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	INDEX	
STATION	DESCRIPTION	SHEETS
25+85.40 -L-	BRIDGE ON I-40 OVER SR 1758 (WESTBOUND LANE)	S01-1 THRU S01-28
25+85.40 -L-	BRIDGE ON I-40 OVER SR 1758 (EASTBOUND LANE)	S02-1 THRU S02-28
24+88.00 -L-	SINGLE 6' × 6' CULVERT EXTENSION	C-1 THRU C-6





PROJE	7 UNTY								
STATI	STATION:								
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH									
	——————————————————————————————————————								
	REVIS	SIONS							
NO. BY:	DATE:	NO. E ର	Y:	DATE:					
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NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 115 TONS PER PILE. DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 195 TONS PER PILE. * PILES LOCATED NEAR THE EXISTING BATTERED PILES CAN BE SHIFTED UP TO 1'-6"TO AVOID INTERFERENCE. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING EXISTING BATTERED PILES BEFORE DRIVING NEW PILES.

DRAWN BY :	DATE :	9/21/15
CHECKED BY :	DATE : .	3/29/16
DESIGN ENGINEER OF RECORD :	. DATE : .	3/29/16

FOUNDATION LAYOUT DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE.





	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPORARY STRUCTURE	REMOVAL OF EXISTING STRUCTURE AT STA. 25+85.40 -L-	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MOD PRE C(G	IFIED 63″ STRESSED NCRETE IRDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP (Stee	12 X 53 Il PILES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	NO.	LIN.FT.	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE			5218	6447		LUMP SUM		5	573.04				230.6		LUMP SUM	LUMP SUM
END BENT NO.1					48.7		6084			11	11	580		375		
END BENT NO.2					48.4		6043			11	11	390		330		
TOTAL	LUMP SUM	LUMP SUM	5218	6447	97.1	LUMP SUM	12127	5	573.04	22	22	970	230.6	705	LUMP SUM	LUMP SUM

DRAWN BY : T.J. KIRSCHBAUM	DATE :	12/14/15
CHECKED BY : R.F. WERTMAN	DATE : _	3/29/16
DESIGN ENGINEER OF RECORD :	DATE : _	3/29/16

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ENT ON WESTBOUND I-40 9.55	
PROPOSED GUARDRAIL -L- = PROPOSED GUARDRAIL (TYP. AS SHOWN) (ROADWAY DETAIL & PAY ITEM) TTTTTTTTTTTTTTT A PAY ITEM TTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTTT	ASSUMED LIVE THIS BRIDGE LRFD BRIDGE THIS BRIDGE FOR OTHER DE FOR OTHER DE FOR SUBMITT FOR FALSEWOF FOR CRANE SA FOR GROUT FO FOR MAINTEN STRUCTURE, SE REMOVABLE FO FORMS IN ACO SPECIFICATIO NEEDLE BEAMS FOR ON THE F INASMUCH AS STEEL CONTATIONS LEAD BASED F REGULATIONS LEAD BASED F REMOVAL OF
WOODS	SPECIFICATION SUBSTITUTION OF THE REINF

VE LOAD = HL-93 OR ALTERNATE LOADING.

E HAS BEEN DESIGNED IN ACCORDANCE WITH THE AAS Design specifications.

IS LOCATED IN SEISMIC ZONE 1.

DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

TAL OF WORKING DRAWINGS, SEE SPECIAL PROVISION

DRK AND FORMWORK, SEE SPECIAL PROVISIONS.

SAFETY, SEE SPECIAL PROVISIONS.

FOR STRUCTURES, SEE SPECIAL PROVISIONS.

NANCE AND PROTECTION OF TRAFFIC BENEATH PROPO SEE SPECIAL PROVISIONS.

ORMS MAY BE USED IN LIEU OF METAL STAY-IN-PL CORDANCE WITH ARTICLE 420-3 OF THE STANDARD ONS.

1S WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED PLANS OR APPROVED BY THE ENGINEER.

THE PAINT SYSTEM ON THE EXISTING STRUCTURAL INS LEAD, THE CONTRACTOR'S ATTENTION IS DIREC 107-1 OF THE STANDARD SPECIFICATIONS.ANY COS ROM COMPLIANCE WITH APPLICALBE STATE OR FEDE S PERTAINING TO HANDLING OF MATERIALS CONTAIN PAINT SHALL BE INCLUDED IN THE BID PRICE FOR EXISTING STRUCTURE AT STATION 25+85.40 -L-".

A CONCRETE IN THE BRIDGE DECK SHALL CONTAIN GROUND GRANULATED BLAST FURNACE SLAG AT THE N RATE SPECIFIED IN ARTICLE 1024-1 AND IN WITH ARTICLES 1024-5 AND 1024-5 OF THE STANDARD ONS.NO PAYMENT WILL BE MADE FOR THIS ON AS IT IS CONSIDERED INCIDENTAL TO THE COST FORCED CONCRETE DECK SLAB.



ΝΟΤΕ	IS:
SHTO	THE EXISTING STRUCTURE CONSISTING OF THREE SPANS @ 45'-O"FT. WITH A CLEAR ROADWAY OF 40' AND REINFORCED CONCRETE DECK ON STEEL GIRDERS ON END BENTS WITH REINFORCED CONCRETE CAPS ON STEEL PILES & INTERIOR BENTS WITH REINFORCED CONCRETE POSTS AND BEAMS LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED.
NS.	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
DSED .ACE	THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STA.26+05.17 -DETEB- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.
D	THE BRIDGE RAILS ON THE TEMPORARY STRUCTURE SHALL BE DESIGNED FOR THE AASHTO LRFD TEST LEVEL 3 (TL-3) CRASH TEST CRITERIA. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.
L TFD	FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
TS FRAI	FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
NING	FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

S NT			PROJE(CT NO. Burł	<u> </u>	-4447 C0	UNTY
И И			STATI	ON: 2 1(<u>5+85.</u> 5+67.	<u>40 - </u> 23 -\	 /
M		TH CAROLINE	DEP/	stat ARTMENT	e of north car OF TRAN RALEIGH	OLINA NSPORTA	TION
		SEAL 037180	G		AL DR	XAWIN	IG
		DocuSigned by: DocuSigned by: Dischered For Ductors	SR 1 Betw	RIDGE 758 BE EEN SR WEST	REA CH 1001 , BOUND	HURCH I AND SR LANE	к ROAD 1761
iff Road		4/12/2017		REVIS	SIONS		SHEET NO.
C 27607-3073 7660	DOCUMEN	NT NOT CONSIDERED	1 №0. ВҮ: 1	DATE:	NO. BY:	DATE:	SUI-S TOTAL SHEETS
o. F-0270	SIGNA	TURES COMPLETED	2		<u></u> Д		28

