



ASSUMED LIVE LOAD -----HL-93 OR ALTERNATE LOADING. DESIGN FILL-----MAX. = 16.4' MIN. = 14.1' FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET. 3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER: (SEE ALSO "STAGING DIAGRAM", SHEET C3.

1. FOR STAGE 1A, WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.

2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

3. REPEAT SEQUENCE ABOVE FOR STAGES 1B, 2A, AND 2B.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

MAIN TRANSVERSE STEEL IN THE TOP AND BOTTOM SLABS SHALL BE CONNECTED ALONG THE LONGITUDINAL CONSTRUCTION JOINT EITHER VIA DOWELS OR MECHANICAL COUPLERS. IF ADEQUATE SPACE DOESN'T EXIST DURING STAGING, MECHANICAL COUPLERS HAVE BEEN DETAILED AND SHALL BE USED INSTEAD OF THE DOWELS. MECHANICAL COUPLERS SHALL BE IN ACCORDANCE WITH SECTION 1070-9 OF THE STANDARD SPECIFICATIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC MANAGEMENT PLAN.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE EXISTING STRUCTURE, CONSISTING OF 2 @ 8' X 11.5' RCBC, LENGTH 59'-3" ALONG C/L W/ NATURAL BOTTOM AND LOCATED AT THE PROPOSED STRUCTURE, SHALL BE REMOVED.

THE EXISTING STRUCTURE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING STRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. THE EXISTING STRUCTURE IS PRESENTLY POSTED FOR LOAD LIMIT.

<u>GRADE DATA</u>

| TOTAL STRUC | CTURE QUANTITI | ES |
|---|--|----------------------------|
| REMOVAL OF EXISTING ASBESTOS ASSESSMEN CULVERT EXCAVATION FOUNDATION CONDITI | G STRUCTURE <u>LUMP SUM</u> T <u>LUMP SUM</u> <u>LUMP SUM</u> ONING MATERIAL <u>430</u> | - - _ _TONS |
| CLASS A CONCRETE BARREL & SILLS @_ WINGS ETC. TOTAL | 4.1 CY/FT 815.8 90.7 906.5 | _ C.Y. _ C.Y. _ C.Y. |
| REINFORCING STEEL BARREL & SILLS WINGS ETC | 149,670 6,256 | _LBS. _LBS. |
| TOTAL | 155,926 | _LBS. |



🛛 Boone, NC 828.355.9933

Tri-Cities, TN

DESIGN DISCHARGE FREQUENCY OF DESIGN DISCHARGE DESIGN HIGH WATER ELEVATION DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING OVERTOPPING ELEVATION



REMOVAL OF THE EXISTING STRUCTURE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE STRUCTURE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

EXCAVATE 1'-O" MIN. BENEATH CULVERT & WING FOOTING ELEVATIONS. REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH ARTICLE 414-4 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

