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	HEET 3 FOR PLAN IE OF INVESTIGA		T LAYOUT	<b>STATE OF NORTH CAROLINA</b> DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS
CONTE LINE L YI Y3	<b>NTS</b> <u>STATION</u> II+83-I6+92 I0+3I-I2+62 I0+28-II+57	<u>PLAN</u> 4 4	<u>PROFILE</u> 5 5 5	GEOTECHNICAL ENGINEERING UNIT ROADWAY SUBSURFACE INVESTIGATIO
SHEET N SHEET N 1 2 3 3A 4 5 6-8 9	<b>GUIDE</b> D. <u>DESCRIPTION</u> TITLE SHEET LEGEND (SOIL & ROO ROADWAY TITLE SHEI INVENTORY TEXT PLAN SHEET(S) PROFILE(S) BORE LOG(S) SOIL TEST RESULTS	CK) ET		COUNTY WAKE PROJECT DESCRIPTION SR 1656 (TRINITY RD.) AT SR 1658 (YOUTH CENTER DR.) INTERSECTION IN RALEIGH. CONSTRUCT PEDESTRIAN TUNNEL UNDER 1656 (TRINITY RD.) INVENTORY

PR

STATE STATE PROJECT REFERENCE NO. TOTAL SHEETS NO. 9 N.C W-5522 1

## **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (99) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAIL

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UNI-FLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY YARY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITORIED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OF ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPHIONO OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONSTRUCTIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACTUAL CONDENSION OF FOR AN THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES: I. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

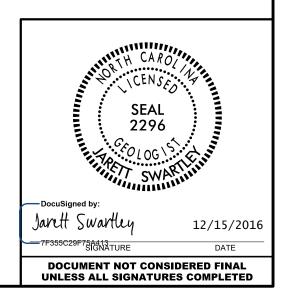
PERSONNEL

J.R. SWARTLEY

O.B. OTI

D.G. PINTER

INVESTIGATED BY **J.R. SWARTLEY** DRAWN BY \_\_\_\_\_\_. SWARTLEY CHECKED BY \_\_\_\_\_\_ N.T. ROBERSON SUBMITTED BY \_\_\_\_\_\_ DATE DECEMBER 2016

