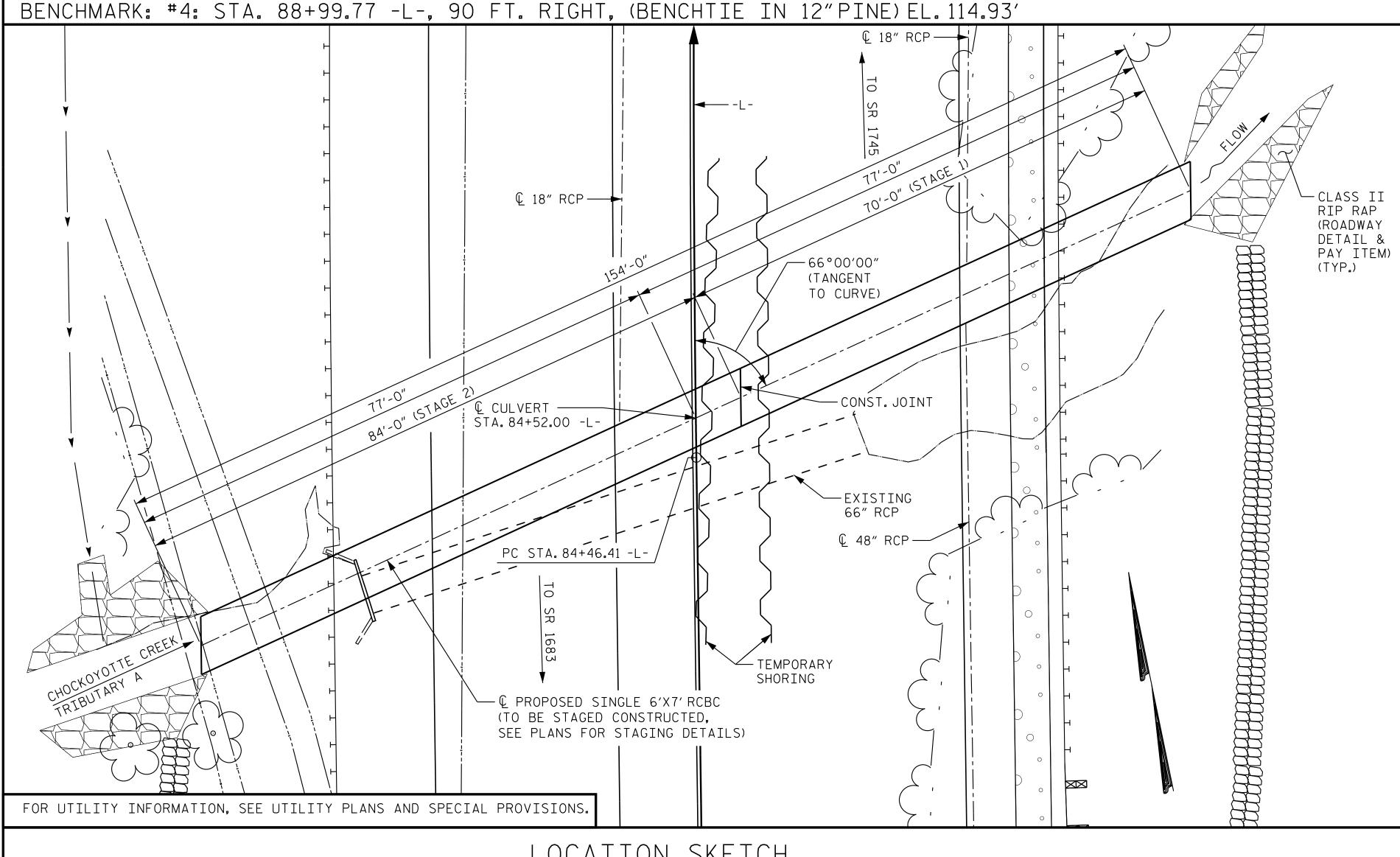
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LOCATION SKETCH

HYDRAULIC DATA

DESIGN DISCHARGE -----360 C.F.S. FREQUENCY OF DESIGN FLOOD -----50 YR. DESIGN HIGH WATER ELEVATION-----110.3 FT. DRAINAGE AREA ------O.87 SQ. MI. BASE DISCHARGE (Q100) -----400 C.F.S. FEMA 100-----348 C.F.S. BASE HIGH WATER ELEVATION -----110.9 FT. FEMA 100-----110.2 FT.

OVERTOPPING FLOOD DATA

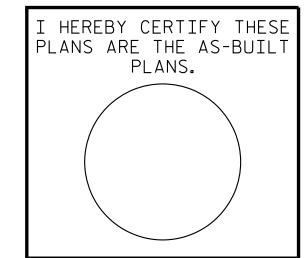
OVERTOPPING DISCHARGE -----550 C.F.S. FREQUENCY OF OVERTOPPING FLOOD ---> 500 YR.

OVERTOPPING FLOOD ELEVATION -----114.31 FT.

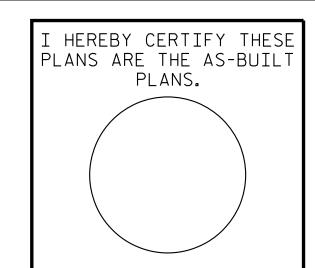
ROADWAY DATA

GRADE POINT ELEV. @ STA 84+52.00 -L- = 113.70' BED ELEVATION @ STA 84+52.00 -L- = 100.90' ROADWAY SLOPES 3:1

TOTAL STRUCTURE QUAN	ITITIES
CLASS A CONCRETE	
STAGE 1	72.2 C.Y.
STAGE 2	81.8 C.Y.
TOTAL	154.0 C.Y.
REINFORCING STEEL	
STAGE 1	8,261 LBS.
STAGE 2	9,187 LBS.
TOTAL	17,448 LBS.
REMOVAL OF EXISTING STRUCTURES	LUMP SUM
CULVERT EXCAVATION	LUMP SUM
FOUNDATION CONDITIONING MATERIAL	
STAGE 1	67 TONS
STAGE 2	56 TONS
TOTAL	123 TONS



SLAB @ & CULVERT -



77'-0" 77'-0" ┽──END PROPOSED CULVERT BEGIN PROPOSED ──── 7′-10″¬ **CULVERT** 20'-4" 36′-10″ 16'-8" 23′-6″ -APPROXIMATE EXISTING EL.101.8 ± — STREAM BED ELEVATION EL. 102.6 ± — EL. 102.1 ± -— EL.102.1 ± EL. 102.0 ± — EL. 100.09 EL.100.90-EL.101.71-EL. 102.1 ± EL. 102.2 ± -TOP OF FLOOR └─ EL.101.6 ±

SEAL 040769

andrew Phillips

5/21/2018

PROFILE ALONG & CULVERT

EL.102.4±

EL.102.0 ± ---

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Raleigh, NC 27601-1772
NC LICENSE #
F-0102

66° SKEW REVISIONS

SHEET NO DATE: NO. BY: DATE: C-1 TOTAL SHEETS

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NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

DESIGN FILL = 6.68 FEET.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN STAGE I CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1. WING FOOTINGS, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB, HEADWALL AND SILL.

