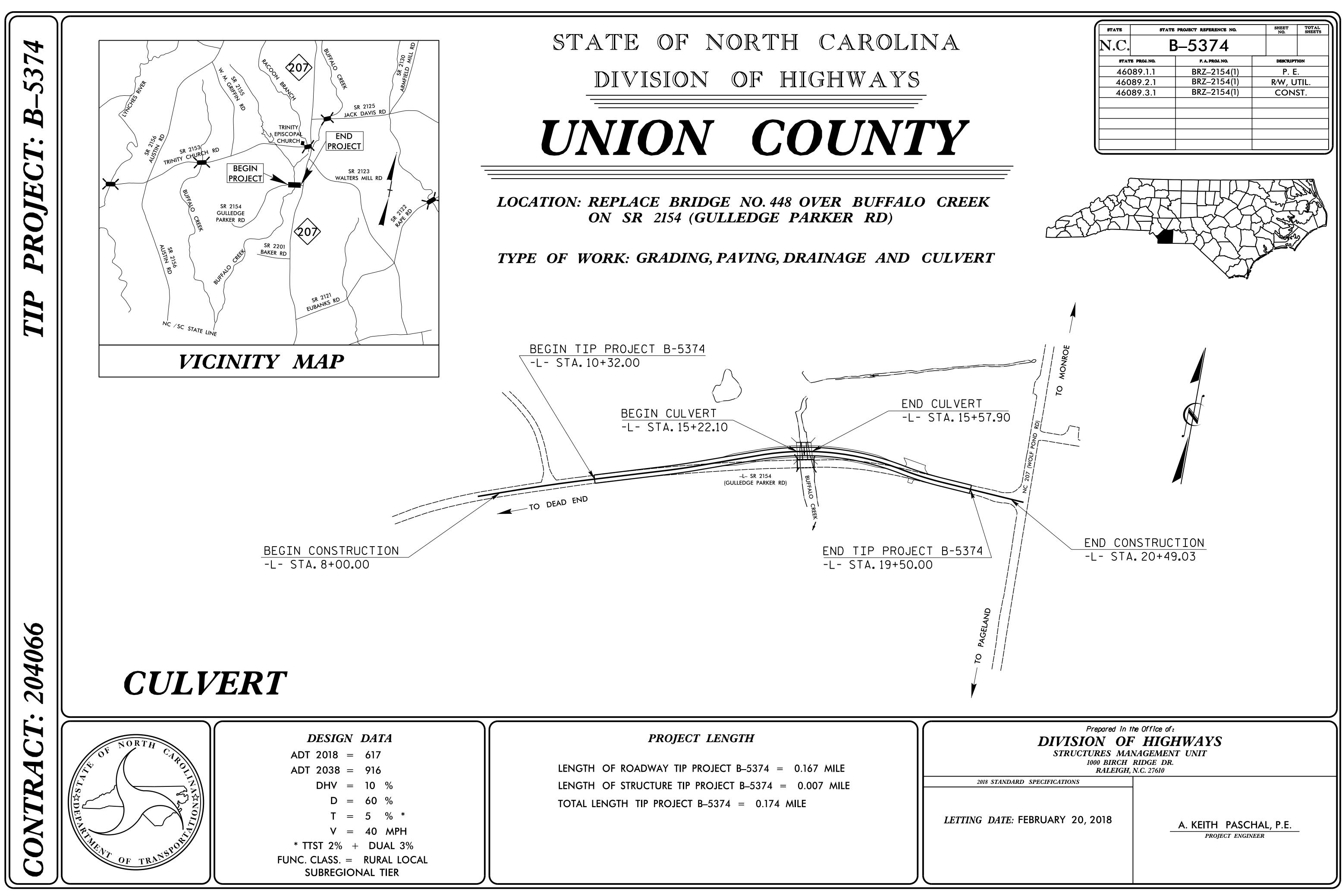
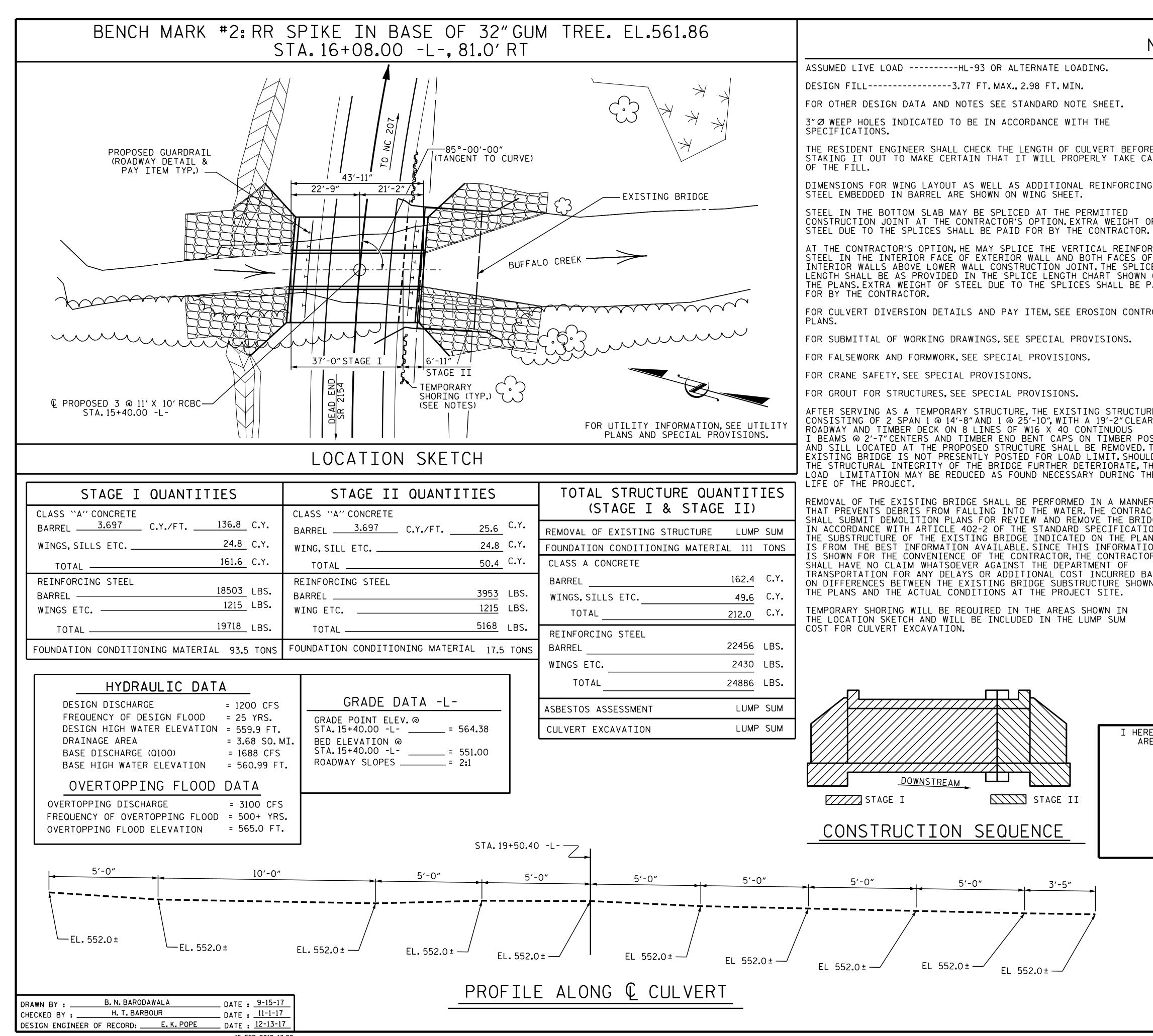
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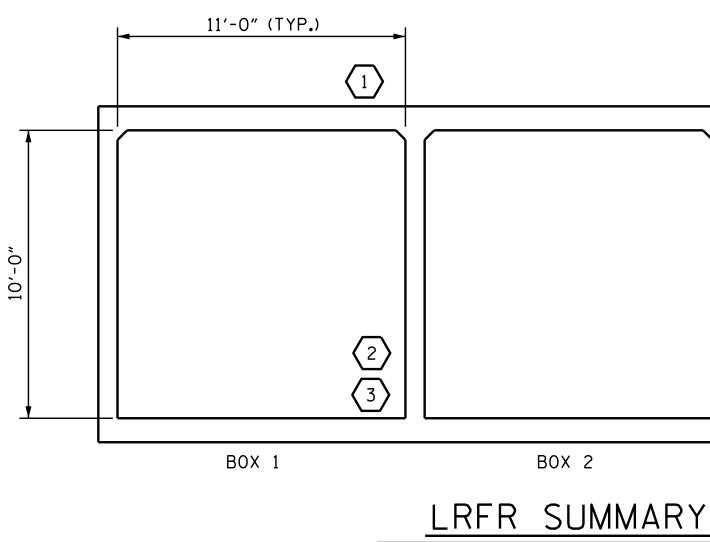




