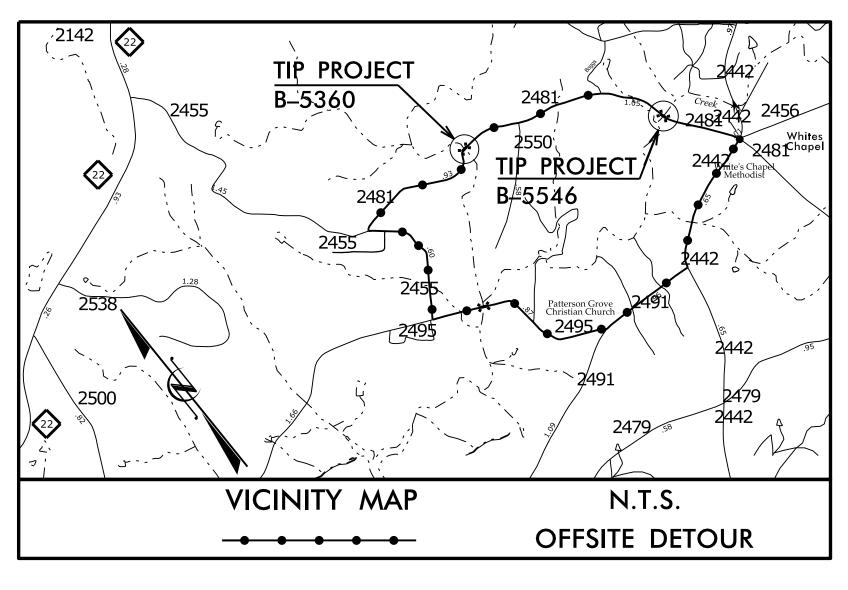
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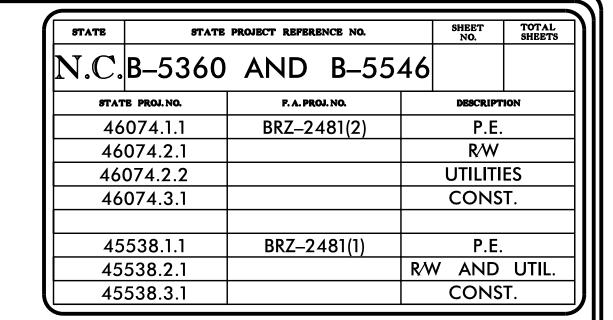
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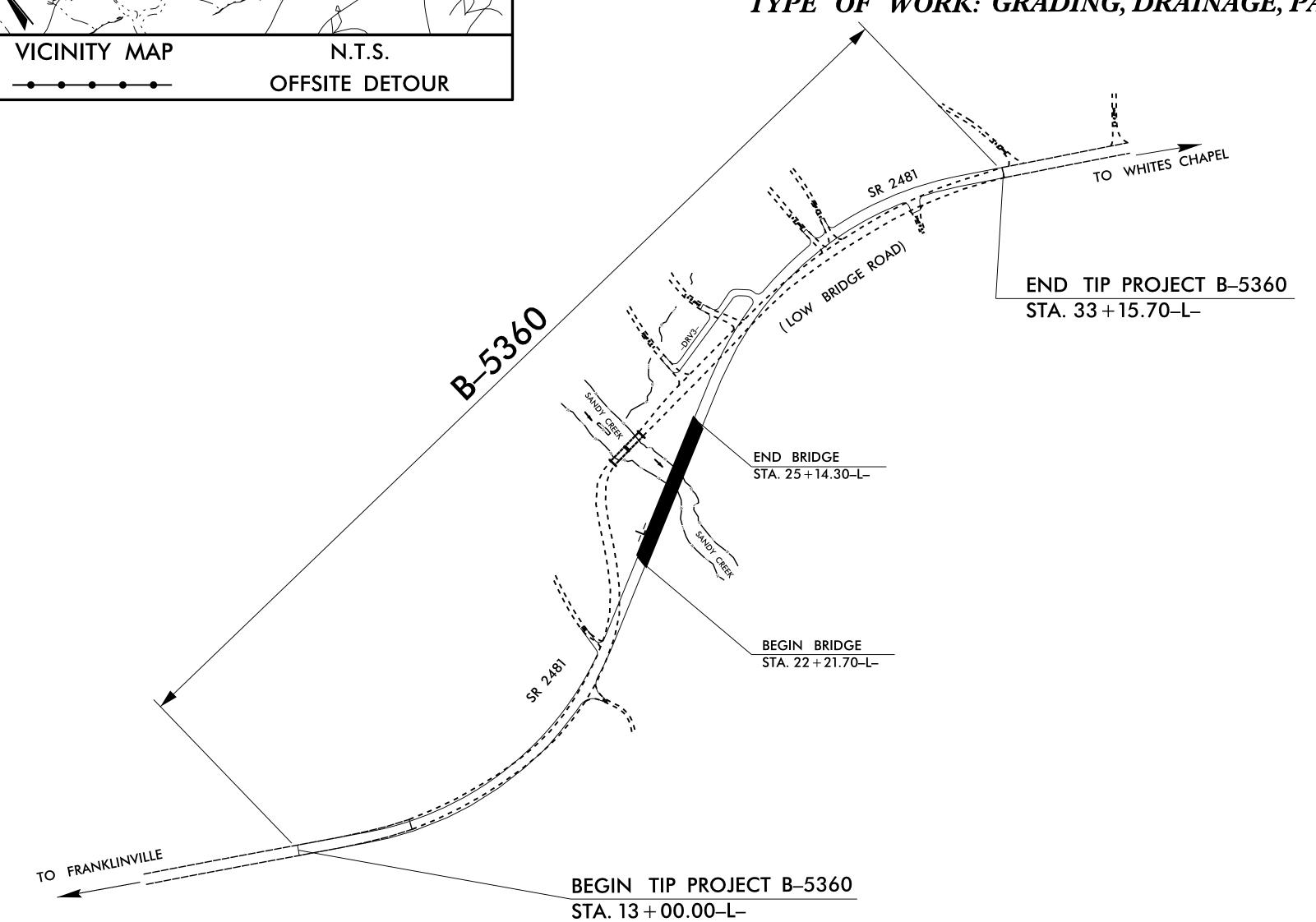


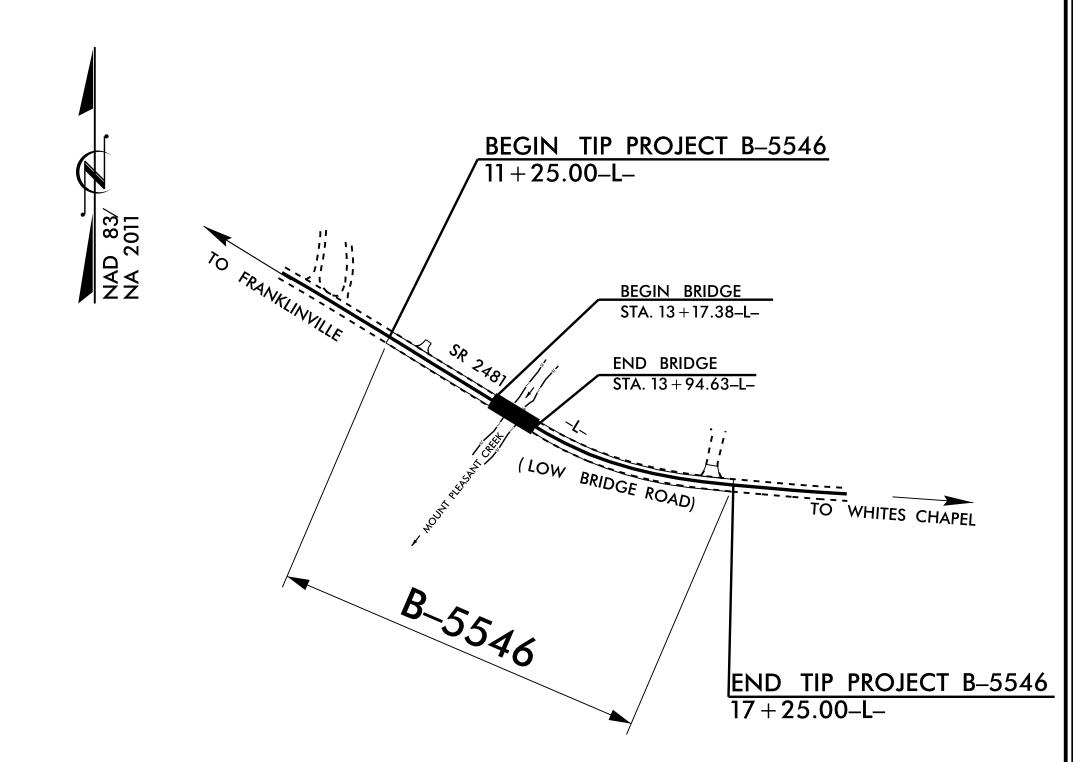
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

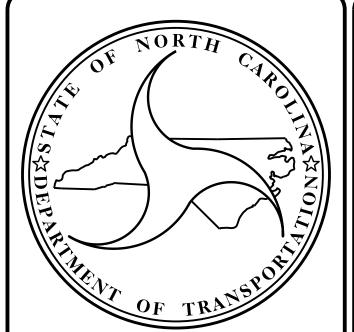
RANDOLPH COUNTY



LOCATION: BRIDGE NO. 374 OVER SANDY CREEK ON SR 2481 (LOW BRIDGE RD.)
AND BRIDGE NO. 307 OVER MOUNT PLEASANT CREEK ON SR 2481 (LOW BRIDGE RD.)
TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURES







DESIGN DATA

ADT (2014) = 400 ADT (2040) = 700 K = 13 % D = 60 %

T = 9 % ** V = 40 MPH ** (TTST 1 %, DUAL 8 %)

FUNC CLASS = RURAL, LOCAL SUB REGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5360 = 0.327 MILES LENGTH STRUCTURE TIP PROJECT B-5360 = 0.055 MILES

TOTAL LENGTH TIP PROJECT B-5360 = 0.382 MILES

LENGTH ROADWAY TIP PROJECT B-5546 = 0.099 MILES LENGTH STRUCTURE TIP PROJECT B-5546 = 0.015 MILES

TOTAL LENGTH TIP PROJECT B-5546 = 0.114 MILES

Prepared in the Office of: DIVISION OF HIGHWAYS STRUCTURES MANAGEMENT LINES

STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.
RALEIGH, N.C. 27610

LETTING DATE :

2012 STANDARD SPECIFICATIONS

OCTOBER 18, 2016

J.M. BAILEY, P.E.

PROJECT ENGINEER

G.W. DICKEY, PE

PROJECT DESIGN ENGINEER

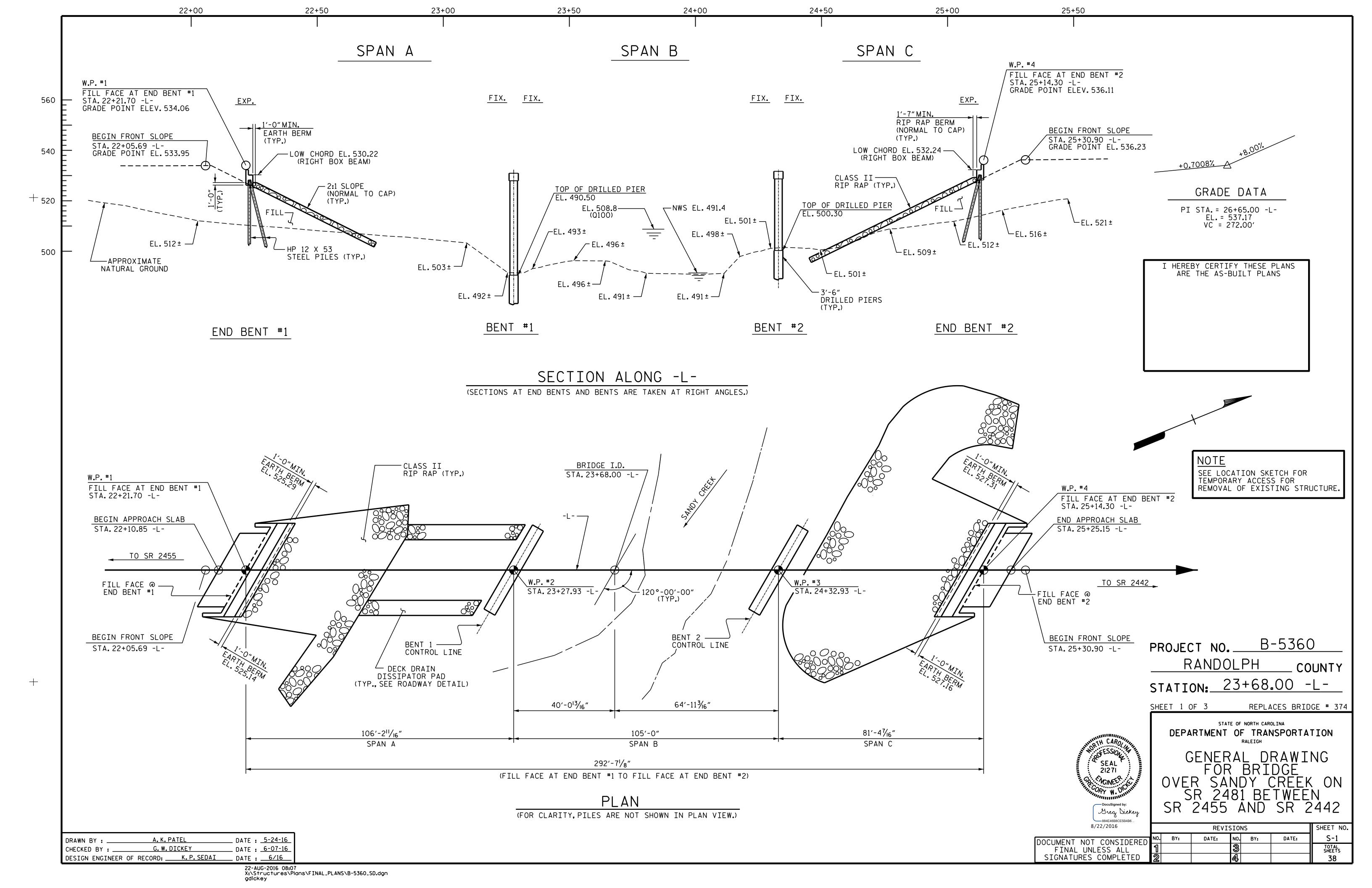
E.E. MURRAY, P.E.

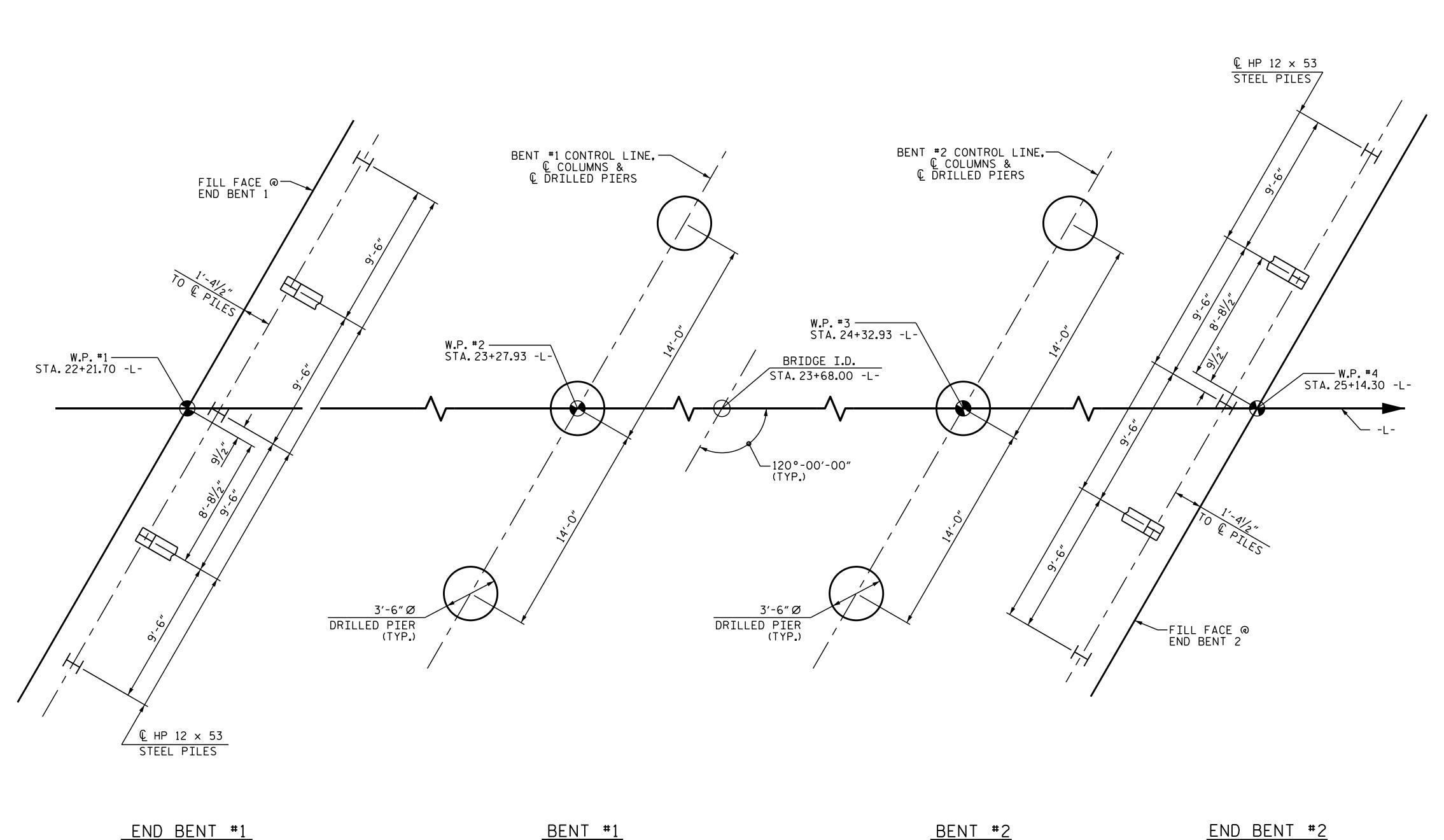
PROJECT ENGINEER

D.A. DAVENIPORT IR PE

D.A. DAVENPORT, JR., PE

PROJECT DESIGN ENGINEER





FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE AT THE BOTTOM OF CAP

NOTES:

FOR PILES, SEE SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENTS NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 150 AND 125 TONS PER PILE. RESPECTIVELY.

DRIVE PILES AT END BENTS NO.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 250 AND 210 TONS PER PILE, RESPECTIVELY.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENTS NO.1 AND 2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 20 TO 30 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENTS NO.1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D) (2) OF THE STANDARD SPECIFICATIONS.

FOR DRILLED PIERS, SEE SPECIAL PROVISIONS AND SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENTS NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 531 TONS AND 483 TONS PER PIER, RESPECTIVELY. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 25 TSF.

INSTALL PERMANENT STEEL CASINGS AT BENT NO.1 BY VIBRATING, SCREWING OR DRIVING PERMANENT CASINGS BEFORE EXCAVATING OR DISTRUBING ANY MATERIALS BELOW ELEVATION 490 FT.

PERMANENT STEEL CASINGS MAY BE REQUIRED FOR DRILLED PIERS AT BENT NO.1. DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 490 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 480 FT AND WITH THE REQUIRED TIP RESISTANCE.

INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 481 FT AND WITH THE REQUIRED TIP RESISTANCE.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS 489 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS 496 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

PROJECT NO. B-5360

RANDOLPH COUNTY

STATION: 23+68.00 -L-

SHEET 2 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE

OVER SANDY CREEK OF

Docusigned by:

Lacy Dickey

884E46BBCE5B4B6...

8/22/2016

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-2

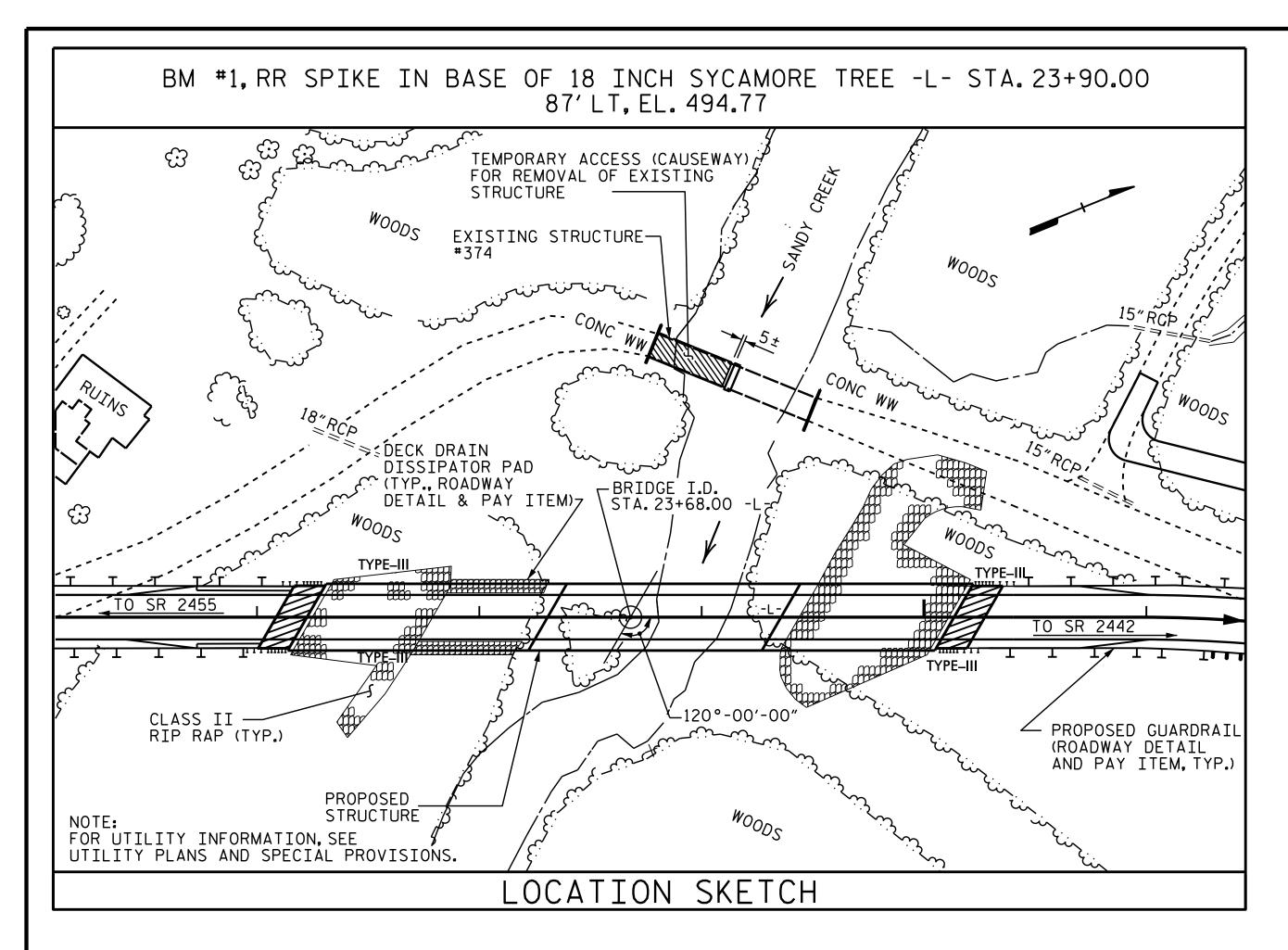
1 3 5000

TOTAL SHEETS

3 38

2481 BETWEEN

DRAWN BY: _____A.K.PATEL DATE: 5-24-16
CHECKED BY: ____G.W.DICKEY DATE: 6-07-16
DESIGN ENGINEER OF RECORD: ____K.P.SEDAI DATE: 6/16



NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY ACCESS (CAUSEWAY), AT THE EXISTING STRUCTURE. FOR USE DURING REMOVAL OF EXISTING STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF 2 SPANS, 1 @ 39'-0". 1 @ 38'-6" WITH A 5" AWS ON TIMBER DECK ON I-BEAMS; A CLEAR ROADWAY WIDTH OF 11'-2", ON MASS CONCRETE ABUTMENTS AND INTERIOR BENT, LOCATED 90' UPSTREAM FROM PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

						_	Т	OTAL	BIL	L OF	MATER	RIA	ΔL								
	CONSTRUCTION, MAINTENANCE, & REMOVAL OF TEMP. ACCESS	REMOVAL OF EXISTING STRUCTURE	3'-6"DIA. DRILLED PIERS IN SOIL	3'-6"DIA. DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-6"DIA. DRILLED PIER	SID INSPECTIONS	CSL TESTING	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	HP STEI	12 X 53 EL PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE:	O"X 3'-3" STRESSED ONCRETE X BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	CU. YDS.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	NO.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE									LUMP SUM						579 . 42			LUMP SUM	30	2897.08	
END BENT NO. 1								30.0		4709		5	175	5		296	329				
BENT NO. 1			1.5	30.0	4.5			45.4		17476	3177										
BENT NO. 2			17.9	40.0				38.3		12544	3125										
END BENT NO. 2								30.0		4709		5	125	5		685	760				
TOTAL	LUMP SUM	LUMP SUM	19.4	70.0	4.5	2	2	143.7	LUMP SUM	39438	6302	10	300	10	579.42	981	1089	LUMP SUM	30	2897.08	LUMP SUM

HYDRAULIC DATA

= 45.1 SQ. MILES

DESIGN DISHARGE

= 5400 CFS FREQUENCY OF DESIGN FLOOD = 25 YEAR DESIGN HIGH WATER ELEVATION = 505.4

DRAINAGE AREA BASE DISCHARGE (Q100)

= 8066 CFS BASE HIGH WATER ELEVATION

OVERTOPPING DATA

OVERTOPPING DISCHARGE = N/A

OVERTOPPING ELEVATION = 535.7

OVERTOPPING FREQUENCY = N/A

SEAL 21271 MOINEER

RALEIGH GENERAL DRAWING SR 2481 BETWEEN

SHEET 3 OF 3

PROJECT NO. B-5360

STATION: 23+68.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RANDOLPH

Greg Dickey --- 884E46B8CE5B4B6.. 8/22/2016

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS SHEET NO S-3 NO. DATE: DATE: BY: TOTAL SHEETS

SR 2455 AND SR 2442

A. K. PATEL, P.E. _ DATE : <u>5-19-16</u> DRAWN BY : _ G.W.DICKEY DATE : 6-07-16 CHECKED BY : _ DESIGN ENGINEER OF RECORD: K.P. SEDAI DATE: 6/16

	LO	ad and re	ESIST	ANCE	E FA(CTOR	RAT	ING	(LRF	R) SI	JMMA	ARY F	OR F	PREST	TRES	SED	CON	CRET	Е ВО	X BE	AM	UNIT	T S	
										STRE	ENGTH	I LIM	MIT S	ГАТЕ				SERVICE III LIMIT STATE						
								MOMENT									MOMENT				_			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
	_	HL-93(Inv)	N/A	1	1.23		1.75	0.247	1.5	В	EL	51.562	0.615	1.33	В	EL	10.312	0.80	0.247	1.23	В	EL	51.562	
DESIGN	-	HL-93(0pr)	N/A		1.72		1.35	0.247	1.94	В	EL	51.562	0.615	1.72	В	EL	10.312	N/A						
LOAD RATING	-	HS-20(Inv)	36.000	(2)	1.72	62.092	1.75	0.247	2.11	В	EL	51.562	0.615	1.78	В	EL	10.312	0.80	0.247	1.72	В	EL	51.562	
		HS-20(0pr)	36.000		2.31	83.078	1.35	0.247	2.73	В	EL	51.562	0.615	2.31	В	EL	10.312	N/A						
		SNSH	13.500		4.11	55 . 518	1.4	0.247	6.28	В	EL	51.562	0.615	5.49	В	EL	10.312	0.80	0.247	4.11	В	EL	51.562	
		SNGARBS2	20.000		2.97	59.376		0.247	4.54	В	EL	51.562	0.615	3.84	В	EL	10.312	0.80	0.247	2.97	В	EL	51.562	
		SNAGRIS2	22.000		2.77	61.009	1.4	0.247	4.24	В	EL	51.562			В	EL	10.312	0.80	0.247	2.77	В	EL	51.562	
	>	SNCOTTS3	27.250		2.04	55.689	1.4	0.247	3.12	В	EL	51.562	0.615	2.73	В	EL	10.312	0.80	0.247	2.04	В	EL	51.562	
	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	SNAGGRS4	34.925		1.67	58.355	1.4	0.247	2.55	В	EL	51.562	0.615	2.23	В	EL	10.312	0.80	0.247	1.67	B	EL	51.562	
		SNS5A	35 . 550		1.64	58.174	1.4	0.247	2 . 5	В	EL	51.562	0.615	2.23	В	EL	10.312	0.80	0.247	1.64	B	EL	51.562	
		SNS6A	39.950		1.49	59.381	1.4	0.247	2.27	В	EL	51.562	0.615	2.02	B	EL	10.312	0.80	0.247	1.49	B	EL	51.562	
LEGAL		SNS7B	42.000		1.41	59.427	1.4	0.247	2.16	В	EL	51.562	0.615	1.96	B	EL	10.312	0.80	0.247	1.41	B	EL	51.562	
LOAD RATING		TNAGRIT3	33.000		1.81	59.670	1.4	0.247	2.76	В	EL	51.562	0.615	2.42	B	EL	10.312	0.80	0.247	1.81	В	EL	51.562	
		TNT4A	33.075		1.81	59 . 935	1.4	0.247	2.77	В	EL	51.562	0.615	2.37	В	EL	10.312	0.80	0.247	1.81	B	EL	51.562	
		TNT6A	41.600		1.47	61.055	1.4	0.247	2.24	B	EL _	51.562	0.615	2.05	B	EL	10.312	0.80	0.247	1.47	B 	EL _	51.562	
	181	TNT7A	42.000		1.47	61.641	1.4	0.247	2.24	В	EL 	51.562	0.615	2.02	B	EL	10.312	0.80	0.247	1.47	B	EL _	51.562	
	-	TNT7B	42.000		1.5	63.019	1.4	0.247	2.29	В	EL 	51.562	0.615	1.93	B	EL	10.312	0.80	0.247	1.50	B	EL	51.562	
		TNAGRIT4	43.000		1.44	61.946	1.4	0.247	2.2	В	EL	51.562	0.615	1.88	B	EL	10.312	0.80	0.247	1.44	B	EL	51.562	
		TNAGT5A	45.000	 / <u>-</u> \	1.36	61.407	1.4	0.247	2.09	B	EL _	51.562	0.615	1.84	B	EL	10.312	0.80	0.247	1.36	B	EL	51.562	
		TNAGT5B	45.000	(3)	1.35	60.917	1.4	0.247	2.07	В	EL	51 . 562	0.615	1.79	В	EL	10.312	0.80	0.247	1.35	B	EL	51.562	

