This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document -

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page. This file or an individual page shall not be considered a certified document.

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

444

Ŕ

REFERENCE

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>
-L-	II+46 TO 42+00	4,5

CROSS SEC	TIONS	
<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	18+75	6
-L-	20+25	6
-L-	21+00	6
-L-	22+50	7
-L-	24+50	7
-L-	27+50	7
-L-	28+25	8
-L-	29+75	8
-L-	31+50	8
-L-	34+75	9
-L-	35+75	9

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY BURKE

PROJECT DESCRIPTION BRIDGES 160 & 162 ON I-40 OVER SR 1758 (BEREA CHURCH ROAD)

INVENTORY

7 383 PROIEC

STATE PROJECT REFERENCE NO. STATE SHEETS NO. 10 N.C **B-4447** 1

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSIFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITORIED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONSTRUCTIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACTUAL CONDENSATION.

NOTES:

- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REDUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

N. MOHS, LG

SDS

G. SKOGLUND

J. JUSTICE

D. JEFFRIES

INVESTIGATED BY <u>N. MOHS, LG</u>

DRAWN BY <u>N. MOHS</u>, LG

CHECKED BY ______. D. BROWN, PE

SUBMITTED BY <u>N. MOHS</u>, LG





NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOLL DESCRIPTION	GRADATION	ROCK DESCRIPTION	
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOC	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SUILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586), SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM, BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ARENACEOUS - APPLIED TO ROCKS THAT HAVE REEN DERIVED FROM SAND OR THAT CONTAIN SAND
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUC AS MINERAL OGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE.	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGULACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS OR HAVING
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SURANGULAR, SUBROLINDED, OR BOUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION		ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS (< 35% PASSING #200) (> 35% PASSING #200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCLUDNTERED, BUT WHICH DUES NUT NECESSARILY RISE TO UK ABOVE THE GROUND SURFACE.
CENSI C = 501 (Hosting 2007) GR0UP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-0 A-1-0 A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL SCOOL	SLIGHTLY COMPRESSIBLE LL < 31	COASTAL PLAIN ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
7. PASSING	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
10 50 MX GRANULAR SILI MU		WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL GRANULAR SILT - CLAY SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	HORIZONTAL.
LL – – 40 MX 41 MN	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLL) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 11 MN MODERATE	LY HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF SO		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. OF MALIOR GRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR UNSUI	ABLE ∇ PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBGRADE POOR POOR	····· O→MA→ SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
		MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FIELD.
		(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACINESS OR PENETRATION RESISTENCE COMPRESSIVE STRENC CONSISTENCY (NEVALUE) (TONG/ET2)		IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LODSE < 4		(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LIS LAIEMAL EXIENT.
GENERALLY LOOSE 4 TO 10 GRANULAR		TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOULS
MATERIAL DENSE 10 TO 30 N/A		VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NUN-COHESIVE) VERY DENSE > 50		SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25	- INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	UF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0		COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4		SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4		ROCK HARDNESS	SAPROLITE (SAP) - RESIDUAL SOLL THAT RETAINS THE RELIC STRUCTURE OR FARRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
0-ENTING (MM) 4.78 2.00 0.42 0.23 0.075 0.055	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND SILT CLA (BLDR.) (COB.) (GR.) SAND SAND (SL.) (CL.		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
		HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	UK SLIP PLANE.
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
SOU MOISTURE - CORRELATION OF TERMS	CL CLAY MOD MODERATELY γ - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE CUIDE FOR FIELD MOISTURE DESCRIPT	$\frac{1}{2} = \frac{1}{2} = \frac{1}$	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION CODE FOR THEE HOISTORE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST <u>SAMPLE ABBREVIATIONS</u> DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - RIII K	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH	LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
	F FINE SL SILI, SILIY ST - SHELBY TUBE	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY	TOPOOL (TEXE SUPERCE SOLID EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID: REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL		
(PI) PL _ PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	BENCH MARK:
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: FEET
	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINKY BEDDED 0.16 - 1.5 FEET	NOTES
REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	BORING ELEVATIONS GENERATED FROM NODOT FILE R4447 IS THE TIN
ATTAIN OPTIMUM MOISTURE	CME-55 6' CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	□ · · · · · · · · · · · · · · · · · · ·	INDURATION	4
PLASTICITY INDEX (PI) DRY STRENGTH	X CME-550 X HARD FACED FINGER BITS	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	CASING W/ ADVANCER POST HOLE DIGGER	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
	VORTABLE HOIST TRICONE'STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
		INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY		SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE:	
HOSE IERS SUCH AS LIGHT, DAME, STREAKED, ETC. HAE USED TO DESCRIBE APPEARANCE.		SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

PROJECT REFERENCE NO.