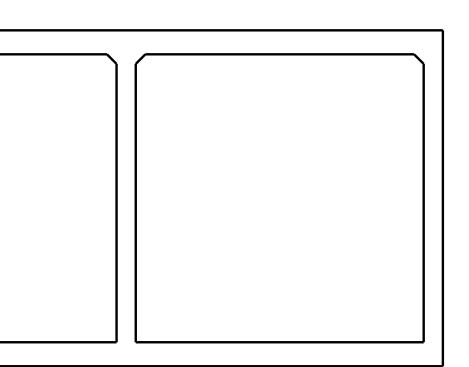
NOT	ES	F.A. F	PROJI	ECT	NO.: B	RZ-2	154(1)
	A 3 FOOT STRIP FACE OF THE WING JOINT.						
	THE REINFORCED (STANDARD 1.0 FOC SEE SECTION 414 CONCRETE IN STA ORDER:	T BLANKET OF THE ST	OF FOUN	NDATION SPECIFIC	CONDITION CATIONS.	NING MATE	RIAL
RE CARE	1.WING FOOTIN INCLUDING 4				M AND FL	OOR SLAB	
1G	2. THE REMAINI 3. INLET SILLS		N OF WAL	LS AND	WINGS FUL	L HEIGHT.	
OF	4. ROOF SLAB, E		AND HEAD	WALL.			
ORCING OF	CONCRETE IN STA ORDER:	GE II CUL	VERT TO	BE POUR	ED IN THE	FOLLOWIN	IG
CE I ON PAID	1.WING FOOTIN FLOOR SLAB STAGE II VE	TO THE CO	NSTRUCTI			NG 4″ OF	
ROL	2. THE REMAINI HEIGHT.	NG PORTIO	N OF STA	AGE II V	ALLS AND	WINGS FU	LL
	3.OUTLET SILL						
	4. ROOF SLAB, E FOR ASBESTOS AS				LITION AN	ID RENOVA	TION
	ACTIVITIES, SEE INASMUCH AS THE				STING STR	UCTURAL S	STEEL
JRE AR OSTS , THE ILD	CONTAINS LEAD, T ARTICLE 107-1 OF FROM COMPLIANCE PERTAINING TO H SHALL BE INCLUDE STRUCTURE AT ST	HE CONTRA THE STANE WITH APP ANDLING O D IN THE	CTOR'S A DARD SPE PLICABLE F MATER]	TTENTIO CIFICAT STATE C ALS CON	N IS DIRE IONS.ANY R FEDERAL ITAINING L	CTED TO COSTS RES REGULATI EAD BASE	GULTING IONS D PAINT
THE THE	FOR EROSION CON FOR CONSTRUCTIO						
ER CTOR	THE REQUIRED BEA 1 TSF. THE REQUIR	ARING CAP	ACITY AT	THE BA	SE OF THE	CULVERT	IS
DGE IONS. ANS ION OR BASED	AT THE CONTRACT APPROVAL, DESIGN CONCRETE BOX CU ON THE PLANS. THI OF BARRELS AS US	AND DETA _VERT IN E DESIGN S SED ON THE	IL DRAW] LIEU OF SHALL PR E CAST-I	INGS FOF THE CAS OVIDE TH N-PLACE	A PRECAS T-IN-PLACE HE SAME SI DESIGN.FC	T REINFO CULVERT ZE AND N R OPTION	RCED SHOWN UMBER AL
WN ON	PRECAST REINFOR THE CONTRACTOR OF REINFORCING TO 400 TONS OF EACH SIZE BAR US TONS OF REINFOR BAR USED. THE BA THEN BE SPLICED LENGTH OF THE SA BAR DIAMETERS. F STEEL SHALL BE C	SHALL PRO STEEL AS F REINFORCI SED, AND FO CING STEEN RS FROM W WITH REPN MPLE, PLUS PAYMENT FO	VIDE IND FOLLOWS: NG STEEL OR PROJE L,TWO 3C WHICH TH LACEMENT S A MINI OR THE S	EPENDEN FOR PRO ONE 30 CTS REO INCH S E SAMPLI BARS O MUM LAF AMPLES	T ASSURAN JECTS REQ INCH SAM JIRING OV AMPLES OF S ARE TAK F THE SIZE S SPLICE O OF REINFOF	CE SAMPLE UIRING UI PLE OF ER 400 EACH SIZ EACH SIZ E AND F THIRTY RCING	S E
	ERTIFY THESE PLAN AS-BUILT PLANS	IS					
		_		UNIC	•		UNTY L-
		S	HEET 1 OF	- 9	REPLAC	ES BRIDGE	NO. 448
	DocuSigned by: A. Keith Parchal		TRIF	RTMENT PLE RETE	OF TRAI RALEIGH	NSPORTA X 10 CUL'	FT.
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LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS																
										STRENGTH	I LIM	IT ST	ATE			
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f†)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	N⁄A	$\langle 1 \rangle$	1.05		1.75	1.45	1	TOP SLAB	4.96	1.05	1	TOP SLAB	10.38	
DESIGN LOAD		HL-93 (OPERATING)	N⁄A		1.36		1.35	1.88	1	TOP SLAB	4.96	1.36	1	TOP SLAB	10.38	
RATING		HS-20 (INVENTORY)	36.000	2	1.16	41.74	1.75	1.63	1	TOP SLAB	4.96	1.16	1	BOTTOM SLAB	10.76	
		HS-20 (OPERATING)	36.000		1.50	54.10	1.35	2.11	1	TOP SLAB	4.96	1.50	1	BOTTOM SLAB	10.76	
		SNSH	13.500		2.37	31.96	1.40	2.97	1	TOP SLAB	4.96	2.37	1	TOP SLAB	10.38	
	ш	SNGARBS2	20.000		2.22	44.38	1.40	2.78	1	TOP SLAB	4.96	2.22	1	TOP SLAB	10.38	
	ICL	SNAGRIS2	22.000		2.37	52.09	1.40	2.96	1	TOP SLAB	4.96	2.37	1	TOP SLAB	10.38	
	VEH.	SNCOTTS3	27.250		1.32	35.85	1.40	1.82	1	TOP SLAB	4.96	1.32	1	TOP SLAB	10.38	
	SLE (S	SNAGGRS4	34.925		1.50	52 . 52	1.40	2.21	1	TOP SLAB	5.25	1.50	1	BOTTOM SLAB	10.76	
	SING	SNS5A	35.550		1.48	52.61	1.40	2.11	1	TOP SLAB	4.96	1.48	1	BOTTOM SLAB	10.76	
		SNS6A	39.950		1.32	52.71	1.40	2.01	1	BOTTOM SLAB	10.79	1.32	1	BOTTOM SLAB	10.76	
LEGAL LOAD		SNS7B	42.000		1.25	52.68	1.40	1.91	1	BOTTOM SLAB	10.79	1.25	1	BOTTOM SLAB	10.76	
RATING	LER	TNAGRIT3	33.000		1.59	52.44	1.40	2.45	1	BOTTOM SLAB	10.79	1.59	1	BOTTOM SLAB	10.76	
	RAI	TNT4A	33.075		1.57	52.03	1.40	2.16	1	TOP SLAB	4.96	1.57	1	TOP SLAB	10.38	
	NI-T	TNT6A	41.600		1.27	52.65	1.40	1.92	1	BOTTOM SLAB	10.79	1.27	1	BOTTOM SLAB	10.76	
	SEMI ST)	TNT7A	42.000		1.32	55.41	1.40	2.03	1	BOTTOM SLAB	10.79	1.32	1	BOTTOM SLAB	10.76	
	CTOR (TT	TNT7B	42.000		1.32	55.41	1.40	2.06	2	BOTTOM SLAB	10.79	1.32	1	BOTTOM SLAB	10.76	
	TRA(TNAGRIT4	43.000		1.23	52.75	1.40	1.91	1	BOTTOM SLAB	10.79	1.23	1	BOTTOM SLAB	10.76	
	TRUCK	TNAGT5A	45.000	3	1.17	52.66	1.40	1.81	1	BOTTOM SLAB	10.79	1.17	1	BOTTOM SLAB	10.76	
	TRI	TNAGT5B	45.000		1.18	52.88	1.40	1.79	1	BOTTOM SLAB	10.79	1.18	1	BOTTOM SLAB	10.76	