LOAD FACTORS:

	DESIGN LOAD RATING	LIMIT STATE	γ_{DC}	γ_{D}
		STRENGTH I	1.25	1.5
	FACTORS	SERVICE III	1.00	1.0

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

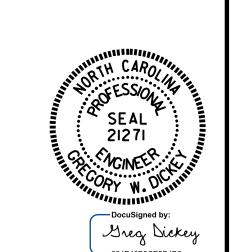
GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5360 RANDOLPH COUNTY STATION: 23+68.00 -L-



8/22/2016

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD

LRFR SUMMARY FOR PRESTRESSED CONCRETE BOX BEAM UNITS

(NON-INTERSTATE TRAFFIC)

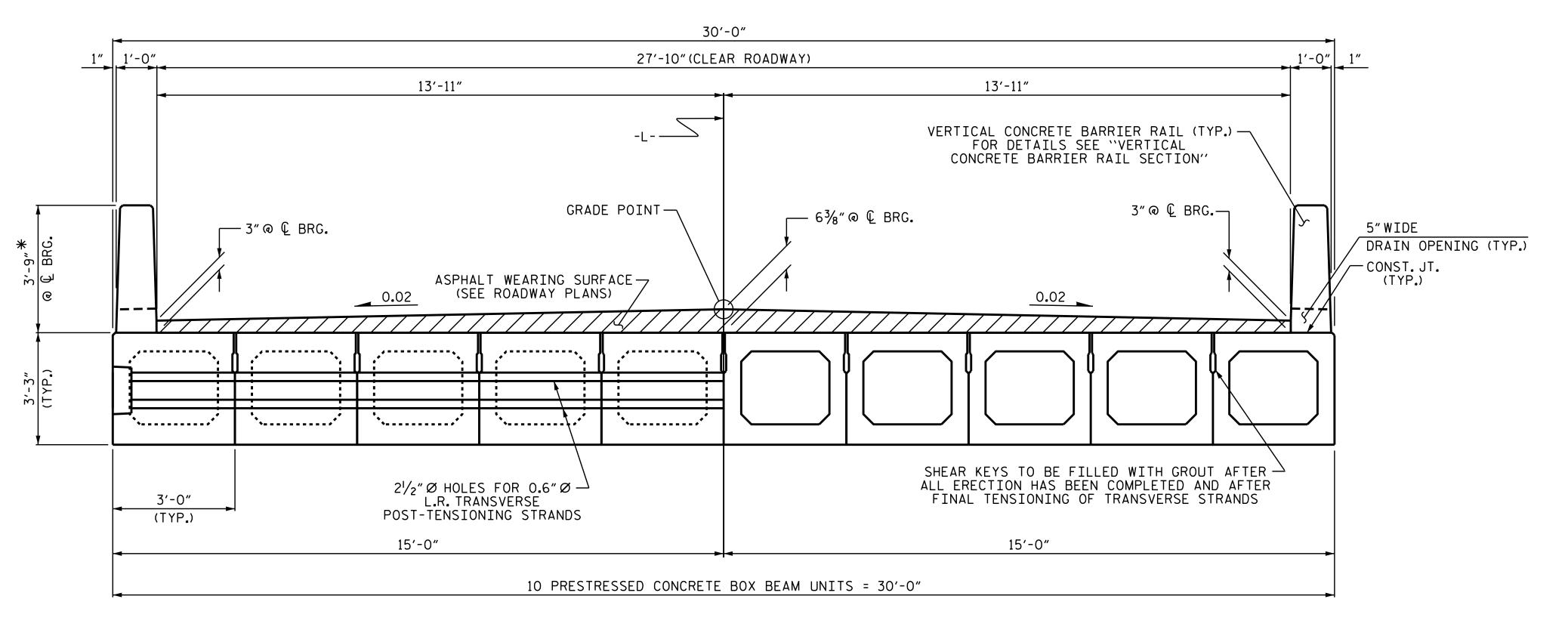
REVISIONS SHEET NO. S-4 DATE: NO. BY: DATE: TOTAL SHEETS 38

END BENT 1 BENT 1 BENT 2 END BENT 2 SPAN A SPAN B SPAN C

LRFR SUMMARY

ASSEMBLED BY: A.K.PATEL DATE: 1/19/16
CHECKED BY: W.F.PARKER DATE: 3/16
DESIGN ENGINEER
OF RECORD: K.P.SEDAI DATE: 6/16 DRAWN BY: MAA I/08 REV. II/I2/08RR MAA/GM REV. IO/I/II MAA/GM

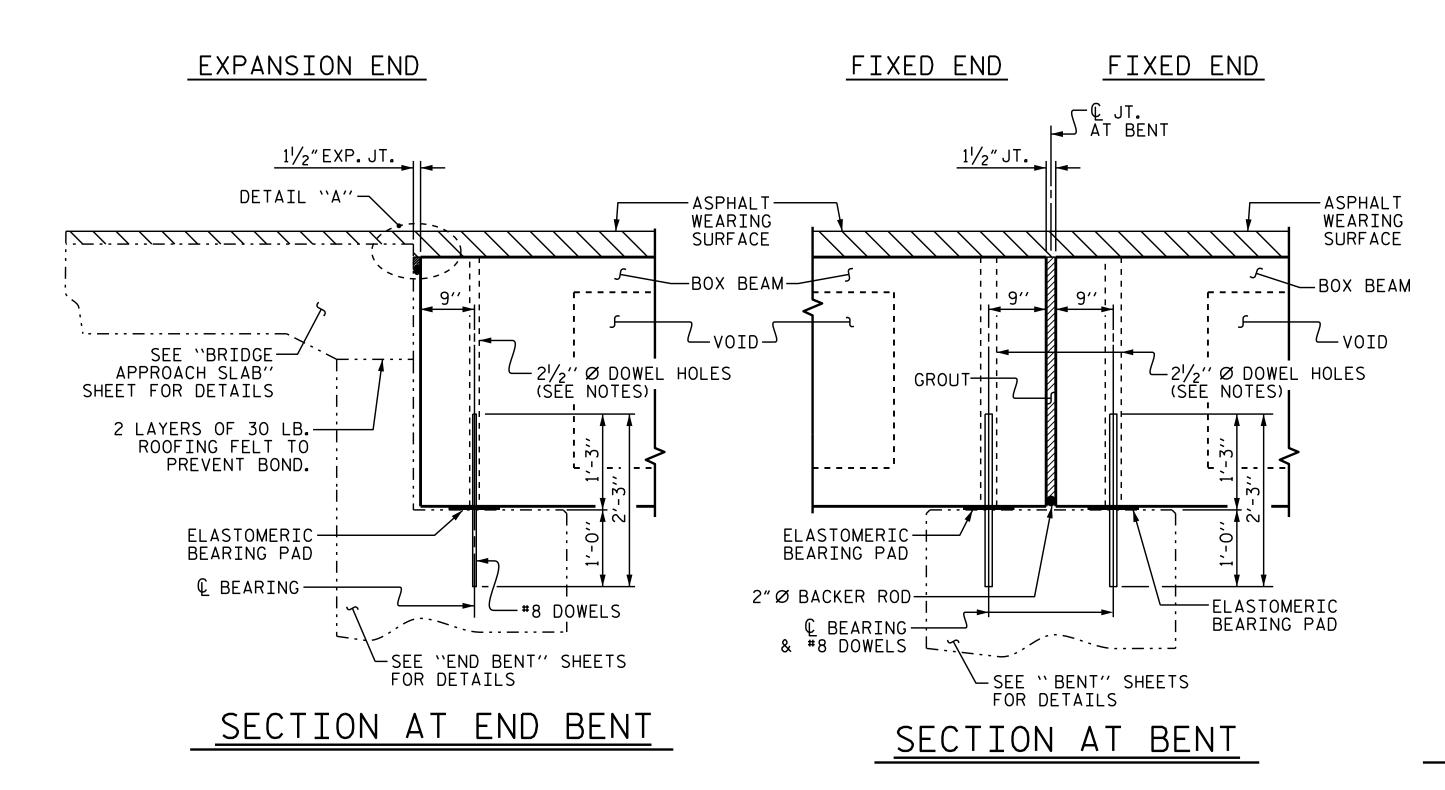
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



HALF SECTION AT INTERMEDIATE DIAPHRAGMS HALF SECTION THROUGH VOIDS

TYPICAL SECTION

*THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.



PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.

DETAIL "A"

JOINT SEALER

MATERIAL

 $1\frac{1}{2}$ " EXP. JT.

© 2″Ø→ `─ BACKER ROD

THREADED INSERT DETAIL

ASSEMBLED BY: A.K.PATEL DATE: 1/13/16 CHECKED BY: W.F.PARKER DATE: 3/16 DRAWN BY: DGE 8/II REV. 10/15 MAA/TMG CHECKED BY : TMG II/II

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6200 PSI (SPANS A & B) AND 5000 PSI (SPAN C).

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE VERTICAL CONCRETE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN VERTICAL CONCRETE BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF VERTICAL CONCRETE BARRIER RAI SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 5" X 6". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

> PROJECT NO. _____B-5360 RANDOLPH _ COUNTY STATION: 23+68.00 -L-

SHEET 1 OF 7

CESSION SEAL 21271 NOINEER --- DocuSigned by:

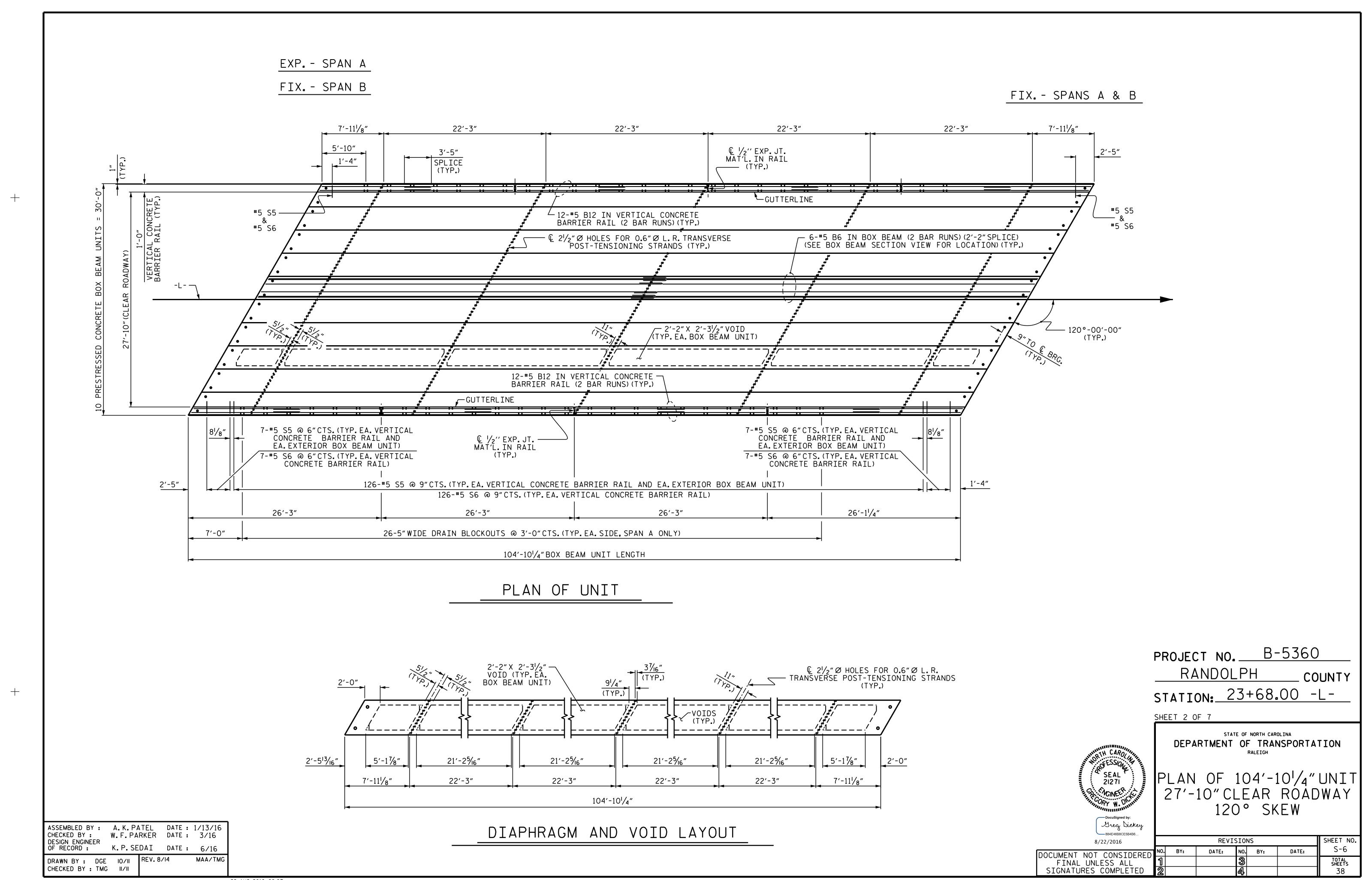
Greg Dickey

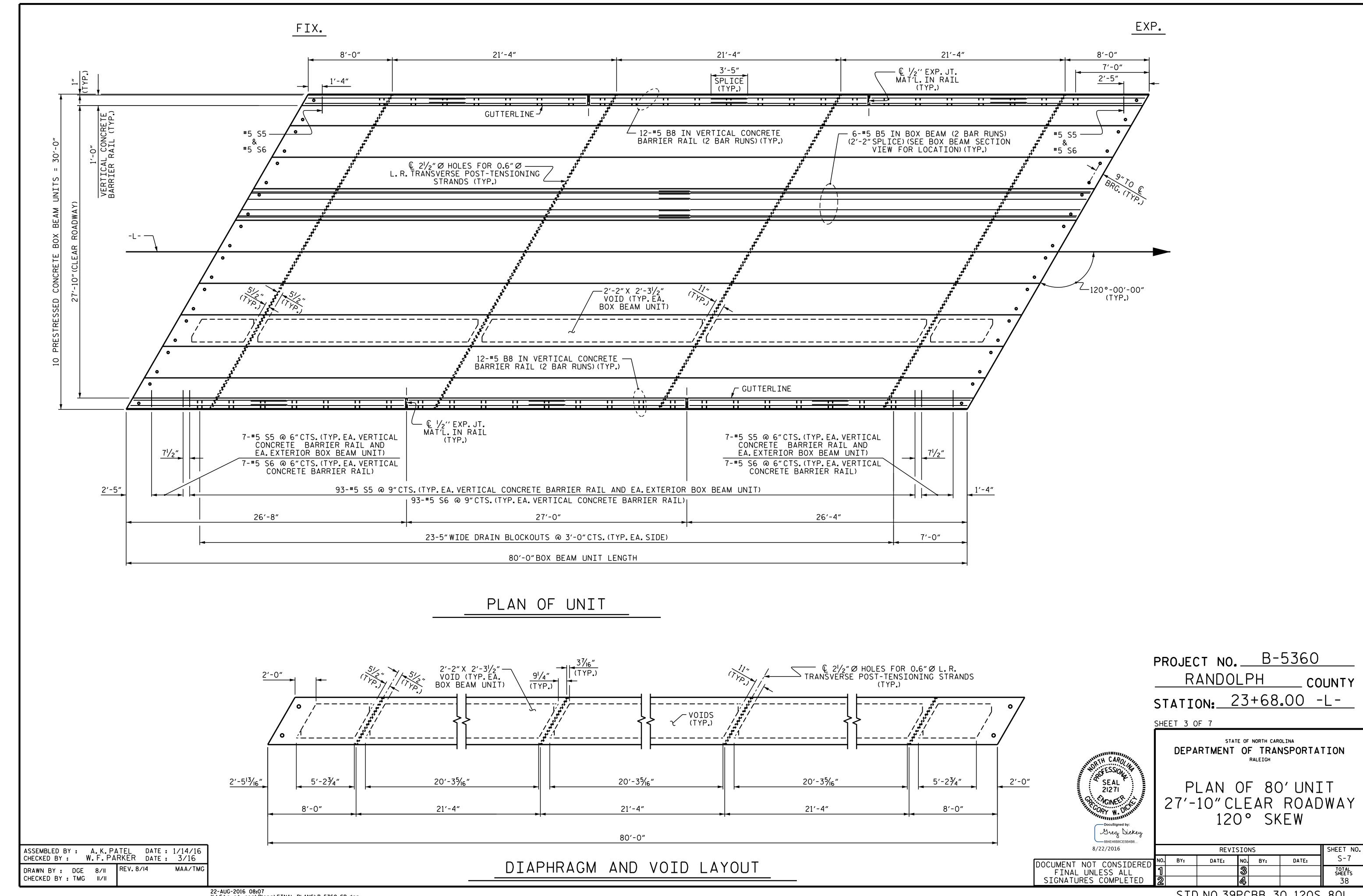
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

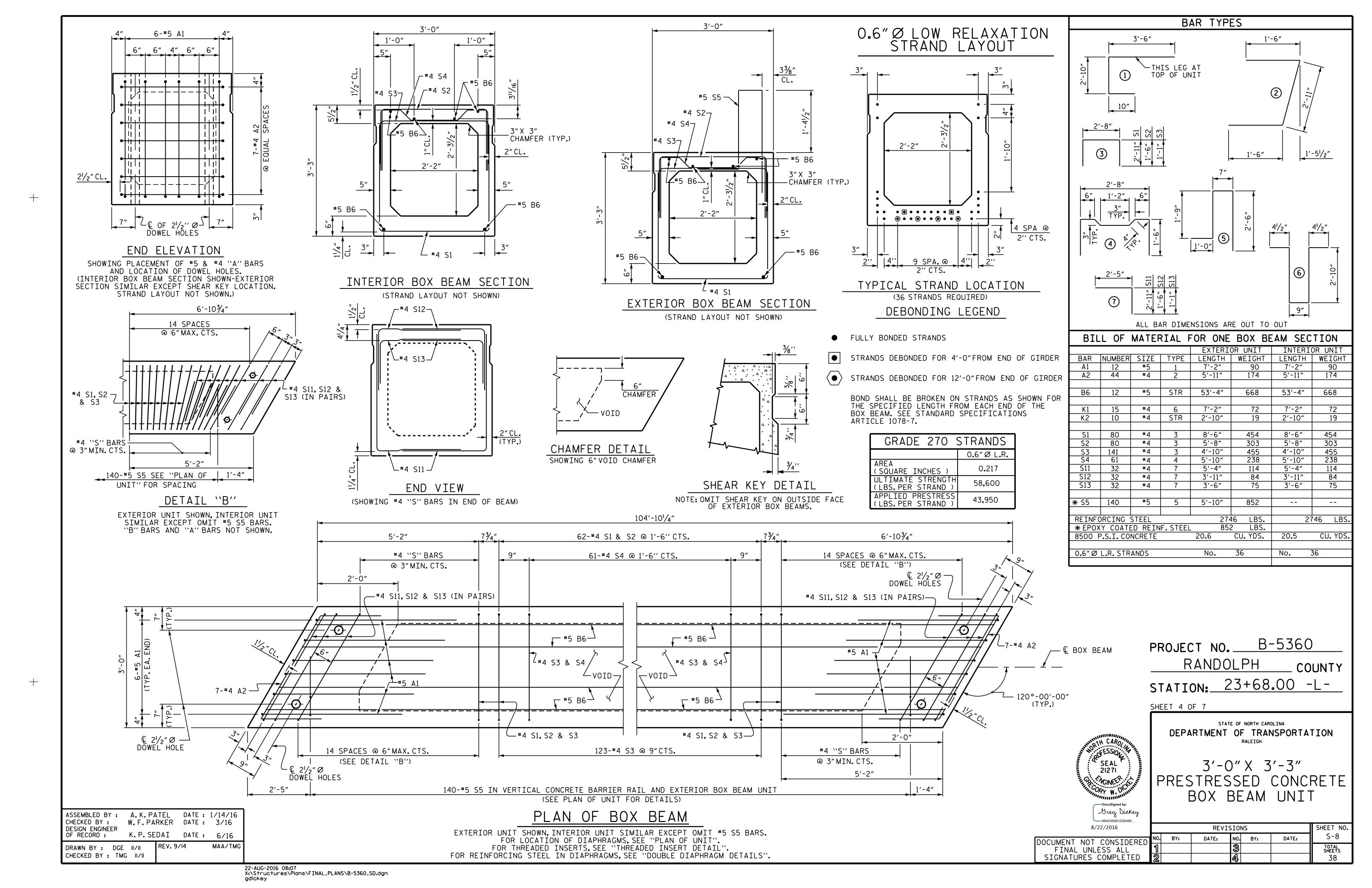
3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

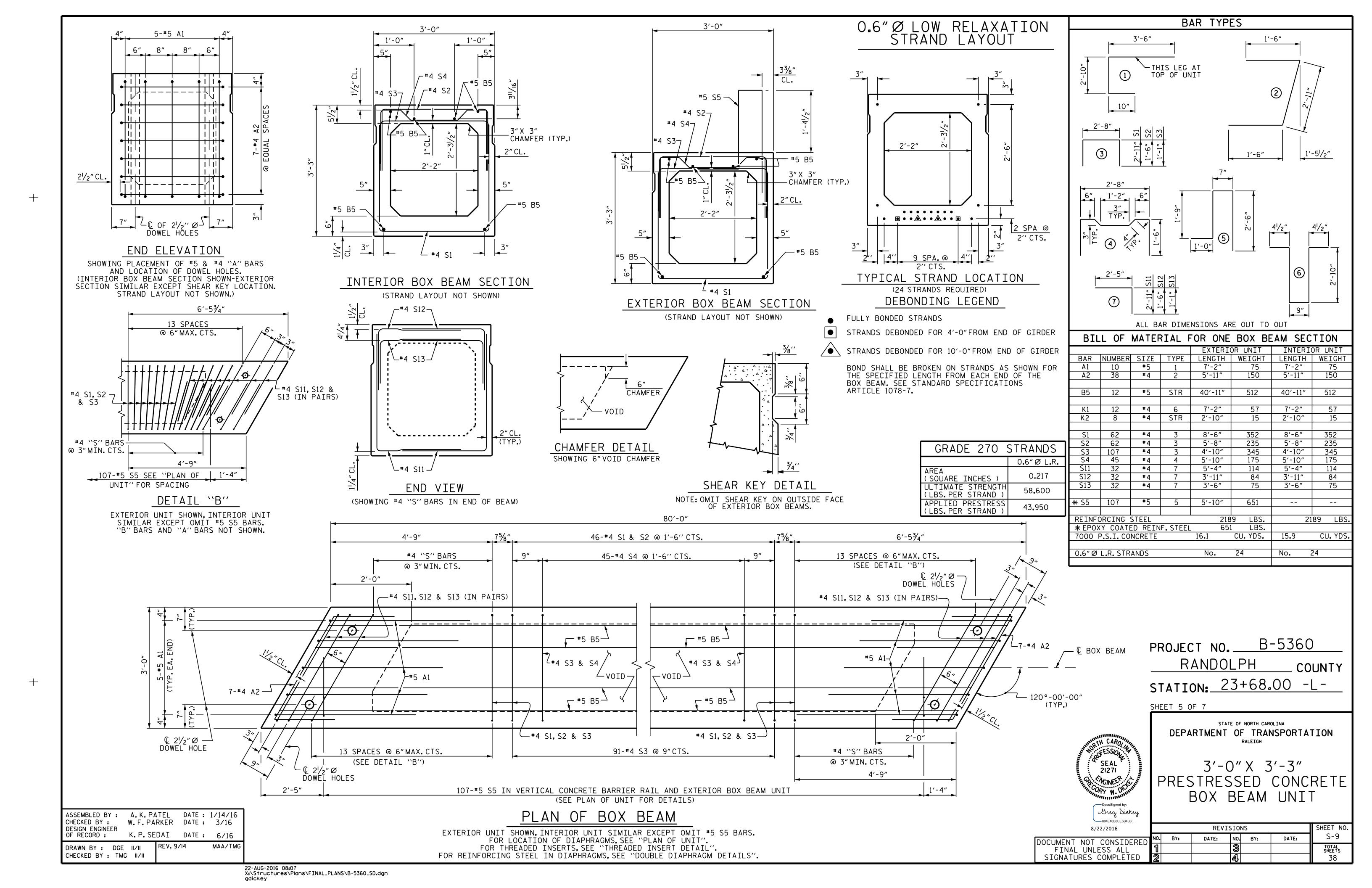
DOCUMENT FINAL SIGNATUR

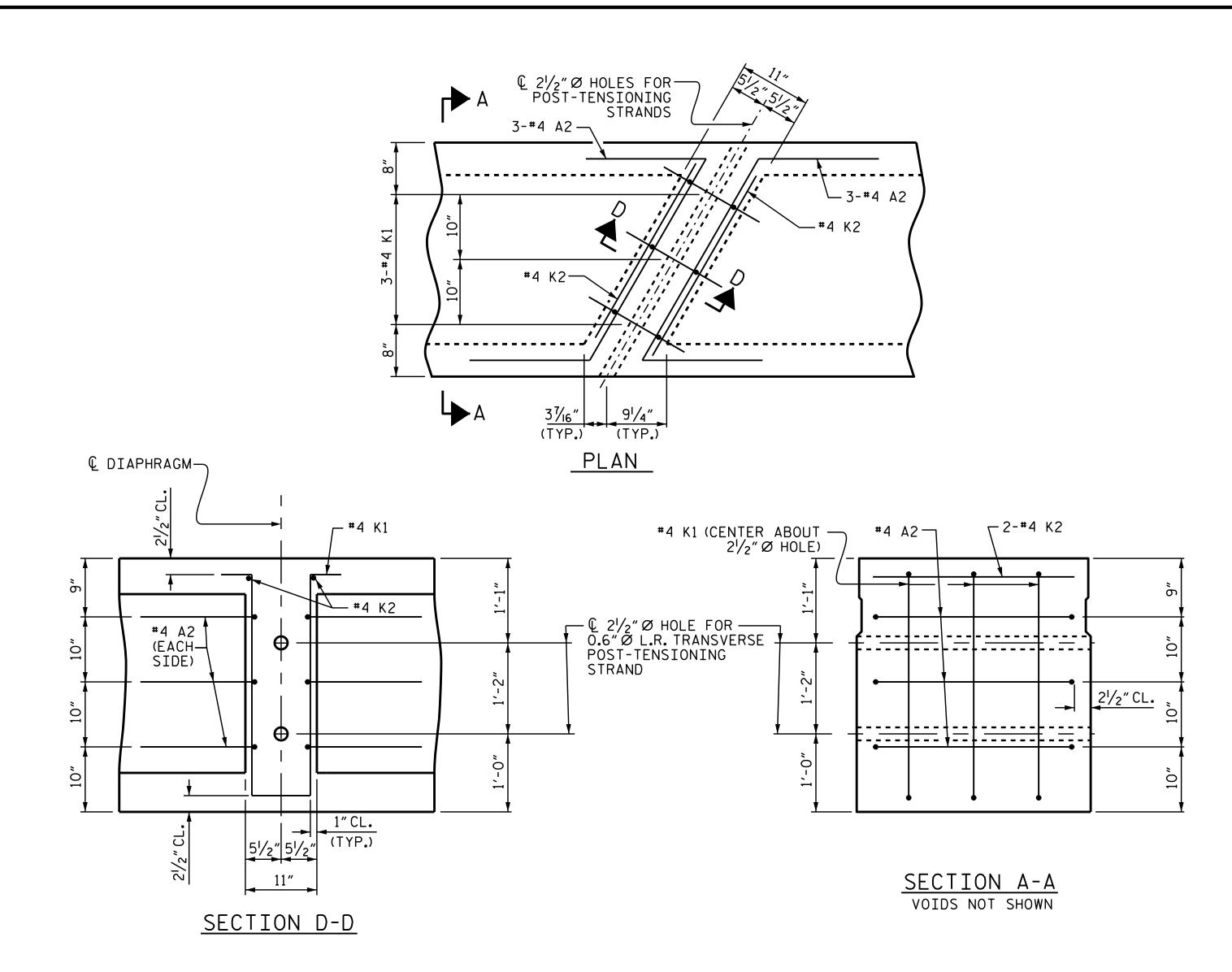
004E40B0CE3B4B0							
8/22/2016		SHEET NO					
NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
UNLESS ALL	1			3			TOTAL SHEETS
RES COMPLETED	2			4			38





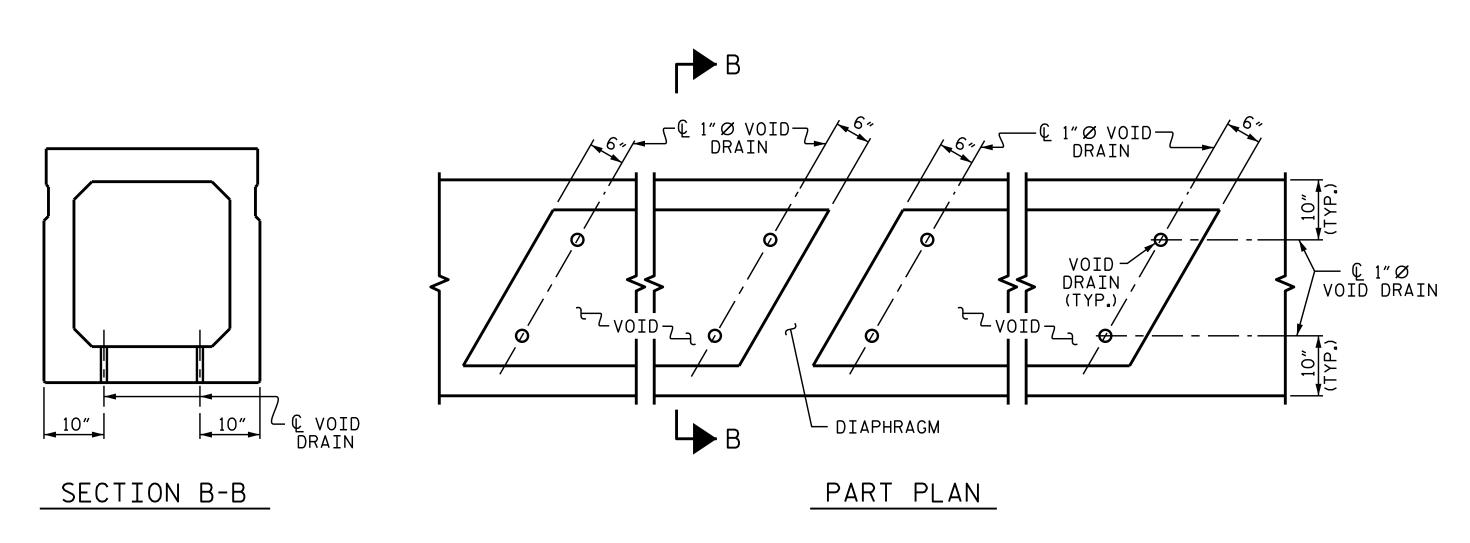






DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 21/2" Ø HOLE.