CONCRETE IN STAGE II CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1 WING FOOTINGS, CURTAIN WALL AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB, HEADWALL AND SILL.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

THE 18" Ø R.C. PIPES AND 48" Ø R.C. PIPE THROUGH THE SIDEWALLS OF THE CULVERT SHALL BE LOCATED BY THE ENGINEER. THE REINFORCING STEEL SHALL BE FIELD BENT AS NECESSARY TO CLEAR PIPES.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING 66" Ø REINFORCED CONCRETE PIPE LOCATED AT THE SAME LOCATION AS THE PROPOSED CULVERT SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE STRUCTURE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF THE EXTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION, HE MAY SUBMIT TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT. SEE SPECIAL PROVISIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

TRAFFIC ON NC 125 SHALL BE MAINTAINED. IN ORDER TO MAINTAIN TRAFFIC THE CULVERT SHALL BE CONSTRUCTED IN SECTIONS AS SHOWN ON THESE PLANS AND/OR AS DIRECTED BY THE ENGINEER.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FEET. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

THIS STRUCTURE IS LOCATED IN SEISMIC ZONE 1.

EL. 101.2 ±

SEE SECTION 414 OF THE STANDARD SPECIFICATIONS FOR CULVERT EXCAVATION AND BACKFILLING.

EXCAVATE AT LEAST 1 FOOT BELOW THE CULVERT AND FOOTINGS, AND REPLACE THE EXCAVATED MATERIAL WITH CLASS VI SELECT MATERIAL MEETING THE REQUIREMENTS OF SECTION 1016 OF THE STANDARD SPECIFICATIONS.

BACKFILL WING WALLS WITH CLASS II OR BETTER SELECT MATERIAL MEETING THE REQUIREMENTS OF SECTION 1016 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. U-5725 HALIFAX COUNTY STATION: 84+52.00 -L-

SHEET 1 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

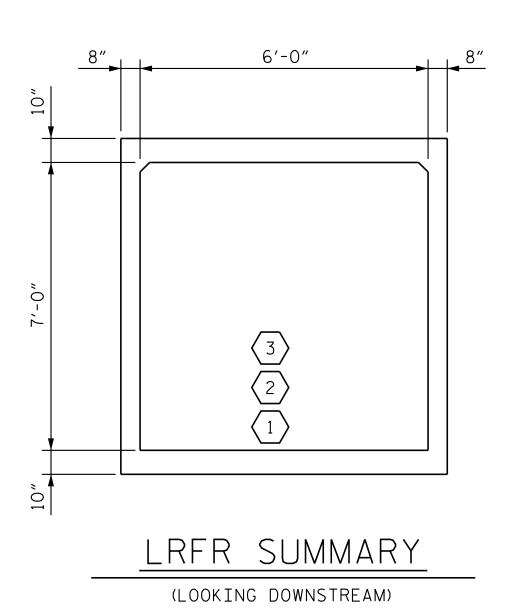
SINGLE 6 FT. X 7 FT. CONCRETE BOX CULVERT

BY:

DRAWN BY: P.G. ROBBS DATE: 5/18 DATE: 5/18 CHECKED BY: <u>C.T.POOLE</u> DESIGN ENGINEER OF RECORD: <u>A.L.PHILLIPS</u> DATE: <u>5/18</u>

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

							STRENGTH I LIMIT STATE									
							MOMENT SHEAR				SHEAR					
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	EL EMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.25		1.75	1.25	1	BOTTOM SLAB	3.67	3.55	1	BOTTOM SLAB	0.83	
DESIGN LOAD		HL-93 (OPERATING)	N/A	٠	1.62		1.35	1.62	1	BOTTOM SLAB	3.67	4.61	1	BOTTOM SLAB	0.83	
RATING		HS-20 (INVENTORY)	36.000	2	1.40	50.40	1.75	1.40	1	BOTTOM SLAB	3.67	3.99	1	BOTTOM SLAB	0.83	
	_	HS-20 (OPERATING)	36.000		1.82	65 . 52	1.35	1.82	1	BOTTOM SLAB	3.67	5.17	1	BOTTOM SLAB	0.83	
		SNSH	13.500		2.28	30.78	1.40	2.28	1	BOT CORNER WALL	0.33	7.26	1	BOTTOM SLAB	0.83	
	Ш	SNGARBS2	20.000		2.20	44.00	1.40	2.20	1	BOT CORNER WALL	0.33	6.79	1	BOTTOM SLAB	0.83	
	ICL	SNAGRIS2	22.000		2.28	50.16	1.40	2.28	1	BOT CORNER WALL	0.33	7.26	1	BOTTOM SLAB	0.83	
	VEHICLE V)	SNCOTTS3	27.250	(3)	1.55	42.24	1.40	1.55	1	BOTTOM SLAB	3.67	4.45	1	BOTTOM SLAB	0.83	
	SLE (S	SNAGGRS4	34.925		1.66	57.98	1.40	1.66	1	BOTTOM SLAB	3.67	4.70	1	BOTTOM SLAB	0.83	
LEGAL	SINGLE (S	SNS5A	35.550		1.67	59.37	1.40	1.67	1	BOTTOM SLAB	3.67	4.73	1	BOTTOM SLAB	0.83	
LOAD RATING		SNS6A	39.950		1.67	66.72	1.40	1.67	1	BOTTOM SLAB	3.67	4.73	1	BOTTOM SLAB	0.83	
KATING		SNS7B	42.000		1.67	70.14	1.40	1.67	1	BOTTOM SLAB	3.67	4.73	1	BOTTOM SLAB	0.83	
		TNAGRIT3	33.000		2.27	74.91	1.40	2.27	1	BOT CORNER WALL	0.33	7.25	1	BOTTOM SLAB	0.83	
		TNT4A	33.075		1.84	60.86	1.40	1.84	1	BOTTOM SLAB	3.67	5.30	1	BOTTOM SLAB	0.83	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNT6A	41.600		1.68	69.89	1.40	1.68	1	BOTTOM SLAB	3.67	4.75	1	BOTTOM SLAB	0.83	
	TRA TRA: TST)	TNT7A	42.000		1.77	74.34	1.40	1.77	1	BOTTOM SLAB	3.67	5.02	1	BOTTOM SLAB	0.83	
	UCK (T	TNT7B	42.000		1.67	70.14	1.40	1.67	1	BOTTOM SLAB	3.67	4.73	1	BOTTOM SLAB	0.83	
	TR SE	TNAGRIT4	43.000		1.84	79.12	1.40	1.84	1	BOTTOM SLAB	3.67	5.30	1	BOTTOM SLAB	0.83	
		TNAGT5A	45.000	•	1.84	82.80	1.40	1.84	1	BOTTOM SLAB	3.67	5.30	1	BOTTOM SLAB	0.83	
		TNAGT5B	45.000	•	1.84	82.80	1.40	1.84	1	BOTTOM SLAB	3.67	5.30	1	BOTTOM SLAB	0.83	