		LOAD ANE) RES	SIST	ANCE	FACT	OR	RATI	ING (LRFF	<) Sl	JMMA	RY F	OR F	PRES	TRES	SED	CON	CRET	E GI	RDEF	?S				
										STRE	NGTH	I LIN	IT ST	TATE				SE	RVICE	E III	LIMIT STATE					
										MOMENT					SHEAR						MOMENT					
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER		
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.09		1.75	0.821	1.25	А	E	56.6	0.956	1.27	А	I	33.7	0.80	0.821	1.09	А	E	56.6	1		
DESIGN		HL-93 (OPERATING)	N/A		1.62		1.35	0.821	1.62	А	E	56.6	0.956	1.78	А	I	22.2	N⁄A						1		
RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	1.56	56.160	1.75	0.821	1.80	А	E	56.6	0.956	1.85	А	I	22.2	0.80	0.821	1.56	А	E	56.6	1		
	_	HS-20 (OPERATING)	36.000		2.33	83.880	1.35	0.821	2.33	А	E	56.6	0.956	2.44	А	I	22.2	N⁄A						1		
		SH	12.500		4.09	51.125	1.40	0.821	5.87	А	E	56.6	0.956	6.57	А	I	22.2	0.80	0.821	4.09	А	E	56.6	1		
		S3C	21.500		2.39	51.385	1.40	0.821	3.43	А	E	56.6	0.956	3.77	А	I	22.2	0.80	0.821	2.39	А	E	56.6	1		
	ICLI	S3A	22.750		2.26	51.415	1.40	0.821	3.25	А	E	56.6	0.956	3.57	А	I	22.2	0.80	0.821	2.26	А	E	56.6	1		
	H <> <	S4A	26.750		1.97	52.698	1.40	0.821	2.83	А	E	56.6	0.956	3.08	А	I	22.2	0.80	0.821	1.97	А	E	56.6	1		
	CCE (S	S5A	30.500		1.74	53.070	1.40	0.821	2.50	А	E	56.6	0.956	2.78	А	I	22.2	0.80	0.821	1.74	А	E	56.6	1		
	NIS	S6A	34.500		1.57	54.165	1.40	0.821	2.25	А	E	56.6	0.956	2.48	А	I	22.2	0.80	0.821	1.57	А	E	56.6	1		
		S7B	38.500		1.42	54.670	1.40	0.821	2.04	А	E	56.6	0.956	2.29	А	I	22.2	0.80	0.821	1.42	А	E	56.6	1		
NATING		S7A	40.000	3	1.39	55.600	1.40	0.821	2.00	А	E	56.6	0.956	2.31	А	I	22.2	0.80	0.821	1.39	А	E	56.6	1		
	ж ж	Т4А	28.250		1.93	54.523	1.40	0.821	2.76	А	E	56.6	0.956	2.97	А	I	22.2	0.80	0.821	1.93	А	E	56.6	1		
	ACTC \ILE	Т5В	32.000		1.69	54.080	1.40	0.821	2.43	А	E	56.6	0.956	2.77	А	I	22.2	0.80	0.821	1.69	А	E	56.6	1		
	-TR/ TTST	ТбА	36.000		1.54	55.440	1.40	0.821	2.21	А	E	56.6	0.956	2.51	А	I	22.2	0.80	0.821	1.54	А	E	56.6	1		
		Т7А	40.000		1.42	56.800	1.40	0.821	2.03	А	E	56.6	0.956	2.31	А	I	22.2	0.80	0.821	1.42	А	E	56.6	1		
	– °'	Т7В	40.000		1.48	59.200	1.40	0.821	2.13	А	E	56.6	0.956	2.19	А	I	22.2	0.80	0.821	1.48	А	E	56.6	1		

END BENT 1

ASSEMBLED BY : T.J.KIRSCH	HBAUM	DATE :	11/16/15
CHECKED BY : A.J.FORFA		DATE :	12/2/15
DRAWN BY : MAA 1/08	REV.II/	12/08RR	MAA/GM
CHECKED BY : GM/DI 2/08	REV.IO	/1/11	MAA/GM



END BENT 2

LRFR SUMMARY



 PLANS PREPARED BY:
 2610 Wycliff Road suite 102 Raleigh NC 27607-3073 (919) 420-7660
 THESE PLANS HAVE BEEN PROPERLY EXAMINED BY TH UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPL WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\rm DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1. THE REDUCTION OF LOAD DISTRIBUTION FACTOR FOR MOMENT IN LONGITUDINAL BEAMS ON SKEWED SUPPORTS (AASHTO TABLE 4.6.2.2.2E-1) WAS NOT APPLIED.

(#) CONTROLLING LOAD RATING							
$\left(1\right)$ DESIGN LOAD RATING (HL-93)							
2 DESIGN LOAD RATING (HS-20)							
<pre>3 LEGAL LOAD RATING **</pre>							
** SEE CHART FOR VEHICLE TYPE							
GIRDER LOCATION							
I – INTERIOR GIRDER EL – EXTERIOR LEFT GIRDER ER – EXTERIOR RIGHT GIRDER							

-	PROJECT NO. <u>B-4447</u> <u>BURKE</u> <u>county</u> Station: <u>25+85.40</u> -L- 15+67.23 -Y-
Docusigned by:	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS (INTERSTATE TRAFFIC) WESTBOUND LANE
THE PLY DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:SO1-413131TOTAL SHEETS 2828
	STR. NO. 1 STD. NO. LRFR2



DocuSign Envelope ID: 85619921-CB87-4629-97DF-3DA95CF924EA

STR.NO.1



DRAWN BY :	DATE : <u>11/10/15</u>
CHECKED BY :A.J. FORFA	DATE : <u>12/15/15</u>
DESIGN ENGINEER OF RECORD :	DATE : <u>3/29/16</u>

DocuSign Envelope ID: 85619921-CB87-4629-97DF-3DA95CF924EA

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TYPICAL SECTION AT INTERMEDIATE DIAPHRAGM

FOR INTERMEDIATE STEEL DIAPHRAGMS, SEE SHEET S-12





SECTION AT INTEGRAL END BENT

DRAWN BY :	DATE :	11/10/15
CHECKED BY :A.J. FORFA	DATE :	12/15/15
DESIGN ENGINEER OF RECORD :	DATE : .	3/29/16

DocuSign Envelope ID: 85619921-CB87-4629-97DF-3DA95CF924EA

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3'-9"_



END BENT DIAPHRAGM



		PROJECT NO. <u>B-4447</u> <u>BURKE</u> COUNTY STATION: <u>25+85.40</u> -L- 15+67.23 -Y- SHEET 3 OF 3
	RTH CAROL ROFESSION SEAL 037180 F WERNING	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE TYPICAL SECTION DETAILS
Road	08901A86EBF6470 7/12/2016	WESTBOUND LANE REVISIONS SHEET NO.
7607-3073 DOCUM 60 F 5-0270 SIG	MENT NOT CONSIDERED INAL UNLESS ALL NATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: SOI 1 1 3



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DRAWN BY :	DATE :_	10/29/15
CHECKED BY : A.J. FORFA	DATE : _	12/15/15
DESIGN ENGINEER OF RECORD :	DATE :_	3/29/16

<u>FIX.</u> E5

SPAN A

FRAMING PLAN



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CE T	@ #2	

		PROJE(STATI	CT NO BURK ON: <u>25</u> 15	B E 5+85. +67.	-4447 co 40 -1 23 -Y	/ UNTY //
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	SEAL 037180 F WER		SUPER	istruc ING	plan	
	DocuSigned by: Dichen F. Vuitno		WESTE	BOUND	LANE	
1	7/12/2016		REVISI	ONS		SHEET NO.
-3073	DOCUMENT NOT CONSIDERED	NO. BY:	DATE: N	IO. BY:	DATE:	S01-9
0	FINAL UNLESS ALL SIGNATURES COMPLETED	1		33 A,		TOTAL SHEETS 28



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	0.2	217	58	,600		43.950				
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	S1	200	#4		1	6′-1	<i>"</i> 813			
	S2 53	24	#6 #⊿		1	6'-1 8'-5	<u> </u>			
	S4	88	#4		3	3'-0	<i>"</i> 176			
	S6 ₩ .S7	224 40	#5 #5	S	4 TR	4'-4 3'-8	<u> </u>			
	S8	2	#5		2	9'-0	<u> </u>			
		57	#5 #3	S S	TR	<u>3'-3</u> 1'-10	<u> </u>			
	S11	8	#5		5	10'-0)″ 83			
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STR.NO.1

STD. NO. PCG7



DocuSign Envelope ID: 85619921-CB87-4629-97DF-3DA95CF924EA

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DETAIL ``C''