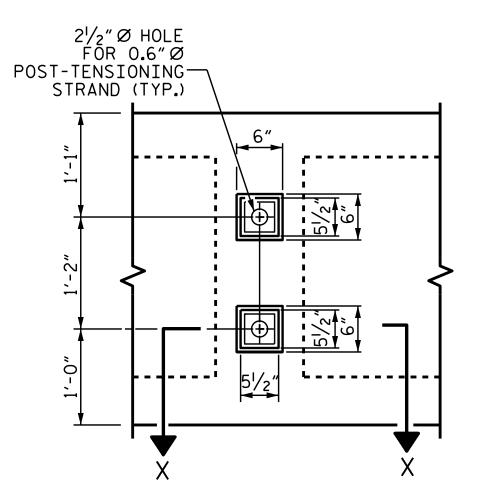


VOID DRAIN DETAILS

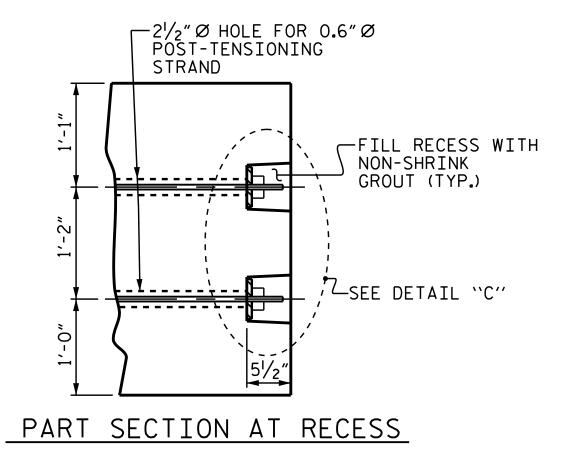
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

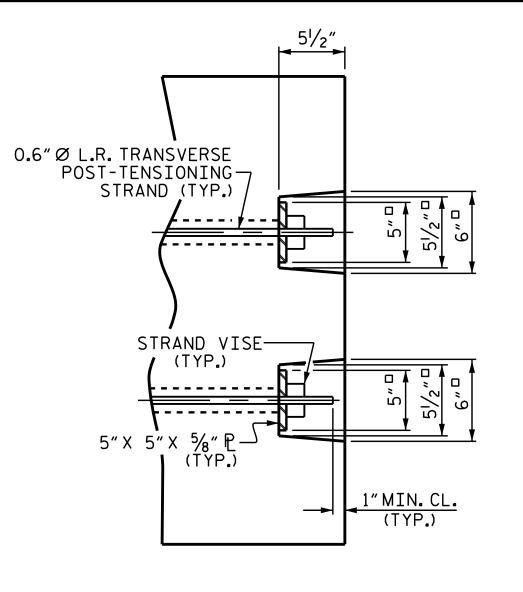
ASSEMBLED BY: A.K.PATEL DATE: 1/14/16 CHECKED BY: W.F.PARKER DATE: 3/16 REV. 8/14 MAA/TMG DRAWN BY : DGE II/II

CHECKED BY : TMG II/II

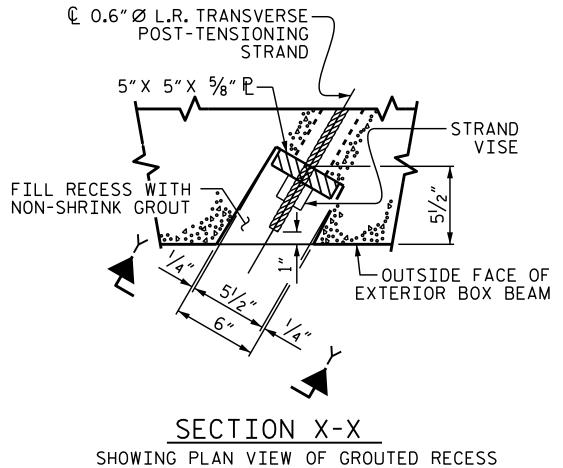


VIEW Y-Y SHOWING ELEVATION VIEW OF GROUTED RECESS





DETAIL "C"



GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS OF EXTERIOR BOX BEAM

DEAD LOAD DEFL	ECTION A	ND CAMBEI	R				
3'-0" × 3'-3"							
	0.0	6"Ø L.R. STRAN	1D				
	SPAN "A"	SPAN "B"	SPAN "C"				
CAMBER (BEAM ALONE IN PLACE)	21/16"	2 ¹ / ₁₆ " 🛉	1 ¹¹ / ₁₆ " 🛉				
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD ***	1⅓6″ ♦	1 <mark>⅓₁₆″ </mark>	3⁄8″ ♥				
FINAL CAMBER	1"	1″ ∤	1 ⁵ ⁄ ₁₆ " ∤				

** INCLUDES FUTURE WEARING SURFACE

PROJECT NO. B-5360 RANDOLPH ____ COUNTY

STATION: 23+68.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

SEAL 21271 COPY W. DILLIN

> — DocuSigned by: Greg Dickey

3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT

REVISIONS 8/22/2016 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

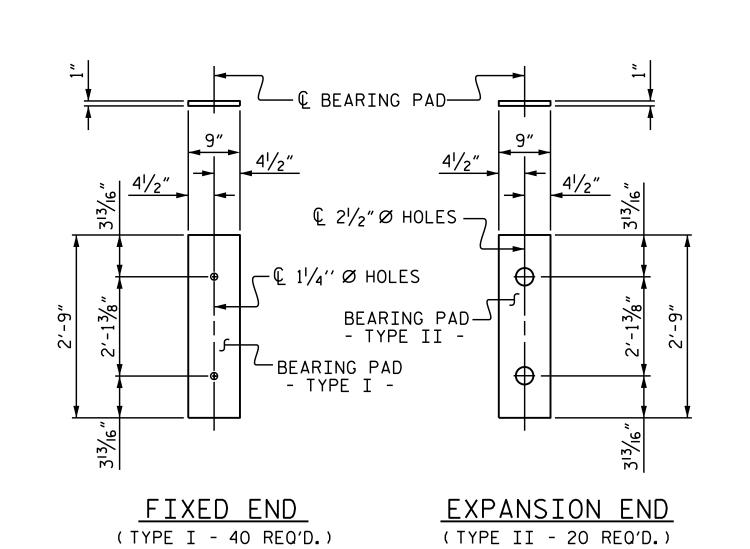
SHEET 6 OF 7

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SHEET NO

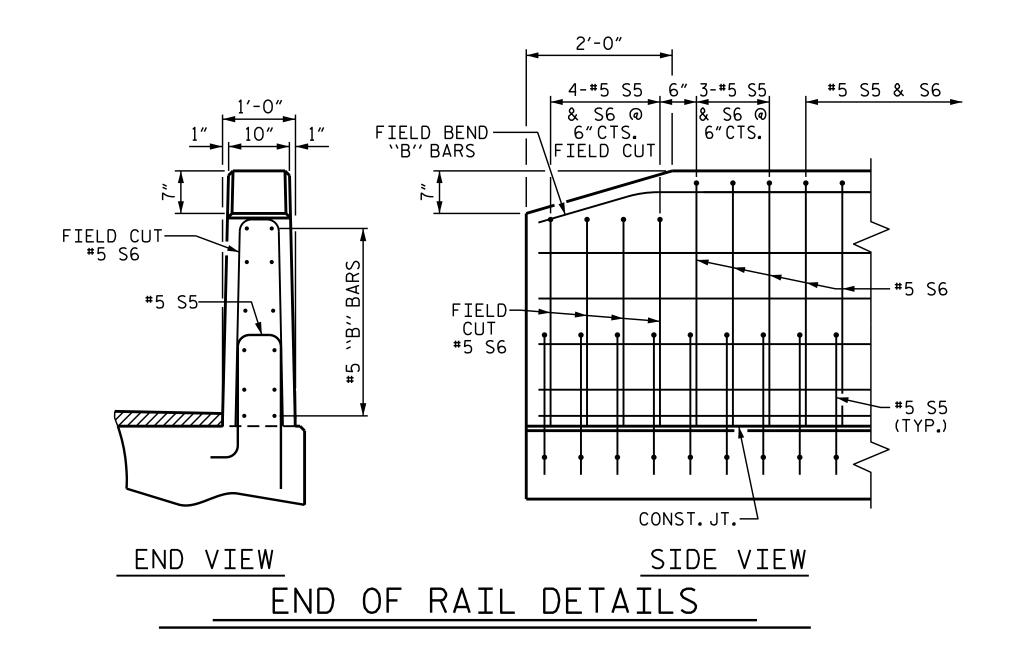
S-10

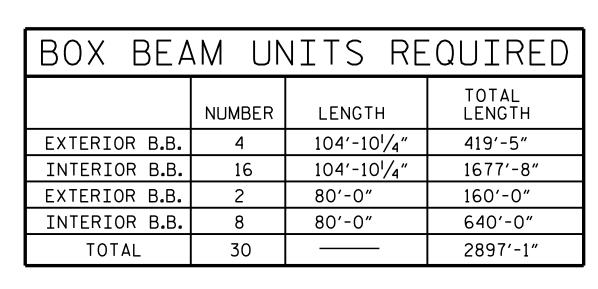
TOTAL SHEETS 38

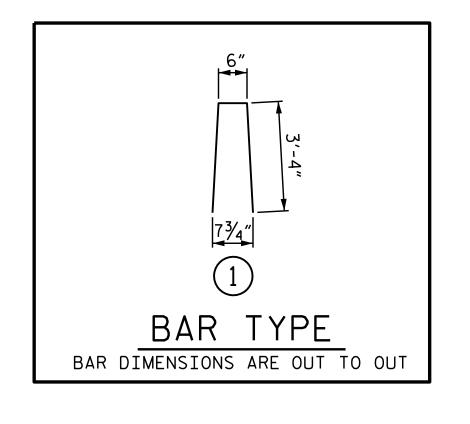


ELASTOMERIC BEARING DETAILS

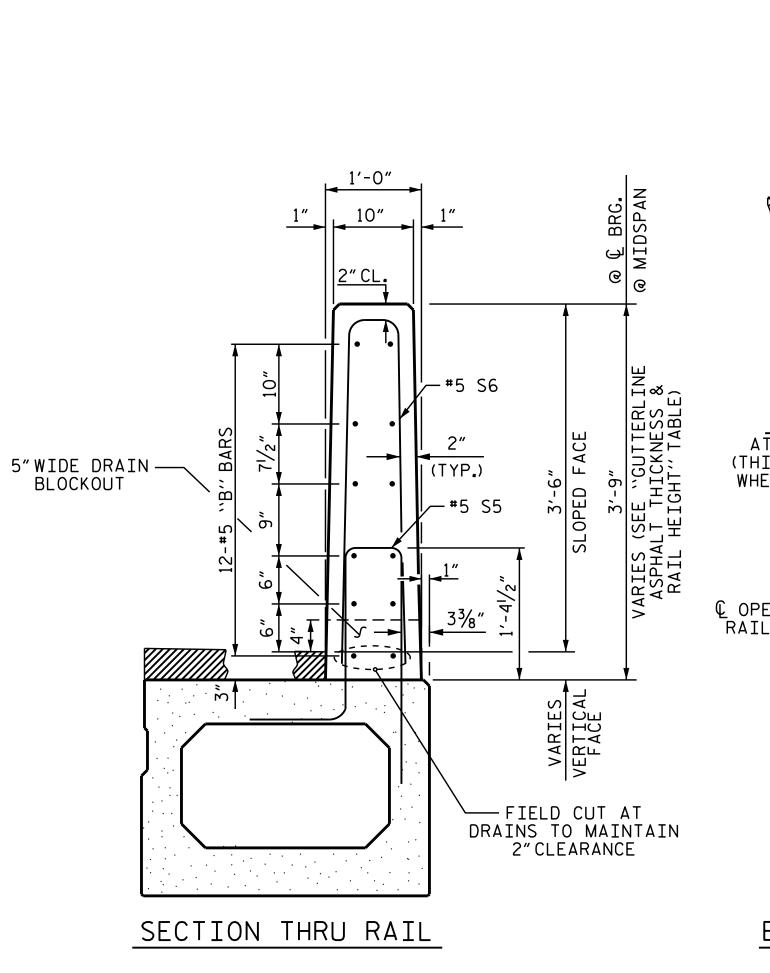
ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

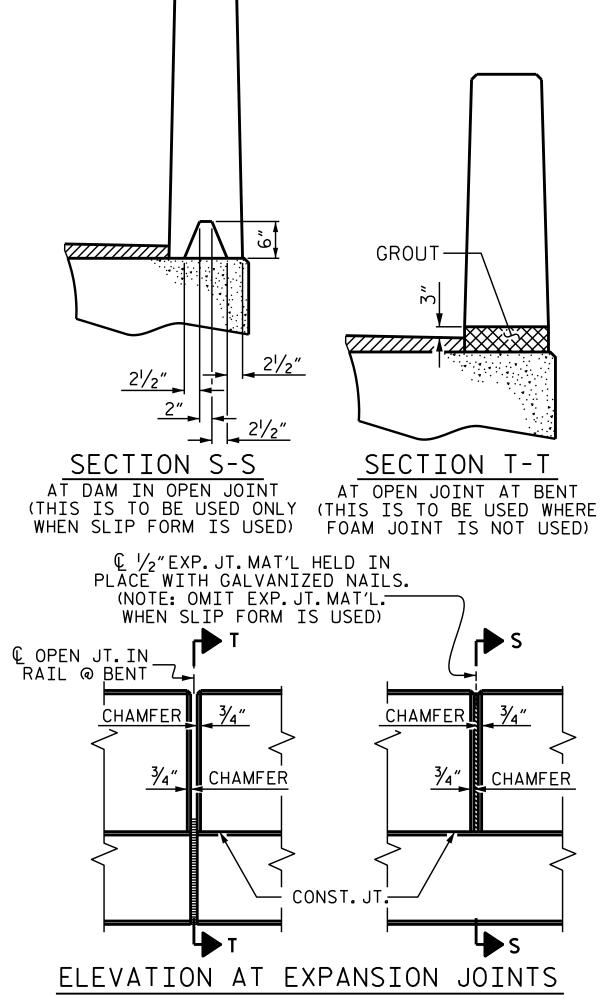






UNIT PSI 104'-101/4"UNITS 6200
104'-10 ¹ / ₄ "UNITS 6200
80'-0"UNITS 5000





BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL (FOR ONE SPAN - 2 REQUIRED)										
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT					
	104'-10 ¹ / ₄ " UNIT									
*B12	192	#5	STR	14'-11"	2987					
* S6	280	#5	1	7'-2"	2093					
<u> </u>	OATED REINFORCING STEEL		LBS.		5080					
CLASS AA	CONCRETE		CU.YDS.		27.2					
TOTAL VER	TICAL CONCRETE BARRIER RAIL		LN.FT.		209.71					

BI	LL OF MATERIAL FOR VERTICAL CONCRE	TE B	ARR:	IER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	80' UNIT				
∗ B8	144	#5	STR	15'-2"	2278
* S6	214	#5	1	7'-2"	1600
* EP0X	 (Y COATED REINFORCING STEEL		LBS.		3878
CLASS	AA CONCRETE		CU.YDS.	1	20.7
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		160.00
	Т	JATC	LN. F	T. = 5	79.42

GUTTERLINE ASPH	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
104'-10 ¹ / ₄ " UNITS	2"	3′-8″
80'UNITS	1"/16"	3′-7 ¹¹ / ₁₆ "

PROJECT NO. B-5360 RANDOLPH _ COUNTY STATION: 23+68.00 -L-

SHEET 7 OF 7

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-0" X 3'-3" PRESTRESSED CONCRETE BOX BEAM UNIT Greg Dickey

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SEAL 21271

COPY W. DILLIN

DocuSigned by:

8/22/2016 **REVISIONS** NO. BY: DATE: DATE:

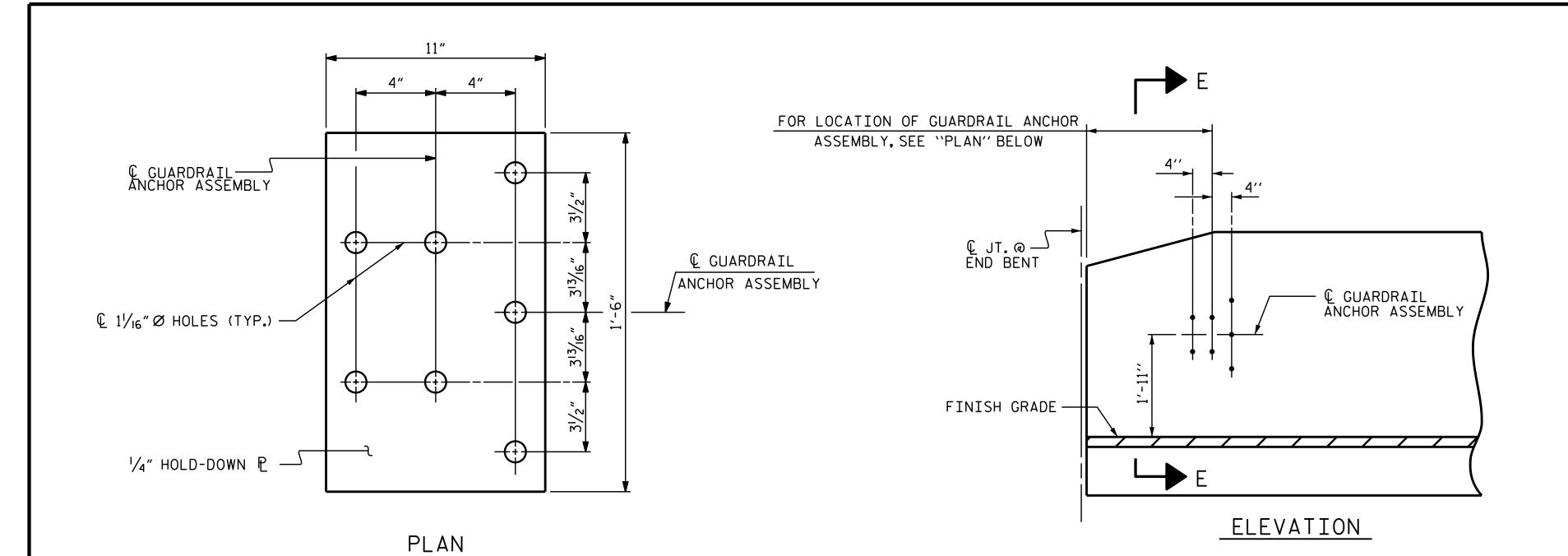
ASSEMBLED BY: A.K.PATEL DATE: 1/15/16 CHECKED BY: W.F.PARKER DATE: 3/16 REV. 4/15 MAA/TMG DRAWN BY : DGE IO/II CHECKED BY : TMG II/II

VERTICAL CONCRETE BARRIER RAIL DETAILS

SHEET NO

S-11

TOTAL SHEETS 38



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

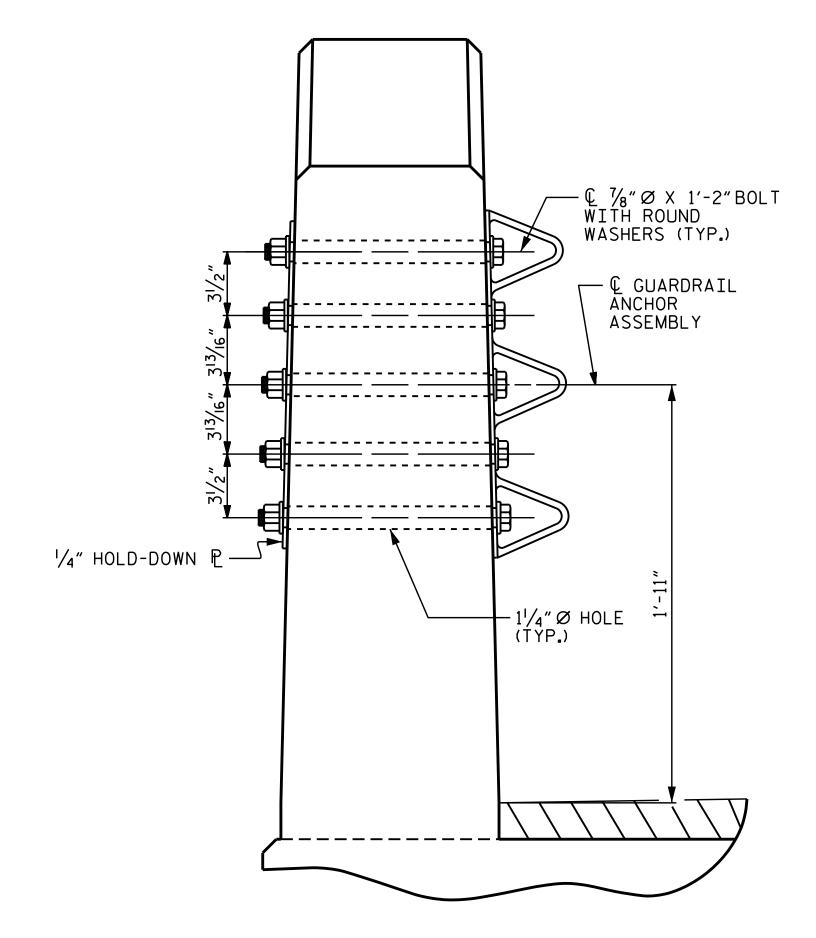
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

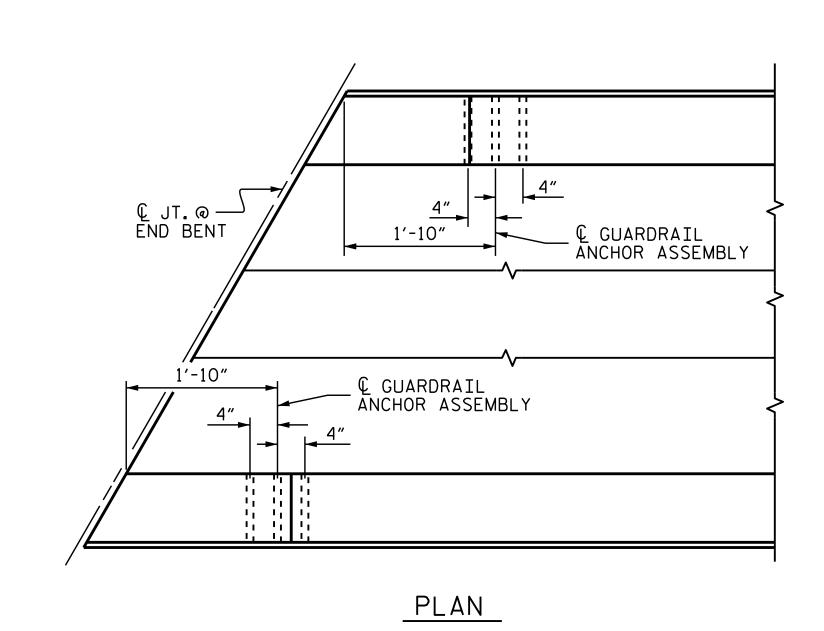


SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS

ASSEMBLED BY : A.K.PATEL CHECKED BY : W.F.PARKER

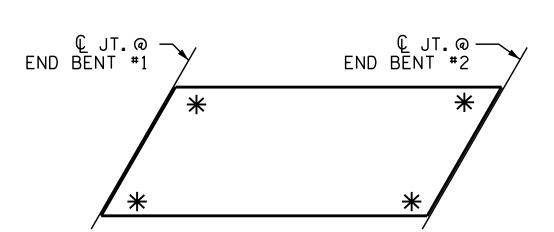
MAA/GM MAA/TMG

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-5360 RANDOLPH COUNTY STATION: 23+68.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE

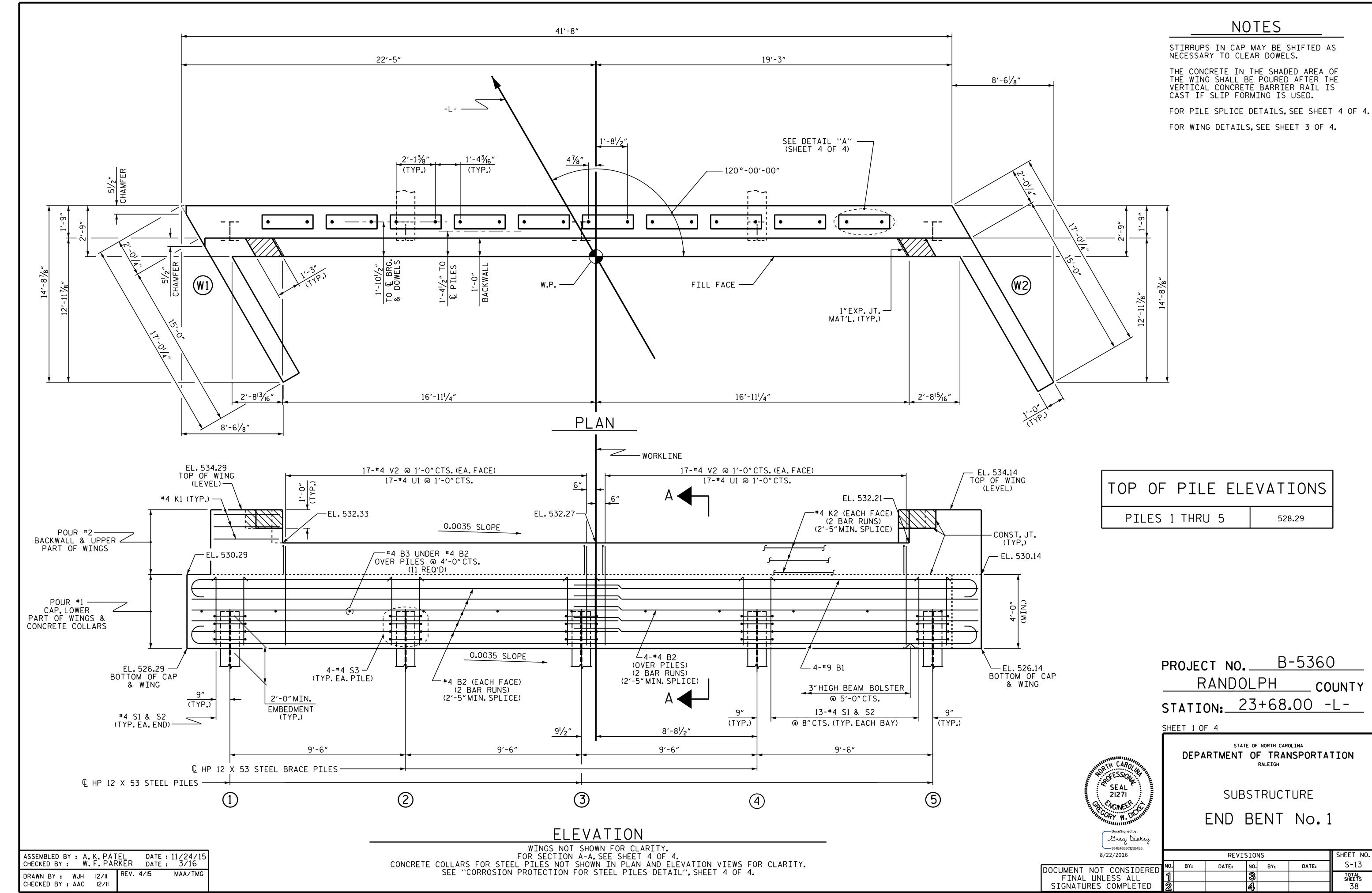
TOTAL SHEETS 38

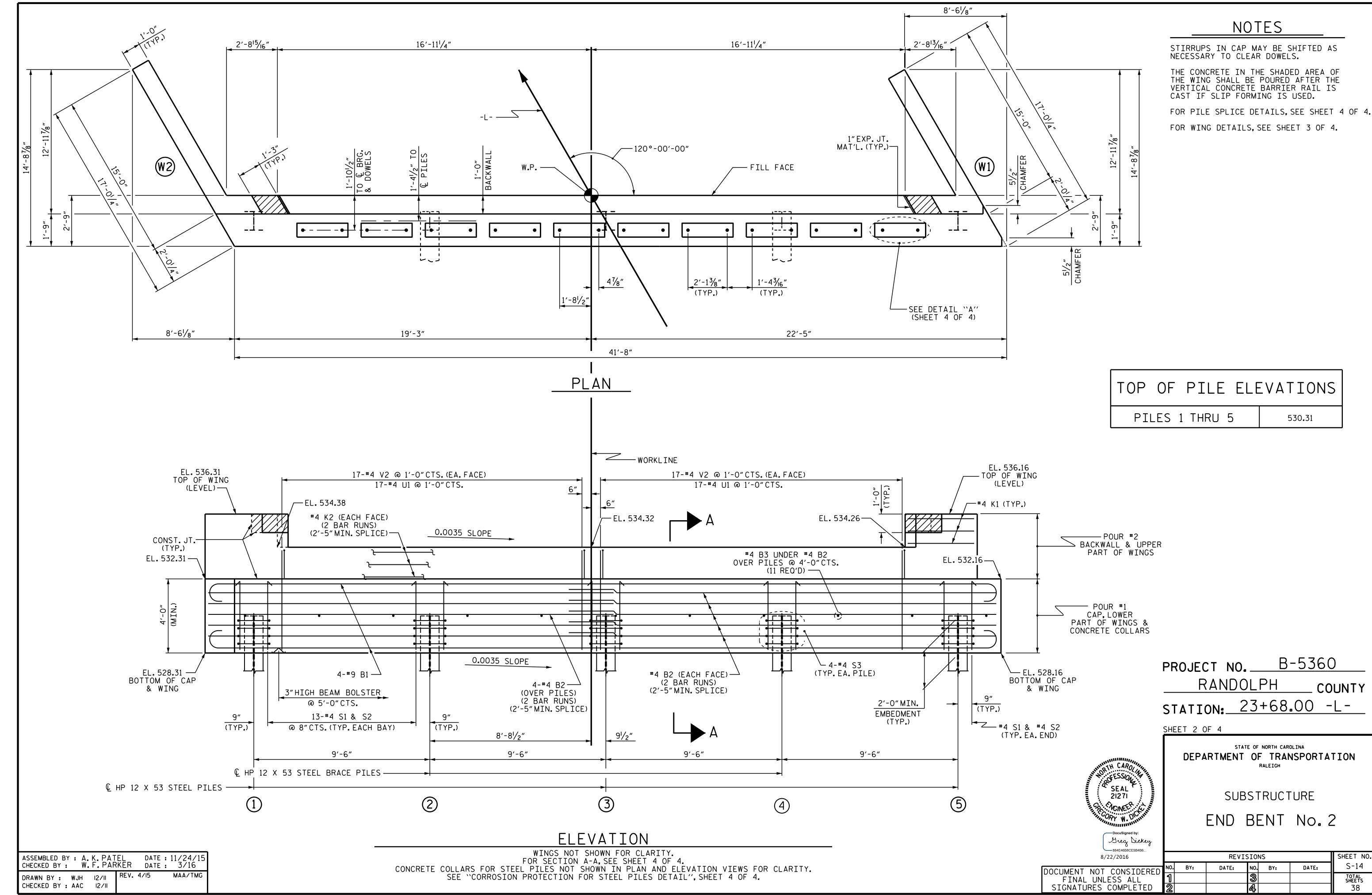
Greg Dickey 8/22/2016

BARRIER RAIL SHEET NO **REVISIONS** S-12 NO. BY: DATE:

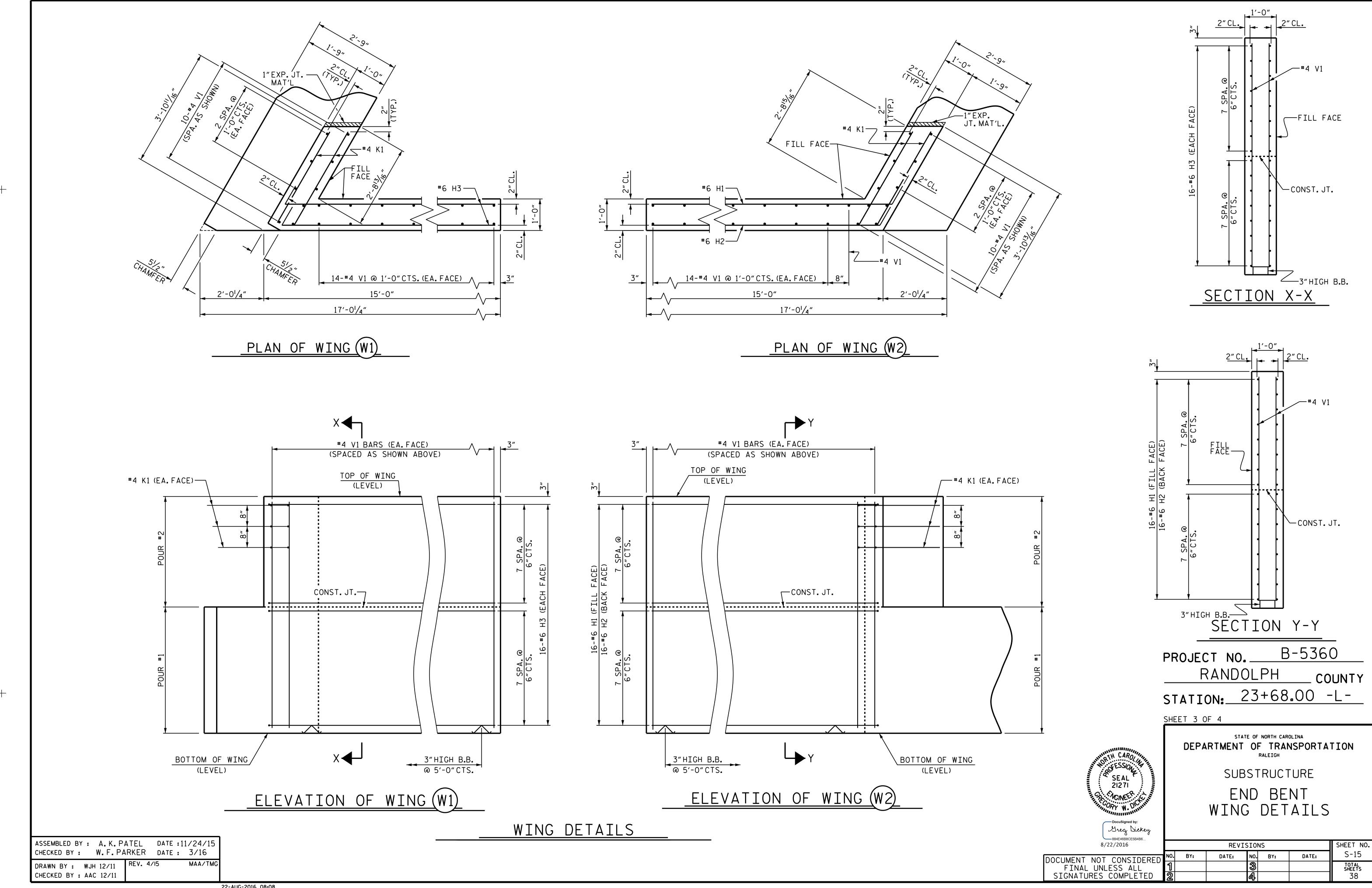
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

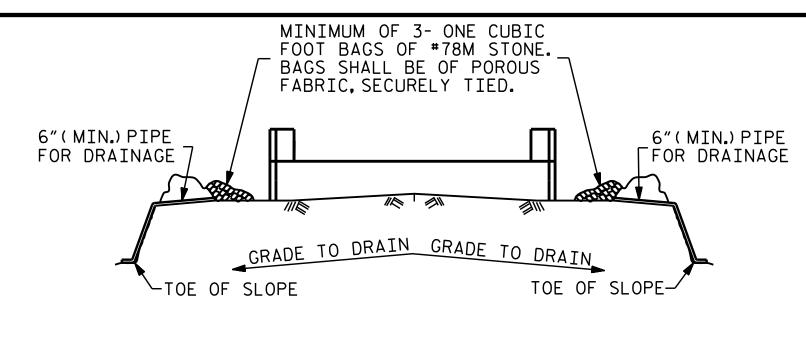
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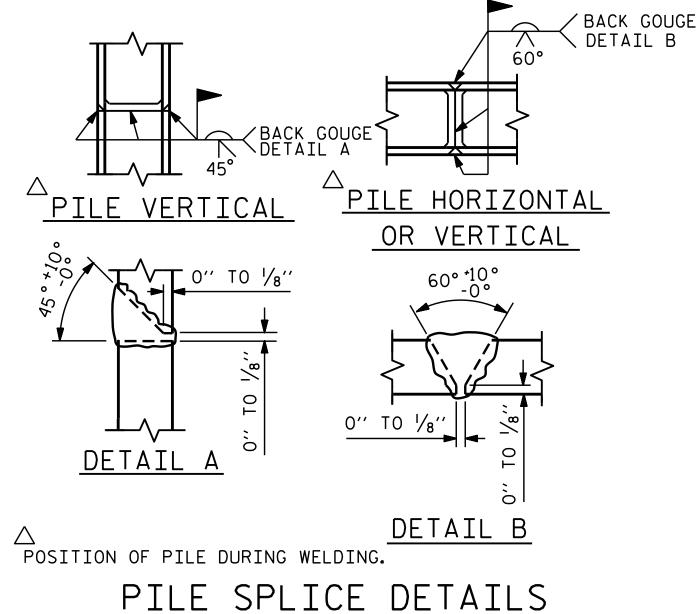


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



1-#4 B2— EA.FACE

FILL FACE—

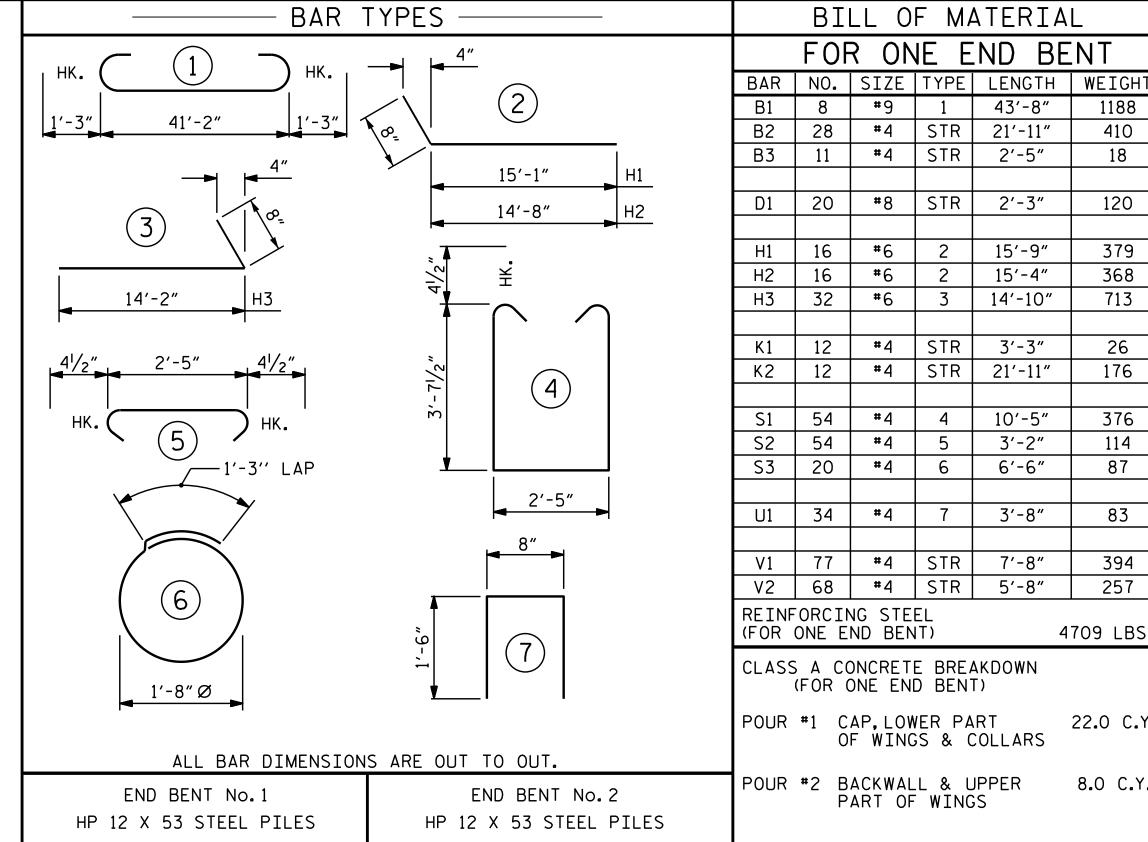
© HP 12 X 53

STEEL PILE—

2-**#**9 B1

2"CL.(TYP.)—

#4 B3 —



4709 LBS. CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT) POUR #1 CAP, LOWER PART 22.0 C.Y. OF WINGS & COLLARS POUR #2 BACKWALL & UPPER 8.0 C.Y. PART OF WINGS NO: 5 LIN. FT.= 175 NO: 5 LIN. FT.= 125 TOTAL CLASS A CONCRETE 30.0 C.Y. STEEL PILE POINTS STEEL PILE POINTS 5 EA. 5 EA. 1'-10¹/₂" 2" CL. | | | — € #8 D1 DOWEL #4 U1— 1-#4 K2 —— EA.FACE #4 V2-CONST. JT.r#4 S2 → 4-#9 B1

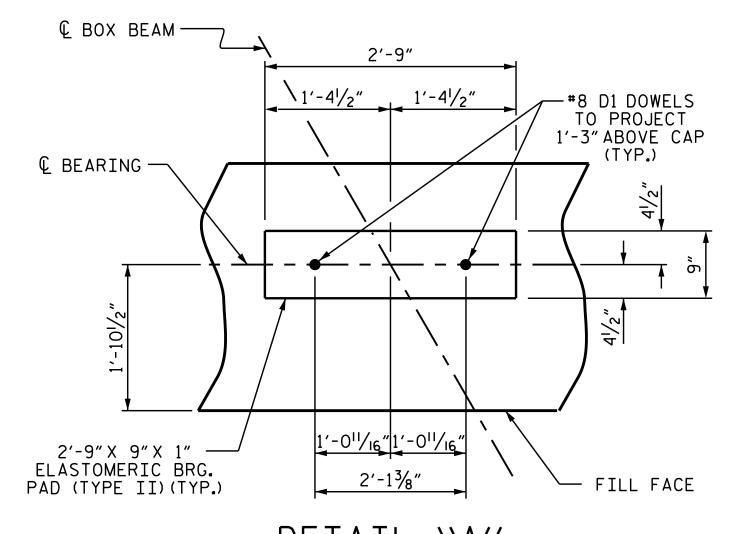
—4-#4 B2 @ 4" CTS.

OVER PILES

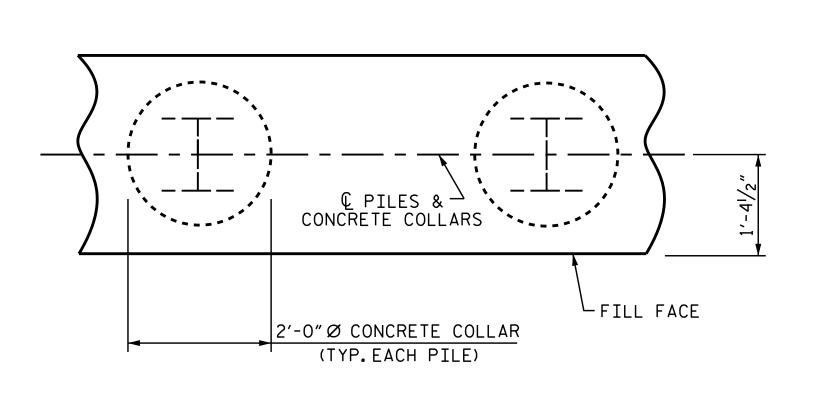
2-#9 B1

-3"HIGH B.B.

. Q HP 12 X 53 STEEL BRACE PILE



DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



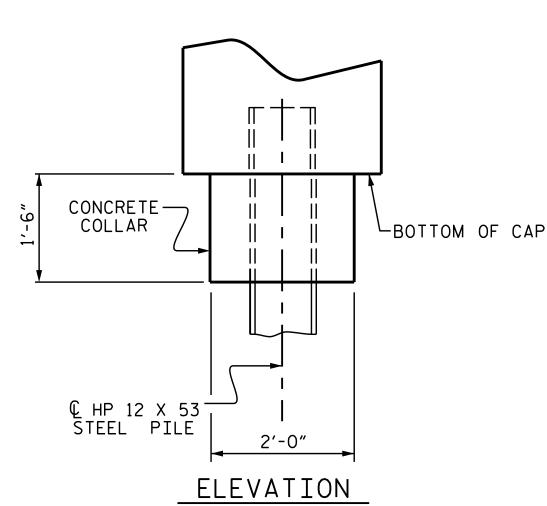
PLAN

CONCRETE-COLLAR © HP 12 X 53 T STEEL PILE 2'-0" ELEVATION

CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

ASSEMBLED BY: A.K.PATEL DATE: 11/24/15 CHECKED BY: W.F.PARKER DATE: 3/16 MAA/TMG REV. 8/14 DRAWN BY: WJH 12/11

CHECKED BY : AAC 12/11



SECTION A-A (CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

 $1'-4\frac{1}{2}"$ $1'-4\frac{1}{2}"$

2'-9"

* COFESSION SEAL 21271 NOINEER

Greg Dickey

RANDOLPH __ COUNTY 23+68.00 -L-STATION:_

B-5360

43′-8″

3 | 14'-10"

10′-5″

3'-2"

6'-6"

4 |

6

1188

410

18

120

379

368

713

26

176

376

114

87

83

394

257

SHEET 4 OF 4

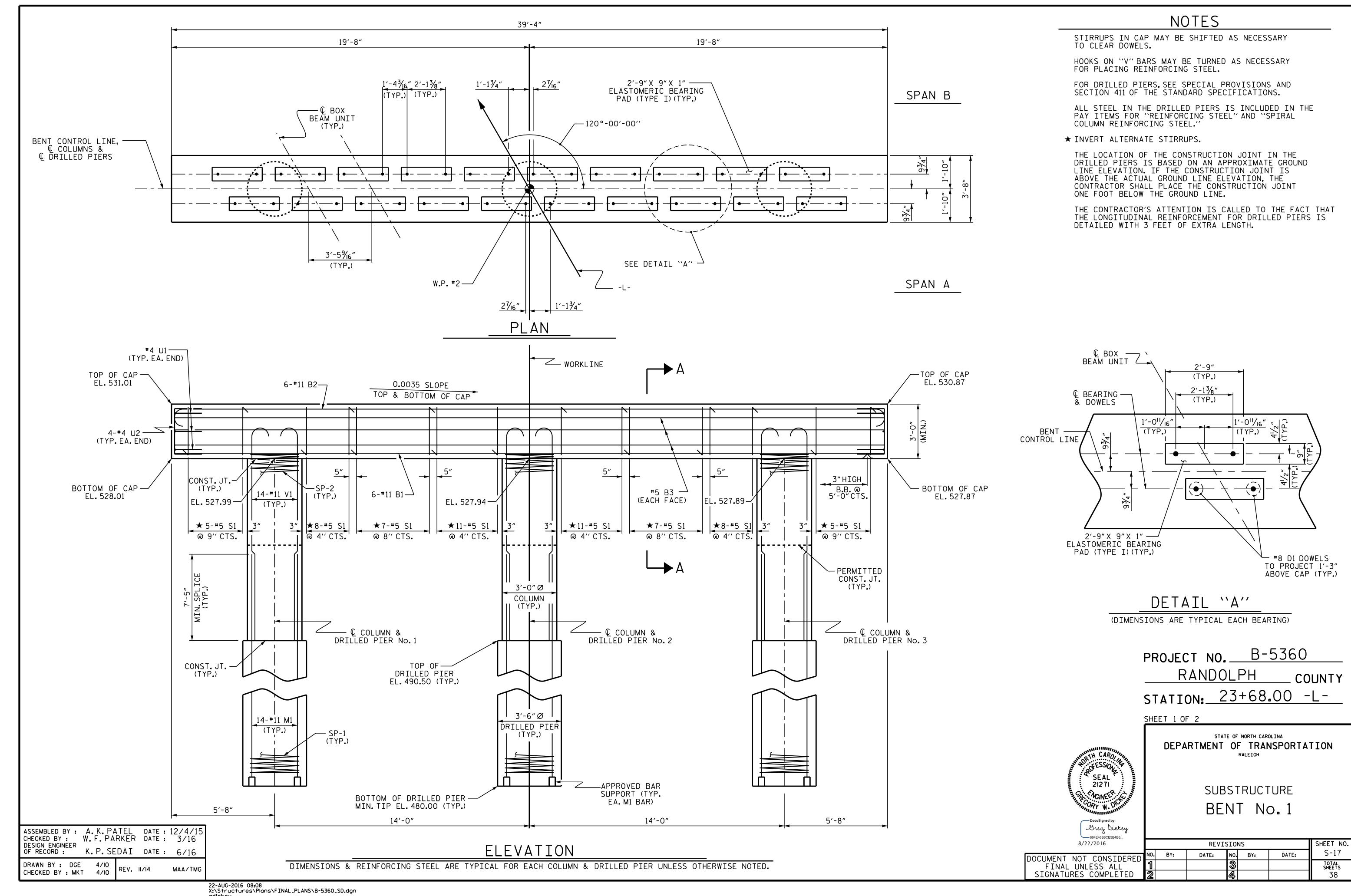
PROJECT NO. _

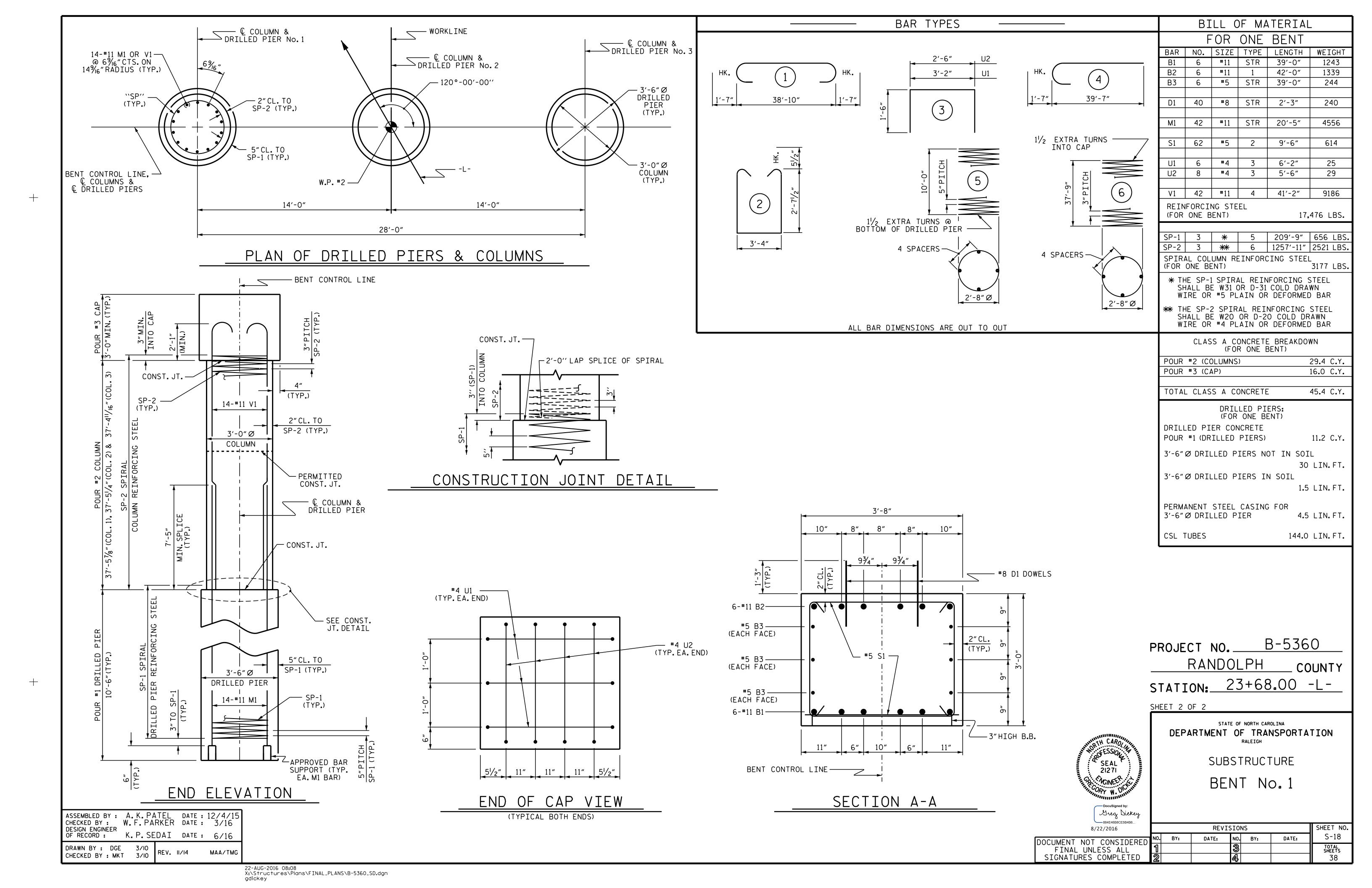
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