DRAWN BY: P.G. ROBBS
CHECKED BY: C.T. POOLE DATE: 5/18 CHECKED BY: C.T. POOLE

DATE: 5/18

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS

DATE: 5/18

UNLESS ALL SIGNATURES COMPLETED

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR		
DC	1.25	0.90		
DW	1.50	0.65		
EV	1.30	0.90		
EH	1.35	0.90		
ES	1.35	0.90		
LS	1.75			
WA	1.00			

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

* * SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-5725

HALIFAX

STATION: 84+52.00 -L-

COUNTY

SHEET 2 OF 8

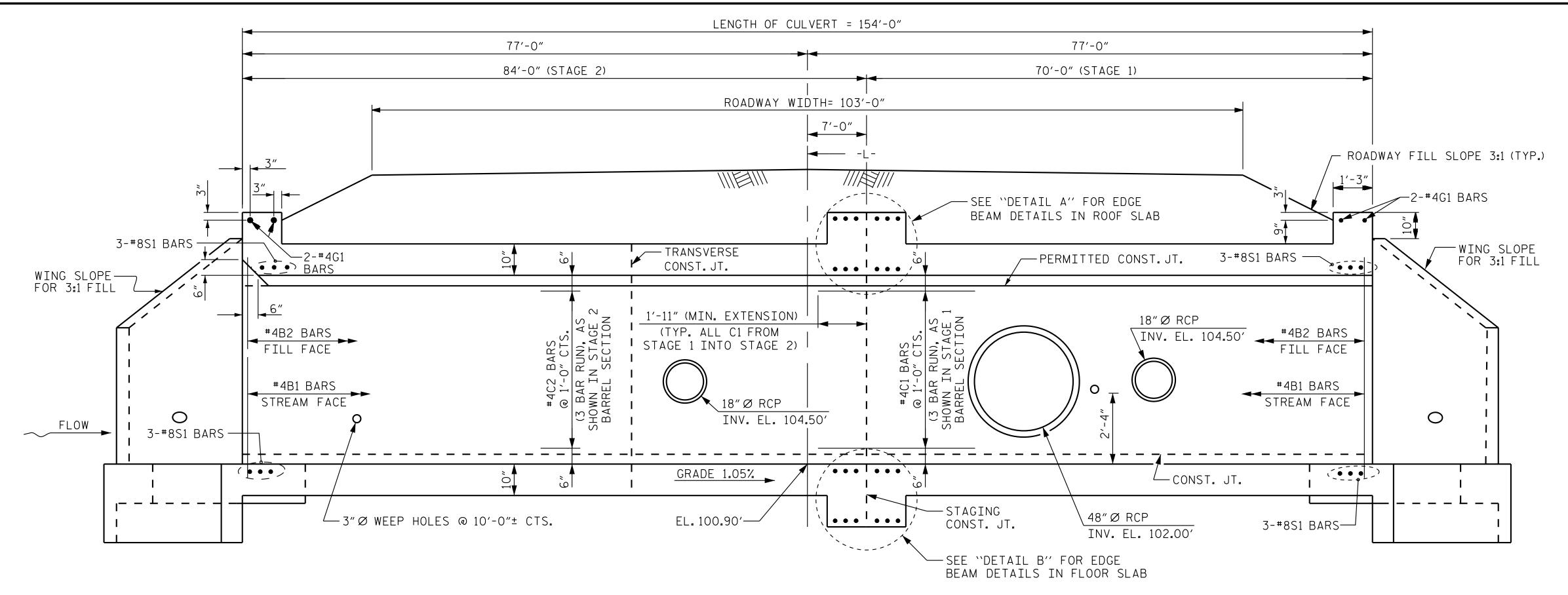
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

