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PROJECT LENGTH		ms consultants, inc. 920 Main Campus Drive Suite 430 Raleigh, NC 27606 NC License Number : C-3239	PLANS PREPARED FOR: DIVISION OF HIGHWAYS 1000 Birch Ridge Dr.
IGTH ROADWAY TIP PROJECT B-5398 =	0.108 mi.	2012 STANDARD SPECIFICATIONS	Kaleigh, NC 27610
GTH STRUCTURE TIP PROJECT B-5398 =	0.036 mi.	LETTING DATE:	
TOTAL LENGTH TIP PROJECT B-5398 =	0.144 mi.	DECEMBER 20, 2016	

STATE	STATE	PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS		
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STAT	E PROJ. NO.	F. A. PROJ. NO.		DESCRIPT	10N		
46	5113.1.1	BRZ–1803(1)		P.E.			
46	0113.2.1	BRZ-1803(1)	/W &	& UTIL.			
46	113.3.1	BRZ–1803(1)		CONS	ST.		





FOUNDATION NOTES

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FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS. CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING.FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 95 TONS PER PILE. DO NOT USE SLURRY CONSTRUCTION FOR DRILLED PIERS AT BENT NO.1 AND BENT NO.2. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 160 TONS PER PILE. THE SPREAD FOOTINGS AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 10 TSF. CHECK FIELD CONDITIONS FOR DRILLED PIERS, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 411 OF THE STANDARD FOR THE REQUIRED RESISTANCE OF 25 TSF JUST BEFORE PLACING CONCRETE. SPECIFICATIONS. DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 375 TONS PER PILE. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF. DRILLED PIERS AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 390 TONS PER PILE. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30 TSF. INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 931 FT AND WITH THE REQUIRED TIP RESISTANCE. INSTALL DRILL PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 948 FT AND WITH THE REQUIRED TIP RESISTANCE. THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 948 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE PROBLEMS DURING THE LIFE OF THE STRUCTURE. THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS ELEVATION 954 FT.SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE PROBLEMS DURING THE LIFE OF THE STRUCTURE SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICAION .. ms consultants 920 Main Campu

DRAWN BY :	J.M. KEPICH	DATE :	02/16
CHECKED BY :	L.M. SAMPLES	DATE :	04/16
DESIGN ENGINEEF	OF RECORD : L.M. SAMPLES	DATE :	05/16