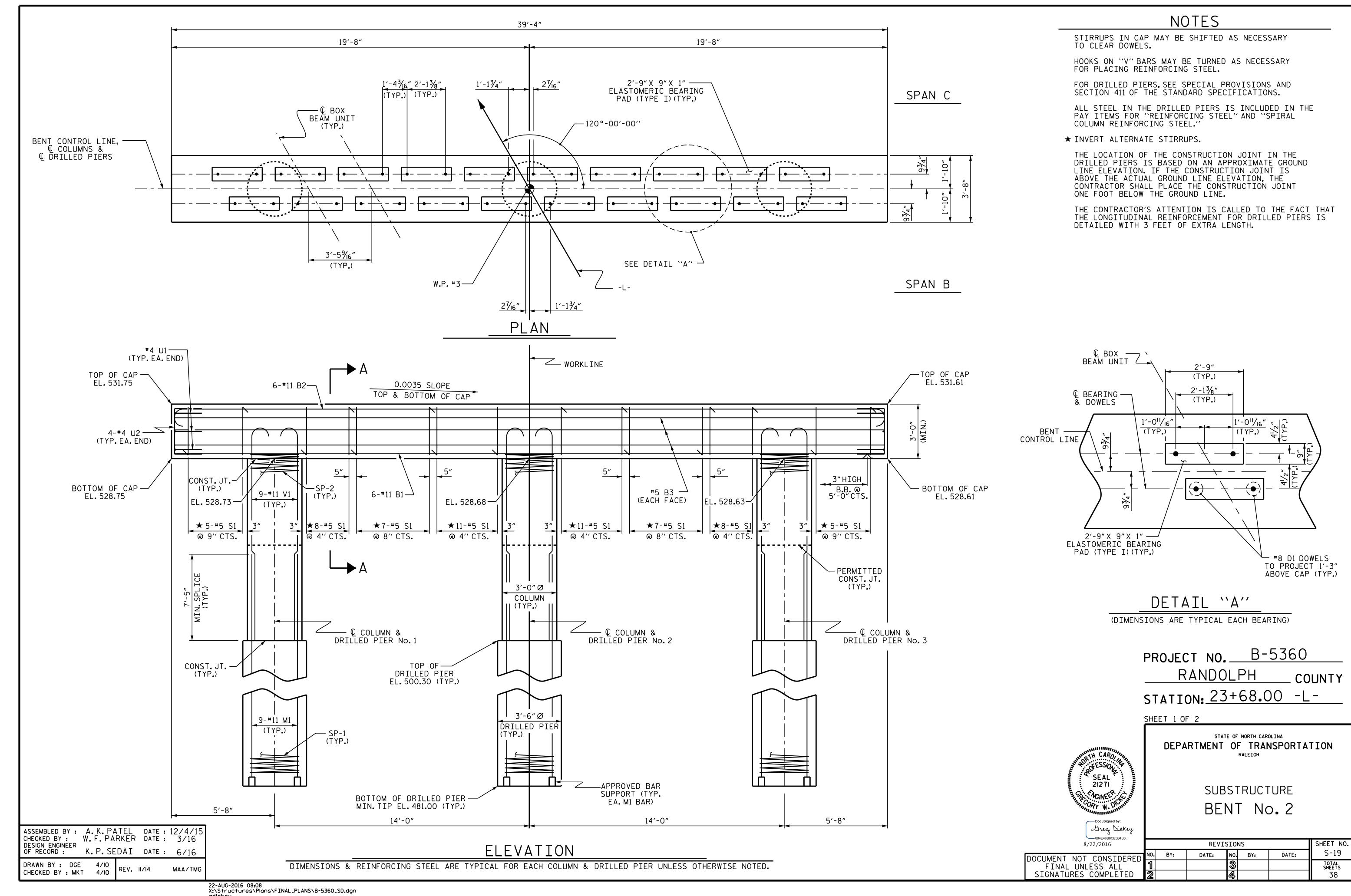
SUBSTRUCTURE

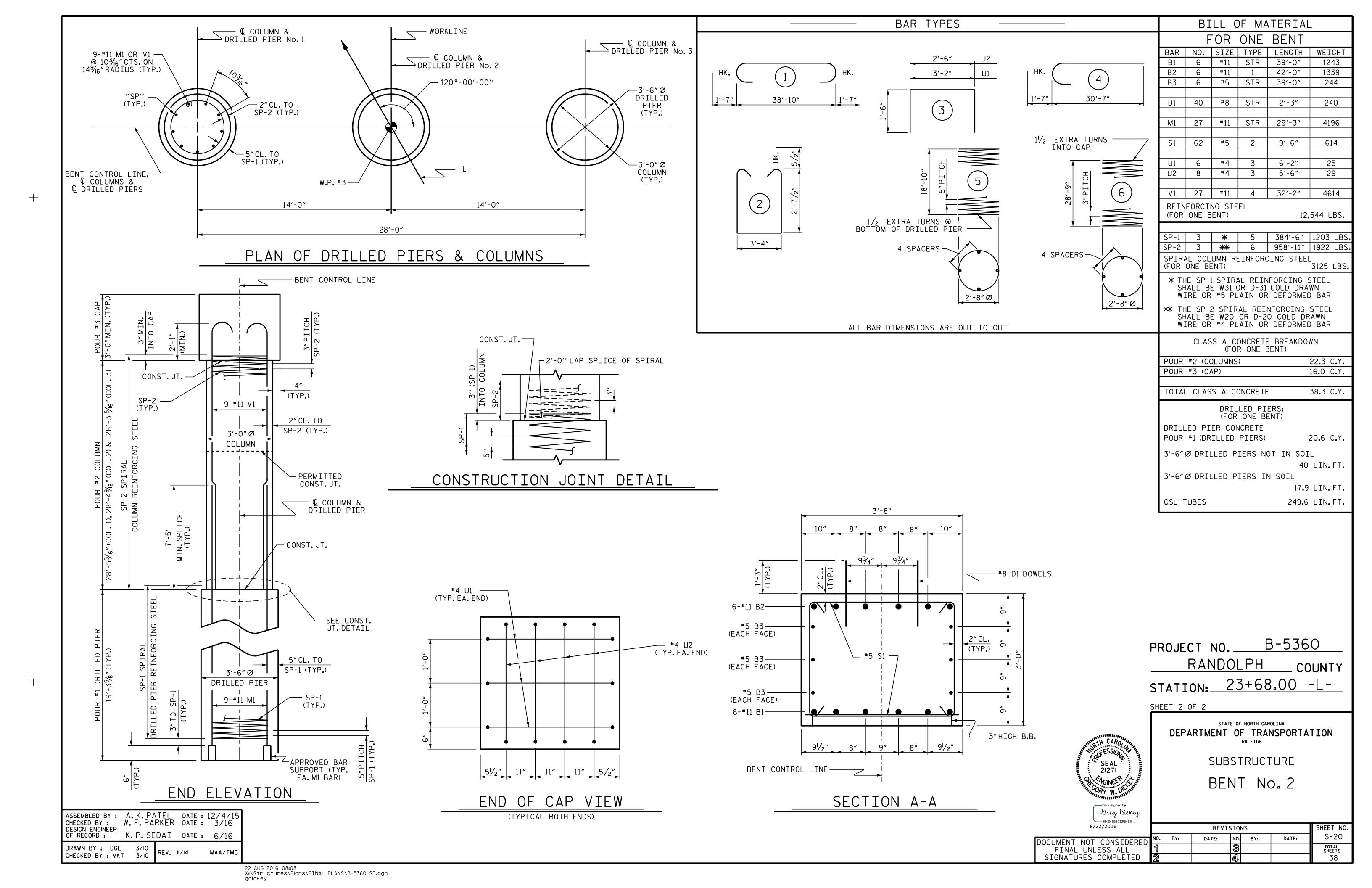
END BENT No.1 & 2 DETAILS

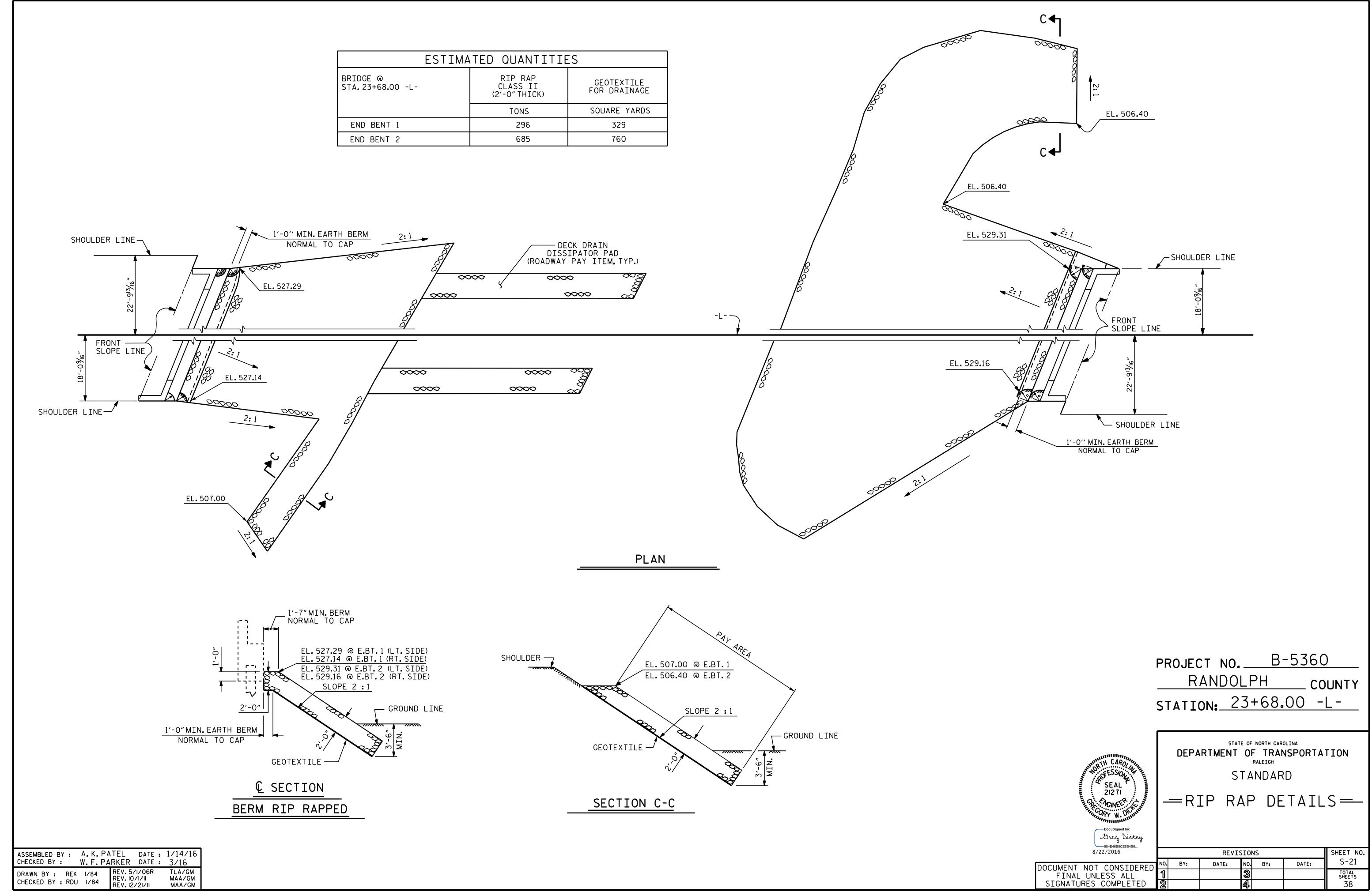
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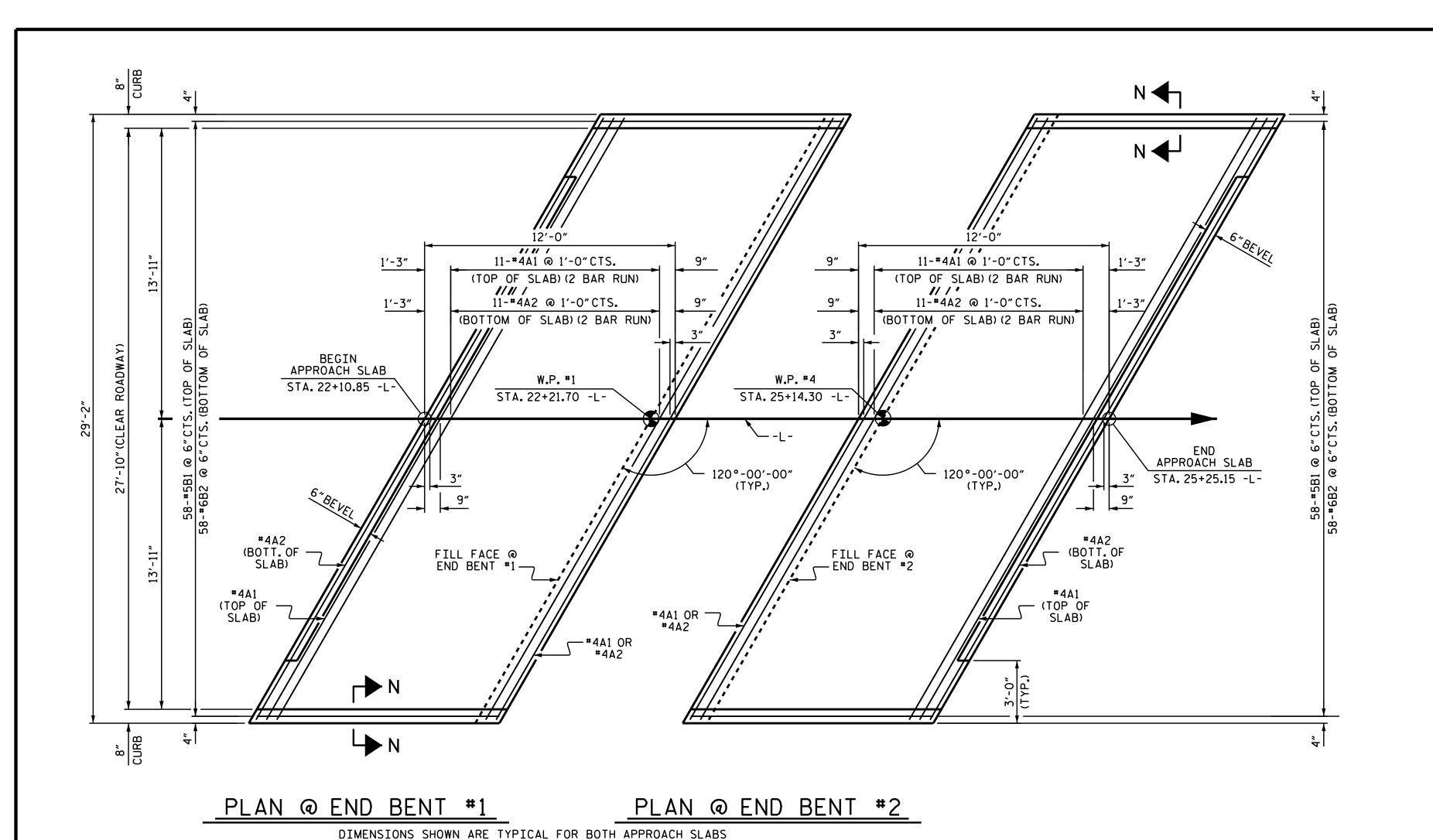


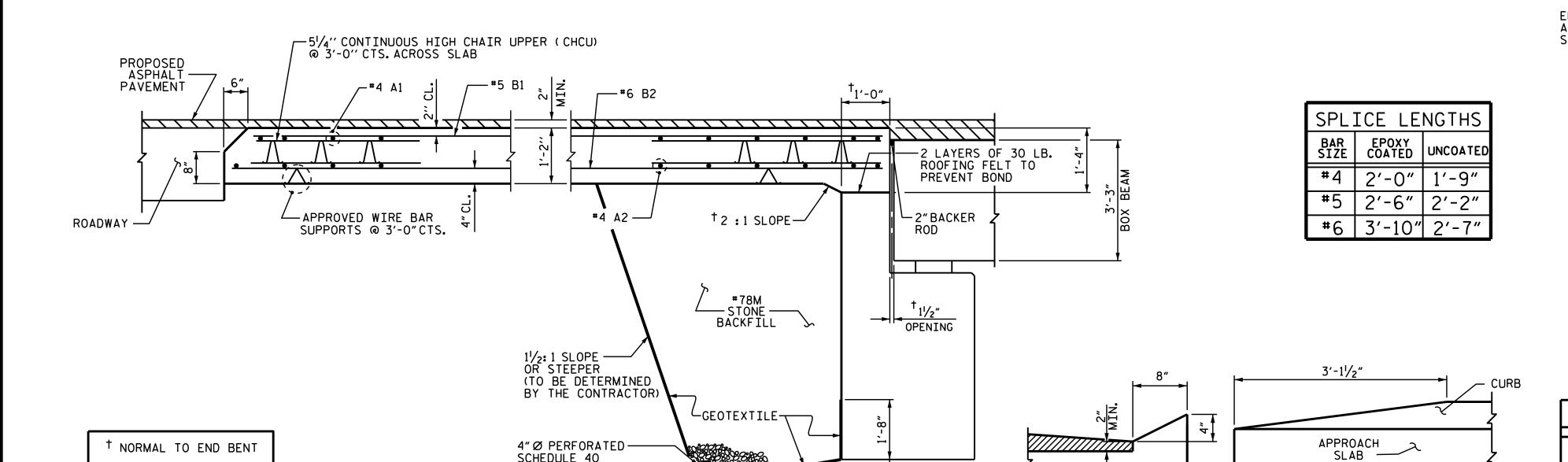












3'-0"

CURB DETAILS

END OF CURB WITHOUT SHOULDER BERM GUTTER

@ END BENT 2

SECTION N-N

SLAB

END OF CURB WITH

SHOULDER BERM GUTTER

@ END BENT 1

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND *78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

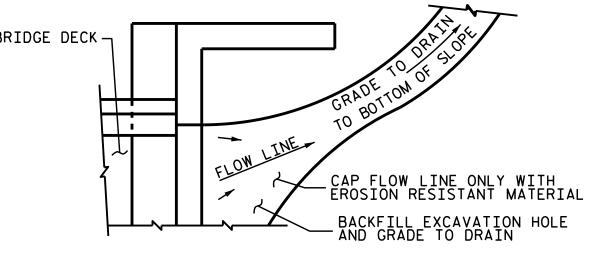
*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

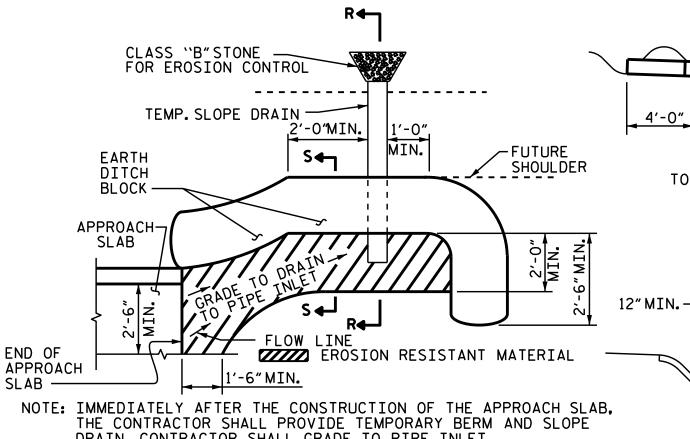
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

FOR JOINT DETAILS, SEE "PRESTRESSED CONCRETE BOX BEAM UNIT" SHEETS.



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

B-5360 PROJECT NO. _ RANDOLPH _ COUNTY 23+68.00 -L-STATION:_

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR NO. SIZE TYPE LENGTH WEIGHT

305

1009

1314

305

1009

LBS.

LBS.

C.Y.

LBS.

LBS.

C. Y.

* A1 | 26 | #4 | STR | 17'-8"

*B1 | 58 | #5 | STR | 11'-1"

* A1 | 26 | #4 | STR | 17'-8" A2 | 26 | #4 | STR | 17'-7"

*B1 | 58 | *5 | STR | 11'-1"

B2 | 58 | #6 | STR | 11'-7"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

REINFORCING STEEL

* EPOXY COATED

CLASS AA CONCRETE

TOE OF FILL—

CLASS "B"STONE —/
FOR EROSION CONTROL

3"EROSION RESISTANT MATERIAL OVER PIPE

-EARTH DITCH BLOCK

FILL SLOPE

SECTION R-R

4'-0" MIN.

SECTION S-S

REINFORCING STEEL

* EPOXY COATED

58 | #6 | STR | 11'-7"

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

A2 | 26 | #4 | STR | 17'-7"

OF ESSION SEAL 21271 CHOINEER

Greg Dickey

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE

> BOX BEAM UNIT (SUB-REGIONAL TIER) 120° SKEW

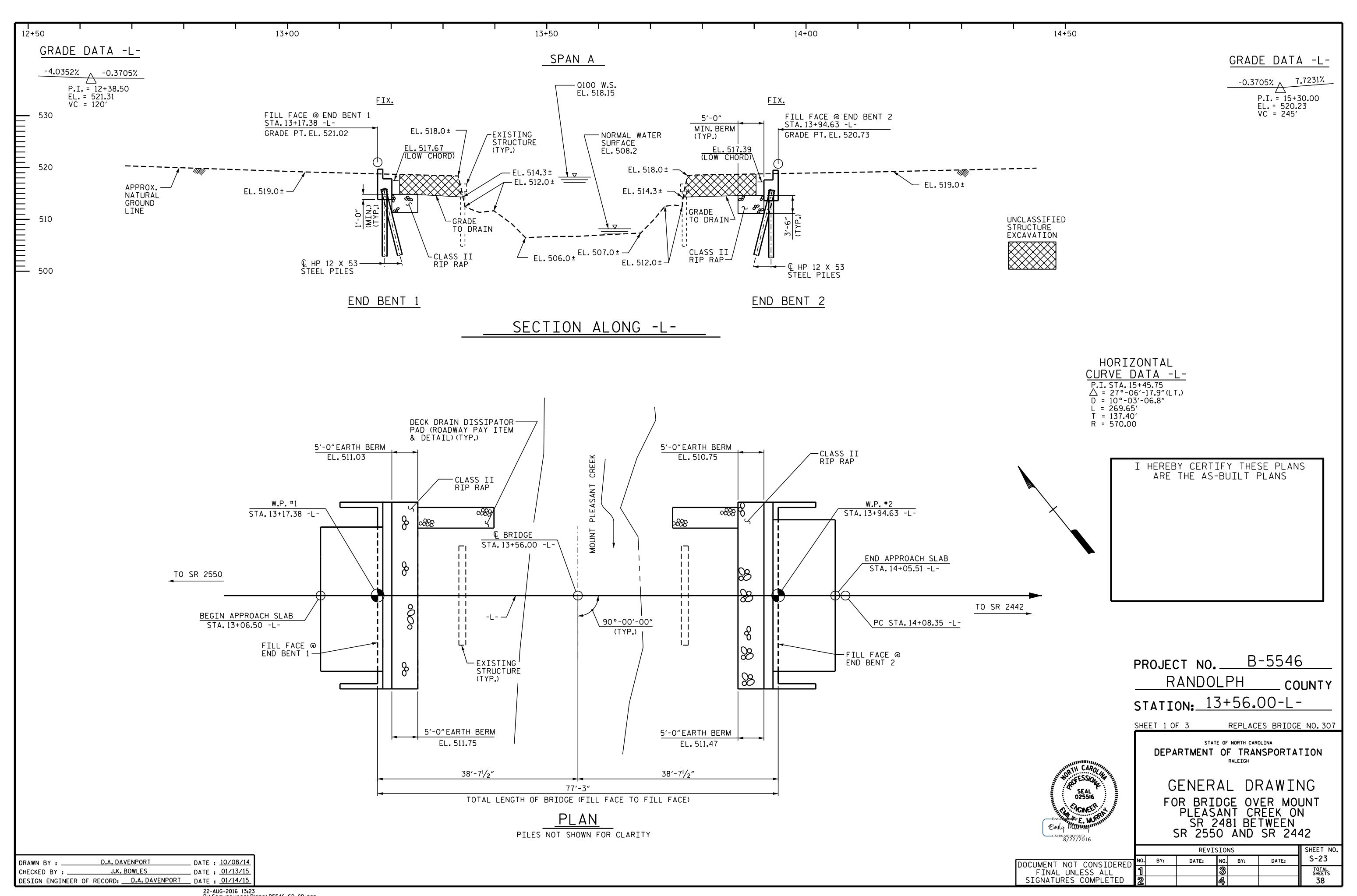
8/22/2016 SHEET NO **REVISIONS** S-22 DATE: DATE: BY: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL TOTAL SHEETS SIGNATURES COMPLETED

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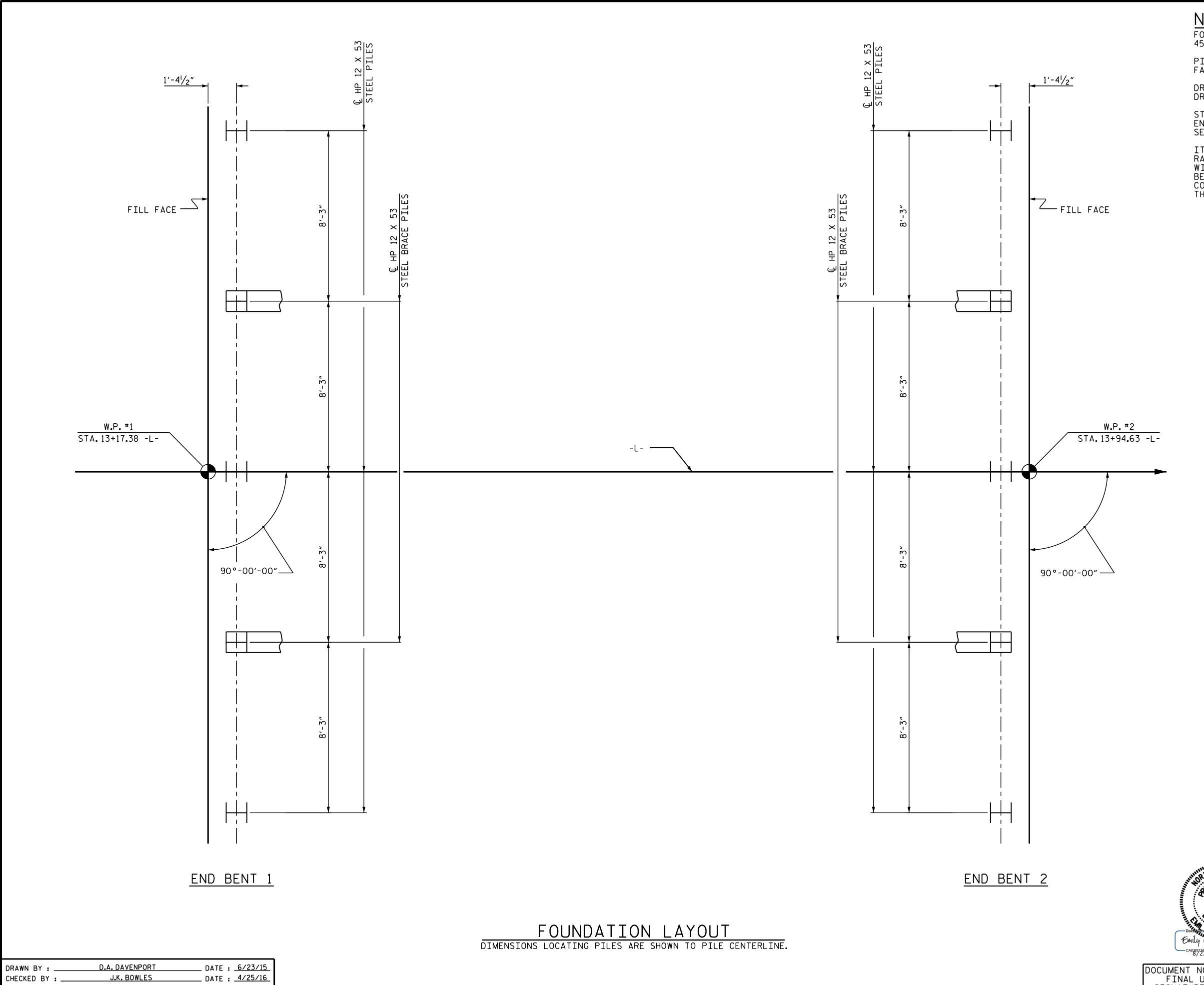
SECTION THRU SLAB

ASSEMBLED BY: A.K.PATEL DATE: 1/20/16 CHECKED BY: W.F.PARKER DATE: 3/16

DRAWN BY : MAA 11/11 CHECKED BY : AAC 11/11 SCHEDULE 40 PVC PIPE



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NOTES

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 114 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 190 TONS PER PILE.

STEEL H-PILE POINTS ARE REQUIRED FOR ALL STEEL H-PILES AT END BENT 1 AND END BENT 2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 30 TO 35 FT.-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT 1 AND END BENT 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING EQUIPMENT IN ACCORDANCE WITH THE PILE PROVISION.

> PROJECT NO. B-5546 RANDOLPH COUNTY

STATION: 13+56.00-L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

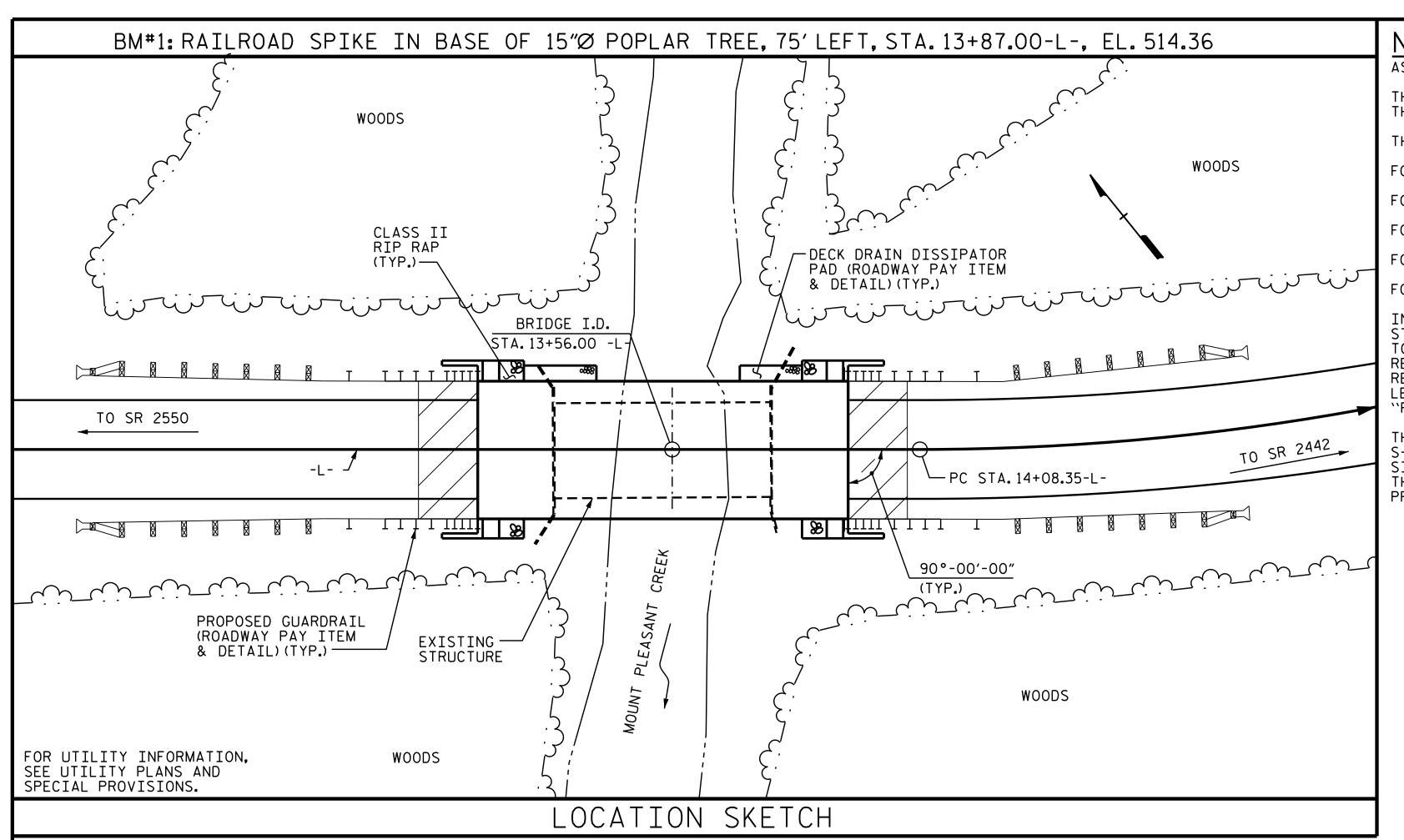
GENERAL DRAWING

FOR BRIDGE OVER MOUNT
PLEASANT CREEK ON
SR 2481 BETWEEN
SR 2550 AND SR 2442

SHEET NO. REVISIONS S-24 NO. BY: DATE: DATE: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 38

17-AUG-2016 09:57 R:\Structures\Plans\B5546_SD_GD.dgn jdhawk

J.K. BOWLES



NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK & FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+56.00-L-".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 44'-6", WITH A TIMBER DECK AND A CLEAR ROADWAY WIDTH OF 19'-1" WITH AN 1" ASPHALT WEARING SURFACE ON STEEL I-BEAMS, ON TIMBER CAPS AND PILES, ONE WHICH IS ENCASED IN CONCRETE AND TIMBER BULKHEADS, LOCATED AT THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS THE EXISTING END BENTS SHALL BE REMOVED DOWN TO EL.514.30 WITH THE REMAINING PORTION LEFT IN PLACE FOR BANK STABILIZATION.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

HYDRAULIC DATA

DESIGN DISCHARGE = 1,900 CFS FREQUENCY OF DESIGN FLOOD = 25 YR.

DESIGN HIGH WATER ELEVATION = 516.80 = 8.1 SQ. MI.

BASE DISCHARGE (Q 100) = 2,801 CFS = 518.15

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE =6,000 CFS FREQUENCY OF OVERTOPPING FLOOD =500+ YR. OVERTOPPING FLOOD ELEVATION =520.90

	TOTAL BILL OF MATERIAL															
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12X53 EL PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRI C	-O"X 2'-9" ESTRESSED CONCRETE OX BEAMS	ASBESTOS ASSESSMENT
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE										150.00				10	750.00	
END BENT NO.1				23.8		3,342	5	125	5		30	45				
END BENT NO.2				23.8		3,342	5	100	5		30	45				
TOTAL	LUMP SUM	1	LUMP SUM	47.6	LUMP SUM	6,684	10	225	10	150.00	60	90	LUMP SUM	10	750.00	LUMP SUM

PROJECT NO. B-5546

RANDOLPH COUNTY

STATION: 13+56.00-L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER MOUNT PLEASANT CREEK ON SR 2481 BETWEEN SR 2550 AND SR 2442

REVISIONS

SHEET NO.

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FINAL UNLESS ALL
SIGNATURES COMPLETED

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REVISIONS

SHEET NO.