| | | | SUMM | ARY | FOR | REIN | FORC | CED (| | RETE BO | X CU | LVER | TS | | | |
|--------|-------------|-------------------|----------------------|---------------------|-----------------------------------|---------------|---|---------------|---------|-----------------|--|---------------|---------|-----------------|--|----------------|
| | | | | | | | | | | STRENGTH | I LIM | IT ST | ATE | | | |
| | | | | | | | | | | MOMENT | | | | SHEAR | | |
| LEVEL | | VEHICLE | WEIGHT (W) (TONS) | CONTROLLING (#) | MINIMUM RATING FACTORS (RF) | TONS = W × RF | LIVE-LOAD FACTORS (Y _{LL}) | RATING FACTOR | BOX NO. | ELEMENT TYPE | DISTANCE FROM LEFT END OF ELEMENT (ft) | RATING FACTOR | BOX NO. | ELEMENT TYPE | DISTANCE FROM LEFT END OF ELEMENT (ft) | COMMENT NUMBER |
| | | HL-93 (INVENTORY) | N⁄A | $\langle 1 \rangle$ | 2.16 | | 1.75 | 2.34 | 1 | TOP SLAB | 5.17 | 2.16 | 1 | EXT.WALL | 1.47 | 1 |
| DESIGN | | HL-93 (OPERATING) | N⁄A | | 2.80 | | 1.35 | 3.04 | 1 | TOP SLAB | 5.17 | 2.80 | 1 | EXT.WALL | 1.47 | |
| RATING | | HS-20 (INVENTORY) | 36.000 | 2 | 2.16 | 77.76 | 1.75 | 2.29 | 1 | TOP SLAB | 5.17 | 2.16 | 1 | EXT.WALL | 1.47 | |
| | | HS-20 (OPERATING) | 36.000 | | 2.80 | 100.80 | 1.35 | 2.96 | 1 | TOP SLAB | 5.17 | 2.80 | 1 | EXT.WALL | 1.47 | |
| | | SNSH | 13.500 | | 2.46 | 33.21 | 1.40 | 6.47 | 1 | EXT.WALL | 6.71 | 2.46 | 1 | EXT.WALL | 1.47 | |
| | | SNGARBS2 | 20.000 | | 2.34 | 46.80 | 1.40 | 5.38 | 1 | TOP SLAB | 5.17 | 2.34 | 1 | EXT.WALL | 1.47 | |
| | ICLE | SNAGRIS2 | 22.000 | | 2.34 | 51.48 | 1.40 | 4.89 | 1 | TOP SLAB | 5.17 | 2.34 | 1 | EXT.WALL | 1.47 | |
| | <pre></pre> | SNCOTTS3 | 27.250 | | 2.70 | 73.58 | 1.40 | 2.87 | 1 | TOP SLAB | 5.17 | 2.70 | 1 | EXT.WALL | 1.47 | |
| | ILE (S | SNAGGRS4 | 34.925 | | 2.45 | 85.57 | 1.40 | 2.63 | 1 | TOP SLAB | 5.17 | 2.45 | 1 | TOP SLAB | 1.43 | |
| | ING | SNS5A | 35.550 | | 2.58 | 91.72 | 1.40 | 2.58 | 1 | TOP SLAB | 5.17 | 2.53 | 1 | TOP SLAB | 1.43 | |
| | | SNS6A | 39.950 | | 2.26 | 90.29 | 1.40 | 2.32 | 1 | TOP SLAB | 5.17 | 2.26 | 1 | TOP SLAB | 1.43 | |
| LEGAL | | SNS7B | 42.000 | | 2.22 | 93.24 | 1.40 | 2.32 | 1 | TOP SLAB | 5.17 | 2.22 | 1 | TOP SLAB | 1.43 | |
| RATING | ER | TNAGRIT3 | 33.000 | | 2.66 | 87.78 | 1.40 | 2.66 | 1 | TOP SLAB | 5.17 | 2.70 | 1 | EXT.WALL | 1.47 | |
| | RAII | TNT4A | 33.075 | | 2.69 | 88.97 | 1.40 | 2.69 | 1 | TOP SLAB | 5.17 | 2.70 | 1 | TOP SLAB | 1.43 | |
| | 1 - T | TNT6A | 41.600 | | 2.27 | 94.43 | 1.40 | 2.35 | 1 | TOP SLAB | 5.17 | 2.27 | 1 | TOP SLAB | 1.43 | |
| | SEN ST) | TNT7A | 42.000 | | 2.25 | 94.50 | 1.40 | 2.37 | 1 | TOP SLAB | 5.17 | 2.25 | 1 | TOP SLAB | 1.43 | |
| | TOR (TT) | TNT7B | 42.000 | | 2.40 | 100.80 | 1.40 | 2.53 | 1 | TOP SLAB | 5.17 | 2.40 | 1 | TOP SLAB | 1.43 | |
| | TRAC | TNAGRIT4 | 43.000 | | 2.17 | 93.31 | 1.40 | 2.24 | 1 | TOP SLAB | 5.17 | 2.17 | 1 | TOP SLAB | 1.43 | |
| | JCK | TNAGRT5A | 45.000 | | 2.37 | 106.65 | 1.40 | 2.44 | 1 | TOP SLAB | 5.17 | 2.37 | 1 | TOP SLAB | 1.43 | |
| | TRL | TNAGRT5B | 45.000 | $\langle 3 \rangle$ | 2.02 | 90.90 | 1.40 | 2.48 | 1 | TOP SLAB | 5.17 | 2.02 | 1 | TOP SLAB | 1.43 | |



LRFR SUMMARY

(LOOKING DOWNSTREAM)

BOX 2

LOAD FACTORS:

| DESIGN LOAD RATING FACTORS | | | | | | | | | |
|----------------------------|---------------|---------------|--|--|--|--|--|--|--|
| LOAD TYPE | MAX FACTOR | MIN FACTOR | | | | | | | |
| DC | 1.25 | 0.90 | | | | | | | |
| DW | 1.50 | 0.65 | | | | | | | |
| ΕV | 1.30 | 0.90 | | | | | | | |
| EH | 1.35 | 0.90 | | | | | | | |
| ES | 1.35 | 0.90 | | | | | | | |
| LS | 1.75 | | | | | | | | |
| WA | 1.00 | | | | | | | | |

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

1. ALL MEASUREMENTS ARE TAKEN FROM THE CENTER OF WALL AND CENTER OF SLAB OF THE BOX NUMBER INDICATED. WALL MEASUREMENTS TAKEN FROM THE CENTER OF BOTTOM SLAB.