BRACE PILES AT END BENT 1 ARE BATTERED 3:12



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s, inc. ıs Drive	DocuSigned by: J. M. J.		DEPA FOL FO OV BETV	ARTMENT	T] DG NR R	NORTH CAR F TRAI RALEIGH E ON E ON Y FO 1818	NSPORTA	TION OUT 803 /ER 1808
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	CONSTRUCTION MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION FOR END BENT	3'-O"DIA. DRILLED PIERS IN SOIL	3'-O"DIA. DRILLED PIERS NOT IN SOIL	SID INSPECTIONS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	HF STEE	P12X53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X 2'- PRESTRESS CONCRETE BOX BEAM	9″ ED ASBESTOS ASSESSMEN S
	LUMP SUM	LUMP SUM		LIN.FT.	LIN.FT.	EACH	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	LIN.FT. 380	TONS	SQ. YDS.	LUMP SUM	NO. LIN. F	T. LUMP SUM
END BENT NO.1 BENT NO.1				31	35			LUMP SUM	23 . 8		3342 12202	2368	5	150		203	226			
BENT NO.2					28.5				22.3		9214	1388								
END BENT NO.2			LUMP SUM						50.0		5714					172	192			
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	31	63.5	1	1	LUMP SUM	121.8	LUMP SUM	30472	3756	5	150	380	375	418	LUMP SUM	30 1900	LUMP SUM
(ROADWAY PAY D DETAIL) CLASS II RIP RAP (TYP.) (TYP.) (TYP.) (CLASS II (TYP.) (TYP.) (CLASS II (TYP.) (CLASS II (TYP.) (CLASS II (TYP.) (CLASS II (TYP.) (TYP.) (CLASS II (TYP.) (CLASS II (CLASS II (TYP.) (CLASS II (CLASS II (TYP.) (CLASS II (CLASS II (CLA	VEGE		CK DRAIN DIS (ROADWAY DE PAY ITEM PAY ITEM PROPOSI STRUCTURE EXISTING STRUCTURE RIVER FORK	SIPATOR PA TAIL AND) (TYP.) BRIDGE STA. 16+ 90°-00' ED 90°-00' ED 90°-00'	Period T+00 A5.00 -L- T+00 A5.00 -L- T+00 A5.00 T- T+00 ROPOSED GI ROPOSED GI R	S NOODS	T 18+0 200 PR CLASS RIP F				FOR FOR PRO FOR FOR FOR FOR FOR REN THE SAM REO INC REO INC REO INC SAM DIA STE ITE AT CAU MAY SPE REM	OTHER DESIGN SUBMITTAL OF VISIONS. FALSEWORK AN CRANE SAFETY GROUT FOR ST ASBESTOS ASS OVATION ACTIV CONTRACTOR S PLES OF REINF UIRING UP TO H SAMPLE OF E UIRING OVER 4 H SAMPLES OF CH THE SAMPLE THE CONTRACTOR EL SHALL BE C MS. THE CONTRACTOR SEWAY, THE CLA BE PLACEMENT PLE, PLUS A MI METERS. PAYME EL SHALL BE C MS. THE CONTRACTOR SEWAY, THE CLA SEWAY, THE CLA SEWAY, THE CLA SEWAY, THE CLA SEWAY, THE CLA SEWAY, THE CLA SEVAL OF TEMPO SMUCH AS THE UCTURAL STEEL ENTION IS DIF NDARD SPECIFI PLIANCE WITH ULATIONS PERT	I DATA WORK ID FOR ID FOR ID FOR ID FOR IN SEE RUCTL SESSME VITIES HALL ORCIN 400 T ACH S IOO TO EACH S ARE DARS IOO TO EACH S ARE NIMUN NT FO ONSID ONSID OR'S O SS II ONSID OR'S O SS II S RIP ONSID OR'S O SS II S RIP ONSID	A AND GE KING DRA MWORK, S SPECIAL JRES, SEE JRES, SEE ENT FOR S, SEE SP PROVIDE NG STEEL TONS OF SIZE BAR ONS OF R SIZE BAR OF THE SI ONS OF R SIZE BAR ONS OF R SIZE BAR OF THE SI ONS OF R SIZE BAR ONS OF R SIZE BAR