

LOAD FACTORS

DESIGN LOAD RATING FACTORS	LIMIT STATE	$\gamma_{DC}$	$\gamma_{DW}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING #	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE								SERVICE III LIMIT STATE								COMMENT NUMBER		
						MOMENT					SHEAR			MOMENT										
						LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPANS	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPANS	GIRDER LOCATION		DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.08	--	1.75	0.966	1.21	A	E	43.9	1.109	1.28	A	I	26.1	0.80	0.966	1.08	A	E	43.9	--	
	HL-93 (OPERATING)	N/A		1.57	--	1.35	0.966	1.57	A	E	43.9	1.109	2.01	A	I	17.1	N/A	--	--	--	--	--	--	--
	HS-20 (INVENTORY)	36.000	2	1.47	52.92	1.75	0.966	1.64	A	E	43.9	1.109	1.97	A	I	17.1	0.80	0.966	1.47	A	E	43.9	--	
	HS-20 (OPERATING)	36.000		2.13	76.68	1.35	0.966	2.13	A	E	43.9	1.109	2.59	A	I	17.1	N/A	--	--	--	--	--	--	--
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.43	46.31	1.40	0.966	4.79	A	E	43.9	1.109	6.17	A	I	17.1	0.80	0.966	3.43	A	E	43.9	--
		SNGARBS2	20.000		2.51	50.20	1.40	0.966	3.50	A	E	43.9	1.109	4.33	A	I	17.1	0.80	0.966	2.51	A	E	43.9	--
		SNAGRIS2	22.000		2.36	51.92	1.40	0.966	3.29	A	E	43.9	1.109	4.01	A	I	17.1	0.80	0.966	2.36	A	E	43.9	--
		SNCOTTS3	27.250		1.70	46.33	1.40	0.966	2.38	A	E	43.9	1.109	3.01	A	I	17.1	0.80	0.966	1.70	A	E	43.9	--
		SNAGGRS4	34.925		1.41	49.24	1.40	0.966	1.96	A	E	43.9	1.109	2.47	A	I	17.1	0.80	0.966	1.41	A	E	43.9	--
		SNS5A	35.550		1.38	49.06	1.40	0.966	1.92	A	E	43.9	1.109	2.49	A	I	17.1	0.80	0.966	1.38	A	E	43.9	--
		SNS6A	39.950		1.26	50.34	1.40	0.966	1.75	A	E	43.9	1.109	2.26	A	I	17.1	0.80	0.966	1.26	A	E	43.9	--
		SNS7B	42.000		1.20	50.40	1.40	0.966	1.67	A	E	43.9	1.109	2.21	A	I	17.1	0.80	0.966	1.20	A	E	43.9	--
	TRUCK TRACTOR SEMI-TRAILER (TTS)	TNAGRIT3	33.000		1.53	50.49	1.40	0.966	2.13	A	E	43.9	1.109	2.71	A	I	17.1	0.80	0.966	1.53	A	E	43.9	--
		TNT4A	33.075		1.53	50.60	1.40	0.966	2.14	A	E	43.9	1.109	2.65	A	I	17.1	0.80	0.966	1.53	A	E	43.9	--
		TNT6A	41.600		1.25	52.00	1.40	0.966	1.74	A	E	43.9	1.109	2.35	A	I	17.1	0.80	0.966	1.25	A	E	43.9	--
		TNT7A	42.000		1.25	52.50	1.40	0.966	1.74	A	E	43.9	1.109	2.30	A	I	17.1	0.80	0.966	1.25	A	E	43.9	--
		TNT7B	42.000		1.28	53.76	1.40	0.966	1.79	A	E	43.9	1.109	2.16	A	I	17.1	0.80	0.966	1.28	A	E	43.9	--
		TNAGRIT4	43.000		1.23	52.89	1.40	0.966	1.71	A	E	43.9	1.109	2.09	A	I	17.1	0.80	0.966	1.23	A	E	43.9	--
TNAGT5A	45.000		1.16	52.20	1.40	0.966	1.62	A	E	43.9	1.109	2.06	A	I	17.1	0.80	0.966	1.16	A	E	43.9	--		
TNAGT5B	45.000		3	1.15	51.75	1.40	0.966	1.61	A	E	43.9	1.109	1.98	A	I	17.1	0.80	0.966	1.15	A	E	43.9	--	

NOTES

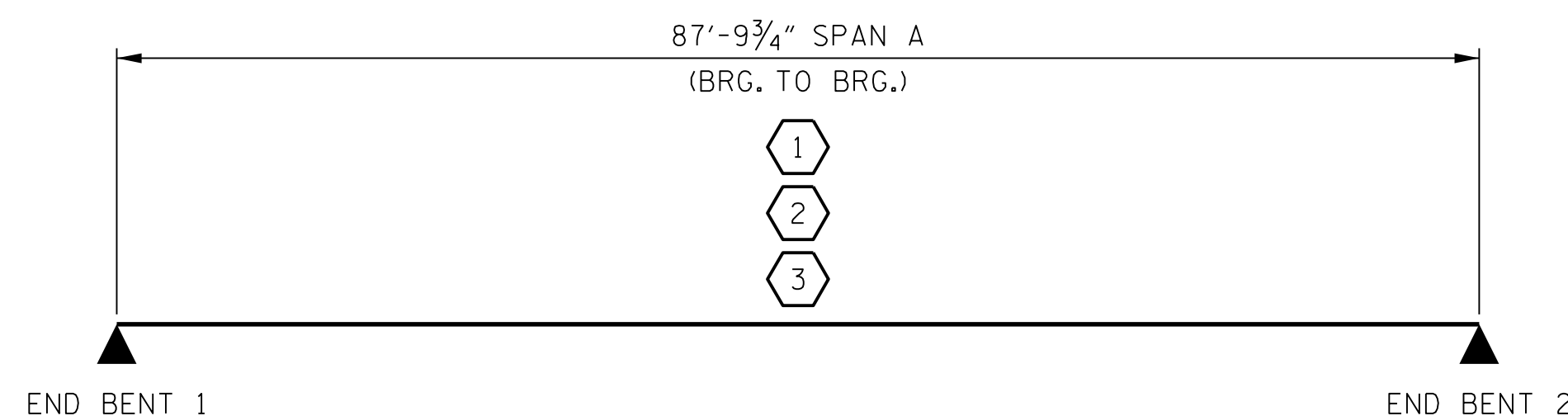
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS

1. RIGHT EXTERIOR GIRDER IS THE CONTROLLING EXTERIOR GIRDER.

#	CONTROLLING LOAD RATING
1	DESIGN LOAD RATING (HL-93)
2	DESIGN LOAD RATING (HS-20)
3	LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE	
GIRDER LOCATION	
I - INTERIOR GIRDER	
E - EXTERIOR GIRDER (RIGHT)	

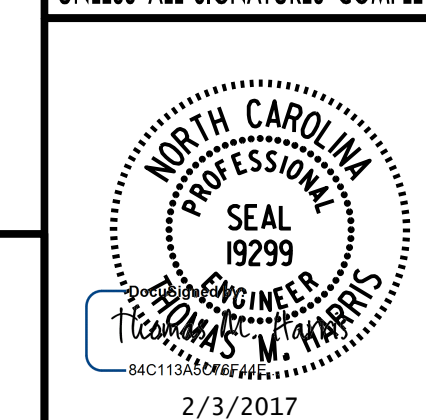


LRFR SUMMARY

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 LRFR SUMMARY FOR  
 PRESTRESSED CONCRETE GIRDERS  
 (NON-INTERSTATE TRAFFIC)  
 (LEFT LANE)

DOCUMENT NOT CONSIDERED FINAL  
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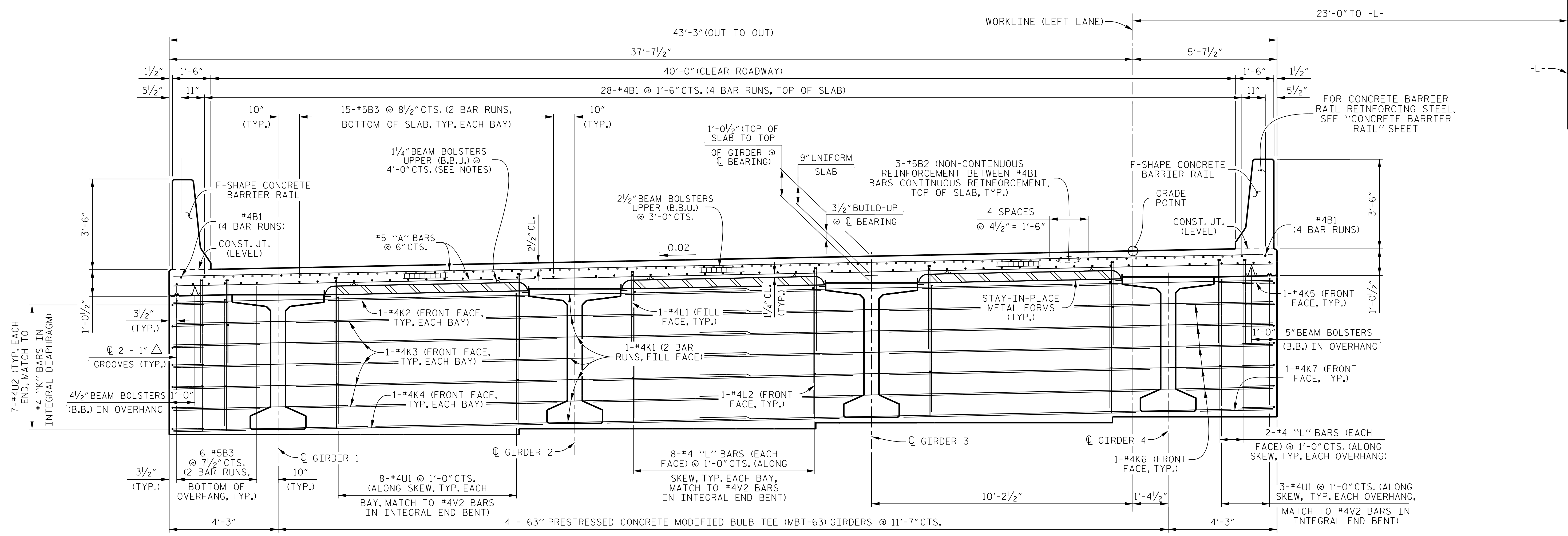
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No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
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2			4			

ASSEMBLED BY : K. E. LOFTON      DATE : 6-16  
 CHECKED BY : A. D. SHAH      DATE : 10-16

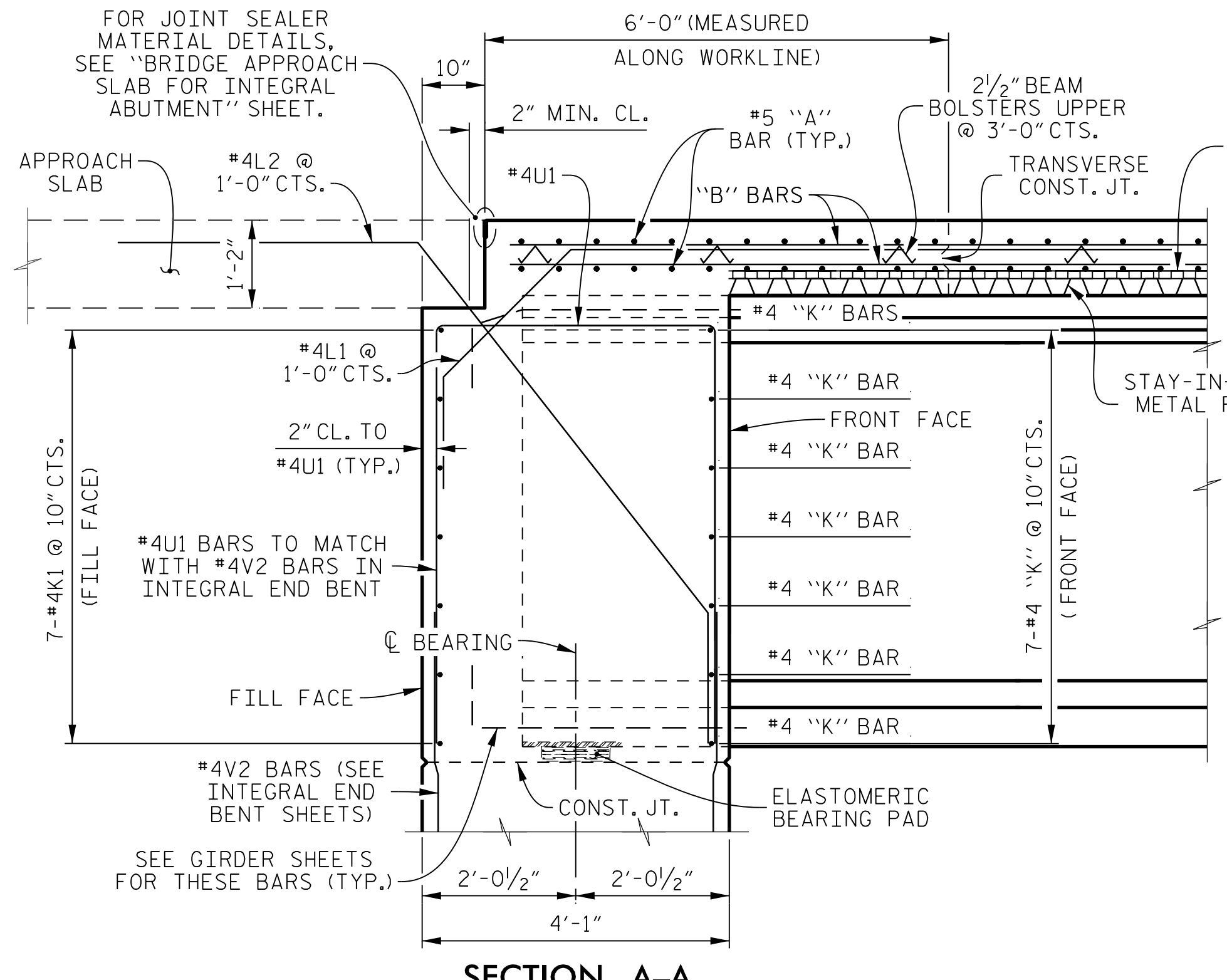
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 CHECKED BY : GM/DI 2/08      REV. 10/1/11 MAA/GM

DRAWN BY : K. E. LOFTON      DATE : 6-16  
 CHECKED BY : A. D. SHAH      DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS      DATE : 10-16

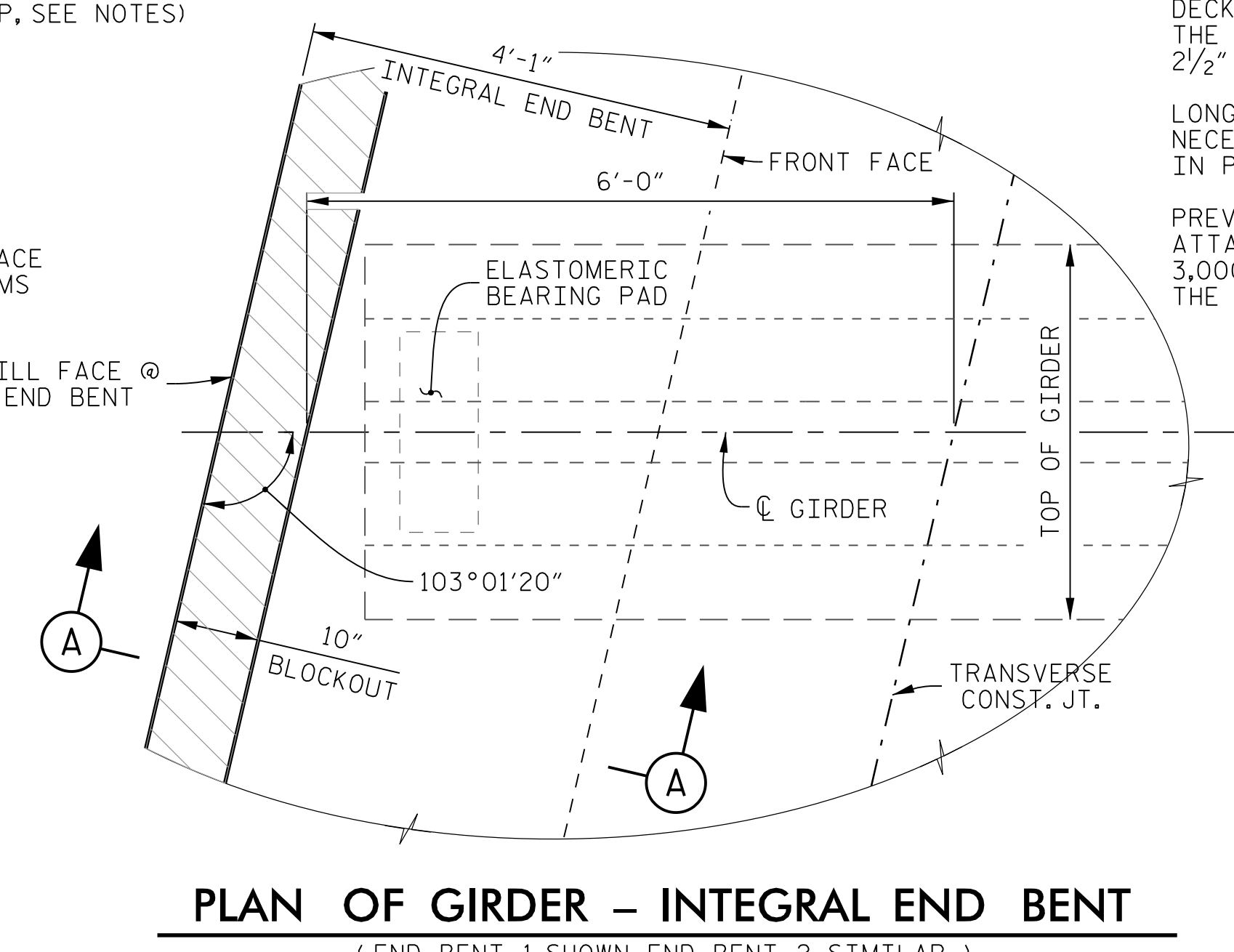
PLANS PREPARED BY :  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
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 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION



TYPICAL SECTION AT INTEGRAL END BENT 1 AND END BENT 2



SECTION THRU INTEGRAL END BENT (END BENT 1 SHOWN, END BENT 2 SIMILAR)



PLAN OF GIRDER - INTEGRAL END BENT (END BENT 1 SHOWN, END BENT 2 SIMILAR)

NOTES

- PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
- LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
- PREVIOUSLY CAST CONCRETE IN SPAN "A" SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.
- BARRIER RAIL IN SPAN "A" SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
- FOR F-SHAPE CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.
- FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.
- FOR END VIEW OF INTEGRAL DIAPHRAGM, SEE SHEET 2 OF 2.

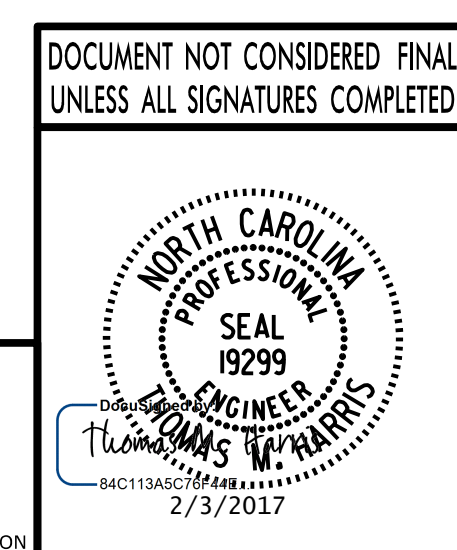
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 CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUPERSTRUCTURE  
 TYPICAL SECTION  
 (LEFT LANE)

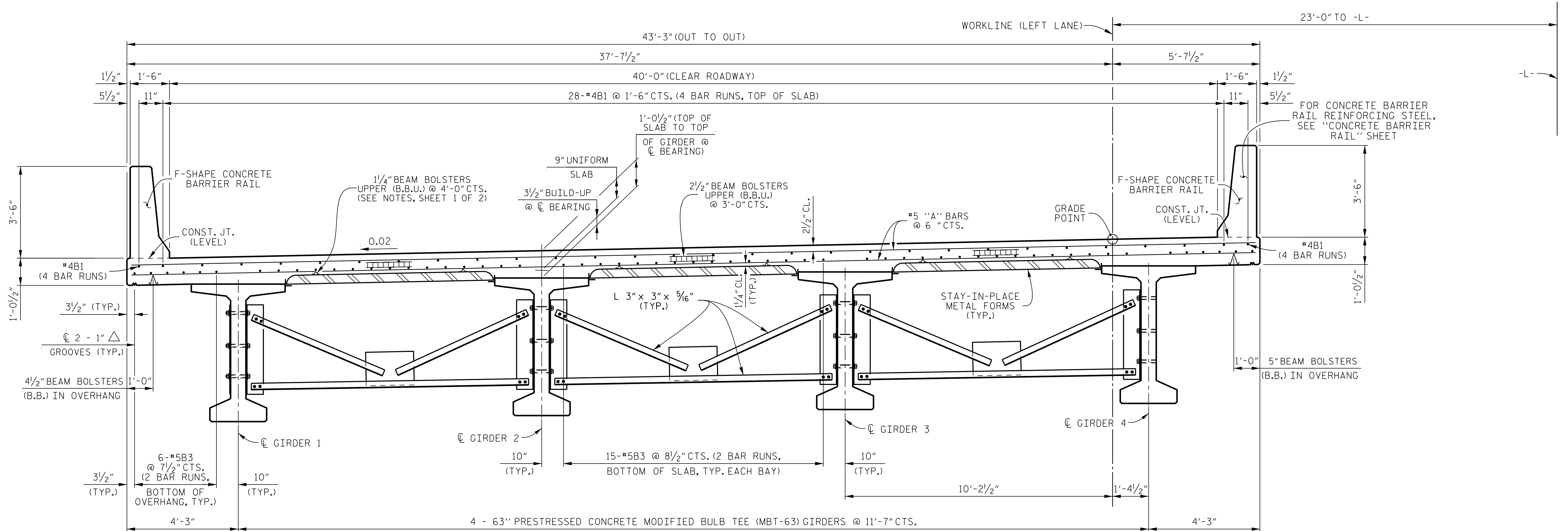
REVISIONS						SHEET No. S9-5
No.	BY:	DATE:	No.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			STR. #9



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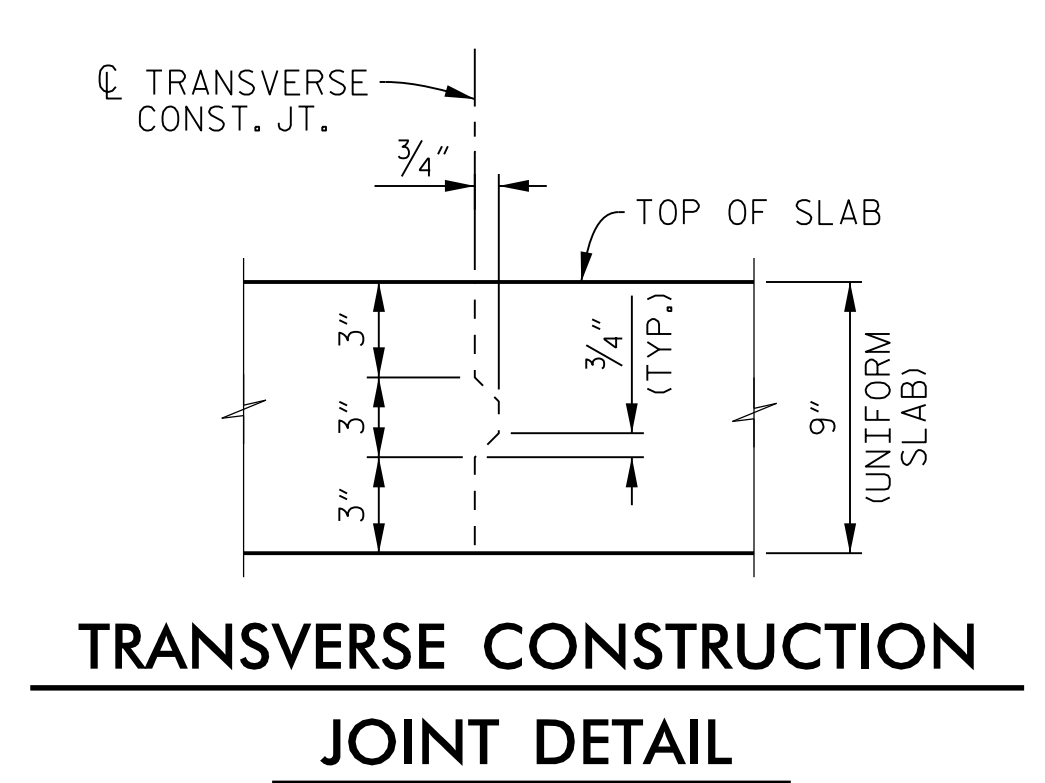
DRAWN BY:	K. E. LOFTON	DATE:	6-16
CHECKED BY:	T. M. HARRIS	DATE:	10-16
DESIGN ENGINEER:	T. M. HARRIS	DATE:	10-16



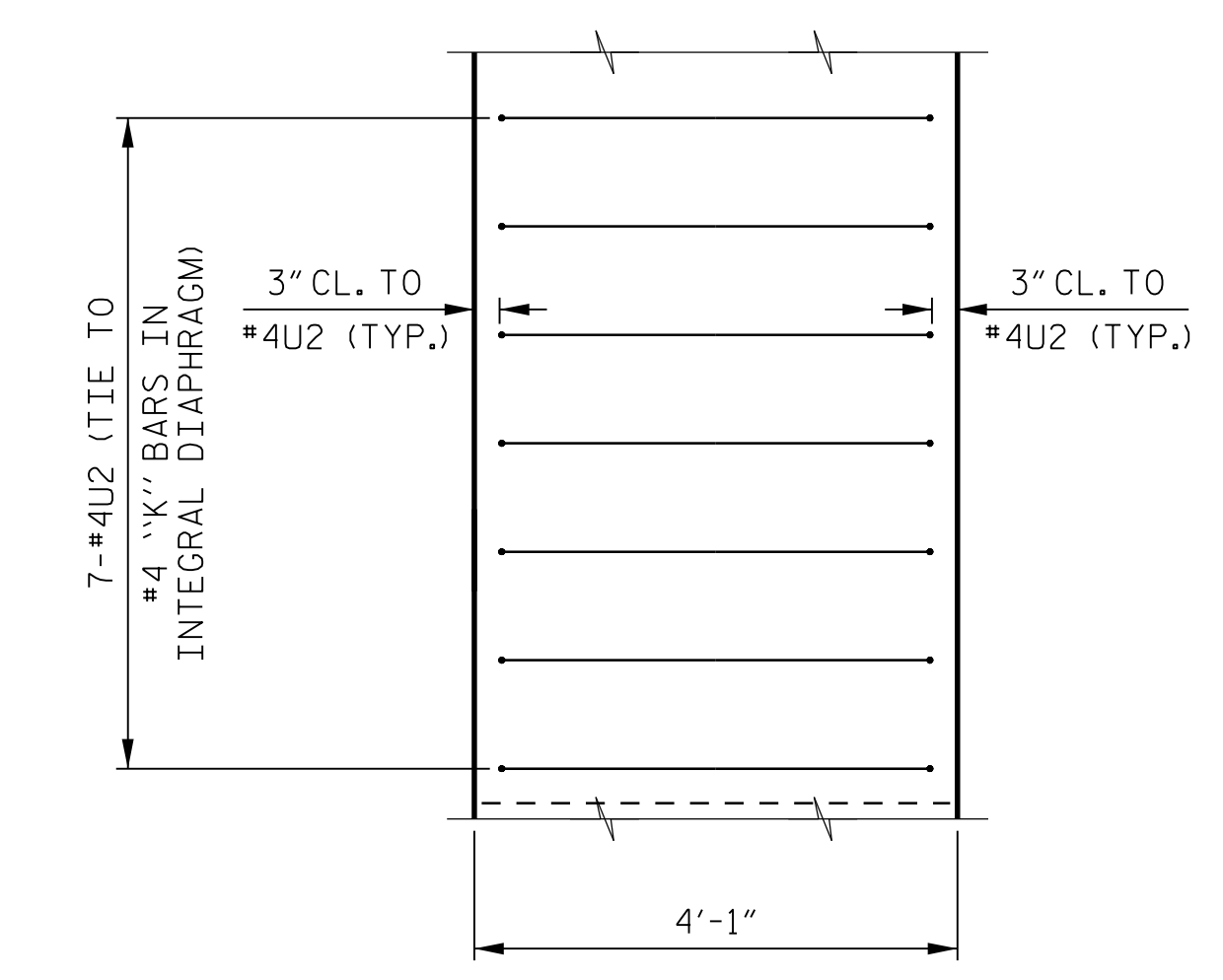


**TYPICAL SECTION AT INTERMEDIATE STEEL DIAPHRAGMS**

FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.



REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT.



**END VIEW OF INTEGRAL DIAPHRAGM**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUPERSTRUCTURE**

**TYPICAL SECTION**

**(LEFT LANE)**

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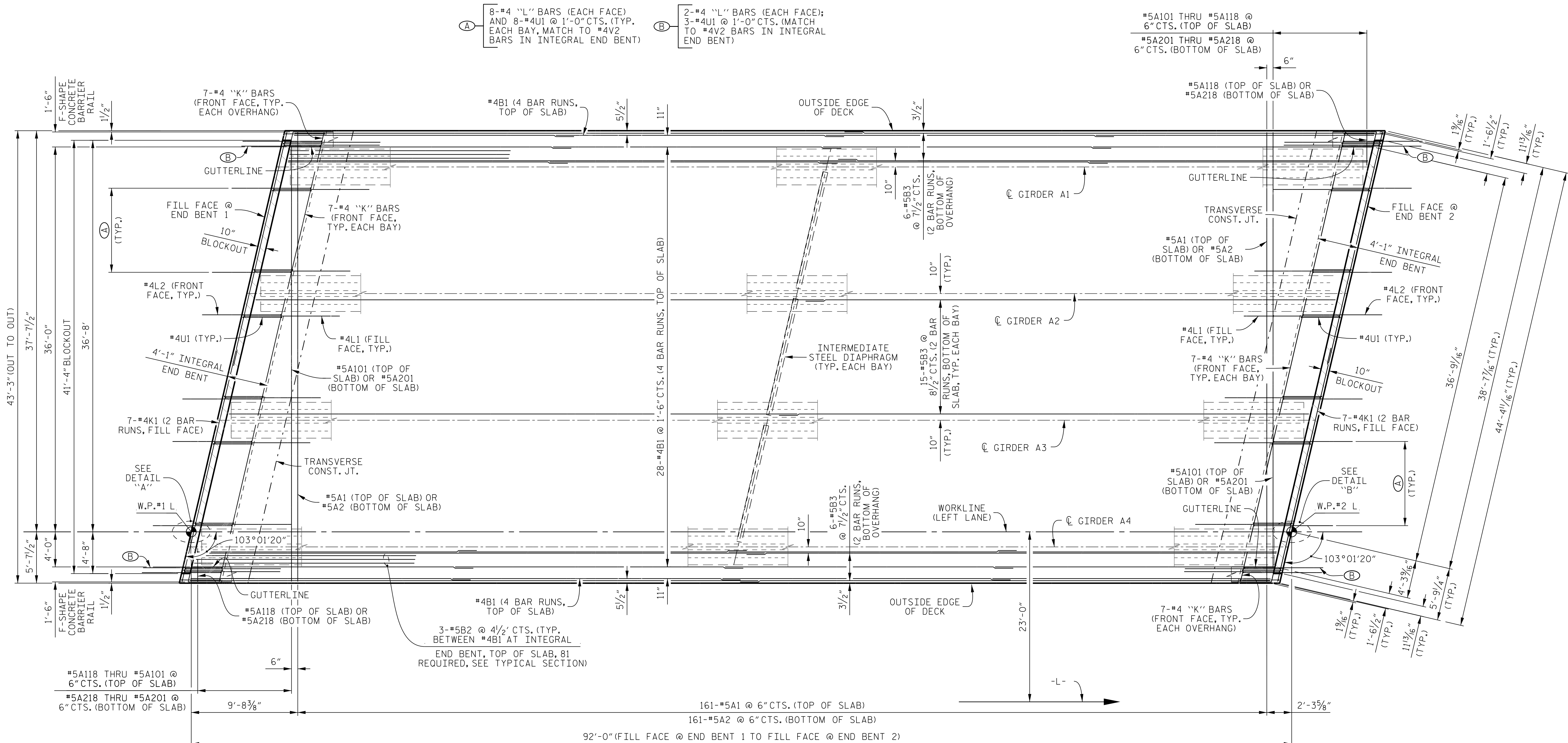
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DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

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No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

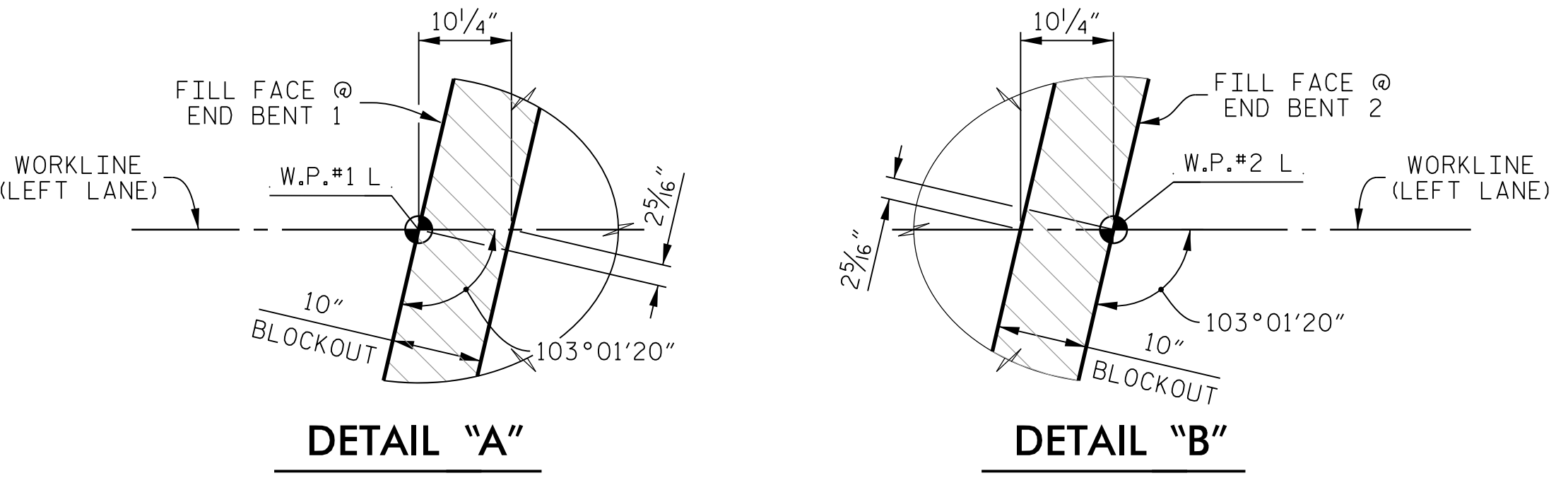
TOTAL SHEETS: 25

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**PLAN OF SPAN**

PROJECT NO. R-2707C  
 CLEVELAND COUNTY  
 STATION: 611+32.01 -L-



**NOTES**

#5 "A" BARS SHALL BE PLACED PERPENDICULAR TO WORKLINE WITH A 2" MINIMUM CLEARANCE ON EACH END.

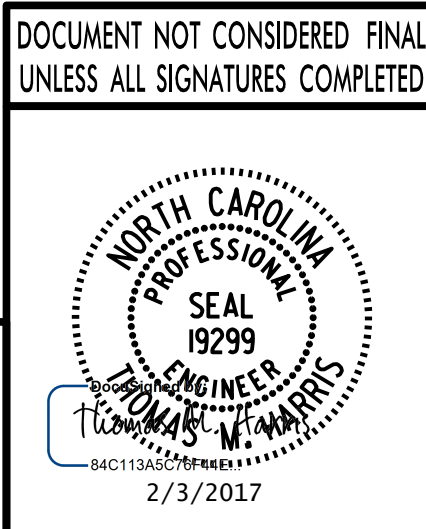
FOR REINFORCING STEEL IN CONCRETE BARRIER RAIL, SEE "CONCRETE BARRIER RAIL" SHEET.

FOR SLAB POURING SEQUENCE AND SPLICE CHART, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.

FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIDERS" SHEET.

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 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

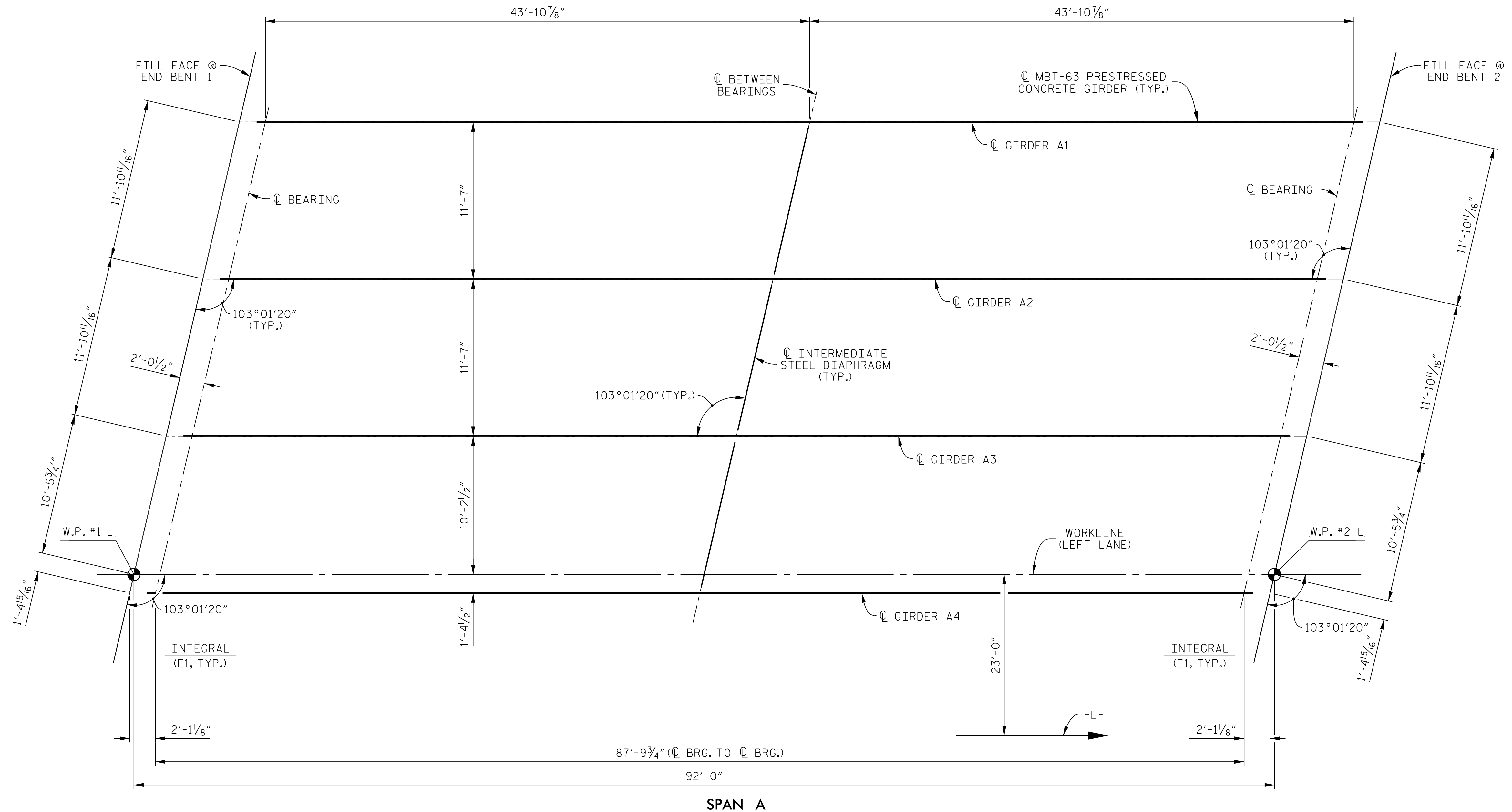
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 Raleigh, NC 27606-3386  
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE PLAN OF SPAN (LEFT LANE)					
REVISIONS					
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		
SHEET No.					S9-7
TOTAL SHEETS					25

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**SPAN A**  
**FRAMING PLAN**

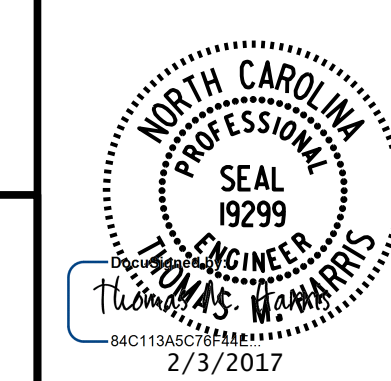
**NOTES**

FOR INTERMEDIATE STEEL DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.

FOR ELASTOMERIC BEARING DETAILS, SEE "ELASTOMERIC BEARING DETAILS PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE" SHEET.

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 611 + 32.01 -L-

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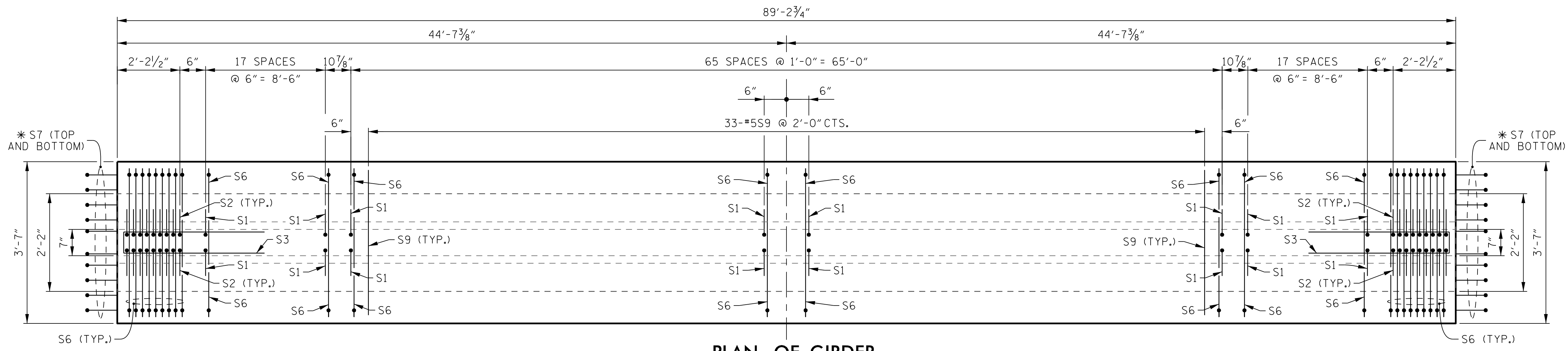


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FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

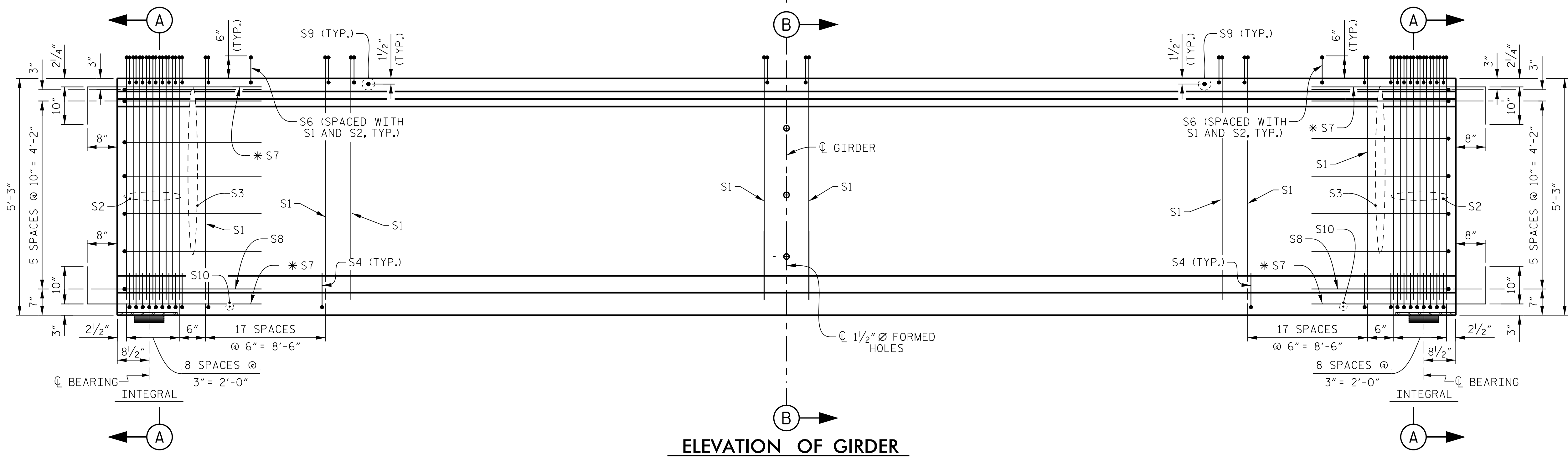
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CHECKED BY : A. D. SHAH DATE : 10-16  
DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
<b>SUPERSTRUCTURE FRAMING PLAN (LEFT LANE)</b>					
REVISIONS					
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		
					SHEET No. <b>S9-8</b>
					TOTAL SHEETS <b>25</b>

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PLAN OF GIRDER



ELEVATION OF GIRDER

**NOTES**  
 FOR SECTION A-A AND SECTION B-B, SEE SHEET 2 OF 3.  
 FOR STRAND PATTERN, SEE SHEET 2 OF 3.  
 FOR ADDITIONAL NOTES, SEE SHEET 3 OF 3.

**0.6" Ø L.R. GRADE 270 STRANDS**

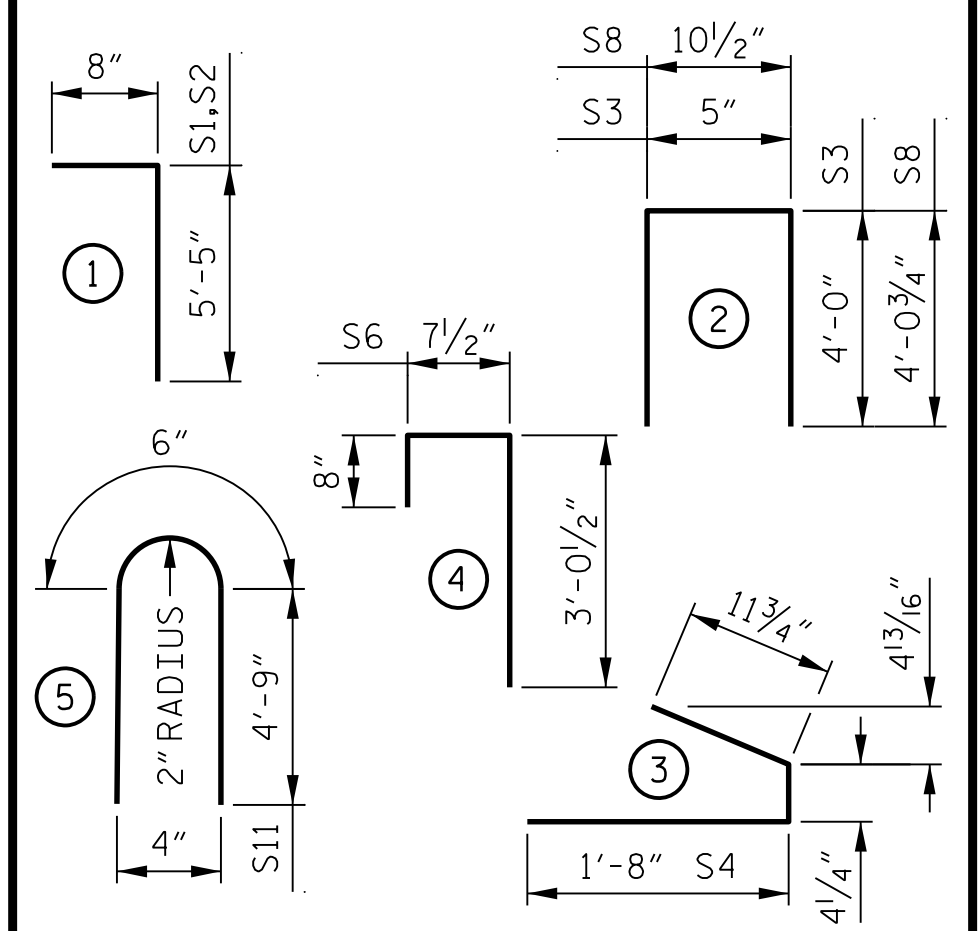
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

**REINFORCING STEEL FOR ONE GIRDER**

BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	204	#4	1	6'-1"	829
S2	36	#5	1	6'-1"	228
S3	12	#4	2	8'-5"	67
S4	108	#4	3	3'-0"	216
S6	240	#5	4	4'-4"	1,085
*S7	40	#5	STR	3'-8"	153
S8	2	#5	2	9'-0"	19
S9	3	#5	STR	3'-3"	112
S10	2	#3	STR	1'-10"	1
S11	4	#5	5	10'-0"	42
S12	8	#4	STR	8'-0"	43

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

**BAR TYPES**  
 ALL BAR DIMENSIONS ARE OUT-TO-OUT.



**QUANTITIES FOR ONE GIRDER**

	REINFORCING STEEL (LB.)	7,000 PSI CONCRETE (C.Y.)	0.6" Ø L.R. STRANDS (No.)
GIRDERS			
A1 THRU A4	2,795	17.7	30

**GIRDERS REQUIRED**

LOCATION	NUMBER	LENGTH	TOTAL LENGTH
SPAN A	4	89'-2 3/4"	356.92'

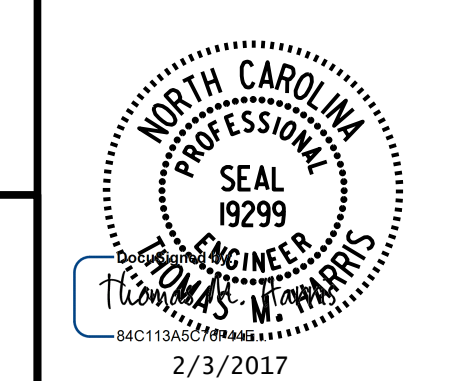
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
**63" PRESTRESSED CONCRETE MODIFIED BULB TEE GIRDER**  
 (LEFT LANE)

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

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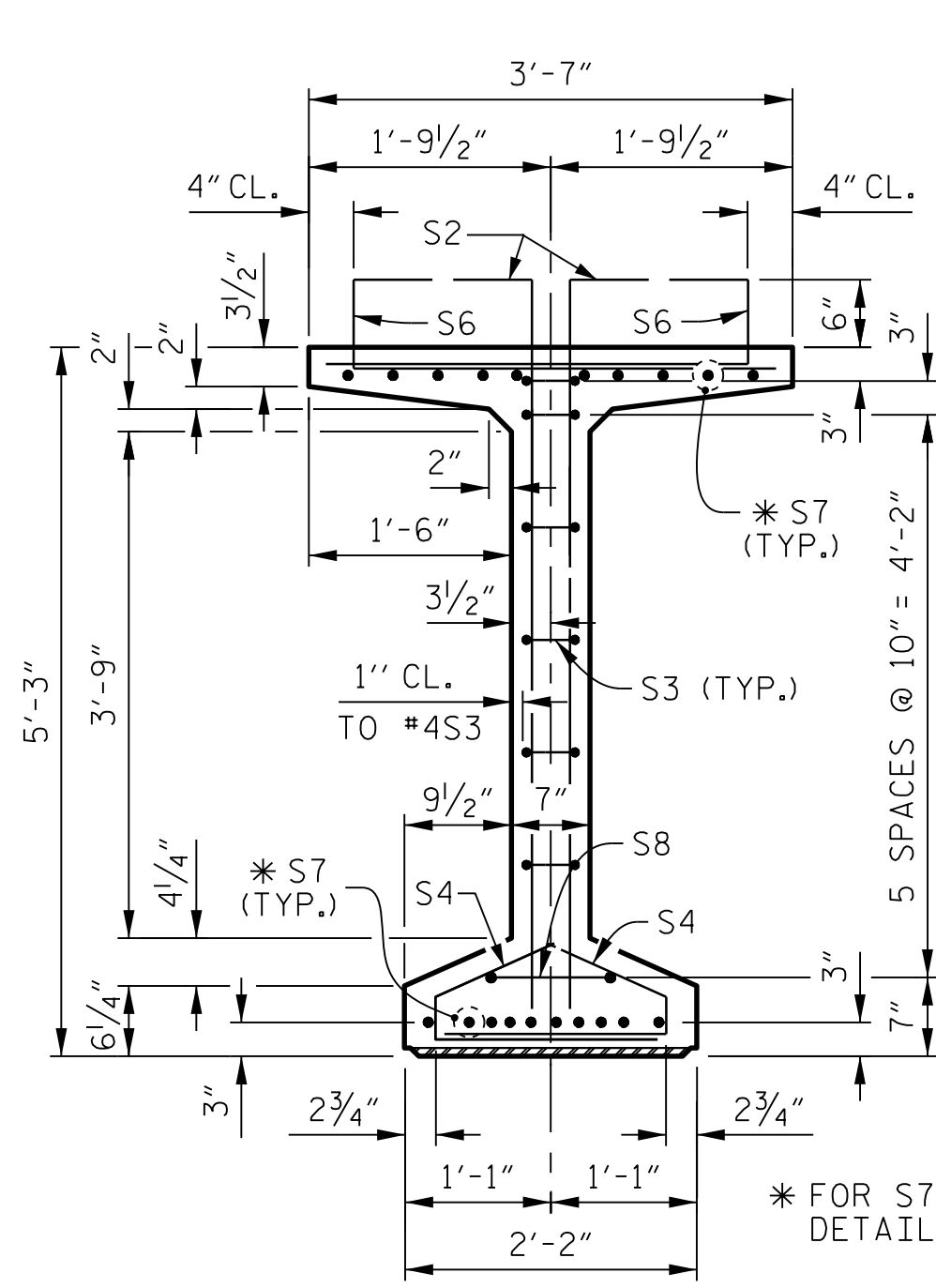
PLANS PREPARED BY:  
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 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY:	K. E. LOFTON	DATE:	6-16
CHECKED BY:	T. M. HARRIS	DATE:	10-16
DESIGN ENGINEER:	T. M. HARRIS	DATE:	10-16

ASSEMBLED BY:	K. E. LOFTON	DATE:	6-16
CHECKED BY:	T. M. HARRIS	DATE:	10-16
DRAWN BY:	EEM 2/6/97	REV. 10/1/11	MAA/GM
CHECKED BY:	VAP 2/6/97	REV. 6/13	MAA/GM
		REV. 1/15	MAA/TMG

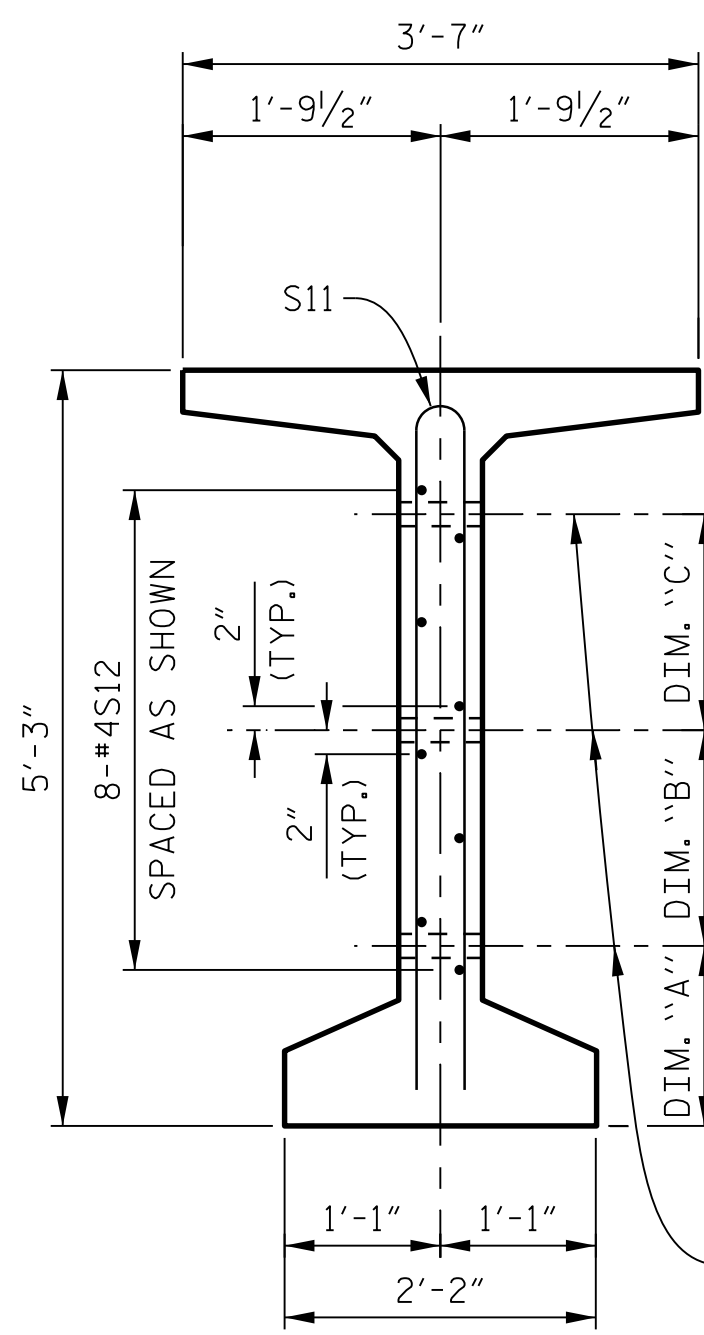
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**SECTION A-A**

\* FOR S7 BAR SPACING, SEE DETAIL "C", SHEET 3 OF 3.

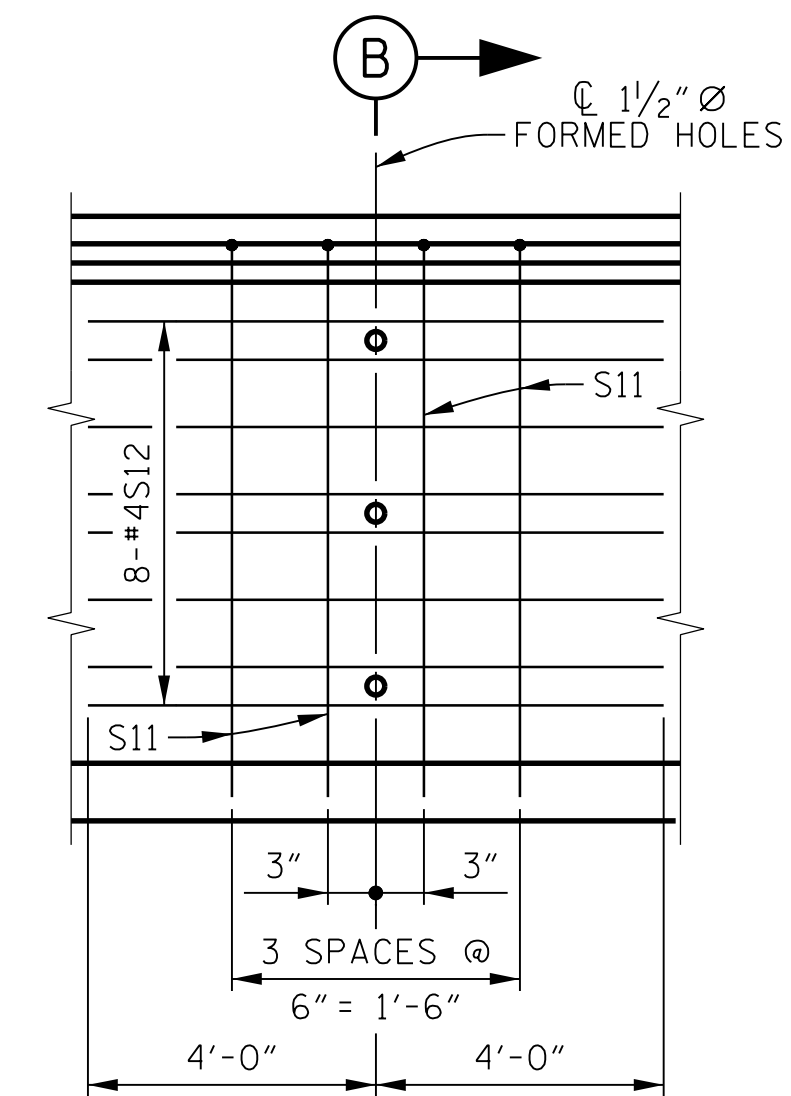


**SECTION B-B**

(S1, S6 AND S9 BARS NOT SHOWN)

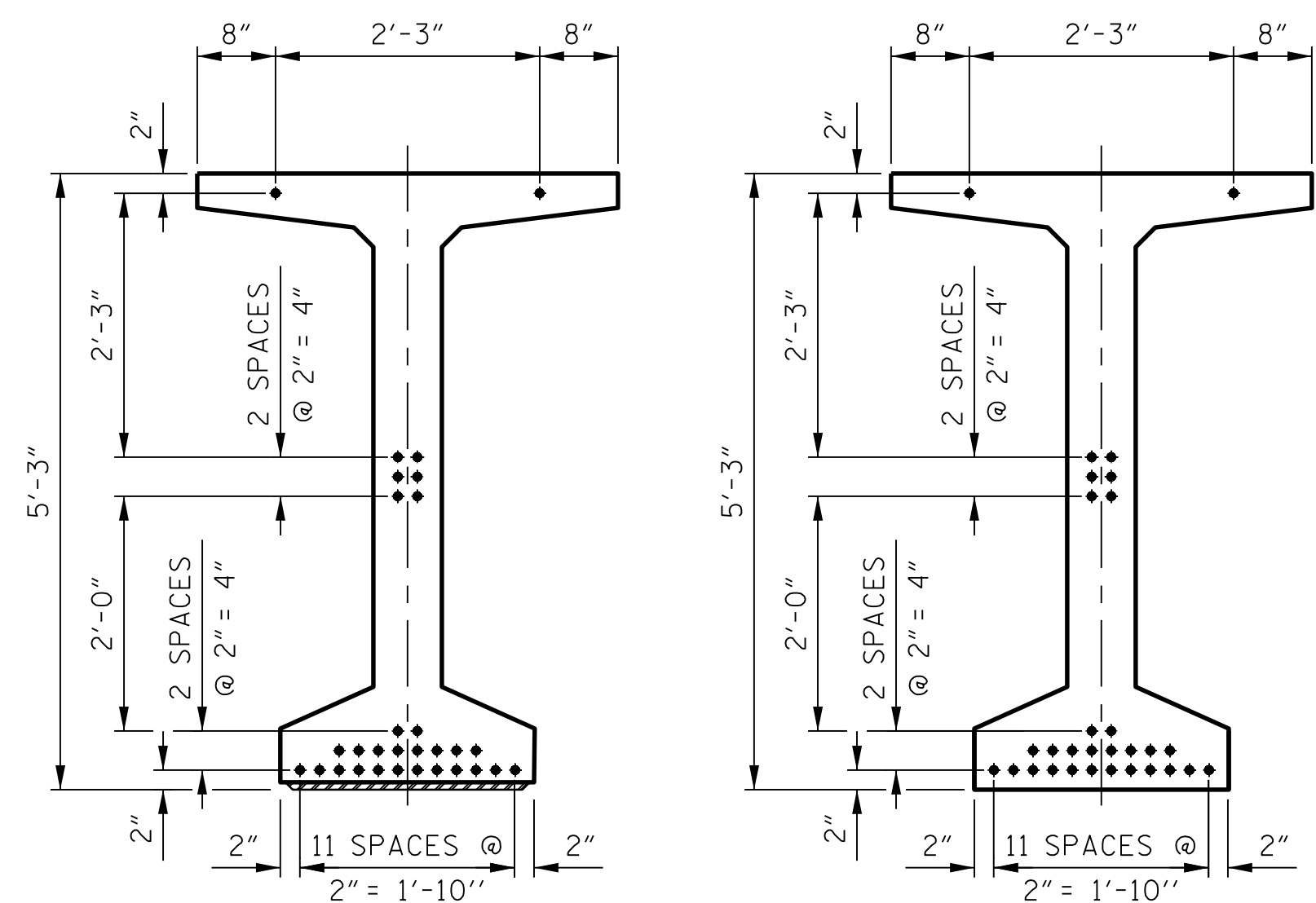
TABLE			
GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"
63" BULB TEE	1'-9"	1'-5"	11 3/4"

Ø 1 1/2" Ø FORMED HOLE, FOR LOCATION SEE ELEVATION, SHEET 1 OF 3.



**PARTIAL ELEVATION**

SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR ALL GIRDERS



**30 - 0.6" Ø LOW RELAXATION STRAND LAYOUT**

• FULLY BONDED STRANDS

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD

**63" PRESTRESSED CONCRETE MODIFIED BULB TEE GIRDER**

(LEFT LANE)

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS  
25

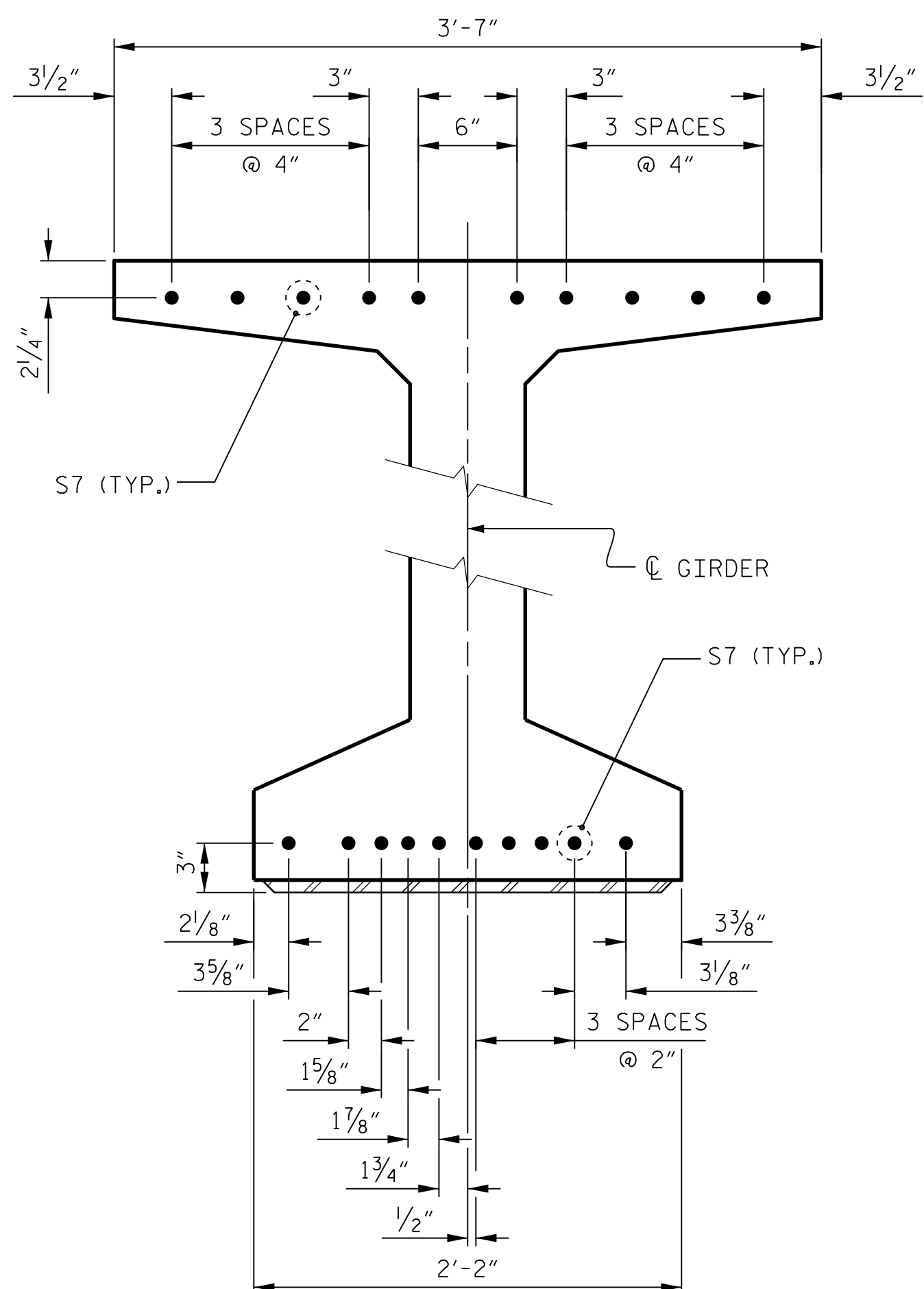
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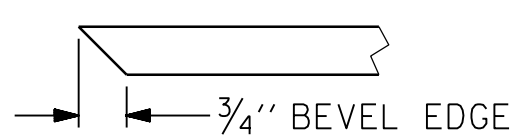
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY : K. E. LOFTON DATE : 6-16  
 CHECKED BY : T. M. HARRIS DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

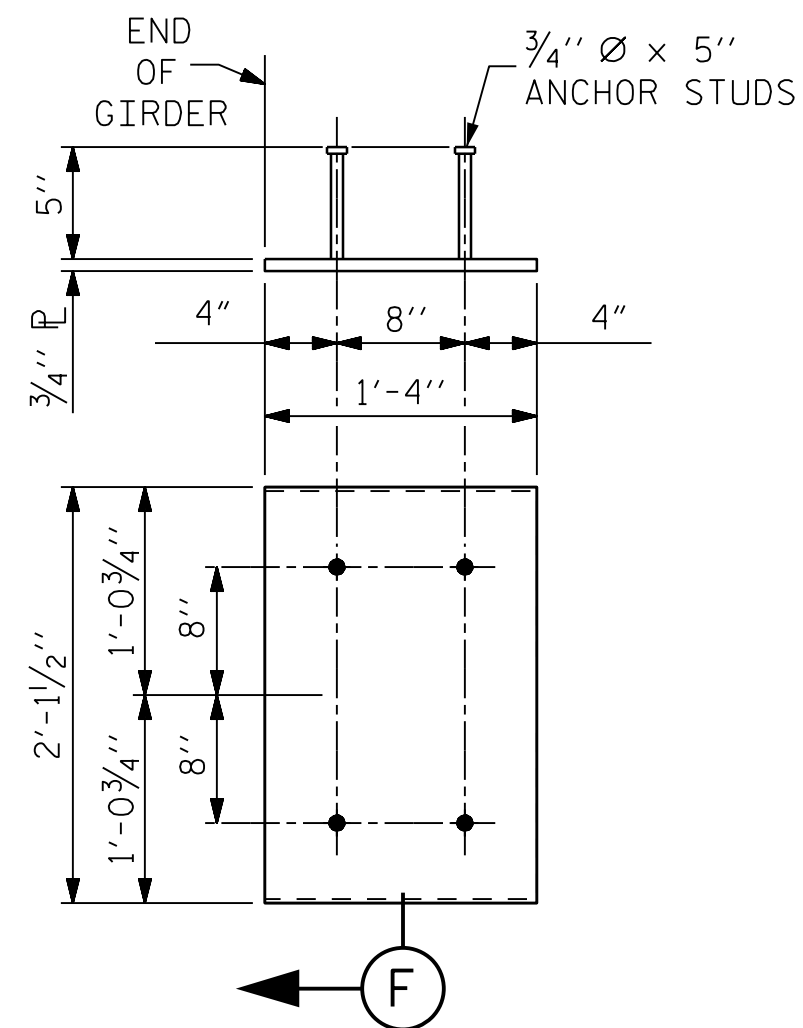
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 CHECKED BY : T. M. HARRIS DATE : 10-16  
 DRAWN BY : EEM 2/6/97 REV. 10/1/11 MAA/GM  
 CHECKED BY : VAP 2/6/97 REV. 6/13 MAA/GM  
 DATE: 10/20/17 12:47:23 PM REV. 1/15 MAA/TMC



**DETAIL "C"**  
(FOR 63" MODIFIED BULB TEES)



**SECTION "F"**  
(SEE NOTES)



**EMBEDDED PLATE "B-1" DETAILS  
FOR 63" MODIFIED BULB TEES**  
(2 REQUIRED PER GIRDER)

**NOTES**

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5,800 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" MODIFIED BULB TEES.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4,500 LBS.

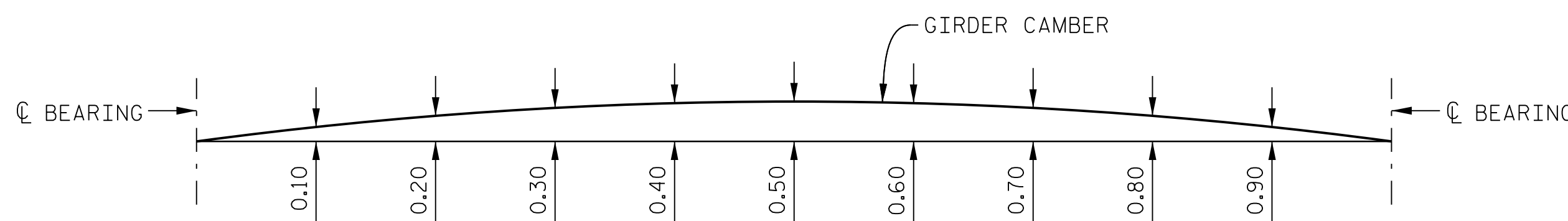
FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

**CAMBER AND DEAD LOAD DEFLECTIONS**

GIRDERS 1 THRU 4	SPAN A										
	CL BRG.	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	CL BRG.
CAMBER (GIRDER ALONE IN PLACE)	↑ 0.000	0.048	0.090	0.123	0.144	0.152	0.144	0.123	0.090	0.048	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. *	↑ 0.000	0.027	0.054	0.075	0.088	0.092	0.088	0.075	0.054	0.027	0.000
FINAL CAMBER	↑ 0	1/4"	7/16"	9/16"	11/16"	11/16"	11/16"	9/16"	7/16"	1/4"	0

\* INCLUDES FUTURE WEARING SURFACE

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).



**GIRDER CAMBER AND DEFLECTIONS**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

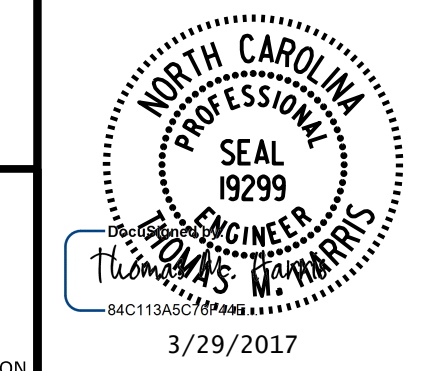
STANDARD

**63" PRESTRESSED CONCRETE  
MODIFIED BULB TEE GIRDER**

(LEFT LANE)

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2			4			

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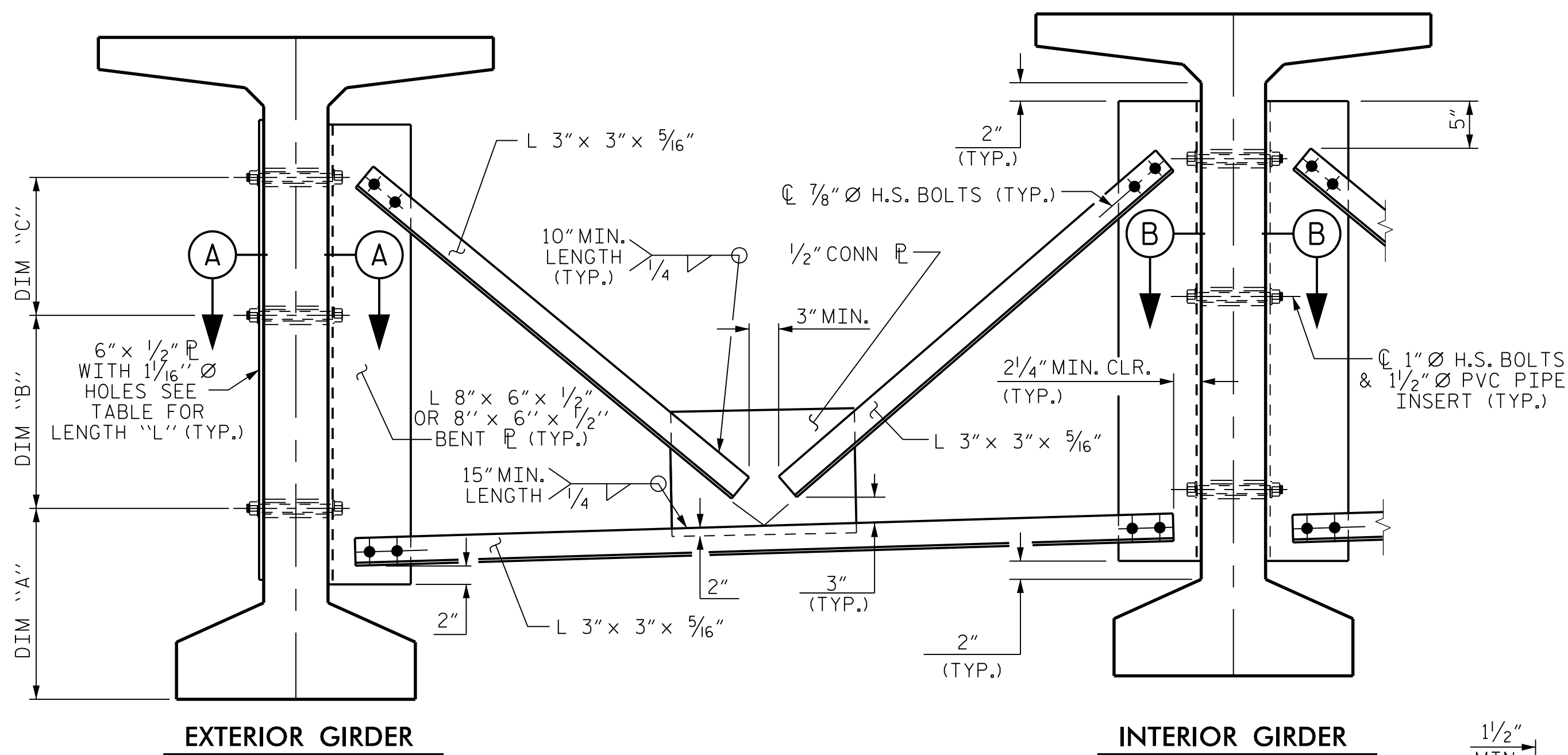


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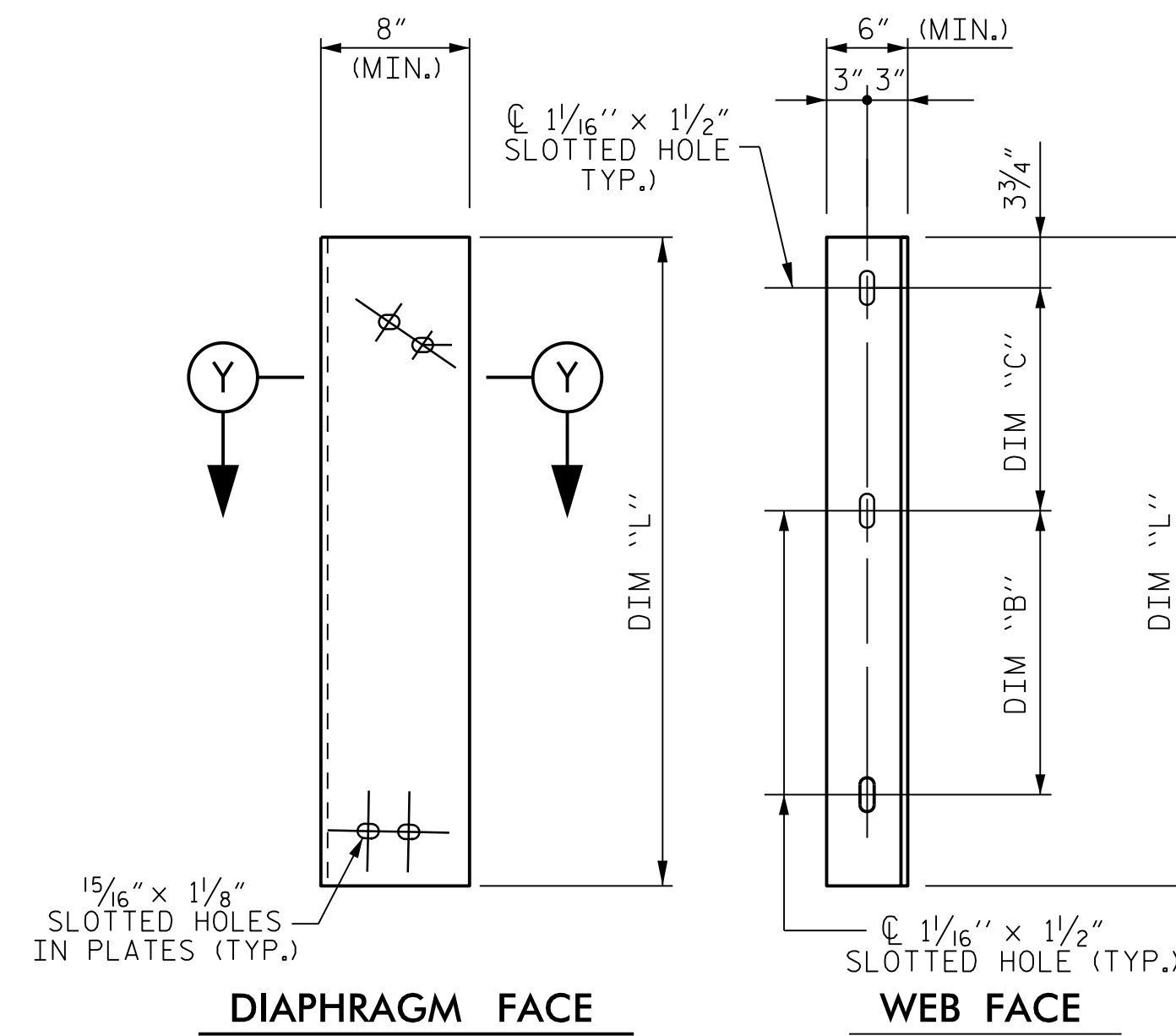
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DRAWN BY : ELR 11/91 REV. 10/11/11 MAA/GM  
CHECKED BY : GRP 11/91 REV. 1/15 MAA/TMG  
REV. 2/15 MAA/TMG





**PART SECTION AT INTERMEDIATE DIAPHRAGM**

(63" BULB TEE)



**CONNECTOR PLATE DETAILS**

**STRUCTURAL STEEL NOTES**

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

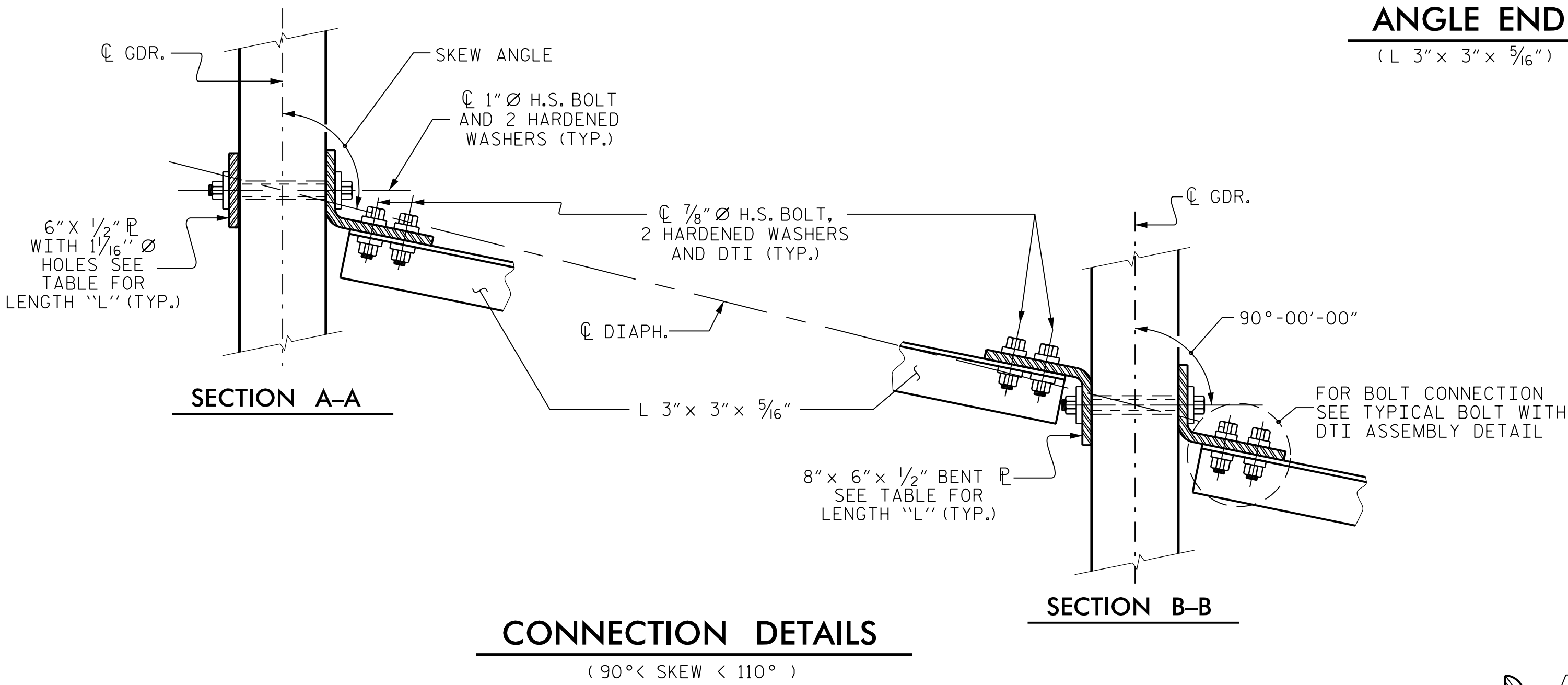
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

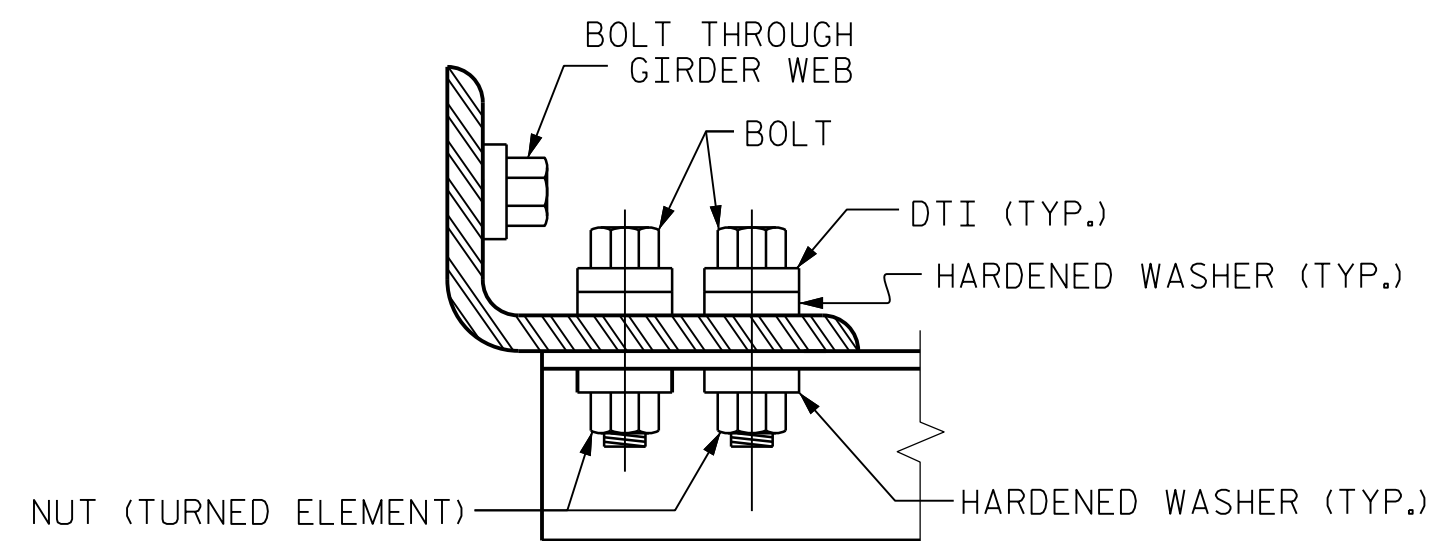
IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



**CONNECTION DETAILS**

(90° < SKEW < 110°)

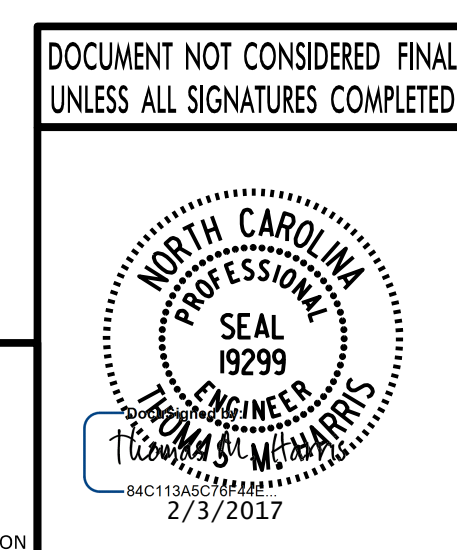


**BOLT WITH DTI ASSEMBLY DETAIL**

TABLE				
GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
63" BULB TEE	1'-9"	1'-5"	11 3/4"	3'-5"

PROJECT NO. R-2707C  
 CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
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 STANDARD  
**INTERMEDIATE STEEL DIAPHRAGMS  
 FOR 63" MODIFIED BULB TEE  
 PRESTRESSED CONCRETE GIRDERS  
 (LEFT LANE)**



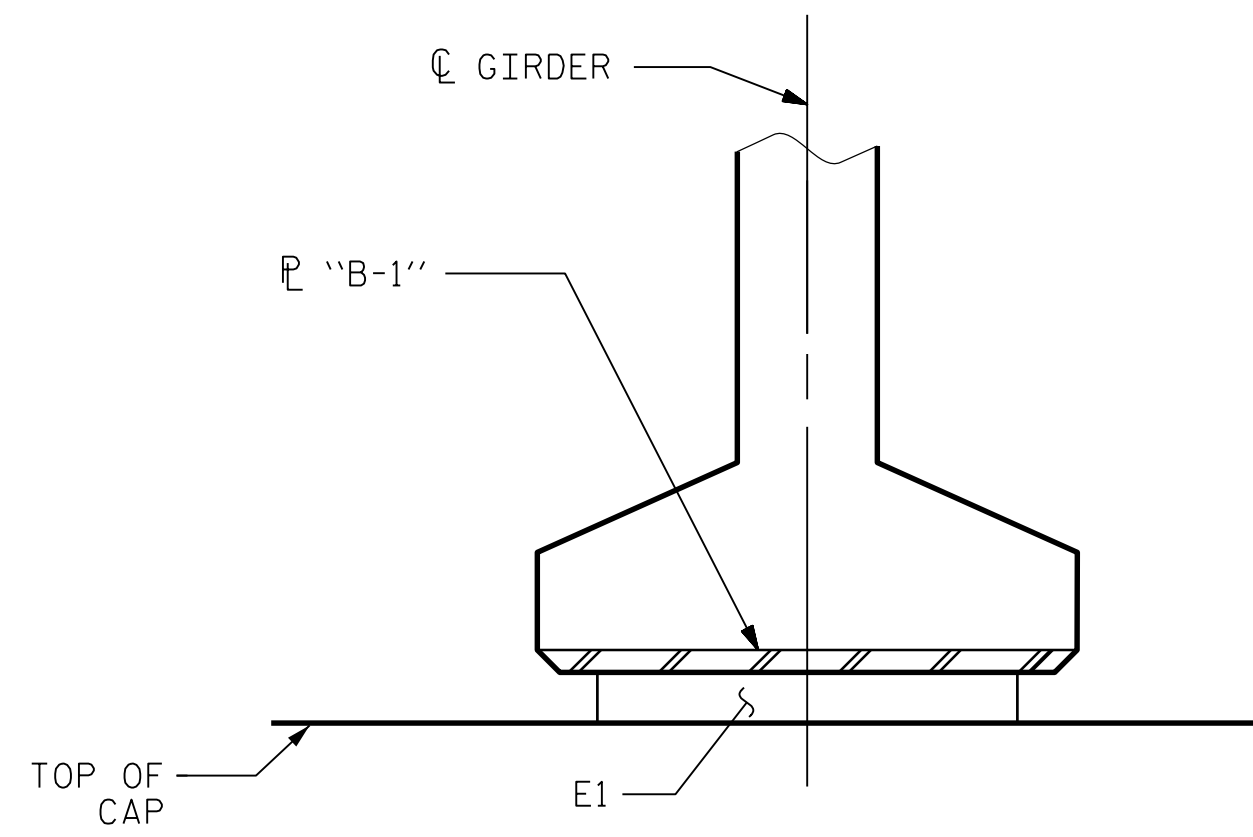
PLANS PREPARED BY:  
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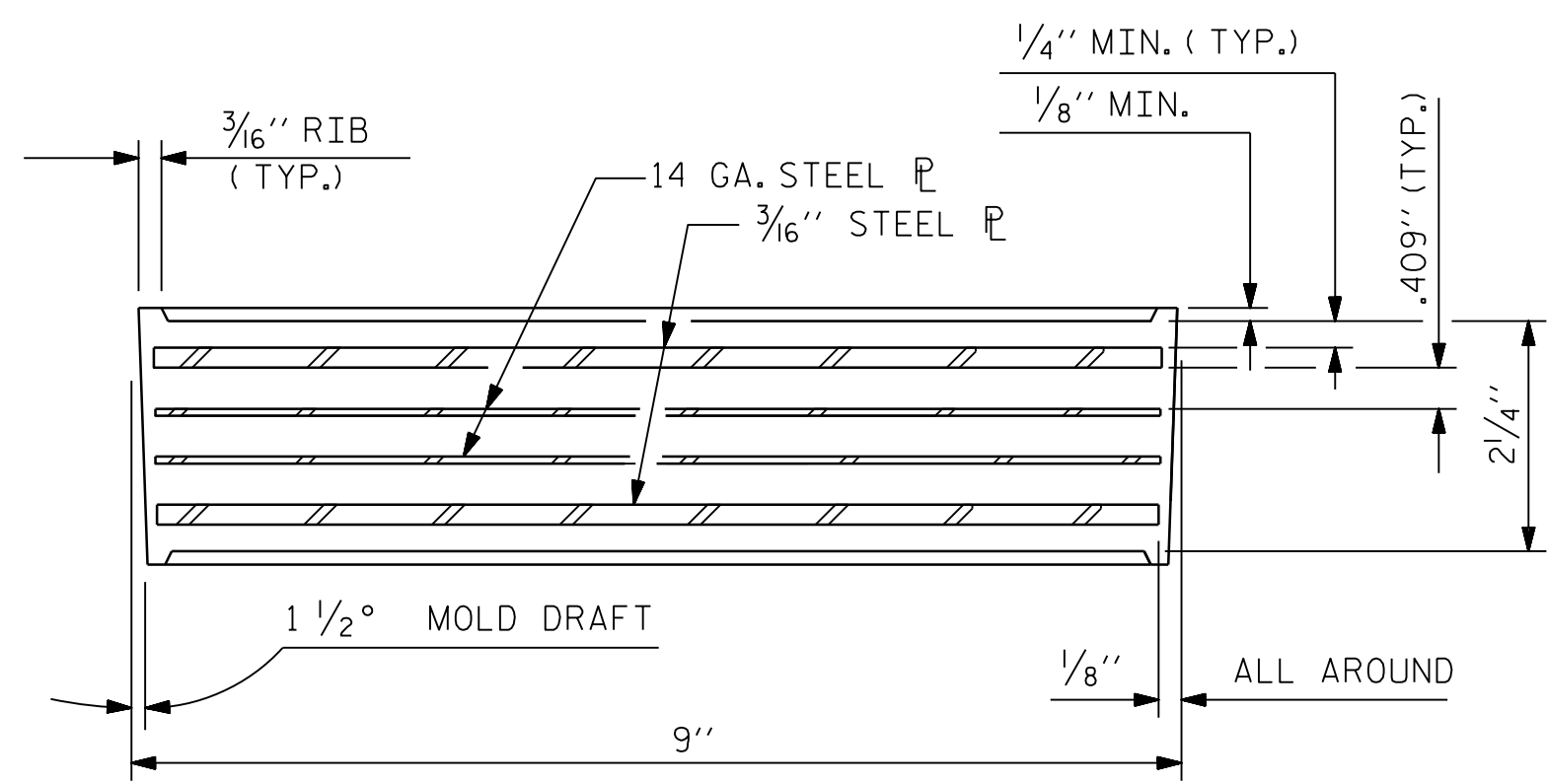
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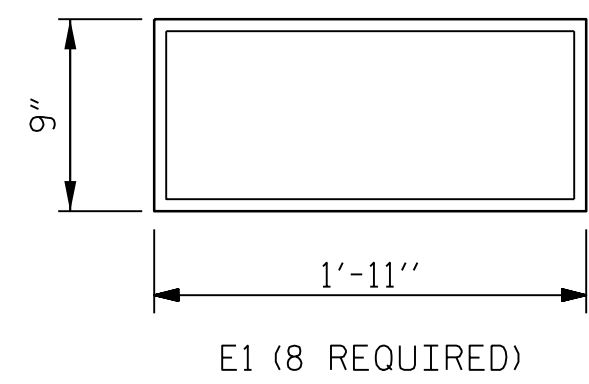
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 CHECKED BY : GM 11/09  
 ADDED 11/23/09  
 REV. 10/1/11 MAA/GM



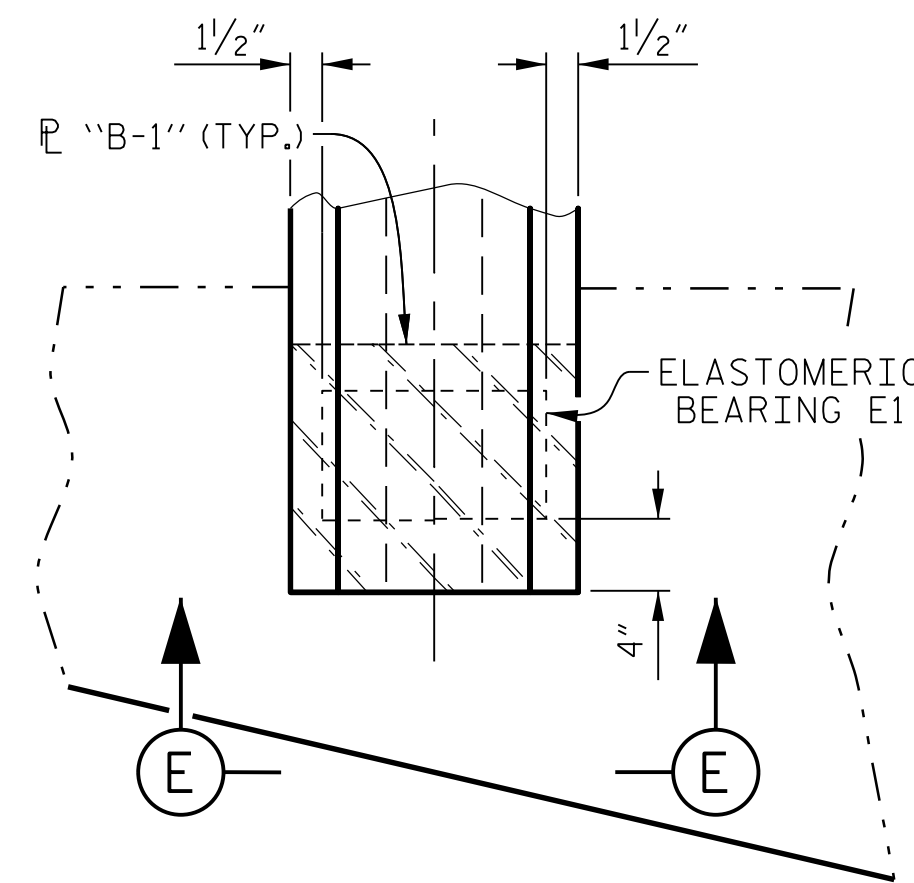
INTEGRAL  
SECTION E-E



TYPICAL SECTION OF ELASTOMERIC BEARINGS



PLAN VIEW OF ELASTOMERIC BEARING  
TYPE V



PARTIAL PLAN - INTEGRAL END BENT

**NOTES**

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

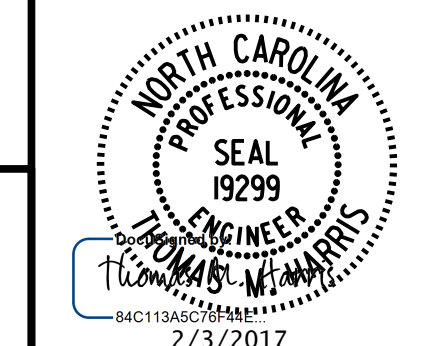
MAXIMUM ALLOWABLE SERVICE LOAD	
	D.L.+ L.L. (NO IMPACT)
TYPE V	365 K

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CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

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**ELASTOMERIC BEARING DETAILS**  
**PRESTRESSED CONCRETE GIRDER**  
**SUPERSTRUCTURE**  
**(LEFT LANE)**

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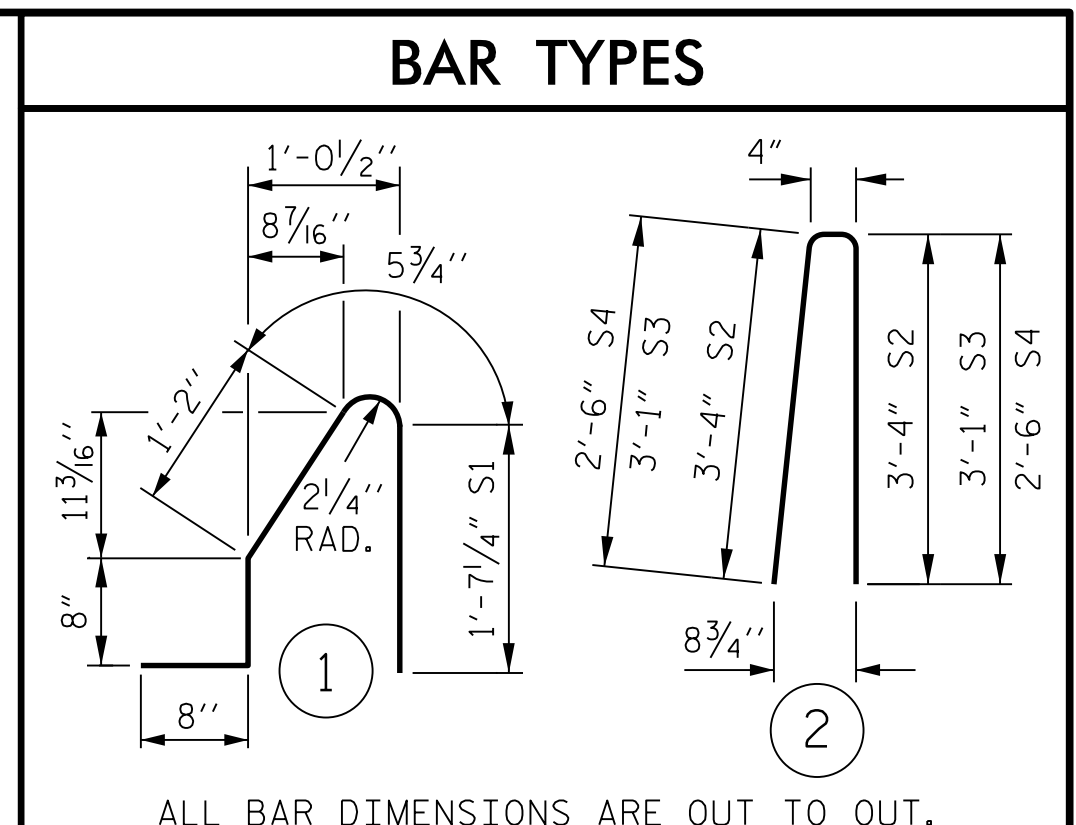
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 REV. 1/15 MAA/TMG

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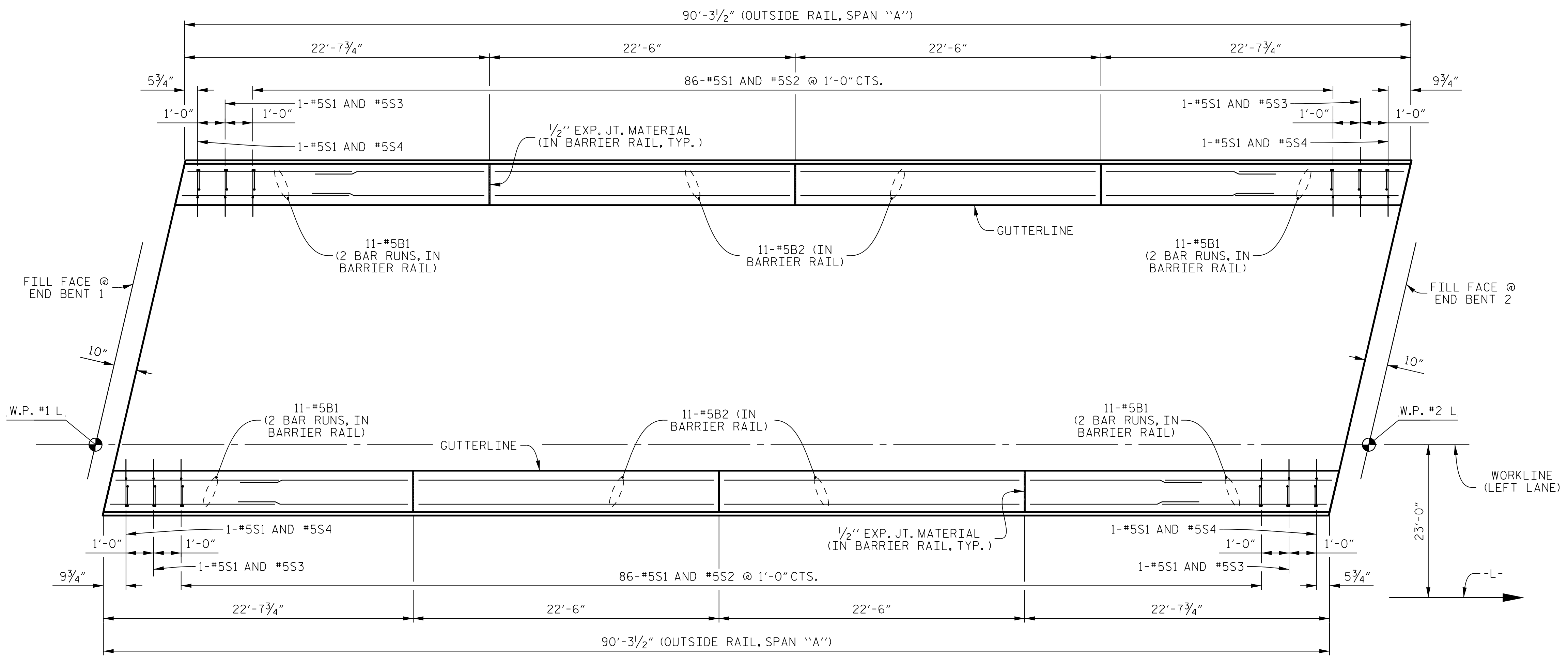


### BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY

BAR No.	SIZE	TYPE	LENGTH	WEIGHT
* B1	88	#5	STR 12'-10"	1,178
* B2	44	#5	STR 22'-1"	1,013
* S1	180	#5	1 4'-7"	860
* S2	172	#5	2 7'-0"	1,256
* S3	4	#5	2 6'-6"	27
* S4	4	#5	2 5'-4"	22

\*EPOXY COATED REINFORCING STEEL 4,356 LBS.  
 CLASS "AA" CONCRETE 24.5 CU. YDS.  
 CONCRETE BARRIER RAIL 180.58 LIN. FT.



### SPLICE LENGTH

BAR	SIZE	LENGTH
B1	#5	3'-5"

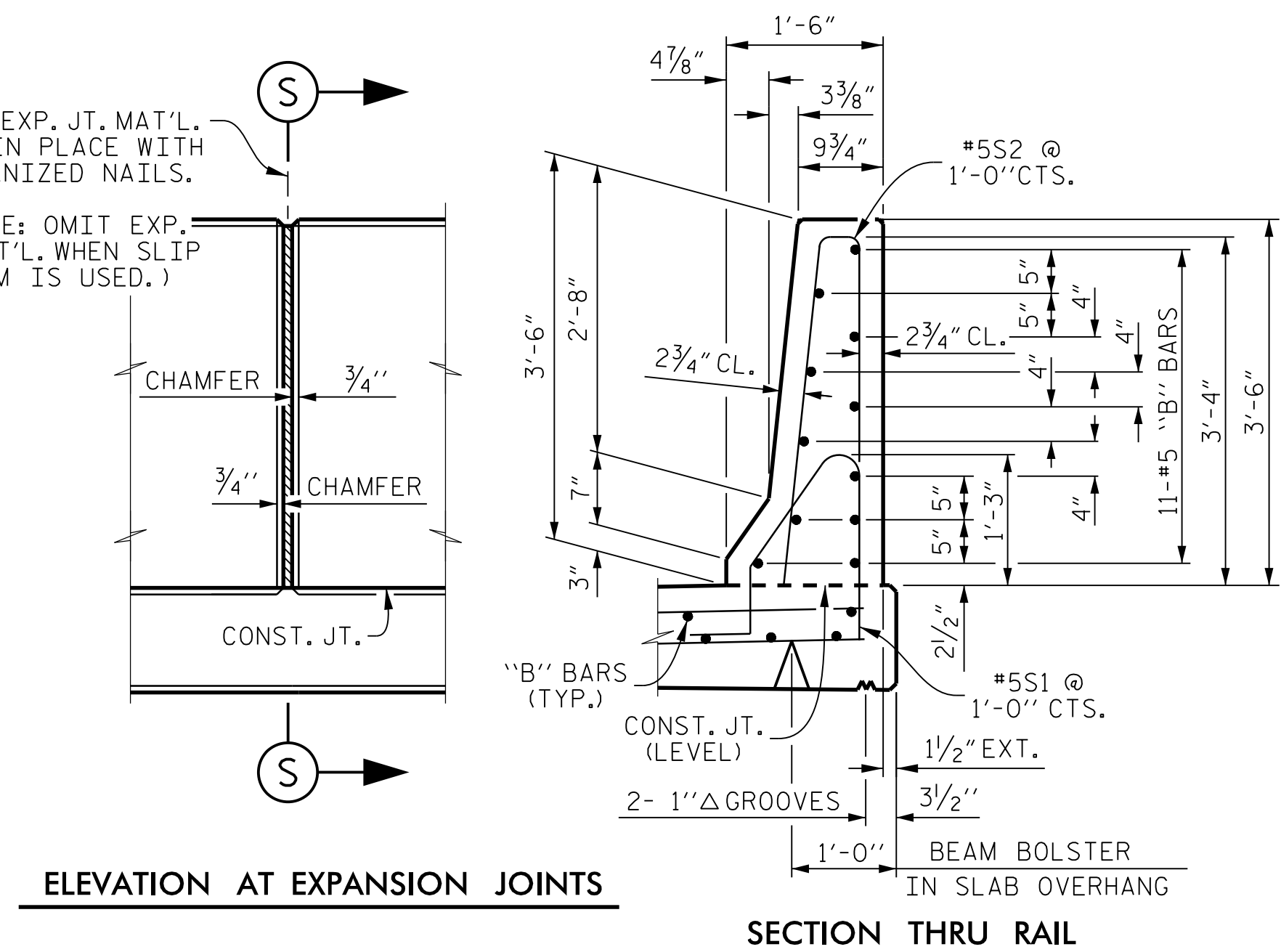
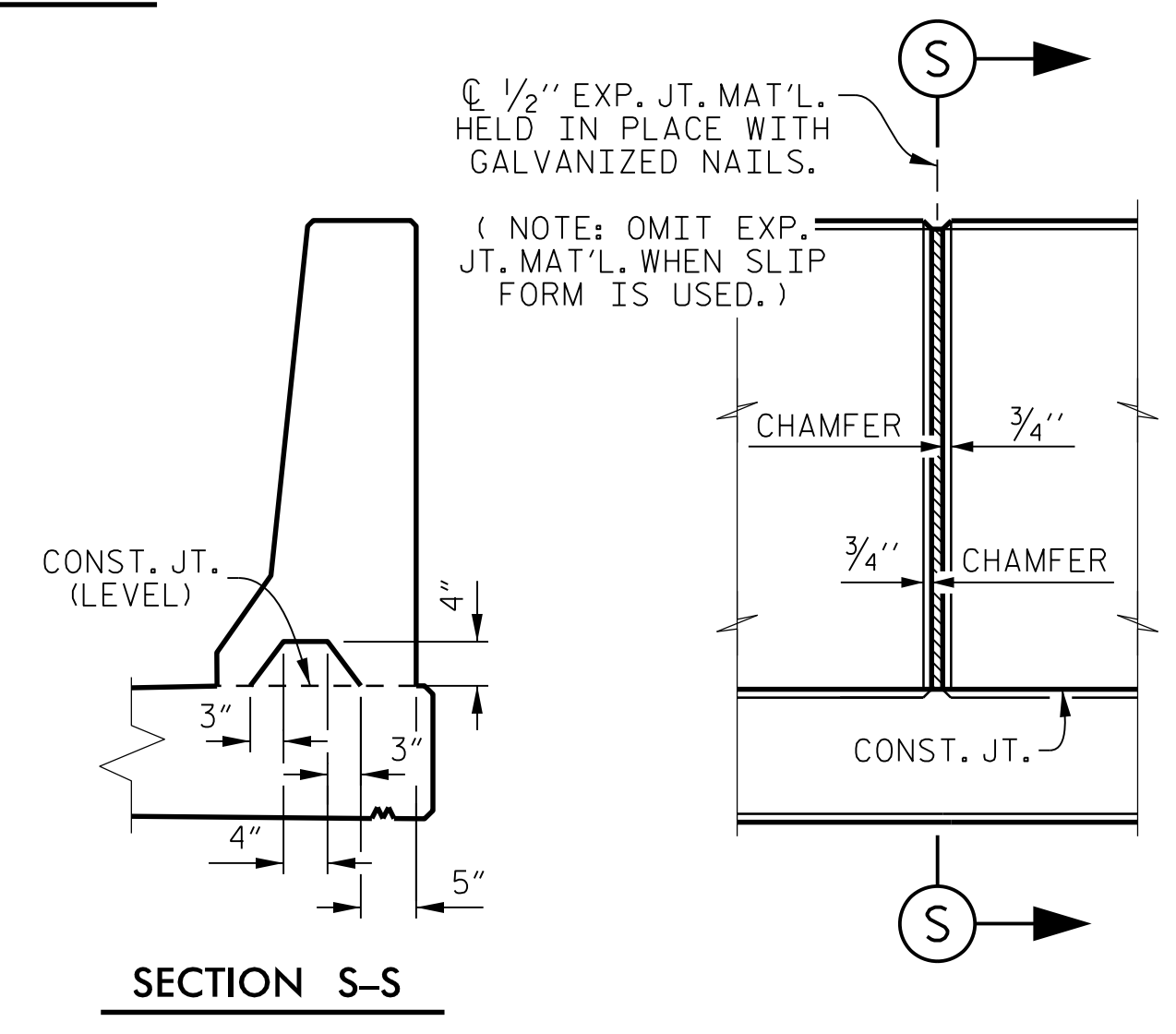
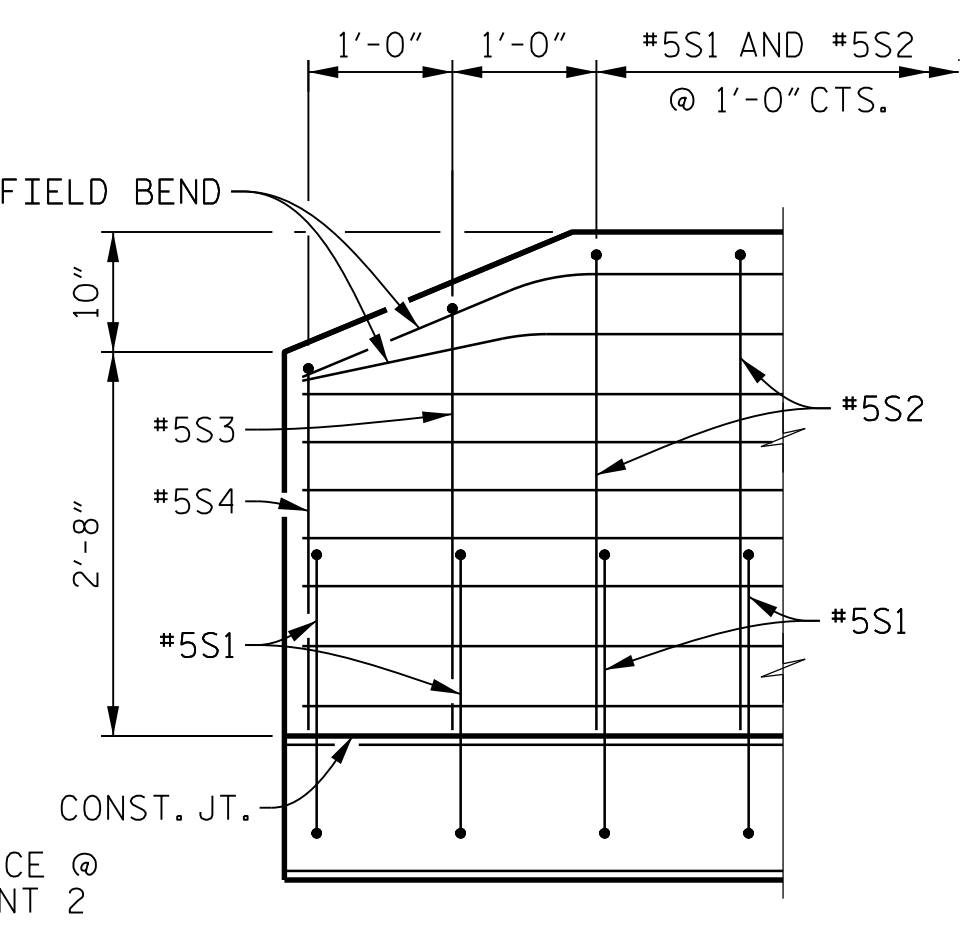
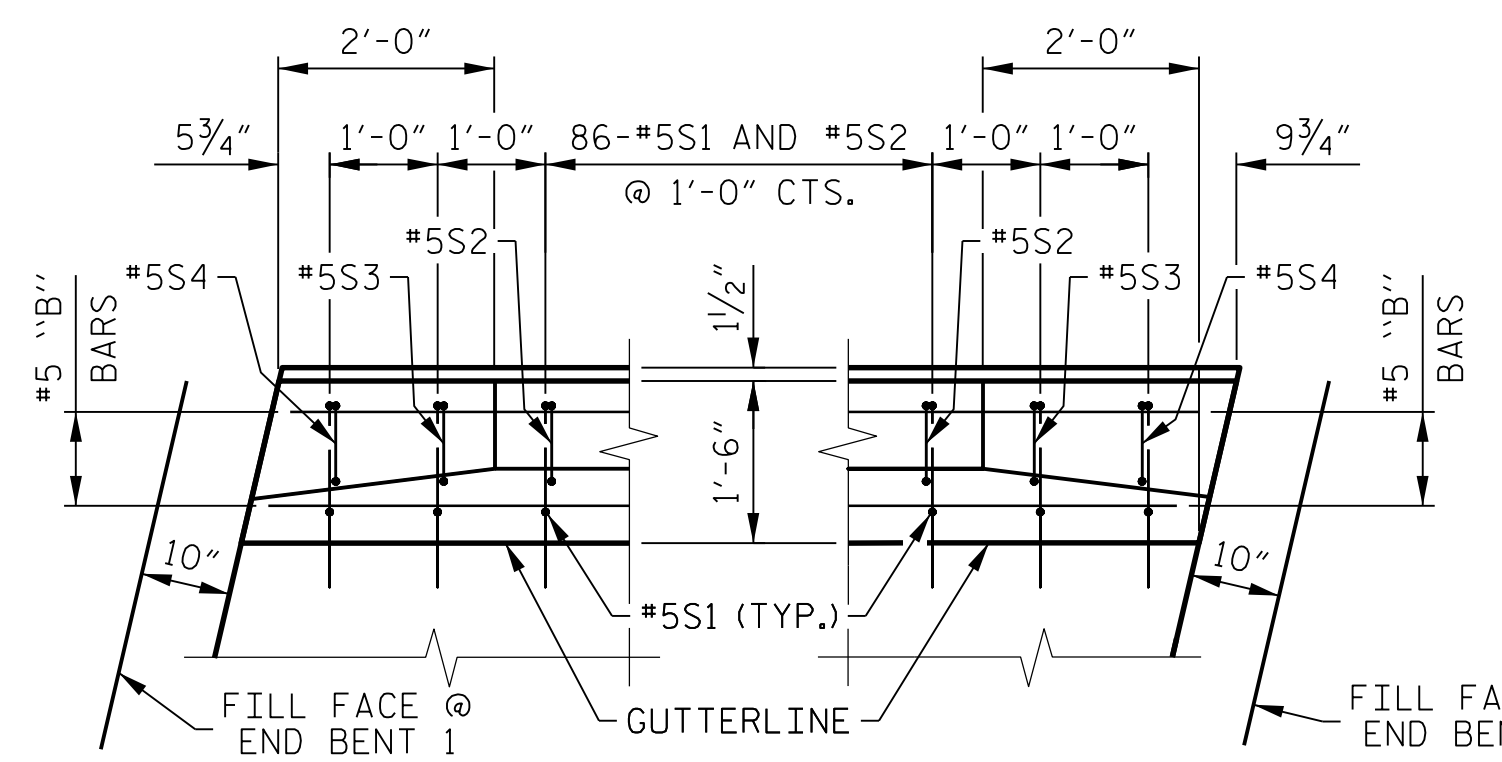
### NOTES

THE BARRIER RAIL IN SPAN "A" SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE #5S1 AND #5S2 BARS MAY BE SHIFTED AS NECESSARY TO CLEAR EXPANSION JOINTS IN RAIL.



PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

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 RALEIGH

STANDARD  
**CONCRETE BARRIER RAIL (LEFT LANE)**

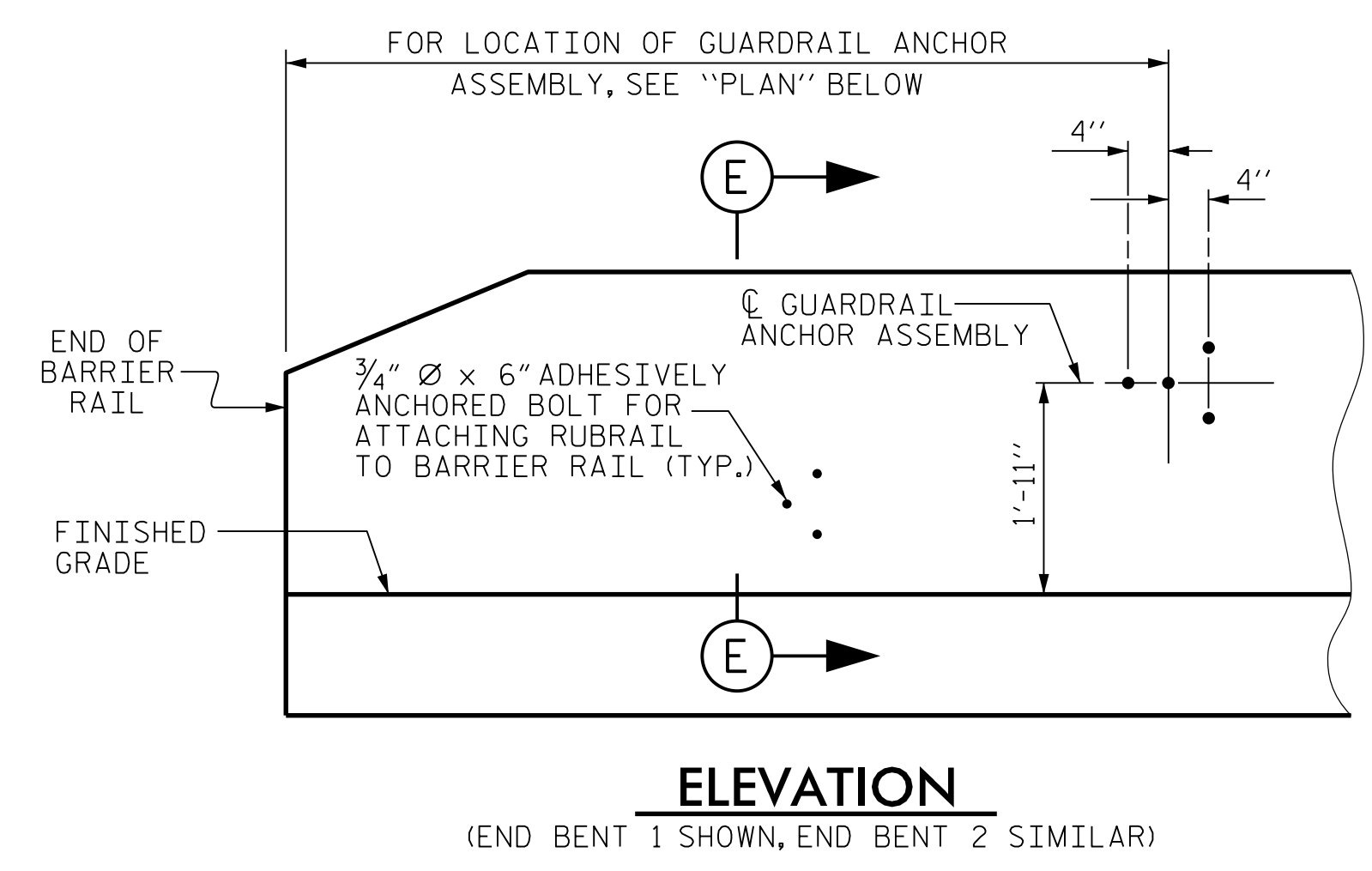
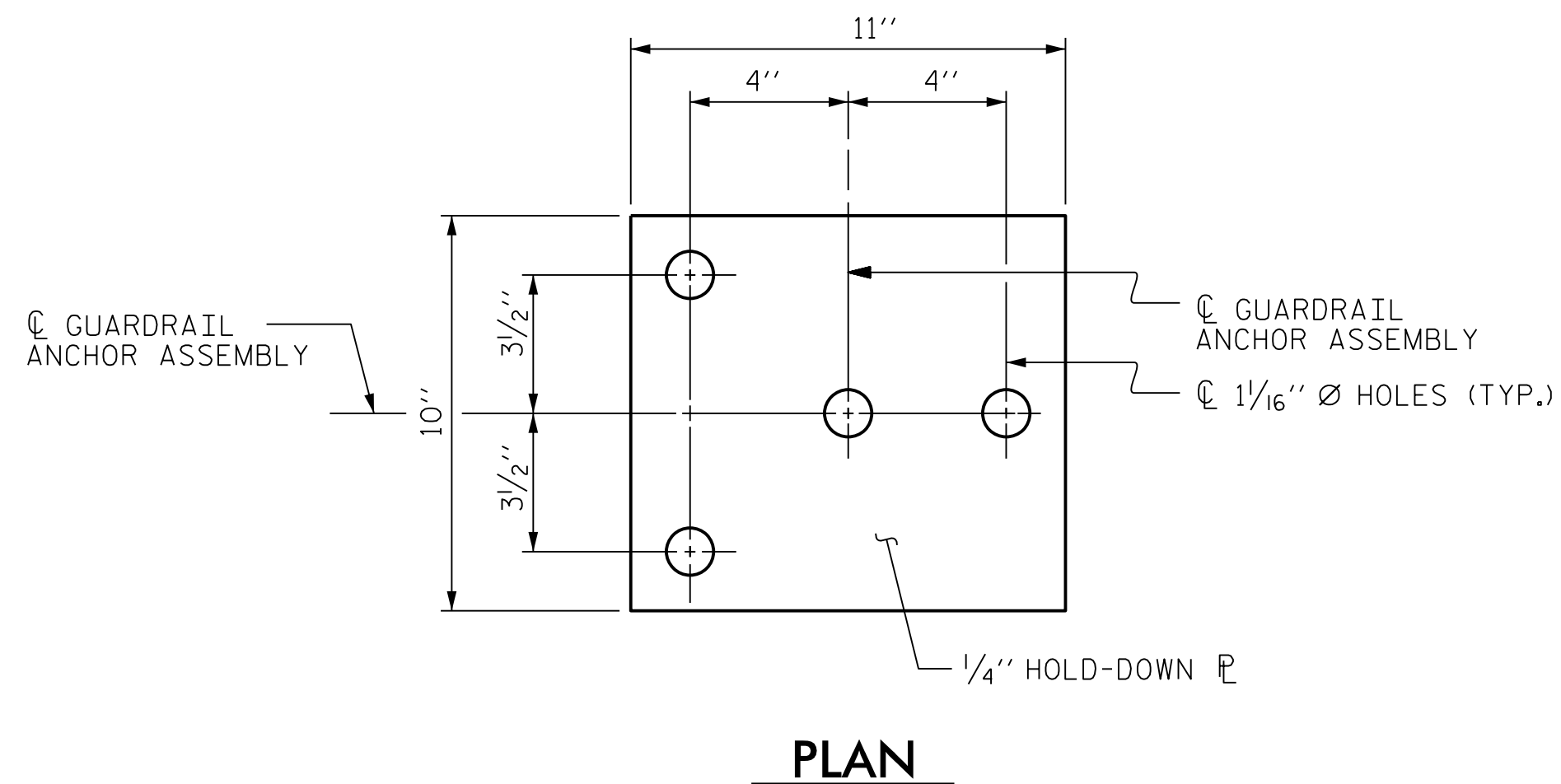
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 DESIGN ENGINEER: **T. M. HARRIS** DATE: **10-16**

ASSEMBLED BY: **K. E. LOFTON** DATE: **6-16**  
 CHECKED BY: **A. D. SHAH** DATE: **10-16**  
 DRAWN BY: **ARB** 5/87 REV. 10/1/11 MAA/GM  
 CHECKED BY: **SJD** 9/87 REV. 7/12 MAA/GM  
 DATE: 10/20/17 REV. 6/13 MAA/GM

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**NOTES**

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

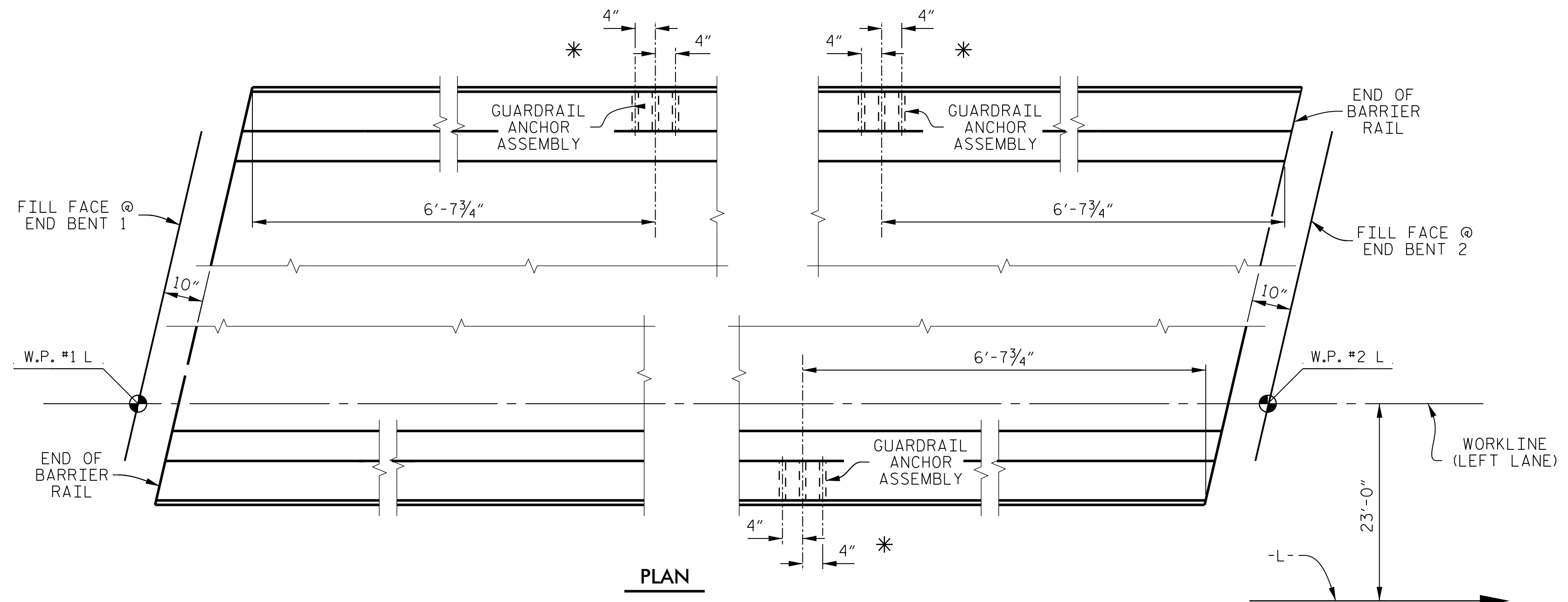
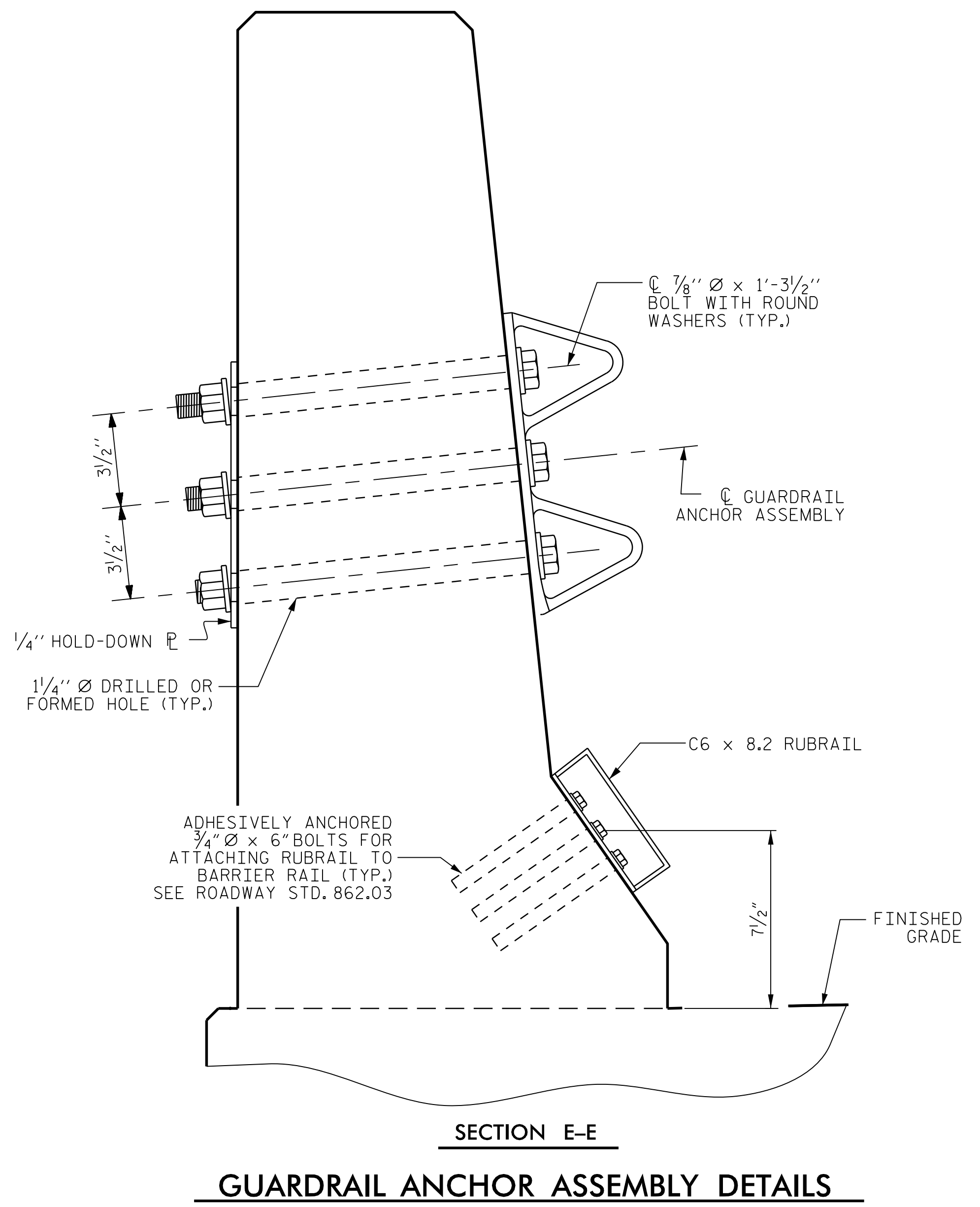
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

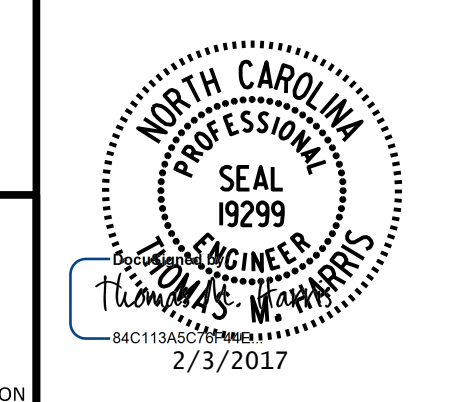
THE C6 x 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø x 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
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 RALEIGH  
 STANDARD  
 GUARDRAIL ANCHORAGE  
 FOR BARRIER RAIL  
 (LEFT LANE)

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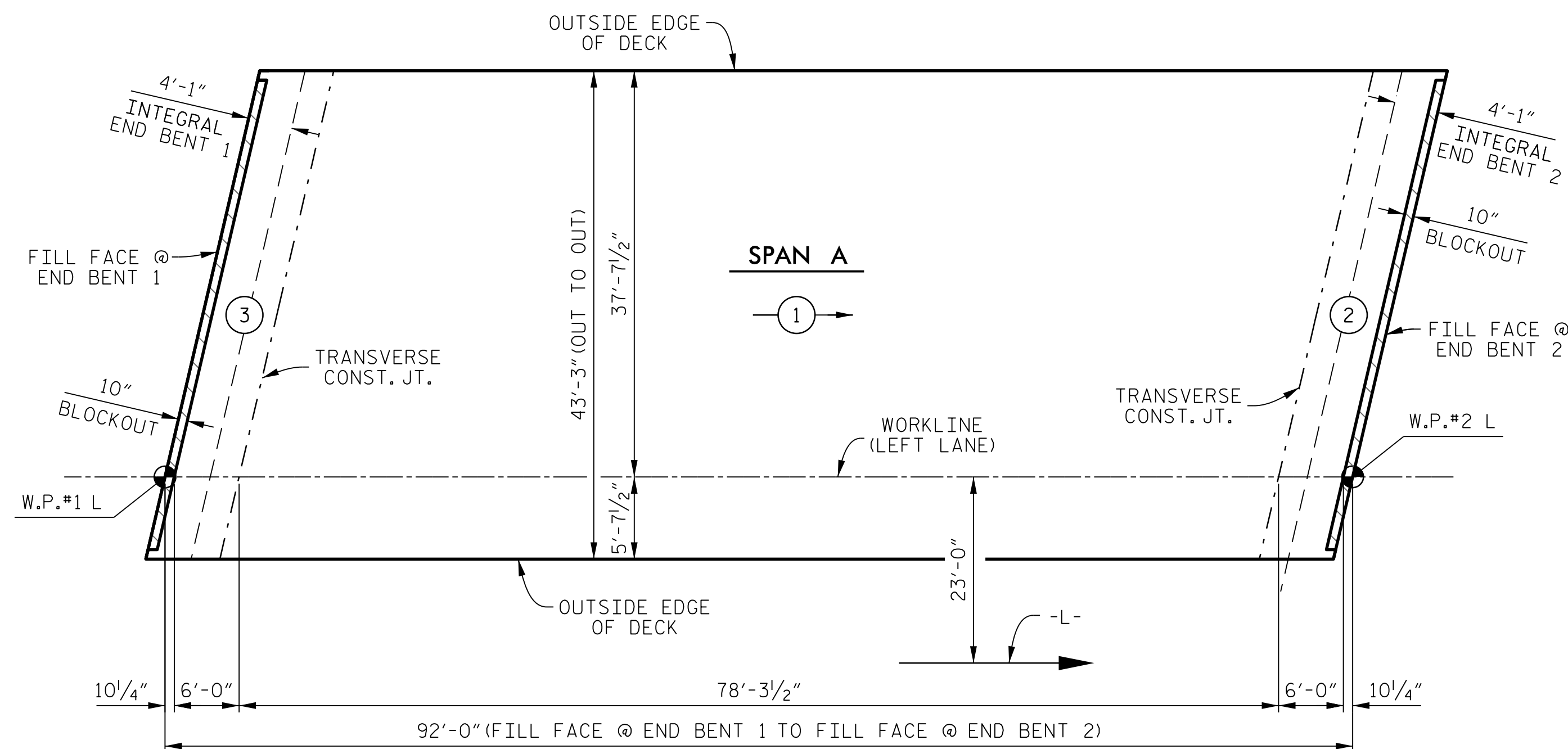
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 DRAWN BY: TLA 5/06 REV. 10/1/11 MAA/GM  
 CHECKED BY: GM 5/06 REV. 7/12 MAA/GM  
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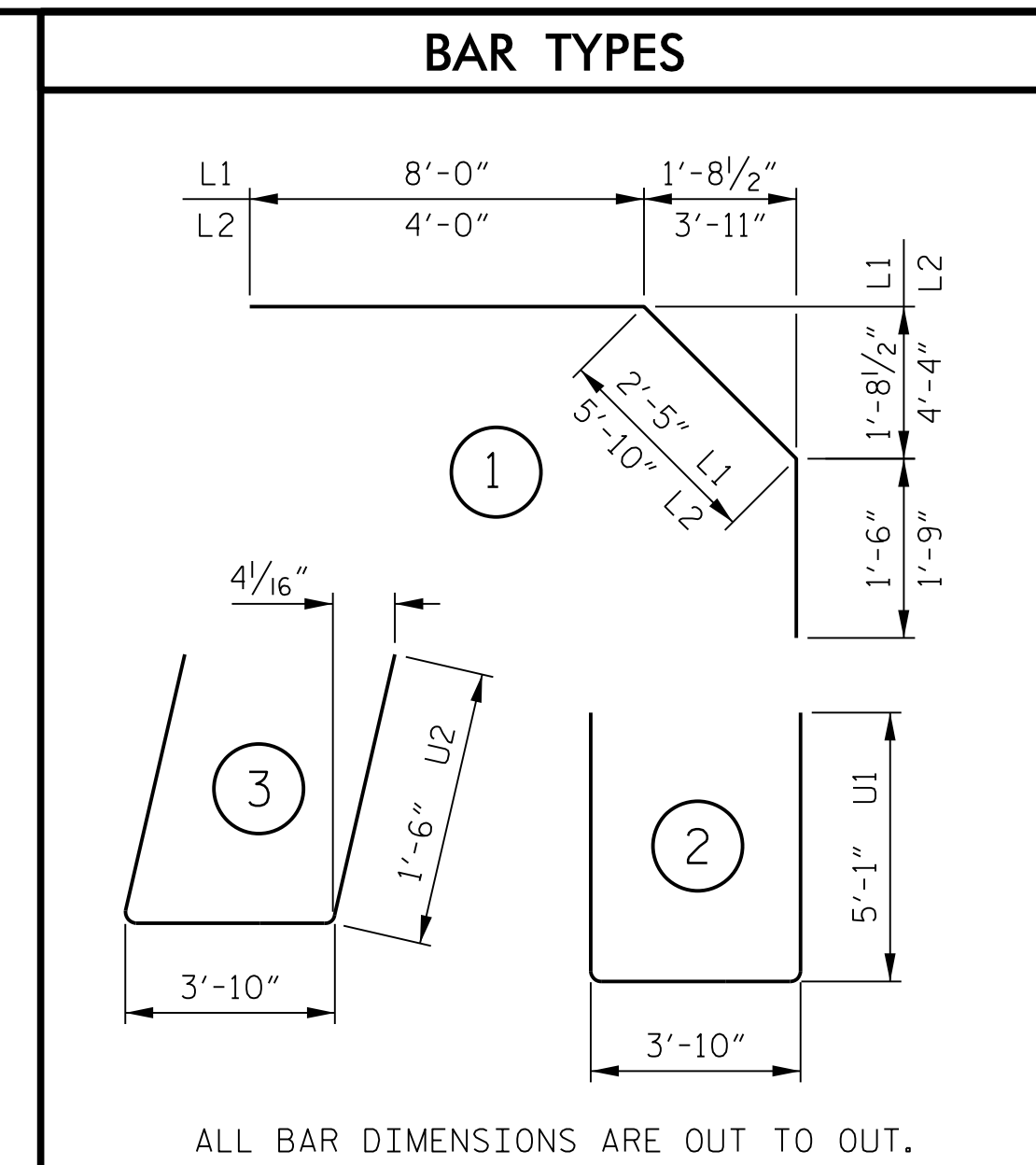




**POURING SEQUENCE**

CLASS AA CONCRETE BREAKDOWN	
POUR 1	116.9 CU. YDS.
POUR 2	44.1 CU. YDS.
POUR 3	44.1 CU. YDS.
<b>TOTAL</b>	<b>205.1 CU. YDS.</b>

**NOTES**  
 POUR 2 AND POUR 3 MAY BE COMBINED.  
 DIRECTION OF POUR 1 MAY BE REVERSED.  
 # INDICATES POUR NUMBER AND DIRECTION OF POUR.

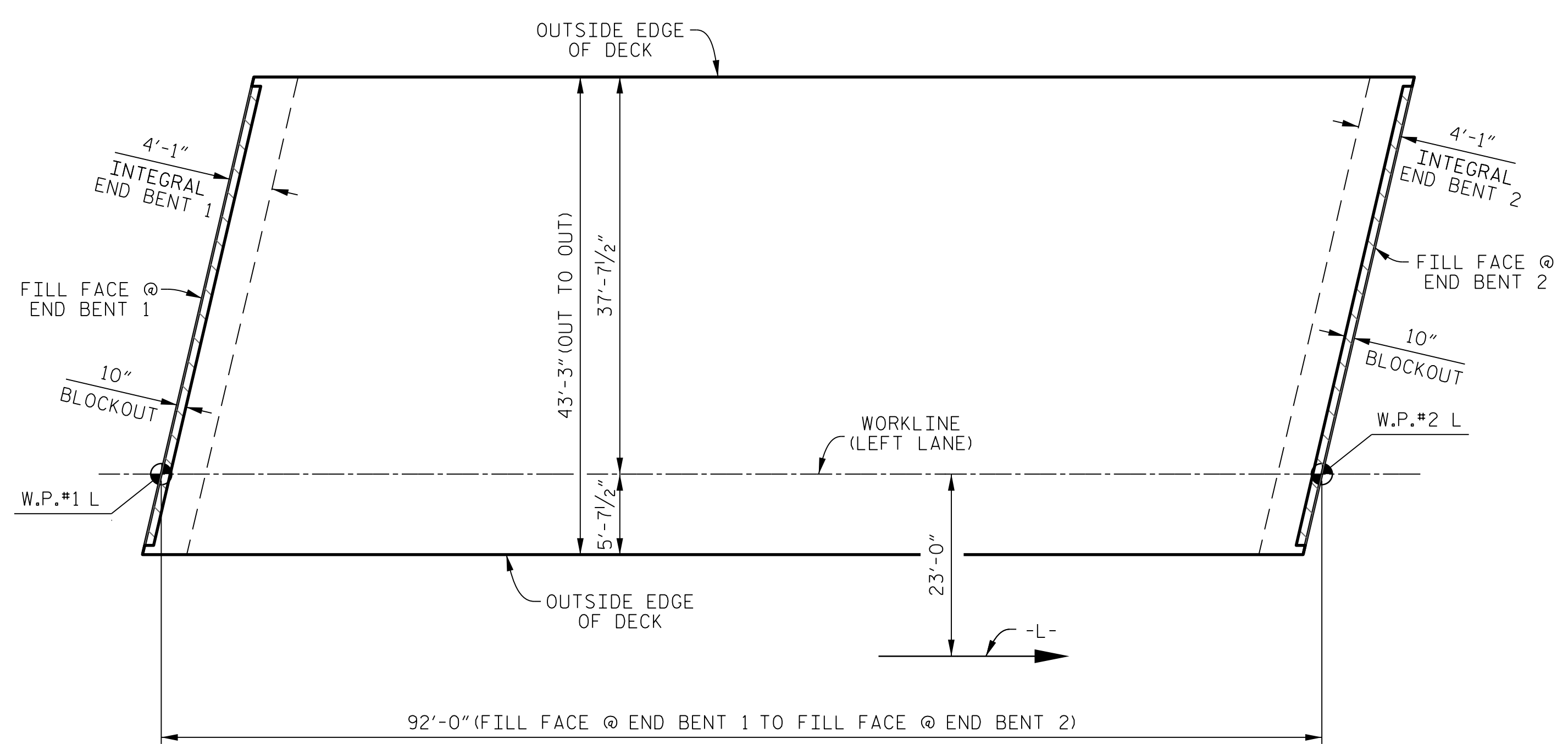


BILL OF MATERIAL					
SPAN A					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
*A1	161	#5	STR	42'-11"	7,207
*A101	2	#5	STR	40'-9"	85
*A102	2	#5	STR	38'-7"	80
*A103	2	#5	STR	36'-5"	76
*A104	2	#5	STR	34'-3"	71
*A105	2	#5	STR	32'-2"	67
*A106	2	#5	STR	30'-0"	63
*A107	2	#5	STR	27'-10"	58
*A108	2	#5	STR	25'-8"	54
*A109	2	#5	STR	23'-6"	49
*A110	2	#5	STR	21'-4"	45
*A111	2	#5	STR	19'-2"	40
*A112	2	#5	STR	17'-0"	35
*A113	2	#5	STR	14'-10"	31
*A114	2	#5	STR	12'-8"	26
*A115	2	#5	STR	10'-6"	22
*A116	2	#5	STR	8'-4"	17
*A117	2	#5	STR	6'-2"	13
*A118	2	#5	STR	4'-0"	8
A2	161	#5	STR	42'-11"	7,207
A201	2	#5	STR	40'-9"	85
A202	2	#5	STR	38'-7"	80
A203	2	#5	STR	36'-5"	76
A204	2	#5	STR	34'-3"	71
A205	2	#5	STR	32'-2"	67
A206	2	#5	STR	30'-0"	63
A207	2	#5	STR	27'-10"	58
A208	2	#5	STR	25'-8"	54
A209	2	#5	STR	23'-6"	49
A210	2	#5	STR	21'-4"	45
A211	2	#5	STR	19'-2"	40
A212	2	#5	STR	17'-0"	35
A213	2	#5	STR	14'-10"	31
A214	2	#5	STR	12'-8"	26
A215	2	#5	STR	10'-6"	22
A216	2	#5	STR	8'-4"	17
A217	2	#5	STR	6'-2"	13
A218	2	#5	STR	4'-0"	8
*B1	120	#4	STR	24'-0"	1,924
*B2	162	#5	STR	18'-5"	3,112
B3	114	#5	STR	46'-1"	5,479
K1	28	#4	STR	22'-11"	429
K2	6	#4	STR	7'-10"	31
K3	30	#4	STR	10'-9"	215
K4	6	#4	STR	9'-4"	37
K5	4	#4	STR	2'-2"	6
K6	20	#4	STR	3'-7"	48
K7	4	#4	STR	2'-11"	8
*L1	60	#4	1	11'-11"	478
*L2	56	#4	1	11'-7"	433
U1	60	#4	2	14'-0"	561
U2	28	#4	3	6'-10"	128
REINFORCING STEEL				14,989	LBS.
*EPOXY COATED REINFORCING STEEL				13,994	LBS.

TOTAL SUPERSTRUCTURE QUANTITIES			
	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL	CLASS "AA" CONCRETE
	LBS.	LBS.	CU. YDS.
SPAN A	14,989	13,994	205.1
<b>** TOTAL</b>	<b>14,989</b>	<b>13,994</b>	<b>205.1</b>

\*\* QUANTITIES FOR CONCRETE BARRIER RAIL ARE NOT INCLUDED. SEE "CONCRETE BARRIER RAIL" SHEET FOR DETAILS.

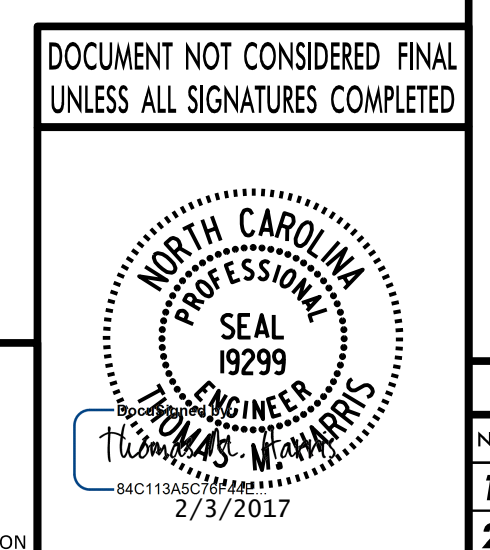
GROOVING BRIDGE FLOORS	
APPROACH SLAB AT END BENT 1	1,089 SQ. FT.
BRIDGE DECK	3,327 SQ. FT.
APPROACH SLAB AT END BENT 2	1,089 SQ. FT.
<b>TOTAL</b>	<b>5,505 SQ. FT.</b>



**LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB**  
 (3,979 SQ. FT.)

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS					
BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

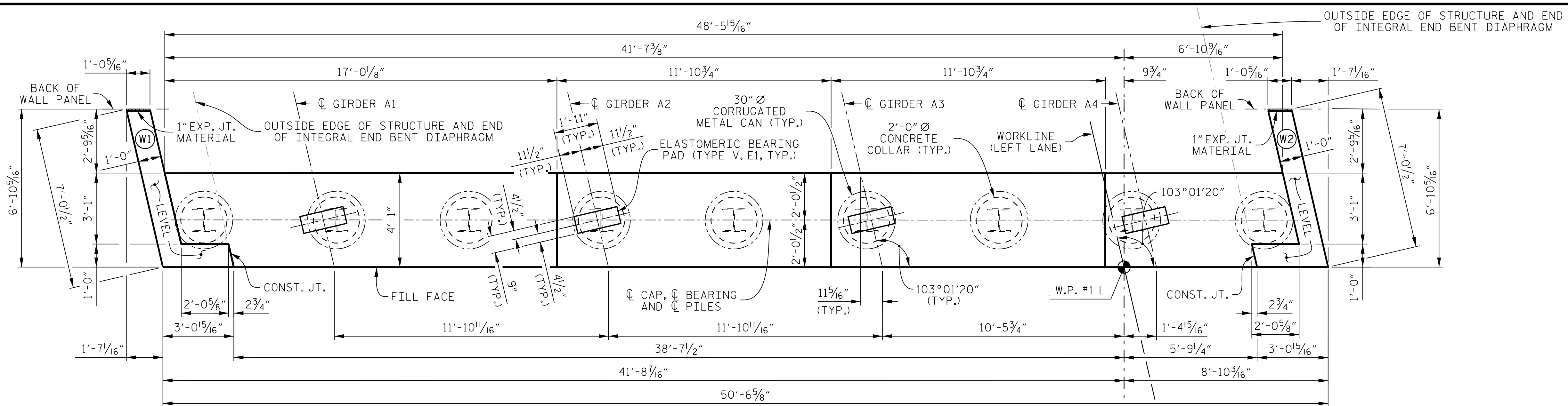
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**



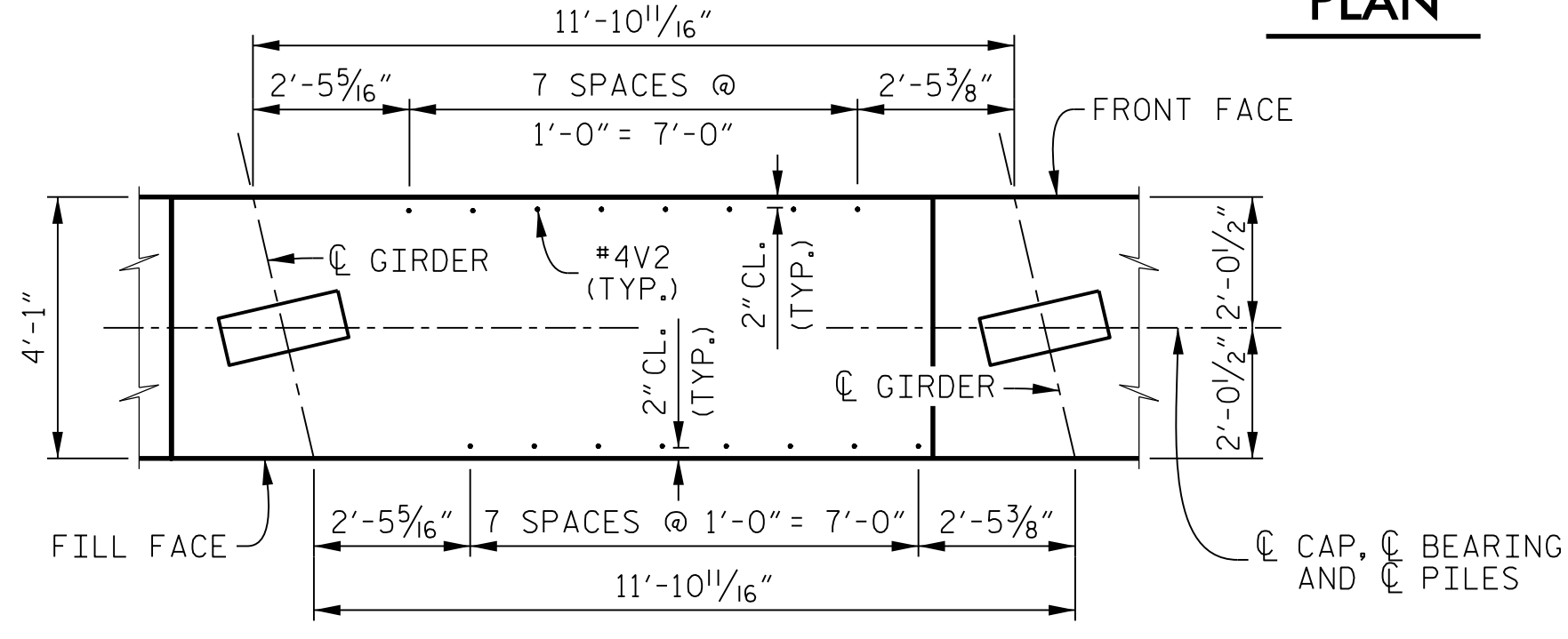
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: **K. E. LOFTON** DATE: **6-16**  
 CHECKED BY: **A. D. SHAH** DATE: **10-16**  
 DESIGN ENGINEER: **T. M. HARRIS** DATE: **10-16**

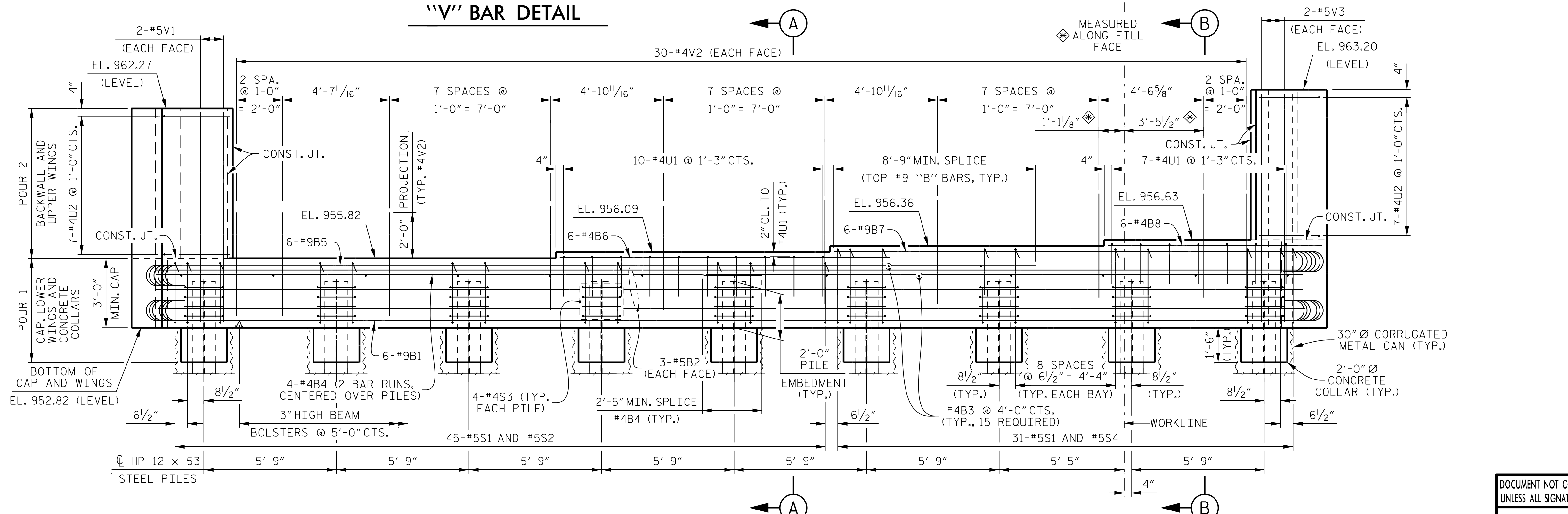
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE BILL OF MATERIAL (LEFT LANE)					
REVISIONS					SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS
					25



**PLAN**



**"V" BAR DETAIL**



**ELEVATION**

**NOTES**

STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4V2 BARS.

THE TOP SURFACE OF THE END BENT CAP UNDER THE INTEGRAL END BENT DIAPHRAGM, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

FOR TEMPORARY DRAINAGE AT END BENT DETAIL, SEE SHEET 3 OF 3.

FOR SECTION A-A AND SECTION B-B, SEE SHEET 3 OF 3.

FOR INTEGRAL END BENT DIAPHRAGM DETAILS, SEE "TYPICAL SECTION" AND "PLAN OF SPAN" SHEETS.

THE COST TO FURNISH AND INSTALL THE 30"Ø CORRUGATED METAL CANS SHALL BE INCLUDED IN THE CONTRACT PRICE FOR MSE RETAINING WALL.

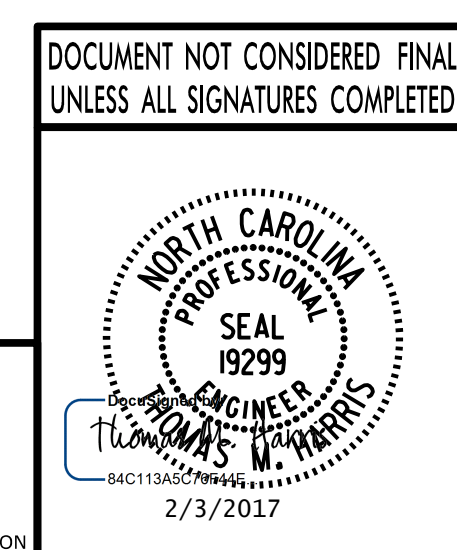
WING (W1) AND WING (W2) DETAILS ARE BASED ON A 5 1/2" WALL PANEL THICKNESS AND USING DOWELS FOR THE COPING. CONTRACTOR MAY ADJUST WINGS SLIGHTLY AS NECESSARY, BASED ON APPROVED MSE WALL SHOP DRAWINGS.

PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUBSTRUCTURE**  
**END BENT 1**  
**(LEFT LANE)**



PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

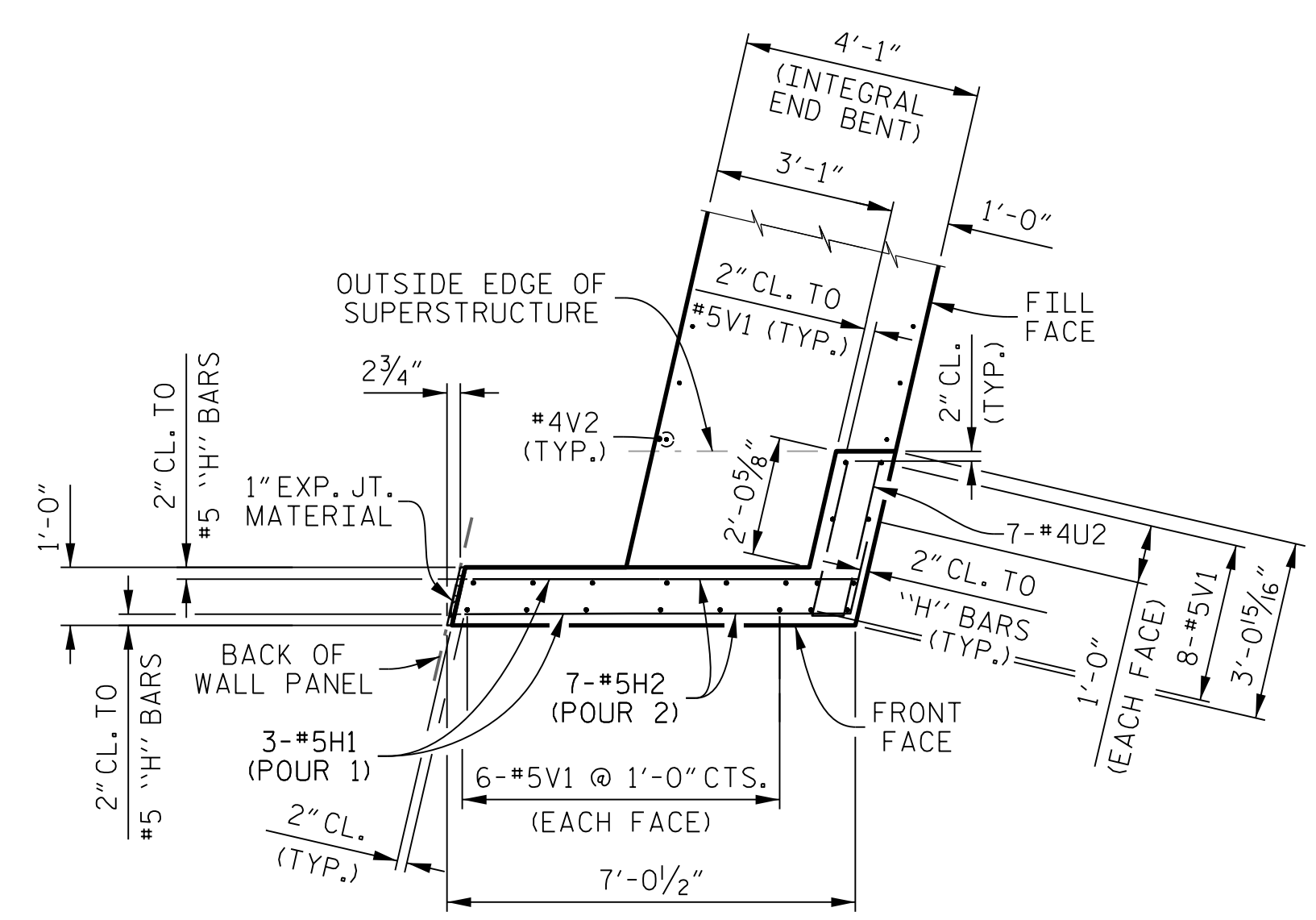
DRAWN BY: **K. E. LOFTON** DATE: **6-16**  
 CHECKED BY: **A. D. SHAH** DATE: **10-16**  
 DESIGN ENGINEER: **T. M. HARRIS** DATE: **10-16**

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

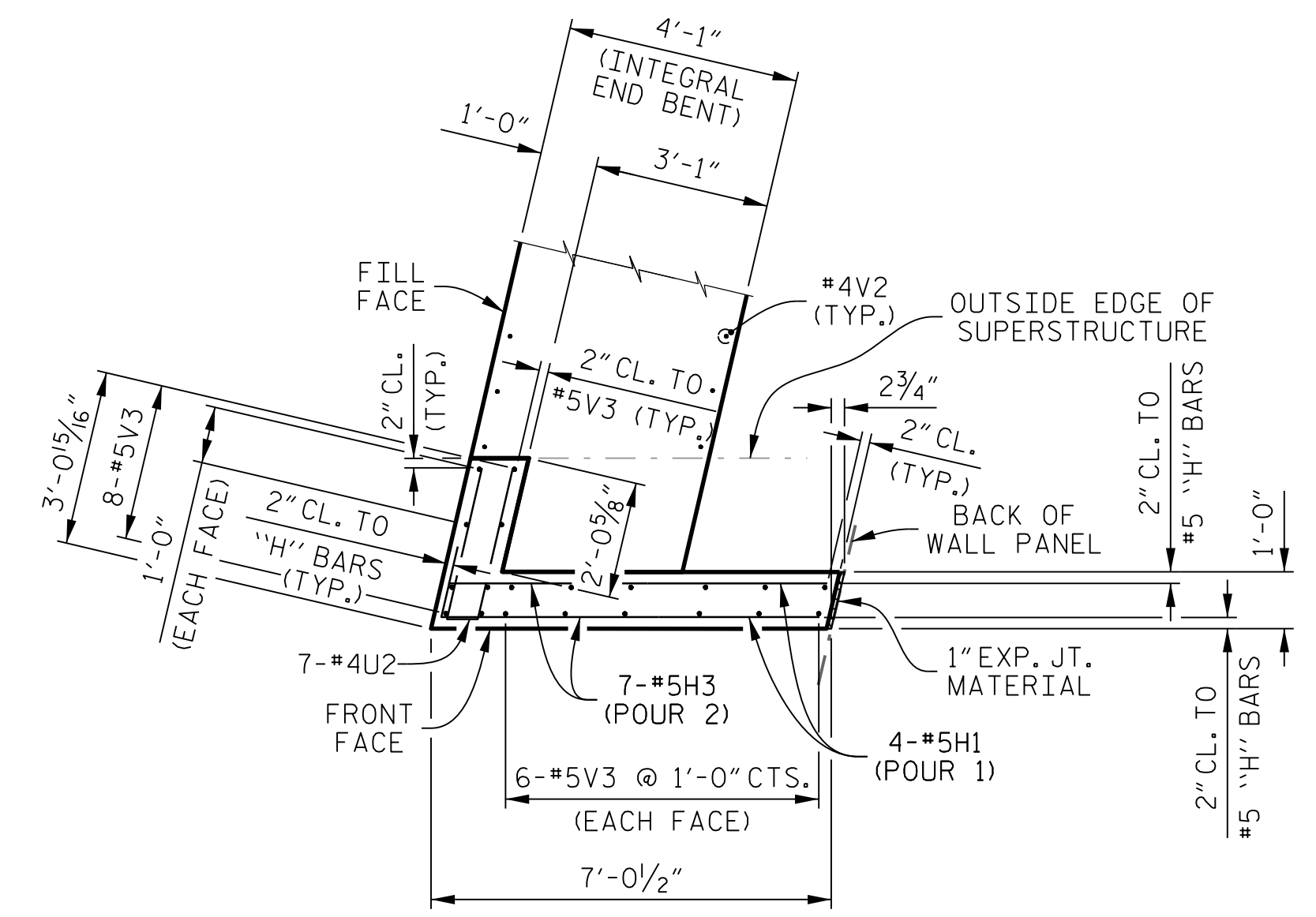
TOTAL SHEETS: **25**  
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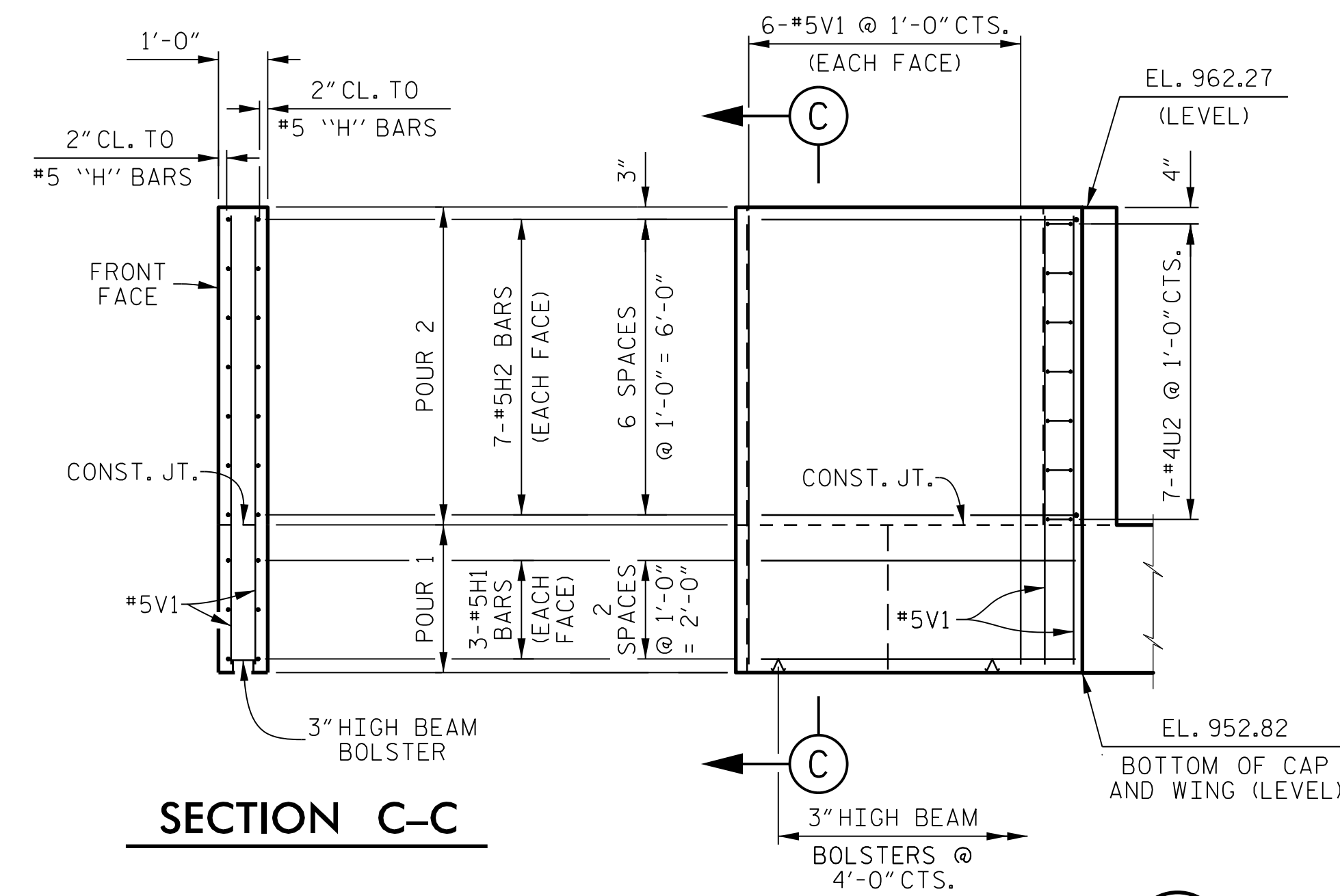




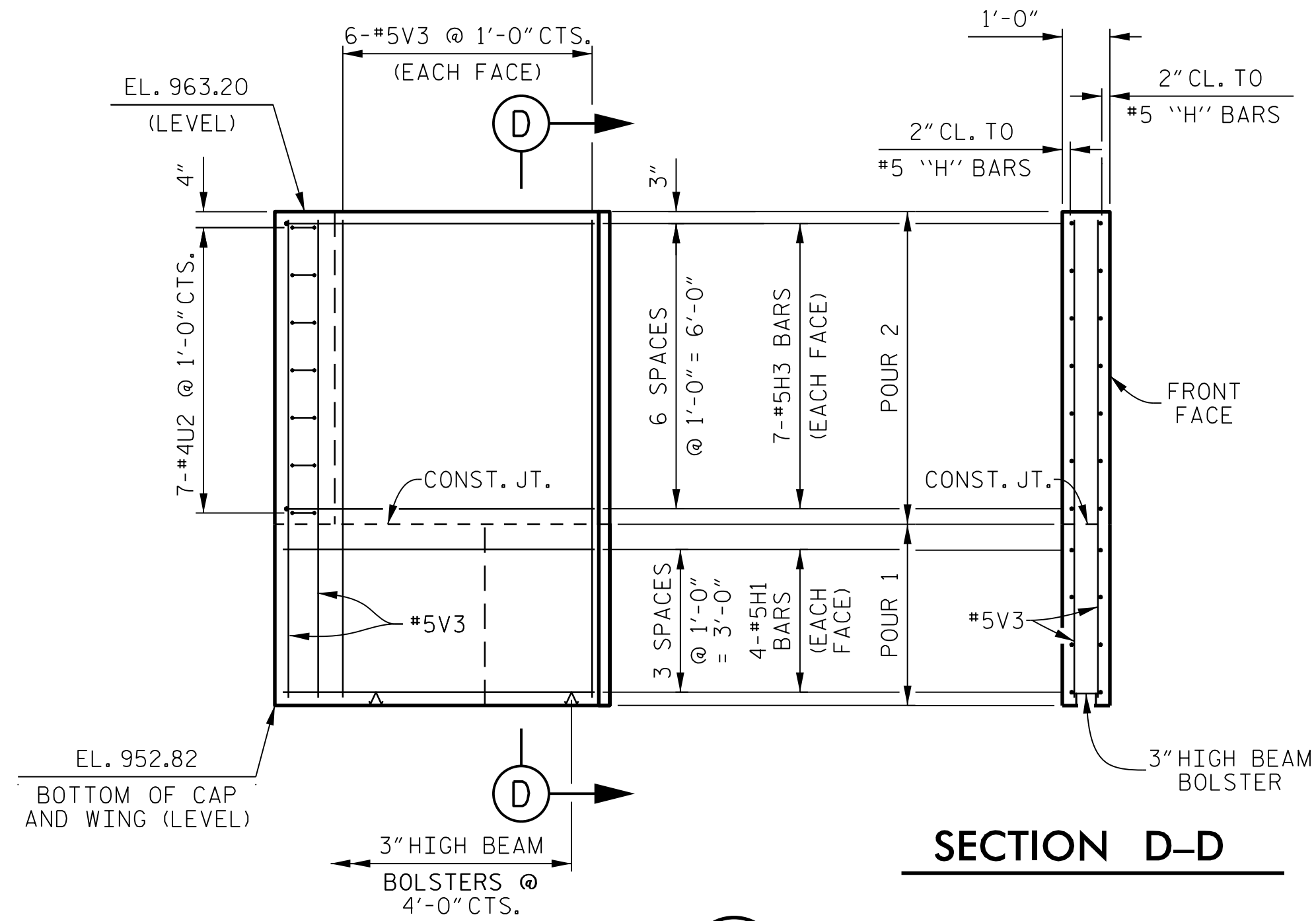
PLAN OF WING (W1)



PLAN OF WING (W2)



ELEVATION OF WING (W1)



ELEVATION OF WING (W2)

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

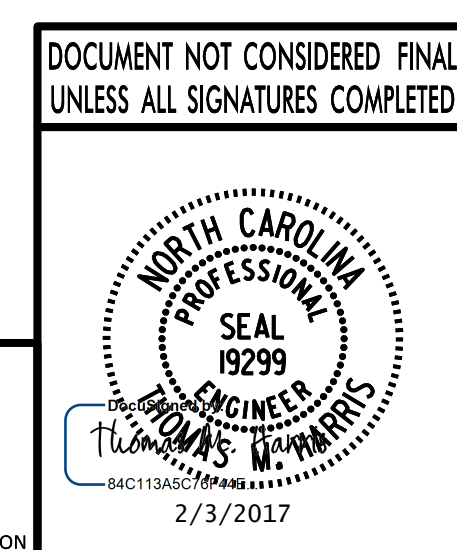
SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUBSTRUCTURE  
 END BENT 1  
 (LEFT LANE)**

REVISIONS				SHEET No.	
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

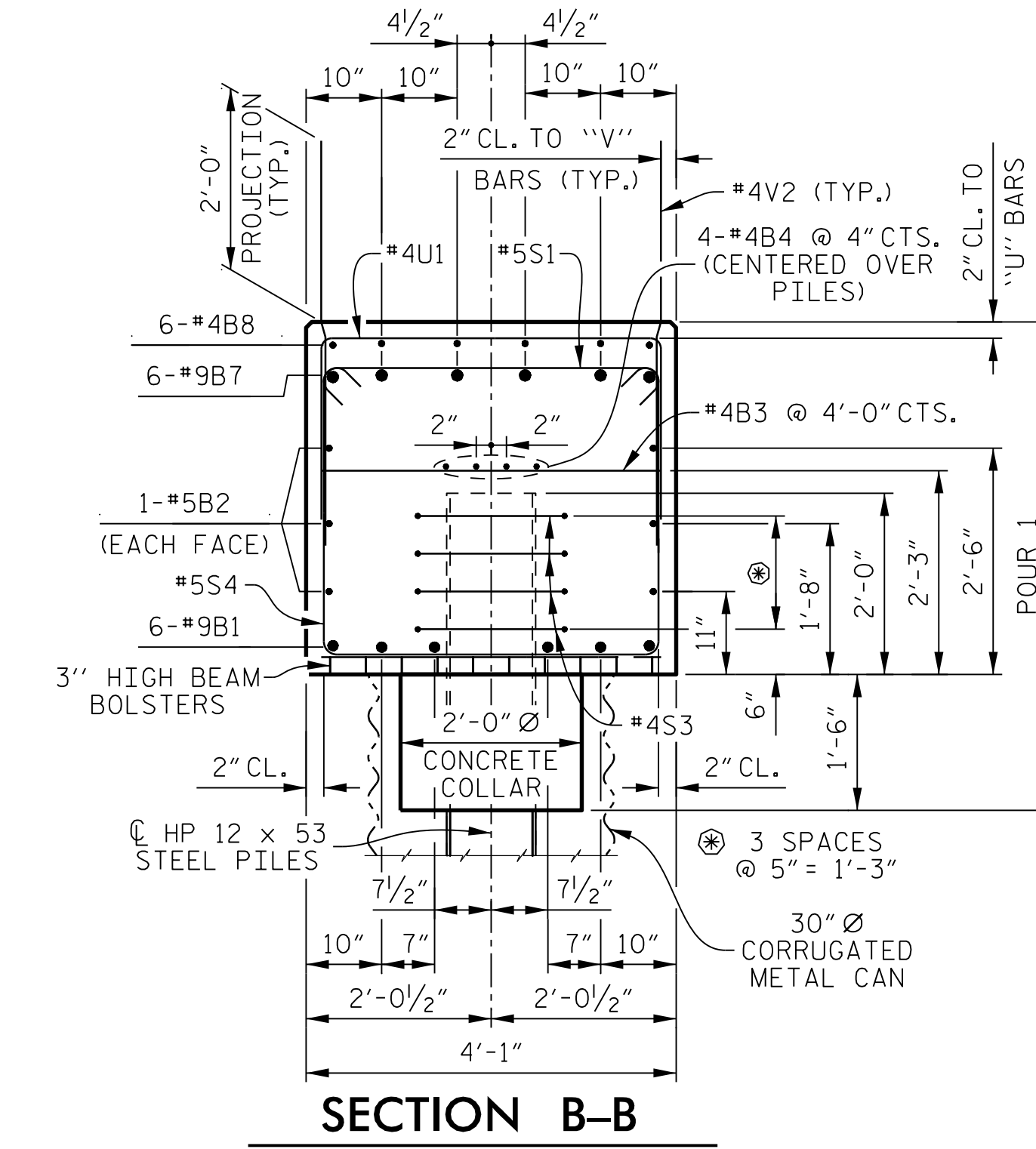
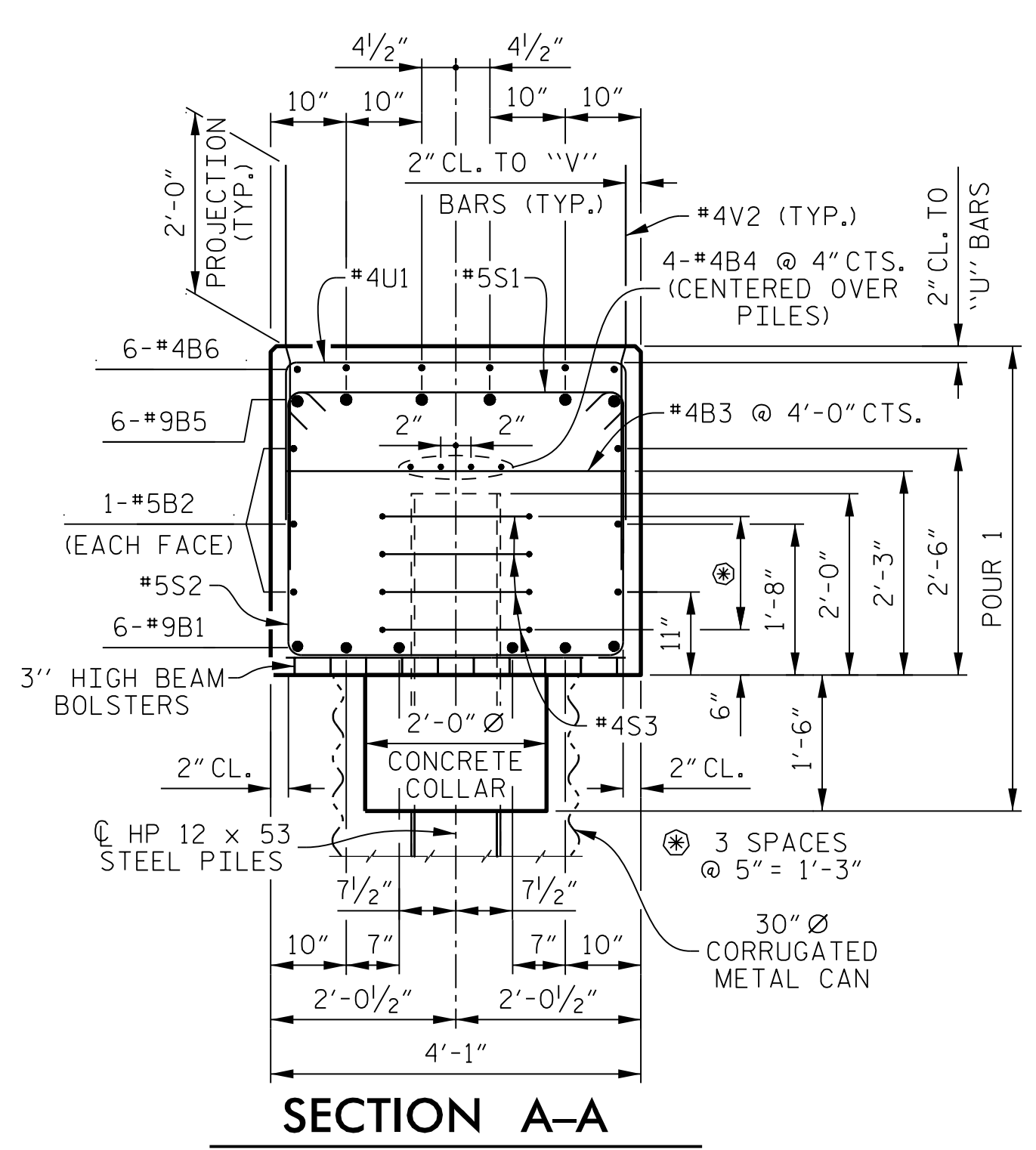
TOTAL SHEETS: 25



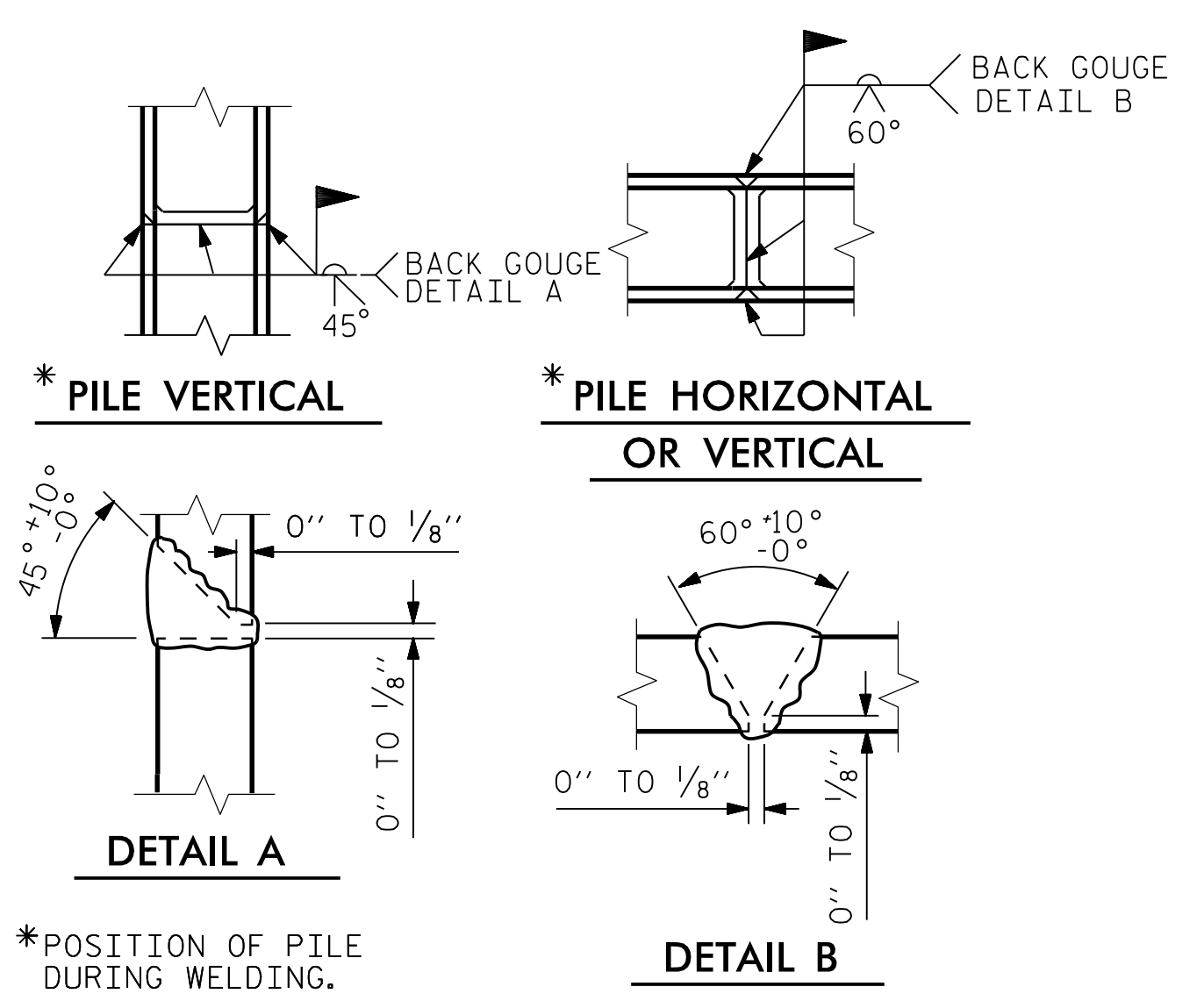
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON DATE: 6-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

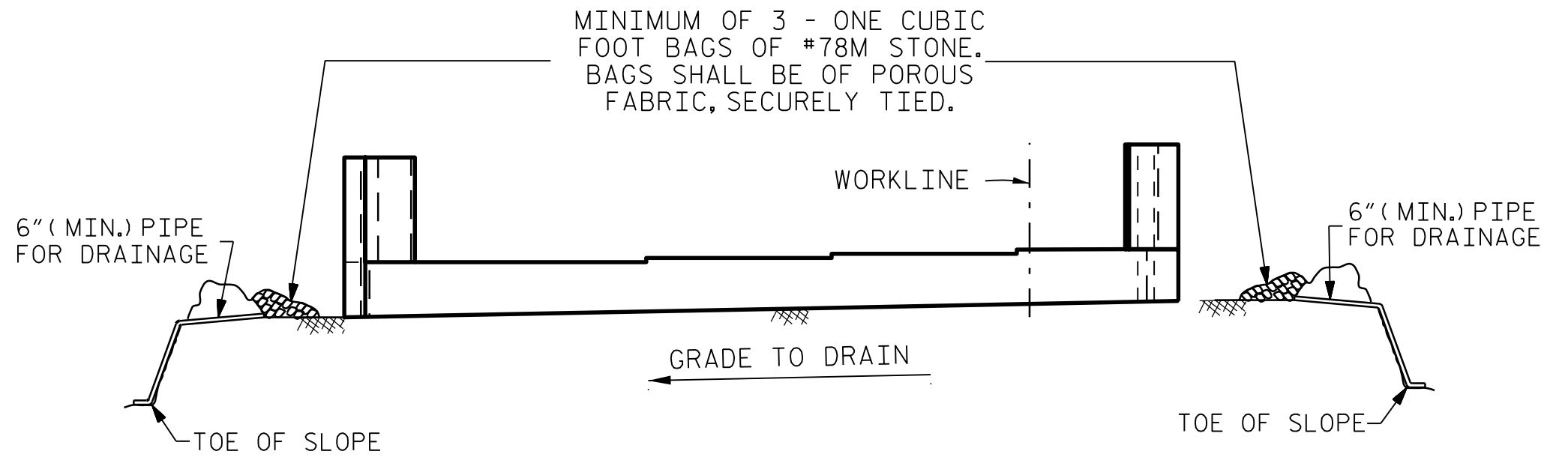
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BAR TYPES				BILL OF MATERIAL	
				END BENT 1	
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
B1	#9	2	52'-8"	1,074	
B2	#5	STR	50'-2"	314	
B3	#4	STR	3'-9"	38	
B4	#4	STR	26'-4"	141	
B5	#9	9	39'-11"	814	
B6	#4	STR	14'-6"	58	
B7	#9	9	21'-7"	440	
B8	#4	STR	8'-9"	35	
H1	#5	STR	6'-7"	96	
H2	#5	5	7'-5"	108	
H3	#5	7	7'-5"	108	
S1	#5	1	4'-8"	370	
S2	#5	4	9'-11"	465	
S3	#4	3	6'-6"	156	
S4	#5	4	11'-0"	356	
U1	#4	6	7'-9"	88	
U2	#4	8	5'-11"	55	
V1	#5	STR	8'-11"	186	
V2	#4	STR	4'-6"	180	
V3	#5	STR	9'-11"	207	
REINFORCING STEEL				5,289	LBS.
CLASS "A" CONCRETE					
POUR 1 COLLARS, CAP AND LOWER WINGS				27.8	CU. YDS.
POUR 2 BACKWALL AND UPPER WINGS				4.4	CU. YDS.
TOTAL				32.2	CU. YDS.
PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES				9	EACH
HP 12 x 53 STEEL PILES				9	REQUIRED
				700.0	LIN. FT.



PILE SPICE DETAILS



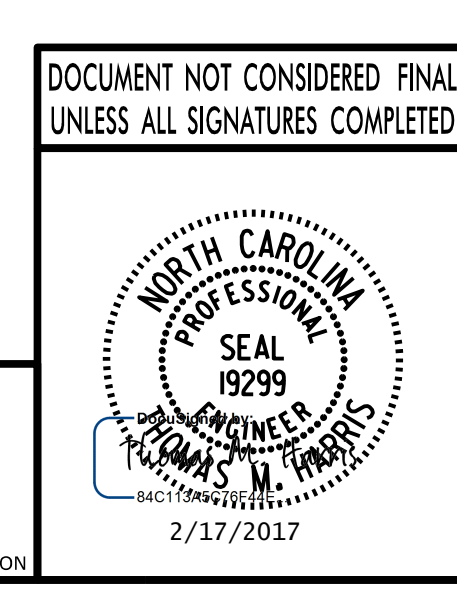
TEMPORARY DRAINAGE AT END BENT

MINIMUM OF 3 - ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.



PLANS PREPARED BY:  
**PARSONS**  
5540 CenterView Drive, Suite 217  
Raleigh, NC 27606-3386  
NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON DATE: 6-16  
CHECKED BY: A. D. SHAH DATE: 10-16  
DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

SUBSTRUCTURE  
END BENT 1  
(LEFT LANE)

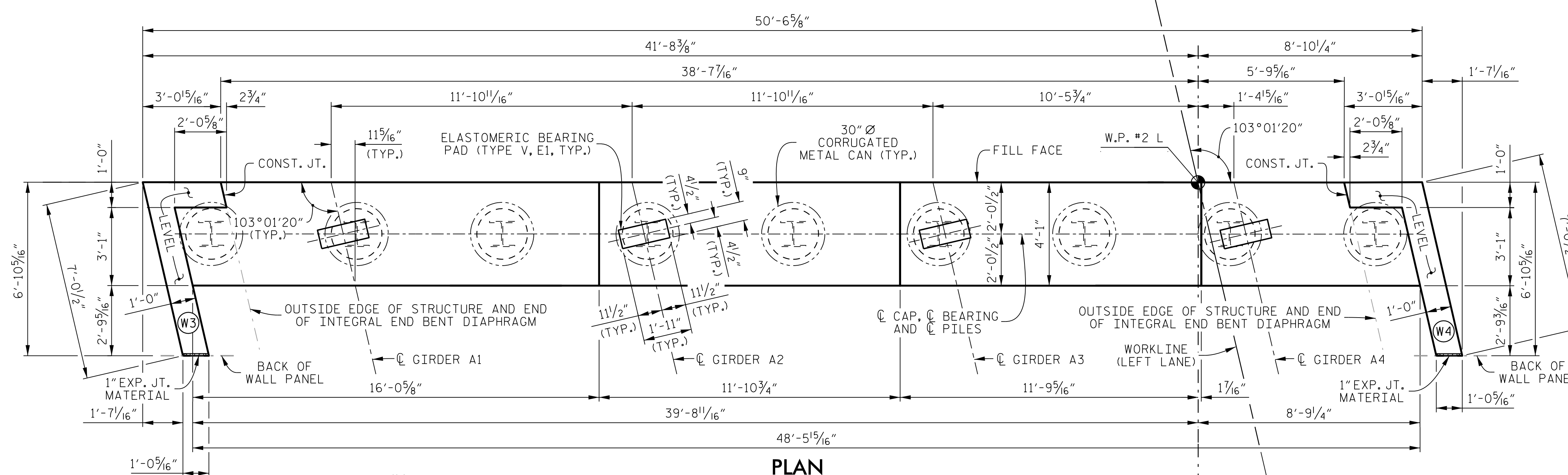
REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	S9-19
1			3			TOTAL SHEETS
2			4			25

STR. #9

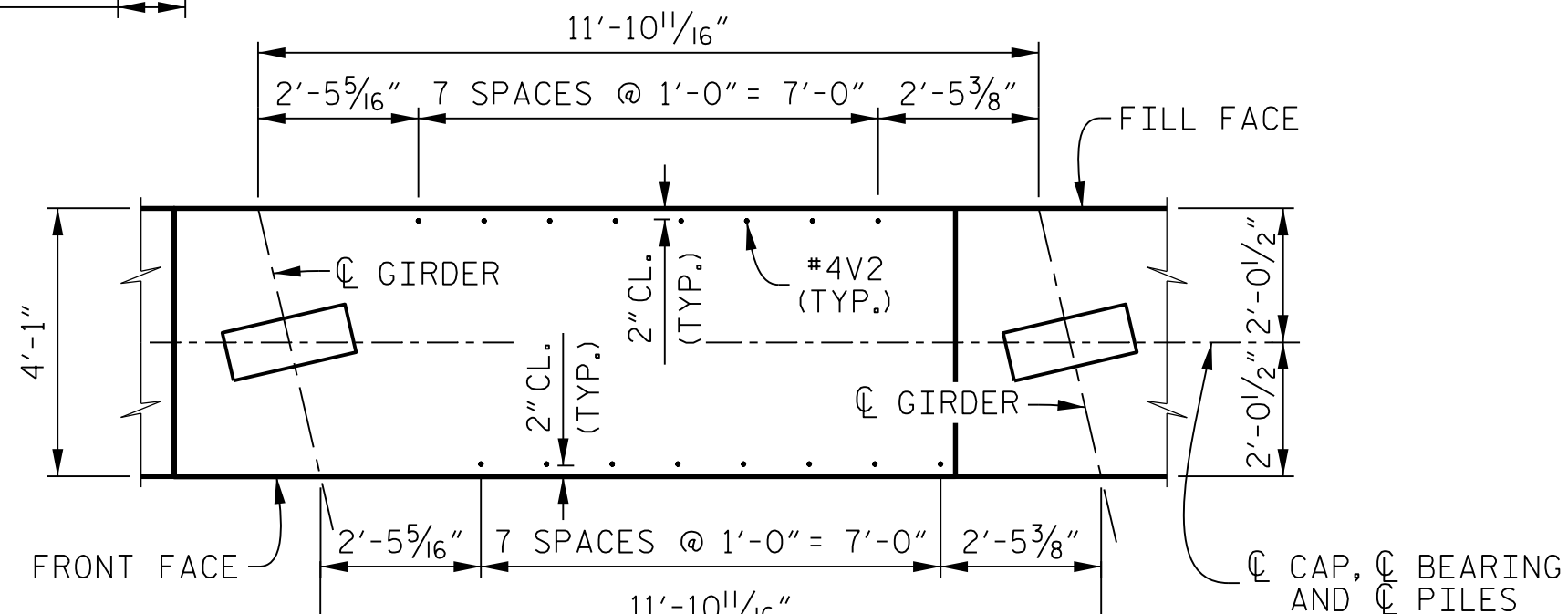


**NOTES**

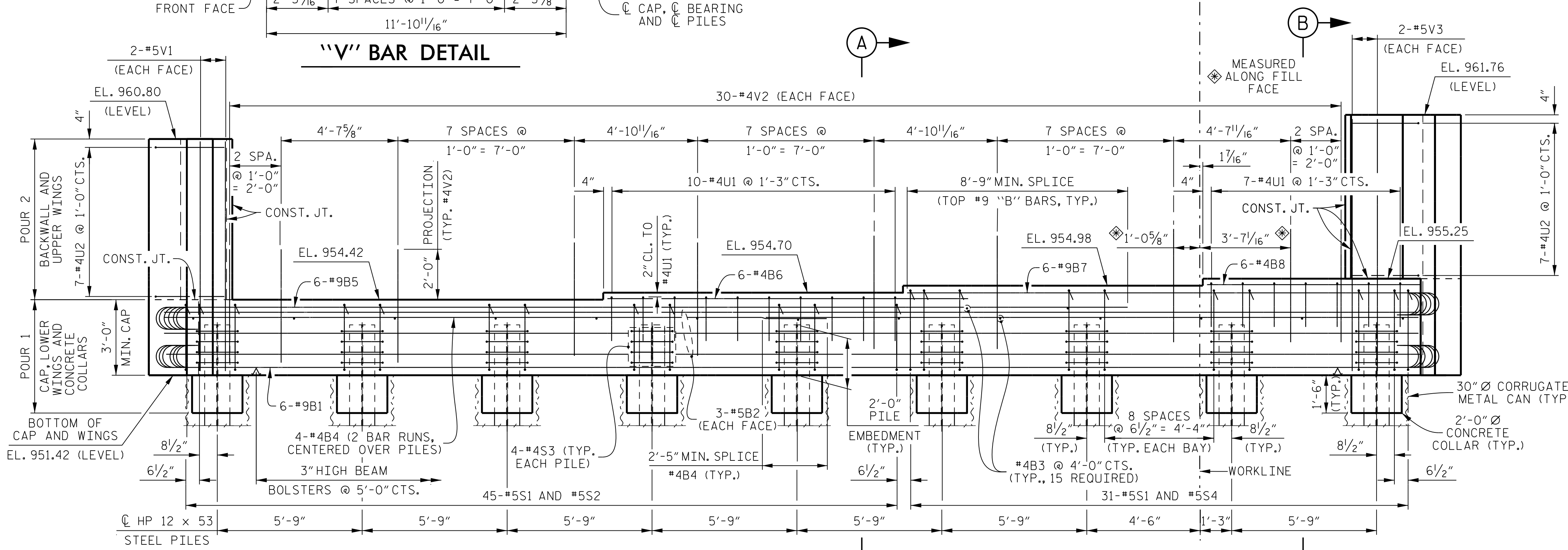
- STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4V2 BARS.
- THE TOP SURFACE OF THE END BENT CAP UNDER THE INTEGRAL END BENT DIAPHRAGM, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
- FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.
- FOR TEMPORARY DRAINAGE AT END BENT DETAIL, SEE SHEET 3 OF 3.
- FOR SECTION A-A AND SECTION B-B, SEE SHEET 3 OF 3.
- FOR INTEGRAL END BENT DIAPHRAGM DETAILS, SEE "TYPICAL SECTION" AND "PLAN OF SPAN" SHEETS.
- THE COST TO FURNISH AND INSTALL THE 30" Ø CORRUGATED METAL CANS SHALL BE INCLUDED IN THE CONTRACT PRICE FOR MSE RETAINING WALL.
- WING W3 AND WING W4 DETAILS ARE BASED ON A 5/2" WALL PANEL THICKNESS AND USING DOWELS FOR THE COPING. CONTRACTOR MAY ADJUST WINGS SLIGHTLY AS NECESSARY, BASED ON APPROVED MSE WALL SHOP DRAWINGS.



**PLAN**



**"V" BAR DETAIL**



**ELEVATION**

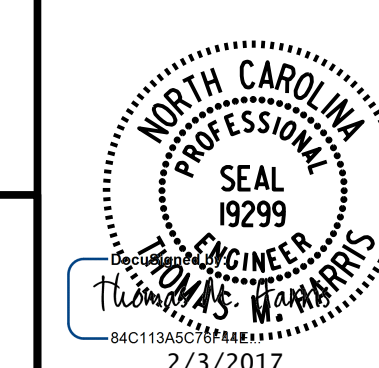
PROJECT NO. R-2707C  
**CLEVELAND COUNTY**  
 STATION: 611+32.01 -L-

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUBSTRUCTURE**  
**END BENT 2**  
**(LEFT LANE)**

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED

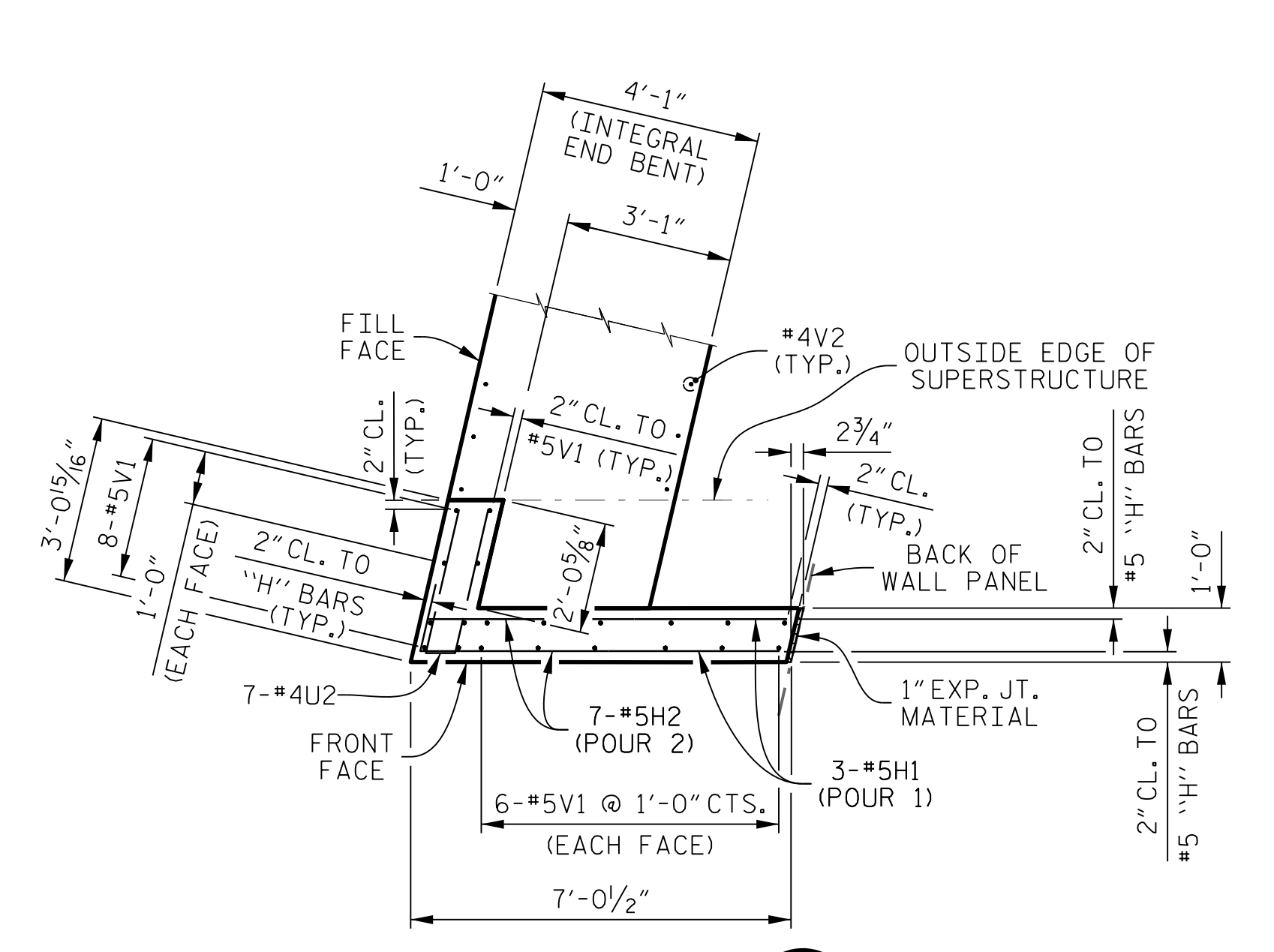


PLANS PREPARED BY:  
**PARSONS**  
 5540 Centerview Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

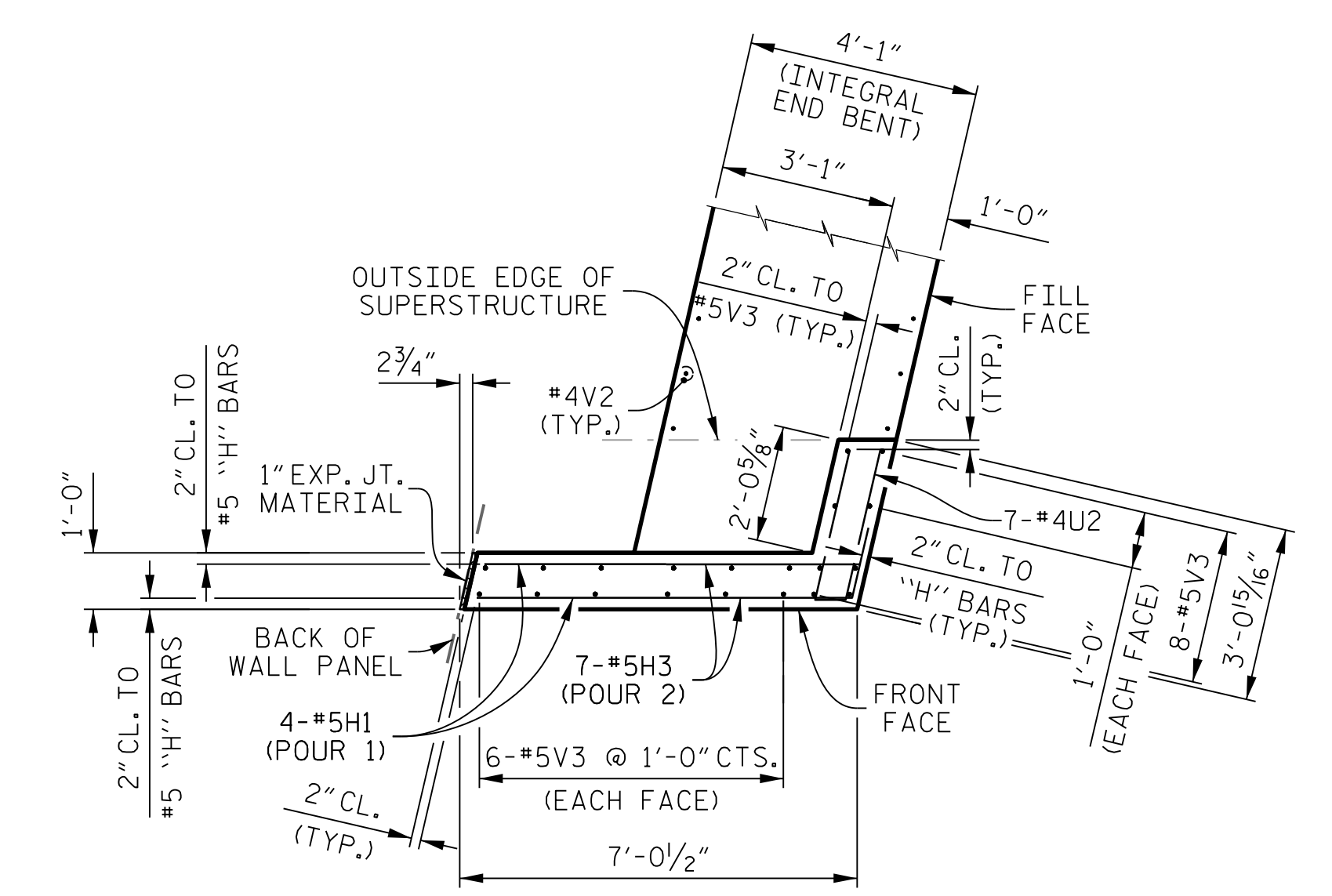
DRAWN BY: K. E. LOFTON DATE: 6-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

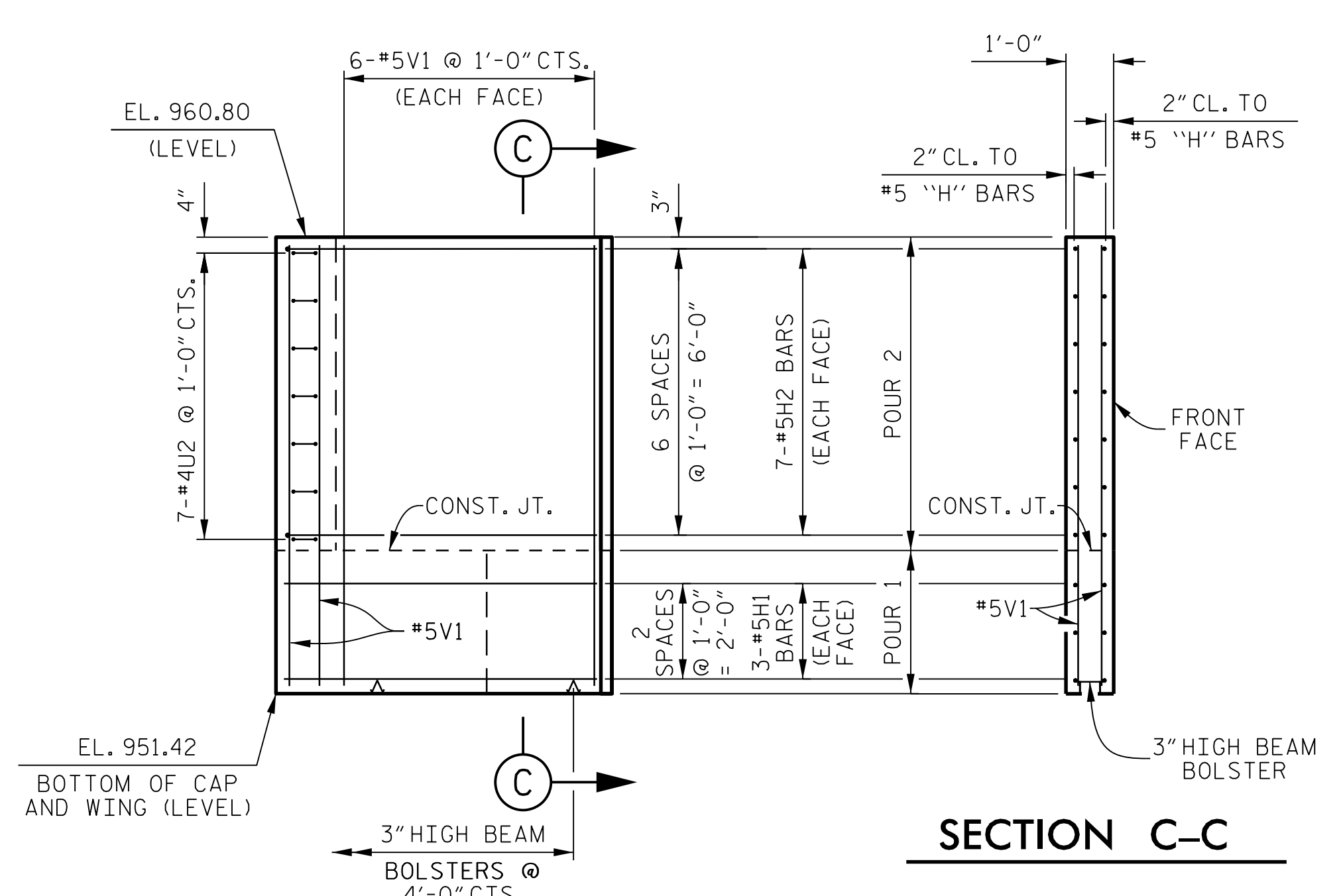
TOTAL SHEETS: **25**



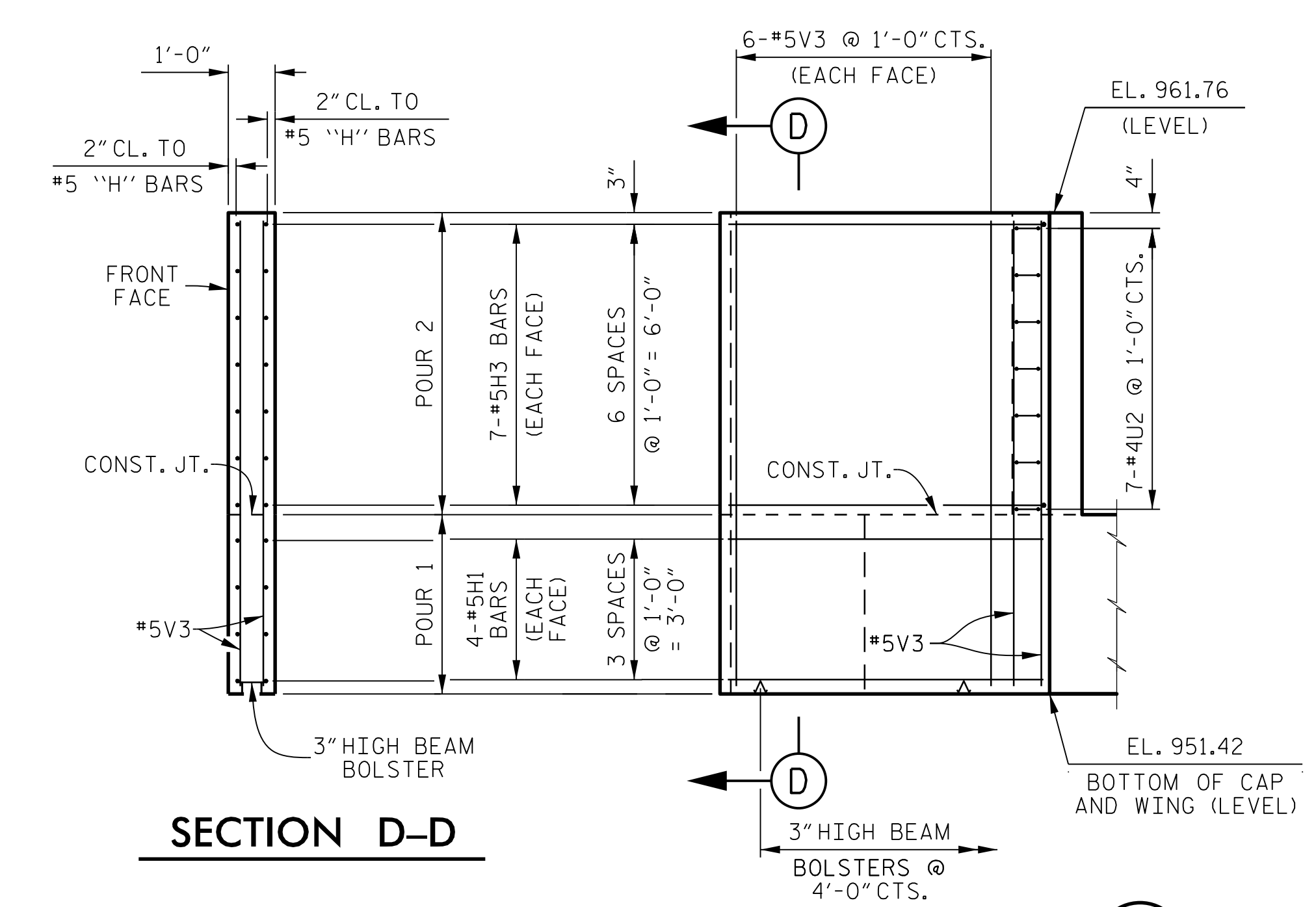
PLAN OF WING W3



PLAN OF WING W4



ELEVATION OF WING W3



ELEVATION OF WING W4

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUBSTRUCTURE  
 END BENT 2  
 (LEFT LANE)**

REVISIONS						SHEET No. S9-21
No.	BY:	DATE:	No.	BY:	DATE:	
1			3			TOTAL SHEETS 25
2			4			

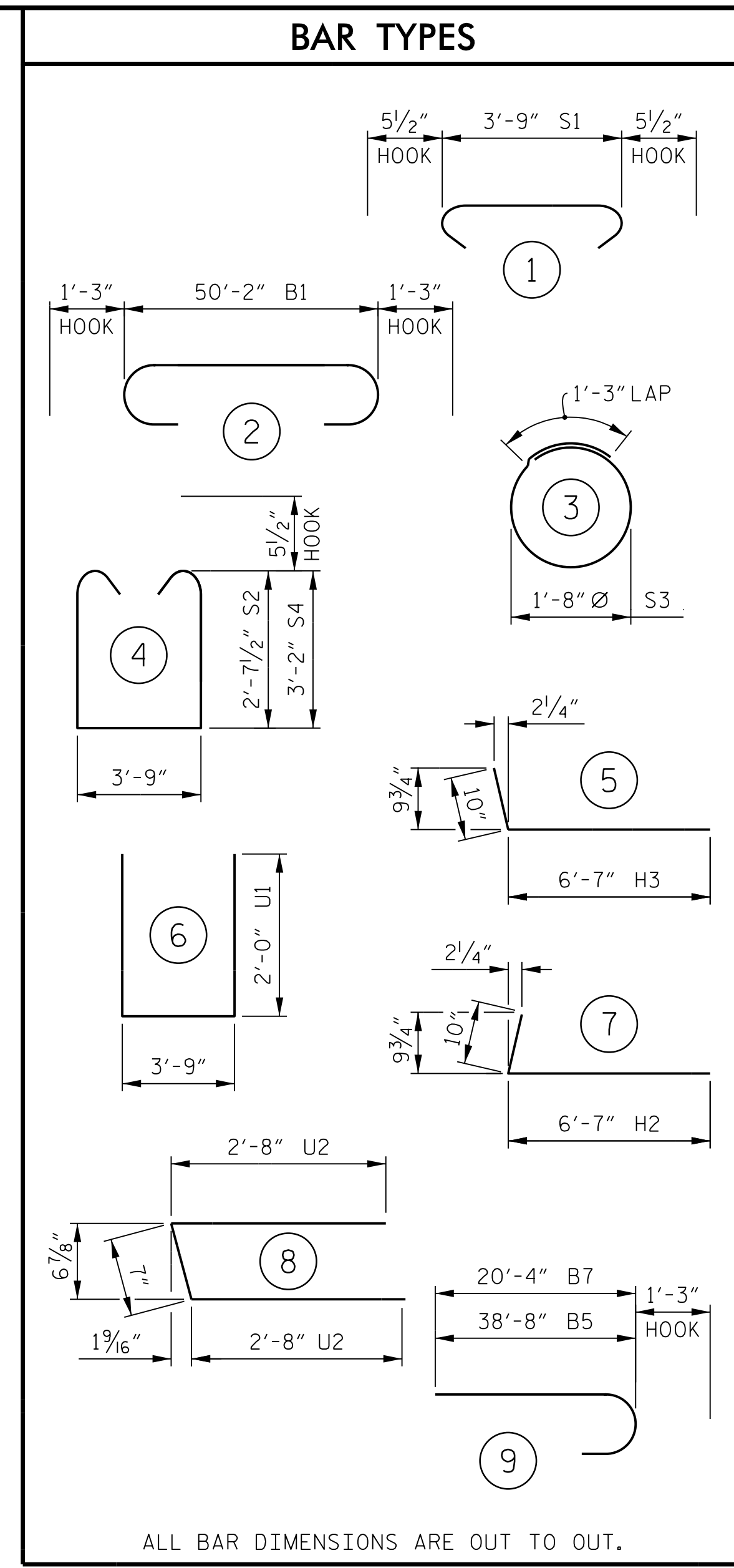
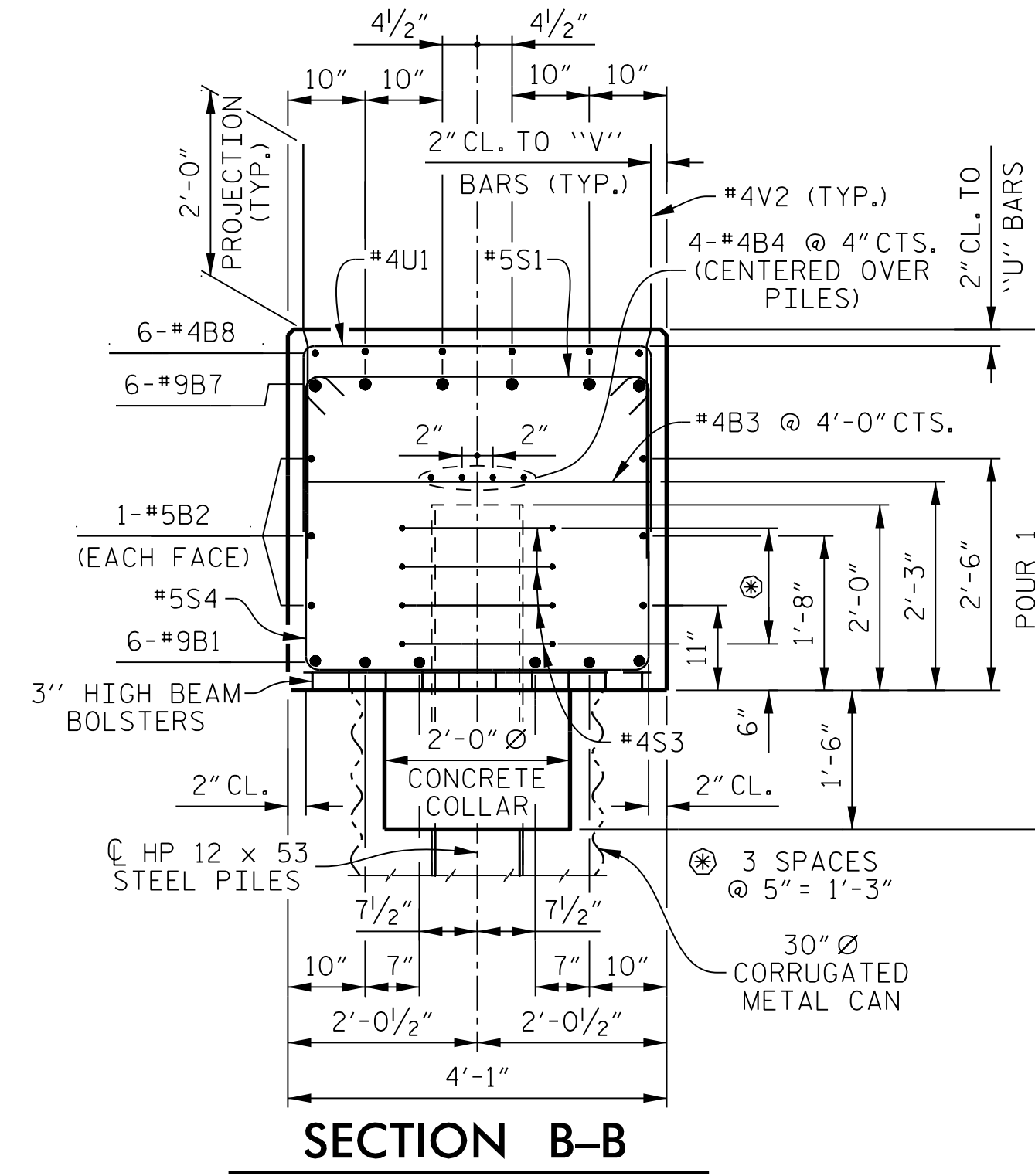
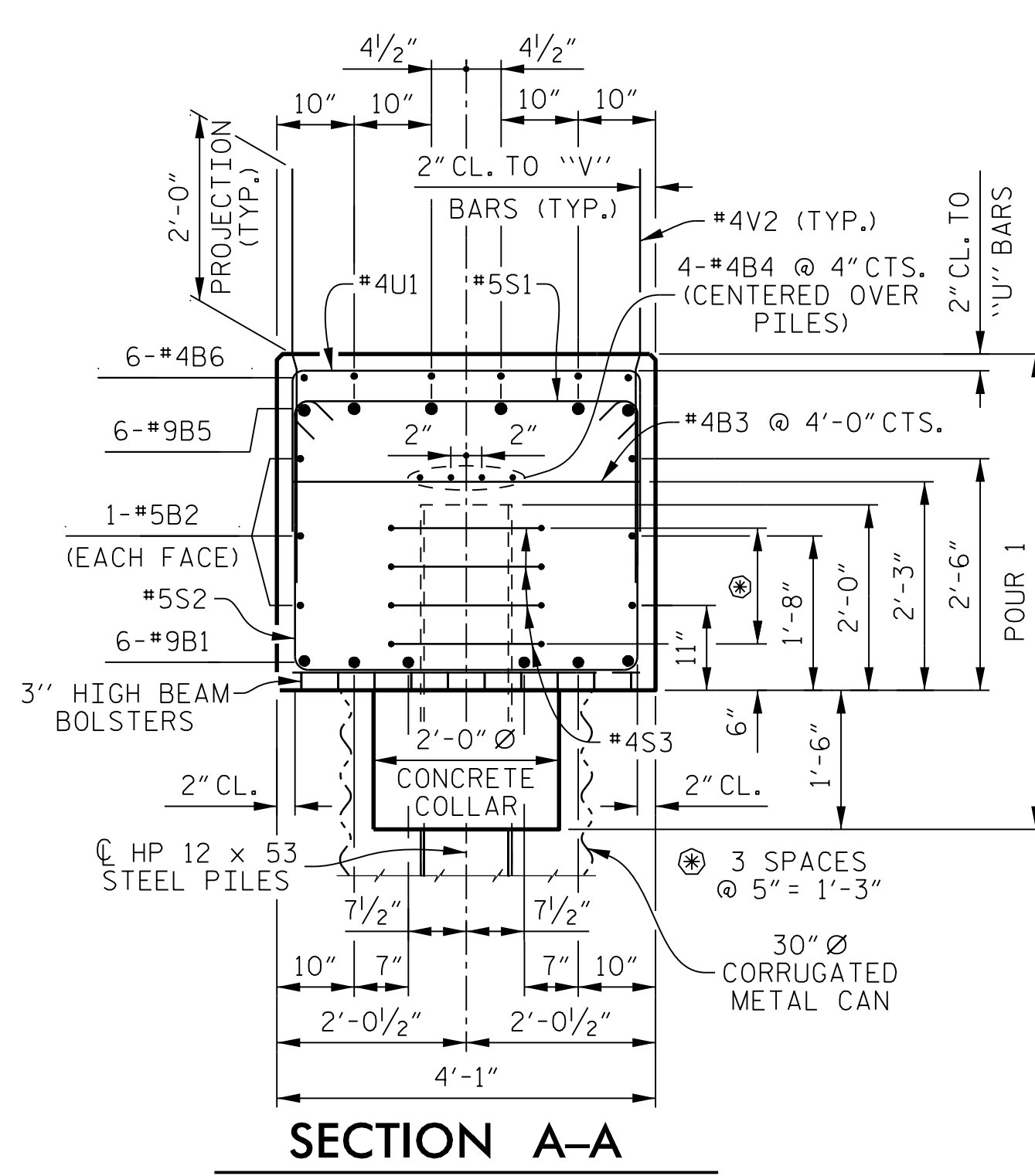
DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



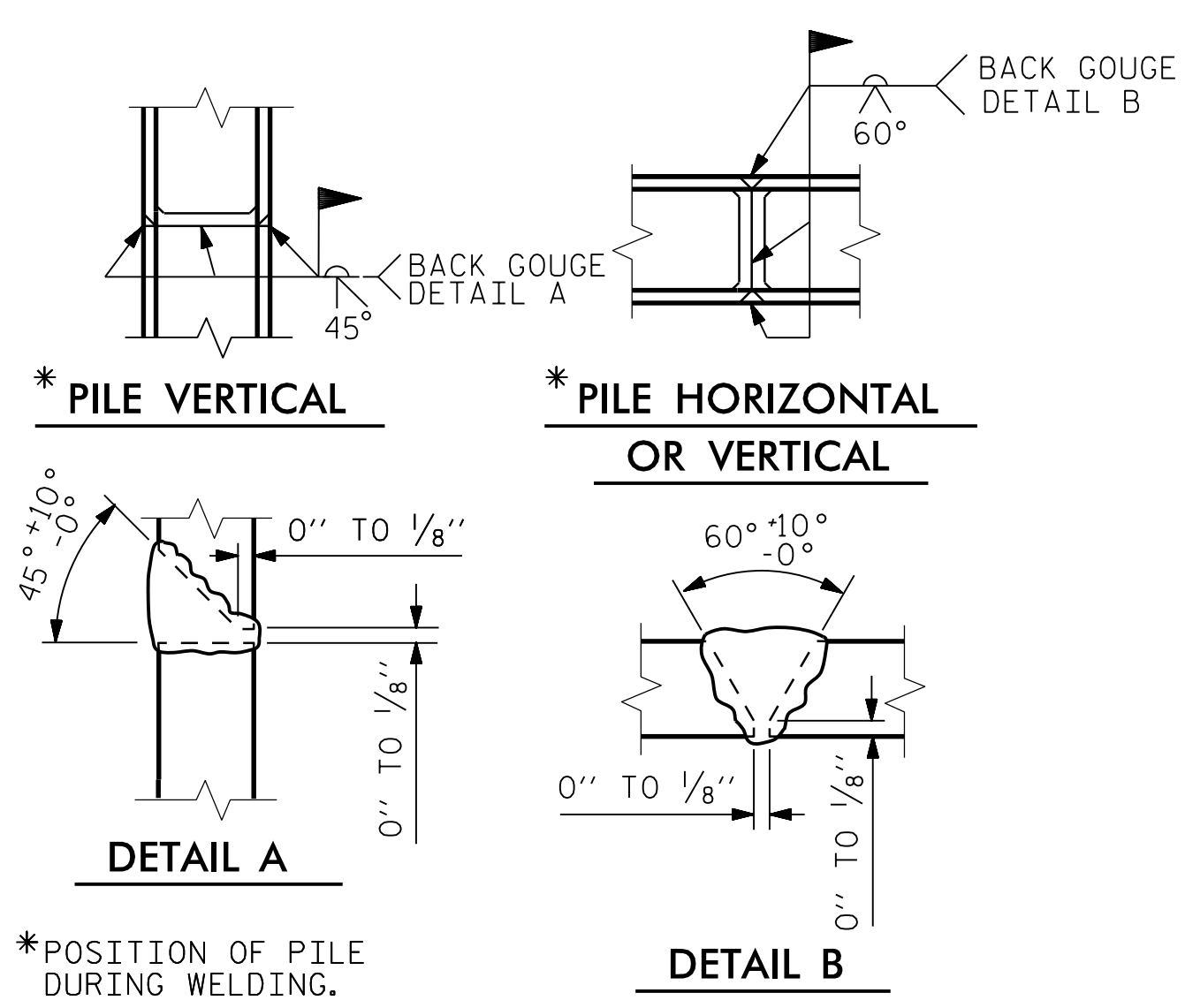
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY: K. E. LOFTON DATE: 6-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

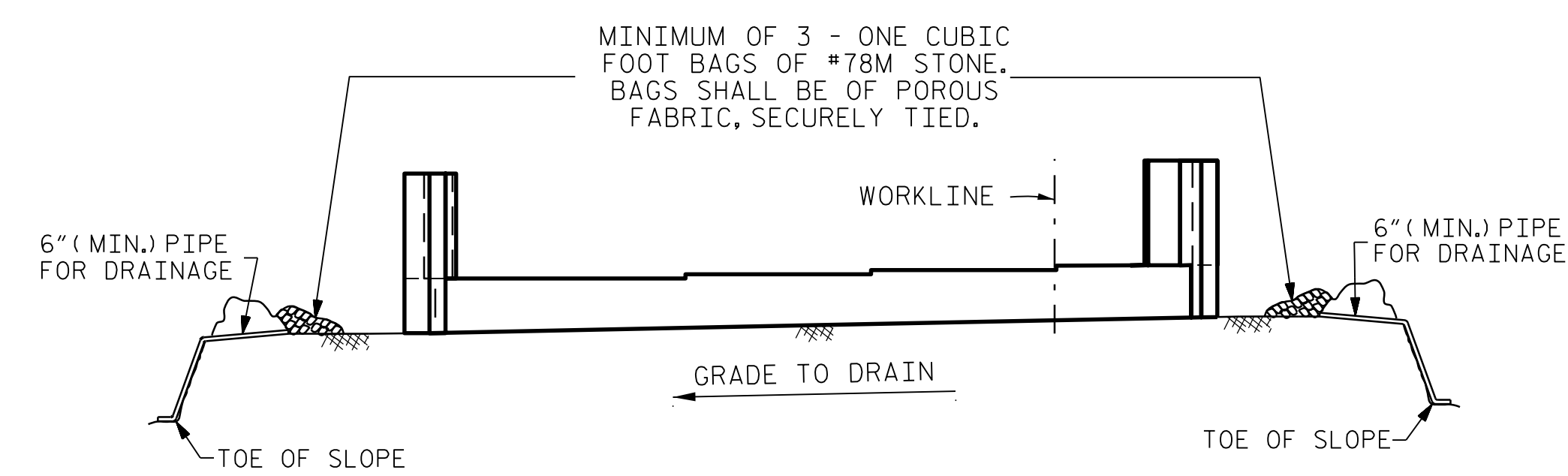




BILL OF MATERIAL					
END BENT 2					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
B1	6	#9	2	52'-8"	1,074
B2	6	#5	STR	50'-2"	314
B3	15	#4	STR	3'-9"	38
B4	8	#4	STR	26'-4"	141
B5	6	#9	9	39'-11"	814
B6	6	#4	STR	14'-6"	58
B7	6	#9	9	21'-7"	440
B8	6	#4	STR	8'-9"	35
H1	14	#5	STR	6'-7"	96
H2	14	#5	7	7'-5"	108
H3	14	#5	5	7'-5"	108
S1	76	#5	1	4'-8"	370
S2	45	#5	4	9'-11"	465
S3	36	#4	3	6'-6"	156
S4	31	#5	4	11'-0"	356
U1	17	#4	6	7'-9"	88
U2	14	#4	8	5'-11"	55
V1	20	#5	STR	8'-11"	186
V2	60	#4	STR	4'-6"	180
V3	20	#5	STR	9'-11"	207
REINFORCING STEEL				5,289	LBS.
CLASS "A" CONCRETE					
POUR 1 COLLARS, CAP AND LOWER WINGS				27.8	CU. YDS.
POUR 2 BACKWALL AND UPPER WINGS				4.4	CU. YDS.
TOTAL				32.2	CU. YDS.
PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES				9	EACH
HP 12 x 53 STEEL PILES				9	REQUIRED
				675.0	LIN. FT.