(#) CONTROLLING LOAD RATING

 $\left< 1 \right>$ DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$ design load rating (HS-20)

 $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

| | PROJECT NO. <u>R-3825B</u> |
|--|--|
| | JOHNSTON COUNTY |
| | STATION: <u>167+03.00</u> -L- |
| | SHEET 10 OF 10 |
| Scenstuled by Handra Willis | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH |
| 7/17/2018 8:24:14 AM EDT | LRFR SUMMARY FOR Reinforced concrete |
| OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | BOX CULVERIS (Non-interstate traffic) |
| ENG.OF RECORD.: CDB | REVISIONS SHEET NO. |
| BY: MAF DATE: 12/17 | NO. BY: DATE: NO. BY: DATE: C1-10 |
| BY: HLW DATE: 12/17 | 1 3 TOTAL SHEETS 2 4 10 |

CULVERT SECTION NORMAL TO ROADWAY

+

+

(LOOKING DOWNSTREAM)

END ELEVATION

| | | DETAIL | |
|------|--------|--------|--------|
| CONN | ECTION | OF W | 'ING F |
| AND | FLOOR | SLAB | WHEN |
| IS 1 | HICKER | THAN | V FOO |
| | | | |

« M PROJECT NO.: 31740-03

& M PROJECT NO.: 31740-03

| SG.ENG.OF RECORD.: CDB | | | SHEET NO. | | | |
|------------------------|-------|---------|-----------|---------|-------|-------|
| WN RY. MAE DATE. | 12/17 | NO. BY: | DATE: | NO. BY: | DATE: | C1-5 |
| VIII DI IVIAI DATLI | 12/11 | 1 | | গ্ৰ | | TOTAL |
| HKD.BY: HLW DATE: | 12/17 | 2 | | <u></u> | | 10 |

| | | | | | (S | TAGE I | I) | |
|-------------------|-------------|-----|-----|-------|-----|--------|-------|-----------------|
| G.ENG.OF RECORD.: | CDB | | | REVIS | SIO | NS | | SHEET NO. |
| IN RY. MAE | DATE. 12/17 | N0. | BY: | DATE: | NO. | BY: | DATE: | C1-6 |
| | DATE 10/17 | 1 | | | 3 | | | TOTAL SHEETS |
| KD.BI: HLW | DAIE: 12/17 | 2 | | | 4 | | | 10 |