BY:
DATE:
NO. BY:
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SHEETS
38

38

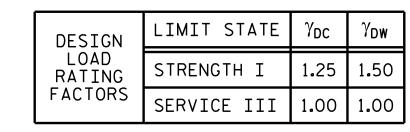
DRAWN BY: D.A. DAVENPORT DATE: 10/08/14
CHECKED BY: J.K. BOWLES DATE: 4/25/16

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE SHEAR MOMENT MOMENT DISTRIBUTION FACTORS (DF) CONTROLLING LOAD RATING OCA.)ISTRIBU1 ACTORS (LIVELOAD FACTORS LIVELOAD FACTORS DISTRIBU FACTORS (GIRDER DIST, LEFT SPAN 1.56 1.484 1.75 0.273 2.04 EL 36.75 0.507 1.48 36.75 HL-93(Inv)N/A 7.35 0.80 0.273 EL 2.028 1.35 0.273 EL 36.75 0.507 2.03 N/A 2.64 EL 7.35 N/A HL-93(0pr) DESIGN $\langle 2 \rangle$ LOAD 36.000 1.947 70.08 1.75 0.273 36.75 0.507 1.98 7.35 0.80 0.273 1.95 36.75 2.67 EL EL HS-20(Inv)RATING 2.561 92.205 1.35 0.273 36.75 0.507 2.56 36.000 3.46 EL 7.35 N/A HS-20(0pr) 13.500 4.403 59.442 0.273 7.55 36.75 0.507 5.89 0.273 4.40 36.75 7.35 0.80 1.4 EL EL SNSH 36.75 36.75 20.000 3.277 65.549 0.273 5.62 0.507 4.18 7.35 0.80 0.273 3.28 SNGARBS2 1.4 EL EL EL 0.507 68.248 0.273 36.75 3.88 36.75 22.000 3.102 1.4 5.32 0.80 0.273 3.10 SNAGRIS2 EL EL 7.35 EL 36.75 36.75 27.250 0.507 59.705 0.273 3.76 2.94 7.35 2.19 2.191 0.80 0.273 SNCOTTS3 EL 34.925 0.273 36.75 0.507 2.44 36.75 7.35 0.273 SNAGGRS4 1.83 63.896 3.14 EL EL 0.80 1.83 EL --36.75 0.507 0.273 35.550 63.605 0.273 2.47 36.75 1.789 3.07 7.35 1.79 SNS5A EL EL EL 39.950 65.556 0.273 36.75 0.507 2.25 7.35 1.64 36.75 SNS6A 1.641 2.81 EL 0.80 0.273 36.75 SNS7B 42.000 1.563 65.632 0.273 2.68 EL 36.75 0.507 2.21 7.35 0.273 1.56 LEGAL LOAD 33.000 2.001 66.029 0.273 36.75 0.507 2.68 7.35 0.80 0.273 2.00 36.75 TNAGRIT3 EL 3.43 RATING 0.273 33.075 66.465 0.273 36.75 0.507 7.35 36.75 EL 0.80 2.01 TNT4A 2.01 1.4 3.45 2.61 EL EL 68.325 0.273 2.82 36.75 0.507 2.35 7.35 1.64 36.75 41.600 1.642 EL EL TNT6A 1.4 EL 0.80 0.273 Α Α 0.507 42.000 1.65 69.313 0.273 2.83 EL 36.75 2.3 EL 7.35 0.273 1.65 EL 36.75 TNT7A 1.4 0.80 36.75 0.507 36.75 TNT7B 42.000 1.706 71.672 0.273 2.93 EL 2.16 Α EL 7.35 0.80 0.273 1.71 EL TNAGRIT4 43.000 1.624 69.831 0.273 2.78 EL 36.75 0.507 2.09 7.35 0.80 0.273 1.62 36.75 36.75 0.273 0.507 2.08 TNAGT5A 45.000 2.63 36.75 0.273 1.53

36.75 0.507

1.99

LOAD FACTORS:



NOTES:

1.51

36.75

0.273

7.35

0.80

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5546 RANDOLPH COUNTY STATION: 13+56.00-L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD LRFR SUMMARY FOR 75' BOX BEAM UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

DATE: BY:

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. REVISIONS S-26 NO. BY: DATE: TOTAL SHEETS 38

ASSEMBLED BY: D.A. DAVENPORT DATE: 10/08/14 CHECKED BY: J.K. BOWLES DATE: 01/20/15

DRAWN BY : TMG II/II CHECKED BY : AAC II/II

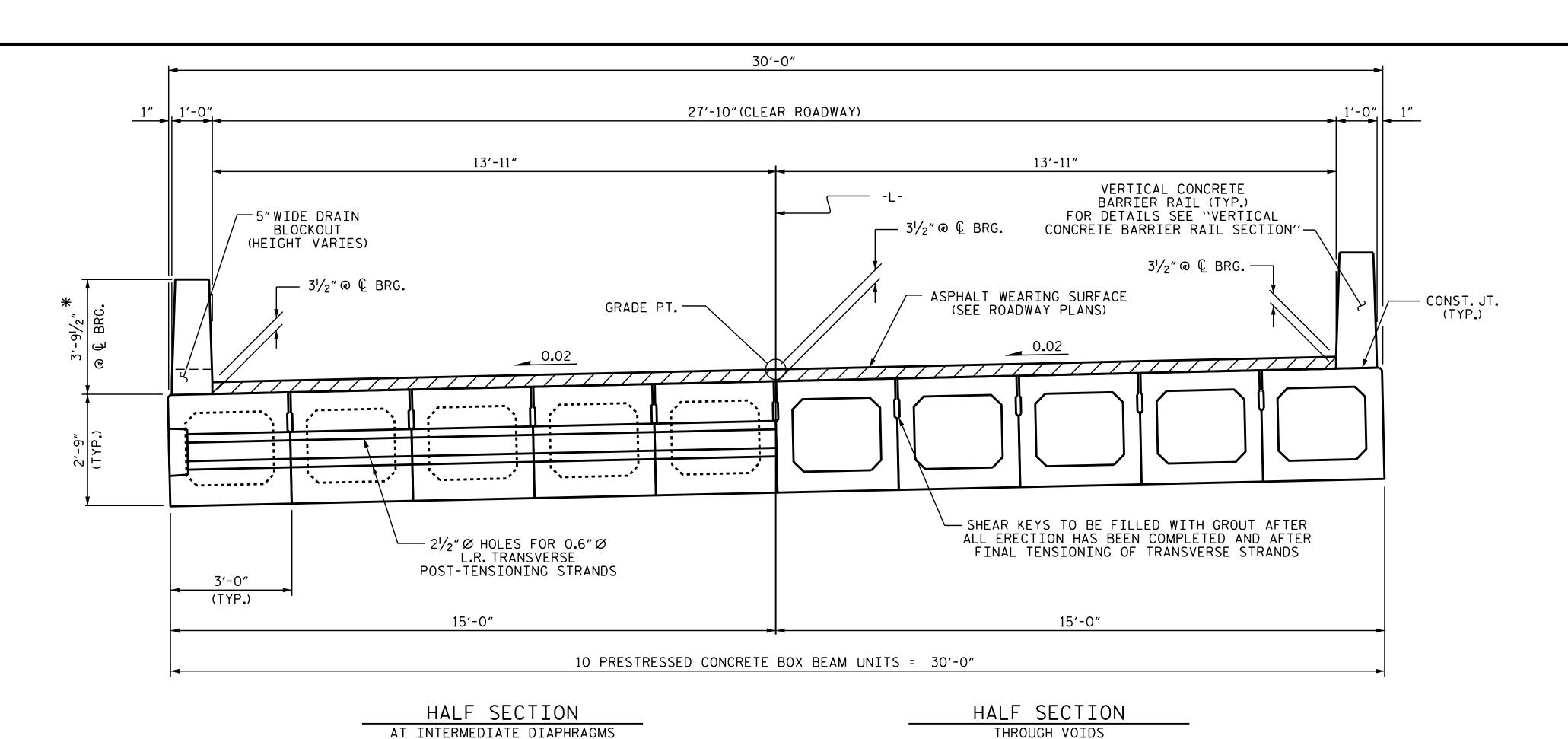
TNAGT5B

45.000

〈3〉 | 1.513 | 68.095 | 1.4

0.273 2.59

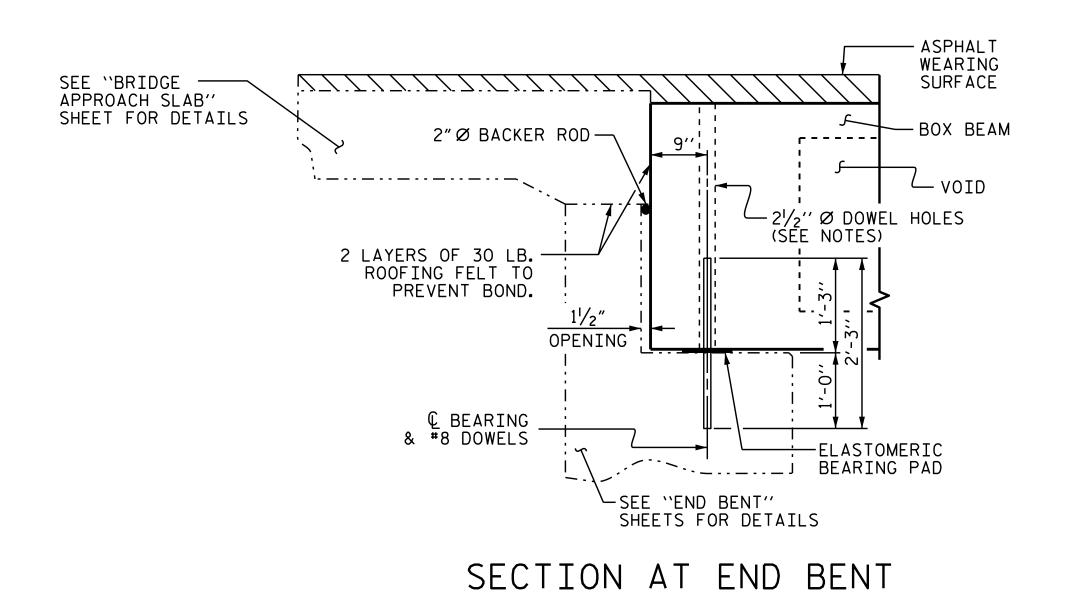
LRFR SUMMARY



TYPICAL SECTION

*THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

FIXED END



PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.

THREADED INSERT DETAIL

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ \emptyset DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 5". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE BOX BEAM UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR BOX BEAM UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

> PROJECT NO. B-5546 RANDOLPH COUNTY STATION: 13+56.00-L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

8/22/2016 BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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SHEET NO. REVISIONS S-27 DATE: DATE: BY: TOTAL SHEETS 38

ASSEMBLED BY : D.A. DAVENPORT DATE :08/04/14

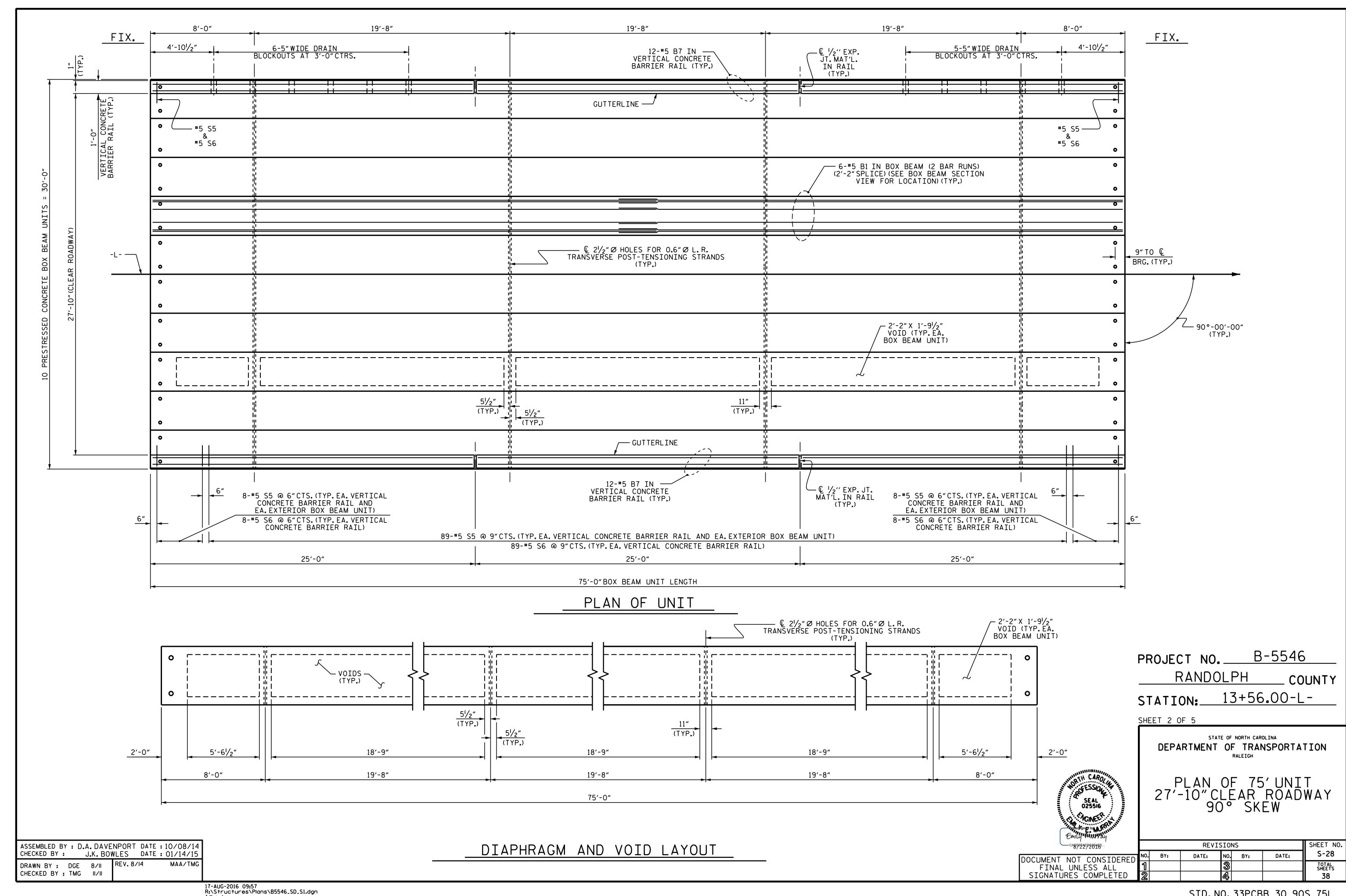
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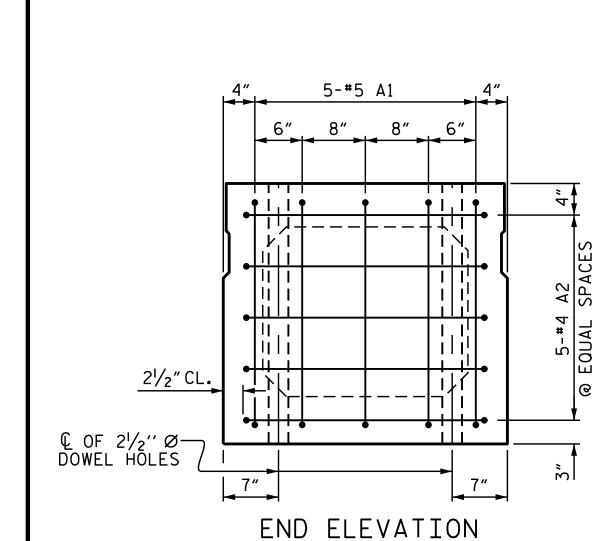
DATE :01/13/15

MAA/TMG

CHECKED BY : J.K. BOWLES

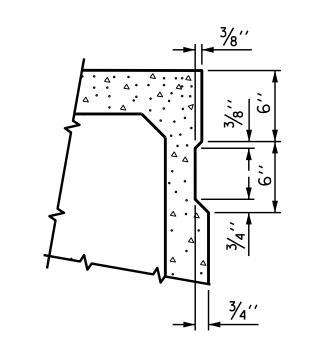
DRAWN BY : DGE 8/II CHECKED BY : TMG II/II





SHOWING PLACEMENT OF #5 & #4 "A" BARS AND LOCATION OF DOWEL HOLES. (INTERIOR BOX BEAM SECTION SHOWN-EXTERIOR SECTION SIMILAR EXCEPT SHEAR KEY LOCATION.

STRAND LAYOUT NOT SHOWN.)

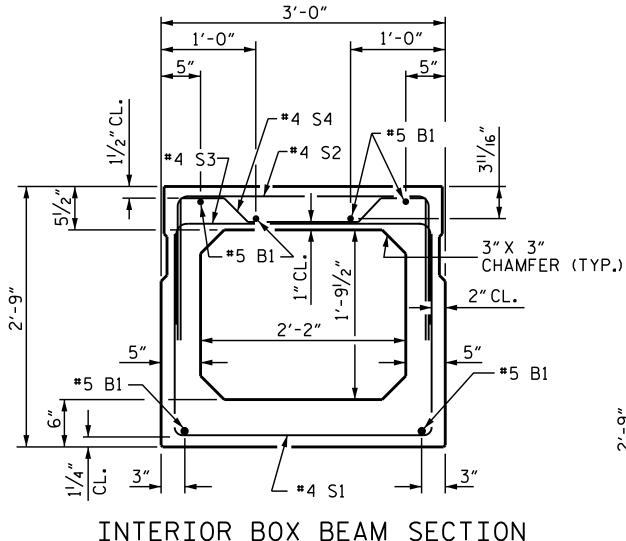


SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.

ASSEMBLED BY : D.A. DAVENPORT DATE :10/08/14 CHECKED BY : J.K. BOWLES DATE :01/14/15

DRAWN BY : DGE IO/II CHECKED BY : TMG II/II

MAA/TMG



INTERIOR BOX BEAM SECTION (STRAND LAYOUT NOT SHOWN)

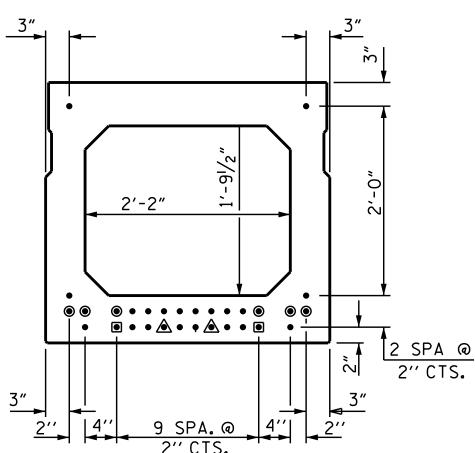
3'-0" $\frac{3\%"}{CL.}$ #5 S5 — #4 S2-#4 S47 #4 S37 — #5 B1 3" X 3" - CHAMFER (TYP.) ∠ #4 S1

EXTERIOR BOX BEAM SECTION

(STRAND LAYOUT NOT SHOWN)

GRADE 270 STRANDS 0.6" Ø L.R. 0.217 (SQUARE INCHES) ULTIMATE STRENGTH (LBS.PER STRAND) 58,600 APPLIED PRESTRESS 43,950 (LBS.PER STRAND)

O.6" Ø LOW RELAXATION STRAND LAYOUT

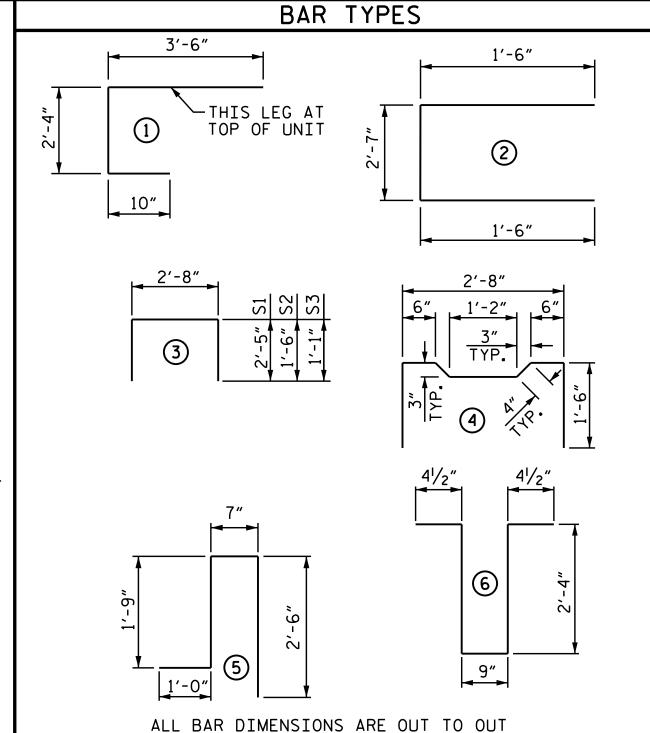


TYPICAL STRAND LOCATION (24 STRANDS REQUIRED) DEBONDING LEGEND

- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 4'-0"FROM END OF GIRDER
- STRANDS DEBONDED FOR 10'-0"FROM END OF GIRDER

OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE BOX BEAM UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL

BOND SHALL BE BROKEN ON STRANDS AS SHOWN FOR THE SPECIFIED LENGTH FROM EACH END OF THE BOX BEAM. SEE STANDARD SPECIFICATIONS ARTICLE 1078-7.



BILL OF MATERIAL FOR ONE BOX BEAM SECTION											
				EXTERI	OR UNIT	INTERIO	OR UNIT				
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT				
A1	10	# 5	1	6′-8″	70	6′-8″	70				
A2	34	#4	2	5′-7″	127	5′-7″	127				
B1	12	# 5	STR	38′-5″	481	38′-5″	481				
K1	12	#4	6	6′-2″	49	6′-2″	49				
K2	8	#4	STR	2'-7"	14	2'-7"	14				
S1	63	#4	3	7′-6″	316	7′-6″	316				
S2	63	#4	3	5′-8″	238	5′-8″	238				
S3	107	#4	3	4'-10"	345	4'-10"	345				
S4	44	#4	4	5′-10″	171	5′-10″	171				
* S5	105	# 5	5	5′-10″	639						
REINFO	RCING	STEEL		1811	LBS.	18	11 LBS.				

639

No. 24

13.4 CU. YDS.

LBS.

13.3 CU. YDS

No. 24

* EPOXY COATED REINF. STEEL

8000 P.S.I. CONCRETE

0.6" Ø L.R. STRANDS

SEAL 025516

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8/22/2016

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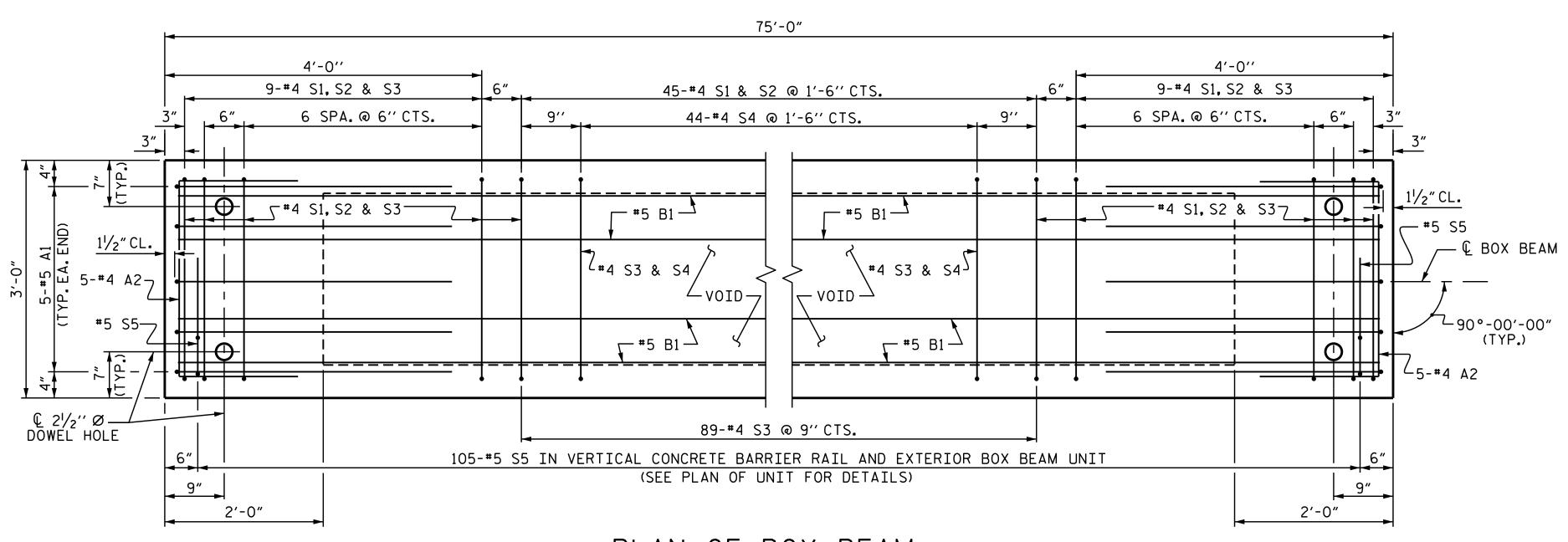
PROJECT NO. B-5546 RANDOLPH COUNTY STATION: 13+56.00-L-SHEET 3 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> STANDARD 3'-0" X 2'-9"

PRESTRESSED CONCRETE BOX BEAM UNIT

		SHEET NO.				
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1			3			TOTAL SHEETS
2			4			38



PLAN OF BOX BEAM

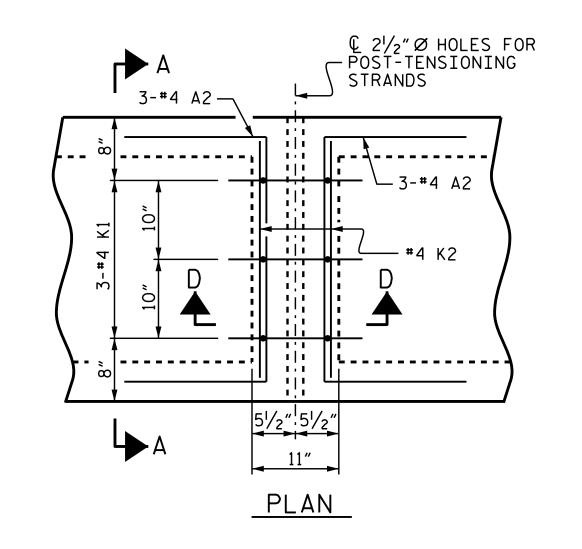
EXTERIOR UNIT SHOWN, INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S5 BARS.

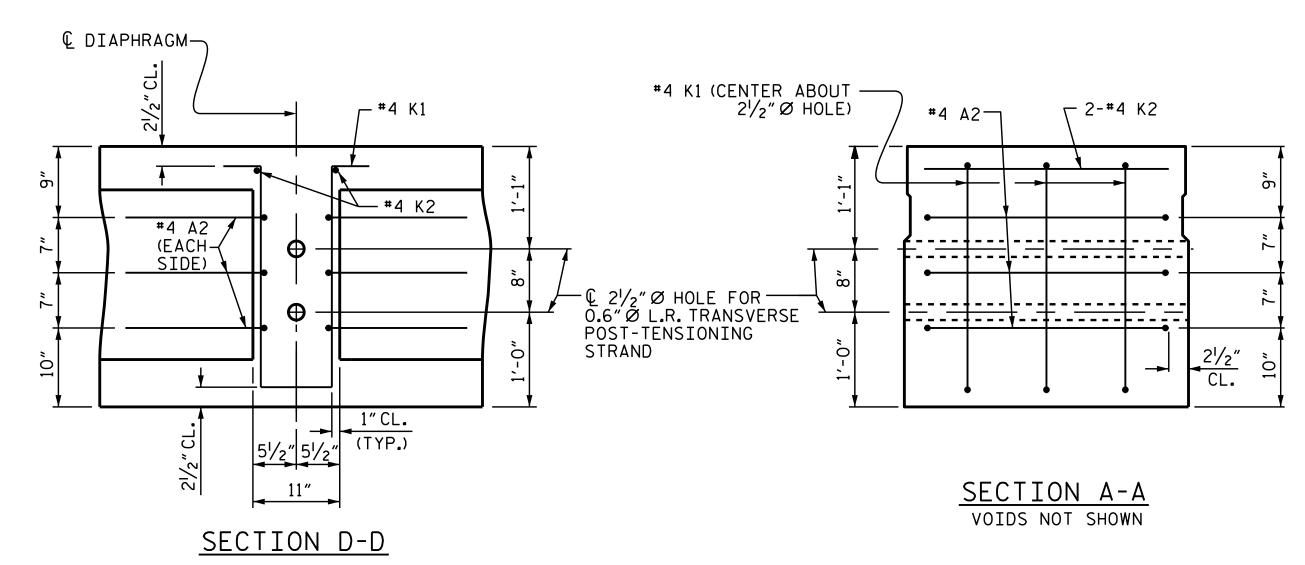
FOR LOCATION OF DIAPHRAGMS, SEE "PLAN OF UNIT".

FOR THREADED INSERTS, SEE "THREADED INSERT DETAIL".

FOR REINFORCING STEEL IN DIAPHRAGMS, SEE "DOUBLE DIAPHRAGM DETAILS".

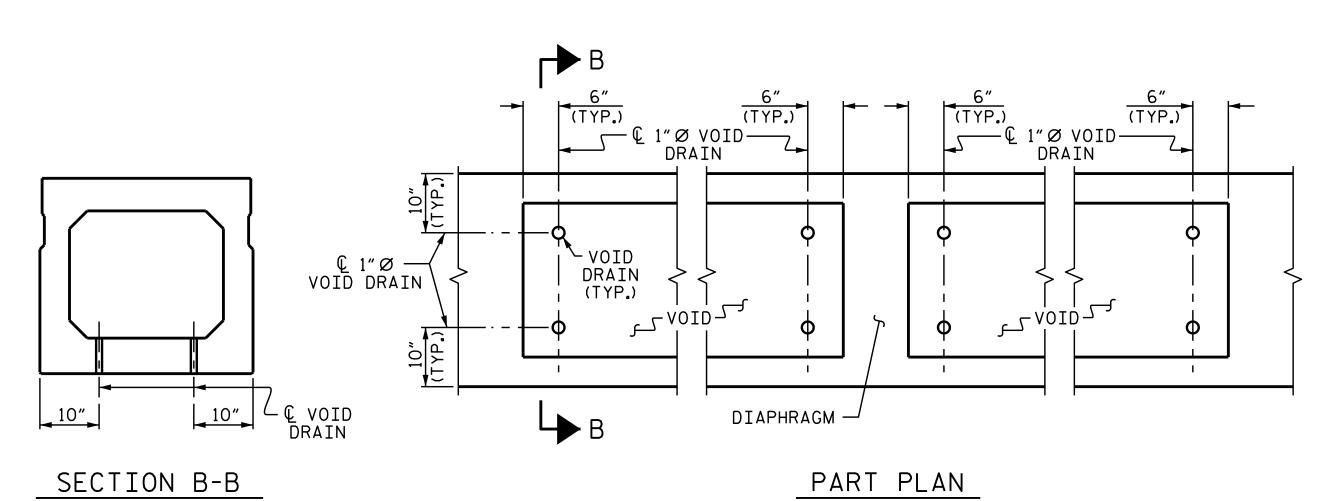
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DOUBLE DIAPHRAGM DETAILS

#4 "S" BARS NOT SHOWN. #4 "S" BARS MAY BE SHIFTED SLIGHTLY TO CLEAR 21/2" Ø HOLE.



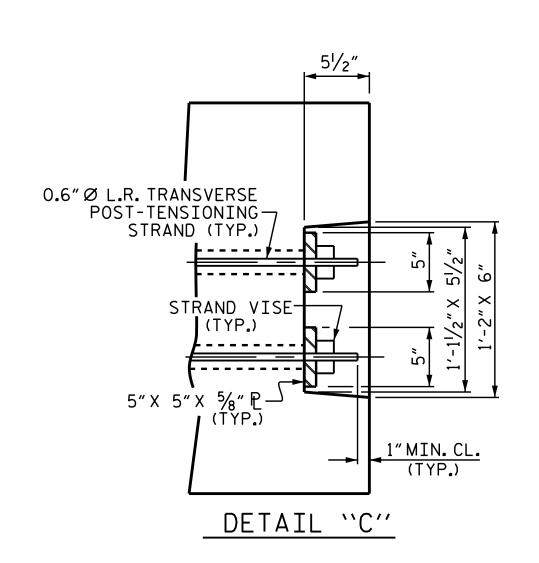
VOID DRAIN DETAILS

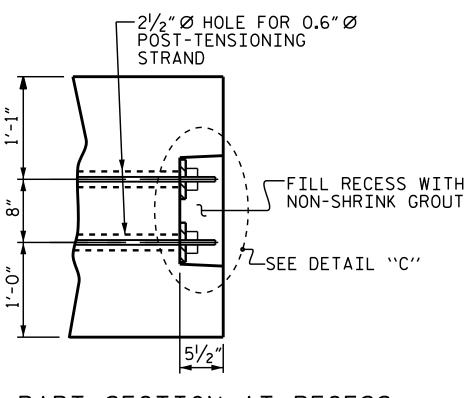
(DIMENSIONS SHOWN ARE TYPICAL FOR EACH VOID)

ASSEMBLED BY : D.A. DAVENPORT DATE :10/08/14 CHECKED BY : J.K. BOWLES DATE :01/14/15

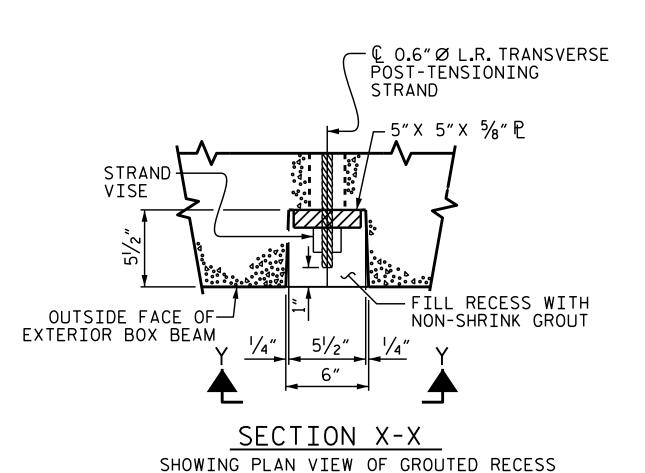
DRAWN BY : DGE IO/II REV. 8/I4 MAA/TMG CHECKED BY : TMG II/II

SHOWING ELEVATION VIEW OF GROUTED RECESS





PART SECTION AT RECESS



GROUTED RECESS DETAIL AT END OF POST-TENSIONED STRANDS
OF EXTERIOR BOX BEAM

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-9"
75'BOX BEAM UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1¾″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	¹/₂′′ ♦
FINAL CAMBER	11/4"

** INCLUDES FUTURE WEARING SURFACE

PROJECT NO. B-5546

RANDOLPH COUNTY

STATION: 13+56.00-L-

SHEET 4 OF 5

SEAL 025516

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CONECTION

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DEPARTMENT OF TRANSPORTATION

STANDARD

3'-0" X 2'-9"

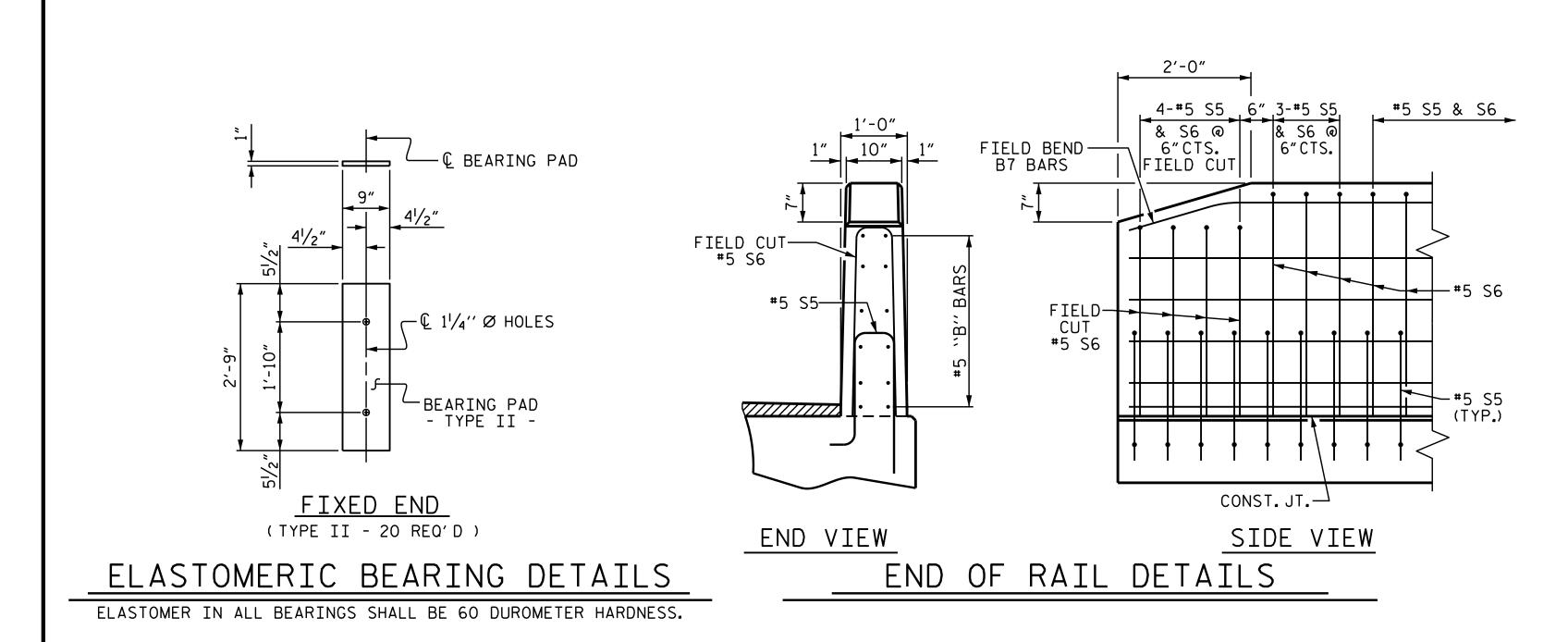
PRESTRESSED CONCRETE

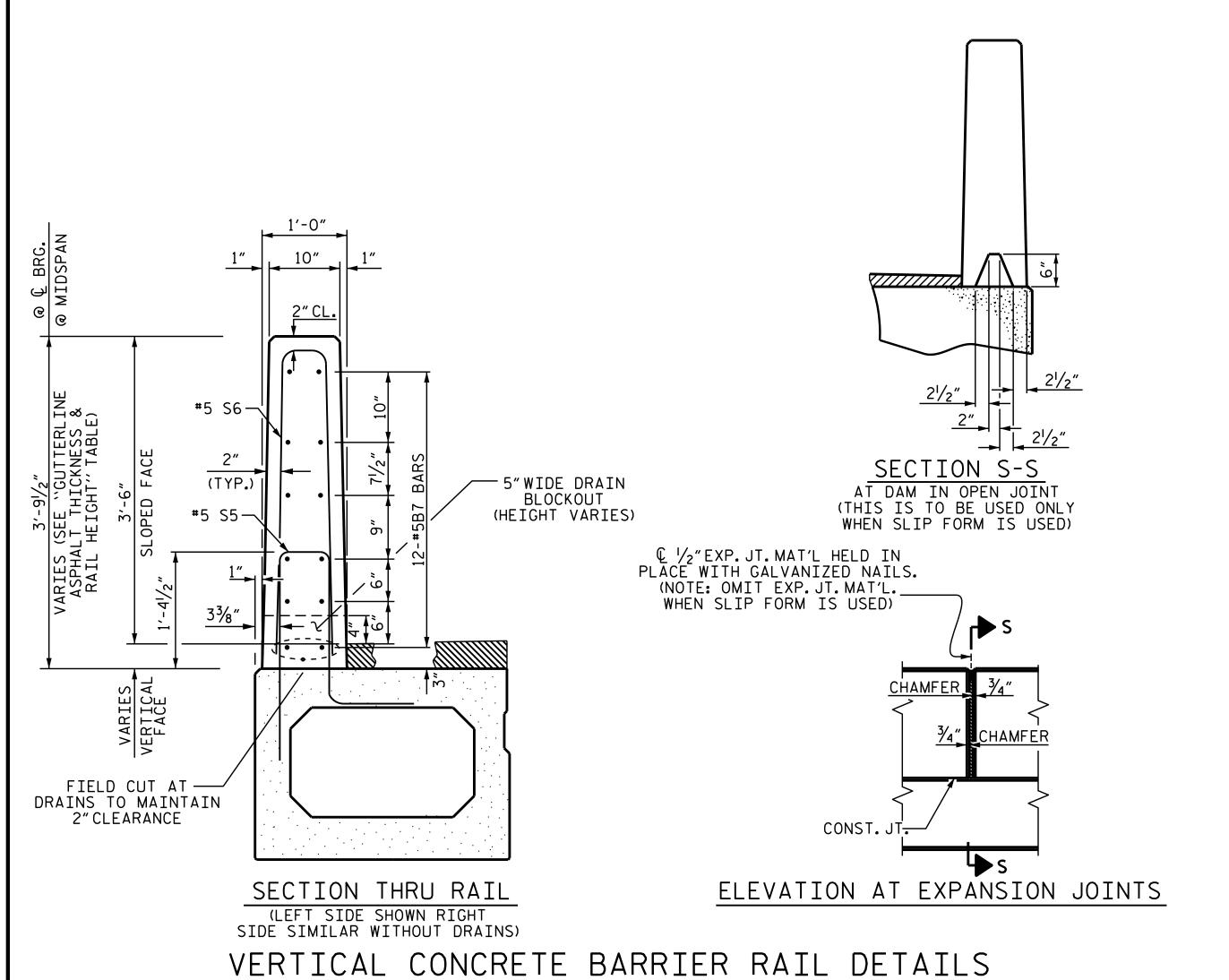
BOX BEAM UNIT

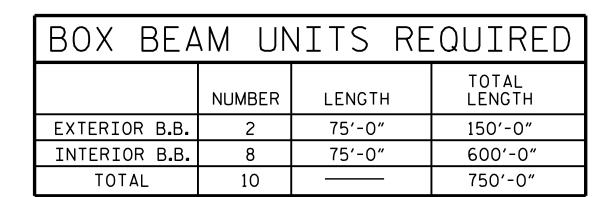
STATE OF NORTH CAROLINA

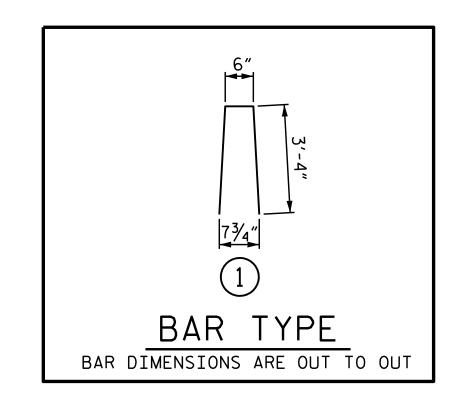
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SS ALL	1			3			TOTAL SHEETS			
OMPLETED	2			4			38			









BI	L OF MATERIAL FOR VERTICAL CONCRE	TE B	ARR	IER R	AIL			
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT			
	75' UNIT							
∗ B7	72	#5	STR	24'-7"	1846			
* S6	210	#5	1	7′-2″	1570			
* EPOXY COATED REINFORCING STEEL LBS.								
CLASS AA CONCRETE CU.YDS.								
TOTAL								

GUTTERLINE ASPHA	ALT THICKNESS &	RAIL HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
75' UNITS	21/4"	3'-81/4''

PROJECT NO. B-5546 RANDOLPH COUNTY STATION: 13+56.00-L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT

8/22/2016

SEAL 025516

CINEER

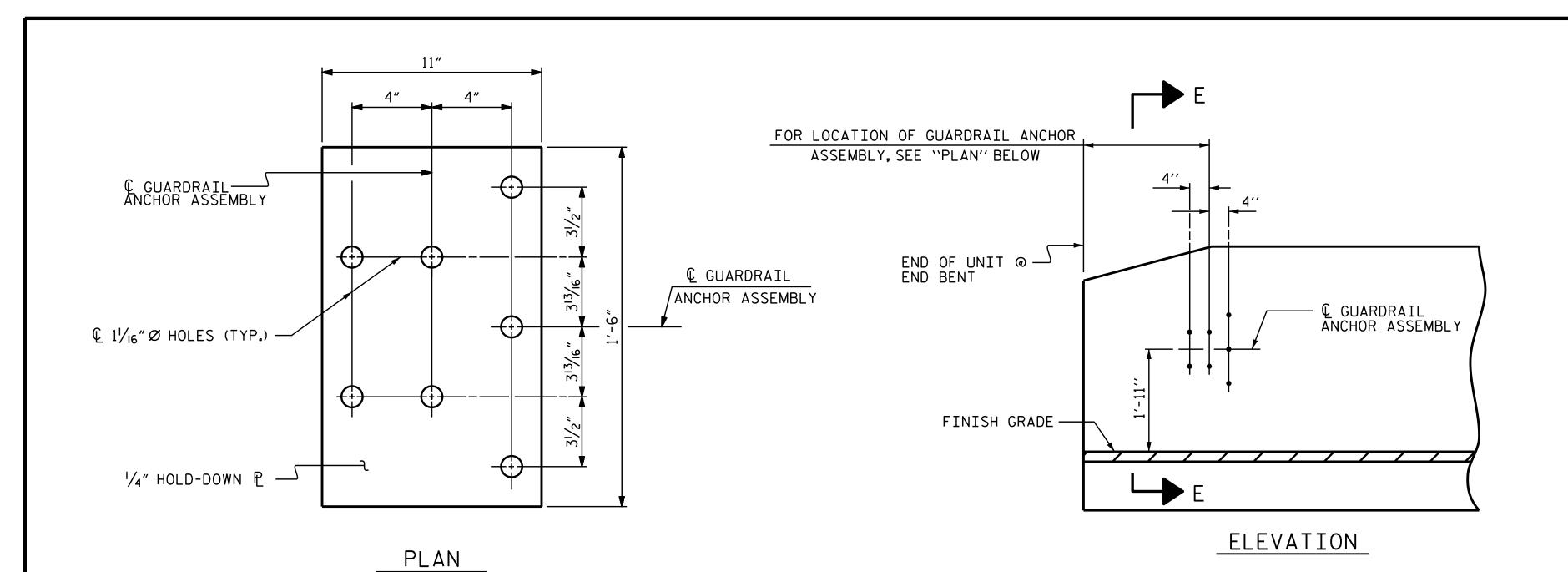
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			SHEET NO.				
7	NO.	BY:	DATE:	NO.	BY:	DATE:	S-31
1	1			3			TOTAL SHEETS
	2			4			38

ASSEMBLED BY :D.A. DAVENPORT DATE :10/08/14 CHECKED BY : J.K. BOWLES DATE :01/20/15

MAA/TMG

DRAWN BY : DGE IO/II CHECKED BY : TMG II/II



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ " \varnothing GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

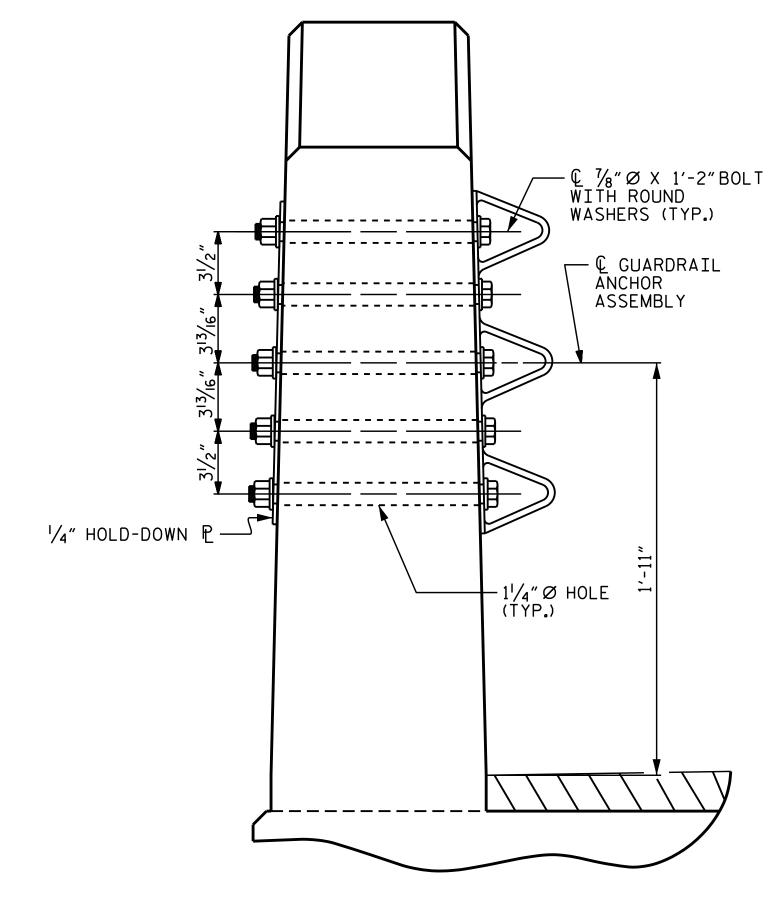
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

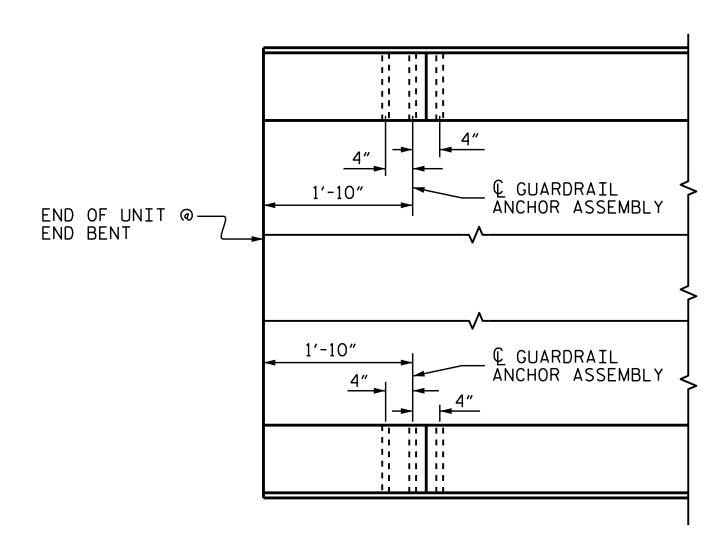
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



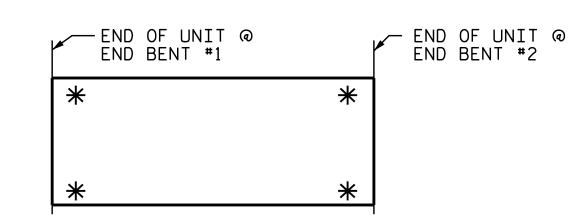
SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

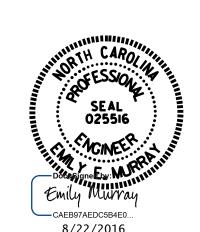
END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-5546 RANDOLPH ___ COUNTY STATION: 13+56.00-L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL

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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL	1	
SIGNATURES COMPLETED	2	

SHEET NO. REVISIONS NO. BY: DATE: DATE:

ASSEMBLED BY: D.A. DAVENPORT DATE: 08/04/14 CHECKED BY: J.K. BOWLES DATE: 01/14/15

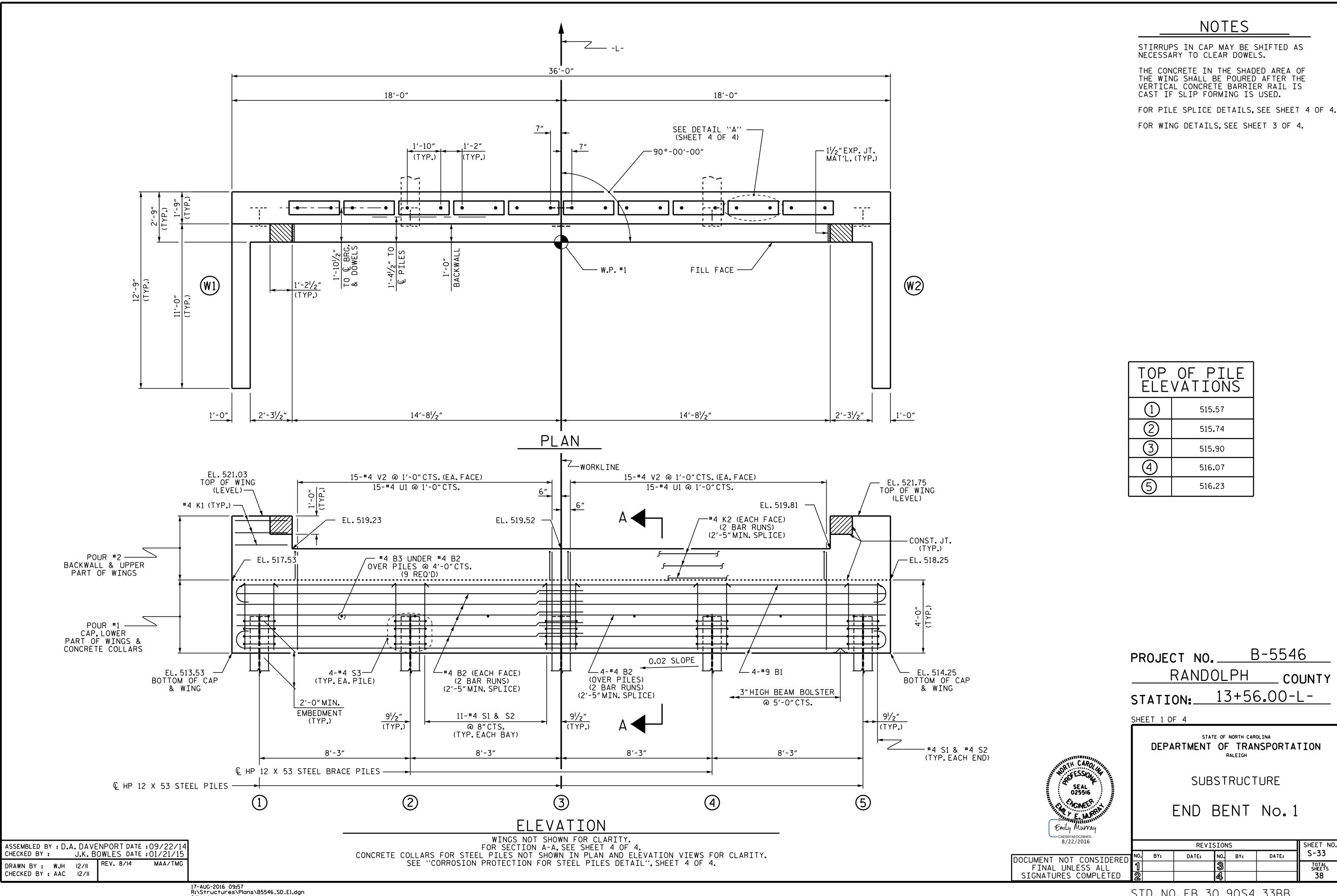
DRAWN BY: MAA 5/10

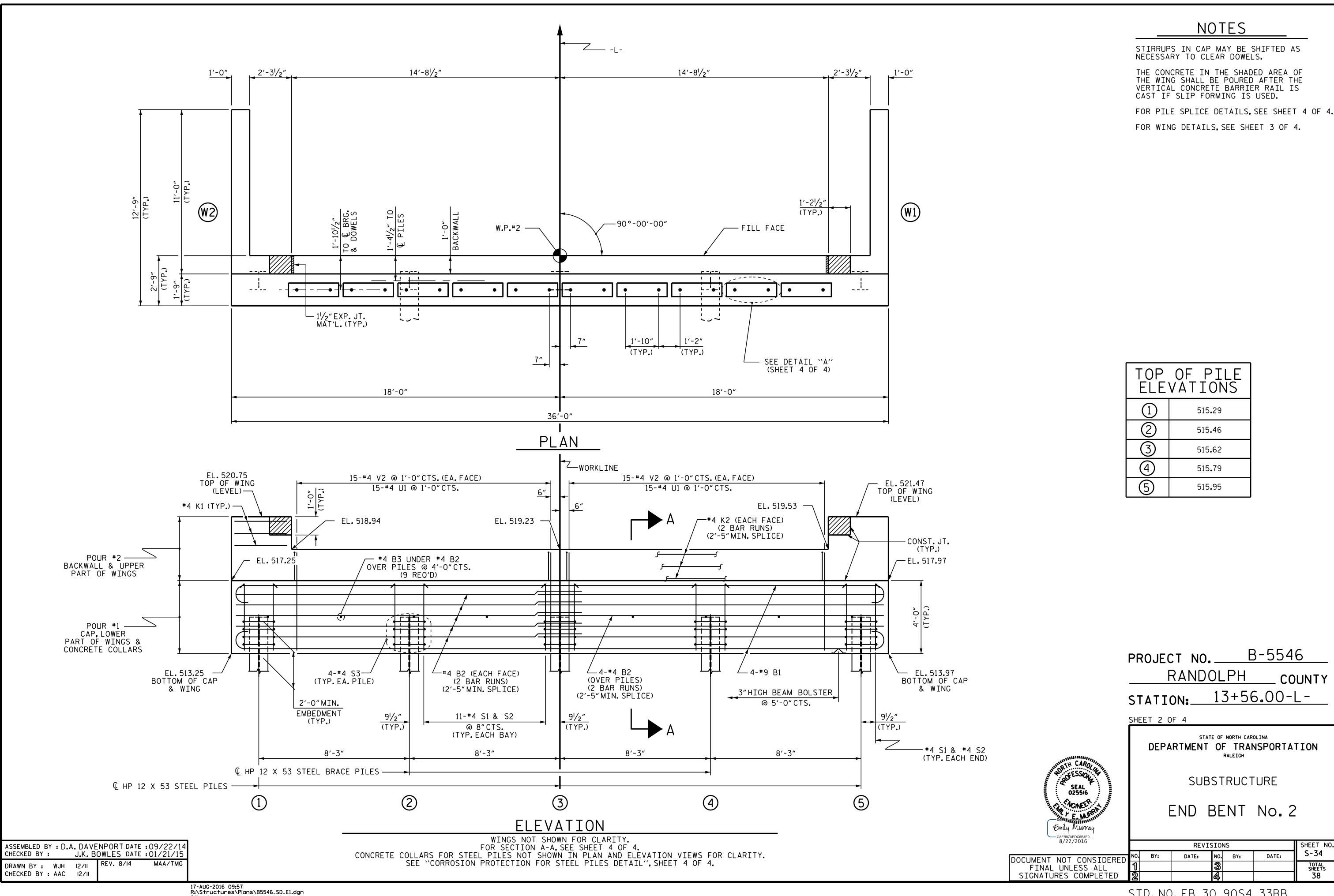
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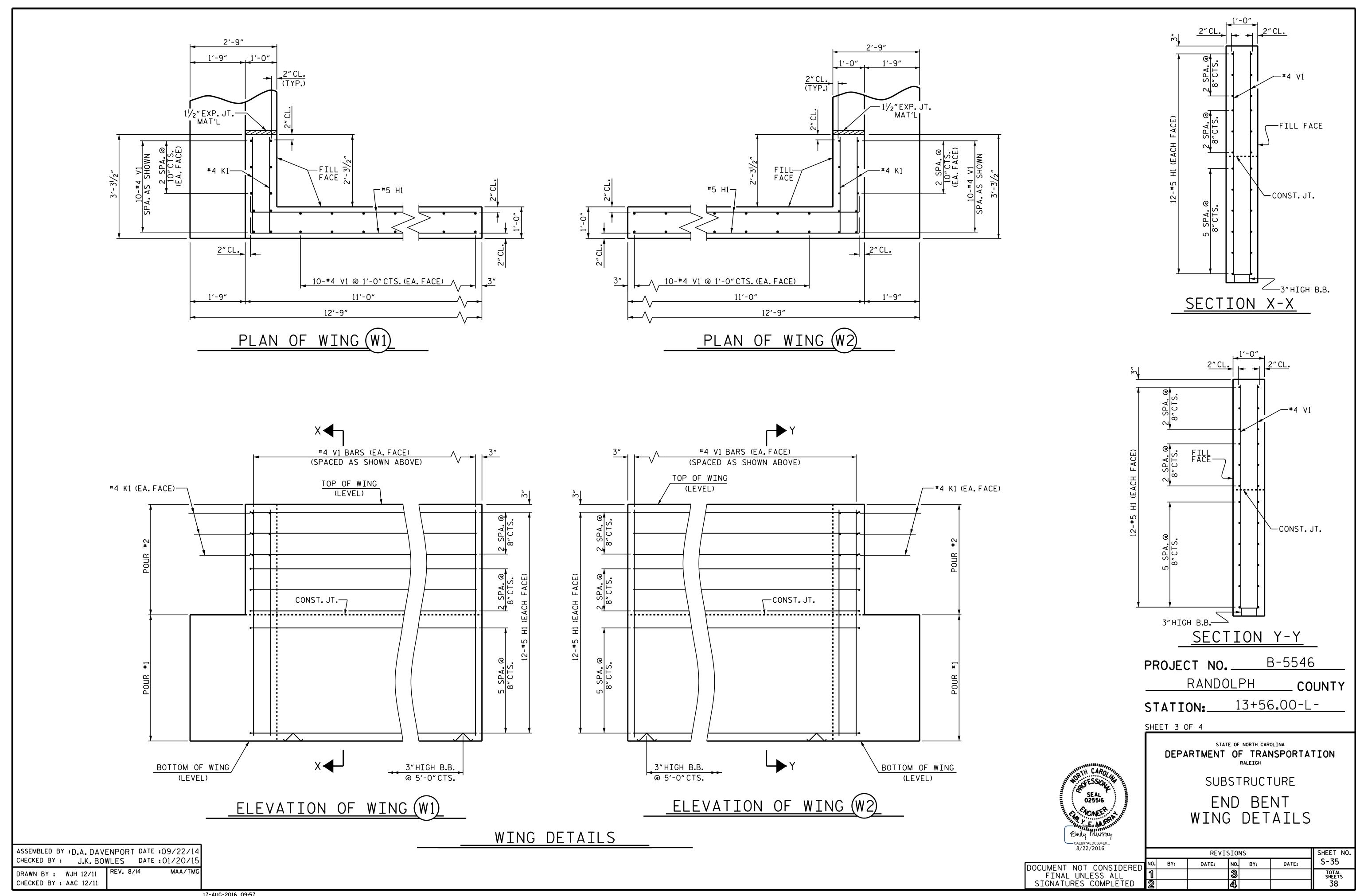
REV. 10/1/II REV. 12/5/II REV. 6/13