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andrew Phillips

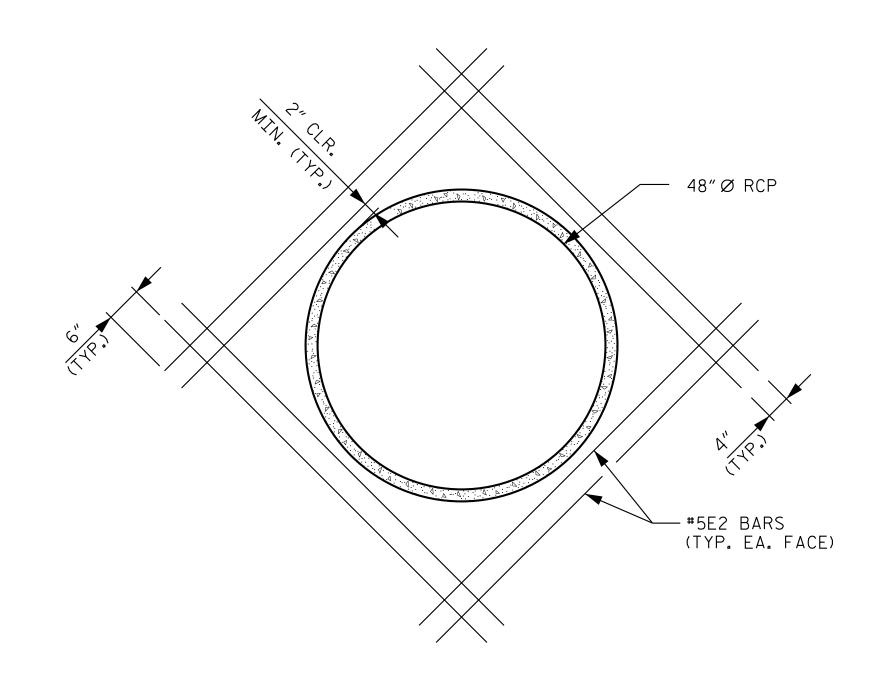
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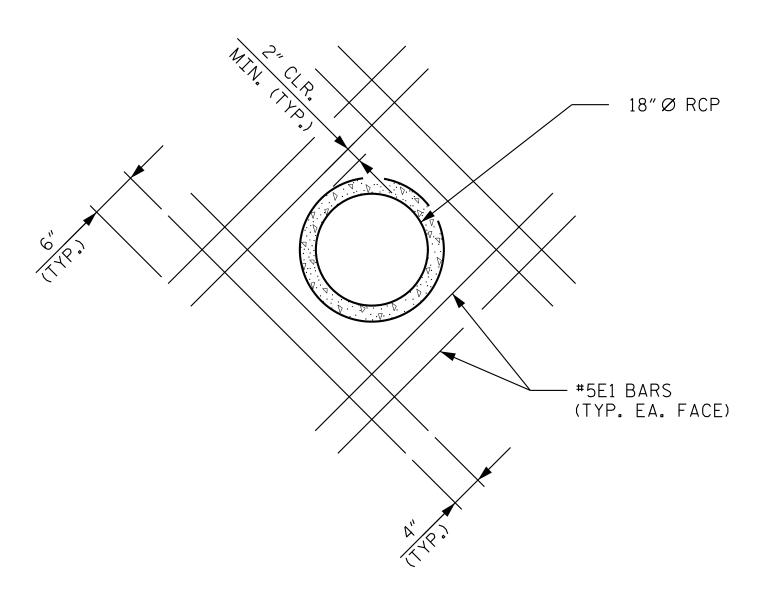
CULVERT SECTION NORMAL TO ROADWAY

FOR APPROXIMATE PLAN VIEW LOCATIONS OF R.C. PIPES, SEE SHEET C1.

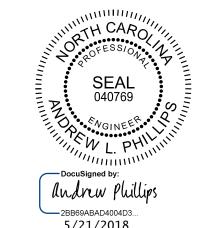


DETAIL OF REINFORCING AROUND 48"DIA.RCP

E2 BARS MAYBE SHIFTED SLIGHTLY AS NECESSARY TO MAINTAIN CLEARANCES AND AVOID "A" BARS.



DETAIL OF REINFORCING AROUND
18"DIA.RCP



Kimley >>> Horn

421 Fayetteville Street, Suite 600

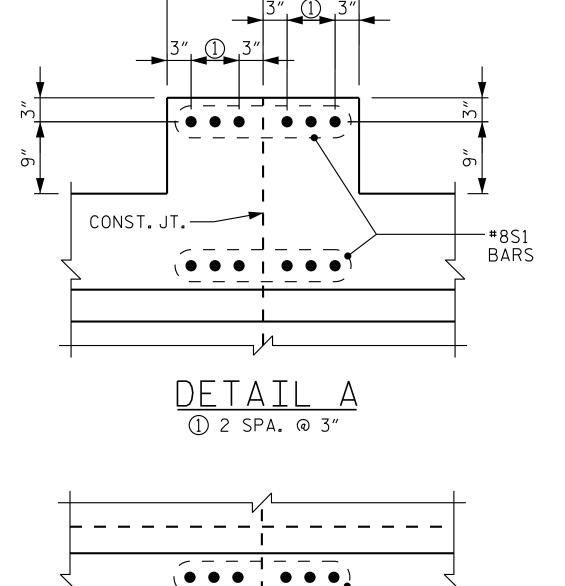
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F-0102

Recomment, together with the concepts and designs presented herein, as an appeal of services intended only for the specific purposed and client for

CONCRETE BO
66° S

REVISIONS

NO. BY: DATE: NO. BY:



DETAIL B

① 2 SPA. @ 3"

 $\tau \bullet \bullet \bullet ! \bullet \bullet \bullet$

CONST.JT.

PROJECT NO. U-5725

HALIFAX COUNTY

STATION: 84+52.00 -L-

BARS

SHEET 3 OF 8

DEPARTMENT OF TRANSPORTATION
RALEIGH
SINGLE 6 FT. X 7 FT.