PILE SPlice DETAILS



MINIMUM OF 3 - ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

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NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

ALL BAR DIMENSIONS ARE OUT TO OUT.

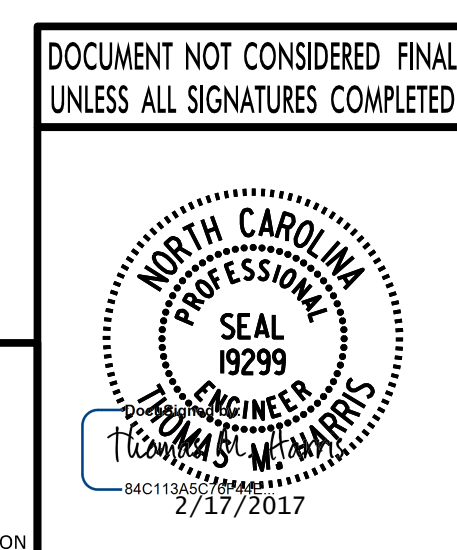
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

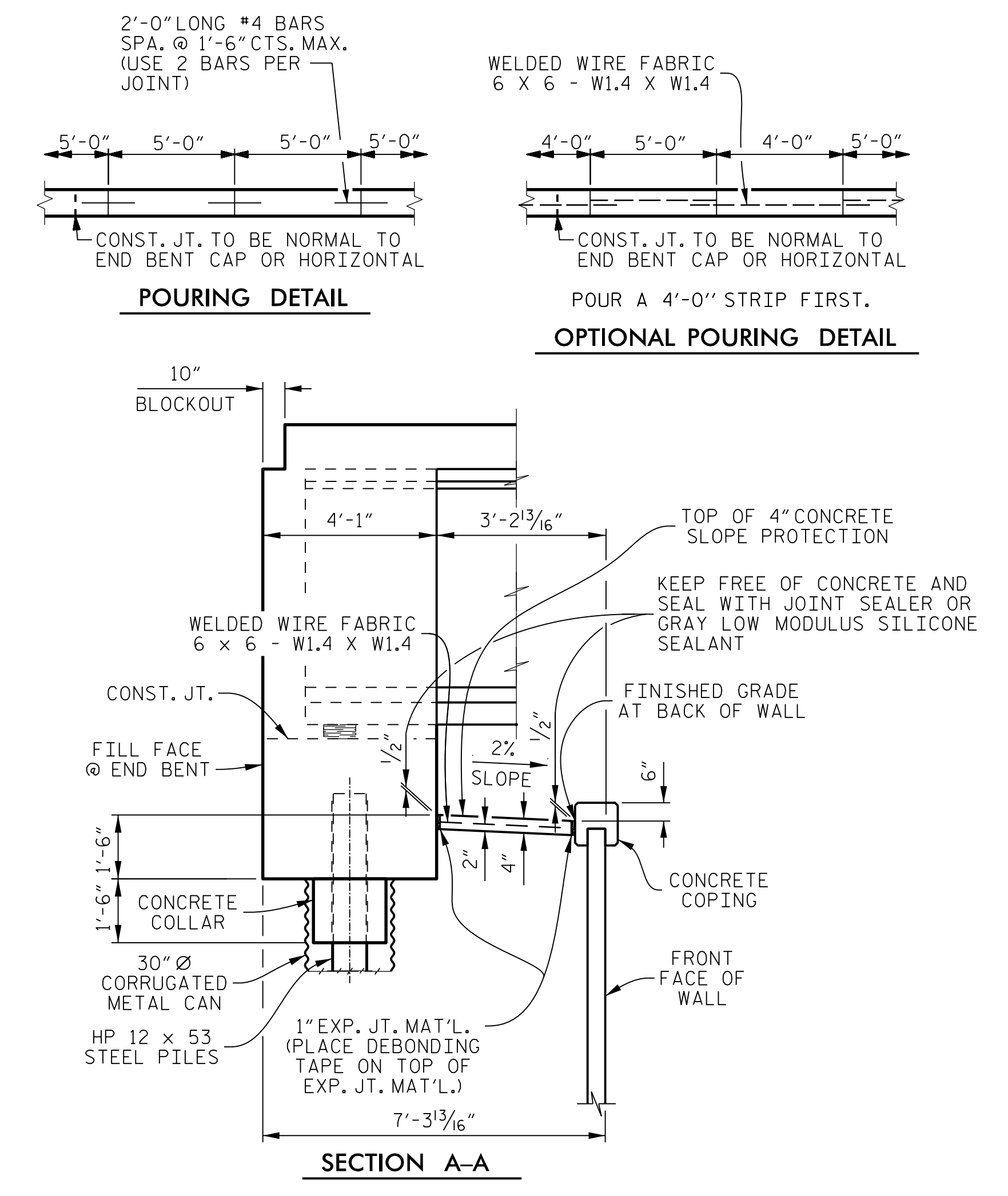
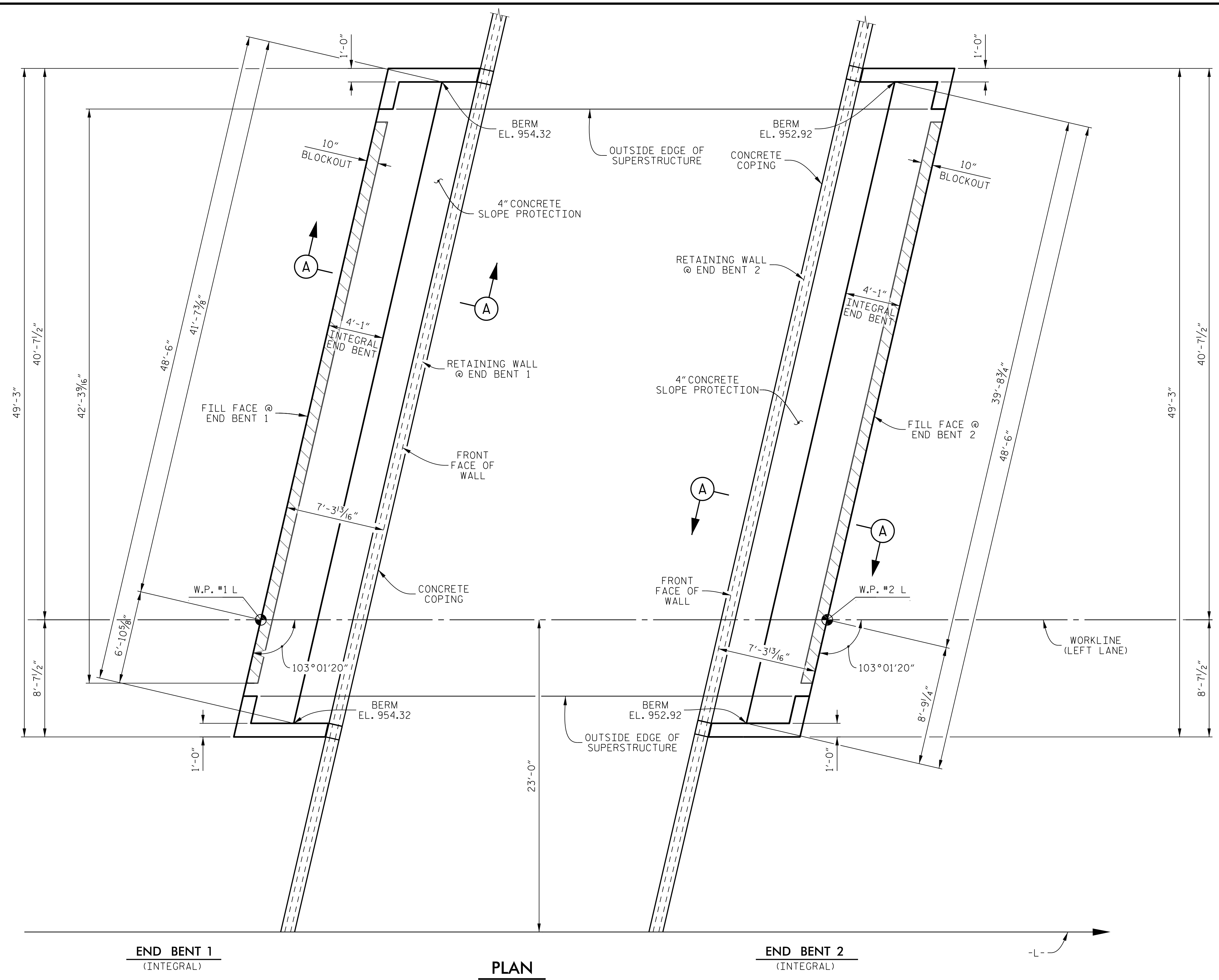
SUBSTRUCTURE  
 END BENT 2  
 (LEFT LANE)

REVISIONS					
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		



PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON DATE: 6-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16



BRIDGE AT STA. 611+32.01 -L- (LEFT LANE)	4" SLOPE PROTECTION	WELDED WIRE FABRIC
	SQUARE YARDS	APPROX. LINEAR FEET
END BENT 1	14	25
END BENT 2	14	25

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

**GENERAL NOTES**

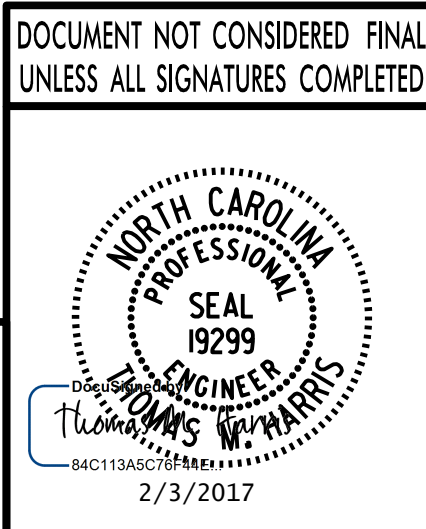
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.  
 STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.  
 MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B", THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 x 6 - W1.4 x W1.4. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

ASSEMBLED BY : K. E. LOFTON	DATE : 6-16
CHECKED BY : A. D. SHAH	DATE : 10-16
DRAWN BY : ELR	5/92
CHECKED BY : GRP	6/92
REV. 10/1/11	MAA/GM
REV. 12/21/11	MAA/GM
REV. 1/16	MAA/TMG

DRAWN BY : K. E. LOFTON	DATE : 6-16
CHECKED BY : A. D. SHAH	DATE : 10-16
DESIGN ENGINEER : T. M. HARRIS	DATE : 10-16

PLANS PREPARED BY :  
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 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD

**SLOPE PROTECTION DETAILS (LEFT LANE)**

REVISIONS					
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

SHEET No. **S9-23**  
 TOTAL SHEETS **25**



**BILL OF MATERIAL**

**APPROACH SLAB AT  
END BENT 1 OR END BENT 2**

BAR No.	SIZE	TYPE	LENGTH	WEIGHT
*A1	52	#4	STR 22'-1"	767
A2	52	#4	STR 21'-11"	761
*B1	83	#5	STR 24'-2"	2,092
B2	83	#6	STR 24'-8"	3,075
REINFORCING STEEL				3,836 LBS.
*EPOXY COATED REINFORCING STEEL				2,859 LBS.
CLASS "AA" CONCRETE (2 APPROACH SLABS REQUIRED)				44.5 CU. YDS.

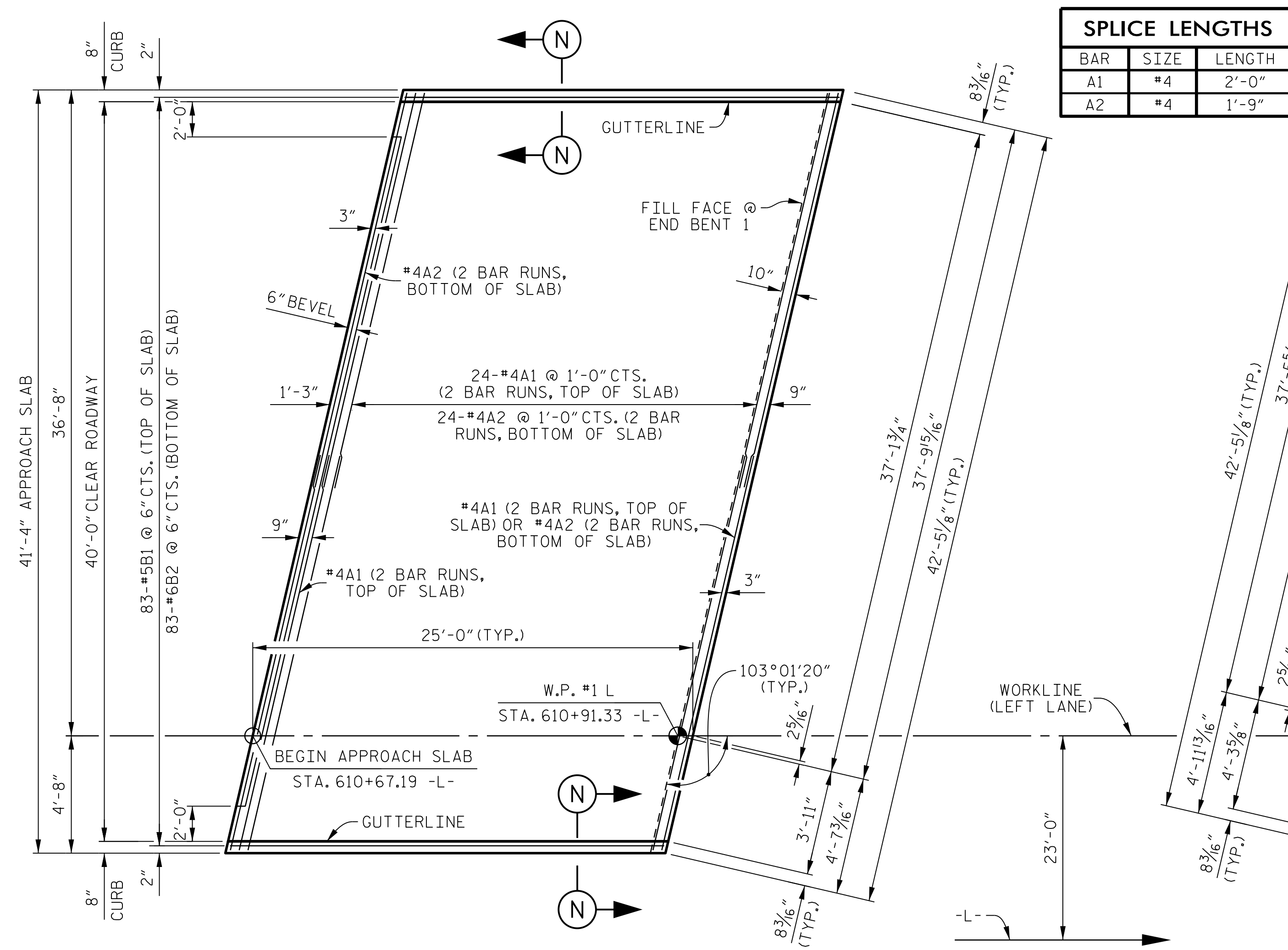
**NOTES**

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWS NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

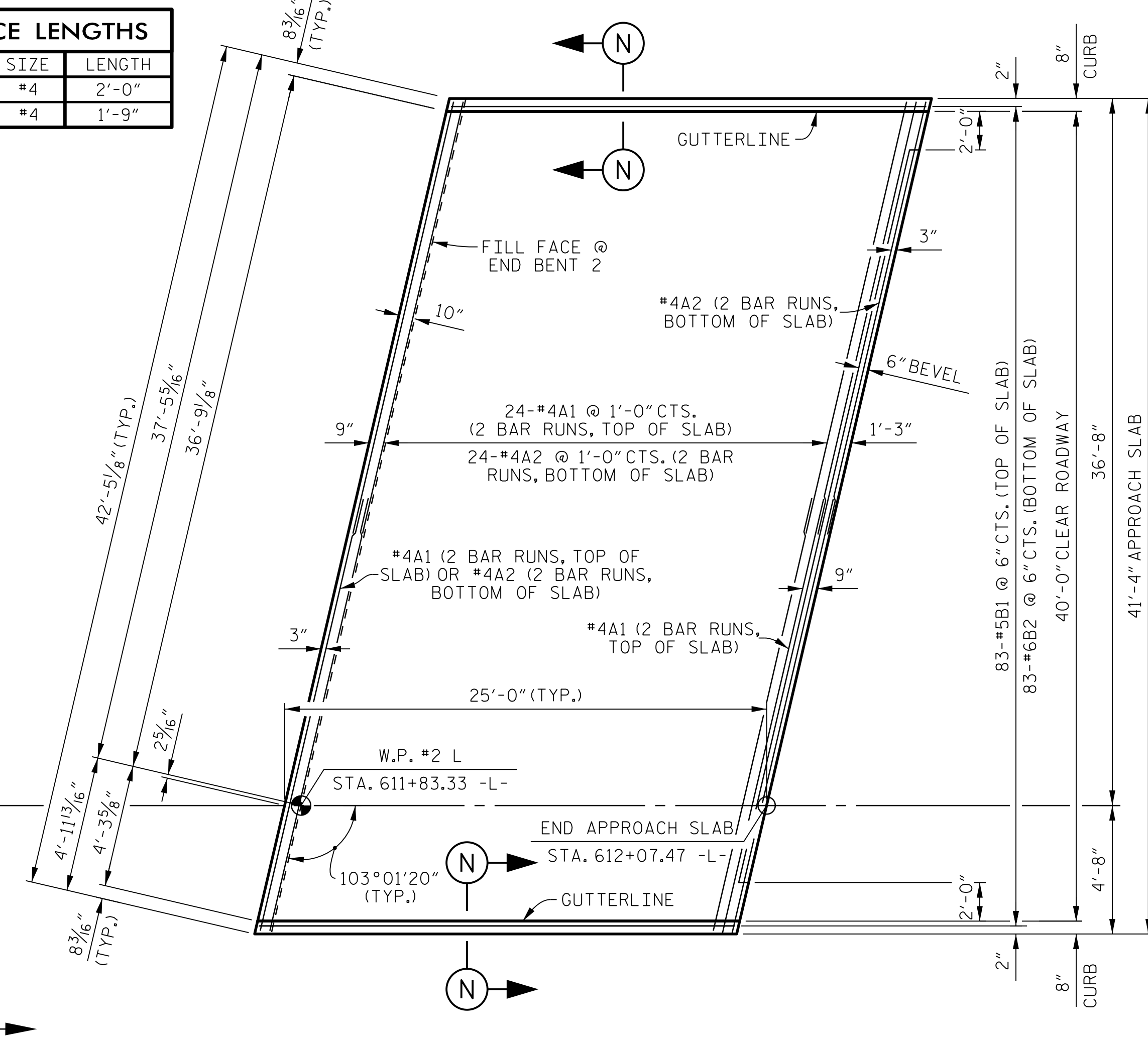
FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.



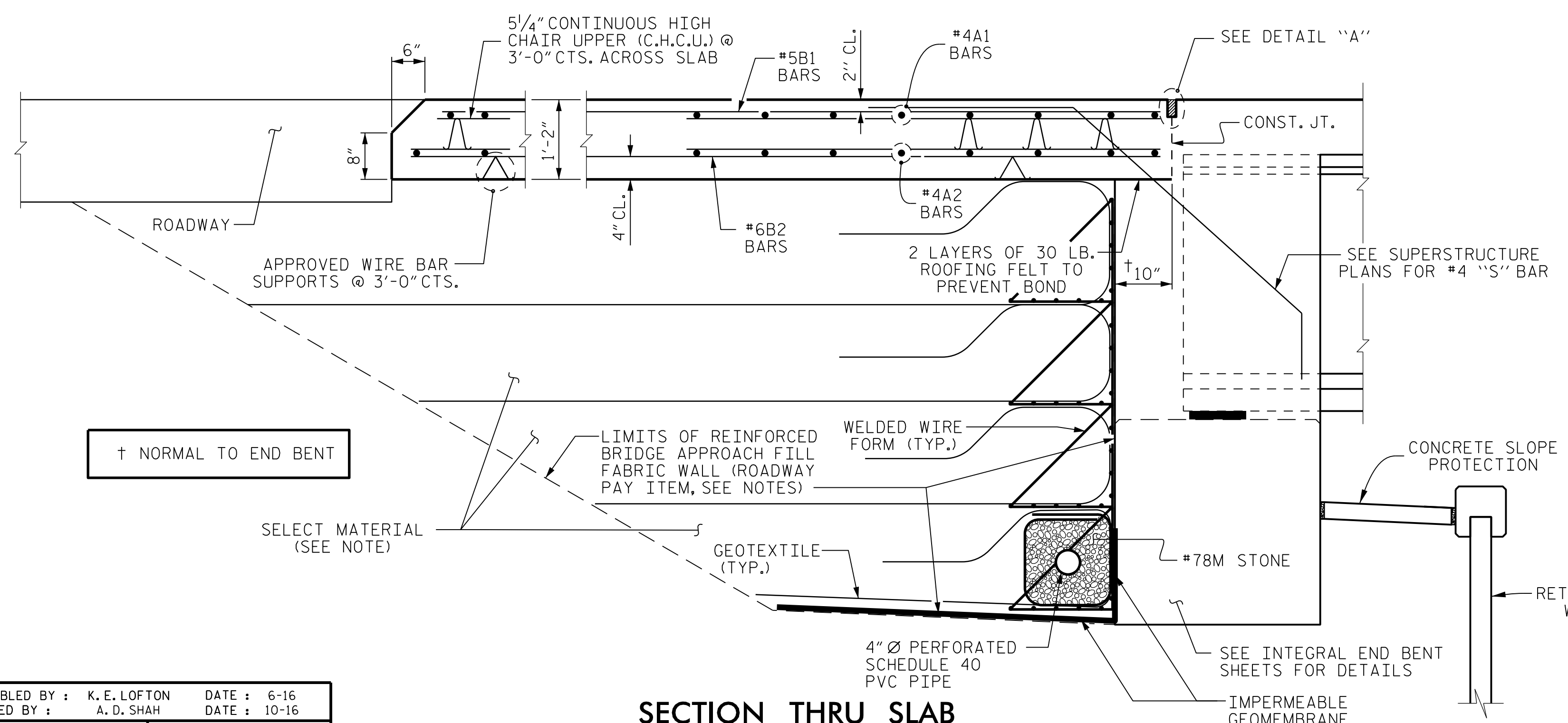
**PLAN OF APPROACH SLAB AT END BENT 1**

**SPLICE LENGTHS**

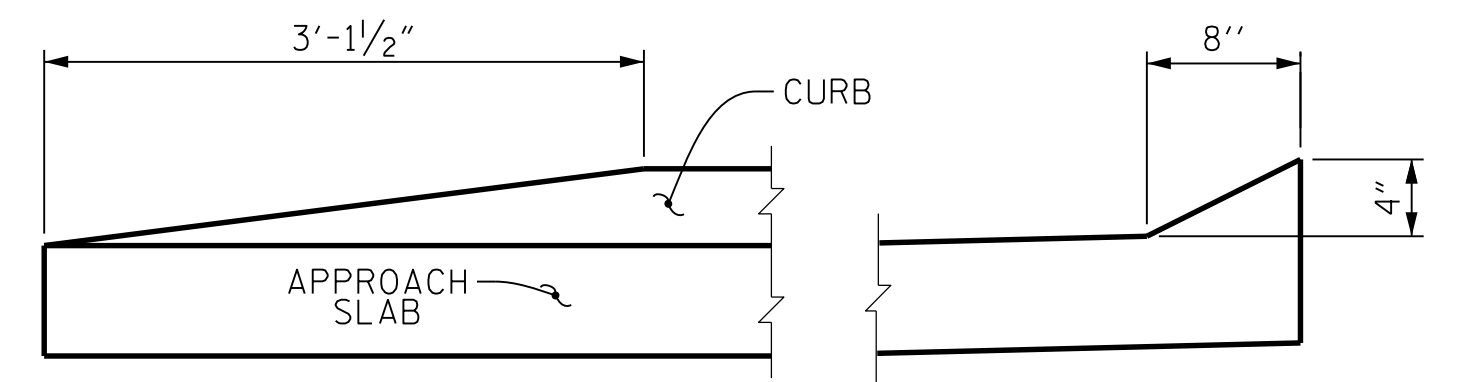
BAR	SIZE	LENGTH
A1	#4	2'-0"
A2	#4	1'-9"



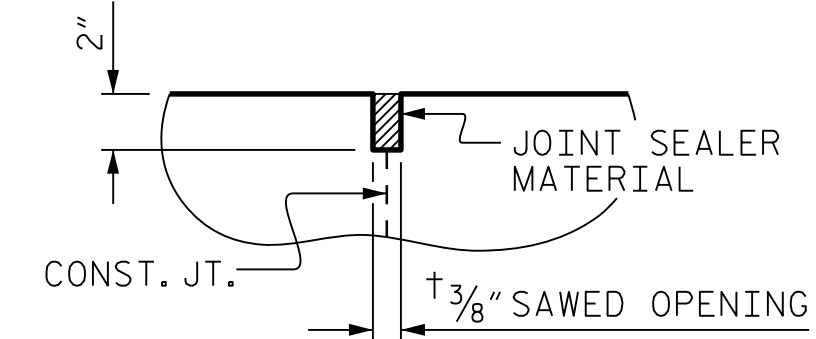
**PLAN OF APPROACH SLAB AT END BENT 2**



**SECTION THRU SLAB**



**CURB DETAILS**



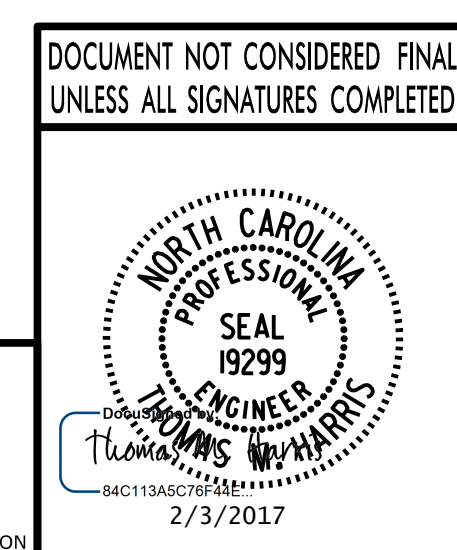
**DETAIL "A"**

PROJECT NO. **R-2707C**  
**CLEVELAND** COUNTY  
 STATION: **611+32.01 -L-**

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
**BRIDGE APPROACH SLAB  
 FOR INTEGRAL ABUTMENT  
 (LEFT LANE)**

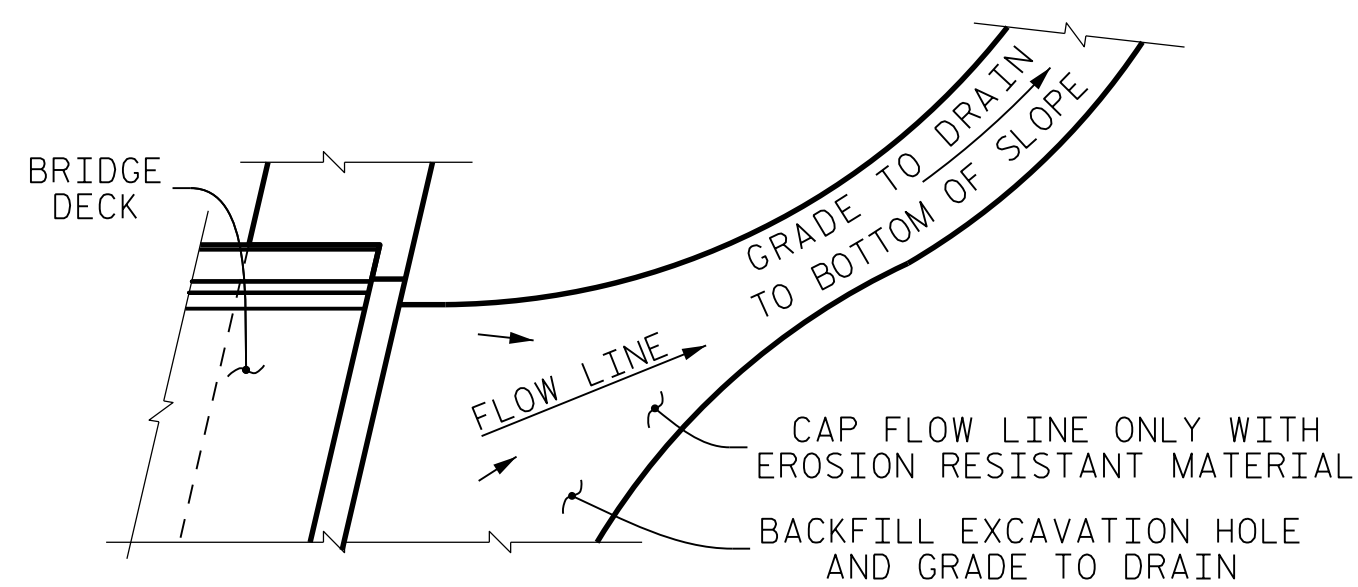
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No.	BY:	DATE:	No.	BY:	DATE:	S9-24
1			3			TOTAL SHEETS
2			4			25



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 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

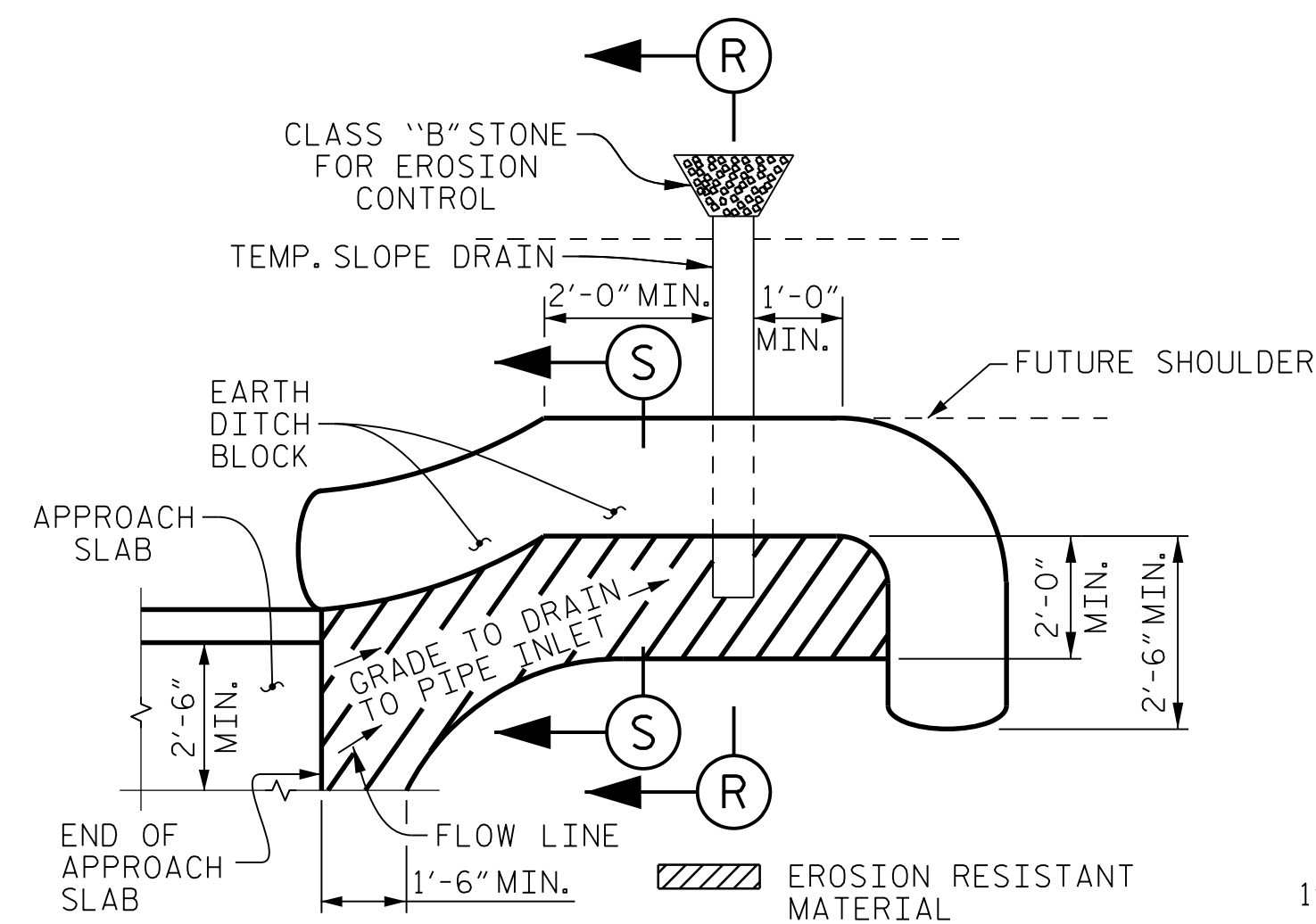
DRAWN BY:	K. E. LOFTON	DATE:	6-16
CHECKED BY:	A. D. SHAH	DATE:	10-16
DESIGN ENGINEER:	T. M. HARRIS	DATE:	10-16

FILE: j:\2707c\_struct\bridge\structures\plans - smc\_final\409\_047\_R2707C\_smc\_01\_9-2-24.dgn  
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IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

**TEMPORARY DRAINAGE DETAIL**

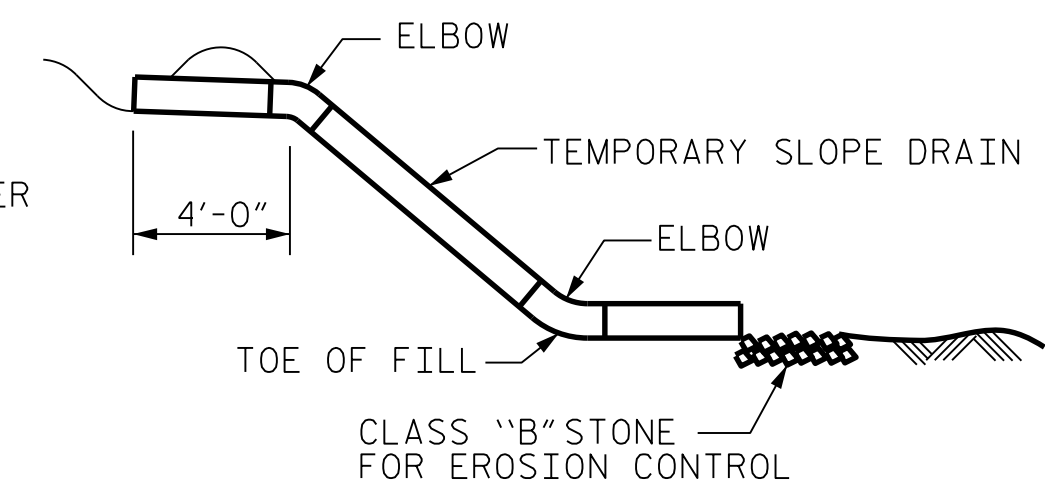


NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

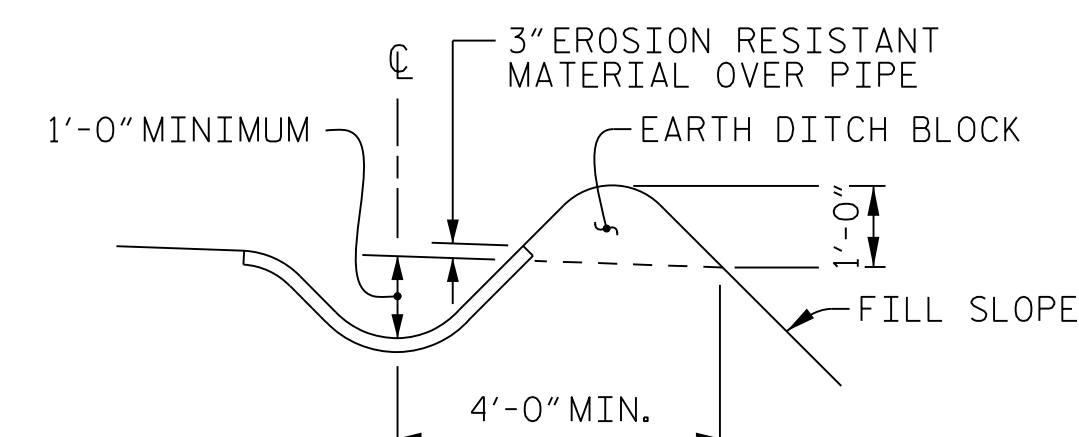
**PLAN VIEW**

**TEMPORARY BERM AND SLOPE DRAIN DETAILS**

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



**SECTION R-R**



**SECTION S-S**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD

**BRIDGE APPROACH  
 SLAB DETAILS  
 (LEFT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	S9-25
1			3			TOTAL SHEETS
2			4			25

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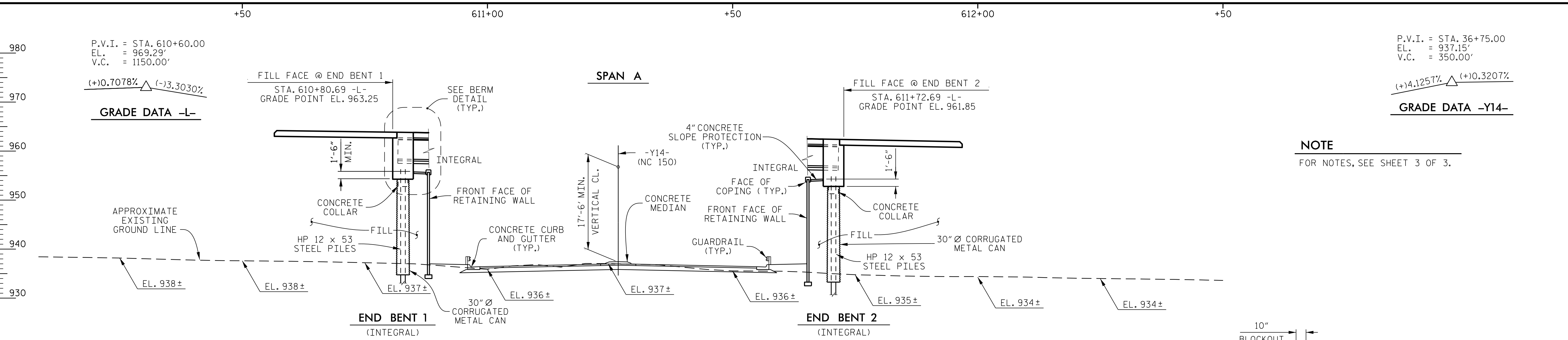
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY :	K. E. LOFTON	DATE :	6-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

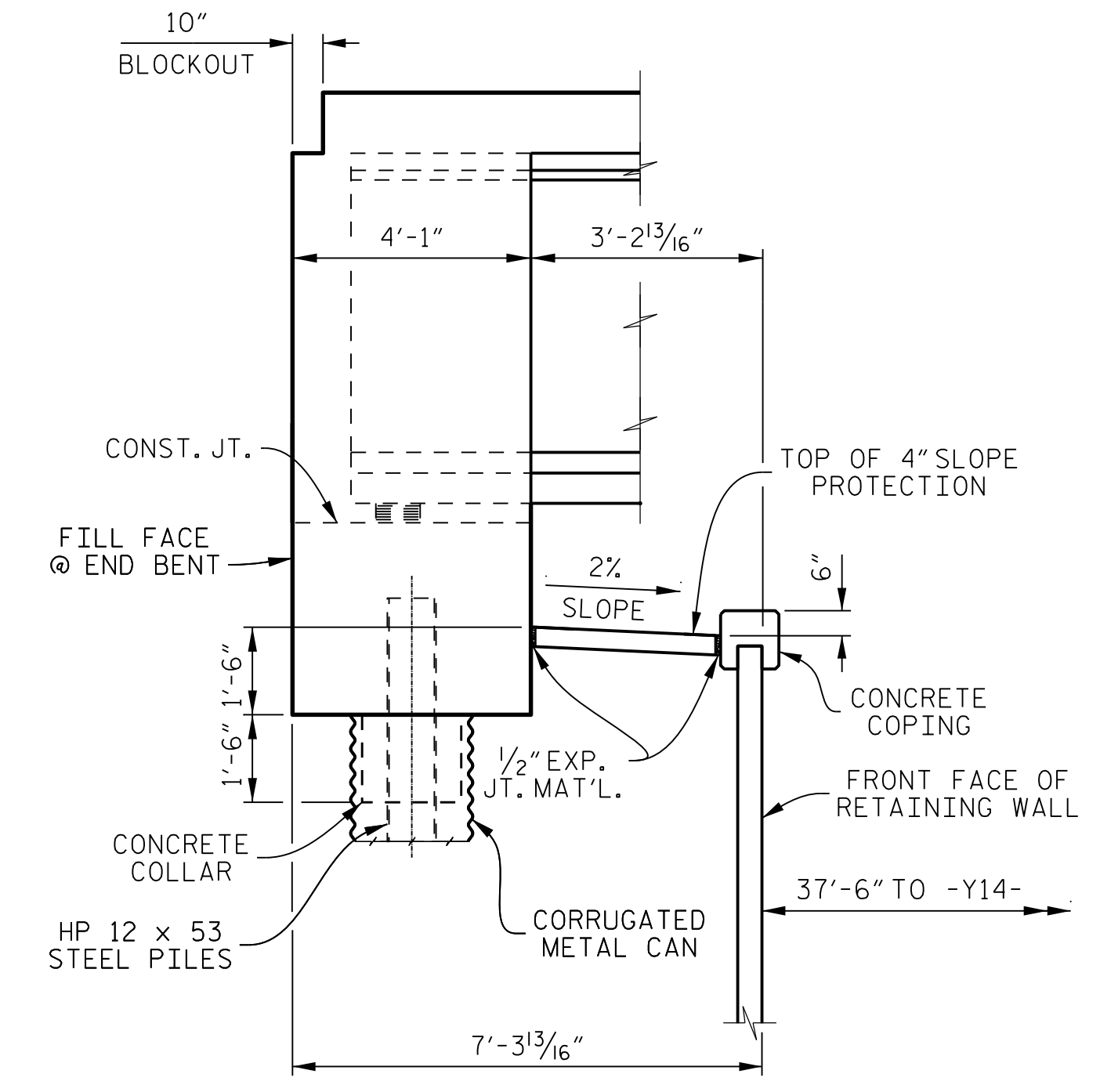
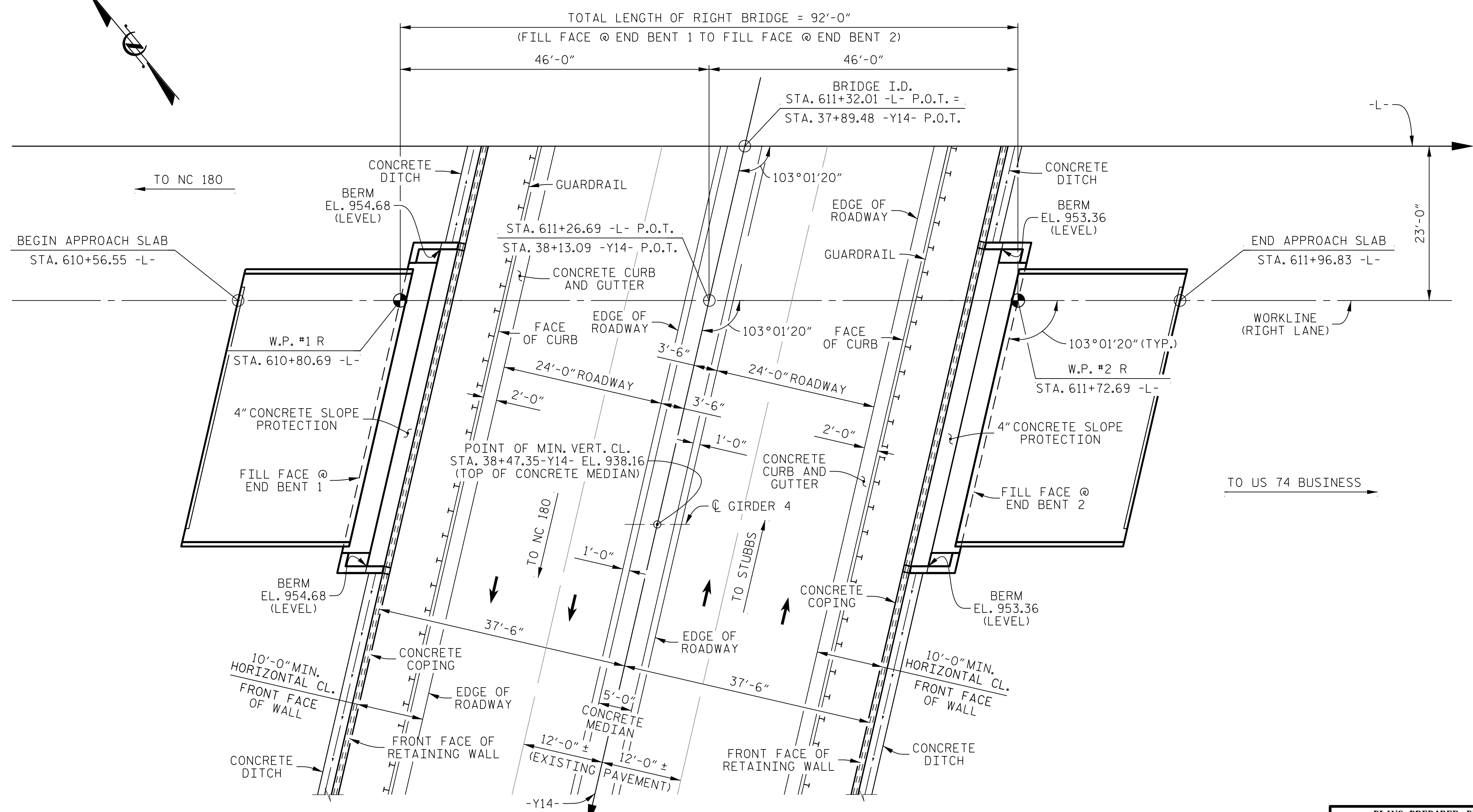
ASSEMBLED BY :	K. E. LOFTON	DATE :	6-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DRAWN BY :	FCJ	11/88	REV. 10/1/11
CHECKED BY :	ABR	11/88	REV. 7/12
			REV. 6/13

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**NOTE**  
FOR NOTES, SEE SHEET 3 OF 3.



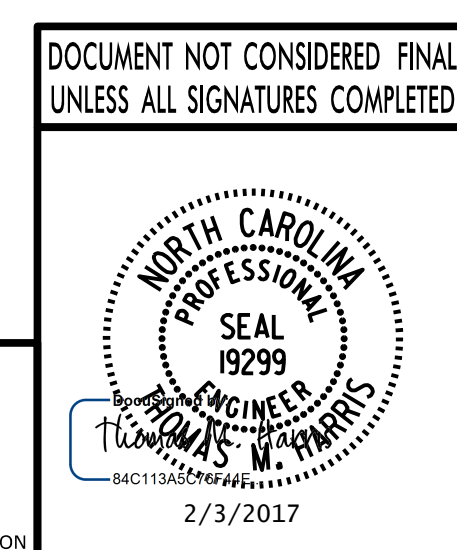
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611 + 32.01 -L-**  
**37 + 89.48 -Y14-**  
 SHEET 1 OF 3      BRIDGE No. 475

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
**BRIDGE OVER NC 150 (-Y14-) ON SHELBY BYPASS (-L-) BETWEEN NC 180 AND US 74 BUSINESS (RIGHT LANE)**

REVISIONS		SHEET No.	
No.	BY:	DATE:	No.
1			3
2			4

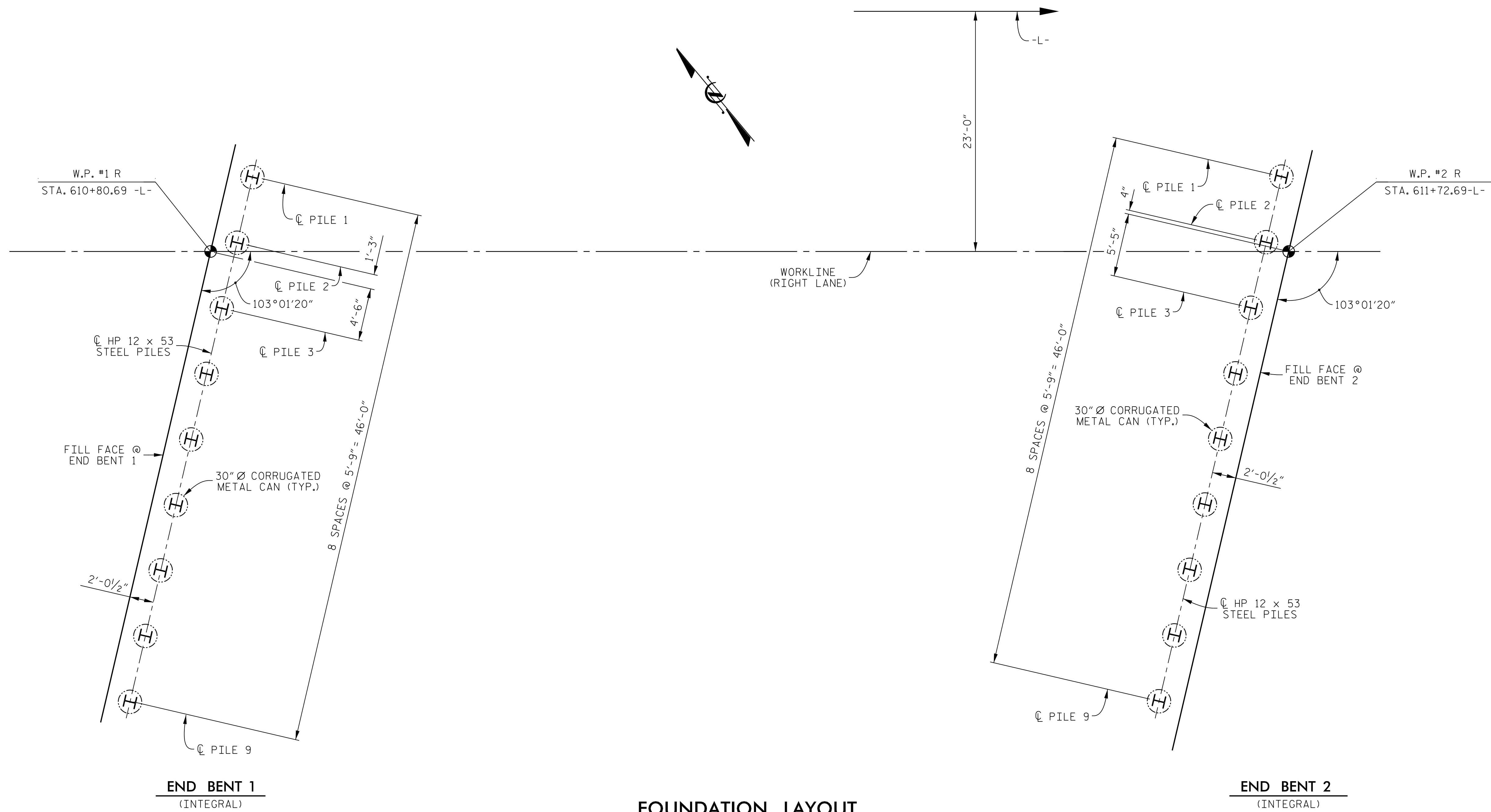
TOTAL SHEETS: 25



PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON      DATE: 10-16  
 CHECKED BY: A. D. SHAH      DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS      DATE: 10-16

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**NOTES**

ALL END BENT PILES ARE VERTICAL HP 12 x 53 STEEL PILES. DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINES AT THE BOTTOM OF THE END BENT CAPS.

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

DRIVE PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 215 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAW.

DRIVE PILES AT END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 215 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAW.

DRIVE PILES AT END BENT 1 AND END BENT 2 AFTER MSE WALL CONSTRUCTION AND AFTER THE 1 MONTH WAITING PERIOD.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 35,000 TO 55,000 FT.-LBS. PER BLOW WILL BE REQUIRED TO DRIVE PILES AT THE END BENTS. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

INSTALLATION OF 30" DIAMETER CORRUGATED METAL CANS FROM THE BOTTOM OF THE PILE CAP TO THE LEVELING PAD ELEVATION IS REQUIRED FOR PILES AT END BENT 1 AND END BENT 2. THE CANS SHALL BE DESIGNED TO WITHSTAND THE PRESSURES FROM COMPACTION OPERATIONS ON ADJACENT FILLS WITHOUT DISTORTION. AT A MINIMUM, CORRUGATED METAL CANS SHALL BE 16-GAUGE WITH A WALL THICKNESS OF 0.064".

LOOSELY BACKFILL CORRUGATED METAL CANS USING SAME MATERIAL AS MSE REINFORCEMENT ZONE PRIOR TO CONSTRUCTION OF THE END BENT PILE CAP. DO NOT COMPACT MATERIAL WITHIN THE CAN.

OBSERVE A 1 MONTH WAITING PERIOD AFTER CONSTRUCTING MSE RETAINING WALL AT END BENT 1 AND END BENT 2 TO THE BOTTOM OF CAP ELEVATION BEFORE BEGINNING END BENT CONSTRUCTION. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS.

OBSERVE A 1 MONTH WAITING PERIOD AFTER CONSTRUCTING THE EMBANKMENT, END BENT AND REINFORCED BRIDGE APPROACH FILL, IF APPLICABLE, BEFORE BEGINNING APPROACH SLAB CONSTRUCTION AT END BENT 1. FOR BRIDGE WAITING PERIODS, SEE ROADWAY PLANS AND SPECIAL PROVISIONS.

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

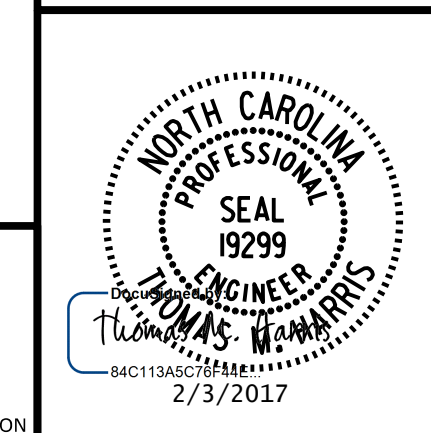
SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**GENERAL DRAWING**  
**BRIDGE OVER NC 150 (-Y14-) ON**  
**SHELBY BYPASS (-L-) BETWEEN**  
**NC 180 AND US 74 BUSINESS**  
**(RIGHT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

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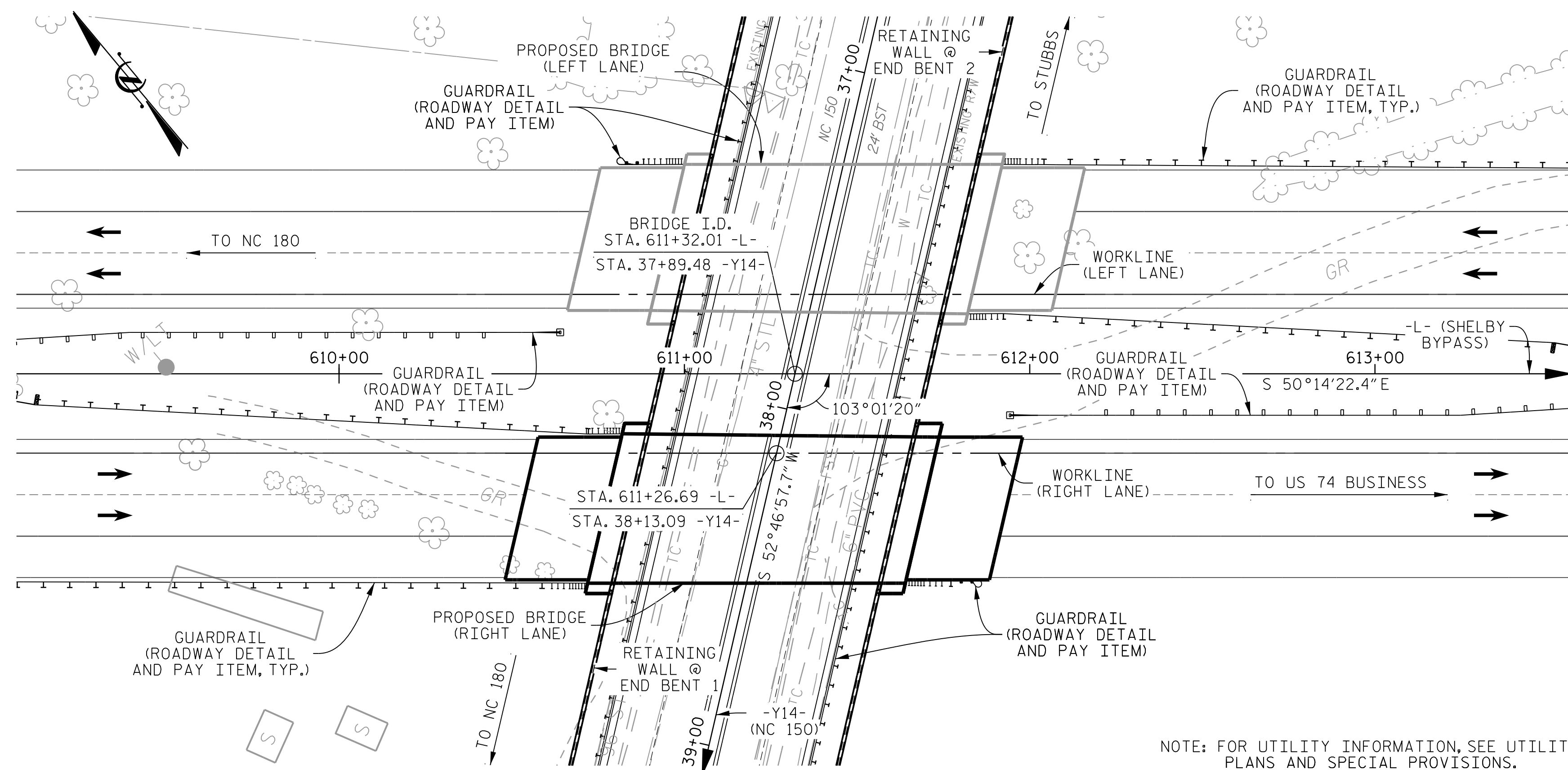
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : T. M. HARRIS DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

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 DATE : 10/20/17 12:47:55 PM



BENCHMARK: #27 - CHISEL SQUARE WITH "X" ON THE NORTH EAST WING WALL OF BRIDGE THAT CROSSES RAILROAD 102.86' RIGHT OF STA. 595+20.66 -L- EL. 944.58 N 579370.5 E 1255991.6



NOTE: FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.

LOCATION SKETCH

NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.  
 THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.  
 THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.  
 FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.  
 THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.  
 REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.  
 FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.  
 FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.  
 FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.  
 FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.  
 FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.  
 FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.  
 FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

TOTAL BILL OF MATERIAL

	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MODIFIED 63" PRESTRESSED CONCRETE GIRDERS		PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES	HP 12 x 53 STEEL PILES		PDA TESTING	CONCRETE BARRIER RAIL	4" SLOPE PROTECTION	ELASTOMERIC BEARINGS
	SQ. FT.	SQ. FT.	CU. YDS.	LUMP SUM	LBS.	No.	LIN. FT.	EACH	No.	LIN. FT.	EACH	LIN. FT.	SQ. YD.	LUMP SUM
SUPERSTRUCTURE	3,979	5,505		LUMP SUM		4	356.92					180.58		LUMP SUM
END BENT 1			31.5		5,258			9	9	720.0			14	
END BENT 2			31.5		5,258			9	9	720.0	1		14	
TOTAL	3,979	5,505	63.0	LUMP SUM	10,516	4	356.92	18	18	1,440.0	1	180.58	28	LUMP SUM

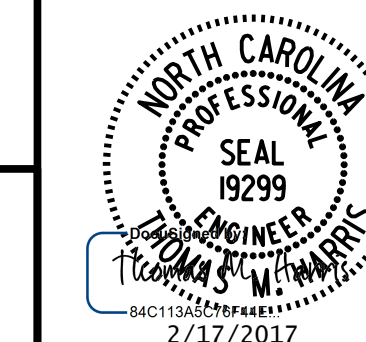
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

GENERAL DRAWING  
 BRIDGE OVER NC 150 (-Y14-) ON  
 SHELBY BYPASS (-L-) BETWEEN  
 NC 180 AND US 74 BUSINESS  
 (RIGHT LANE)

DOCUMENT NOT CONSIDERED FINAL  
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PLANS PREPARED BY:  
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 5540 CenterView Drive, Suite 217  
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 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY: K. E. LOFTON DATE: 10-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

**LOAD FACTORS**

DESIGN LOAD RATING FACTORS	LIMIT STATE	$\gamma_{DC}$	$\gamma_{DW}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

**NOTES**

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.  
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

**COMMENTS**

1. LEFT EXTERIOR GIRDER IS THE CONTROLLING EXTERIOR GIRDER.

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING (#)	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	MOMENT					SHEAR					LIVE-LOAD FACTORS ( $\gamma_{LL}$ )	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPANS	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPANS		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	1	1.08	--	1.75	0.966	1.21	A	E	43.9	1.109	1.28	A	I	26.1	0.80	0.966	1.08	A	E	43.9	--	
	HL-93 (OPERATING)	N/A		1.57	--	1.35	0.966	1.57	A	E	43.9	1.109	2.01	A	I	17.1	N/A	--	--	--	--	--	--	--
	HS-20 (INVENTORY)	36.000	2	1.47	52.92	1.75	0.966	1.64	A	E	43.9	1.109	1.97	A	I	17.1	0.80	0.966	1.47	A	E	43.9	--	
	HS-20 (OPERATING)	36.000		2.13	76.68	1.35	0.966	2.13	A	E	43.9	1.109	2.59	A	I	17.1	N/A	--	--	--	--	--	--	--
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.43	46.31	1.40	0.966	4.79	A	E	43.9	1.109	6.17	A	I	17.1	0.80	0.966	3.43	A	E	43.9	--
		SNGARBS2	20.000		2.51	50.20	1.40	0.966	3.50	A	E	43.9	1.109	4.33	A	I	17.1	0.80	0.966	2.51	A	E	43.9	--
		SNAGRIS2	22.000		2.36	51.92	1.40	0.966	3.29	A	E	43.9	1.109	4.01	A	I	17.1	0.80	0.966	2.36	A	E	43.9	--
		SNCOTTS3	27.250		1.70	46.33	1.40	0.966	2.38	A	E	43.9	1.109	3.01	A	I	17.1	0.80	0.966	1.70	A	E	43.9	--
		SNAGGRS4	34.925		1.41	49.24	1.40	0.966	1.96	A	E	43.9	1.109	2.47	A	I	17.1	0.80	0.966	1.41	A	E	43.9	--
		SNS5A	35.550		1.38	49.06	1.40	0.966	1.92	A	E	43.9	1.109	2.49	A	I	17.1	0.80	0.966	1.38	A	E	43.9	--
		SNS6A	39.950		1.26	50.34	1.40	0.966	1.75	A	E	43.9	1.109	2.26	A	I	17.1	0.80	0.966	1.26	A	E	43.9	--
		SNS7B	42.000		1.20	50.40	1.40	0.966	1.67	A	E	43.9	1.109	2.21	A	I	17.1	0.80	0.966	1.20	A	E	43.9	--
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.53	50.49	1.40	0.966	2.13	A	E	43.9	1.109	2.71	A	I	17.1	0.80	0.966	1.53	A	E	43.9	--
		TNT4A	33.075		1.53	50.60	1.40	0.966	2.14	A	E	43.9	1.109	2.65	A	I	17.1	0.80	0.966	1.53	A	E	43.9	--
		TNT6A	41.600		1.25	52.00	1.40	0.966	1.74	A	E	43.9	1.109	2.35	A	I	17.1	0.80	0.966	1.25	A	E	43.9	--
		TNT7A	42.000		1.25	52.50	1.40	0.966	1.74	A	E	43.9	1.109	2.30	A	I	17.1	0.80	0.966	1.25	A	E	43.9	--
		TNT7B	42.000		1.28	53.76	1.40	0.966	1.79	A	E	43.9	1.109	2.16	A	I	17.1	0.80	0.966	1.28	A	E	43.9	--
		TNAGRIT4	43.000		1.23	52.89	1.40	0.966	1.71	A	E	43.9	1.109	2.09	A	I	17.1	0.80	0.966	1.23	A	E	43.9	--
TNAGT5A	45.000		1.16	52.20	1.40	0.966	1.62	A	E	43.9	1.109	2.06	A	I	17.1	0.80	0.966	1.16	A	E	43.9	--		
TNAGT5B	45.000	3	1.15	51.75	1.40	0.966	1.61	A	E	43.9	1.109	1.98	A	I	17.1	0.80	0.966	1.15	A	E	43.9	--		

# CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

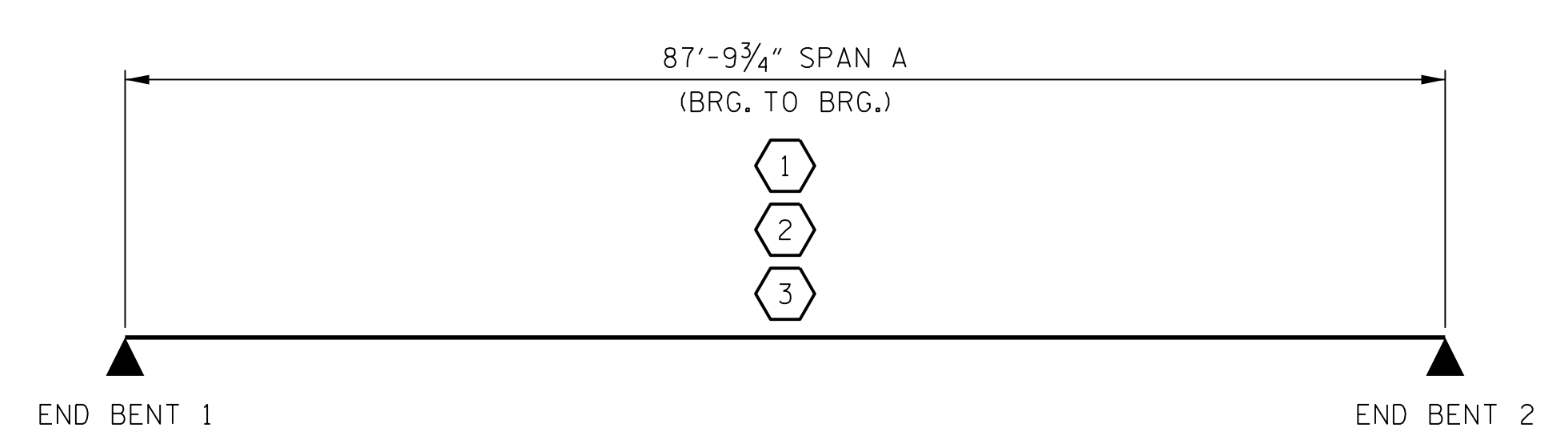
2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER  
E - EXTERIOR GIRDER (LEFT)



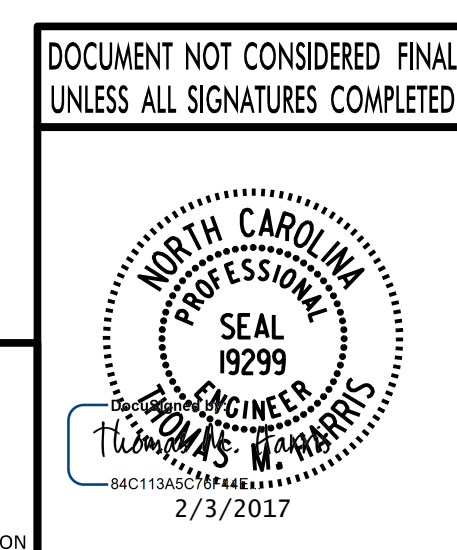
**LRFR SUMMARY**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

STANDARD  
LRFR SUMMARY FOR  
PRESTRESSED CONCRETE GIRDERS  
(NON-INTERSTATE TRAFFIC)  
(RIGHT LANE)

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	510-4
1			3			TOTAL SHEETS
2			4			25

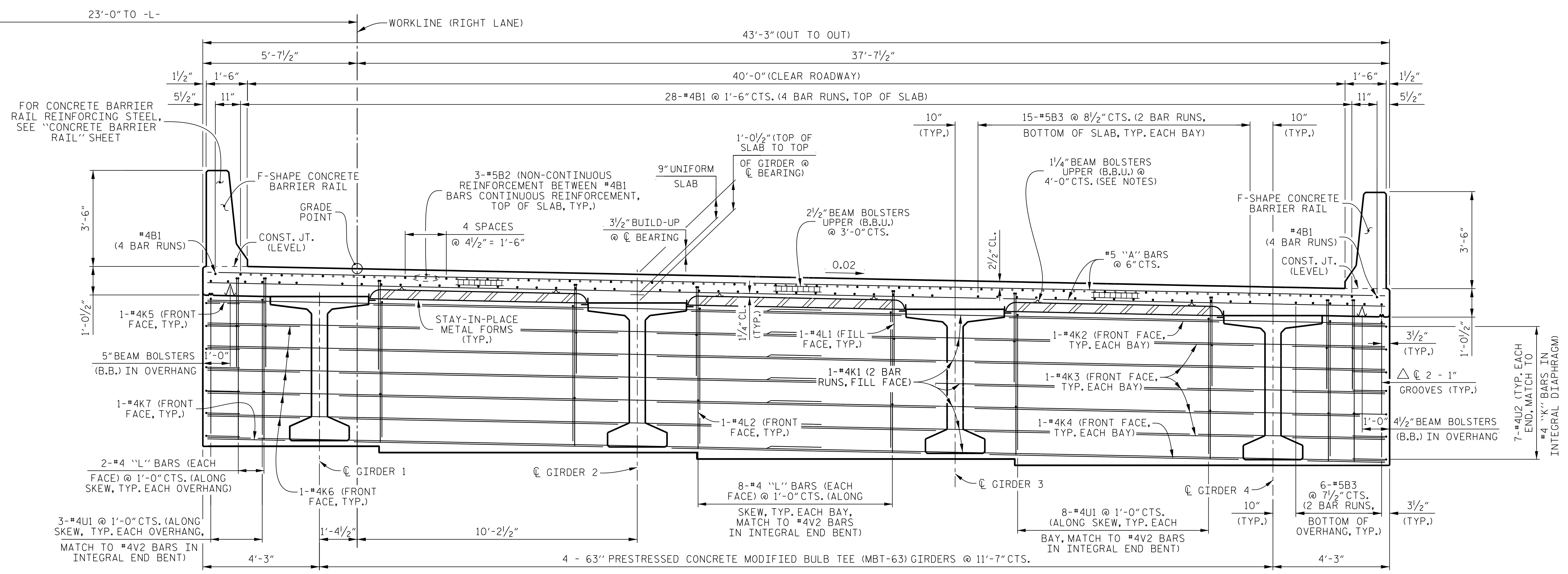


PLANS PREPARED BY:  
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NC LICENSE No. F-0246

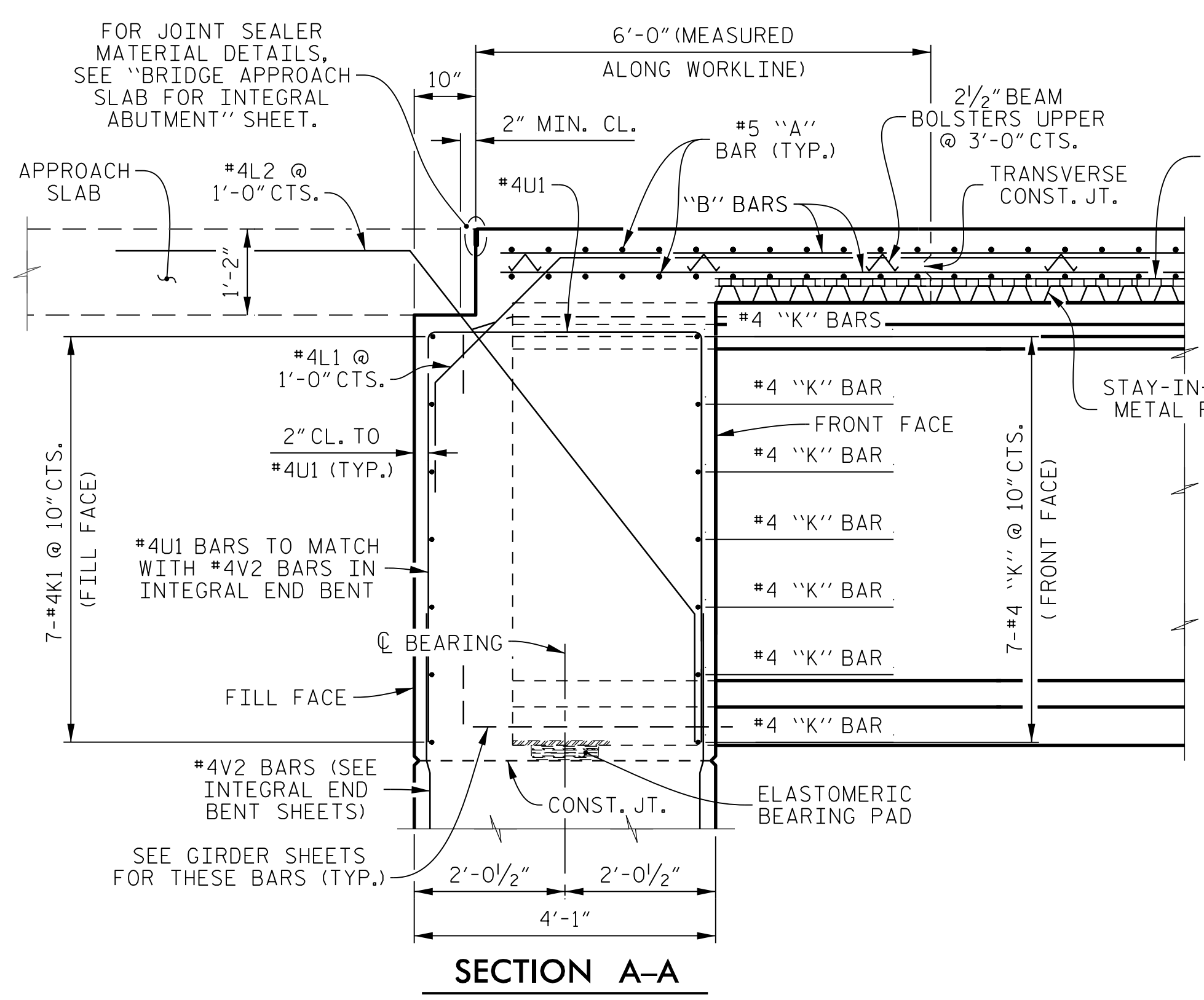
DRAWN BY : K. E. LOFTON    DATE : 10-16  
CHECKED BY : A. D. SHAH    DATE : 10-16  
DESIGN ENGINEER : T. M. HARRIS    DATE : 10-16

ASSEMBLED BY : K. E. LOFTON    DATE : 10-16  
CHECKED BY : A. D. SHAH    DATE : 10-16  
DRAWN BY : MAA    1/08    REV. 11/2/08RR MAA/GM  
CHECKED BY : GM/DI 2/08    REV. 10/1/11    MAA/GM

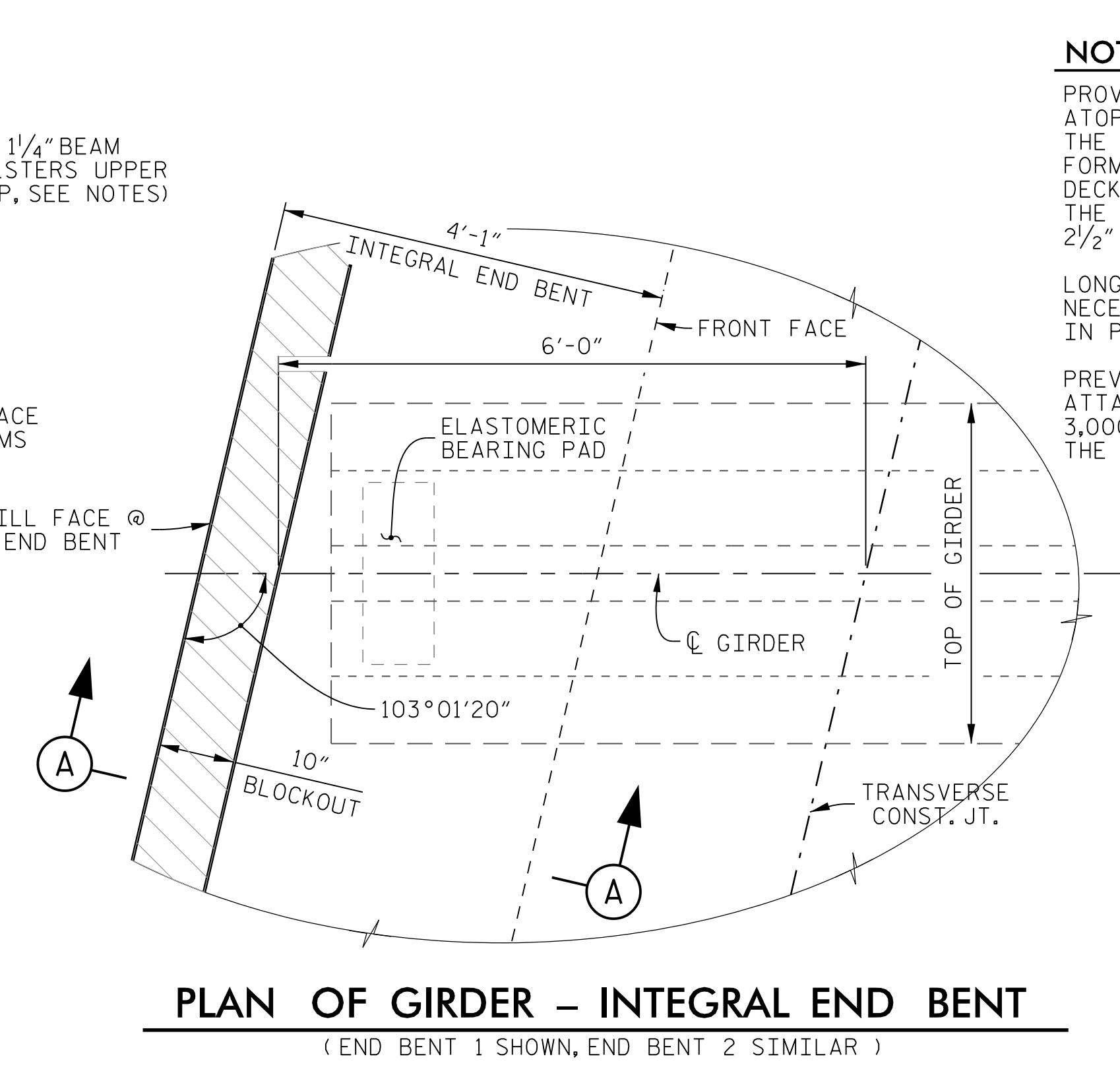




**TYPICAL SECTION AT INTEGRAL END BENT 1 AND END BENT 2**



**SECTION THRU INTEGRAL END BENT**  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)



**PLAN OF GIRDER - INTEGRAL END BENT**  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)

**NOTES**

- PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
- LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
- PREVIOUSLY CAST CONCRETE IN SPAN "A" SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE SPAN.
- BARRIER RAIL IN SPAN "A" SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
- FOR F-SHAPE CONCRETE BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.
- FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.
- FOR END VIEW OF INTEGRAL DIAPHRAGM, SEE SHEET 2 OF 2.

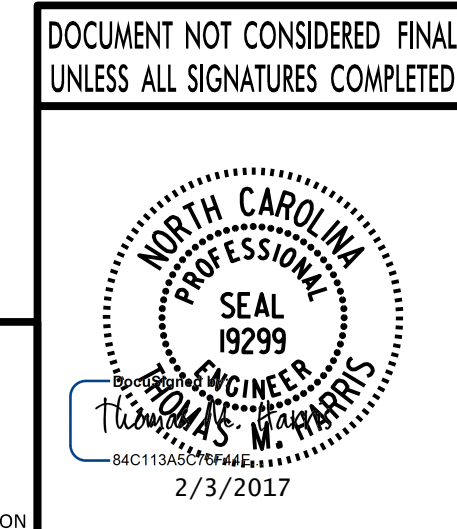
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUPERSTRUCTURE**  
**TYPICAL SECTION**  
**(RIGHT LANE)**

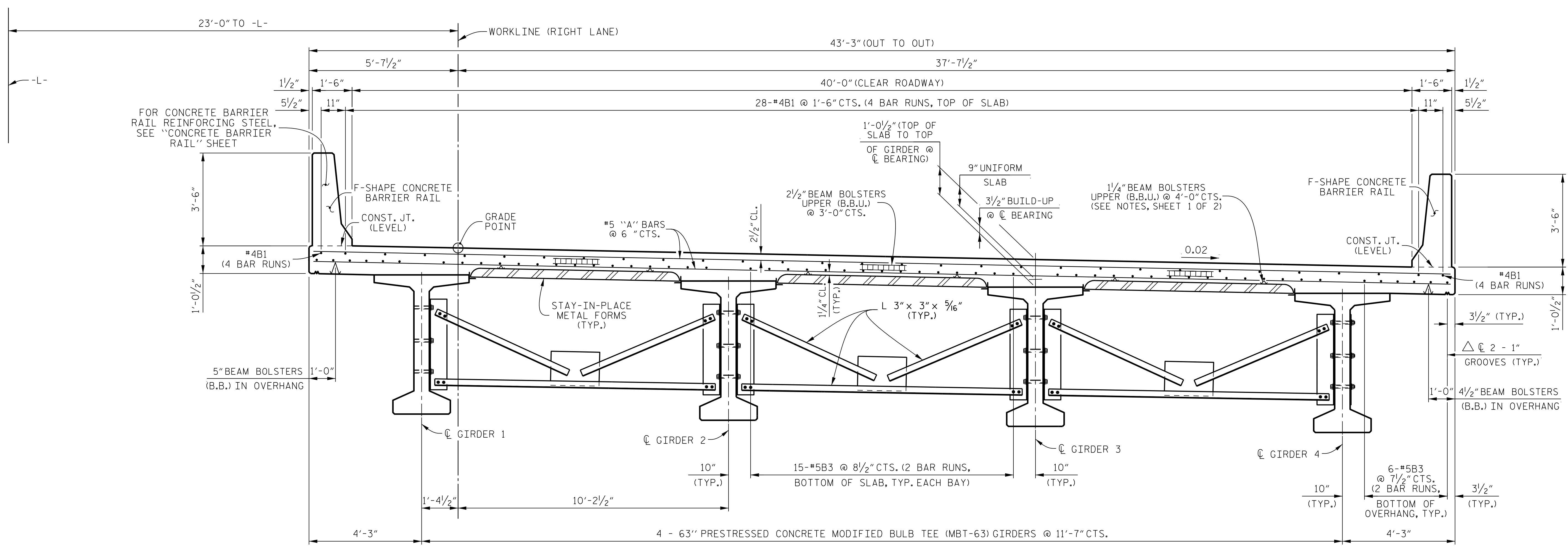
REVISIONS						SHEET No.
No.	BY	DATE	No.	BY	DATE	TOTAL SHEETS
1			3			25
2			4			



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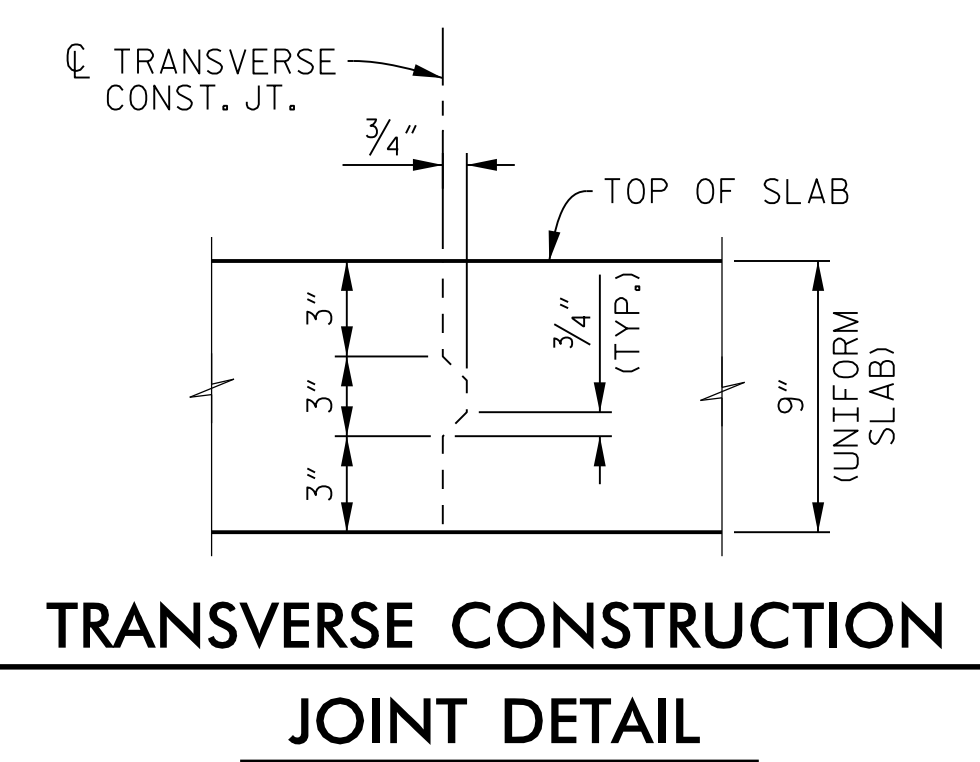
DRAWN BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	T. M. HARRIS	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

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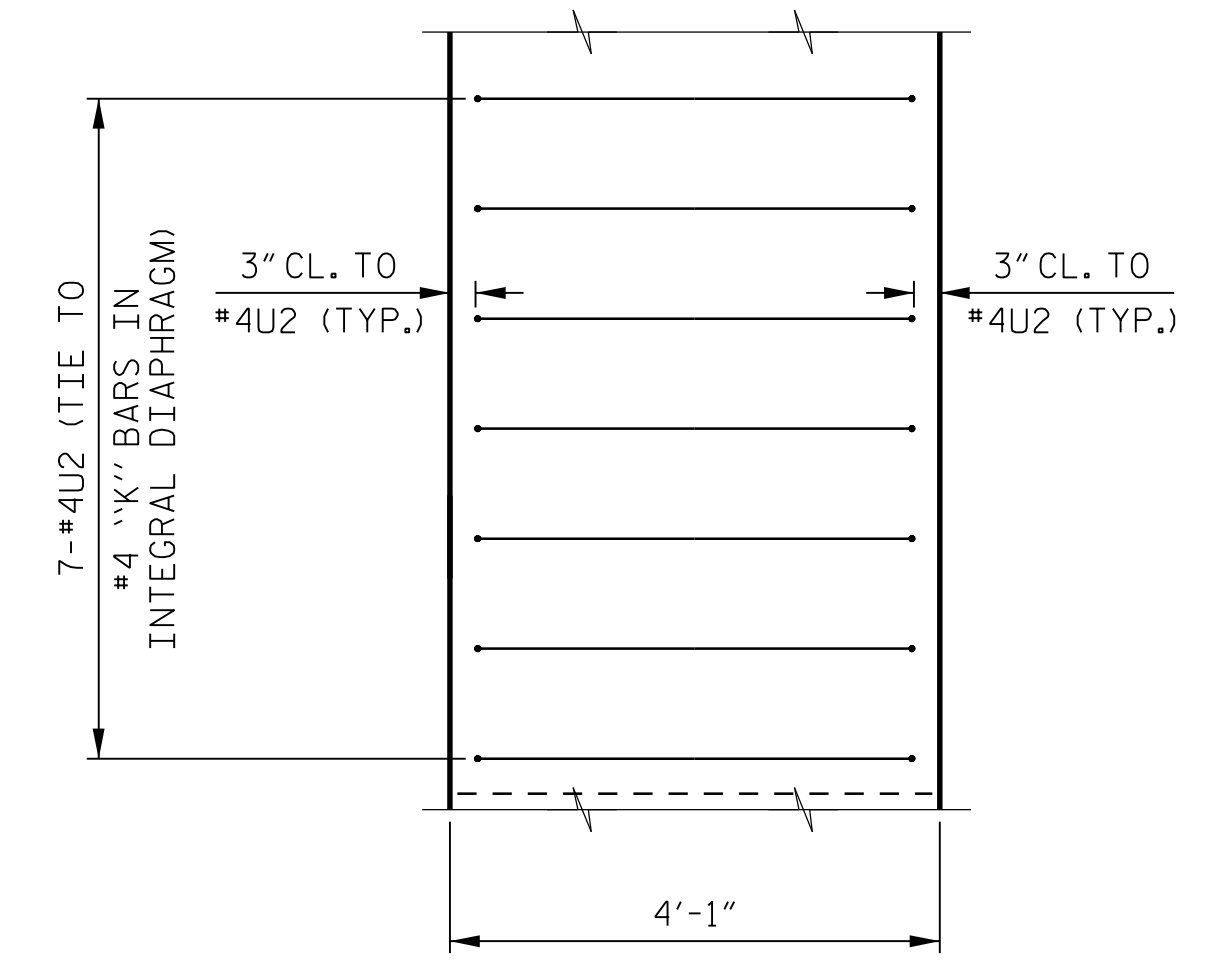


**TYPICAL SECTION AT INTERMEDIATE STEEL DIAPHRAGMS**

FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.



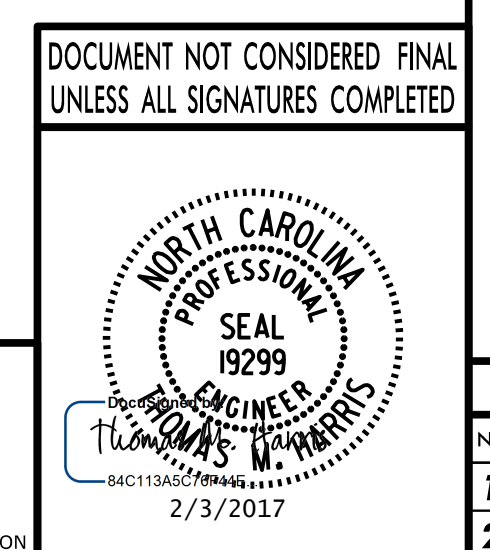
**TRANSVERSE CONSTRUCTION JOINT DETAIL**  
 REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THROUGH JOINT.



**END VIEW OF INTEGRAL DIAPHRAGM**

PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**  
 SHEET 2 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**SUPERSTRUCTURE**  
**TYPICAL SECTION**  
**(RIGHT LANE)**



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 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

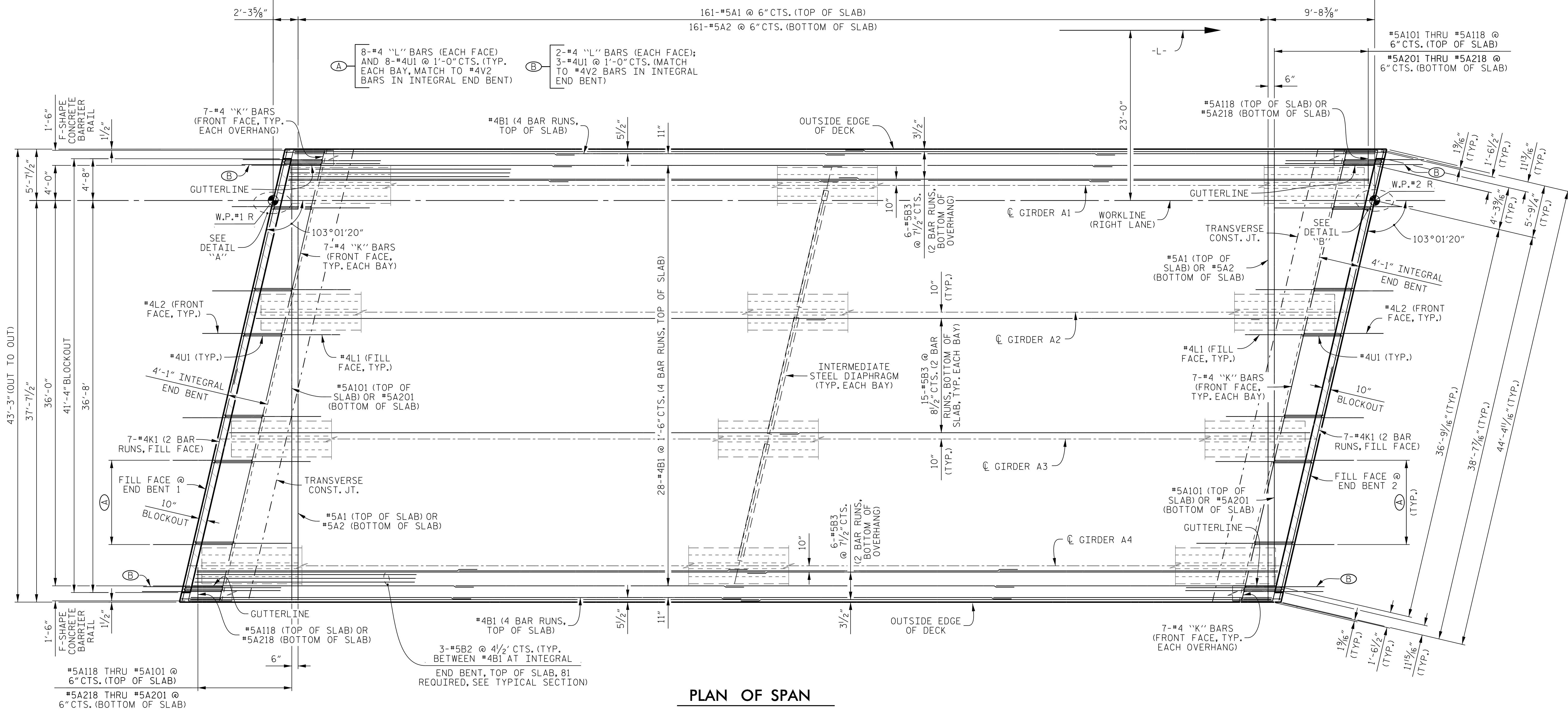
DRAWN BY: **K. E. LOFTON** DATE: **10-16**  
 CHECKED BY: **T. M. HARRIS** DATE: **10-16**  
 DESIGN ENGINEER: **T. M. HARRIS** DATE: **10-16**

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

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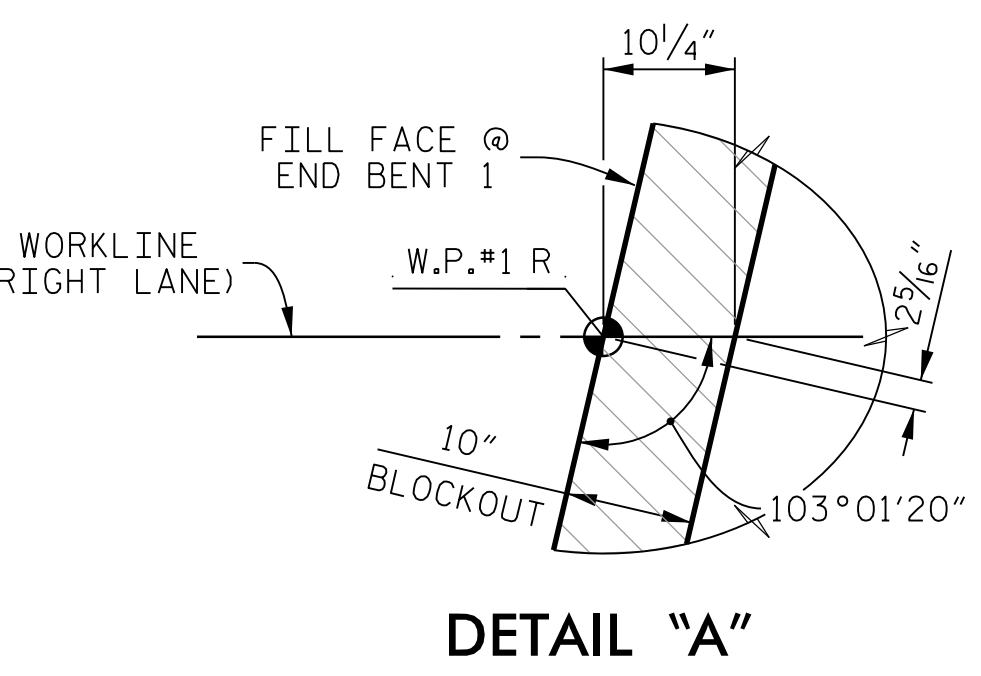
92'-0" (FILL FACE @ END BENT 1 TO FILL FACE @ END BENT 2)



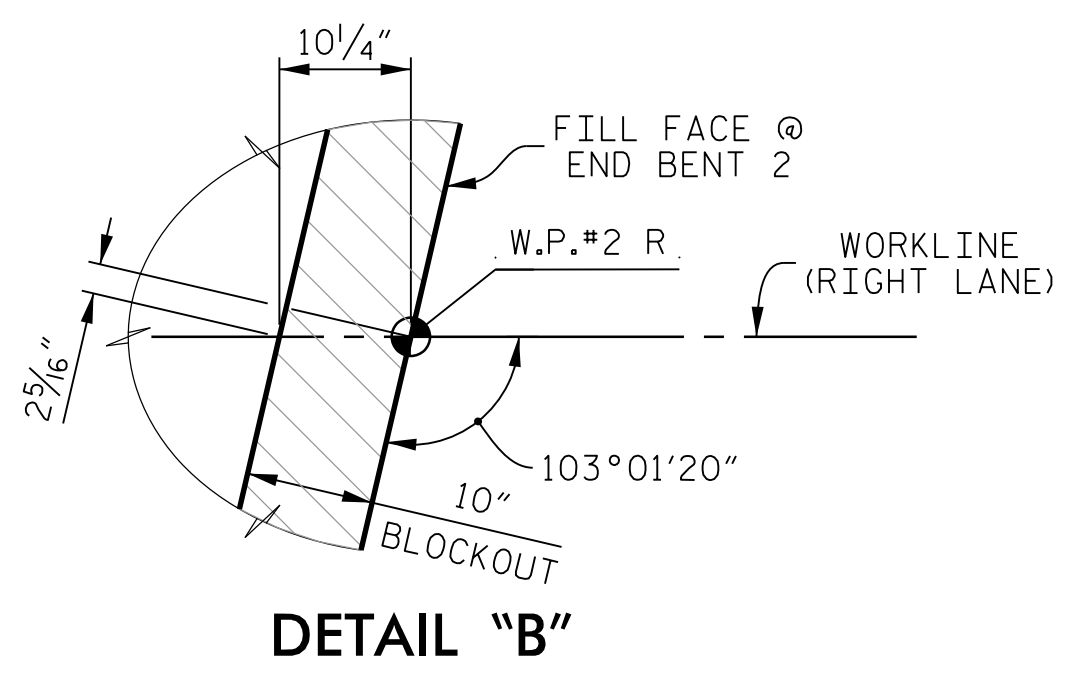
PLAN OF SPAN

PROJECT NO. R-2707C  
 CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

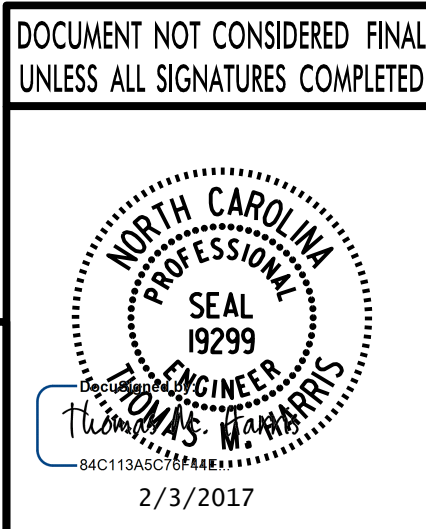
**NOTES**  
 #5 "A" BARS SHALL BE PLACED PERPENDICULAR TO WORKLINE WITH A 2" MINIMUM CLEARANCE ON EACH END.  
 FOR REINFORCING STEEL IN CONCRETE BARRIER RAIL, SEE "CONCRETE BARRIER RAIL" SHEET.  
 FOR SLAB POURING SEQUENCE AND SPLICE CHART, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.  
 FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIDERS" SHEET.



DETAIL "A"



DETAIL "B"

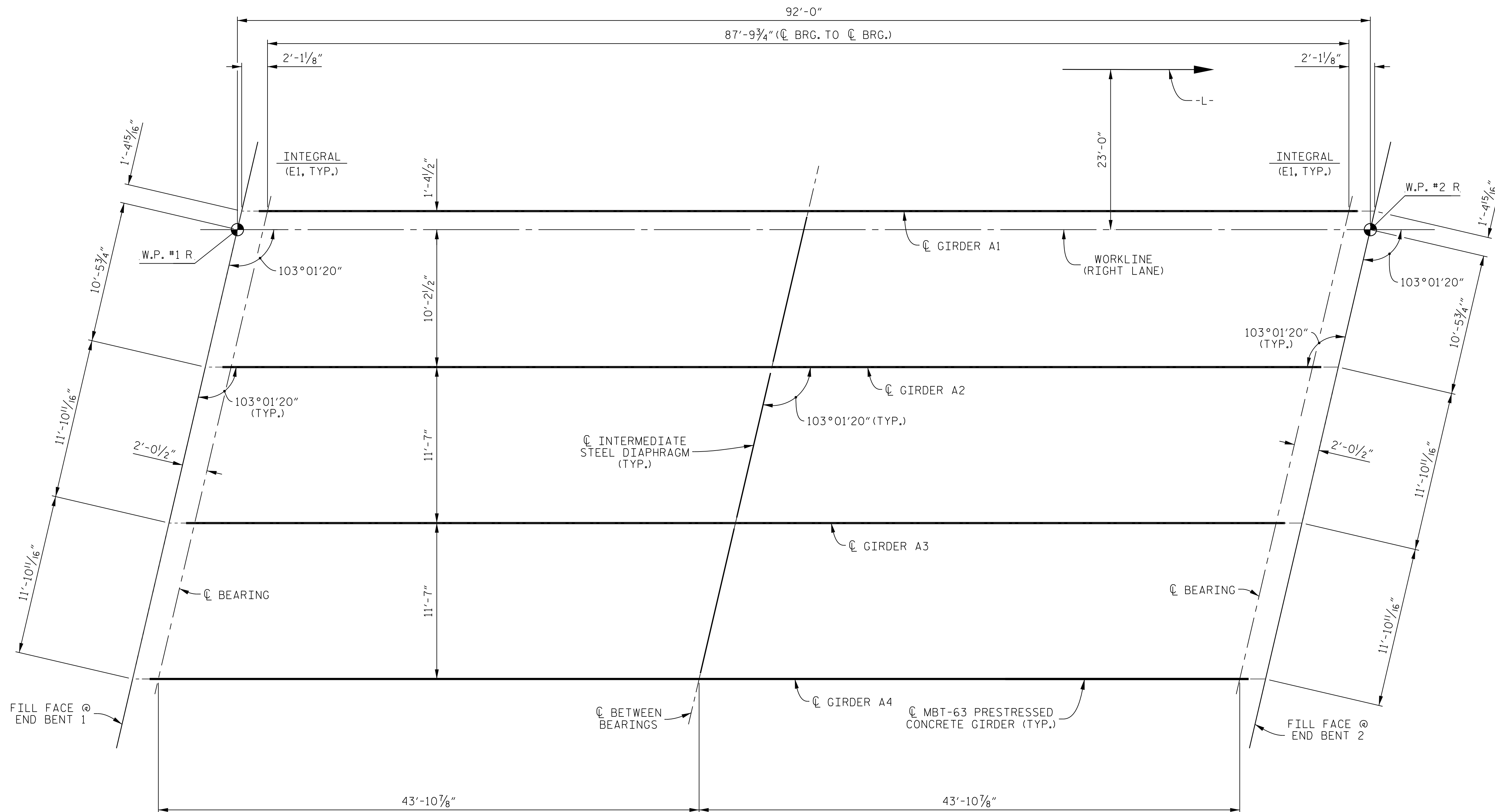


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CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH			
SUPERSTRUCTURE PLAN OF SPAN (RIGHT LANE)			
REVISIONS			
No.	BY:	DATE:	No.
1			3
2			4
SHEET No.			S10-7
TOTAL SHEETS			25

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 DATE: 10/20/17 12:48:12 PM



SPAN A  
FRAMING PLAN

**NOTES**

FOR INTERMEDIATE STEEL DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.

FOR ELASTOMERIC BEARING DETAILS, SEE "ELASTOMERIC BEARING DETAILS PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE" SHEET.