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																					
										S	PAN	А									
0.6″Ø LOW RELAXATION									G	IRDE	IRS 1	. &	5								
TWENTIETH POINTS	0	.05	.10	. 15	.20	.25	.30	. 35	. 40	. 45	. 50	.55	.60	. 65	.70	.75	.80	.85	.90	. 95	0
CAMBER (GIRDER ALONE IN PLACE)	0	0.046	0.091	0.134	0.173	0.207	0.237	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.237	0.207	0.173	0.134	0.091	0.046	0
∗DEFLECTION DUE TO SUPERIMPOSED D.L. 🕴	0	0.028	0.056	0.084	0.109	0.132	0.151	0.166	0.177	0.184	0.186	0.184	0.177	0.166	0.151	0.132	0.109	0.084	0.056	0.028	0
FINAL CAMBER	0	3/16″	7/16″	5/ ₈ ″	³ ⁄4″	7⁄8″	1 / ₁₆ ″	11/8″	1 ³ / ₁₆ ″	1 /4″	1 /4″	1 ¹ /4″	1 ³ / ₁₆ ″	1 /8″	1 / ₁₆ ″	7⁄8″	³ ⁄4″	5⁄8″	7/16″	3/16″	0
		SPAN A																			
0.6″Ø LOW RELAXATION									GIF	RDERS	52,	38	k 4								
TWENTIETH POINTS	0	.05	.10	. 15	.20	.25	.30	.35	. 40	. 45	.50	. 55	.60	.65	.70	. 75	.80	.85	.90	. 95	0
CAMBER (GIRDER ALONE IN PLACE)	0	0.046	0.091	0.134	0.173	0.207	0.237	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.237	0.207	0.173	0.134	0.091	0.046	0
*DEFLECTION DUE TO SUPERIMPOSED D.L.	0	0.029	0.060	0.090	0.118	0.143	0.164	0.180	0.192	0.199	0.202	0.199	0.192	0.180	0.164	0.143	0.118	0.090	0.060	0.029	0
FINAL CAMBER	0	3/16″	3/8″	1/2"	¹¹ / ₁₆ ″	3⁄4″	7⁄8″	15/16″	1″	1 / ₁₆ ″	1 / ₁₆ ″	11/16″	1″	15/16″	7⁄8″	3/4″	¹¹ / ₁₆ ″	1/2″	3/8″	3/16″	0

* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER', WHICH IS GIVEN IN INCHES (FRACTION FORM).

ASSEMBLED BY : T.J. KIRSCH	HBAUM DATE :	10/9/15
CHECKED BY : A.J.FORFA	DATE :	12/15/15
DRAWN BY: ELR 11/91 CHECKED BY:GRP 11/91	REV. 10/1/11 REV. 1/15 REV. 2/15	MAA/GM MAA/TMG MAA/TMG





DETAIL ``D''



2610 Wycliff Road Suite 102

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED. I HAVE DETERMINED THAT THEY CO WITH EXISTING NORTH CAROLINA CODES, AND HAV BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6400 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

DEPTH OF 1/4".

A 2" × 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" MODIFIED BULB TEES.

0F 4500 lbs.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ''B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS.SEE SPECIAL PROVISIONS.

			PROJECT NO. B-4447 BURKE COUR STATION: 25+85.40 -L- 15+67.23 -Y- SHEET 2 OF 2 2	NTY -
		RTH CAROLINA OFESSION SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATI RALEIGH STANDARD	ON
		DocuSigned by:	PRESTRESSED CONCRETE GIN DETAILS	RDER
]	1	Clichaul F Vuitors 08901A86EBF6470 7/12/2016	WESTBOUND LANE	
THE MPLY E	DOCUMEN FIN	AL UNLESS ALL	REVISIONS SF NO. BY: DATE: NO. BY: DATE: SF 1 3	HEET NO. SO1-11 TOTAL SHEETS 28
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CHECKED BY : GM II/09

Excellence Delivered As Promised NC Lic. No. F-0270

BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL $\frac{1}{4}$ TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ " projection beyond the nut.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM ``A''	DIM ``B''	DIM ``C''	DIM ``L''
63" BULB TEE	1'-7¾″	1'-3" 1'-3"		3′-5″

			PROJE	CT NO BURK ON:_25 15	B E +85. +67.	-4447 CO 40 - 23 -Y	/ UNTY /
		DocuSigned by:	DEPA DIAPI B	STATE O RTMENT O ST INTERME HRAGMS ULB TEE CONCRE WESTB	DF NORTH CAR DF TRAN RALEIGH ANDAR DIATE FOR 6 PRES TE GI	NSPORTA SD STRESSE RDERS LANE	TION L IFIED ED
THE		7/12/2016			ONS		SHEET NO. S01-12
vii ∟ i 	DOCUMEN FIN SIGNA	IT NOT CONSIDERED AL UNLESS ALL TURES COMPLETED	1), BT: }	DATE:	TOTAL SHEETS 28
			STR.NO.1	S	TD. NO	.PCG11	







NOTES

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT. The elastomer in the steel reinforced bearings shall

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

	PROJE 	CT NOE BURKE :on:25+85 15+67,	3-4447 CO _40 - _23 -\	7 UNTY L 7
DocuSigned	DEP CAROL SEAL 37180 GINEER F WERTING F WERTIN	STATE OF NORTH CA ARTMENT OF TRA RALEIGH STANDAF STRESSED CON SUPERSTRL WESTBOUND	NSPORTA NSPORTA BEAR LS	TION TING
T 7/12/2016	CONSTDERED NO. BY:	REVISIONS DATE: NO. BY:	DATE:	SHEET NO. SO1-13
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NTEC	BAR TYPES
NOT BE CAST UNTIL ALL SLAB AND HAS REACHED A MINIMUM 3,000 PSI. IN BARRIER RAILS SHALL BE EPOXY BARS SHALL BE INSTALLED, USING SYSTEM, AFTER SAWING THE JOINT. *5 S3, S4, S5 AND S6 BARS IS FOR THE ADHESIVE BONDING INTS, 1/2" IN DEPTH, SHALL BE ACES OF THE BARRIER RAIL AND ICLE 825-10(B) OF THE STANDARD TRACTION JOINT SHALL BE LOCATE TWEEN BARRIER RAIL EXPANSION CTION JOINT IS REQUIRED AT IL SEGMENTS LESS THAN 20 FEET ACTION JOINTS ARE REQUIRED FOR AN 10 FEET IN LENGTH.	$D = \begin{bmatrix} \frac{1^{1} - 0^{1}/2^{n'}}{8^{1}/6^{n'}} & \frac{1^{1} - 0^{1}/2^{n'}}{8^{1}/6^{n'}} & \frac{1^{1} - 0^{1}/2^{n'}}{8^{1}/6^{n'}} & \frac{1^{1}}{8^{1}/6^{n'}} & \frac{1^{1}}{8^{1}/6^{n'}} & \frac{1^{1}}{8^{1}/6^{n'}} & \frac{1^{1}}{8^{1}/6^{n'}} & \frac{1^{1}}{8^{1}/4^{n'}} & \frac{1^{1}}{8^{1}/4^{$
	BILL OF MATERIAL
CTS.	FOR CONCRETE BARRIER RAIL ONLY
	BAR NO. SIZE TYPE LENGTH WEIGHT * B1 66 *5 STR 22'-7" 1555
	★ B2 44 #5 STR 22'-8" 1040
3 A R S	* S1 220 #5 1 4'-10" 1109
	** S2 220 #5 2 7'-0" 1606 ** S3 4 #5 3 4'-2" 17
	* S4 4 #5 STR 4'-0" 17 * S5 8 #5 3 3'-5" 29
	** 35 8 **5 5 5 23 ** S6 8 **5 STR 3'-3" 27
	*EPOXY COATED
	REINFORCING STEEL5,400 LBS.CLASS AA CONCRETE31.2 CU XDS
	CONCRETE BARRIER RAIL 230.6 LIN. FT.
<u>SECTION S-S</u> AT DAM IN OPEN JOINT (THIS IS TO BE USED ONL WHEN SLIP FORM IS USED	Y)))))))))))))
S S	SIGNATURES COMPLETED
	PROJECT NO. <u>B-4447</u>
$\frac{3}{4''}$	BURKE COUNTY
	STATION: 25+85.40 -L-
CHAMFER	15+67.23 -Y-
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
	raleigh $ST\Delta ND \Delta RD$
L S S	
	CUNCRETE RADDTED DATI
EXPANSION JOINIS	DANNIEN NAIL

BARRIER RAIL DETAILS

WESTBOUND LANEREVISIONSSHEET NO.NO.BY:DATE:NO.SHEET NO.13--SO1-1424--TOTAL24-28STR. NO. 1STD. NO. CBR1



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SUPERSTRU	CTURE REI	ENFORCIN	IG STEEL
LENGT	hs are b <i>i</i>	ASED ON	THE
FOLLOWING	MINIMUM	SPLICE	LENGTHS

BAR SIZE	SUPERSTF EXCEPT A SLABS, P AND BARR	RUCTURE Approach Arapet, Ier Rail	APPROAC	PARAPET AND BARRIER	
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL
#4	2'-0"	1'-9″	2'-0"	1'-9″	2'-9"
#5	2'-6"	2'-2"	2′-6″	2'-2"	3′-5″
#6	3′-0″	2'-7″	3′-10″	2'-7"	4'-4"
#7	5′-3″	3′-6″			
#8	6'-10"	4'-7"			

GROOVING	BRIDGE FL	OORS
APPROACH SLABS	1950	SQ.FT.
BRIDGE DECK	4497	_SQ.FT.
TOTAL	6447	_SQ.FT.



SUP	ERSTRUCT	URE BILL OF	MATERIA
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COA REINFORC STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
POUR #1	141.6		
POUR #2	82.2		
TOTALS **	223.8	13,994	12,031
** OLIANTT	TTES FOR BAR	RTER RATI ARE NOT 1	

** QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED



PLANS PREPARED BY: 2610 Wycliff Suite 102 Raleigh NC 2 (919) 420-760 Excellence Delivered As Promised	F Road 27607-3073 560 F-0270	THESE UNDER WITH BEEN	PLANS SIGNED. EXISTI PROPERI	HAVE I HA NG NC _Y AD	BEEN VE DE ORTH C APTED	PROF TERM AROL FOR	PERLY INED INA USE	(EXAN THAT CODES IN T	/INED The , and HIS /) BY Y CON HAV AREA.
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	BIL	L OF	- MA	TERIAL	_	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
米 A1	172	#5	STR	44'-11"	8058	
A2	172	#5	STR	44'-11"	8058	
米 A101	4	#5	STR	38′-2″	159	
₩ A102	4	#5	STR	31'-2"	130	
米 A103	4	#5	STR	24'-1"	101	
米 A104	4	#5	STR	17'-0"	71	
米 A105	4	#5	STR	10'-0"	42	
₩ A106	4	#5	STR	2'-11"	12	
A201	4	#5	STR	38′-2″	159	
A202	4	#5	STR	31′-2″	130	
A203	4	#5	STR	24'-1"	101	
A204	4	#5	STR	17'-0"	71	
A205	4	#5	STR	10'-0"	42	
A206	4	#5	STR	2'-11"	12	
米 B1	160	#4	STR	24'-8″	2636	
B2	68	#5	STR	58′-7″	4155	
K1	20	#4	STR	26'-10"	359	
K2	8	#4	STR	7'-1″	38	
K3	32	#4	STR	8'-8"	185	
K4	4	#4	STR	5′-3″	14	
K5	16	#4	STR	6'-1"	65	
米 S1	56	#4	1	11'-11"	446	
* S2	52	#4	1	10'-10"	376	
U1	76	#4	2	11'-11"	605	
REINFORCING STEEL = 13,994 * EPOXY COATED REINF.STEEL = 12,031						



			PROJE STATI	CT NO. <u>BUR</u> ł on:2 1	<u>B</u> <u>(E</u> 5+85. 5+67.	<u>-444</u> cc _40 - 23 -`	7)UNTY <u>L –</u> { –
	ſ	NUMBER CAROL	DEP.	STAT	e of north car OF TRAN RALEIGH	OLINA NSPORTA	TION
		SEAL 037180		SUPE	RSTRUC	CTURE	
		A CHARACTER AND A CHARACTER AND F WERMINING	B	ILL C	F MA	TERI	AL
		Docusigned by: Dichard F. Vuitros		WEST	BOUND	LANE	
THE		7/12/2016		REVIS	SIONS	Ι	SHEET NO.
MPLY F	DOCUMEN	T NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	
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BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

	#4 D1 TO PROJECT 1'-9"(MIN.) ABOVE CAP	
	CONST.JT.	
	5-#4 B6	
	5-#9 B2	
	1-#5 B3 (EA.FACE)	
	#4 B5 —	
	1-#5 B3 (EA.FACE)	
	1-#5 B3 (EA.FACE)	
	4-#10 B1	
	2'-0"ØX1'-6" Concrete collar	
	© HP 12 X 53 Steel Piles	
5		

DRAWN BY :	DATE :2/2/16
CHECKED BY :	DATE: <u>4/6/16</u>
DESIGN ENGINEER OF RECORD :	DATE : <u>4/6/16</u>



1′−0″ 1'-10<mark>1/</mark>2″ 1'-10<mark>/</mark>/2" 3′-9″ SECTION A-A

OF NO	DRTH CAROLINA
OF	TRANSPORTATION
RAI	LEIGH

IN	TEGRA	L
END	BENT	#1

NO. BY:

SHEET NO

S01-20

TOTAL SHEETS

28

DATE:

REVISIONS

DATE:

	INTEGRAL END BENT #1
ma	WESTBOUND LANE

Docusigned by: Archaul F. Vierton 08901A86EBF6470. 4/12/2017 OCUMENT NOT CONSIDERE FINAL UNLESS ALL SIGNATURES COMPLETED

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BY:

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BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

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NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

		CONST.JT.—
5-#4 B6		
5-#9 B2		
1-#5 B3 (EA.	FACE)	
		#4 B5
1-#5 B3 (EA.	FACE)	
		<u>2″CL.(TYP</u> .
1-#5 B3 (EA.	FACE)	
4-#10 B1		
		2'-0″ØX 1'-6″ Concrete collar
		€ HP 12 X 53 STEEL PILES

DRAWN BY :	
CHECKED BY : R.F. WERTMAN	DATE: <u>4/6/16</u>
DESIGN ENGINEER OF RECORD :	DATE :

1'-0" 1'-10[|]/2" 1'-10[|]/2" 3′-9″ SECTION A-A

STATE OF NORTH CAROLINA						
DEPARTMENT OF TRANSPORTATION						
RALEIGH						
SUBSTRUCTURE						

INTEGRAL

END BENT #2

WESTBOUND LANE

NO. BY:

SHEET NO.

S01-24

TOTAL SHEETS

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DATE:

REVISIONS

DATE:

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_	FIN	AL UNI	LESS ALL	
70	SIGNA	TURES	COMPLET	ED

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BY:

SHEET 4 OF 4

PLAN

DETAILS FOR SLOPE PROTECTION

-++-

- WELDED WIRE FABRIC

6 X 6 - W1.4 X W1.4

4″

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED.I HAVE DETERMINED THAT THEY CC WITH EXISTING NORTH CAROLINA CODES, AND HAV BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

PLANS PREPARED BY: Suite 102 **Gannett Fleming** Raleigh NC 27607-3073 (919) 420-7660 Excellence Delivered As Promised NC Lic. No. F-0270

SECTION A-A

2610 Wycliff Road

GENERAL NOTES

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING. SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS ``B''. THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE ''POURING DETAIL'' WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE ``OPTIONAL POURING DETAIL' WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.25+85.40 -L- STA.15+67.23 -Y-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
END BENT 1	375	830
END BENT 2	330	825

* QUANTITY SHOWN IS BASED ON 5' POURS.

		PROJECT NO. <u>B-4447</u> <u>BURKE</u> COUNTY STATION: <u>25+85.40</u> -L-
		19+61』とう -1- SHEET 1 OF 2
	Bocusigned by: Bocusigned by: Bocusi	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD SLOPE PROTECTION DETAILS WESTBOUND LANE
THE	7/12/2016	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: SO1-25
/E	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 28
		STR. NO. 1 STD. NO. SP1

		PROJECT NO. <u>B-4447</u> BURKE COUNTY
_		STATION: 25+85.40 -L- 15+67.23 -Y- Sheet 2 of 2
	RTH CAROL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
	SEAL P 037180 ROLEF VGINEER	SLOPE PROTECTION DETAILS
	DocuSigned by: Dichen F. Vutno 08901A86EBF6470	WESTBOUND LANE
THE 1PLY		REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: SO1-26
-	FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 28
		STR. NO. 1 STD. NO. SP2

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NOTES:

APPROACH SLAB SHALL NOT BE CONSTRUCTE COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL F. GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4' #78M STONE, WELDED WIRE FORM, AND SELEC ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROAD GRADED TO DRAIN THE WATER AWAY FROM THE BRIDGE AND SHALL BE PAVED. SEE ROA

THE JOINT OPENING AT THE APPROACH SLA SHALL BE SAWED NO MORE THAN 12 HOURS SLAB IS CAST. THE JOINT SHALL BE CLEAN BEFORE THE SEALANT IS APPLIED. THE JOI SHALL CONFORM TO THE REQUIREMENTS OF THE STANDARD SPECIFICATIONS.

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

	BILL OF MATERIAL					
D PRIOR TO	FOR ONE APPROACH SLAB (2 REQUIRED)					SLAB
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
ABRIC WALL INCLUDING	米 A1	52	#4	STR	22'-11"	796
Ø DRAINAGE PIPE,	A2	52	#4	STR	22′-9″	790
CT MATERIAL, SEE						
	米 B1	87	#5	STR	24'-2"	2193
CH SLAB SHALL BE	B2	87	#6	STR	24'-8"	3223
THE FILL FACE OF						
ADWAT PLANS.				_		
B/DECK INTERFACE	REINFORCING STEEL LBS. 4013				4013	
AFTER THE APPROACH NED OF ALL DEBRIS	* EPO REI	XY CO NFORO	DATED CING S	TEEL	LBS.	2989
SECTION 1028-3 OF						
	CLASS	S AA	CONCRE	TE	C. Y.	46.9

SPLI	CE LENG	THS CHART
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3′-10″	2'-7"

+

ASSEMBLED BY : T.J. KIRSC CHECKED BY : A.J. FORFA	HBAUM DATE : DATE :	11/10/15 12/15/15
DRAWN BY : FCJ II/88 CHECKED BY : ARB II/88	REV. 10/1/11 REV. 7/12 REV. 6/13	MAA/GM MAA/GM MAA/GM

ORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

2610 Wycliff Road Suite 102

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED. I HAVE DETERMINED THAT THEY CON WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL BACKFILL EXCAVATION HOLE AND GRADE TO DRAIN

			PROU STAT	ECT N BU ION: 2 of 2	0. RKE 25- 15+	<u>B</u> + 85.	-444 ⁻ cc 40 - 23 -`	7 UNTY L – 7 –
		SEAL 037180 F WERLING	DE	BRID	STATE OF STAI GE AB	NORTH CARG	DINA D PROA(IALS	tion CH
		DocuSigned by: Dichard For Vienton		WES	stbc	UND	LANE	
		7/12/2016		RE	VISION	IS	D • - -	SHEET NO.
/E •	DOCUMEN FIN SIGNA	NT NOT CONSIDER IAL UNLESS ALL TURES COMPLETEI	сер <mark>No. ву</mark> 1 D 2	DATE:	NO. 3 4	BY:	DATE:	TOTAL SHEETS 28
			STR.NC	. 1	57	D. NO) RASA	

SID. NO. DAS4

STR.NO.2

NOTES:

PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 115 TONS PER PILE. DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 195 TONS PER PILE. * PILES LOCATED NEAR THE EXISTING BATTERED PILES CAN BE SHIFTED UP TO 1'-6"TO AVOID INTERFERENCE. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING EXISTING BATTERED PILES BEFORE DRIVING NEW PILES.

DRAWN BY :	DATE :	9/21/15
CHECKED BY :	DATE : .	3/21/16
DESIGN ENGINEER OF RECORD :	DATE : .	3/21/16

STR.NO.2

BY:

ocuSigned by:

OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Docusigned by. Dicheen F. Vuitno 08901A86EBF6470... 7/12/2016

BRIDGE ON I-40 OVER SR 1758 BEREA CHURCH ROAD BETWEEN SR 1001 AND SR 1761 EASTBOUND LANE

NO. BY:

REVISIONS

DATE:

SHEET NO.

S02-2

TOTAL SHEETS

28

DATE:

				T	OTAL	BILL	\bigcirc	- MA	TERIAL	-					
	REMOVAL OF EXISTING STRUCTURE AT STA. 25+85.40 -L-	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MOD PRE C(G	IFIED 63" STRESSED ONCRETE IRDERS	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES		12 X 53 El PILES	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS	ASBESTOS ASSESSMENT
	LUMP SUM	SQ.FT.	SQ.FT.	CU.YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	NO.	LIN.FT.	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE		5218	6447		LUMP SUM		5	573.04				230.6		LUMP SUM	LUMP SUM
END BENT NO.1				49.0		6119			11	11	580		360		
END BENT NO.2				48.7		6044			11	11	445		330		
TOTAL	LUMP SUM	5218	6447	97.7	LUMP SUM	12163	5	573.04	22	22	1025	230.6	690	LUMP SUM	LUMP SUM

DRAWN BY :	/15
CHECKED BY : R.F. WERTMAN DATE : J/21	/16
DESIGN ENGINEER OF RECORD :	/16

NOTE	S:
SHTO	THE EXISTING STRUCTURE CONSISTING OF THREE SPANS @ 45'-O"FT. WITH A CLEAR ROADWAY OF 28' AND REINFORCED CONCRETE DECK ON STEEL GIRDERS ON END BENTS WITH REINFORCED CONCRETE CAPS ON STEEL PILES & INTERIOR BENTS WITH REINFORCED CONCRETE POSTS AND BEAMS LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED.
NS.	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
DSED .ACE	THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STA.26+05.17 -DETEB- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.
D	THE BRIDGE RAILS ON THE TEMPORARY STRUCTURE SHALL BE DESIGNED FOR THE AASHTO LRFD TEST LEVEL 3 (TL-3) CRASH TEST CRITERIA. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.
L TFD	FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
TS FRAI	FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.
NING	FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

			PROJE	CT NO Riirki	<u> </u>	-444	
			STATI	001111 0N: <u>25</u> 15	- +85. +67.	0	UNIY /
			<u>Sheet 3 c</u>)F 3			
		TH CARO	DEPA	STATE C	OF NORTH CAR DF TRAI RALEIGH	OLINA NSPORTA	TION
		SEAL 037180	G	ENERA	_ DF	RAWIN	IG
		DocuSigned by:	B SR 1 Betwi	RIDGE C 758 BER EEN SR EASTB	N I- EA CH 1001 , OUND	40 OVE HURCH H AND SR LANE	R Road 1761
Dood		08901A86EBF6470 4/12/2017		REVISI			SHEET NO
коаа 7607-3073	DOCUMEN	NT NOT CONSIDERE	NO. BY:	DATE: NO). BY:	DATE:	SO2-3
-0270	FIN SIGNA	IAL UNLESS ALL TURES COMPLETED	2	ୁ ଜୁ)		SHEETS

		LOAD AND) RES	SIST	ANCE	FACT	OR	RATI	ING (LRFR	R) SL	JMMA	ry f	OR F	PRES	TRES	SED	CON	CRET	E GI	RDEF	?S		
								STRENGTH I LIMIT STATE									SE	SERVICE III LIMIT STATE						
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#) LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD Factors (Y _{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93 (INVENTORY)	N⁄A	$\langle 1 \rangle$	1.09		1.75	0.821	1.25	А	E	56.6	0.956	1.27	А	I	33.7	0.80	0.821	1.09	А	E	56.6	1
DESIGN		HL-93 (OPERATING)	N⁄A		1.62		1.35	0.821	1.62	А	E	56.6	0.956	1.78	А	I	22.2	N⁄A						1
RATING		HS-20 (INVENTORY)	36.000	2	1.56	56.160	1.75	0.821	1.80	А	E	56.6	0.956	1.85	А	I	22.2	0.80	0.821	1.56	А	E	56.6	1
		HS-20 (OPERATING)	36.000		2.33	83.880	1.35	0.821	2.33	А	E	56.6	0.956	2.44	А	I	22.2	N⁄A						1
		SH	12.500		4.09	51.125	1.40	0.821	5.87	А	E	56.6	0.956	6.57	А	I	22.2	0.80	0.821	4.09	А	E	56.6	1
		S3C	21.500		2.39	51.385	1.40	0.821	3.43	А	E	56.6	0.956	3.77	А	I	22.2	0.80	0.821	2.39	А	E	56.6	1
	ICLE	S3A	22.750		2.26	51.415	1.40	0.821	3.25	А	E	56.6	0.956	3.57	А	I	22.2	0.80	0.821	2.26	А	E	56.6	1
	<pre>K</pre>	S4A	26.750		1.97	52.698	1.40	0.821	2.83	А	E	56.6	0.956	3.08	А	I	22.2	0.80	0.821	1.97	А	E	56.6	1
	C (S	S5A	30.500		1.74	53.070	1.40	0.821	2.50	А	E	56.6	0.956	2.78	А	I	22.2	0.80	0.821	1.74	А	E	56.6	1
	DNIS	S6A	34.500		1.57	54.165	1.40	0.821	2.25	А	E	56.6	0.956	2.48	А	I	22.2	0.80	0.821	1.57	А	E	56.6	1
		S7B	38.500		1.42	54.670	1.40	0.821	2.04	А	E	56.6	0.956	2.29	А	I	22.2	0.80	0.821	1.42	А	E	56.6	1
NATING		S7A	40.000	3	1.39	55.600	1.40	0.821	2.00	А	E	56.6	0.956	2.31	А	I	22.2	0.80	0.821	1.39	А	E	56.6	1
	ц К К	Т4А	28.250		1.93	54.523	1.40	0.821	2.76	А	E	56.6	0.956	2.97	А	I	22.2	0.80	0.821	1.93	А	E	56.6	1
	ACTC ALLEI (ILLEI	Т5В	32.000		1.69	54.080	1.40	0.821	2.43	А	E	56.6	0.956	2.77	А	I	22.2	0.80	0.821	1.69	А	E	56.6	1
	TR/- TR/- TTST	ТбА	36.000		1.54	55.440	1.40	0.821	2.21	А	E	56.6	0.956	2.51	А	I	22.2	0.80	0.821	1.54	A	E	56.6	1
		Т7А	40.000		1.42	56.800	1.40	0.821	2.03	A	E	56.6	0.956	2.31	А	I	22.2	0.80	0.821	1.42	A	E	56.6	1
		T7B	40.000		1.48	59.200	1.40	0.821	2.13	А	E	56.6	0.956	2.19	А	I	22.2	0.80	0.821	1.48	А	E	56.6	1

END BENT 1

ASSEMBLED BY : T.J. KIRSCHB	BAUM DATE :	/ 6/ 5
CHECKED BY : A.J. FORFA	DATE :	2/2/ 5
DRAWN BY : MAA 1/08	REV. II/I2/08RR	MAA/GM
CHECKED BY : GM/DI 2/08	REV. I0/I/II	MAA/GM

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END BENT 2

LRFR SUMMARY

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THUS UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLETER WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1. THE REDUCTION OF LOAD DISTRIBUTION FACTOR FOR MOMENT IN LONGITUDINAL BEAMS ON SKEWED SUPPORTS (AASHTO TABLE 4.6.2.2.2E-1) WAS NOT APPLIED.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

-	PROJECT NO. <u>B-4447</u> <u>BURKE</u> <u>COUNTY</u> STATION: <u>25+85.40</u> -L- 15+67.23 -Y-
DocuSigned by:	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS (INTERSTATE TRAFFIC) EASTBOUND LANE
HE PLY DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:SO2-413SImage: Sheet state s
	STR. NO. 2 STD. NO. LRFR2

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PRO FOR	V M

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DRAWN BY :	DATE : .	11/10/15
CHECKED BY :	DATE : .	3/21/16
DESIGN ENGINEER OF RECORD :	DATE : .	3/21/16

TYPICAL SECTION AT INTEGRAL END BENT

IOVABLE FORM.

DRAWN BY :	_ DATE :	11/10/15
CHECKED BY :	DATE :	3/21/16
DESIGN ENGINEER OF RECORD :	_ DATE :.	3/21/16

TYPICAL SECTION AT INTERMEDIATE DIAPHRAGM

FOR INTERMEDIATE STEEL DIAPHRAGMS, SEE SHEET S-12

DRAWN BY :T	J. KIRSCHBAUM	DATE : _	11/10/15
CHECKED BY :	R.F. WERTMAN	DATE :_	3/22/16
DESIGN ENGINEE	R OF RECORD :	DATE : _	3/22/16

3'-9"_

END BENT DIAPHRAGM

	PROJECT NO. <u>B-4447</u> <u>BURKE</u> COL STATION: <u>25+85.40</u> -L 15+67.23 -Y- SHEET 3 OF 3	JNTY
DocuSigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATE RALEIGH SUPERSTRUCTURE TYPICAL SECTION DETAILS	ion
Road <i>Uchent</i> Vu 08901A86EBF6470 7/12/2016	REVISIONS	SHEET NO.
7607-3073 60 FINAL UNLESS A SIGNATURES COMPL	DERED NO. BY: DATE: NO. BY: DATE: L 3 3 4 3 1	TOTAL SHEETS 28

DocuSign Envelope ID: 0478A425-7732-4086-911E-7DFB65CADB42

STR.NO.2

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<u>FIX.</u> E5

DRAWN BY :	DATE :_	10/29/15
CHECKED BY : R.F. WERTMAN	DATE :	3/23/16
DESIGN ENGINEER OF RECORD :	DATE :_	3/23/16

FIX. E5

<u>SPAN A</u>

FRAMING PLAN

	PROJECT NO. <u>B-4447</u> <u>BURKE</u> county Station: <u>25+85.40</u> -L- 15+67.23 -Y-
OFESSION SEAL	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE
PRO F WERMIN	FRAMING PLAN
DocuSigned by: Dichard F. Dutono	EASTBOUND LANE
ad 7/12/2016	REVISIONS SHEET NO.
07-3073 DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: SO2-9
FINAL UNLESS ALL SIGNATURES COMPLETED	U S SHEETS 2 4 28

	0.6″ 0	ØL.F	R. GRAE)E 2 ⁻	70 STF	RANDS						
			ULTI	ΜΑΤΕ		APPLIED						
			STRE	NGTH		ESTRESS						
	$\bigcirc 217$	HES)	LBS. PER		(LBS.	4 <u>3</u> 950						
	0.211		50,6	00		JU, JUU						
	REINFO	RCIN	IG STE	EEL	FOR C	DNE GDR						
	BAR NU S1 2	MBER	\$12E #4	1 YP	E LENC	SIH WELGHI 1″ 813						
	S2 (24	#6	1	6'-	1″ 219						
	S3 S4	12 88	#4 #4	2	3'-	5″ <u>67</u> 5″ 176						
	S6 2	24	#5 #5	4 сте	4' - 4' - 4' - 4' - 4' - 4' - 4' - 4' -	4″ 1012 8″ 153						
	× 37 · S8	2	#5 #5	2	9'-(155 0" 19						
	S9 !	57 2	#5 #3	STF STF	R 3'-: R 1'-1	3″ 193 0″ 1						
	S10 S11	8	#5	5	10'-	0″ 83						
	\$12 * NOTE:	16 S7 B/	#4 ARS SH	<u> Sif</u> All B	<u>r 8'-(</u> E BENT	D" 86 BEFORE						
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		· ـ · ـ	REINFORC	ING 8	000 PST	0.6″Ø						
			STEEL	C	ONCRETE	L.R. STRANDS						
			LB.		C.Y.	No.						
	EXTERIOR GI	RDER RDER	2822 2822		22.7 22.7	40						
		GIR	DERS	REQI	JIRED							
	NUMBER		LEN	GTH	TOT	AL LENGTH						
	5		114'-	7 ⁵ /16″	5	573′-0 ¹ /2″						
Dr				R-	444	7						
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	B	UR	KE		C(DUNTY						
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		1	5+6	7.2	3 -`	Ý –						
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1 NO. 1	BY: [DATE:	NO. BY	: 	DATE:	SUZ-IU TOTAL						
2			<u>a</u>			28						

STR.NO.2

STD. NO. PCG7

DETAIL ``C''

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																					
		SPAN A																			
0.6″Ø LOW RELAXATION									(.	SIRDE	ERS 1	. &	5								
TWENTIETH POINTS	0	.05	.10	. 15	.20	.25	.30	.35	.40	. 45	.50	. 55	.60	.65	.70	.75	.80	.85	.90	.95	0
CAMBER (GIRDER ALONE IN PLACE)	0	0.046	0.091	0.134	0.173	0.207	0.237	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.237	0.207	0.173	0.134	0.091	0.046	0
*DEFLECTION DUE TO SUPERIMPOSED D.L.	0	0.028	0.056	0.084	0.109	0.132	0.151	0.166	0.177	0.184	0.186	0.184	0.177	0.166	0.151	0.132	0.109	0.084	0.056	0.028	0
FINAL CAMBER	0	3/16″	7/16″	5/8 <i>"</i>	³ ⁄4″	7⁄8″	1 / ₁₆ ″	11/8″	1 ³ / ₁₆ ″	1 /4″	1 /4″	11/4″	1 ³ / ₁₆ ″	1 /8″	1 / ₁₆ ″	7⁄8″	³ / ₄ ″	⁵ /8″	7/16″	3/16″	0
										S	PAN	А									
0.6″Ø LOW RELAXATION									GI	RDER	S 2,	3 &	<u>4</u>								
TWENTIETH POINTS	0	.05	.10	. 15	.20	. 25	.30	. 35	. 40	. 45	.50	. 55	. 60	. 65	.70	. 75	.80	. 85	.90	. 95	0
CAMBER (GIRDER ALONE IN PLACE)	0	0.046	0.091	0.134	0.173	0.207	0.237	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.237	0.207	0.173	0.134	0.091	0.046	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	0	0.029	0.060	0.090	0.118	0.143	0.164	0.180	0.192	0.199	0.202	0.199	0.192	0.180	0.164	0.143	0.118	0.090	0.060	0.029	0
FINAL CAMBER	0	3/16″	3/8″	1/2″	¹¹ /16″	3⁄4″	7⁄8″	15/16″	1″	1 / ₁₆ ″	11/16″	11/16″	1″	15/16″	7⁄8″	3⁄4″	¹¹ /16″	1/2″	3/8″	3/16″	0

* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER', WHICH IS GIVEN IN INCHES (FRACTION FORM).

ASSEMBLED BY : T.J.KIRSCH CHECKED BY : R.F.WERTMA	HBAUM DATE : N DATE :	10/9/15 3/24/16
DRAWN BY: ELR 11/91 CHECKED BY:GRP 11/91	REV. 10/1/11 REV. 1/15 REV. 2/15	MAA/GM MAA/TMG MAA/TMG

DETAIL ``D''

2610 Wycliff Road

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED. I HAVE DETERMINED THAT THEY CO WITH EXISTING NORTH CAROLINA CODES, AND HAV BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6400 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER. THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

DEPTH OF 1/4".

A 2" \times 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" MODIFIED BULB TEES.

0F 4500 lbs.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ''B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE CONTRACTOR HAS THE OPTION TO PROVIDE,AT NO ADDITIONAL COST TO THE DEPARTMENT,2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS.SEE SPECIAL PROVISIONS.

			PR ST	OJEC ATI(T NO. <u>BURI</u> DN: _2 1	<u>KE</u> <u>5+</u> 5+	B 85. 67.	-444 co 40 - 23 -	7 DUNTY L – 7 –
		SEAL 037180 BOCKER DocuSigned by:	Ρ	depa REST	RTMENT	TAN DET	IORTH CARC TRAN ALEIGH NDAR ONCI	DINA NSPORTA D RETE G S	TION SIRDER
	,	Achen F. Vuitro			EAST	BO	UND	LANE	
THE		7/12/2016			REVI	SIONS	5		SHEET NO.
MPLY E	DOCUMEN FIN SIGNA	IT NOT CONSIDERED AL UNLESS ALL TURES COMPLETED	™. 1 2	BY:	DATE:	NO. 3 4	BY:	DATE:	SUZ-11 TOTAL SHEETS 28
			STF	R. NO. 2		ST	D. N	O.PCGS	3

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STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL $\frac{1}{4}$ TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ " projection beyond the nut.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM ``A''	DIM ``B''	DIM ``C''	DIM ``L''
63" BULB TEE	1'-73⁄4″	1'-3"	1'-3"	3'-5″

		PROJECT NO. <u>B-4447</u> <u>BURKE</u> COUNTY STATION: <u>25+85.40</u> -L- 15+67.23 -Y-
	Bocusigned by: 08901A86EBF6470	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS EASTBOUND LANE
THE MPLY E	7/12/2016 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:SO2-1213TOTAL SHEETSTOTAL SHEETS28
		STR. NO. 2 STD. NO. PCG11

NOTES

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT. The elastomer in the steel reinforced bearings shall

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

F	PROJECT NO. <u>B-4447</u> <u>BURKE</u> county station: <u>25+85.40</u> -L- 15+67.23 -Y-
Docusigned by: Docusigned by: Dosuble Decusion Description Descrip	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD ELASTOMERIC BEARING DETAILS — PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE EASTBOUND LANE
Y DOCUMENT NOT CONSTDERED	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:SO2-13
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 28
S	STR. NO. 2 STD. NO. EB4

	BAR TYPES
 JOTES I NOT BE CAST UNTIL ALL SLAB AND HAS REACHED A MINIMUM of 3,000 PSI. IN BARRIER RAILS SHALL BE EPOXY 6 BARS SHALL BE INSTALLED, USING SYSTEM, AFTER SAWING THE JOINT. 6 BARS SHALL BE INSTALLED, USING SYSTEM, AFTER SAWING THE JOINT. 6 BARS SHALL BE NSTALLED, USING SYSTEM, AFTER SAWING THE JOINT. 7 JOINTS, 1/2" IN DEPTH, SHALL BE PACES OF THE BARRIER RAIL AND RACES OF THE BARRIER RAIL AND STRACTION JOINT SHALL BE LOCATED BETWEEN BARRIER RAIL EXPANSION RACTION JOINT IS REQUIRED AT RAIL SEGMENTS LESS THAN 20 FEET RACTION JOINTS ARE REQUIRED FOR HAN 10 FEET IN LENGTH. 	BAR TYPES BAR TYPES $\frac{1'-0!/2''}{87/6''}$ $\frac{37/6''}{53/4''}$ $\frac{1}{5}/2''$ $\frac{1}{5}/2''$ $\frac{4!/4'' S3}{3!/4'' S5}$ $\frac{5!/2''}{5}$ $\frac{4!/4'' S3}{3!/4'' S5}$ $\frac{5!/2''}{5}$ $\frac{4!/4'' S3}{5}$ $\frac{5!/2''}{5}$ $\frac{4!/4'' S3}{5}$ $\frac{5!/2''}{5}$ $\frac{5!}{5}$
O"CTS.	ALL BAR DIMENSIONS ARE OUT TO OUT BILL OF MATERIAL FOR CONCRETE BARRIER RAIL ONLY BAR NO. SIZE TYPE LENGTH WEIGH * B1 66 **5 STR 22'-7" 1555 * B2 44 **5 STR 22'-8" 1040 * S1 220 **5 1 4'-10" 1109 * S2 220 **5 2 7'-0" 1606 * S3 4 **5 3 4'-2" 17
LEVEL) (LEVEL) (LEVEL) (UEV	* S4 4 #5 STR 4'-0" 17 * S5 8 #5 3 3'-5" 29 * S6 8 #5 STR 3'-3" 27 * EPOXY COATED REINFORCING STEEL 5,400 LBS CLASS AA CONCRETE 31.2 CU. YDS. CONCRETE BARRIER RAIL 230.6 LIN. FT.
WHEN SLIP FORM IS USED)	Pocusigned by: Content of the formation Document not considered Final unless all SIGNATURES COMPLETED ROJECT NO. <u>B-4447</u> BURKE COUNT
ST	FATION: <u>25+85.40 -L-</u> 15+67.23 -Y-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD CONCRETE BARRIER RAIL