2



	STATE	STATE	PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
	N.C.		3–4447	3	10
	STAT	E PROJ.NO.	P. A. PROJ. NO.	DESCRIPT	10N
	38	3/1.1.1	BRNHS-40-1(159)115	PE	
				S	1
RES	74	XXX	RHYXX	WE CA	A
۲LU ۲	ŦY	<u>tan</u>	ATCH A	Sen .	Ľ,
Ľ			LAN LAN	EV/	
			V Y		
END	TIP P	ROJECT	B-4447		
/ _L_ P	POT ST	A. 42 + 0	0.00		
- /					
\neg					
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~					
			INCOMPLETE	PLAN	IS
			PRELIMINAR	Y PLA	NS
			DO NOT USE FOR CO	NSTRUCTION	
HYDRAIILICS F	VGINEE	R	γ <u> </u>		=
IIIDAAULIUS EI	OMEE		1		
			OF NOA	TH	
			ATE	CAROI	
SIGNATURE:	P.E	:			
ROADWAY DE	SIGN				
ENGINEER	2		A REAL	A A A A A A A A A A A A A A A A A A A	/
			OF TRI	MSPS	
	рг				
SIGNATURE	r.E.	-	1		



October 9, 2015

STATE PROJECT:	38371.1.1 (B-4447)
COUNTY:	Burke
DESCRIPTION:	Bridges No. 160 & 162 on I-40 over SR 1758 (Berea Church Road)

SUBJECT: Geotechnical Report – Inventory

# **Project Description**

This project consists of the construction of a detour alignment (-DETEB-) to the south of I-40 (-L-) for the construction of a temporary roadway and bridge over SR 1758 (Berea Church Road, -Y-). The detour will allow two lanes of eastbound traffic to be maintained while the existing bridges No. 160 & 162 are replaced. The project begins 0.3 miles west of SR 1758 and continues for 0.6 miles east.

The geotechnical field investigation was conducted in September of 2015. The drilling investigation was conducted by Soil Drilling Services based in Cornelius, North Carolina, and overseen by Stewart personnel. An ATV-mounted CME-550 drill machine with an automatic hammer was used during the investigation. Eleven Standard Penetration Test (SPT) borings were performed at selected sites along the alignment to collect data on soils that will be encountered during the proposed earthwork. Representative soil samples were collected in the field for visual classification, with 7 samples being subjected to laboratory analysis.

The following alignments, totaling 1.2 miles were investigated. Subsurface cross sections are included in this report:

<u>Alignment</u>	Stations
-L-	11+46 to 42+00
-DETEB-	10+00 to 40+06

# Physiography & Geology

The project is located in the rolling terrain of Burke County. The surrounding land is a mix of homes, woods, and pastures. Geologically is underlain by metamorphic Mica Schist of the Inner Piedmont Belt. These rocks were formed millions of years ago during continental collisions between ancient North America and Africa that formed the Appalachian Mountains.

# Areas of Special Geotechnical Interest

1. Weathered Rock – Weathered Rock (Mica Schist) was encountered in the following borings:

<u>Alignment</u>	Station & Offset (ft)
-L-	18+85, 85 RT
-L-	20+25, 135 RT
-L-	21+00, 104 LT
-L-	22+50, 86 RT
-L-	24+50, 137 RT
-L-	29+75, 130 RT
-L-	31+50, 90 RT

# Soil Properties

Soils encountered at the site include artificial fill and residual soils. The artificial fill consists of wet, loose, silty sand (A-2-4). This material is associated with the realignment of a small creek during the construction of I-40. Residual soils are the weathered remnants of the parent rock material. These soils include dry to moist, medium dense to dense, silty sand (A-2-4), and medium stiff to hard, sandy silt, clayey silt, and silty clay (A-4, A-5, A-7). Roadway embankment is expected in the sections of I-40 leading up to the bridges over SR 1758.

# **Rock Properties**

Weathered rock (Mica Schist) was encountered in several borings within and below proposed roadway cut slope sections. Differential weathering of the parent rock has created alternating layers of weathered schist and soil that are shown on the cross sections. This material is exposed at the ground surface along portions of the project alignment and is expected to be mechanically ripable.

# **Debris Piles**

Two debris piles were found within the project limits during the subsurface investigation. These piles are located at -L- 25+25, 140 feet RT, and -L- 25+75, 200 feet RT. The piles consist of large logs, timber, and felled trees from the clearing of the Piedmont Natural Gas right of way that runs along the right side of the project.

# Groundwater

Groundwater was encountered at -L- 24+50, 137 feet RT at a depth of 5.4 feet, near a small creek that flows north through a culvert under I-40. Groundwater at this location may impact proposed culvert work. Borings performed at the top of the proposed cut slopes to below the ditch line did not encounter groundwater, and therefore groundwater is not expected to impact construction of the slopes.



