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REFERENCE: B-5121 / B-5317

PROJECT: 42263 / 46031

SEE SHEET 3 FOR PLAN SHEET LAYOUT  
AT TIME OF INVESTIGATION

**STATE OF NORTH CAROLINA**  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5121 / B-5317	1	48

**CONTENTS**

<u>LINE</u>	<u>STATION</u>	<u>PLAN</u>	<u>PROFILE</u>
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-Y5-	10+00 to 14+19	4	11
-Y6-	12+60 to 20+00	4-5	11
-YIRPC-	10+00 to 19+16	4-5	11
-YIRPD-	10+00 to 17+27	5	12
-DR2-	10+00 to 11+50	5	12
-Y2RPB-	10+00 to 16+43	7	12
-Y2LPC-	10+00 to 13+11	7	12
-FLYOVER-	10+00 to 25+76	6-7	13-14

**CROSS SECTIONS**

<u>LINE</u>	<u>STATION</u>	<u>SHEETS</u>
-L-	20+00 to 42+00	15-26
-Y5-	10+50 to 13+25	27-28
-Y6-	15+00 to 19+27	29-31
-YIRPC-	12+60.64 to 15+52.58	32-34
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**APPENDICES**

<u>APPENDIX</u>	<u>TITLE</u>	<u>SHEETS</u>
A	LABORATORY RESULTS	45 - 46

# ROADWAY SUBSURFACE INVESTIGATION

COUNTY WAKE  
PROJECT DESCRIPTION REPLACE BRIDGE NO. 227 ON  
US 70/US 401/NC 50 (CAPITAL BOULEVARD) OVER  
PEACE STREET AND  
REPLACE BRIDGE NO. 213 ON US 70/NC 50 (WADE  
AVENUE) OVER US 401 (CAPITAL BOULEVARD)  
**INVENTORY**

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

TURNAGE, J. R.

DUGGINS, W. T.

EKLUND, M. A.

ALEXANDER, M. J.

FRAWLEY, M. H.

INVESTIGATED BY TERRACON CONSULTANTS

DRAWN BY ALEXANDER, M. J.

CHECKED BY NASH, A. A.

SUBMITTED BY TERRACON CONSULTANTS

DATE AUGUST 2015

NOT CONSIDERED FINAL UNLESS ALL SIGNATURES ARE COMPLETED



DocuSigned by:

Matthew J. Alexander 8/20/2015

0FB0038EEA08452 SIGNATURE DATE



**TIP PROJECT: B-5121 / B-5317**

**CONTRACT:**

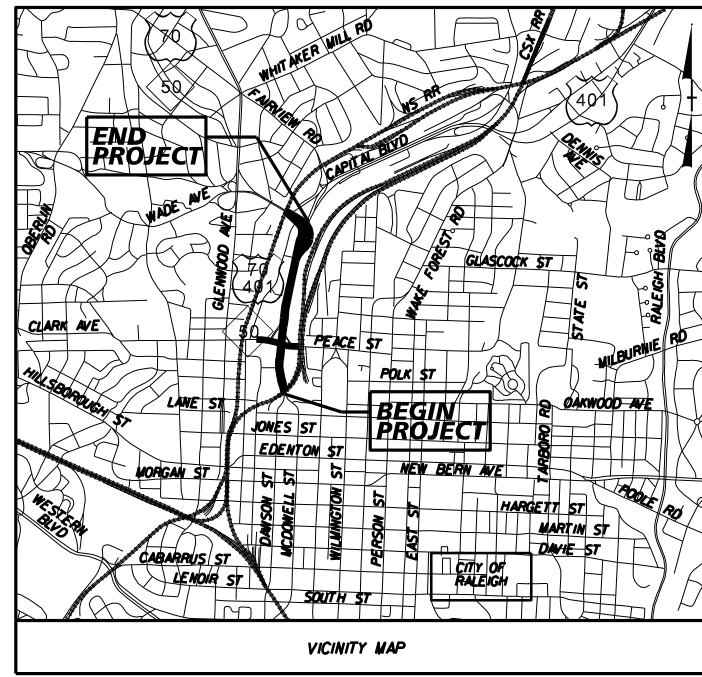
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STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

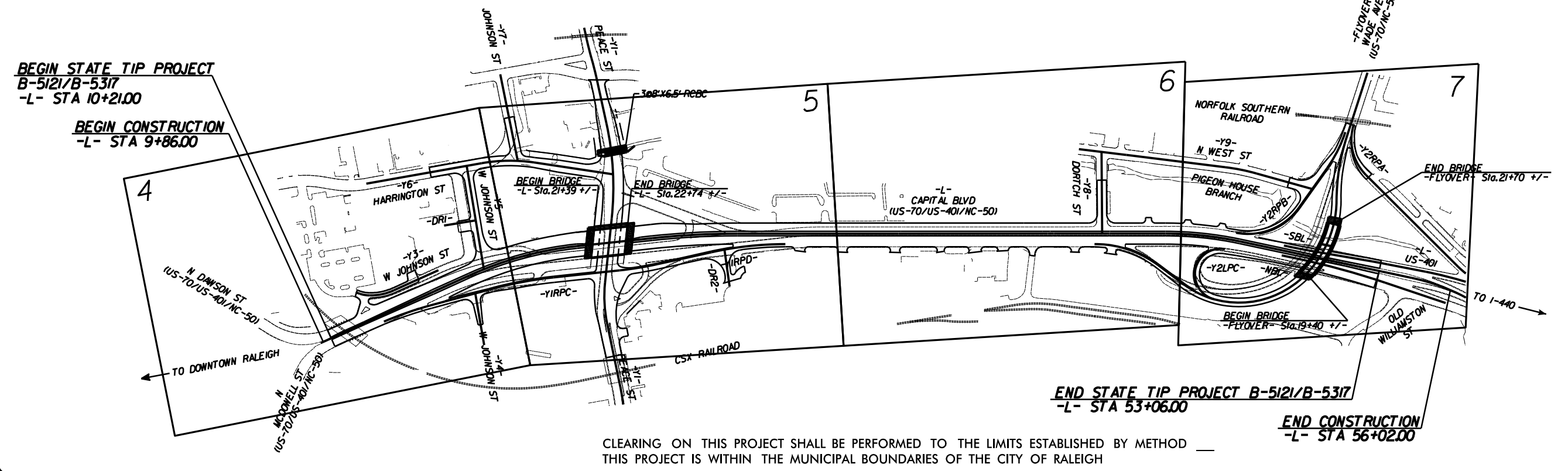
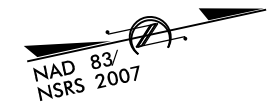
**WAKE COUNTY**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5121/B-5317	3	48
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
42263.1.1	BRNHS-0070(119)	P.E. (B-5121)	
46031.1.1	BRNHS-0070(149)	P.E. (B-5317)	

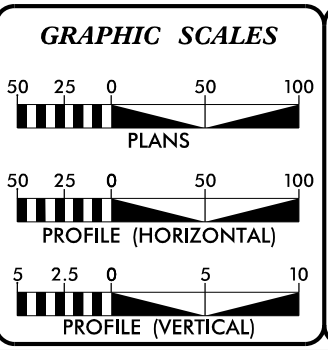
INCOMPLETE PLANS  
DO NOT USE FOR R/W ACQUISITION  
PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION



**LOCATION: BRIDGE NO. 277 ON US-70/US-401/NC-50 (CAPITAL BOULEVARD) OVER PEACE STREET AND BRIDGE NO. 213 ON US-70/NC-50 (WADE AVENUE) OVER US 401 (CAPITAL BOULEVARD)**  
**TYPE OF WORK: GRADING, PAVING, DRAINAGE, SIGNING, SIGNALS, STRUCTURES, AND CULVERT**



CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD \_\_\_\_  
THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF THE CITY OF RALEIGH



**DESIGN DATA**

AADT 2016	=	58,083
AADT 2036	=	70,416
K	=	10%
D	=	55%
T	=	5%*
V	=	40 MPH

CLASSIFICATION:  
URBAN ARTERIAL

\* 1% TTST 4% DUAL  
STATEWIDE TIER

**PROJECT LENGTH**

LENGTH ROADWAY TIP PROJECT B-5121/B-5317	=	0.786 MILES
LENGTH STRUCTURE TIP PROJECT B-5121/B-5317	=	0.026 MILES
TOTAL LENGTH TIP PROJECT B-5121/B-5317	=	0.812 MILES

PLANS PREPARED FOR THE NCDOT BY:

**Kimley»Horn**

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: JUNE 19, 2015

LETTING DATE: JUNE 21, 2016

**JEFFREY W. MOORE, P.E.**  
PROJECT ENGINEER

**J. JASON PACE, P.E.**  
PROJECT DESIGN ENGINEER

**BRENDA L. MOORE, P.E., CPM**  
PROJECT ENGINEER  
NCDOT ROADWAY DESIGN  
ENGINEERING COORDINATION SECTION

HYDRAULICS ENGINEER

\_\_\_\_\_  
P.E.

SIGNATURE:  
ROADWAY DESIGN ENGINEER

\_\_\_\_\_  
P.E.

SIGNATURE:

DIVISION OF HIGHWAYS  
STATE OF NORTH CAROLINA

STATE HIGHWAY DESIGN ENGINEER

\_\_\_\_\_  
P.E.

Date: August 2015  
 WBS Number: 42263.1.1 / 46031.1.1  
 TIP Number: B-5121 / B-5317  
 F.A. Number: BRNHS-0070(119) / BRNHS-0070(149)  
 County: Wake  
 Description: Replace Bridge No. 227 on US 70/US 401/NC 50 (Capital Boulevard) over Peace Street and Replace Bridge No. 213 on US 70/NC 50 (Wade Avenue) over US 401 (Capital Boulevard)

**Subject: Roadway Geotechnical Report - Inventory**

**Project Description**

The project is located within the city limits of Raleigh in Wake County. The project begins just south of the CSX Railroad bridge over Capital Boulevard and just north of the Dawson Street and McDowell Street split near downtown. The project continues north for approximately 0.8 miles to a point 275 feet north of the existing Wade Avenue bridge over Capital Boulevard. The project also includes the realignment of several secondary alignments. The project corridor is in an urban setting that has experienced substantial development over hundreds of years.

The geotechnical subsurface investigation was performed in January through April of 2015. Diedrich D-50 and CME 75 rotary drills were utilized to advance the soil test borings. All of the drilling equipment utilized on this project was equipped with automatic Standard Penetration Test (SPT) hammers. In addition to soil test borings performed along the corridor, historic maps were reviewed in order to better understand the development that has taken place along the corridor.

The following alignments were investigated by soil testing or visual reconnaissance:

<u>Alignment</u>	<u>Stations</u>
-L-	10+21 to 53+06
-Y5-	10+00 to 14+19
-Y6-	12+60 to 20+00
-Y1RPC-	10+00 to 19+16
-Y1RPD-	10+00 to 17+27
-DR2-	10+00 to 11+50
-Y2RPB-	10+00 to 16+43
-Y2LPC-	10+00 to 13+11
-FLYOVER-	10+00 to 25+76

**Areas of Special Geotechnical Interest**

**1. High Plasticity Clays**

High plasticity clays were encountered in proposed cuts at the following location:

<u>Alignment</u>	<u>Stations</u>
-Y1RPD-	15+80 to 16+90

High plasticity clays were encountered near proposed subgrade or in fill sections at the following locations:

<u>Alignment</u>	<u>Stations</u>
-L-	33+50 to 38+00
-FLYOVER-	22+26 to 24+00

**2. Artificial Fill**

Artificial fill was encountered at the following locations:

<u>Alignment</u>	<u>Stations</u>
-Y5-	10+25 to 13+48
-Y6-	10+22 to 19+55
-DR2-	10+34 to 11+50
-FLYOVER-	14+04 to 17+68

**3. Weathered Rock**

Weathered rock was encountered in cut sections or near proposed subgrade at the following location:

<u>Alignment</u>	<u>Stations</u>
-Y1RPC-	12+54 to 15+50

**Physiography and Geology**

The project is located in the Piedmont Physiographic Province. The residual soils in the area were derived from the weathering of the parent bedrock encountered at depth along the corridor. Geologically, the project corridor is mapped as being underlain by an injected gneiss formation in the Raleigh Belt. The formation consists of biotite gneiss and schist intruded by sills and dikes of granite, pegmatite, and aplite. The existing topography along the project consists of a ridgeline along the east of the corridor with the CSX Railroad at the top and, in general, slopes gently toward Pigeon House Branch to the west. Pigeon House Branch meanders to the west of Capital Boulevard and flows into an existing box culvert at Peace Street which continues outside of the project limits. There is approximately 50 feet of elevation difference between the upland and lowland portions of the project.



PROJECT REFERENCE NO.	SHEET NO.
B-5121 / B-5317	3B

### Soil Properties

Soils encountered during the investigation along the corridor consist predominately of roadway embankment, artificial fill, alluvial, and residual.

The roadway embankment soils predominately consist of sandy silts (A-4), sandy clays (A-6) and clayey sands (A-2-6). The silts encountered in the approaches to the existing bridge at Peace Street were very soft to medium stiff. The roadway embankment soils encountered along the -L- alignment from the Peace Street interchange north consisted of clayey sands and sandy clays with a majority of this material being encountered as utility trench backfill. The material encountered was very loose to medium dense clayey sands and soft to very stiff sandy clays.

Artificial fill was encountered in developed areas surrounding the existing roadways. The material encountered varied across the project consisting of silty to clayey sands (A-2-4 and A-2-6, respectively) and sandy clays (A-6). The artificial fill encountered along the -Y6- alignment contained concrete and brick rubble. Glass and metal debris was encountered in the boring on the -DR2- alignment. The borings performed at the proposed retaining wall along the -FLYOVER- alignment encountered wood throughout the fill. The artificial fill ranged in thickness from surficial fill for landscape areas to approximately 15 feet.

Alluvial soils were encountered beneath artificial fill and roadway embankments soils and consist of silty and clayey sands (A-2-4 and A-2-6) and clayey silt (A-5). These soils are not anticipated to impact embankment stability because the amount and duration of overburden pressure.

The residual soils encountered on a majority of the project were saprolitic. The residual soils consist of medium to high plasticity silty clays (A-7), low to medium plasticity sandy clays (A-6), fine sandy and clayey silts (A-4 and A-5), and silty and clayey sands (A-2-4 and A-2-6, respectively). The high plasticity clay soils (PI>25) were encountered near the surface at the locations indicated above. In general the silty and sandy layers were encountered alternating to the top of weathered rock. The residual soils were observed to have alternating layers of fine and coarse grained soils before transitioning into weathered rock.

### Rock Properties

Weathered rock along the corridor is derived from the underlying gneiss and schist. The weathered rock along the project corridor is suitable for use as fill. Weathered rock was encountered in proposed cut sections at the approximate location listed in the "Areas of Special Geotechnical Interest" section.

The crystalline rock gneiss was encountered during the investigation. Where encountered, the rock was blue-gray with white and black. The rock was cored at the Wade Avenue overpass location (-FLYOVER-) and RQD values of approximately 85% were reported. The crystalline rock was not encountered within 6 feet of proposed grades during the investigation along the corridor.

### Groundwater

In general, the corridor drains to Pigeon House Branch which runs to the west of Capital Boulevard for the length of the project. Drainage is accomplished by surface runoff of developed areas and existing stormwater management infrastructure.

Groundwater was not encountered within 6 feet of proposed grades during the investigation.

**Respectfully Submitted,  
Terracon Consultants, Inc.**

DocuSigned by:  
*Matthew J. Alexander*  
0FB0038EEA06452...  
Matthew J. Alexander, PE  
Project Geotechnical Engineer

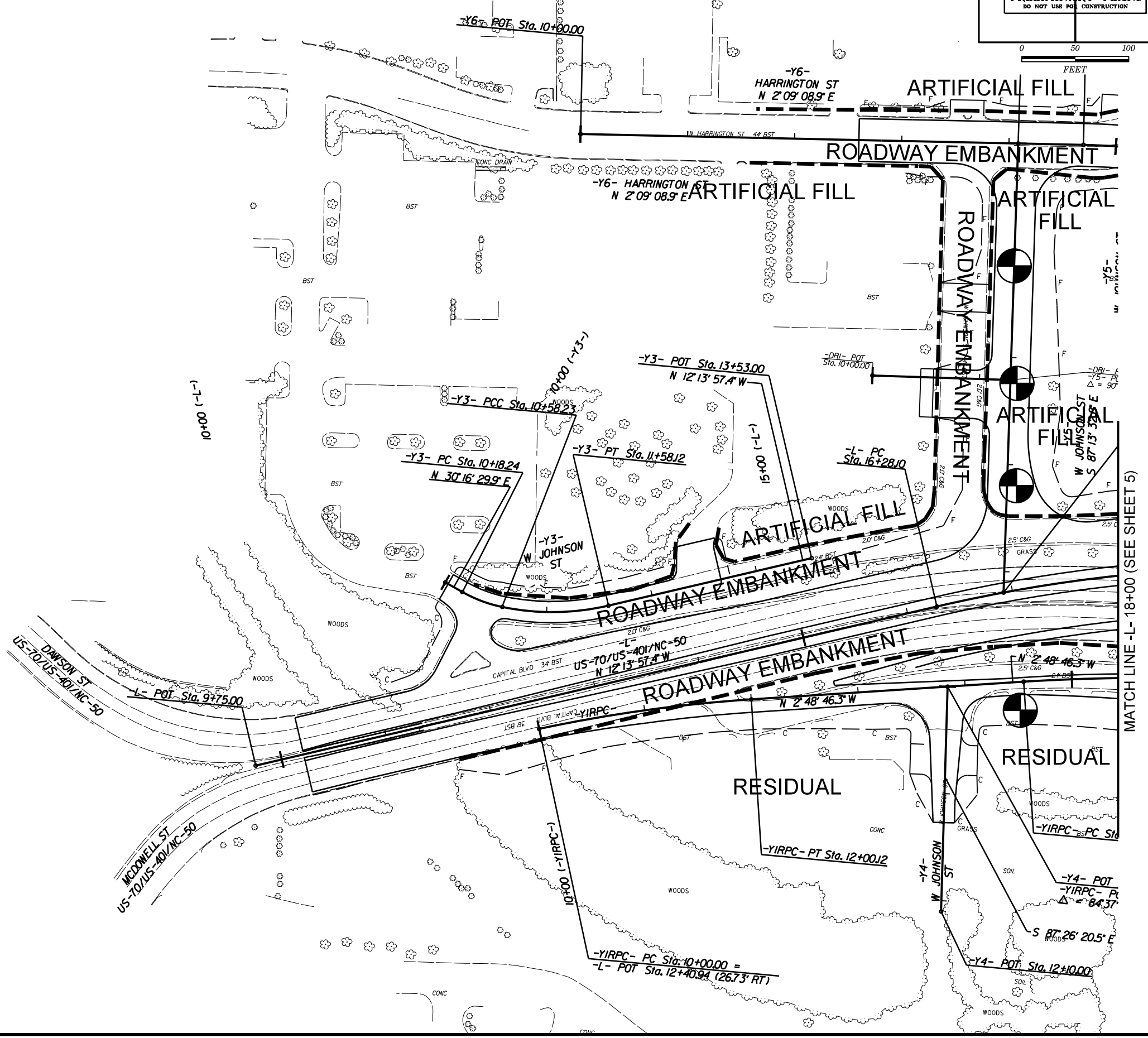
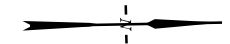
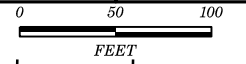
DocuSigned by:  
*Andrew A. Nash*  
671DA68582E04FE...  
Andrew A. Nash, PE  
Geotechnical Department Manager

Y5- POT Sta. 10+00  
 $\Delta = 90'37'18.3"$

PROJECT REFERENCE NO.  
B-5121 / B-5317

SHEET NO.  
4

INCOMPLETE PLANS  
DO NOT USE FOR E/W ACQUISITION  
PRELIMINARY PLANS  
DO NOT USE FOR CONSTRUCTION



MATCH LINE - L-18+00 (SEE SHEET 5)

I = 100.7  
 R = 650.00'  
 SE = RC  
 RO = 60'

PROJECT REFERENCE NO.  
 B-5121 / B-5317

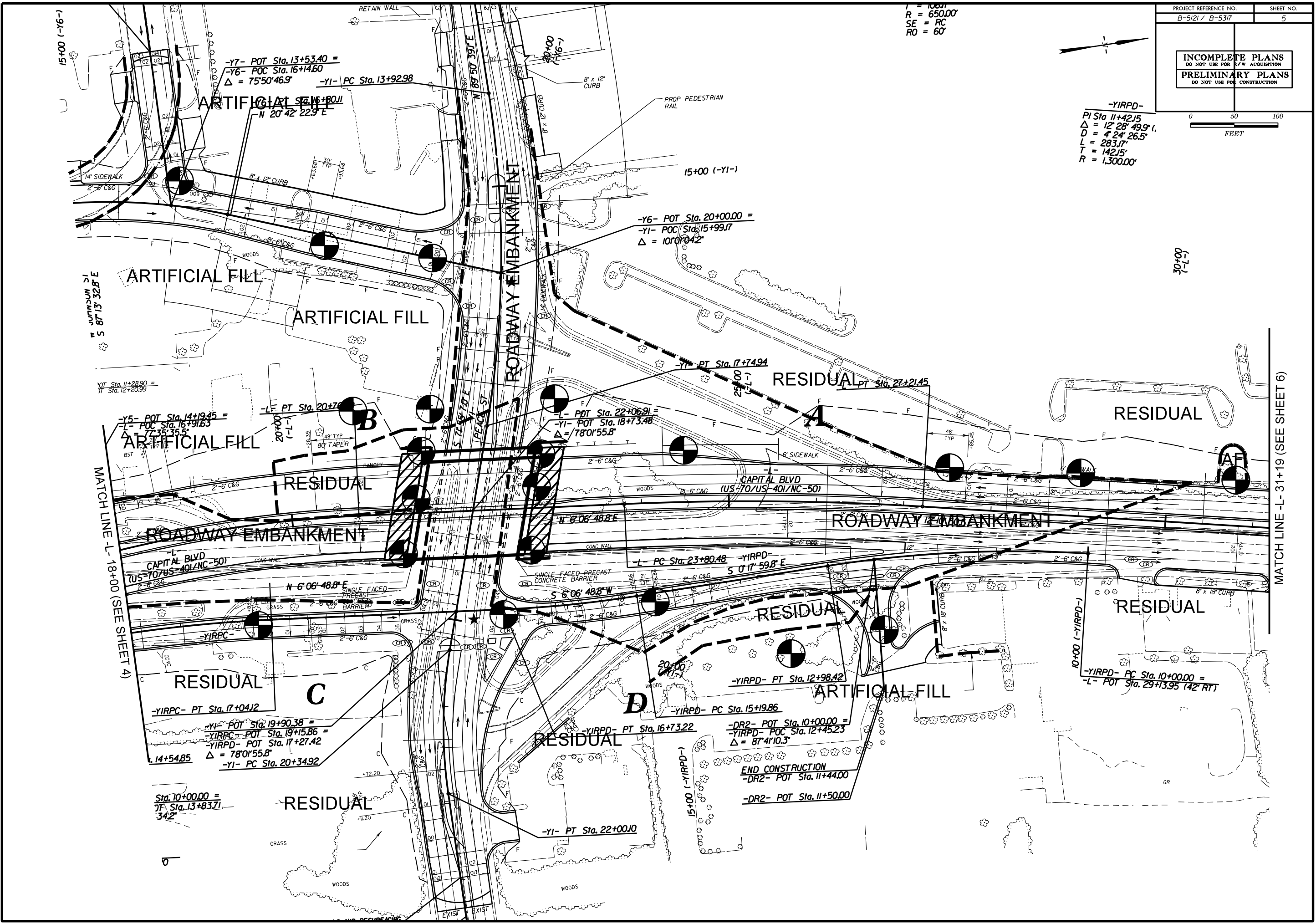
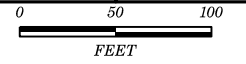
SHEET NO.  
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**INCOMPLETE PLANS**  
 DO NOT USE FOR A/W ACQUISITION

**PRELIMINARY PLANS**  
 DO NOT USE FOR CONSTRUCTION



-YIRPD-  
 PI Sta 11+42.15  
 $\Delta = 12^\circ 28' 49.9''$   
 $D = 424' 26.5''$   
 $L = 283.17'$   
 $T = 142.15'$   
 $R = 1,300.00'$



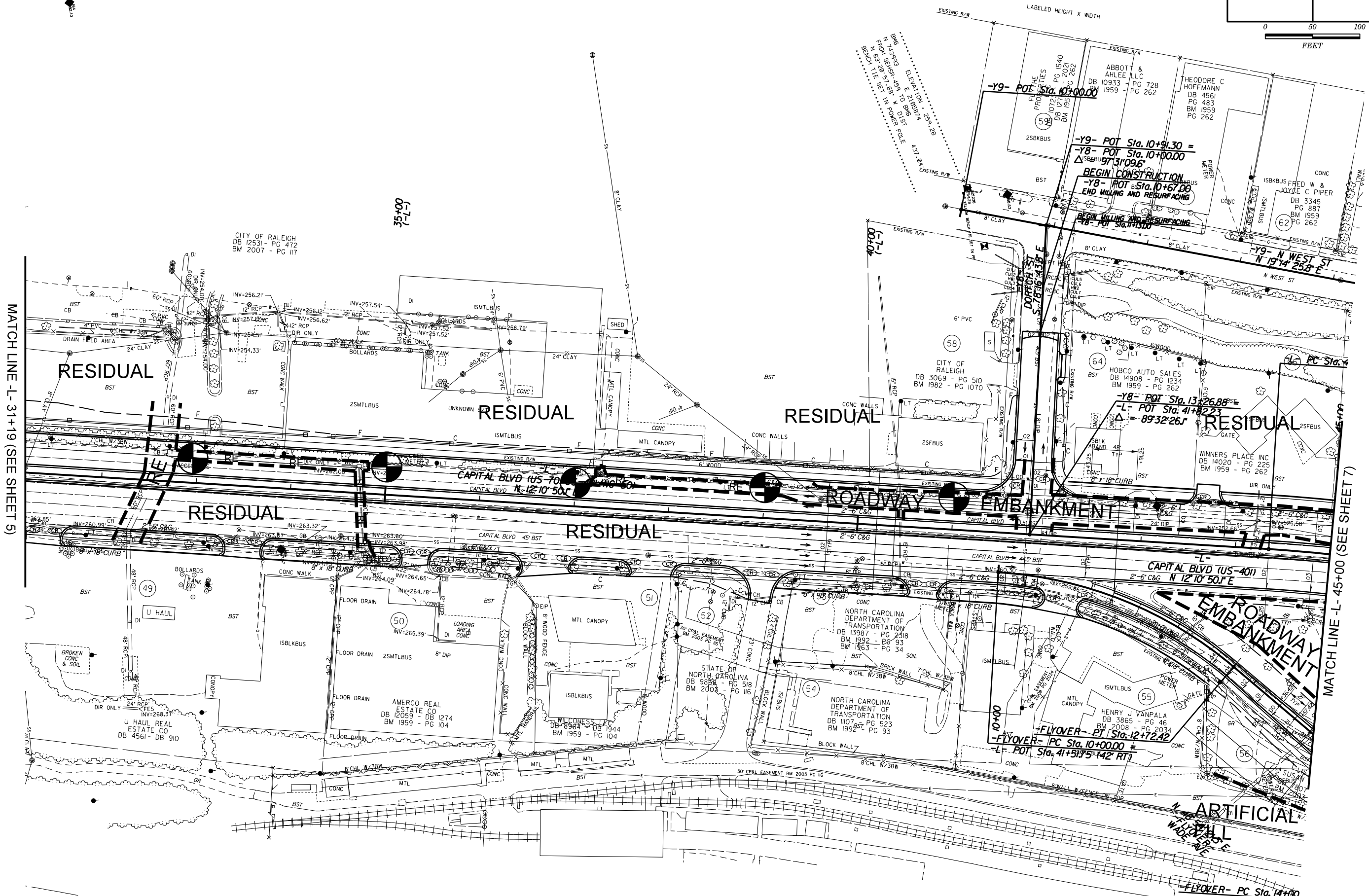
MATCH LINE -L- 31+19 (SEE SHEET 6)