OF THE SI ONS OF R SIZE BAR ONS OF R SIZE BAR OF THE SI OF THE SI ONS OF R SIZE BAR OF THE SI ONS OF R SIZE BAR OF THE SI OF THE SI ONS OF R SIZE BAR OF THE SI OF THE SI ONS OF R SIZE BAR ONS OF R SIZE BAR OF THE SI OF THE	NERAL NOTE: WINGS, SEE EE SPECIAL PROVISIONS SPECIAL PI BRIDGE DEM ECIAL PROV INDEPENDEN AS FOLLOWS REINFORCING R USED, AND F EINFORCING R USED, AND F EINFORCING R USED, AND F EINFORCING R USED. THE MUST THEN I SIZE AND I PLICE OF TH AMPLES OF F CIDENTAL TO ND UPON REI P USED IN PE PROTECT RUCTION, MA AT STATION I ON THE EX AD, THE CON ICLE 107-1 COST RESUL TATE OR FE NDLING OF	S, SEE SHEE SPECIAL PROVISION S. ROVISIONS. OLITION AN ISIONS. NT ASSURAN S: FOR PROJECT STEEL, TWO BARS FROM BE SPLICED LENGTH OF HIRTY BAR REINFORCINO D VARIOUS F MOVAL OF T HE CAUSEW ION. SEE INTENANCE INTENANCE ISTING TRACTOR'S OF THE TING FROM DERAL MATERIALS	T SN. ON T PRESINTE THE REDU IS. THE PLAN INFO THE DEPA COST BRID CE COND JECTS IE 30 REMO TO A COND JECTS IE 30 REMO TO A COND THE THIS 18-E G PAY FOR ASPH HE ROAD ACCO	IMBER PILES SENTLY POSTED EGRITY OF THE PROPOSED BRI JCED AS FOUND SUBSTRUCTURE NS IS FROM TH ORMATION IS S CONTRACTOR S ARTMENT OF TH INCURRED BA OGE SUBSTRUCT OTTIONS AT TH OVAL OF THE E ALLOW DEBRIS L REMOVE THE ORDANCE WITH S STRUCTURE H VALUATING SCA EROSION CONT HALT WEARING OWAY PLANS.	SHALL BE RE FOR LOAD L BRIDGE DET DGE, A LOAD NECESSARY OF THE EXI IS BEST INFO SHOWN FOR T HALL HAVE N ANSPORTATI SED ON DIFI URE SHOWN O IE PROJECT S XISTING BR TO FALL IN BRIDGE AND ARTICLE 402 AS BEEN DES OUR AT BRID SURFACE IS PROJ	MOVED. THE EX IMIT. SHOULD ERIORATE DUR LIMIT MAY BI DURING THE L STING BRIDGE ORMATION AVA HE CONVENIEND ON FOR ANY DI FERENCES BETW ON THE PLANS SITE. IDGE SHALL BE TO THE WATER. SUBMIT PLAN 2-2 OF THE ST SIGNED IN ACCORS. ES, SEE EROSIC INCLUDED IN BURKE SUBMIT 16
ESIGN DISCHARGE REQUENCY OF DESIG SIGN HIGH WATER RAINAGE AREA ASE DISCHARGE (Q1) ASE HIGH WATER EL	AULIC DA GN DISCHARGE ELEVATION	A I A = 8,300 C.F = 25 YRS. = 962.70 = 74.5 SQ.I = 11,000 C. = 964.70	F.S. MI. .F.S.	OVERTOF FREQUEN OVERTOF	<u>PING DISCU</u> CY OF OVEF PING FLOOD	PING FL HARGE RTOPPING FLOO DELEVATION	<u></u>	DATA ,000 C.F.S. 0+ YRS. 7.8 @ STA.18+89	9.1 -L-		BID STA THE SHEI EAC ENG LUM EXC SPE	PRICE FOR "R TION 16+45.00 MATERIAL SHO ET S-01 SHALL H SIDE OF CEN INEER. THIS W P SUM PRICE F AVATION. SEE CIFICATIONS.	EMOVA -L-″ OWN IN BE EX TERLI ORK W FOR UN SECTI	N THE CR CAVATED NE ROADV TILL BE F NCLASSIF ON 412 O	OSS-HATCHEI FOR A DIS NAY AS DIRI AID FOR AT IED STRUCT F THE STAN	D AREA ON TANCE OF 4 ECTED BY TI THE CONTR URE DARD nsultants, i	3 FT HE RACT inc.	DocuSigned Lin M. J. 5663D099A9 10/14/ PFESS/01/14/ SEAL 037031	SHEET C	STATE OF PARTMENT OF GENERA OR BRID(OVER HENF TWEEN SR