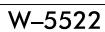


# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

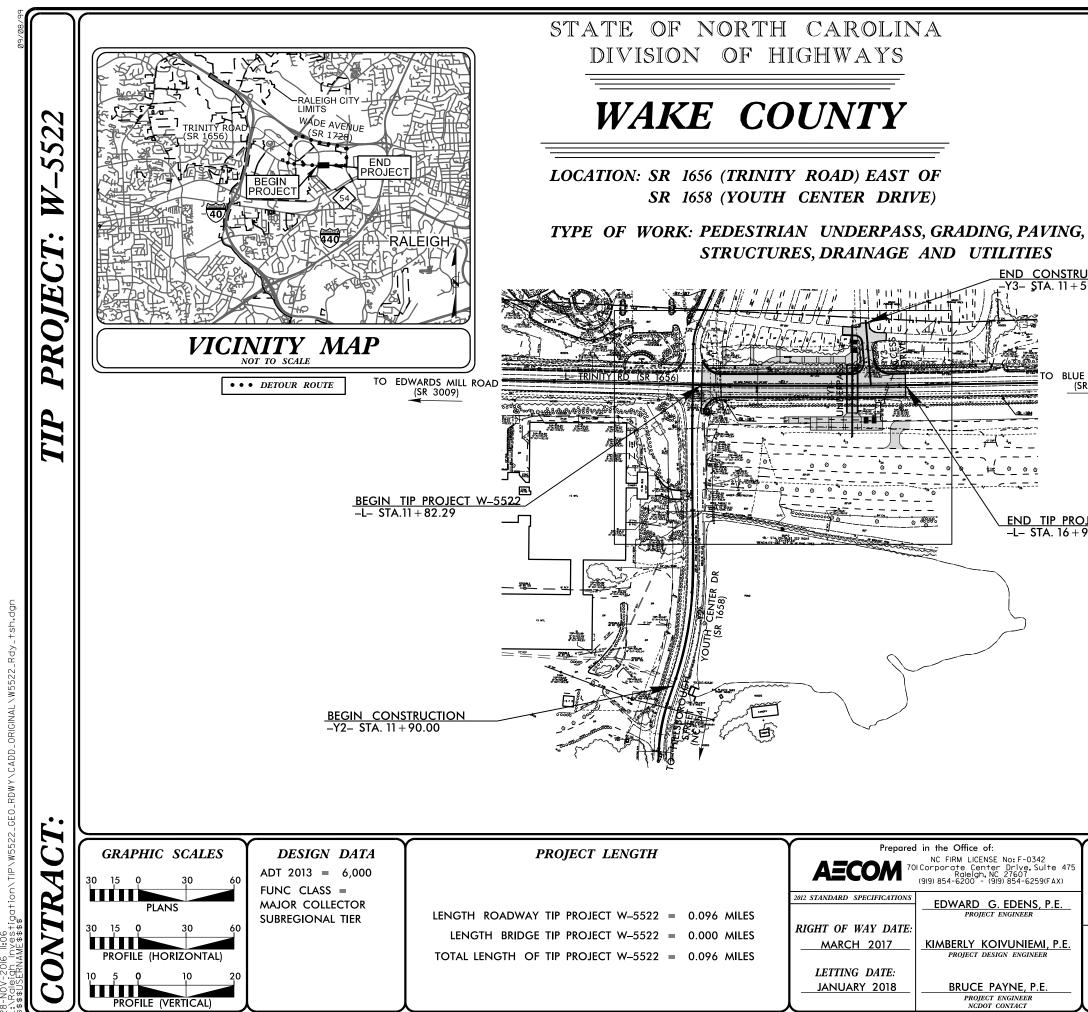
			SC	IL DE	SCRI	PTION							GF	RADATION			T			F	ROCK DES	CRIPTION
	CONSIDERED RATED WITH										WELL GRADED - INDICAT											DULD YIELD SPT REFUSAL IF TESTE TAL PLAIN MATERIAL WOULD YIELD
ACCORDI	NG TO THE	STANDARD	PENETRAT	ION TEST	(AASH	TO T 206	ASTM D	586). SOIL	CLASSIFIC	CATION	UNIFORMLY GRADED - IN GAP-GRADED - INDICATES						SPT REFUSAL	. IS PEI	NETRATION E	BY A SPLI	IT SPOON SAM	PLER EQUAL TO OR LESS THAN 0.1 SITION BETWEEN SOIL AND ROCK
CONSISTE	ASED ON TH NCY.COLOR.	TEXTURE.	MOISTURE,	AASHTO C	LASSIF	ICATION,	AND OTHE	R PERTINE	NT FACTOR				ANGULAR	ITY OF GRAI	٧S		REPRESENTED	BY A	ZONE OF WE	EATHERED	ROCK.	
	5 MINERALOO													SOIL GRAINS IS D	ESIGNATED E	BY THE TERMS:	ROCK MATERI	ALS AR	FICEL	20		MATERIAL THAT WOULD YIELD SPT
	SI	OIL LE	GEND #	AND A	ASHT	O CLA	SSIFI	CATION			ANGULAR, SUBAN				TION		WEATHERED ROCK (WR)					IT IF TESTED.
GENERAL CLASS.		GRANULAR M				-CLAY MATE		ORC	GANIC MATERI	ALS				CAL COMPOS		FIC	CRYSTALLINE		P.P.		TO COARSE GF	AIN IGNEOUS AND METAMORPHIC RO REFUSAL IF TESTED. ROCK TYPE IN
GROUP		≤ 35% PASS	A-2		A-4	5% PASSING	_	A-1. A-2	A-4, A-5					N THEY ARE CONSID			ROCK (CR)		<u>XX</u>	GNEISS	5, GABBRO, SC⊢	IIST, ETC.
	A-1-a A-1-b		4 A-2-5 A-2	2-6 A-2-7			A-7-5, A-7-6	A-3	A-6, A-7					RESSIBILITY			NON-CRYSTAL ROCK (NCR)	LINE		SEDIME	ENTARY ROCK	TAIN METAMORPHIC AND NON-COASTA THAT WOULD YEILD SPT REFUSAL
SYMBOL	000000000000000000000000000000000000000		44	333							SLIGH MODE!	ITLY CO RATELY	OMPRESSIBLE	F	LL < 31 LL = 31	- 50	COASTAL PLA	IN				S PHYLLITE, SLATE, SANDSTONE, ETO IMENTS CEMENTED INTO ROCK, BUT
% PASSING	000000000000000000000000000000000000000				<u></u>				SILT-			Y COMF	PRESSIBLE		LL > 50		SEDIMENTARY (CP)			SPT R		TYPE INCLUDES LIMESTONE, SANDS
*10 5 *40 3	50 MX 30 MX 50 MX	51 MN						GRANULAR SOILS	CLAY	MUCK. PEAT		P		<u>GE OF MATEF</u>			-				WEATH	ERING
	5 MX 25 MX		X 35 MX 35	MX 35 MX	36 MN	36 MN 36 M	1N 36 MN		SOILS		ORGANIC MATERIAL		GRANULAR SOILS	SILT - CLAY SOILS		R MATERIAL	FRESH				HT.FEW JOINTS	MAY SHOW SLIGHT STAINING. ROCK
MATERIAL PASSING #40											TRACE OF ORGANIC MA LITTLE ORGANIC MATT		2 - 3% 3 - 5%	3 - 5% 5 - 12%	TRACE LITTLE	1 - 10% 10 - 20%			R IF CRYSTA			
LL	-		X 41 MN 40					SOILS LITTL			MODERATELY ORGANIC		5 - 10%	12 - 20%	SOME	20 - 35%	(V SLI.)					OME JOINTS MAY SHOW THIN CLAY C HINE BRIGHTLY, ROCK RINGS UNDER H
PI	6 MX		X 10 MX 11				_	MODE	RATE	HIGHLY	HIGHLY ORGANIC		> 10%	> 20%	HIGHLY	35% AND ABOVE	4		CRYSTALLINE			
GROUP INDEX	0	0	0	4 MX	8 MX	12 MX 16 M	IX NO MX	amoun Orga		SOILS					TE: V AETE		SLIGHT (SLI.)					ND DISCOLORATION EXTENDS INTO RO N GRANITOID ROCKS SOME OCCASIONA
USUAL TYPES S OF MAJOR	GRAVEL, AND		SILTY OR CL GRAVEL AND		SIL 1 SOIL		LAYEY SOILS	MAT	TER					BORE HOLE IMMEDIA		( DRILLING		CRYST	ALS ARE DUL	L AND DIS	SCOLORED. CRY	STALLINE ROCKS RING UNDER HAMMER