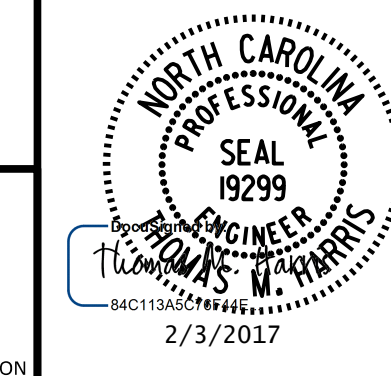
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611 + 32.01 -L-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUPERSTRUCTURE  
 FRAMING PLAN  
 (RIGHT LANE)

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

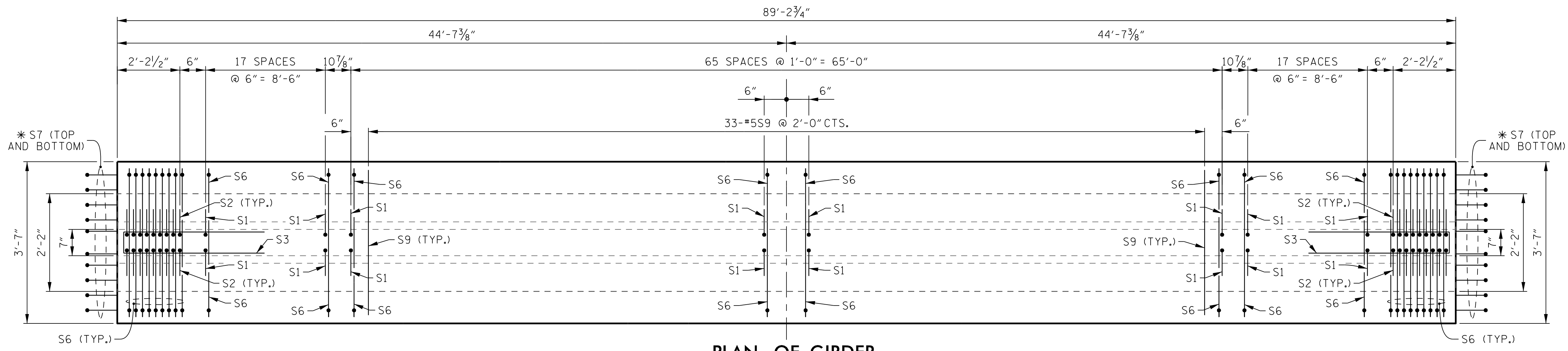
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 UNLESS ALL SIGNATURES COMPLETED



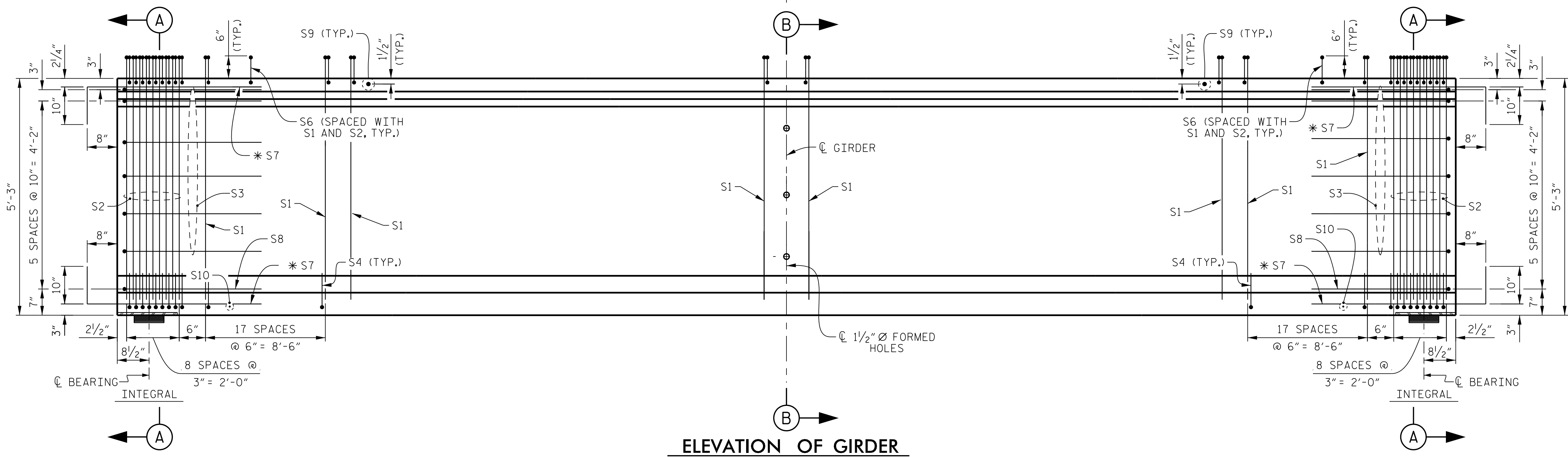
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 Raleigh, NC 27606-3386  
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 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16





PLAN OF GIRDER



ELEVATION OF GIRDER

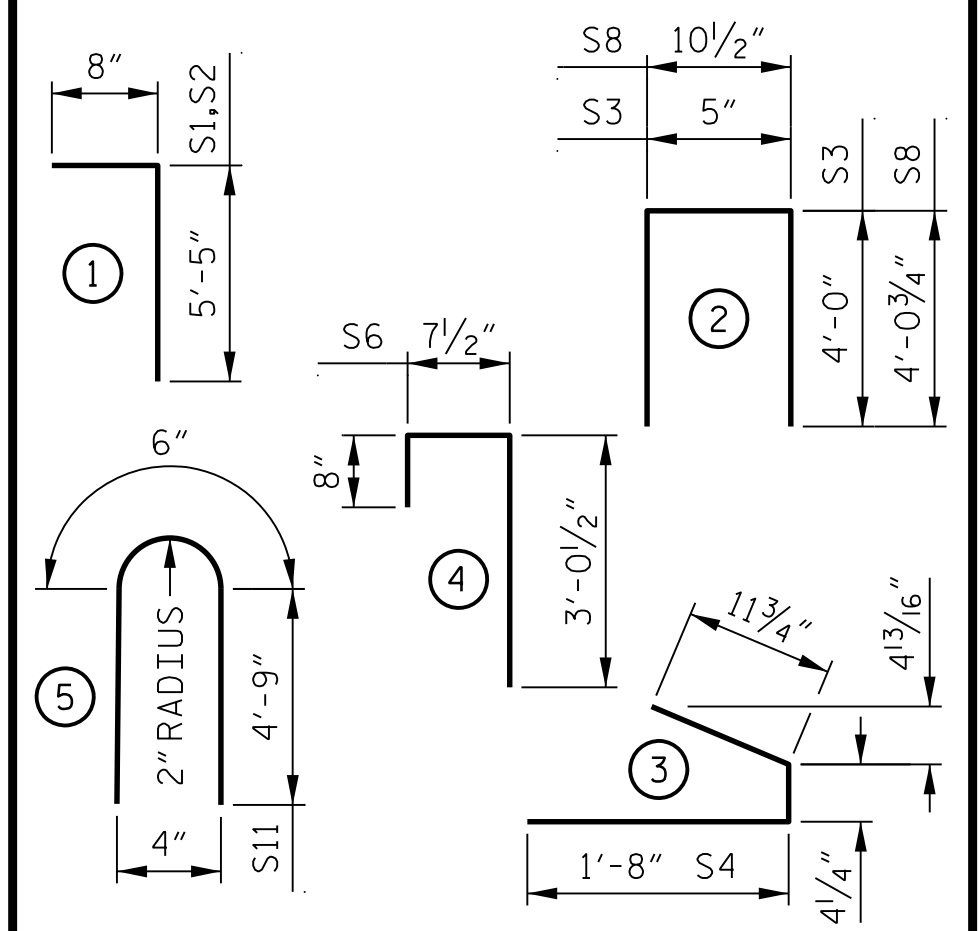
**NOTES**  
 FOR SECTION A-A AND SECTION B-B, SEE SHEET 2 OF 3.  
 FOR STRAND PATTERN, SEE SHEET 2 OF 3.  
 FOR ADDITIONAL NOTES, SEE SHEET 3 OF 3.

0.6" Ø L.R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GIRDER					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	204	#4	1	6'-1"	829
S2	36	#5	1	6'-1"	228
S3	12	#4	2	8'-5"	67
S4	108	#4	3	3'-0"	216
S6	240	#5	4	4'-4"	1,085
*S7	40	#5	STR	3'-8"	153
S8	2	#5	2	9'-0"	19
S9	33	#5	STR	3'-3"	112
S10	2	#3	STR	1'-10"	1
S11	4	#5	5	10'-0"	42
S12	8	#4	STR	8'-0"	43

\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

**BAR TYPES**  
 ALL BAR DIMENSIONS ARE OUT-TO-OUT.



QUANTITIES FOR ONE GIRDER			
	REINFORCING STEEL (LB.)	7,000 PSI CONCRETE (C.Y.)	0.6" Ø L.R. STRANDS (No.)
GIRDERS			
A1 THRU A4	2,795	17.7	30

GIRDERS REQUIRED			
LOCATION	NUMBER	LENGTH	TOTAL LENGTH
SPAN A	4	89'-2 3/4"	356.92'

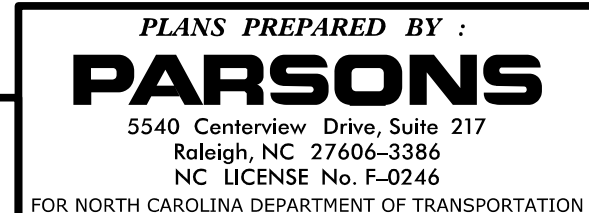
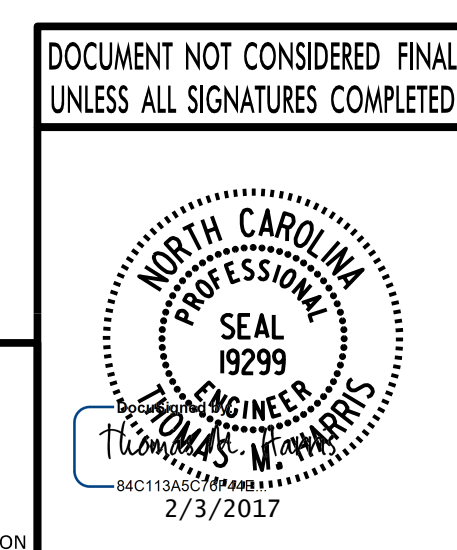
PROJECT NO. **R-2707C**  
**CLEVELAND** COUNTY  
 STATION: **611+32.01 -L-**

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
**63" PRESTRESSED CONCRETE MODIFIED BULB TEE GIRDER (RIGHT LANE)**

REVISIONS					
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

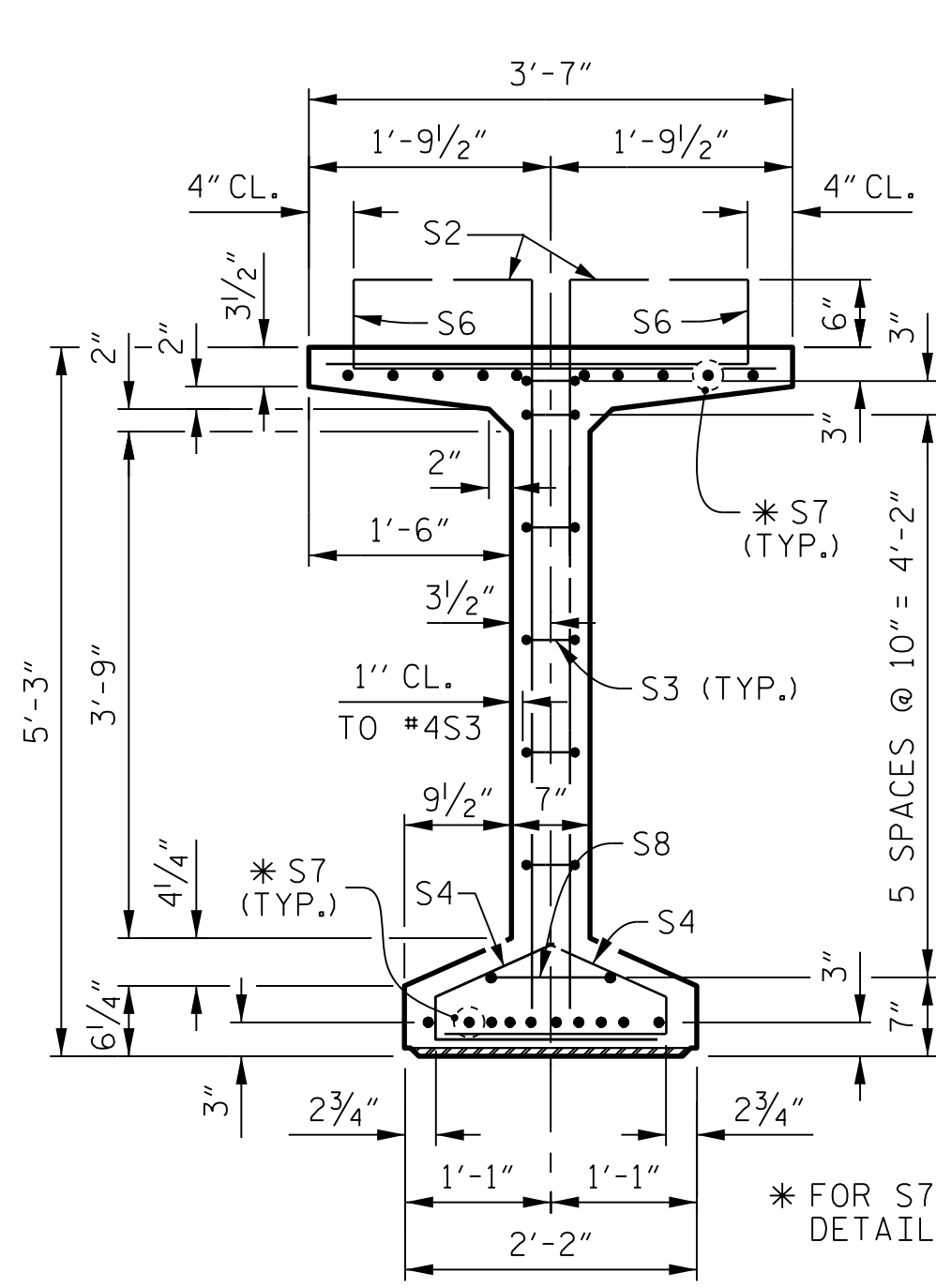
SHEET No. **S10-9**  
 TOTAL SHEETS **25**



ASSEMBLED BY : K. E. LOFTON	DATE : 10-16
CHECKED BY : A. D. SHAH	DATE : 10-16
DRAWN BY : EEM 2/6/97	REV. 10/1/11 MAA/GM
CHECKED BY : VAP 2/6/97	REV. 6/13 MAA/GM
	REV. 1/15 MAA/TMC

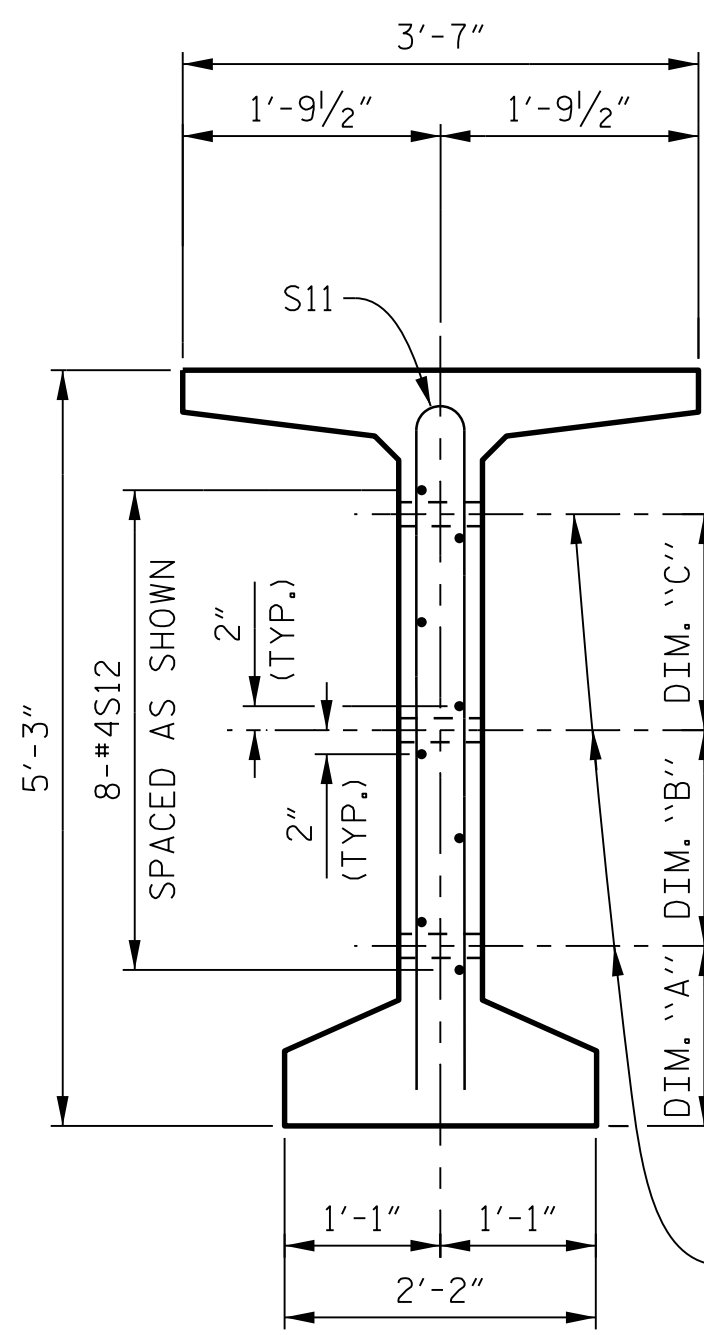
DRAWN BY : K. E. LOFTON	DATE : 10-16
CHECKED BY : A. D. SHAH	DATE : 10-16
DESIGN ENGINEER : T. M. HARRIS	DATE : 10-16

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**SECTION A-A**

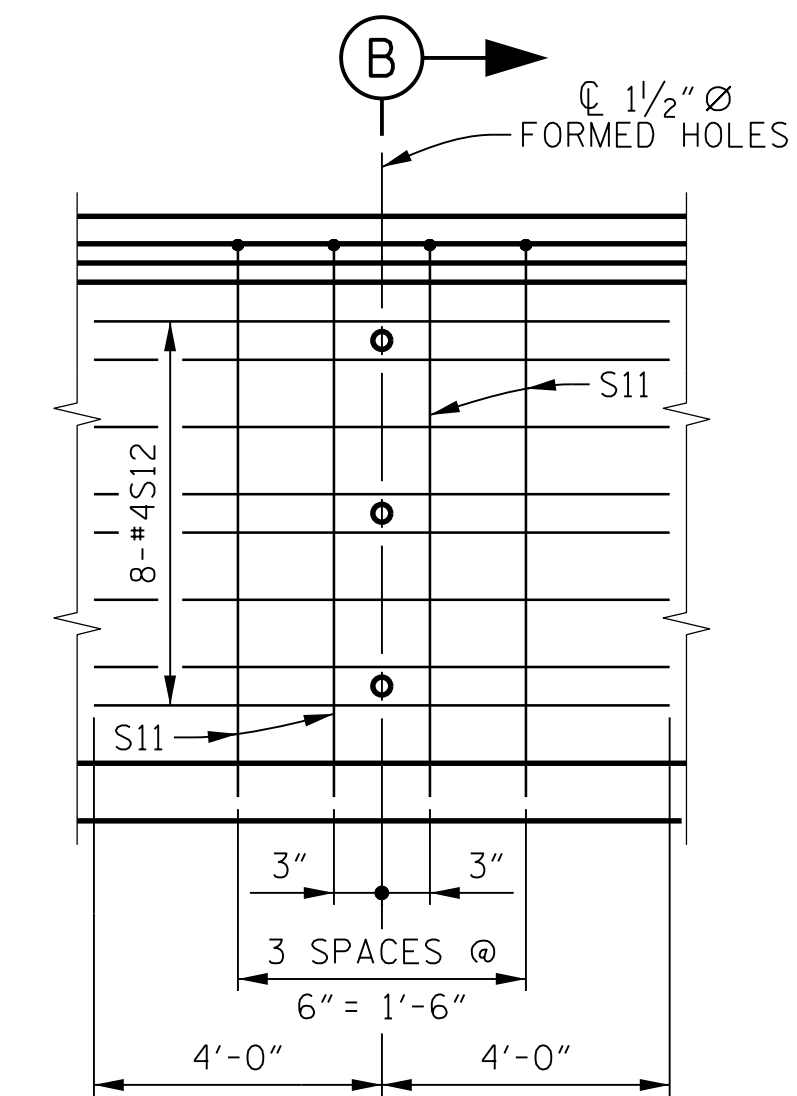
\* FOR S7 BAR SPACING, SEE DETAIL "C", SHEET 3 OF 3.



**SECTION B-B**

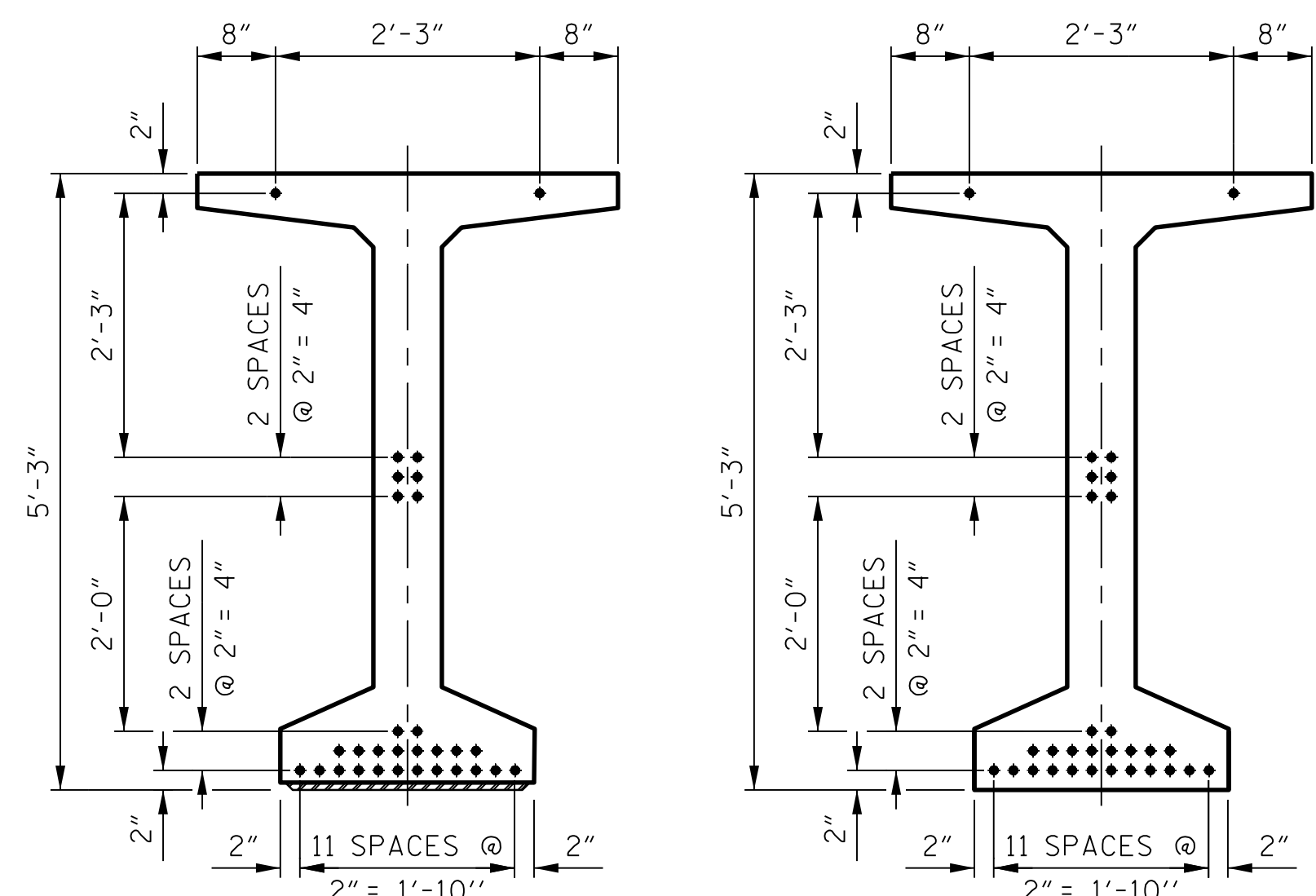
(S1, S6 AND S9 BARS NOT SHOWN)

TABLE			
GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"
63" BULB TEE	1'-9"	1'-5"	11 3/4"



**PARTIAL ELEVATION**

SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR ALL GIRDERS



**30 - 0.6" Ø LOW RELAXATION STRAND LAYOUT**

• FULLY BONDED STRANDS

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD

**63" PRESTRESSED CONCRETE  
 MODIFIED BULB TEE GIRDER**

**(RIGHT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

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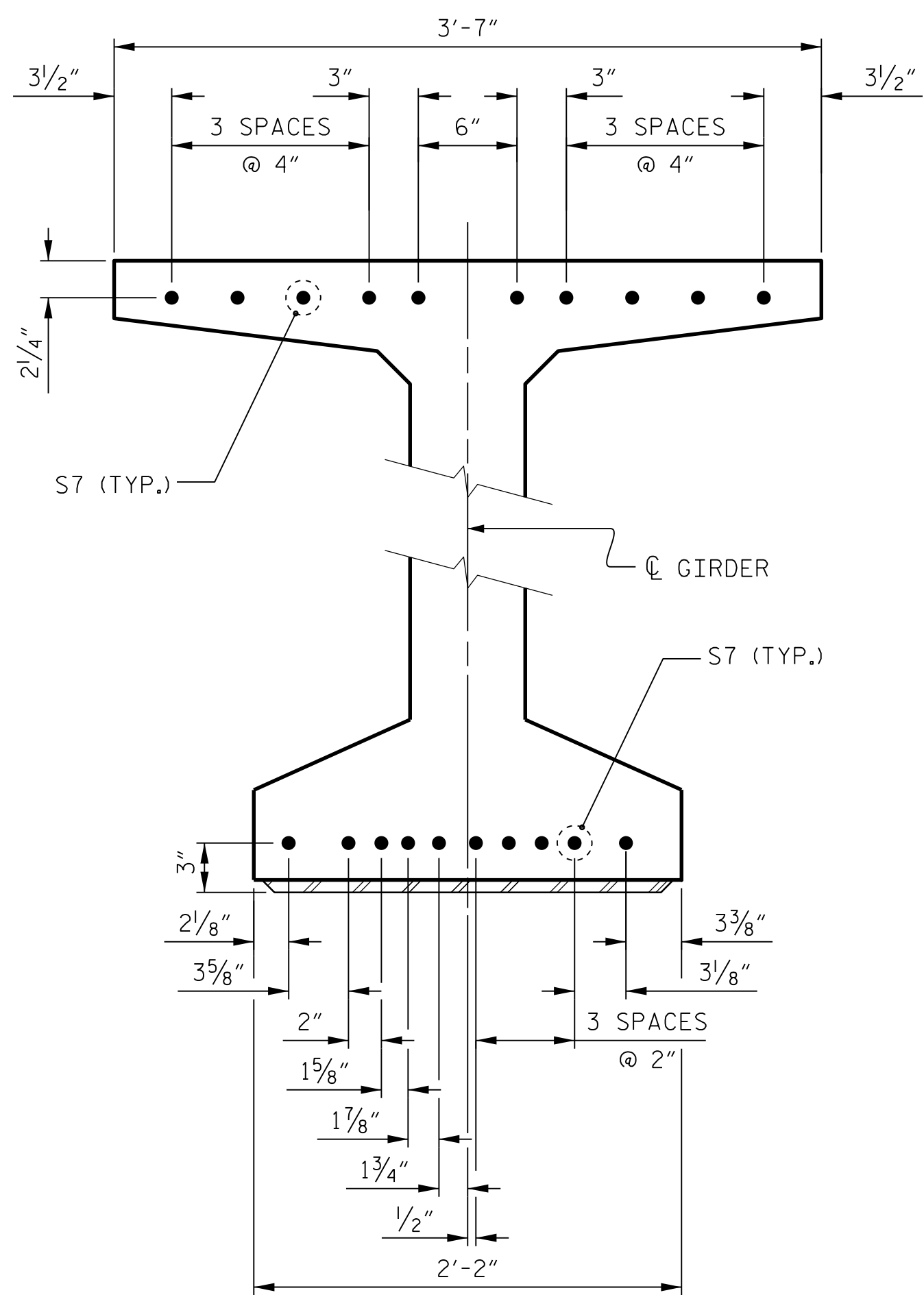


PLANS PREPARED BY:  
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 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

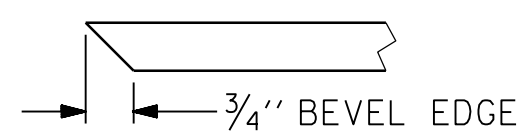
DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

ASSEMBLED BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DRAWN BY : EEM 2/6/97 REV. 10/1/11 MAA/GM  
 CHECKED BY : VAP 2/6/97 REV. 6/13 MAA/GM  
 DATE: 10/20/17 REV. 1/15 MAA/TMC

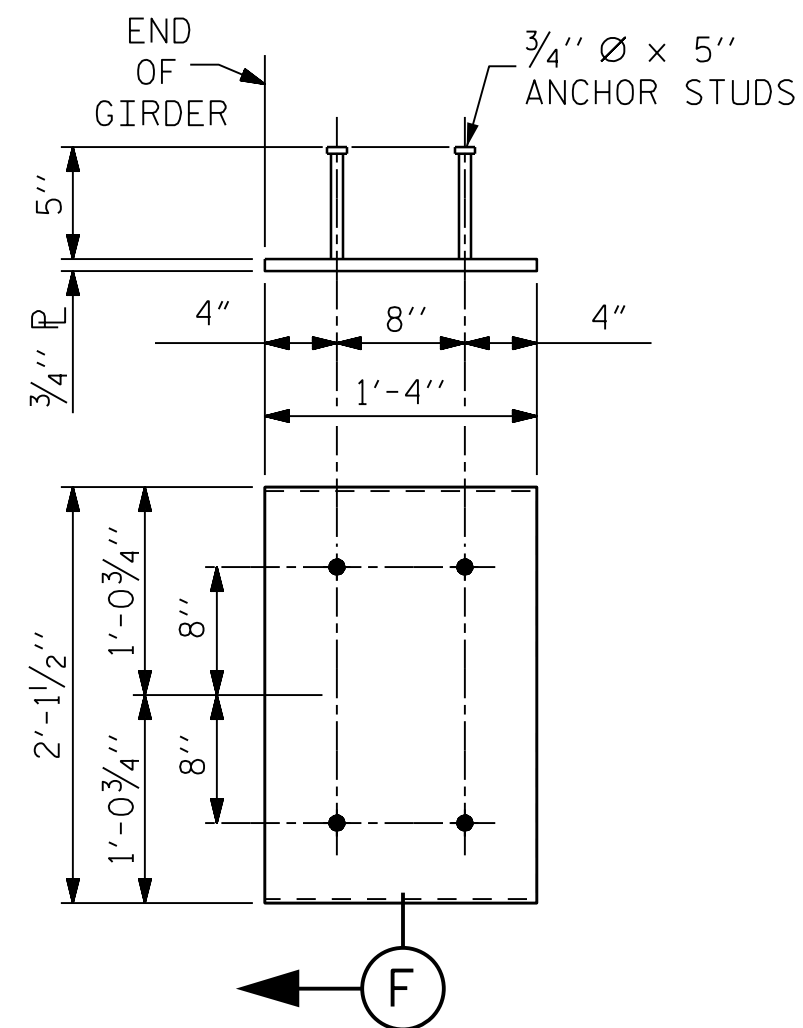




**DETAIL "C"**  
(FOR 63" MODIFIED BULB TEES)



**SECTION "F"**  
(SEE NOTES)



**EMBEDDED PLATE "B-1" DETAILS FOR 63" MODIFIED BULB TEES**  
(2 REQUIRED PER GIRDER)

**NOTES**

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5,800 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

A 2" x 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE OF THE 63" MODIFIED BULB TEES.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4,500 LBS.

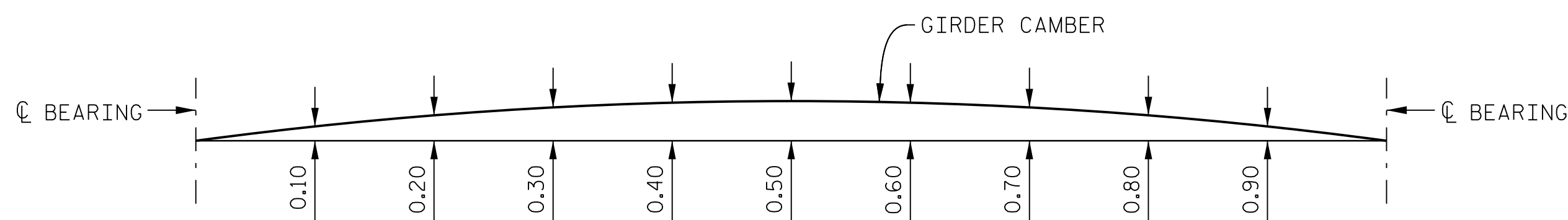
FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

**CAMBER AND DEAD LOAD DEFLECTIONS**

GIRDERS 1 THRU 4	SPAN A										
	CL BRG.	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	CL BRG.
CAMBER (GIRDER ALONE IN PLACE)	↑ 0.000	0.048	0.090	0.123	0.144	0.152	0.144	0.123	0.090	0.048	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. *	↑ 0.000	0.027	0.054	0.075	0.088	0.092	0.088	0.075	0.054	0.027	0.000
FINAL CAMBER	↑ 0	1/4"	7/16"	9/16"	11/16"	11/16"	11/16"	9/16"	7/16"	1/4"	0

\* INCLUDES FUTURE WEARING SURFACE

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).



**GIRDER CAMBER AND DEFLECTIONS**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

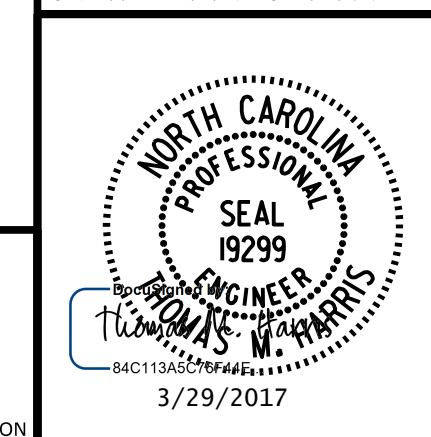
STANDARD

**63" PRESTRESSED CONCRETE MODIFIED BULB TEE GIRDER**

**(RIGHT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

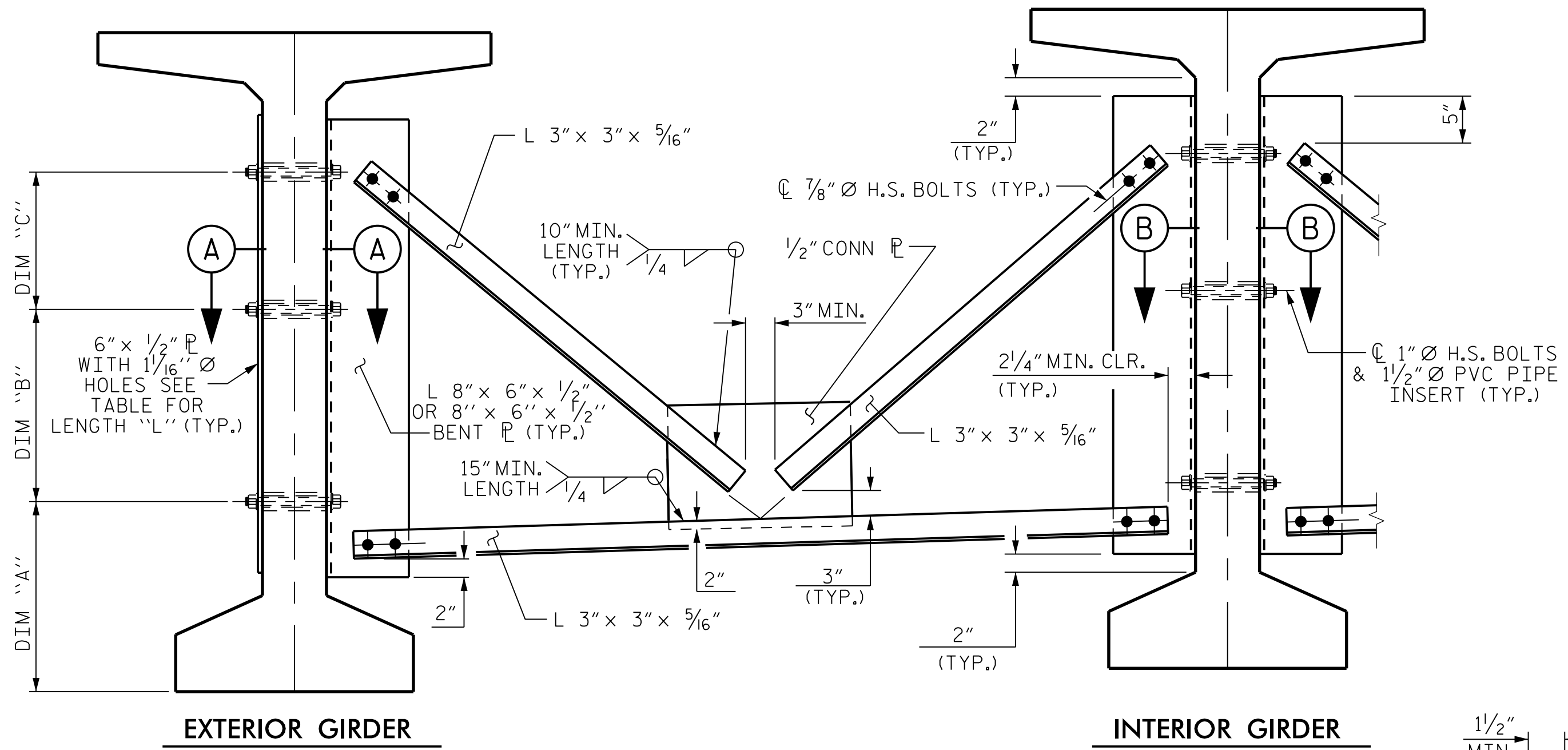
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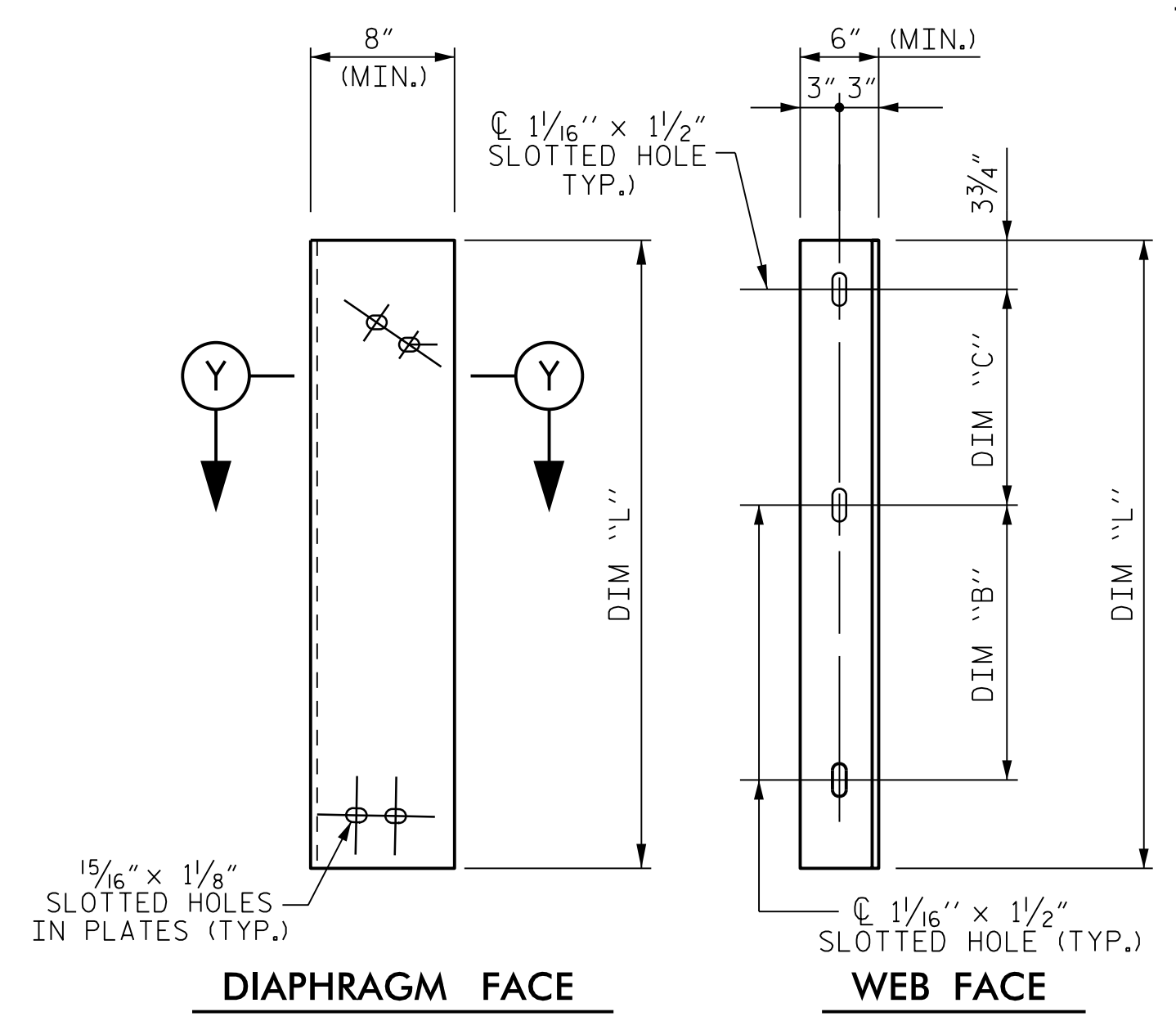
PLANS PREPARED BY:  
**PARSONS**  
5540 CenterView Drive, Suite 217  
Raleigh, NC 27606-3386  
NC LICENSE No. F-0246  
FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

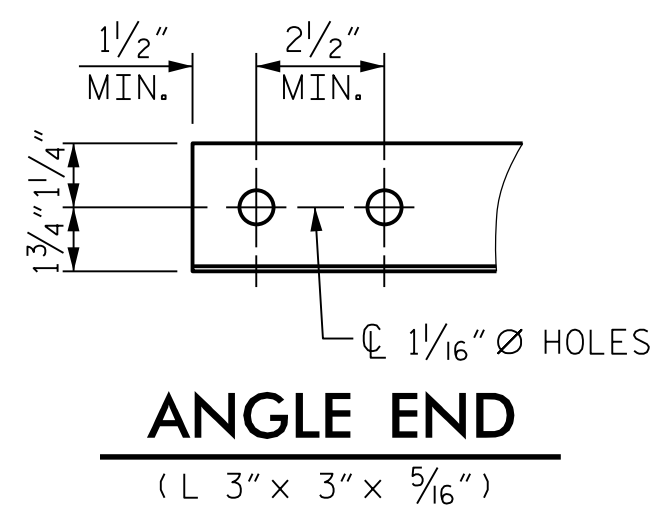
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CHECKED BY :	A. D. SHAH	DATE :	10-16
DRAWN BY :	ELR	11/91	REV. 10/11/11
CHECKED BY :	GRP	11/91	REV. 1/15
			REV. 2/15
			MAA/TMG
			MAA/TMG



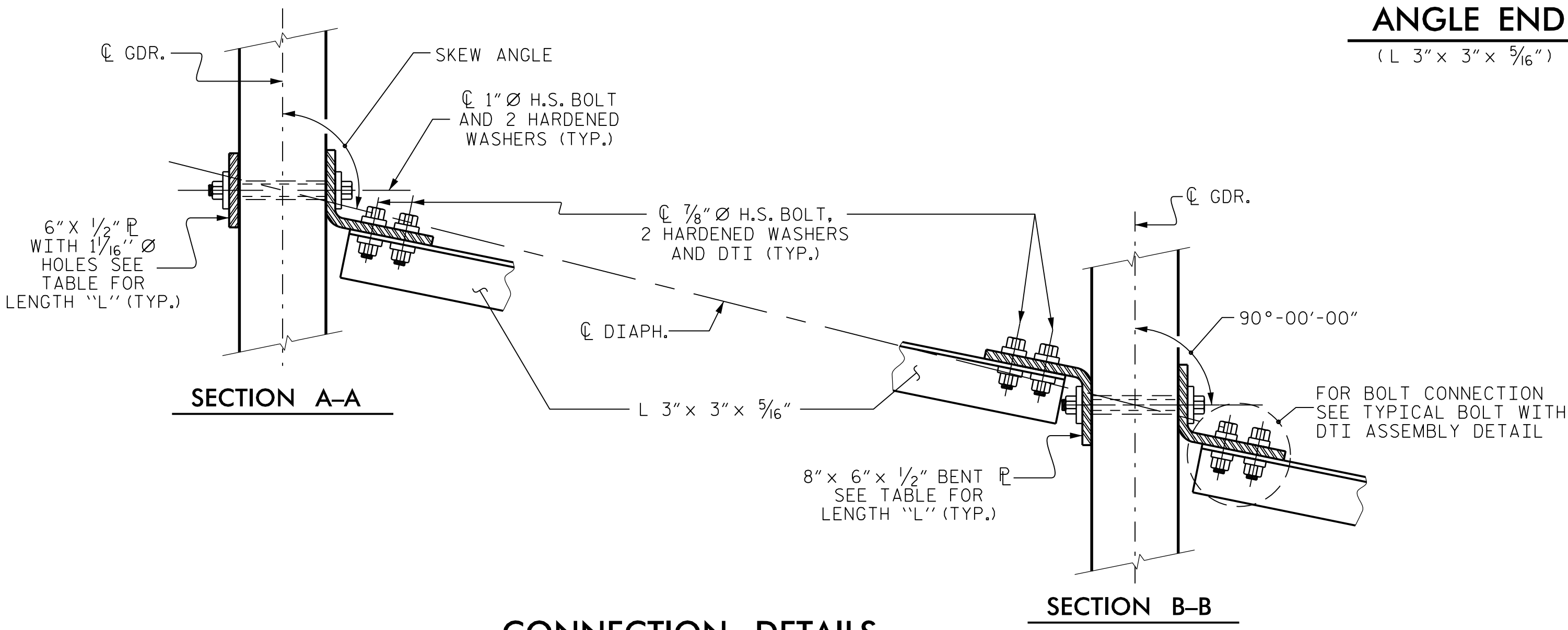
**PART SECTION AT INTERMEDIATE DIAPHRAGM**  
(63" BULB TEE)



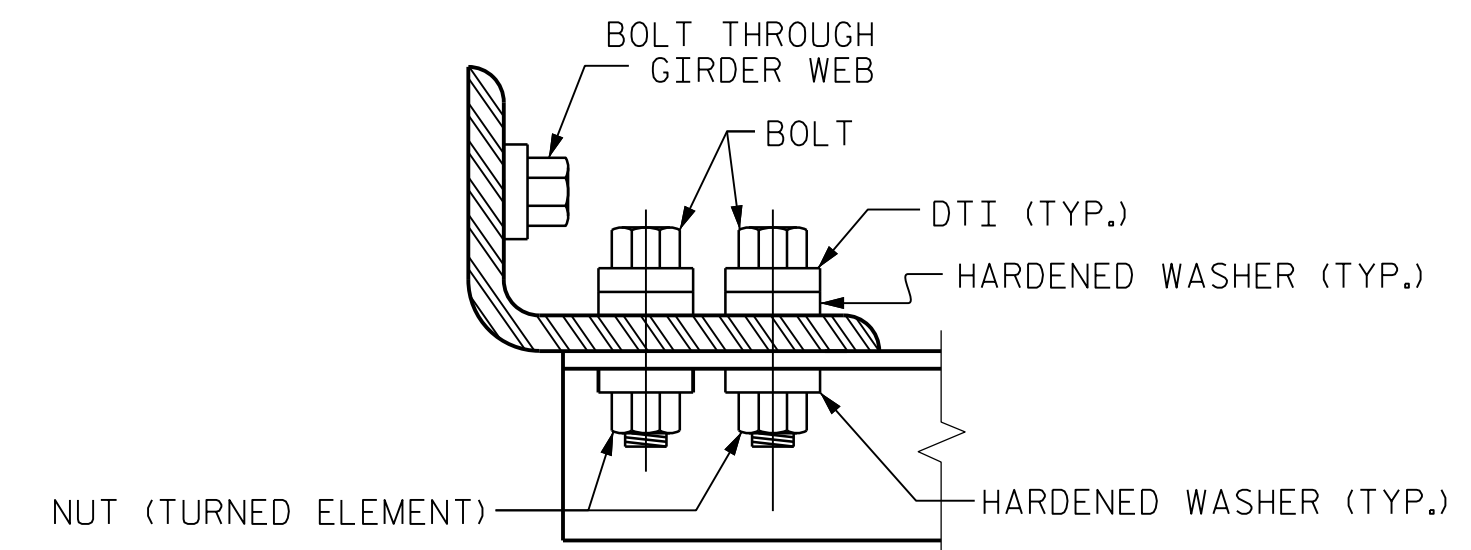
**CONNECTOR PLATE DETAILS**



**ANGLE END**  
(L 3" x 3" x 5/16")



**CONNECTION DETAILS**  
(90° < SKEW < 110°)



**BOLT WITH DTI ASSEMBLY DETAIL**

**STRUCTURAL STEEL NOTES**

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

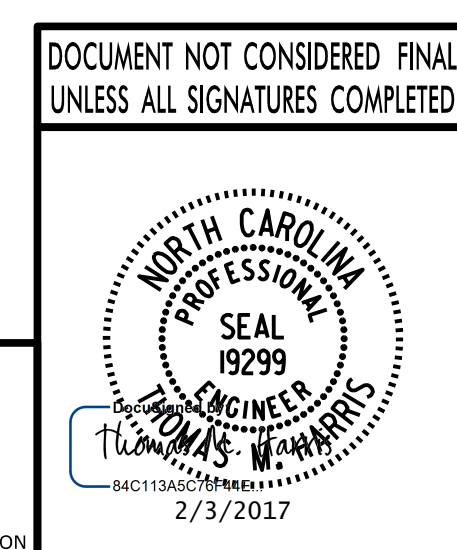
TABLE				
GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
63" BULB TEE	1'-9"	1'-5"	11 3/4"	3'-5"

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD  
**INTERMEDIATE STEEL DIAPHRAGMS  
 FOR 63" MODIFIED BULB TEE  
 PRESTRESSED CONCRETE GIRDERS  
 (RIGHT LANE)**

REVISIONS						SHEET No. S10-12
No.	BY:	DATE:	No.	BY:	DATE:	
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2			4			



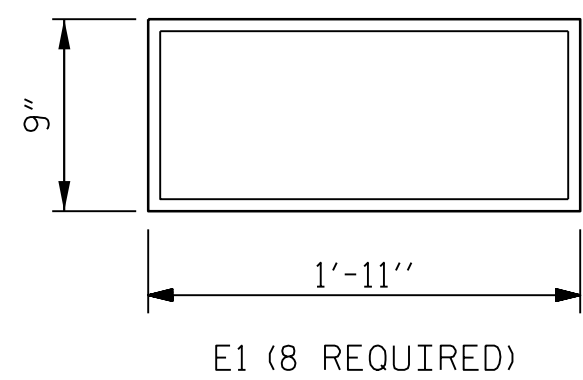
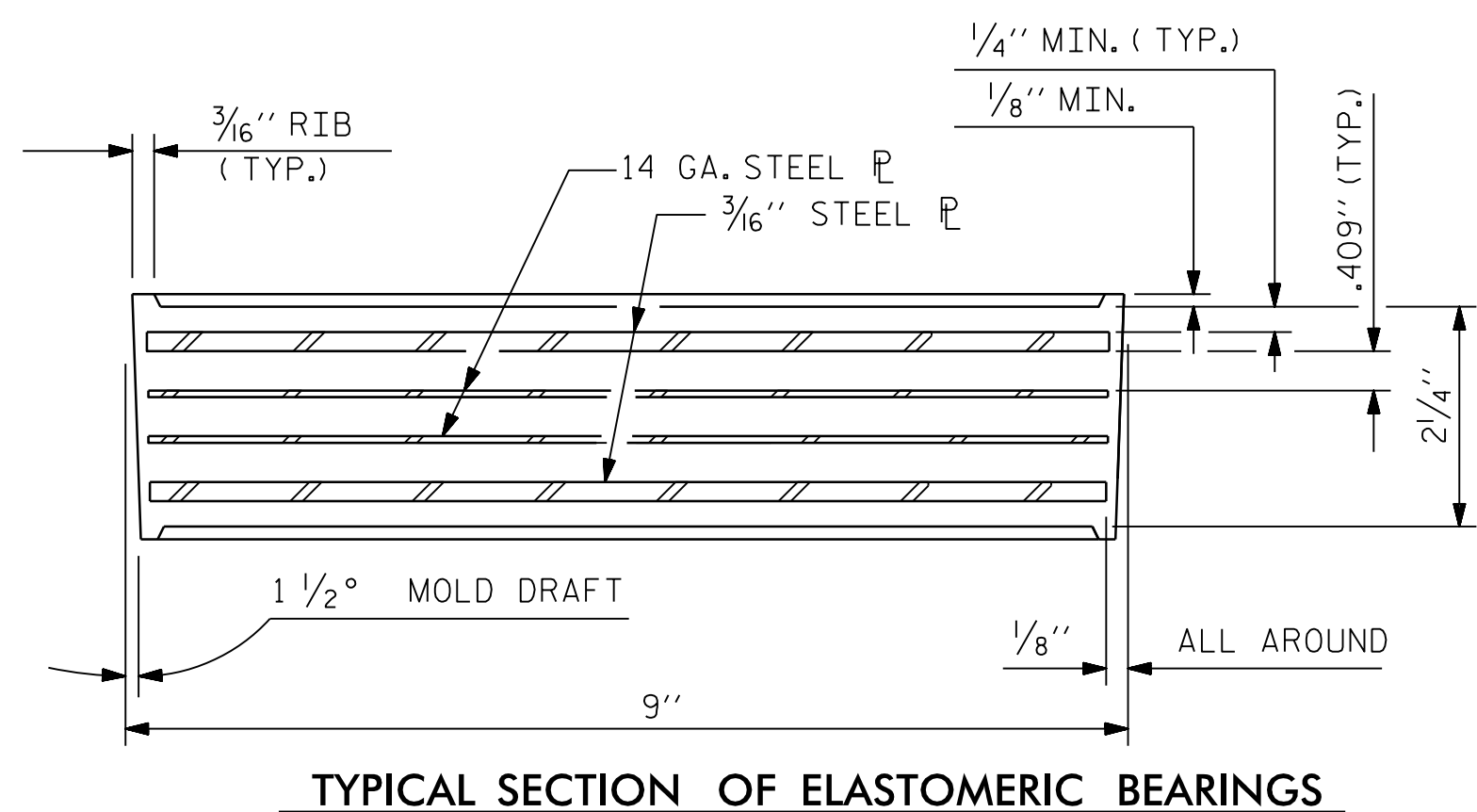
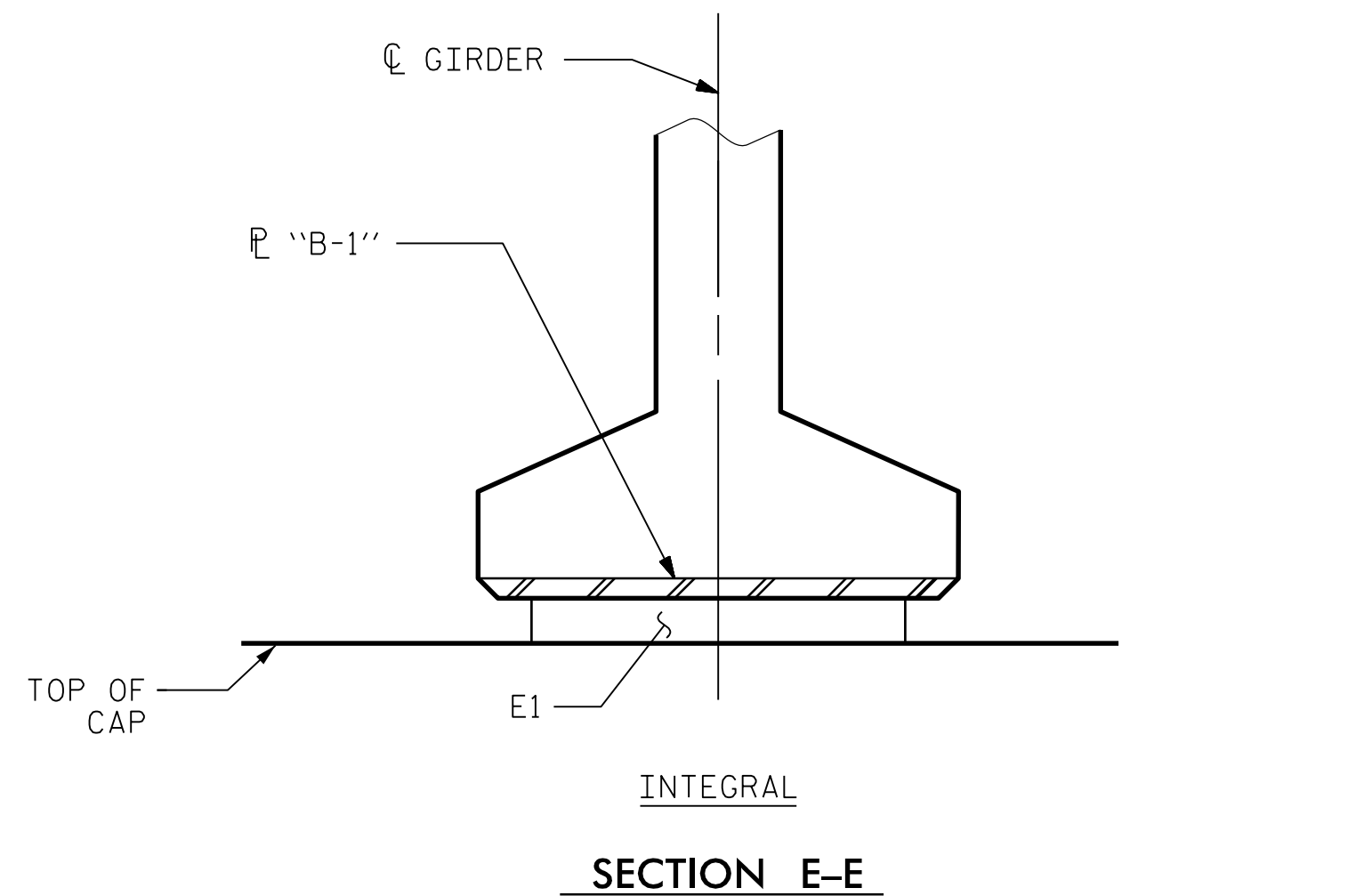
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

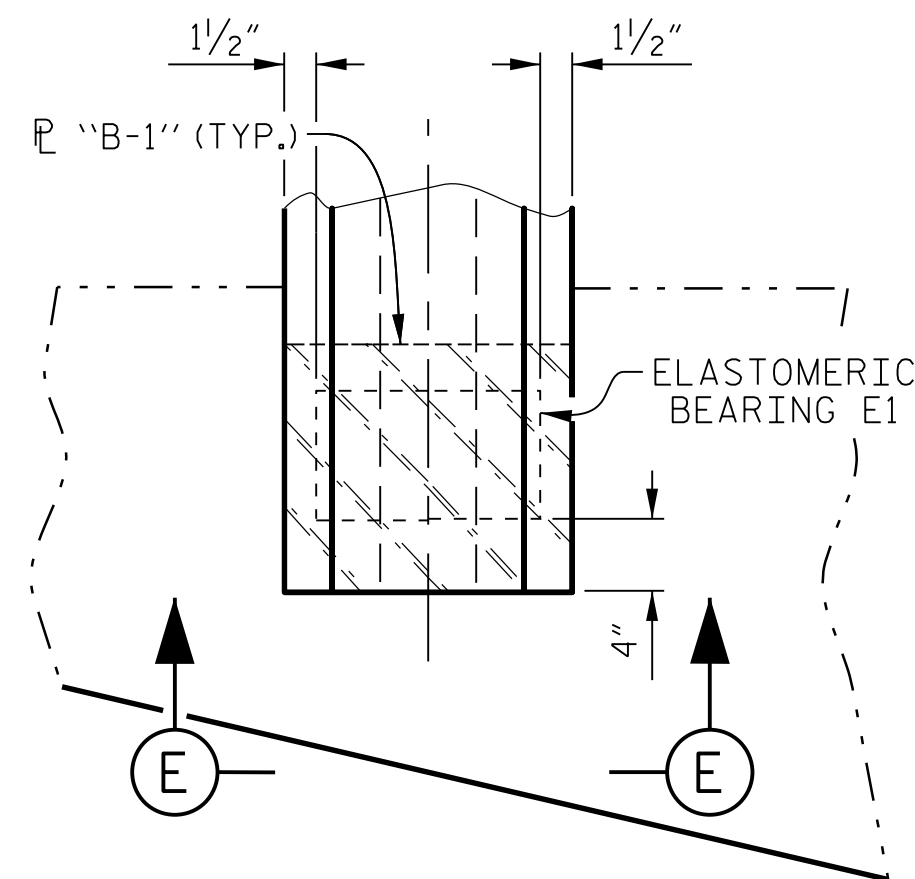
ASSEMBLED BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DRAWN BY : RWW 11/09  
 CHECKED BY : GM 11/09  
 ADDED 11/23/09  
 REV. 10/1/11 MAA/GM

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PLAN VIEW OF ELASTOMERIC BEARING  
TYPE V



PARTIAL PLAN - INTEGRAL END BENT

**NOTES**

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

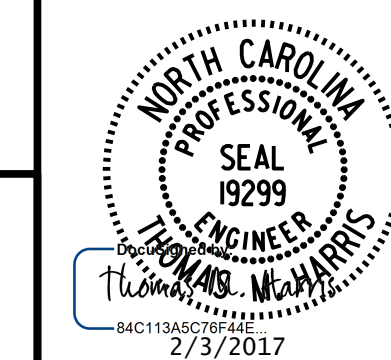
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

MAXIMUM ALLOWABLE SERVICE LOAD	
	D.L.+ L.L. (NO IMPACT)
TYPE V	365 K

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
**ELASTOMERIC BEARING DETAILS**  
**PRESTRESSED CONCRETE GIRDER**  
**SUPERSTRUCTURE**  
**(RIGHT LANE)**

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED



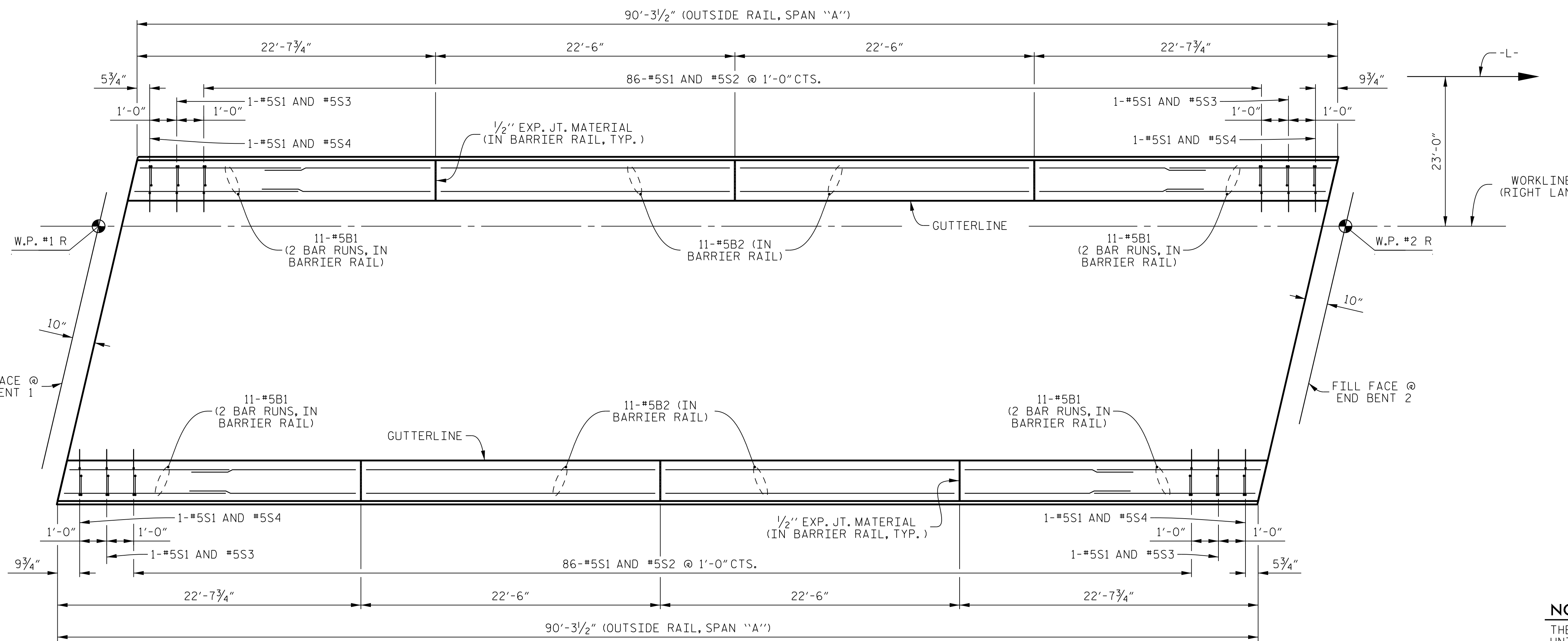
PLANS PREPARED BY :  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

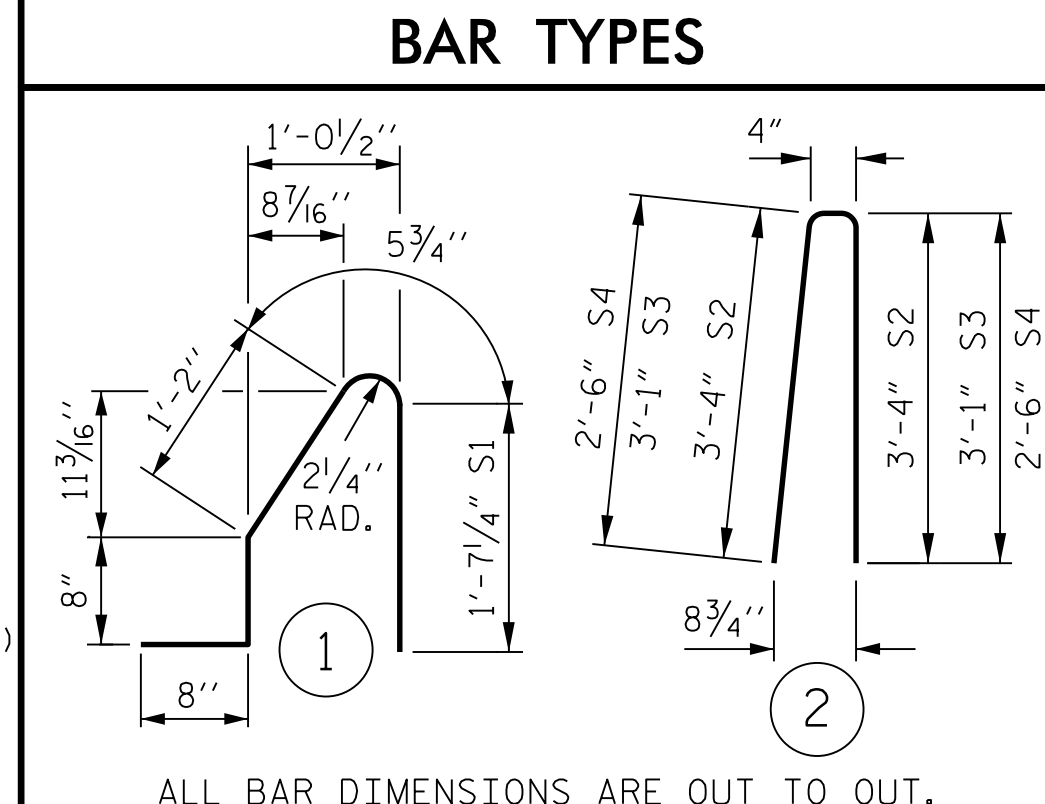
ASSEMBLED BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DRAWN BY : EEM 2/97 REV. 10/1/11 MAA/GM  
 CHECKED BY : VAP 2/97 REV. 6/13 AAC/MAA  
 DATE: 10/20/17 DATE: 12-08-21 MAA/TMG

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 DATE: 10/20/17 12:48:21 PM



SPLICE LENGTH		
BAR	SIZE	LENGTH
B1	#5	3'-5"

**PLAN OF BARRIER RAIL**



**BILL OF MATERIAL**

FOR CONCRETE BARRIER RAIL ONLY					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
* B1	88	#5	STR 12'-10"	1,178	
* B2	44	#5	STR 22'- 1"	1,013	
* S1	180	#5	1 4'- 7"	860	
* S2	172	#5	2 7'- 0"	1,256	
* S3	4	#5	2 6'- 6"	27	
* S4	4	#5	2 5'- 4"	22	
* EPOXY COATED REINFORCING STEEL				4,356	LBS.
CLASS "AA" CONCRETE				24.5	CU. YDS.
CONCRETE BARRIER RAIL 180.58 LIN. FT.					

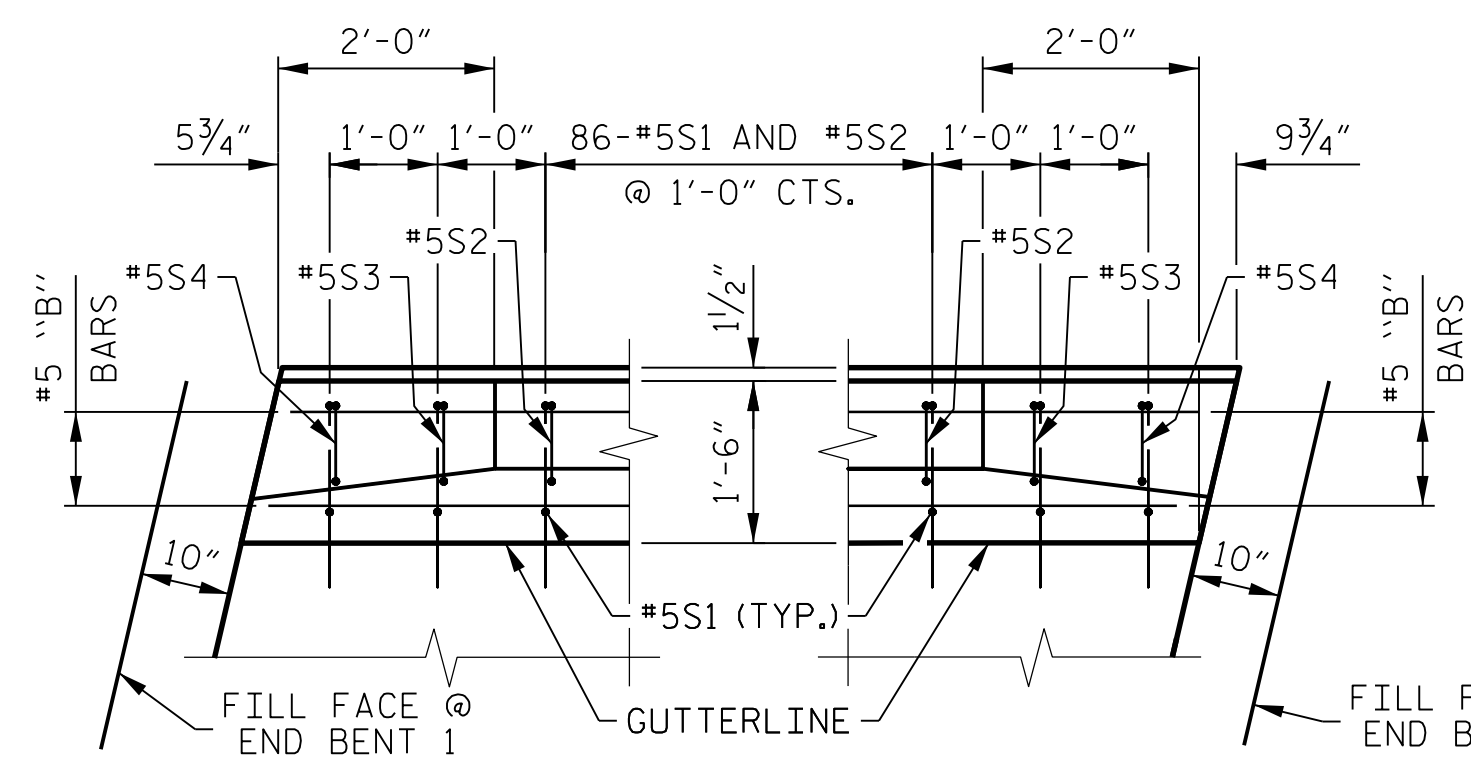
**NOTES**

THE BARRIER RAIL IN SPAN "A" SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

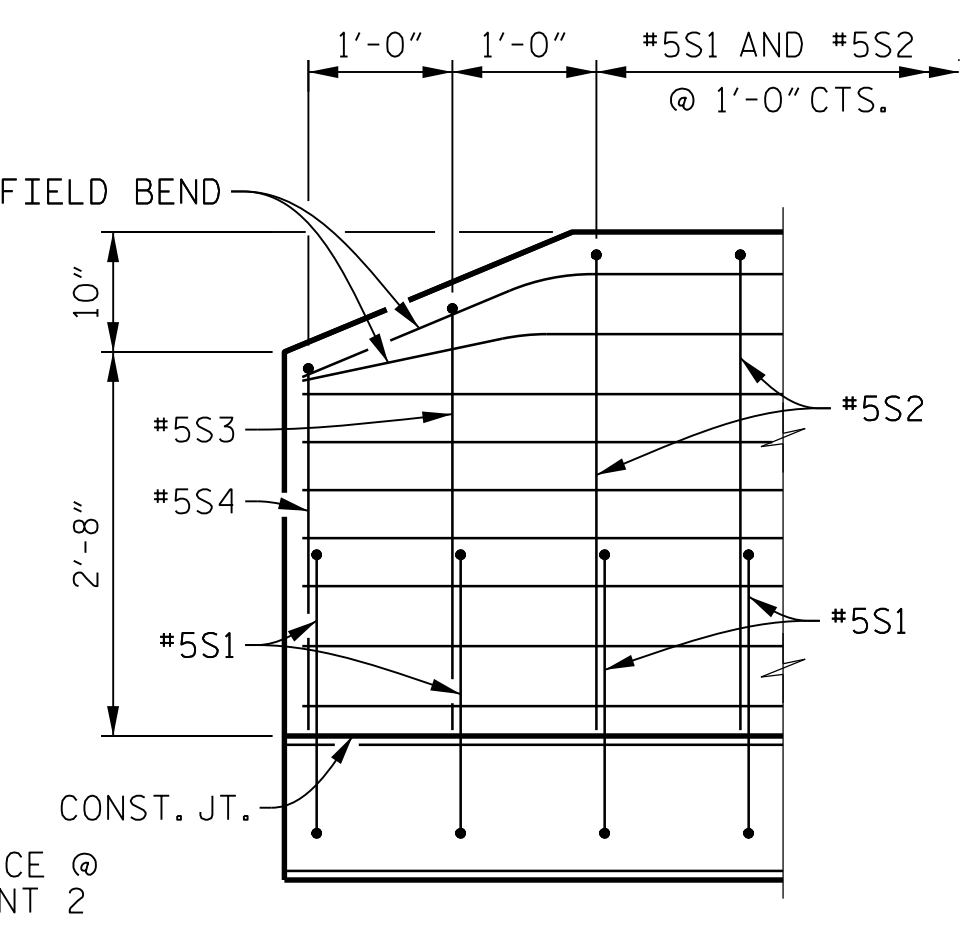
ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

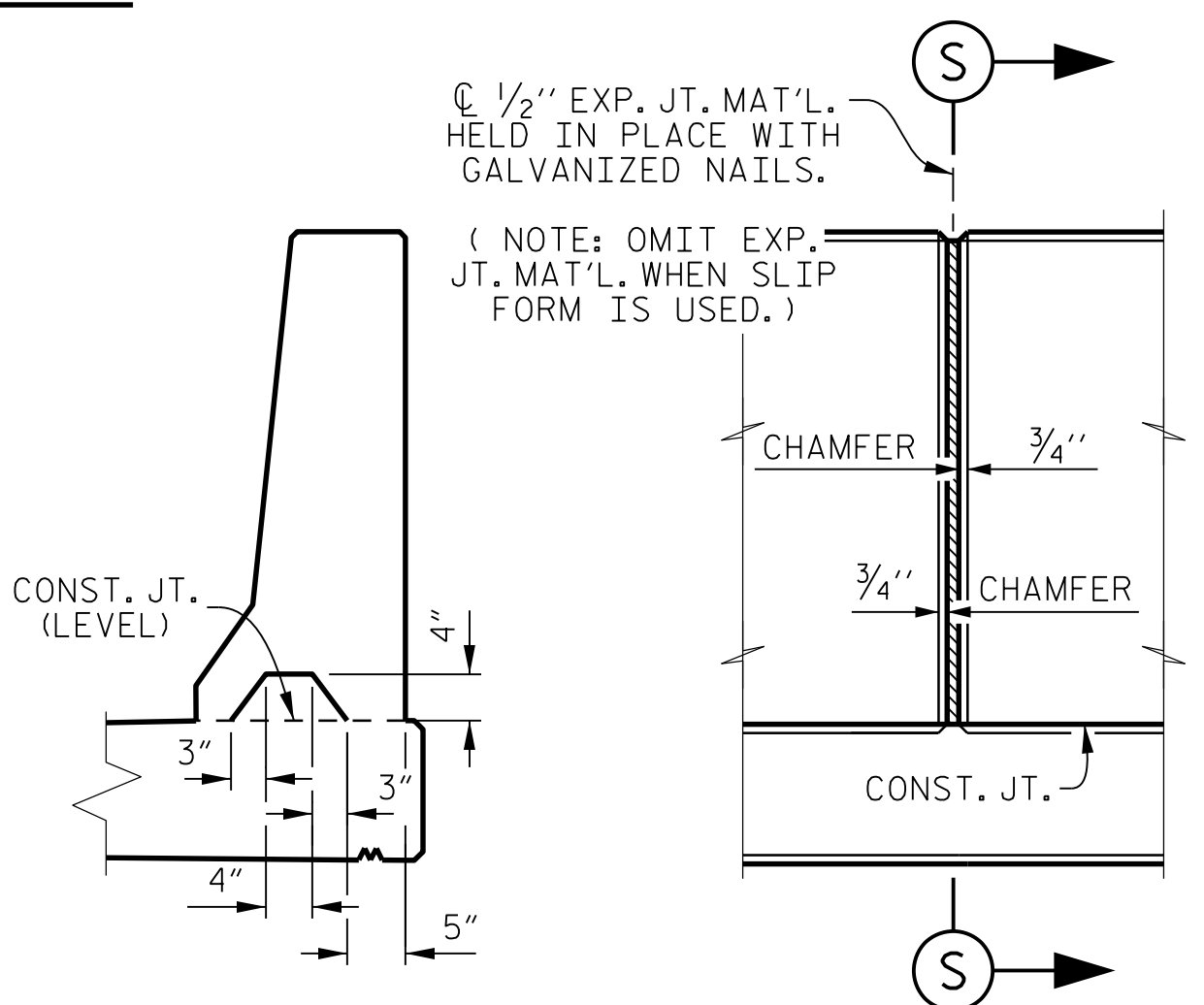
THE #5S1 AND #5S2 BARS MAY BE SHIFTED AS NECESSARY TO CLEAR EXPANSION JOINTS IN RAIL.



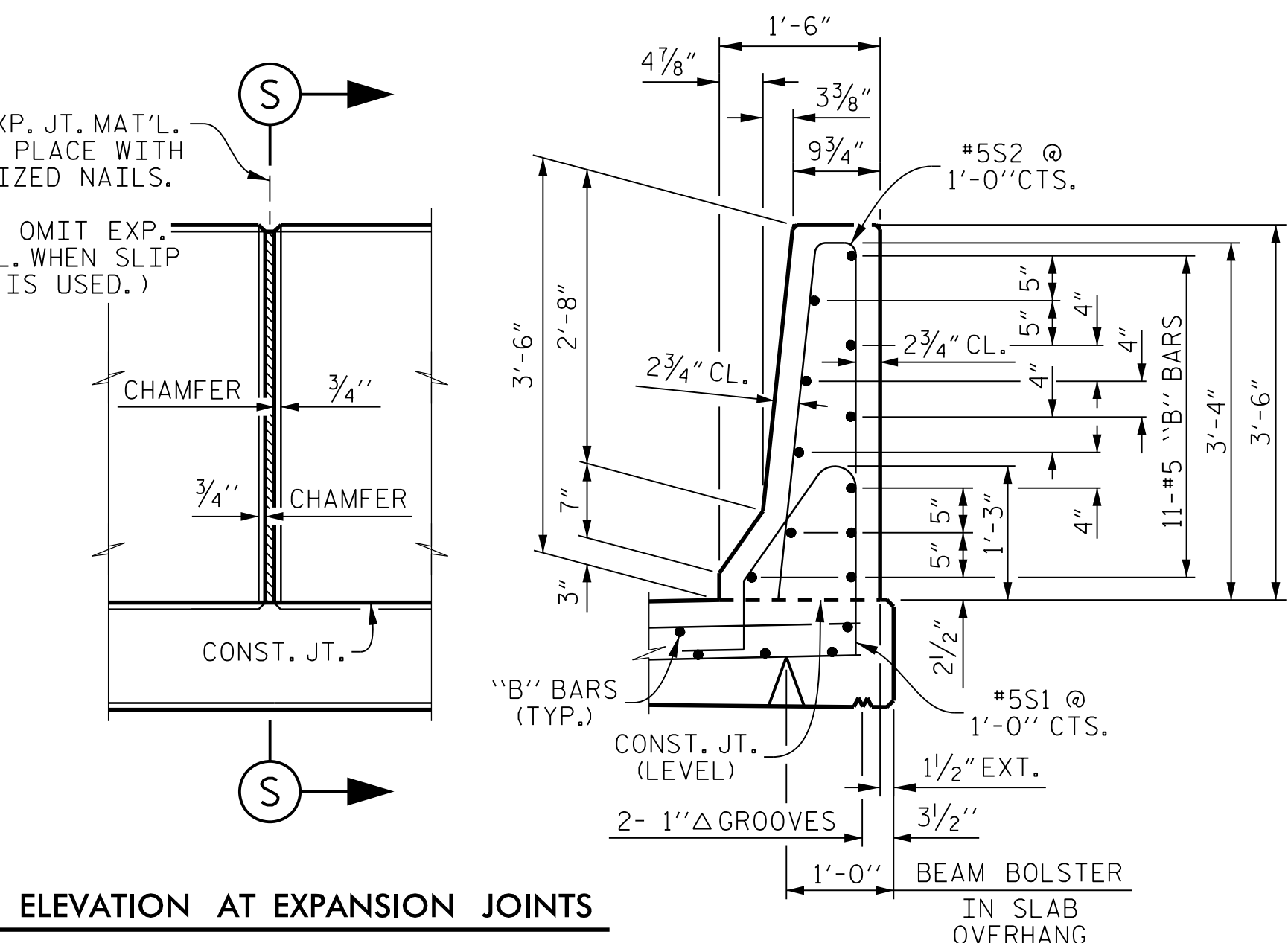
**END OF RAIL DETAILS**



**ELEVATION VIEW**



**SECTION S-S**  
AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)



**ELEVATION AT EXPANSION JOINTS**

**SECTION THRU RAIL**

ASSEMBLED BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DRAWN BY :	ARB 5/87	REV. 10/1/11	MAA/GM
CHECKED BY :	SJD 9/87	REV. 7/12	MAA/GM
		REV. 6/13	MAA/GM

DRAWN BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

PLANS PREPARED BY :  
**PARSONS**  
5540 CenterView Drive, Suite 217  
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NC LICENSE No. F-0246

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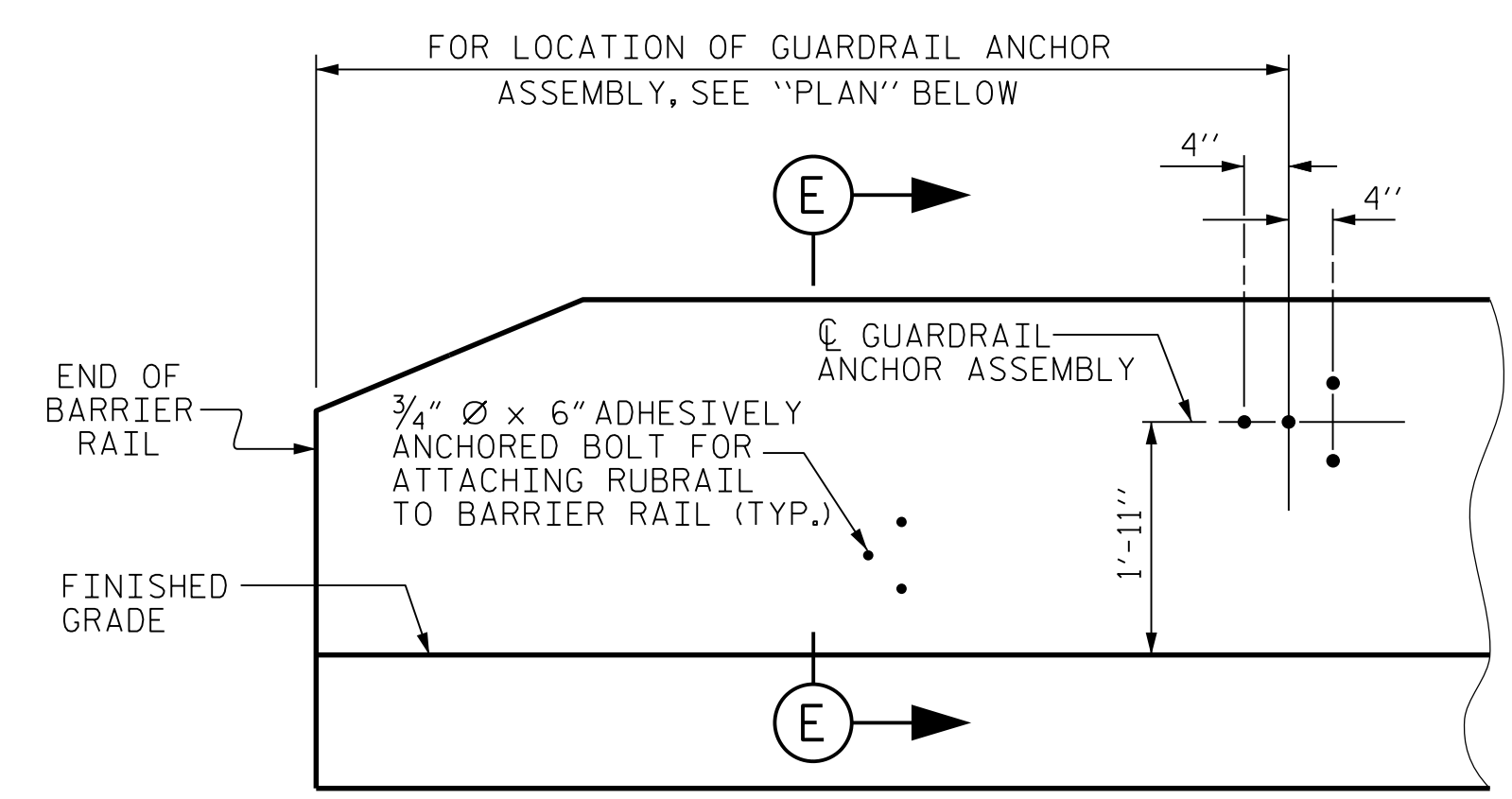
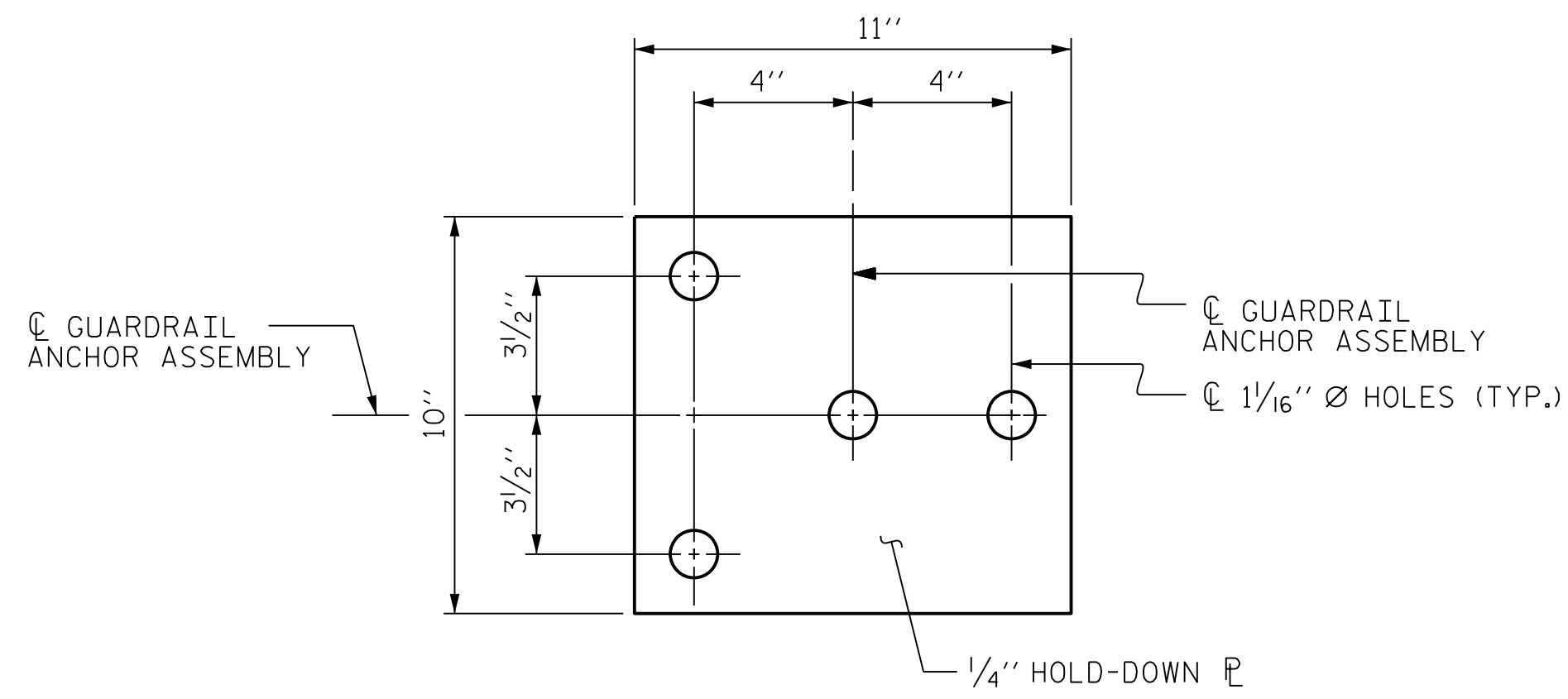
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
STATION: **611+32.01 -L-**

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

**STANDARD CONCRETE BARRIER RAIL (RIGHT LANE)**

REVISIONS						SHEET No.	
No.	BY:	DATE:	No.	BY:	DATE:	ST10-14	
1			3			TOTAL SHEETS	
2			4			25	





**NOTES**

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

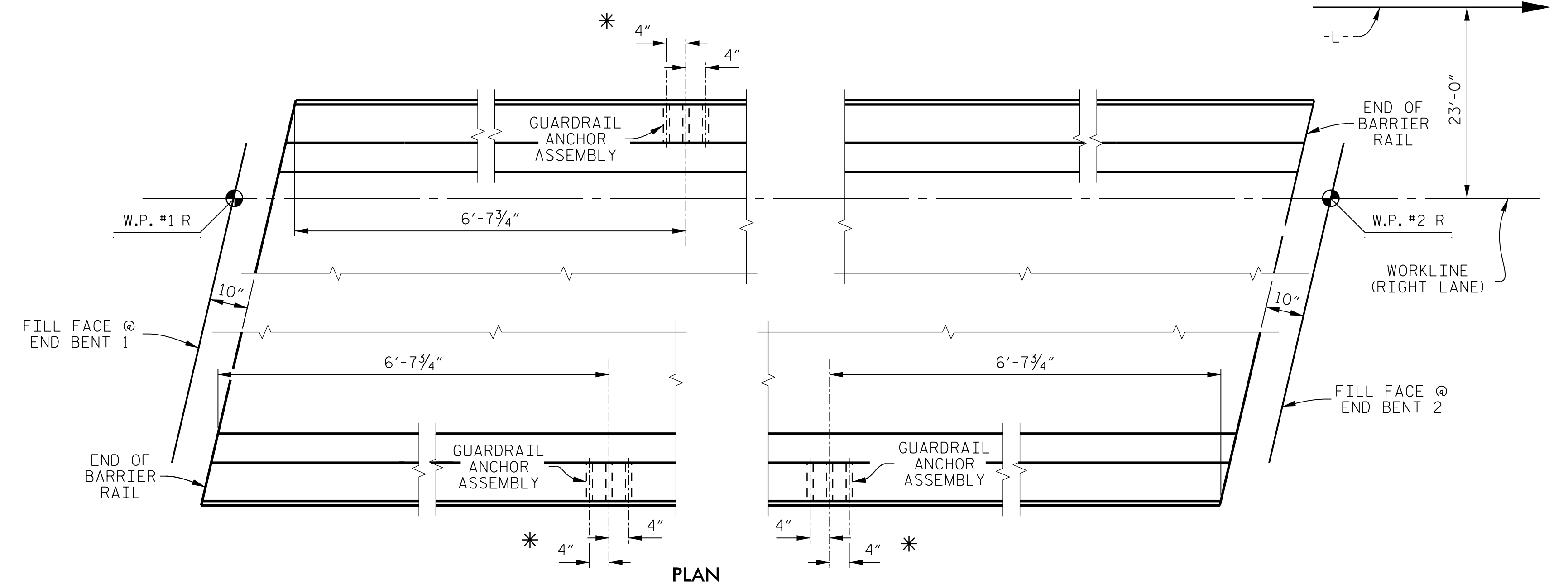
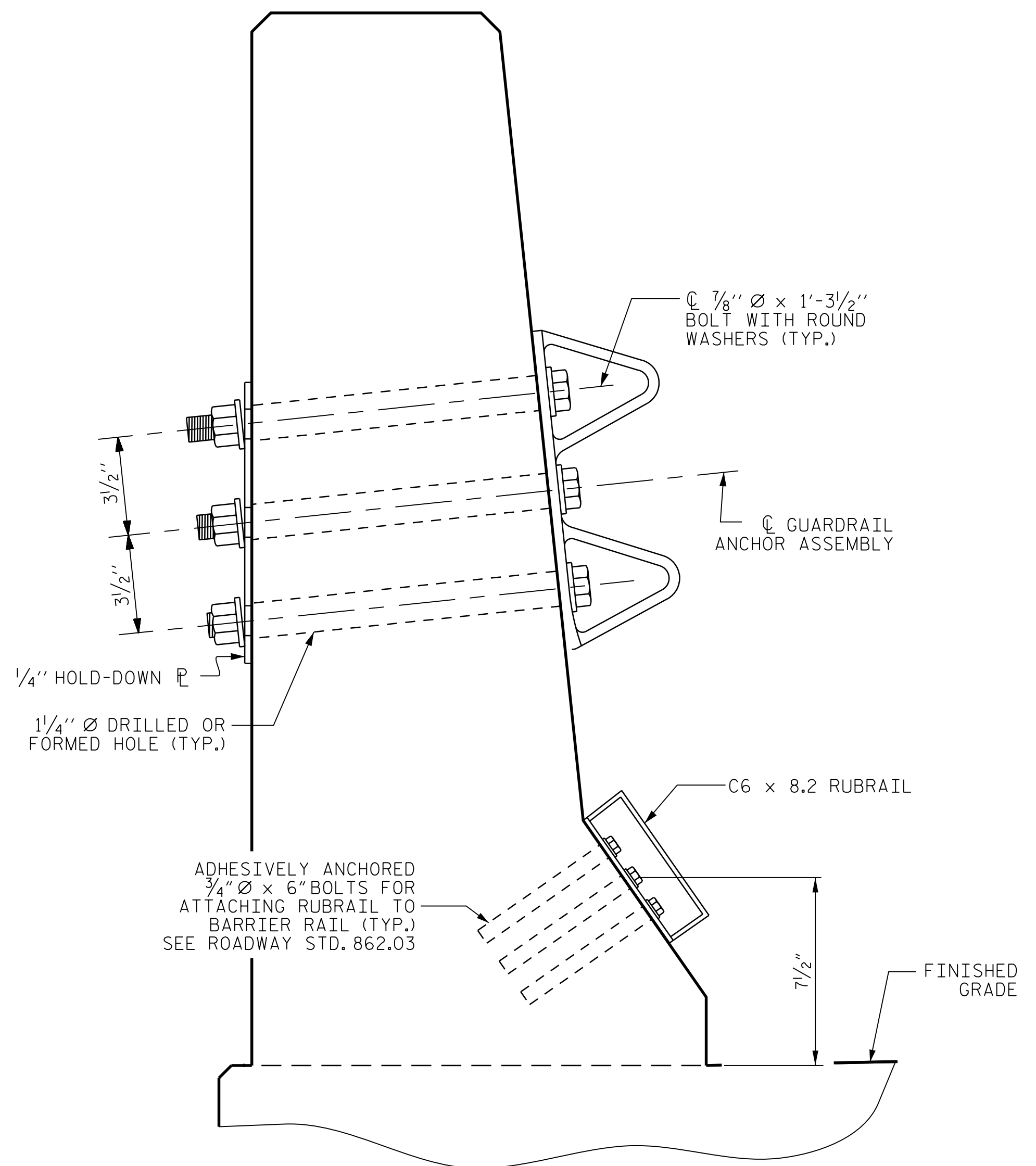
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

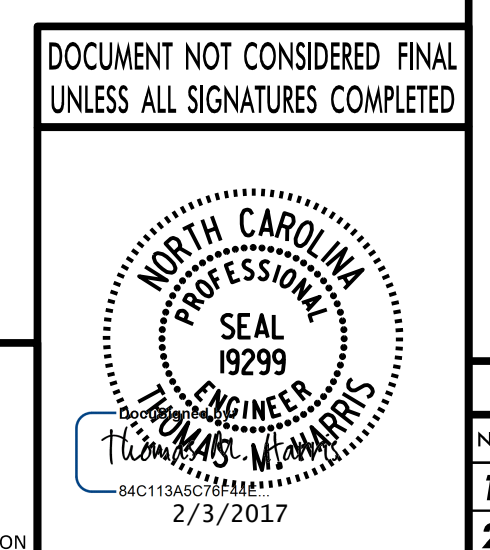
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 x 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø x 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



PROJECT NO. R-2707C  
 CLEVELAND COUNTY  
 STATION: 611+32.01 -L-



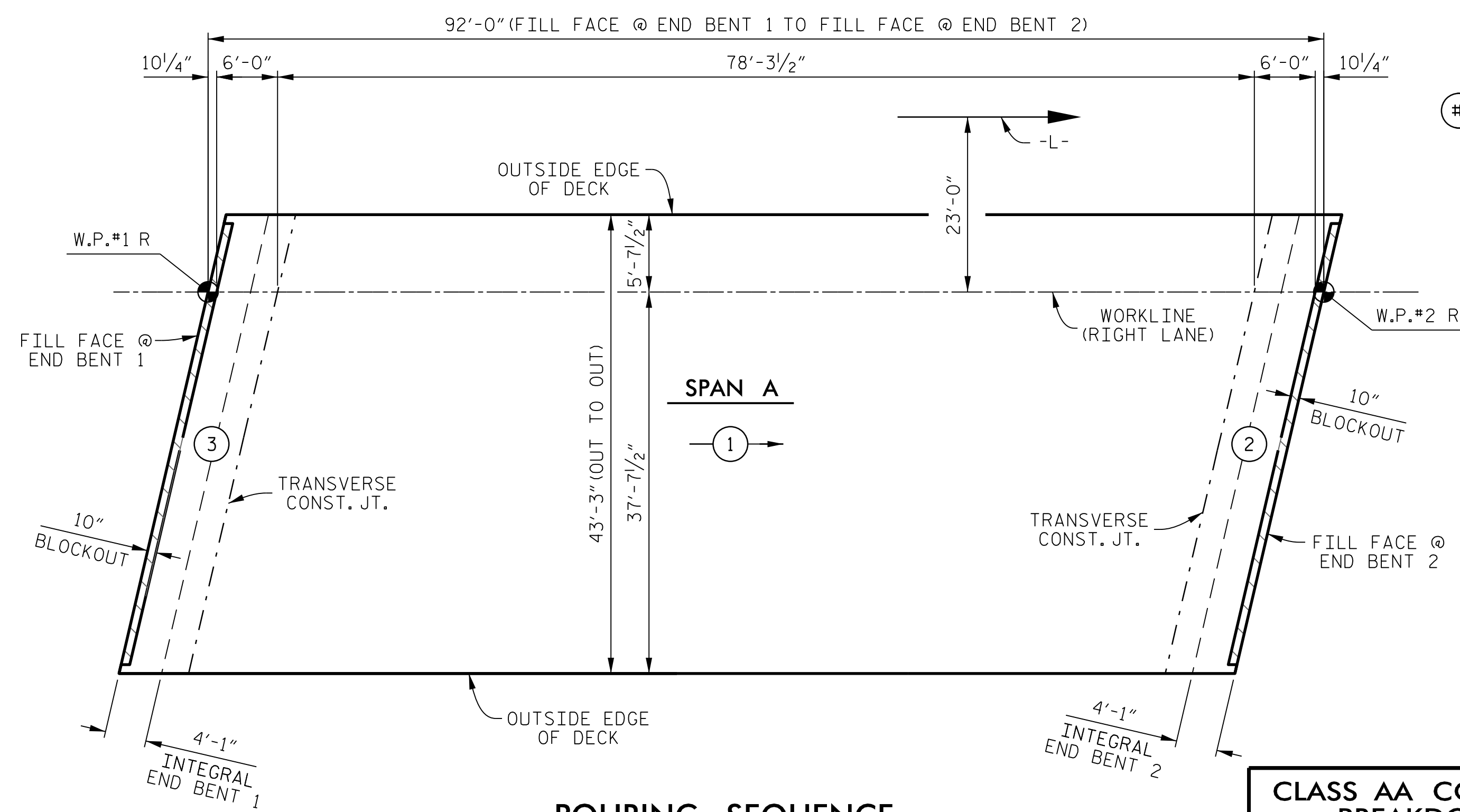
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
 GUARDRAIL ANCHORAGE  
 FOR BARRIER RAIL  
 (RIGHT LANE)

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

PLANS PREPARED BY :  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
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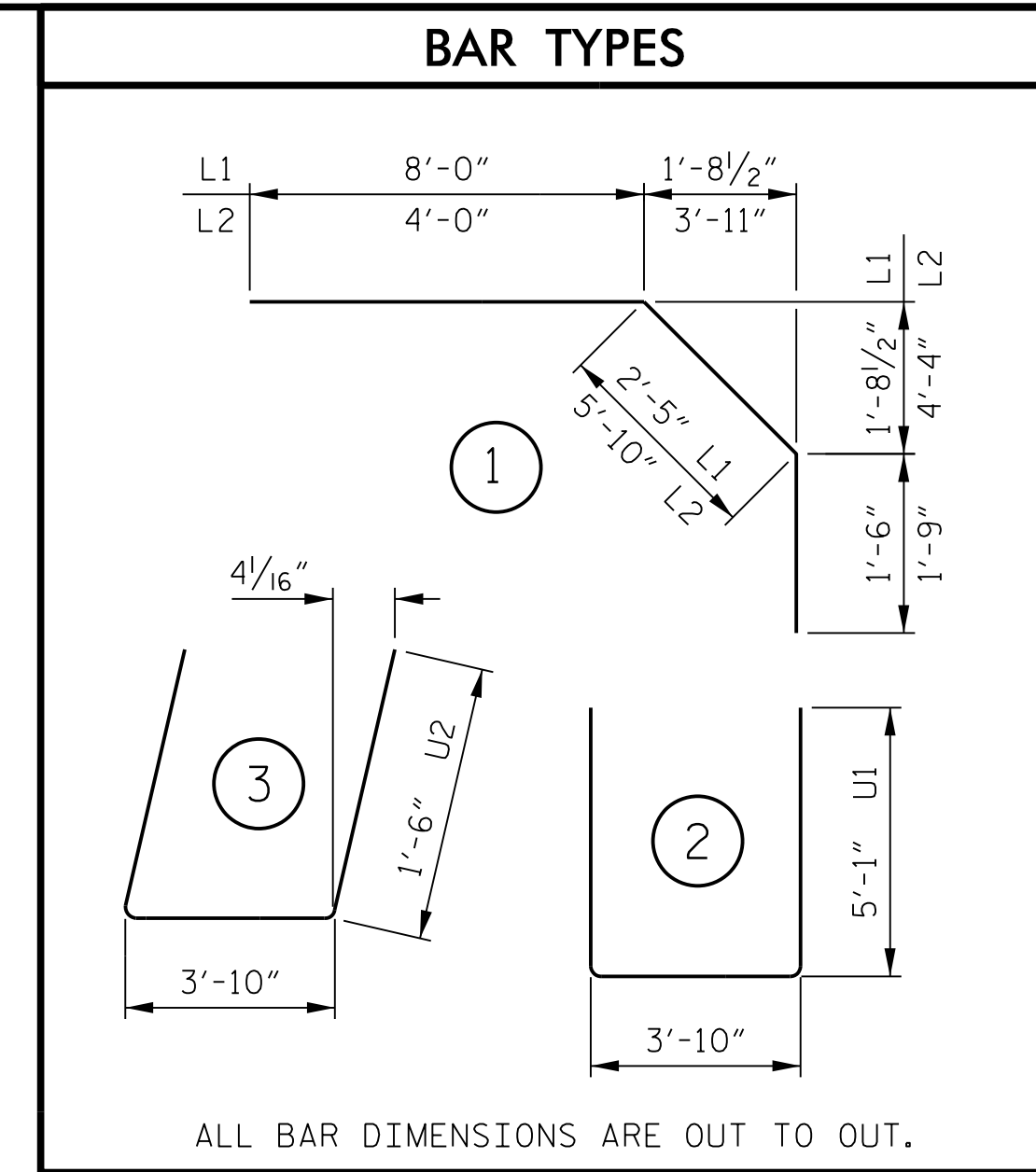
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**POURING SEQUENCE**

CLASS AA CONCRETE BREAKDOWN	
POUR 1	116.9 CU. YDS.
POUR 2	44.1 CU. YDS.
POUR 3	44.1 CU. YDS.
<b>TOTAL</b>	<b>205.1 CU. YDS.</b>

**NOTES**  
 POUR 2 AND POUR 3 MAY BE COMBINED.  
 DIRECTION OF POUR 1 MAY BE REVERSED.  
 # INDICATES POUR NUMBER AND DIRECTION OF POUR.

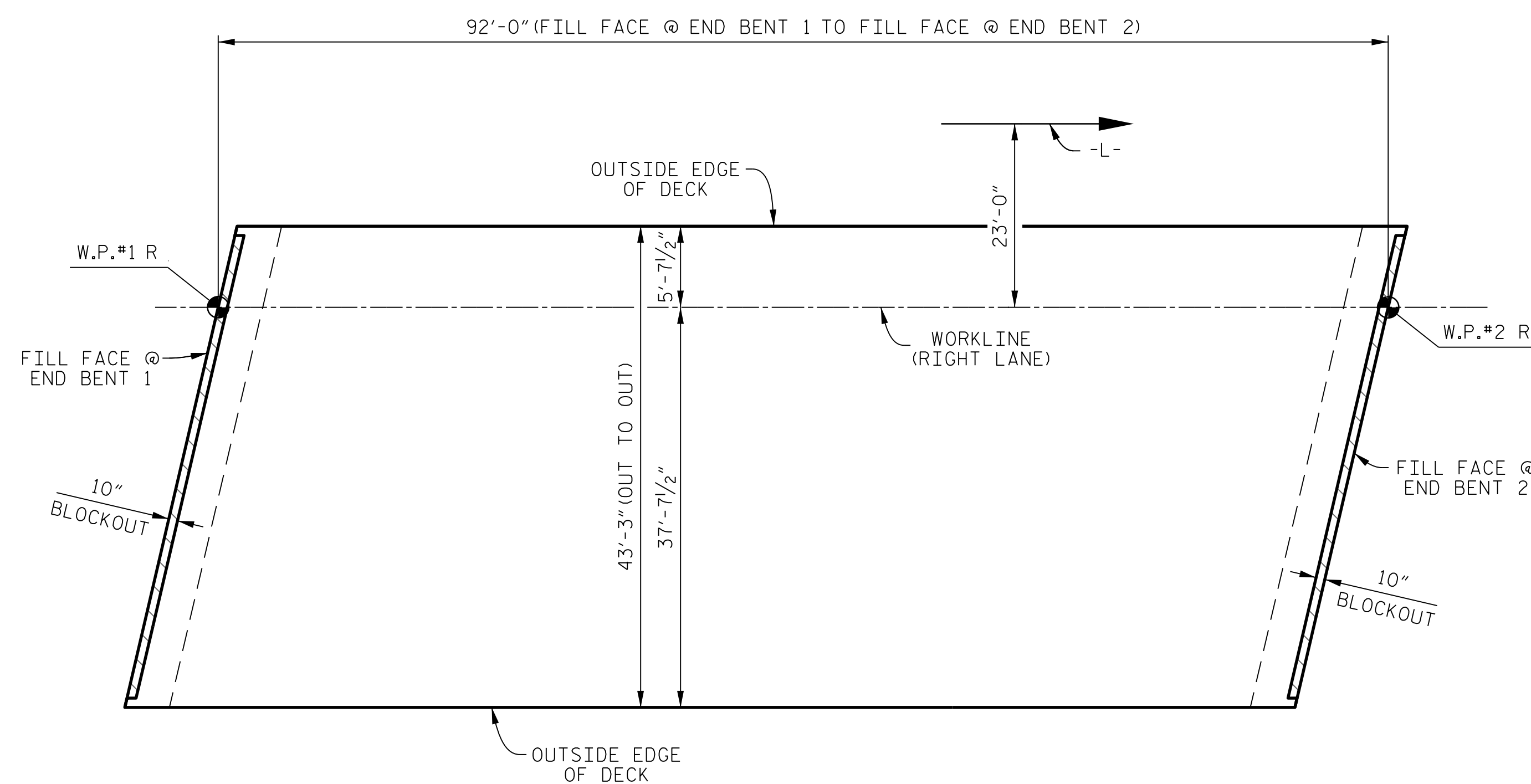


BILL OF MATERIAL					
SPAN A					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
*A1	161	#5	STR 42'-11"	7,207	
*A101	2	#5	STR 40'-9"	85	
*A102	2	#5	STR 38'-7"	80	
*A103	2	#5	STR 36'-5"	76	
*A104	2	#5	STR 34'-3"	71	
*A105	2	#5	STR 32'-2"	67	
*A106	2	#5	STR 30'-0"	63	
*A107	2	#5	STR 27'-10"	58	
*A108	2	#5	STR 25'-8"	54	
*A109	2	#5	STR 23'-6"	49	
*A110	2	#5	STR 21'-4"	45	
*A111	2	#5	STR 19'-2"	40	
*A112	2	#5	STR 17'-0"	35	
*A113	2	#5	STR 14'-10"	31	
*A114	2	#5	STR 12'-8"	26	
*A115	2	#5	STR 10'-6"	22	
*A116	2	#5	STR 8'-4"	17	
*A117	2	#5	STR 6'-2"	13	
*A118	2	#5	STR 4'-0"	8	
A2	161	#5	STR 42'-11"	7,207	
A201	2	#5	STR 40'-9"	85	
A202	2	#5	STR 38'-7"	80	
A203	2	#5	STR 36'-5"	76	
A204	2	#5	STR 34'-3"	71	
A205	2	#5	STR 32'-2"	67	
A206	2	#5	STR 30'-0"	63	
A207	2	#5	STR 27'-10"	58	
A208	2	#5	STR 25'-8"	54	
A209	2	#5	STR 23'-6"	49	
A210	2	#5	STR 21'-4"	45	
A211	2	#5	STR 19'-2"	40	
A212	2	#5	STR 17'-0"	35	
A213	2	#5	STR 14'-10"	31	
A214	2	#5	STR 12'-8"	26	
A215	2	#5	STR 10'-6"	22	
A216	2	#5	STR 8'-4"	17	
A217	2	#5	STR 6'-2"	13	
A218	2	#5	STR 4'-0"	8	
*B1	120	#4	STR 24'-0"	1,924	
*B2	162	#5	STR 18'-5"	3,112	
B3	114	#5	STR 46'-1"	5,479	
K1	28	#4	STR 22'-11"	429	
K2	6	#4	STR 7'-10"	31	
K3	30	#4	STR 10'-9"	215	
K4	6	#4	STR 9'-4"	37	
K5	4	#4	STR 2'-2"	6	
K6	20	#4	STR 3'-7"	48	
K7	4	#4	STR 2'-11"	8	
*L1	60	#4	1 11'-11"	478	
*L2	56	#4	1 11'-7"	433	
U1	60	#4	2 14'-0"	561	
U2	28	#4	3 6'-10"	128	
REINFORCING STEEL				14,989	LBS.
*EPOXY COATED REINFORCING STEEL				13,994	LBS.

TOTAL SUPERSTRUCTURE QUANTITIES			
	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL	CLASS "AA" CONCRETE
	LBS.	LBS.	CU. YDS.
SPAN A	14,989	13,994	205.1
<b>** TOTAL</b>	<b>14,989</b>	<b>13,994</b>	<b>205.1</b>

\*\* QUANTITIES FOR CONCRETE BARRIER RAIL ARE NOT INCLUDED. SEE "CONCRETE BARRIER RAIL" SHEET FOR DETAILS.

GROOVING BRIDGE FLOORS	
APPROACH SLAB AT END BENT 1	1,089 SQ. FT.
BRIDGE DECK	3,327 SQ. FT.
APPROACH SLAB AT END BENT 2	1,089 SQ. FT.
<b>TOTAL</b>	<b>5,505 SQ. FT.</b>



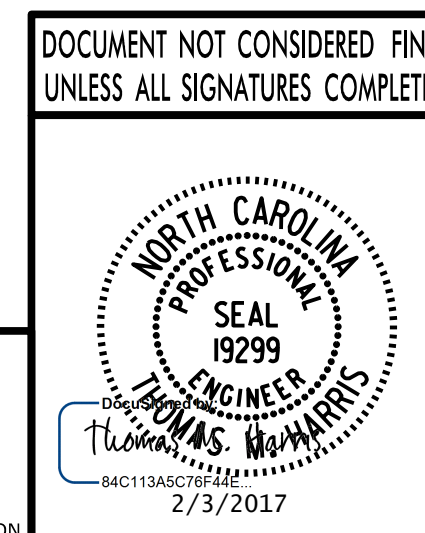
**LAYOUT FOR COMPUTING AREA OF REINFORCED CONCRETE DECK SLAB**

(3,979 SQ. FT.)

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

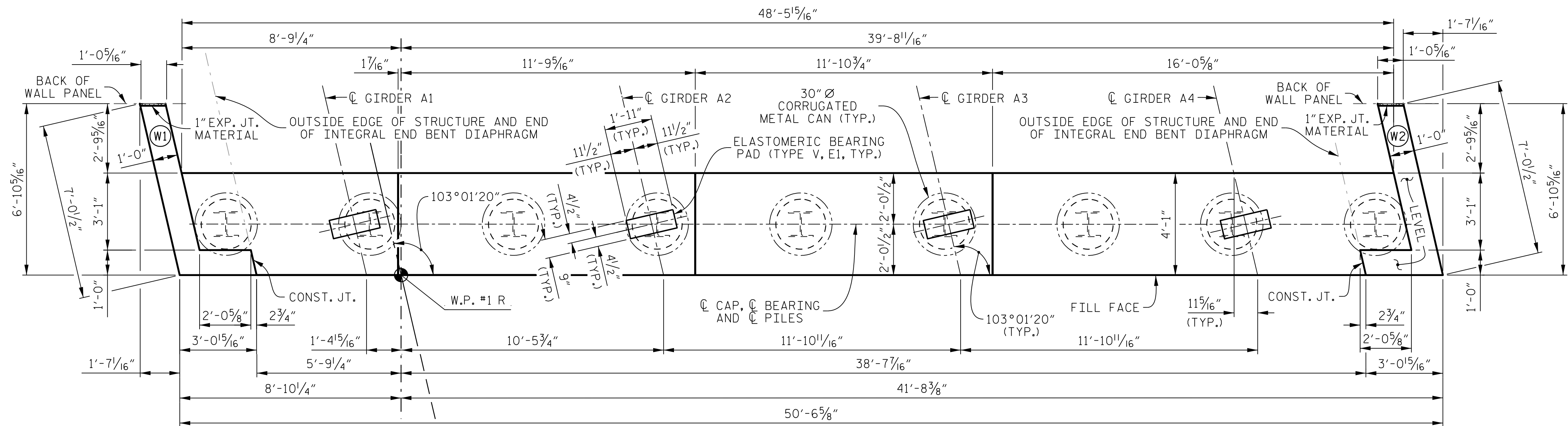
PLANS PREPARED BY : **PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246



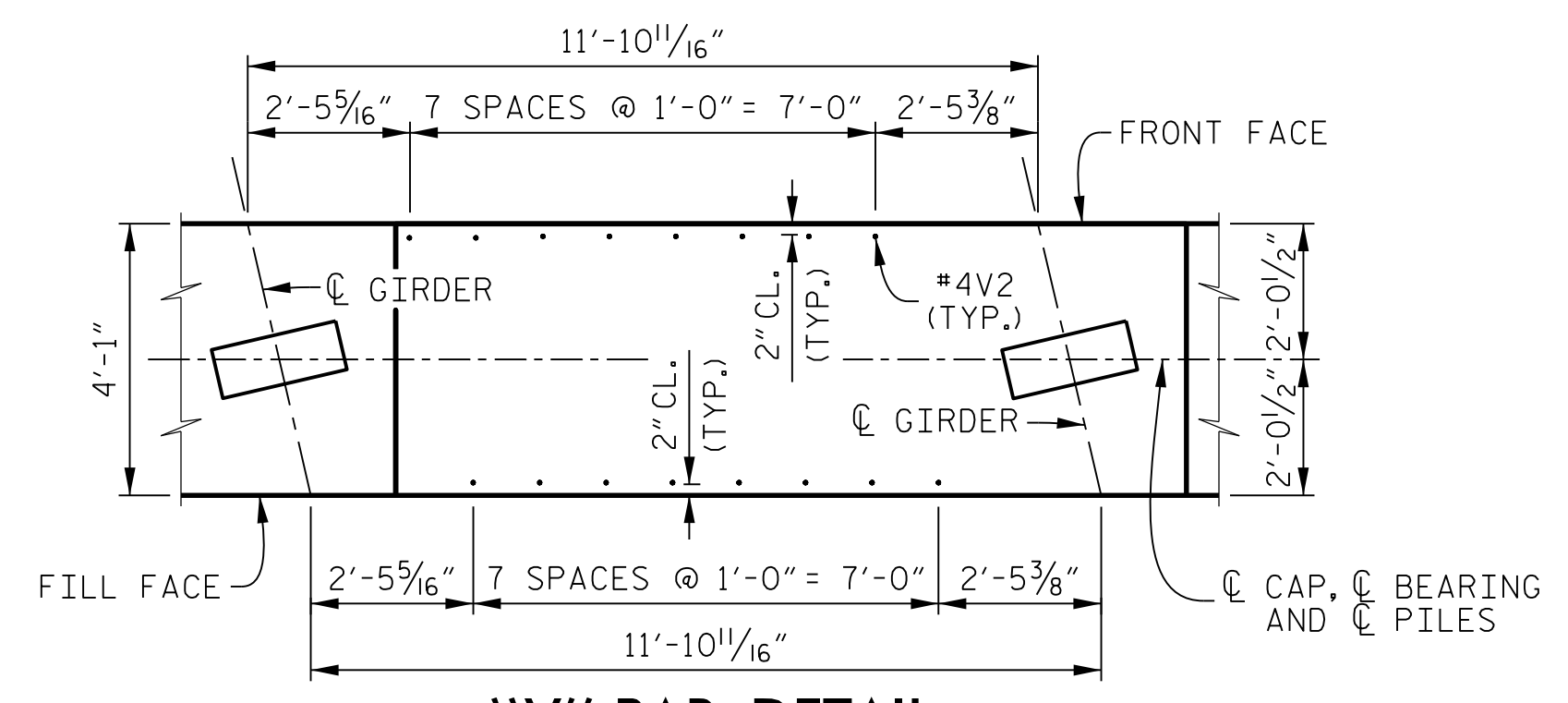
PROJECT NO. **R-2707C**  
**CLEVELAND** COUNTY  
 STATION: **611+32.01 -L-**

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUPERSTRUCTURE					
BILL OF MATERIAL					
(RIGHT LANE)					
REVISIONS					SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		
					TOTAL SHEETS
					25

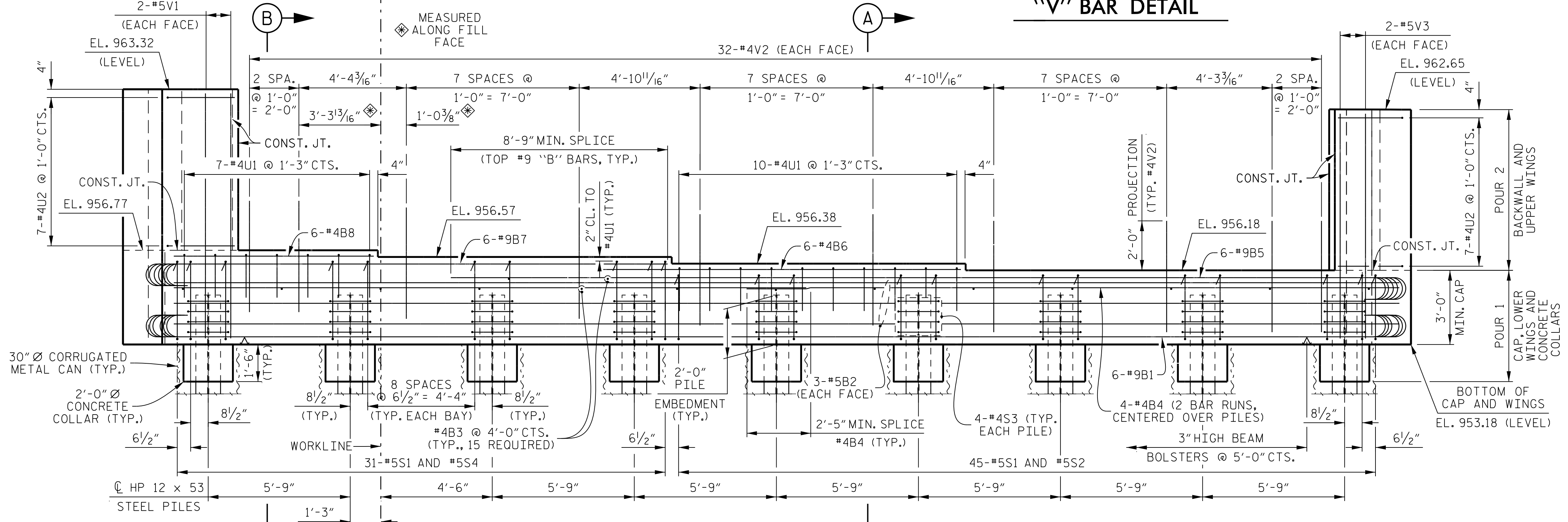




**PLAN**



**"V" BAR DETAIL**



**ELEVATION**

**NOTES**

- STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4V2 BARS.
- THE TOP SURFACE OF THE END BENT CAP UNDER THE INTEGRAL END BENT DIAPHRAGM, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".
- FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.
- FOR TEMPORARY DRAINAGE AT END BENT DETAIL, SEE SHEET 3 OF 3.
- FOR SECTION A-A AND SECTION B-B, SEE SHEET 3 OF 3.
- FOR INTEGRAL END BENT DIAPHRAGM DETAILS, SEE "TYPICAL SECTION" AND "PLAN OF SPAN" SHEETS.
- THE COST TO FURNISH AND INSTALL THE 30" Ø CORRUGATED METAL CANS SHALL BE INCLUDED IN THE CONTRACT PRICE FOR MSE RETAINING WALL.
- WING (W1) AND WING (W2) DETAILS ARE BASED ON A 5 1/2" WALL PANEL THICKNESS AND USING DOWELS FOR THE COPING. CONTRACTOR MAY ADJUST WINGS SLIGHTLY AS NECESSARY, BASED ON APPROVED MSE WALL SHOP DRAWINGS.

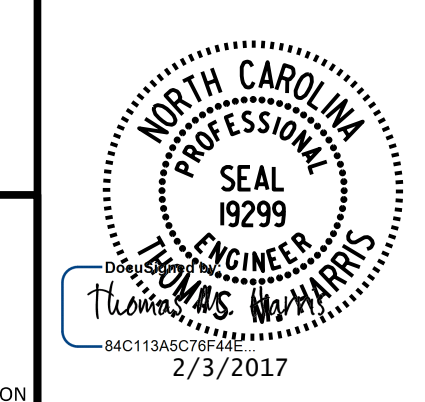
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

SHEET 1 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**SUBSTRUCTURE**  
**END BENT 1**  
**(RIGHT LANE)**

REVISIONS						SHEET No. <b>S10-17</b>
No.	BY:	DATE:	No.	BY:	DATE:	
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2			4			

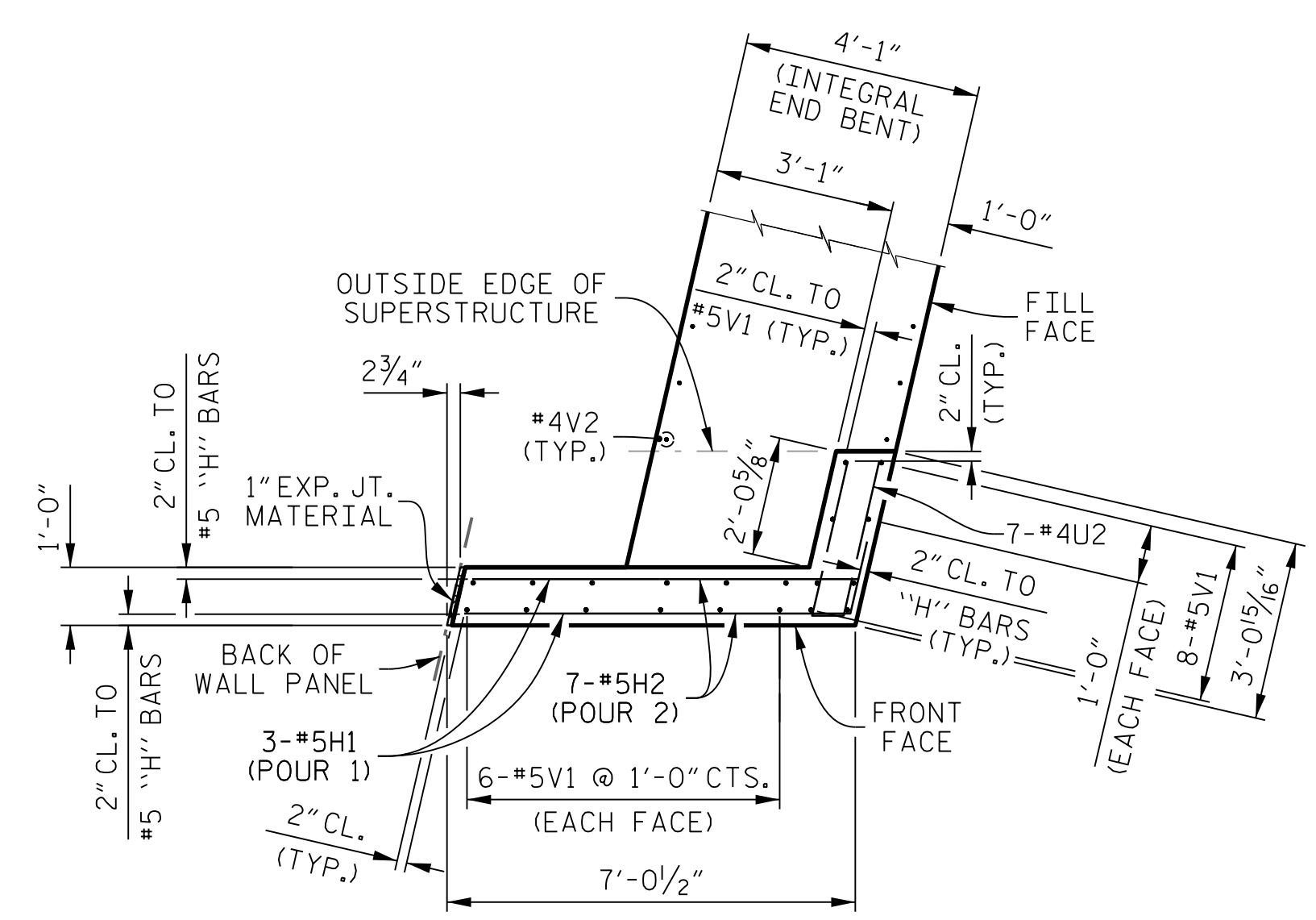
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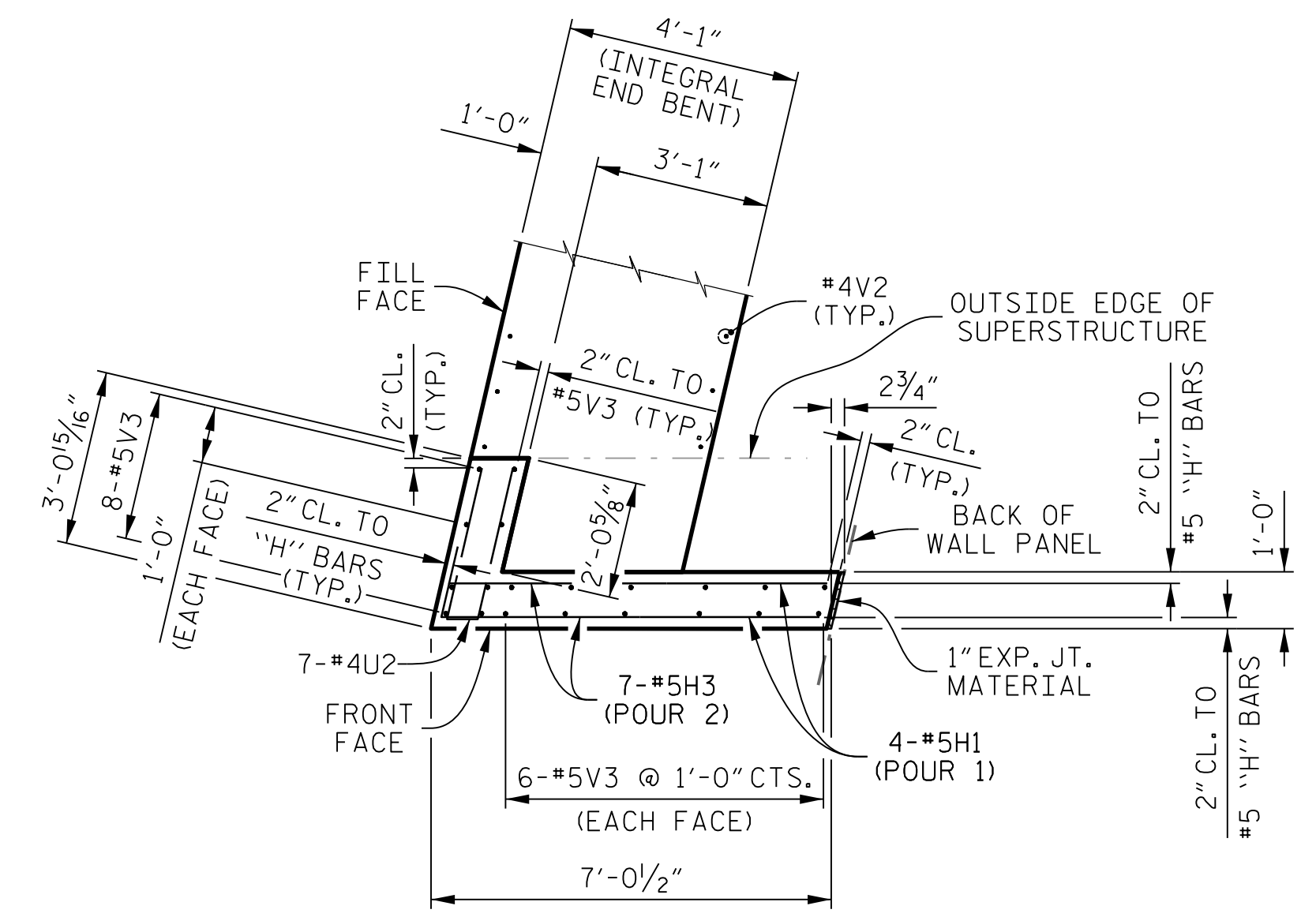
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: **K. E. LOFTON** DATE: **10-16**  
 CHECKED BY: **A. D. SHAH** DATE: **10-16**  
 DESIGN ENGINEER: **T. M. HARRIS** DATE: **10-16**

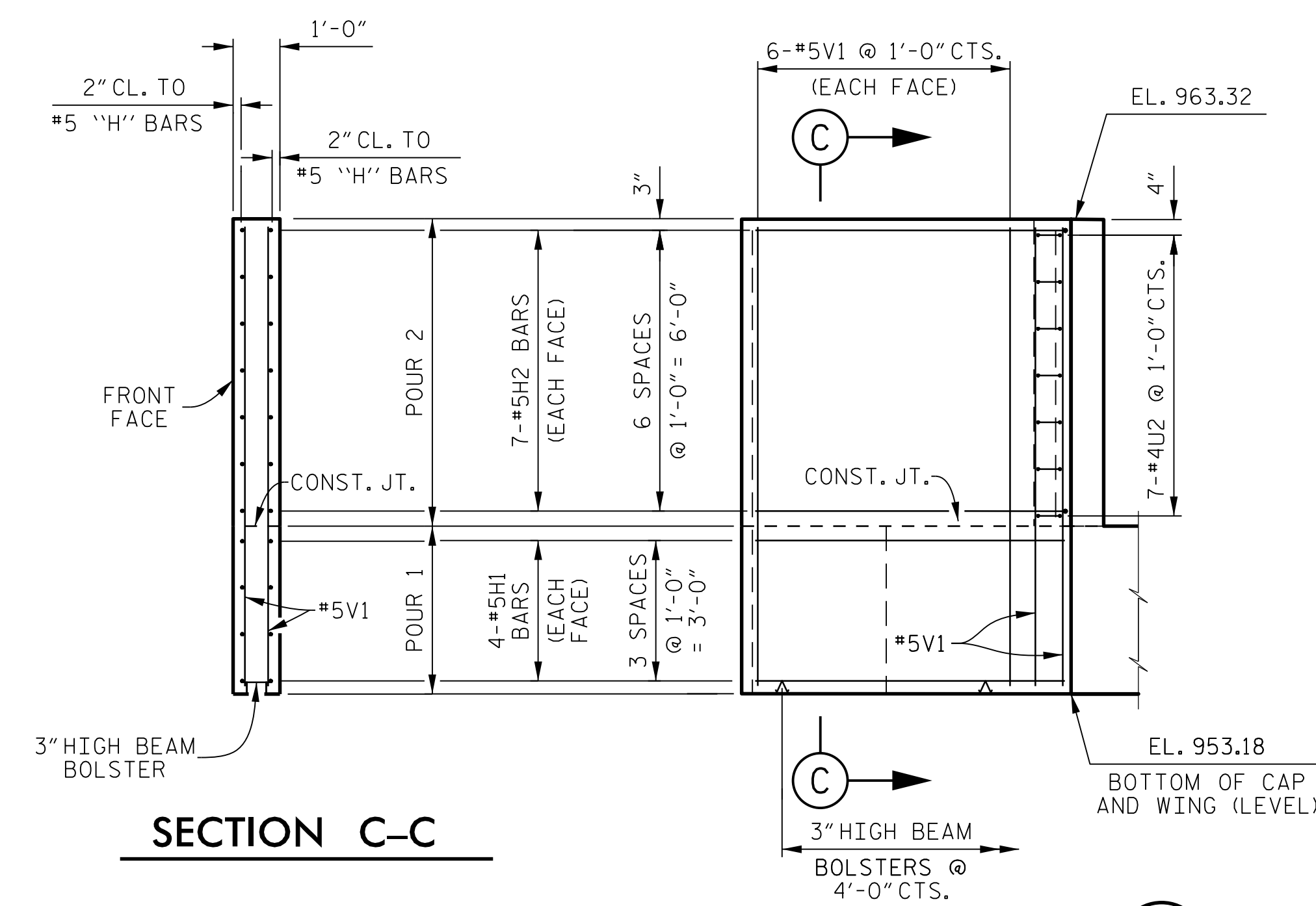
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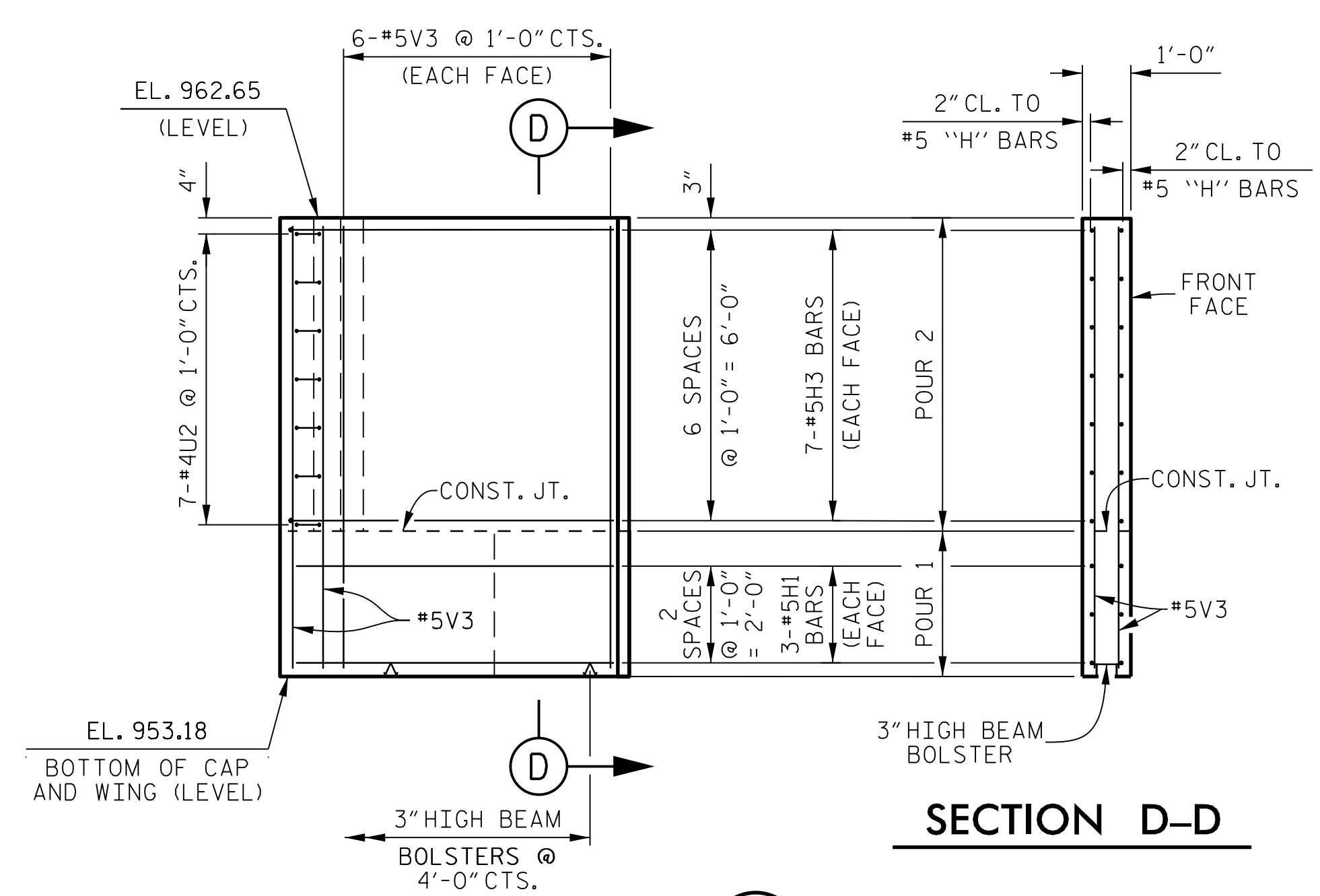
PLAN OF WING (W1)



PLAN OF WING (W2)



ELEVATION OF WING (W1)



ELEVATION OF WING (W2)

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 3

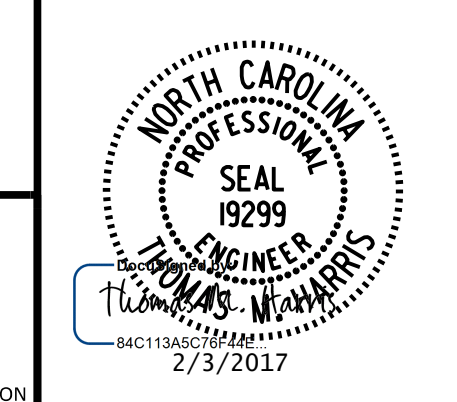
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUBSTRUCTURE  
 END BENT 1  
 (RIGHT LANE)**

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS: 25

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED

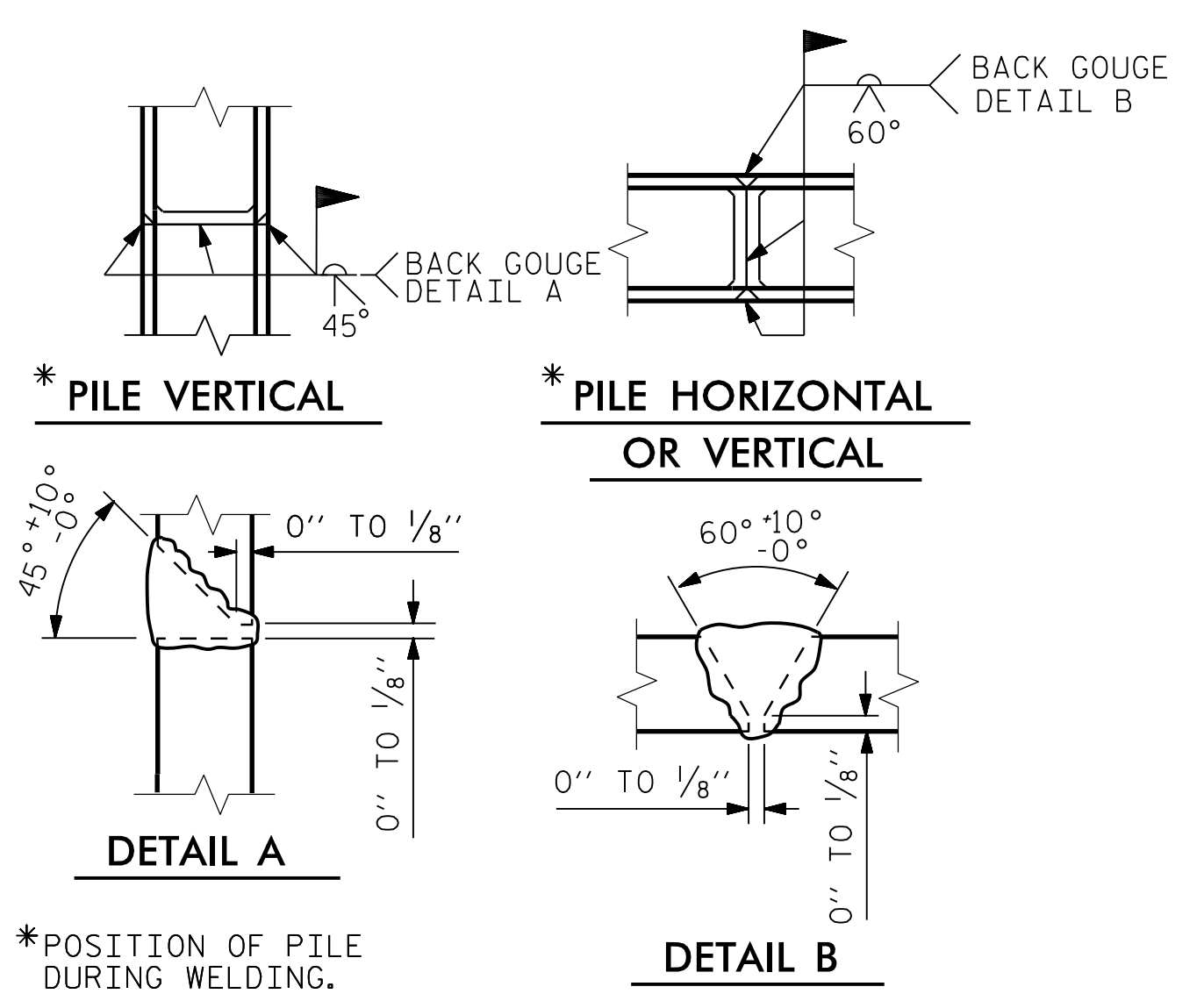
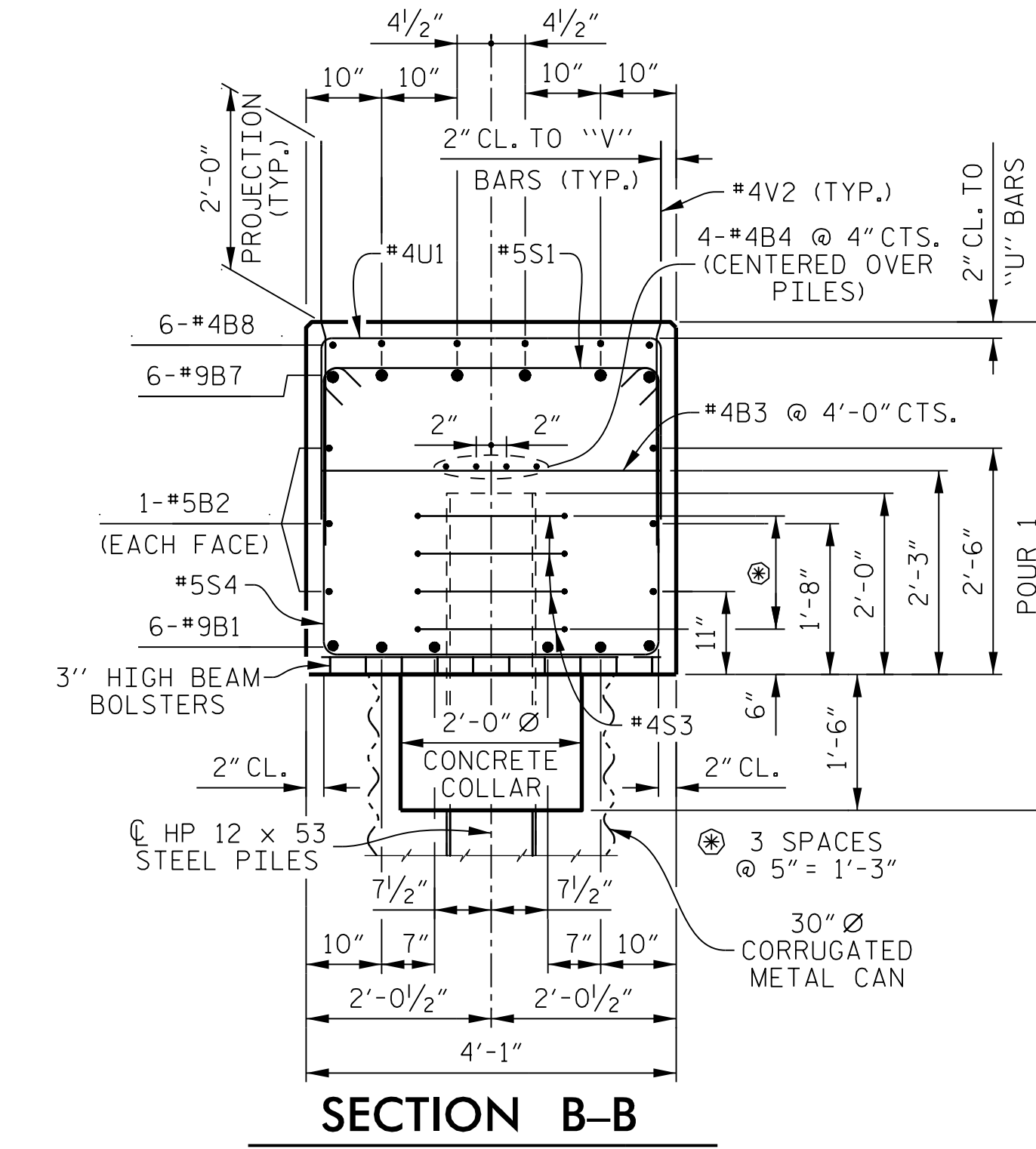
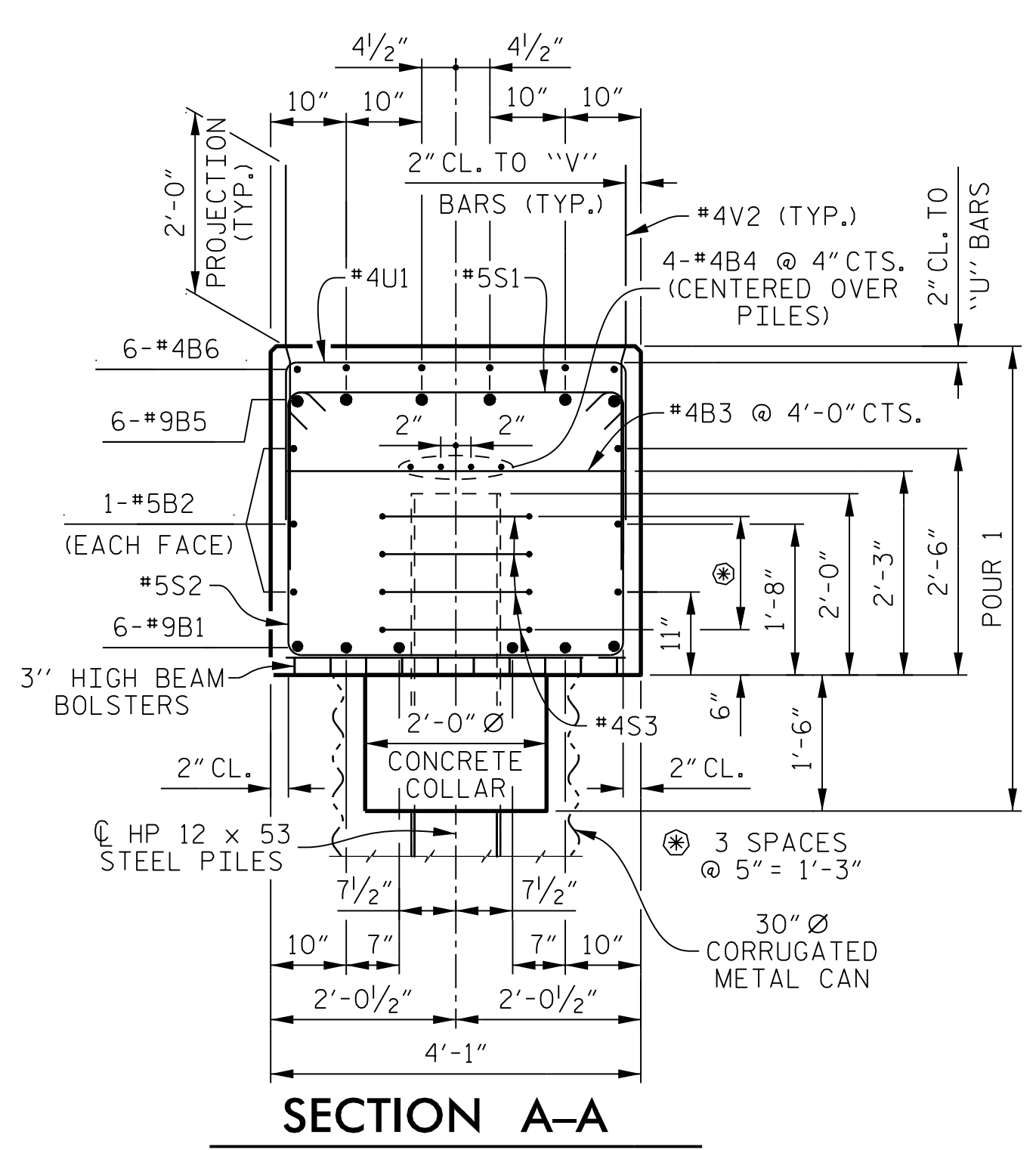


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 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

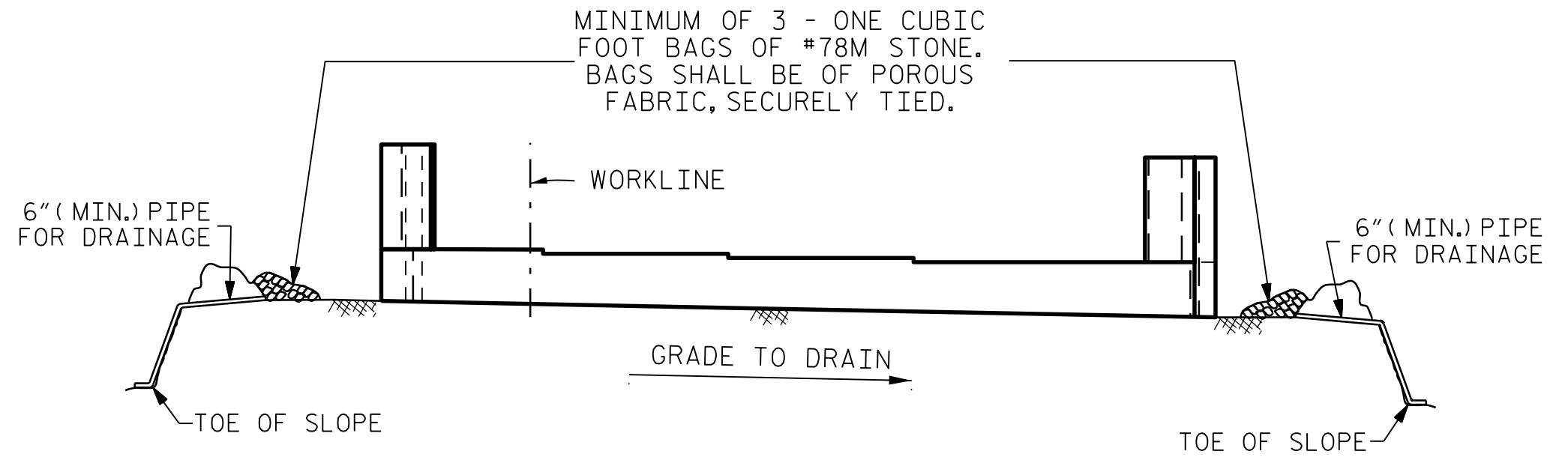
DRAWN BY: K. E. LOFTON DATE: 10-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

FILE: j:\2707c\structures\workbooks\plans - smc\_final\410\_035\_R2707C\_smc\_eb12\_510-18.dgn  
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PILE SPlice DETAILS



MINIMUM OF 3 - ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.

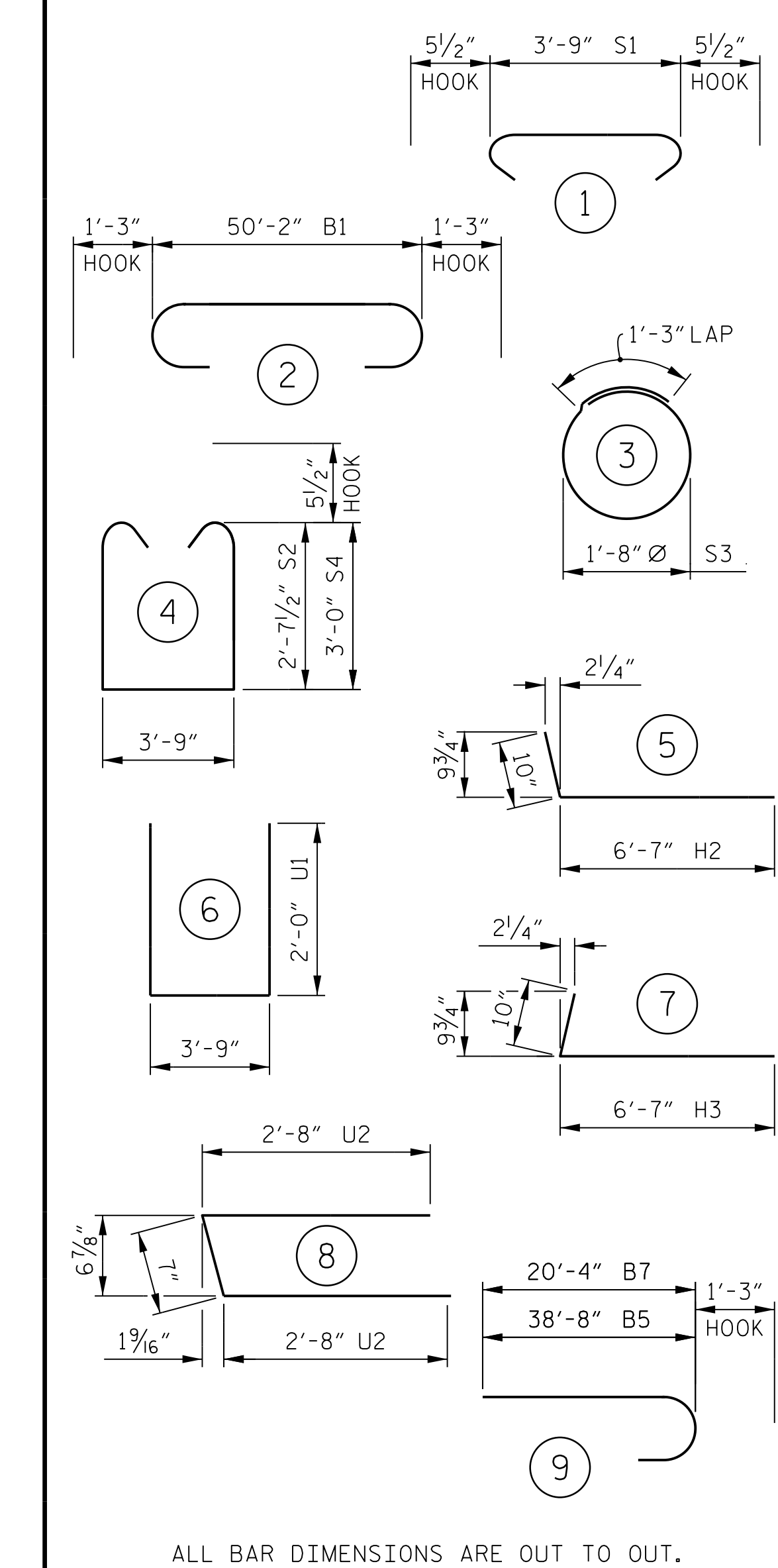
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

END BENT 1					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
B1	6	#9	2	52'-8"	1,074
B2	6	#5	STR	50'-2"	314
B3	15	#4	STR	3'-9"	38
B4	8	#4	STR	26'-4"	141
B5	6	#9	9	39'-0"	796
B6	6	#4	STR	14'-8"	59
B7	6	#9	9	21'-7"	440
B8	6	#4	STR	8'-5"	34
H1	14	#5	STR	6'-7"	96
H2	14	#5	7	7'-5"	108
H3	14	#5	5	7'-5"	108
S1	76	#5	1	4'-8"	370
S2	45	#5	4	9'-11"	465
S3	36	#4	3	6'-6"	156
S4	31	#5	4	10'-8"	345
U1	17	#4	6	7'-9"	88
U2	14	#4	8	5'-11"	55
V1	20	#5	STR	9'-9"	203
V2	60	#4	STR	4'-6"	180
V3	20	#5	STR	9'-0"	188
REINFORCING STEEL			5,258 LBS.		
CLASS "A" CONCRETE					
POUR 1 COLLARS, CAP AND LOWER WINGS			27.1 CU. YDS.		
POUR 2 BACKWALL AND UPPER WINGS			4.4 CU. YDS.		
TOTAL			31.5 CU. YDS.		
PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES			9 EACH		
HP 12 x 53 STEEL PILES			9 REQUIRED		
			720.0 LIN. FT.		

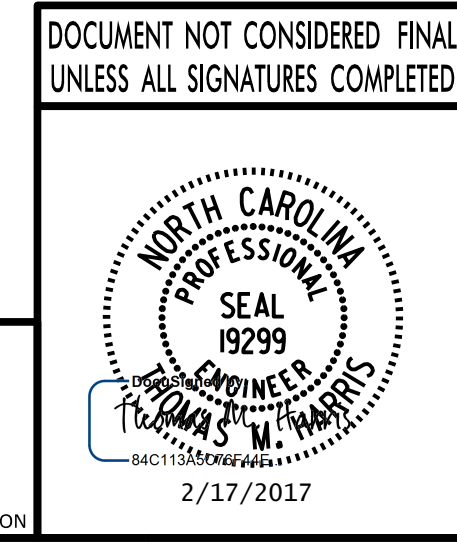
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUBSTRUCTURE  
 END BENT 1  
 (RIGHT LANE)

REVISIONS		SHEET No.	
No.	BY:	DATE:	No.
1			3
2			4
			TOTAL SHEETS
			25

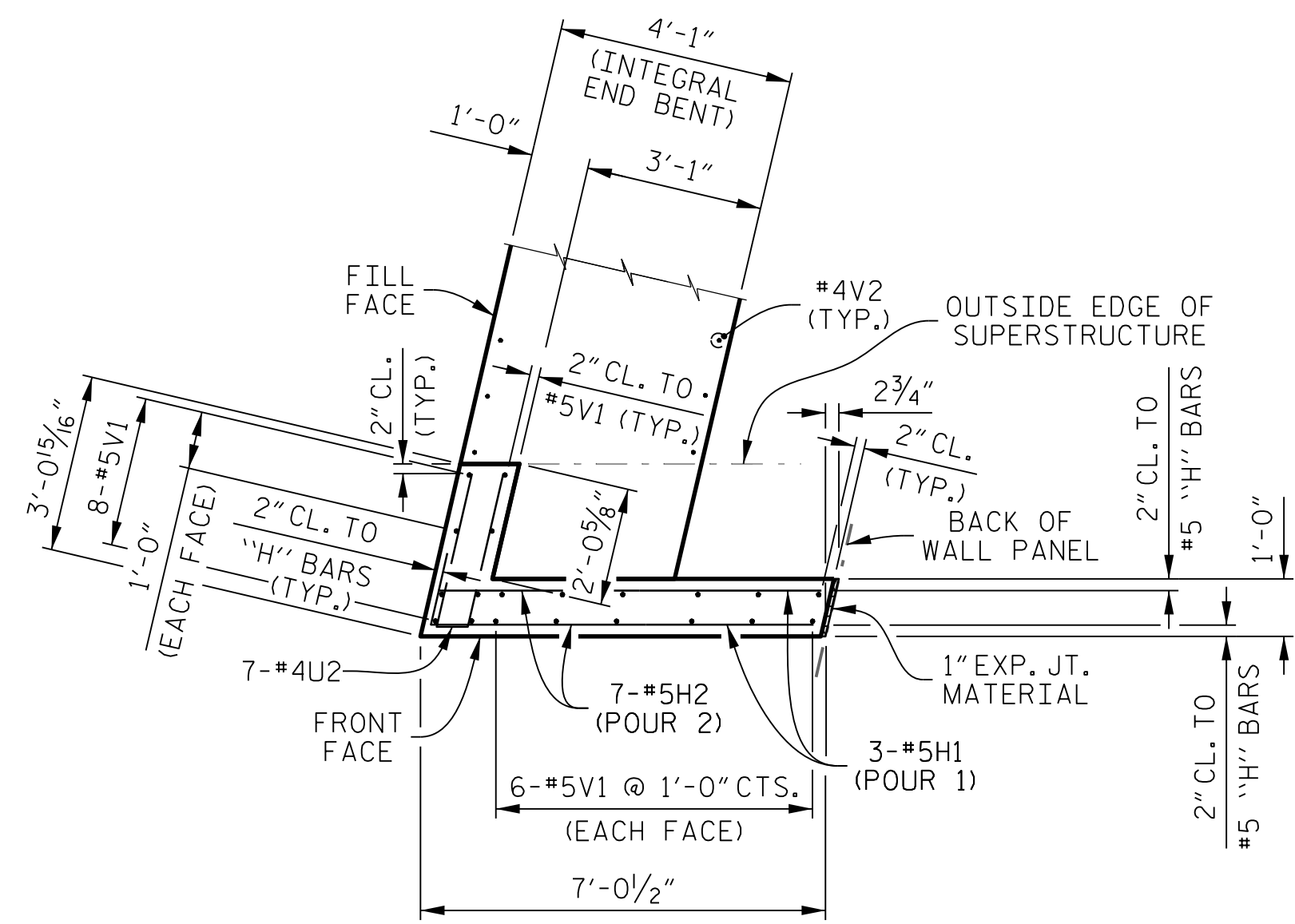


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 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

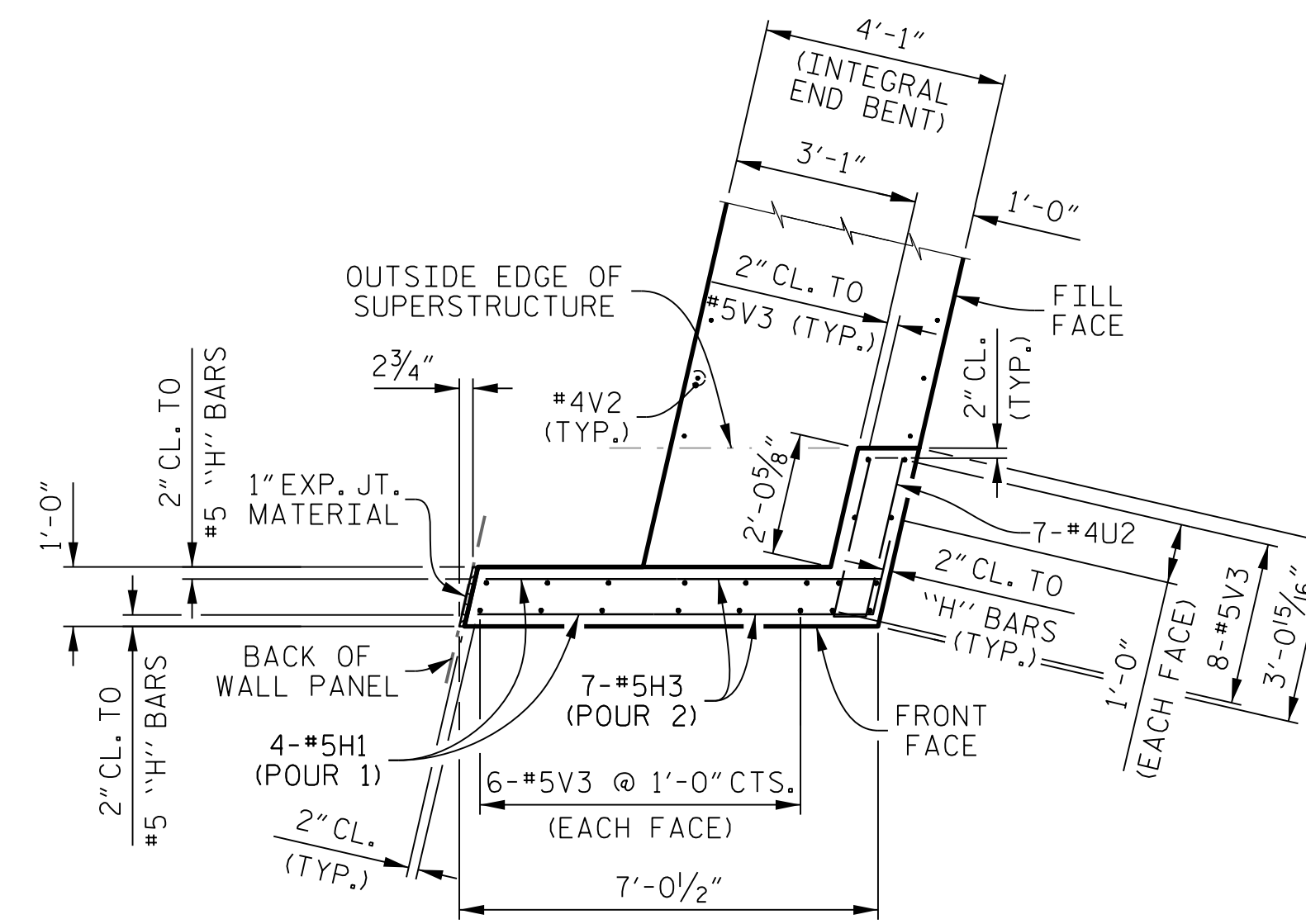
DRAWN BY: K. E. LOFTON DATE: 10-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16



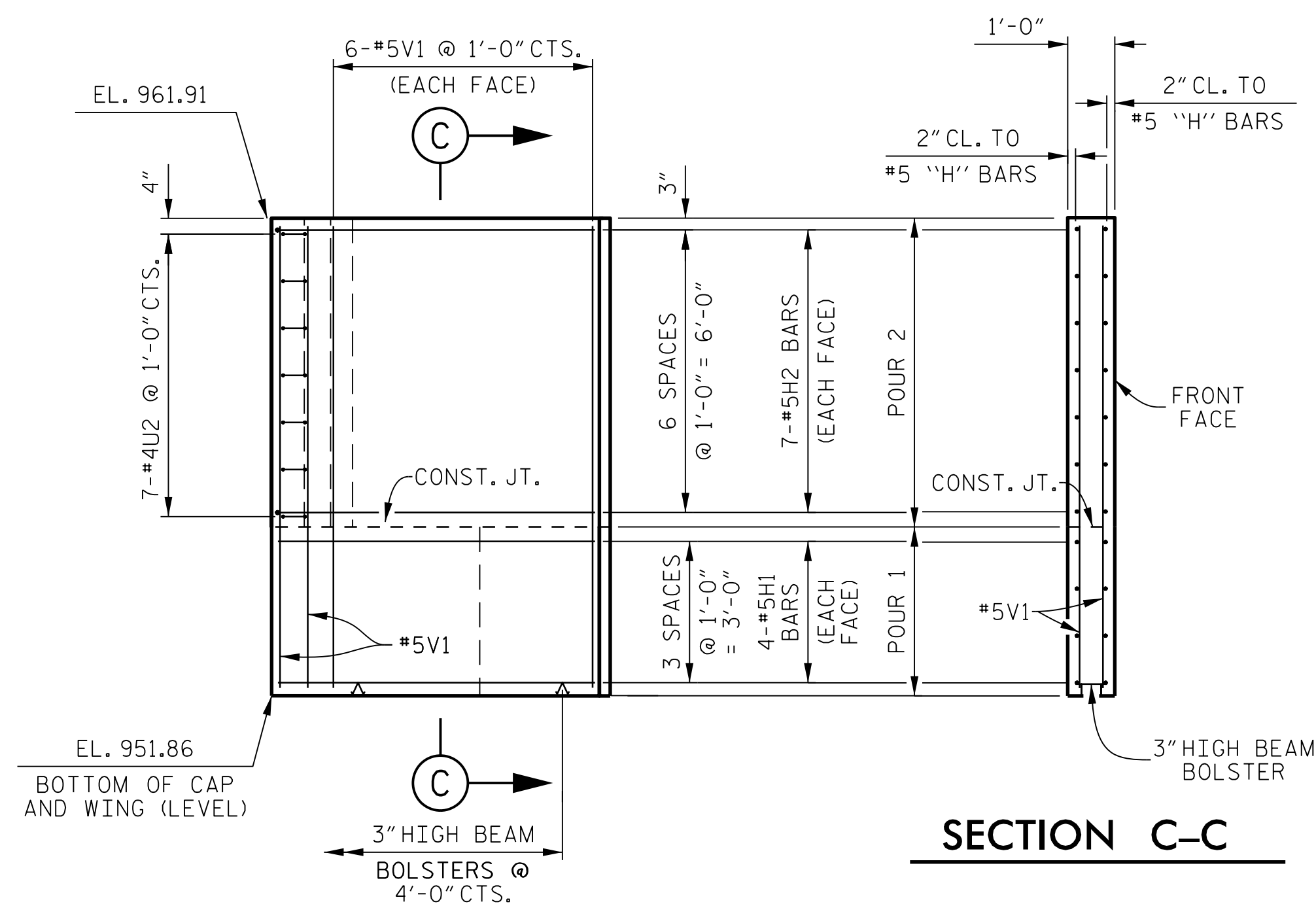




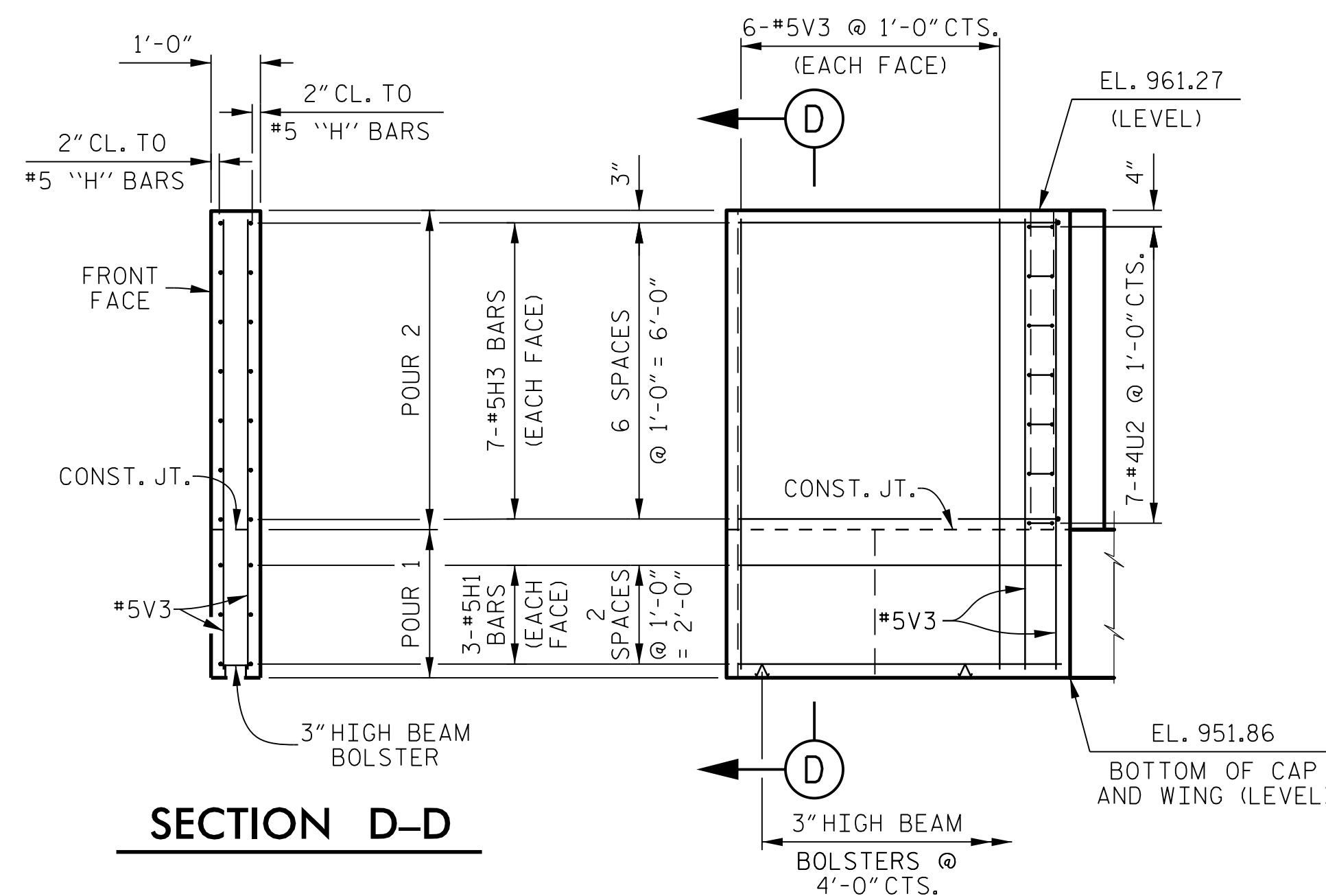
PLAN OF WING W3



PLAN OF WING W4



ELEVATION OF WING W3



ELEVATION OF WING W4

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 3

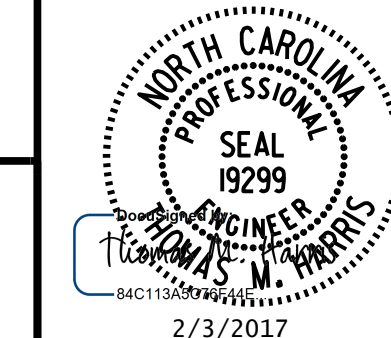
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUBSTRUCTURE  
 END BENT 2  
 (RIGHT LANE)

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

STR. #10

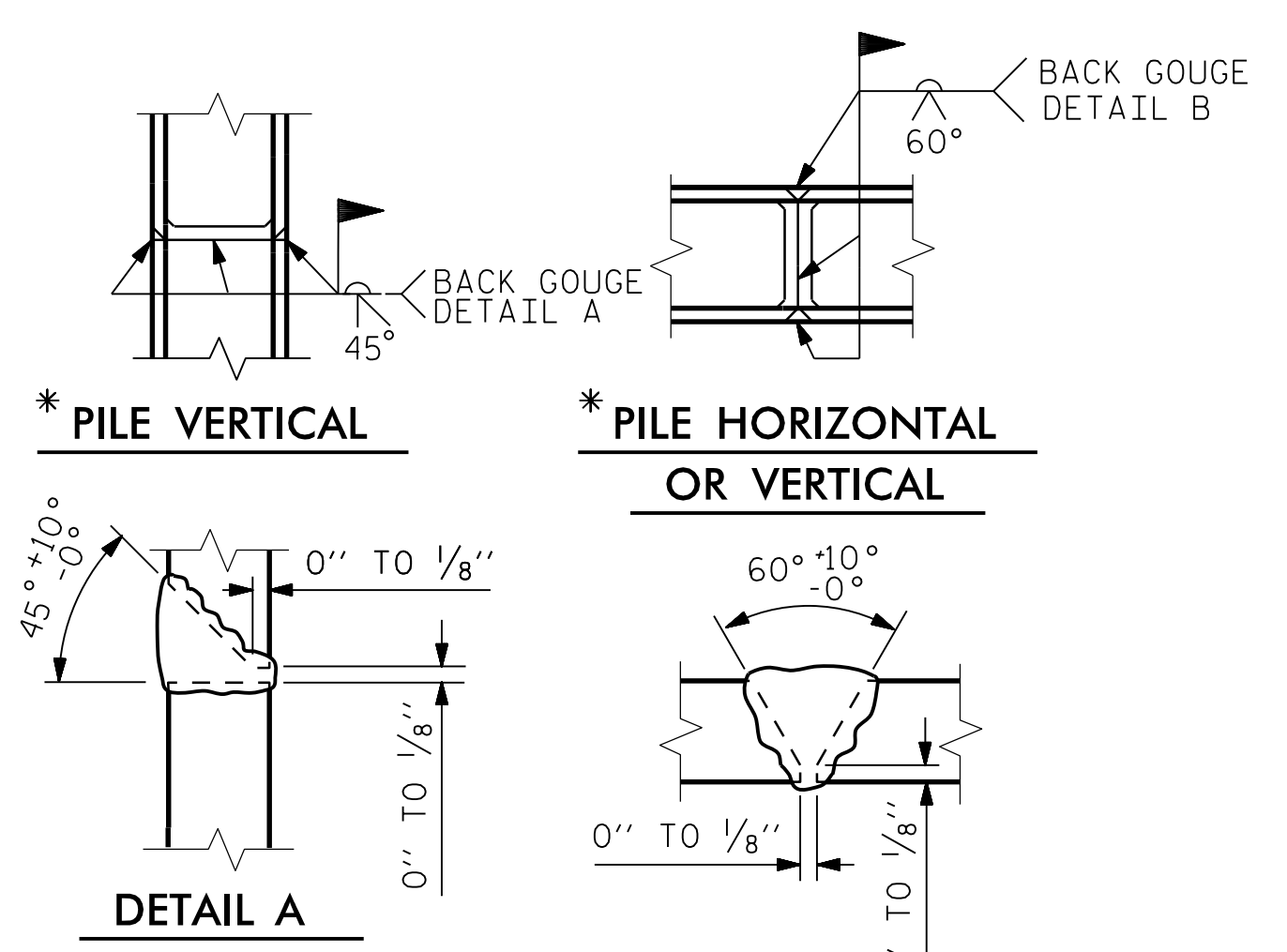
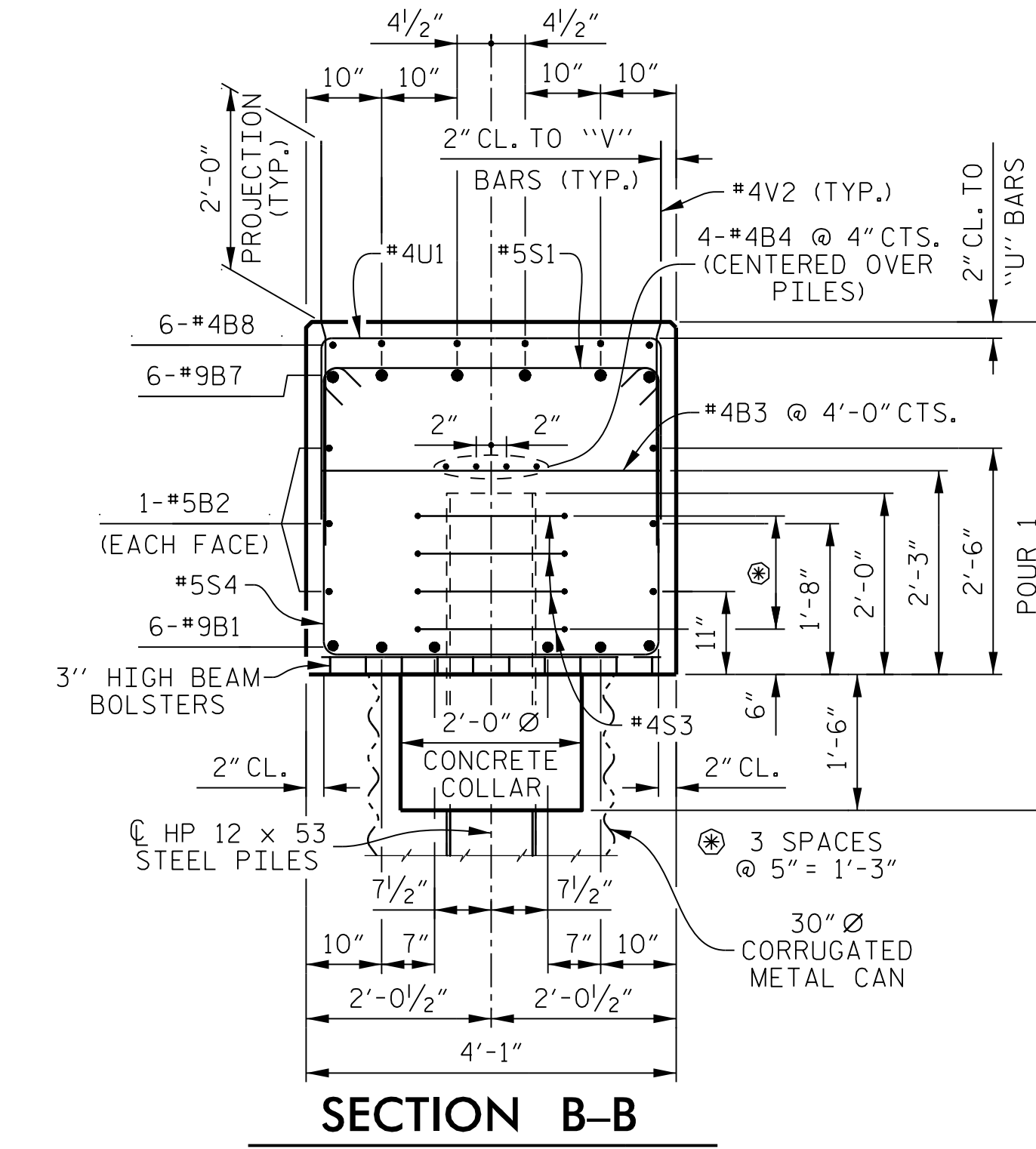
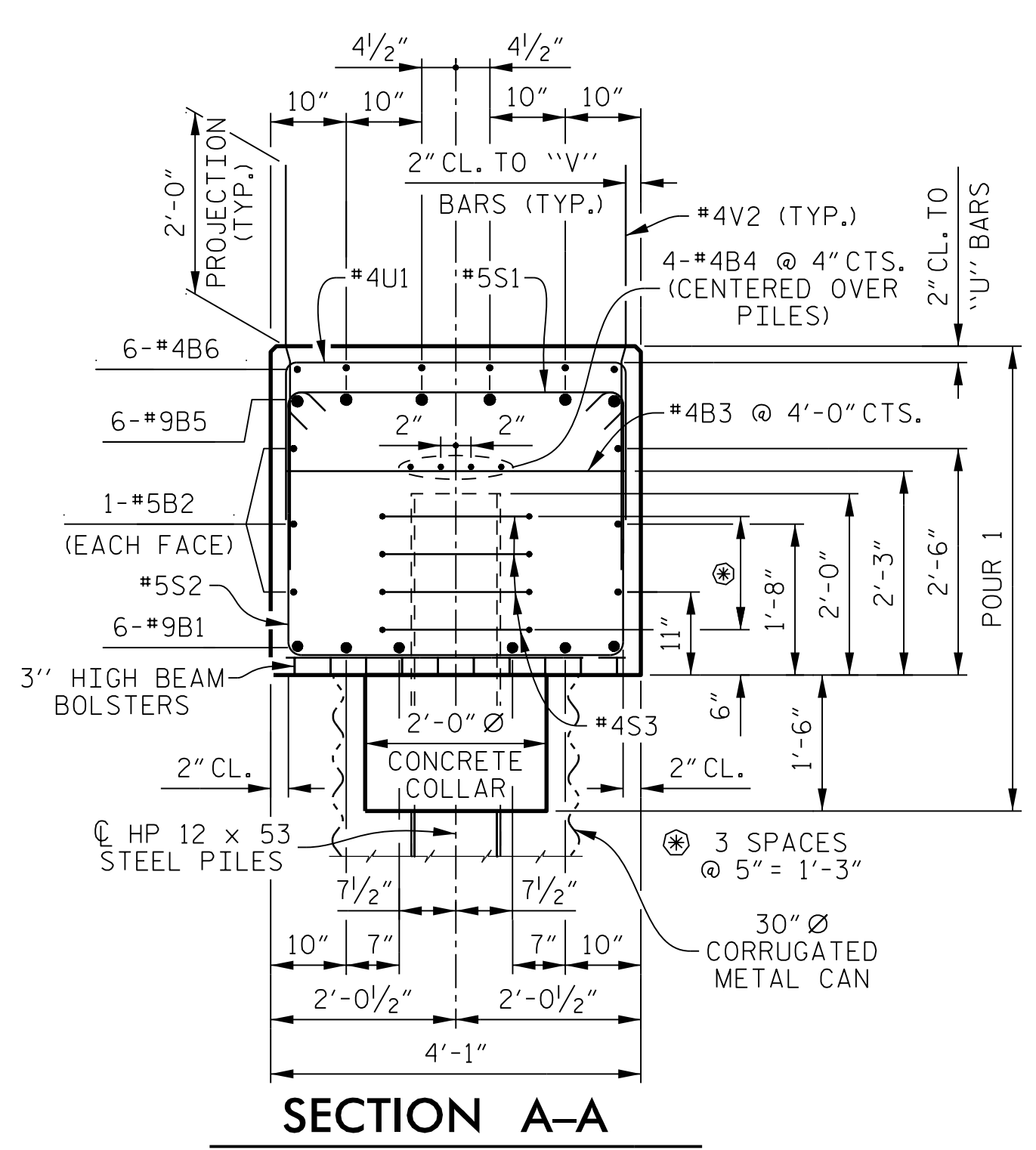
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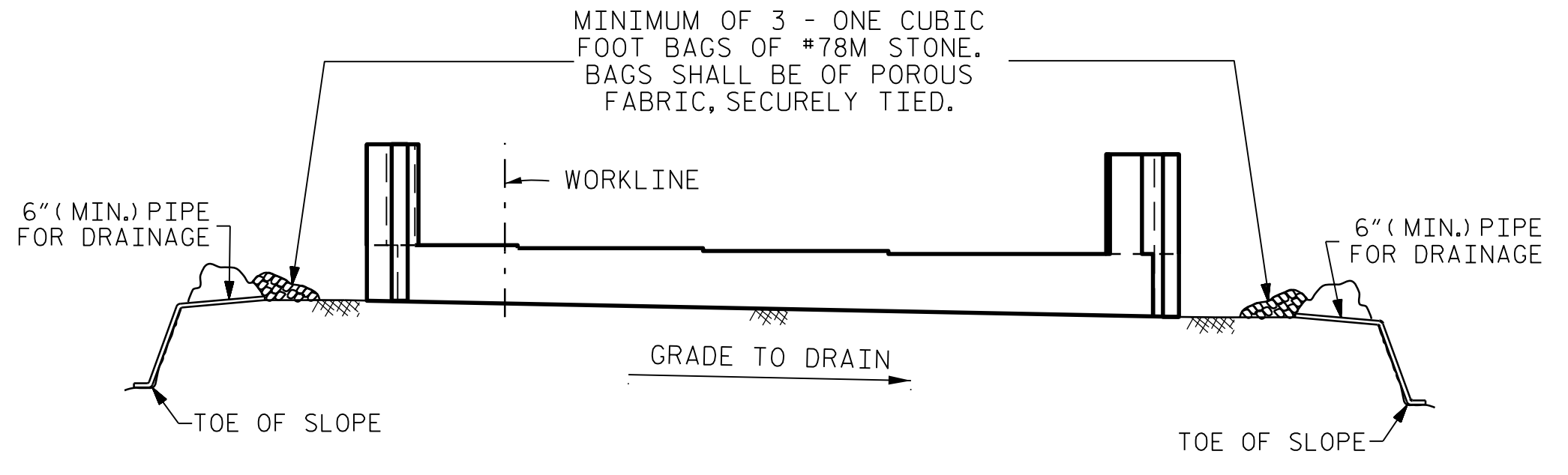
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON DATE: 10-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16

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 DATE: 2/2/2017 3:52:12 PM



PILE SPlice DETAILS



MINIMUM OF 3 - ONE CUBIC FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED.

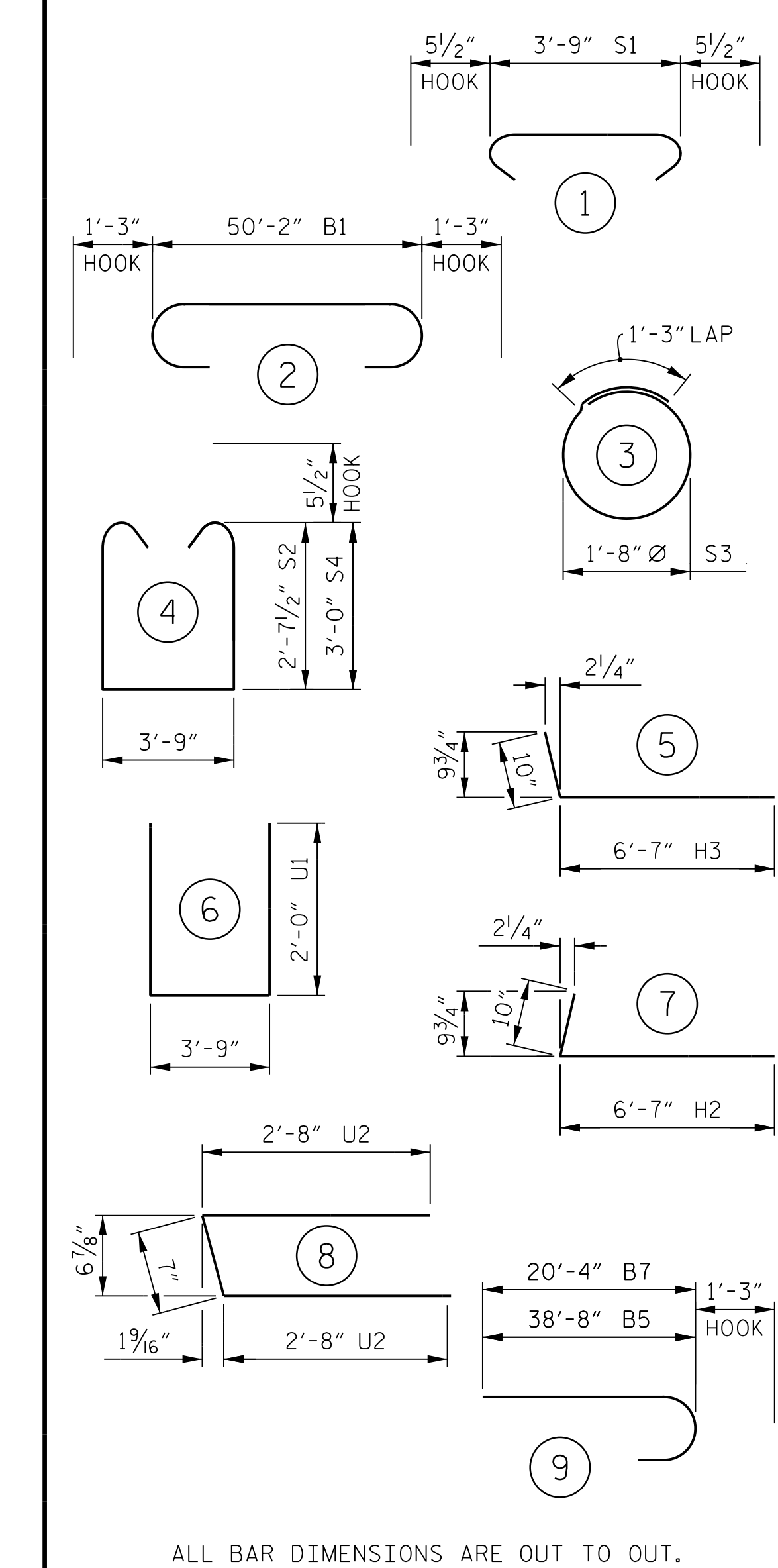
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TEMPORARY DRAINAGE AT END BENT

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

END BENT 2					
BAR No.	SIZE	TYPE	LENGTH	WEIGHT	
B1	6	#9	2	52'-8"	1,074
B2	6	#5	STR	50'-2"	314
B3	15	#4	STR	3'-9"	38
B4	8	#4	STR	26'-4"	141
B5	6	#9	9	39'-0"	796
B6	6	#4	STR	14'-8"	59
B7	6	#9	9	21'-7"	440
B8	6	#4	STR	8'-5"	34
H1	14	#5	STR	6'-7"	96
H2	14	#5	7	7'-5"	108
H3	14	#5	5	7'-5"	108
S1	76	#5	1	4'-8"	370
S2	45	#5	4	9'-11"	465
S3	36	#4	3	6'-6"	156
S4	31	#5	4	10'-8"	345
U1	17	#4	6	7'-9"	88
U2	14	#4	8	5'-11"	55
V1	20	#5	STR	9'-9"	203
V2	60	#4	STR	4'-6"	180
V3	20	#5	STR	9'-0"	188
REINFORCING STEEL				5,258	LBS.
CLASS "A" CONCRETE					
POUR 1 COLLARS, CAP AND LOWER WINGS				27.1	CU. YDS.
POUR 2 BACKWALL AND UPPER WINGS				4.4	CU. YDS.
TOTAL				31.5	CU. YDS.
PILE DRIVING EQUIPMENT SETUP FOR HP 12 x 53 STEEL PILES				9	EACH
HP 12 x 53 STEEL PILES				9	REQUIRED
				720.0	LIN. FT.

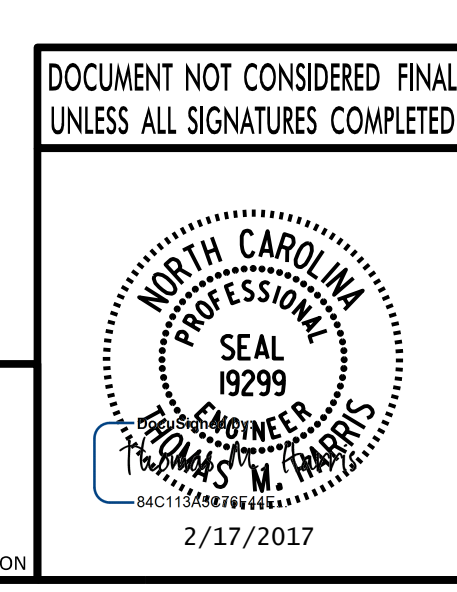
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

SUBSTRUCTURE  
 END BENT 2  
 (RIGHT LANE)

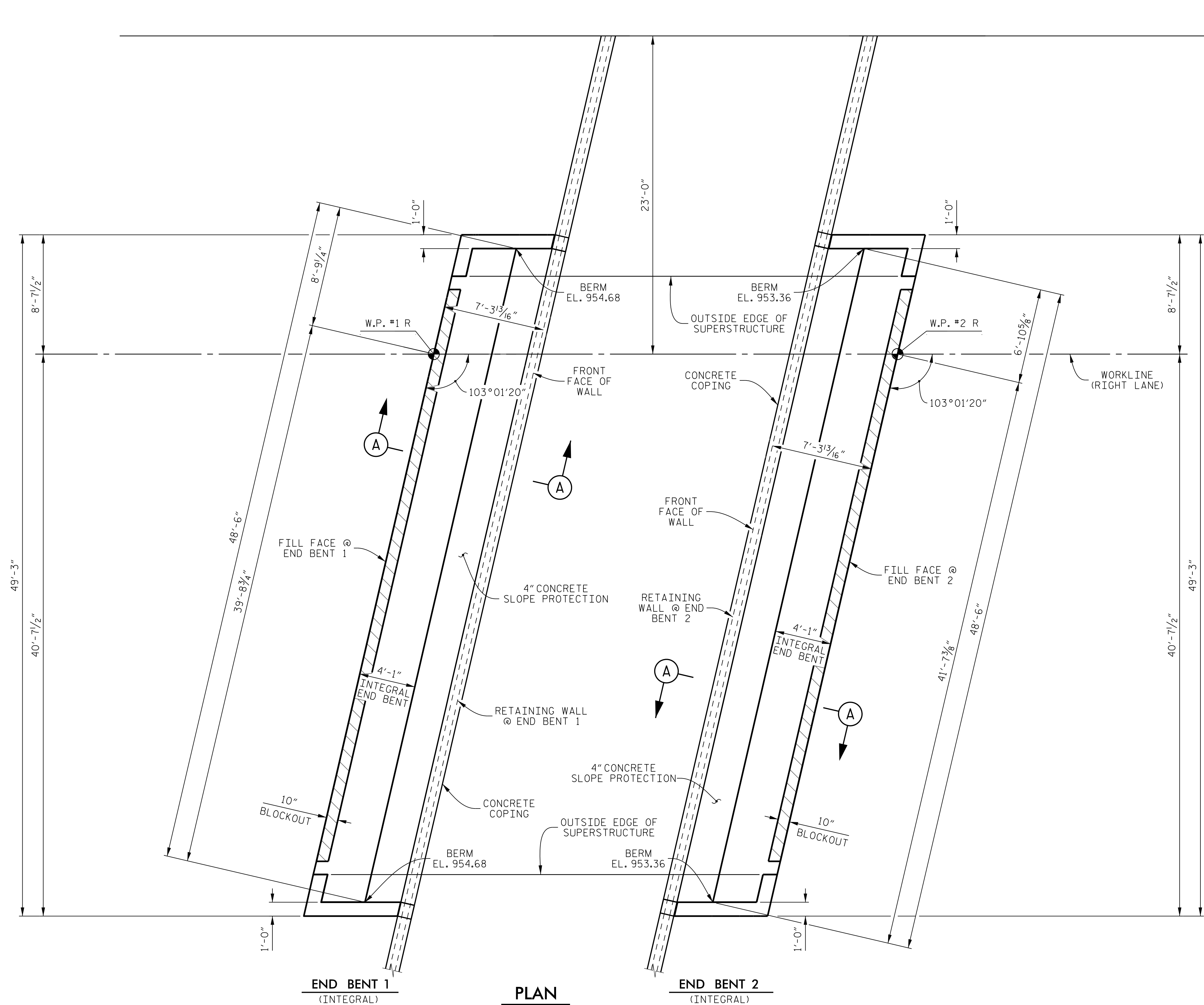
REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	S10-22
1			3			TOTAL SHEETS
2			4			25



PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: K. E. LOFTON DATE: 10-16  
 CHECKED BY: A. D. SHAH DATE: 10-16  
 DESIGN ENGINEER: T. M. HARRIS DATE: 10-16



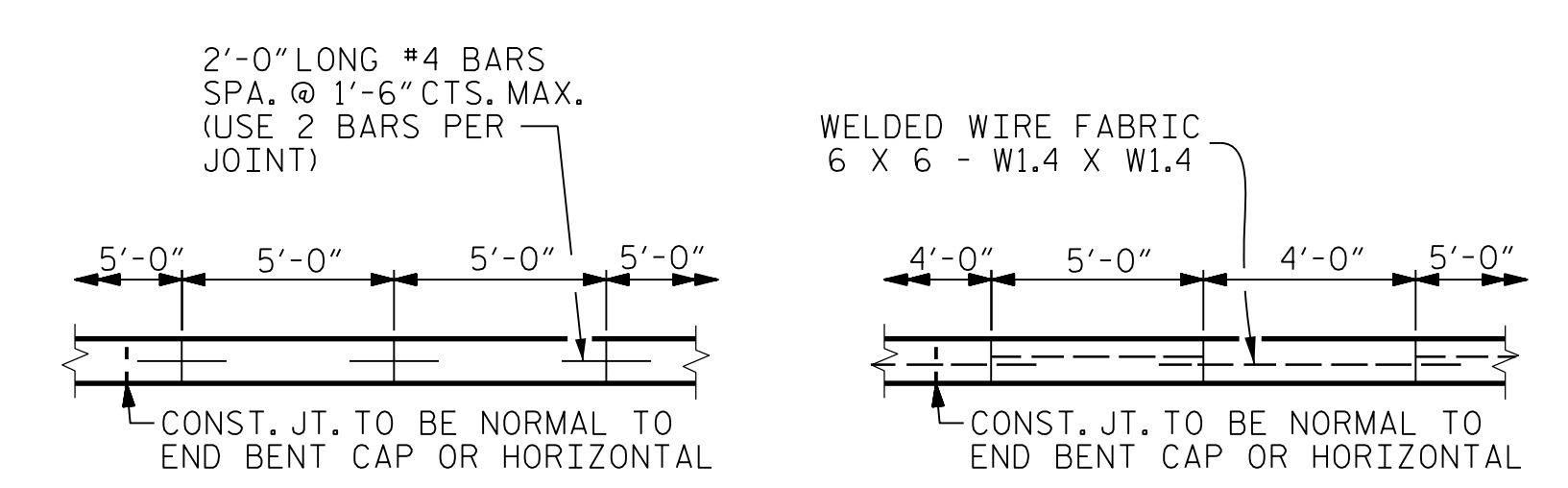


**END BENT 1 (INTEGRAL) PLAN**  
 APPROACH SLABS AND CONCRETE DITCHES NOT SHOWN  
**END BENT 2 (INTEGRAL)**

**GENERAL NOTES**

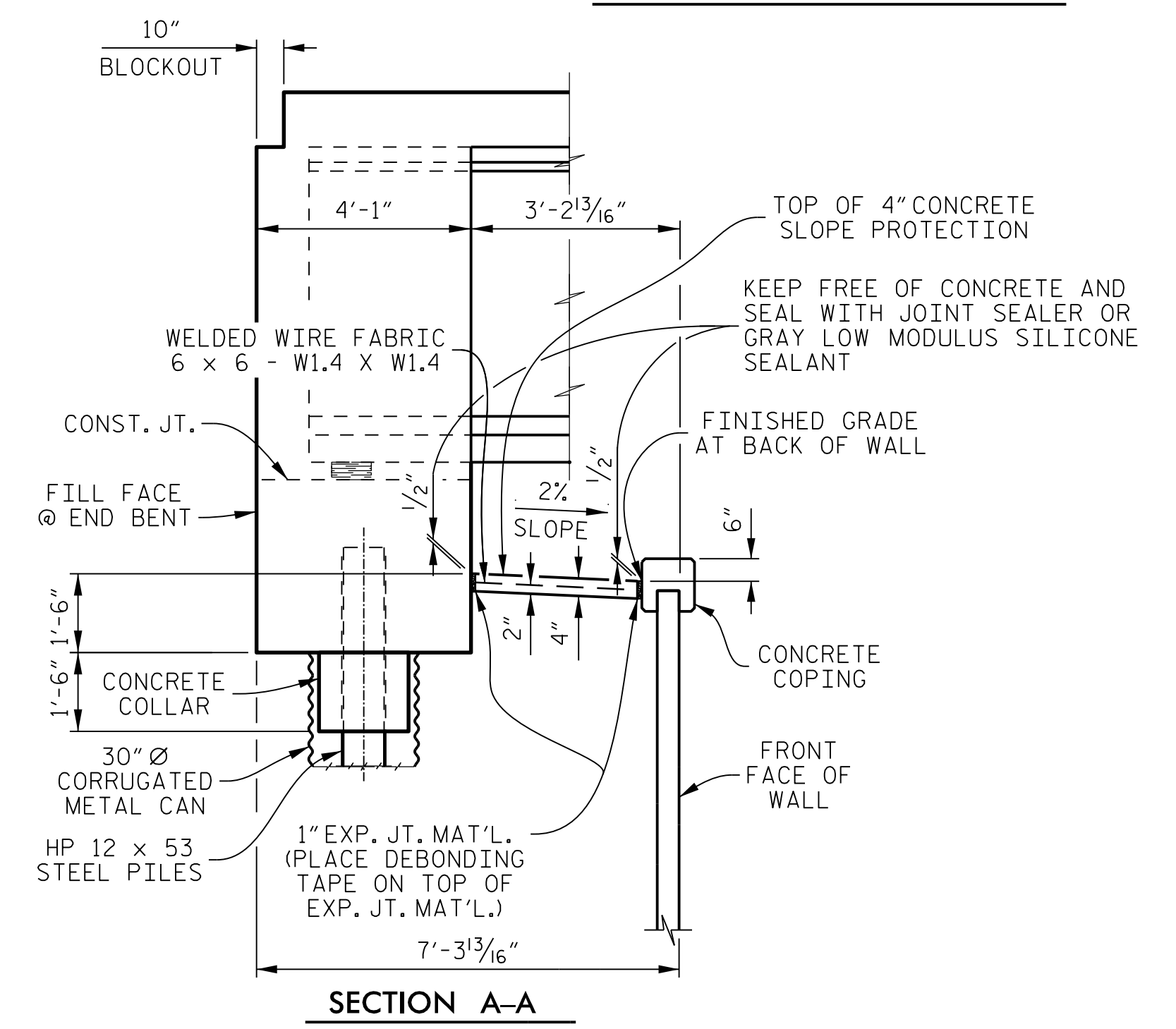
SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS.  
 STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT.  
 MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B", THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 x 6 - W1.4 x W1.4. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.



**POURING DETAIL**

**OPTIONAL POURING DETAIL**



**SECTION A-A**

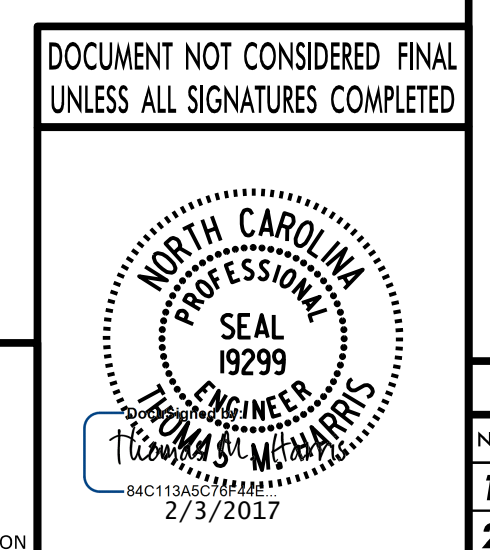
BRIDGE AT STA. 611 + 32.01 -L- (RIGHT LANE)	4" SLOPE PROTECTION	WELDED WIRE FABRIC
	SQUARE YARDS	APPROX. LINEAR FEET
END BENT 1	14	25
END BENT 2	14	25

PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611 + 32.01 -L-**

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**STANDARD**

**SLOPE PROTECTION DETAILS (RIGHT LANE)**



PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DESIGN ENGINEER :	T. M. HARRIS	DATE :	10-16

REVISIONS			SHEET No.		
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS: **25**

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 DATE: 10/20/17 12:48:41 PM

ASSEMBLED BY :	K. E. LOFTON	DATE :	10-16
CHECKED BY :	A. D. SHAH	DATE :	10-16
DRAWN BY :	ELR 5/92	REV. 10/1/11	MAA/GM
CHECKED BY :	GRP 6/92	REV. 12/21/11	MAA/GM
		REV. 1/16	MAA/TMG



**BILL OF MATERIAL**

**APPROACH SLAB AT END BENT 1 OR END BENT 2**

BAR No.	SIZE	TYPE	LENGTH	WEIGHT
*A1	52	#4	STR 22'-1"	767
A2	52	#4	STR 21'-11"	761
*B1	83	#5	STR 24'-2"	2,092
B2	83	#6	STR 24'-8"	3,075
REINFORCING STEEL				3,836 LBS.
*EPOXY COATED REINFORCING STEEL				2,859 LBS.
CLASS "AA" CONCRETE (2 APPROACH SLABS REQUIRED)				44.5 CU. YDS.

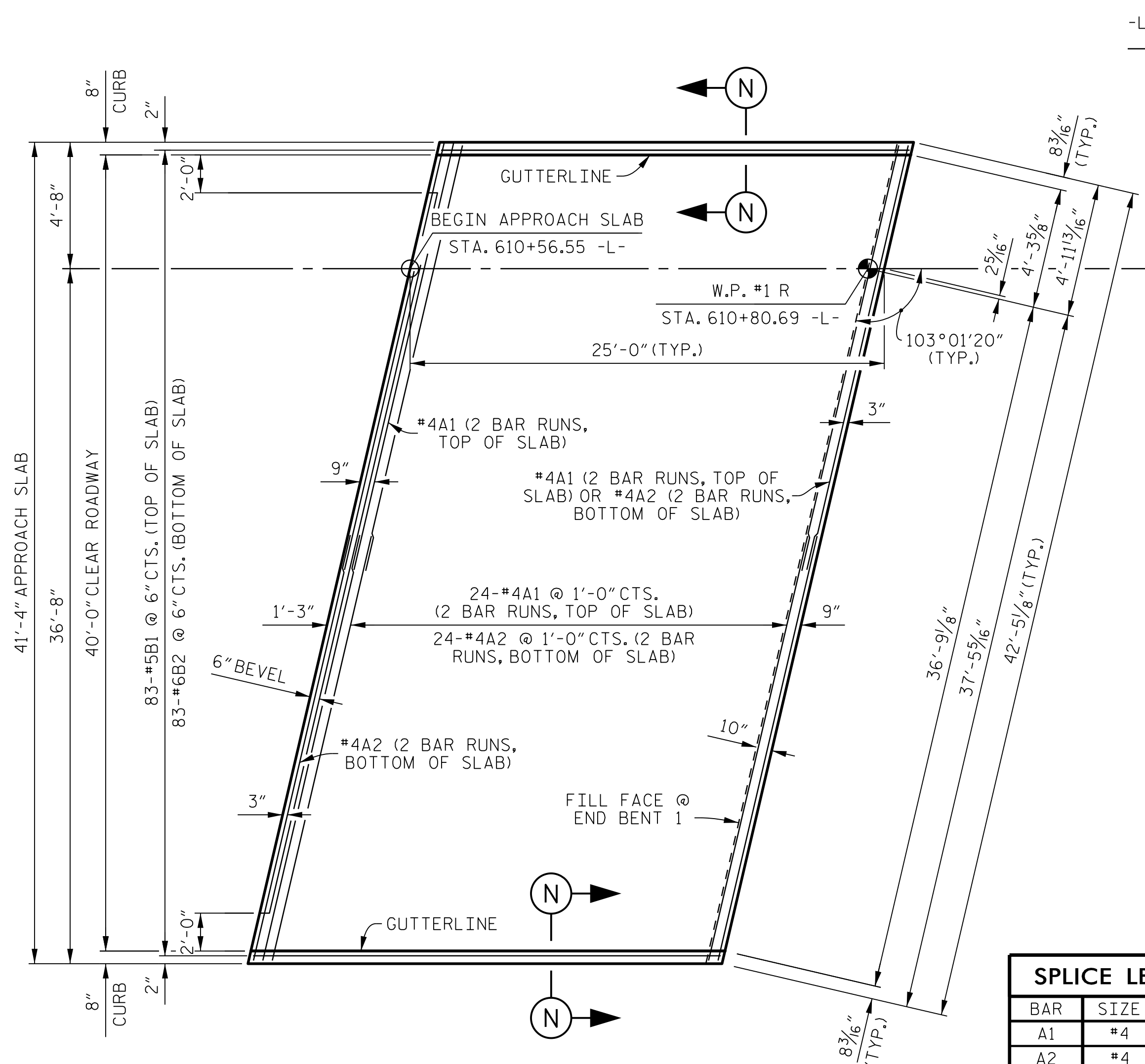
**NOTES**

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

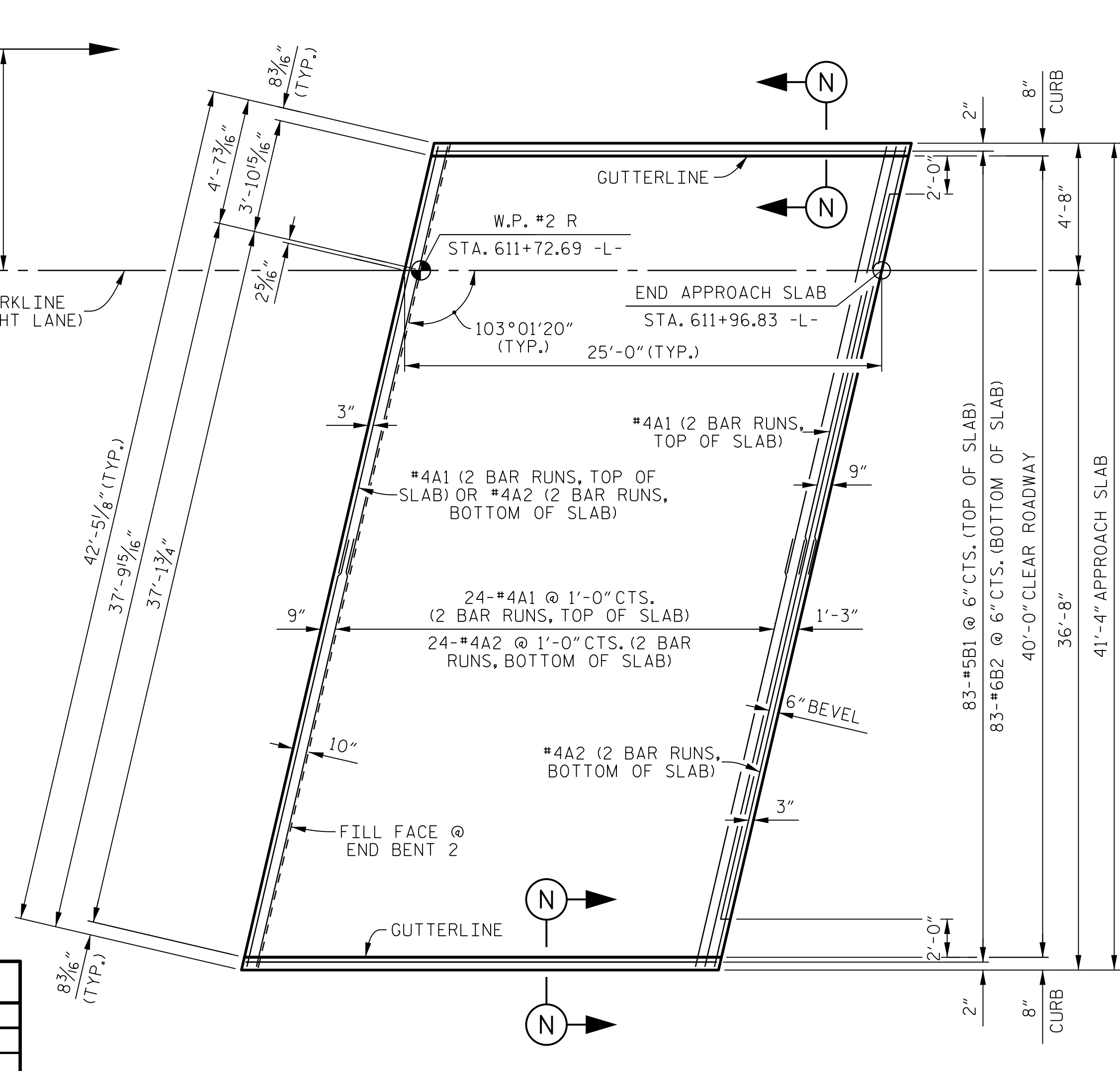
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWS NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.



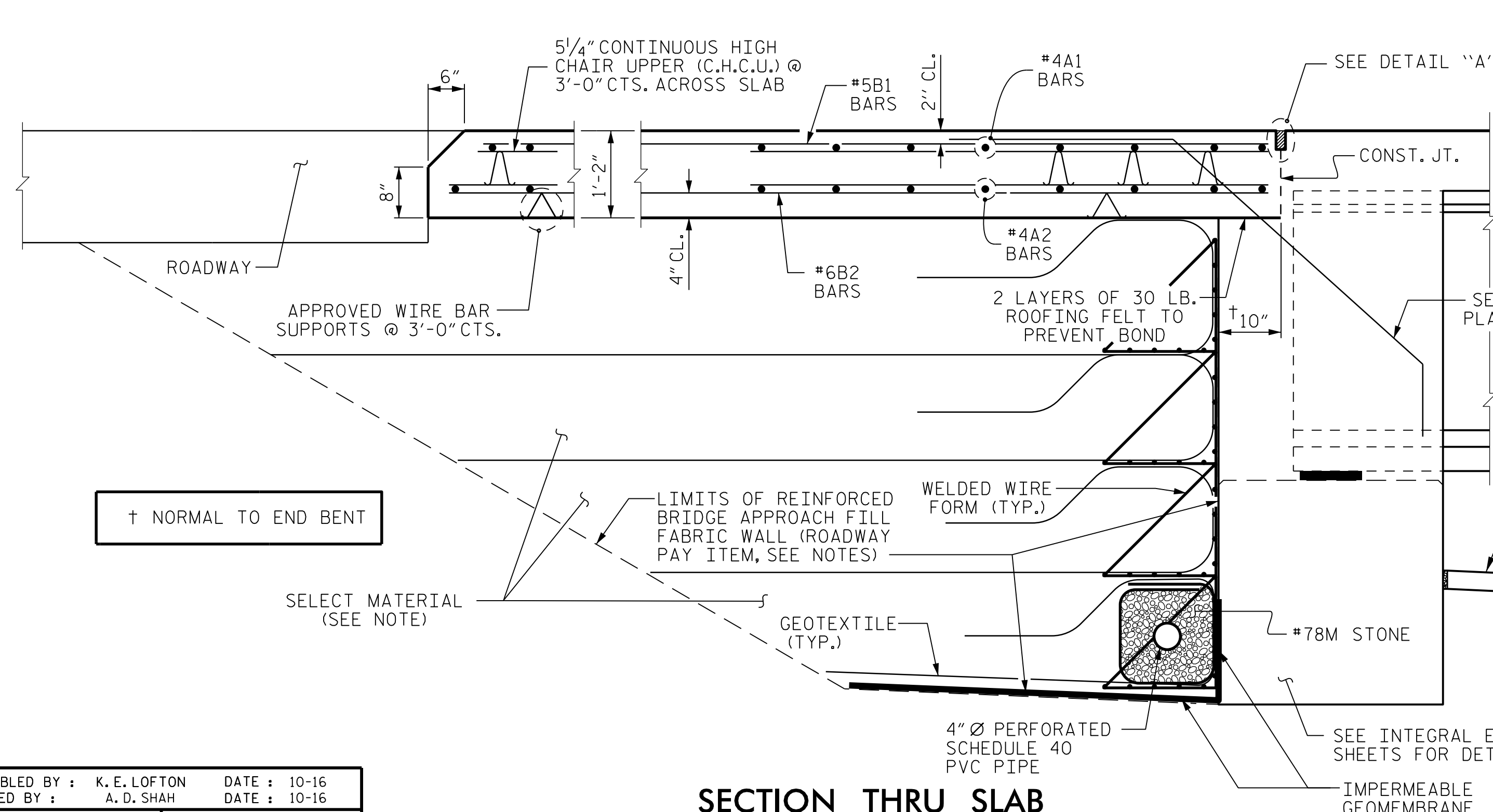
**PLAN OF APPROACH SLAB AT END BENT 1**



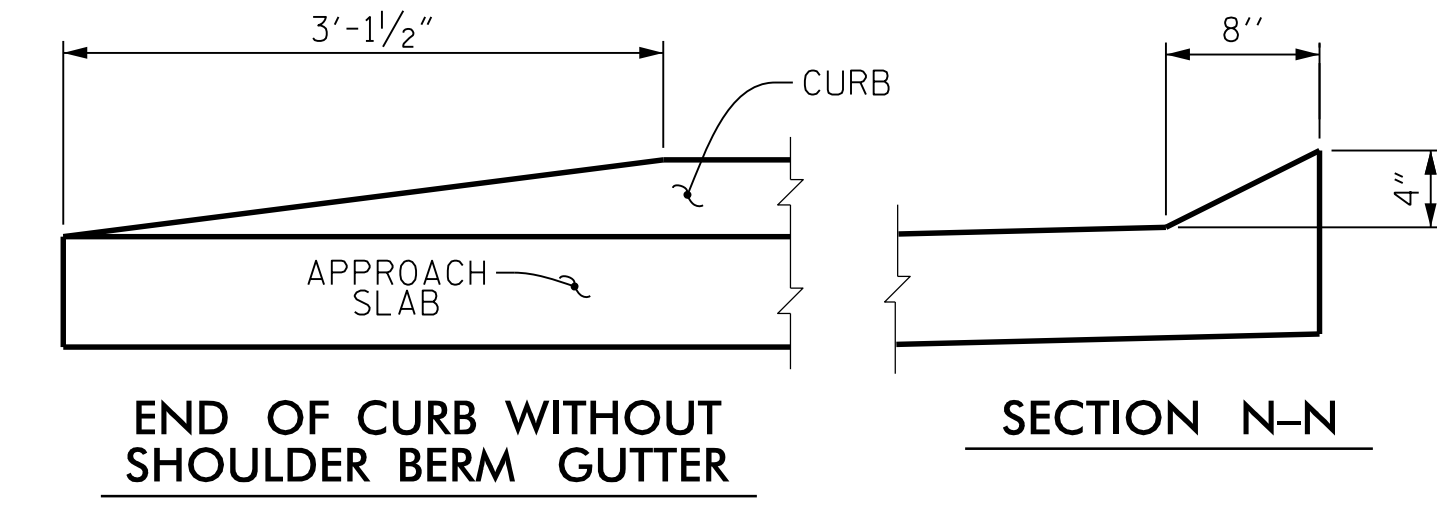
**PLAN OF APPROACH SLAB AT END BENT 2**

**SPLICE LENGTHS**

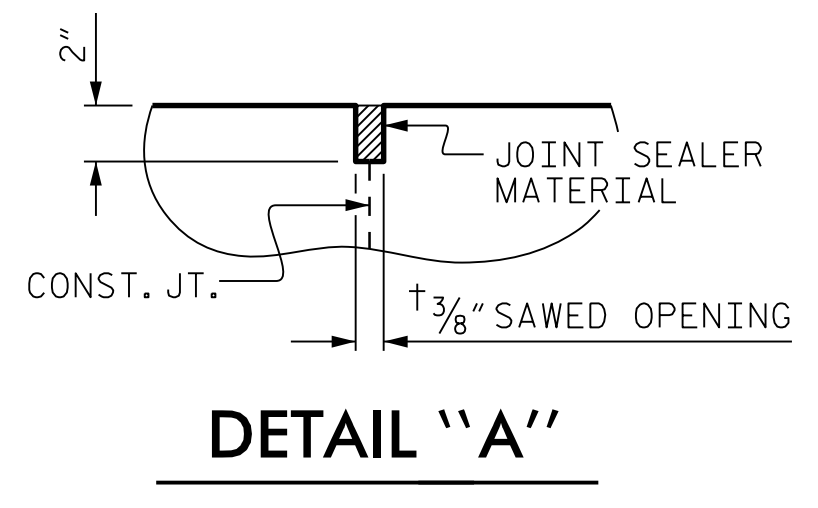
BAR	SIZE	LENGTH
A1	#4	2'-0"
A2	#4	1'-9"



**SECTION THRU SLAB**



**CURB DETAILS**



**DETAIL "A"**

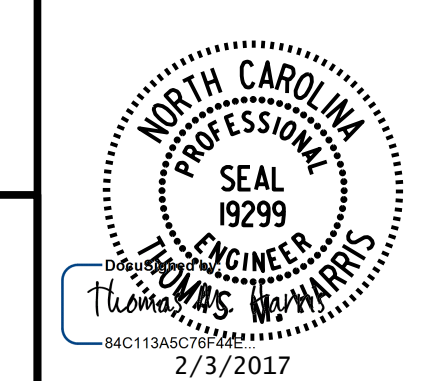
PROJECT NO. **R-2707C**  
**CLEVELAND COUNTY**  
 STATION: **611+32.01 -L-**

SHEET 1 OF 2

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 STANDARD  
**BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT (RIGHT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	S10-24
1			3			TOTAL SHEETS
2			4			25

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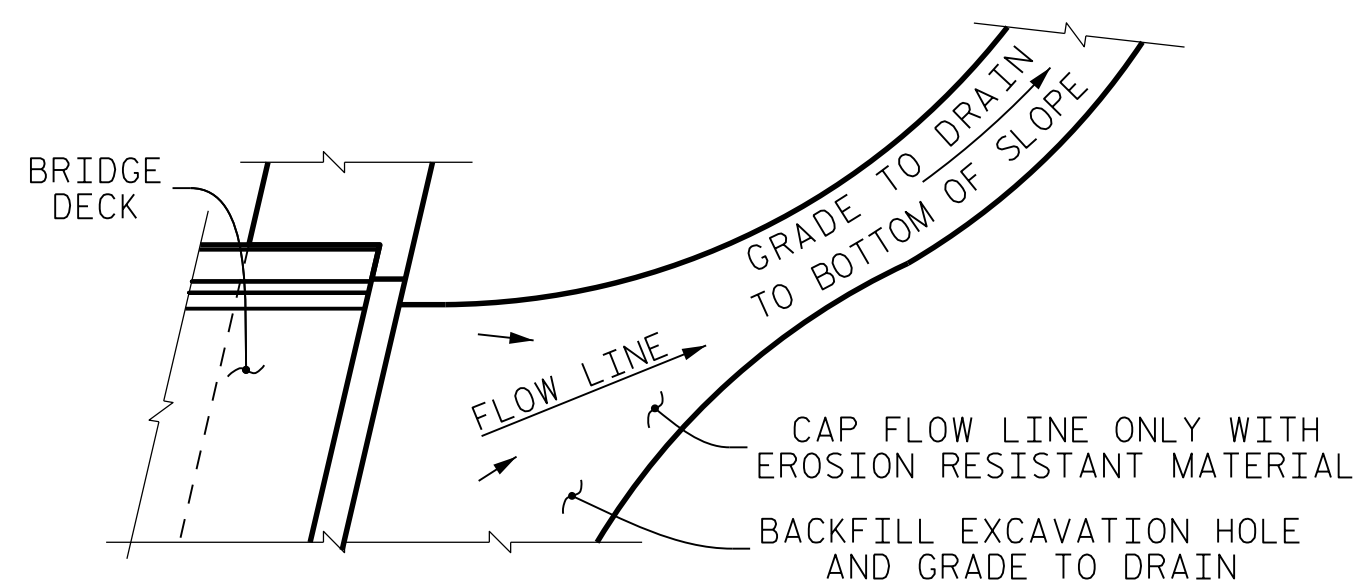
PLANS PREPARED BY:  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246

DRAWN BY: **K. E. LOFTON** DATE: 10-16  
 CHECKED BY: **A. D. SHAH** DATE: 10-16  
 DESIGN ENGINEER: **T. M. HARRIS** DATE: 10-16

ASSEMBLED BY: **K. E. LOFTON** DATE: 10-16  
 CHECKED BY: **A. D. SHAH** DATE: 10-16  
 DRAWN BY: **TLA** 10/05  
 CHECKED BY: **GM** 5/06

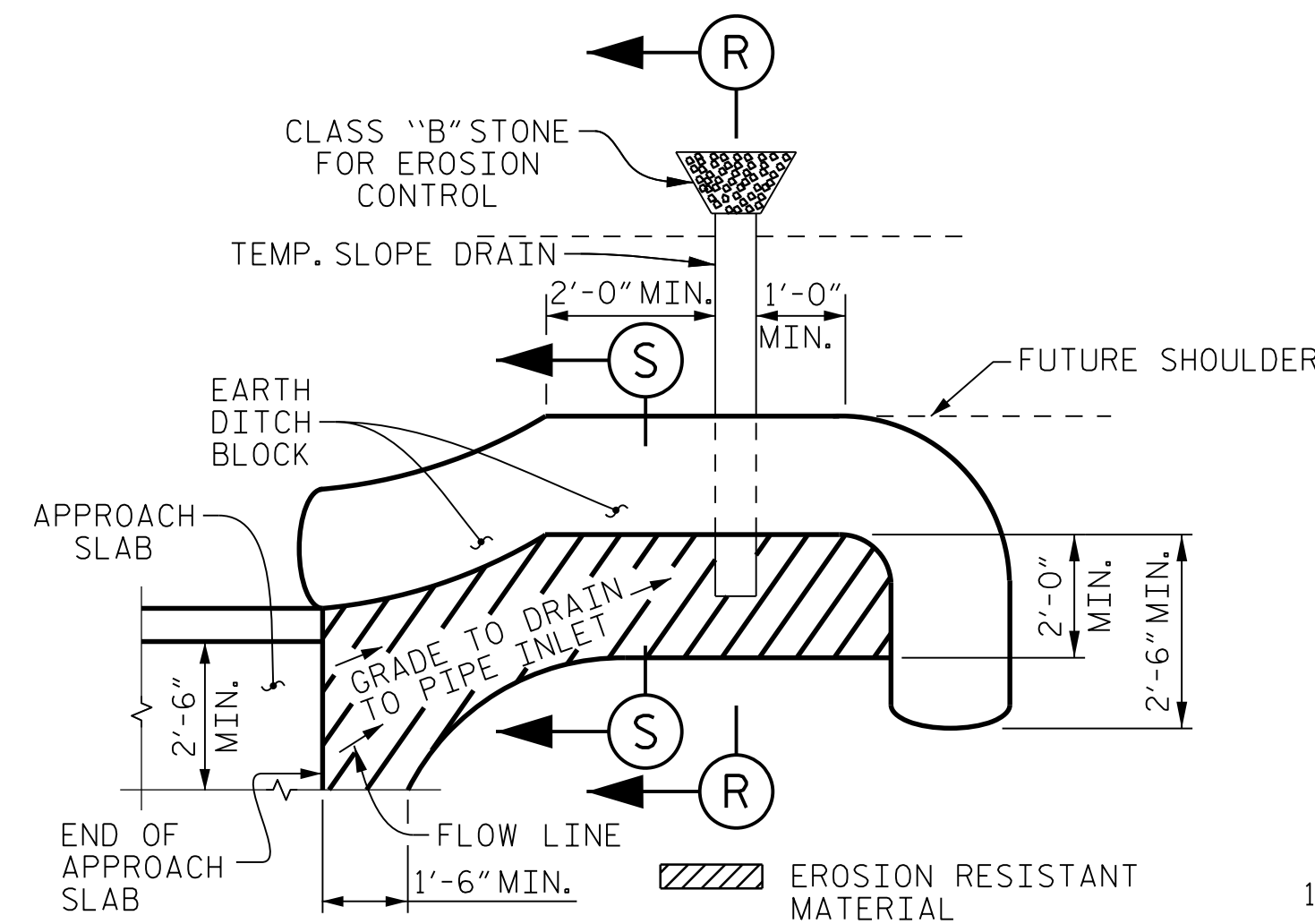
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IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

**TEMPORARY DRAINAGE DETAIL**

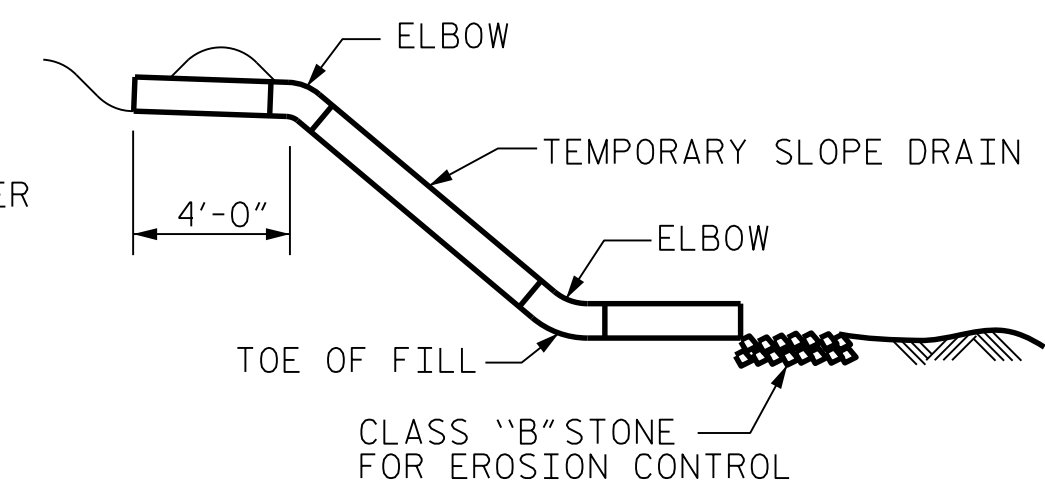


NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

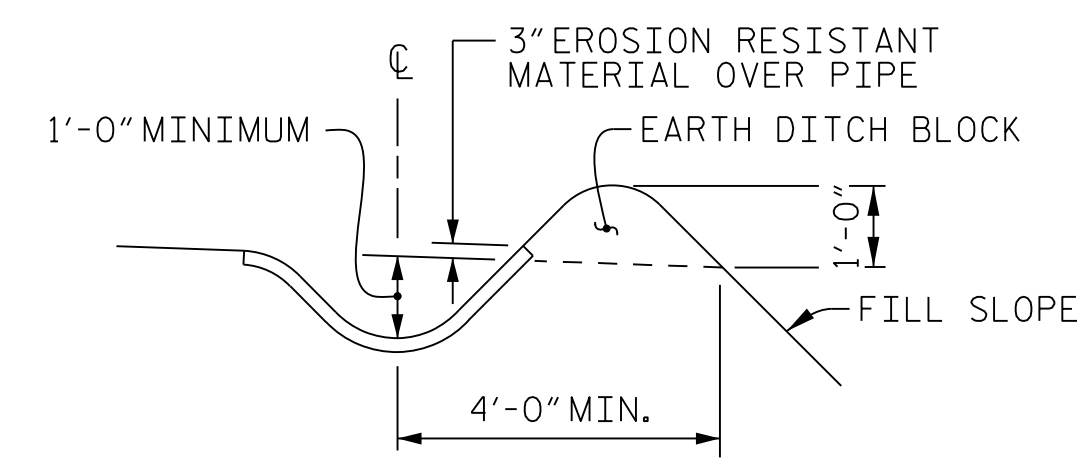
**PLAN VIEW**

**TEMPORARY BERM AND SLOPE DRAIN DETAILS**

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



**SECTION R-R**



**SECTION S-S**

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 611+32.01 -L-

SHEET 2 OF 2

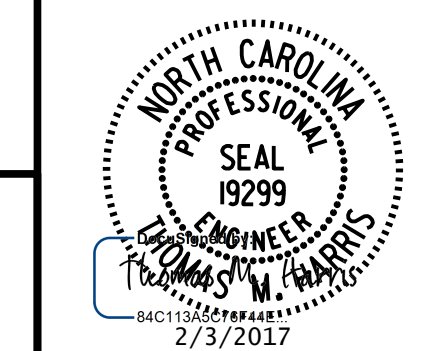
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

STANDARD

**BRIDGE APPROACH  
 SLAB DETAILS  
 (RIGHT LANE)**

REVISIONS						SHEET No.
No.	BY:	DATE:	No.	BY:	DATE:	TOTAL SHEETS
1			3			25
2			4			25

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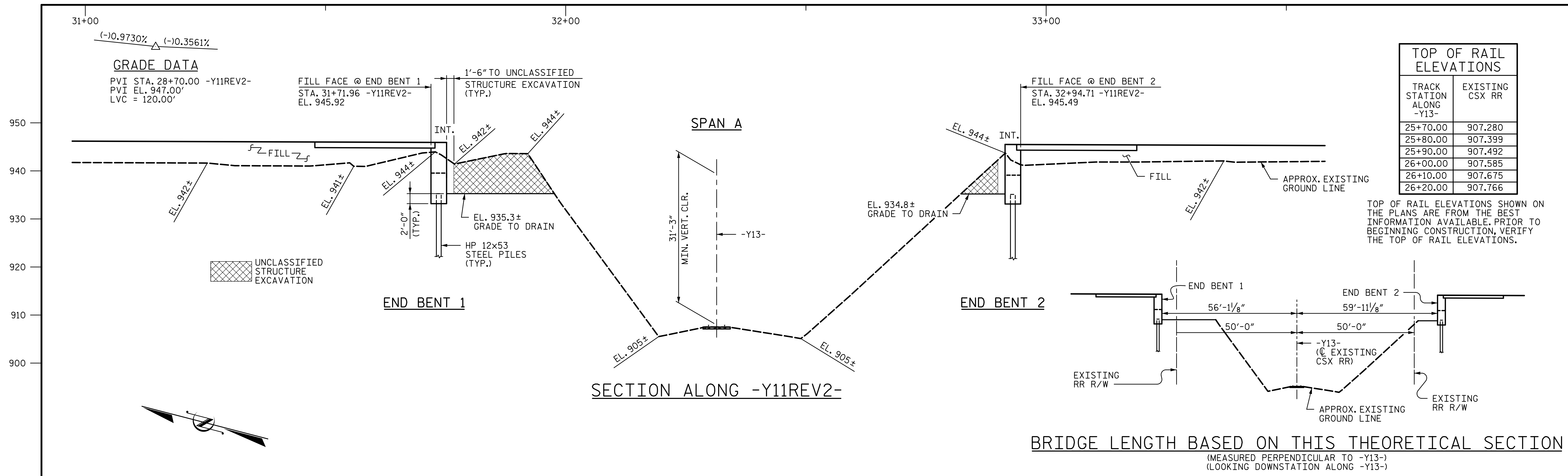


PLANS PREPARED BY :  
**PARSONS**  
 5540 CenterView Drive, Suite 217  
 Raleigh, NC 27606-3386  
 NC LICENSE No. F-0246  
 FOR NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DRAWN BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DESIGN ENGINEER : T. M. HARRIS DATE : 10-16

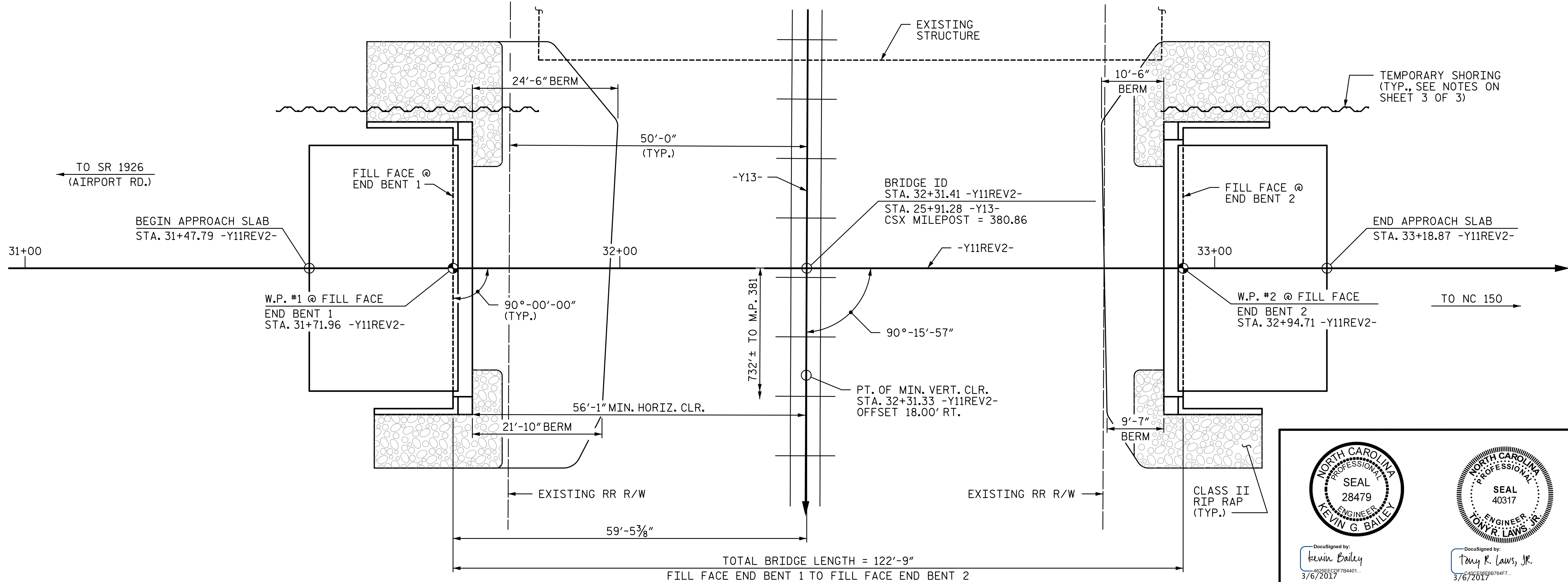
ASSEMBLED BY : K. E. LOFTON DATE : 10-16  
 CHECKED BY : A. D. SHAH DATE : 10-16  
 DRAWN BY : FCJ 11/88 REV. 10/1/11 MAA/GM  
 CHECKED BY : ABR 11/88 REV. 7/12 MAA/GM  
 DATE: 10/20/17 REV. 6/13 MAA/GM

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TOP OF RAIL ELEVATIONS	
TRACK STATION ALONG -Y13-	EXISTING CSX RR
25+70.00	907.280
25+80.00	907.399
25+90.00	907.492
26+00.00	907.585
26+10.00	907.675
26+20.00	907.766

TOP OF RAIL ELEVATIONS SHOWN ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE, PRIOR TO BEGINNING CONSTRUCTION, VERIFY THE TOP OF RAIL ELEVATIONS.