DRAWN BY :	J.M. KEPICH	DATE :	02/16
CHECKED BY :	L.M. SAMPLES	DATE :	04/16
DESIGN ENGINE	ER OF RECORD : L.M. SAMPLES	DATE	05/16





Raleigh, NC 2760 NC License Numb

TH THE Eet Sn.	THE EXISTING 5 SPAN S LENGTHS OF 40-2" AND T WITH REINFORCED CONCE ON 4 LINES OF STEEL B CLEAR ROADWAY ON VAR REINFORCED CONCRETE F OR TIMBER PILES AND ON ON TIMBER PILES SHALL PRESENTLY POSTED FOR INTEGRITY OF THE BRID THE PROPOSED BRIDGE, A REDUCED AS FOUND NECE	STRUCTURE HREE INTE RETE DECK EAMS SPAC IOUS SUBS POST AND I REINFORCE LOAD LIM DGE DETER A LOAD LIM	CONSIST RIOR SPA AND ASPE CED AT 7' TRUCTURE BEAM BEN D CONCRE VED. THE E IT. SHOULD IT. SHOULD IORATE DU MIT MAY RING THF	ING OF TA AN LENGTH HALT OVEF -O"CTS.W S INCLUD TS WITH TE END BE EXISTING D THE STF JRING COM BE POSTE I TFF OF	WO END SF IS OF 40'- RLAY SUPP ITH A 22' ING DRILLED F ENTS SUPF BRIDGE I RUCTURAL NSTRUCTIC D AND MA THF PROJI	PAN O" ORTED -O" PIERS PORTED S ON OF Y BE FCT.
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LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)
		HL-93(Inv)	N/A	$\langle 1 \rangle$	1.109		1.75	0.272	1.47	В	EL	44.25	0.493	1.26	В	EL	4.425	0.80	0.272	1.11	В	EL	44.25
DESTGN		HL-93(0pr)	N/A		1.633		1.35	0.272	1.9	В	EL	44.25	0.493	1.63	В	EL	4.425	N/A					
LOAD		HS-20(Inv)	36.000	2	1.507	54.255	1.75	0.272	1.99	В	EL	44.25	0.493	1.65	В	EL	4.425	0.80	0.272	1.51	В	EL	44.25
RAIING		HS-20(0pr)	36.000		2.14	77.039	1.35	0.272	2.59	В	EL	44.25	0.493	2.14	В	EL	4.425	N/A					
		SNSH	13.500		3.519	47.501	1.4	0.272	5.82	В	EL	44.25	0.493	5.05	В	EL	4.425	0.80	0.272	3.52	В	EL	44.25
		SNGARBS2	20.000		2.572	51.43	1.4	0.272	4.25	В	EL	44.25	0.493	3.55	В	EL	4.425	0.80	0.272	2.57	В	EL	44.25
		SNAGRIS2	22.000		2.415	53.122	1.4	0.272	4	В	EL	44.25	0.493	3.27	В	EL	4.425	0.80	0.272	2.41	В	EL	44.25
		SNCOTTS3	27.250		1.749	47.674	1.4	0.272	2.89	В	EL	44.25	0.493	2.52	В	EL	4.425	0.80	0.272	1.75	В	EL	44.25
	SV	SNAGGRS4	34.925		1.443	50.381	1.4	0.272	2.39	В	EL	44.25	0.493	2.06	В	EL	4.425	0.80	0.272	1.44	В	EL	44.25
		SNS5A	35.550		1.412	50.195	1.4	0.272	2.34	В	EL	44.25	0.493	2.07	В	EL	4.425	0.80	0.272	1.41	В	EL	44.25
		SNS6A	39.950		1.287	51.435	1.4	0.272	2.13	В	EL	44.25	0.493	1.88	В	EL	4.425	0.80	0.272	1.29	В	EL	44.25
LEGAL		SNS7B	42.000		1.226	51.483	1.4	0.272	2.03	В	EL	44.25	0.493	1.83	В	EL	4.425	0.80	0.272	1.23	В	EL	44.25
		TNAGRIT3	33.000		1.568	51.733	1.4	0.272	2.59	В	EL	44.25	0.493	2.24	В	EL	4.425	0.80	0.272	1.57	В	EL	44.25
RATING		TNT4A	33.075		1.572	52.007	1.4	0.272	2.6	В	EL	44.25	0.493	2.2	В	EL	4.425	0.80	0.272	1.57	В	EL	44.25
		TNT6A	41.600		1.278	53.17	1.4	0.272	2.11	В	EL	44.25	0.493	1.92	В	EL	4.425	0.80	0.272	1.28	В	EL	44.25
	ST	TNT7A	42.000		1.281	53.782	1.4	0.272	2.12	В	EL	44.25	0.493	1.89	В	EL	4.425	0.80	0.272	1.28	В	EL	44.25
		TNT7B	42.000		1.315	55.229	1.4	0.272	2.18	В	EL	44.25	0.493	1.79	В	EL	4.425	0.80	0.272	1.31	В	EL	44.25
		TNAGRIT4	43.000		1.258	54.101	1.4	0.272	2.08	В	EL	44.25	0.493	1.74	В	EL	4.425	0.80	0.272	1.26	В	EL	44.25
		TNAGT5A	45.000		1.19	53.537	1.4	0.272	1.97	В	EL	44.25	0.493	1.71	В	EL	4.425	0.80	0.272	1.19	В	EL	44.25
		TNAGT5B	45.000	$\overline{\langle 3 \rangle}$	1.178	53.027	1.4	0.272	1.95	В	EL	44.25	0.493	1.66	В	EL	4.425	0.80	0.272	1.18	В	EL	44.25



SPAN A

ASSEMBLED BY : J.M. KEPICH D CHECKED BY : L.M. SAMPLES D	DATE: 02/16 DATE: 04/16	
CHECKED BY : AAC II/II		DESIGN ENGINEER OF RECORD : L.M. SAMPLES DATE : 05/16

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ms consultants, inc. 920 Main Campus Drive Suite 430



LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

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NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

<pre>(#) CONTROLLING LOAD RATING</pre>
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER



MBER ź COMMENT



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THREADED INSERT DETAIL

ms consultants 920 Main Campu Suite 430 Raleigh, NC 2760 NC License Num

NOTES

ALL PRESTRESS STRANDS AND REQUIREMENTS SPECIFICATIO	SING STRANDS SHALL SHALL CONFORM TO A WHICH SHALL BE IN NS.	BE 7-WIRE LOW RELAXATION GRADE 270 AASHTO M203 EXCEPT FOR SAMPLING ACCORDANCE WITH THE STANDARD
ALL REINFORC 60 AND SHALL CONCRETE BOX	ING STEEL CAST WITH BE INCLUDED IN THE BEAMS.	THE BOX BEAM SECTIONS SHALL BE GRADE NE UNIT PRICE BID FOR PRESTRESSED
FLAME CUTTIN Allowed.	G OF THE TRANSVERSI	SE POST-TENSIONING STRAND IS NOT
RECESSES FOR OF THE STRAN	TRANSVERSE STRANDS	S SHALL BE GROUTED AFTER THE TENSIONING
THE 2 ¹ /2"ØDOW FILLED WITH M ENDS OF BOX M TO 1 ¹ /2"ABOVE	WEL HOLES AT FIXED NON-SHRINK GROUT.TH BEAM SECTIONS SHAL THE TOP OF DOWELS	ENDS OF BOX BEAM SECTIONS SHALL BE THE 2 ¹ /2″Ø DOWEL HOLES AT EXPANSION LL BE FILLED WITH JOINT SEALER MATERIAL AND THEN FILLED WITH GROUT.
THE JOINT SEA SL LOW MODUL TO THE REQUI STANDARD SPE	ALER MATERIAL SHALL US SILICONE SEALAN REMENTS OF TYPE M CIFICATIONS.	L CONFORM TO THE REQUIREMENTS OF TYPE NT.THE 2″Ø BACKER RODS SHALL CONFORM BOND BREAKER.SEE SECTION 1028 OF THE
THE TRANSFER BE DONE WHEN LESS THAN 4,0	OF LOAD FROM THE THE CONCRETE HAS F OOO PSI FOR SPANS A	ANCHORAGES TO THE BOX BEAM UNIT SHALL REACHED A COMPRESSIVE STRENGTH OF NOT A AND C AND 6,000 PSI FOR SPAN B.
ALL REINFORCE EPOXY COATED	ING STEEL IN VERTIO	CAL CONCRETE BARRIER RAILS SHALL BE
PRESTRESSING	STRANDS SHALL BE (CUT FLUSH WITH THE BOX BEAM UNIT ENDS.
APPLY EPOXY	PROTECTIVE COATING	G TO BOX BEAM UNIT ENDS.
VERTICAL GROUND IN ALL EXPOSE WITH ARTICLE CONTRACTION VERTICAL BAR IS REQUIRED 20 FEET IN LE SEGMENTS LESS	OVED CONTRACTION J ED FACES OF THE VEF 825-10(B) OF THE ST JOINT SHALL BE LOCA RIER RAIL EXPANSIO AT MIDPOINT OF VER ENGTH AND NO CONTR S THAN 10 FEET IN L	JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED RTICAL BARRIER RAIL AND IN ACCORDANCE ANDARD SPECIFICATIONS. A VERTICAL CATED AT EACH THIRD POINT BETWEEN ON JOINTS. ONLY ONE CONTRACTION JOINT RTICAL BARRIER RAIL SEGMENTS LESS THAN RACTION JOINTS ARE REQUIRED FOR THOSE LENGTH.
THE LOCATION NECESSARY TO STEEL.	OF THE VOID DRAIN CLEAR PRESTRESSIN	NS MAY BE SHIFTED SLIGHTLY WHERE NG STRANDS OR TRANSVERSE REINFORCING
FOR GROUT FO	R STRUCTURES, SEE S	SPECIAL PROVISIONS.
THE PERMITTE CONTRACTOR T	D THREADED INSERTS O ATTACH FALSEWORK	5 ARE DETAILED AS AN OPTION FOR THE K AND FORMWORK DURING CONSTRUCTION.
THE PERMITTE BY THE CONTR WITH SECTION THREADED INS	D THREADED INSERTS ACTOR, SPACED AT 4'- 1076 OF THE STANDA ERTS MAY BE USED A	S IN THE EXTERIOR UNITS SHALL BE SIZED -O"CENTERS AND GALVANIZED IN ACCORDANCE ARD SPECIFICATIONS.STAINLESS STEEL AS AN ALTERNATE.
THE PERMITTE	D THREADED INSERTS FOLLOWING REMOVAL	S SHALL BE GROUTED BY THE CONTRACTOR OF THE FALSEWORK.
THE COST OF PRICE BID FO	THE PERMITTED THREAR R THE PRECAST UNIT	ADED INSERTS SHALL BE INCLUDED IN THE
APPLY EPOXY BEAM UNITS T	PROTECTIVE COATING HAT REQUIRE DRAINS	G TO EXTERIOR FACE OF THE EXTERIOR BOX S IN THE VERTICAL BARRIER RAIL.
THE DRAIN OP	ENING AT THE GUTTE	ERLINE SHALL BE 4"X5". THE HEIGHT OF THE
TOP OF THE BO	OX BEAM UNIT TO TH	HE TOP OF THE DRAIN OPENING.
		PROJECT NO. <u>B-5398</u>
		BURKE COUNTY
		STATION: 16+45.00 -L-
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		STD.NO.PCBB1