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	115'-3 /	[∕] <u>l6</u> ″∕►
5'-3" 19'-7 ¹ /2"	-LEBL-	
25'-7/2"	© SAWED JOINT @ END BENT #1	€ SAWED JOINT @ END BENT #2
RI	LAYOUT FOR COM EINFORCED CONCR (SQ.FT.=	PUTING AREA Ete deck slab ——— 5218)
6'-0"	103'-3 / ₁₆ "	<u>6'-0"</u>
	© SAWED JOINT @ END BENT #1 -LEBL- ── -1	© SAWED JOINT © END BENT #2
	TRANSVERSE CONST. JT. SPAN	A
	POURING S	<u>Sequence</u>

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SUP	ERSTRU	CTURE	REIN	IFORC	ING	STE	EL
	LENGTH	HS ARE	. BAS	ED O	ΝΤ	HE	
FOLL	OWING	MININ	IUM S	SPLI(CE L	ENG	ΓHS

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH BAR SLABS, PARAPET, SIZE AND BARRIER RAIL		APPROAC	PARAPET AND BARRIER	
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL
#4	2'-0"	1'-9″	2'-0"	1'-9″	2'-9"
#5	2'-6"	2'-2"	2′-6″	2'-2"	3′-5″
#6	3′-0″	2'-7"	3′-10″	2'-7"	4'-4"
#7	5′-3″	3′-6″			
#8	6'-10"	4'-7"			

GROOVING	BRIDGE FL	OORS
APPROACH SLABS	1950	SQ.FT.
BRIDGE DECK	4497	_SQ.FT.
TOTAL	6447	_SQ.FT.

SUP	ERSTRUCT	URE BILL OF	MATERIAL
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
POUR #1	141.6		
POUR #2	82.2		
TOTALS **	223.8	13,994	12,031
** OLIANTT	TTES FOR BAR	RTER RATI ARE NOT T	

** QUANTITIES FOR BARRIER RAIL ARE NOT INCLUDED

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT

PLANS PREPARED BY: 2610 Wycliff Road Suite 102 Raleigh NC 27607-307. (919) 420-7660 Excellence Delivered As Promised	THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMP WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.
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ISIONS	ARE	OUT	ТО	OUT		

	BIL	L OF	F MA	TERIAL	_
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	172	#5	STR	44'-11"	8058
A2	172	#5	STR	44'-11"	8058
米 A101	4	#5	STR	38′-2″	159
₩ A102	4	#5	STR	31'-2"	130
米 A103	4	#5	STR	24'-1"	101
₩ A104	4	#5	STR	17'-0"	71
米 A105	4	#5	STR	10'-0"	42
₩ A106	4	#5	STR	2'-11"	12
A201	4	#5	STR	38′-2″	159
A202	4	#5	STR	31'-2"	130
A203	4	#5	STR	24'-1"	101
A204	4	#5	STR	17'-0"	71
A205	4	#5	STR	10'-0"	42
A206	4	#5	STR	2'-11"	12
₩ B1	160	#4	STR	24'-8"	2636
B2	68	#5	STR	58′-7″	4155
K1	20	#4	STR	26'-10"	359
K2	8	#4	STR	7'-1"	38
K3	32	#4	STR	8'-8"	185
K4	4	#4	STR	5'-3"	14
K5	16	#4	STR	6'-1"	65
米 S1	56	#4	1	11'-11"	446
* S2	52	#4	1	10'-10"	376
U1	76	#4	2	11'-11"	605
REINF * EPOX	ORCING (Y COAT	STEEL ED RE	INF.ST	= 1 FEEL = :	13,994 12,031

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		F WERMUN	BI	ILL O	F MA	TERI	AL
		DocuSigned by: Dichard F. Vuitan		EAST	BOUND	LANE	
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STATE OF NORTH CAROLINA							
DEPARTMENT OF TRANSPORTATION							
RALEIGH							

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HORIZONTAL-4″ SECTION B-B

4″

PLANS PREPARED BY:

SECTION A-A

Gannett Fleming Raleigh NC 27607-3073 (919) 420-7660

Excellence Delivered As Promised NC Lic. No. F-0270

- WELDED WIRE FABRIC

6 X 6 - W1.4 X W1.4

2610 Wycliff Road

Suite 102

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED.I HAVE DETERMINED THAT THEY CC WITH EXISTING NORTH CAROLINA CODES, AND HAV BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

GENERAL NOTES

STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH, SEE GENERAL DRAWING. SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS ``B''. THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE ``POURING DETAIL'' WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE ``OPTIONAL POURING DETAIL' WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.25+85.40 -L- STA.15+67.23 -Y-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
END BENT 1	360	900
END BENT 2	330	760

* QUANTITY SHOWN IS BASED ON 5' POURS.

		PROJECT NO. <u>B-4447</u> <u>BURKE</u> <u>county</u> Station: <u>25+85.40</u> -L- <u>15+67.23</u> -Y-
		SHEET 1 OF 2
	Bocusigned by: Cachand & Market	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD SLOPE PROTECTION DETAILS EASTBOUND LANE
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SLOPE PIVOT POINT <u>STA.26+65.32</u> -LEBL-EL.1210.26

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MAA/GM

NOTES:

APPROACH SLAB SHALL NOT BE CONSTRUCTED COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL FA GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" #78M STONE, WELDED WIRE FORM, AND SELEC ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH GRADED TO DRAIN THE WATER AWAY FROM T THE BRIDGE AND SHALL BE PAVED. SEE ROAD

THE JOINT OPENING AT THE APPROACH SLAB SHALL BE SAWED NO MORE THAN 12 HOURS A SLAB IS CAST. THE JOINT SHALL BE CLEANE BEFORE THE SEALANT IS APPLIED. THE JOIN SHALL CONFORM TO THE REQUIREMENTS OF S THE STANDARD SPECIFICATIONS.

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

	BILL OF MATERIAL						
) PRIOR TO	FOR ONE APPROACH SLAB (2 REQUIRED)						
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
BRIC WALL INCLUDING	* A1	52	#4	STR	22'-11"	796	
Ø DRAINAGE PIPE, T MATERIAL, SEE	A2	52	#4	STR	22'-9″	790	
	米 B1	87	#5	STR	24'-2"	2193	
H SLAB SHALL BE HE FILL FACE OF	B2	87	#6	STR	24'-8"	3223	
JWAY PLANS.				•			
3/DECK INTERFACE	REINFORCING STEEL * EPOXY COATED REINFORCING STEEL				LBS.	4013	
FTER THE APPROACH ED OF ALL DEBRIS NT SEALER MATERTAL					LBS.	2989	
SECTION 1028-3 OF						10.0	
	LLASS AA LUNLKEIE C.Y.					46.Y	

SPLICE LENGTHS CHART					
BAR SIZE	EPOXY COATED	UNCOATED			
#4	2'-0"	1'-9"			
#5	2'-6"	2'-2"			
#6	3'-10"	2′-7″			

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ASSEMBLED BY : T.J.KIRSCH CHECKED BY : R.F.WERTMA	HBAUM DATE : N DATE :	11/10/15 3/29/16
DRAWN BY : FCJ II/88 CHECKED BY : ARB II/88	REV. 10/1/11 REV. 7/12 REV. 6/13	MAA/GM MAA/GM MAA/GM

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

2610 Wycliff Road Suite 102

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY UNDERSIGNED. I HAVE DETERMINED THAT THEY CON WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL BACKFILL EXCAVATION HOLE AND GRADE TO DRAIN

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DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR

EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

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