MATCH LINE -L- 18+00 (SEE SHEET 4)



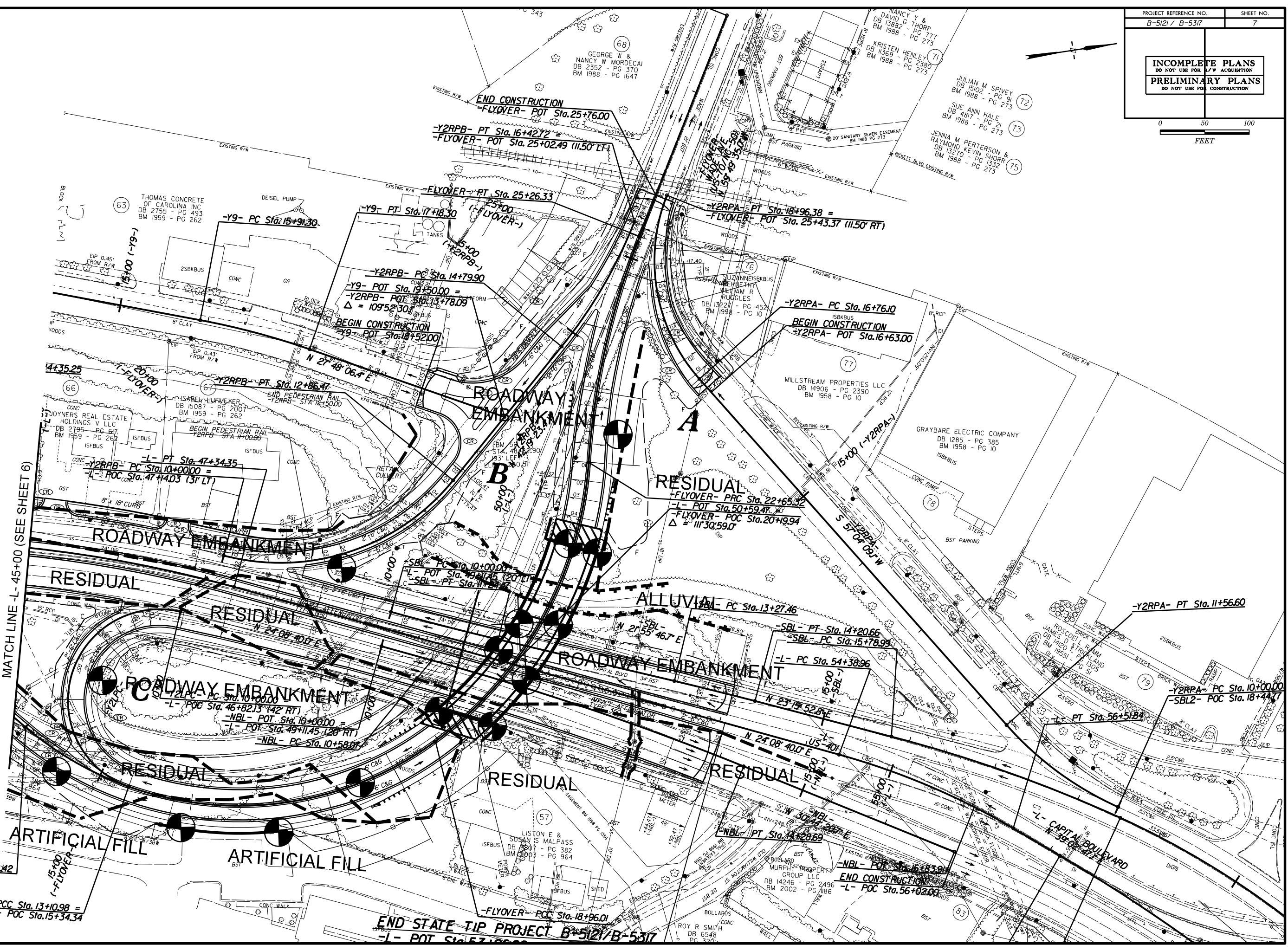
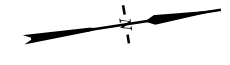
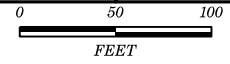
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PROJECT REFERENCE NO. B-5121 / B-5317	SHEET NO. 6
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MATCH LINE -L- 31+19 (SEE SHEET 5)

MATCH LINE -L- 45+00 (SEE SHEET 7)

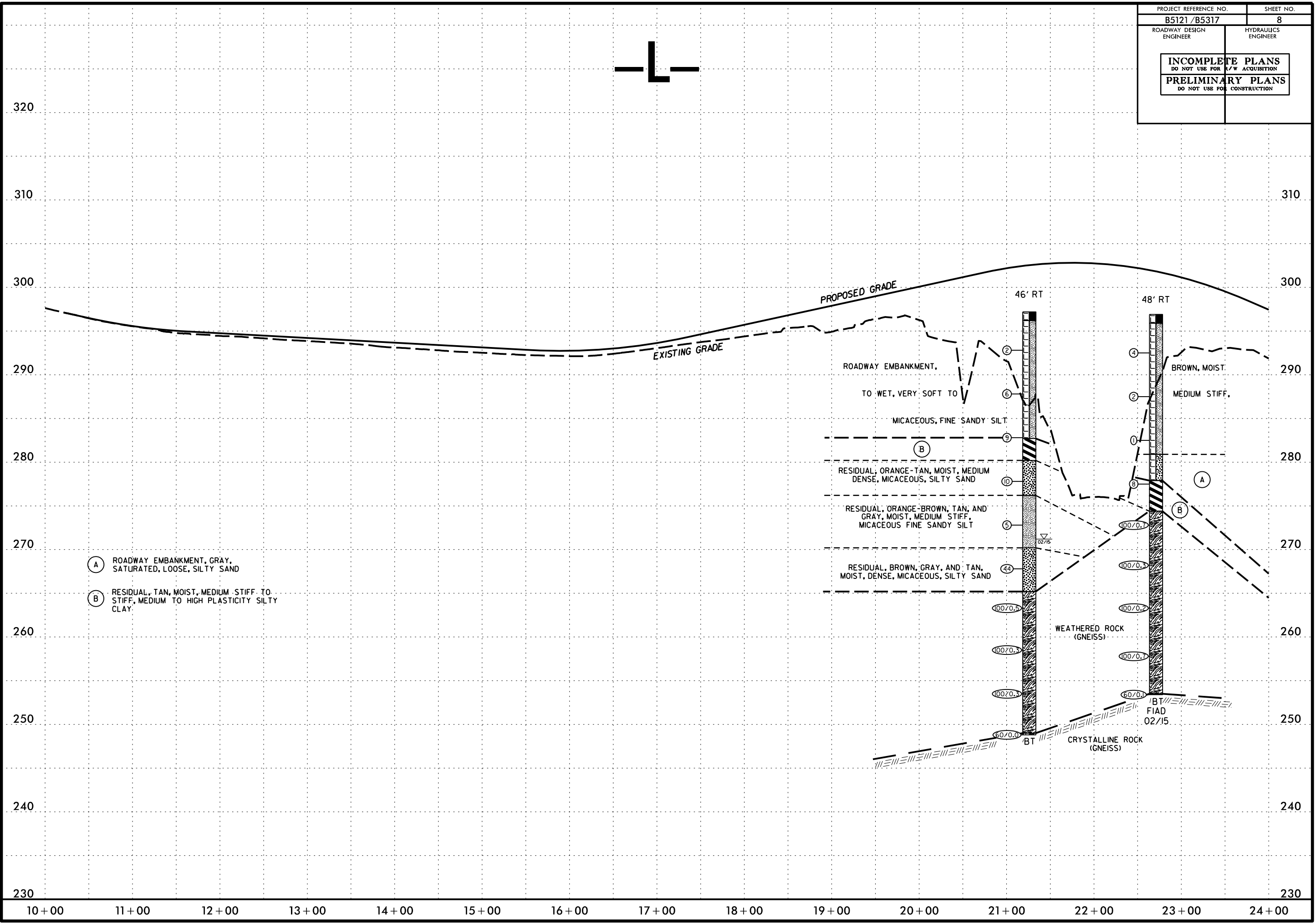


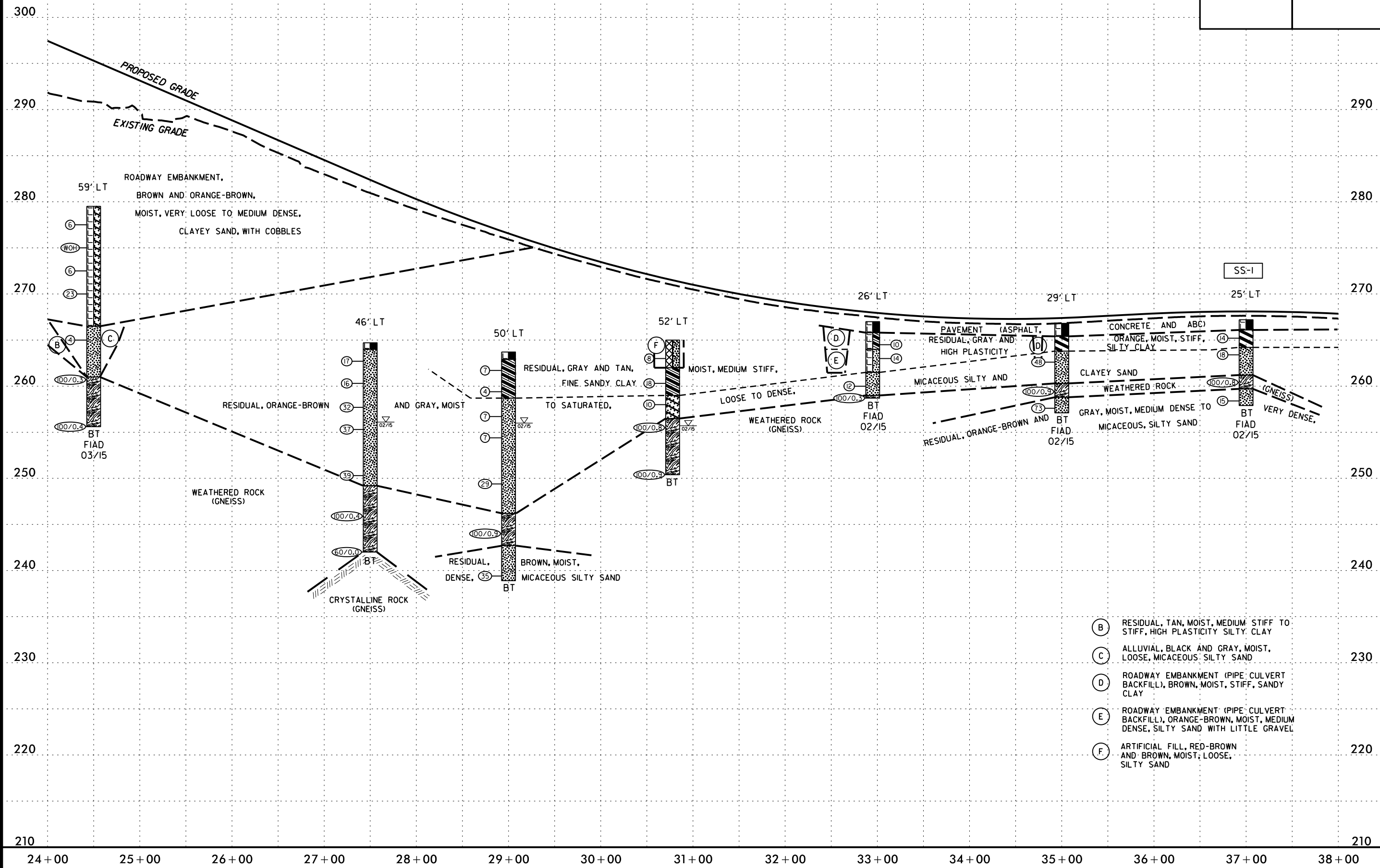
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END STATE TIP PROJECT B-5121/B-5317  
-L- POT Sta. 57+10.00

PROJECT REFERENCE NO. B5121 / B5317	SHEET NO. 8
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

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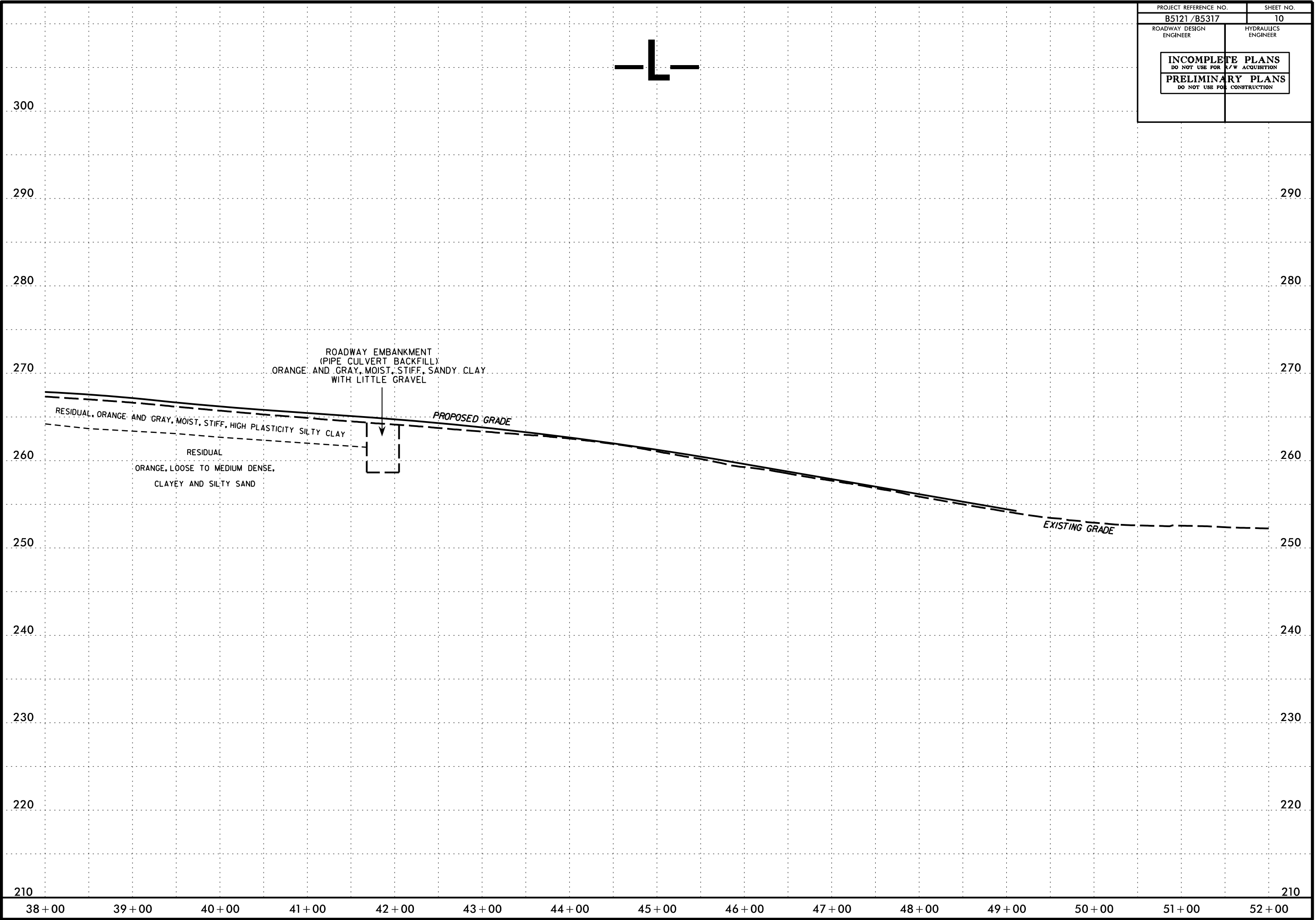


- (B) RESIDUAL, TAN, MOIST, MEDIUM STIFF TO STIFF, HIGH PLASTICITY SILTY CLAY
- (C) ALLUVIAL, BLACK AND GRAY, MOIST, LOOSE, MICACEOUS SILTY SAND
- (D) ROADWAY EMBANKMENT (PIPE CULVERT BACKFILL), BROWN, MOIST, STIFF, SANDY CLAY
- (E) ROADWAY EMBANKMENT (PIPE CULVERT BACKFILL), ORANGE-BROWN, MOIST, MEDIUM DENSE, SILTY SAND WITH LITTLE GRAVEL
- (F) ARTIFICIAL FILL, RED-BROWN AND BROWN, MOIST, LOOSE, SILTY SAND

5/14/99  
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PROJECT REFERENCE NO.	SHEET NO.
B5121 / B5317	10
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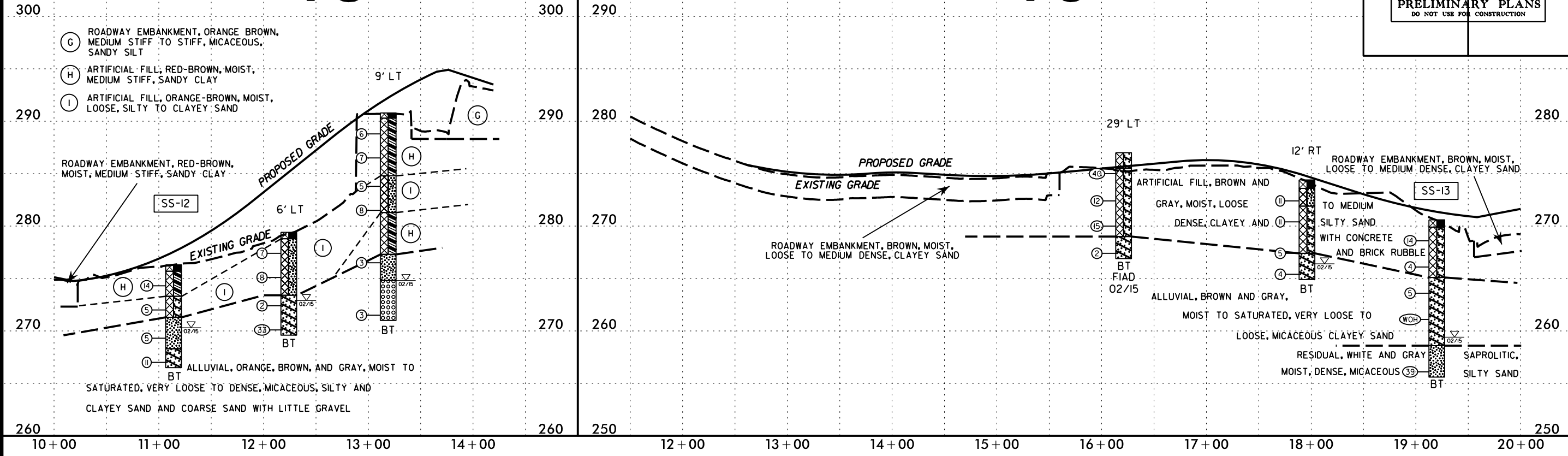
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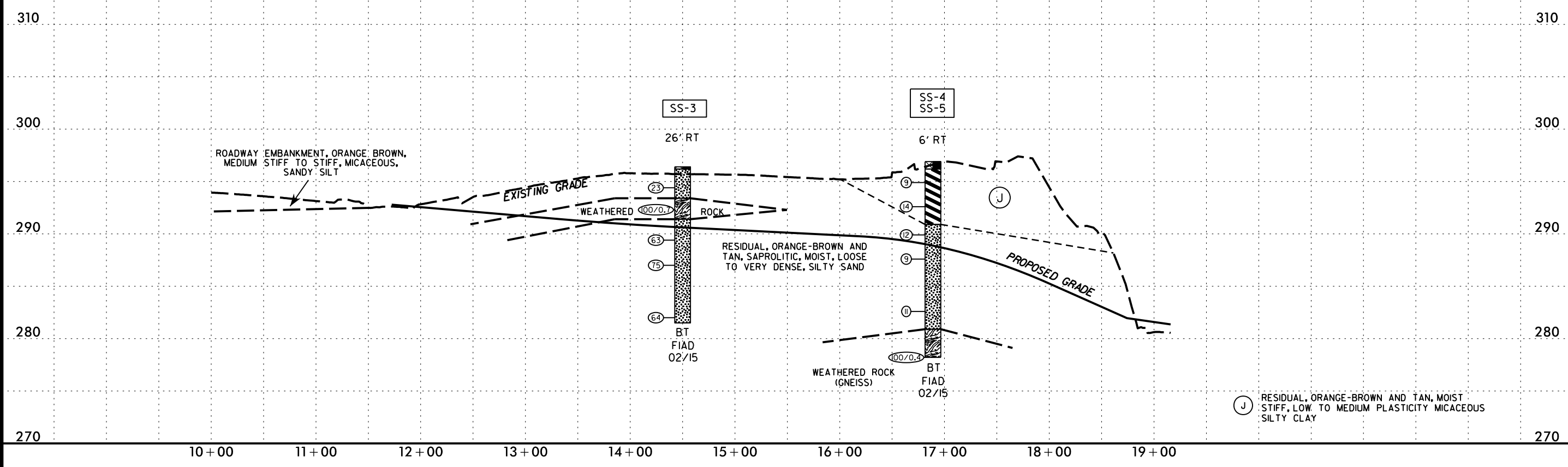
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<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

# -Y5-

# -Y6-



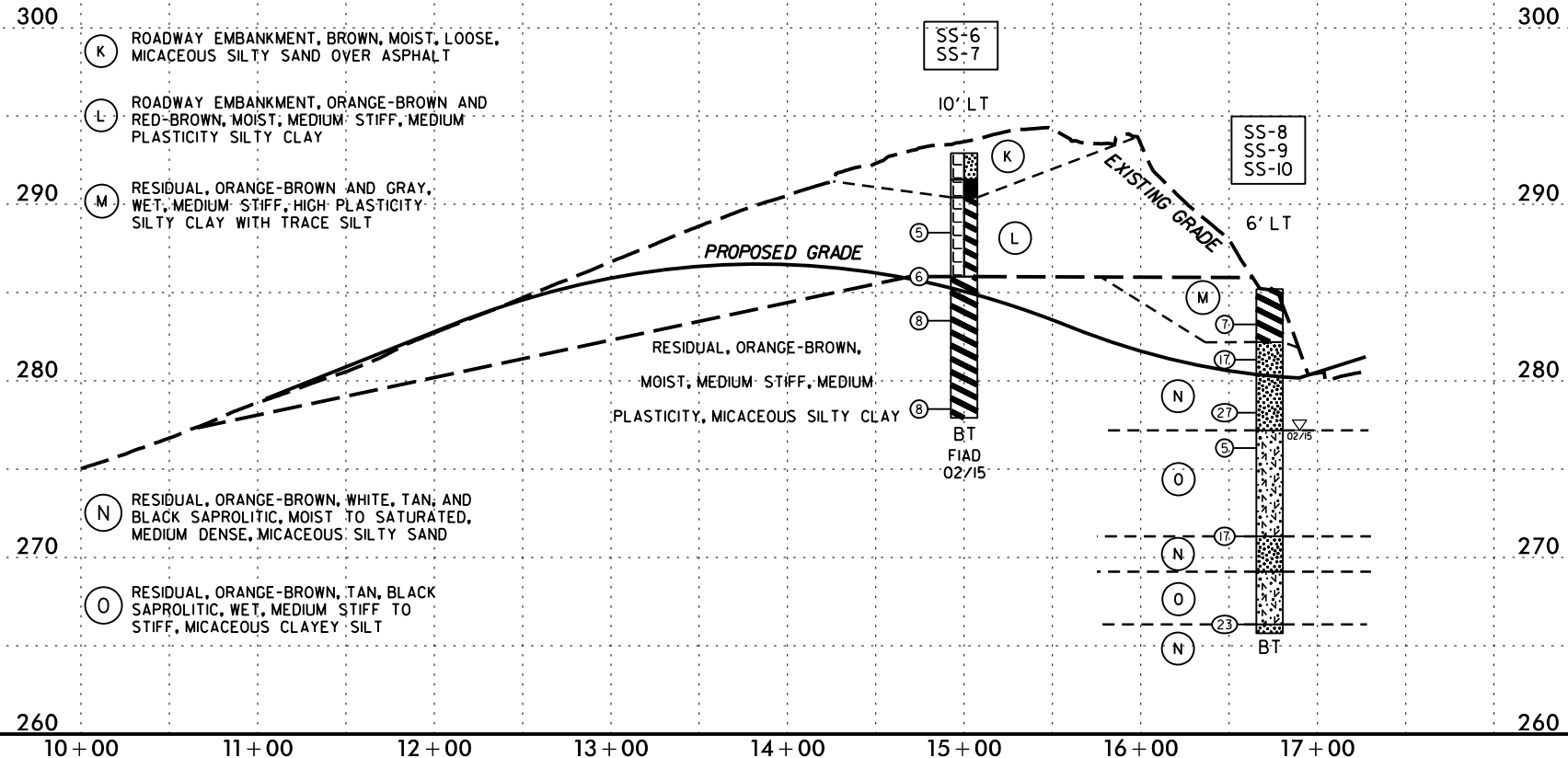
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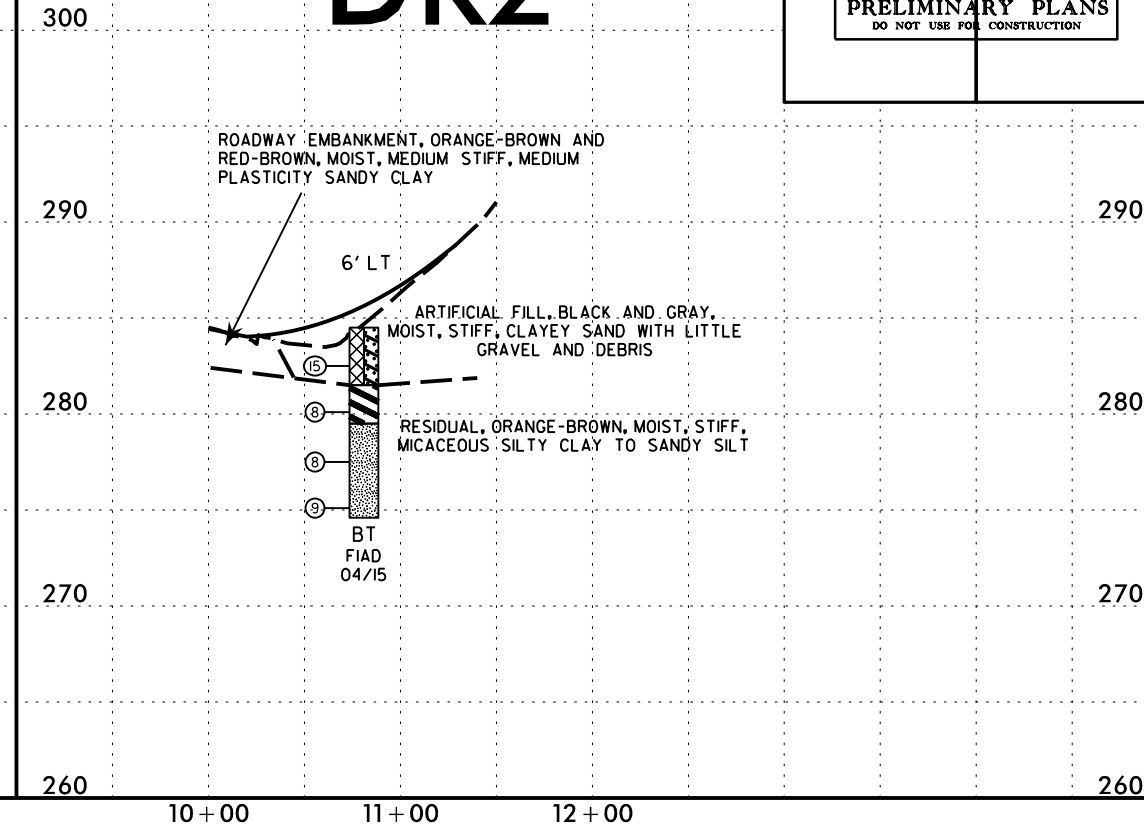
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5/28/99