MAA/GM

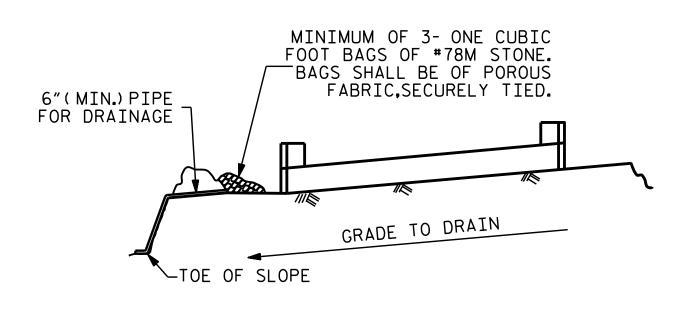
MAA/GM MAA/GM







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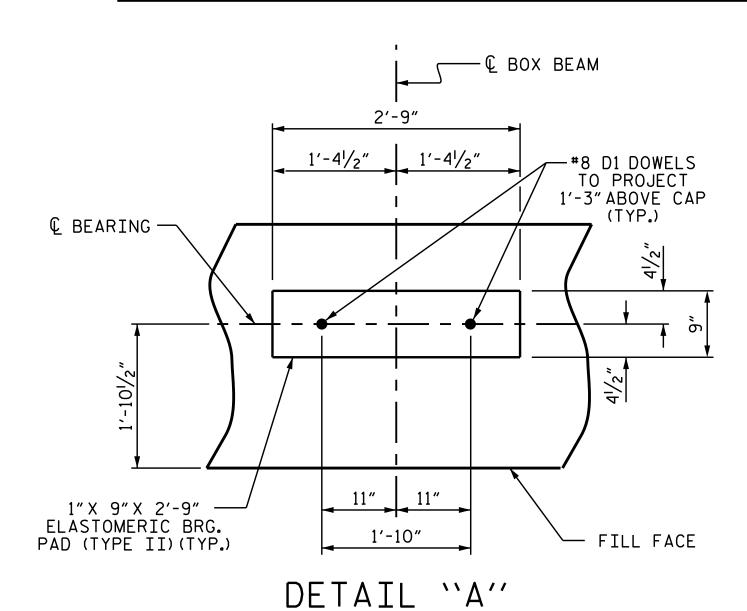


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

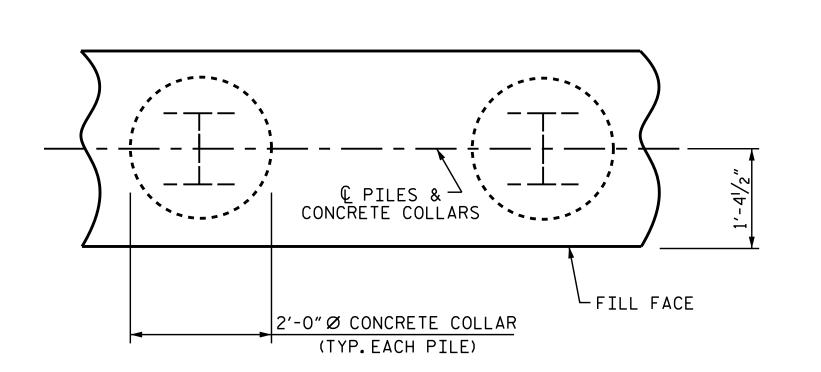
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)



PLAN CORROSION PROTECTION FOR STEEL PILES DETAIL (END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)

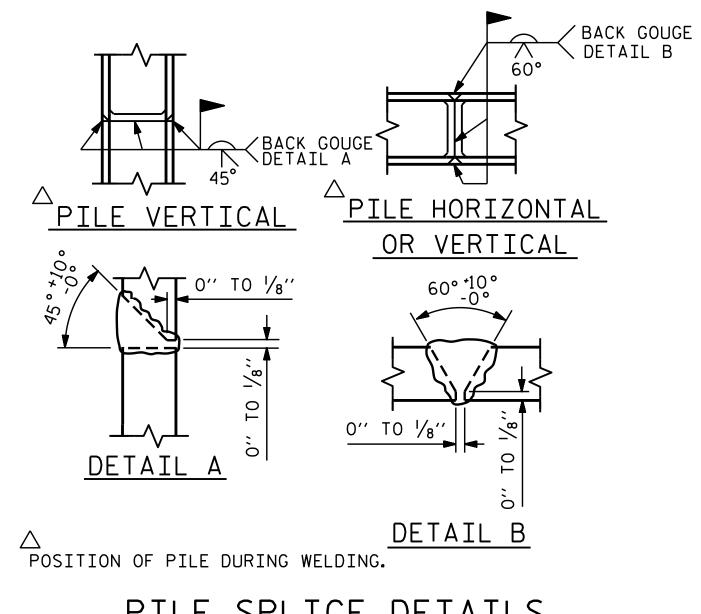
CONCRETE —

© HP 12 X 53 STEEL PILE

ELEVATION

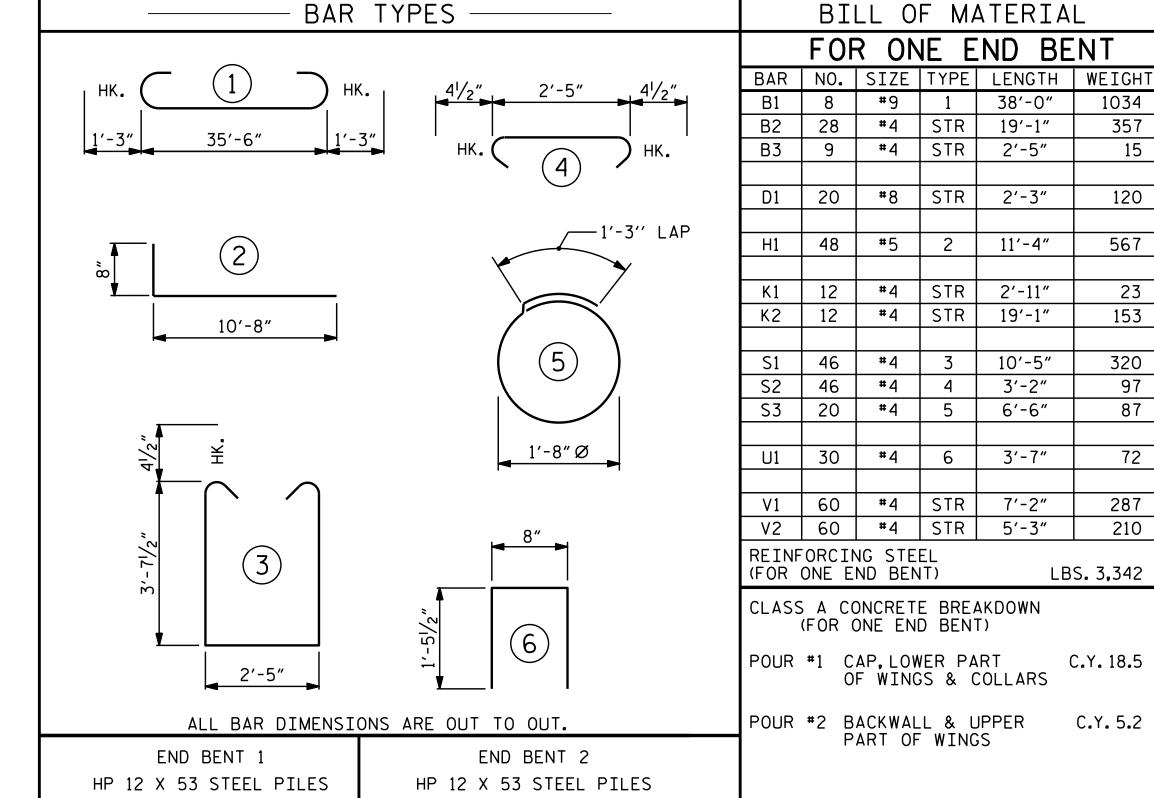
COLLAR

ASSEMBLED BY :D.A. DAVENPORT DATE :09/22/14 CHECKED BY: J.K. BOWLES DATE: 04/25/16 REV. 8/14 MAA/TMG DRAWN BY: WJH 12/11 CHECKED BY : AAC 12/11



PILE SPLICE DETAILS

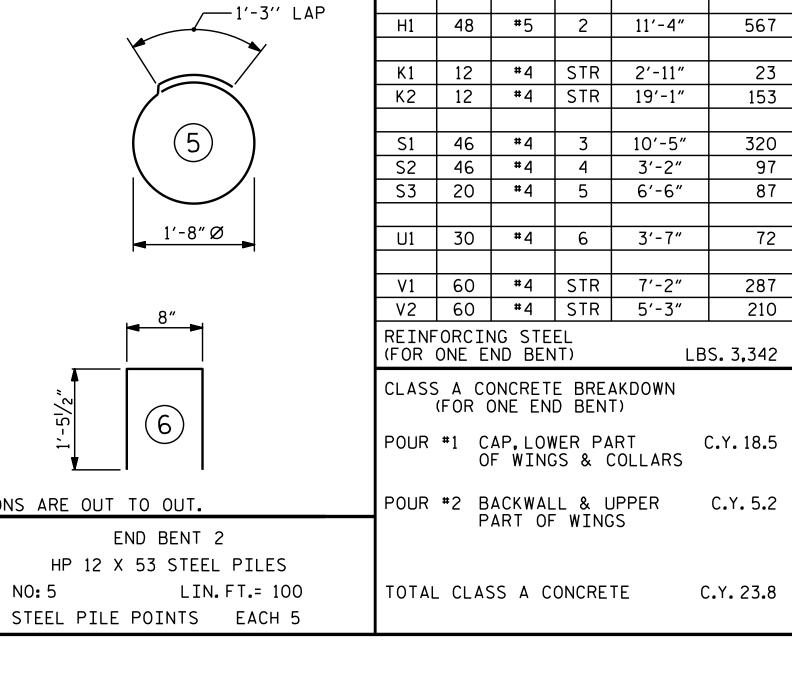
-BOTTOM OF CAP



NO: 5

LIN. FT.= 125

STEEL PILE POINTS EACH 5



BILL OF MATERIAL

#4 | STR |

#4 | STR |

FOR ONE END BENT

#9 | 1 | 38'-0"

#8 | STR | 2'-3"

19′-1"

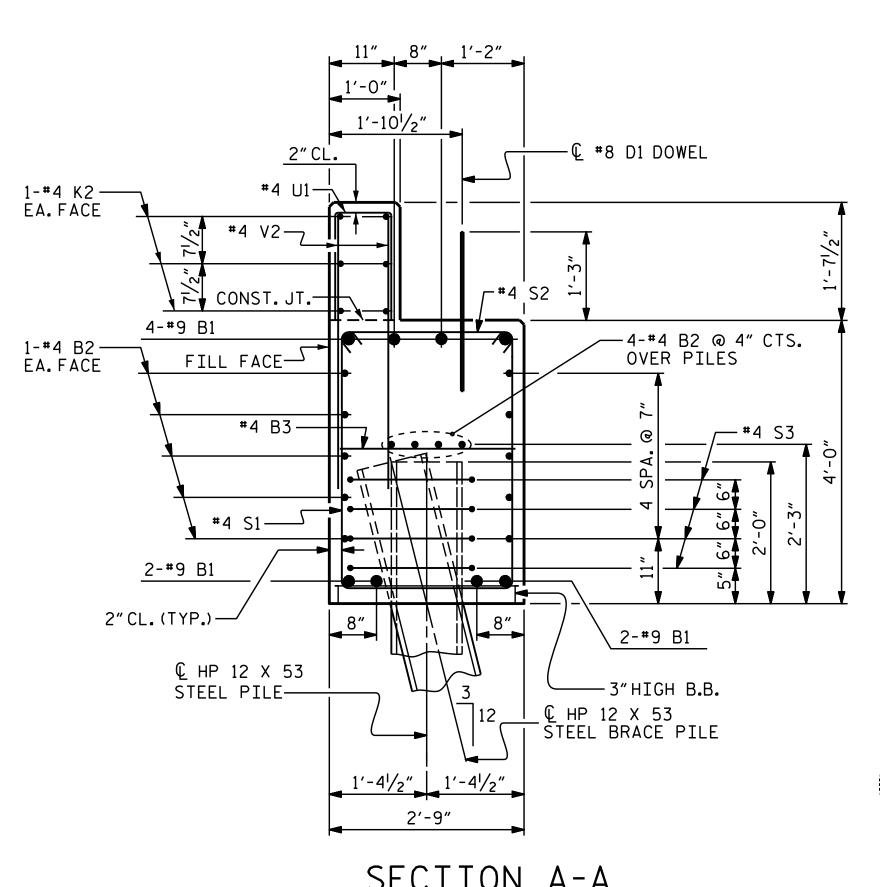
2′-5"

1034

357

15

120



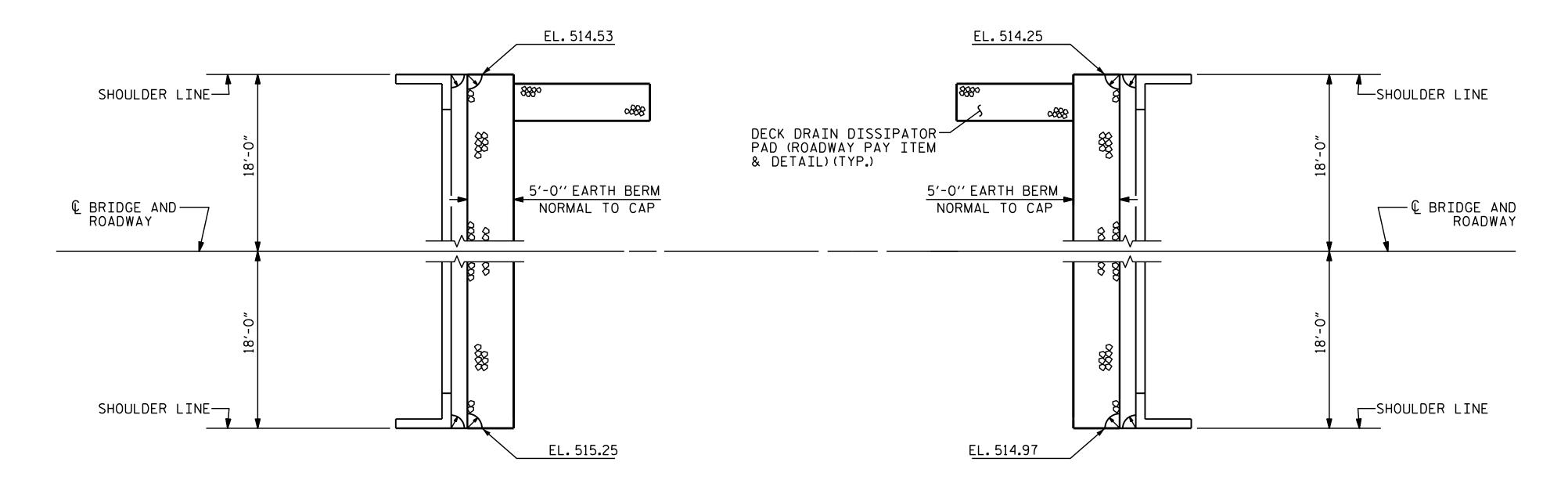
B-5546 PROJECT NO. ____ RANDOLPH COUNTY 13+56.00-L-SHEET 4 OF 4

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > SUBSTRUCTURE

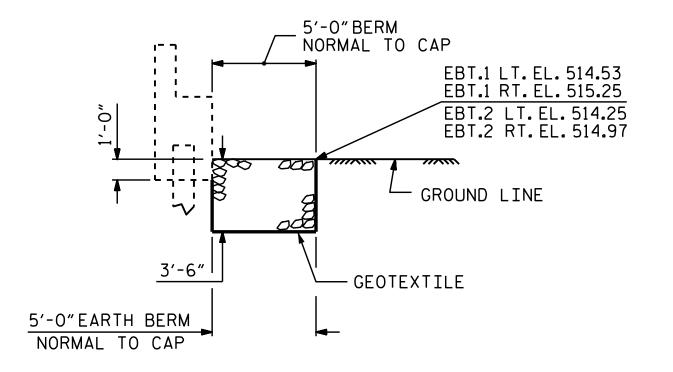
END BENT 1 & 2 DETAILS

SHEET NO. REVISIONS S-36 NO. BY: DATE: DATE: BY: TOTAL SHEETS 38

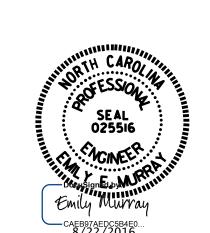


ESTIMATED QUANTITIES BRIDGE @ STA.13+56.00-L-RIP RAP CLASS II GEOTEXTILE FOR DRAINAGE SQUARE YARDS TONS END BENT 1 30 45 END BENT 2 30 45

END BENT 1 END BENT 2



BERM RIP RAPPED PROJECT NO. B-5546 RANDOLPH COUNTY STATION: 13+56.00-L-

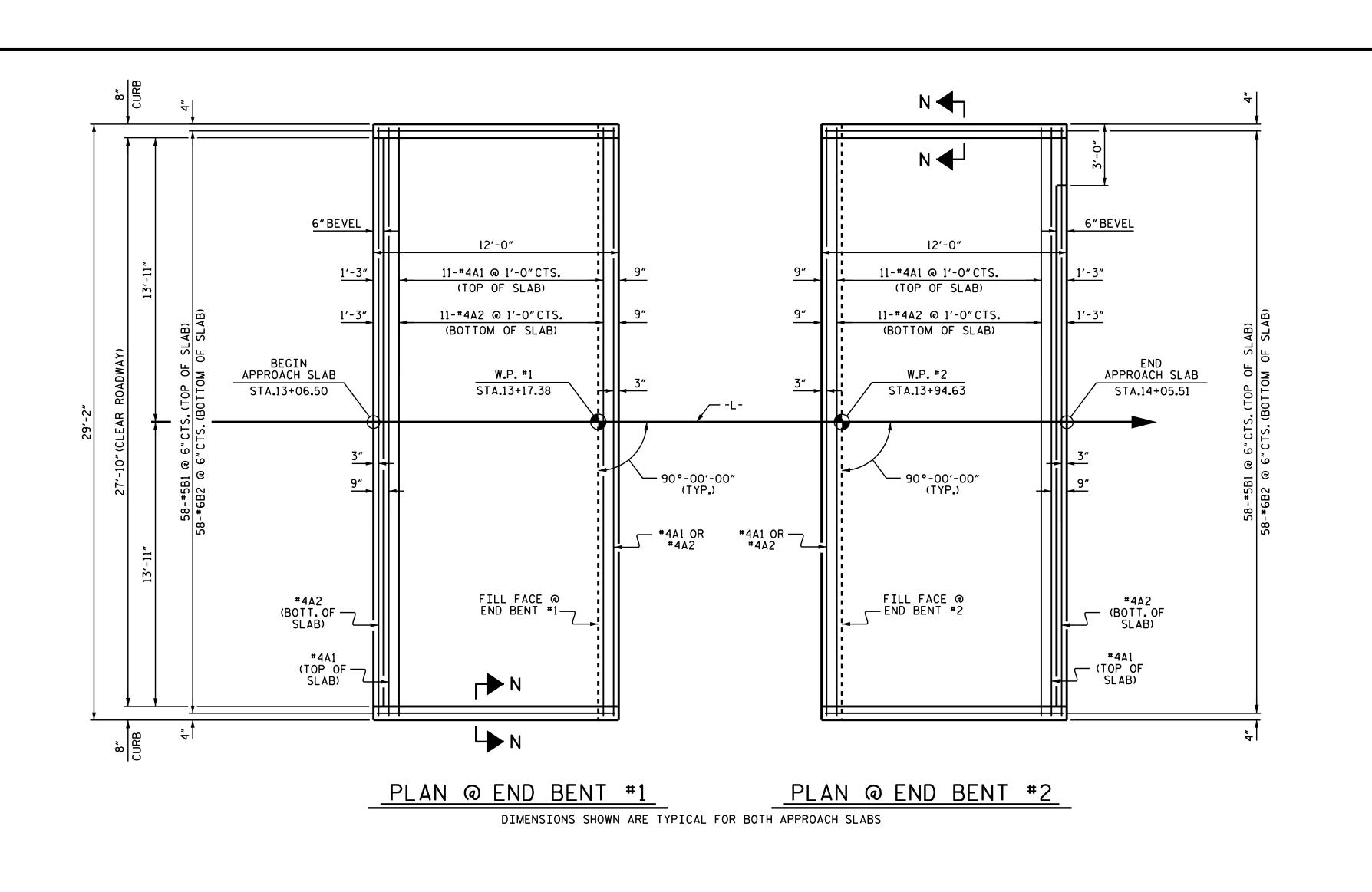


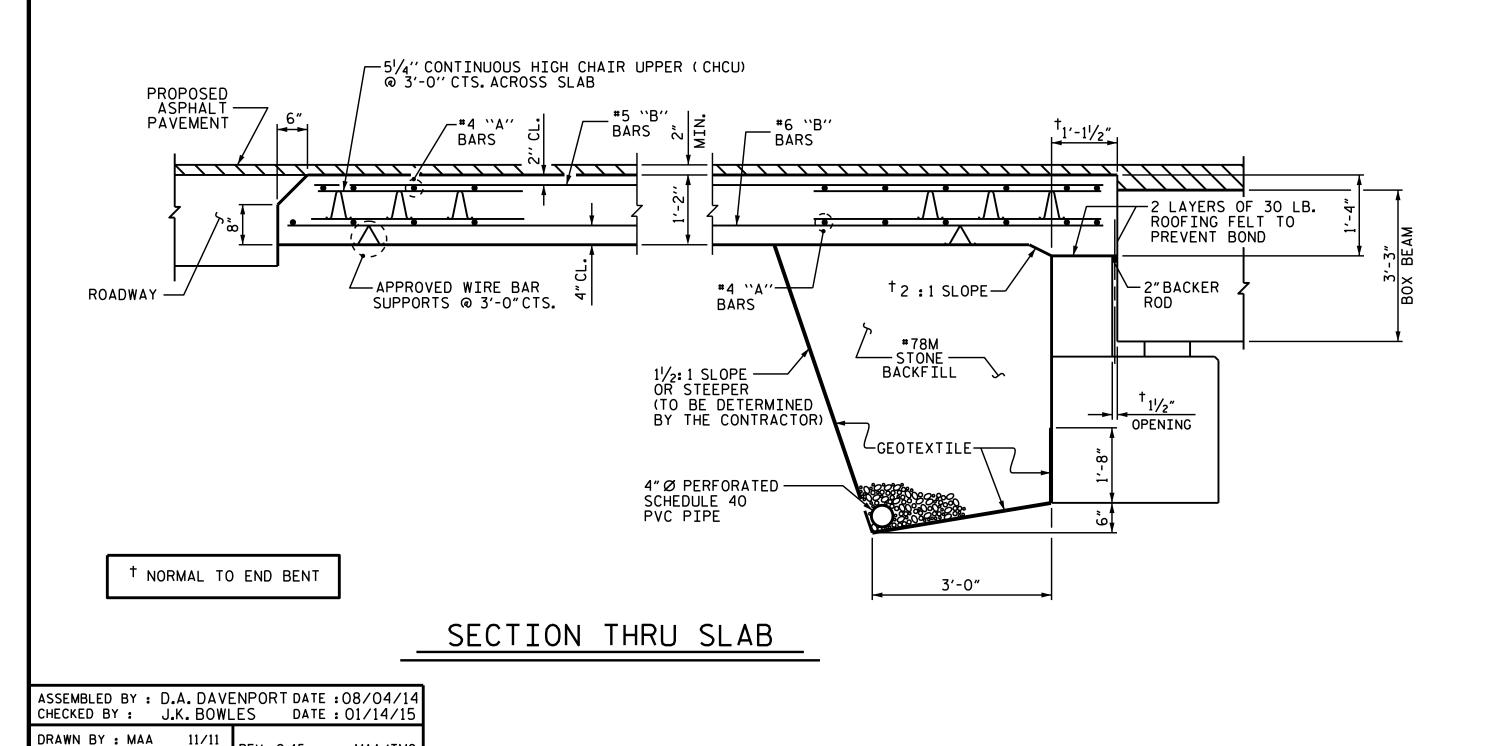
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

-RIP RAP DETAILS-

6/22/2010							
3, ==, =3=3			REVI	SION	S		SHEET NO
CUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-37
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			38

ASSEMBLED BY : D.A. DAVENPORT DATE :10/08/14 CHECKED BY : J.K. BOWLES DATE : 4/25/16 REV. 5/I/06R REV. I0/I/II REV. I2/2I/II TLA/GM MAA/GM MAA/GM DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84





NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND *78M STONE BACKFILL, SEE ROADWAY PLANS. GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

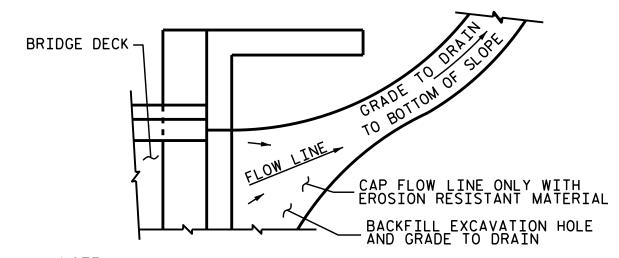
*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. TEMPORARY DRAINAGE DETAIL

* A1 | 13 | #4 | STR | 28'-10" 250 A2 | 13 | #4 | STR | 28'-10" 250 *B1 | 58 | #5 | STR | 11'-2" 676 B2 | 58 | #6 | STR | 11'-8" 1016 REINFORCING STEEL LBS. 1266 * EPOXY COATED REINFORCING STEEL LBS. 926 CLASS AA CONCRETE C.Y. 15.4 APPROACH SLAB AT EB #2 BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT * A1 | 13 | #4 | STR | 28'-10" 250 A2 | 13 | #4 | STR | 28'-10" 250 *B1 | 58 | *5 | STR | 11'-2" 676 B2 | 58 | *****6 | STR | 11'-8" 1016 REINFORCING STEEL LBS. 1266 * EPOXY COATED REINFORCING STEEL LBS. 926 CLASS AA CONCRETE 15.4 C. Y.

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

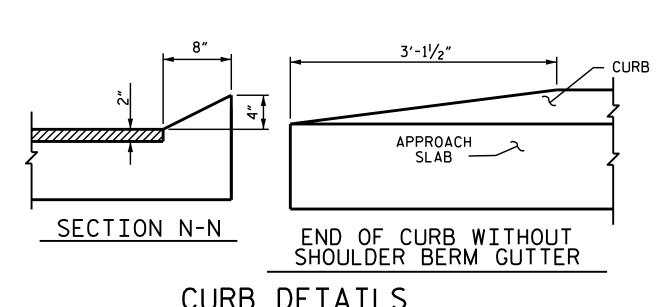
CLASS "B"STONE — FOR EROSION CONTROL . _ _ _ _ _ _ _ . TEMP. SLOPE DRAIN 4'-0" '-0"MIN. S◀┐ SHOULDER DITCH TOE OF FILL -BLOCK -CLASS "B"STONE —/
FOR EROSION CONTROL APPROACH-SLAB SECTION R-R 3"EROSION RESISTANT MATERIAL OVER PIPE ---EARTH DITCH BLOCK END OF APPROACH EROSION RESISTANT MATERIAL SLAB ---NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL 4'-0" MIN. - FILL SLOPE MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER.
THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED
TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

PLAN VIEW TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

GESSION

SEAL 025516



CURB DETAILS

SPLICE LENGTHS								
BAR SIZE	EPOXY COATED	UNCOATED						
#4	2'-0"	1'-9"						
# 5	2'-6"	2'-2"						
#6	3'-10"	2'-7"						

B-5546 PROJECT NO. RANDOLPH COUNTY 13+56.00-L-

SECTION S-S

DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE BOX BEAM UNIT

STATE OF NORTH CAROLINA

(SUB-REGIONAL TIER) 90° SKEW

SHEET NO.

S-38

TOTAL SHEETS

38

REVISIONS DATE: DATE: BY: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

MAA/TMG

CHECKED BY : AAC 11/11

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

COMPRESSION PERPENDICULAR TO GRAIN

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

OF TIMBER ----

- - - - -

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL

STRUCTURAL STEEL:

BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

GM 17-AUG-2016 09

375 LBS. PER SQ. IN.

(MINIMUM)

30 LBS. PER CU. FT.

REV. 6-16-95 EEM (x) RGW REV. 5-7-03 RWW (x) JTE REV. 10-1-11 MAA (x) GM REV. 8-16-99 RWW (x) LES REV. 5-1-06 TLA (x) GM

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