STATE OF NORTH CAROLINA

CONCRETE BOX CULVERT

66° SKEW

REVISIONS

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TOTAL SHEETS

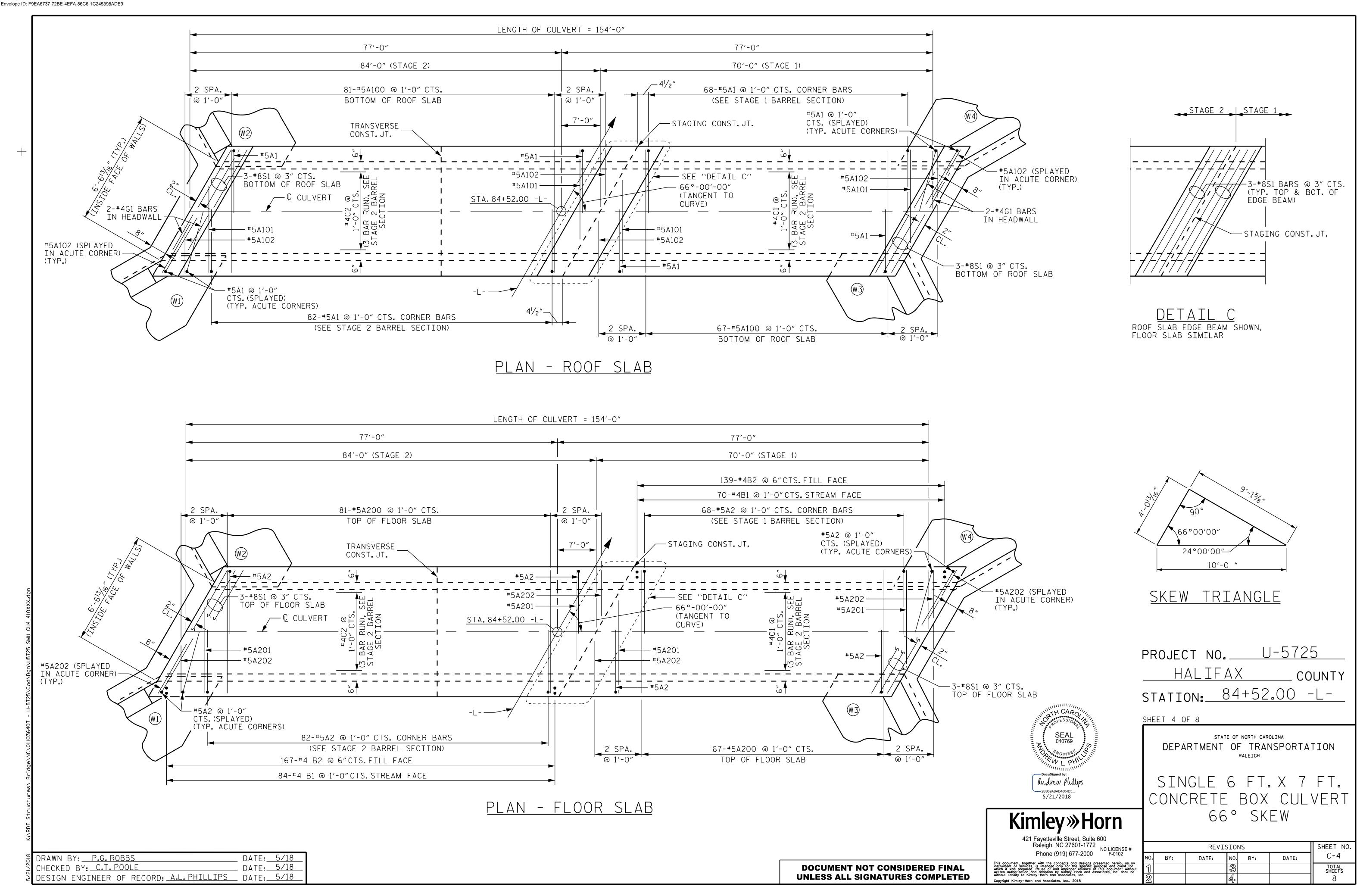
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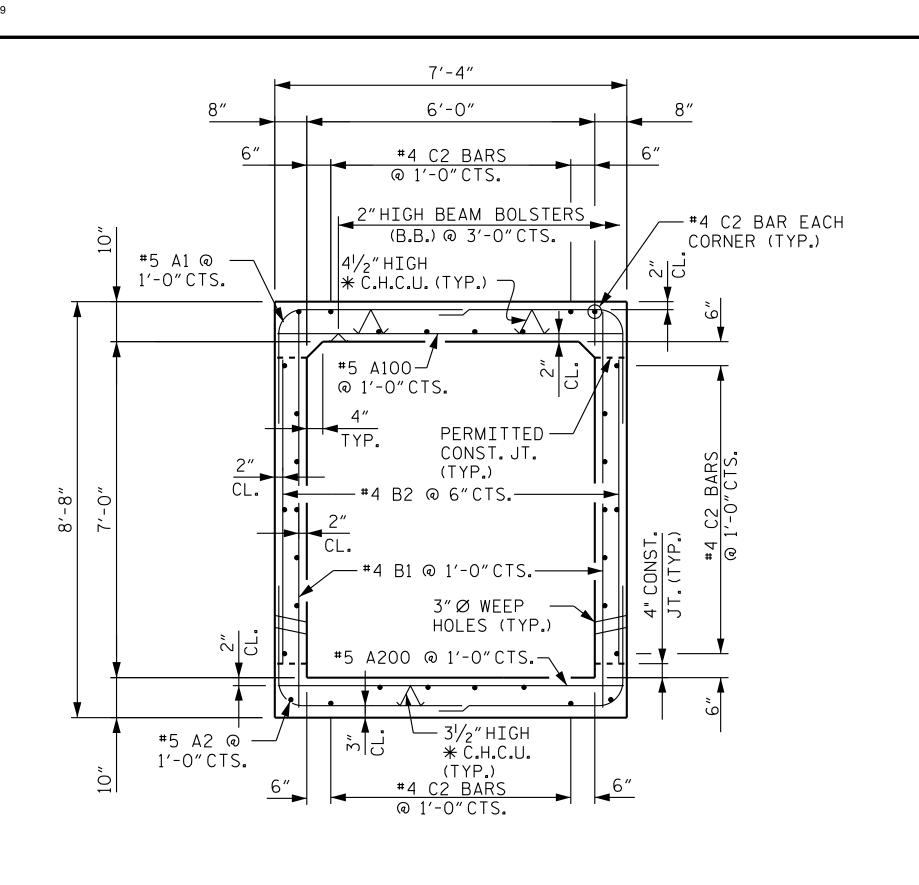
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DESIGN ENGINEER OF RECORD: A.L.PHILLIPS DATE: 5/18

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STAGE 2 RIGHT ANGLE SECTIONS OF BARREL

* ALL CONTINUOUS HIGH CHAIR UPPER (C.H.C.U.) @ 3'-0"CTS. THERE ARE 32 C2 BARS (3 BAR RUN) IN SECTION OF BARREL

CHECKED BY: C.T. POOLE

DATE: 5/18

DESIGN ENGINEER OF RECORD: A.L. PHILLIPS

DATE: 5/18

CHECKED BY: C.T. POOLE

-* #6 D1 DOWEL (TYP.) @ 4'-0" MAX. CTS. -2 LAYERS OF 30 LB ROOFING FELT TO PREVENT BOND (TYP.)

SECTION A-A

* DOWELS MAY BE PUSHED INTO GREEN CONCRETE AFTER SLAB HAS BEEN FLOAT FINISHED.