MATERIALS	SAND	SHILD		JHINU	501		30123				 			VEL AFTER <u>24</u>			MODERATE (MOD.)					COLORATION AND WEATHERING EFFECTS
GEN. RATING AS SUBGRADE		EXCELLENT	to good		F	air to po	DR	FAIR TO POOR	POOR	UNSUITABLE				ATURATED ZONE, OR	WATER BEA	RING STRATA		DULL S	SOUND UNDER			OWS SIGNIFICANT LOSS OF STRENGTH
	F	PI OF A-7-5	SUBGROUP IS	≤ LL - 3	300;PIO	- A-7-6 SU	BGROUP IS	> LL - 30				SPRIM	ING OR SEEP				MODERATELY		FRESH ROCK.			STAINED. IN GRANITOID ROCKS, ALL F
			CONSIS	<b>FENCY</b>	OR	DENSE	NESS					1	MISCELLA	NEOUS SYMBO	DLS		SEVERE	AND DI	ISCOLORED AN	AND A MAJO	DRITY SHOW K	OLINIZATION. ROCK SHOWS SEVERE L
PRIMARY S			ACTNESS			E OF STA			E OF UNC			ANKMEN.	NT (RF) 25/0	25 DIP & DIP DIR	FCTION		(MOD. SEV.)		AN BE EXCAV S <i>TED, WOULD</i>			'S PICK. ROCK GIVES "CLUNK" SOUND
	PRIMART SUL TTPE CONSISTENCY PENETRATIUM RESISTENCE COMPRESSIVE STRENGT (N-VALUE) (TONS/FT <sup>2</sup> )						2)	WITH SOIL DES	SCRIPT	ION	OF ROCK STRU			SEVERE					STAINED. ROCK FABRIC CLEAR AND E			
GENERAL		VE	RY LOOSE LOOSE			< 4 4 TO 10					SOIL SYMBOL		•	DPT DMT TEST BOP		SLOPE INDICATOR	(SEV.)					I GRANITOID ROCKS ALL FELDSPARS A RONG ROCK USUALLY REMAIN.
GRANULA MATERIA		MED	IUM DENS	E		10 TO 3	2		N/A			ILL (AF			Ā	CONE PENETROMETER					N VALUES >	
(NON-CO		VE	DENSE RY DENSE			30 TO 5 > 50	0				THAN ROADWAY			AUGER BORING	$\mathbf{\Theta}$	TEST	VERY SEVERE					STAINED. ROCK FABRIC ELEMENTS AR NIL STATUS, WITH ONLY FRAGMENTS OF
		-	RY SOFT			< 2			< 0.25		INFERRED SOIL	L BOUN		- CORE BORING	•	SOUNDING ROD	(V SEV.)	REMAIN	NING. SAPROL	LITE IS AN	EXAMPLE OF	ROCK WEATHERED TO A DEGREE THAT
GENERAL SILT-CL		MED	SOFT			2 TO 4 4 TO 8			0.25 TO 0			'K I INF	<u>.</u> MW	) MONITORING WE	a 📥	TEST BORING	COMPLETE					N. IF TESTED, WOULD YIELD SPT N V DISCERNIBLE, OR DISCERNIBLE ONLY
MATERIA	L		STIFF			8 TO 15	i		1 TO 2		_			PIEZOMETER	$\Psi$	WITH CORE		SCATTE	ERED CONCEN	NTRATIONS.		BE PRESENT AS DIKES OR STRINGERS
(COHESIV	/E)	VE VE	RY STIFF HARD			15 TO 3 > 30	٥		2 TO 4 > 4		ALLUVIAL SOIL	L BOUN	NDARY Z	INSTALLATION	$\bigcirc$	- SPT N-VALUE		ALSO A	AN EXAMPLE.			DDNEGO
			TEXTI	JREO	r Gr	AIN S	IZE					R	RECOMMEN	DATION SYMB	OLS						ROCK HA	
U.S. STD. SIE	VE SIZE		4	10	40	60	200	270					ICLASSIFIED E	XCAVATION -		SIFIED EXCAVATION -	VERY HARD				E GEOLOGIST'S	PICK. BREAKING OF HAND SPECIMENS PICK.
OPENING (MM	1)		4.76	2.00	0.42			0.053					ISUITABLE WAS		USED I	ABLE, BUT NOT TO BE N THE TOP 3 FEET OF	HARD				E OR PICK ONL	Y WITH DIFFICULTY. HARD HAMMER B
BOULDER		BBLE	GRAVEL		COARS SAND		F INE SAND		SILT	CLAY			CEPTABLE DE	XCAVATION - GRADABLE ROCK	EMBAN	KMENT OR BACKFILL	MODERATELY		TACH HAND S			JGES OR GROOVES TO 0.25 INCHES DE
(BLDR.)		0B.)	(GR.)		(CSE. S	D.)	(F SD.	) (	SL.)	(CL.)			ABBI	REVIATIONS			HARD	EXCAVA	ATED BY HAR	RD BLOW O		T'S PICK, HAND SPECIMENS CAN BE D
GRAIN MM SIZE IN.		75 3		2.0		0.25		0.05	0.005		AR - AUGER REFUSAL BT - BORING TERMINATED	ı		MEDIUM - MICACEOUS		- VANE SHEAR TEST - WEATHERED	MEDIUM		DERATE BLOW		0 05 INCHES	DEEP BY FIRM PRESSURE OF KNIFE O