| | | | | | | | | | | | | | | BIL | L C |)FMA | ATER] | IAL - | | | | | | |
|-------------------|--------|---------|-------|----------|-----------|--------|--------|---------|-------------------|----------|-----------|--------|--------|--------|-------|----------|-----------|--------|--------|---------|-------|-----------|----------|------------------------|
| STAGE 1A STAGE 1B | | | | | | | | | STAGE 2A STAGE 2B | | | | | | | | В | | | | | | | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | _ |
| A1 | 378 | 6 | | 8'-2" | 4,637 | A1 | 378 | 6 | | 8'-2" | 4,637 | A1 | 302 | 6 | | 8'-2" | 3,704 | A1 | 302 | 6 | | 8'-2" | 3,704 | - |
| | | | | | | | | | | | | | | | | | | | | | | | | |
| A100 | 374 | 7 | STR. | 14'-6" | 11,085 | A300 | 374 | 7 | STR. | 11'-8" | 8,919 | A500 | 298 | 7 | STR. | 14'-6" | 8,832 | A700 | 298 | 7 | STR. | 11'-8" | 7,106 | _ VERTI |
| A101 | 374 | (| SIR. | 8,-0,, | 6,116 | A 400 | 375 | 7 | STP | 11′-8″ | 89/3 | A501 | 298 | (| SIR. | 8'-0" | 4,873 | 4800 | 299 | 7 | STP | 11′-8″ | 7 1 3 0 | - |
| A200 | 375 | 7 | STR. | 14'-6" | 11,114 | A400 | 515 | | JIN. | | 0,940 | A600 | 299 | 7 | STR. | 14'-6" | 8.862 | A800 | 233 | | JIN. | | 7,130 | - |
| A201 | 375 | 7 | STR. | 8'-0" | 6,132 | | | | | | | A601 | 299 | 7 | STR. | 8'-0" | 4,889 | | | | | | | - |
| | | | | | | | | | | | | | | | | | | | | | | | | |
| B1 | 330 | 4 | STR. | 12'-10" | 2,829 | B1 | 110 | 4 | STR. | 12'-10" | 943 | B1 | 264 | 4 | STR. | 12'-10" | 2,263 | B1 | 88 | 4 | STR. | 12'-10" | 754 | _ |
| B2 | 330 | 4 | SIR. | 3'-4" | (35 | B2 | 110 | 4 | STR. | 3'-4" | 245 | B2 | 264 | 4 | SIR. | 3'-4" | 588 | B2 | 88 | 4 | STR. | 3'-4" | 196 | _ |
| 60 | | | SIR. | 12 -10 | 2,000 | 83 | IIU | | SIR. | 12 -10 | 2,000 | 63 | 00 | 1 | SIR. | 12 -10 | 2,300 | 83 | 00 | 1 | JIK. | 12 -10 | 2,308 | - |
| C1 | 304 | 4 | STR. | 28'-10" | 5,855 | C1 | 216 | 4 | STR. | 28'-10" | 4,160 | C2 | 228 | 4 | STR. | 30'-6" | 4,645 | C2 | 162 | 4 | STR. | 30'-6" | 3,301 | - |
| | | | | | | | | | | | | | | | | | | | | | | | | |
| D1 | 3 | 6 | STR. | 2'-1" | 9 | D2 | 3 | 6 | STR. | 4'-1" | 18 | D1 | 3 | 6 | STR. | 2'-1" | 9 | D2 | 3 | 6 | STR. | 4'-1" | 18 | _ |
| <u> </u> | Q | 5 | стр | 15'-9" | 1 3 1 | C2 | 0 | 5 | стр | 10/-10// | 90 | C1 | | 5 | C T D | 15/_9″ | 65 | C2 | 1 | | | 10/-10// | 15 | |
| 61 | 0 | 5 | JIN. | 15-0 | 151 | 62 | 0 | 5 | SIR. | 10 -10 | 30 | 61 | 4 | 5 | JIN. | 15 -0 | 65 | 62 | 4 | 5 | JIR. | 10 -10 | 45 | - |
| S1 | 12 | 8 | STR. | 15'-8" | 502 | S2 | 12 | 8 | STR. | 10'-10" | 347 | S1 | 12 | 8 | STR. | 15'-8" | 502 | S2 | 12 | 8 | STR. | 10'-10" | 347 | - |
| | | | | | | | | | | | | | | | | | | | | | | | | _ |
| | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | – BAR SIZE |
| | | | | | | | | | | | | | | | | | | | | | | | | - #4 |
| REINFO | DRCING | STEEL | (STAG | E 1A): 5 | 2,030 LB. | REINFO | DRCING | STEEL | (STAGE | E 1B): 3 | l,187 LB. | REINFO | ORCING | STEEL | (STAG | E 2A): 4 | 1,542 LB. | REINFO | DRCING | STEEL | (STAG | E 2B): 24 | ,911 LB. | #5 |
| CLASS | A CON | CRETE (| STAGE | 1A) | | CLASS | A CONC | CRETE (| STAGE : | 1B) | | CLASS | A CON | CRETE | STAGE | 2A) | | CLASS | A CON | CRETE (| STAGE | 2B) | | - [#] 6 #7 |
| C | IVERT | BARREI | | 262.9 | ΓY | | IVERT | 3ARRFI | | 188 7 | CΥ | C | IVERT | RARREI | a | 209.9 | ΓY | | IVERT | RARREI | a | 150 7 | ΓY | |
| SI | LLS: | DANNEL | | 0.5 | С.Ү. | SI | LLS: | | 0 | 1.3 | C.Y. | SI | LLS: | DANNEL | | 0.5 | C.Y. | SI | LLS: | DANNEL | | 1.3 | C.Y. | TOTAL REINFORCING STE |
| L | | | | | | 1 | | | | | | 1 | | | | | | 1 | | | | | | CLASS A CONCRETE |
| | | | | | | | | | | | | | | | | | | | | | | | | CUL |

| | PROJECT NO. <u>R-3825B</u> | | | | | | | |
|---|---|--|--|--|--|--|--|--|
| | JOHNSTON COUNTY | | | | | | | |
| | STATION: <u>167+03.45</u> -L- | | | | | | | |
| | SHEET 8 OF 10 | | | | | | | |
| SpecyAldred W. | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | | | |
| 7/17/2018 8:24:14 AM EDT | DOUBLE 12'X12' CONCRETE BOX CULVERT | | | | | | | |
| DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | ON MILL CREEK | | | | | | | |
| SG.ENG.OF RECORD.: CDB | REVISIONS SHEET NO. | | | | | | | |
| WN.BY: MAF DATE: 12/17 HKD.BY: HLW DATE: 12/17 | NO. BY: DATE: NO. BY: DATE: C1=0 1 3 3 5 5 5 5 5 10 2 4 10 10 10 10 10 10 | | | | | | | |

North Carolina 843.974.5650 828 • 253 • 2796 🛛 Middlesboro, KY Raleigh, NC
 Charlotte, NC
 606·248·6600 919·977·9455 704·357·0488 🗆 Atlanta, GA 770.627.3590 Copyright © 2006 Vaughn & Melton, Inc. All Rights Reserved PROJECT NO. R-3825B JOHNSTON _ COUNTY STATION: 167+03.00 -L-SHEET 9 OF 10 CARO FSSI STATE OF NORTH CAROLINA SEOAIstigneart Hourdy Wil DEPARTMENT OF TRANSPORTATION RALEIGH VGINE WINGS FOR 7/17/2018 8:24:14 AM EDT CONCRETE BOX CULVERT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED H = 12' - 0''SLOPE = 3:1 90° SKEW DSG. ENG. OF RECORD.: CDB SHEET NO. REVISIONS C1-9 DATE: DATE: NO. BY: BY: DATE: 12/17 DWN.BY: MAF TOTAL SHEETS DATE: 12/17 CHKD.BY: HLW 10

865.546.5800

864.574.4775

Spartanburg, SC

🛛 Charleston, SC

Vaughn & Melfon

Consulting Engineers

Asheville,

BAF

| R TYPES | | BIL | L OF | MA | TERIAL | _ |
|--|-------------------------|------------------------|----------|-----|---------------------------------------|---|
| | | F | UR L | | WING | |
| | BAR | NO. | SIZE | | LENGIH | WEIGHI |
| | HI | | #4 | SIR | 28'-2" | 132 |
| | | 3 | #4 | SIR | 24'-4" | 49 |
| | НЗ | 2 | #4 | SIR | 10/11/ | 24 |
| $ \left(\begin{array}{c} 2 \end{array} \right) = \left($ | | 3 | #4 | SIR | 10'-11" | 22 |
| | H5 | 5 | #4 | SIR | 4'-3" | 9 |
| | H6 | 5 | #4 | SIR | 29'-2" | 97 |
| - | H (| 22 | #4 | 1 | 5'-1" | 45 |
| $6'' RAD \longrightarrow 1$ | N 1 1 | 17 | # 6 | 0 | | 117 |
| | | 13 | #6 #F | 2 | 6'-0" | |
| | | 15 | #5 #4 | 2 | 6'-0" | 81 |
| | N S | 15 | # 4 | 2 | 60 | 52 |
| 1'-6'/4" | S1 | 3 | #6 | STR | 6'-0" | 27 |
| | Τ1 | 6 | #4 | STR | 30'-0" | 270 |
| 7'-10″ 9″ | V/1 | 3 | #6 | STR | 12′-1″ | 54 |
| | V2 | 3 | #6 | STR | 11'-٦" | 51 |
| ('-5" 9 " | V3 | 3 | #6 | STR | 10'-6" | 47 |
| 6'-6" 7" | V 4 | 3 | #5 | STR | 9'-8" | 30 |
| 5'-9" 7" | V.5 | 3 | #5 | STR | 8'-10" | 28 |
| | V6 | 3 | #5 | STR | 8'-1" | 25 |
| 5'-1" | V7 | 3 | #4 | STR | 7'-3" | 15 |
| 4′-5″ 6″ | V8 | 3 | #4 | STR | 6'-6" | 13 |
| | V9 | 3 | #4 | STR | 5′-8″ | 11 |
| | V10 | 3 | #4 | STR | 4'-10" | 10 |
| | V11 | 1 | #4 | STR | 12'-6" | 8 |
| | V12 | 2 | #4 | STR | 11'-5" | 15 |
|) нк. | V13 | 2 | #4 | STR | 10'-4" | 14 |
| | V14 | 2 | #4 | STR | 9'-3" | 12 |
| | V15 | 2 | #4 | STR | 8'-2" | 11 |
| TO OUT. | V16 | 2 | #4 | STR | 7'-2" | 10 |
| | V17 | 2 | #4 | STR | 6'-1" | 8 |
| | V18 | 2 | #4 | STR | 5'-0" | 7 |
| 4 WINGS 85.3 CY | Z1 | 1 | #6 | 3 | 8'-7" | 13 |
| 2 END CURTAIN WALLS 5.4 CY | Z2 | 8 | #6 | 3 | 8'-0" | 96 |
| TOTAL 90.7 CY | Z3 | 8 | #5 | 3 | 7'-1" | 59 |
| | Z4 | 8 | #5 | 3 | 6'-4" | 53 |
| | Z5 | 7 | #4 | 3 | 5'-7" | 26 |
| | Z6 | 7 | #4 | 3 | 4'-11" | 23 |
| | REINF FOR 1 (4 RE | FORCIN WING Q'D) | IG STE | EL | 1 | ,564 LBS |
| | | | | | □ Boone 828-3 □ Tri-Cl 423-4 | 9, NC 55 • 9933 †les, TN 67 • 8401 |