DRAWN BY .	J.M. KEPICH	DATE .	02/16
CHECKED BY :	L.M. SAMPLES	DATE :	04/16
DESIGN ENGINE	R OF RECORD : L.M. SAMPLES	DATE :	05/16





ASSEMBLED BY :M. KEF Checked by :l.M. Samf	DICH DATE : 02/16 DIES DATE : 04/16		
DRAWN BY : DGE 08/10 Checked by : TMG 11/11	REV. 8/14 MAA/TMG	DESIGN ENGINEER OF RECORD : L.M. SAMPLES DATE : 05/16	





GRADE 270 STRANDS					
	0.6″ØL.R.				
AREA (SQUARE INCHES)	0.217				
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600				
APPLIED PRESTRESS (LBS.PER STRAND)	43,950				

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 24



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STD.NO.33PCBB4_90S_90L



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SHOWING ELEVATION VIEW OF GROUTED RECESS





DEAD LOAD DEFLECTION A	ND CAMBER
	3'-0" × 2'-9"
90' BOX BEAM UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2⅔₄″ ⋠
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	3∕₄″ ∳
FINAL CAMBER	2″ 🕴

** INCLUDES FUTURE WEARING SURFACE

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-9"
50' BOX BEAM UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	¹ ∕2″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	^ı ∕8″ ∳
FINAL CAMBER	3/8″

** INCLUDES FUTURE WEARING SURFACE



STD.NO.33PCBB5_90S



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BOX BEAM UNITS REQUIRED							
SPANS A & C	NUMBER	LENGTH	TOTAL LENGTH				
EXTERIOR B.B.	4	50'-0"	200'-0"				
INTERIOR B.B.	16	50'-0"	800'-0"				
SPAN B	NUMBER	LENGTH	TOTAL LENGTH				
EXTERIOR B.B.	2	90'-0"	180'-0"				
INTERIOR B.B.	8	90'-0"	720'-0"				
TOTAL	30		1900′-0″				

BI	L OF MATERIAL FOR VERTICAL CONCRE	ΤΕ Β	ARR]	ER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	90' UNIT				
₩ B10	96	# 5	STR	22'-1"	2211
* S6	252	# 5	1	7'-2″	1884
	50' UNIT				
₩ B7	96	# 5	STR	24'-7"	2461
* S6	288	# 5	1	7′-2″	2153
* EPOX	COATED REINFORCING STEEL		LBS.		8709
CLASS	AA CONCRETE		CU.YDS.		49.2
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		380.0

GUTTERLINE ASPHA	ALT THICKNESS & F	RAIL HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
90' UNITS	11/2″	3′-7½″
50' UNITS	31/8″	3′-9 <mark>½</mark> ″



ms consultants, inc. 920 Main Campus Drive Suite 430 Raleigh, NC 27606

$ \begin{array}{c} 6'' \\ 3'-4'' \\ 7\frac{3'}{4''} \\ 1 \end{array} $
BAR TYPE BAR DIMENSIONS ARE OUT TO OUT





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ANCHORS FOR GUARDRAIL

ms consultants, inc. 920 Main Campus Drive Suite 430 Raleigh, NC 27606

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 78" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.