5120 114.		-	סודסזר					TEDMC			CL CLAY		MOD	MODERATELY	7-	UNIT WEIGHT	HARD	CAN BE	E EXCAVATED	D IN SMALL	_ CHIPS TO PE	ICES 1 INCH MAXIMUM SIZE BY HARD
SOIL	MOISTURE :			<u> LU</u> ELD MOIS							CPT - CONE PENETRATION CSE COARSE	1 TEST		ION PLASTIC ORGANIC	$\gamma_{d}$	DRY UNIT WEIGHT	COLT		OF A GEOLOG			NFE OR PICK. CAN BE EXCAVATED IN
	ERBERG LIN			DESCRIPT		GUI	DEFORF	IELD MOIS	STURE DES	CRIPTION	DMT - DILATOMETER TES		PMT -	PRESSUREMETER TE		MPLE ABBREVIATIONS	SOFT	FROM (	CHIPS TO SE	EVERAL INC	HES IN SIZE	BY MODERATE BLOWS OF A PICK POIN
			-	SATURATI	ED -				WET, USU4		DPT - DYNAMIC PENETRAT e - VOID RATIO	TUN TE		SAPROLITIC SAND, SANDY	S - I SS -	SPLIT SPOON	VEDV				INGER PRESSU	
LL		IIMIT		(SAT.)		FRO	M BELOW	THE GRO	UND WATE	R TABLE	F - FINE FOSS FOSSILIFEROUS			SILT, SILTY SLIGHTLY		SHELBY TUBE ROCK	VERY SOFT					VATED READILY WITH POINT OF PICK. FINGER PRESSURE. CAN BE SCRATCH
PLASTIC						SEM	ISOLID: F	EQUIRES D	DRYING TO		FRAC FRACTURED, FRAC	TURES	TCR -	TRICONE REFUSAL		RECOMPACTED TRIAXIAL		FINGER				
RANGE <			-	WET - (W	D			MUM MOIS			FRAGS FRAGMENTS HI HIGHLY		w - M V - VE	OISTURE CONTENT	CBR	<ul> <li>CALIFORNIA BEARING RATIO</li> </ul>		RACT	TURE SP		-	BEDDING
FLL.														ON SUBJECT	PROJE		VERY WIDE	ε	MOR	SPACINO RE THAN 10		TERM VERY THICKLY BEDDED
	OM _ OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE					ISTURE	DRILL UNITS:	1	ANCING TOOLS:		HAMMER		WIDE MODERATE			3 TO 10 F 1 TO 3 FE		THICKLY BEDDED 1. THINLY BEDDED 0.1				
SL .		AGE LIMIT						DITIONAL			CME-45C		CLAY BITS		X AU	TOMATIC MANUAL	CLOSE		Ø.	0.16 TO 1 F	-00T	VERY THINLY BEDDED 0.0
			-	DRY - (D	)			MUM MOIS	WATER TO	J			6" CONTINUOU	S FLIGHT AUGER	CORE SI	ZE:	VERY CLO	SE	LESS	S THAN 0.1	16 FEET	THICKLY LAMINATED 0.00 THINLY LAMINATED <
	PLASTICITY							CME-55	×	8 HOLLOW AU	IGERS	-в_	П-н					INDUR				
	PLASTICITY INDEX (PI) DRY STRENGTH						тн	× СМЕ-550		FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEN					NG OF MATERIAL BY CEMENTING, HE							
	NON PLASTIC 0-5 VERY LOW						VANE SHEAR TEST		TUNGCARBID	E INSERTS	<u>□</u> -N _		FRIABL	_E				INGER FREES NUMEROUS GRAINS: Y HAMMER DISINTEGRATES SAMPLE.				
	GHTLY PLAS ERATELY PL				6-15 16-25				SLIGHT MEDIUM				CASING	W/ ADVANCER		OLS: ST HOLE DIGGER						
	ILY PLASTI				OR MO	RE			HIGH		PORTABLE HOIST			STEEL TEETH		ND AUGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH BREAKS EASILY WHEN HIT WITH HAMMER.					
				CC	DLOR						۱ <sub>¬</sub>		TRICONE	TUNGCARB.		UNDING ROD	INDURA	ATED				FICULT TO SEPARATE WITH STEEL
DESCRIPT	IONS MAY 1	INCLUDE (	OLOR OR	COLOR C	OMBINA	TIONS (T	AN, RED,	YELLOW-BF	ROWN, BLUE	GRAY).			CORE BIT			NE SHEAR TEST						REAK WITH HAMMER.
MOI	DIFIERS SU	CH AS LI	GHT, DARK,	STREAKE	ED, ETC	ARE US	ED TO DE	SCRIBE A	PPEARANCE								EXTRE	MELY I	NDURATED			BLOWS REQUIRED TO BREAK SAMPLE ACROSS GRAINS.