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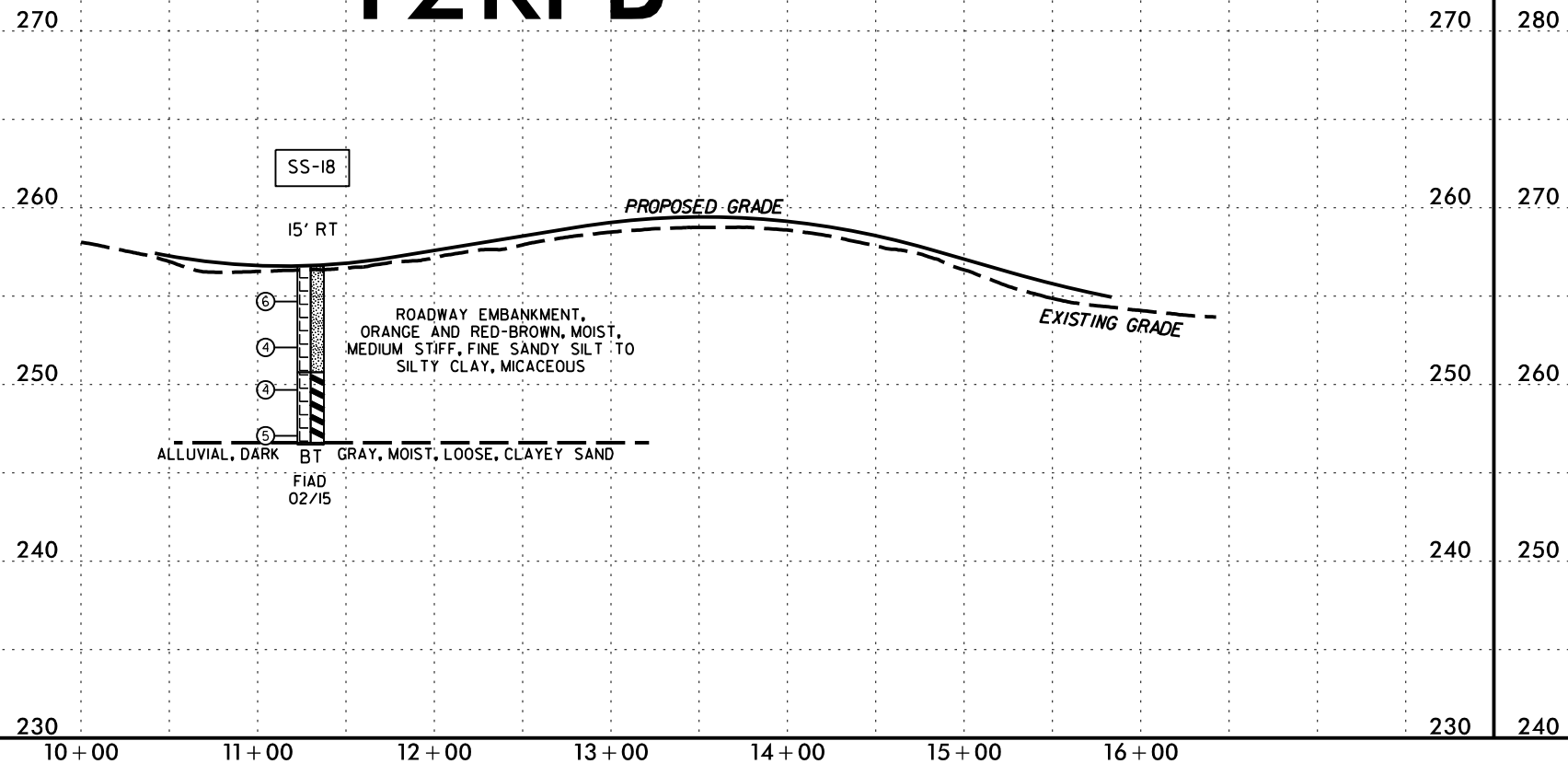


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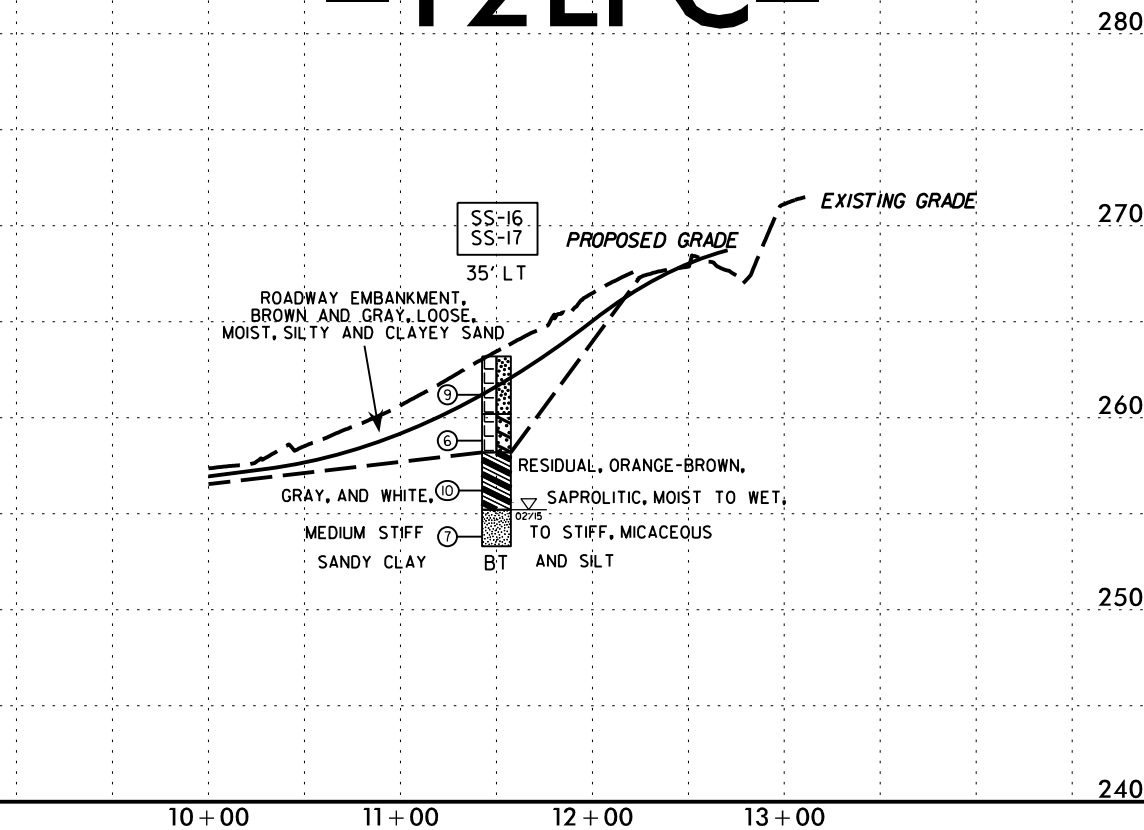


PROJECT REFERENCE NO. B5121/B5317	SHEET NO. 12
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<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

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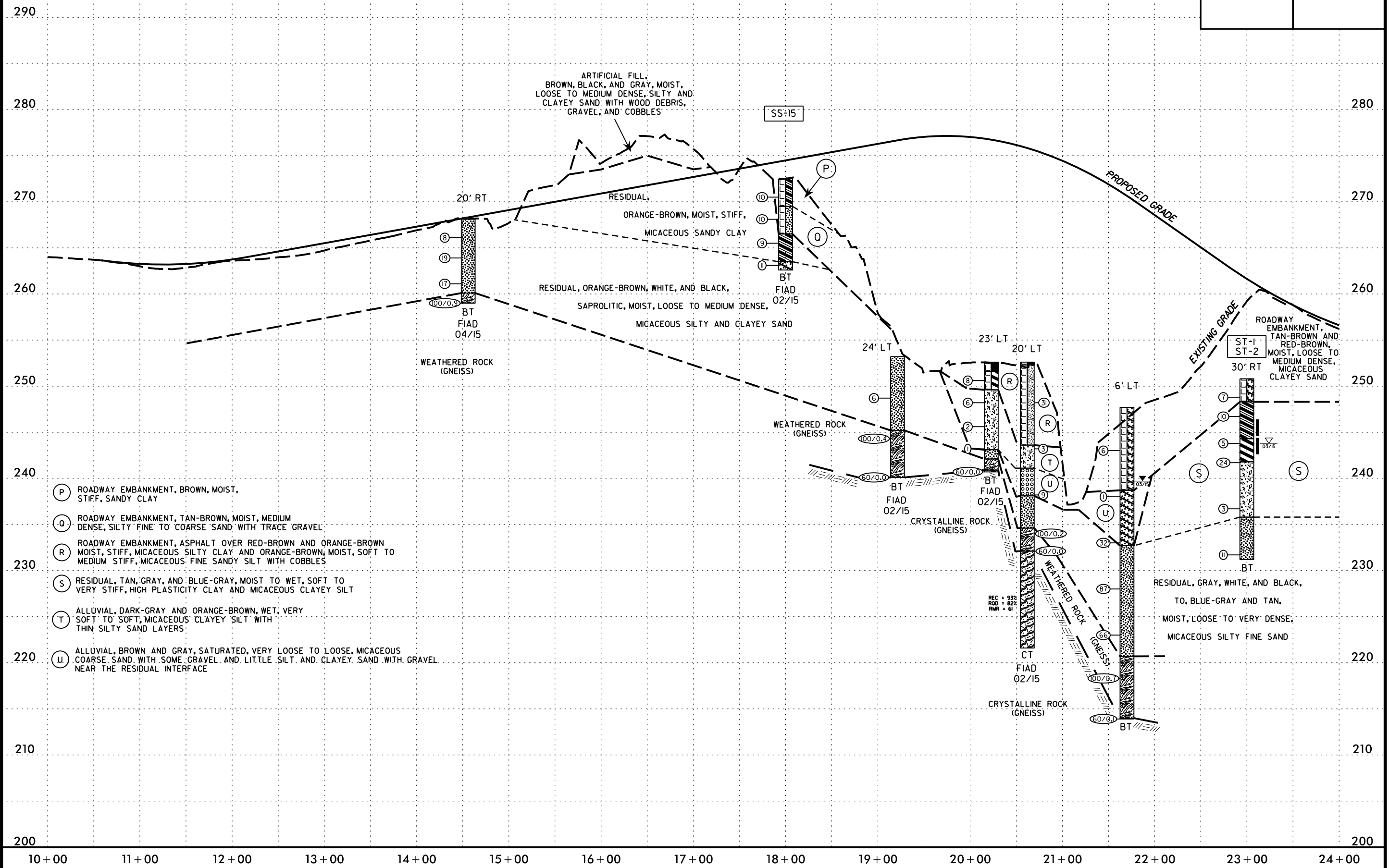
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PROJECT REFERENCE NO. B5121 / B5317	SHEET NO. 13
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR A/W ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

# -FLYOVER-



- (P) ROADWAY EMBANKMENT, BROWN, MOIST, STIFF, SANDY CLAY
- (Q) ROADWAY EMBANKMENT, TAN-BROWN, MOIST, MEDIUM DENSE, SILTY FINE TO COARSE SAND WITH TRACE GRAVEL
- (R) ROADWAY EMBANKMENT, ASPHALT OVER RED-BROWN AND ORANGE-BROWN MOIST, STIFF, MICACEOUS SILTY CLAY AND ORANGE-BROWN, MOIST, SOFT TO MEDIUM STIFF, MICACEOUS FINE SANDY SILT WITH COBBLES
- (S) RESIDUAL, TAN, GRAY, AND BLUE-GRAY, MOIST TO WET, SOFT TO VERY STIFF, HIGH PLASTICITY CLAY AND MICACEOUS CLAYEY SILT
- (T) ALLUVIAL, DARK-GRAY AND ORANGE-BROWN, WET, VERY SOFT TO SOFT, MICACEOUS CLAYEY SILT WITH THIN SILTY SAND LAYERS
- (U) ALLUVIAL, BROWN AND GRAY, SATURATED, VERY LOOSE TO LOOSE, MICACEOUS COARSE SAND WITH SOME GRAVEL AND LITTLE SILT AND CLAYEY SAND WITH GRAVEL NEAR THE RESIDUAL INTERFACE

REC = 93%  
 ROD = 82%  
 RMR = 61

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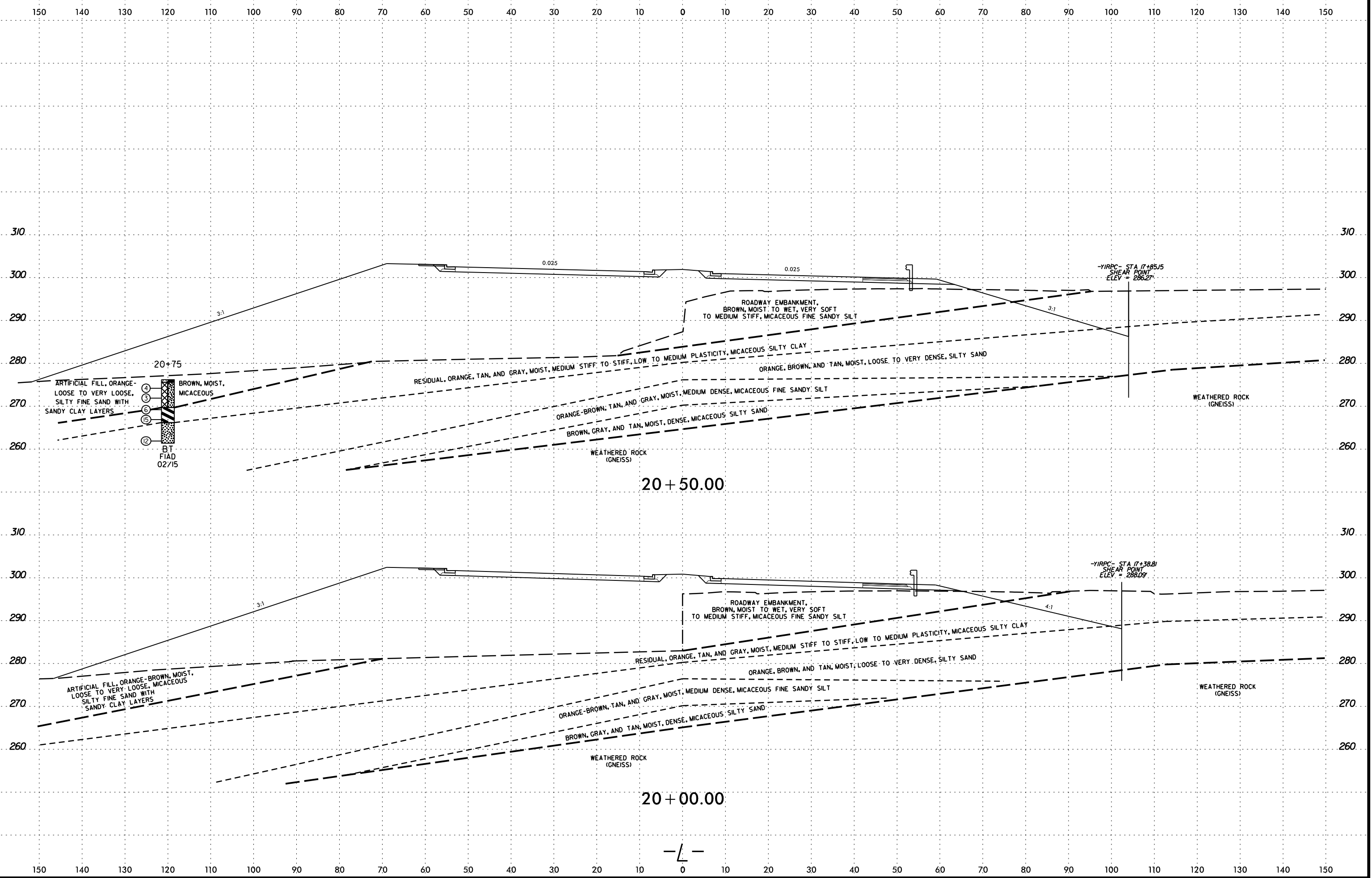
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B5121 / B5317	14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR ACQUISITION	
<b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

# -FLYOVER-



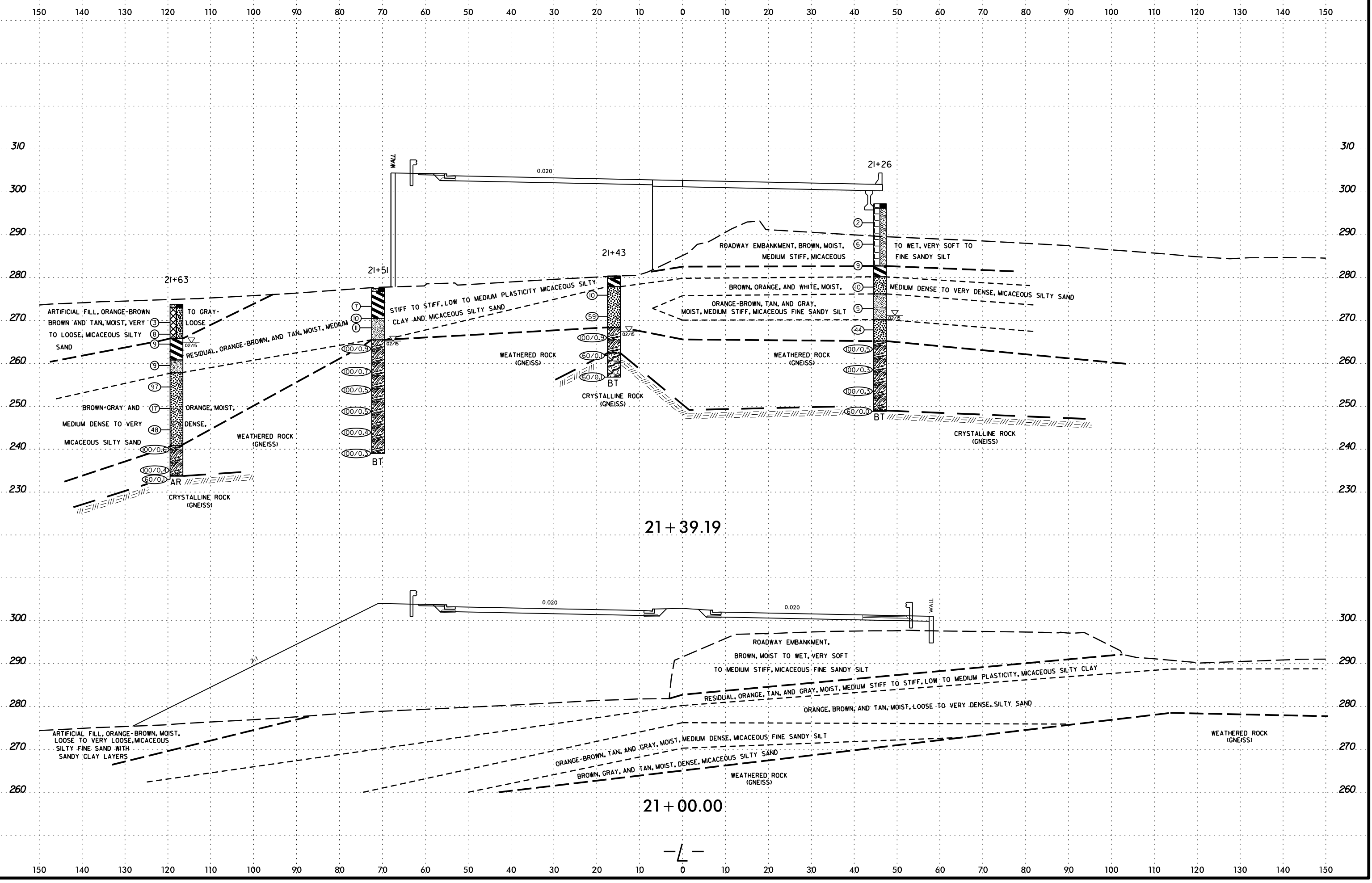
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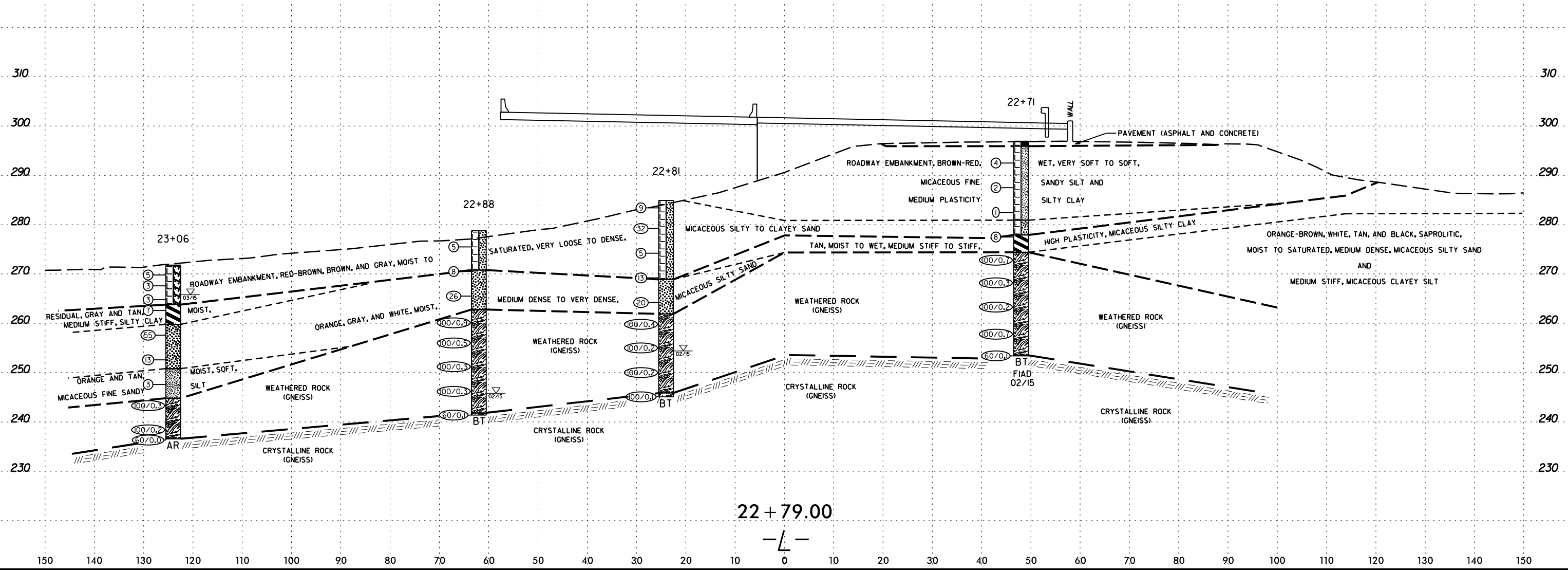
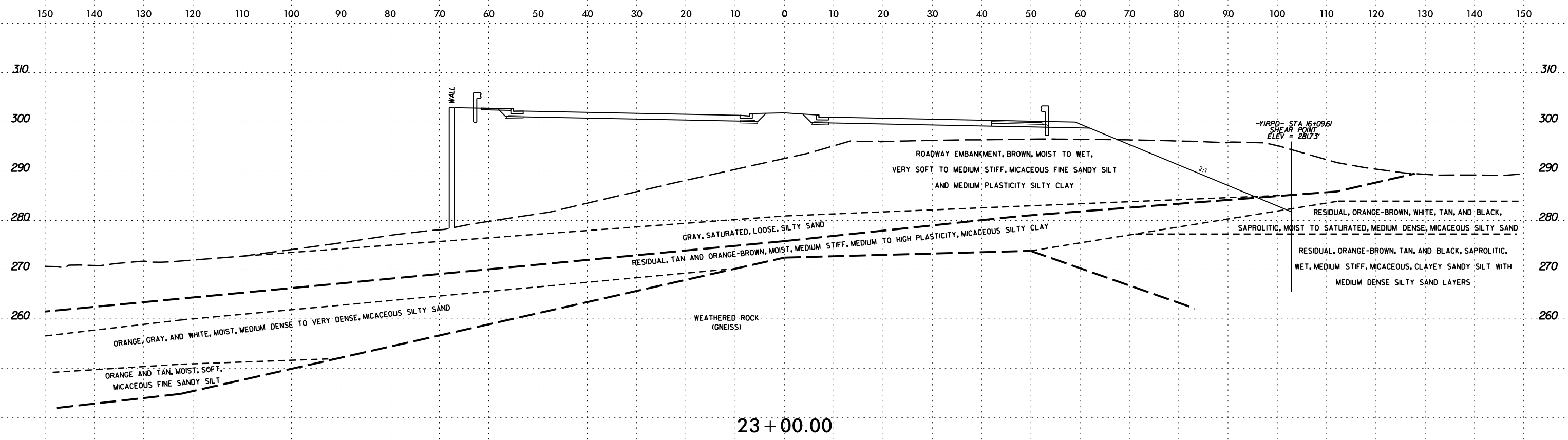
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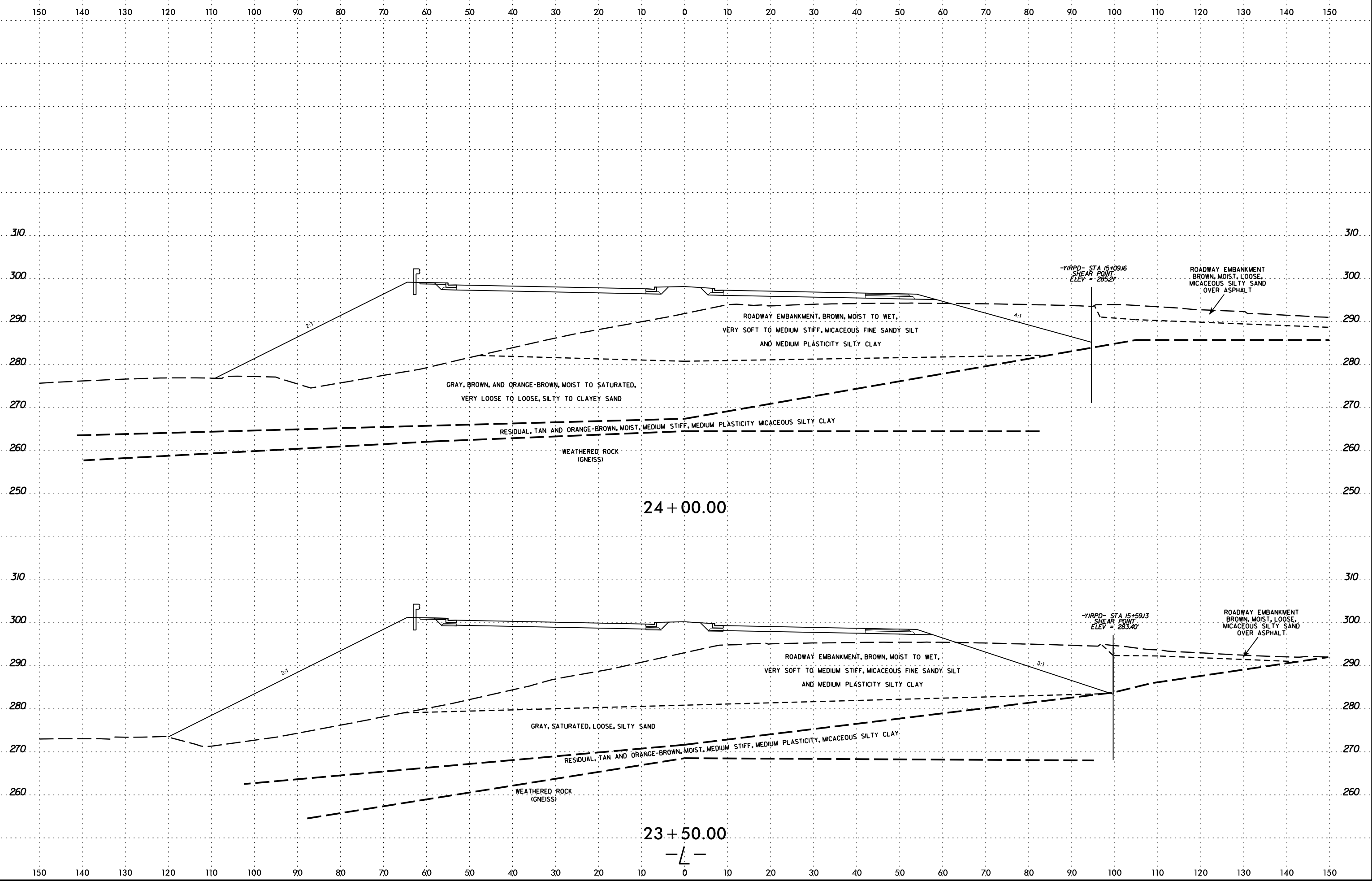
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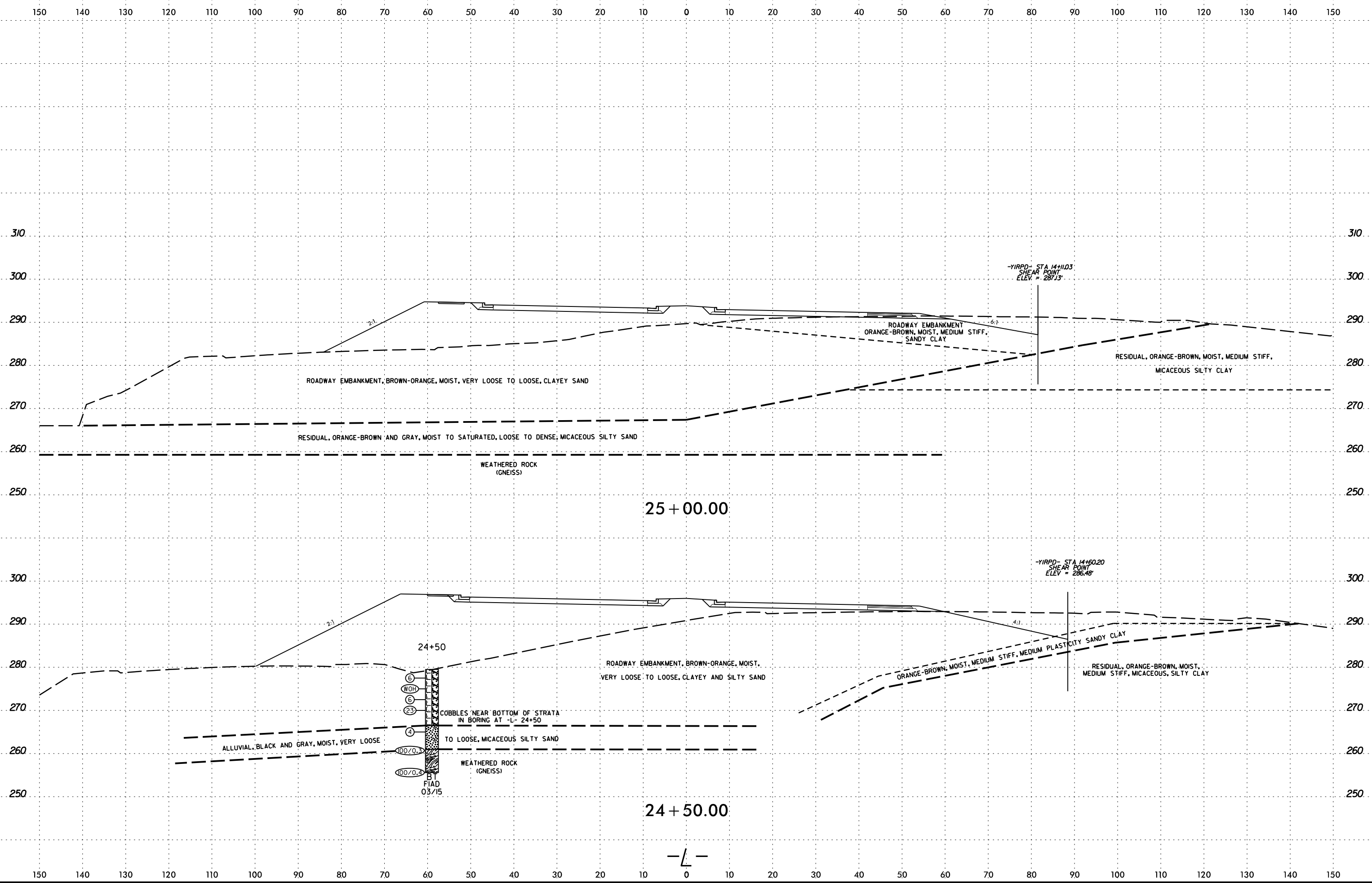