SHORING LOCATION NO.1A

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE UTILITY INSTALLATION FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT.LEFT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT. LEFT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS AND GROUNDWATER ELEVATION:

UNIT WEIGHT (γ) = 110 LB/CF FRICTION ANGLE $(\Phi) = 28$ DEGREES COHESION (c) = O LB/SFGROUNDWATER ELEVATION = 157.0 FT. ±

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT.LEFT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL SITE CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

DRIVEN PILING FOR TEMPORARY SHORING FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT. LEFT MAY NOT PENETRATE BELOW ELEVATION 149 FT. DUE TO OBSTRUCTIONS, VERY DENSE OR HARD SOIL, BOULDERS OR WEATHERED OR HARD ROCK.

DO NOT USE A TEMPORARY WALL FOR TEMPORARY SHORING FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT.LEFT.

IT MAY BE PREFERRED TO USE A TEMPORARY SOIL NAIL WALL FOR TEMPORARY SHORING FROM STATION -L- 113+92±, 103.5 FT. LEFT, TO STATION -L- 114+80±, 93.0 FT. LEFT. FOR TEMPORARY SOIL NAIL WALLS, SEE TEMPORARY SOIL NAIL WALLS PROVISION.

SHORING NOTE 5

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS.FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

| OUNDATION CONDITIONING VAL BY ENGINEER. | Boone, NC 828·355·9933 Tri-Cities, TN 423·467·8401 Knoxville, TN 865·546·5800 Spartanburg, SC 864·574·4775 Asheville, Charleston, SC 843·974·5650 Raleigh, NC Charlotte, NC Gobyright © 2006 Vaughn & Melton, Inc. All Rights Reserved |
|---|--|
| JLIC DATA | PROJECT NO. <u>R-3825B</u> |
| = 37 CFS DISCHARGE = 50 YRS EVATION = 166.6 FT = 0.33 SQ MI = 39 CFS ATION = 166.67 FT | <u>JOHNSTON</u> COUNTY STATION: <u>114+23.00</u> -L- Sheet 1 of 7 |
| PING DATA GE = 129 (+) CFS PING = 500 (+) YRS DN = 170.9 FT | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 84" OD WELDED STEEL PIPE W/ RISER STRUCTURE |
| DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | ON UT TO NEUSE RIVER UNDER NC 42 BETWEEN SR 1704 AND SR 2677 |
| SG.ENG.OF RECORD.: CB WN.BY: MAF DATE: 3/18 HKD.BY: HLW DATE: 3/18 | REVISIONSSHEET NO.NO.BY:DATE:NO.C2-113TOTAL SHEETSTOTAL SHEETS247 |

& M PROJECT NO.: 31740-03

| OUTLET HEADWALL OULET EL. 143 | INVERT 5.90 |
|---|---|
| | PROJECT NO. <u>R-3825B</u> |
| | JOHNSTON COUNTY |
| | STATION: <u>114+23.00</u> -L- |
| | SHEET 2 OF 7 |
| Laordar Willie CC287F0F0292461 | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 84"OD WELDED STEEL PTPF W/ RTSFR STRUCTURF |
| 7/17/2018 8:15:56 AM EDT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | ON UT TO NEUSE RIVER UNDER NC 42 BETWEEN SR 1704 AND SR 2677 |
| SG.ENG.OF RECORD.: CB WN.BY: MAF DATE: 3/18 HKD.BY: HLW DATE: 3/18 | REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:C2-21347 |

PLAN VIEW AT PIPE INLET

SKEW TRIANGLE

| ſ | PROJECT NO. <u>R-3825B</u> |
|---|--|
| | JOHNSTON COUNTY |
| | STATION: <u>114+23.00</u> -L- |
| | SHEET 3 OF 7 |
| 7/17/2018 8:15:56 AM EDT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 84" OD WELDED STEEL PIPE W/ RISER STRUCTURE ON UT TO NEUSE RIVER UNDER NC 42 BETWEEN SR 1704 AND SR 2677 |
| SG.ENG.OF RECORD.: CB | REVISIONS SHEET NO. |
| WN. BY: MAFDATE: 3/18HKD. BY: HLWDATE: 3/18 | DATE: NO. BY: DATE: C2 3 1 3 TOTAL SHEETS 7 |