P	roject reference nc	).	SHEET NO.				
	B-4447		4				
	R/W SHEET N	10.					
RC	DADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER				
	INCOMPLE DO NOT USE FOR		PLANS COUISITION				
	PRELIMINA DO NOT USE FO	RY CONST	PLANS TRUCTION				



	PROJECT REFERENCE NO. SHEET NO.						
	B-4447	5					
	RW SHEET NO						
	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER					
	INCOMPLET	E PLANS					
	PRELIMINAE DO NOT USE FOR	Y PLANS CONSTRUCTION					
		J					
<u>END</u> CONS	T. TIP PROJECT B-4447						
-DETWB- ST ×	A. 42+00	\××××-					
··· ·· ·· ·· ··	^						
		$\backslash$					
	<u>المناحدة المنابعة من </u>	سننسناسناس					
		<u>_</u>					
	B						
<u> </u>							
	8						







6	1		1	1	Ī		i i	Ī	1	I	I	1	I	i i	T	1	i i	ī	0	5 10	PROL	REFERENCE NO	SHEFT NO
23/6					1			1	1				   	1 I 1 I 1 I						<b>ŤTT</b>	TROJ	B-4447	9
^{مَ}	140 130	120 110	100 9	90 80	70	60	50 40	0 30	20	0 10	Ó	10 2	20 3	30 40	50	60	70 80	90	100	110	120	130 140	150
	·			+				<u> </u> !				·											·
				1 1																			
					1	1								1 I 1 I 1 I									
·	-L- SLOFL	OPE			   			   	   	   	   	   	   	     +					!		-4	!	·
1				AVEN EDOM	1																		1
1	ELECTRONIC FILE	S RECEIVED FROM NO DRAWN THROUGH T	CDOT GEU. INFERF THE BORINGS WITH	RED H BOTH PROJEC	CTED					I I													1
	ONTO THE CROSS	SECTION.		+							<del>1</del>	 	- - -	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	<mark> </mark>	$-\frac{1}{1}$					- +		
					1																		
					1																		1
				+			·	<del> </del> 								- +					- +	 	· +
										1													
1					1														1				1
   	l L		 I I		   	   				   	+		- L	+		- ±		 ! !			- 1		·
					   																		1
1			   		   	   		   		   	1				   			   			1		   
1		· <del></del>			·					   		   				1		 I I			T	 	   
					1																		
	     		   	1 1 1 +				·						i i 					 	   		     	
	· · - · - · - · - · - · - · - · -						·!							i +									·
				1 I I I I I		1	ii	i		SOL			SULT	S			i i	<u> </u>					1
					SAMP	LE		DE	PTH	AASHTO		<u>1 111/</u> %	6 BY WEIG	GHT %	PASSING	(SIEVES)	%	%					
	·			+	NO.	OFFSE	T STATIO	N INTE	ERVAL	CLASS.	L.L. $P.I.$	C.SAND F.	SAND SI	ILT CLAY	10 40	200	MOISTURE O	RGANIC					
						<u>35   80 H</u>	71 35+80	0 8.5	- 10.0	A-7-5(7)	50 18	30.56 1	16.41 14	4.74 38.29 92	2. 72 78. 2	9 55.79	18.4						
1220																							1990
1220	·			+		+	·	+						+									
														-DETEB-				1					
1210														35 + 79.13			SS-35						
									1	 I I	1202 23	   	   	¥ ¦	I I	1	35+80						
											1202.20												
	· ¦			; 						·		ROADWAY EM	I <u>BANKMENT</u>										1200
						<						RE	ESIDUAL.RED	BROWN, MOIST, VERY	STIFF.MODER	RATELY PLAS	sтис, 🔟	SILTY CLAY					
																	(5-						
<i>1190</i>			—	+											INST VERY S								<i>1190</i> _
													L L	DRANGE AND WHITE,	UISI VERI SI	I IFF, SAFRUL		ANDI SILI					
										35	+75.00	)											
								<u>-</u>															·
1220																							1220
	· i i							<u>-</u>						<u></u>		- <u>-</u>							
1210					1	1								ę	-DETEB-			1					1210
										ព					34+78.91								
						~											_EXISTING G	ROUND					
								— — <u> </u>	IOADWAT L		<u>1202</u> 90					- <u>+</u>	<u> </u>					· · · · · · · · · · · · · · · · · · ·	1200
			R	RESIDUAL.ORANG	æ 🎽 📓	AND BROWN.MO	DIST.VERY STIFF	F TO HARD, SA	PROLITIC.	SANDY SILT													
					47-			1	1														
				$\frac{1}{1}$ $\frac{1}{1}$			·							· · · · · · · · · · · · · · · · · · ·									
) ) + <del>()</del> + <del>()</del>					(09/15)				1	24	75 00												
\$ \$ □								1	1	54	+/3.00	נ											
				+						 				· · · · · · · · · · · · · · · · · · ·		+					- +		· +
								1		•	-1-												
÷	140 130	120 110	100 9	90 80	70	60	50 40	0 30	20	o 10	ò	10 2	20 3	30 40	50	60	70 80	90	100	110	120	130 140	150