TYPES —	BILL OF MATERIAL						
		FOF	R ON	IE E	ND BE	NT	
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	B1	8	#9	1	38'-0"	1034	
¬"	B2	28	#4	STR	19'-1"	357	
НК. () НК.	B3	9	#4	STR	2'-5″	15	
` (4) <i>`</i>							
\mathbf{C}	D1	20	# 8	STR	2'-3"	120	
	H1	48	# 5	2	11'-4"	567	
	К1	12	#4	STR	2'-11"	23	
	K2	12	#4	STR	19'-1"	153	
	S1	46	#4	3	10'-5"	320	
\sim	<u>S2</u>	46	#4	4	3'-2"	97	
	53	20	#4	5	6'-6"	87	
1′-8″ Ø	1.11	70	# 1	<u> </u>	7/ 7//	70	
	UI	- 50	- 4	б	5-1	12	
	\/1	60	#1	STD	7'-2"	287	
	V1 V2	60	 #⊿	STR	5'-3"	210	
8″►					5 5	210	
	(FOR	ONE E	ND BEN	EL T)	3	342 LBS.	
	CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT)						
	POUR #1 CAP,LOWER PART 18.5 C.Y OF WINGS & COLLARS						
ONS ARE OUT TO OUT.	POUR	#2 B P	ACKWAL ART OF	L & L WING	IPPER S	5.2 C.Y.	
	TOTAL	. CLAS	SS A C	ONCRE	ΓE	23.8 C.Y.	

	1	RT							
	RENT 1								
	BAR		C ST7F	TYPE		WETCHT			
	B1	10	#11	1	35'-2"	1868			
	B2	6	# 5	STR	32'-2"	201			
1'-7"	D1	40	*8	STR	2'-3"	240			
	M1	30	#11	STR	31'-10"	5074			
1 ¹ / ₂ EXTRA TURNS	S1	52	# 5	2	9′-6″	515			
INTO CAP	U1	6	#4	3	6'-2"	25			
	U2	8	#4	3	5'-6"	29			
PITCH	V1	30	#11	4	26′-8″	4250			
	(FOR	BENT	NG STE 1)	EL	12,	202 LBS.			
	SP-1	3	*	5	353′-6″	1106			
	SP-2	3	**	6	630′-10″	1264			
4 SPACERS	SPIR. (FOR	AL COL BENT 1	UMN RE	EINFORG	CING STEEL 2				
	* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN								
	₩. علام لا	IRE UR	ר כ י ס לסדס	AIN OF		STEEL			
2'-2"Ø	SH W	IALL BI	E W20 #4 PL	OR D-2 AIN OF	O COLD DR COLD DR COLD DR	AWN D BAR			
		CLAS	SAC((F)	ONCRETE	E BREAKDON T 1)	WN			
	POUR	#2 (CC	DLUMNS)		12.5 C.Y.			
	POUR #3 (CAP) 13.2 C.Y								
	TOTAL CLASS A CONCRETE 25.7 C.Y								
			DRIL (FO	LED PI R BENT	ERS: 1)				
	DRILI POUR	_ED PI #1(DR	ER CON ILLED	NCRETE PIERS)		17.3 C.Y.			
	3'-0"	Ø DRII	LED P	IER NO	T IN SOIL 35	I TN. FT.			
	3'-0"	Ø DRII	LED P	IER IN	SOIL 31				
	CSI -	TUBES			282	LIN.FT.			
1 DOWELS					202	· · · · · · · · · · · · · · · · ·			
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s									
О- , ́р									
F	PROJ	ECT	NO.	[<u> 3-539</u>	8			
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	BILL OF MATERIAL						
			B	ENT	2		
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	B1	10	#11	1	35'-2"	1868	
	В2	6	#5	SIR	52'-2"	201	
1'-7" 18'-10"	D1	40	#8	STR	2'-3"	240	
	M1	30	#11	STR	19'-4"	3082	
1 ¹ / ₂ EXTRA TURNS INTO CAP	S1	52	#5 #4	2	9'-6"	515	
	U2	<u>а</u> 8	#4	د 3	<u> </u>	25 29	
		-		-			
17'-0" 3" PIT	V1 REIN (FOR	30 FORCII BENT	#11 NG STE 2)	4 EL	<u>20'-5"</u>	3254 214 LBS.	
	SP-1	3	*	5	153′-10″	481	
	SP-2	3	**	6	464'-4"	931	
4 SPACERS	SPIRA (FOR	AL COL BENT 2	umn RE 2)	LINFORC	ING STEEL 1	412 LBS.	
	* TH SH WI	IE SP-1 IALL BI IRE OR	l SPIR4 E W31 (#5 PL	AL REIN DR D-31 AIN OR	FORCING S COLD DRA DEFORMED	STEEL WN) BAR	
2'-2"Ø	*** TH SH W]	IE SP-2 IALL BI TRE OR	2 SPIR E W2O #4 PL	AL REII OR D-2 AIN OR	NFORCING O COLD DR DEFORMED	STEEL AWN) BAR	
		CLAS	SAC((F)	ONCRETE	E BREAKDON [2)	٧N	
	POUR	#2 (C()		9.1 C.Y.	
	POUR	#3 (C/	4P)			13.2 C.Y.	
	TOTAL	_ CLAS	S A C(ONCRETE		22.3 C.Y.	
				LED PI R RENT	ERS: 2)		
	DRILL	ED PI.	ER CON	NCRETE	_ /		
	POUR	#1 (DR	ILLED	PIERS)		7.5 C.Y.	
	3'-0"	Ø DRII	LED P	IER NO	T IN SOIL 28.50	LIN.FT.	
	3'-0"	Ø DRII	LED P	IER IN	SOIL 0	LIN.FT.	
D1 DOWELS	CSL T	UBES			132.00	LIN.FT.	
* 0 -							
Ŵ							
P	ROJE	СТ	NO.	E	<u>8-539</u> 8	3	
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DRAWN BY :	J.M. KEPICH	DATE	02/16
CHECKED BY :	L.M. SAMPLES	DATE :	04/16
DESIGN ENGINEE	R OF RECORD : L.M. SAMPLES	DATE :	05/16