A201	2	#5	STR	5′-4″	11	
A202	3	#5	STR	3'-1"	10	
B1	168	#4	STR	8'-3"	926	
B2	334	#4	STR	6'-4"	1413	
C2	96	#4	STR	29'-2"	1870	
D1	3	#6	STR	1′-5″	6	
E1	16	#5	STR	3'-11"	65	
G1	2	#4	STR	7'-7"	10	
S1	18	#8	STR	7'-7"	364	
STAGE 2						
REINFORCING STEEL 8,360 LBS						
•						

BAR

Α2

A100

A101

A102

A200

168

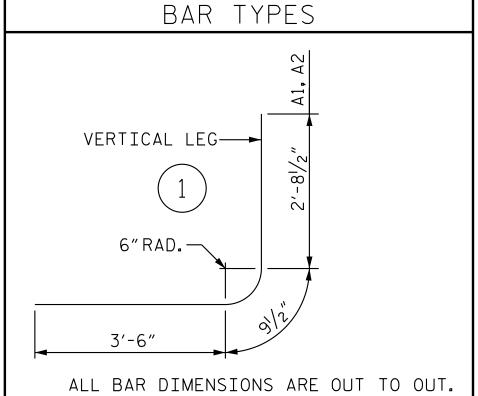
81

STAGE 2 QUANTITIES CLASS A CONCRETE

BARREL @ 0.802 CY/FT	67.4 CY
WINGS, ETC.	13.6 CY
SILL	0.2 CY
EDGE BEAMS	0.6 CY
TOTAL	81 . 8 CY

REINFORCING STEEL

BARREL, SILLS, & EDGE BEAMS 8,360 LBS 827 LBS WINGS, ETC. 9,187 LBS TOTAL



OF MATERIAL STAGE 2

#5 | 1 | 7'-1"

#5 | STR | 7'-0"

#5 STR 5'-4"

#5 STR 3'-1"

| #5 | STR | 7'-0" | 591

#5 1

| SIZE | TYPE | LENGTH | WEIGHT

7'-1"

1241

1241

11

10

SPLICE LENGTHS

BAR SIZE LENGTH #4 1'-11" C2 #4 1'-11"

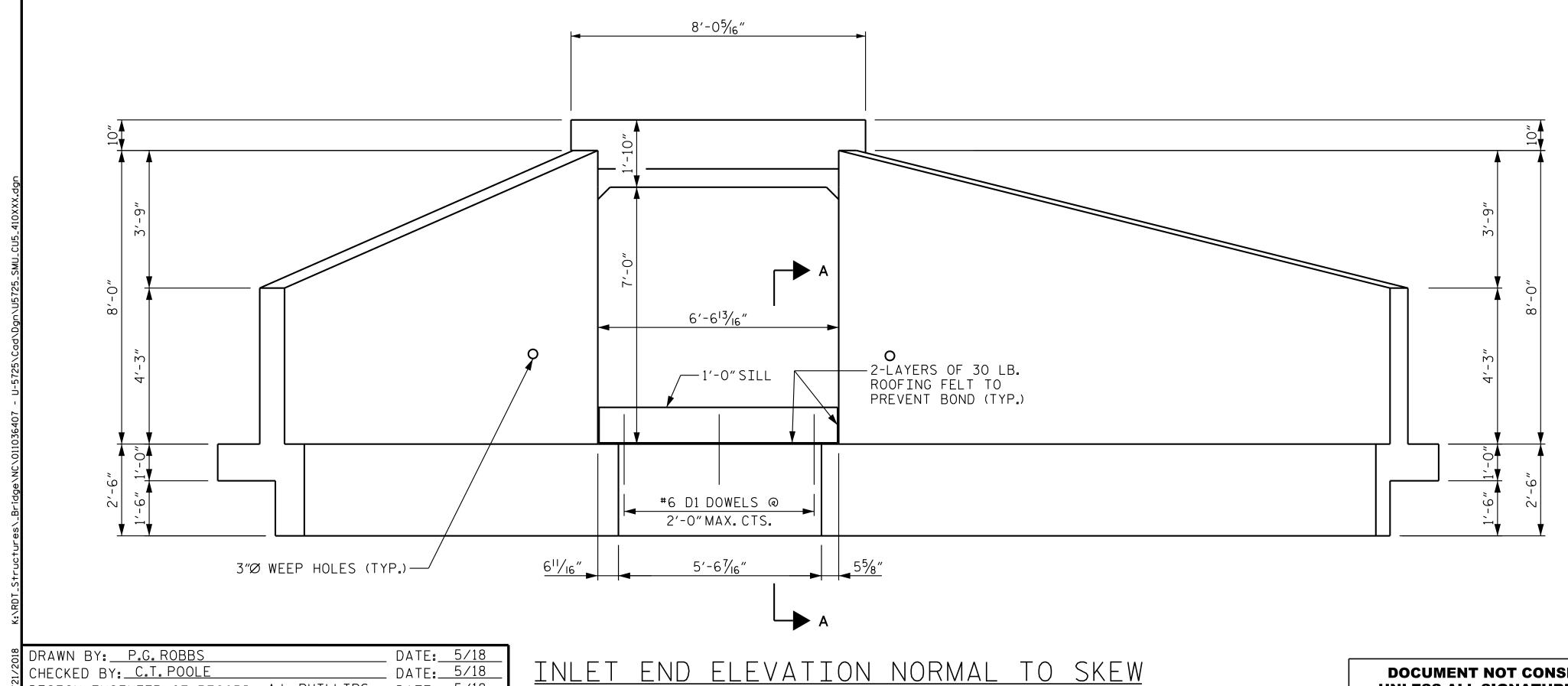
PROJECT NO. U-5725 HALIFAX COUNTY STATION: 84+52.00 -L-