**PLAN**

(END BENT PILES NOT SHOWN FOR CLARITY)

PROJECT NO. **R-2707C**  
**CLEVELAND** COUNTY  
 STATION: **32+31.41 -Y11REV2-**  
**25+91.28 -Y13-**  
 SHEET 1 OF 3 MILEPOST 380.86 BRIDGE NO. 476

**NORTH CAROLINA PROFESSIONAL SEAL**  
 28479  
 ENGINEER  
 KEVIN G. BAILEY

**NORTH CAROLINA PROFESSIONAL SEAL**  
 40317  
 ENGINEER  
 TONY R. LAWS, JR.

DocuSigned by:  
 Kevin Bailey  
 3/6/2017

DocuSigned by:  
 Tony R. Laws, Jr.  
 3/6/2017

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
**GENERAL DRAWING**  
**BRIDGE ON -Y11REV2- (NC 180)**  
**OVER -Y13- (CSX RR)**  
**BETWEEN SR 1926 & NC 150**

DRAWN BY : <b>MBC</b>	DATE : <b>9-16</b>	DESIGN ENGINEER OF RECORD : <b>T. LAWS</b>	DATE : <b>9-16</b>
CHECKED BY : <b>AJP</b>	DATE : <b>9-16</b>		

REVISIONS				SHEET NO.
NO.	BY:	DATE:	NO.	DATE:
1			3	
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

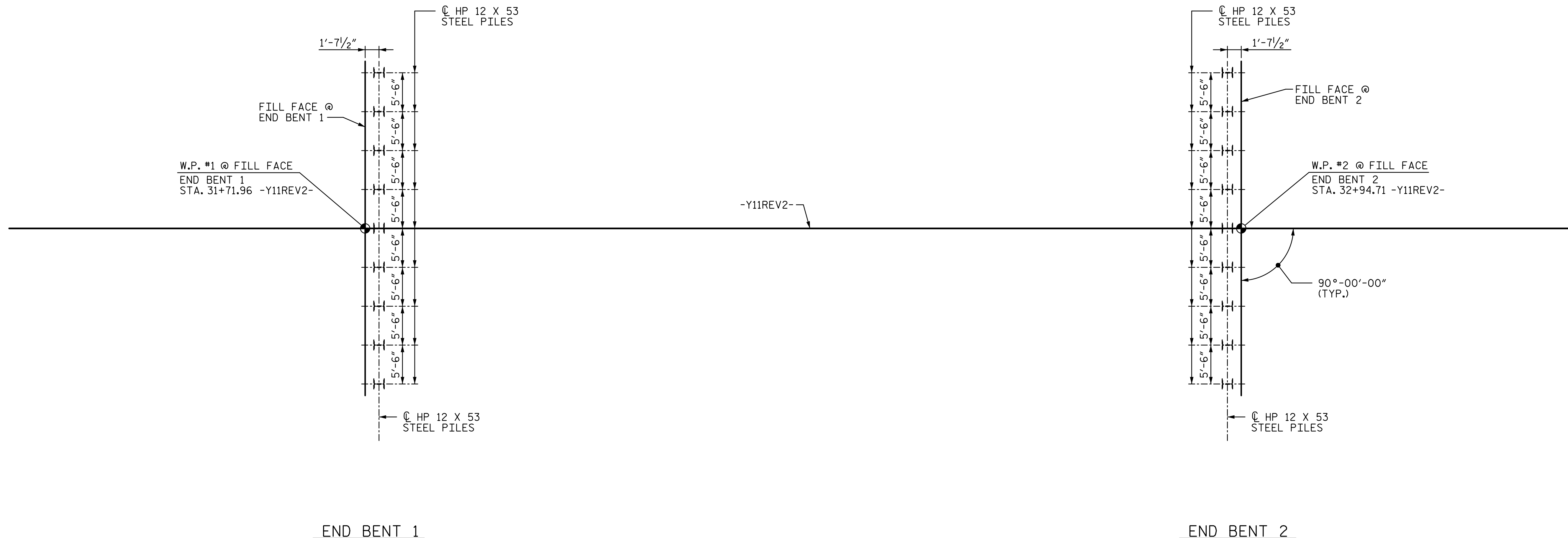
**STV** 100 years

STV ENGINEERS, INC.  
 900 West Trade St., Suite 715  
 Charlotte, NC 28202  
 NC License Number F-0991

SHEET NO. **S11-01**  
 TOTAL SHEETS **27**

STR. #11





FOUNDATION LAYOUT

**FOUNDATION NOTES:**

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 110 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 183 TONS PER PILE.

PROJECT NO. R-2707C

CLEVELAND COUNTY

STATION: 32+31.41 -Y11REV2-

25+91.28 -Y13-

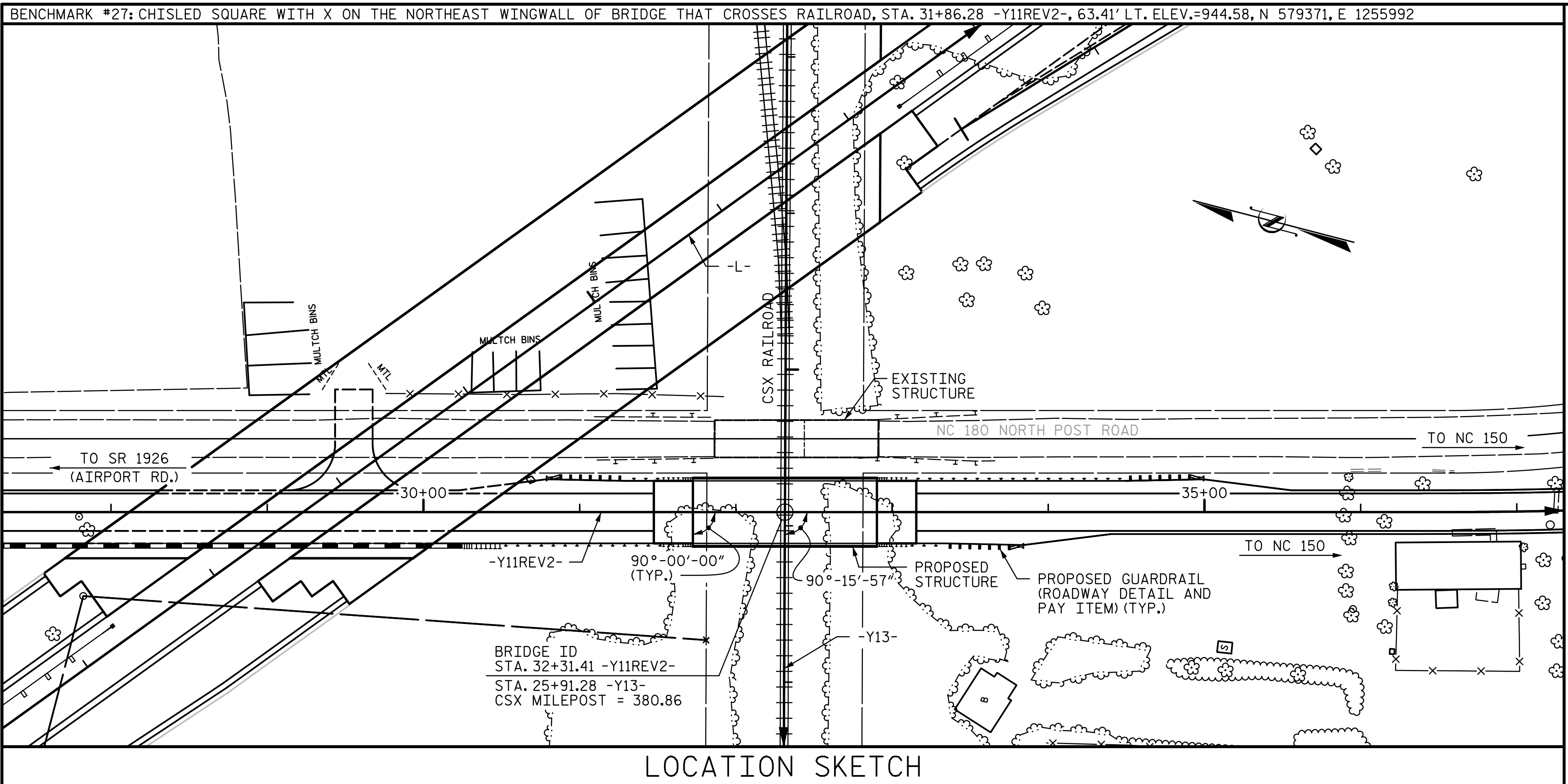
SHEET 2 OF 3

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DRAWN BY : <u>MBC</u>	DATE : <u>9-16</u>	DESIGN ENGINEER OF RECORD: <u>T. LAWS</u>	DATE : <u>9-16</u>
CHECKED BY : <u>AJP</u>	DATE : <u>9-16</u>		

				STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH <b>GENERAL DRAWING</b> <b>FOUNDATION LAYOUT</b>	
DocuSigned by: Kevin Bailey 3/6/2017		DocuSigned by: Tony R. Laws, Jr. 3/6/2017			
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				REVISIONS	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S11-02 TOTAL SHEETS 27

**STV** 100 years  
 STV ENGINEERS, INC.  
 900 West Trade St., Suite 715  
 Charlotte, NC 28202  
 NC License Number F-0991



FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.

TOTAL BILL OF MATERIAL

	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	MODIFIED 63" PRESTRESSED CONCRETE GIRDERS	PILE DRIVING EQUIP. SETUP FOR HP 12x53 STEEL PILES	HP 12x53 STEEL PILES	CONCRETE BARRIER RAIL	60" CHAIN LINK FENCE	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	SQ. FT.	SQ. FT.	CU. YD.	LUMP SUM	LBS.	NO. LIN. FT.	EA.	NO. LIN. FT.	LIN. FT.	LIN. FT.	TONS	SQ. YD.	LUMP SUM	LUMP SUM
SUPERSTRUCTURE			5,309	6,330		LUMP SUM		5 604.58			242.2	228.0			LUMP SUM	LUMP SUM
END BENT 1					45.3		6,193		9	9 495			68	75		
END BENT 2					45.3		6,193		9	9 360			68	75		
TOTAL	LUMP SUM	LUMP SUM	5,309	6,330	107.2	LUMP SUM	12,386	5 604.58	18	18 855	242.2	228.0	136	150	LUMP SUM	LUMP SUM

GENERAL NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR FOUNDATION NOTES, SEE "FOUNDATION LAYOUT" SHEET.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE RAILROAD TRACK TOP OF RAIL ELEVATIONS SHOWN ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE TOP OF RAIL ELEVATIONS AND REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

STEEL SHEET PILING REQUIRED FOR SHORING SHALL BE HOT ROLLED.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA (ON SHEET 1 OF 3) SHALL BE EXCAVATED FOR A DISTANCE FROM THE CENTERLINE OF ROADWAY 38'± (LEFT) AND 34'± (RIGHT) AT BOTH END BENTS AND TO AN ELEVATION OF 935.3± AT END BENT 1 AND 934.8± AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF (1) 47'-6" AND (1) 57'-6" SPANS WITH REINFORCED CONCRETE DECK ON STEEL I-BEAMS, WITH A CLEAR ROADWAY OF 24'-0" AND SUPPORTED BY REINFORCED CONCRETE ABUTMENTS, REINFORCED CONCRETE POST AND BEAM BENT ON SPREAD FOOTINGS SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 32+31.41 -Y11REV2-".

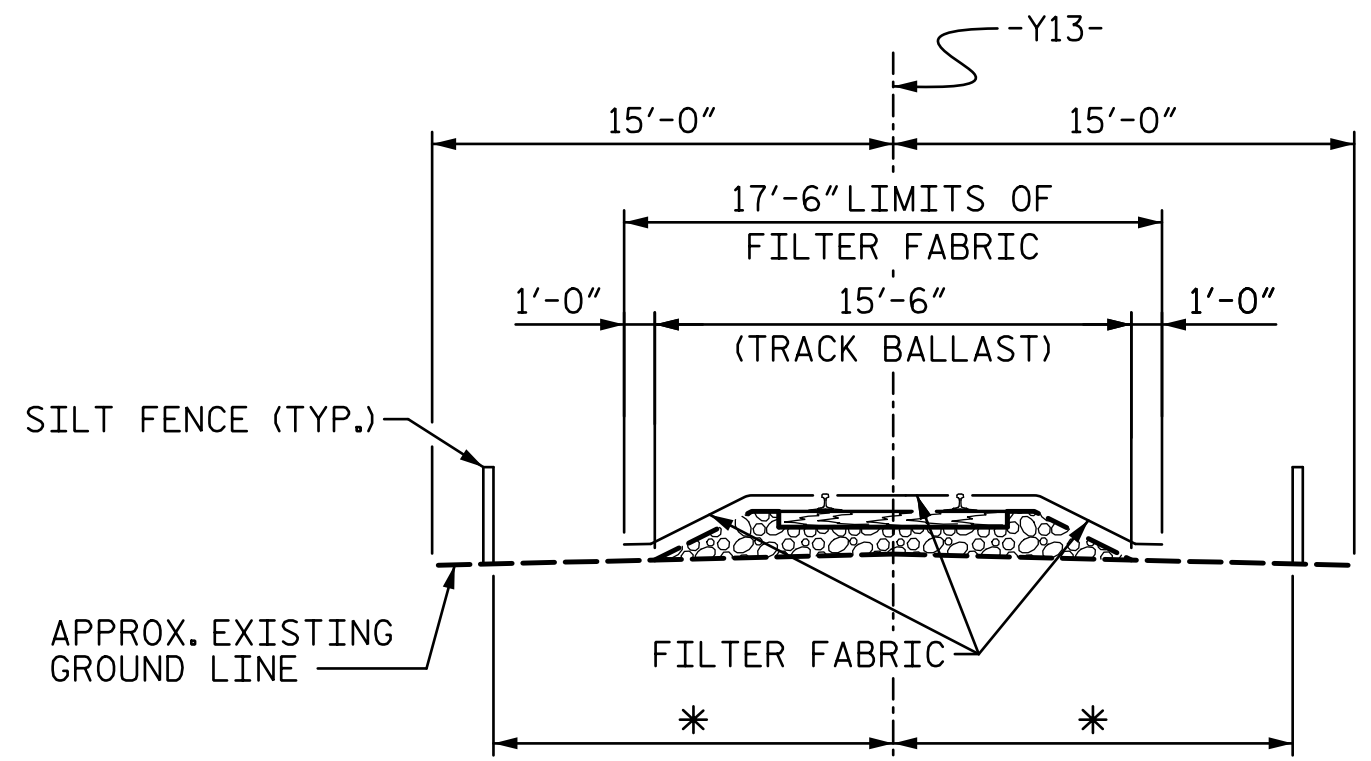
FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

ANY SOIL EXCAVATED DUE TO CONSTRUCTION ACTIVITIES ON CSXT RIGHT OF WAY SHALL NOT BE REMOVED FROM THE PROPERTY. ANY EXCESS SOIL THAT IS NOT REUSED WITHIN THE CSXT RIGHT OF WAY SHALL BE TESTED BY A RAILROAD REPRESENTATIVE FOR CONTAMINATION AND DISPOSAL ACCORDINGLY AT AN APPROVED LANDFILL. CSXT WILL NOT BEAR ANY COSTS RELATED TO DISPOSAL OF SOILS GENERATED DUE TO CONSTRUCTION ACTIVITY RELATED TO THIS PROJECT.

FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.

FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
25+91.28 -Y13-  
 SHEET 3 OF 3



NOTES:

RAILROAD EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO PERFORMING ANY WORK IN THE RAILROAD RIGHT-OF-WAY.

ADDITIONAL EROSION CONTROL MEASURES FOR PROTECTION OF RAILROAD DITCHES MAY BE REQUIRED AS DIRECTED BY THE ENGINEER.

NO SEPARATE PAYMENT WILL BE MADE FOR RAILROAD EROSION CONTROL MEASURES.

LIMITS OF SILT FENCE AND FILTER FABRIC PARALLEL TO RAILROAD SHALL EXTEND A MINIMUM OF 25'-0" OUTSIDE EDGE OF SUPERSTRUCTURE OR TOE OF SLOPE ON CONSTRUCTION. A GREATER LENGTH OF SILT FENCE OR FILTER FABRIC MAY BE REQUIRED IF SO DIRECTED BY THE ENGINEER.

FILTER FABRIC TO BE NAILED TO TIMBER RAIL TIES WITH PRIME SOURCE "GRIP CAP" OR EQUIVALENT. FILTER FABRIC ON SHOULDER TO BE SECURED AS DIRECTED BY THE ENGINEER AND RAILROAD.

RAILROAD EROSION CONTROL DETAIL

\* TO BE DETERMINED BY THE RESIDENT ENGINEER IN CONSULTATION WITH THE RAILROAD ENGINEER.

DRAWN BY : <u>MBC</u>	DATE : <u>9-16</u>	DESIGN ENGINEER OF RECORD: <u>T. LAWS</u>	DATE : <u>9-16</u>
CHECKED BY : <u>AJP</u>	DATE : <u>9-16</u>		

DocuSigned by:  
Kevin Bailey  
4025EEDCF784401...  
3/30/2017

DocuSigned by:  
Tony R. Laws, Jr.  
CA0C06F6E764F7...  
3/30/2017

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH  
GENERAL DRAWING  
LOCATION SKETCH,  
GENERAL NOTES AND TOTAL  
BILL OF MATERIAL

REVISIONS

NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

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STV 100 years

STV ENGINEERS, INC.  
900 West Trade St., Suite 715  
Charlotte, NC 28202  
NC License Number F-0991

SHEET NO. S11-03  
TOTAL SHEETS 27

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LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	$\gamma_{DC}$	$\gamma_{DW}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING (#)	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					COMMENT NUMBER			
						LIVE-LOAD FACTORS (%LL)	MOMENT					SHEAR					LIVE-LOAD FACTORS (%LL)	MOMENT						
							DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)		DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN		GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.08	--	1.75	0.80	1.18	A	E	59.75	0.90	1.22	A	I	11.4	0.80	0.80	1.08	A	E	59.75		
	HL-93 (OPERATING)	N/A		1.54	--	1.35	0.80	1.54	A	E	59.75	0.90	1.70	A	I	11.4	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.57	56.52	1.75	0.80	1.73	A	E	59.75	0.90	1.83	A	I	11.4	0.80	0.80	1.57	A	E	59.75		
	HS-20 (OPERATING)	36.000		2.24	80.64	1.35	0.80	2.24	A	E	59.75	0.90	2.42	A	I	11.4	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500		3.80	51.30	1.40	0.80	5.24	A	E	59.75	0.90	6.10	A	I	11.4	0.80	0.80	3.80	A	E	59.75	
		SNGARBS2	20.000		2.72	54.40	1.40	0.80	3.74	A	E	59.75	0.90	4.19	A	I	11.4	0.80	0.80	2.72	A	E	59.75	
		SNAGRIS2	22.000		2.53	55.66	1.40	0.80	3.48	A	E	59.75	0.90	3.84	A	I	11.4	0.80	0.80	2.53	A	E	59.75	
		SNCOTTS3	27.250		1.89	51.50	1.40	0.80	2.60	A	E	59.75	0.90	2.94	A	I	11.4	0.80	0.80	1.89	A	E	59.75	
		SNAGGRS4	34.925		1.53	53.44	1.40	0.80	2.11	A	E	59.75	0.90	2.34	A	I	11.4	0.80	0.80	1.53	A	E	59.75	
		SNS5A	35.550		1.50	53.33	1.40	0.80	2.07	A	E	59.75	0.90	2.34	A	I	11.4	0.80	0.80	1.50	A	E	59.75	
		SNS6A	39.950		1.36	54.33	1.40	0.80	1.87	A	E	59.75	0.90	2.10	A	I	11.4	0.80	0.80	1.36	A	E	59.75	
		SNS7B	42.000		1.30	54.60	1.40	0.80	1.78	A	E	59.75	0.90	2.03	A	I	11.4	0.80	0.80	1.30	A	E	59.75	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		1.65	54.45	1.40	0.80	2.28	A	E	59.75	0.90	2.55	A	I	11.4	0.80	0.80	1.65	A	E	59.75	
		TNT4A	33.075		1.66	54.90	1.40	0.80	2.28	A	E	59.75	0.90	2.50	A	I	11.4	0.80	0.80	1.66	A	E	59.75	
		TNT6A	41.600		1.34	55.74	1.40	0.80	1.84	A	E	59.75	0.90	2.12	A	I	11.4	0.80	0.80	1.34	A	E	59.75	
		TNT7A	42.000		1.34	56.28	1.40	0.80	1.84	A	E	59.75	0.90	2.08	A	I	11.4	0.80	0.80	1.34	A	E	59.75	
		TNT7B	42.000		1.36	57.12	1.40	0.80	1.88	A	E	59.75	0.90	2.00	A	I	11.4	0.80	0.80	1.36	A	E	59.75	
		TNAGRIT4	43.000		1.31	56.33	1.40	0.80	1.81	A	E	59.75	0.90	1.94	A	I	11.4	0.80	0.80	1.31	A	E	59.75	
		TNACT5A	45.000	③	1.24	55.80	1.40	0.80	1.71	A	E	59.75	0.90	1.89	A	I	11.4	0.80	0.80	1.24	A	E	59.75	
TNACT5B	45.000	③	1.24	55.80	1.40	0.80	1.70	A	E	59.75	0.90	1.84	A	I	11.4	0.80	0.80	1.24	A	E	59.75			

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1.
- 2.
- 3.
- 4.

# CONTROLLING LOAD RATING

① DESIGN LOAD RATING (HL-93)

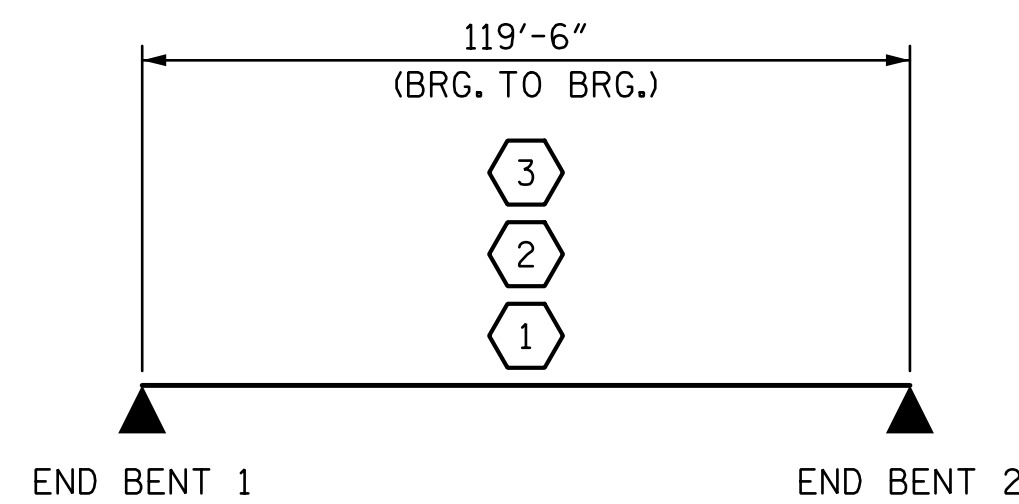
② DESIGN LOAD RATING (HS-20)

③ LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER  
E - EXTERIOR GIRDER



LRFR SUMMARY

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

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DRAWN BY: MBC DATE: 9-16  
 CHECKED BY: KGB DATE: 9-16  
 DESIGN ENGINEER OF RECORD: T. LAWS DATE: 9-16

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

LRFR SUMMARY FOR  
 PRESTRESSED  
 CONCRETE GIRDERS  
 (NON-INTERSTATE TRAFFIC)

REVISIONS

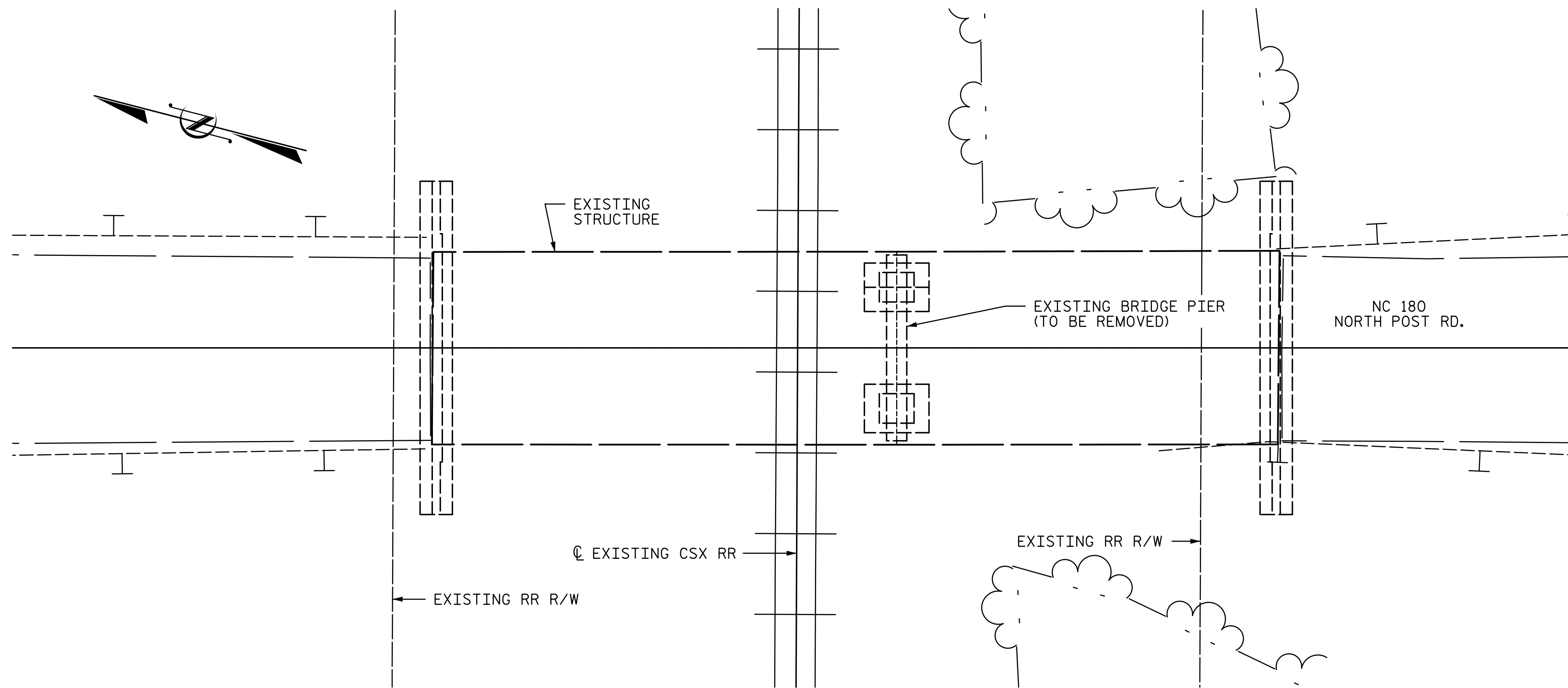
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2			4		

SHEET NO. S11-04  
 TOTAL SHEETS 27

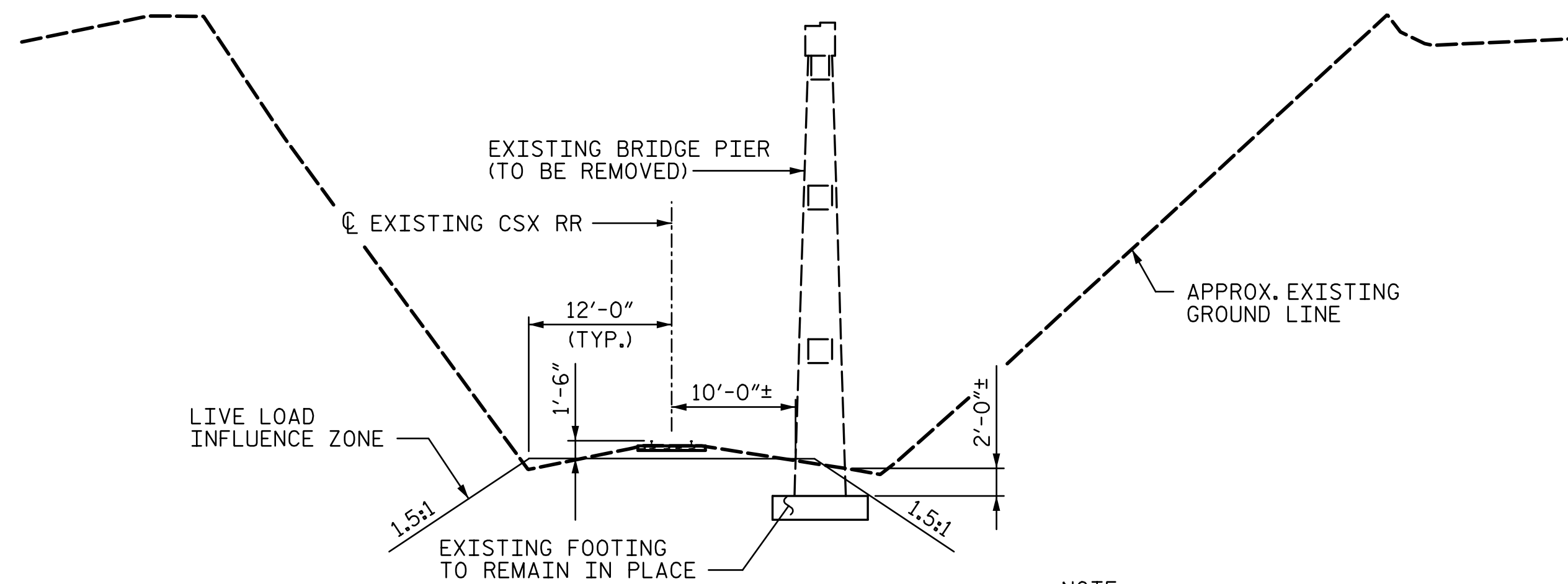
STV ENGINEERS, INC.  
 900 West Trade St., Suite 715  
 Charlotte, NC 28202  
 NC License Number F-5991

DocuSigned by:  
 Tony R. Laws, Jr.  
 3762017

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



**PLAN FOR STRUCTURE REMOVAL**  
 (FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.)



**SECTION VIEW**  
 (LOOKING DOWNSTATION ALONG  $\phi$  EXISTING CSX RR)

NOTE:  
 FOOTINGS SHALL BE LEFT IN PLACE. COLUMNS SHALL BE REMOVED TO 2'-0" BELOW GROUND LEVEL. METHOD FOR COLUMN REMOVAL SHALL BE INCLUDED IN EXISTING BRIDGE DEMOLITION PLAN. SEE RAILROAD AGREEMENT FOR EXPECTATIONS, REQUIREMENTS AND TIMING FOR COLUMN REMOVAL.

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

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 Tony R. Laws, Jr.  
 3/6/2017

**STV** 100 years  
 STV ENGINEERS, INC.  
 900 West Trade St., Suite 715  
 Charlotte, NC 28202  
 NC License Number F-5991

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

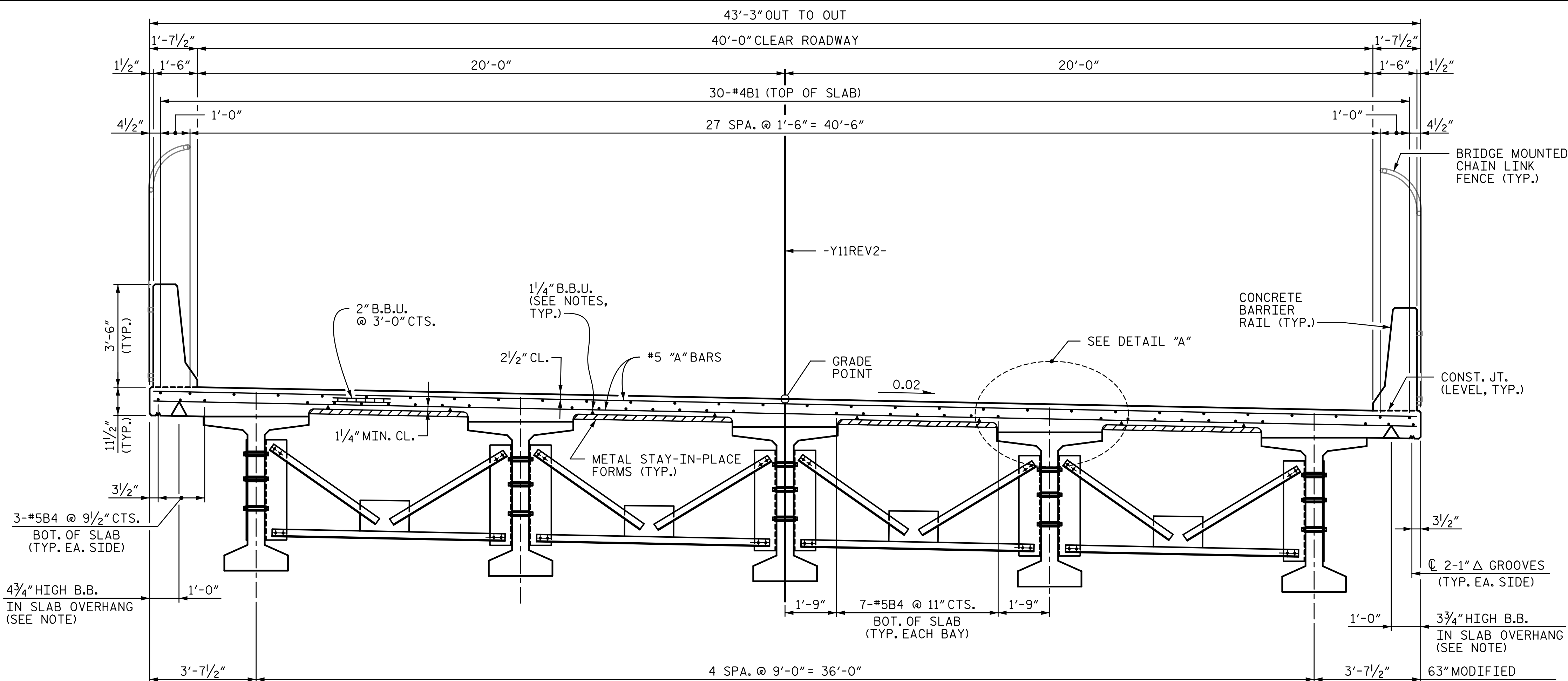
**EXISTING STRUCTURE REMOVAL**

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S11-05
1			3			TOTAL SHEETS
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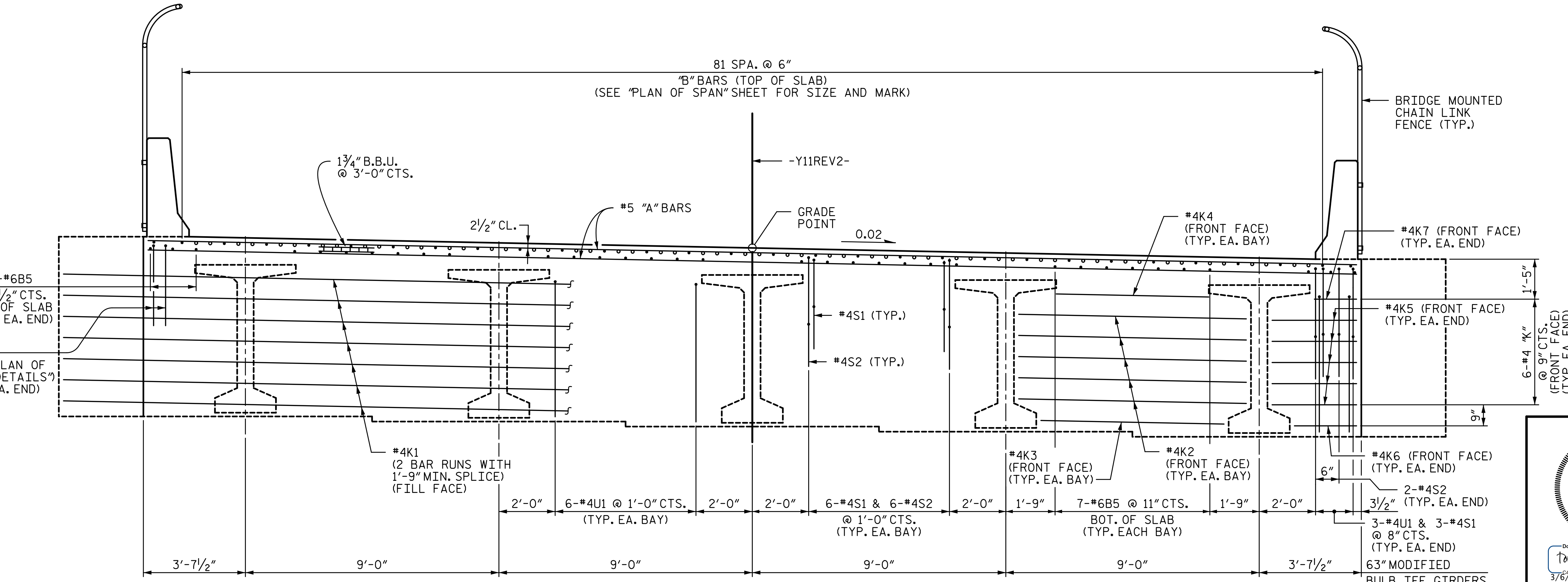
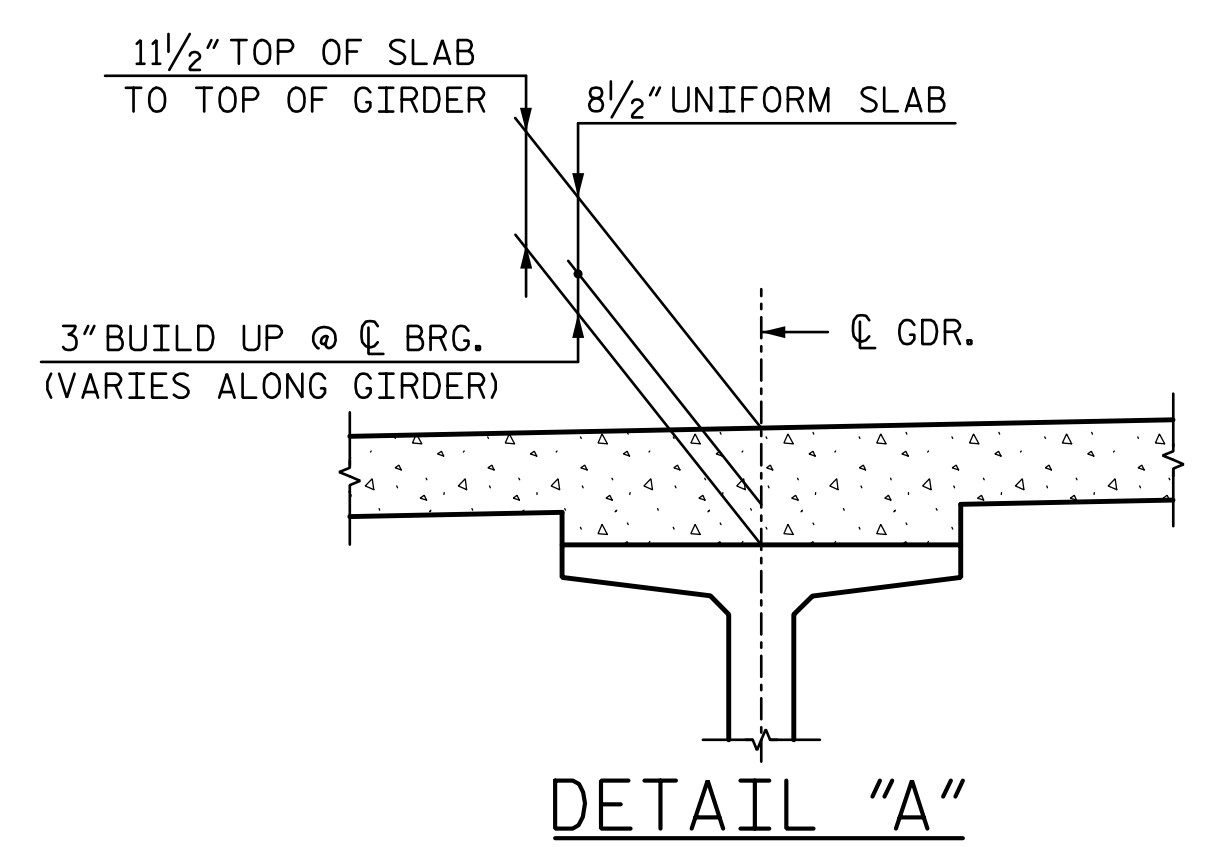




TYPICAL SECTION AT INTERMEDIATE DIAPHRAGMS

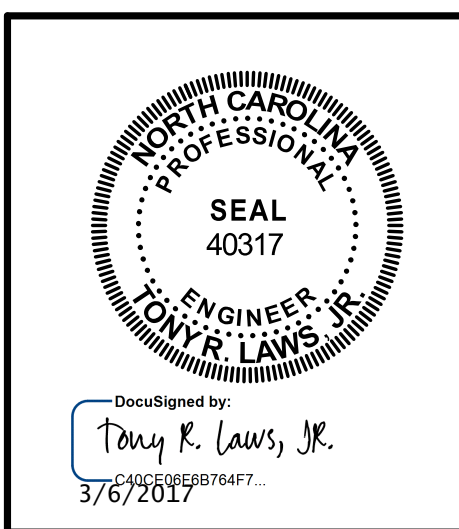
NOTES:

- PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS, WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.
- LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.
- BARRIER RAIL SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
- FOR INTERMEDIATE DIAPHRAGM DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.
- FOR BARRIER RAIL DETAILS, SEE "CONCRETE BARRIER RAIL" SHEET.
- FOR INTEGRAL BACKWALL AND WING WALL DIMENSIONS, DETAILS AND ELEVATIONS, SEE "PLAN OF SPANS DETAILS" SHEETS.
- HEIGHT OF BEAM BOLSTER IS CALCULATED @ @ BEARING. CONTRACTOR SHALL ADJUST HEIGHTS, AS NECESSARY TO MAINTAIN PROPER CLEARANCE, DUE TO GIRDER CAMBER.
- PREVIOUSLY CAST CONCRETE SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.



ELEVATION OF INTEGRAL END BENT DIAPHRAGM

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-



STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH  
 SUPERSTRUCTURE  
 TYPICAL SECTION &  
 INTEGRAL BACKWALL

REVISIONS				SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S11-06  
TOTAL SHEETS 27

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 CHECKED BY: JTG DATE: 9-16  
 DESIGN ENGINEER OF RECORD: T. LAWS DATE: 9-16

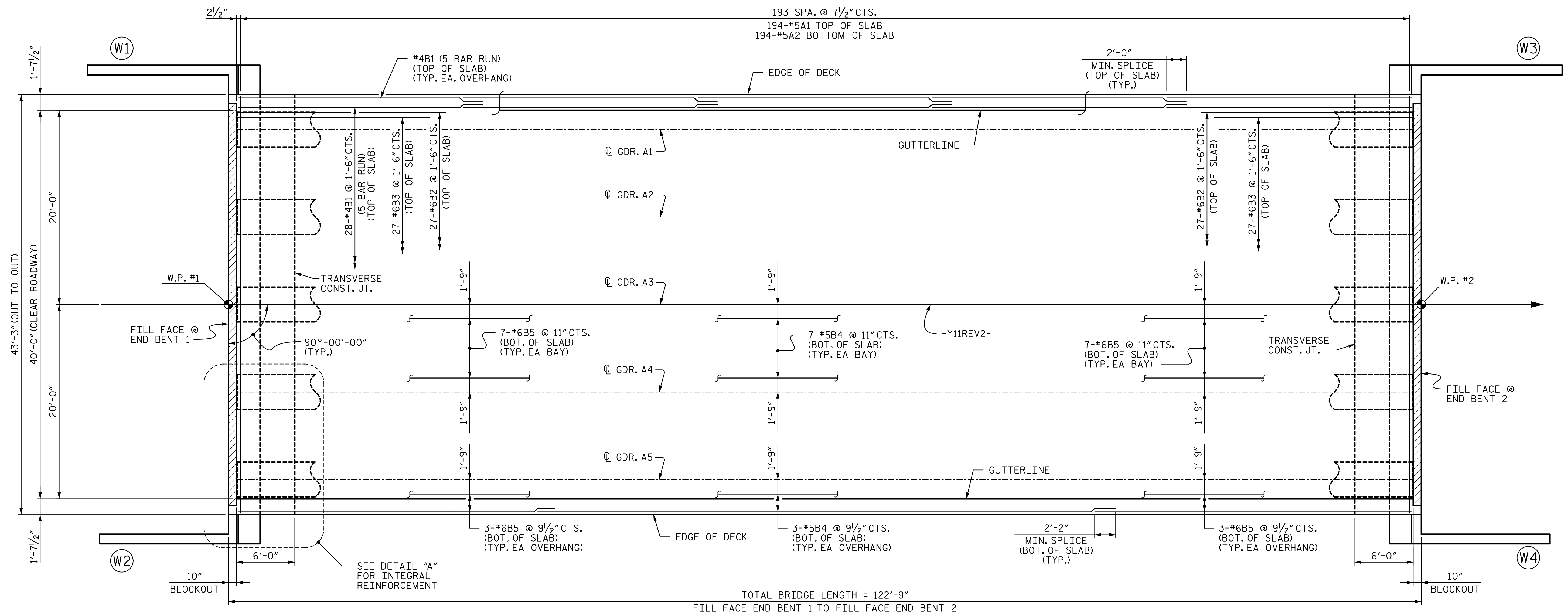
(END BENT 1 SHOWN, END BENT 2 SIMILAR)  
 (LOOKING IN DIRECTION OF STATIONING,  
 WING REINFORCEMENT NOT SHOWN FOR CLARITY.)

DOCUMENT NOT CONSIDERED FINAL  
 UNLESS ALL SIGNATURES COMPLETED

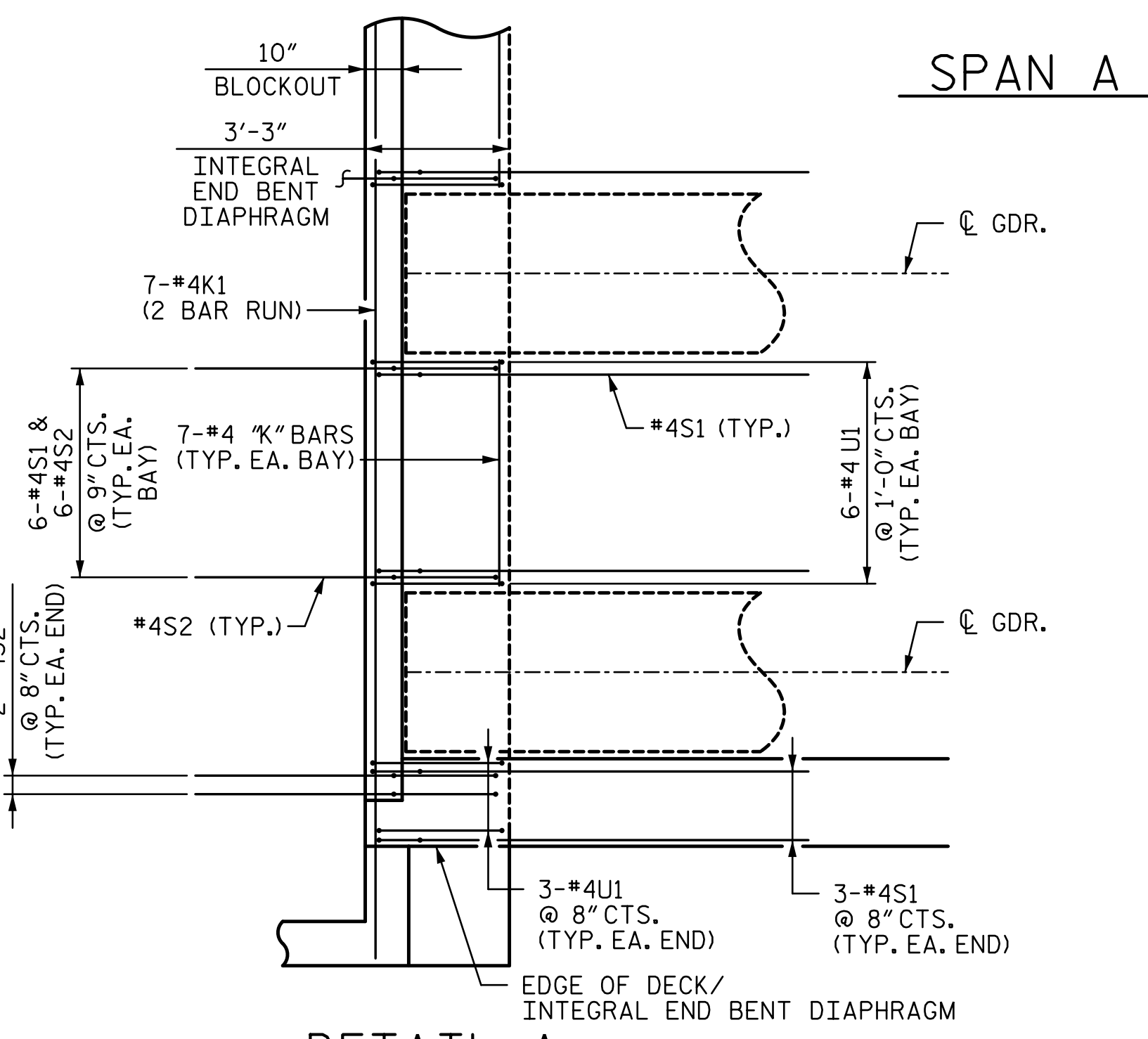
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TOTAL BRIDGE LENGTH = 122'-9"  
 FILL FACE END BENT 1 TO FILL FACE END BENT 2



**DETAIL A**  
 (END BENT 1 SHOWN, END BENT 2 MIRRORED)  
 (#4U2 BARS NOT SHOWN FOR CLARITY,  
 SEE "PLAN OF SPAN DETAILS" SHEETS)

**NOTES:**

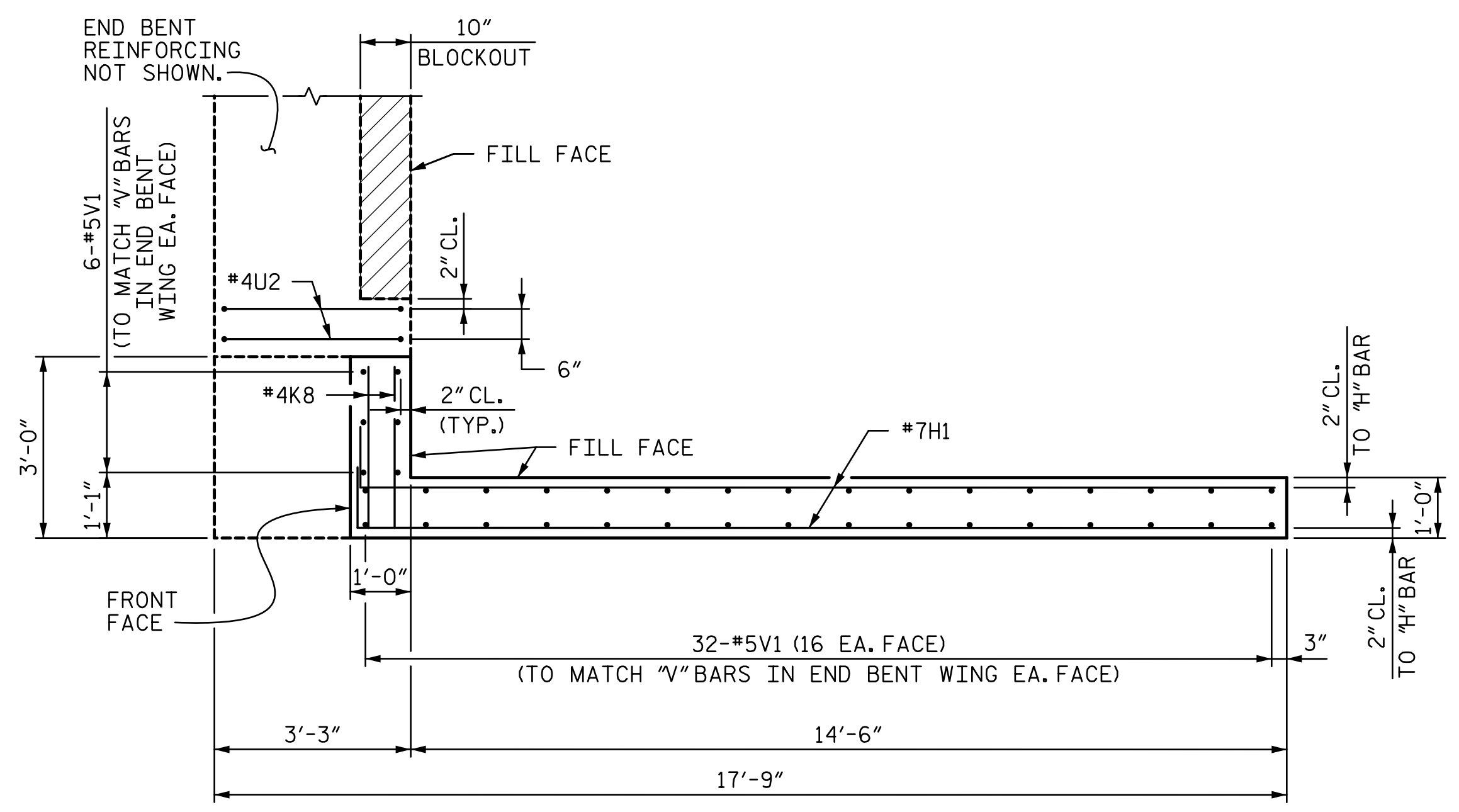
1. FOR CONCRETE BARRIER RAIL DIMENSIONS, REINFORCEMENT AND JOINT SPACING, SEE "CONCRETE BARRIER RAIL" SHEET.
2. FOR POURING SEQUENCE, SEE "SUPERSTRUCTURE BILL OF MATERIAL" SHEET.
3. FOR TRANSVERSE CONST. JOINT IN DECK SLAB, SEE "SUPERSTRUCTURE DETAILS" SHEET.
4. FOR SPACING OF TOP AND BOTTOM "B" BARS, SEE "TYPICAL SECTION AND INTEGRAL BACKWALL" SHEET.

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 1 OF 3

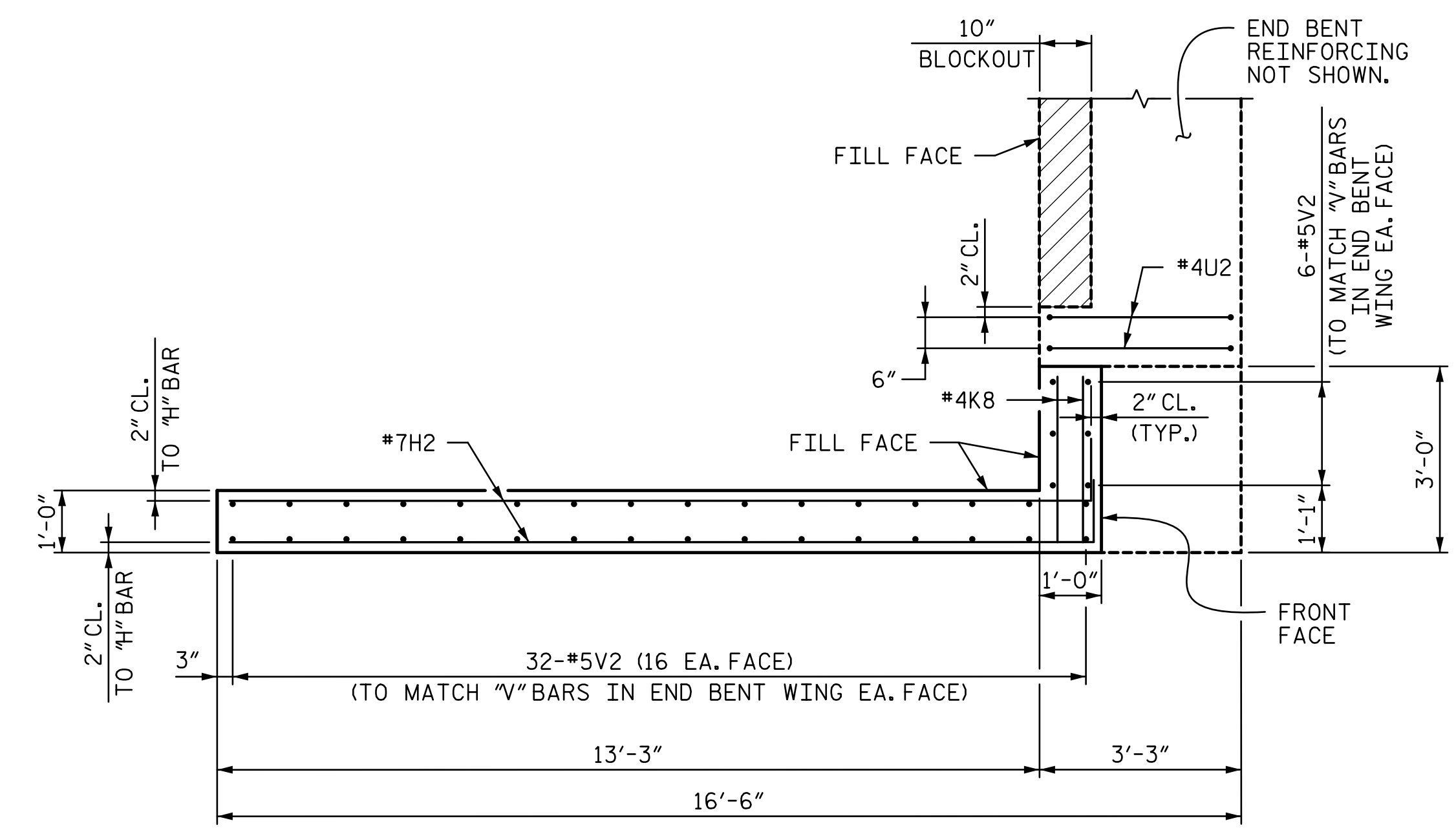
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	 SEAL 40317 ENGINEER TONY R. LAWS, JR. DocuSigned by: Tony R. Laws, Jr. 3/6/2017		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE PLAN OF SPAN		SHEET NO. S11-08 TOTAL SHEETS 27		
	REVISIONS						
	NO.	BY:	DATE:	NO.		BY:	DATE:
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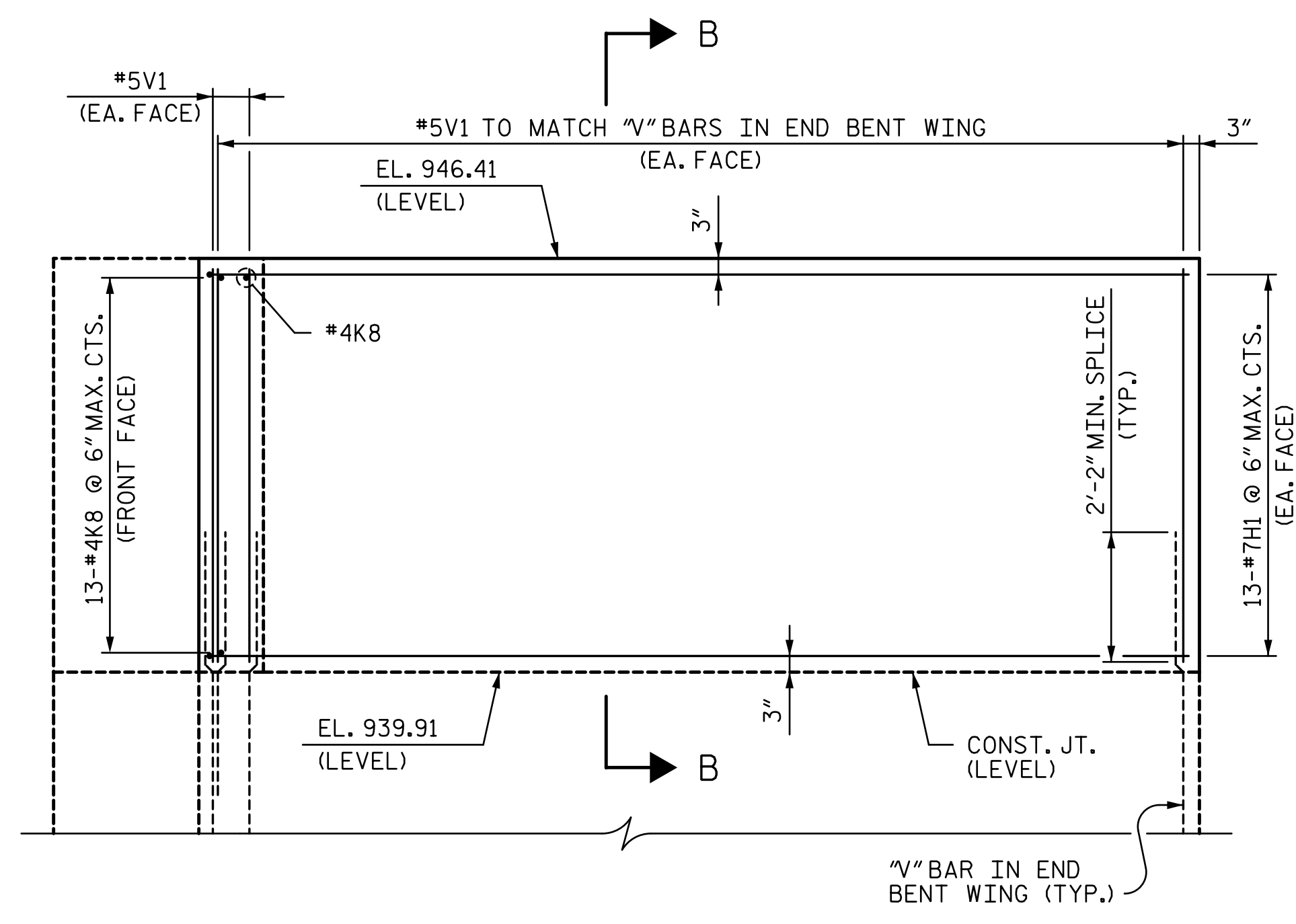
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 CHECKED BY: JTG DATE: 9-16



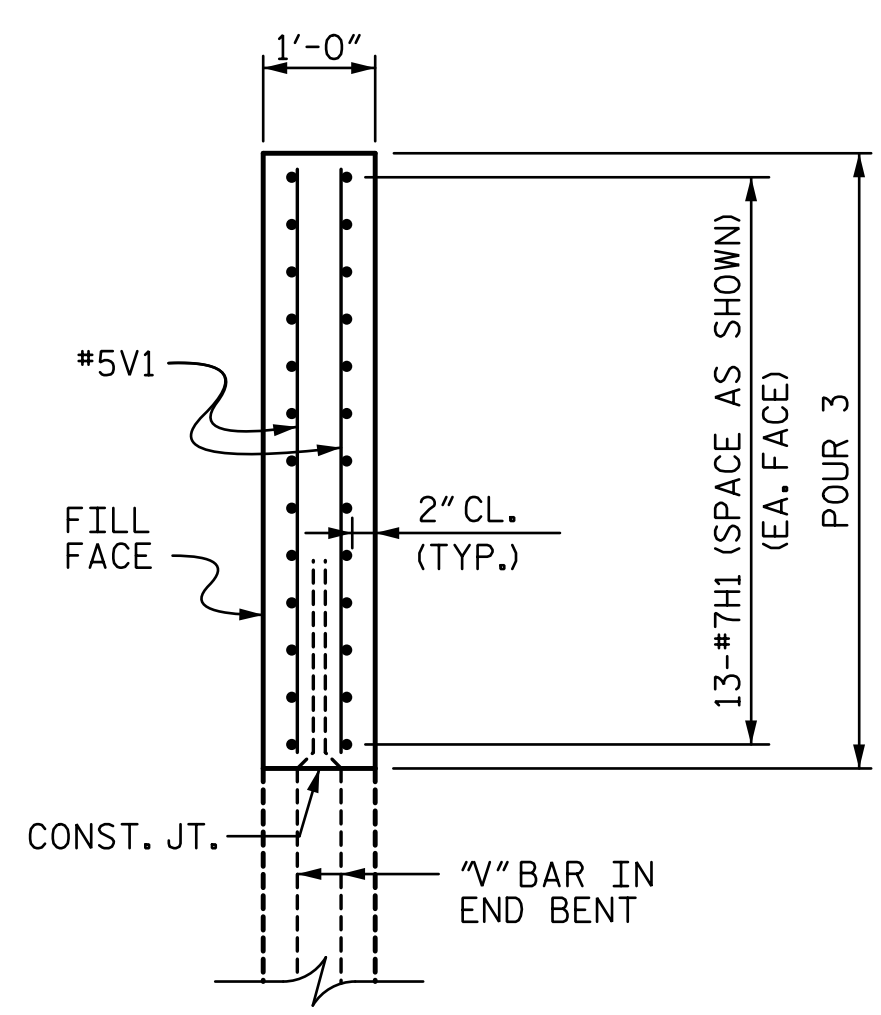
WING WALL PLAN (W1)



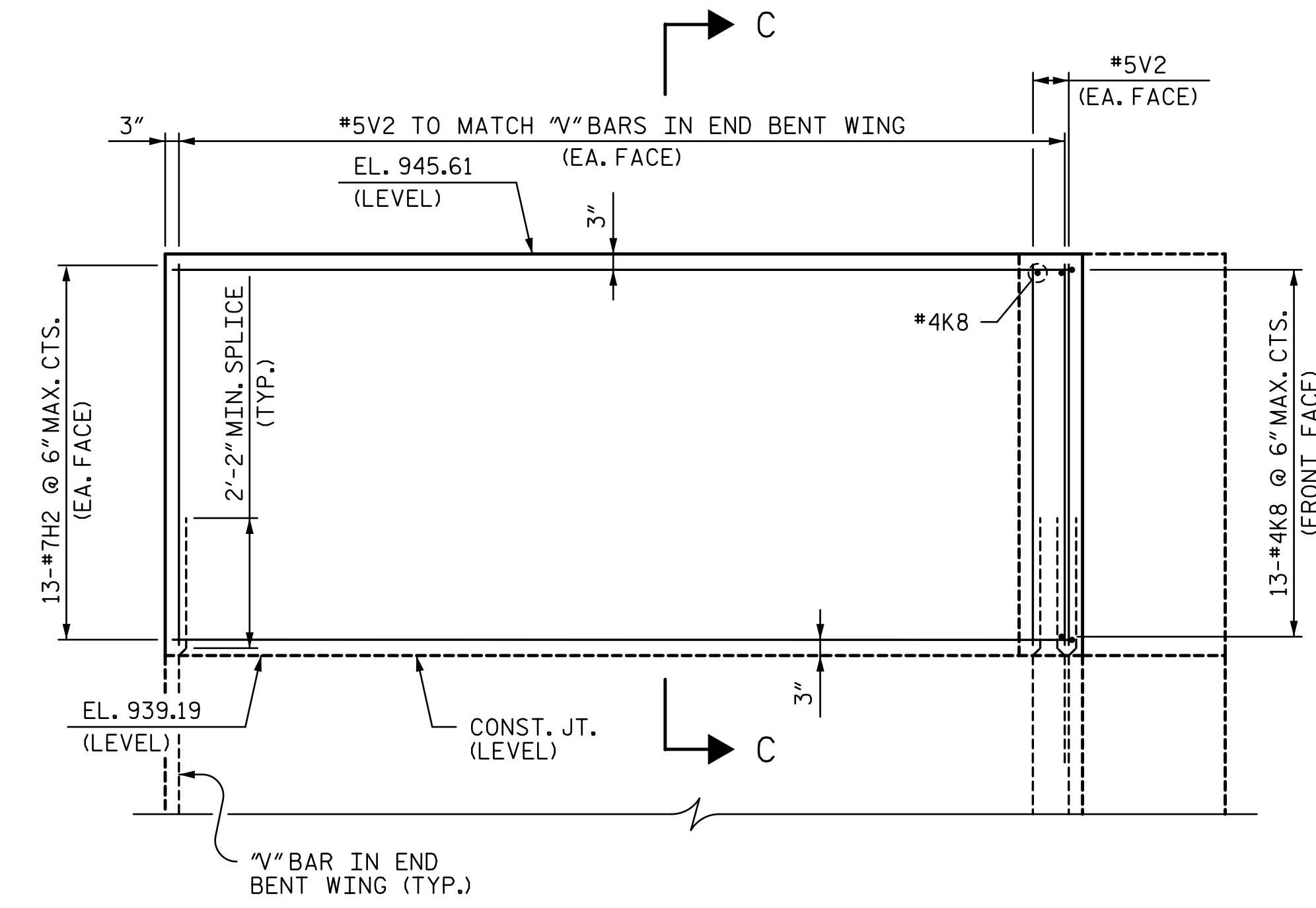
WING WALL PLAN (W2)



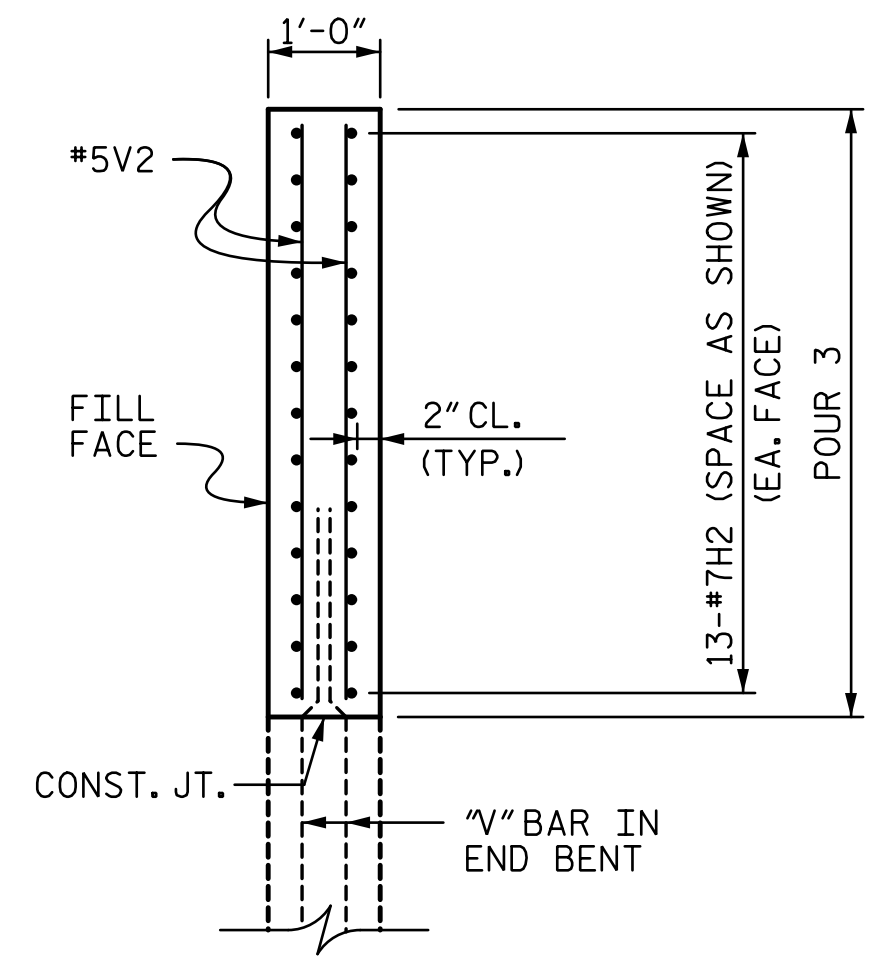
WING WALL ELEVATION (W1)



SECTION B-B



WING WALL ELEVATION (W2)



SECTION C-C

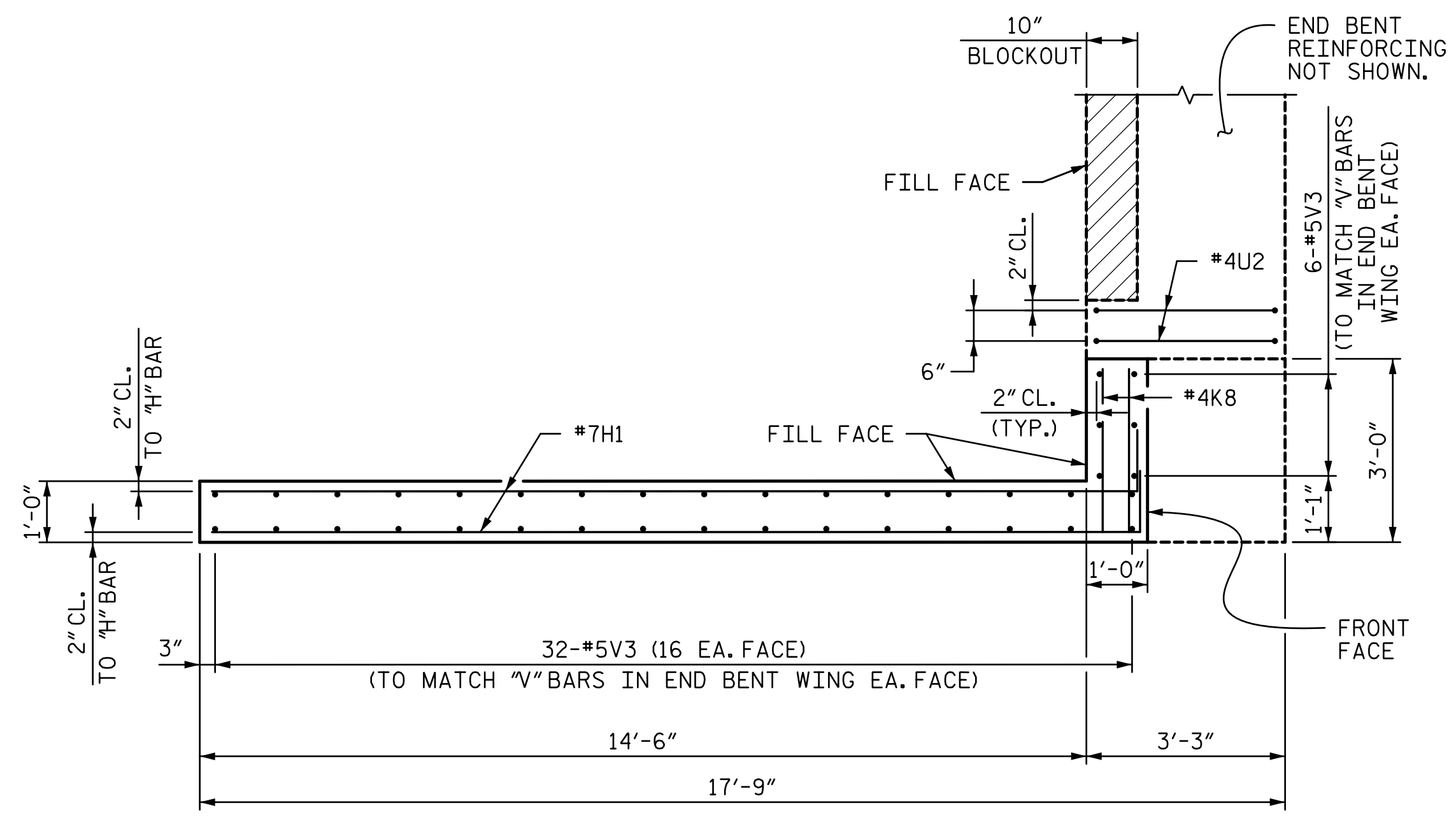
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 2 OF 3

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH <b>SUPERSTRUCTURE</b> <b>PLAN OF SPAN</b> <b>DETAILS</b>		SHEET NO. S11-09																		
	DocuSigned by: <b>Tony R. Laws, Jr.</b> <small>3/6/2017 3:07:47 PM</small>	<b>STV</b> 100 years STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-5991		TOTAL SHEETS 27																		
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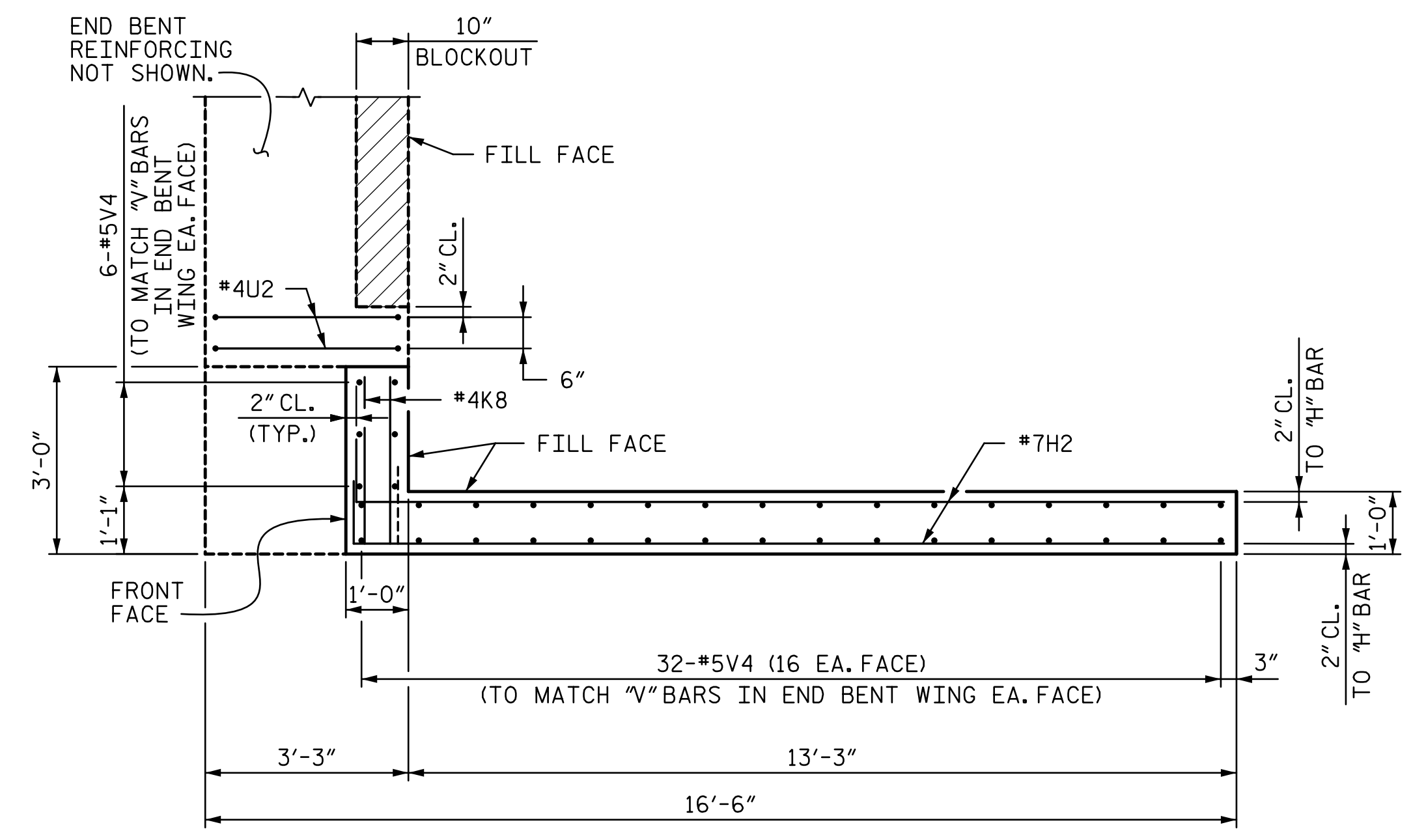
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 3/6/2017

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CHECKED BY : <u>JTG</u>	DATE : <u>9-16</u>		

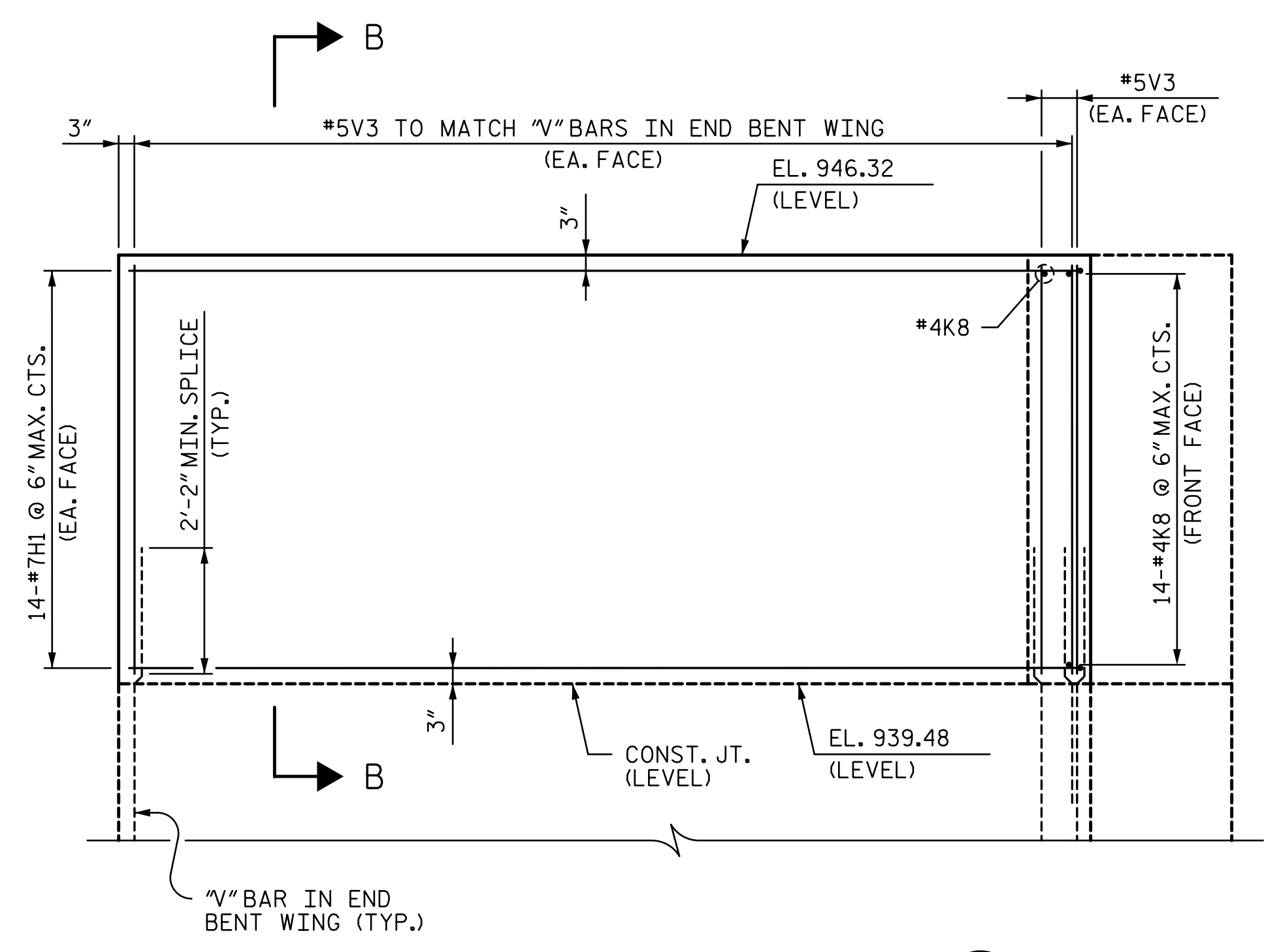




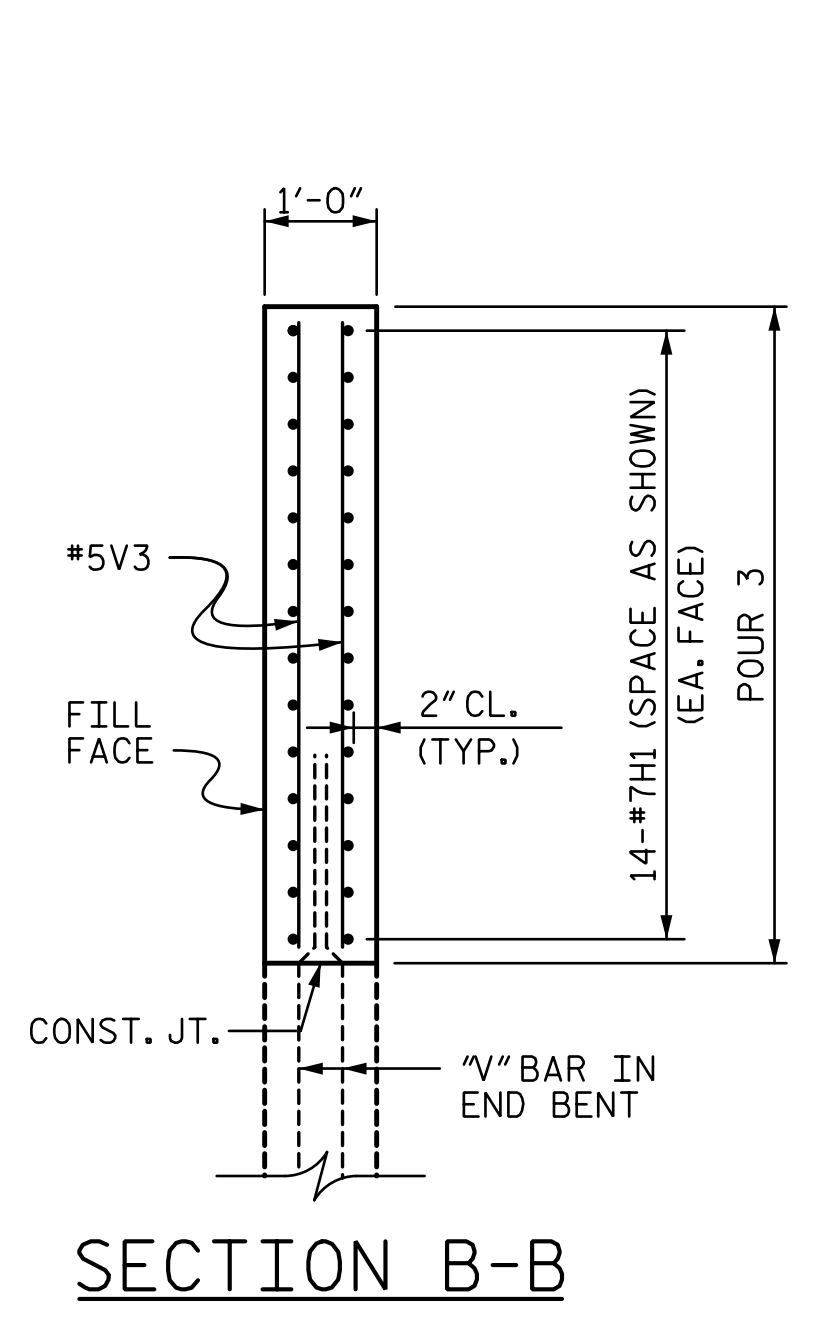
WING WALL PLAN (W3)



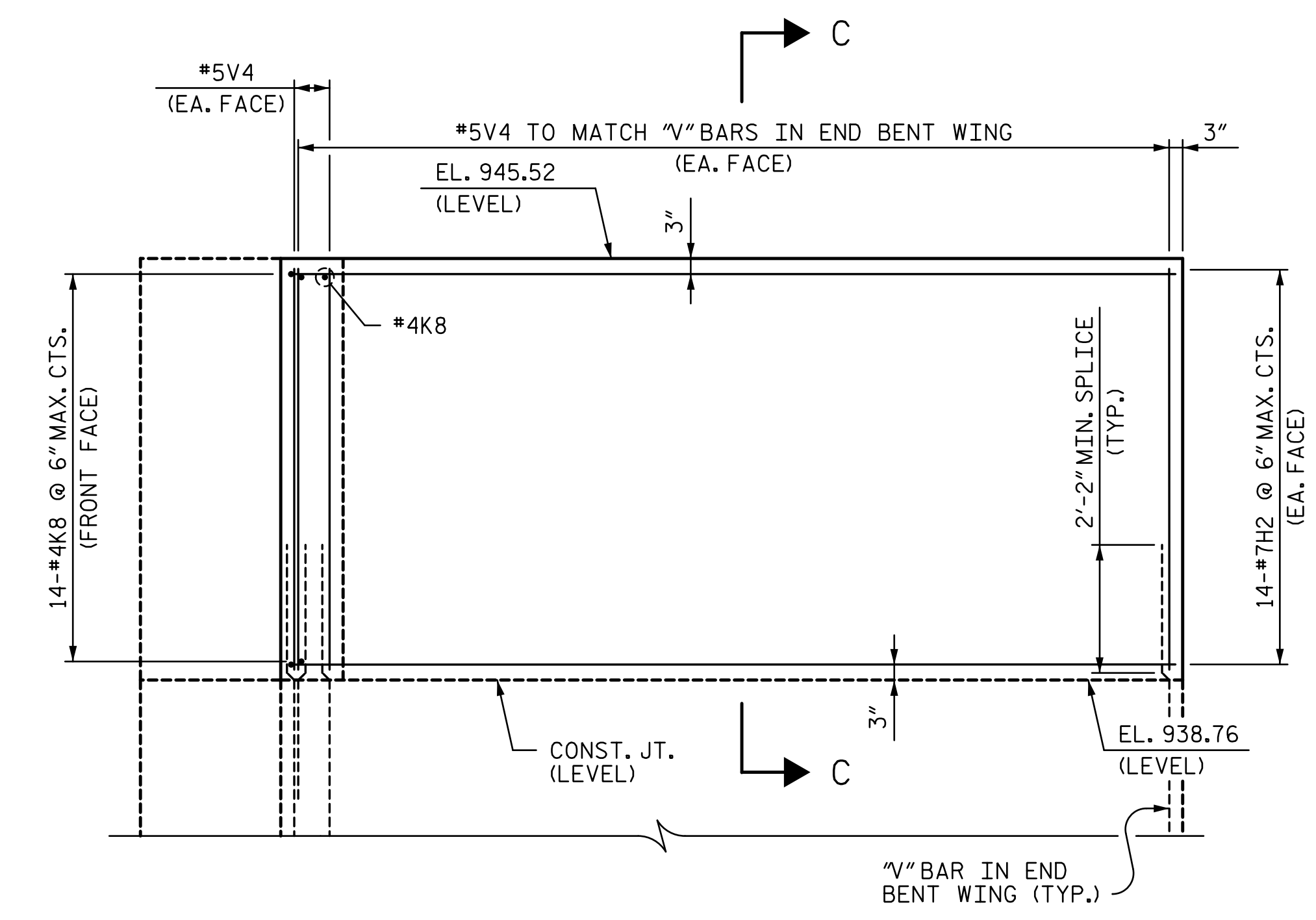
WING WALL PLAN (W4)



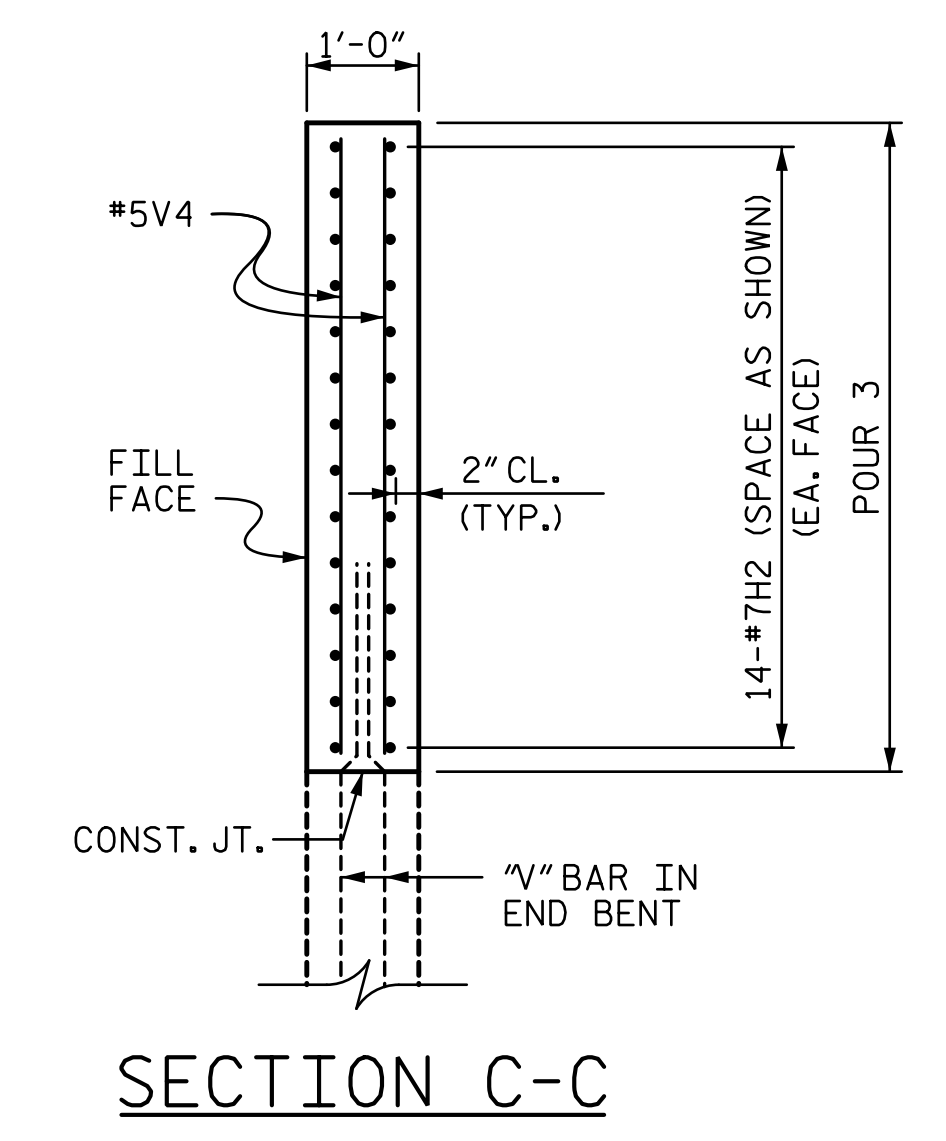
WING WALL ELEVATION (W3)



SECTION B-B



WING WALL ELEVATION (W4)



SECTION C-C

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 3 OF 3

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CHECKED BY : <u>JTG</u>	DATE : <u>9-16</u>		

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**Tony R. Laws, Jr.**  
 37672017

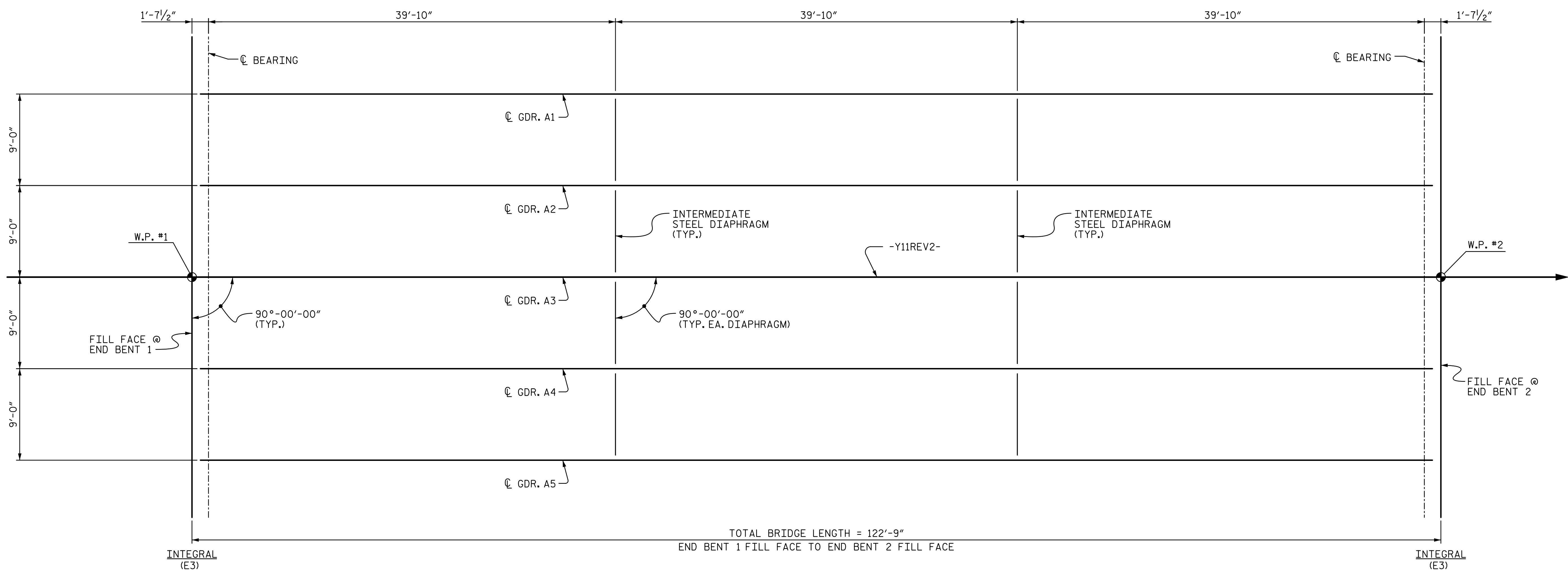
STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**SUPERSTRUCTURE**

**PLAN OF SPAN DETAILS**

REVISIONS			
NO.	BY:	DATE:	DATE:
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SHEET NO.	S11-10
TOTAL SHEETS	27

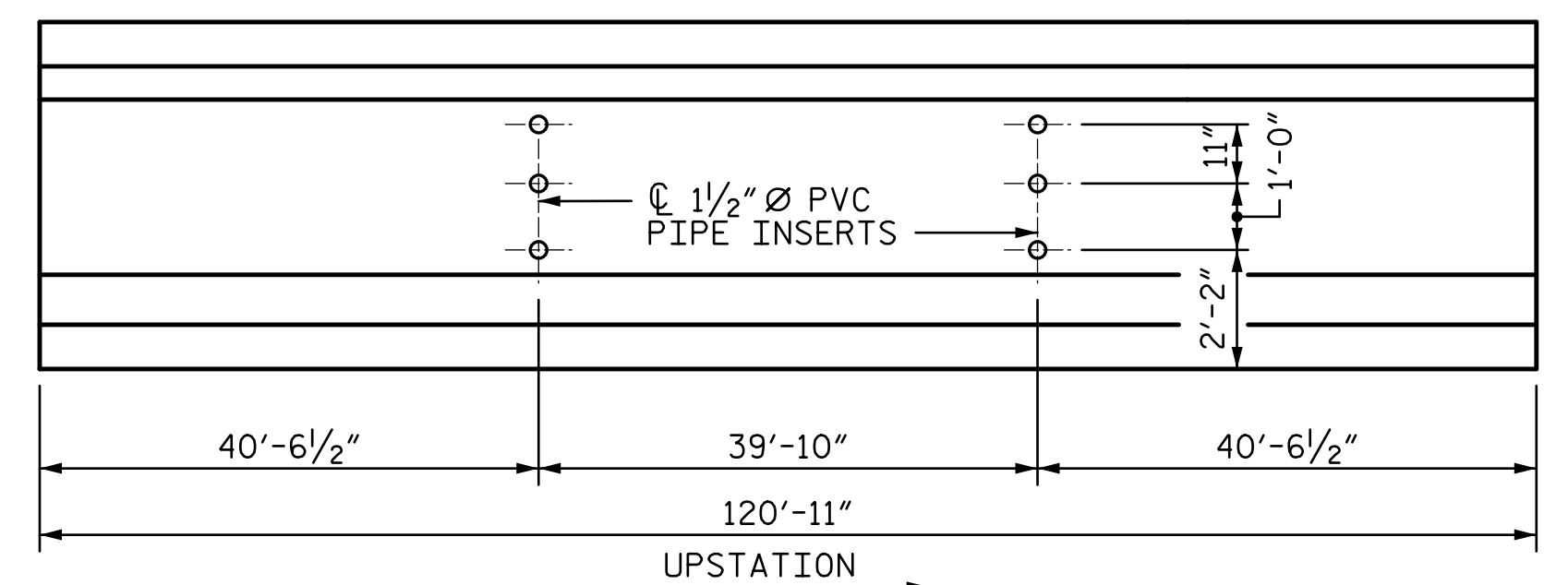


TOTAL BRIDGE LENGTH = 122'-9"  
 END BENT 1 FILL FACE TO END BENT 2 FILL FACE

SPAN A

**FRAMING PLAN**

FOR INTERMEDIATE STEEL DIAPHRAGM DETAILS,  
 SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR 63" MODIFIED  
 BULB TEE PRESTRESSED CONCRETE GIRDERS" SHEET.



**NOTES:**  
 ALL GIRDER ALONE IN PLACE CAMBERS AND DEFLECTIONS  
 ARE SHOWN IN DECIMAL FEET.  
 ▲ DOES NOT INCLUDE FUTURE WEARING SURFACE.

DEAD LOAD DEFLECTION TABLE																					
GIRDERS 1 & 5																					
TWENTIETH POINTS	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.046	0.091	0.134	0.173	0.207	0.236	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.236	0.207	0.173	0.134	0.091	0.046	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. ▲ ↓	0.000	0.030	0.059	0.088	0.115	0.138	0.158	0.175	0.186	0.193	0.196	0.193	0.186	0.175	0.158	0.138	0.115	0.088	0.059	0.030	0.000
FINAL CAMBER ↑	0"	3/16"	3/8"	9/16"	1 1/16"	1 3/16"	1 5/16"	1"	1 1/16"	1 1/8"	1 1/8"	1 1/8"	1 1/16"	1"	1 5/16"	1 3/16"	1 1/16"	9/16"	3/8"	3/16"	0"

DEAD LOAD DEFLECTION TABLE																					
GIRDERS 2-4																					
TWENTIETH POINTS	0.00	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	1.00
CAMBER (GIRDER ALONE IN PLACE) ↑	0.000	0.046	0.091	0.134	0.173	0.207	0.236	0.260	0.277	0.287	0.291	0.287	0.277	0.260	0.236	0.207	0.173	0.134	0.091	0.046	0.000
DEFLECTION DUE TO SUPERIMPOSED D.L. ▲ ↓	0.000	0.032	0.062	0.093	0.122	0.147	0.168	0.185	0.198	0.205	0.208	0.205	0.198	0.185	0.168	0.147	0.122	0.093	0.062	0.032	0.000
FINAL CAMBER ↑	0"	3/16"	3/8"	1/2"	5/8"	3/4"	13/16"	7/8"	1 1/16"	1"	1"	1"	1 5/16"	7/8"	1 3/16"	3/4"	5/8"	1/2"	3/8"	3/16"	0"

DRAWN BY : MBC DATE : 9-16  
 CHECKED BY : AJP DATE : 9-16  
 DESIGN ENGINEER OF RECORD: T. LAWS DATE : 9-16

GIRDER INSERTS

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

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3762017

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
RALEIGH

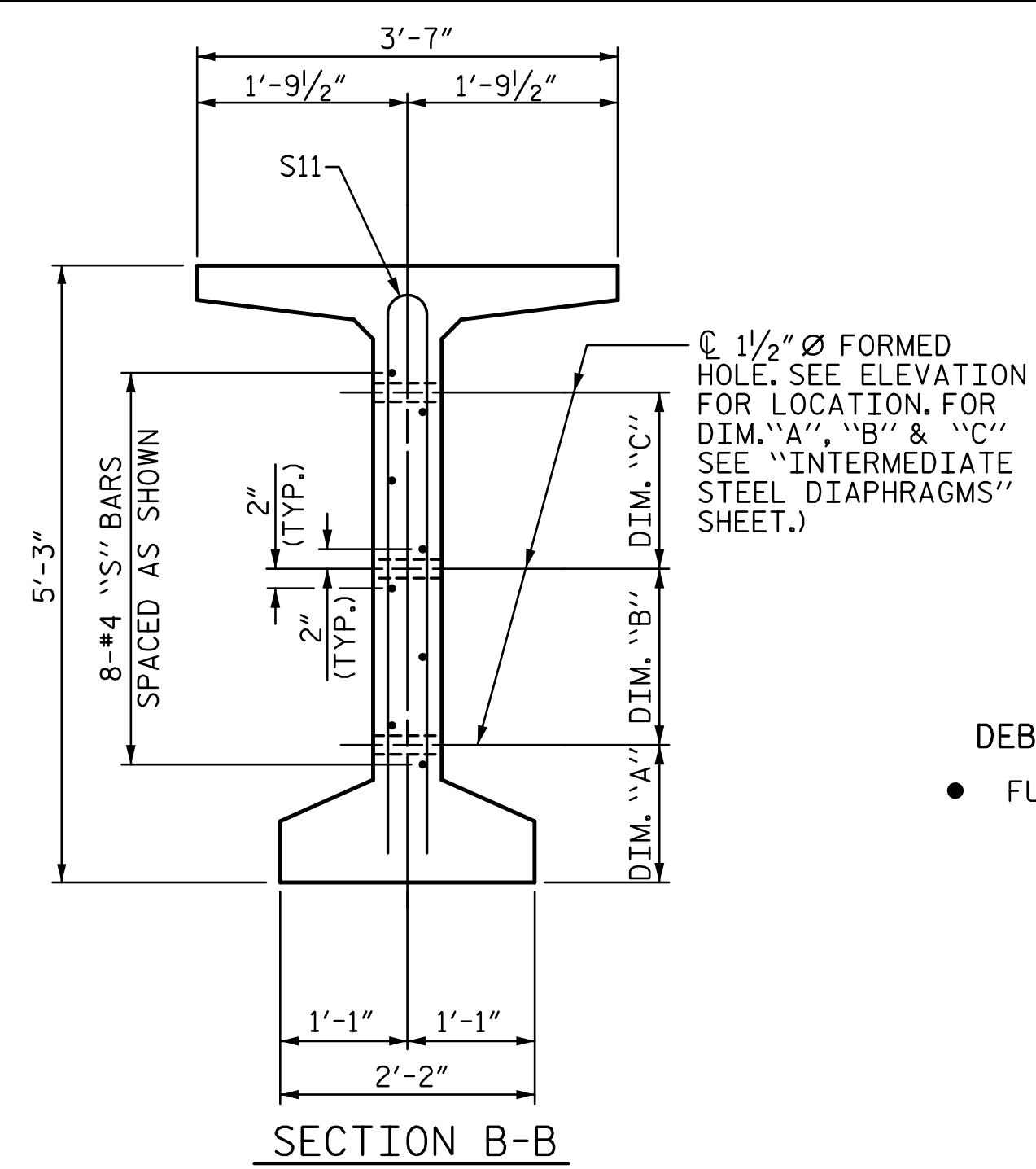
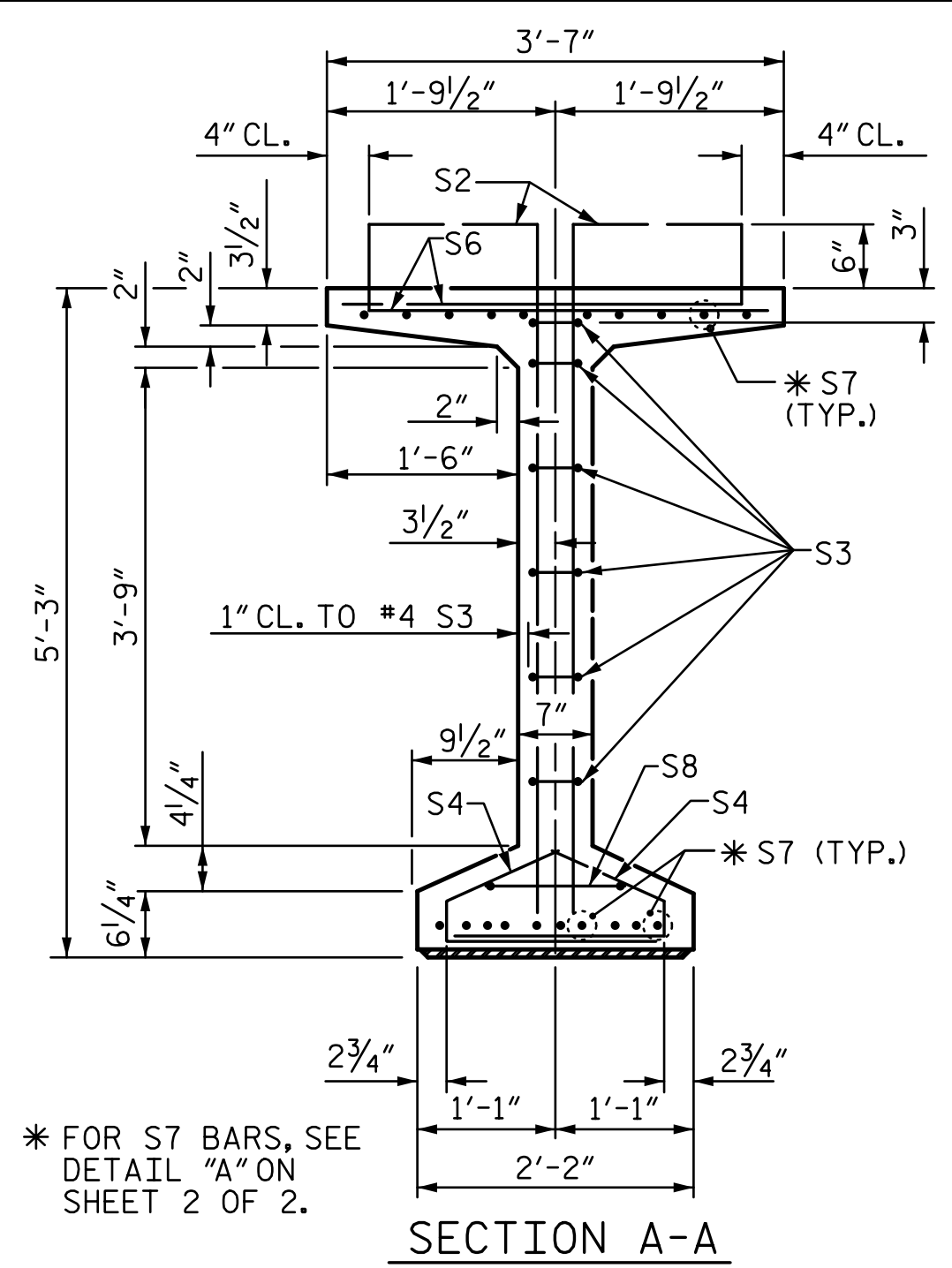
**SUPERSTRUCTURE**

**FRAMING PLAN &  
 DEAD LOAD DEFLECTIONS**

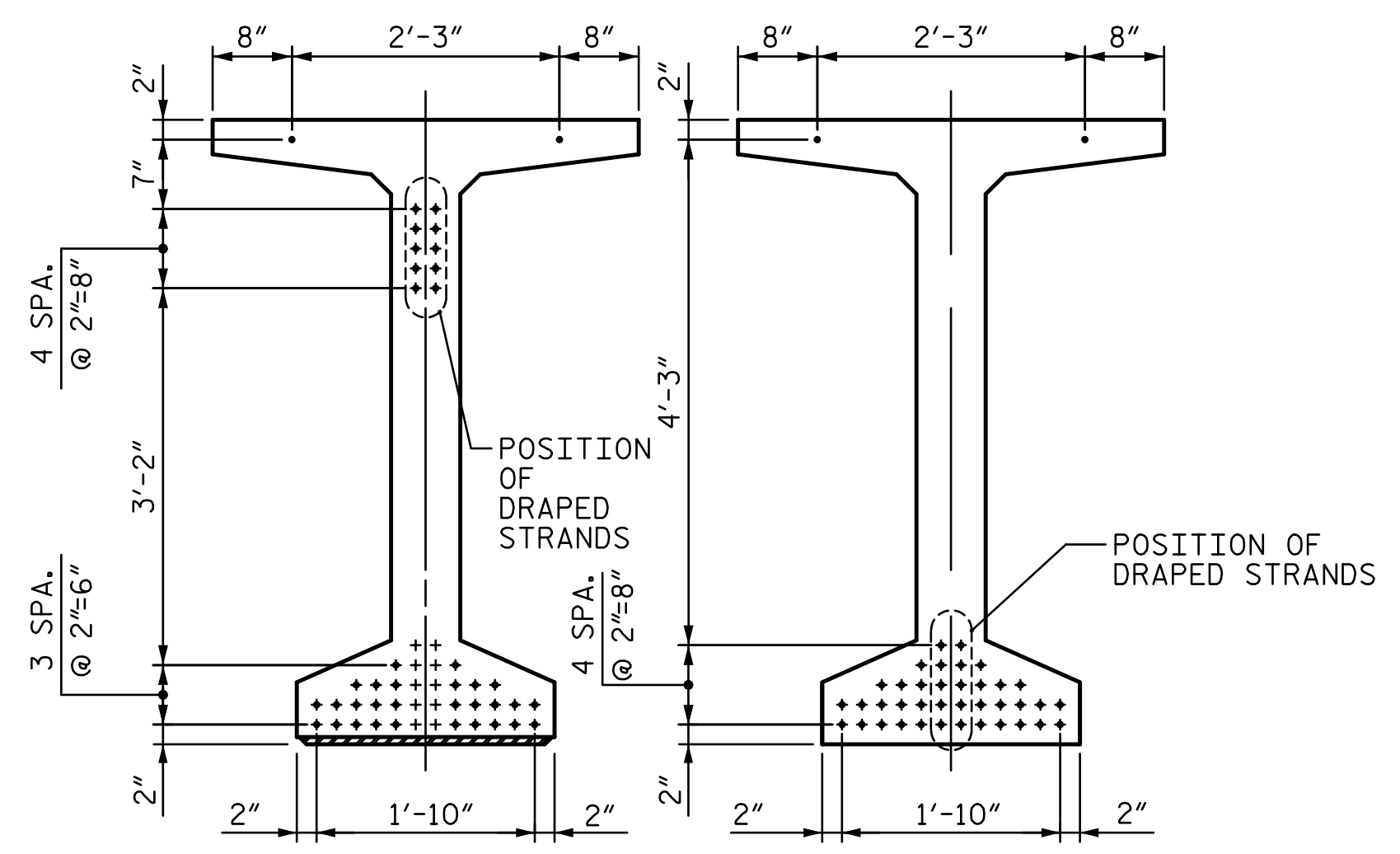
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2			4			27

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DEBONDING LEGEND  
 ● FULLY BONDED STRANDS



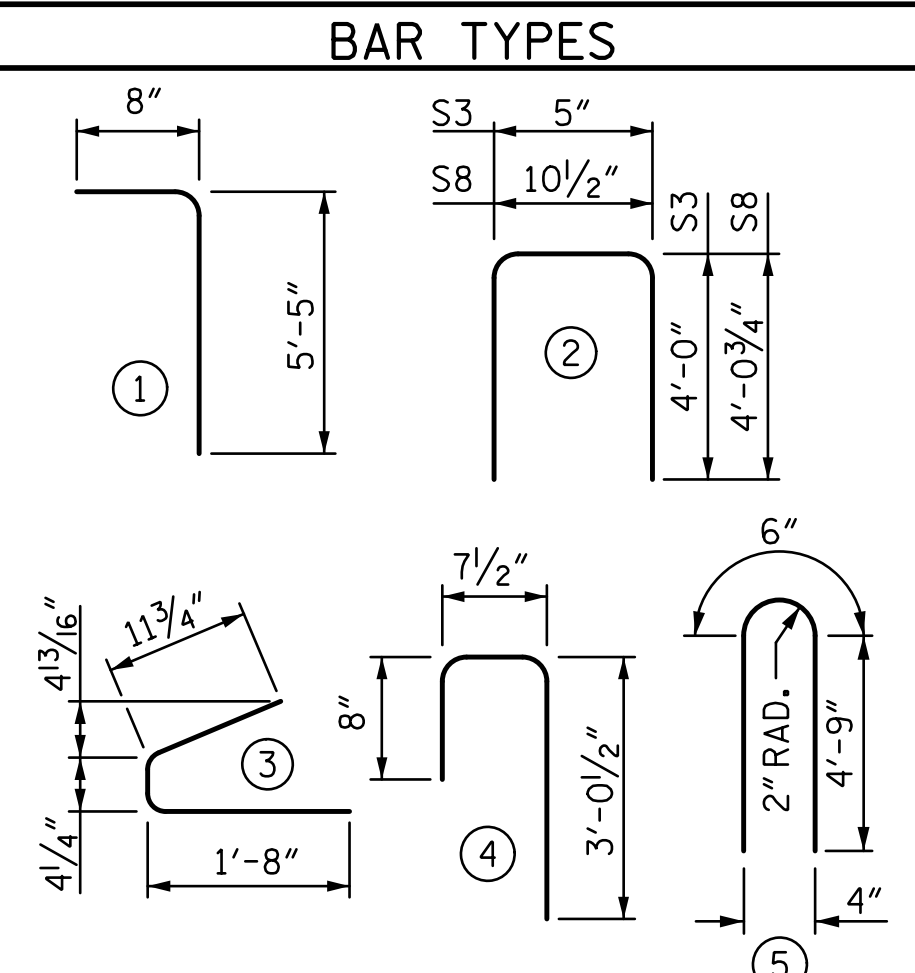
AT END OF GIRDER      AT C. OF GIRDER  
 0.6" Ø LOW RELAXATION STRAND LAYOUT

- NOTES:
- FOR DIAPHRAGM HOLE LOCATIONS, SEE "FRAMING PLAN" SHEET.
  - FOR ADDITIONAL GIRDER DETAILS, SEE SHEET 2 OF 2.
  - FOR NOTES, SEE SHEET 2 OF 2.
  - THE UPLIFT FORCE DUE TO DRAPED STRANDS IS 30 KIPS.

0.6" Ø L. R. GRADE 270 STRANDS		
AREA (SQUARE INCHES)	ULTIMATE STRENGTH (LBS. PER STRAND)	APPLIED PRESTRESS (LBS. PER STRAND)
0.217	58,600	43,950

REINFORCING STEEL FOR ONE GDR					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
S1	190	#4	1	6'-1"	772
S2	24	#6	1	6'-1"	219
S3	12	#4	2	8'-5"	67
S4	72	#4	3	3'-0"	144
S6	214	#5	4	4'-4"	967
*S7	40	#5	STR	3'-8"	153
S8	2	#5	2	9'-0"	19
S9	69	#5	STR	3'-3"	234
S10	2	#3	STR	1'-10"	1
S11	8	#5	5	10'-0"	83
S12	16	#4	STR	8'-0"	86

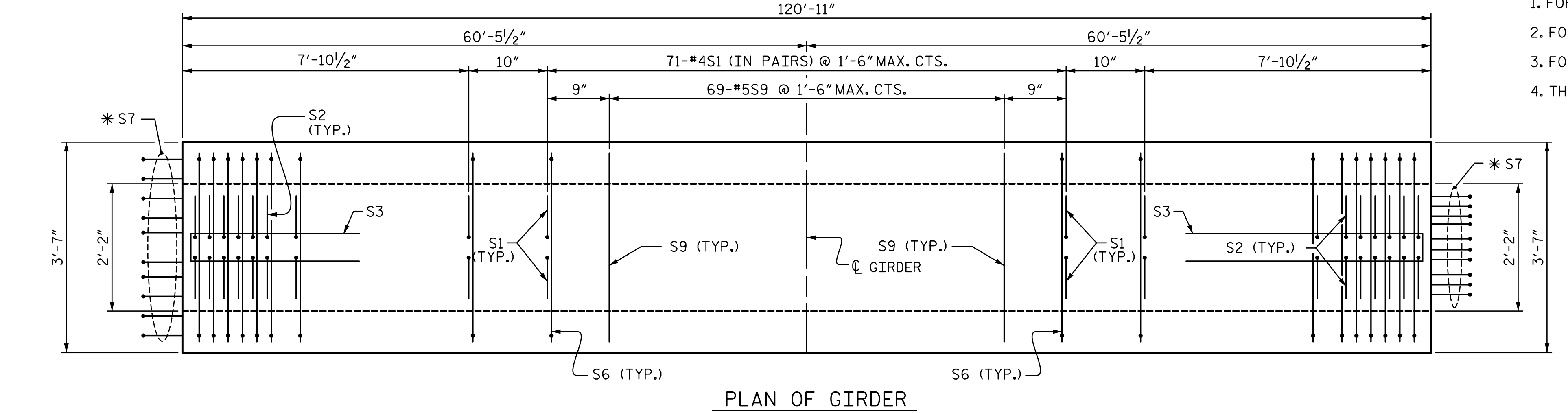
\* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.



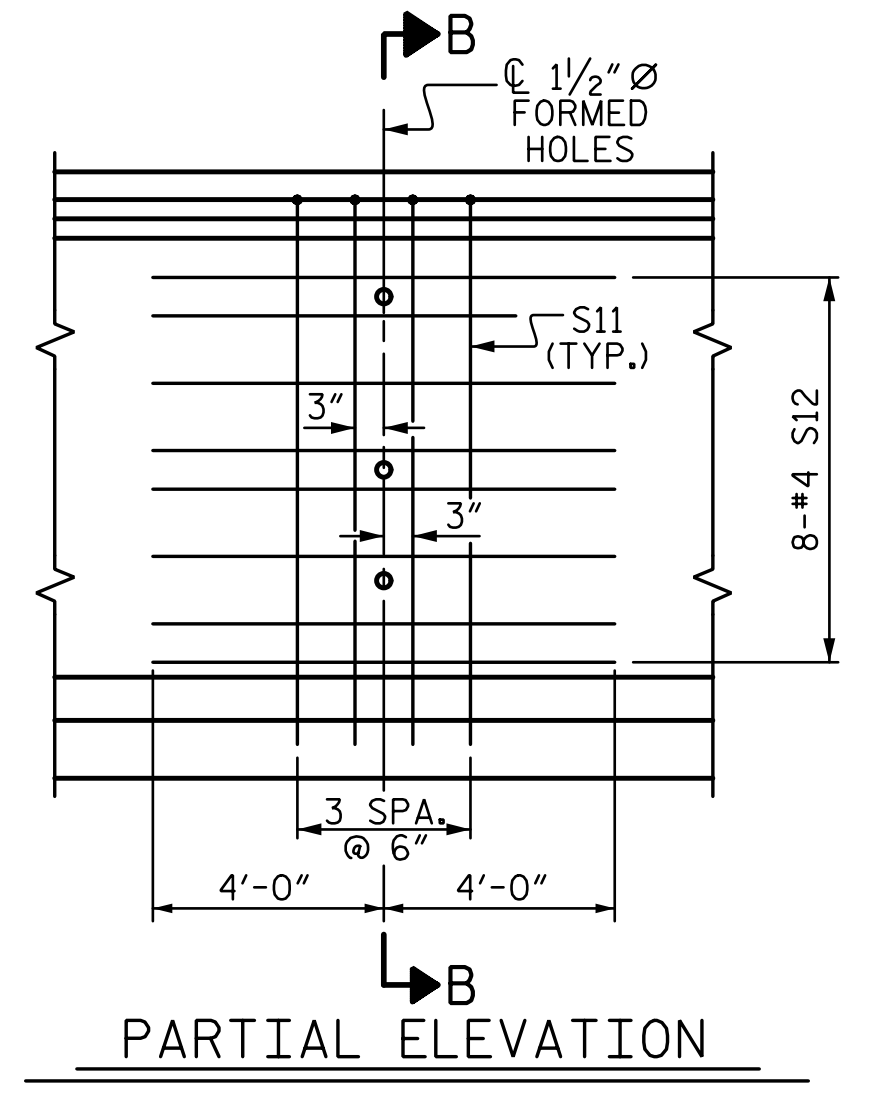
ALL BAR DIMENSIONS ARE OUT-TO-OUT

QUANTITIES FOR ONE GIRDER		
REINFORCING STEEL	9000 PSI CONCRETE	0.6" Ø L.R. STRANDS
LB.	C.Y.	No.
2745	24.0	40

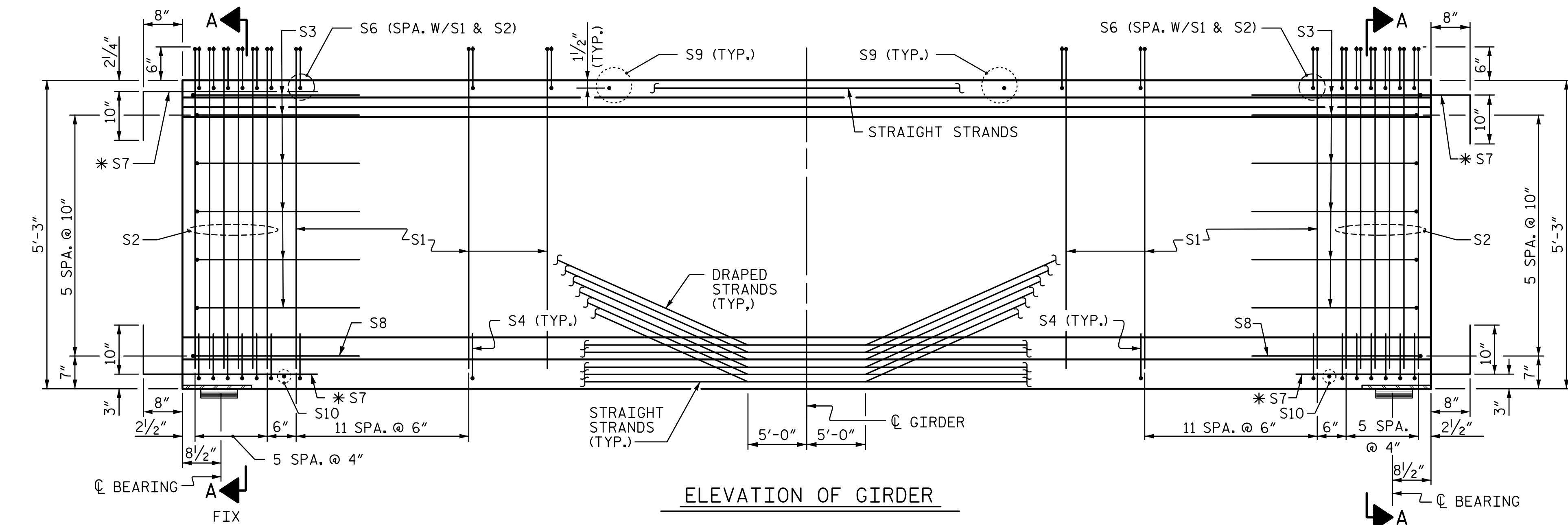
GIRDERS REQUIRED		
NUMBER	LENGTH	TOTAL LENGTH
5	120'-11"	604'-7"



PLAN OF GIRDER



PARTIAL ELEVATION



ELEVATION OF GIRDER

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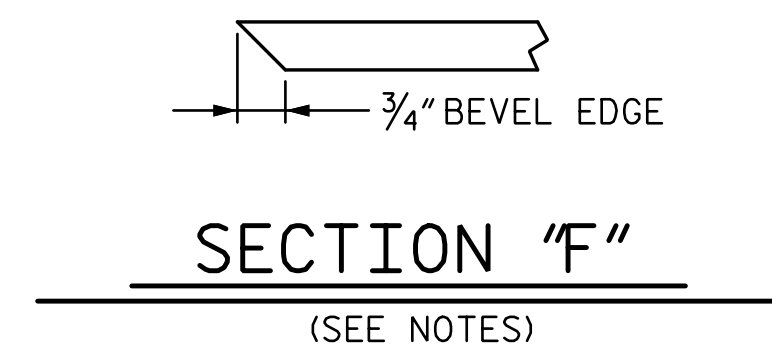
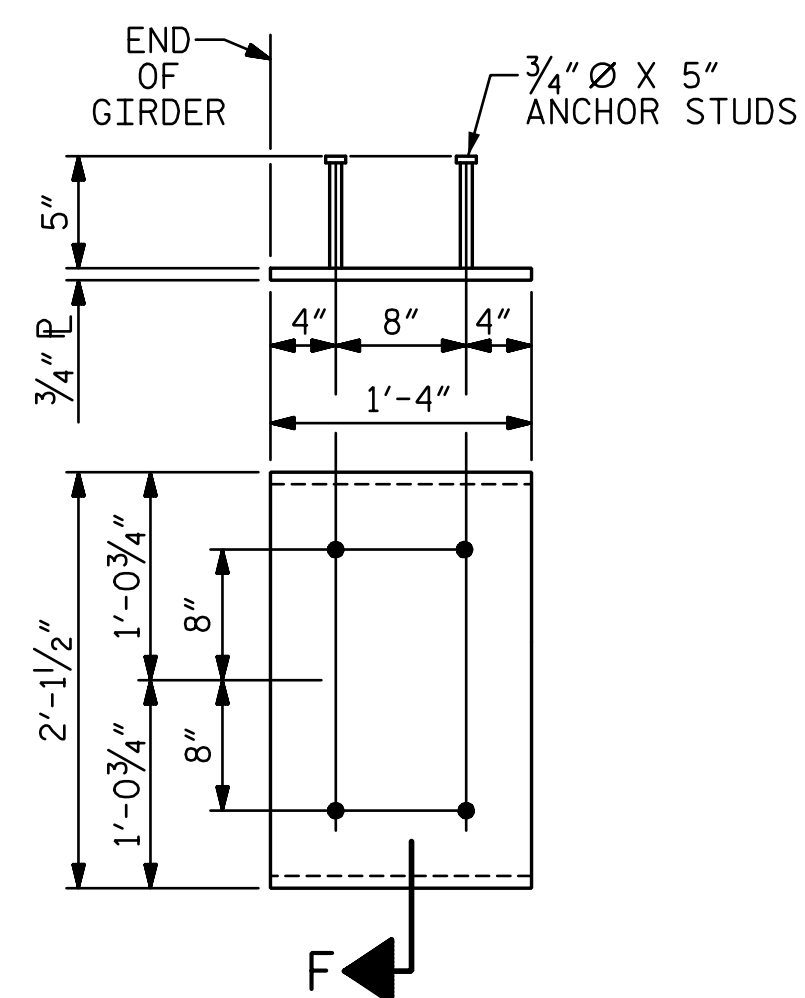
DocuSigned by:  
 Tony R. Laws, Jr.  
 3/6/2017

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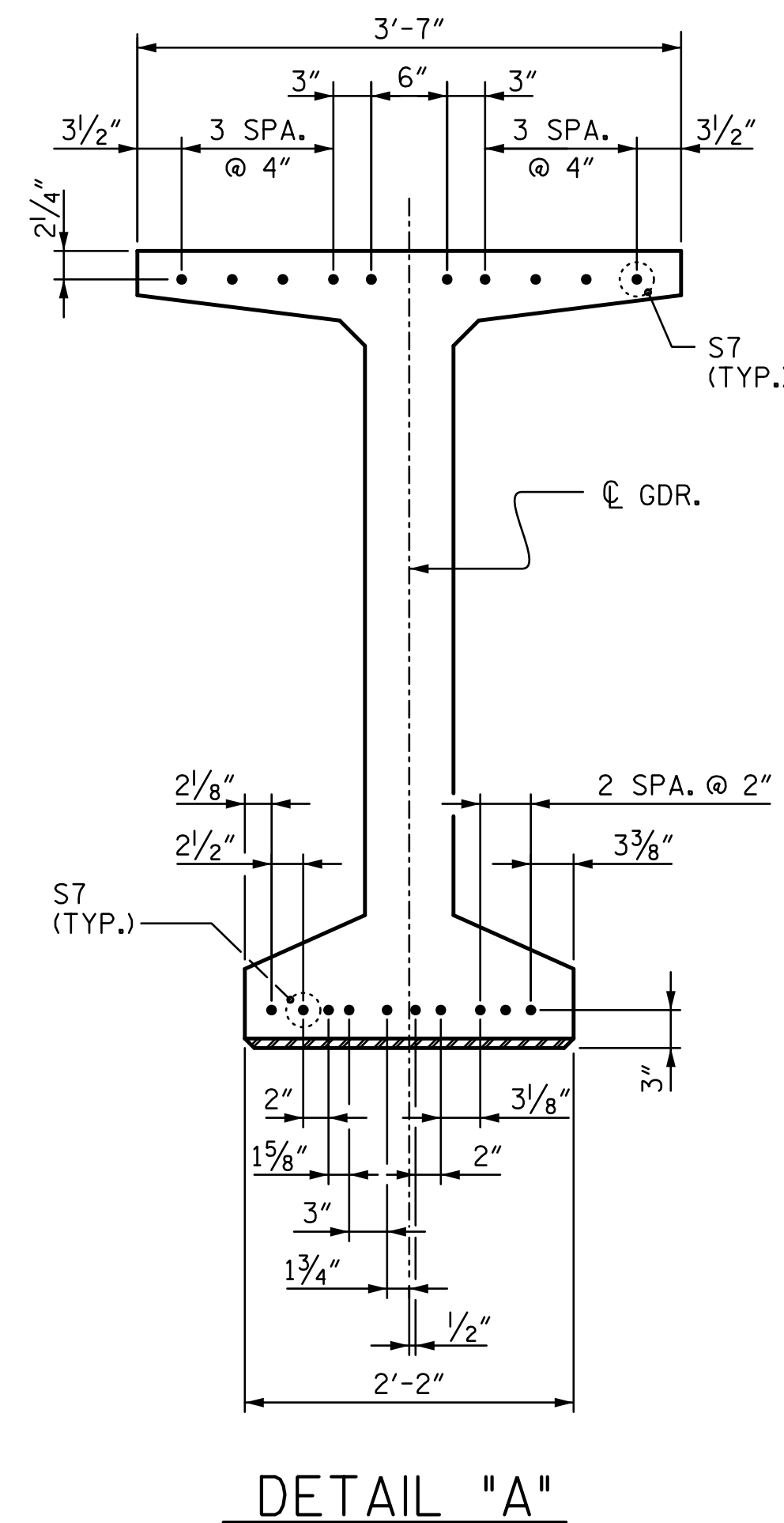
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
63" PRESTRESSED CONCRETE MODIFIED BULB TEE					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

SHEET NO. S11-12
TOTAL SHEETS 27



**EMBEDDED PLATE "B-1" DETAILS**  
(2 REQ'D. PER GIRDER)



**NOTES**

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR ENDWALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE PRESTRESSED STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 7,200 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6" OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN 1/2" OF THE THEORETICAL LOCATION SHOWN.

A 2" X 2" CHAMFER IS ALLOWED AT THE INTERSECTION OF THE WEB AND THE BOTTOM FLANGE.

THE CONTRACTOR HAS THE OPTION TO PROVIDE, AT NO ADDITIONAL COST TO THE DEPARTMENT, 2 ADDITIONAL STRANDS AT THE TOP OF THE GIRDER TO FACILITATE TYING OF THE REINFORCING STEEL. THESE STRANDS SHALL BE PULLED TO A LOAD OF 4,500 lbs.

FOR EMBEDDED CLIPS FOR PRESTRESSED CONCRETE MEMBERS, SEE SPECIAL PROVISIONS.

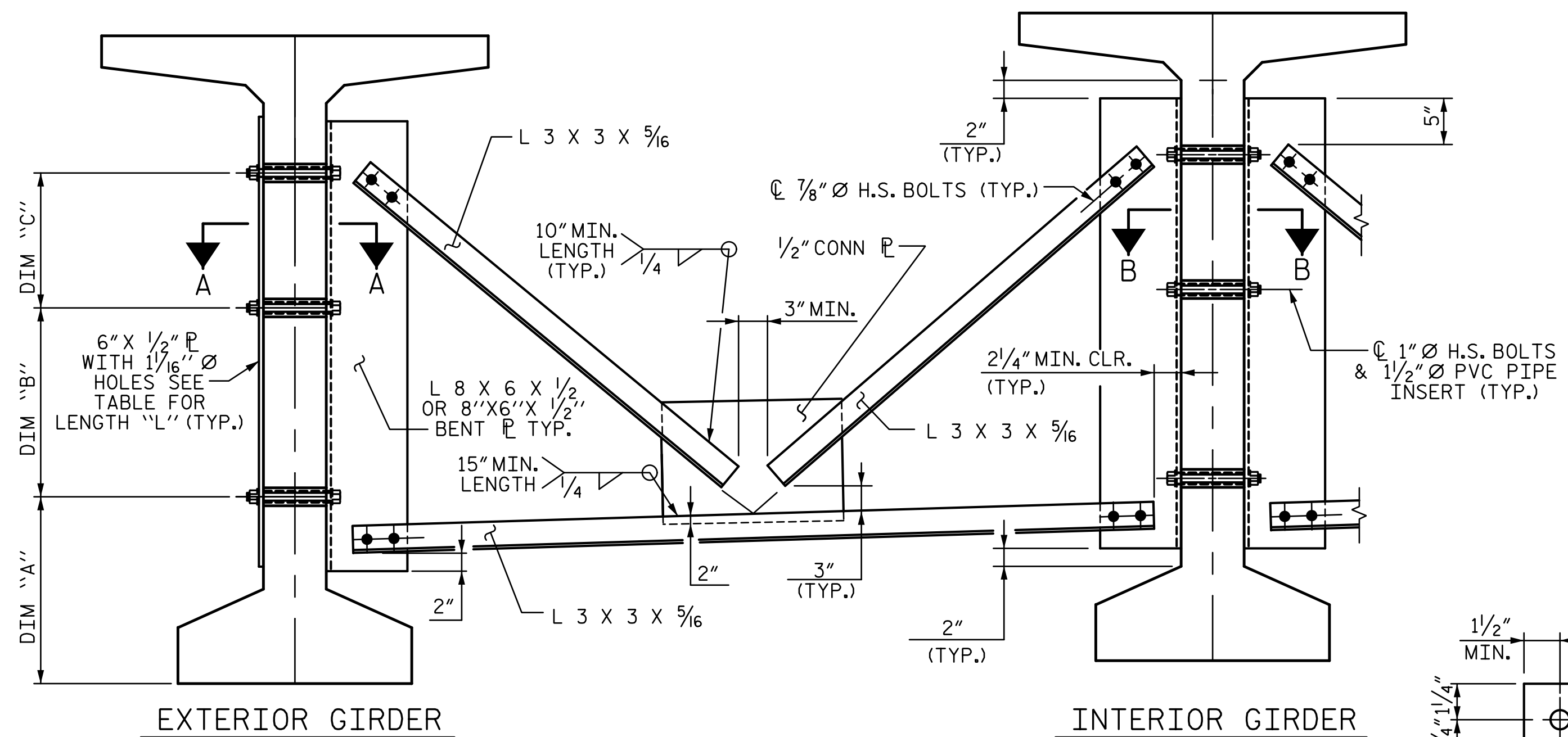
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PROJECT NO. R-2707C  
CLEVELAND COUNTY  
STATION: 32+31.41 -Y11REV2-  
SHEET 2 OF 2

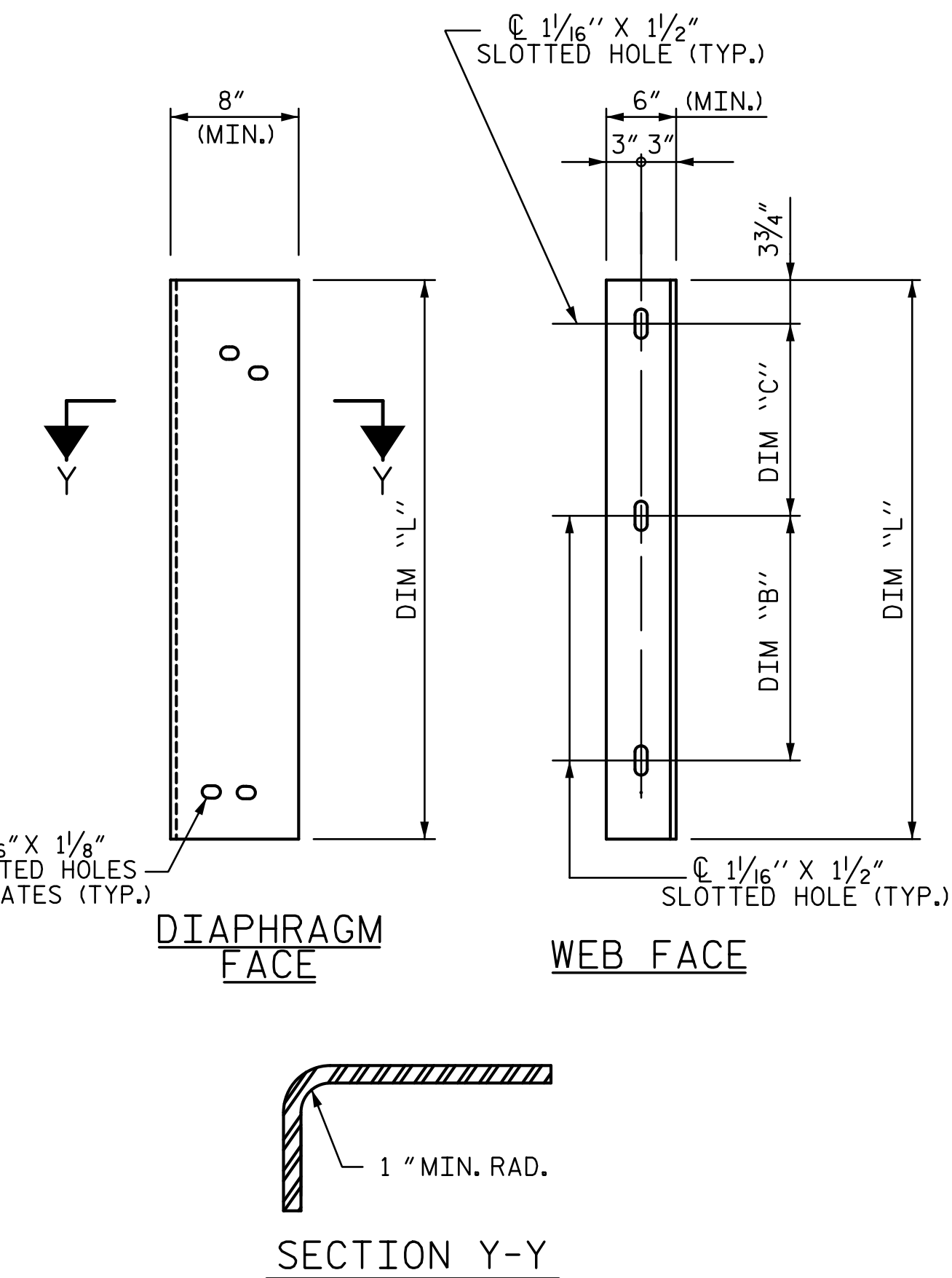
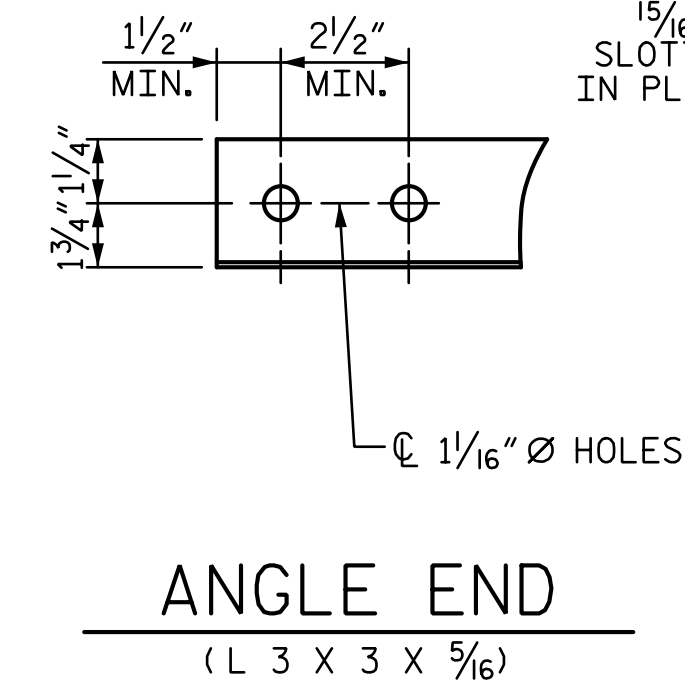
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	DocuSigned by: Tony R. Laws, Jr. 3/23/2017	<b>63" PRESTRESSED CONCRETE                  MODIFIED BULB TEE                  DETAILS</b>		TOTAL SHEETS 27
	STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-5991	REVISIONS		

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CHECKED BY : <u>AJP</u>	DATE : <u>9-16</u>		

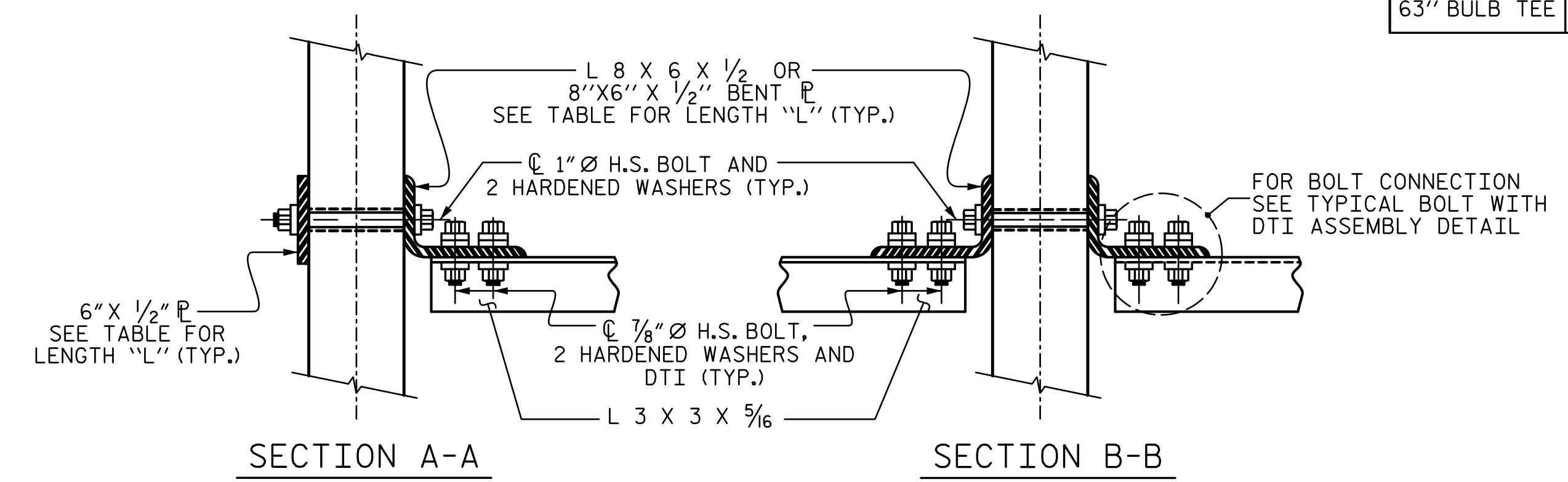




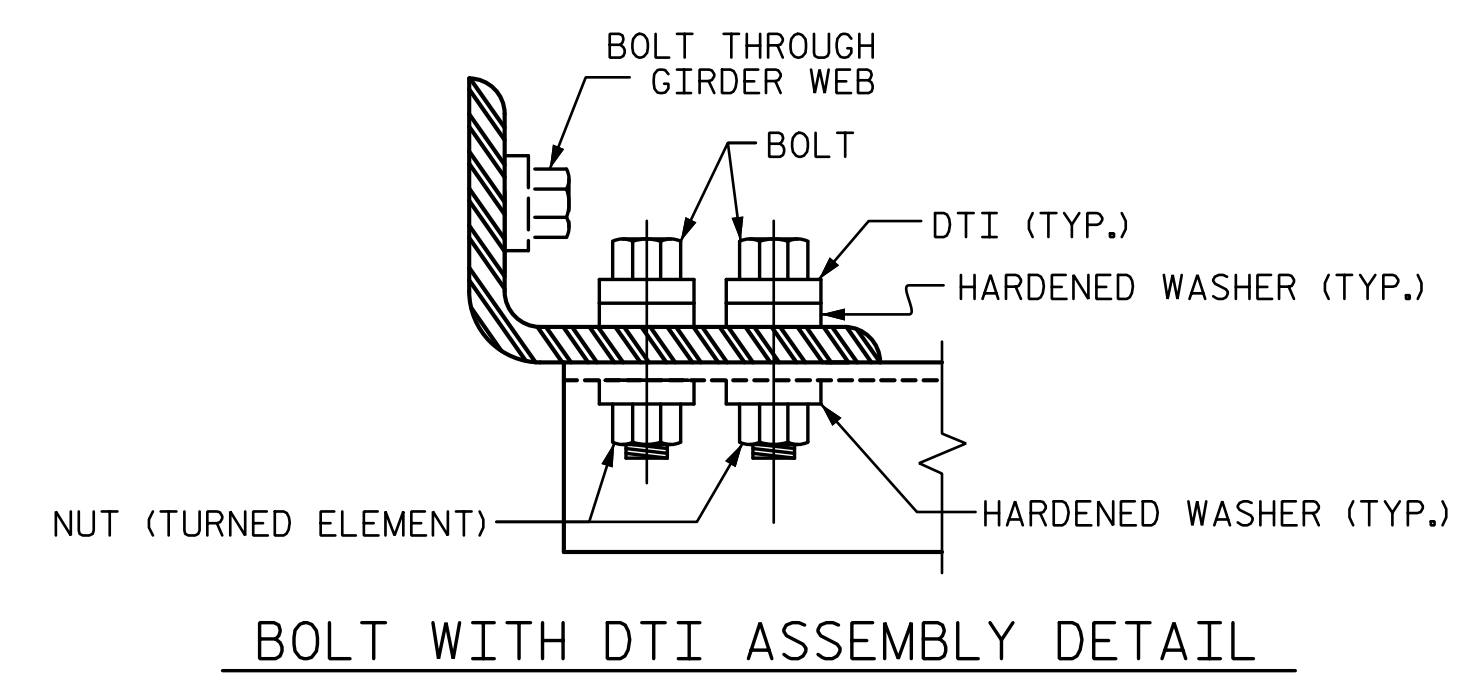
PART SECTION AT INTERMEDIATE DIAPHRAGM



CONNECTOR PLATE DETAIL



CONNECTION DETAILS



BOLT WITH DTI ASSEMBLY DETAIL

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE ANGLE MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

TABLE

GIRDER TYPE	DIM "A"	DIM "B"	DIM "C"	DIM "L"
63" BULB TEE	2'-2"	1'-0"	11"	3'-5"

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**INTERMEDIATE STEEL  
 DIAPHRAGMS FOR 63" MODIFIED  
 BULB TEE PRESTRESSED  
 CONCRETE GIRDERS**

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**Tony R. Laws, Jr.**  
 3/6/2017

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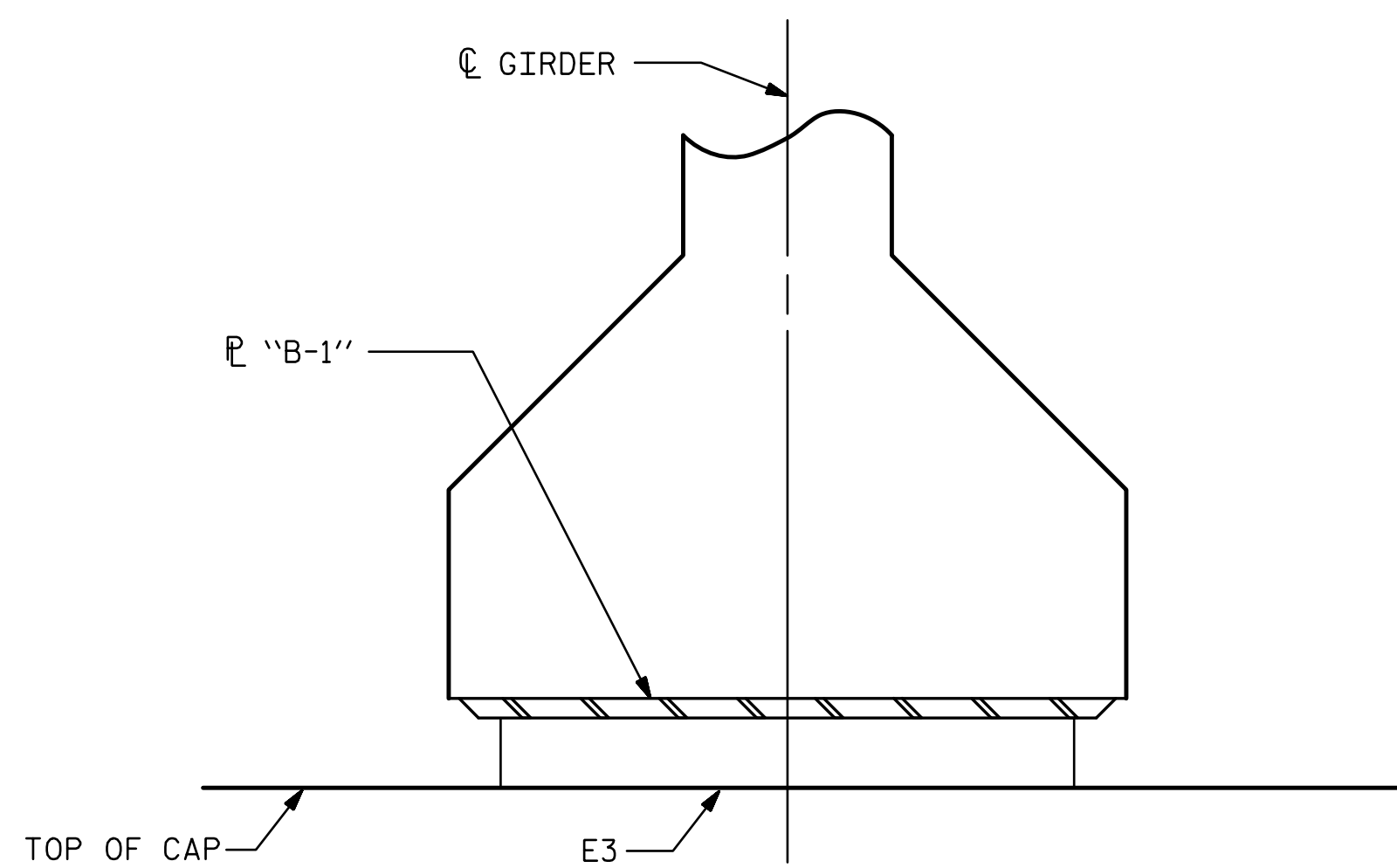
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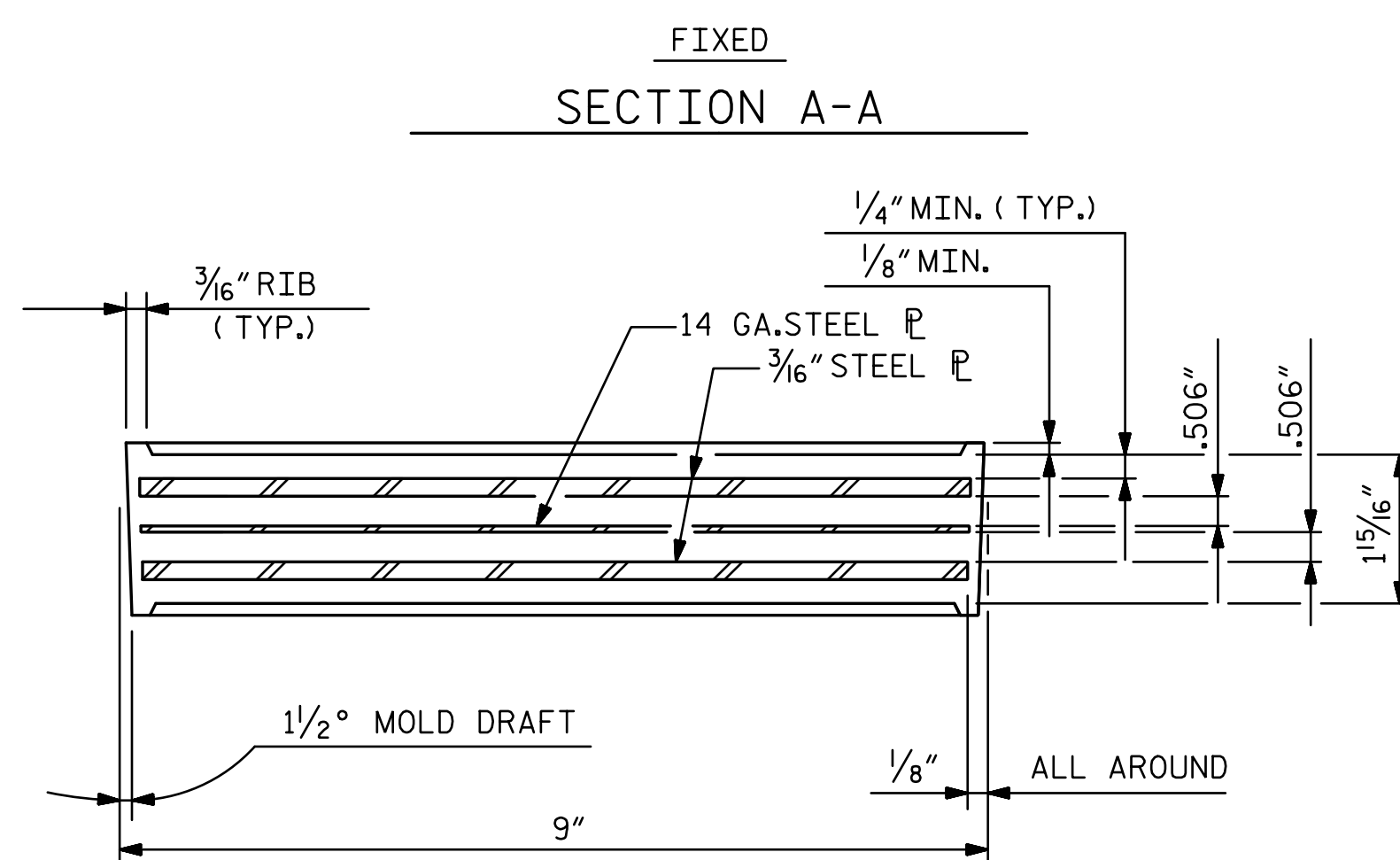
**NOTES**

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

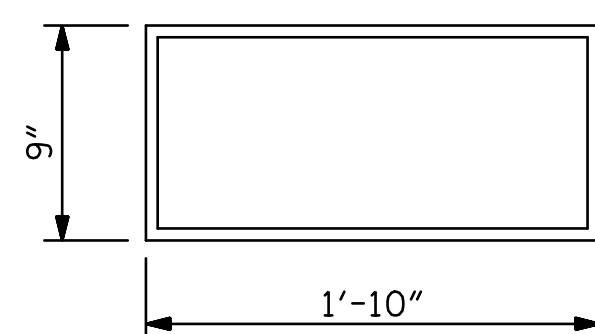
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.



MAXIMUM ALLOWABLE SERVICE LOADS	
D.L.+L.L. (NO IMPACT)	
TYPE IV	225 k



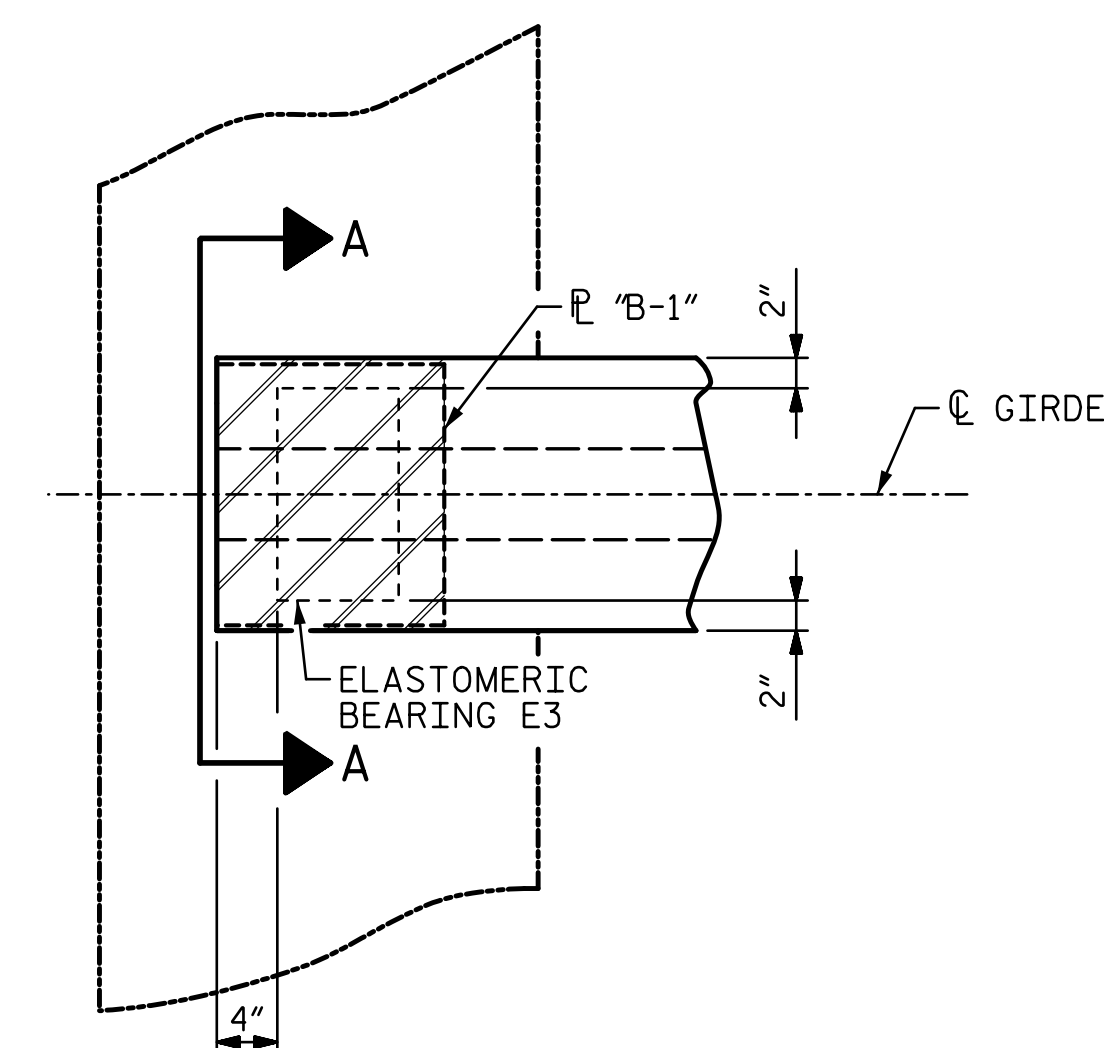
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E3 (10 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

**TYPE IV**



PLAN VIEW  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)

PROJECT NO. R-2707C  
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 STATION: 32+31.41 -Y11REV2-

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	<table border="1" style="width: 100%;"> <thead> <tr> <th colspan="6">REVISIONS</th> </tr> <tr> <th>NO.</th> <th>BY:</th> <th>DATE:</th> <th>NO.</th> <th>BY:</th> <th>DATE:</th> </tr> </thead> <tbody> <tr> <td>1</td> <td></td> <td></td> <td>3</td> <td></td> <td></td> </tr> <tr> <td>2</td> <td></td> <td></td> <td>4</td> <td></td> <td></td> </tr> </tbody> </table>		REVISIONS						NO.	BY:	DATE:	NO.	BY:	DATE:	1			3			2			4	
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NOTES

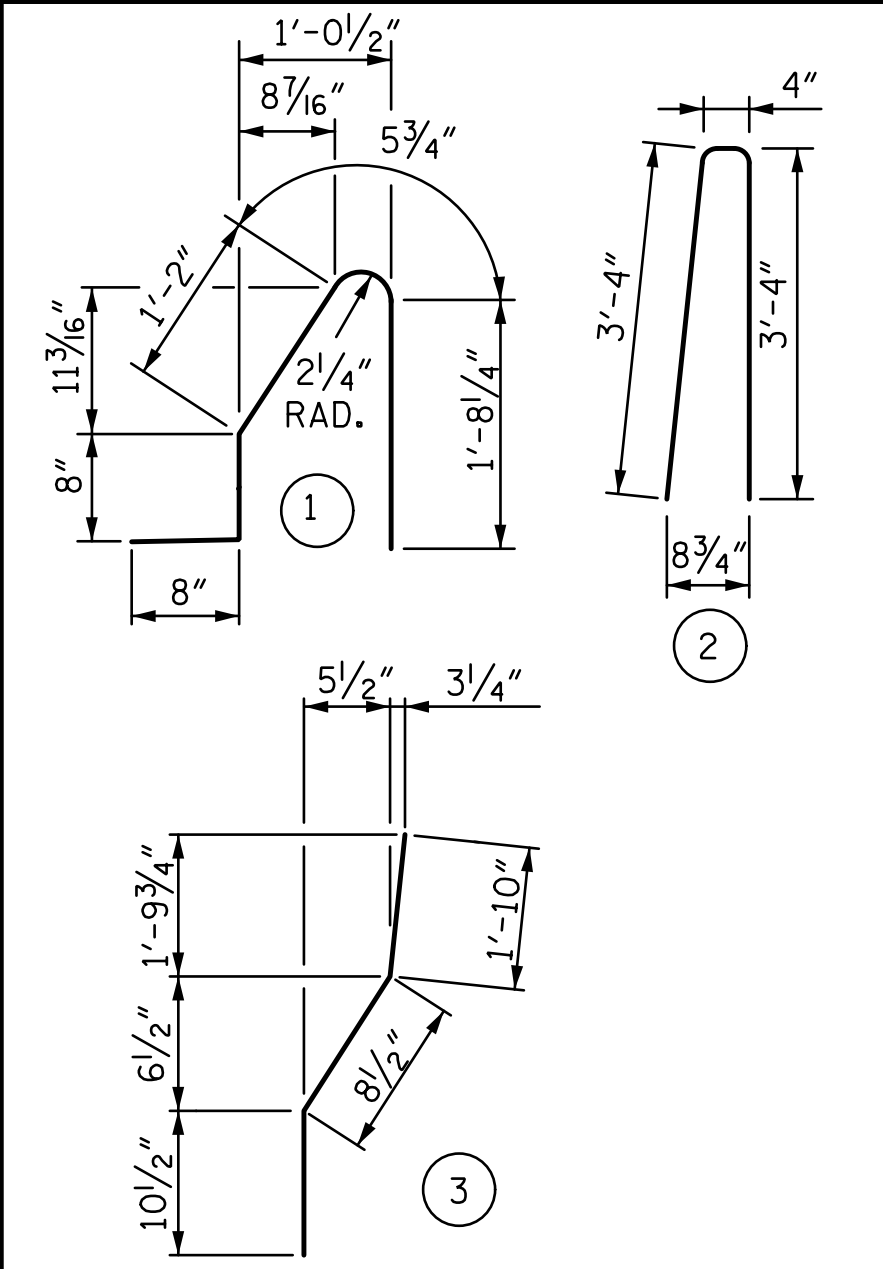
THE BARRIER RAIL IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SPAN HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

#5S1 AND #5S2 BARS MAY BE SHIFTED SLIGHTLY IN ORDER TO MAINTAIN A 2" MINIMUM CLEARANCE TO THE 1/2" EXPANSION JOINT MATERIAL IN RAIL.

BAR TYPES

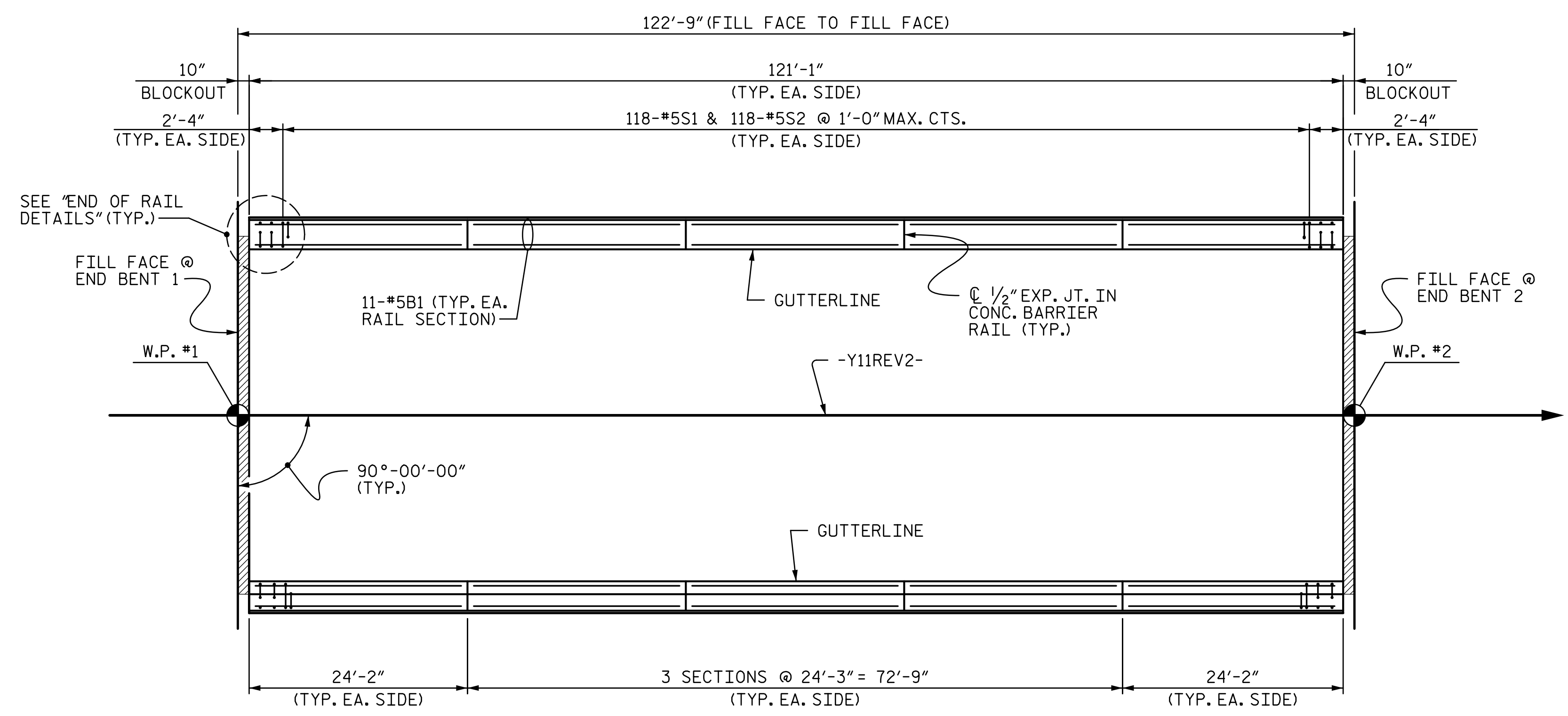


ALL BAR DIMENSIONS ARE OUT TO OUT

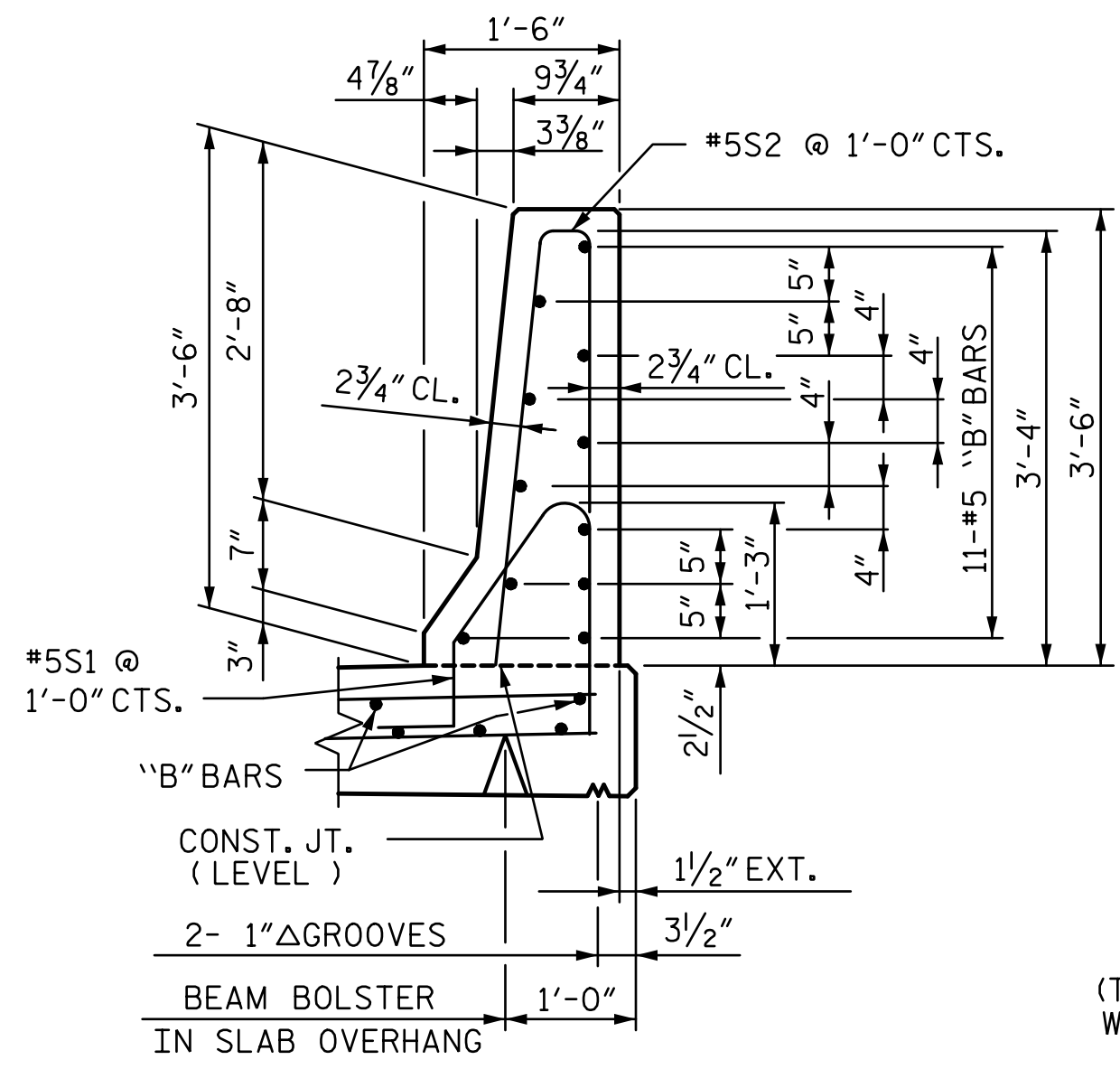
BILL OF MATERIAL

FOR CONCRETE BARRIER RAIL ONLY						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
* B1	110	#5	STR	23'-9"	2725	
* S1	236	#5	1	4'-8"	1149	
* S2	236	#5	2	7'-0"	1723	
* S5	8	#5	3	3'-5"	29	
* S6	8	#5	STR	3'-3"	27	
* EPOXY COATED REINFORCING STEEL					5653 LBS.	
CLASS AA CONCRETE					32.4 CU. YDS.	
CONCRETE BARRIER RAIL					242.2 LTN. FT.	

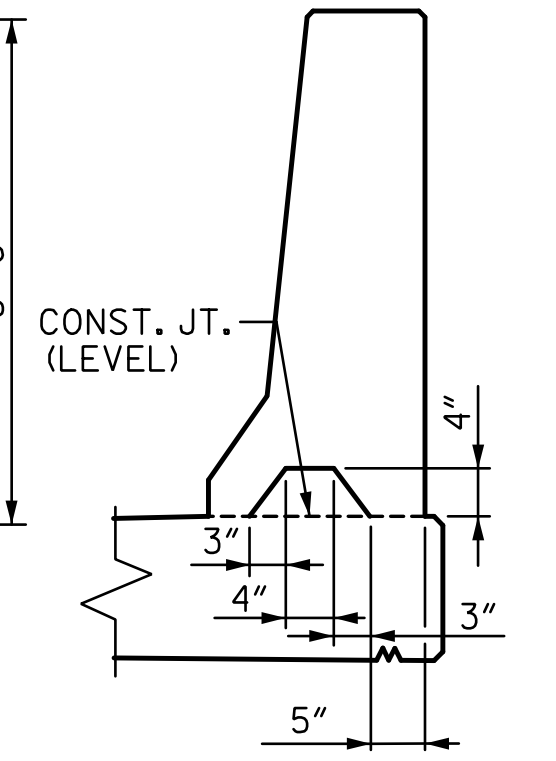
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CLEVELAND COUNTY  
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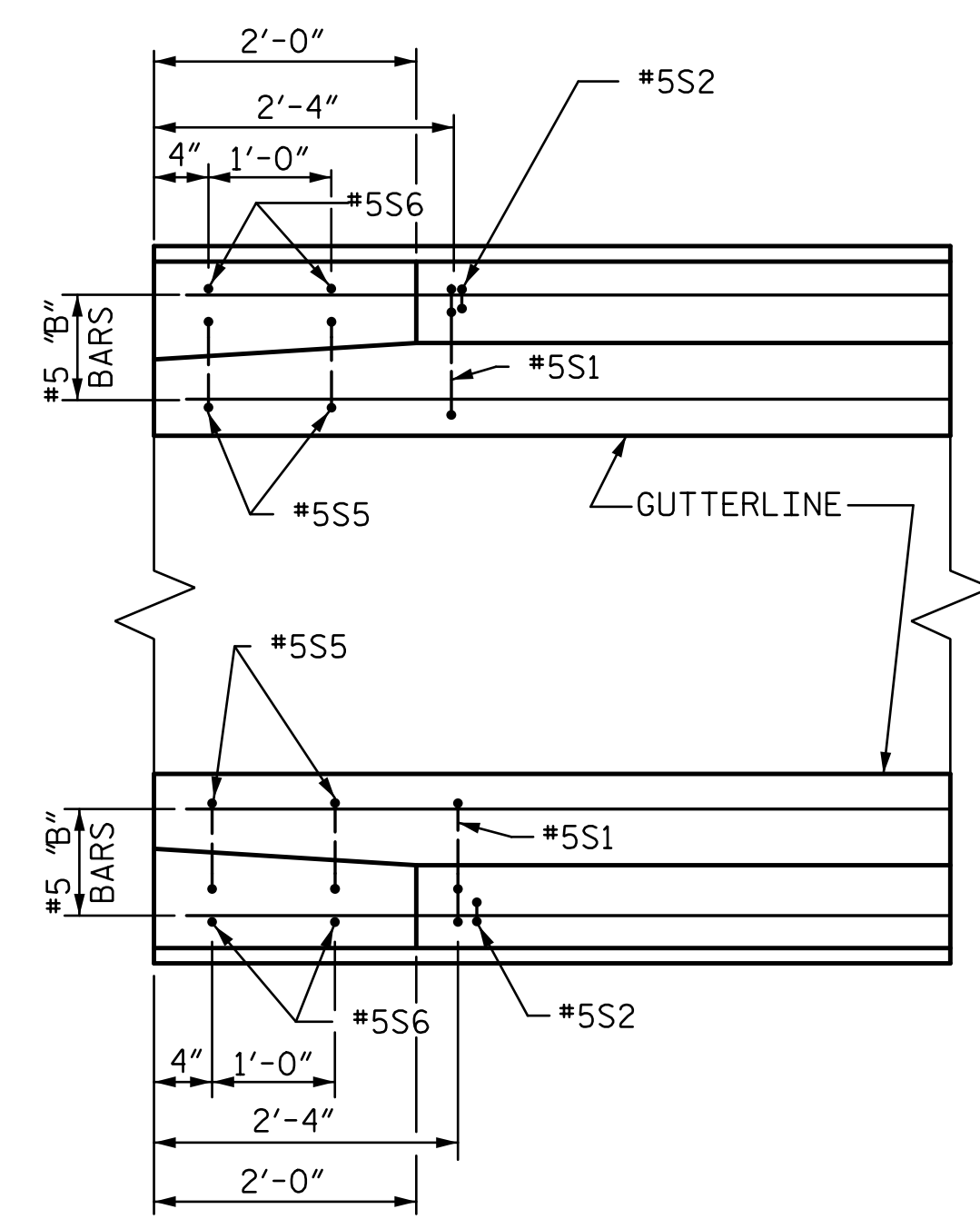
PLAN OF BARRIER RAIL



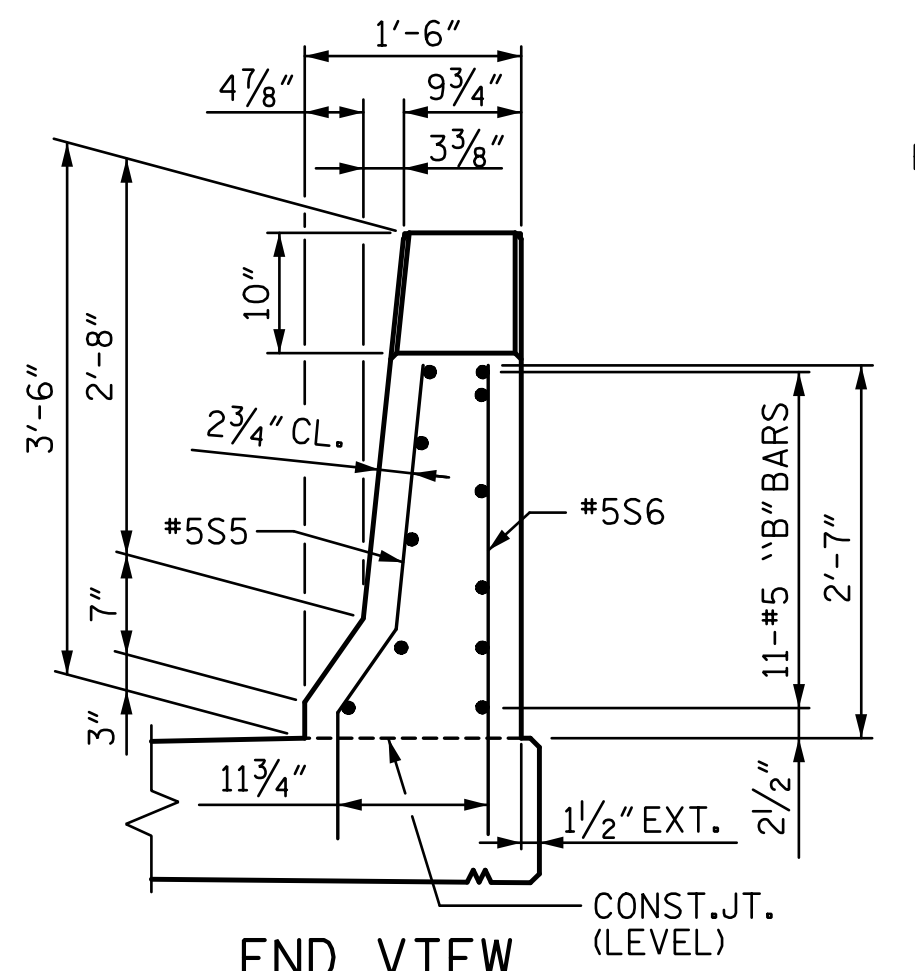
SECTION THRU RAIL



SECTION S-S  
 AT DAM IN OPEN JOINT  
 (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

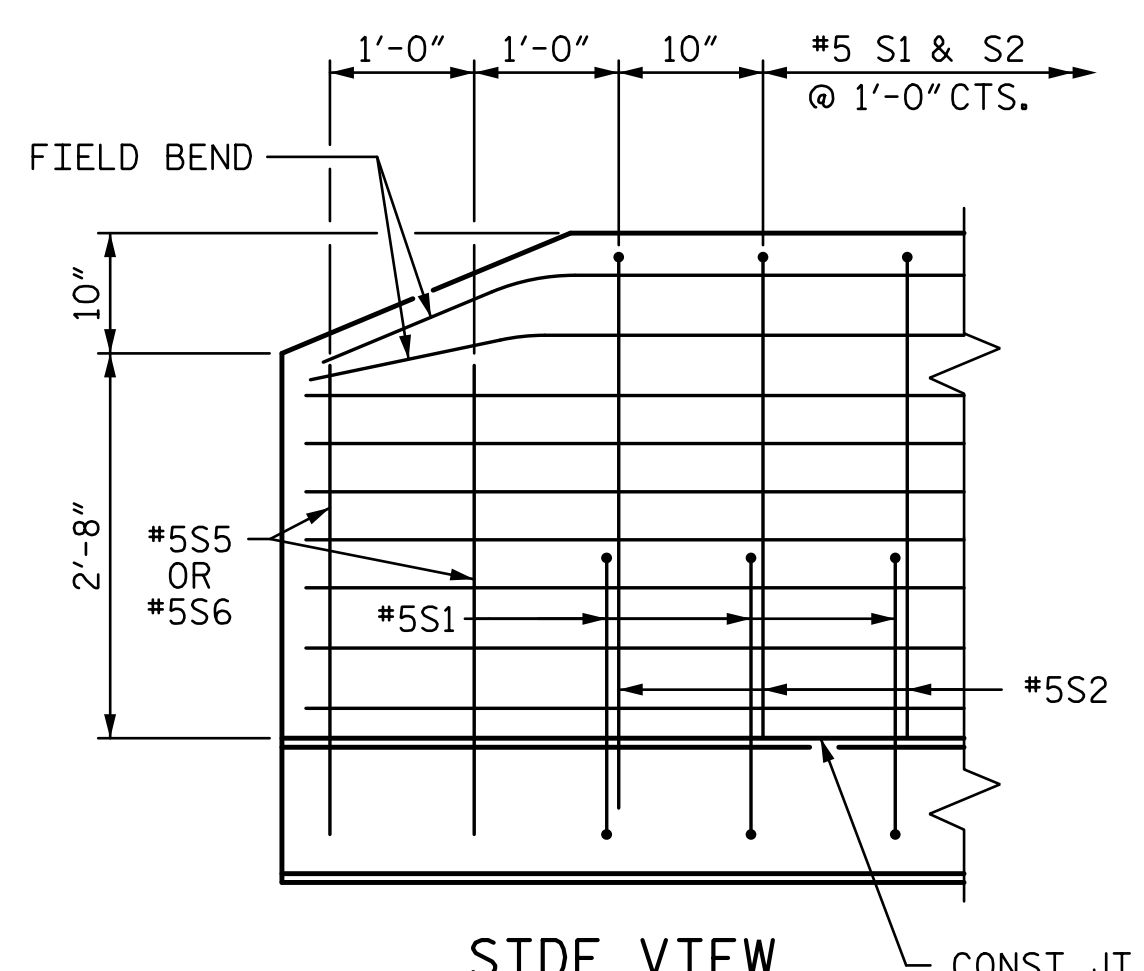


PLAN



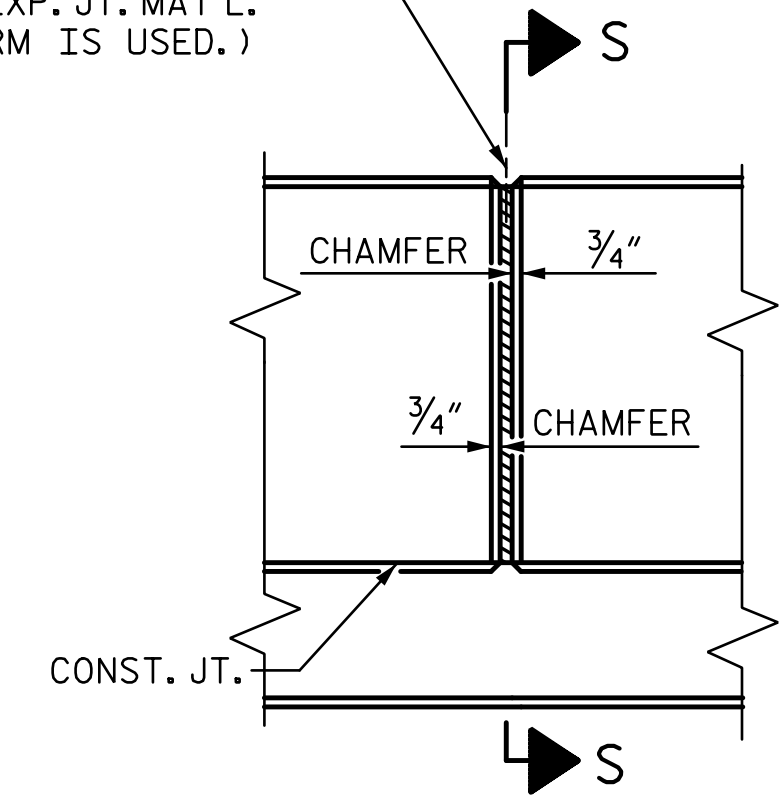
END VIEW

END OF RAIL DETAILS



SIDE VIEW

1/2" EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.  
 (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED.)



ELEVATION AT EXPANSION JOINTS  
 BARRIER RAIL DETAILS

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 RALEIGH

SEAL  
 40317  
 ENGINEER  
 TONY R. LAWS, JR.  
 3/6/2017

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 NC License Number F-5991

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TOTAL SHEETS: 27

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

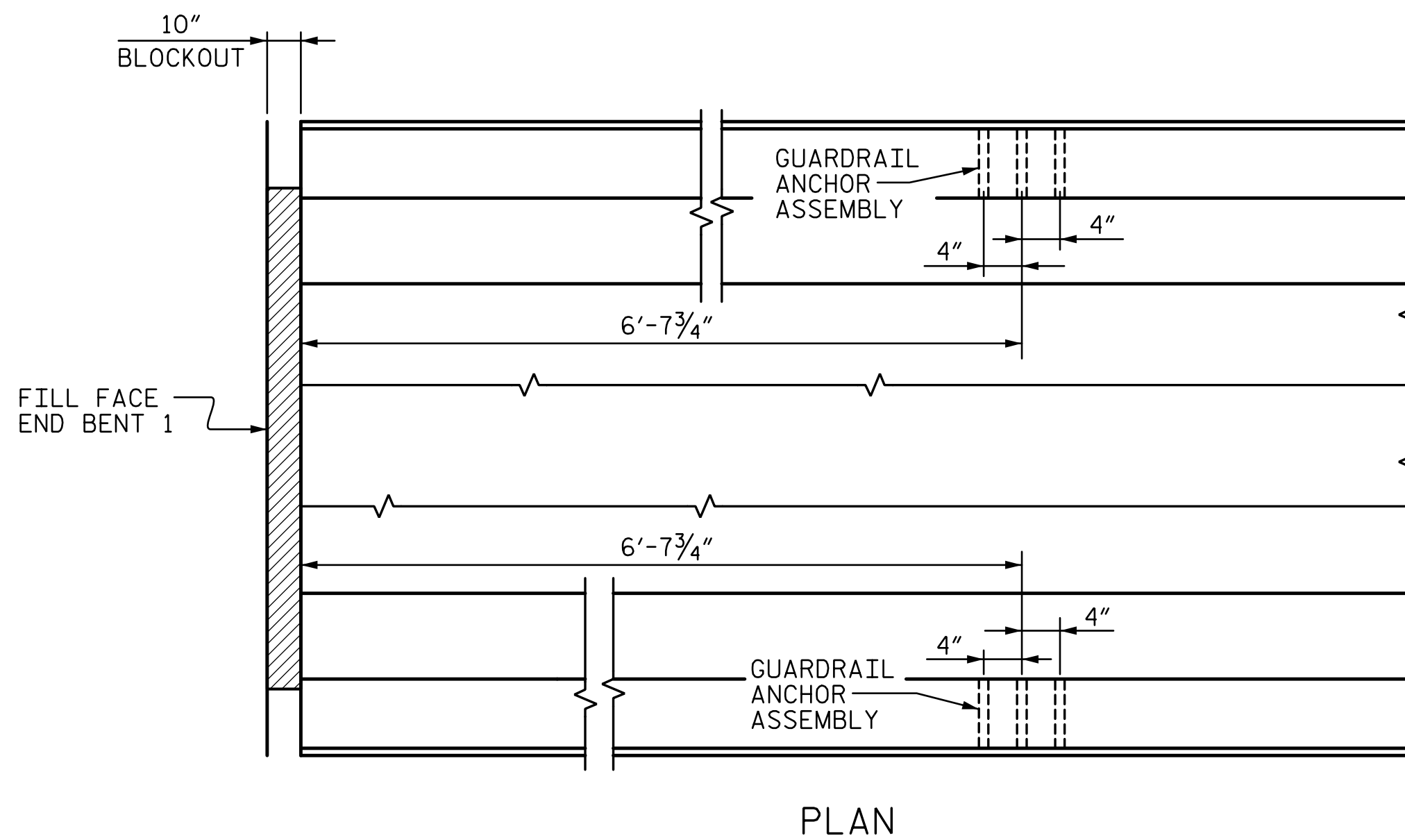
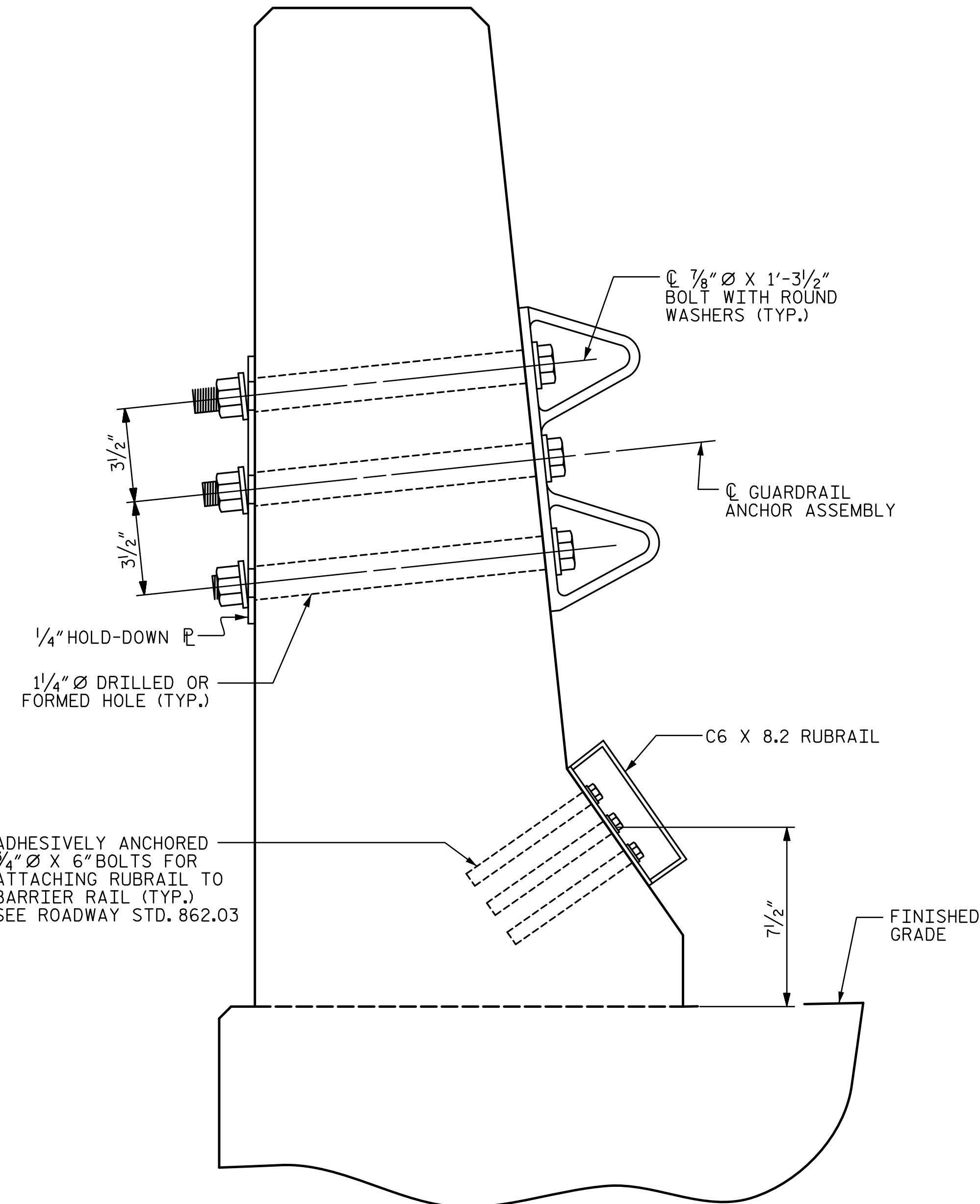
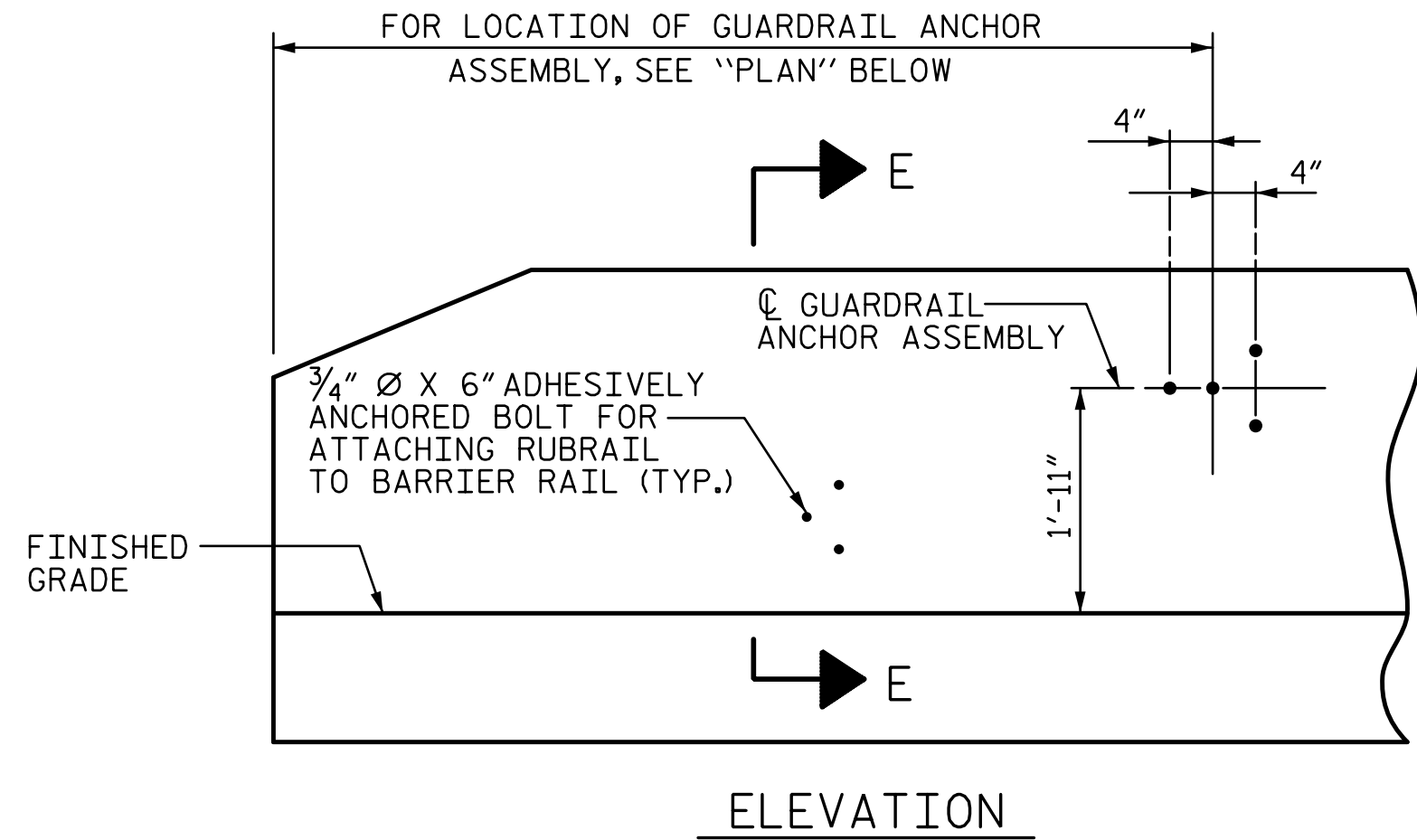
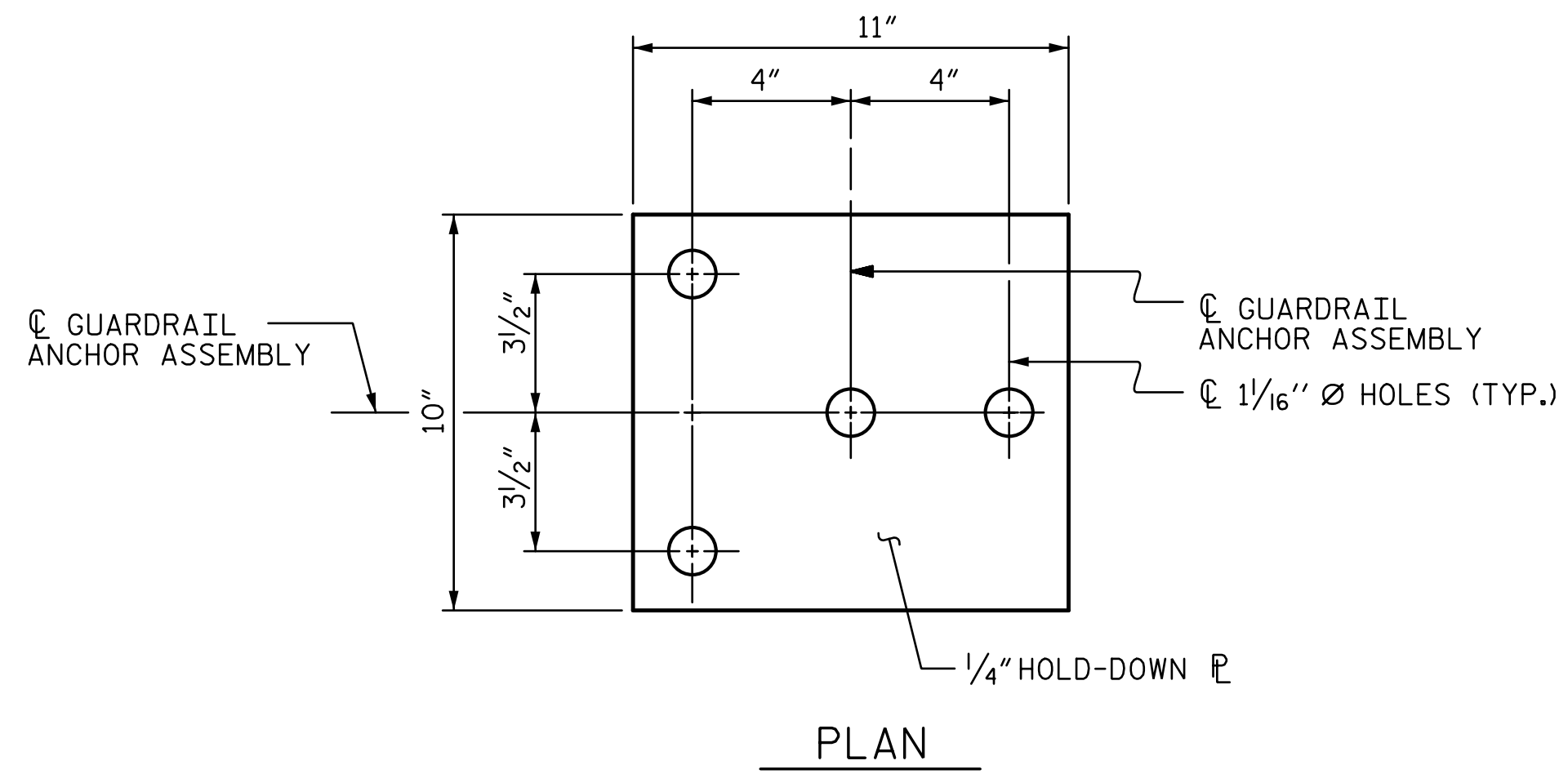
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENTS

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

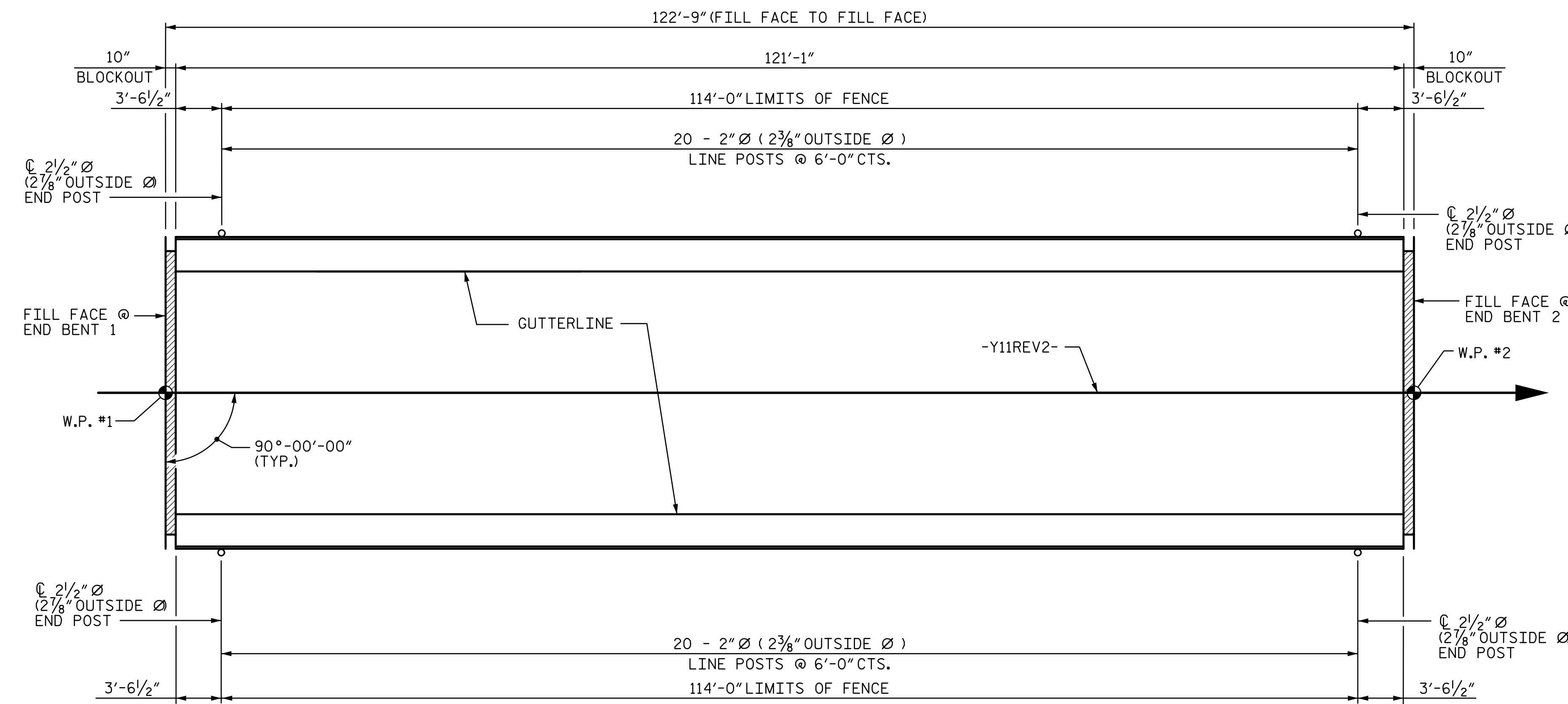
PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH  <b>GUARDRAIL ANCHORAGE                  FOR BARRIER RAIL</b>																			
	DocuSigned by: Tony R. Laws, Jr. License No. 40317 3/6/2017	REVISIONS <table border="1"> <tr> <th>NO.</th> <th>BY:</th> <th>DATE:</th> <th>NO.</th> <th>BY:</th> <th>DATE:</th> </tr> <tr> <td>1</td> <td></td> <td></td> <td>3</td> <td></td> <td></td> </tr> <tr> <td>2</td> <td></td> <td></td> <td>4</td> <td></td> <td></td> </tr> </table>	NO.	BY:	DATE:	NO.	BY:	DATE:	1			3			2			4			SHEET NO. S11-17  TOTAL SHEETS 27
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STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-5991																					

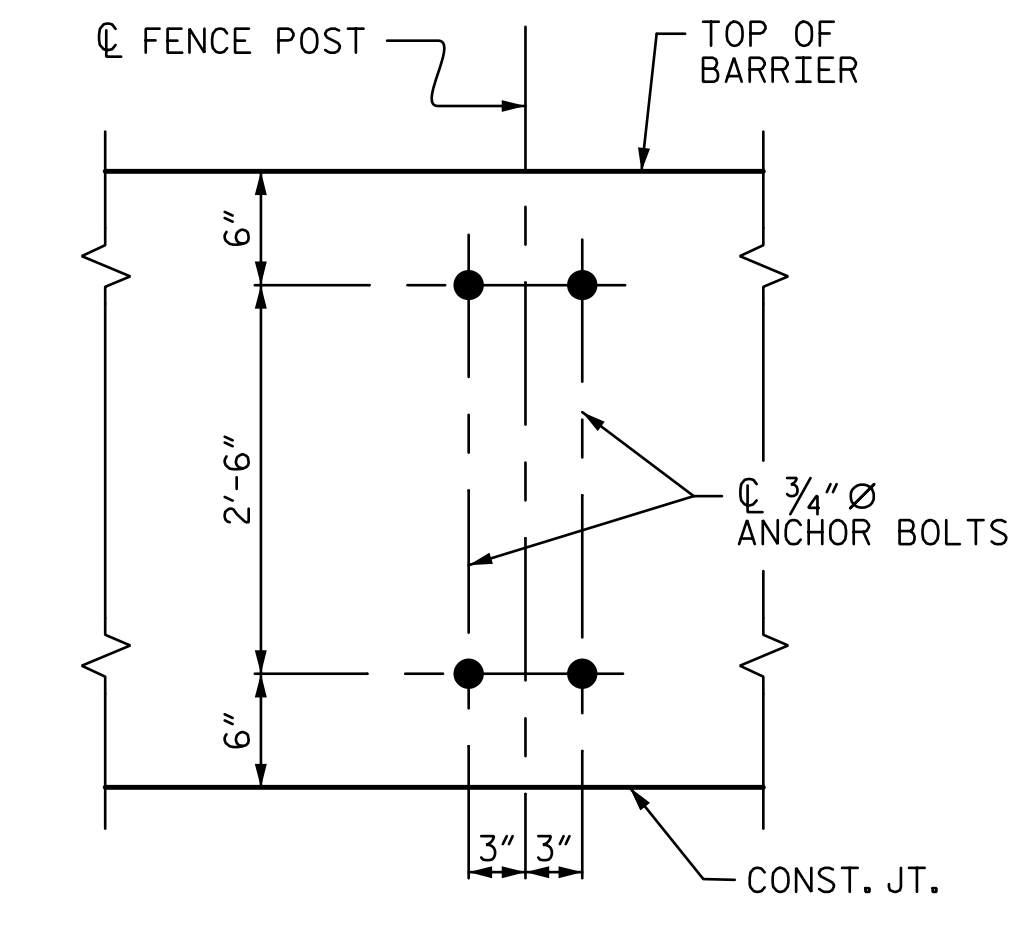
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CHECKED BY : <u>AJP</u>	DATE : <u>9-16</u>		

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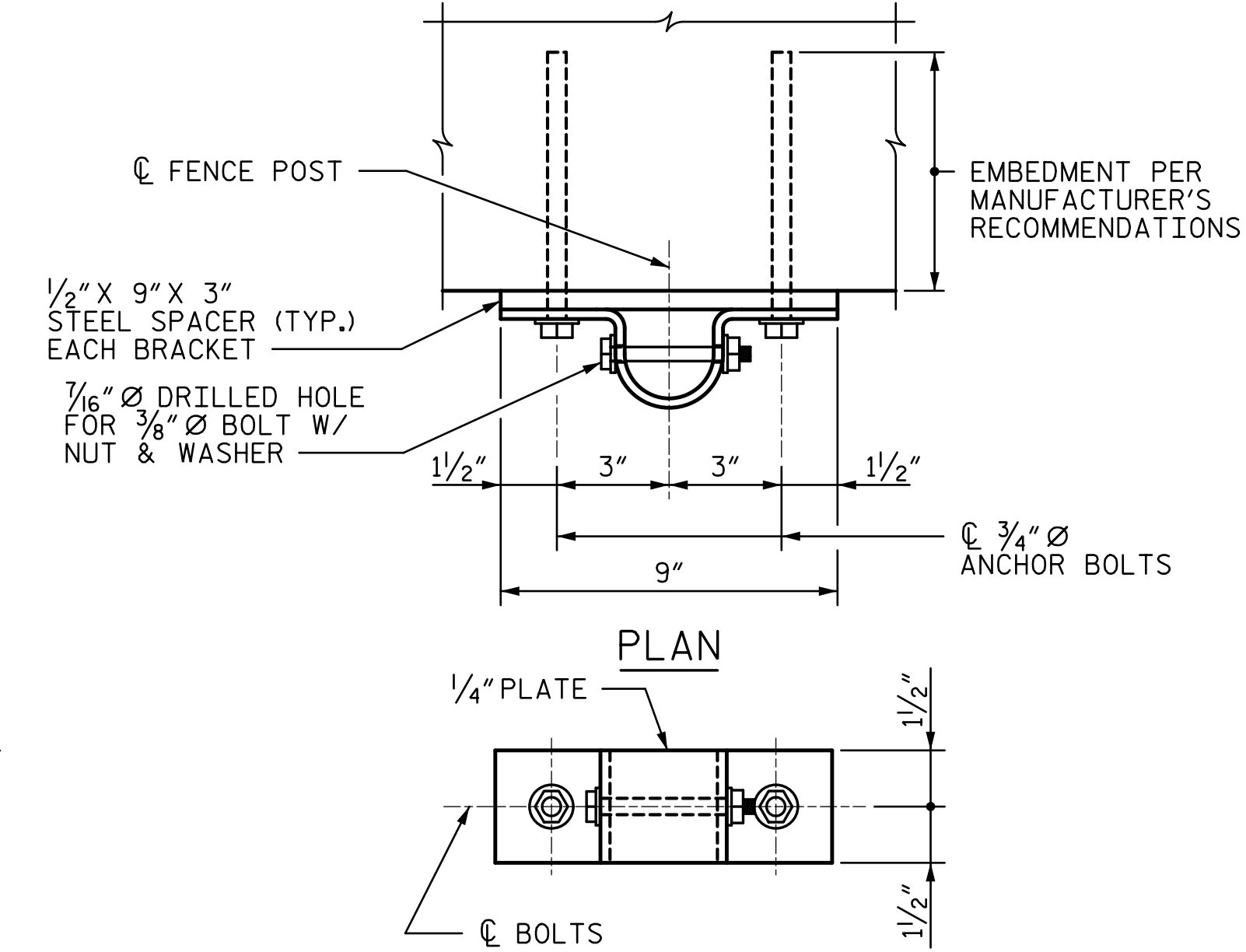


**PLAN OF FENCE POST SPACING**  
 PAY LENGTH 228.0 FEET

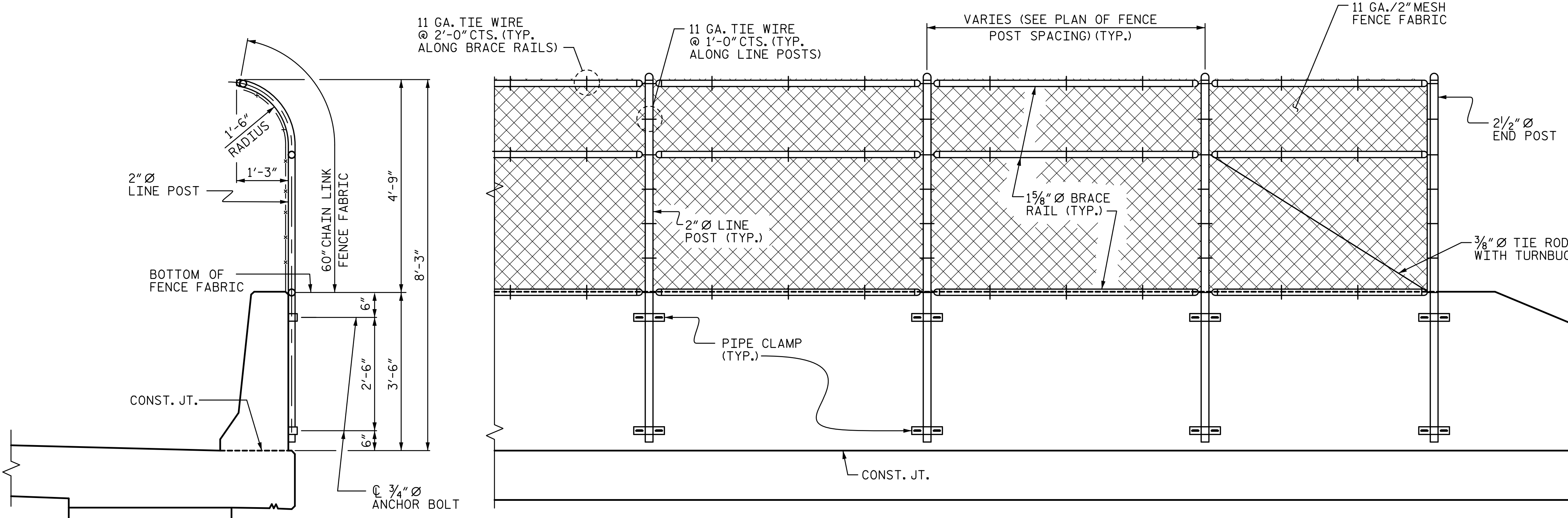


**BOLT SETTING DETAIL**

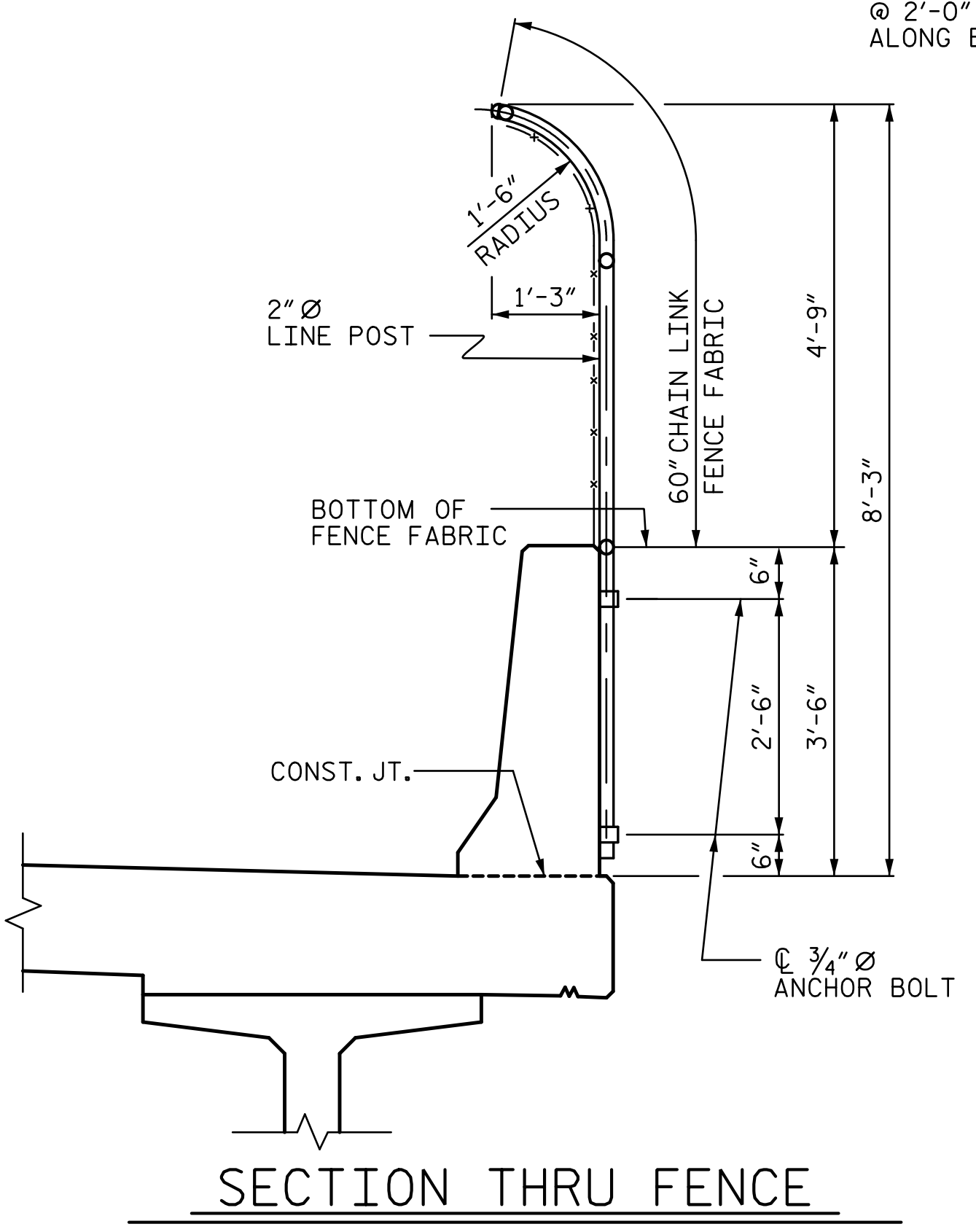
**NOTES:**  
 FOR BRIDGE MOUNTED CHAIN LINK FENCE, SEE SPECIAL PROVISIONS.  
 BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS, AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø GALVANIZED BOLTS, NUTS, AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)  
 LEVEL ONE FIELD TESTING IS REQUIRED AND THE YIELD LOAD OF THE ANCHOR BOLTS IS 10.0 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS, SEE THE STANDARD SPECIFICATIONS.  
 ALL FENCE MATERIAL SHALL MEET THE REQUIREMENTS OF SECTION 1050 OF THE STANDARD SPECIFICATIONS. GALVANIZE ALL STEEL PARTS AND HARDWARE IN ACCORDANCE WITH ARTICLE 1076 OF THE STANDARD SPECIFICATIONS.  
 FENCE POST LOCATIONS SHALL BE SHIFTED, AS NECESSARY, TO MAINTAIN 1'-0" MINIMUM DISTANCE FROM ANCHOR BOLT TO JOINTS IN CONCRETE BARRIER RAIL.  
 DIMENSIONS TAKEN ALONG OUTSIDE FACE OF CONCRETE BARRIER RAIL.  
 CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO AVOID DAMAGING THE REINFORCING STEEL IN CONCRETE BARRIER RAIL.



PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-



**PARTIAL ELEVATION**



**SECTION THRU FENCE**

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**BRIDGE MOUNTED CHAIN LINK FENCE**

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SHEET NO. S11-18  
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CHECKED BY: <u>AJP</u>	DATE: <u>9-16</u>		

**SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS**

BAR SIZE	SUPERSTRUCTURE EXCEPT APPROACH SLABS, PARAPET, AND BARRIER RAIL		APPROACH SLABS		PARAPET AND BARRIER RAIL
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"
#5	2'-6"	2'-2"	2'-6"	2'-2"	3'-5"
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"
#7	5'-3"	3'-6"			
#8	6'-10"	4'-7"			

**GROOVING BRIDGE FLOORS**

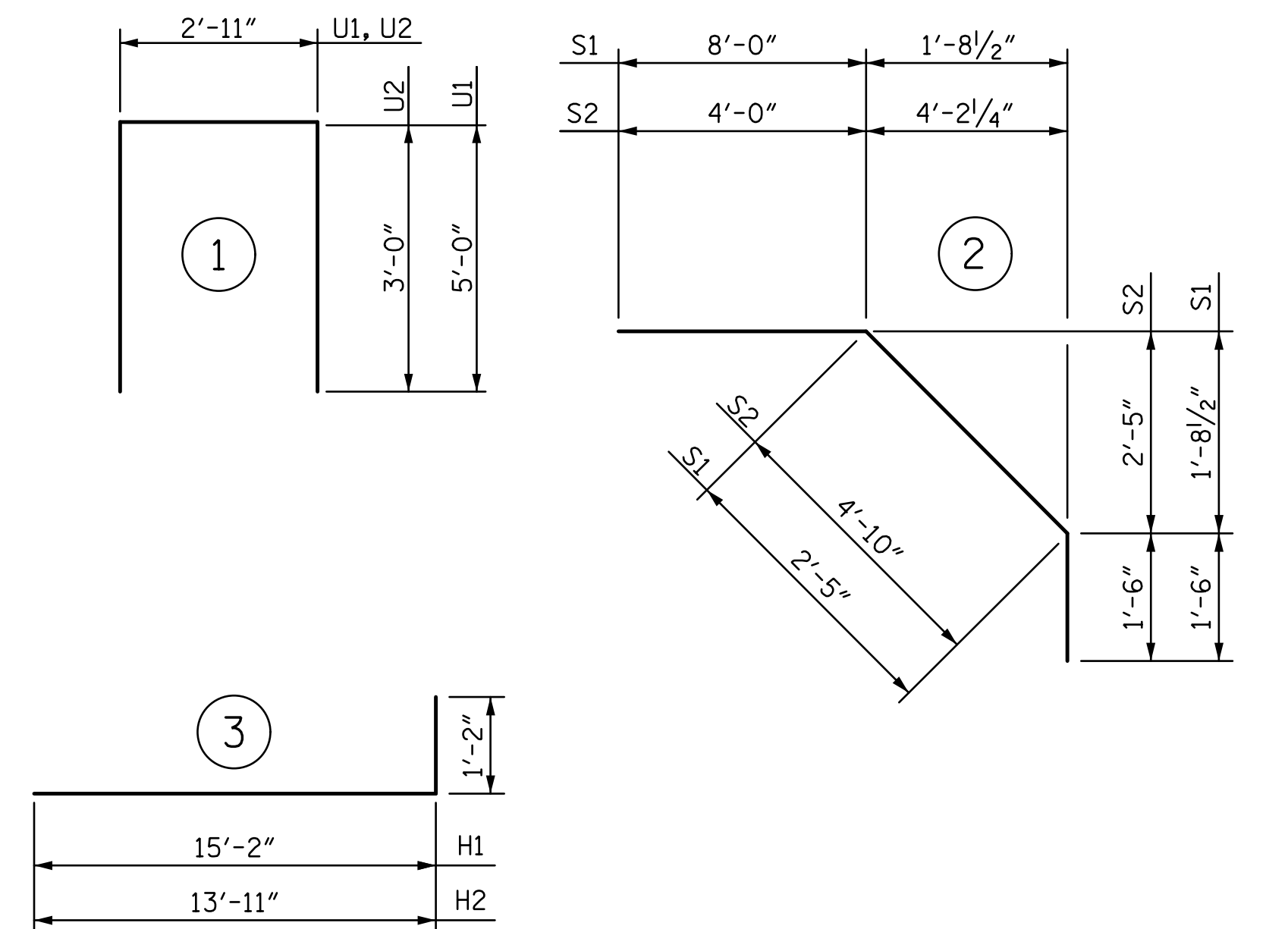
APPROACH SLABS	1,850 SQ.FT.
BRIDGE DECK	4,480 SQ.FT.
TOTAL	6,330 SQ.FT.

SUPERSTRUCTURE BILL OF MATERIAL				
	CLASS AA CONCRETE	CLASS A CONCRETE	*EPOXY COATED STEEL REINFORCING	STEEL REINFORCING
	(CU. YDS.)	(CU. YDS.)	(LBS.)	(LBS.)
SPAN A				
POUR 1	147.4			
POUR 2	68.4			
POUR 3		16.6		
TOTAL**	215.8	16.6	16,329	20,319

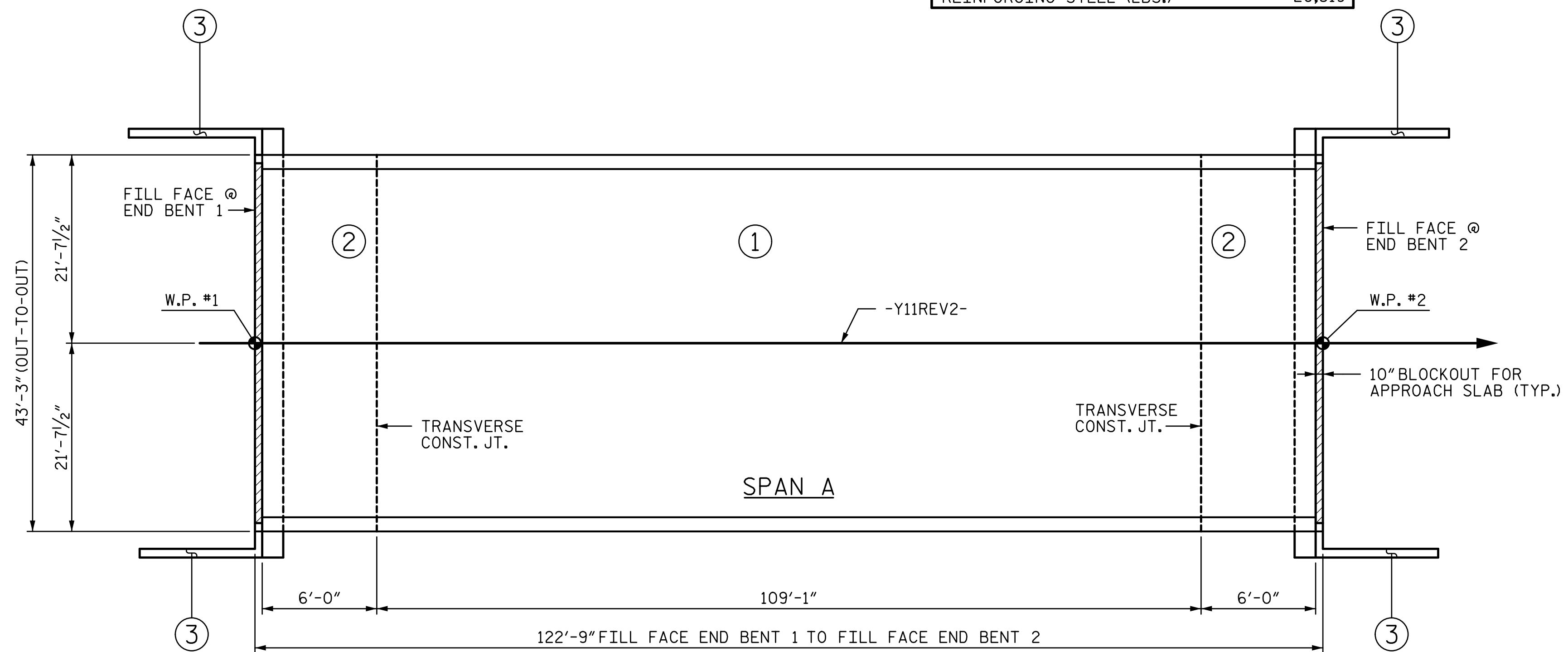
- \*\* QUANTITIES FOR CONCRETE BARRIER RAIL ARE NOT INCLUDED
- POUR 2 INCLUDES CONCRETE FOR SUPERSTRUCTURE PORTION OF INTEGRAL END BENT. ALL COSTS ASSOCIATED WITH THE SUPERSTRUCTURE PORTION OF THE INTEGRAL END BENT, INCLUDING BUT NOT LIMITED TO, MATERIALS, LABOR AND ALL INCIDENTALS SHALL BE INCLUDED IN THE CONTRACT BID PRICE FOR REINFORCED CONCRETE DECK SLAB. NO ADDITIONAL PAYMENT WILL BE MADE.
- POUR 3 INCLUDES CONCRETE FOR SUPERSTRUCTURE PORTION OF WING WALLS. ALL COSTS ASSOCIATED WITH THE SUPERSTRUCTURE PORTION OF THE WING WALLS, INCLUDING BUT NOT LIMITED TO, MATERIALS, LABOR AND ALL INCIDENTALS SHALL BE INCLUDED IN THE CONTRACT BID PRICE FOR REINFORCED CONCRETE DECK SLAB. NO ADDITIONAL PAYMENT WILL BE MADE.

REINFORCING BAR SCHEDULE					
MARK	NO.	SIZE	TYPE	LENGTH	WEIGHT
*A1	194	#5	STR	42'-10"	8,667
A2	194	#5	STR	42'-10"	8,667
*B1	150	#4	STR	25'-9"	2,580
*B2	54	#6	STR	27'-6"	2,230
*B3	54	#6	STR	24'-6"	1,987
B4	34	#5	STR	41'-9"	1,481
B5	68	#6	STR	41'-9"	4,264
H1	54	#7	(3)	16'-4"	1,803
H2	54	#7	(3)	15'-1"	1,665
K1	28	#4	STR	22'-4"	418
K2	40	#4	STR	8'-1"	216
K3	8	#4	STR	6'-6"	35
K4	8	#4	STR	5'-1"	27
K5	20	#4	STR	3'-0"	40
K6	4	#4	STR	2'-2"	6
K7	4	#4	STR	1'-6"	4
K8	58	#4	STR	2'-8"	103
*S1	60	#4	(2)	11'-11"	478
*S2	56	#4	(2)	10'-4"	387
U1	60	#4	(1)	12'-11"	518
U2	8	#4	(1)	8'-11"	48
V1	38	#5	STR	6'-4"	251
V2	38	#5	STR	6'-3"	248
V3	38	#5	STR	6'-8"	264
V4	38	#5	STR	6'-7"	261
*EPOXY COATED REINF. STEEL (LBS.)					16,329
REINFORCING STEEL (LBS.)					20,319

**BAR TYPES**



ALL BAR DIMENSIONS ARE OUT TO OUT.



**POURING DIAGRAM AND LAYOUT FOR COMPUTING AREA REINFORCED CONCRETE DECK SLAB (SQ. FT. = 5,309)**

(#) = INDICATES POUR NUMBER

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

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37672017

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

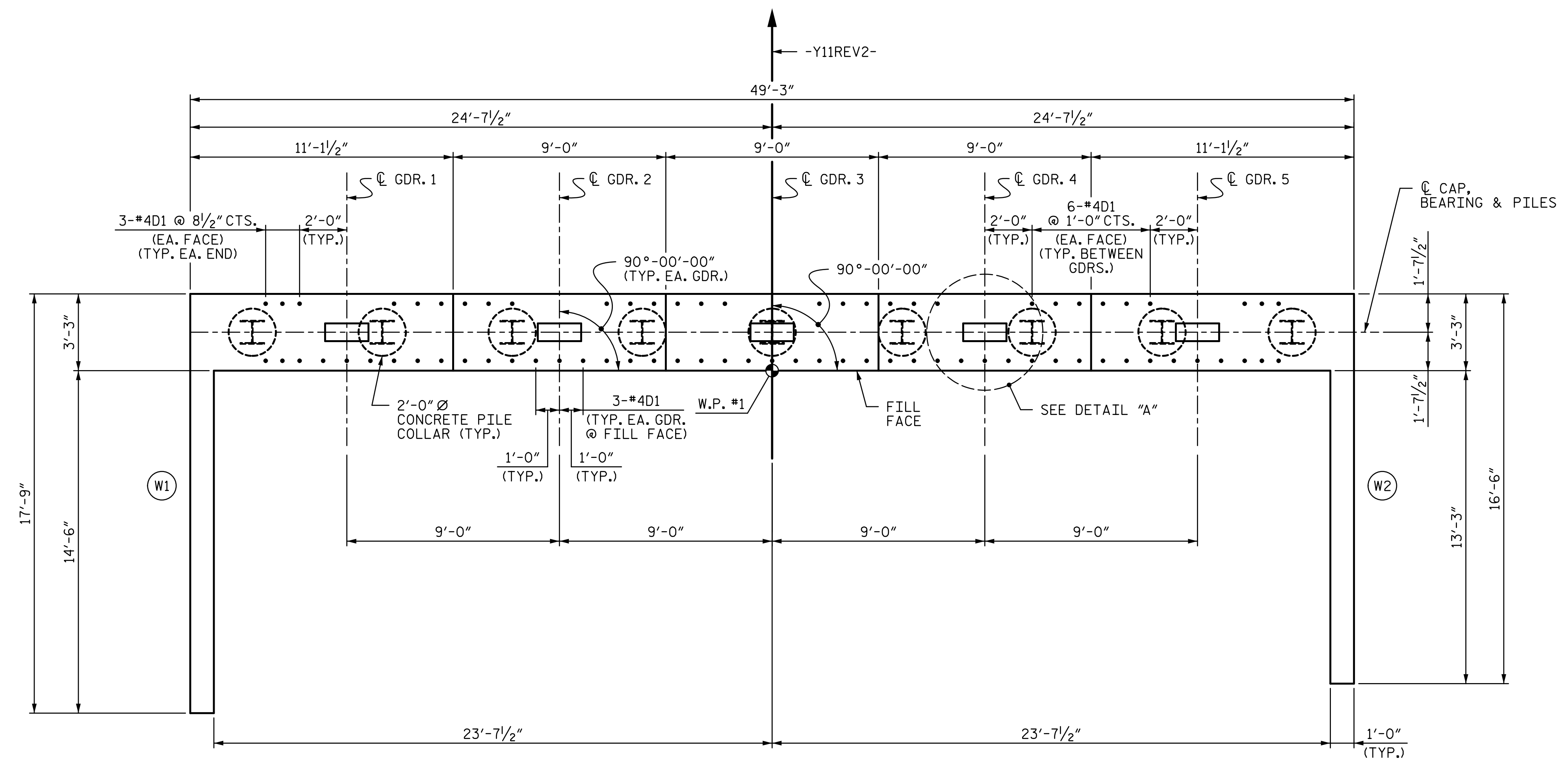
**SUPERSTRUCTURE  
 BILL OF MATERIAL**

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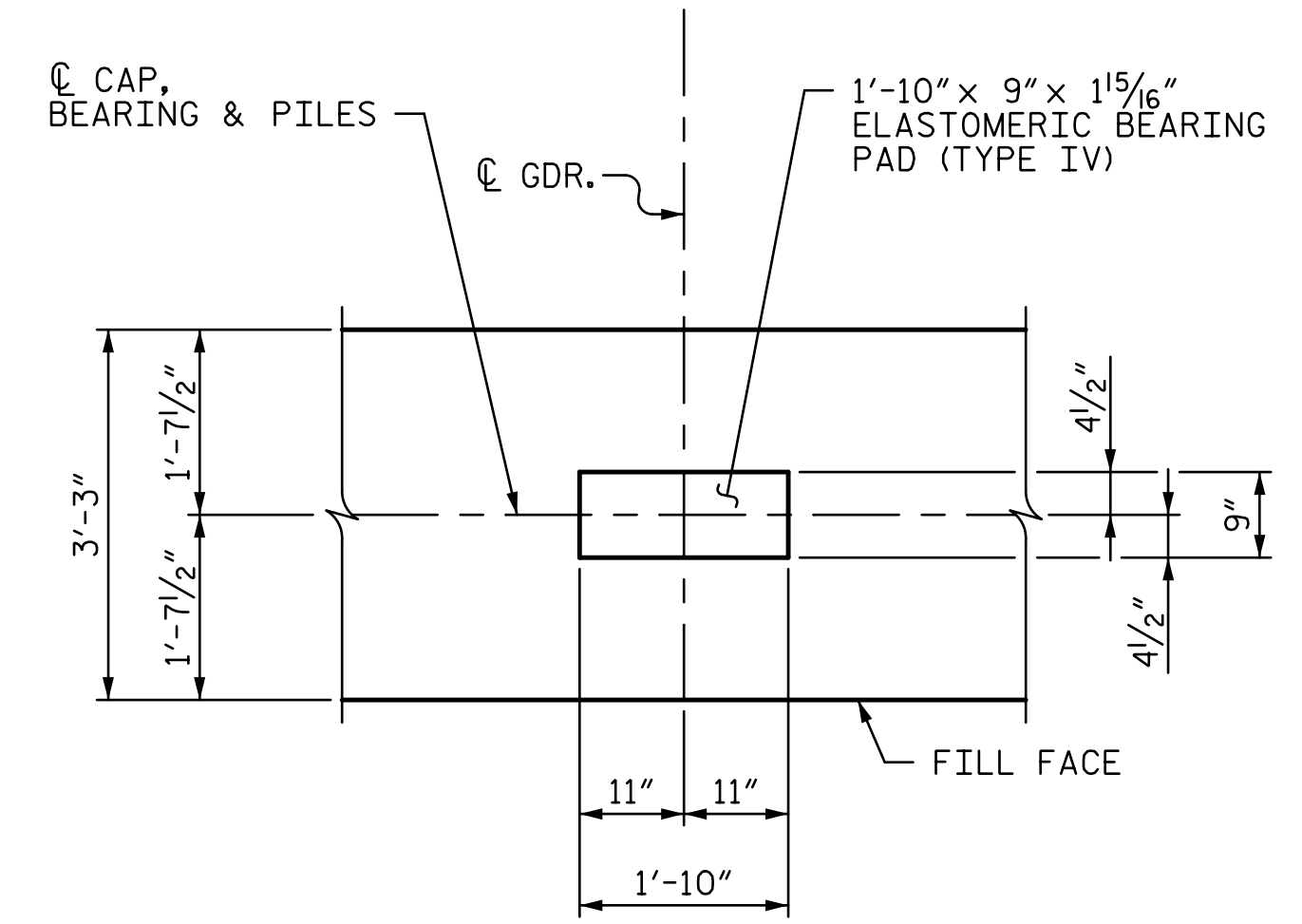
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 S11-19  
 TOTAL SHEETS  
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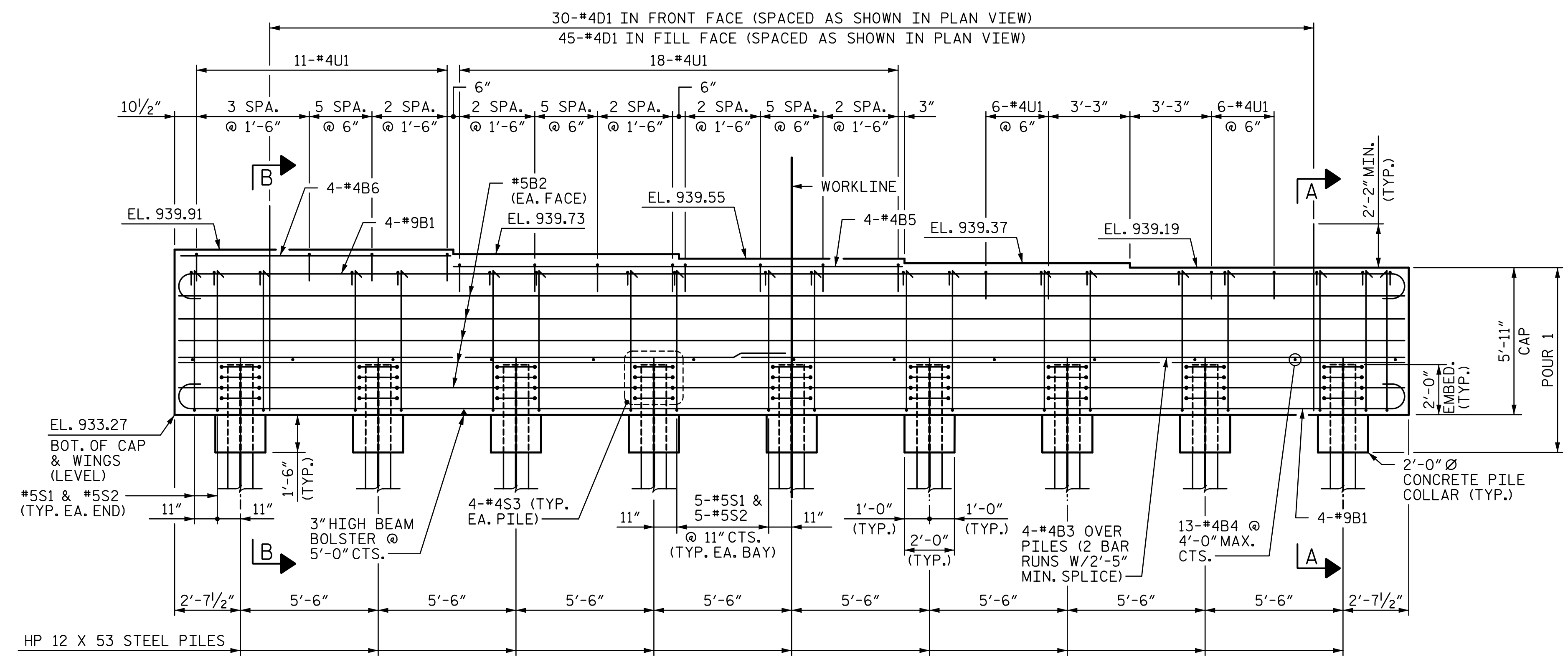
**NOTES:**  
 FOR NOTES, SEE SHEET 3 OF 3.  
 FOR SECTIONS A-A AND B-B, SEE SHEET 3 OF 3.



**PLAN**



**DETAIL "A"**  
 (DETAILS AND DIMENSIONS ARE TYPICAL FOR EACH BEARING)



**ELEVATION**

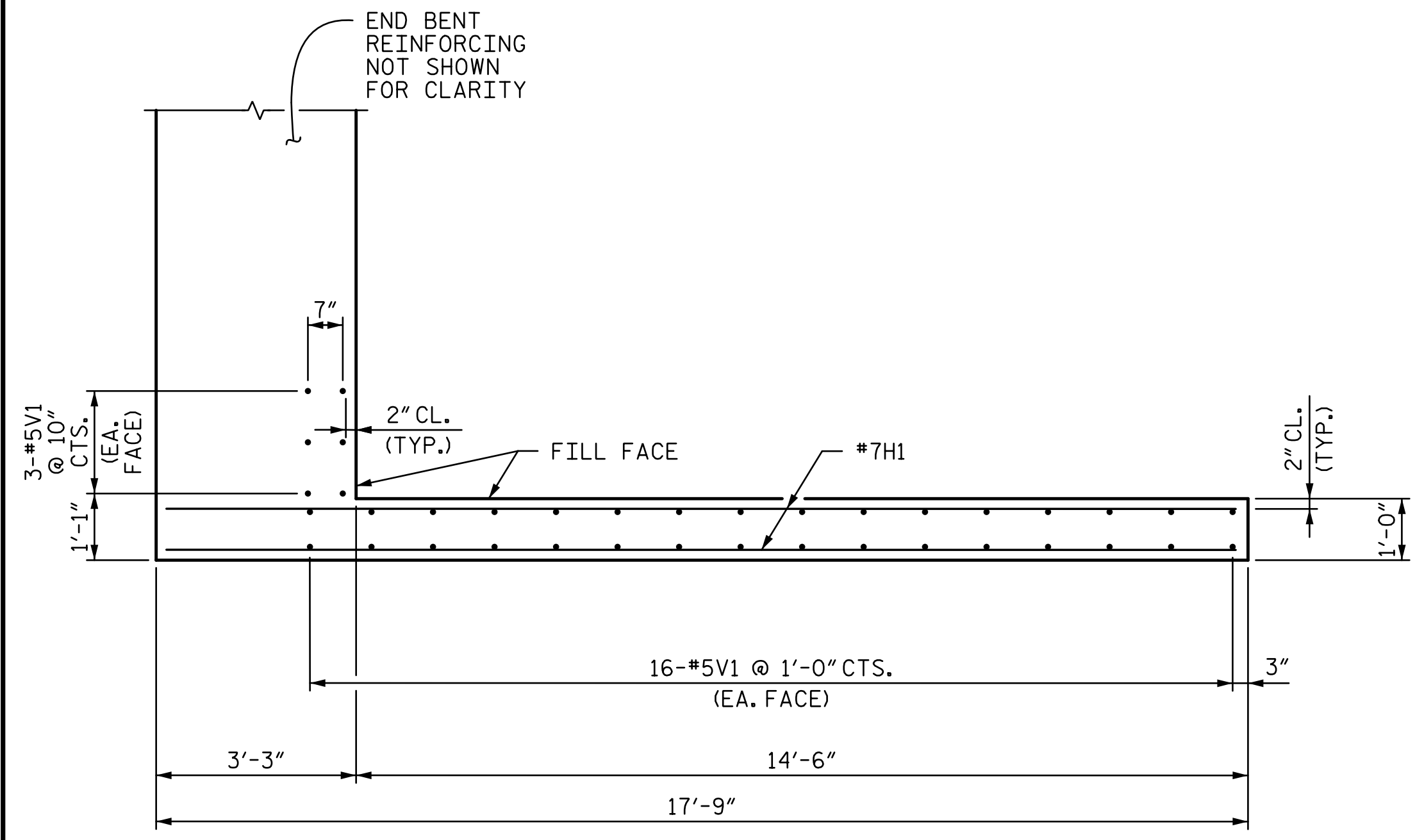
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PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 1 OF 3

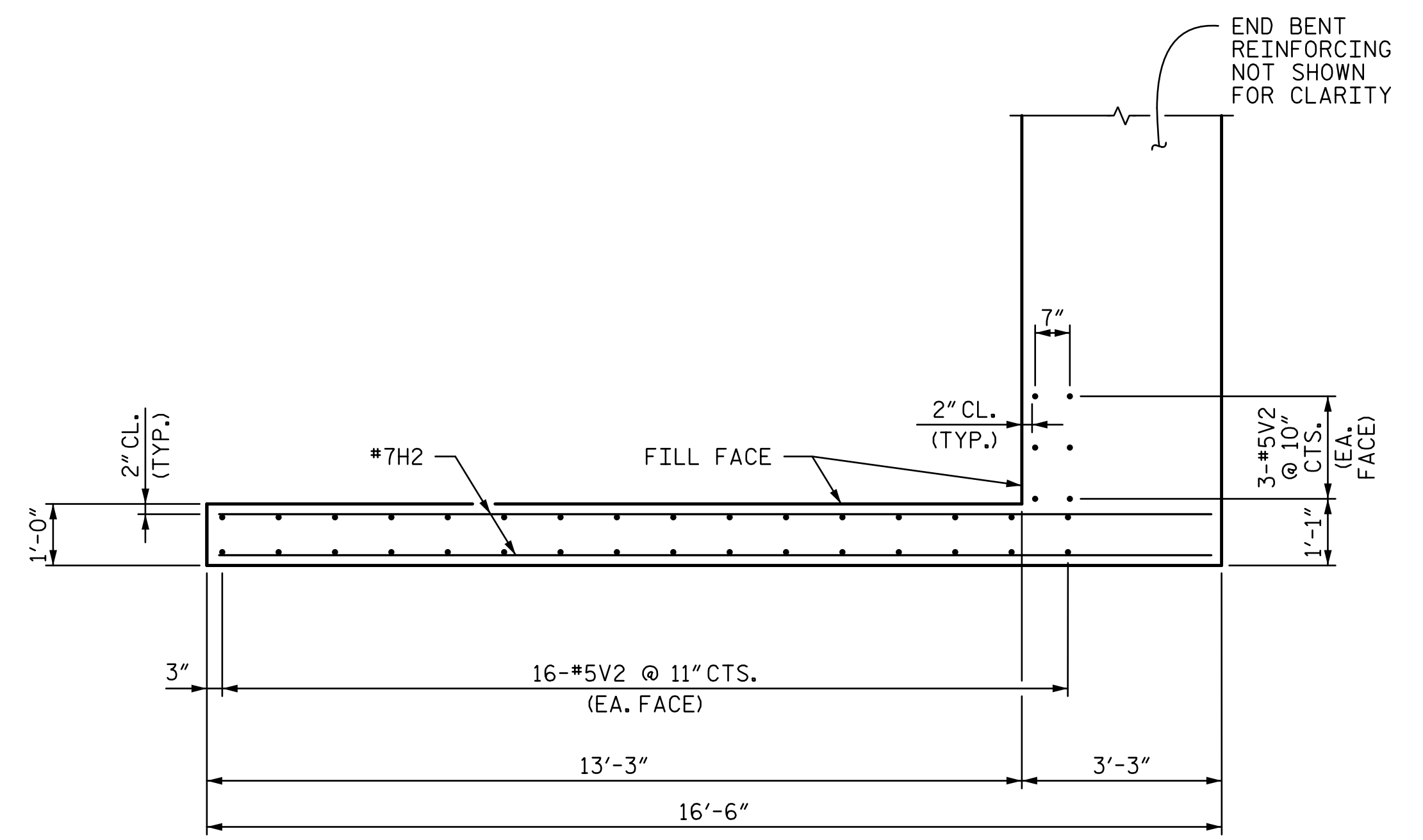
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		REVISIONS				

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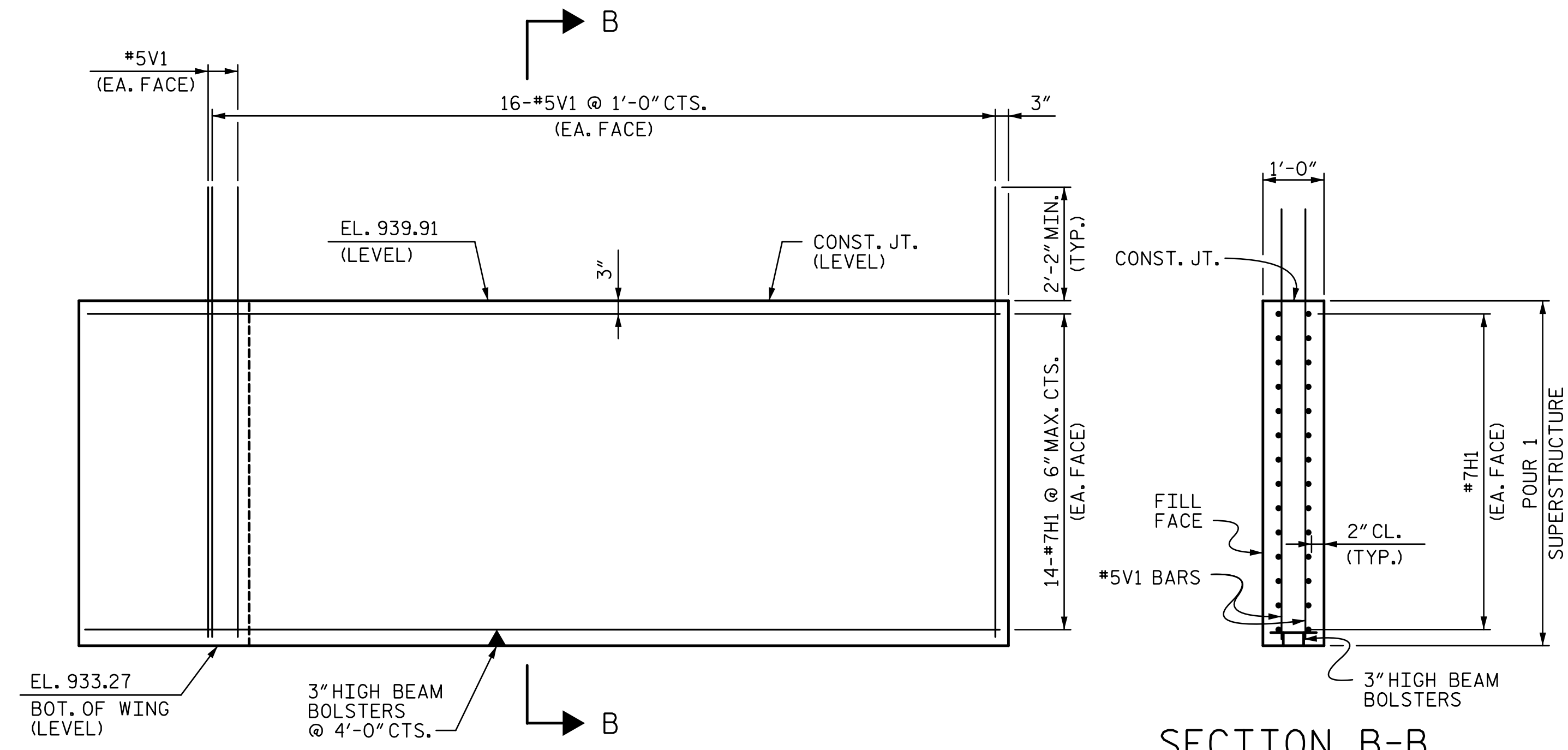
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WING WALL PLAN (W1)

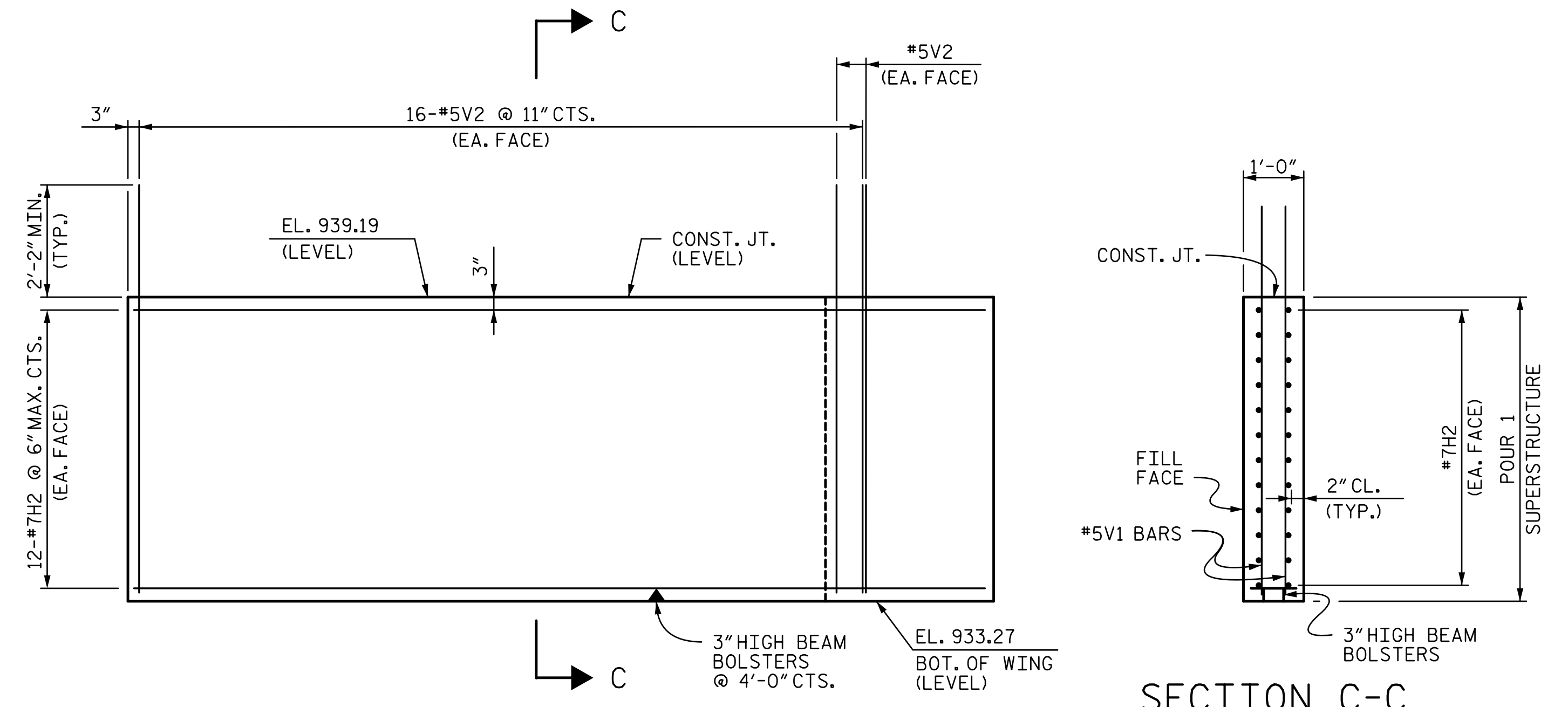


WING WALL PLAN (W2)



WING WALL ELEVATION (W1)

SECTION B-B



WING WALL ELEVATION (W2)

SECTION C-C

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 2 OF 3

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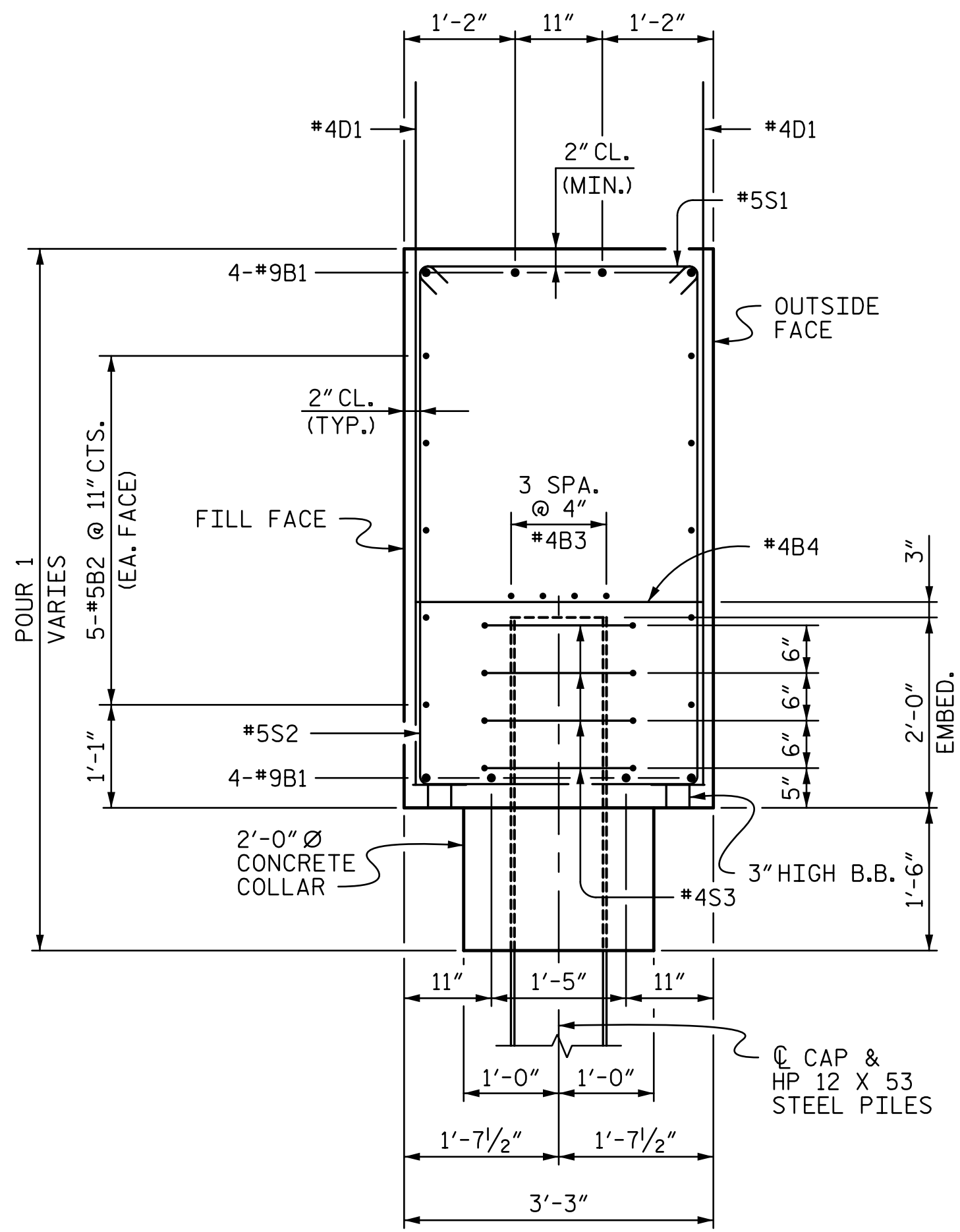
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**END BENT 1 (INTEGRAL)**

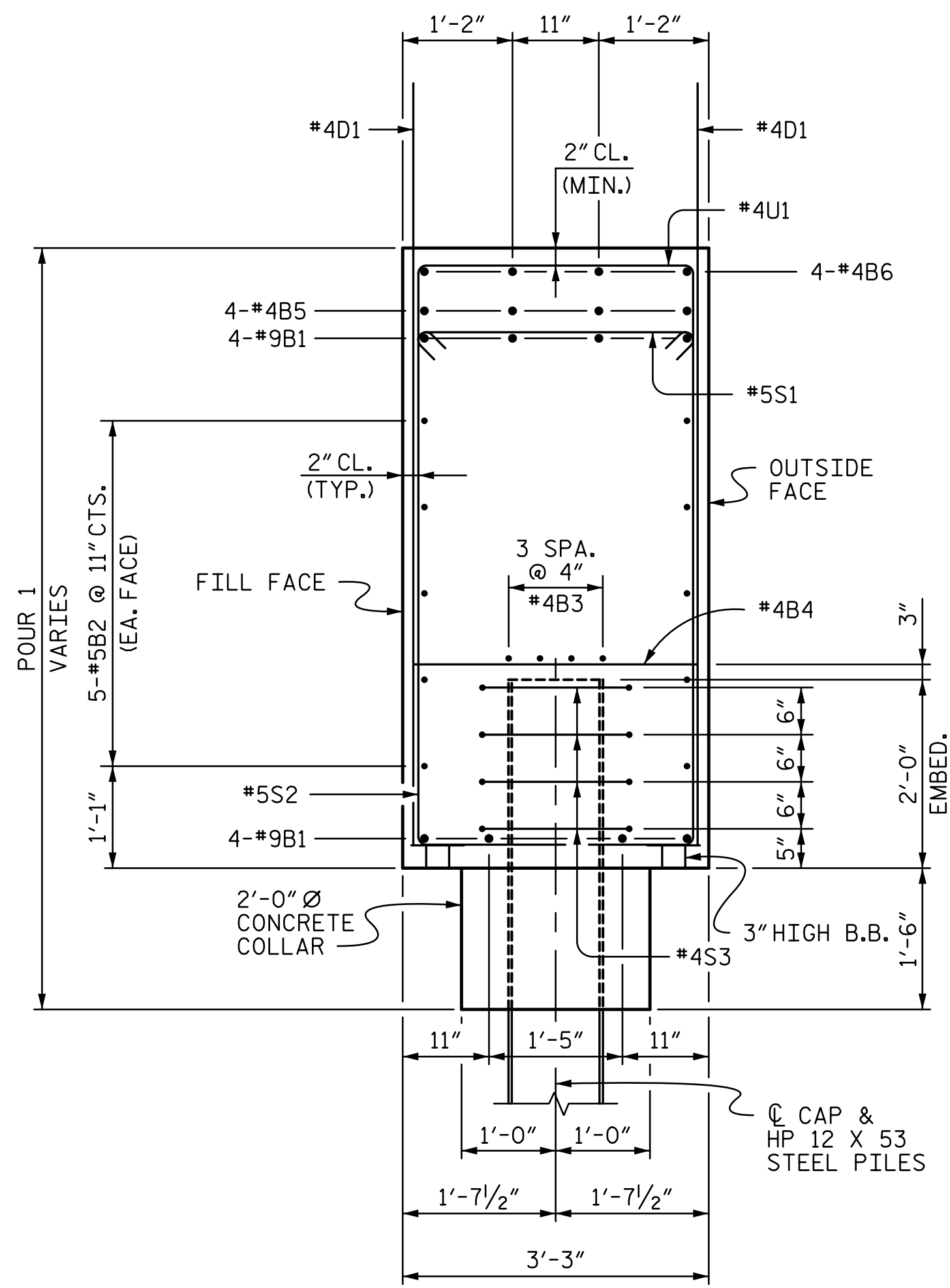
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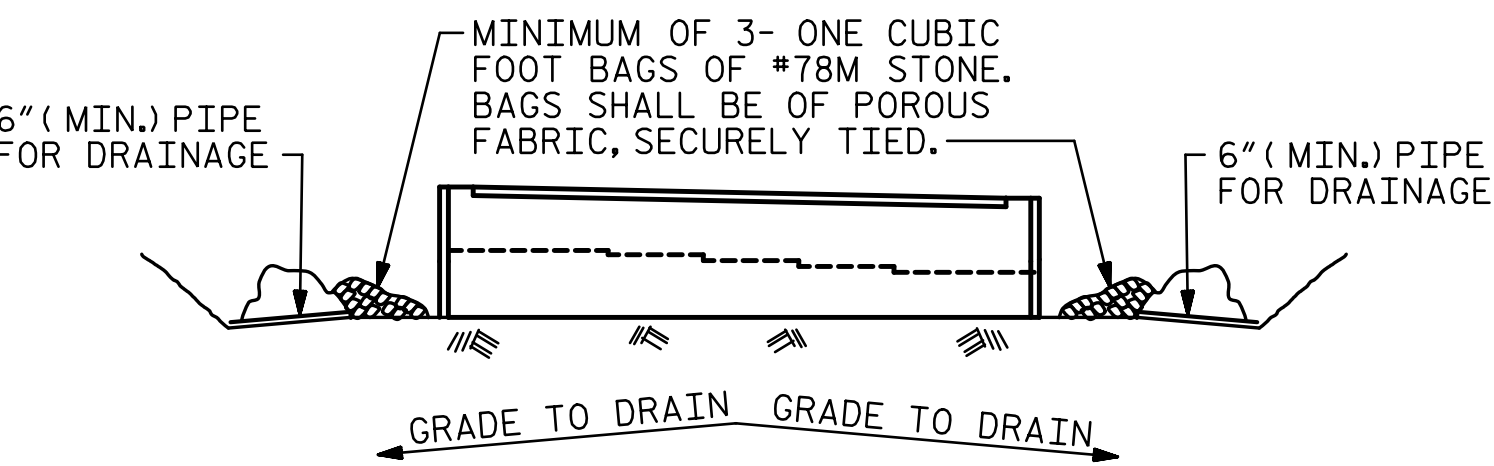




SECTION A-A



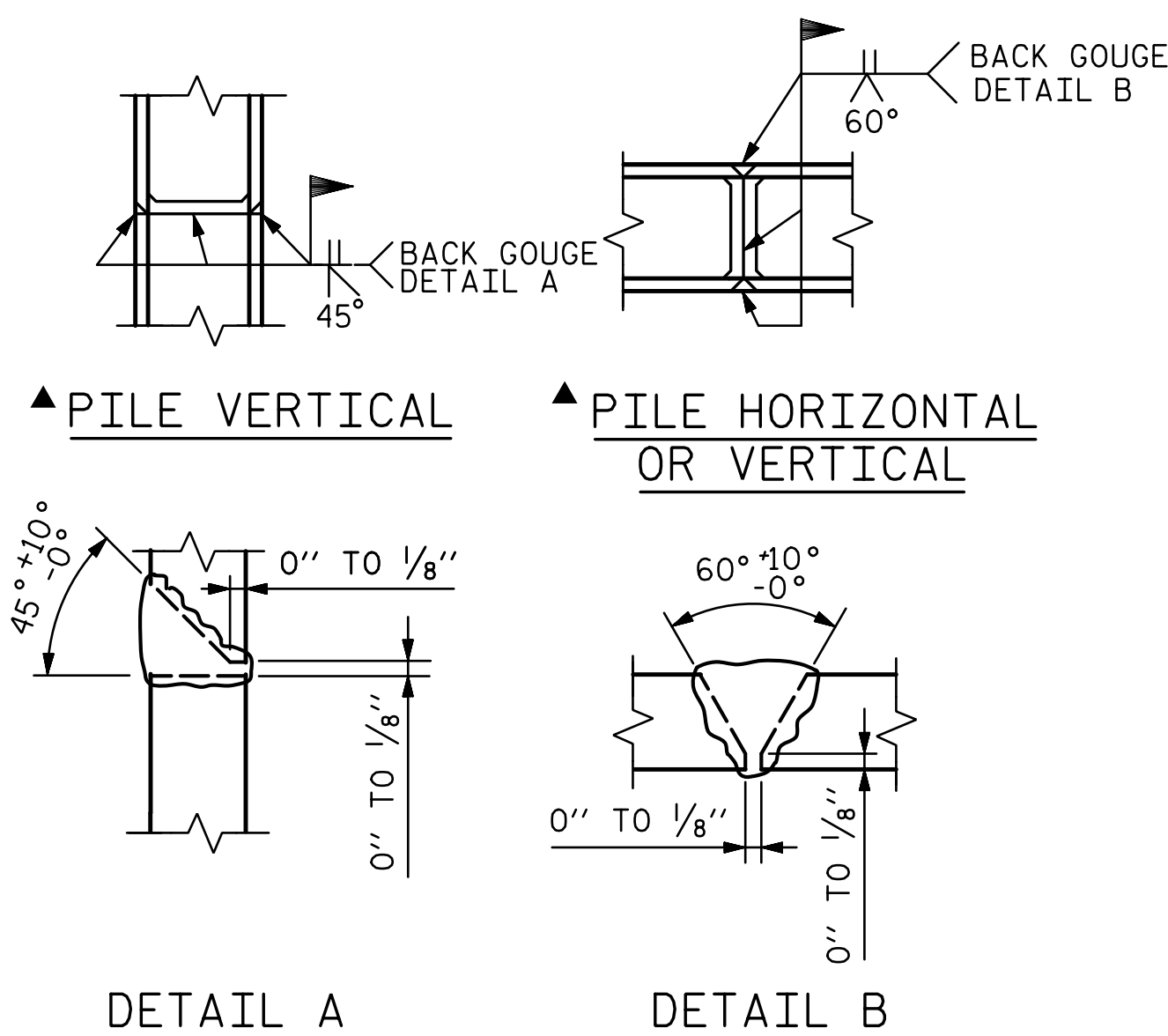
SECTION B-B



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

**TEMPORARY DRAINAGE AT END BENT**

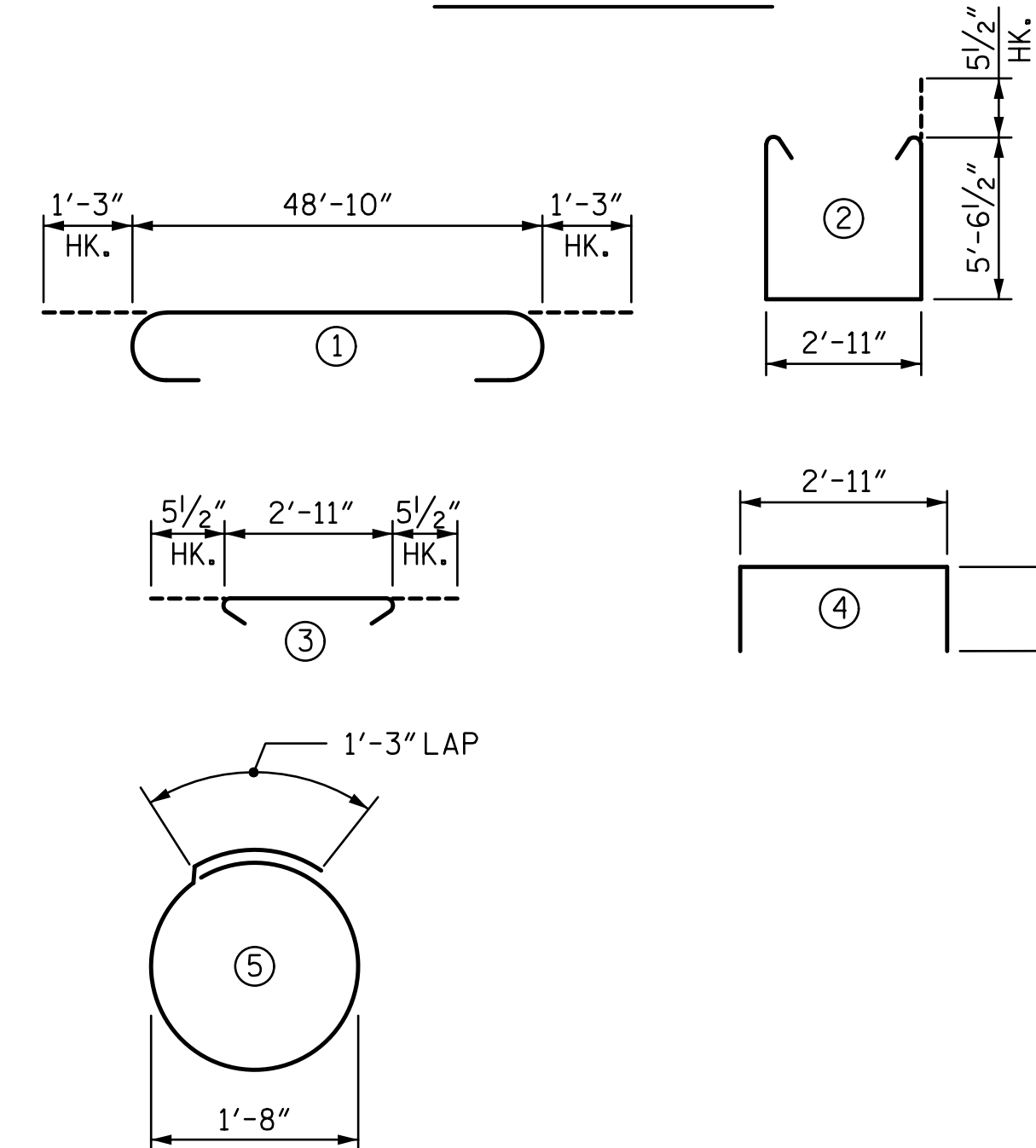


DETAIL A DETAIL B

**PILE SPLICE DETAILS**

▲ POSITION OF PILE DURING WELDING.

**BAR TYPES**



ALL BAR DIMENSIONS ARE OUT TO OUT.

**BILL OF REINFORCING**

MARK	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1	8	#9	①	51'-4"	1,396
B2	10	#5	STR	48'-11"	510
B3	8	#4	STR	25'-8"	137
B4	13	#4	STR	2'-11"	25
B5	4	#4	STR	17'-6"	47
B6	4	#4	STR	10'-7"	28
D1	75	#4	STR	8'-6"	426
H1	28	#7	STR	17'-5"	997
H2	24	#7	STR	16'-2"	793
S1	44	#5	②	14'-11"	685
S2	44	#5	③	3'-10"	176
S3	36	#4	⑤	6'-5"	154
U1	41	#4	④	5'-11"	162
V1	38	#5	STR	8'-8"	343
V2	38	#5	STR	7'-11"	314

**QUANTITIES**

REINFORCING STEEL	LBS.	END BENT 1
REINFORCING STEEL		6,193
CLASS A CONCRETE		
POUR 1 (CAP, COLLARS & LOWER WING):	CU. YARDS	45.3
HP 12 X 53 STEEL PILES	NO.	9
	LIN. FEET	495.0
PILE DRIVING EQUIP. SETUP FOR HP 12x53 STEEL PILES	EA.	9

**NOTES:**

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #4D1 BARS.

FOR INTEGRAL BACKWALL REINFORCEMENT, SEE "TYPICAL SECTION AND INTEGRAL BACKWALL" SHEET.

FOR FOUNDATION NOTES, SEE "FOUNDATION LAYOUT" SHEET.

THE TOP SURFACE OF THE END BENT CAP AND WINGS, EXCEPT THE BEARING AREA, SHALL BE RAKED TO A DEPTH OF 1/4".

INSTALL THE 4" DIA. DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE UPPER PORTION OF THE INTEGRAL END BENT CAP AND THE UPPER PORTION OF THE WINGS SHALL BE POURED WITH THE SUPERSTRUCTURE. SEE SUPERSTRUCTURE PLANS.

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SHEET 3 OF 3

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RALEIGH

SUBSTRUCTURE

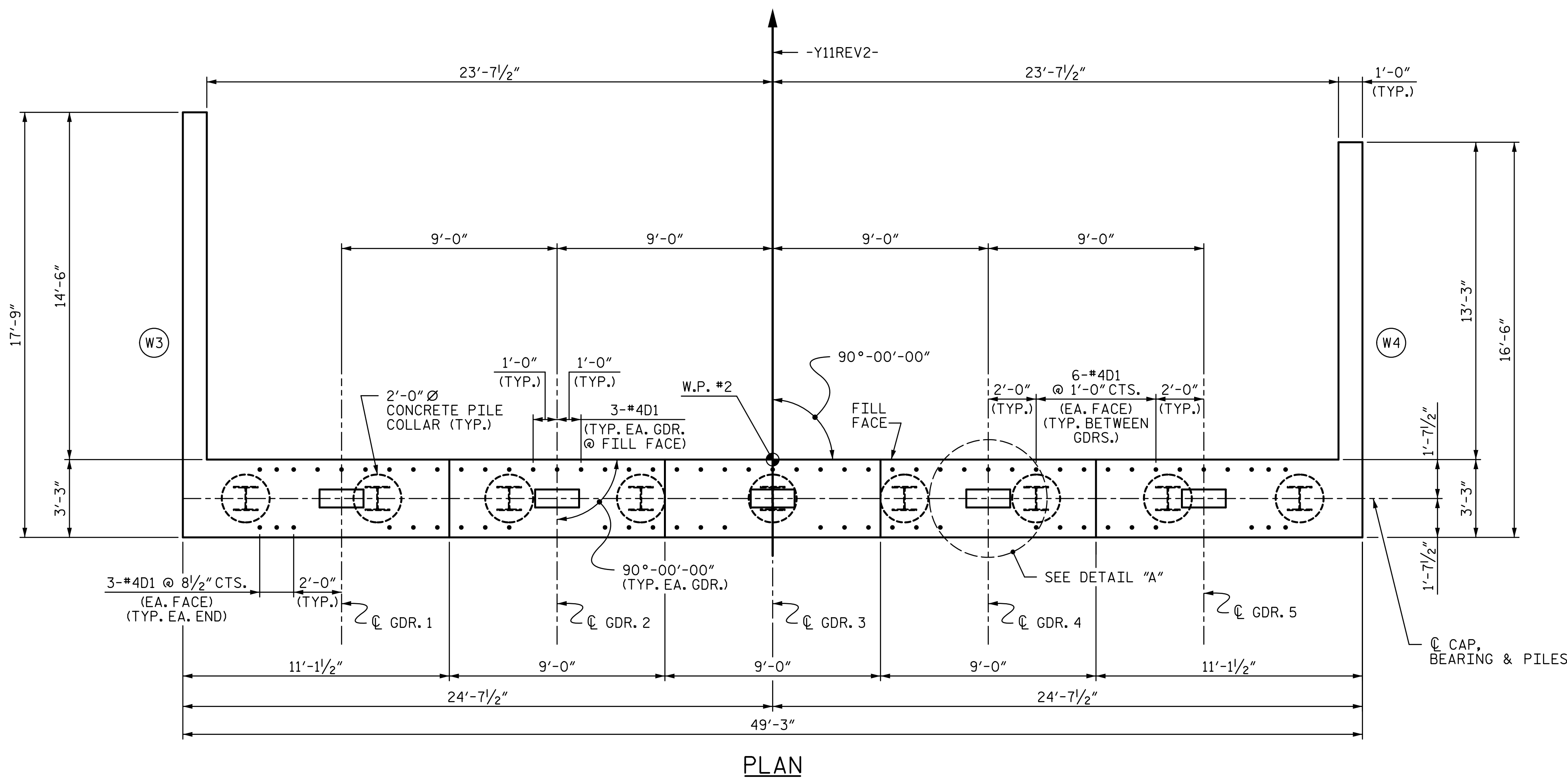
END BENT 1  
(INTEGRAL)

REVISIONS

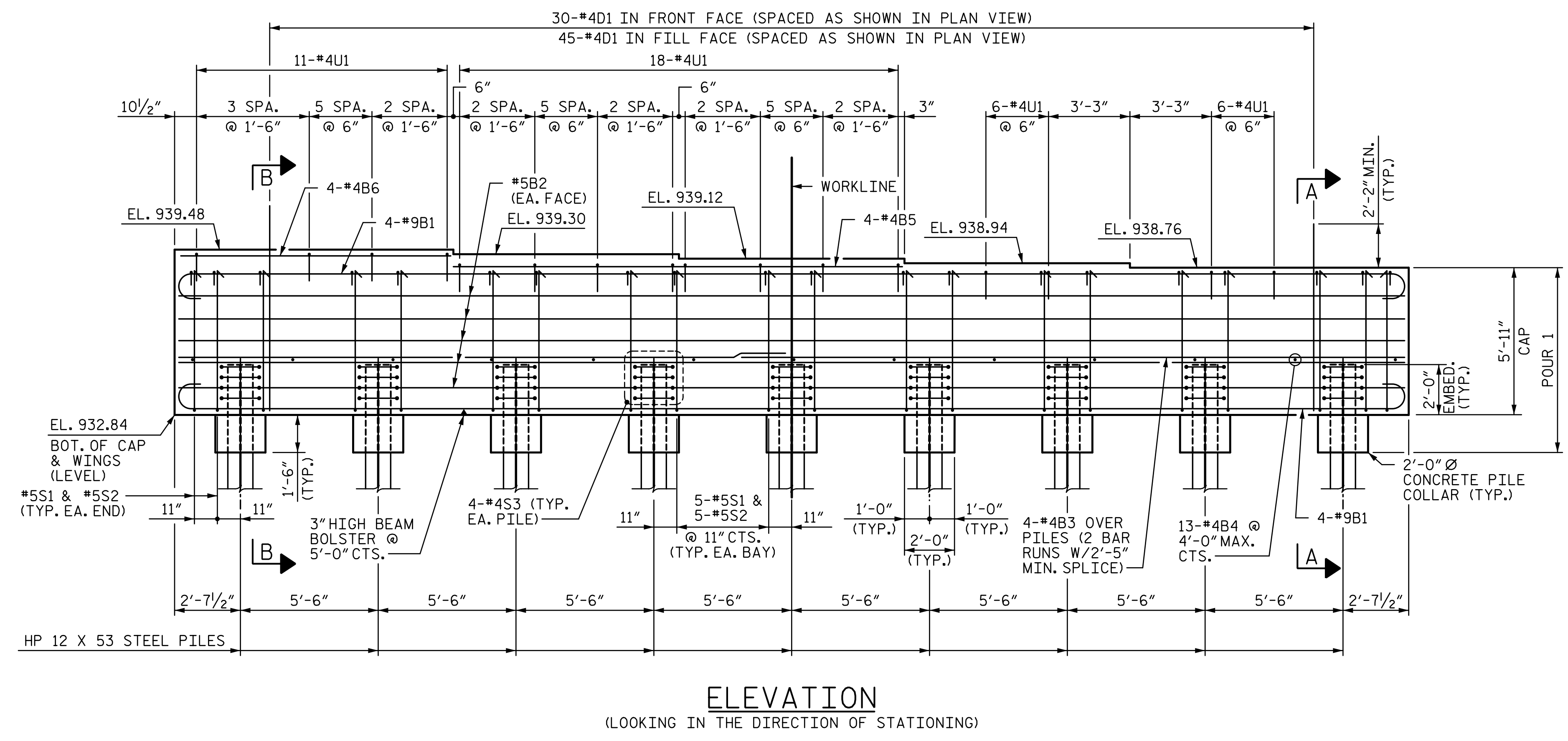
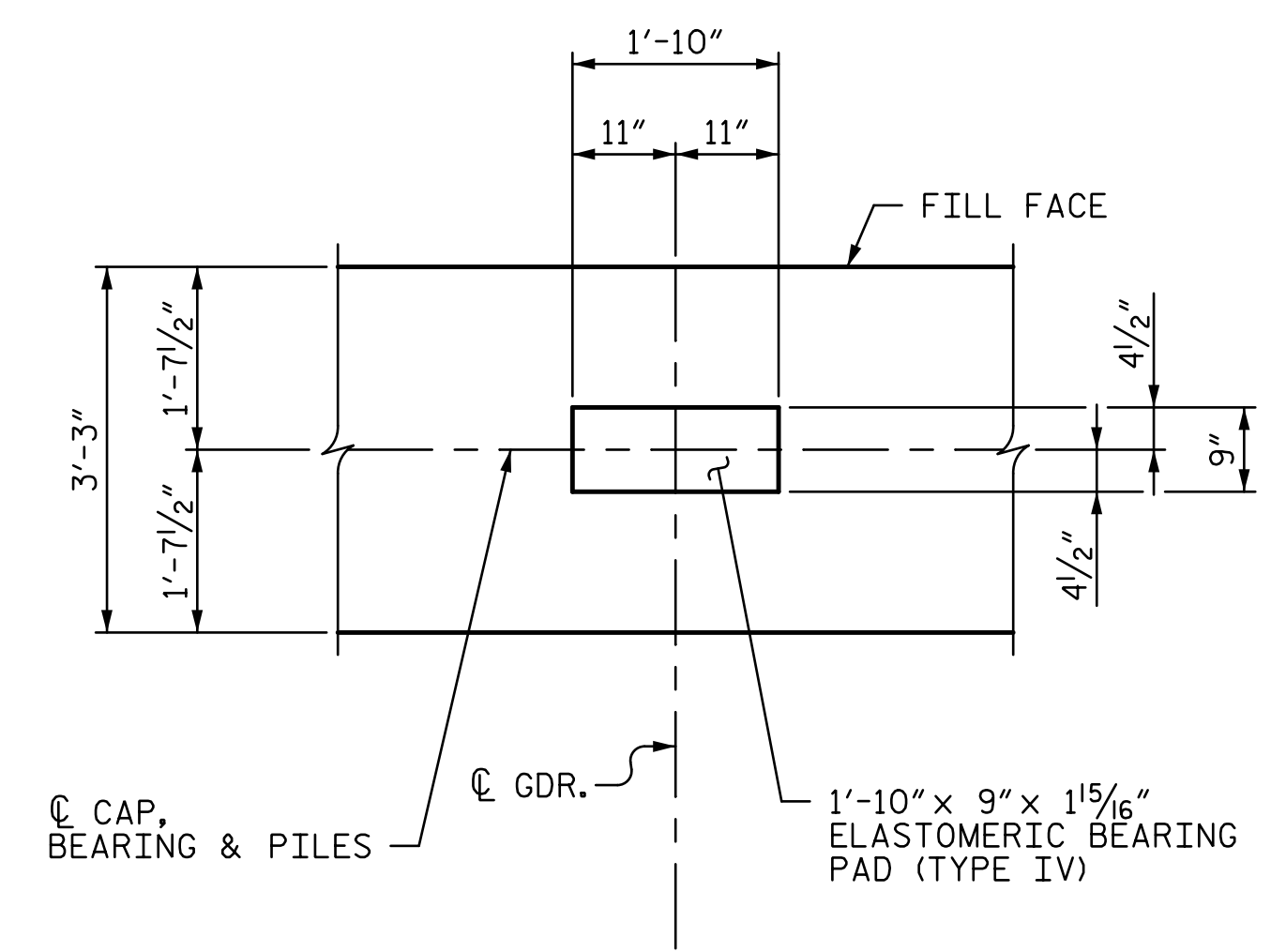
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**NOTES:**  
 FOR NOTES, SEE SHEET 3 OF 3.  
 FOR SECTIONS A-A AND B-B, SEE SHEET 3 OF 3.



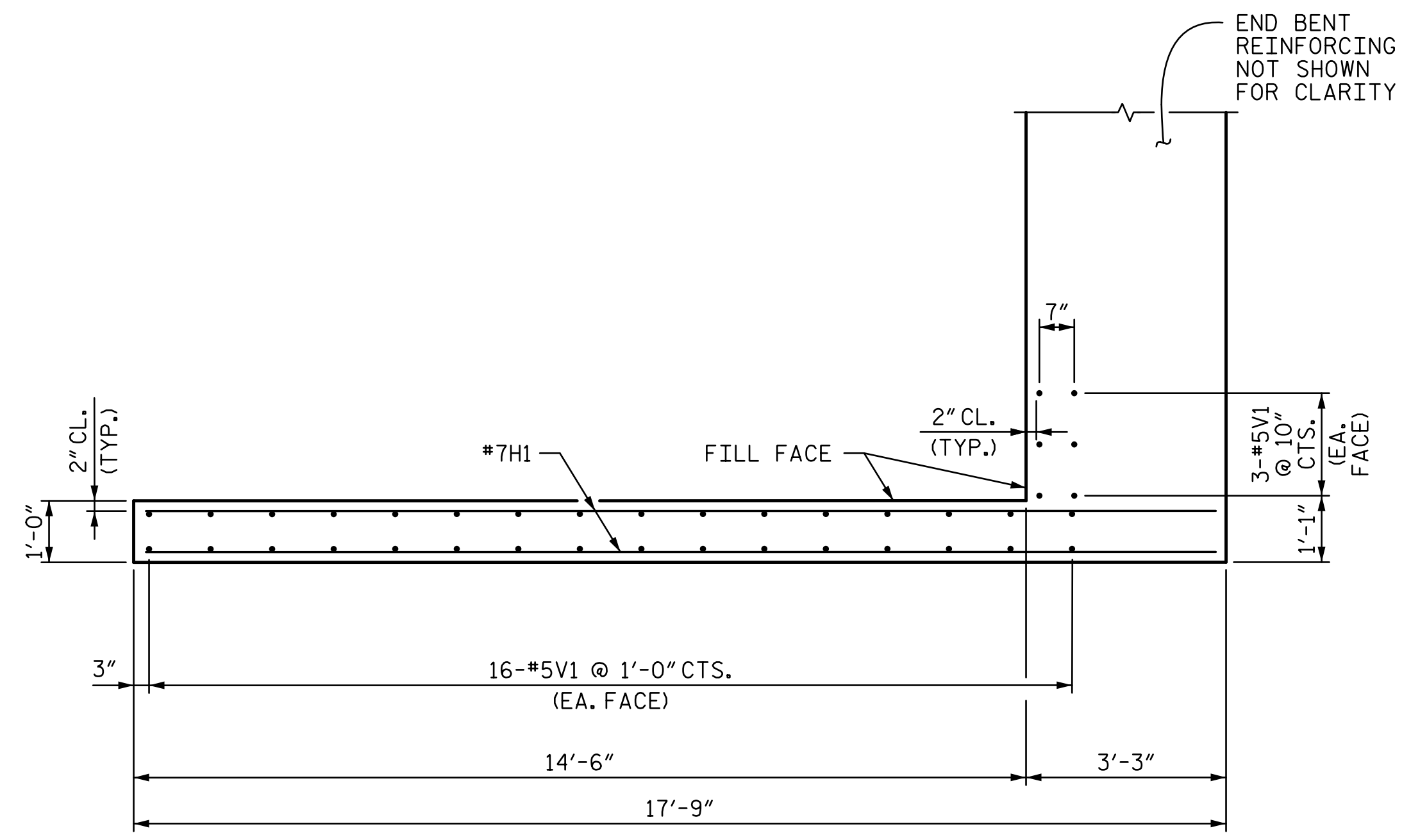
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CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-  
 SHEET 1 OF 3

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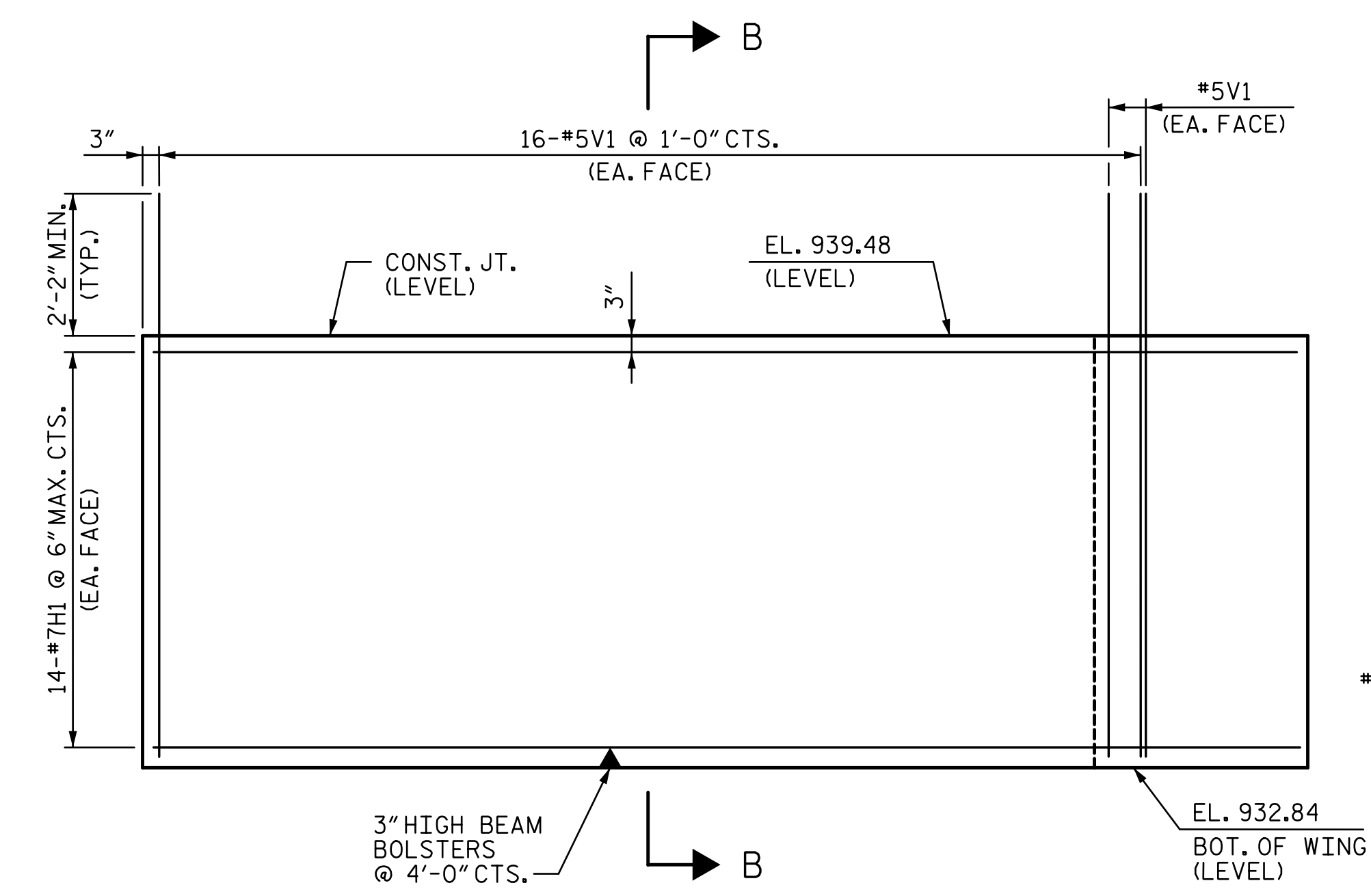
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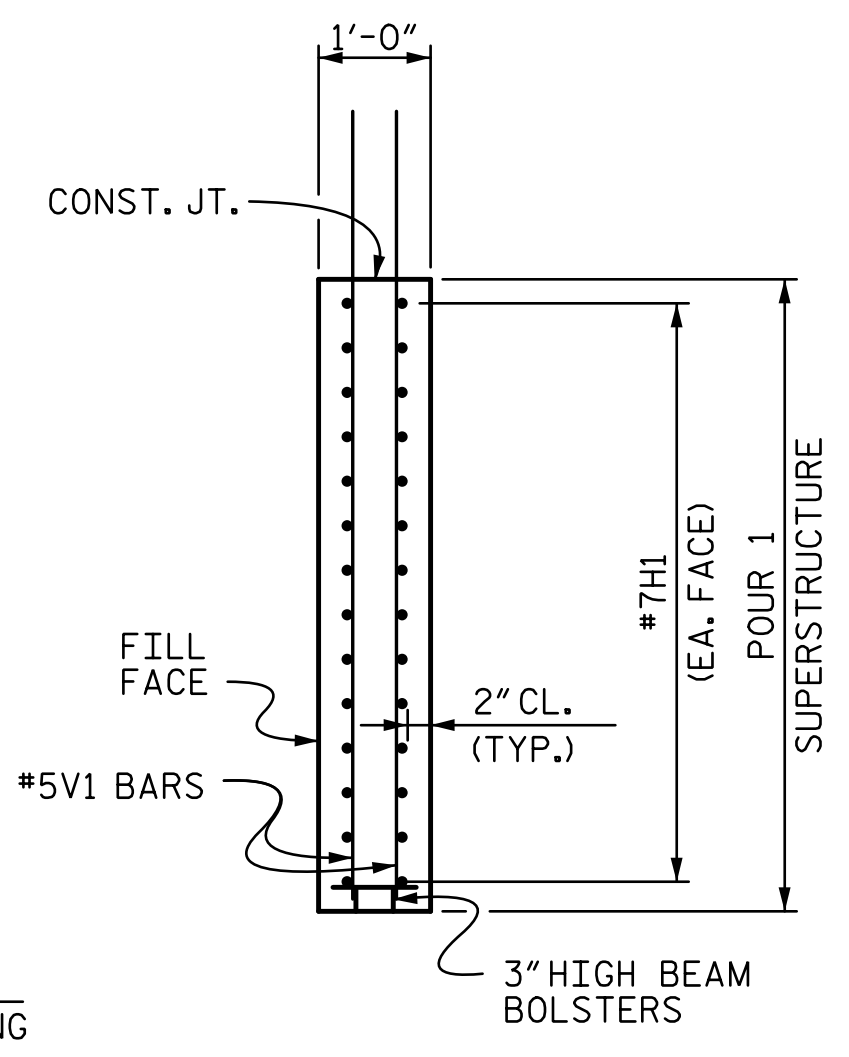




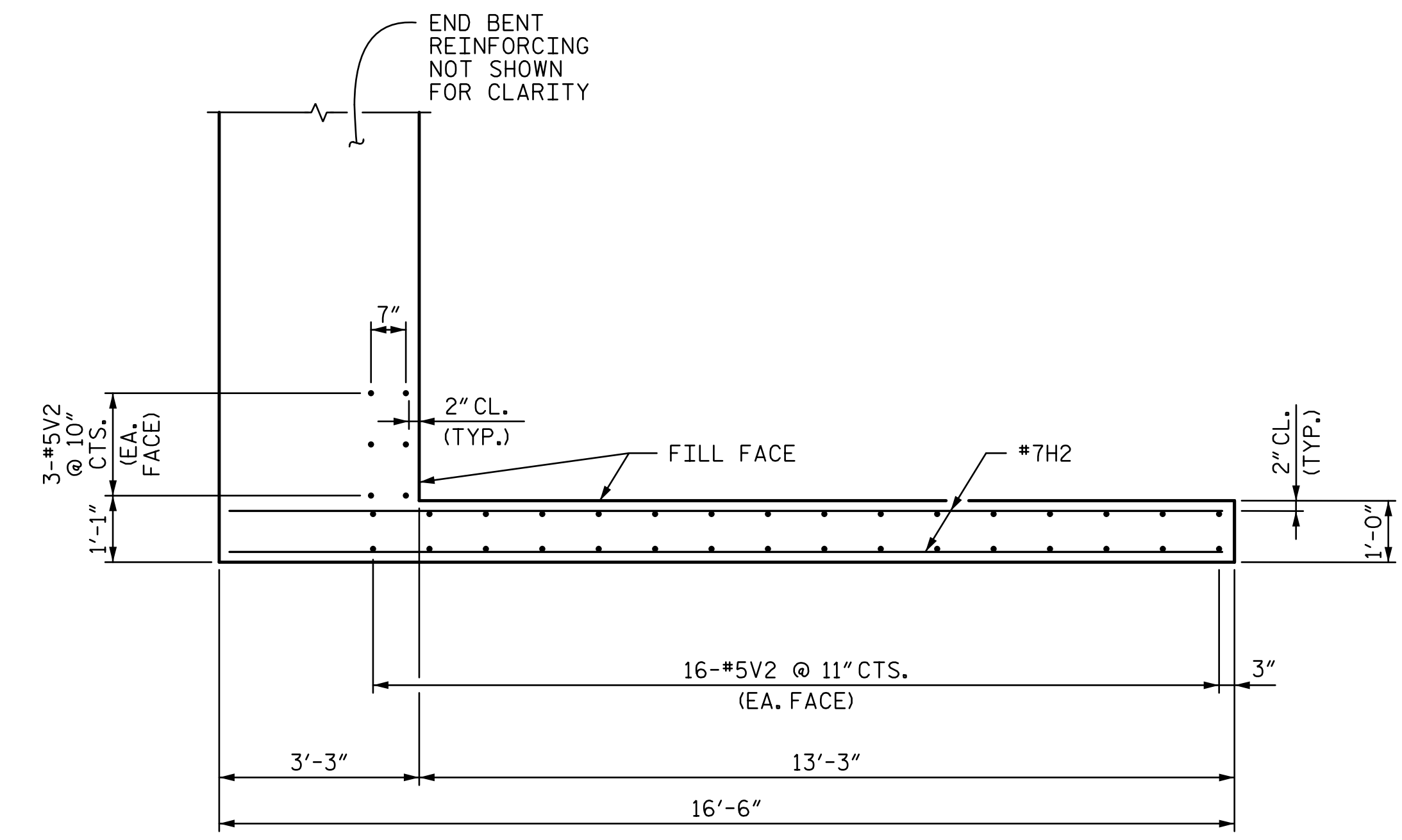
WING WALL PLAN (W3)



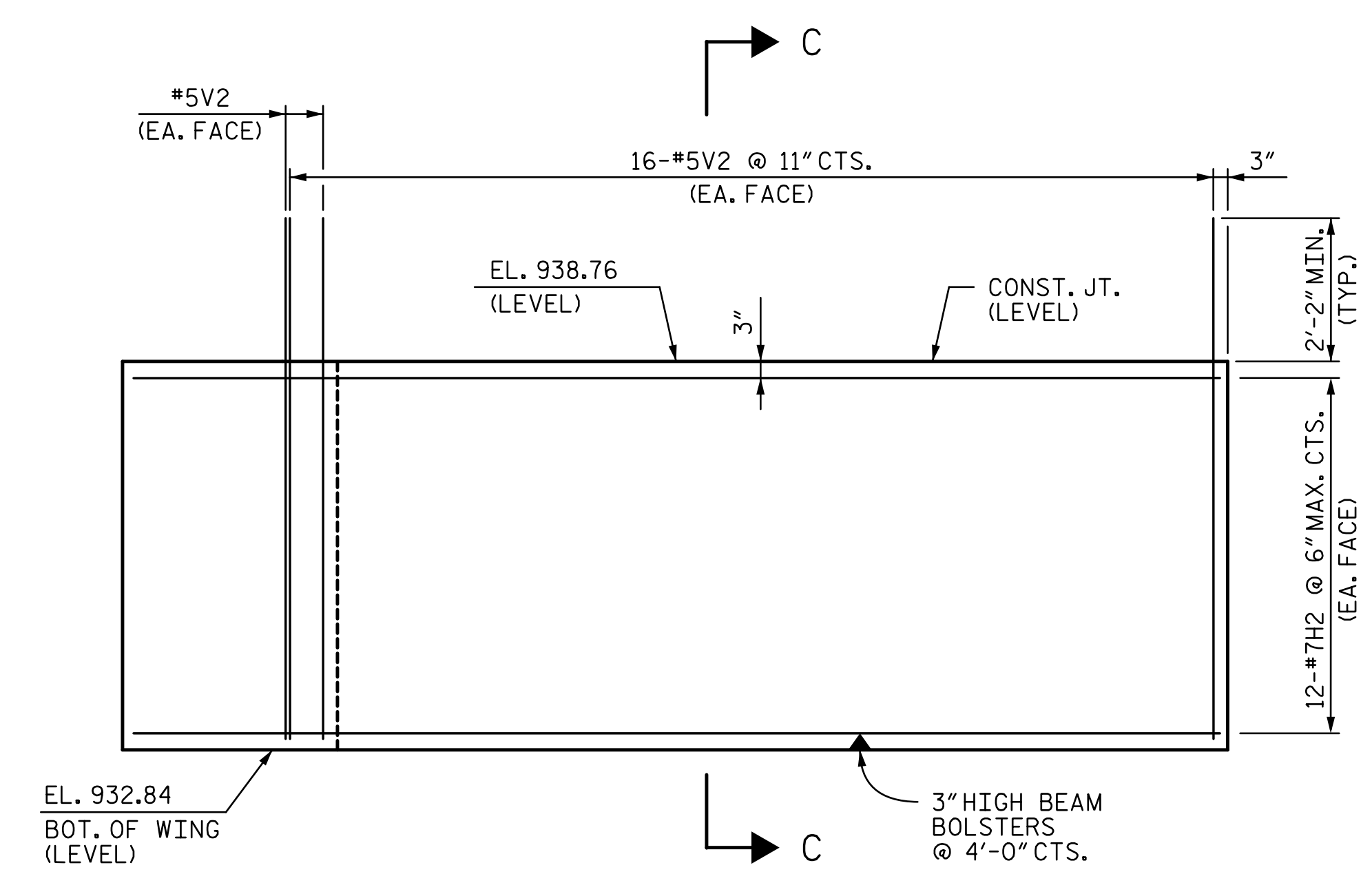
WING WALL ELEVATION (W3)



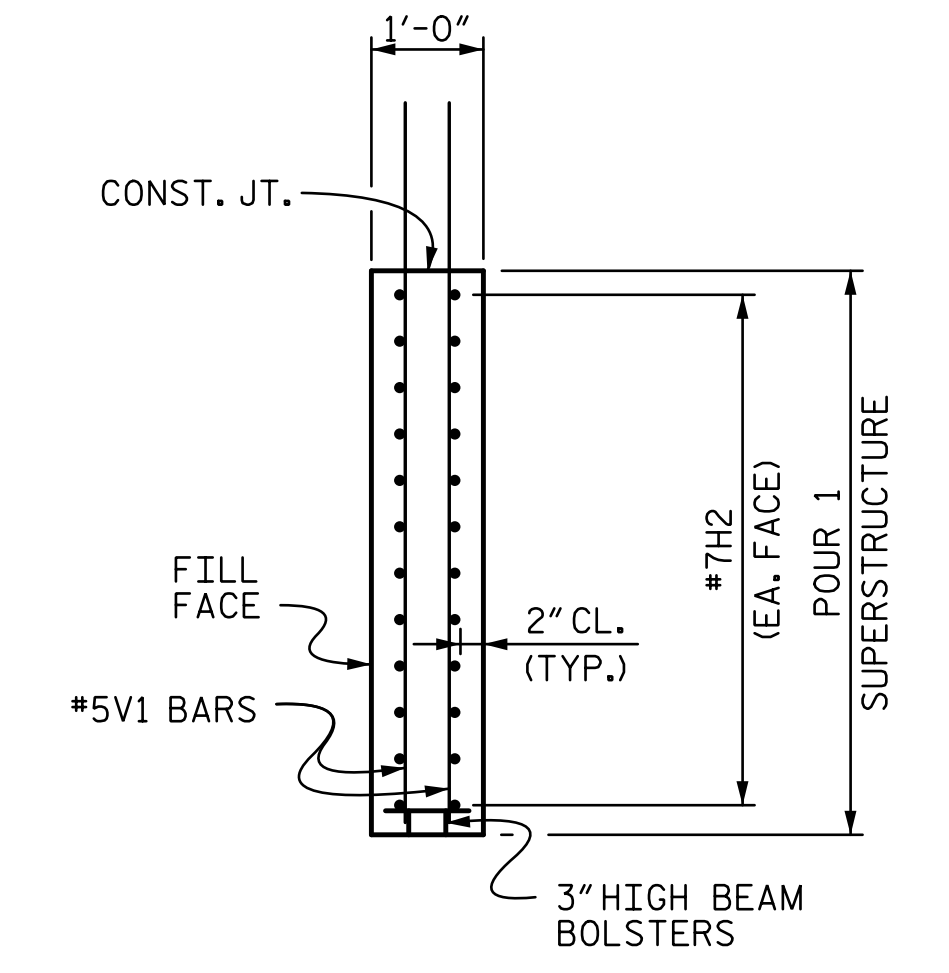
SECTION B-B



WING WALL PLAN (W4)



WING WALL ELEVATION (W4)



SECTION C-C

PROJECT NO. R-2707C  
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 SHEET 2 OF 3

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DRAWN BY : <u>MBC</u>	DATE : <u>9-16</u>	DESIGN ENGINEER OF RECORD: <u>T. LAWS</u>	DATE : <u>9-16</u>
CHECKED BY : <u>AJP</u>	DATE : <u>9-16</u>		

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DocuSigned by:  
Tony R. Laws, Jr.  
3/6/2017

STATE OF NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
RALEIGH

SUBSTRUCTURE

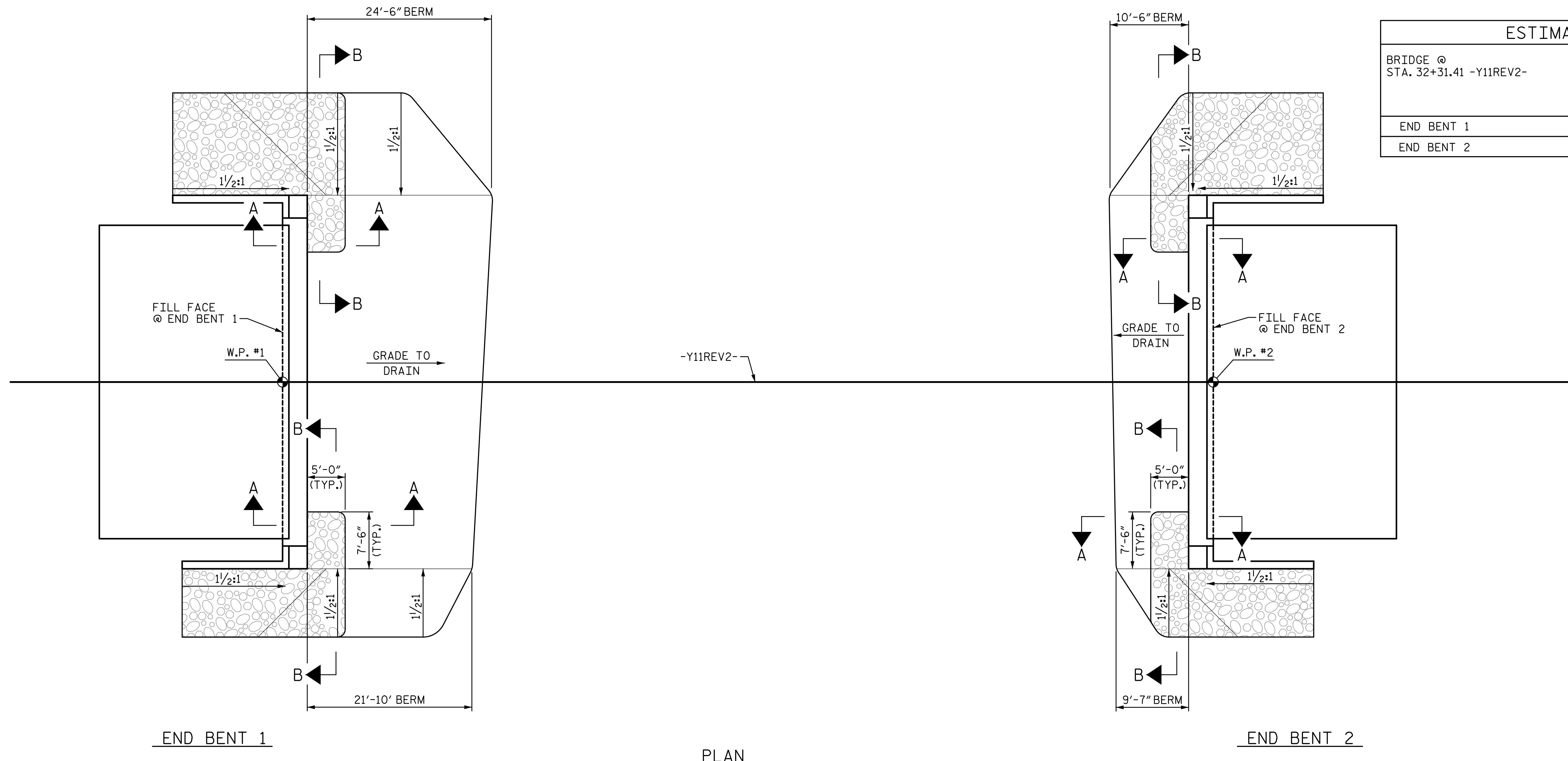
**END BENT 2  
(INTEGRAL)**

REVISIONS				SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS	27
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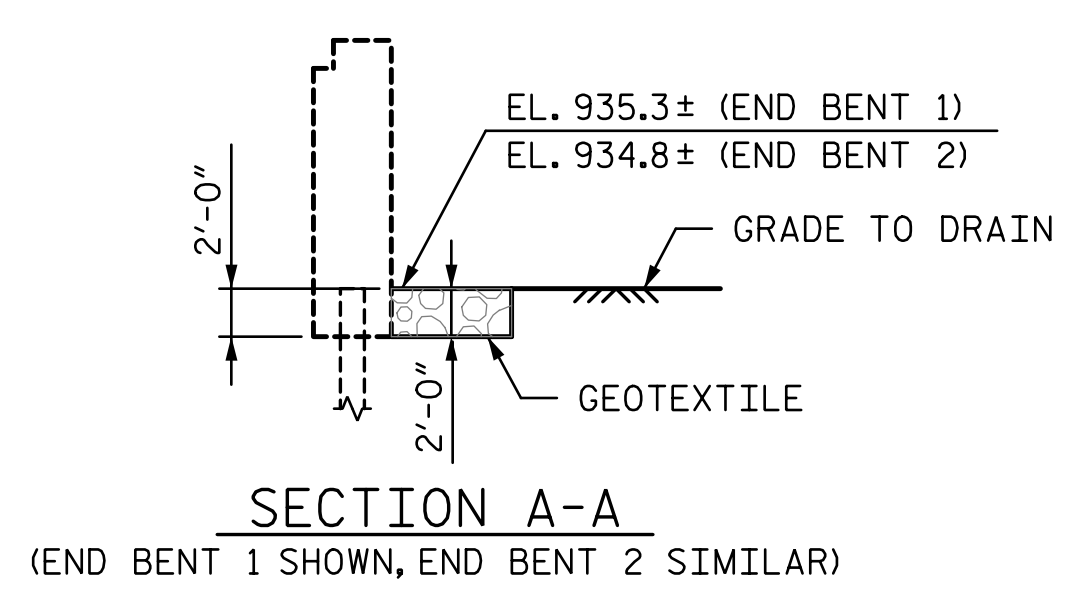


ESTIMATED QUANTITIES		
BRIDGE @ STA. 32+31.41 -Y11REV2-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE
	TONS	SQUARE YARDS
END BENT 1	68	75
END BENT 2	68	75

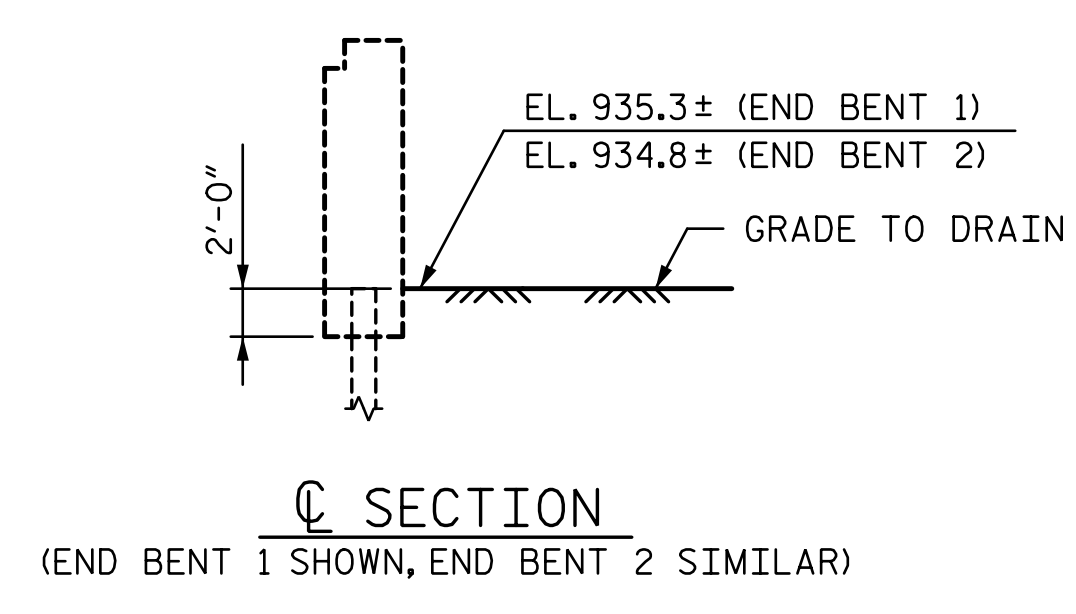
END BENT 1

PLAN

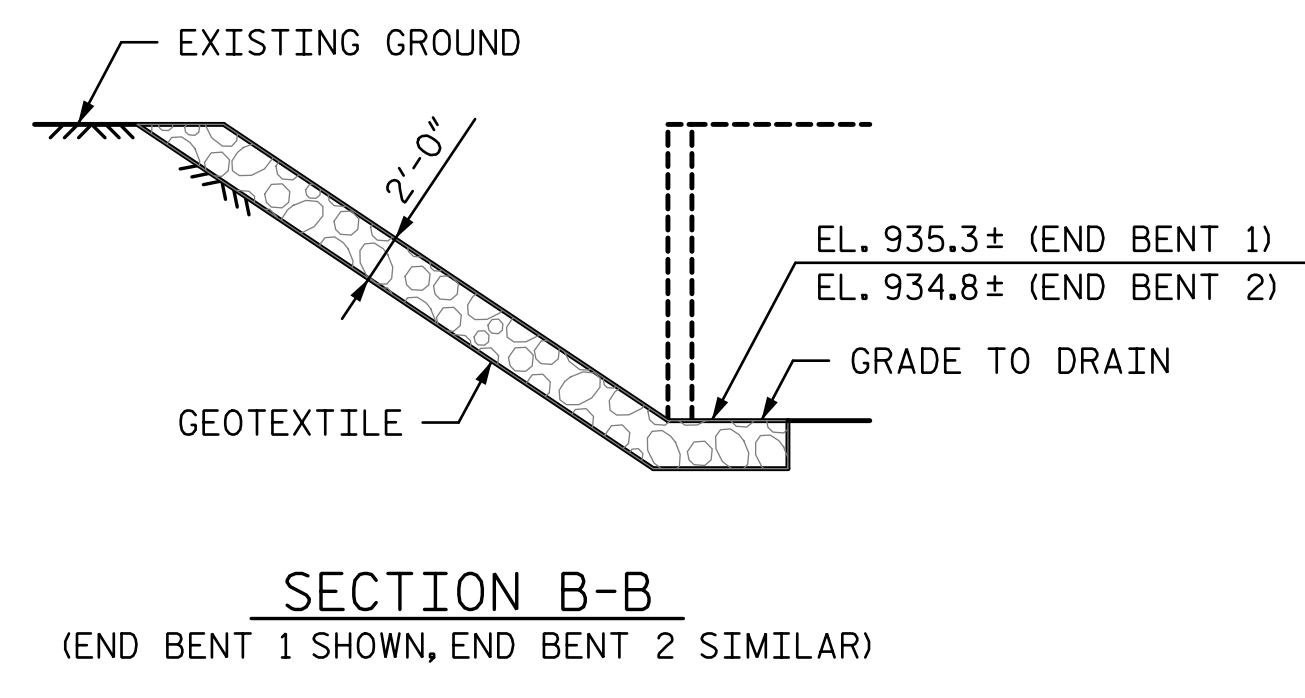
END BENT 2



SECTION A-A  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)



SECTION C-C  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)



SECTION B-B  
(END BENT 1 SHOWN, END BENT 2 SIMILAR)

PROJECT NO. R-2707C  
CLEVELAND COUNTY  
 STATION: 32+31.41 -Y11REV2-

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**Tony R. Laws, Jr.**  
3/6/2017

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DEPARTMENT OF TRANSPORTATION  
RALEIGH

**RIP RAP DETAILS  
AND GRADING PLAN**

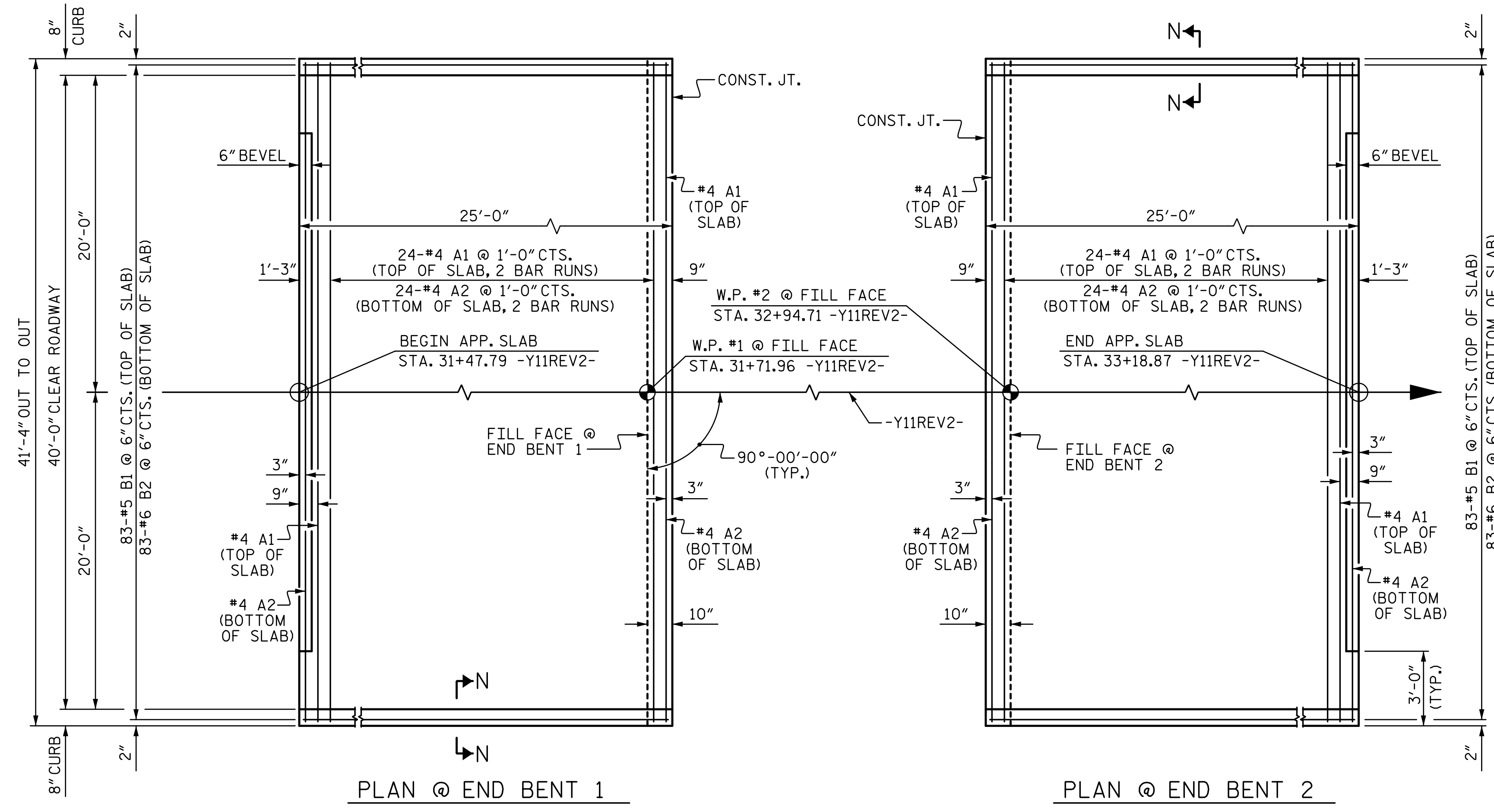
SHEET NO.  
S11-26

TOTAL SHEETS  
27

REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		

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PLAN @ END BENT 1  
 PLAN @ END BENT 2  
 DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS

### NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL FABRIC WALL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, WELDED WIRE FORM, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

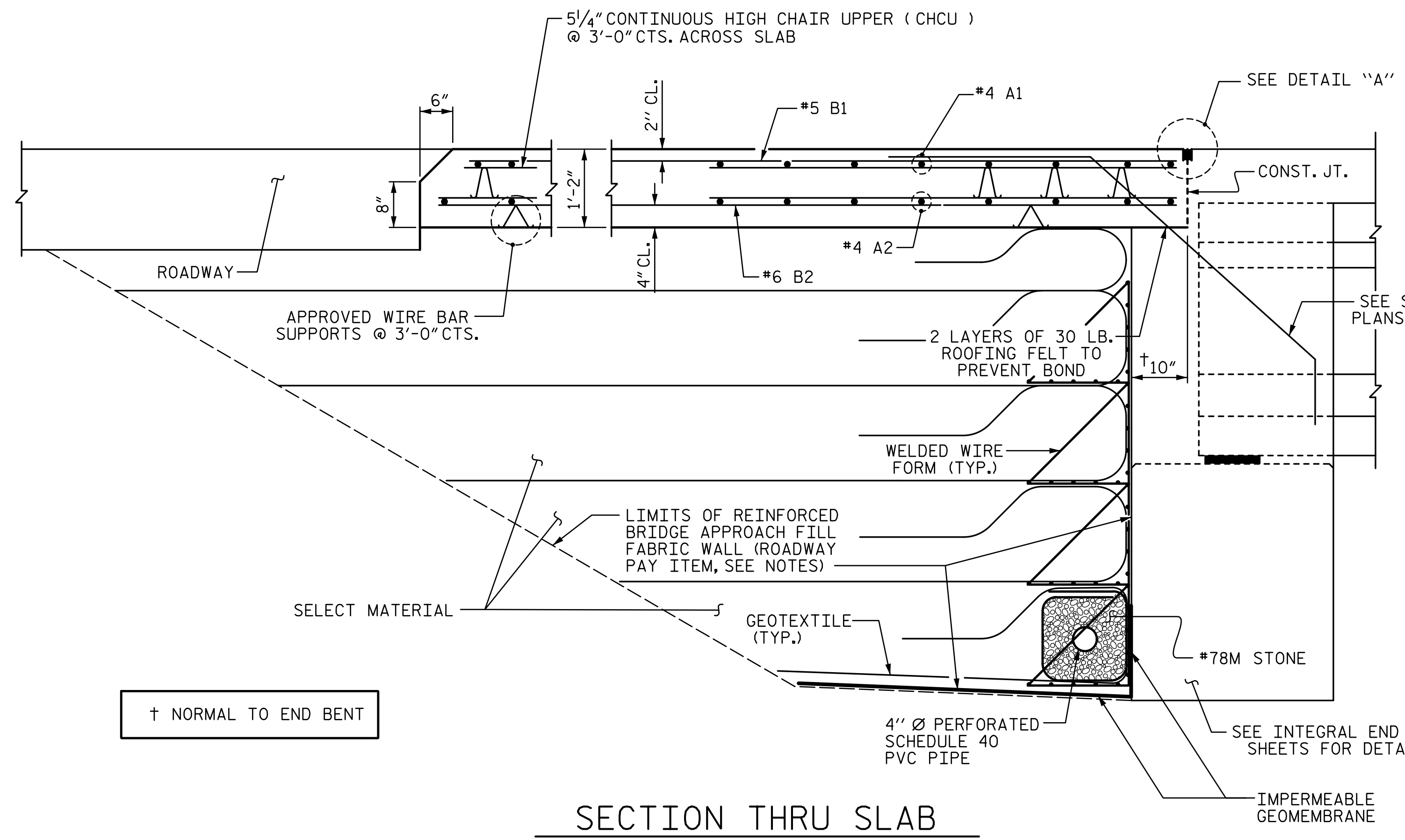
### BILL OF MATERIAL

FOR ONE APPROACH SLAB (2 REQ'D)

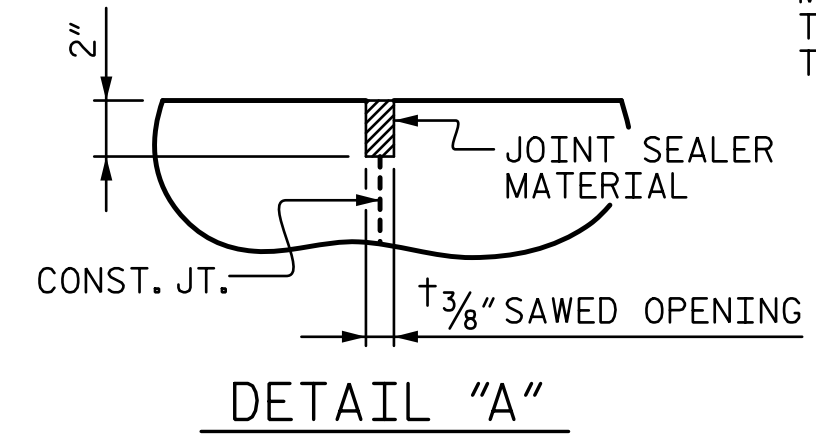
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
* A1	52	#4	STR	21'-6"	747
A2	52	#4	STR	21'-5"	744
* B1	83	#5	STR	24'-1"	2,085
B2	83	#6	STR	24'-7"	3,065
REINFORCING STEEL				LBS.	3,809
* EPOXY COATED REINFORCING STEEL				LBS.	2,832
CLASS AA CONCRETE				C. Y.	44.7

### SPLICE CHART

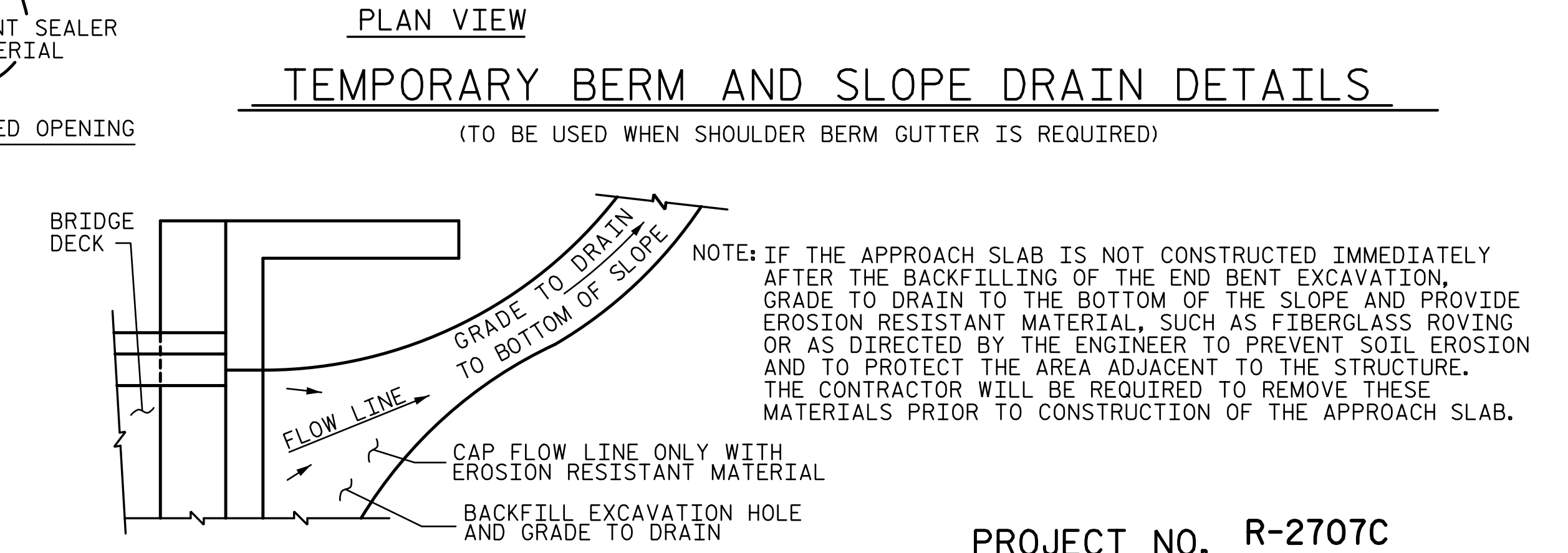
BAR	SIZE	SPLICE
* A1	#4	2'-0"
A2	#4	1'-9"



SECTION THRU SLAB



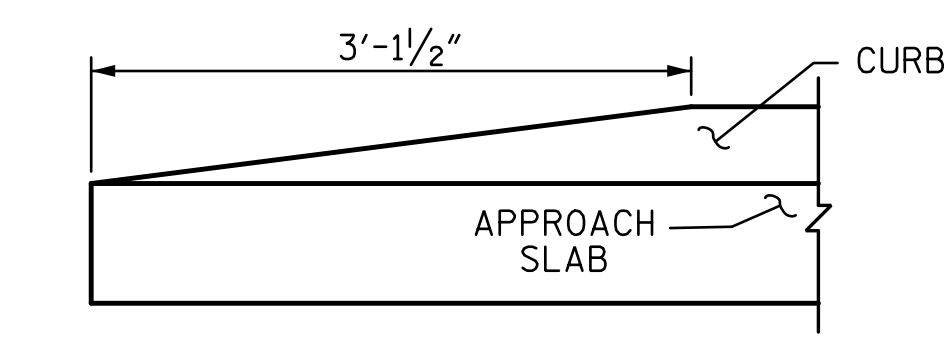
DETAIL "A"



TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER

TEMPORARY DRAINAGE DETAIL

NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

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PROJECT NO. **R-2707C**  
 CLEVELAND COUNTY  
 STATION: **32+31.41 -Y11REV2-**

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

**BRIDGE APPROACH SLAB FOR INTEGRAL ABUTMENT**

SEAL 40317  
 ENGINEER  
 TONY R. LAWS, JR.  
 3/6/2017

STV ENGINEERS, INC.  
 900 West Trade St., Suite 715  
 Charlotte, NC 28202  
 NC License Number F-5991

REVISIONS			
NO.	BY:	DATE:	DATE:
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SHEET NO. **S11-27**  
 TOTAL SHEETS **27**

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 3/6/2017



## STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS	-----	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	-----	SEE PLANS
IMPACT ALLOWANCE	-----	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF		
STRUCTURAL STEEL - AASHTO M270 GRADE 36	-	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W	-	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50	-	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION		
GRADE 60	--	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	-----	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	-----	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR		
UNTREATED - EXTREME FIBER STRESS	-----	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	-----	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	-----	30 LBS. PER CU. FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.  
ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.  
METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINIS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

# ENGLISH

JANUARY, 1990