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NOTES

U-BARS IN STEM MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 2 OF 3.

DRAWN BY :	J.M. KEPICH	DATE :	02/16
CHECKED BY :	L.M. SAMPLES	DATE :	04/16
DESIGN ENGINEER	OF RECORD : L.M. SAMPLES	DATE :	05/16

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ms consultants 920 Main Campus Drive Suite 430 Raleigh, NC 2760 NC License Numb

TYPES -			ΒI	LL O	F M	ATERIA	L
			FC	DR E	ND	BENT	2
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	(2)	B1	4	#9	STR	35′-6″	483
ω		B2	16	# 5	STR	35′-6″	592
╺┝							
►	10'-8" H1	D1	20	*8	SIR	2'-3"	120
	8'-9" V3	н	84	#5	2	11'- <i>A</i> "	993
1-	-1		-07		2	11 7	555
		К1	12	#4	STR	2'-11"	23
		К2	12	#4	STR	19'-1"	153
			37	#4	STR	6'-3"	154
			8	#5 #0		35'-6"	296
		T4	37	#5	1	7'-5"	286
							200
		U1	30	#4	6	3'-7"	72
		U2	38	#4	6	6′-5″	163
NS ARE OUT	TO OUT.						
		V1	60	#5	STR	10'-5"	652
		V2	60 70	#4 #5		5'-3" Q'_5"	210
			טט אג	רי #5	2 	יב- צ' אי_ס״	<u> </u>
		RETNE	<u>, 30</u> Orctn	<u>, J</u> NG STF	<u>, , , , , , , , , , , , , , , , , , , </u>		
		(FOR	END B	ENT 2)		Ę	5714 LBS.
		CLASS	S A CO	ONCRET	E BRE	AKDOWN	
			(FOR	END B	ENT 22)	
		POUR	#1 F	OOTING	;		18.0 C.Y.
		POUR	#2 S	TFM &	LOWF	R PART	26.8 C.Y.
			- 0	FWINC	SS		
			#7 D		1 0 1		E O C V
		PUUR	то Р	ACKWAL ART OF	-L & L F WINC	S	J.Z U.T.
		τοται	CLAS	SS A C	ONCRE	TE	50.0 C.Y.
						N I I	
		FOUNL		N EACA	VALLU		
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NO.	TES	0				
FOR	BERM	WIDTH	DIMENSIONS,	SEE	GENERAL	DRAWING.

ESTIMATED QUANTITIES					
RIDGE @ TA.16+45.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	203	226			
END BENT 2	172	192			
TOTAL	375	418			

\$\$\$\$USERNAME\$\$\$

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STD.NO.BAS_BB_30_90S

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION. SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED.CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY. 1990