	B. N. BARC I. T. BARB		DATE : DATE :	9-15-17 10-25-17
DRAWN BY : WMC CHECKED BY : GM	7/ 7/	REV. 10/	'I / II	MAA/GM



BOX 3

OWNSTREAM)

LOAD FACTORS:

DESIGN LUAD RATING FACTORS					
LOAD TYPE	MAX FACTOR	MIN FACTOR			
DC	1.25	0.90			
DW	1.50	0.65			
EV	1.30	0.90			
EH	ЕН 1.35				
ES	1.35	0.90			
LS	LS 1.75				
WA	1.00				

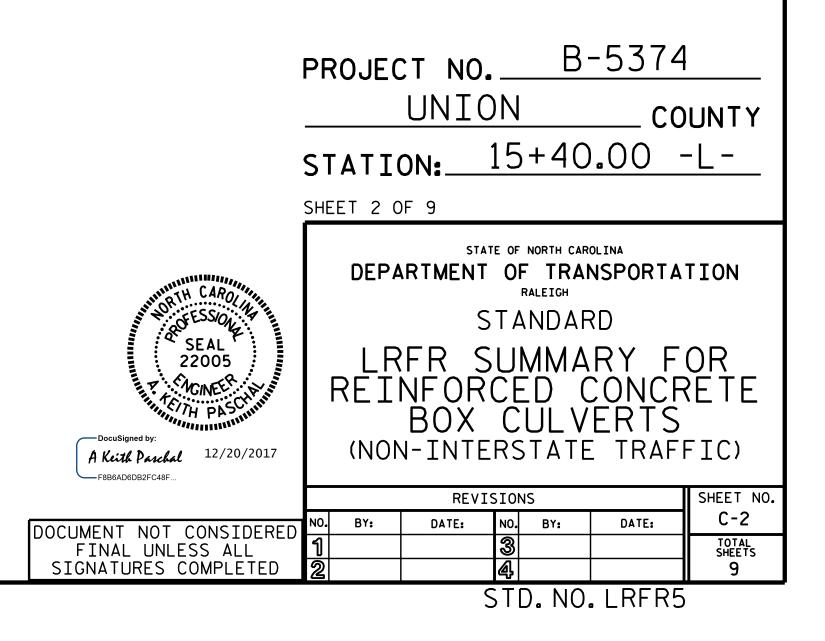
DESTGN LOAD RATING FACTORS

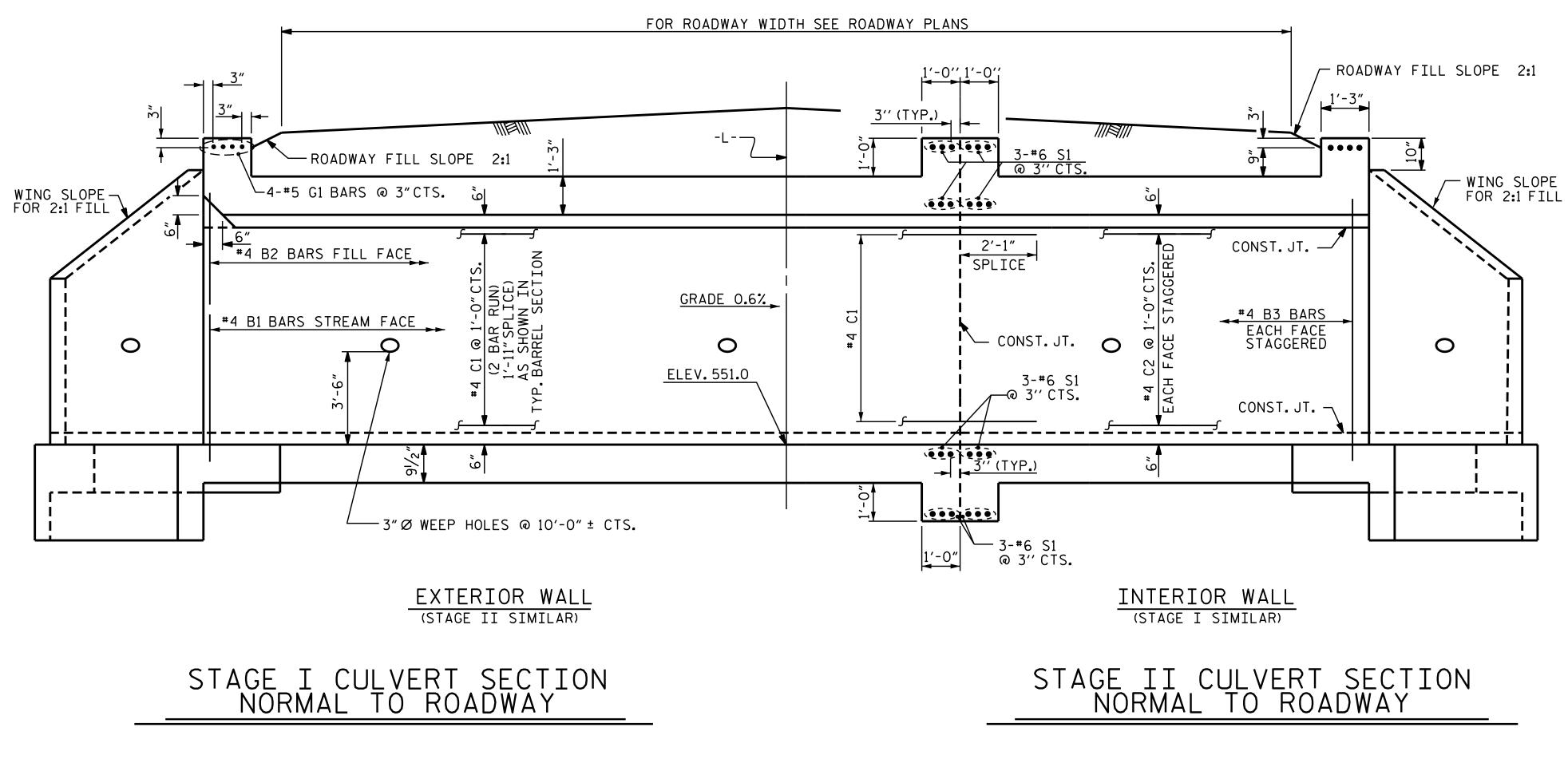
NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

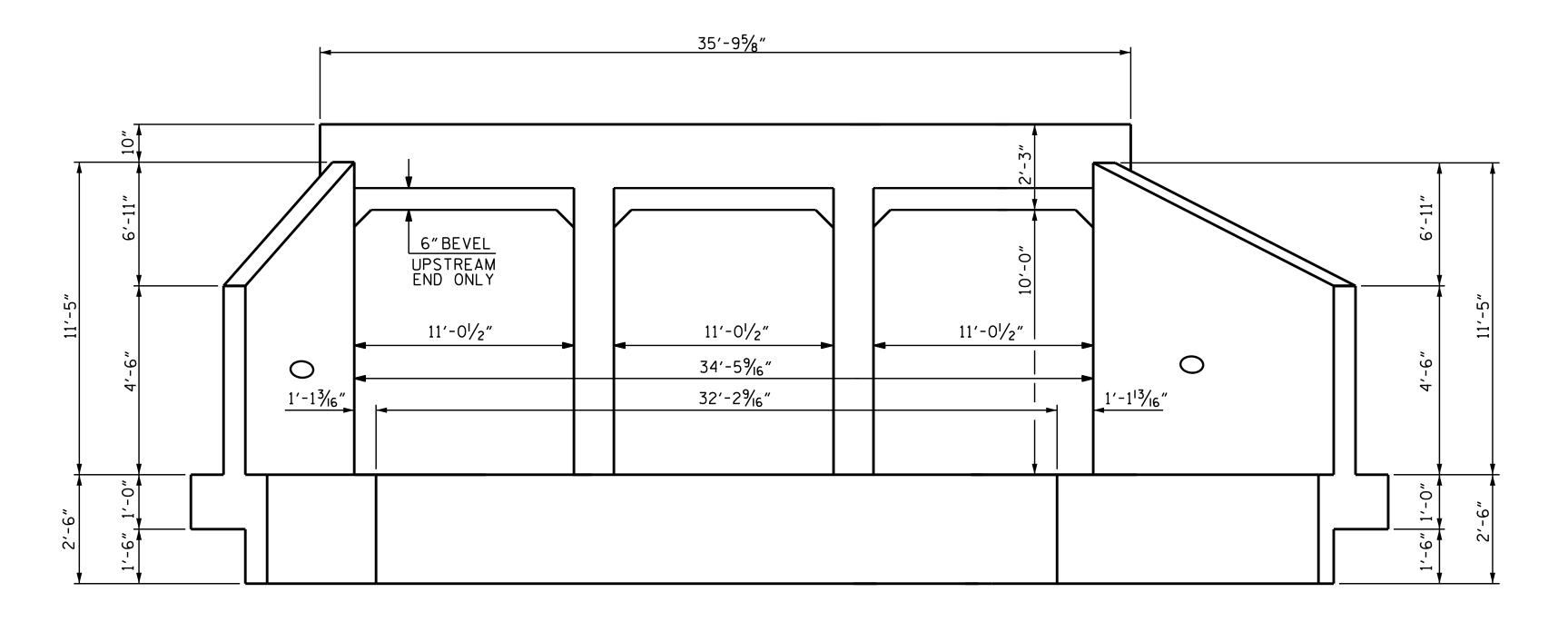


- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- ** SEE CHART FOR VEHICLE TYPE