### PROJECT REFERENCE NO.



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D. AN INFERRED	TERMS AND DEFINITIONS
SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
FOOT PER 60 IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.
	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. A REAL - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CK THAT CLUDES GRANITE,	WHICH IT S BROONTERED, BUT WHICH DOES NOT NECESSARILY RISE TO A BOVE THE GROUND SURFACE.
L PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
IF TESTED.	$\underline{\text{COLLUVIUM}}$ - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
MAY NOT YIELD TONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
	$\underline{\text{DIKE}}$ - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
RINGS UNDER	$\underline{\text{DIP}}$ - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
DATINGS IF OPEN, AMMER BLOWS IF	<u>DIP DIRECTION (DIP AZIMUTH)</u> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
CK UP TO - FELDSPAR	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
S. IN Y. ROCK HAS AS COMPARED	<u>FLOAT</u> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
ELDSPARS DULL DSS OF STRENGTH	<u>FORMATION (FM.)</u> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
VIDENT BUT RE KAOLINIZED	<u>LEDDE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
NE KHOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
E DISCERNIBLE STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
ONLY MINOR ALUES < 100 BPF	OF AN INTERVENING IMPERVIOUS STRATUM.
IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
. SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
S REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
OWS REQUIRED	<u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDE ROCKS.
EP CAN BE ETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
R PICK POINT. BLOWS OF THE	STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF)OF A 140 LB.HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
FRAGMENTS T. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD)- A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GRAFTER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
ED READILY BY	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	BENCH MARK: •see note
THICKNESS	
4 FEET 5 - 4 FEET	ELEVATION: FEET
6 - 1.5 FEET	NOTES:
3 - 0.16 FEET 8 - 0.03 FEET	•Elevations derived from geopak and the .tin file
0.008 FEET	(W5522_Is_tnl.tin/ dated 9/27/2016
AT, PRESSURE, ETC.	
HI, FRESSURE, EIC.	
EEL PROBE;	
PROBE;	
:	DATE: 8-15-14



	STATE	STATI	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
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PAT McCRORY Governor

NICHOLAS J. TENNYSON Secretary

November 29, 2016

STATE PROJECT:	50143.1.FD1 (W-5522)
FEDERAL PROJECT:	HSIP-1656(5)
COUNTY:	Wake
DESCRIPTION:	SR 1656 (Trinity Rd.) at SR 1658 (Youth Center Dr.) Intersection in Raleigh.
	Construct Pedestrian Tunnel Under SR 1656 (Trinity Rd.)

SUBJECT: Geotechnical Report – Inventory

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## **Project Description**

This project lies in the city of Raleigh in Wake County. The project consists of constructing a pedestrian tunnel (-Y1-) underneath Trinity Rd. (-L-). In addition, Trinity Rd. will be upgraded to include sidewalks and curb and gutter. The total mainline (-L-) project length is 0.145 miles.

Six SPT borings were performed at various offset locations along the -L- alignment by the NCDOT Geotechnical Engineering Unit. The work was performed on October of 2016. A CME-550 was used during the field investigation. Representative samples were collected for visual classification in the field and were submitted for laboratory analysis by the Materials and Tests Unit.

The following alignments, totaling 0.18 miles were investigated. Subsurface soil profiles of these alignments are included in this report.

<u>Line</u>	<u>Stations</u>
-L-	11+82 to 16+92
-Y1-	10+00 to 12+62
-Y3-	10+00 to 11+57

## **Areas of Special Geotechnical Interest**

1) <u>Highly Plastic Clays</u>: Clays with a PI > 25 were encountered in the following locations:

<u>Line</u>	<u>Station</u>	<u>Offset</u>
-L-	16+07	79 RT
-L-	15+31	74 RT
-L-	14+28	73 RT

## **Physiography and Geology**

The project is located in the Piedmont physiographic province of North Carolina. The project corridor is urban commercial. Grassy fields lie along the project corridor. The terrain is generally flat. Geologically, the soils in this region are derived from the underlying phyllites and schists belonging to the Raleigh Belt. These rocks were formed by regional metamorphism in this area during the Permian Period.

Soils encountered during this investigation are separated into 2 categories: Roadway Embankment and Residual soils.

Roadway Embankment soils are likely derived from nearby sources and are similar to Residual soils in composition. These soils generally consist of stiff, brown and tan, sandy silt (A-4).

Residual soils are derived from the weathering of underlying rock in the area. These soils consist of tan, red and brown, medium stiff to hard, saprolitic, sandy silt (A-4), clayey silt (A-5) and silty clay (A-7-6).

Groundwater was encountered in all borings. Groundwater elevation ranges from 420± to 430± feet above sea level.

