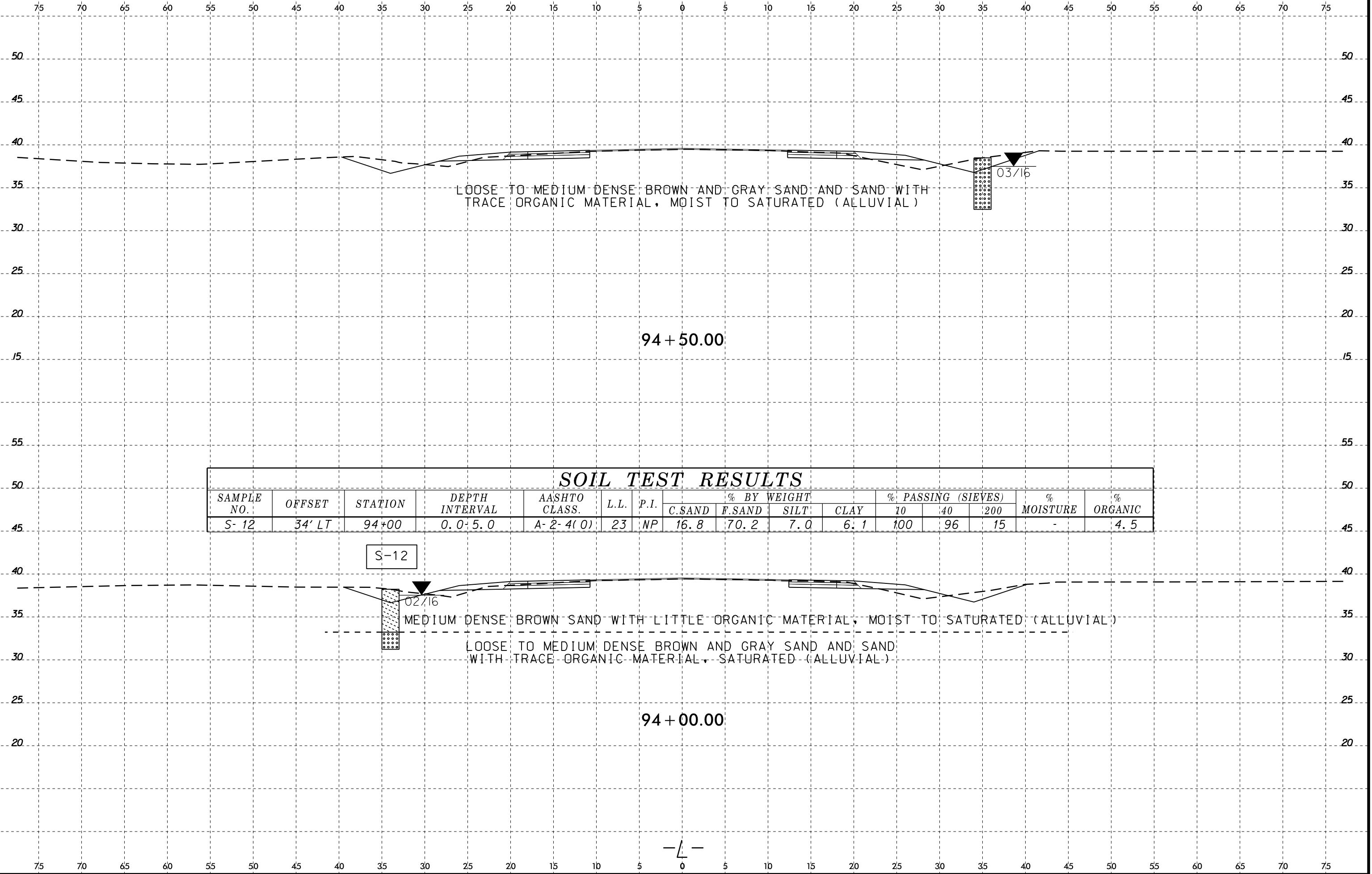
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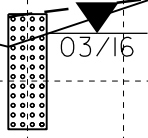
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LOOSE TO MEDIUM DENSE BROWN AND GRAY SAND AND SAND WITH TRACE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)



94 + 50.00

**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-12	34' LT	94+00	0.0-5.0	A-2-4(0)	23	NP	16.8	70.2	7.0	6.1	100	96	15	-	4.5

S-12

02/16

MEDIUM DENSE BROWN SAND WITH LITTLE ORGANIC MATERIAL, MOIST TO SATURATED (ALLUVIAL)

LOOSE TO MEDIUM DENSE BROWN AND GRAY SAND AND SAND WITH TRACE ORGANIC MATERIAL, SATURATED (ALLUVIAL)

94 + 00.00