& M PROJECT NO.: 31740-03

| SPLICE LEI | NGTH CHART |
|----------------|-------------------------|
| BAR SIZE | SPLICE LENGTH |
| #4 #5 #6 | 1'-9" 2'-2" 2'-9" |

SECTION A-A (SEE SHEET C2-4 FOR LOCATION)

SECTION B-B

CHANNEL EXCAVATION: =10 CY DOWNSTREAM

BED MATERIALS; Sand, Small to medium rocks, small boulders

> OUTLET BENCH FACING DOWNSTREAM

| | PROJECT NO. <u>R-3825B</u> | | | | | | |
|---|--|--|--|--|--|--|--|
| | JOHNSTON COUNTY | | | | | | |
| | STATION: <u>114+23.00</u> -L- | | | | | | |
| | SHEET 5 OF 7 | | | | | | |
| 7/17/2018 8:15:56 AM EDT DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH 84" OD WELDED STEEL PIPE W/ RISER STRUCTURE ON UT TO NEUSE RIVER UNDER NC 42 BETWEEN SR 1704 AND SR 2677 | | | | | | |
| SG.ENG.OF RECORD.: CB | REVISIONS SHEET NO. | | | | | | |
| WN.BY: MAF DATE: 3/18 | NO. BY: DATE: NO. BY: DATE: U2-5 | | | | | | |
| HKD.BY: HLW DATE: 3/18 | UVSHEETS247 | | | | | | |

NOTE: DETAILS SCALED TO MATCH NCDOT 838.39.

+

+

| | | | | | | | | | BILL OF MATERIAL | | | | | |
|-------|------|------------|------------|------|---------|------------------|-------|-----|------------------|---------|------|------------|---------|---------------|
| | | | | | | | | | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| I | I | I | I | I | | | | | G1 | 6 | #4 | STR | 10'-8" | 43 |
| Z | NG | N7 | N8 N | 6N | N1(| N11 | | | G2 | 8 | #4 | STR | 3'-3" | 17 |
| | - | | · • | • | | | | | G3 | 8 | #4 | STR | 2'-3" | 12 |
| | | | | | | | | | G4 | 12 | #4 | STR | 1'-9" | 14 |
| | | | | | | | | | | | | | | |
| | | | | | | | | | H1 | 12 | #4 | STR | 9'-2" | 73 |
| | /5' | /2 ' | 1/2 " |)//2 | /2 ' | / ₂ " | | | H2 | 4 | #4 | STR | 6'-6" | 17 |
| ת | 0 | 9 | 00 | -10 | 0 | - 2 | | | H3 | 4 | #4 | STR | 3'-0" | 8 |
| · | ω | , <u>_</u> | è | ъ, | വ് | 4 | | | Н4 | 20 | #4 | 1 | 3'-3" | 43 |
| | | | | | | | | | H5 | 4 | #4 | STR | 9'-10" | 26 |
| | | | | | | | | | | | | | | |
| ł | . ↓ | | , t | . ↓ | . ↓ | | 7 | | J1 | 4 | #6 | STR | 6'-7" | 40 |
| | | | | | | | | | J2 | 4 | #6 | STR | 5'-0" | 30 |
| | | | | | | | | | | | | | | |
| | | | | | | | | | N1 | 6 | #5 | 2 | 10'-11" | 68 |
| | | | | | | | | | N2 | 2 | #5 | 2 | 3'-11" | 8 |
| | | | | | | | | | N3 | 2 | #5 | STR | 2'-2" | 5 |
| | | | | | | | | | N4 | 2 | #5 | STR | 1'-4" | 3 |
| | | | | | | | | | N5 | 2 | #5 | STR | 1'-1" | 2 |
| | | | | | | | | | N6 | 4 | #4 | 2 | 10'-2" | 27 |
| | | 4'- | 0″ | | | | 7″. | | N7 | 2 | #4 | 2 | 9'-8" | 13 |
| | | | | | | | • • | | N8 | 2 | #4 | 2 | 8'-10" | 12 |
| | | 4'- | 1″ | | | | 7″ | | N9 | 2 | #4 | 2 | 8'-0" | 11 |
| | | ⊿′ – | 2″ | | | | 7″ | | N10 | 2 | #4 | 2 | ('-2" | 10 |
| | | 1 | <u> </u> | | | | • | | NII | 2 | #4 | 2 | 6'-4" | 8 |
| | | 4'- | 4″ | | | | 7″ | | <u>т</u> 1 | 10 | #5 | <u>STD</u> | 11/ 0// | 115 |
| | | | 0// | | | | 7// | | | 10 マ | #5 | SIR | 15/_10″ | 50 |
| | | 2 -1 | 0 | | | | | | | ך ב | #Q | SIR | 15'-10" | 127 |
| | | | | | | | | | 15 | J | 0 | 311 | 15 10 | 121 |
| | | | | | | | | | /1 | 6 | #⊿ | STR | 8'-3" | रर |
| | / | \frown | \ | | |) | HK. | | V1 V2 | 2 | #⊿ | STR | 1'-3" | 2 |
| | (| ्उ |) | | | | | | V3 | 2 | ±∠ | STR | 2'-2" | <u>्</u> र |
| | | \smile | | | | | | | V4 | 2 | #4 | STR | 1'-4" | 2 |
| | | | | | | | | | V5 | 2 | #4 | STR | 1'-1" | 1 |
| | | | | | | | | | V6 | 4 | #4 | STR | 7'-5" | 20 |
| ТΝ | | | с ст | EEI | | | | | V7 | 4 | #4 | STR | 7'-0" | 19 |
| R | 2 W | ING | S & | OUTI | LET | | 1,067 | LBS | V8 | 4 | #4 | STR | 6'-2" | 16 |
| | | | | | | | | | V9 | 4 | #4 | STR | 5'-3" | 14 |
| | | | | | | | | | V10 | 4 | #4 | STR | 4'-5" | 12 |
| AS | S A | 10 O | NCRE | ΤE | | | | | V11 | 4 | #4 | STR | 3'-7" | 10 |
| | 2 W | ING | S | | | | 8.0 | СҮ | | | | | | |
| | 1 Ol | JTLE | Т | | | | 3.7 | СҮ | Z1 | 22 | #5 | 3 | 3'-5" | 78 |
| | END | CUI | RTAI | N W/ | ALL | | 1.1 | СҮ | Z2 | 6 | #4 | 3 | 4'-11" | 20 |
| | | | | TOT | TAL | | 12.8 | СҮ | Z3 | 6 | #4 | 3 | 4'-9" | 19 |
| | | | | | | | | | Z 4 | 6 | #4 | 3 | 4'-8" | 19 |
| | | | | | | | | | Z5 | 6 | #4 | 3 | 4'-7" | 18 |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | |

| | PROJECT NO. <u>R-3825B</u> | | | | | | | | |
|--|--|--|--|--|--|--|--|--|--|
| | JOHNSTON COUNT | | | | | | | | |
| | STATION: <u>114+23.00</u> -L- | | | | | | | | |
| | SHEET 7 OF 7 | | | | | | | | |
| | STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | | | | |
| CAROLINIA CAROLINA CAROLINA CAROLINA CAROLINA CAROLINA CAROLINIA CAROLINIA CAROL | WING DETAILS | | | | | | | | |
| HORADULLS HORADULLS HORADULLS | H = 8'-0" SLOPE = 2:1 90° SKEW | | | | | | | | |
| 7/17/2018 8:15:56 AM EDT | REVISIONS SHEET NO. | | | | | | | | |
| DOCUMENT NOT CONSIDERED FINAL UNLESS ALL | NO. BY: DATE: NO. BY: DATE: C2-7 | | | | | | | | |
| SIGNATURES COMPLETED | 2 4 7 | | | | | | | | |

DESIGN DATA:

| SPECIFICATIONS | A.A.S.H.T.O. (CURRENT) |
|---|------------------------|
| LIVE LOAD | SEE PLANS |
| IMPACT ALLOWANCE | SEE A.A.S.H.T.O. |
| STRESS IN EXTREME FIBER OF | |
| STRUCTURAL STEEL - AASHTO M270 GRADE 36 - | 20,000 LBS.PER SQ.IN. |
| - AASHTO M270 GRADE 50W - | 27,000 LBS.PER SQ.IN. |
| - AASHTO M270 GRADE 50 - | 27,000 LBS.PER SQ.IN. |
| REINFORCING STEEL IN TENSION | |
| GRADE 60 | 24,000 LBS.PER SQ.IN. |
| CONCRETE IN COMPRESSION | 1,200 LBS.PER SQ.IN. |
| CONCRETE IN SHEAR | SEE A.A.S.H.T.O. |
| STRUCTURAL TIMBER - TREATED OR | |
| UNTREATED - EXTREME FIBER STRESS | 1,800 LBS.PER SQ.IN. |
| COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER | 375 LBS.PER SQ.IN. |
| EQUIVALENT FLUID PRESSURE OF EARTH | 30 LBS.PER CU.FT. |
| | (MINIMUM) |

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

+

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS

LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ "ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