SHEET 5 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SINGLE 6 FT.X 7 FT. CONCRETE BOX CULVERT 66° SKEW

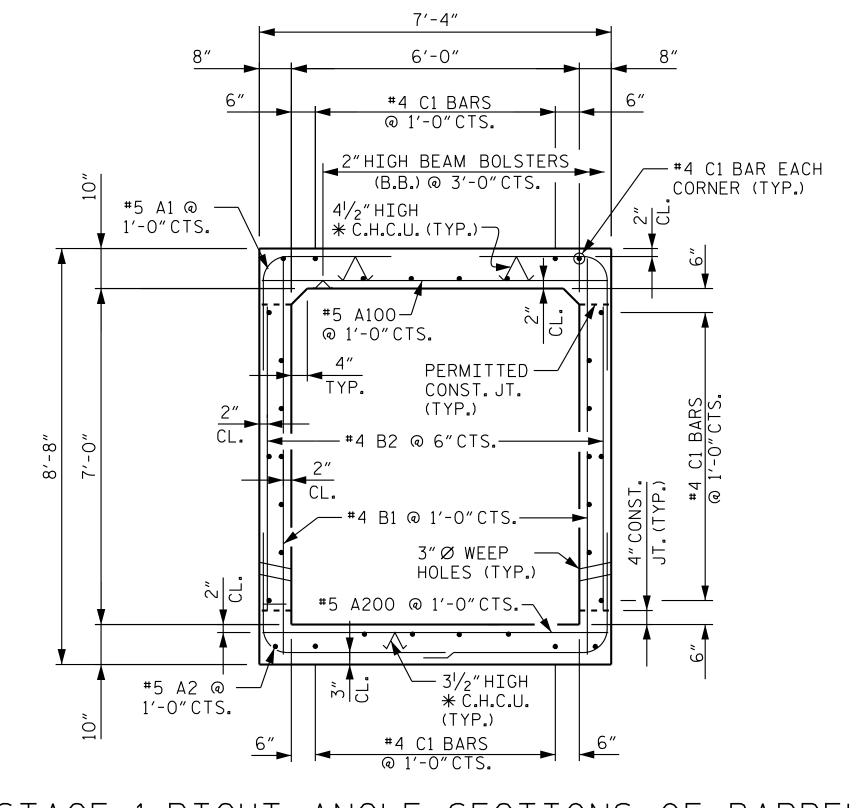
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<u>OUTLET END ELEVATION NORMAL TO SKEW</u>

* ALL CONTINUOUS HIGH CHAIR UPPER (C.H.C.U.) @ 3'-0"CTS. THERE ARE 32 C1 BARS (3 BAR RUN) IN SECTION OF BARREL

WINGS, ETC. 8′-05/₁₆″ TOTAL 6′-6¹³/₁₆" -2-LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND (TYP.) 1'-0" SILL #6 D1 DOWELS @ 2'-0" MAX. CTS. 3"Ø WEEP HOLES (TYP.)─ 4′-10¹⁵/₁₆ 6¹¹/₁₆" 1'-1[|]/₄

BII	_L OF	МАТ	ERIA	AL STA	GE 1			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
Α1	140	#5	1	7'-1"	1034			
Α2	140	#5	1	7'-1"	1034			
A100	67	#5	STR	7′-0″	489			
A101	2	#5	STR	5′-4″	11			
A102	3	#5	STR	3'-1"	10			
A200	67	#5	STR	7′-0″	489			
A201	2	#5	STR	5′-4″	11			
A202	3	#5	STR	3'-1"	10			
B1	140	#4	STR	8'-3"	772			
B2	278	#4	STR	6′-4″	1176			
C1	96	#4	STR	25′-3″	1619			
D1	3	#6	STR	1'-5"	6			
E1	16	#5	STR	3'-11"	65			
E2	16	#5	STR	6′-10″	114			
G1	2	#4	STR	7′-7″	10			
S1	18	#8	STR	7′-7″	364			
STAG	STAGE 1 REINFORCING STEEL 7,214 LBS							

STAGE 1 QUANTIT	IES
CLASS A CONCRETE	
BARREL 0.802 X CY/FT	56.2 CY
WINGS, ETC.	15.2 CY
SILL	0.2 CY
EDGE BEAMS	0.6 CY
TOTAL	72.2 CY
REINFORCING STEEL	
BARREL, SILLS, & EDGE BEAMS WINGS, ETC.	7,214 LBS 1,047 LBS

8,261 LBS

BAR TYPES VERTICAL LEG-6"RAD.— 3′-6″ ALL BAR DIMENSIONS ARE OUT TO OUT.

SPLICE LENGTHS BAR SIZE LENGTH B1 #4 1'-11" C1 #4 1'-11"