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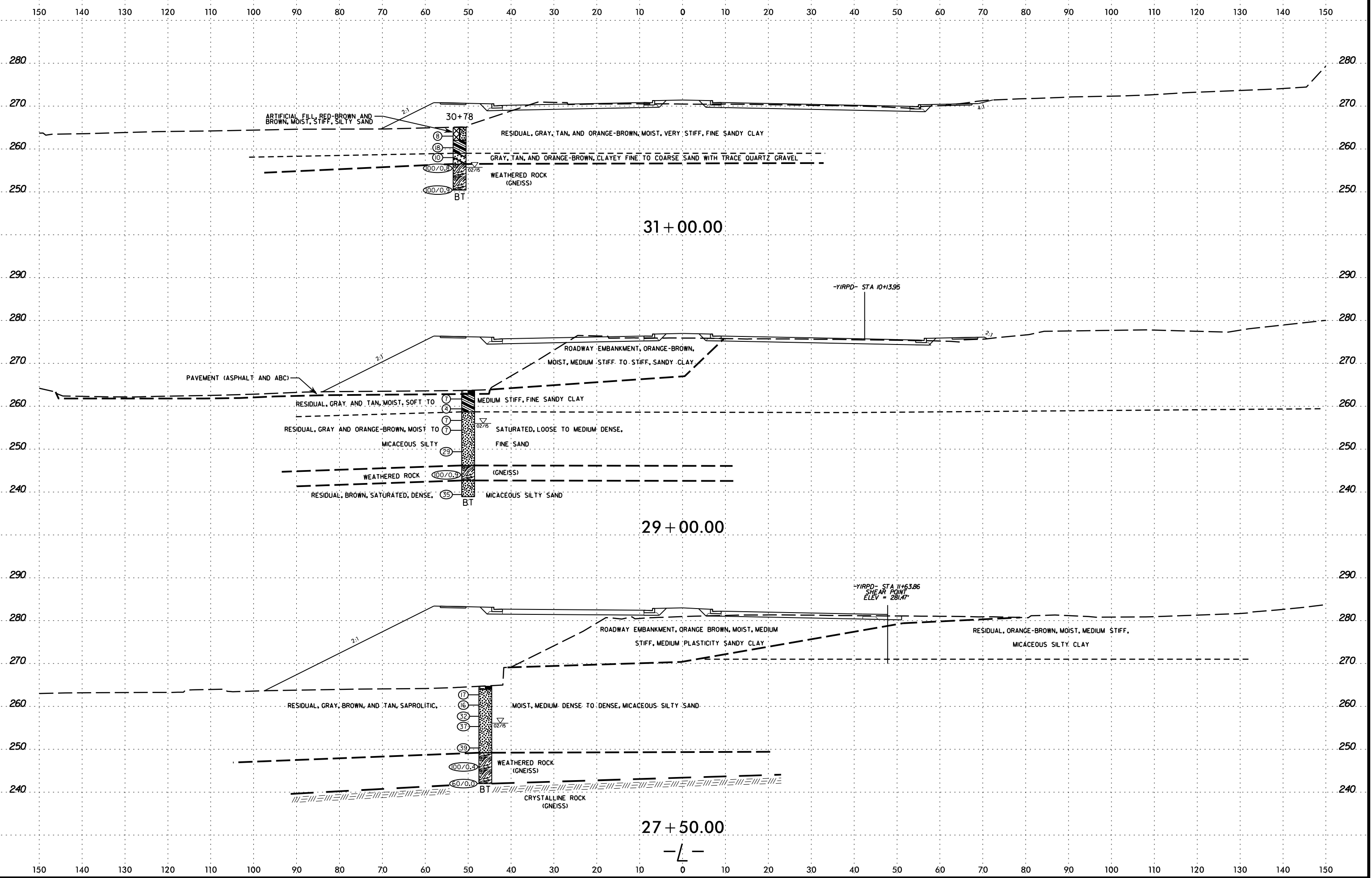


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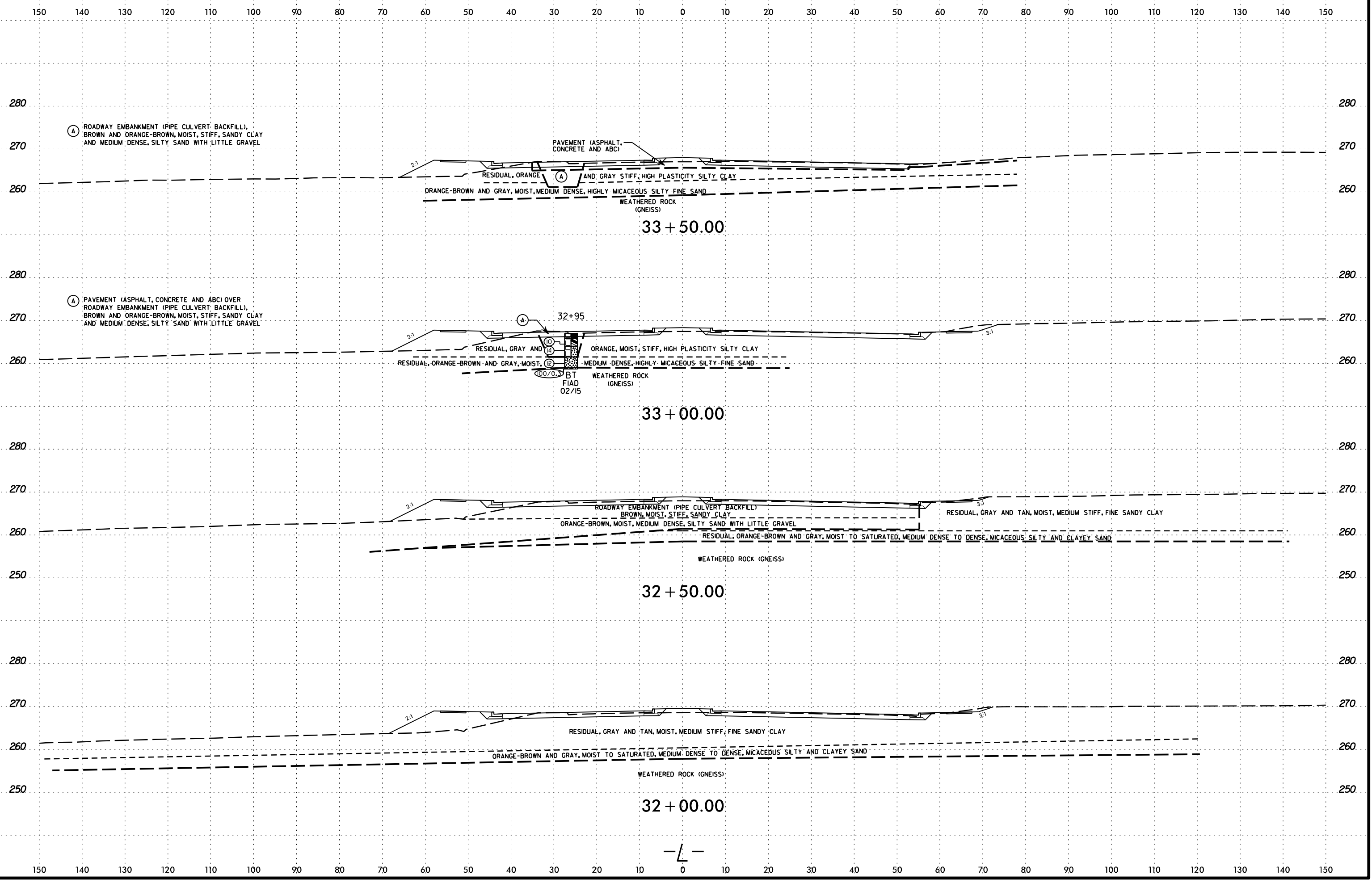
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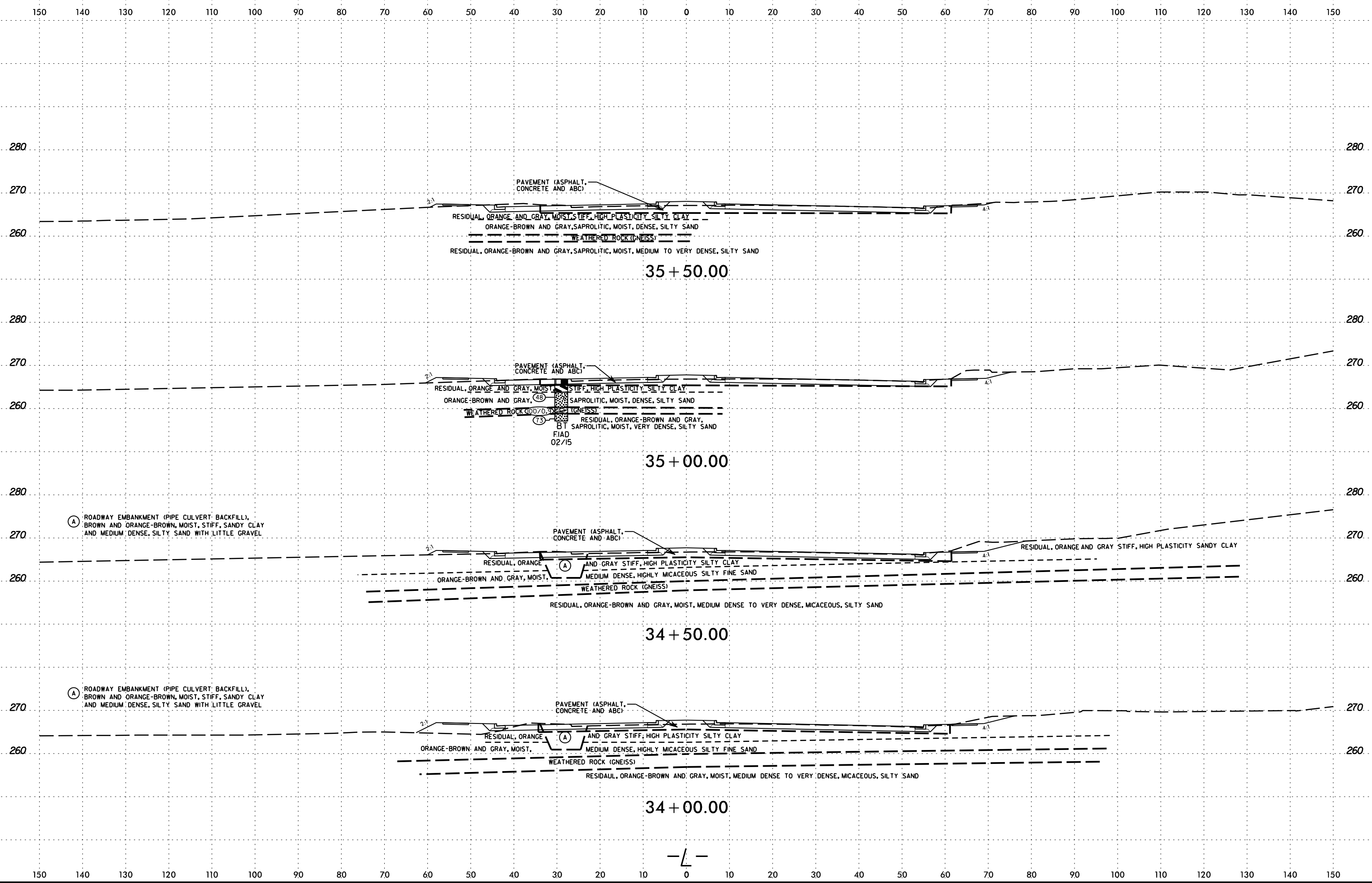
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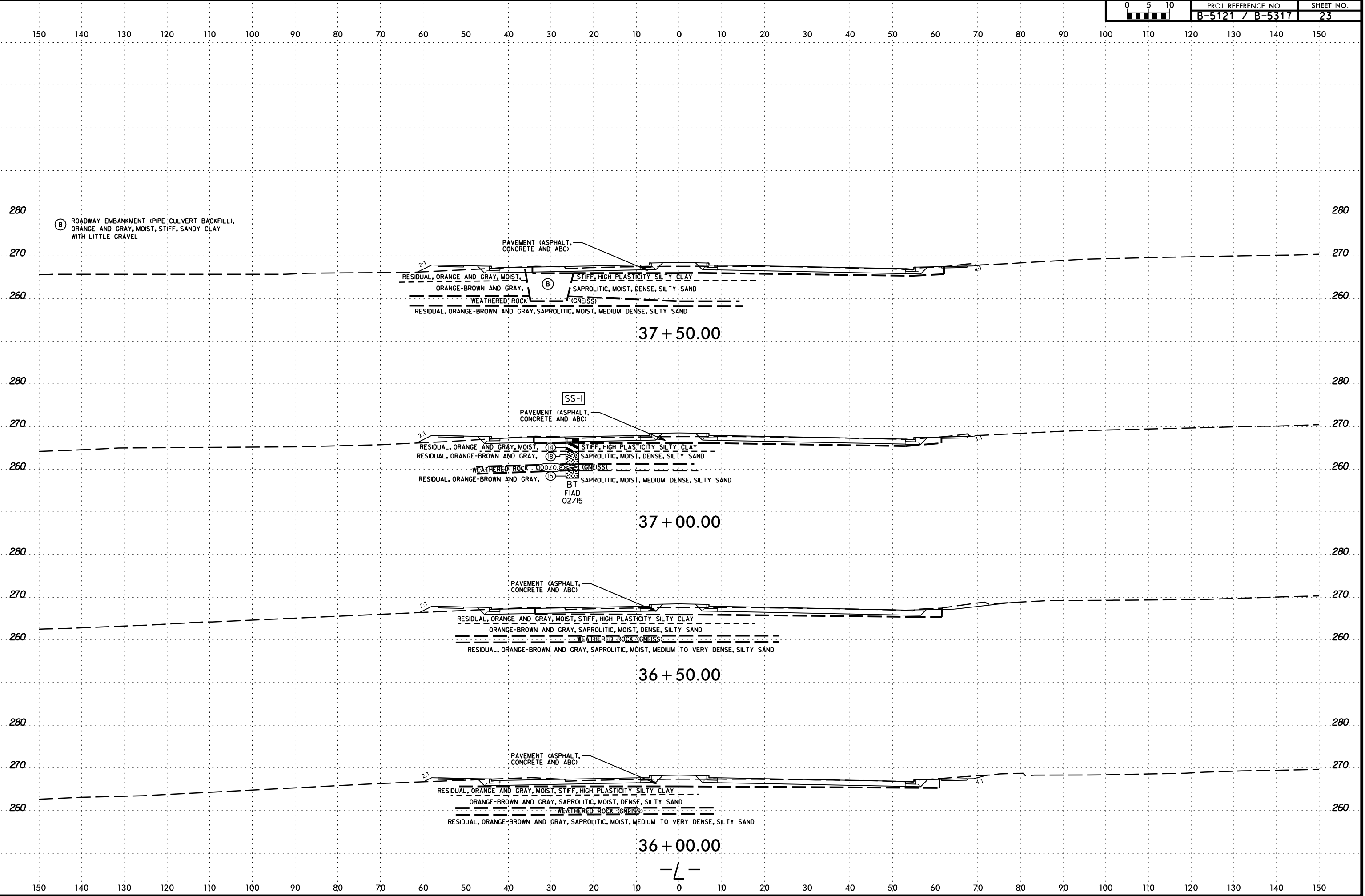
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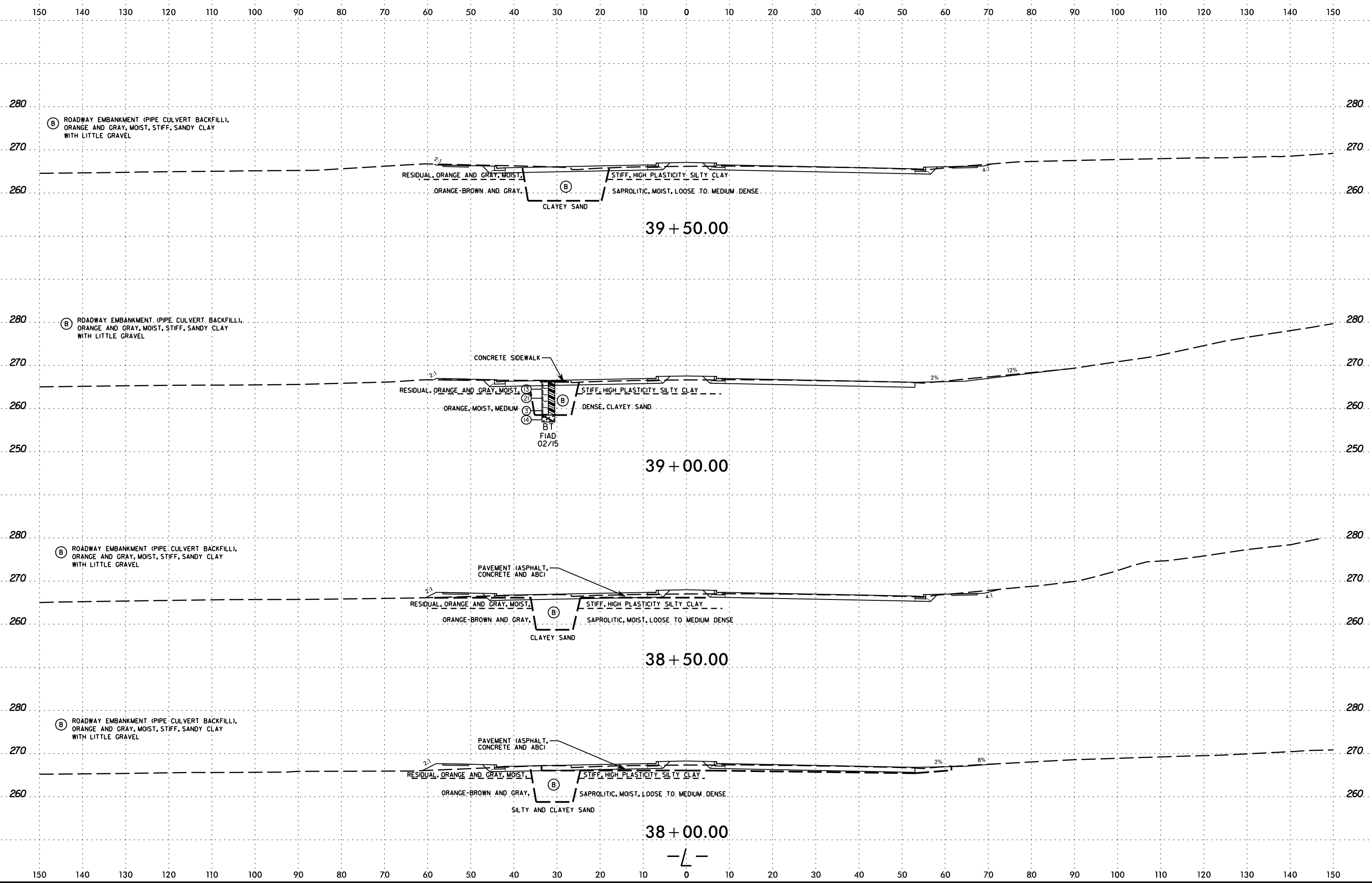
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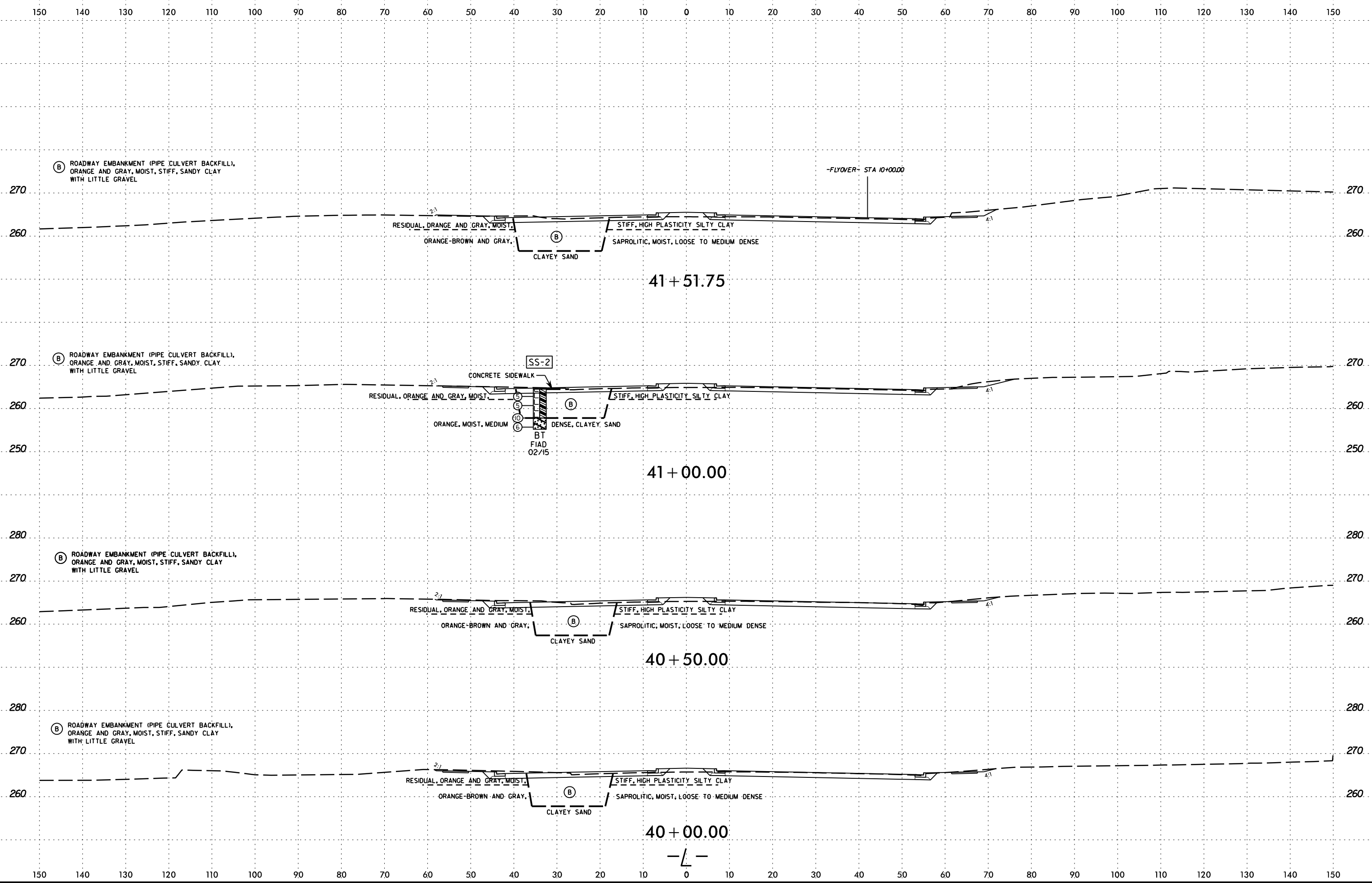
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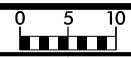