## **Soil Properties**

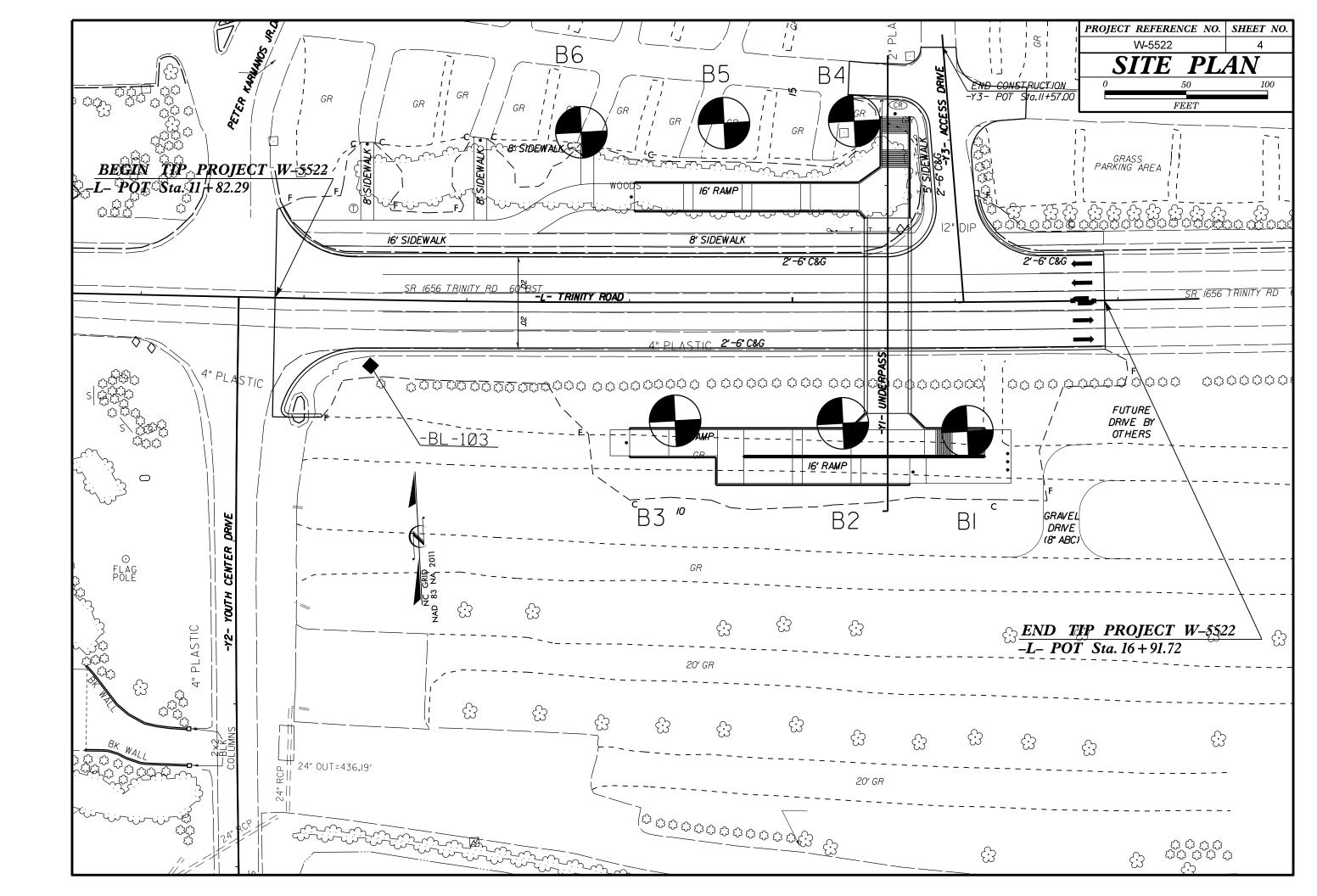
## **Groundwater**

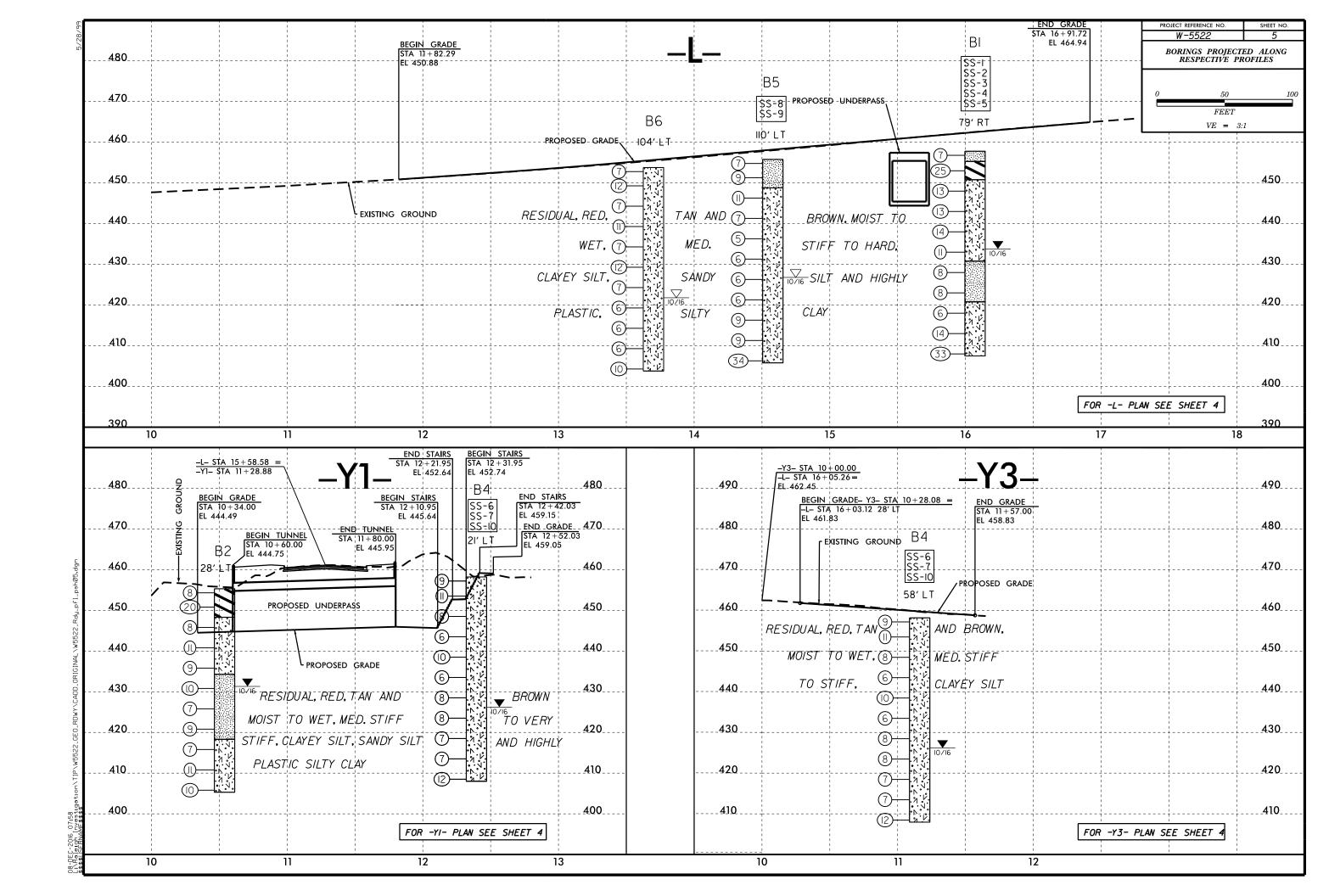
Respectfully submitted,



Jarett Swartle 7E355C29E75A413 12/15/2016

Jarett Swartley, L.G. **Project Geological Engineer** 

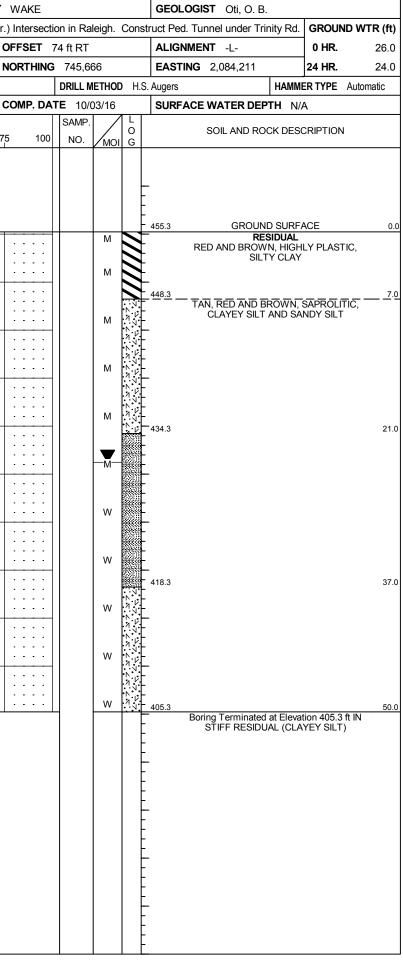




## GEOTECHNICAL BORING REPORT BORE LOG

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		<b>_EV</b> . 45				OTAL DEP			NORTHING	745,6	57		EAST	NG 2,084,288	24 HR.	24.0	COL	LAR ELE	<b>EV.</b> 45	55.3 ft		тс	DTAL DE	<b>PTH</b> 50.0	ft	NO
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		Pinter, D				TART DAT			COMP. DA					ACE WATER DEPTH	N/A		DRIL	LER Pi					ART DA	TE 10/03/		CC
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410	409.0	48.8					<u></u>	<u> </u>	· · · · · ·			N N V	-				410	-	ŧ							
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## SHEET 6



# GEOTECHNICAL BORING REPORT BORE LOG

WBS 50143.1.FD1		TY WAKE	GEOLOGIST Oti, O. B.	WBS 50143.1.FD1	<b>TIP</b> W-5522	COUNTY WAKE	GEOLOGIST Oti, O. B.
			truct Ped. Tunnel under Trinity Rd. GROUND WTR (ft)				Construct Ped. Tunnel under Trinity Rd. GROUND WTR (ft)
BORING NO. B3	STATION 14+28	OFFSET 73 ft RT		BORING NO. B4	STATION 15+38	OFFSET 111 ft LT	
COLLAR ELEV. 453.2 ft					TOTAL DEPTH 50.2 ft		
	TOTAL DEPTH 50.0 ft	NORTHING 745,673	EASTING 2,084,109 24 HR. FIAD	COLLAR ELEV. 458.2 ft		NORTHING 745,851	EASTING         2,084,228         24 HR.         32.0
DRILL RIG/HAMMER EFF./DATE RFC		DRILL METHOD H.S		DRILL RIG/HAMMER EFF./DATE RFOO			
<b>DRILLER</b> Pinter, D. G.	UNT BLOWS PER FOO	COMP. DATE         10/03/16           OT         SAMP.         ▼         ↓	SURFACE WATER DEPTH N/A	DRILLER Pinter, D. G.	INT BLOWS P		SURFACE WATER DEPTH N/A
ELEV DRIVE ELEV (ft) DEPTH (ft) 0.5ft 0.5ft	0.5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV DRIVE ELEV (ft) DEPTH BLOW COU (ft) 0.5ft 0.5ft			O SOIL AND ROCK DESCRIPTION G
455				460			
453.2 0.0	12 19		453.2 GROUND SURFACE 0.0 RESIDUAL	458.2 0.0 3 3		M	458.2         GROUND SURFACE         0.0           RESIDUAL
450 449.7 3.5			TAN, RED AND BROWN, HIGHLY PLASTIC, SILTY CLAY	455 454 5 2 7			TAN, RED AND BROWN, SAPROLITIC, CLAYEY SILT
449.7 - 3.5 7 11	14	· · · · ·     M 🗙	-	454.5 3.7 454.5 454.5	6 · <u>1</u> · · · · · ·	···· SS-6 M	
			446.2 7.0				
445 444.7 8.5 3 3	5		TAN, RED AND BROWN, SAPROLITIC, SANDY SILT AND CLAYEY SILT	450 449.5 8.7		····	
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410 409.7 43.5			_	415 414.5 43.7		····	
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			Boring Terminated at Elevation 403.2 ft IN		• <u>•</u> 12		Boring Terminated at Elevation 408.0 ft IN
			SŤIFF RESIDUAL (CLAYEY SILT)				SŤIFF RESIDUAL (CLAYEY SILT)
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## SHEET 7

# GEOTECHNICAL BORING REPORT BORE LOG

					W-5522 COUNTY WAKE	
<b>WBS</b> 50143.1.FD1		JNTY WAKE	GEOLOGIST Oti, O. B.			GEOLOGIST Oti, O. B.
			ruct Ped. Tunnel under Trinity Rd. GROUND WTR (ft)	,	at SR 1658 (Youth Center Dr.) Intersection in Raleigh.	
BORING NO. B5	<b>STATION</b> 14+58	OFFSET 110 ft LT	ALIGNMENT -L- 0 HR. 29.0		ION         13+70         OFFSET         104 ft LT	ALIGNMENT -L- 0 HR. 32.
COLLAR ELEV. 455.8 ft	TOTAL DEPTH 50.0 ft	NORTHING 745,854	<b>EASTING</b> 2,084,148 <b>24 HR.</b> FIAD		<b>L DEPTH</b> 50.0 ft <b>NORTHING</b> 745,853	EASTING 2,084,061 24 HR. FIAI
	RFO0067 CME-550X 85% 07/12/2016	DRILL METHOD H.S		DRILL RIG/HAMMER EFF./DATE RF00067 CME-		
DRILLER Pinter, D. G.	START DATE 10/05/16	COMP. DATE 10/05/16	SURFACE WATER DEPTH N/A		T DATE 10/05/16 COMP. DATE 10/05/16	SURFACE WATER DEPTH N/A
	V COUNT         BLOWS PER F           0.5ft         0.5ft         0         25         50	COOT         SAMP.         L         O           75         100         NO.         MOI         G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH (ft)	ELEV (ft)         DRIVE ELEV (ft)         DEPTH         BLOW COUNT           0.5ft         0.5ft         0.5ft         0	BLOWS PER FOOT SAMP. 25 50 75 100 NO. MOI	O SOIL AND ROCK DESCRIPTION G
460			_	455 453.8 + 0.0		_ 453.8 GROUND SURFACE
455 455.8 0.0 2	3 4		455.8 GROUND SURFACE 0.0 RESIDUAL	450 450.3 3.5 2 4	M	RESIDUAL       Y-     TAN, RED AND BROWN, SAPROLITIC,       Y-     CLAYEY SILT
452.3 3.5	4 5 k · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	TAN, RED AND BROWN, SAPROLITIC, SANDY SILT AND CLAYEY SILT		M	
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442.3 13.5 2	3 4				M M	
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437.3 18.5	2 3 45		-	430 430.3 23.5 3 4 8		
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407.3 48.5 7		······································	405.8 50.0 Boring Terminated at Elevation 405.8 ft IN			Boring Terminated at Elevation 403.8 ft IN     STIFF RESIDUAL (CLAYEY SILT)
			HĂRD RESIDUAL (CLAYEY SILT)			- - -
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## SHEET 8

				SOL	L $2$	TES	ST R	$\overline{ESUI}$	LTS						
SAMPLE	OFFSET	STATION	DEPTH	AASHTO	L.L.	P.I.			WEIGHT			SING (SI		%	%
NO.	011011	811110H	INTERVAL	CLASS.	Б.Б.	1.11	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS- 1	79′ RT	16 +07	3.8-5.3	A-7-5(30)	71	37	9.9	17.5	10.2	62.4	96	91	74	-	-
SS- 2	79′ RT	16 +07	8.8-10.3	A-5(0)	47	NP	13.3	47.1	21.5	18.1	99	91	52	-	-
SS- 3	79′ RT	16 +07	18.8-20.3	A-5(0)	42	NP	15.7	48.5	23.7	12.1	95	86	49	-	-
SS- 4	79′ RT	16 +07	28.8-30.3	A-4(0)	39	NP	13.1	59.8	19.0	8.1	97	90	40	-	-
SS- 5	79′ RT	16 +07	38.8-40.3	A-5(0)	43	NP	17.7	51.2	19.0	12.1	91	80	42	-	-
SS- 6	111' LT	15+38	3.7-5.2	A-5(5)	44	7	11.9	33.4	26.5	28.2	100	94	64	-	-
SS- 7	111' LT	15+38	23.7-25.2	A-5(0)	49	NP	21.6	37.9	28.5	12.1	95	80	50	-	-
SS- 8	110' LT	14 +58	8.5-10.0	A-5(3)	42	5	12.5	39.9	29.5	18.1	98	91	58	-	-
SS- 9	110′ LT	14 +58	28.5-30.0	A- 5( 0)	45	NP	23.2	33.6	29.1	14.1	92	77	50	-	-
SS- 10	111' LT	15+38	43.7-45.2	A-5(0)	44	NP	15.5	52.2	18.2	14.1	92	83	42	-	-