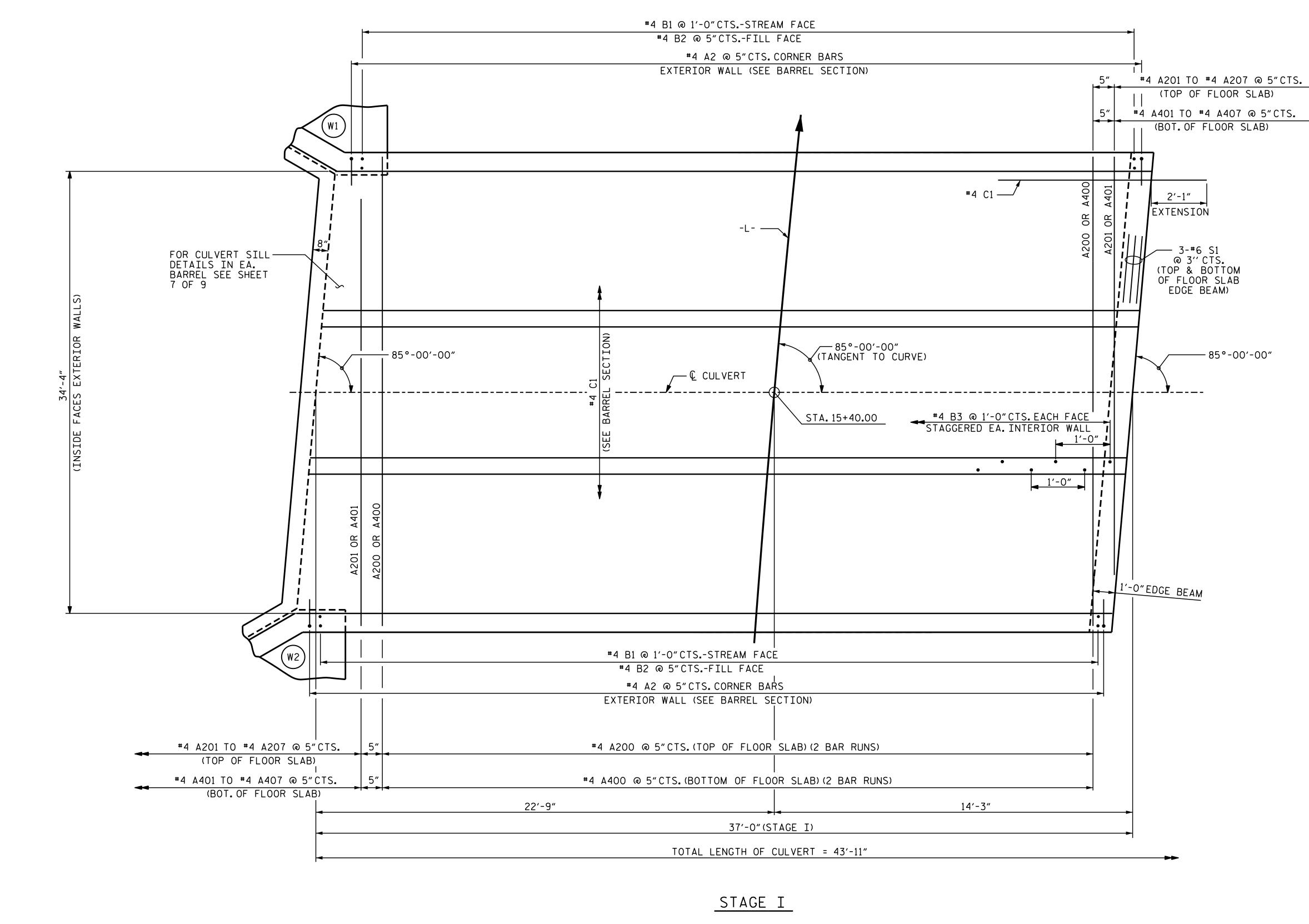


DRAWN BY :	B. N. BARODAWALA	DATE : <u>9-15-17</u>
CHECKED BY :	H. T. BARBOUR	DATE : <u>10-25-17</u>
DESIGN ENGINEER	OF RECORD: <u>E.K.POPE</u>	DATE : <u>12-13-17</u>

END ELEVATION NORMAL TO SKEW

I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS

	PROJEC	UNI	<u>NC</u>			4 OUNTY - L –
	SHEET 3 C)F 9				
SEAL 22005 HILL TO CINES OF ALL AND	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					ATION
SEAL	TRT	PIF 1	1 F	ΞТ.	X 1() FT.
22005				-		
THE TOWNER OF ANTINI	CONC		- L		CUL	
DocuSigned by: A Kith Parchal 12/20/2017 F8B6AD6DB2FC48F	85° SKEW					
	REVISIONS SHEET NO.					
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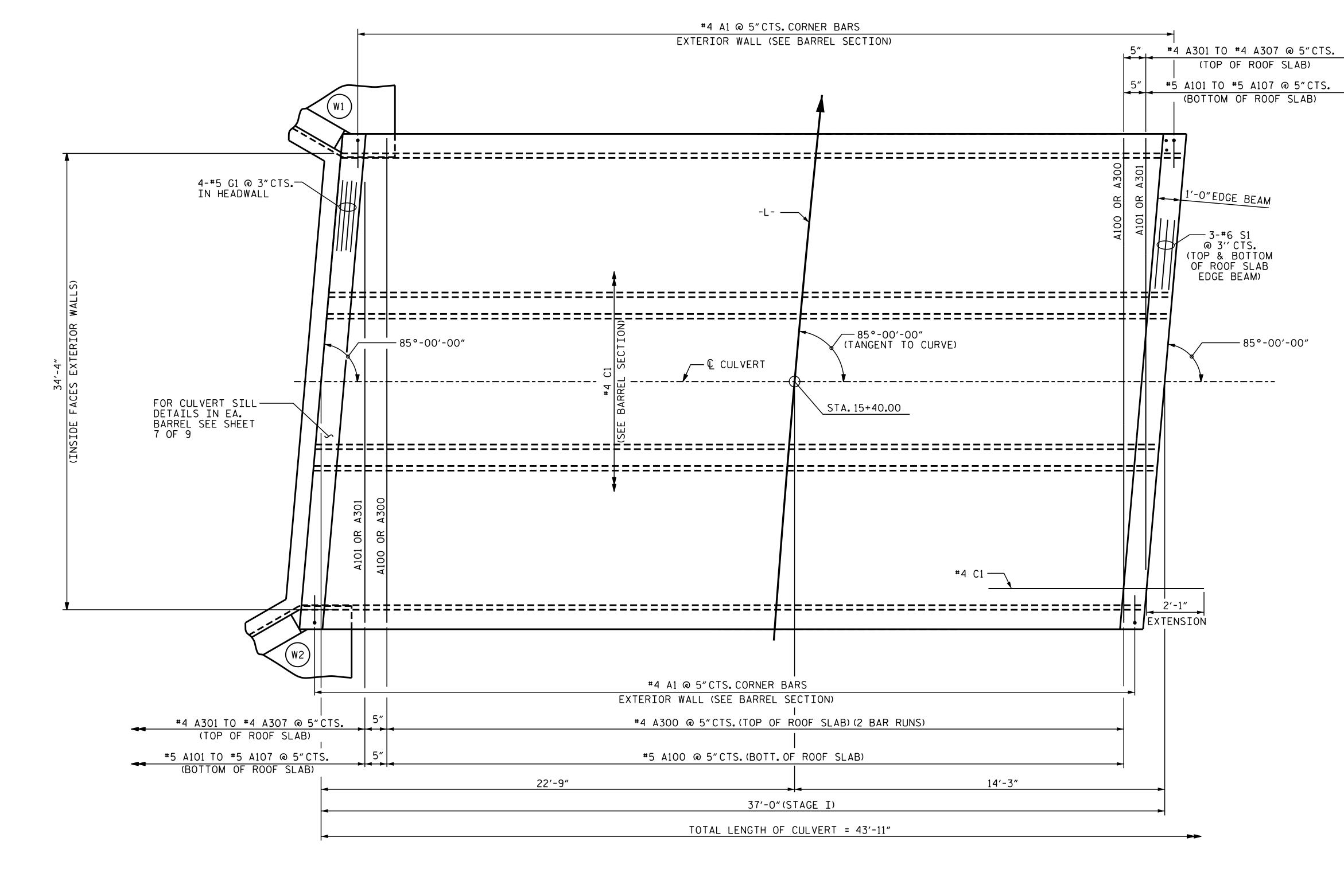


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DRAWN BY :	B. N. BAR	ODAWALA	DATE : <u>9-15-17</u>
CHECKED BY :	н. т. ви	ARBOUR	DATE : <u>10-25-17</u>
DESIGN ENGINEER	OF RECORD:	E.K.POPE	DATE : <u>12-13-17</u>

<u> Plan - Floor Slab</u>

	PROJECT NO. B-5374 UNION COUNTY STATION: 15+40.00 -L- SHEET 4 OF 9 SHEET 4 OF 9				
DocuSigned by:	DEPARTMENT OF TRANSPORTATION RALEIGH TRIPLE 11 FT. X 10 FT. CONCRETE BOX CULVERT				
A Kiith Parchal 12/20/2017 F8B6AD6DB2FC48F	85° SKEW				
	REVISIONS SHEET NO.				
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DRAWN BY :	B. N. BARODAWALA	DATE : <u>9-15-17</u>
CHECKED BY :	H. T. BARBOUR	DATE : <u>10-25-17</u>
DESIGN ENGINEER	OF RECORD: <u>E.K.POPE</u>	DATE : <u>12-13-17</u>

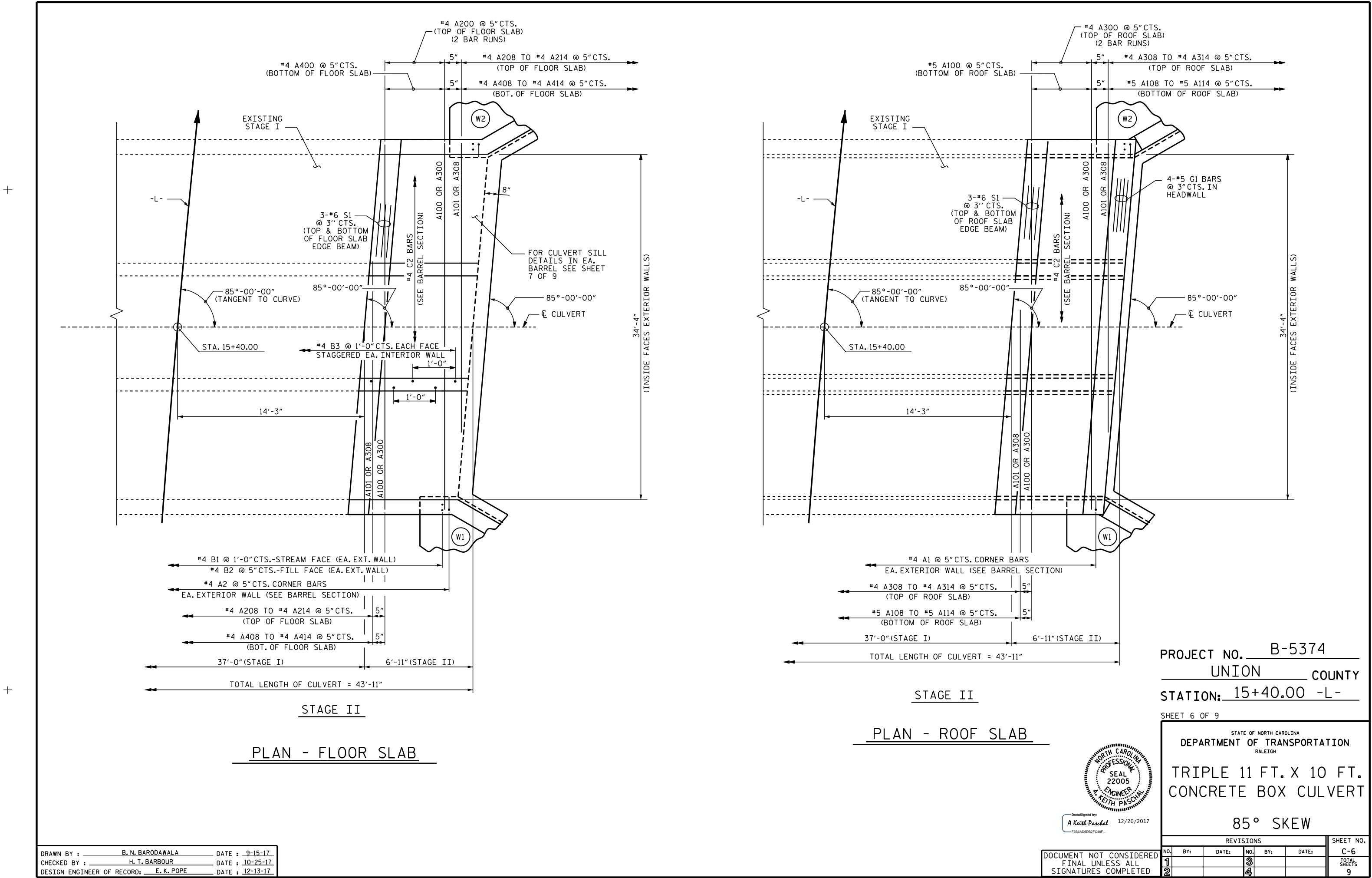
STAGE I

<u> Plan - Roof Slab</u>

	PROJECT NO. <u>B-5374</u> <u>UNION</u> COUNTY STATION: <u>15+40.00</u> -L-				
	SHEET 5 OF 9				
TH CAROLAN	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
DocuSigned by:	TRIPLE 11 FT.X 10 FT. CONCRETE BOX CULVERT				
DocuSigned by: A Keith Parchal 12/20/2017 F8B6AD6DB2FC48F	85° SKEW				
	REVISIONS SHEET NO.				
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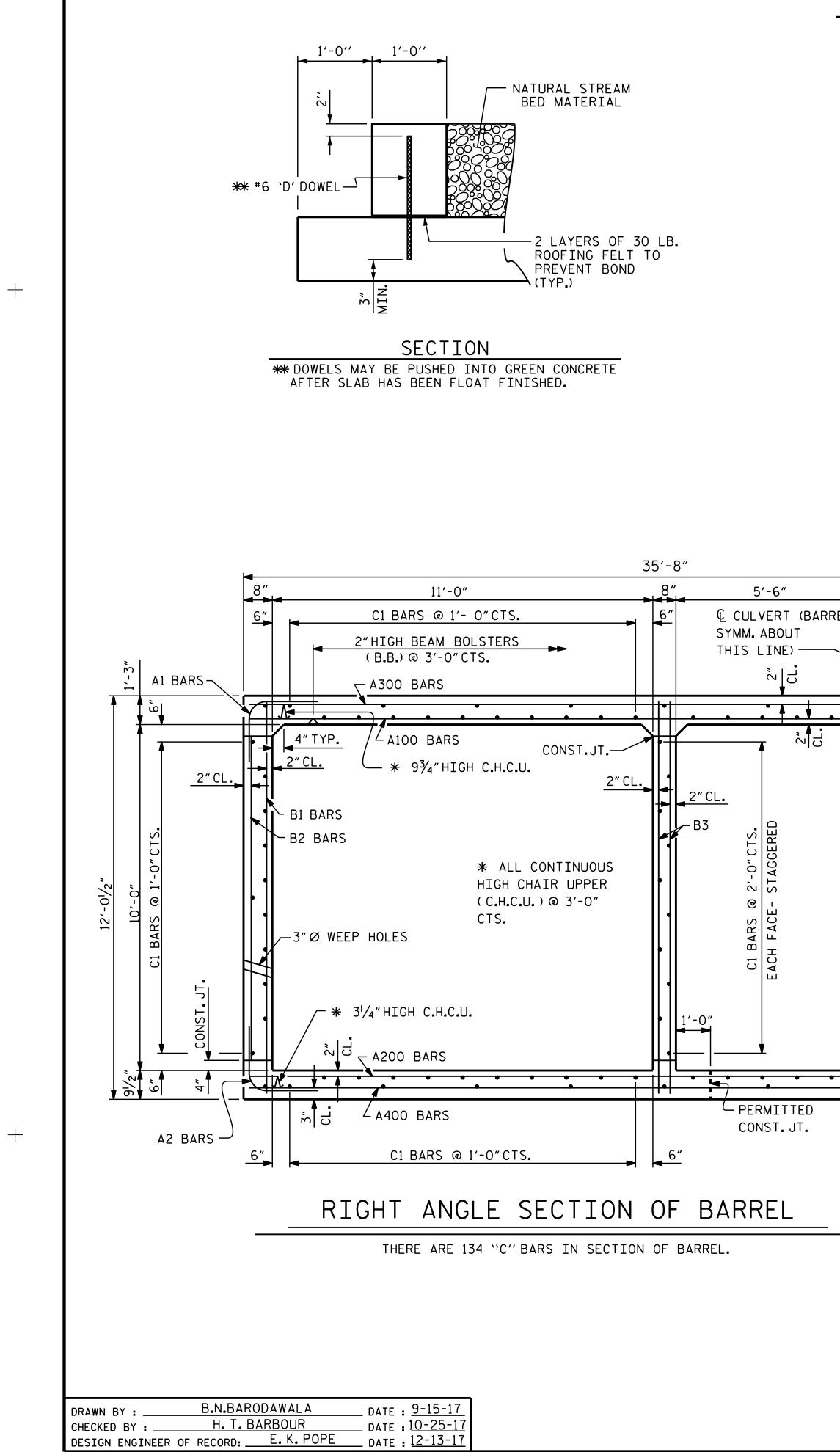
(BOTTOM OF ROOF SLAB)

— 85°-00'-00"

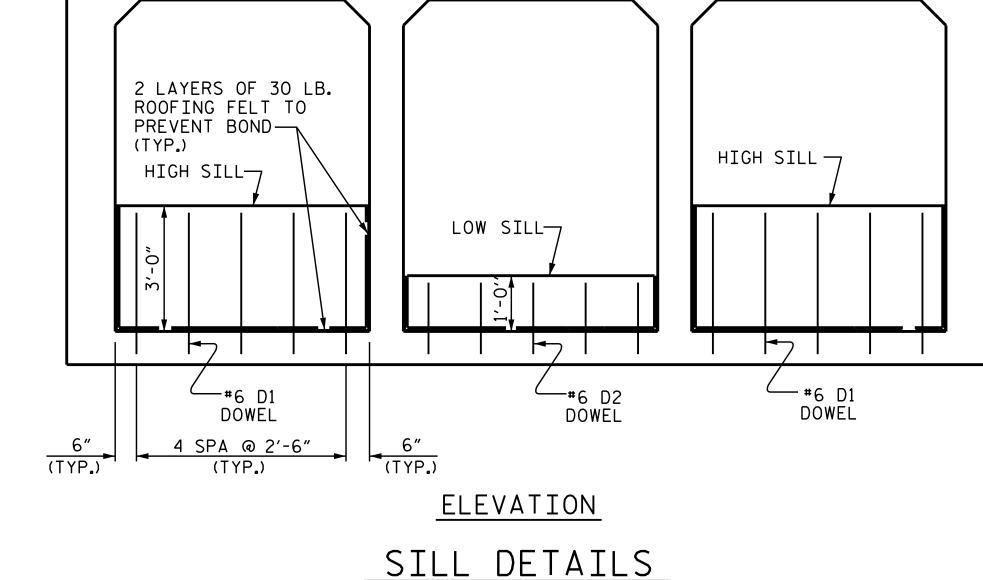


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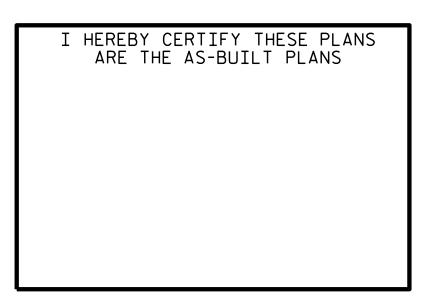
kpaschal



NOTES		BAR T	YPE	
MATERIAL EXCAVATED FROM THE EXISTING STREAM BED OR FLOOD PLAIN SHALL BE STOCKPILED FOR USE IN THE PROPOSED CULVERT AND SHALL PROVIDE A CONTINUOUS LOW FLOW CHANNEL AS SHOWN. THE MATERIAL SHALL BE NATURAL STONE WITH A GRADATION SIZE SIMILAR TO THAT OF CLASS II RIP RAP.BED MATERIAL SHALL BE SUBJECT TO APPROVAL BY THE ENGINEER, AND MAY BE SUBJECT TO PERMIT CONDITIONS.	VER ⁻	TICAL LEG — \	A1 A2	BAR A1 A2 A100 A101
THE STOCKPILED MATERIAL SHALL BE PLACED TO PROVIDE A DEPTH OF 1 FOOT IN LOW FLOW BARREL, AND 3 FEET IN THE HIGH FLOW BARRELS.		(1) 6″ R.→	3'-7"	A102 A103 A104
THE TOP OF BED MATERIAL IN THE LOW FLOW BARREL SHOULD MATCH THE STREAM BED ELEVATION.		о п.		A105 A106
BED MATERIAL SHALL BE SUPPLEMENTED BY CLASS II RIP RAP AS NECESSARY IN THE HIGH FLOW BARREL ONLY.			3/12	A107
BED MATERIAL SHALL BE PLACED ON TOP OF THE SUPPLEMENTAL FILL, IF USED, TO PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE.	A1 A2	2'-7 ¹ /2" 1'-10 ¹ /2"		A200 A201
THE ENTIRE COST OF WORK REQUIRED TO PLACE EXCAVATED OR SUPPLEMENTAL MATERIAL AS SHOWN ON THE PLANS SHALL BE				A202 A203
INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION.			E OUT TO OUT	A204 A205
THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.		CE LENGTH		A206 A207
SHALL DE INCLUDED IN THE VARIOUS FAT ITEMS.	BAR	SIZE	SPLICE LENGTH	
	A200	# 4	1'-9"	A300 A301
	A300	#4	2'-5"	A302
	A400	#4	1'-9"	A303
	B1	#4	1'-5"	A304 A305
	B3	#4	1'-5″	A306
	C1	#4	1'-11"	A307
				A400
				A401
RELS				A402
\sim $ $				A403
				A404
				A405 A406
\Rightarrow $ $ / / / /	\frown /			A408 A407
) (
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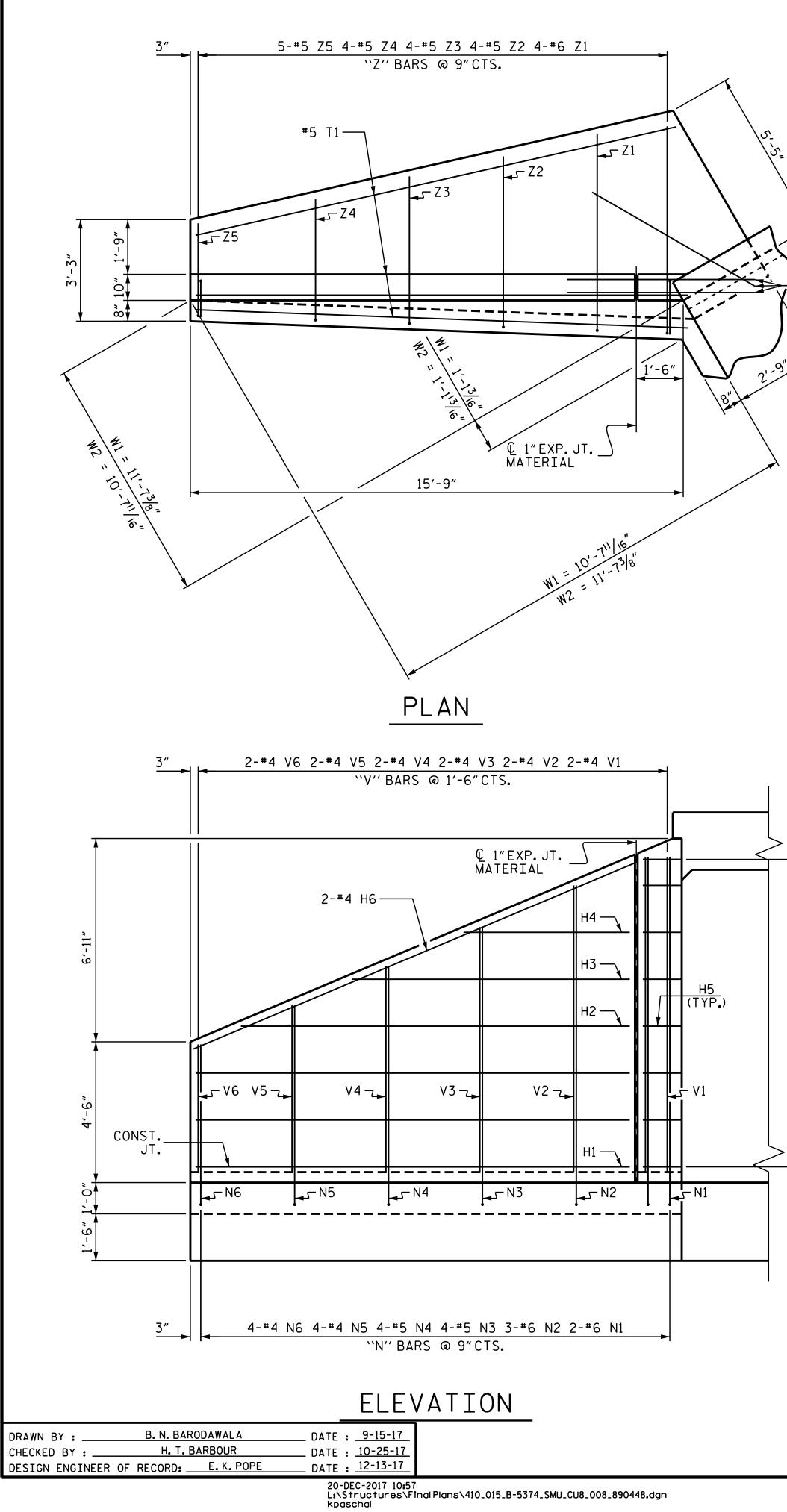






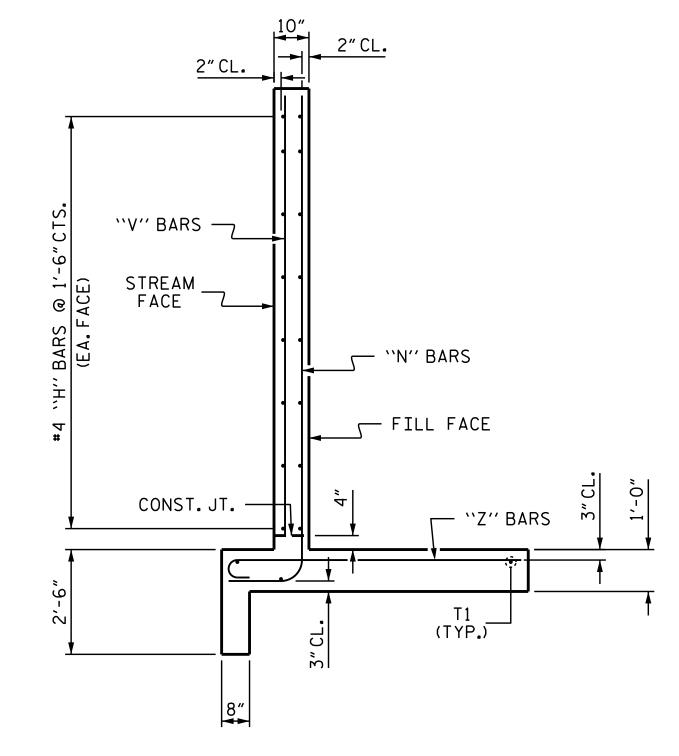
									_		
	REINFORCED					SCH					
			AGE			STAGE II					
BAR A1	NO. 176	SIZE #4	TYPE 1	LENGTH 7'-O"	WEIGHT 823	BAR A1	NO. 32	SIZE #4	TYPE 1	LENGTH 7'-O"	WEIGHT 150
A2	176	#4	1	4'-7"	539	A2	32	#4	1	4'-7"	98
A100	81	#5	STR	35'-4"	2985	A100	9	#5	STR	35'-4"	332
A100	2	#5	STR	32'-1"	67	A100	2	#5	STR	31'-7"	66
A102	2	# 5	STR	27'-2"	57	A109	2	#5	STR	26′-8″	56
A103	2	# 5	STR	22'-4"	47	A110	2	#5	STR	21'-11"	46
A104	2	# 5	STR	17'-7"	37	A111	2	#5	STR	17'-2"	36
A105 A106	2	#5 #5	STR STR	<u>12'-10"</u> 8'-1"	27 17	A112 A113	2	#5 #5	STR STR	12'-5" 7'-7"	26 16
A107	2	#5	STR	3'-4"	7	A113 A114	2	# 5	STR	2'-10"	6
1200	16.2	#4		10/ 7//	2011	4200	10	# 1	CTD	18'-7"	227
A200 A201	162 4	#4 #4	STR STR	18'-7" 16'-11"	2011 45	A200 A208	18 4	#4 #4	STR STR	18 - 7**	223 45
A202	2	#4	STR	27'-2"	36	A209	2	#4	STR	26'-8"	36
A203	2	#4	STR	22'-4"	30	A210	2	#4	STR	21'-11"	29
A204	2	#4	STR	17'-7"	23	A211	2	#4	STR	17'-2"	23
A205 A206	2	#4 #4	STR STR	<u>12'-10"</u> 8'-1"	<u>17</u> 11	A212 A213	2	#4 #4	STR STR	12'-5" 7'-7"	17 10
A206 A207	2	#4	STR	3'-4"	4	A213 A214	2	*4 #4	STR	2'-10"	4
A300	162	#4 #4	STR	18'-11"	2047	A300	18	#4 #4	STR	18'-11"	227
A301 A302	4	#4 #4	STR STR	17'-2" 27'-2"	46 36	A308 A309	4	#4 #4	STR STR	17'-0" 26'-8"	45 36
A302	2	#4	STR	22'-4"	30	A310	2	#4	STR	21'-11"	29
A304	2	#4	STR	17'-7"	23	A311	2	#4	STR	17'-2″	23
A305	2	#4	STR	12'-10"	17	A312	2	#4	STR	12'-5"	17
A306 A307	2 2	#4 #4	STR STR	8'-1" 3'-4"	11 4	A313 A314	2	#4 #4	STR STR	7'-8" 2'-11"	10 4
AJUT	<u> </u>		511			AJIA			511	2 11	
A400	162	#4	STR	18'-7"	2011	A400	18	#4	STR	18′-7″	223
A401	4	#4	STR	16'-11"	45	A408	4	#4	STR	16'-8"	45
A402 A403	2	#4 #4	STR STR	27'-2" 22'-4"	36 30	A409 A410	2	#4 #4	STR STR	26'-8" 21'-11"	36 29
A404	2	#4	STR	17'-7"	23	A411	2	#4	STR	17'-2"	23
A405	2	#4	STR	12'-10"	17	A412	2	#4	STR	12'-5″	17
A406	2	#4 #4	STR	8'-1"	11	A413	2	#4 #4	STR	7'-7"	10
A407	2		STR	3'-4"	4	A414	2		STR	2'-10"	4
B1	74	#4	STR	11'-6″	568	B1	14	#4	STR	11'-6″	108
B2	178	#4	STR	9'-4"	1110	B2	32	#4	STR	9'-4"	200
B3	148	#4	STR	11'-6″	1137	B3	28	#4	STR	11'-6″	215
C1	268	#4	STR	20'-6"	3670	C2	134	#4	STR	6'-7"	589
	10	#0		7/ ///	50		10	#6	CTD	7/ ///	50
D1 D2	10 5	#6 #6	STR STR	3'-4" 1'-4"	50 10	D1 D2	10 5	#6 #6	STR STR	3'-4" 1'-4"	50 10
			••••	• ·					••••		
G1	4	# 5	STR	35'-4"	147	G1	4	#5	STR	35′-4″	147
S1	12	#6	STR	35'-4"	637	S1	12	#6	STR	35′-4″	637
REINF	ORCIN	G STE	EL	LBS.	18503	REINF	ORCIN	IG STE	EL	LBS.	3953
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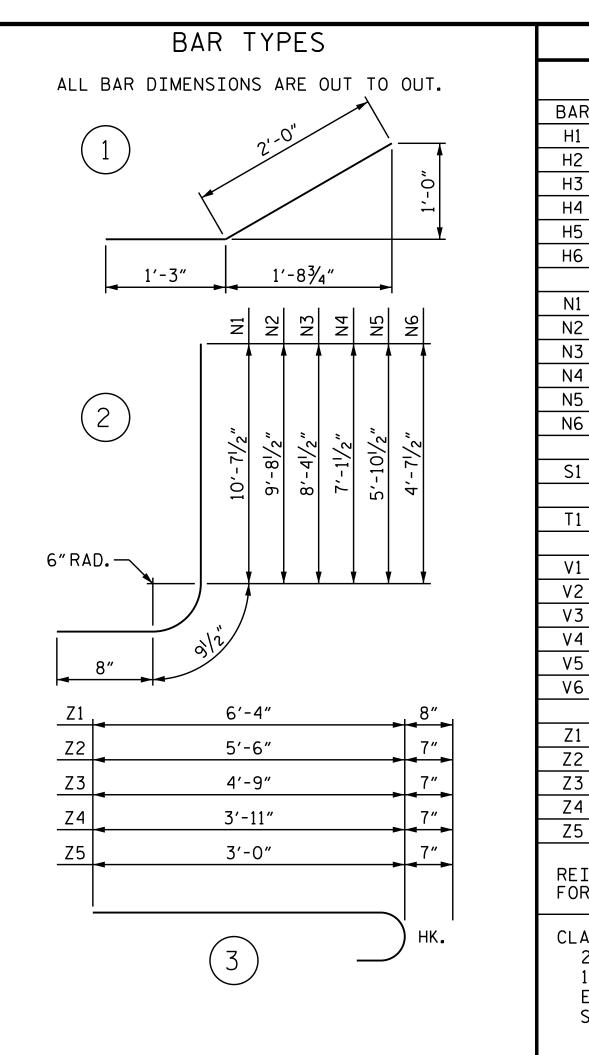
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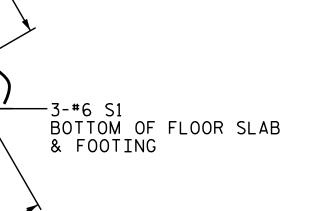


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TYPICAL WING SECTION





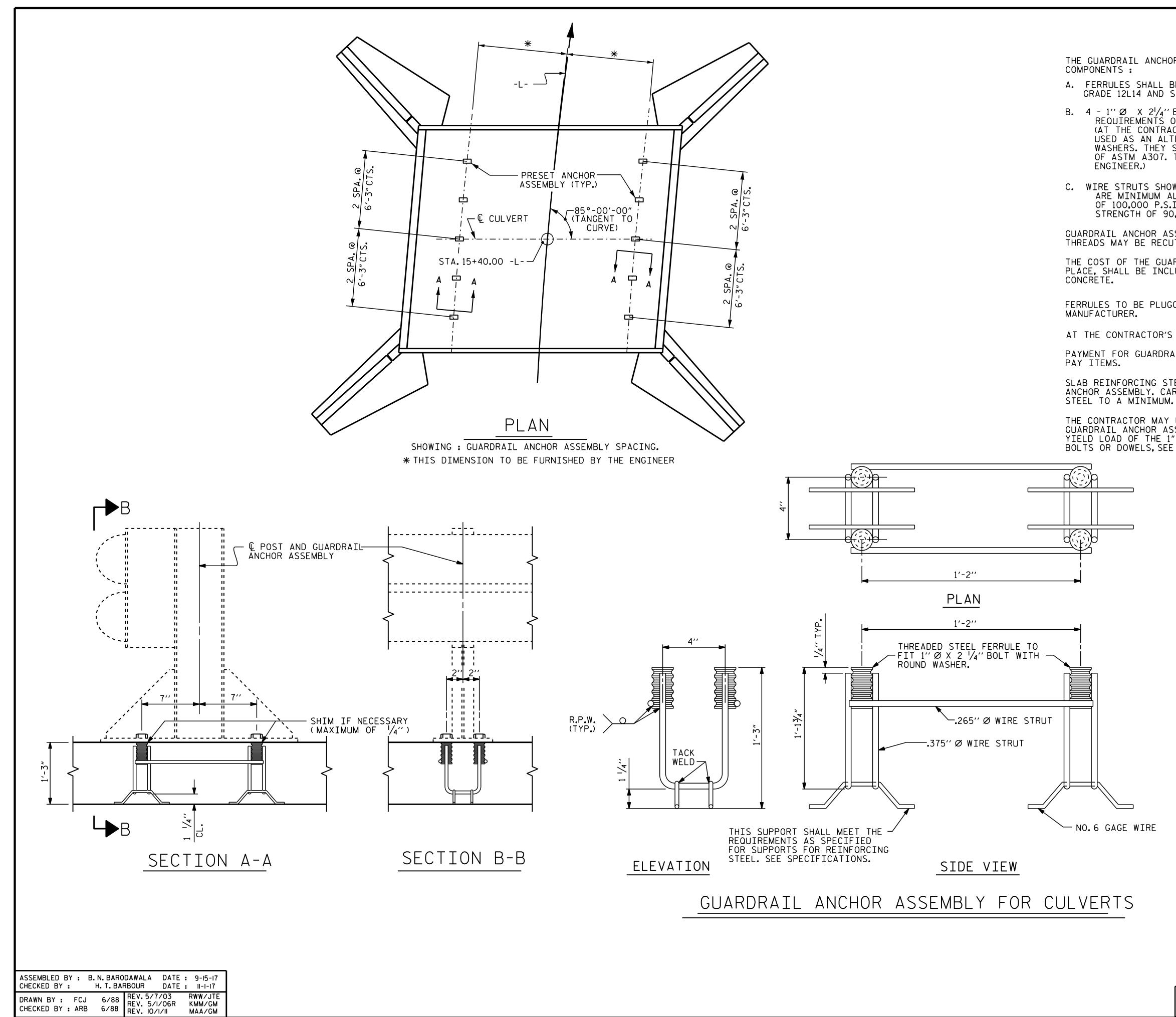


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BI	LL OF	F MA	TERIAL	_		BIL	L OF	MA	TERIAL	_
STAGE I							STA	AGE	II	
AR NO	. SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
11 12	#4	STR	13'-10″	111	H1	12	#4	STR	13'-10″	111
2 4	#4	STR	12′-5″	33	H2	4	#4	STR	12′-5″	33
3 4	#4	STR	8'-10"	24	Н3	4	#4	STR	8'-10"	24
4 4	#4	STR	5'-4"	14	H4	4	#4	STR	5'-4"	14
5 32	* #4	1	3'-3"	69	H5	32	#4	1	3'-3"	69
6 4	#4	STR	15′-5″	41	H6	4	#4	STR	15′-5″	41
11 4	#6	2	12'-1"	73	N1	4	#6	2	12'-1"	73
2 6	#6	2	11'-2"	101	N2	6	#6	2	11'-2"	101
3 8	#5	2	9'-10″	82	N3	8	# 5	2	9'-10"	82
4 8	#5	2	8'-7"	72	N4	8	# 5	2	8'-7"	72
5 8	#4	2	7'-4"	39	N5	8	#4	2	7'-4"	39
6 8	#4	2	6′-1″	33	N6	8	#4	2	6′-1″	33
61 6	#6	STR	6'-0"	54	S1	6	#6	STR	6'-0"	54
1 6	#5	STR	15′-9″	99	T1	6	# 5	STR	15′-9″	99
'1 4	#4	STR	10'-1"	27	V1	4	#4	STR	10'-1"	27
2 4	#4	STR	9′-2″	24	٧2	4	#4	STR	9′-2″	24
3 4	#4	STR	7'-10″	21	٧3	4	#4	STR	7'-10″	21
4 4	#4	STR	6'-7"	18	٧4	4	#4	STR	6'-7"	18
5 4	#4	STR	5'-4"	14	٧5	4	#4	STR	5′-4″	14
6 4	#4	STR	4'-1"	11	٧6	4	#4	STR	4'-1"	11
1 8	#6	3	7'-0"	84	Z1	8	# 6	3	7'-0"	84
2 8 3 8	#5	3	6'-1"	51	Z2	8	# 5	3	6'-1"	51
	#5	3	5'-4"	45	Z3	8	# 5	3	5'-4"	45
4 8	#5	3	4'-6"	38	Z4	8	# 5	3	4'-6"	38
5 10	#5	3	3'-7"	37	Z5	10	# 5	3	3'-7"	37
EINFORCING STEEL DR 2 WINGS 1215 LBS FOR 2 WINGS 1215 LB					15 LBS					
ASS A CONCRETE 2 WINGS 1 HEADWALL & 2 EDGE BEAM END CURTAIN WALL SILLS TOTAL 24.8 CY CLASS A CONCRETE 2 WINGS 1 HEADWALL & 2 EDGE BEAM 2.0 CY SILLS CLASS A CONCRETE 2 WINGS 1 HEADWALL & 2 EDGE E END CURTAIN WALL SILLS TOTAL CLASS A CONCRETE 2 WINGS 1 HEADWALL & 2 EDGE E END CURTAIN WALL SILLS TOTAL						. BEAM 4 2 2	5.6 CY 1.3 CY 1.0 CY 1.9 CY 4.8 CY			

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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".

B. 4 - 1"Ø X 2¼4" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1"Ø X 2¼4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE

C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A $\frac{1}{16}$ WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A"

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1"Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

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DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SO.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

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STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS

LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



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