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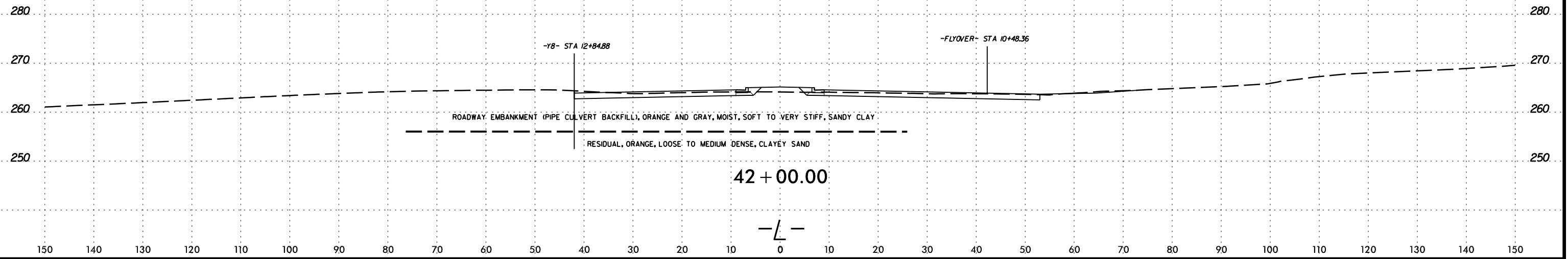
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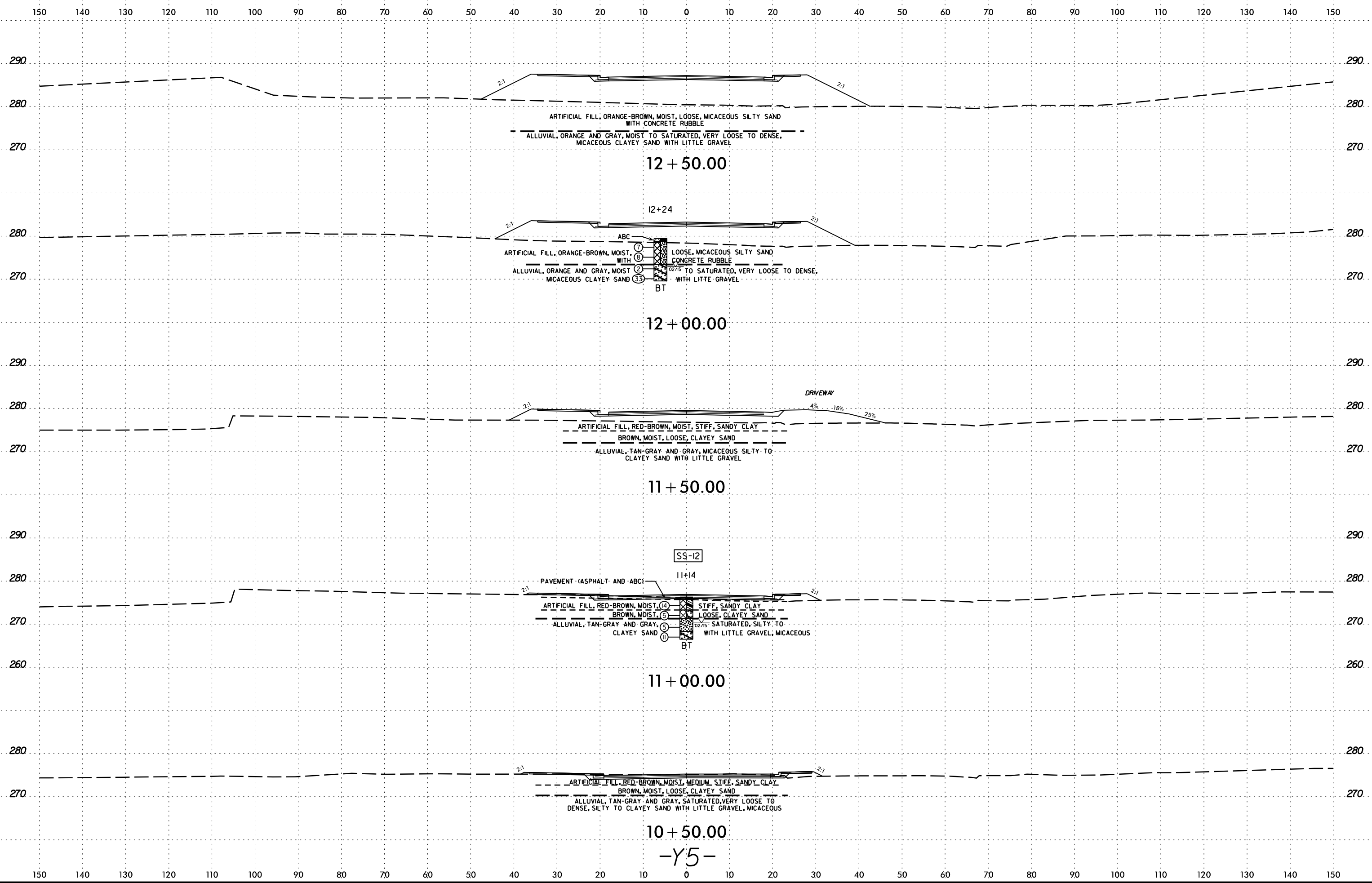
PROJ. REFERENCE NO.  
B-5121 / B-5317

SHEET NO.  
26

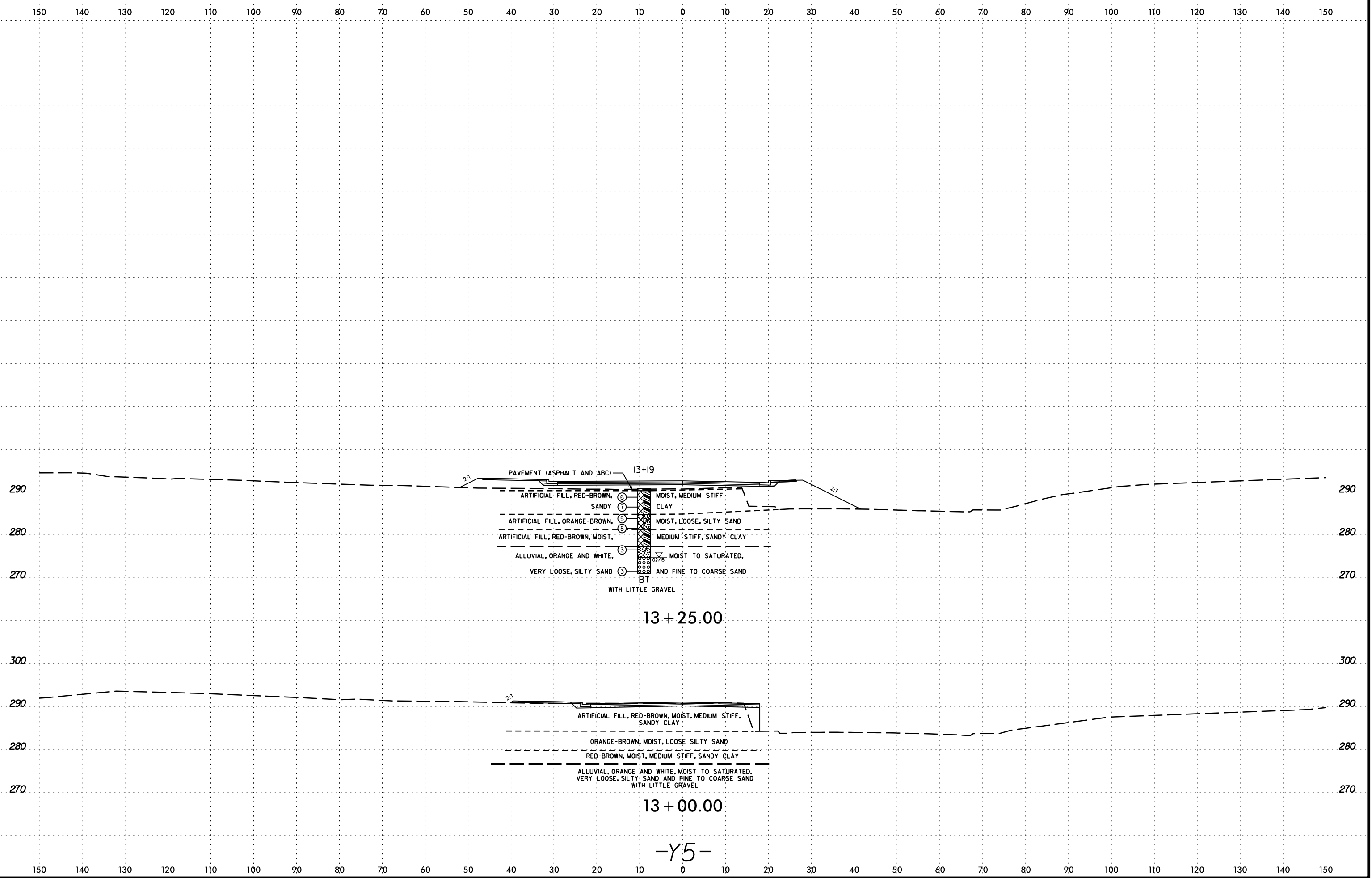
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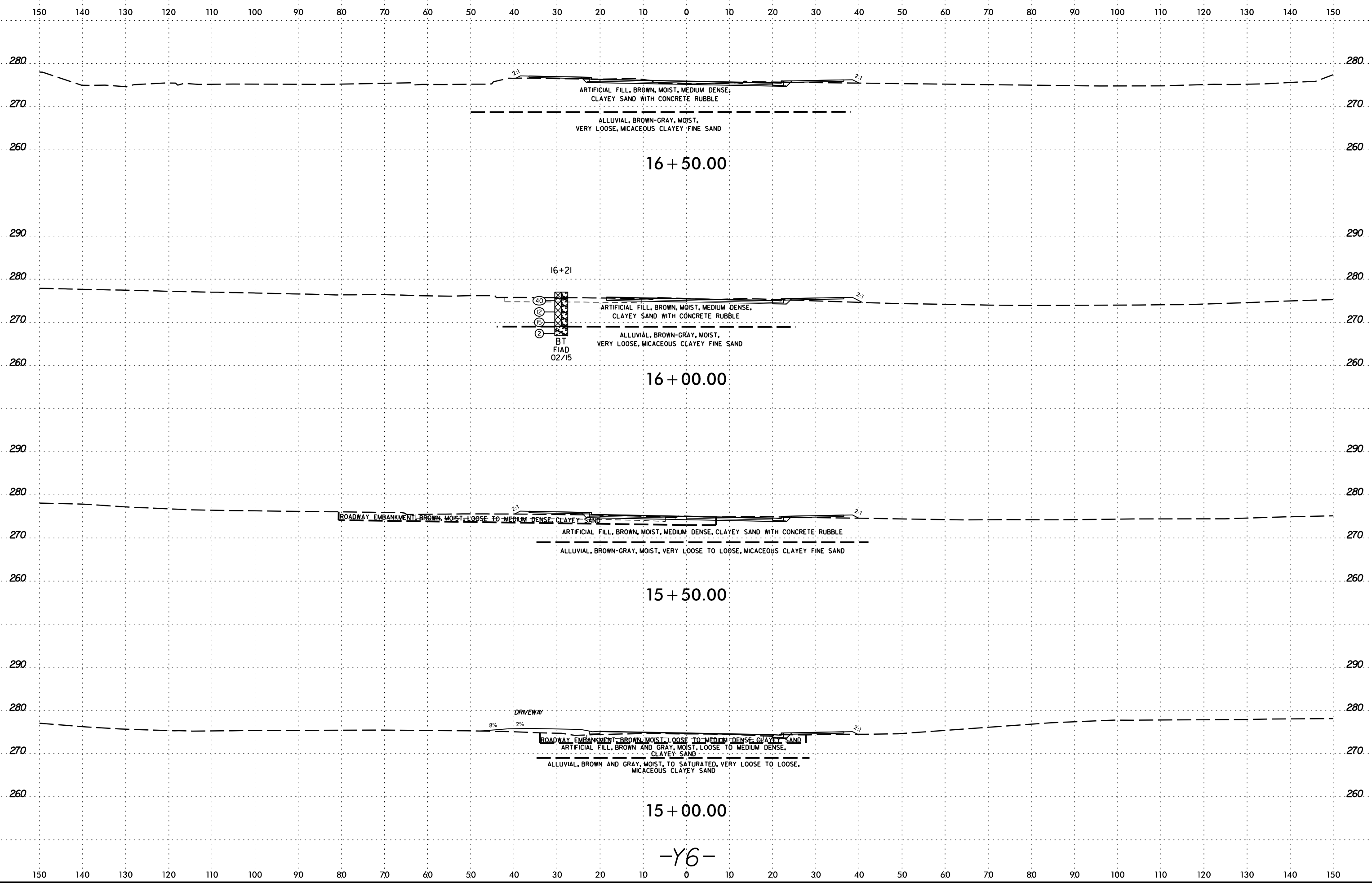
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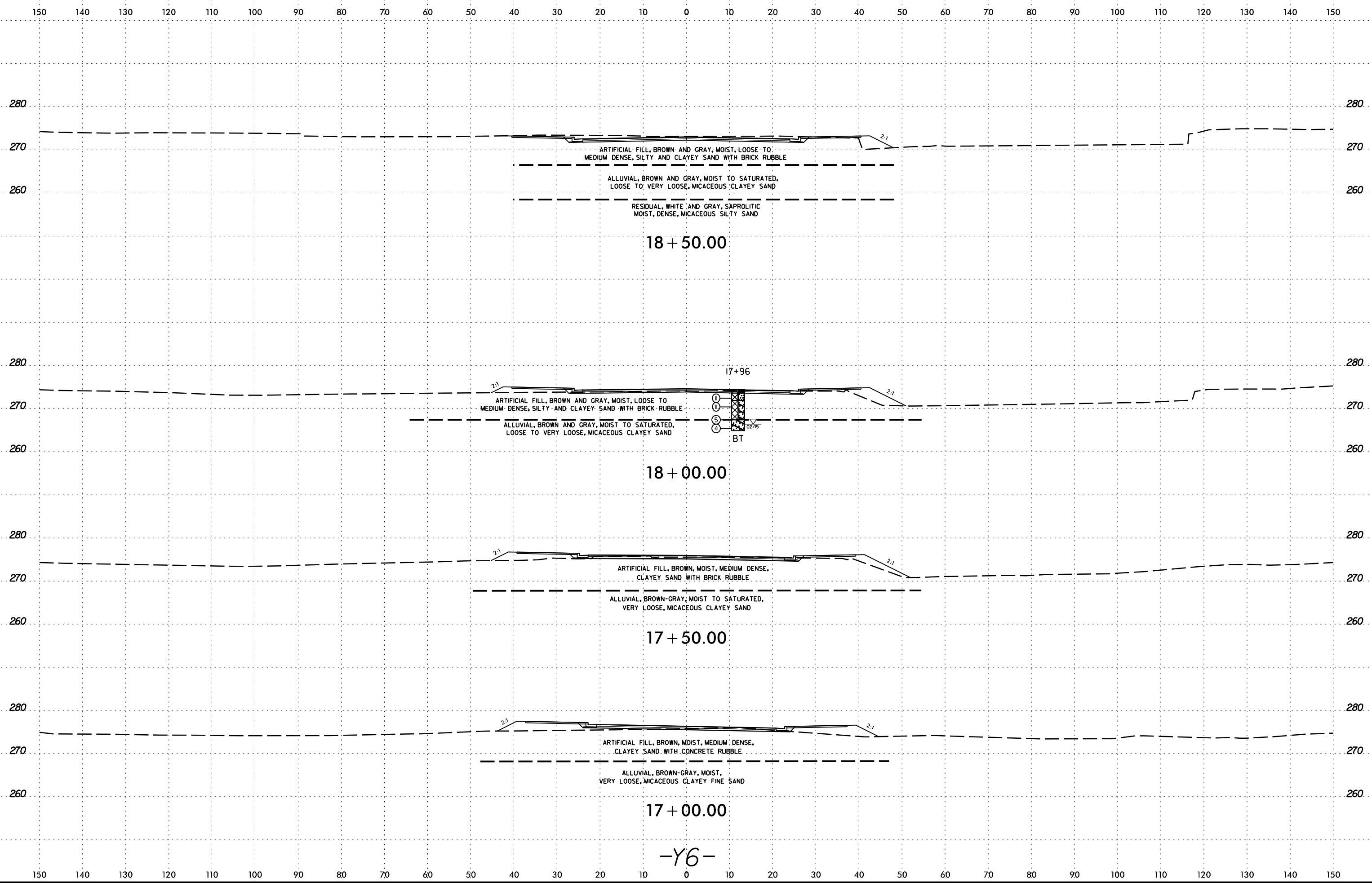


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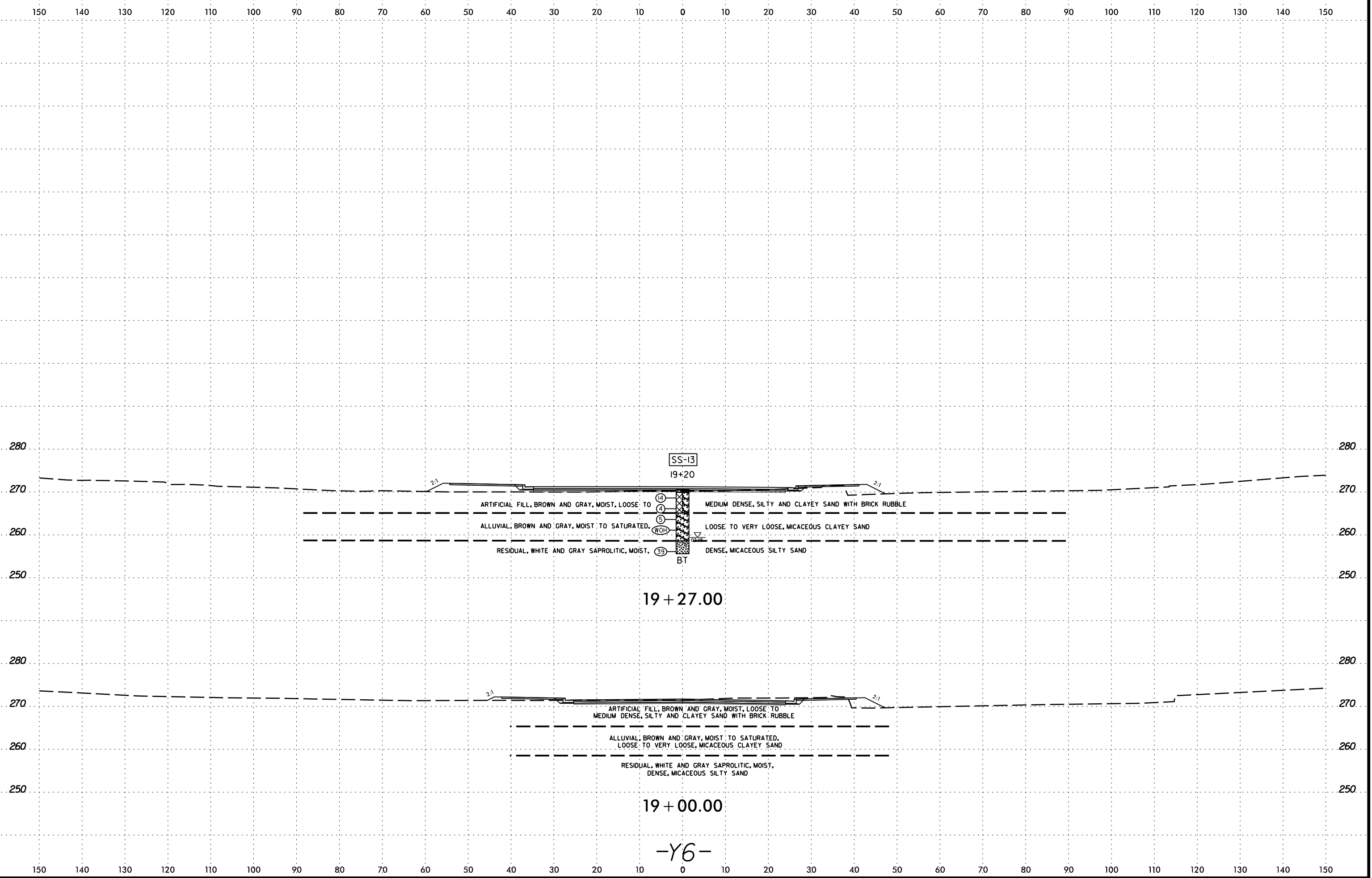
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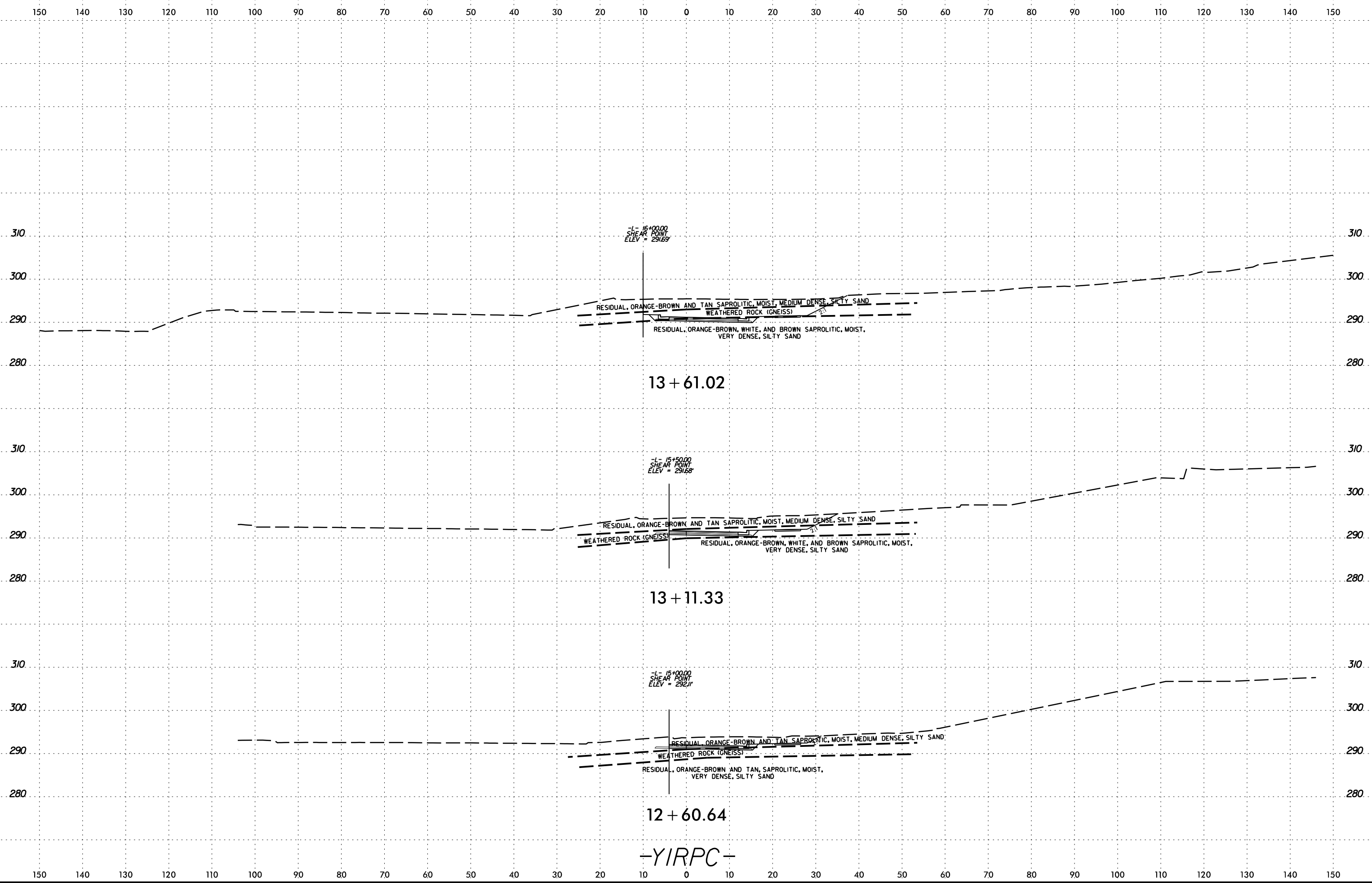
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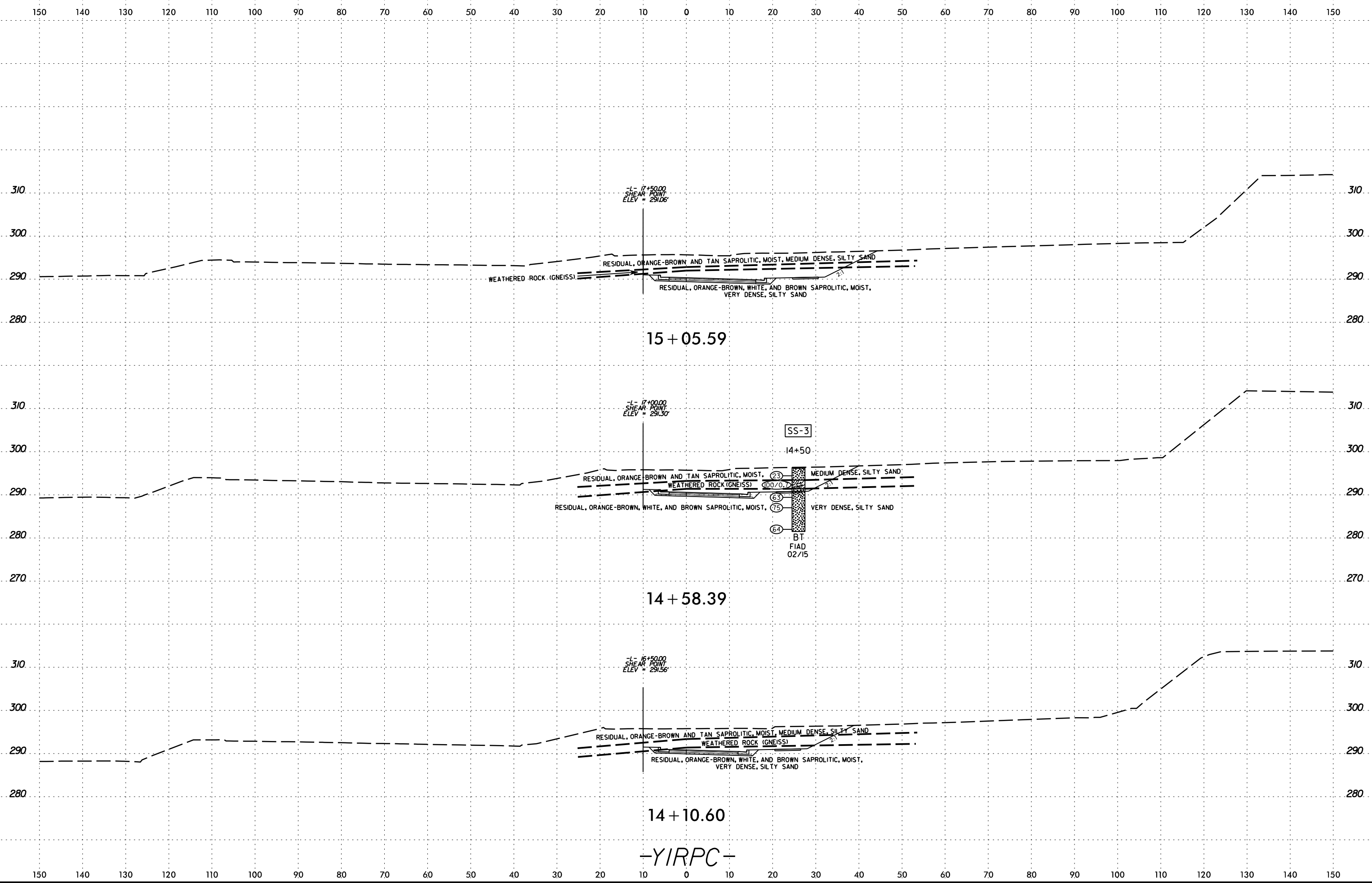
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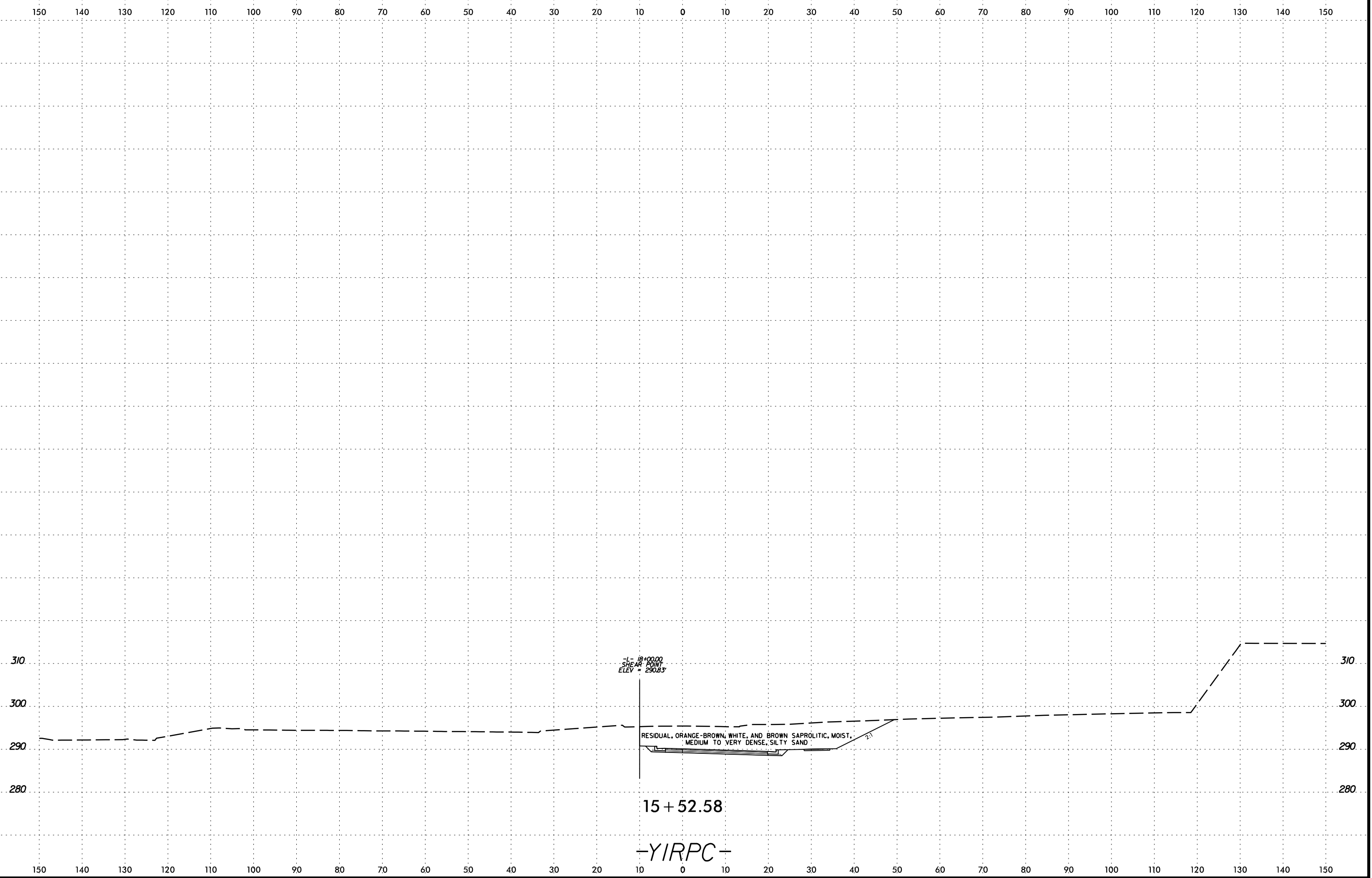
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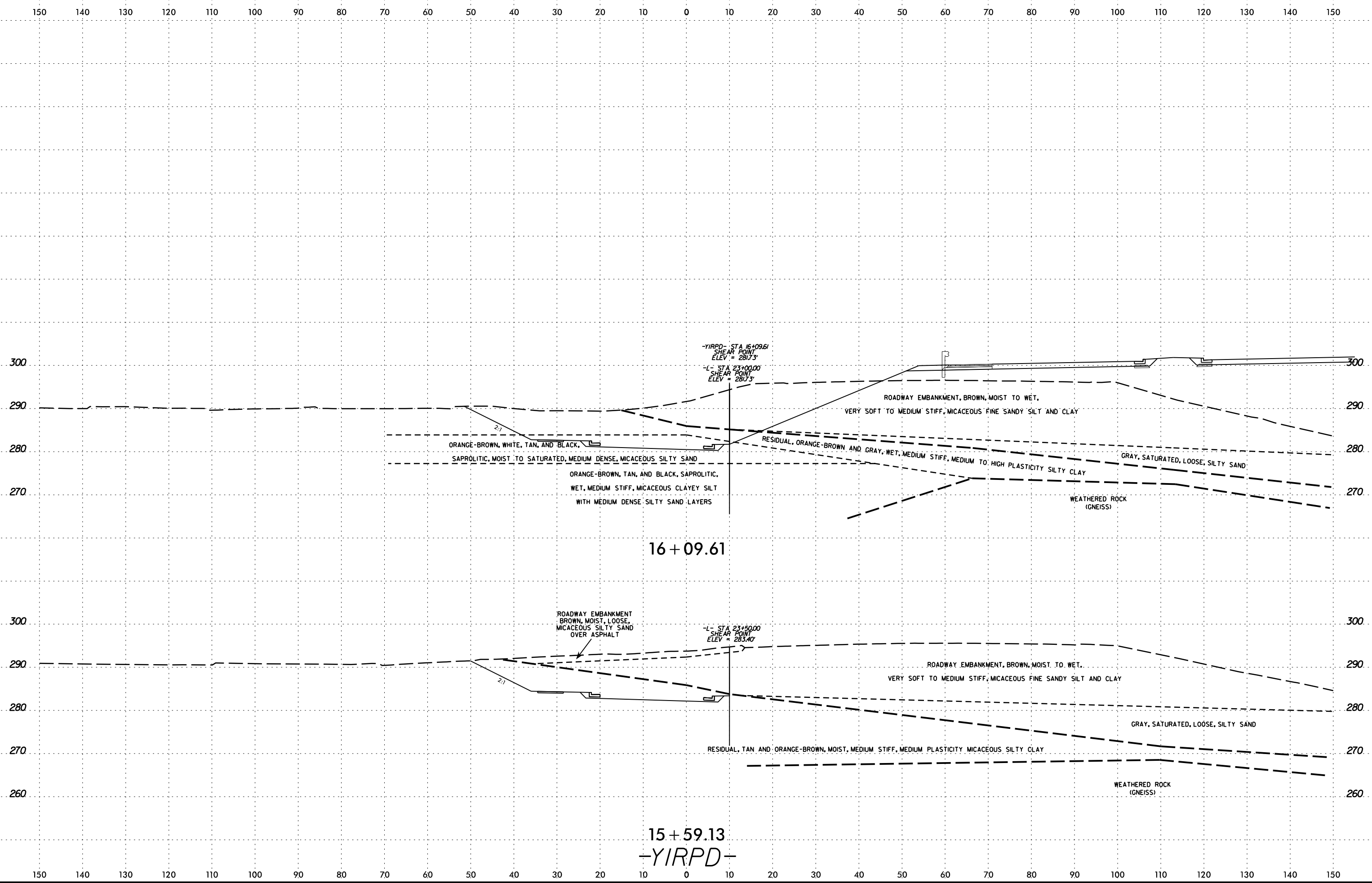
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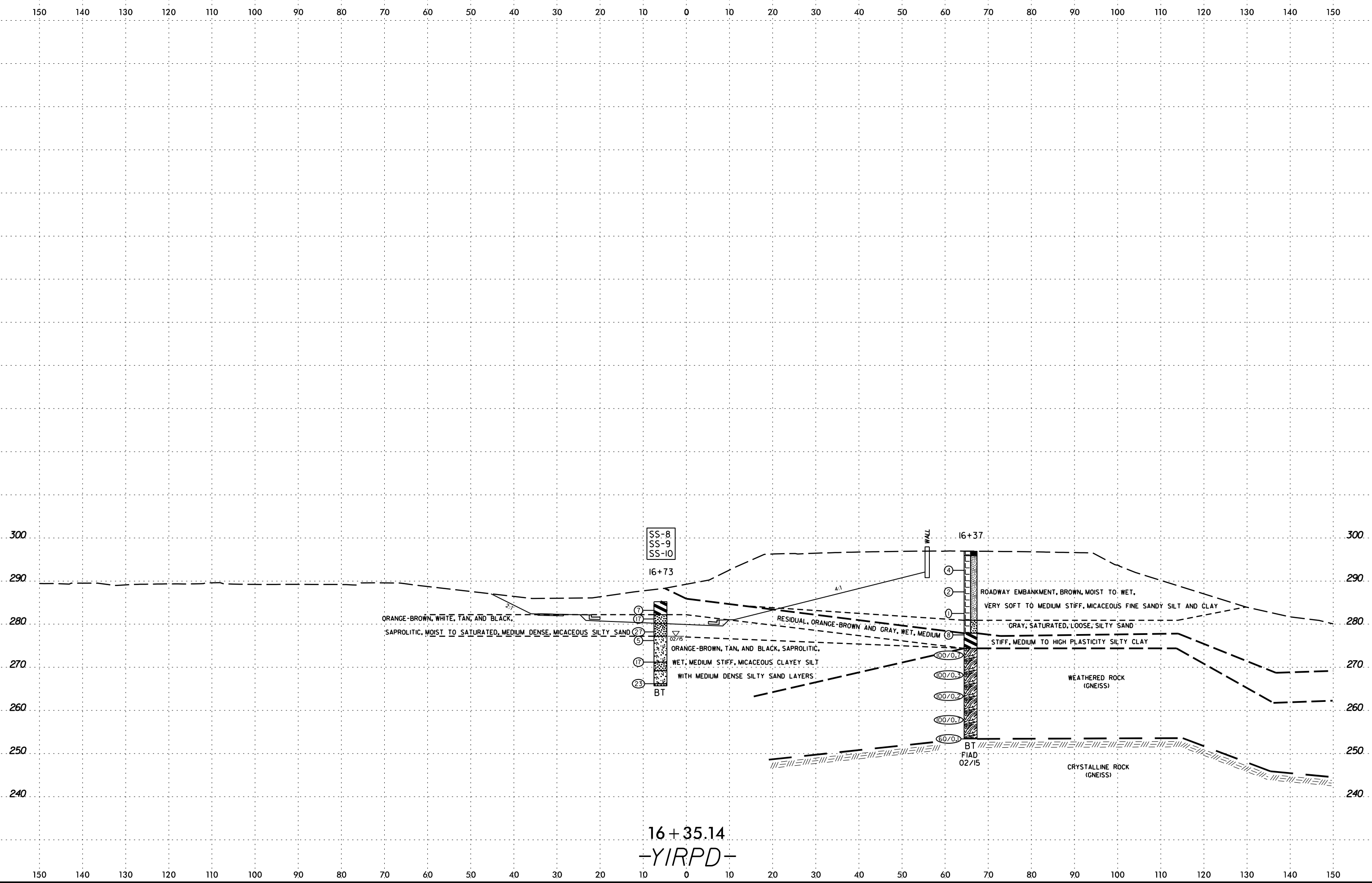


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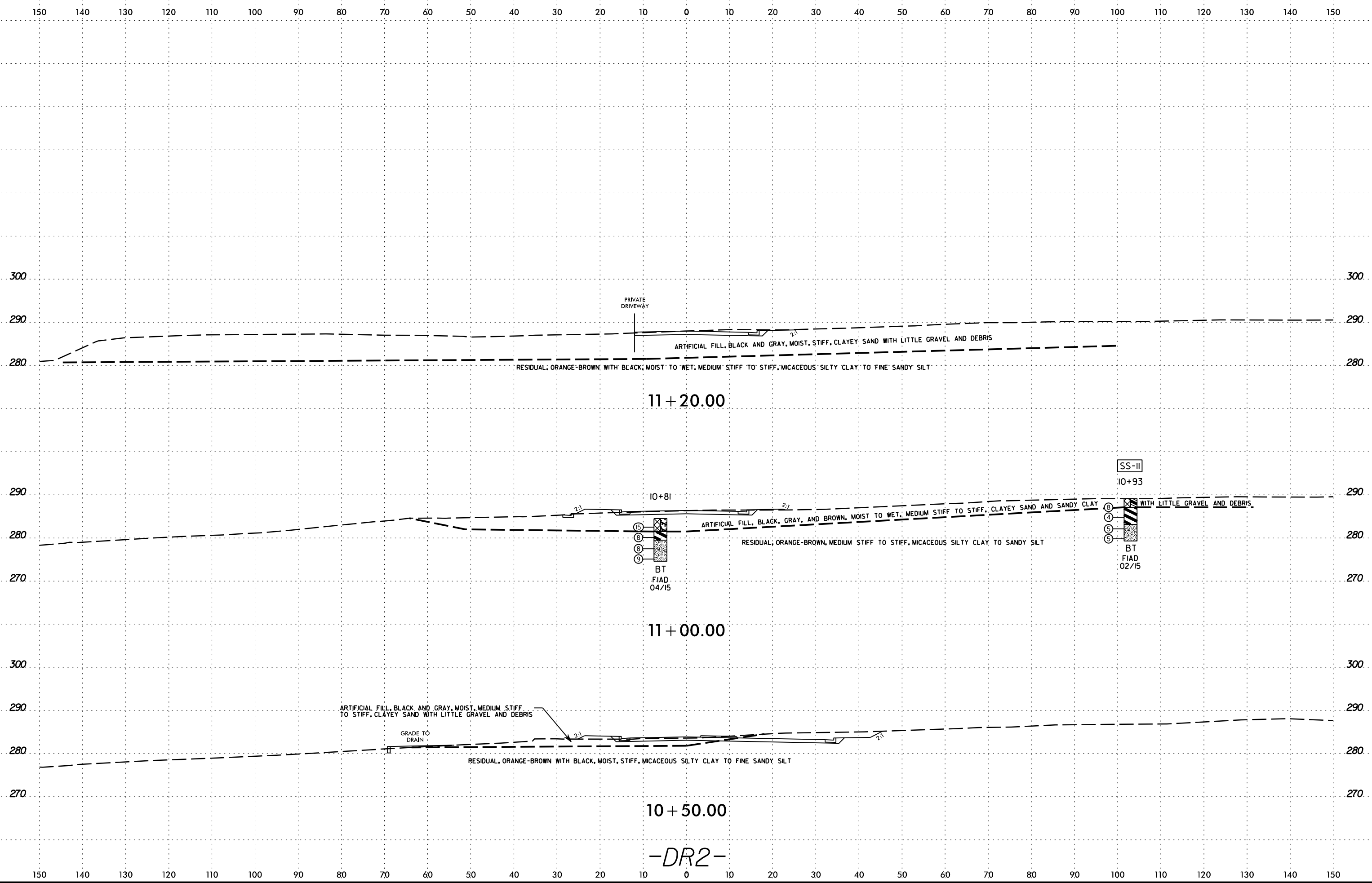


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16+35.14  
-YIRPD-

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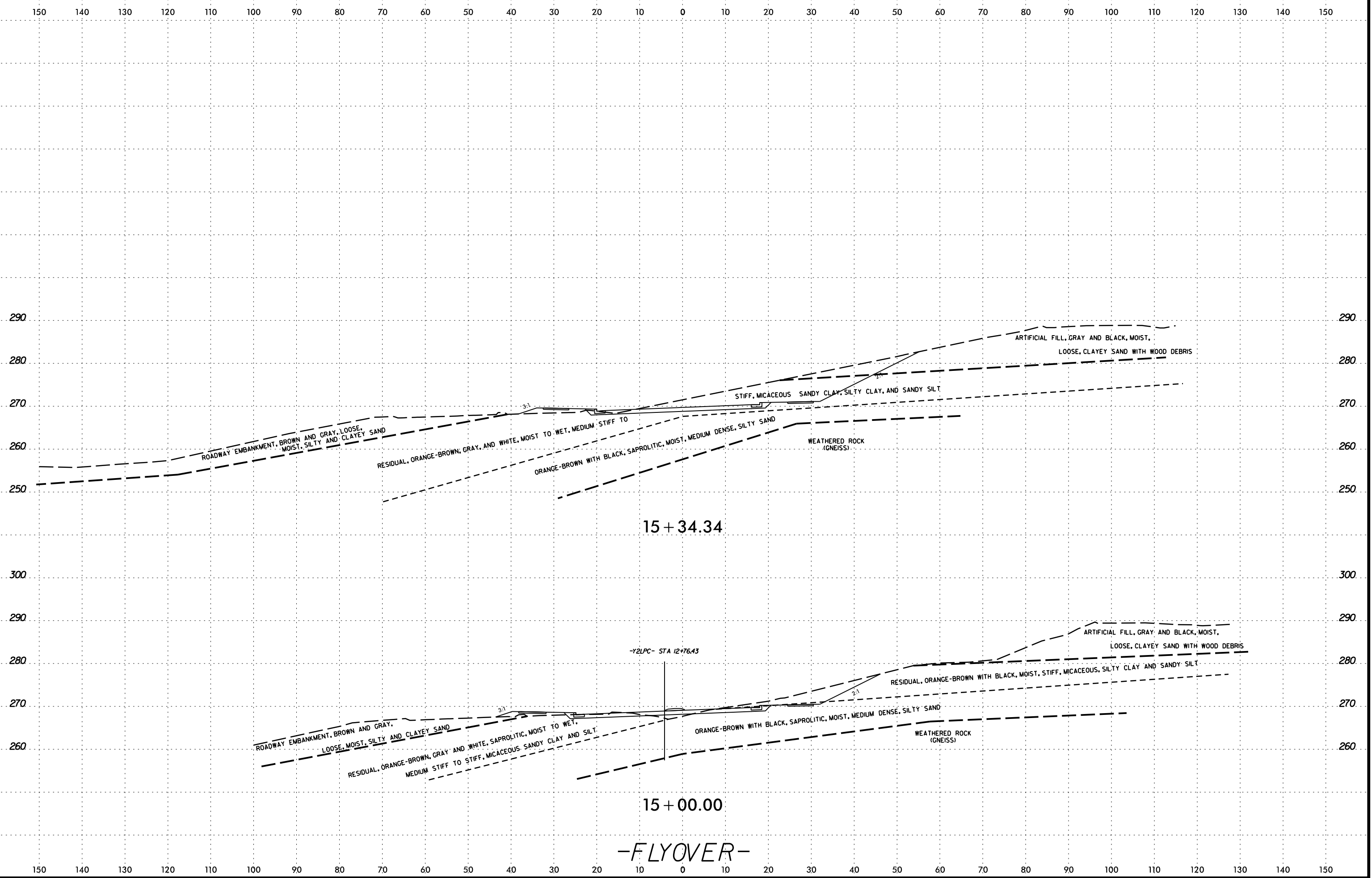


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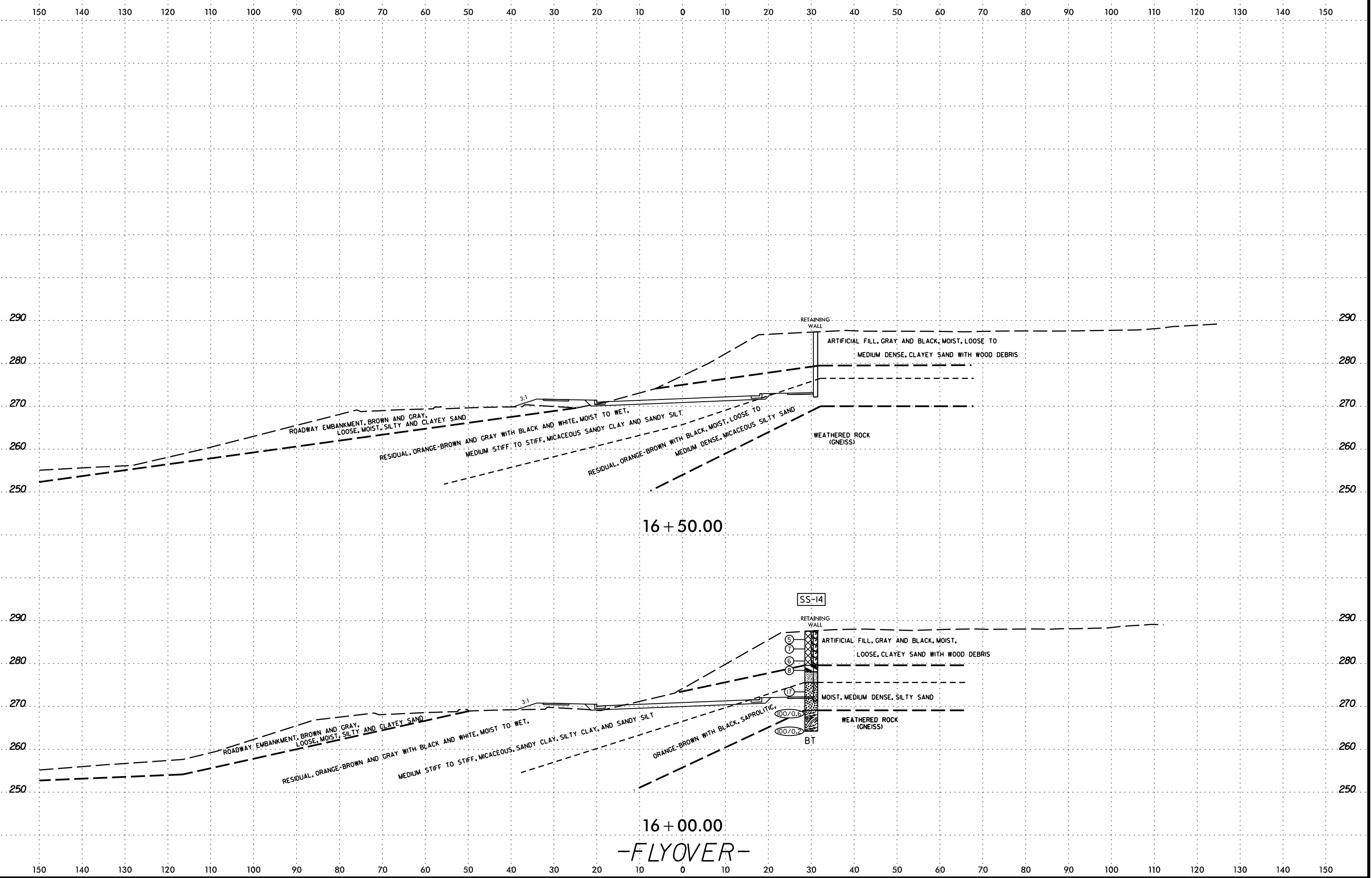
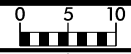


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m.leander



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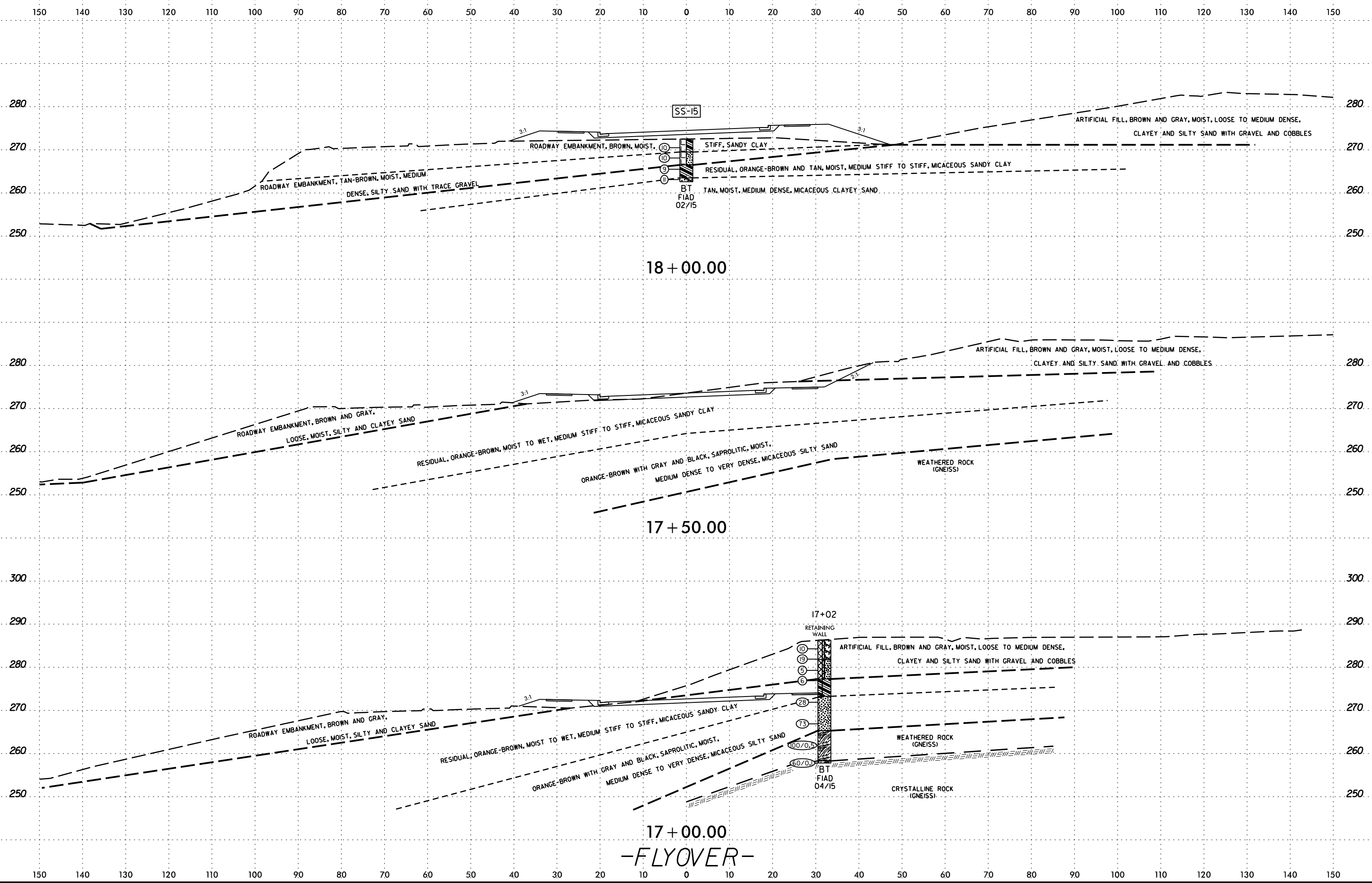
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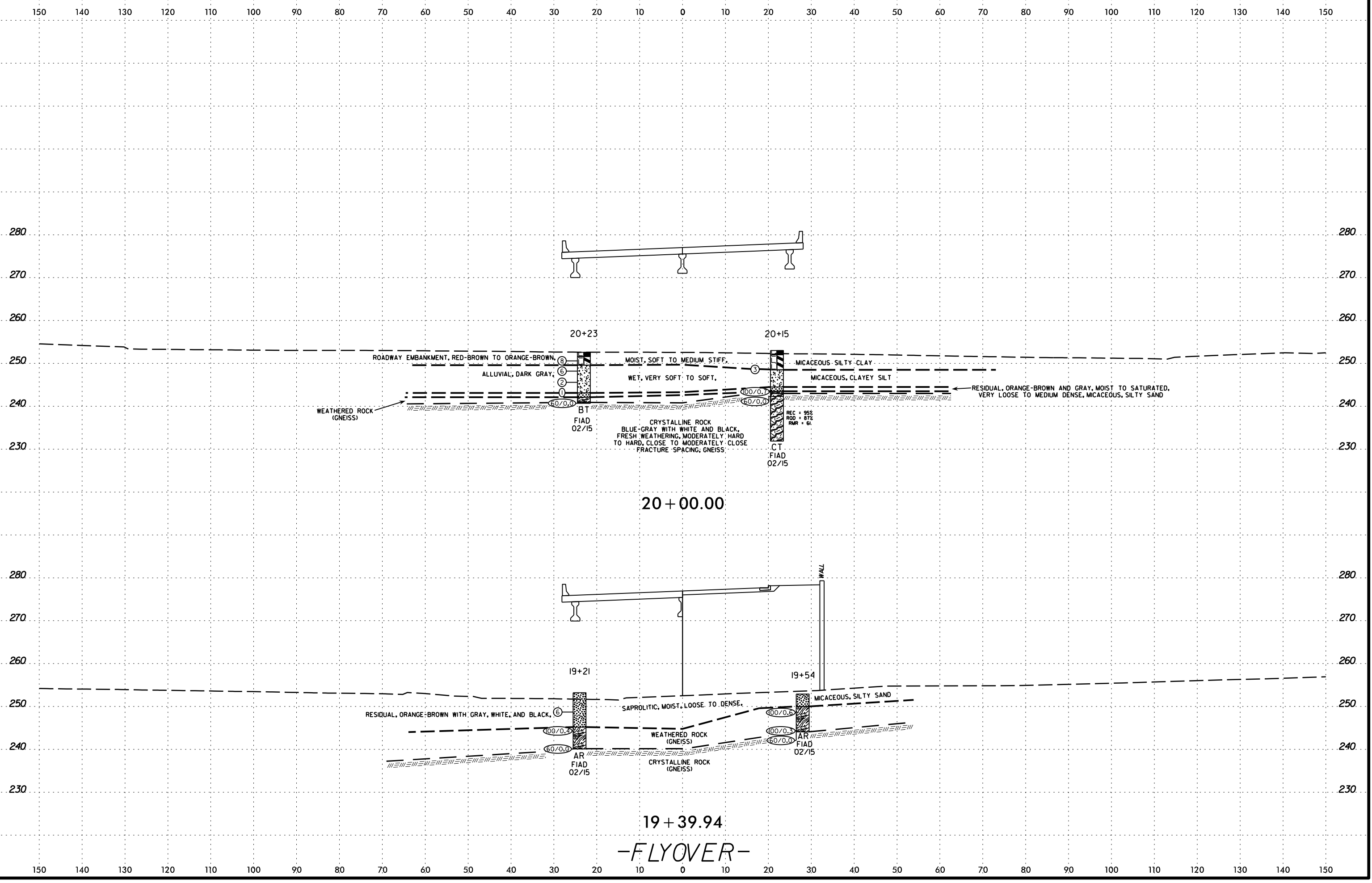
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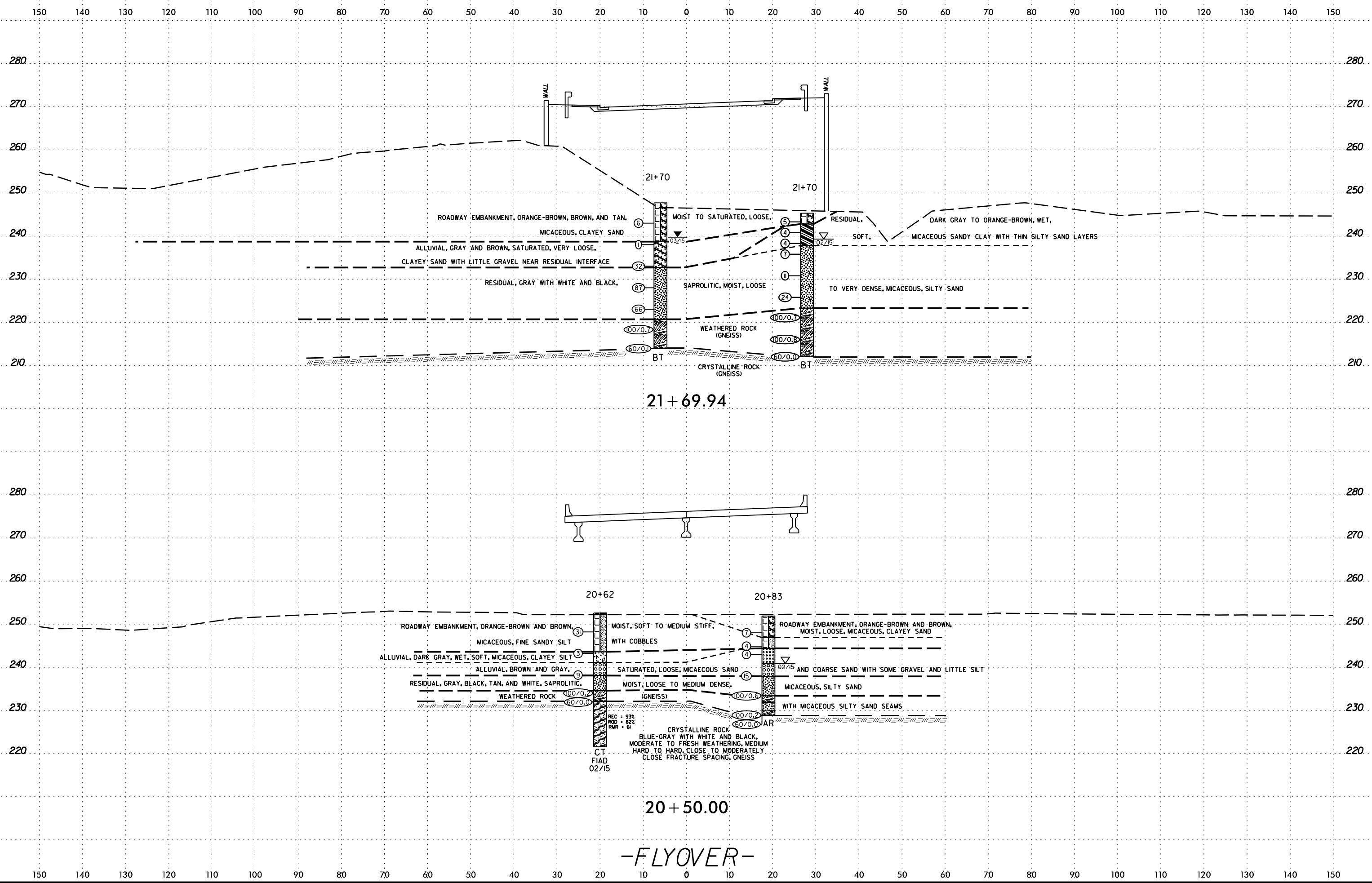


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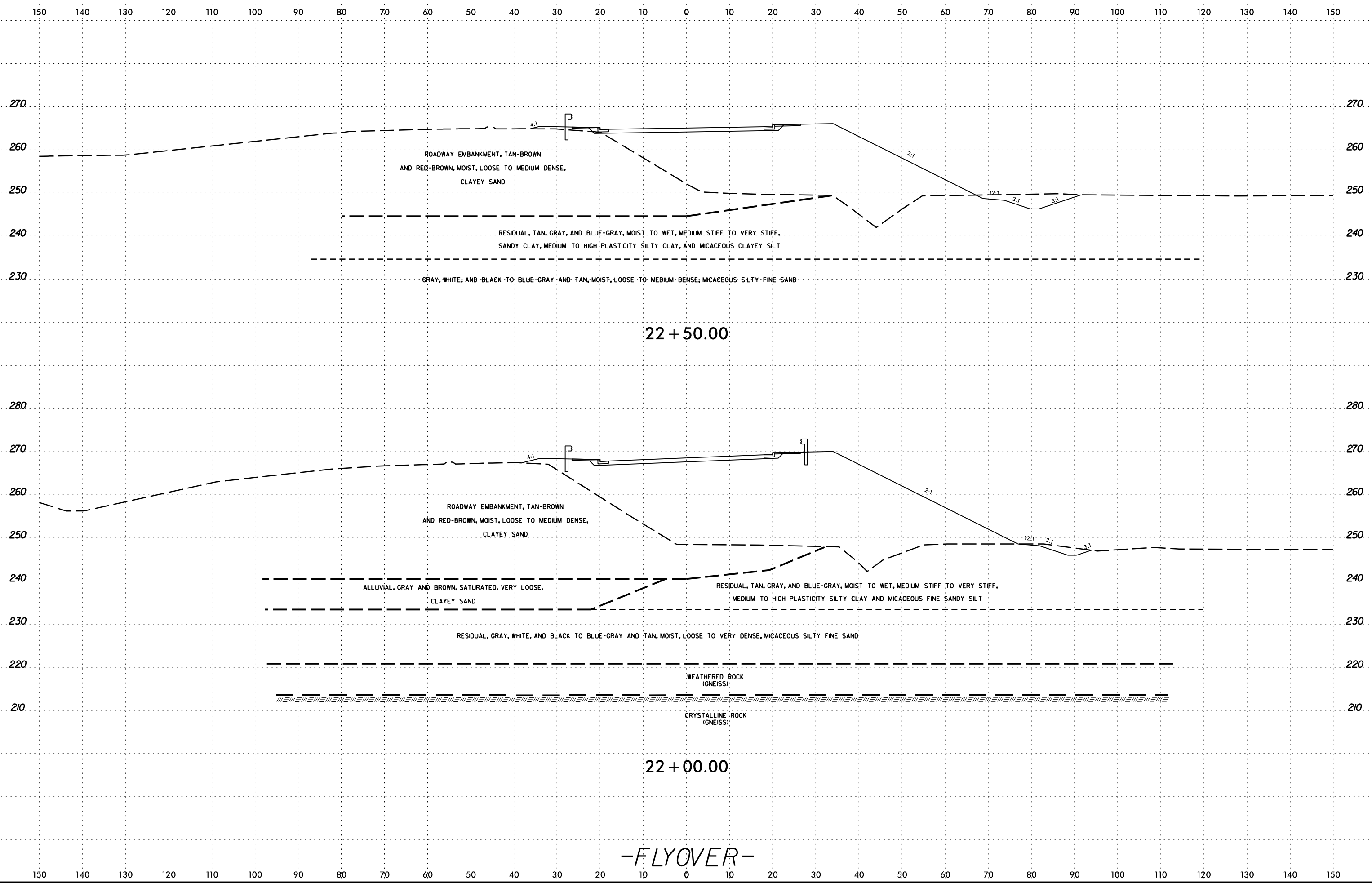
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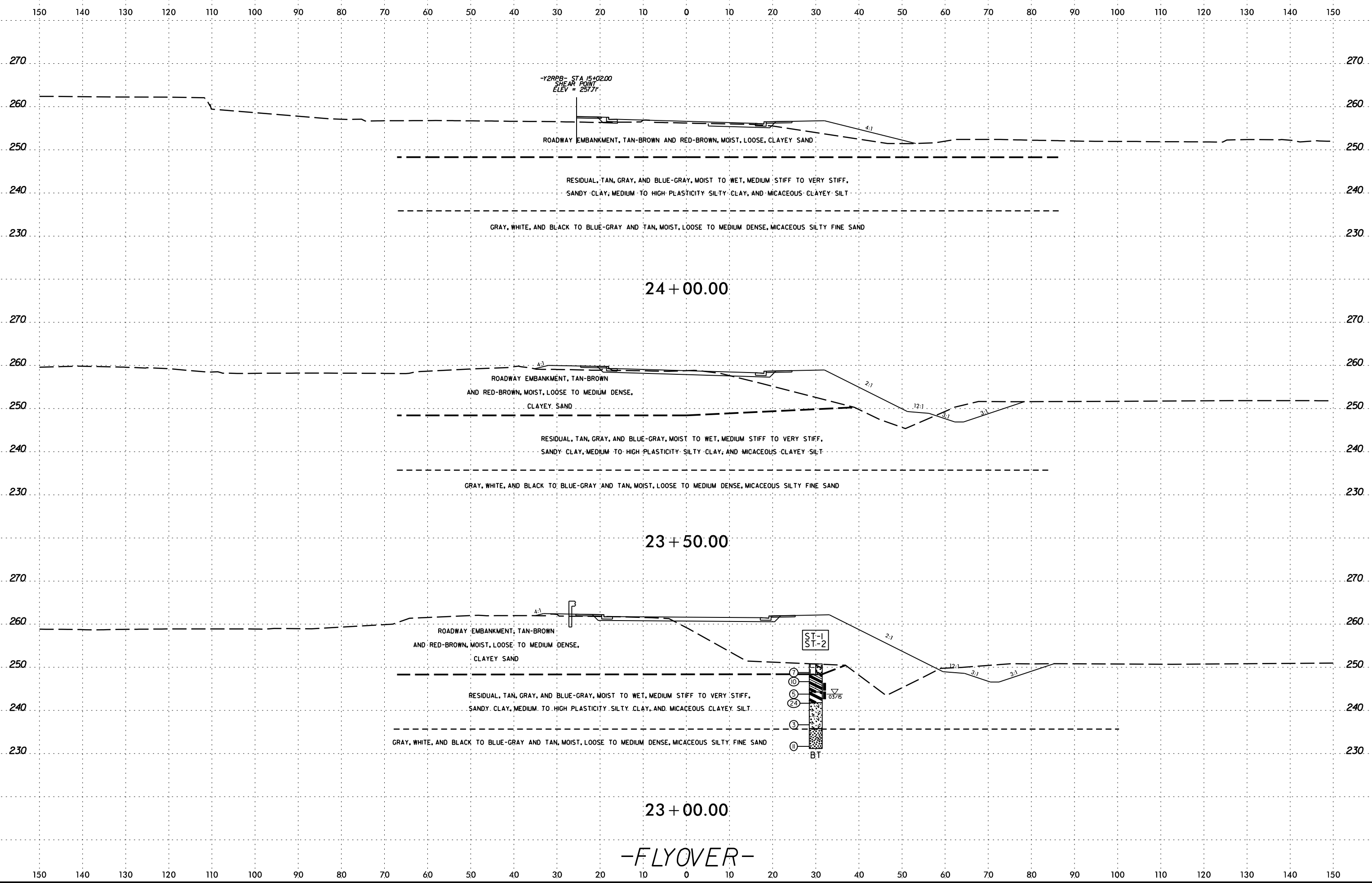


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m.leander

**PROJECT: 42263 /46031**

**REFERENCE: B-5121 /B-5317**

*NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT  
SUBSURFACE INVESTIGATION  
APPENDIX A  
LABORATORY RESULTS*

INITIALS

DATE



**SOIL LABORATORY TESTING SUMMARY**

PROJECT NUMBER: 42263.1.1 / 46031.1.1

ID (TIP): B-5121 / B-5317

COUNTY: Wake

DESCRIPTION: Replace Bridge 227 over Peace Street on US 70 / Replace Bridge over US 401 on US 70 / NC 50

Boring No.	Sample No.	Alignment	Station	Offset (feet)	Depth Interval (feet)	AASHTO Class.	L.L.	P.I.	% by Weight				% Retained #4 Sieve	% Passing (sieves)			% Moisture	% Organic	
									Coarse Sand	Fine Sand	Silt	Clay		#10	#40	#200			
L3700	SS-1	-L-	37+00	30' LT	1.1 - 2.6	A-7-6 (10)	49	28	23.7	27.2	5.4	43.7	0	98	90	50	21.5	-	
L4100	SS-2	-L-	41+00	34' LT	1.0 - 2.5	A-6 (6)	40	20	27.3	21.6	19.7	31.4	3	92	77	48	18.2	-	
Y1RPC1450	SS-3	-Y1RPC-	14+50	26' RT	1.0 - 2.5	A-2-4 (0)	31	5	42.8	34.0	11.5	11.7	2	94	70	26	11.7	-	
Y1RPC1689	SS-4	-Y1RPC-	16+89	6' RT	1.0 - 2.5	A-7-6 (11)	53	25	27.6	20.0	9.7	42.7	1	97	84	54	25.7	-	
Y1RPC1689	SS-5	-Y1RPC-	16+89	6' RT	3.3 - 4.8	A-7-5 (6)	50	14	22.2	32.5	10.9	34.4	0	100	90	52	22.6	-	
Y1RPD1500	SS-6	-Y1RPD-	15+00	10' LT	3.5 - 5.0	A-7-5 (7)	51	19	27.2	18.3	17.1	37.4	5	92	75	50	26.7	-	
Y1RPD1500	SS-7	-Y1RPD-	15+00	10' LT	6.0 - 7.0	A-7-5 (9)	49	19	25.3	21.3	13.2	40.2	1	98	82	55	26.3	-	
Y1RPD1673	SS-8	-Y1RPD-	16+73	6' LT	1.0 - 2.5	A-7-6 (12)	50	26	21.9	20.6	9.2	48.3	1	93	81	57	25.6	-	
Y1RPD1673	SS-9	-Y1RPD-	16+73	6' LT	6.0 - 7.5	A-2-4 (0)	27	NP	43.4	40.4	9.3	6.9	0	100	77	21	11.0	-	
Y1RPD1673	SS-10	-Y1RPD-	16+73	6' LT	8.0 - 9.5	A-5 (1)	45	5	21.9	41.3	25.6	11.2	0	100	91	46	46.5	-	
DR2-1093	SS-11	-DR2-	10+93	CL	3.3 - 4.8	A-7-6 (6)	47	19	27.1	27.6	7.0	38.3	0	100	87	49	33.1	-	
Y5-1114	SS-12	-Y5-	11+14	CL	1.0 - 2.5	A-6 (3)	37	18	29.4	26.7	11.7	32.2	8	88	73	42	18.9	-	
Y6-1920	SS-13	-Y6-	19+20	CL	1.0 - 2.5	A-2-6 (0)	31	12	38.2	27.6	10.7	23.5	7	85	65	32	12.7	-	
FLY1600	SS-14	-FLYOVER-	16+00	30' RT	8.2 - 9.5	A-7-5 (18)	55	24	9.2	24.1	16.9	49.8	0	100	96	71	36.7	-	
FLY1800	SS-15	-FLYOVER-	18+00	CL	1.0 - 2.5	A-6 (3)	34	14	23.7	28.4	18.2	29.7	6	87	75	44	16.9	-	
Y2LPC1150	SS-16	-Y2LPC-	11+50	35' LT	1.0 - 2.5	A-2-4 (0)	30	8	43.4	26.5	13.3	16.8	28	61	44	21	11.6	-	
Y2LPC1150	SS-17	-Y2LPC-	11+50	35' LT	3.4 - 4.9	A-2-6 (0)	31	11	39.5	25.9	10.0	24.6	12	80	60	30	20.0	-	
Y2RPB1130	SS-18	-Y2RPB-	11+30	15' RT	1.0 - 2.5	A-4 (0)	35	6	23.6	37.1	12.8	26.5	1	95	84	43	20.0	-	
FLY2305	ST-1	-FLYOVER-	23+05	30' RT	4.5 - 6.3	A-6 (3)	36	15	20.9	37.9	14.2	27.0	2	92	92	45	19.2	-	
FLY2305	ST-2	-FLYOVER-	23+05	30' RT	6.5 - 8.3	A-7-6 (17)	50	29	12.7	24.7	12.0	50.6	1	97	96	65	39.2	-	

CLASSIFICATIONS FOR SHELBY TUBE SAMPLES ST-1 AND ST-2 WERE PERFORMED BY GEOTECHNICS IN ACCORDANCE WITH NCDOT SPECIFICATIONS.

*Stephanie H. Huffman*  
 Certified Lab Technician Signature

114-01-1203  
 Certification Number

REFERENCE: B-5121

PROJECT: 42263

**STATE OF NORTH CAROLINA**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
**GEOTECHNICAL ENGINEERING UNIT**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-5121	1	9

**CONTENTS**

<u>SHEET NO.</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND
3-5	PLAN SHEETS
6-8	BORELOG REPORTS
9	LABORATORY TESTING SUMMARY

**SUBSURFACE INVENTORY**

COUNTY WAKE

PROJECT DESCRIPTION REPLACE BRIDGE NO. 227 ON CAPITAL BOULEVARD OVER PEACE STREET AND BRIDGE NO. 213 ON WADE AVENUE OVER CAPITAL BOULEVARD - TEMPORARY SHORING

**CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
  2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

TURNAGE, J. R.

CORY, K. W.

ALEXANDER, M. J.

INVESTIGATED BY TERRACON CONSULTANTS

DRAWN BY FIELDS, W. D.

CHECKED BY ALEXANDER, M. J.

SUBMITTED BY TERRACON CONSULTANTS

DATE DECEMBER 2015



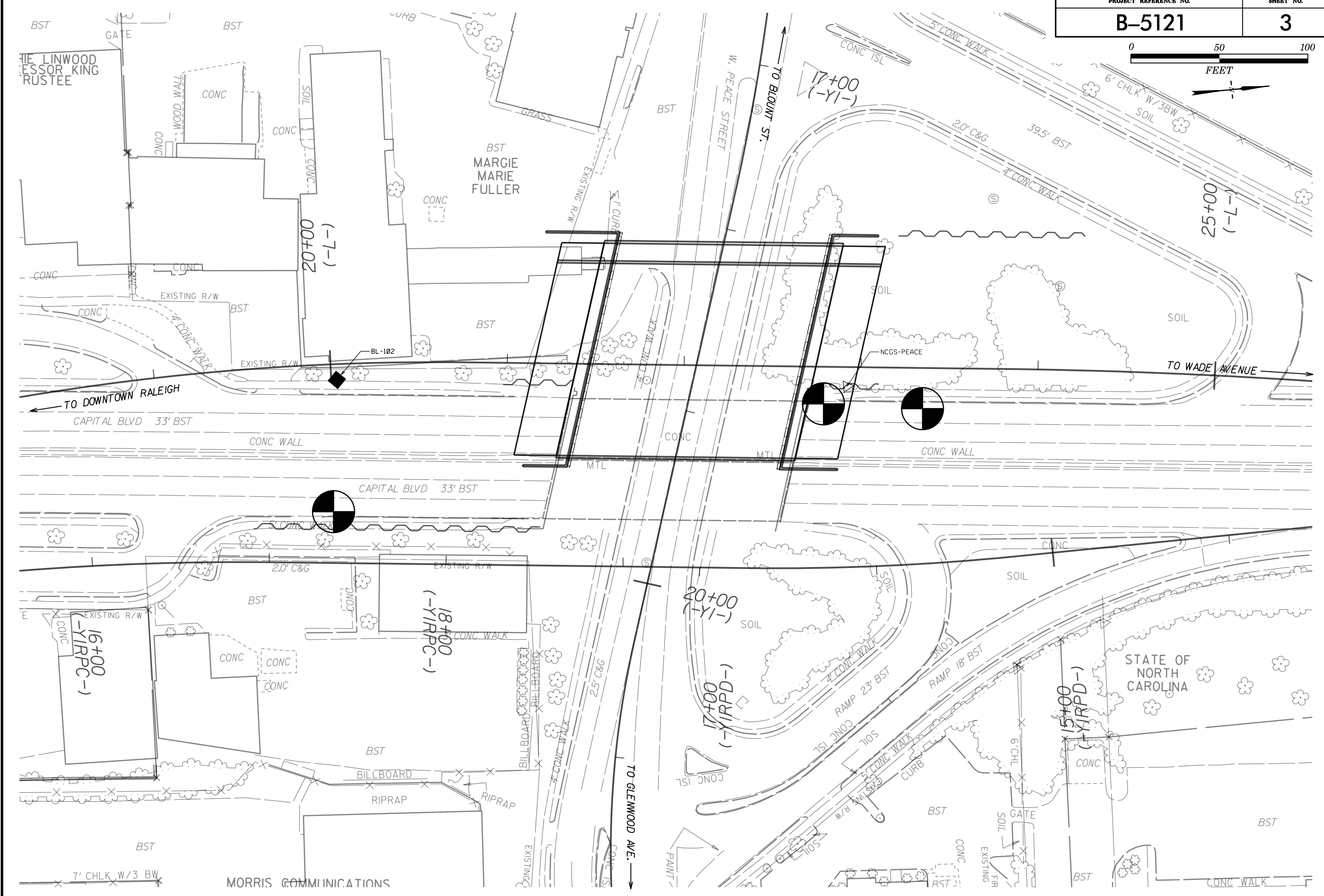
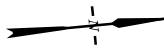
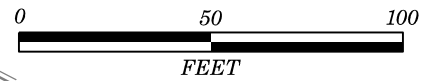
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Matthew J. Alexander 12/22/2015

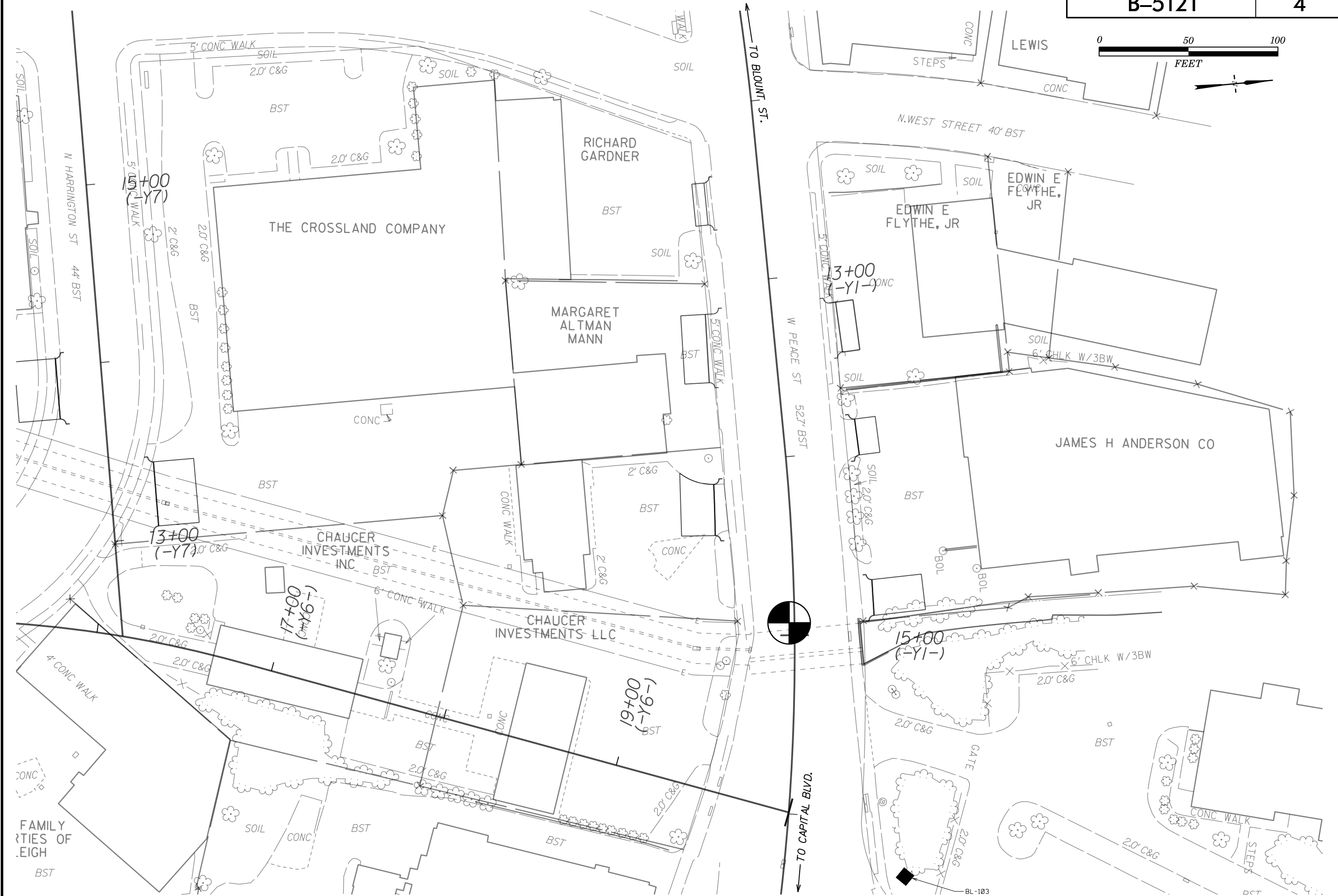
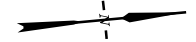
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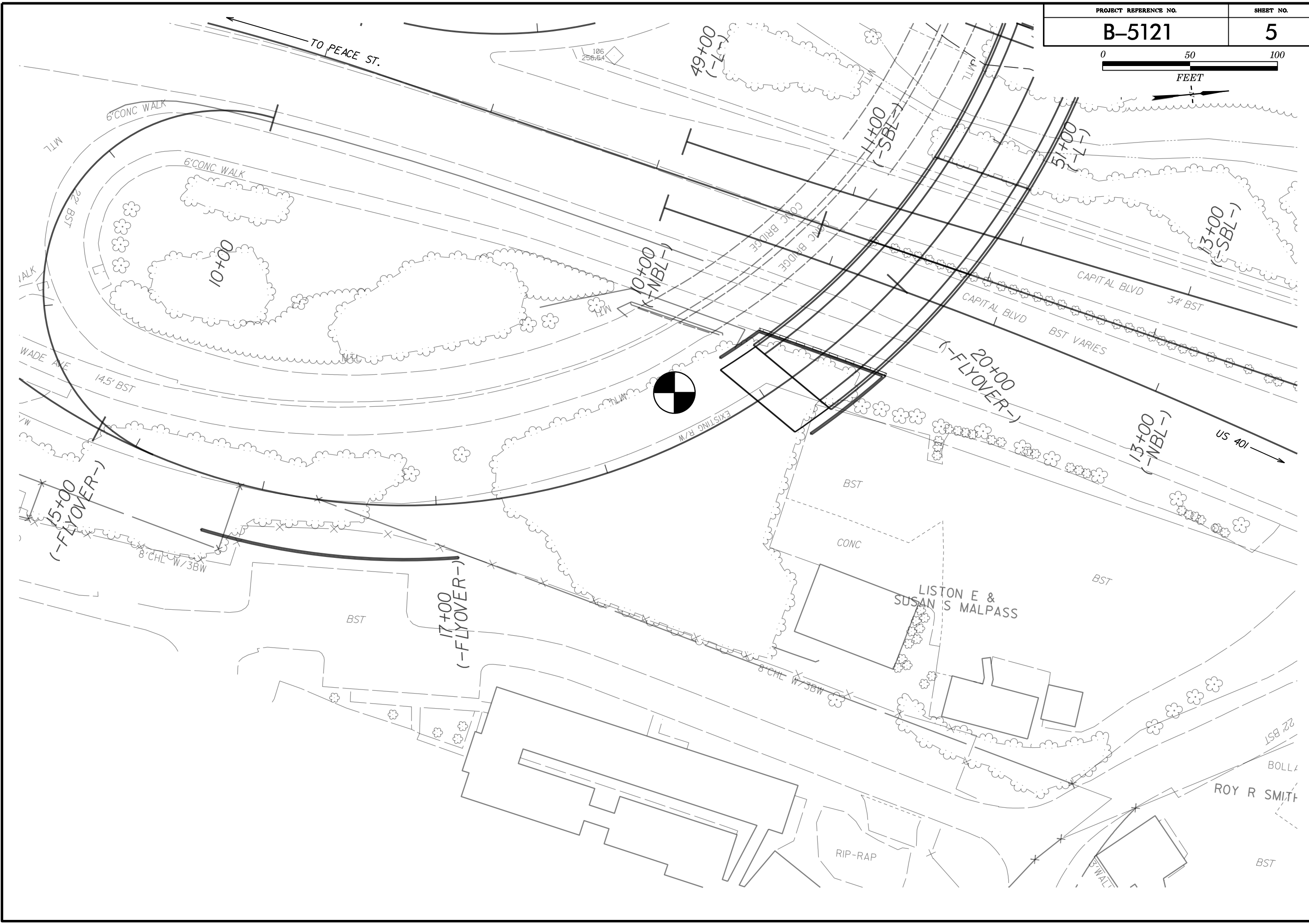
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
**GEOTECHNICAL ENGINEERING UNIT**  
**SUBSURFACE INVESTIGATION**  
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS																																																												
<p>SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, <i>VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i></p>										<p><b>WELL GRADED</b> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <b>UNIFORMLY GRADED</b> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <b>GAP-GRADED</b> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.</p>										<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>										<p><b>ALLUVIUM (ALLUV.)</b> - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. <b>AQUIFER</b> - A WATER BEARING FORMATION OR STRATA. <b>ARENACEOUS</b> - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <b>ARGILLACEOUS</b> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. <b>ARTESIAN</b> - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. <b>CALCAREOUS (CALC.)</b> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. <b>COLLUVIUM</b> - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. <b>CORE RECOVERY (REC.)</b> - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>DIKE</b> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. <b>DIP</b> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. <b>DIP DIRECTION (DIP AZIMUTH)</b> - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. <b>FAULT</b> - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. <b>FISSILE</b> - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. <b>FLOAT</b> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOADED FROM PARENT MATERIAL. <b>FLOOD PLAIN (FP)</b> - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. <b>FORMATION (FM)</b> - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. <b>JOINT</b> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. <b>LEDGE</b> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. <b>LENS</b> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. <b>MOTTLED (MOT.)</b> - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. <b>PERCHED WATER</b> - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. <b>RESIDUAL (RES.) SOIL</b> - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. <b>ROCK QUALITY DESIGNATION (RQD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. <b>SAPROLITE (SAP.)</b> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. <b>SILL</b> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. <b>SLICKENSIDE</b> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. <b>STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT)</b> - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. <b>STRATA CORE RECOVERY (SREC.)</b> - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. <b>STRATA ROCK QUALITY DESIGNATION (SROD)</b> - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. <b>TOPSOIL (TS.)</b> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>																																																												
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b>										<b>ANGULARITY OF GRAINS</b>										<b>WEATHERED ROCK (WR)</b>										<b>NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES &gt; 100 BLOWS PER FOOT IF TESTED.</b>																																																												
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <th colspan="5">GRANULAR MATERIALS (&lt;= 35% PASSING #200)</th> <th colspan="5">SILT-CLAY MATERIALS (&gt; 35% PASSING #200)</th> <th colspan="5">ORGANIC MATERIALS</th> </tr> <tr> <th>GROUP CLASS.</th> <th>A-1</th> <th>A-3</th> <th>A-2</th> <th>A-2</th> <th>A-4</th> <th>A-5</th> <th>A-6</th> <th>A-7</th> <th>A-1, A-2</th> <th>A-3</th> <th>A-4, A-5</th> <th>A-6, A-7</th> <th colspan="5"></th> </tr> <tr> <th>SYMBOL</th> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td colspan="5"></td> </tr> </table>										GRANULAR MATERIALS (<= 35% PASSING #200)					SILT-CLAY MATERIALS (> 35% PASSING #200)					ORGANIC MATERIALS					GROUP CLASS.	A-1	A-3	A-2	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7						SYMBOL																		<b>MINERALOGICAL COMPOSITION</b>										<b>CRYSTALLINE ROCK (CR)</b>										<b>FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</b>									
GRANULAR MATERIALS (<= 35% PASSING #200)					SILT-CLAY MATERIALS (> 35% PASSING #200)					ORGANIC MATERIALS																																																																																
GROUP CLASS.	A-1	A-3	A-2	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-3	A-4, A-5	A-6, A-7																																																																														
SYMBOL																																																																																										
<b>COMPRESSION</b>										<b>NON-CRYSTALLINE ROCK (NCR)</b>										<b>FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</b>																																																																						
<b>PERCENTAGE OF MATERIAL</b>										<b>COASTAL PLAIN SEDIMENTARY ROCK (CP)</b>										<b>COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</b>																																																																						
<b>GROUND WATER</b>										<b>WEATHERING</b>										<b>FRESH</b>										<b>ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.</b>																																																												
<b>MISCELLANEOUS SYMBOLS</b>										<b>VERY SLIGHT (IV SL.)</b>										<b>ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.</b>																																																																						
<b>RECOMMENDATION SYMBOLS</b>										<b>SLIGHT (SL.)</b>										<b>ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.</b>																																																																						
<b>ABBREVIATIONS</b>										<b>MODERATE (MOD.)</b>										<b>SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.</b>																																																																						
<b>EQUIPMENT USED ON SUBJECT PROJECT</b>										<b>MODERATELY SEVERE (MOD. SEV.)</b>										<b>ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL</b>																																																																						
<b>TEXTURE OR GRAIN SIZE</b>										<b>SEVERE (SEV.)</b>										<b>ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES &gt; 100 BPF</b>																																																																						
<b>SOIL MOISTURE - CORRELATION OF TERMS</b>										<b>VERY SEVERE (IV SEV.)</b>										<b>ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</b>																																																																						
<b>PLASTICITY</b>										<b>COMPLETE</b>										<b>ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.</b>																																																																						
<b>COLOR</b>										<b>ROCK HARDNESS</b>										<b>VERY HARD</b>										<b>CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.</b>																																																												
<b>FRACATURE SPACING</b>										<b>HARD</b>										<b>CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.</b>																																																																						
<b>BEDDING</b>										<b>MODERATELY HARD</b>										<b>CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.</b>																																																																						
<b>INDURATION</b>										<b>MEDIUM HARD</b>										<b>CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.</b>																																																																						
<b>SOIL MOISTURE SCALE (ATTERBERG LIMITS)</b>										<b>SOFT</b>										<b>CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.</b>																																																																						
<b>FIELD MOISTURE DESCRIPTION</b>										<b>VERY SOFT</b>										<b>CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.</b>																																																																						
<b>GUIDE FOR FIELD MOISTURE DESCRIPTION</b>										<b>FRACATURE SPACING</b>										<b>BEDDING</b>																																																																						
<b>LIQUID LIMIT (LL)</b>										<b>VERY WIDE</b>										<b>VERY THICKLY BEDDED</b>																																																																						
<b>WET - (W)</b>										<b>WIDE</b>										<b>THICKLY BEDDED</b>																																																																						
<b>SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE</b>										<b>MODERATELY CLOSE</b>										<b>THINLY BEDDED</b>																																																																						
<b>MOIST - (M)</b>										<b>CLOSE</b>										<b>VERY THINLY BEDDED</b>																																																																						
<b>SOLID; AT OR NEAR OPTIMUM MOISTURE</b>										<b>VERY CLOSE</b>										<b>THICKLY LAMINATED</b>																																																																						
<b>DRY - (D)</b>										<b>INDURATED</b>										<b>THINLY LAMINATED</b>																																																																						
<b>REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE</b>										<b>EXTREMELY INDURATED</b>										<b>&lt; 0.008 FEET</b>																																																																						





BL-103



**GEOTECHNICAL BORING REPORT**  
**BORE LOG**

WBS 42263		TIP B-5121		COUNTY WAKE		GEOLOGIST ALEXANDER, M. J.											
SITE DESCRIPTION REPLACE BRIDGES NO. 227 AND 213 - TEMPORARY SHORING							GROUND WTR (ft)										
BORING NO.	STATION	OFFSET		ALIGNMENT		0 HR.											
L_2279RT	22+79	21 ft RT		-L-		28.0											
COLLAR ELEV.	TOTAL DEPTH	NORTHING		EASTING		24 HR.											
296.4 ft	40.6 ft	742,136		2,105,893		FIAD											
DRILL RIG/HAMMER EFF./DATE				DRILL METHOD		HAMMER TYPE											
TER6847 CME-75 93% 05/20/2015				H.S. Augers		Automatic											
DRILLER		START DATE		COMP. DATE		SURFACE WATER DEPTH											
TURNAGE, J. R.		11/15/15		11/15/15		N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
300																	
295																	
	293.3	3.1															
					WOH	1	2										
290																	
	288.3	8.1			2	2	2										
285																	
	283.3	13.1			1	1	2										
280																	
	278.3	18.1			3	7	9										
275																	
	273.3	23.1			6	6	8										
270																	
	268.3	28.1			13	13	15										
265																	
	263.3	33.1			10	17	42										
260																	
	258.3	38.1			55	45/0.2											
	255.9	40.5			60/0.1												

WBS 42263		TIP B-5121		COUNTY WAKE		GEOLOGIST ALEXANDER, M. J.											
SITE DESCRIPTION REPLACE BRIDGES NO. 227 AND 213 - TEMPORARY SHORING							GROUND WTR (ft)										
BORING NO.	STATION	OFFSET		ALIGNMENT		0 HR.											
L_2335RT	23+35	23 ft RT		-L-		28.2											
COLLAR ELEV.	TOTAL DEPTH	NORTHING		EASTING		24 HR.											
295.5 ft	33.6 ft	742,191		2,105,901		FIAD											
DRILL RIG/HAMMER EFF./DATE				DRILL METHOD		HAMMER TYPE											
TER6847 CME-75 93% 05/20/2015				H.S. Augers		Automatic											
DRILLER		START DATE		COMP. DATE		SURFACE WATER DEPTH											
TURNAGE, J. R.		11/16/15		11/16/15		N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100							
300																	
295																	
	292.3	3.2			1	1	3										
290																	
	287.3	8.2			2	2	2										
285																	
	282.3	13.2			2	5	6										
280																	
	277.3	18.2			4	14	9										
275																	
	272.3	23.2			20	29	25										
270																	
	267.3	28.2			28	50	50/0.3										
265																	
	262.3	33.2			100/0.4												

NCDOT BORE DOUBLE B5121\_GEO\_SHORING.GPJ NC\_DOT.GDT 12/1/15

# GEOTECHNICAL BORING REPORT

## BORE LOG

WBS 42263		TIP B-5121		COUNTY WAKE		GEOLOGIST ALEXANDER, M. J.										
SITE DESCRIPTION REPLACE BRIDGES NO. 227 AND 213 - TEMPORARY SHORING							GROUND WTR (ft)									
BORING NO. Y1RPC_1736		STATION 17+36		OFFSET 29 ft LT		ALIGNMENT -Y1RPC-										
COLLAR ELEV. 296.5 ft		TOTAL DEPTH 34.6 ft		NORTHING 741,854		EASTING 2,105,928										
DRILL RIG/HAMMER EFF./DATE TER6847 CME-75 93% 05/20/2015				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic										
DRILLER TURNAGE, J. R.		START DATE 11/16/15		COMP. DATE 11/16/15		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
300																
295	293.4	3.1	3	3	4								M	PAVEMENT 0.2' ASPHALT, 0.7' CONCRETE AND 0.5' AGGREGATE BASE COURSE	1.4	
290	288.4	8.1	15	9	6								M	RESIDUAL ORANGE-BROWN AND ORANGE, SILTY CLAY, MICACEOUS		
285	283.4	13.1	6	9	10								M			
280	278.4	18.1	10	19	28								M	BROWN, TAN AND GRAY, SILTY SAND, MICACEOUS	11.0	
275	273.4	23.1	14	22	32								M			
270	268.4	28.1	30	52	48/0.4								M	WEATHERED ROCK (GNEISS)	28.5	
265	263.4	33.1	25	28	38								M	RESIDUAL BROWN, WHITE AND BLACK, SAPROLITIC, SILTY SAND, MICACEOUS	34.6	
															Boring Terminated at Elevation 261.9 ft IN RESIDUAL (SILTY SAND)	

WBS 42263		TIP B-5121		COUNTY WAKE		GEOLOGIST ALEXANDER, M. J.										
SITE DESCRIPTION REPLACE BRIDGES NO. 227 AND 213 - TEMPORARY SHORING							GROUND WTR (ft)									
BORING NO. Y1_1493RT		STATION 14+93		OFFSET 3 ft RT		ALIGNMENT -Y1-										
COLLAR ELEV. 271.3 ft		TOTAL DEPTH 24.9 ft		NORTHING 742,149		EASTING 2,105,495										
DRILL RIG/HAMMER EFF./DATE TER6847 CME-75 93% 05/20/2015				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic										
DRILLER TURNAGE, J. R.		START DATE 11/16/15		COMP. DATE 11/16/15		SURFACE WATER DEPTH N/A										
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100						
275																
270	267.9	3.4	1	1	1								M	PAVEMENT 1.4' ASPHALT, 0.6' CONCRETE	2.0	
265	262.9	8.4	WOH	1	1								M	ROADWAY EMBANKMENT ORANGE-BROWN, SILTY CLAY, MICACEOUS		
260	257.9	13.4	2	2	3								Sat	ALLUVIAL DARK GRAY AND BROWN, SANDY GRAVEL WITH TRACE CLAY	8.0	
255	252.9	18.4	10	14	26								SS-4	RESIDUAL ORANGE, BLACK AND WHITE, SAPROLITIC, FINE SILTY SAND, MICACEOUS	12.0	
250	247.9	23.4	9	14	27								SS-5	WITH TRACE GRAVEL BELOW 20 FT		
													Sat		Boring Terminated at Elevation 246.4 ft IN RESIDUAL (SILTY SAND)	24.9

NCDOT BORE DOUBLE B5121\_GEO\_SHORING.GPJ NC\_DOT.GDT 12/1/15



WBS 42263		TIP B-5121		COUNTY WAKE		GEOLOGIST ALEXANDER, M. J.									
SITE DESCRIPTION REPLACE BRIDGES NO. 227 AND 213 - TEMPORARY SHORING							GROUND WTR (ft)								
BORING NO. FLY_1855		STATION 18+55		OFFSET 28 ft LT		ALIGNMENT -FLYOVER-	0 HR. 21.5								
COLLAR ELEV. 270.2 ft		TOTAL DEPTH 34.1 ft		NORTHING 744,684		EASTING 2,106,589	24 HR. FIAD								
DRILL RIG/HAMMER EFF./DATE TER6847 CME-75 93% 05/20/2015				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic									
DRILLER TURNAGE, J. R.		START DATE 11/17/15		COMP. DATE 11/17/15		SURFACE WATER DEPTH N/A									
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					
275															
270															270.2 GROUND SURFACE 0.0
265	266.8	3.4	1	1	2	3						W			
260	261.8	8.4	7	7	9	16						D		262.2 RESIDUAL 8.0	
255	256.8	13.4	9	12	13	25						D			
250	251.8	18.4	6	7	8	15						M		253.2 BROWN, ORANGE-TAN AND GRAY, SILTY SAND, MICACEOUS WITH SEAMS OF WHITE SANDY GRAVEL 17.0	
245	246.8	23.4	5	9	21	30						Sat.			
240	241.8	28.4	100/0.2											243.2 WEATHERED ROCK (GNEISS) 27.0	
	236.8	33.4	56	44/0.2										236.1 Boring Terminated at Elevation 236.1 ft IN WEATHERED ROCK (GNEISS) 34.1	

### LABORATORY TESTING SUMMARY

PROJECT NUMBER: 42263

ID (TIP): B-5121

COUNTY: WAKE

DESCRIPTION: REPLACE BRIDGE NO. 227 ON CAPITAL BLVD. OVER PEACE ST. AND BRIDGE NO. 213 ON WADE AVE. OVER CAPITAL BLVD. - TEMPORARY SHORING

Sample No.	Alignment	Station	Offset (feet)	Depth Interval (feet)	AASHTO Class.	L.L.	P.I.	% by Weight				% Retained #4 Sieve	% Passing (sieves)			% Moisture	% Organic
								Coarse Sand	Fine Sand	Silt	Clay		#10	#40	#200		
SS-1	-L-	22+79	21 RT	3.1 - 4.6	A-7-6 (12)	53	30	28.7	20.0	14.0	37.3	1	98	79	53	25.9	--
SS-2	-L-	23+35	23 RT	13.2 - 14.7	A-7-6 (5)	47	25	36.7	26.8	11.9	24.6	1	97	75	39	20.7	--
SS-3	-Y1RPC-	17+36	29 LT	18.1 - 19.6	A-2-4 (0)	26	NP	38.8	43.0	12.4	5.8	0	100	84	24	12.6	--
SS-4	-Y1-	14+93	3 RT	13.4 - 14.9	A-2-4 (0)	34	NP	36.7	43.8	14.0	5.5	0	100	81	25	39.6	--
SS-5	-Y1-	14+93	3 RT	18.4 - 19.9	A-2-4 (0)	26	NP	39.5	45.3	12.4	2.8	0	100	84	20	16.2	--
SS-6	-FLYOVER-	18+55	28 LT	8.4 - 9.9	A-7-5 (22)	67	34	19.9	16.7	9.1	54.3	0	100	90	65	23.8	--
SS-7	-FLYOVER-	18+55	28 LT	18.4 - 19.9	A-2-4 (0)	27	NP	43.9	36.6	10.9	8.6	3	92	70	22	11.8	--
ST-1*	-L-	22+79	21 RT	10.1 - 12.9	A-4(0)	43	NP	24.8	31.2	20.9	23.1	0	100	87	49	27.5	--
* ST-1 classification testing performed by Geotechnics																	
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*Stephanie H. Huffman*  
 Certified Lab Technician Signature  
 114-01-1203  
 Certification Number