PROJECT NO. U-5725 HALIFAX _ COUNTY

STATION: 84+52.00 -L-

SHEET 6 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SINGLE 6 FT.X 7 FT. CONCRETE BOX CULVERT 66° SKEW

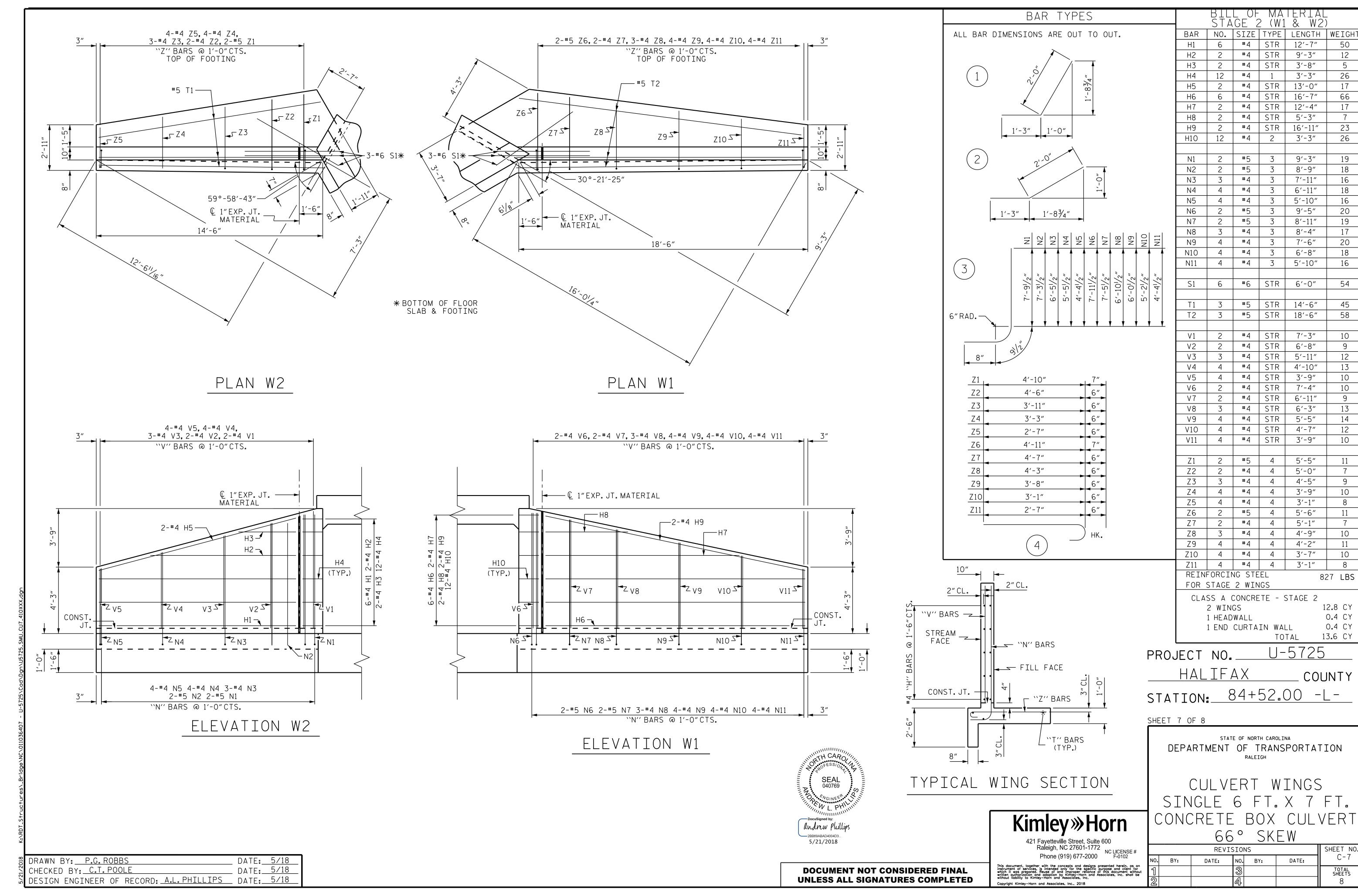
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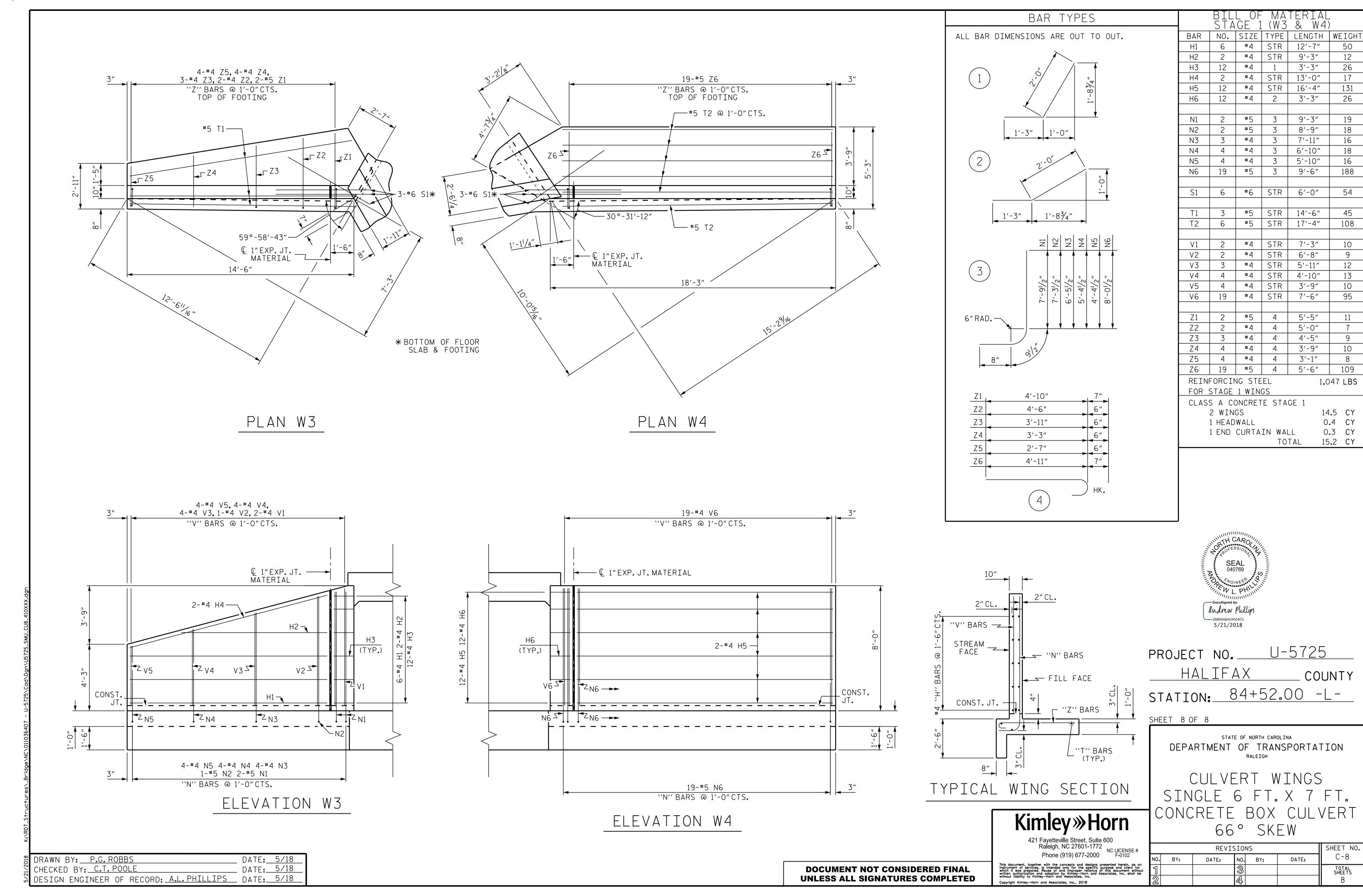
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DRAWN BY: P.G. ROBBS DATE: 5/18 CHECKED BY: C.T. POOLE DATE: 5/18
DESIGN ENGINEER OF RECORD: A.L. PHILLIPS DATE: 5/18 CHECKED BY: C.T. POOLE





STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS LIVE LOAD ----- SEE PLANS STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24.000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION - - - - - - - - 1.200 LBS. PER SQ. IN. CONCRETE IN SHEAR ------ SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ---- 375 LBS.PER SQ.IN. EQUIVALENT FLUID PRESSURE OF EARTH ---- 30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED \(\frac{3}{4}\)" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1\(\frac{1}{2}\)" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A \(\frac{1}{4}\)" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A \(\frac{1}{4}\)" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \(\frac{1}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH