NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

| PROJ. REFERENCE NO. 34915.1.1 (U-3308) F.A. PROJ. STP-55 (20) |
|---|
| |
| COUNTY DURHAM |
| PROJECT DESCRIPTION DURHAM-NC 55 (ALSTON AVE.) FROM NC |
| 147 (BUCK DEAN FREEWAY) TO US 70 BUS/NC 98 |
| (HOLLOWAY ST.) |
| INVENTORY |

| N.C. 34915.1 | .1 (U-3308) | 1 | ı | 34 |
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| STATE PROJ.NO. | F. A. PROJ. NO. | DES | CRIP | TION |
| 34915.1.1 | STP-55 (20) | | P.E. | |
| | | RW | & | UTIL, |

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PERSONNEL **N.D. MOHS**

> TIERRA PERSONNEL

J. HOWARD

C. BRUINSMA

S. HAN

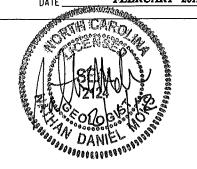
F. COX

INVESTIGATED BY N.D. MOHS

CHECKED BY N.T. ROBERSON

SUBMITTED BY N.T. ROBERSON

FEBRUARY 2012



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NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

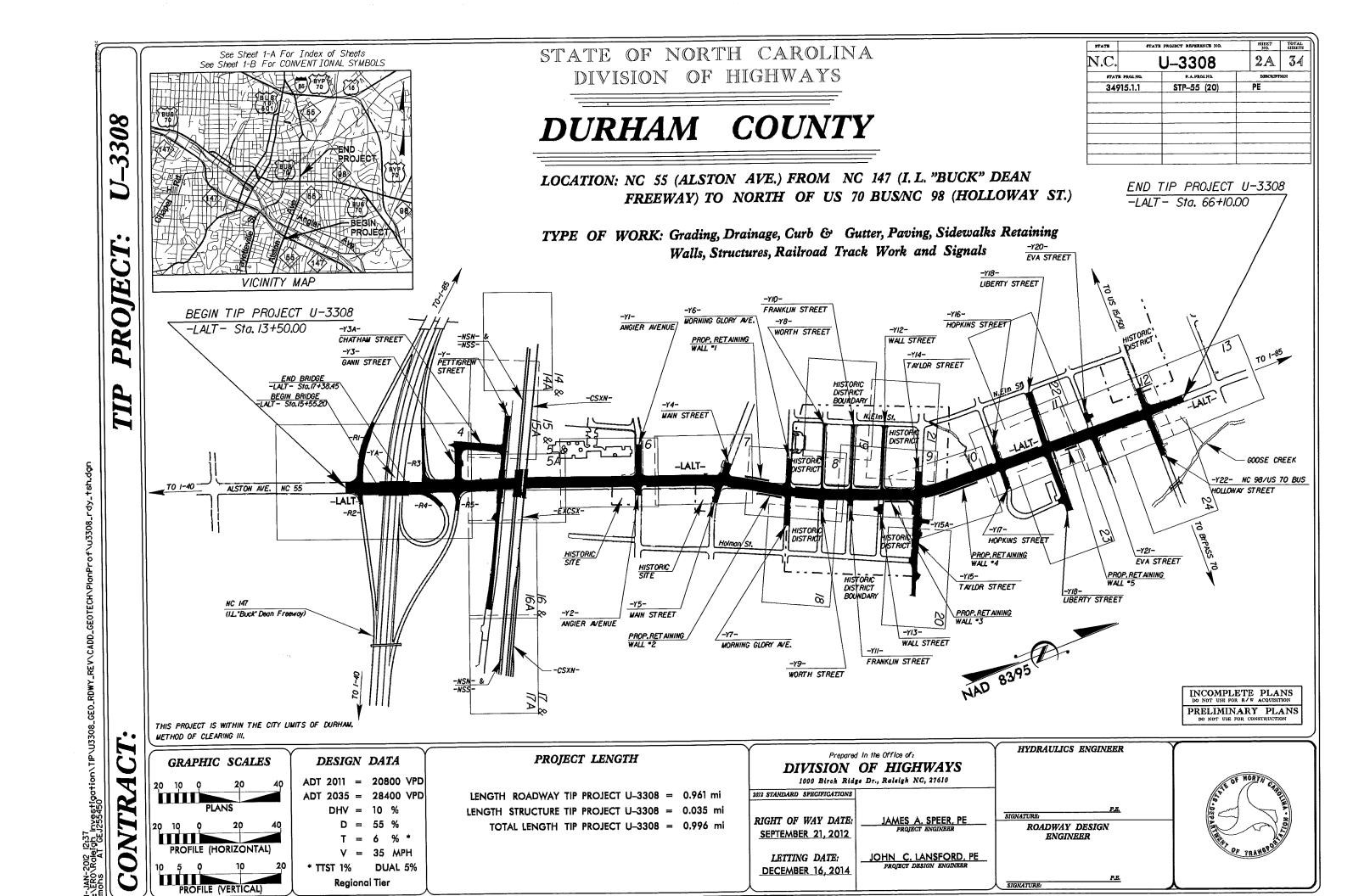
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

| | SOIL AND ROCK | LEGEND, TERMS, S | SYMBOLS, AND ABBREVI | ATIONS | |
|--|--|--|---|--|--|
| SOIL DESCRIPTION | GRADATION | | | DESCRIPTION | TERMS AND DEFINITIONS |
| SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR MEATHERED EARTH MATERIALS | WELL GRADED - INDICATES A GODD REPRESENTATION OF PARTICLE SIZES FROM FI UNIFORM - INDICATES THAT SDIL PARTICLES ARE ALL APPROXIMATELY THE SAME | SIZE (ALSO B | ROCK LINE INDICATES THE LEVEL AT WHICH NON-C | T IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. | ALLUVIUM (ALLUV.) - SOILS THAT HAYE BEEN TRANSPORTED BY WATER. |
| I THAT CAN BE DENETRATED WITH A CONTINIOUS FLIGHT POWER AUGER, AND TIELD LESS IMAN | ONITION OF THE STATE OF THE STA | | | | ADUIFER - A WATER BEARING FORMATION OR STRATA. |
| 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1306), SUIL | ANGULARITY OF GRAINS | | OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLL | | ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. |
| CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: | THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: | ANGULAR. | SV//63V//6 | LAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 | OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. |
| VERY STIFF, GRAV, SULY CLAV, MOST WITH WITERSEDOED FINE SAMO LATERS, HIGHLY PLASTIC, 4-7-6 | SUBANGULAR, SUBROUNDED, DR ROUNDED. | | CK (WR) BLOWS PER FOO | OT IF TESTED. | ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH ODES NOT NECESSARILY RISE TO DR ABOVE THE |
| SOIL LEGEND AND AASHTO CLASSIFICATION | MINERALOGICAL COMPOSITION | | | E GRAIN IGNEDUS AND METAMORPHIC ROCK THAT PT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, | CROUND SURFACE. |
| GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS DRGANIC MATERIALS | MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KADLIN, ETC. ARE USED IN WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. | DESCRIPTIONS ROC | GNEISS, GABBRO, | SCHIST, ETC. | CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE, |
| 0.405 0.70 0.405 0.70 0.405 | COMPRESSIBILITY | | N-LRYSTALLINE SEDIMENTARY RO | E GRAIN METAMORPHIC AND NON-COASTAL PLAIN DCK THAT WOULD YELD SPT REFUSAL IF TESTED, ROCK TYPE | COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. |
| GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-6, A-7 CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-8 A-3 A-6, A-7 | SLIGHTLY COMPRESSIBLE LIOUIO LIMIT LESS MODERATELY COMPRESSIBLE LIOUIO LIMIT EDUAL | THAN 31 COA | STAL PLAIN COASTAL PLAIN | SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD | CORE RECOVERY (REC.) - TOTAL LENGTH DF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL |
| SYMBOL 000000000000000000000000000000000000 | MODERATELY COMPRESSIBLE LIOUID LIMIT EDUAL HIGHLY COMPRESSIBLE LIOUID LIMIT GREATI | | IMENTARY ROCK SPT REFUSAL, R | ROCK TYPE INCLUDES LIMESTONE, SANOSTONE, CEMENTED | LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. |
| % PASSING SILT- | PERCENTAGE OF MATERIAL | TOT 7 | | ATHERING | <u>DIKE</u> - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. |
| # 10 50 HX GRANULAR CLAY PEAT | ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER | MATERIAL FRE | FSH ROCK FRESH, CRYSTALS BRIGHT, FEW JO | DINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER | DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINEO FROM THE |
| 40 38 MX 58 MX 55 MX 6 MX 35 MX 35 MX 35 MX 35 MX 35 MX 36 MX 36 MX 36 MX 36 MX 50 MX 50 MX 50 MX 36 MX 36 MX 6 MX 6 MX 6 MX 6 MX 6 MX | TRACE OF DRGANIC MATTER 2 - 3% 3 - 5% TRACE LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE | 1 - 10% | HAMMER IF CRYSTALLINE. | | HORIZONTAL. |
| LIGOTO LIMIT 48 HX 41 MN 48 HX 41 MN 48 HX 41 MN SDILS WITH | MODERATELY DRGANIC 5 - 10% 12 - 20% SOME | 20 - 35% VER | RY SLIGHT ROCK GENERALLY FRESH, JOINTS STAIN SLI.) CRYSTALS ON A BROKEN SPECIMEN FAC | NED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF | <u>DIP DIRECTION (DIP AZIMUTH) -</u> THE DIRECTION OR BEARING OF THE HORIZDATAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. |
| PLASTIC INDEX 6 MX NP 10 HX 10 HX 11 HN 11 HN 11 HN 11 HN LITTLE OR HIGHLY | HIGHLY DRGANIC >10% >20% HIGHLY | 35% AND ABOVE | OF A CRYSTALLINE NATURE. | | FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE |
| GRUDP INJEX 8 8 1 HX 8 HX 12 HX 18 HX 17 HX 18 H | GROUND WATER WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER ORILLIN | | | NEO AND DISCOLORATION EXTENOS INTO ROCK UP TO AY. IN GRANITOIO ROCKS SOME OCCASIONAL FELOSPAR | SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. |
| USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC | STATIC WATER LEVEL AFTER 24 HOURS | 102. | CRYSTALS ARE DULL AND DISCOLORED. | . CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. | FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. |
| MATERIALS SAND SAND BRAVEL AND SHARD SOLES | 1 | 440 | OC.) GRANITOIO ROCKS, MOST FELOSPARS AR | DISCOLORATION AND WEATHERING EFFECTS. IN RE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS | FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOCGED FROM PARENT MATERIAL. |
| GEN. RATING AS A EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE | PERCHEO WATER, SATURATED ZONE, DR WATER BEARING STI | HAIA | OULL SOUND UNDER HAMMER BLOWS AN | NO SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED | FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY |
| SUBGRADE PI OF A-7-5 SUBGROUP IS \leq LL - 30; PI OF A-7-6 SUBGROUP IS $>$ LL - 30 | SPRING OR SEEP | MOO | DERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED | O OR STAINED. IN GRANITOIO ROCKS, ALL FELOSPARS DULL | THE STREAM. |
| CONSISTENCY OR DENSENESS | MISCELLANEOUS SYMBOLS | SEV | YERE AND DISCOLORED AND A MAJORITY SHO | DW KADLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH DGIST'S PICK, ROCK GIVES 'CLUNK' SDUND WHEN STRUCK. | FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACEO IN THE FIELD. |
| COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED | ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SPT CPT DET CHT TEST BORING ST TPT DET CHT TEST BORING | SAMPLE | ID. SEV.) AND CAN BE EXCAVATED WITH A GEOLU 15 TESTED, WOULD YIELD SPT REFUSAL | | JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. |
| PRIMARY SOIL TYPE CONSISTENCY PENETRATION RESISTENCE (N-VALUE) COMPRESSIVE STRENGTH (TONS/FT ²) | | | | O OR STAINEO.ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED ANITOID ROCKS ALL FELOSPARS ARE KADLINIZED TO SOME | LEDGE - A SHELF-LIKE RIOGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO |
| GENERALLY VERY LOOSE 4 | L SOIL SYMBOL AUGER BORING | SS - SPLIT SPOON | EXTENT. SOME FRAGMENTS OF STRONG | ROCK USUALLY REMAIN. | ITS LATERAL EXTENT. |
| GRANULAR MEDIUM DENSE 10 TO 30 N/A | ARTIFICIAL FILL (AF) OTHER CORE PORTING | SAMPLE | IF TESTED, YIELDS SPT N YALUES > 1 | | LENS - A BODY OF SOIL OR ROCK THAT THINS DUT IN ONE OR MDRE DIRECTIONS. MOTTLED (HOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLDRS. MOTTLING IN |
| MATERIAL DENSE 30 TO 50 (NON-COHESIVE) VERY DENSE >50 | THAN ROADWAY EMBANKMENT | | SEV.) THE MASS IS EFFECTIVELY REDUCED T | O DR STAINEO, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT TO SOIL STATUS, WITH DNLY FRAGMENTS OF STRONG ROCK | SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD ORAINAGE. |
| VERY SOFT <2 <0.25 | — INFERRED SOIL BOUNDARY MONITORING WELL | SHILLE | REMAINING, SAPROLITE IS AN EXAMPLE | E OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR BRIC REMAIN. IF TESTED, YIELDS SPT N YALUES < 100 BPF | PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. |
| GENERALLY SOFT 2 TO 4 0.25 TO 0.50 | INFERRED ROCK LINE | RS - ROCK SAMPLE RT - RECOMPACTED TRIAXIAL COM | | NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SHALL AND | RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. |
| MATERIAL STIFF B TO 15 1 TO 2 | ₹₹₽₽₹ ALLUVIAL SOIL BOUNOARY | SAMPLE SAMPLE | SCATTERED CONCENTRATIONS. DUARTZ | MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS | ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF |
| (COHESIVE) VERY STIFF 15 TD 30 2 TO 4 HARD >30 >4 | 25/826 DIP & DIP DIRECTION OF SLOPE INDICATOR INSTALLATION | CBR - CALIFORNIA BEARING | ALSO AN EXAMPLE. | CHARDNESS | ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. |
| TEXTURE OR GRAIN SIZE | ROCK STRUCTURES SPT N-VALUE | RATIO SAMPLE | | SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES | SAPROLITE (SAP.) - RESIOUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE |
| U.S. STD. SIEVE SIZE 4 10 40 60 200 270 | SOUNDING ROD REF SPT REFUSAL | VE. | ERY HARO CANNOT BE SCRATCHED BY KNIFE OR SEVERAL HARO BLOWS OF THE GEOLO | | PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND |
| OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053 | ABBREVIATIONS | H | HARO CAN BE SCRATCHED BY KNIFE OR PIC TO DETACH HAND SPECIMEN. | CK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED | RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL |
| BOULOER COBBLE GRAVEL SAND SAND SILT CLAY | HI HOOLI ILL OOIL | 20 - MOISTURE CONTENT | | CK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE | TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR |
| (BLDR.) (CDB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.) | BT - BORING TERMINATED MED MEOIUM CL CLAY MICACEOUS | | ARO EXCAVATED BY HARD BLOW OF A GEO | DLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED | SLIP PLANE. |
| GRAIN MM 305 75 2.0 0.25 0.05 0.005 | CPT - CONE PENETRATION TEST MOD MODERATELY | WEA WEATHERED | BY MDOERATE BLOWS. HEDIUM CAN BE GRODVED OR GOUGED 0.05 IN | NCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. | STANDARO PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N DR BPF) DF A 140 LB. HAMMER FALLING 30 INCHES REQUIREO TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH |
| SIZE IN. 12 3 | CSE COARSE NP - NON PLASTIC DMT - DILATOMETER TEST ORG ORGANIC | | HARO CAN BE EXCAVATED IN SMALL CHIPS | TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE | A 2 INCH OUTSIDE DIAMETER SPLIT SPDDN SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS |
| SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION | - OPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST | - | | BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS | THAN 0.1 FOOT PER 60 BLOWS. |
| SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION (ATTERBERG LIMITS) DESCRIPTION | F - FINE SO SANO, SANOY | l ° | | SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN | STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. |
| - SATURATED - USUALLY LIDUID; VERY WET, USUALLY | FOSS, - FOSSILIFEROUS SL SILT, SILTY FRAC FRACTUREO, FRACTURES SLI SLIGHTLY | 1, | | EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH | STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY |
| (SAT.) FROM BELOW THE GROUND WATER TABLE | FRAGS FRAGMENTS TCR - TRICONE REFUSAL | | SDFT OR MORE IN THICKNESS CAN BE BROW | KEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY | TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. |
| PLASTIC SEMISOLID; REDUIRES DRYING TO | FQUIPMENT USED ON SUBJECT PROJ | FCT | FRACTURE SPACING | BEDDING | IOPSOIL (IS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. |
| RANGE - WET - (W) ATTAIN OPTIMUM MOISTURE | L. | MMER TYPE: | IERM SPACING | TERM THICKNESS | BENCH MARK: |
| | | AUTDMATIC X MANUAL | VERY WIDE MORE THAN 10 FEET | VERY THICKLY BEODED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET | |
| OM OPTIMUM MOISTURE - MOIST - (M) SOLIO; AT OR NEAR OPTIMUM MOISTURE | MOBILE B CLAY BITS | ACTORITIES A TOTAL | WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET | THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET | ELEVATION: FT. |
| SL SHRINKAGE LIMIT | 6° CONTINUOUS FLIGHT AUGER CO | RE SIZE: | CLOSE 0.16 TD 1 FEET VERY CLOSE LESS THAN 0.16 FEET | THICKLY LAMINATED 0.008 - 0.03 FEET | NOTES: |
| - DRY - (D) ATTAIN OPTIMUM MOISTURE | X BK-51 X 8' HOLLDW AUGERS |]-B | | THINLY LAMINATED < 0.008 FEET | ቹ: TRIASSIC |
| PLASTICITY | | ¬ | | DURATION NING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. | 1 |
| PLASTICITY INDEX (PI) DRY STRENGTH | TUNGCARBIDE INSERTS |]-H | DUDDING | G WITH FINGER FREES NUMEROUS GRAINS; | |
| NONPLASTIC 0-5 VERY LOW SLIGHT | CME-550 | AND TODLS: | FRIABLE GENTLE | BLOW BY HAMMER DISINTEGRATES SAMPLE. | |
| MEO. PLASTICITY 16-25 MEDIUM | PORTABLE HOIST TRICONE STEEL TEETH | POST HOLE DIGGER | | CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; | |
| HIGH PLASTICITY 26 OR MORE HIGH | TRICONE 'TUNG,-CARB, | HAND AUGER | BREAKS | S EASILY WHEN HIT WITH HAMMER. | |
| COLOR | CORE BIT | SDUNDING ROD | INOURATEO GRAINS DIFFICL | ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; ULT TO BREAK WITH HAMMER. | |
| DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). | | VANE SHEAR TEST | EXTREMELY INDURATED SHARP | HAMMER BLOWS REQUIRED TO BREAK SAMPLE; | |
| MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. | | | SAMPLE | E BREAKS ACROSS GRAINS. | PEVISED 02/23/06 |

PROJECT REFERENCE NO. 34915.1.1 (U-3308)

SHEET NO.





STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

Beverly Eaves Perdue GOVERNOR

P.O. BOX 25201, RALEIGH, N.C. 27611-5201

Gene A. Conti, Jr.
SECRETARY

February 1, 2012

STATE PROJECT:

34915.1.1 (U-3308)

FEDERAL PROJECT:

STP-55 (20)

COUNTY:

Durham

DESCRIPTION:

NC 55 (Alston Ave.) from NC 147 (Buck Dean Freeway) to North of US 70

Business/NC 98 (Holloway St.)

SUBJECT:

Geotechnical Report – Inventory

Project Description

This project consists of widening Alston Ave. (-L-, NC 55) in Durham to four lanes with median and turn lanes. The project begins just south of the intersection of the Durham Expressway (NC 147, -L- Sta. 16+42) and extends 1 mile north to Gilbert St. Replacement bridges will be constructed at the Durham Expressway crossing. Replacement structures will also be placed over Alston Ave. on Pettigrew St. (-Y-Sta. 18+19), Norfolk Southern Railroad (-NSS- Sta. 21+21, and –NSN- Sta. 21+11), and CSX Railroad (-CSXN- Sta. 13+22). Some of the smaller intersecting streets are being re-aligned during the widening. Six retaining walls are also planned along the project.

The geotechnical field investigation was conducted during January and July of 2006. Tierra Engineering was contracted to perform the subsurface investigation, they in turn subcontracted Mid-Atlantic Drilling for drilling only. Geologists from Tierra Engineering sampled and logged the borings. ATV-mounted CME-45 and truck mounted D-50 drill machines were used during the field investigation. Standard Penetration Tests were performed in selected borings and additional borings were advanced using continuous flight augers. Representative soil samples were collected for visual classification in the field and submitted for laboratory analysis by NCDOT's Materials and Tests Unit.

The following alignments, totaling 2.22 miles, were investigated. Subsurface soil profiles, or cross-sections, of these alignments are included in this report:

| Line | | Station | |
|--------|-------|----------------|-------|
| -LALT- | 13+50 | to | 66+10 |
| -Y- | 14+00 | to | 21+49 |
| -Y4- | 10+00 | to | 13+85 |

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| -Y7- | 10+43 | to | 14+05 |
|-------|-------|----|-------|
| -Y8- | 10+00 | to | 13+89 |
| -Y10- | 10+00 | to | 14+05 |
| -Y13- | 10+00 | to | 11+22 |
| -Y14- | 10+00 | to | 11+40 |
| -Y15- | 10+00 | to | 12+05 |
| -Y18- | 10+20 | to | 17+83 |
| -Y20- | 10+00 | to | 11+52 |
| -Y22- | 12+55 | to | 17+90 |
| -R1- | 11+00 | to | 15+11 |
| -R3- | 10+00 | to | 13+60 |
| -NSN- | 12+00 | to | 30+40 |

Areas of Special Geotechnical Interest

1) <u>Highly Plastic Clay Soils</u>: Occurrences of highly plastic clay soil (Plasticity Index greater than 25) are noted below:

| Alignment | Station | <u>Offset</u> |
|-----------|----------------|---------------|
| -LALT- | 14+89 | 72' RT |
| -LALT- | 21+78 | 37' LT |
| -LALT- | 25+02 | 67' LT |
| -LALT- | 30+06 | 15' RT |
| -LALT- | 30+07 | 63' LT |
| -LALT- | 30+45 | 56' LT |
| -LALT- | 35+53 | 7' RT |
| -LALT- | 37+20 | 6' LT |
| -LALT- | 58+96 | 35' LT |
| -Y- | 13+92 | 27' LT |
| -Y18- | 10+32 | 35' LT |
| -NSN- | 12+05 | 26' LT |
| -NSN- | 14+04 | 23' LT |
| -NSN- | 15+92 | 15' LT |
| -NSN- | 16+34 | 34' RT |
| -NSN- | 17+95 | 21' LT |
| -NSN- | 19+87 | 29' LT |
| -NSN- | 22+46 | 32' LT · |
| -NSN- | 24+58 | 47' RT |
| -NSN- | 24+79 | 32' RT |
| -NSN- | 30+03 | 27' RT |
| -R1- | 14+03 | 22' RT |

2) <u>Non-Crystalline Rock</u>: Non-Crystalline (Sedimentary Triassic) rock was encountered in the following borings:

| Alignment | Station | <u>Offset</u> |
|-----------|----------------|---------------|
| -LALT- | 50+51 | 68' RT |
| -Y- | 19+90 | 34' RT |
| -Y4- | 12+84 | 38' LT |
| -Y14- | 11+10 | 22' RT |
| -Y14- | 11+63 | 19' RT |

Physiography and Geology

The project is located in the gently rolling terrain of the Eastern Piedmont area of North Carolina. A mixture of businesses, single-family homes, and churches are located along the project corridor. Small creeks and ephemeral streams run across the project corridor, generally from right to left across the alignment.

Geologically the project lies within the Durham Triassic Basin which is part of a series of failed rifts stretching northward from South Carolina to New Jersey. These rift valleys formed during extensional, normal faulting in the Triassic Period some 200 million years ago. This rifting and separation of Africa from North America eventually formed the Atlantic Ocean.

The geology of the project area consists of Triassic age sedimentary rocks, primarily siltstone and sandstone, and recent alluvial soils. The depositional nature of the Triassic sediments created alternating beds of siltstone, mudstone and sandstone. Combined with the active tectonic environment at the time of deposition, the soils derived from these deposits are highly variable and often laterally discontinuous.

Soil Properties

Soils encountered at the project site include roadway embankment soils, alluvial sediments, and Triassic residual soils.

Roadway embankment soil is common along the -L- alignment. The embankments range up to 24 feet in height Where sampled, the embankment soil consists of dry to moist, stiff to very stiff, silty clay, sandy clay and sandy silt(AASHTO classifications of A-7-6, A-6, and A-4), with some loose to medium dense, dry to moist, silty sand (A-2-4).

Alluvial soils were deposited along small creeks, which flow across the alignment in several areas (-L-52+50, -Y22-18+90, -NSN-17+95 -NSN-30+03). These soils consist of moist to wet, very loose to medium dense, silty sand (A-2-4) and soft to stiff, silty clay (A-7-6).

The Triassic residual soils are derived from the in-place weathering of the underlying sedimentary bedrock. These soils consist primarily of moist, medium stiff to hard, silty clay (A-7-5 and A-7-6) and moist to dry, loose to dense, silty sand (A-2-4 and A-2-5). Silty soils consisting of moist, medium stiff to very stiff, sandy silt (A-4) occur within the project area as well. Areas containing highly plastic soils

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(plasticity indices of greater than 25) are listed above in the section "Areas of Special Geotechnical Interest".

Rock Properties

Weathered rock and non-crystalline rock occur in several areas of the project. The weathered rock is derived from the underlying sedimentary rock of the Durham Triassic Basin, and consists of sandstone and siltstone. Weathered and non-crystalline siltstone in this area is known to be highly degradable when exposed to air and water.

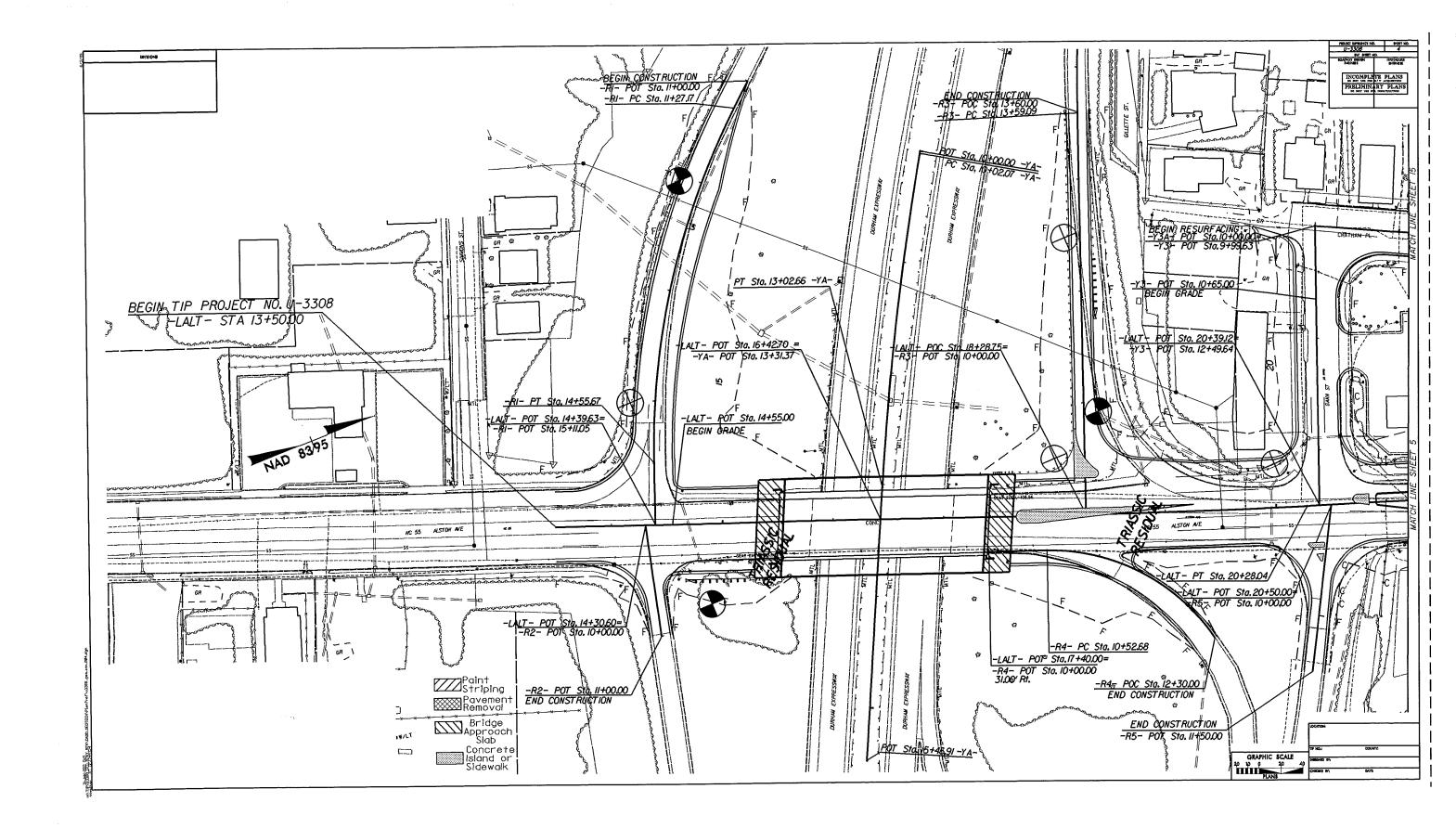
Groundwater

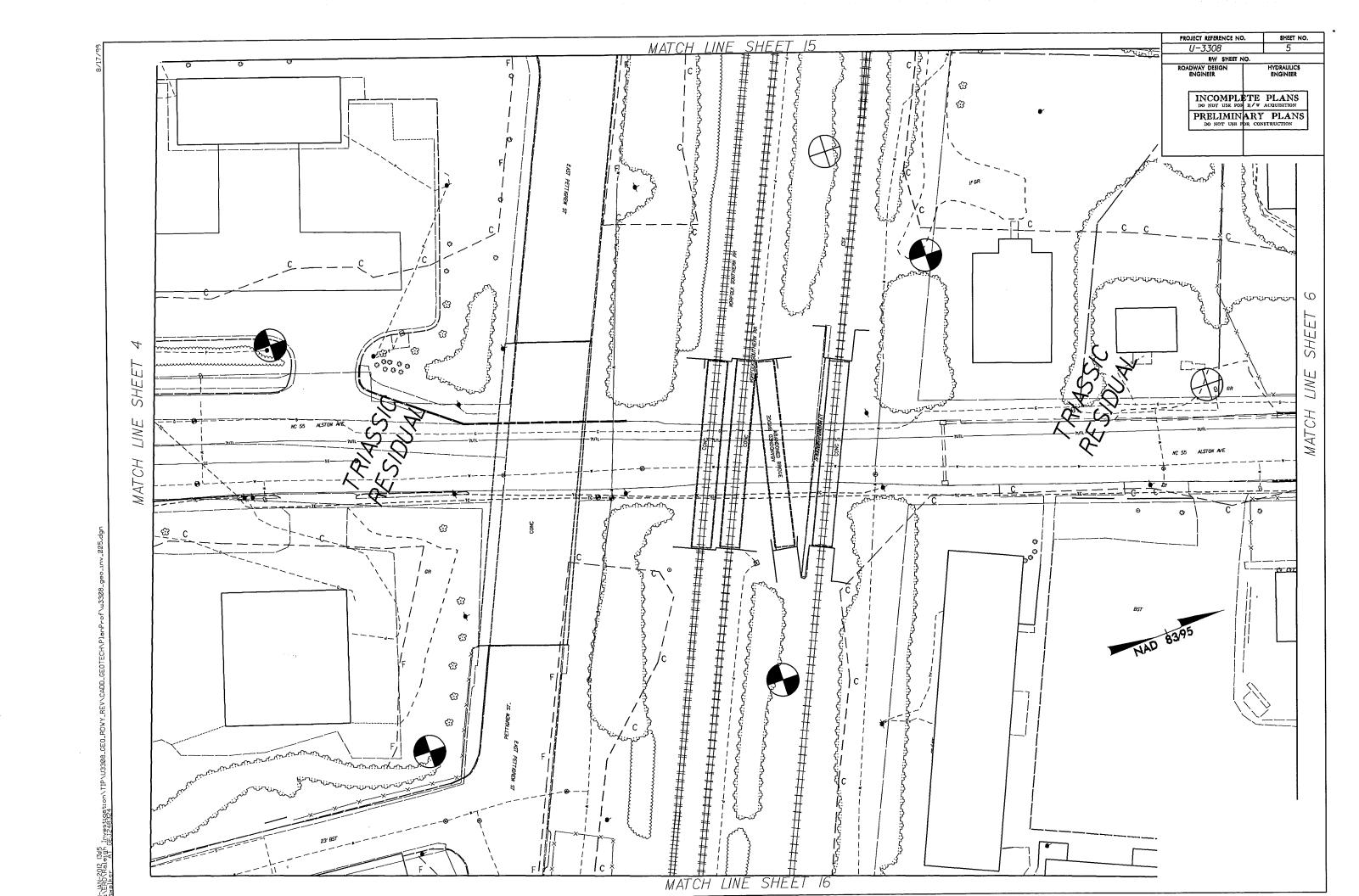
Groundwater was encountered in several borings completed on this project. Groundwater, when encountered in Triassic residual soil or weathered rock, was variable across the project, ranging from 2 feet to 12 feet below the ground surface. Groundwater in borings located in alluvial areas ranged from 5 feet to 6 feet in depth. Based on the investigation, groundwater is not anticipated to cause problems during construction.

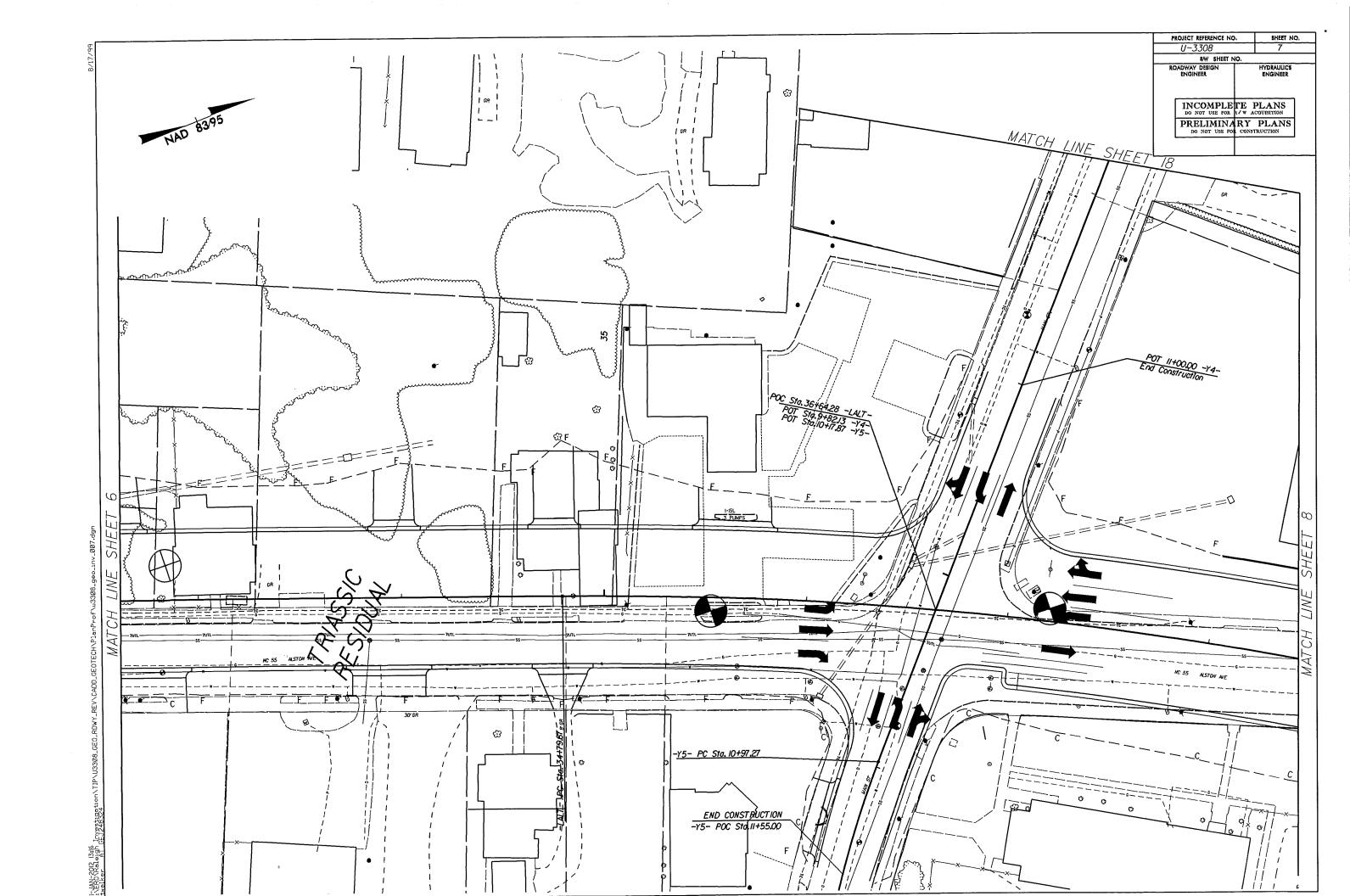
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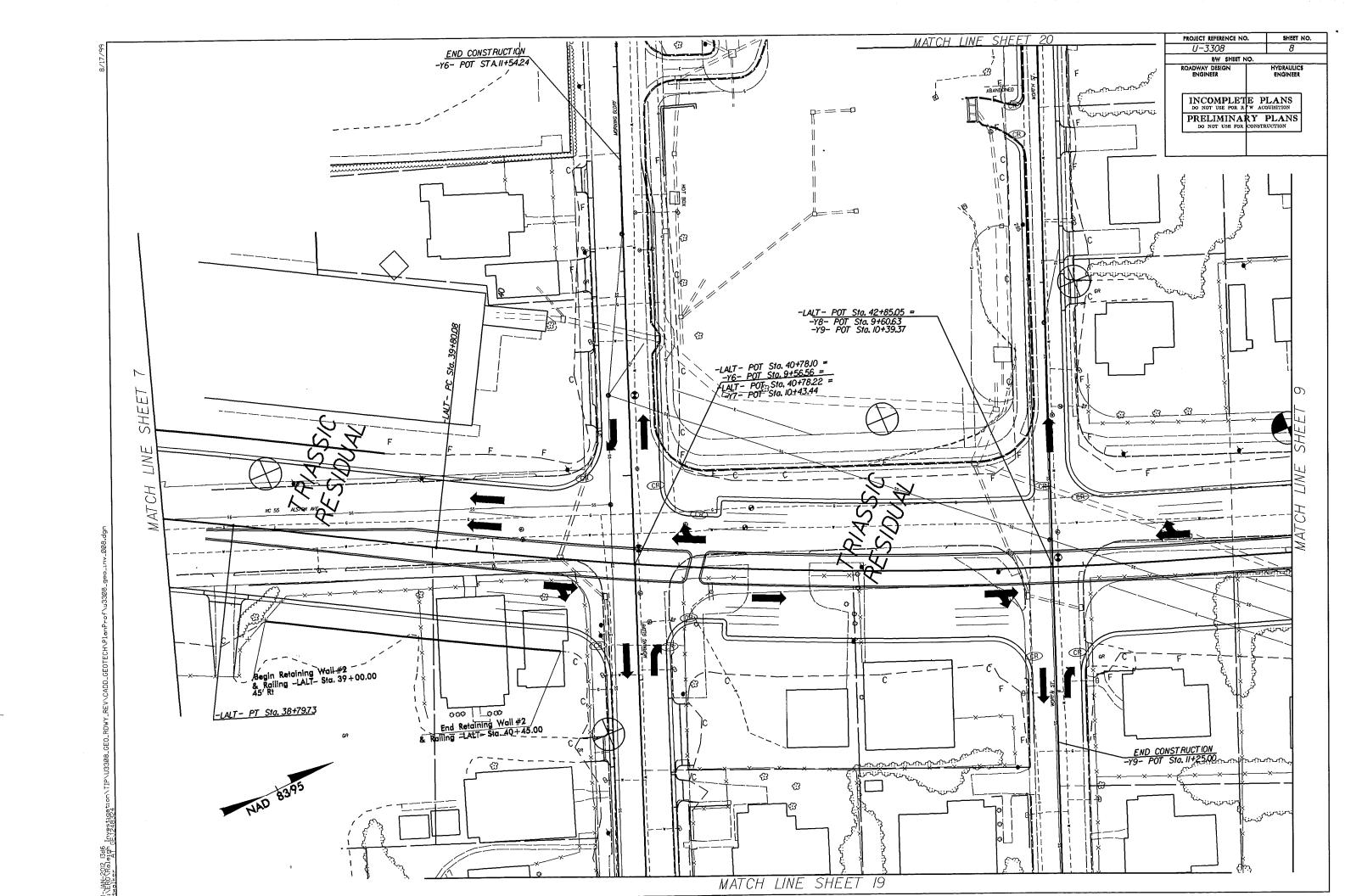
Nathan Mohs, LG

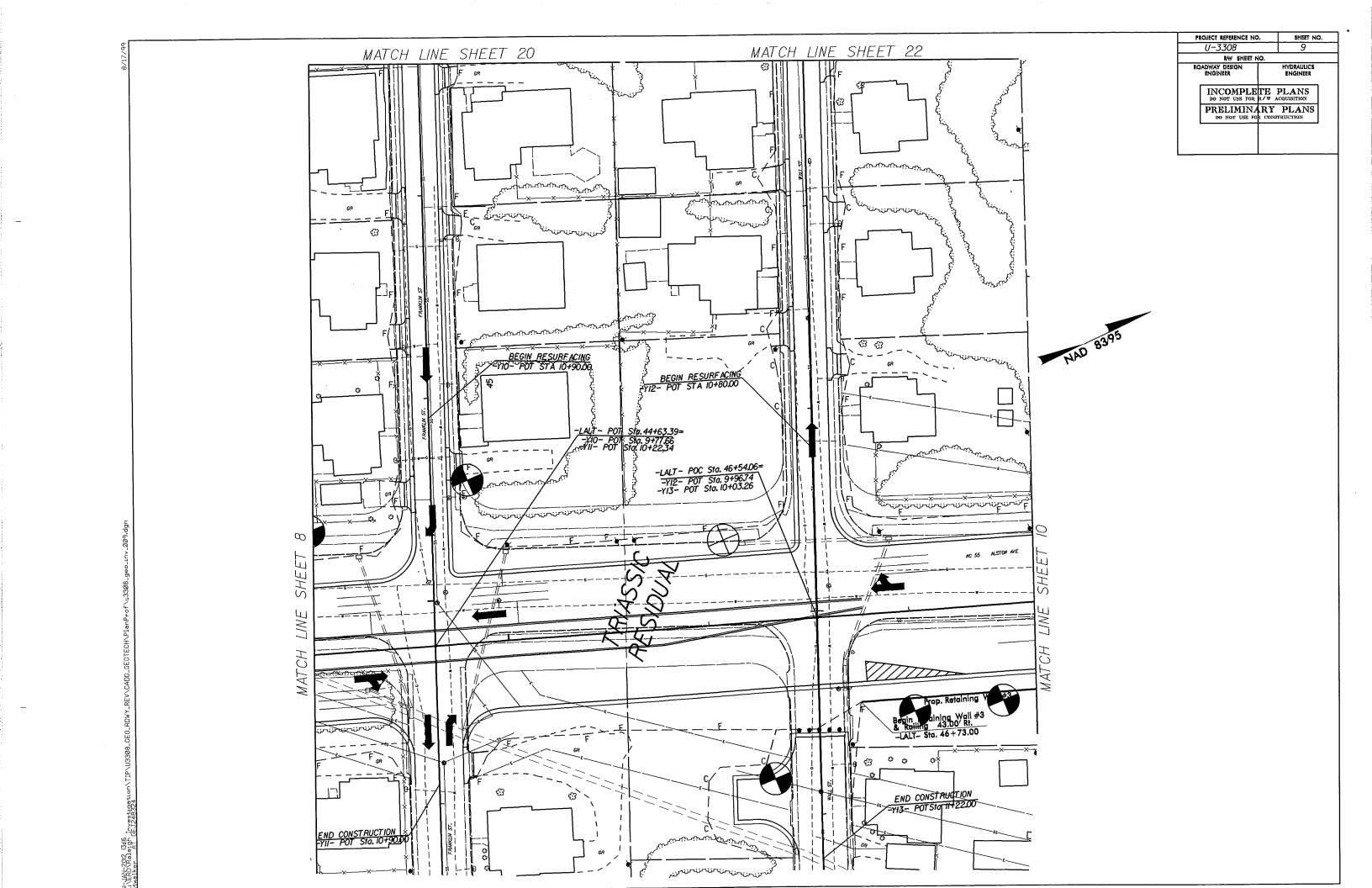
Project Geologic Engineer

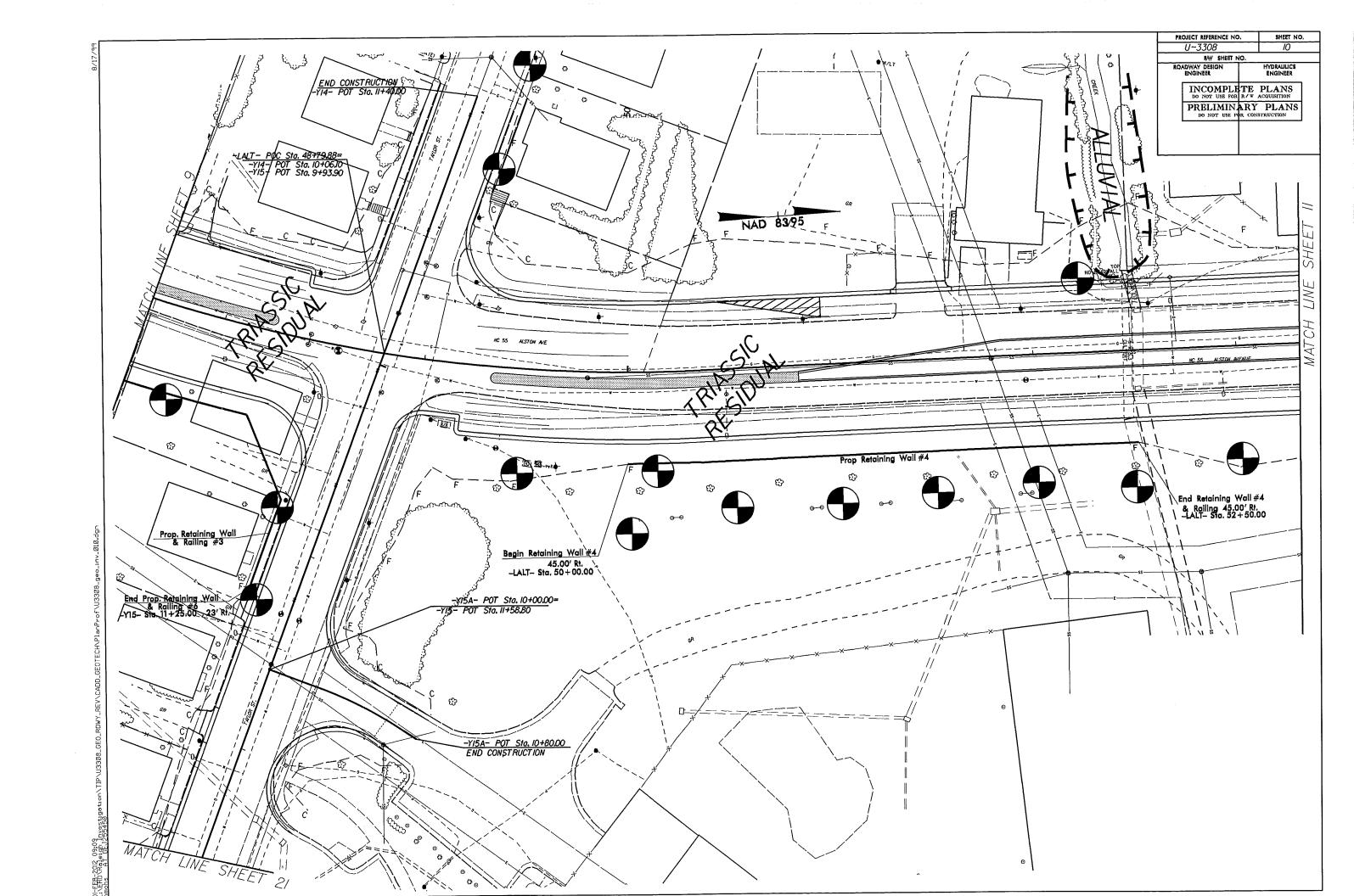


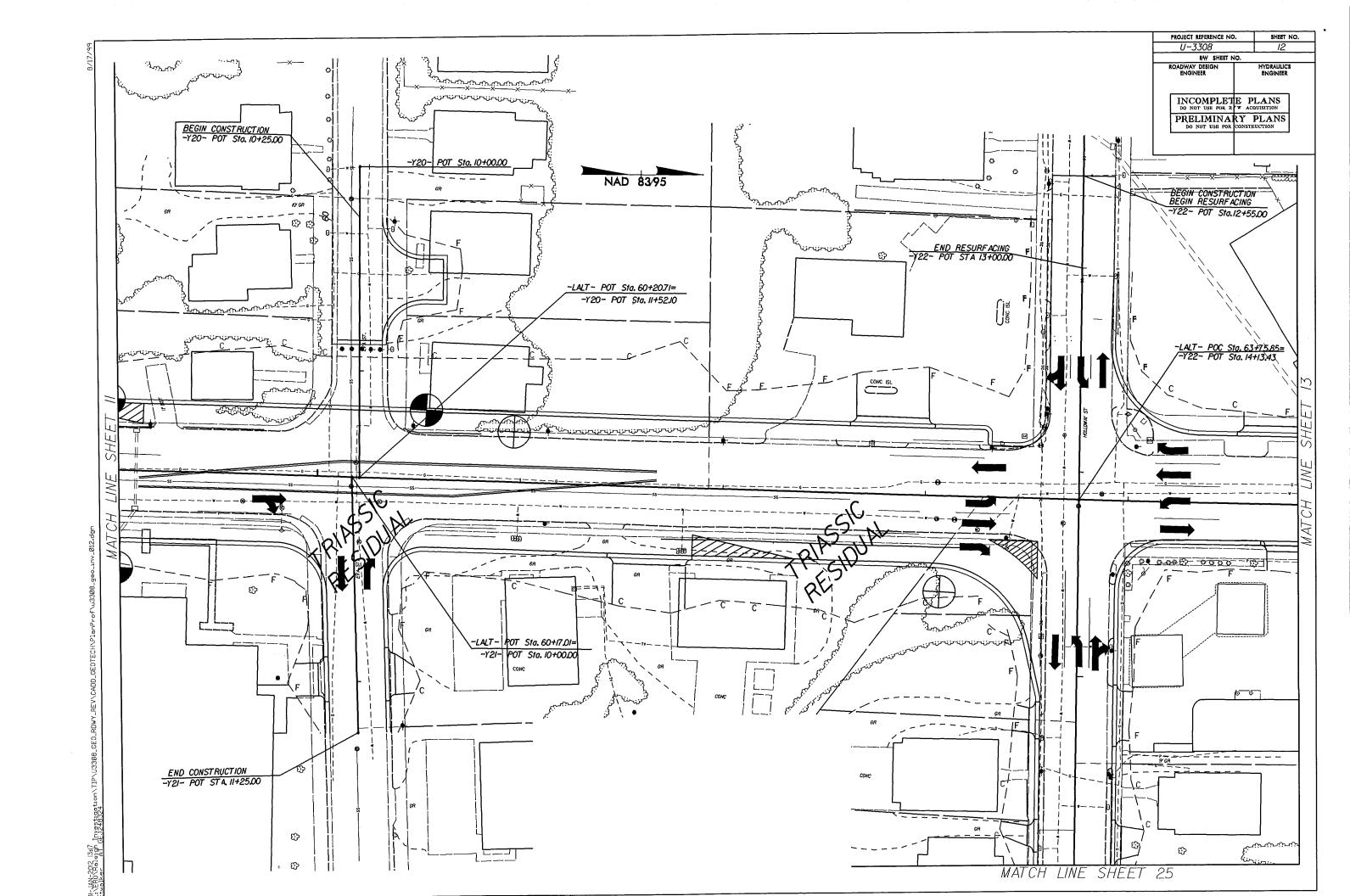


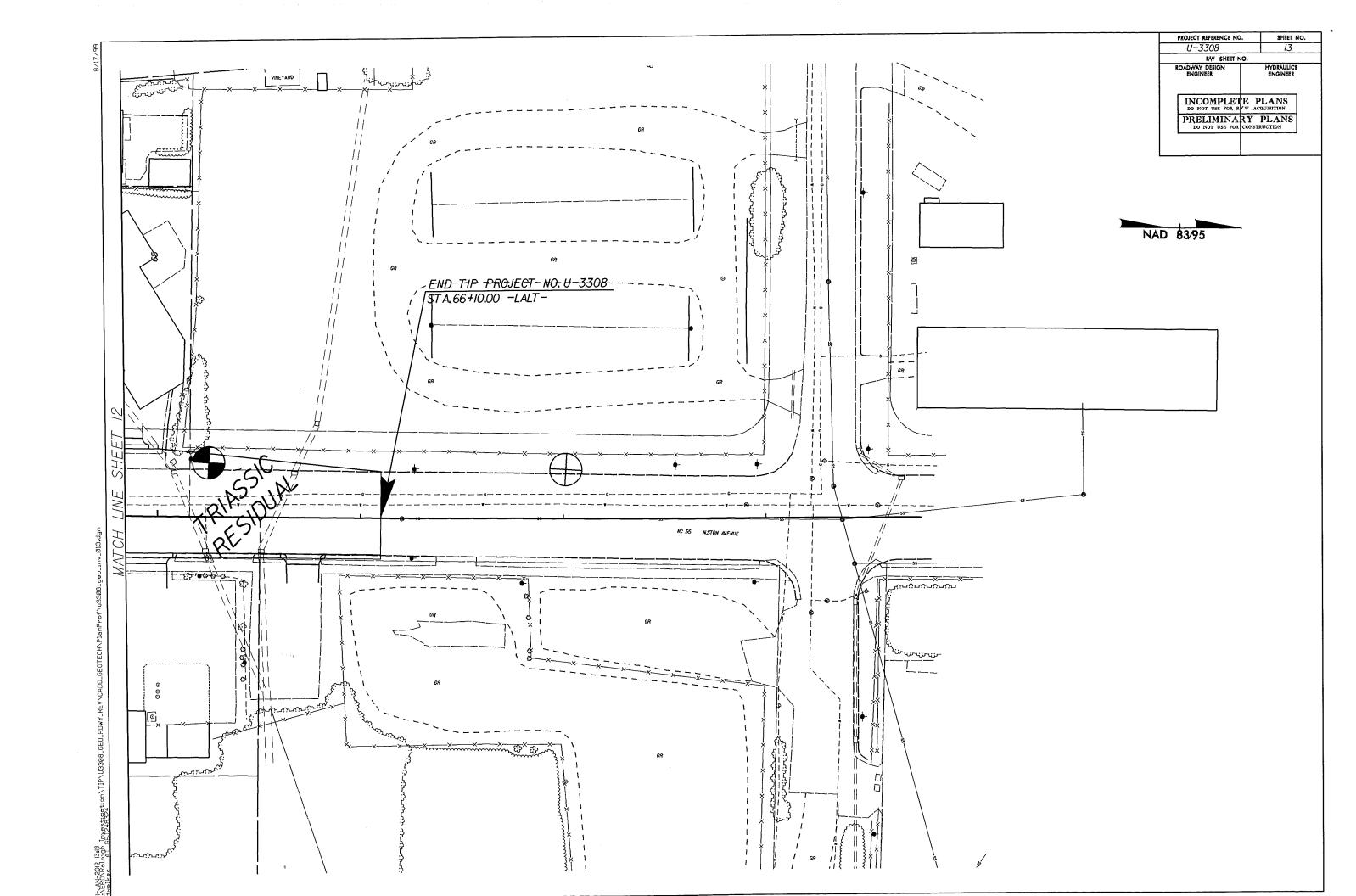


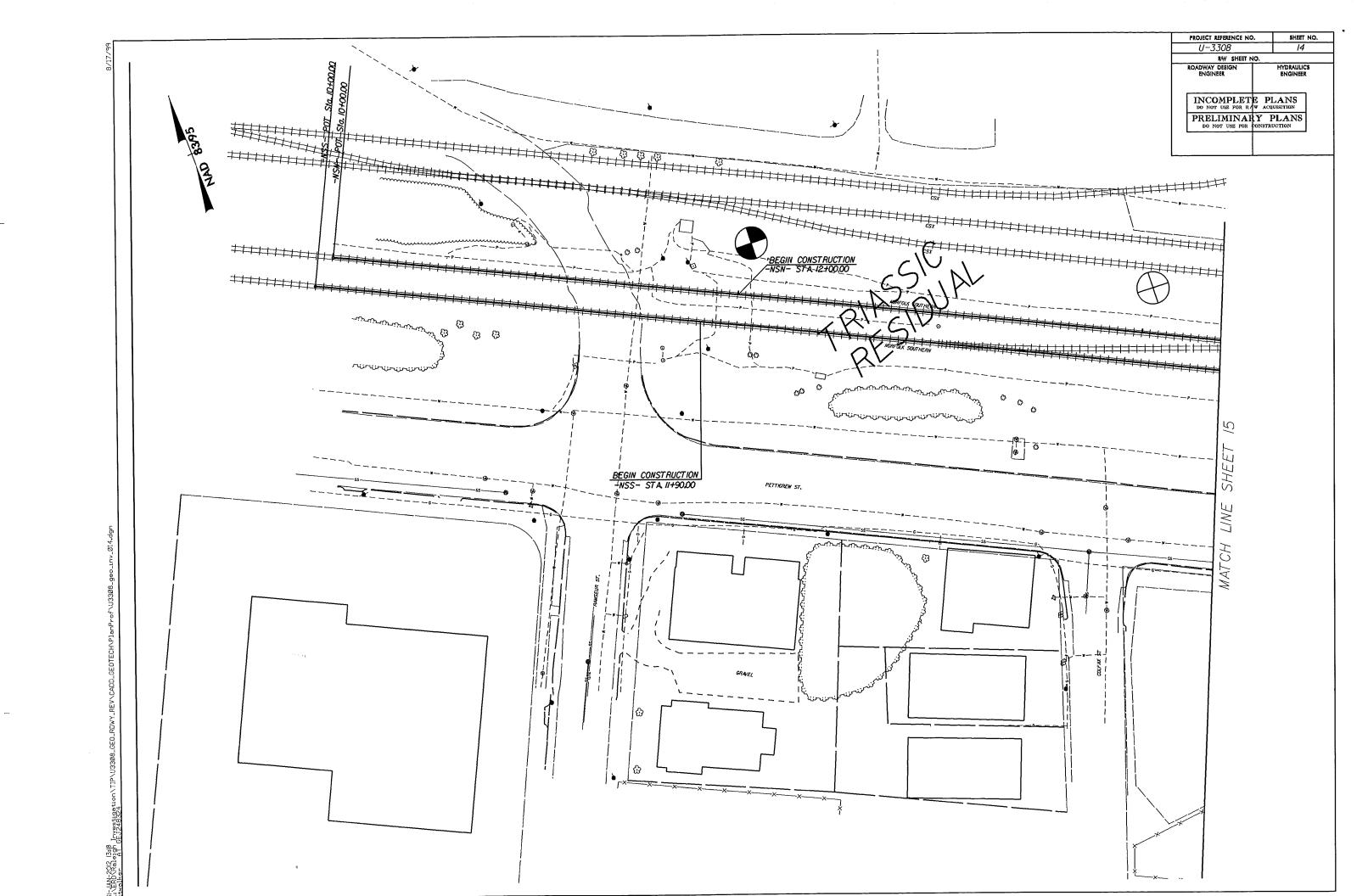


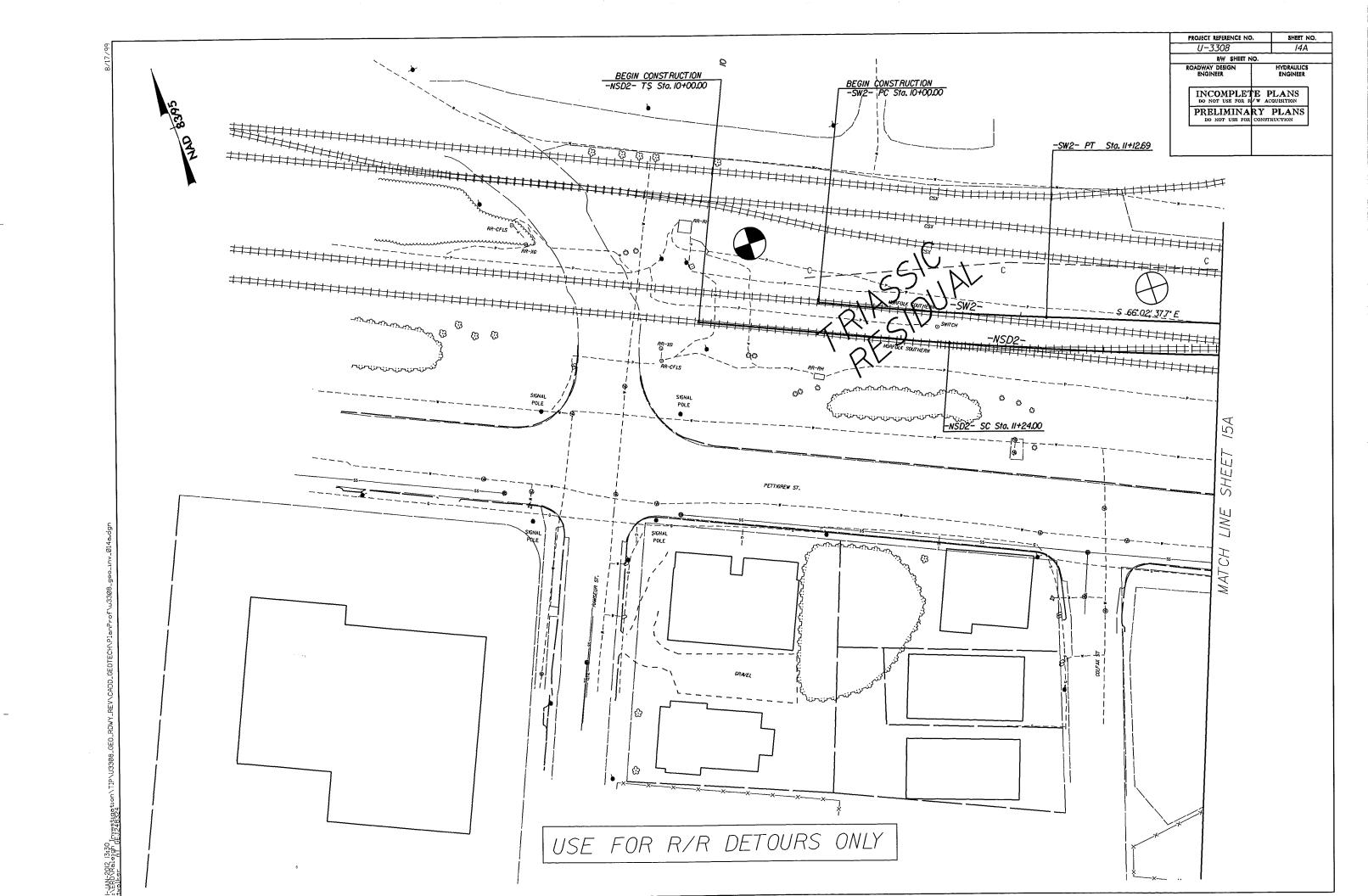


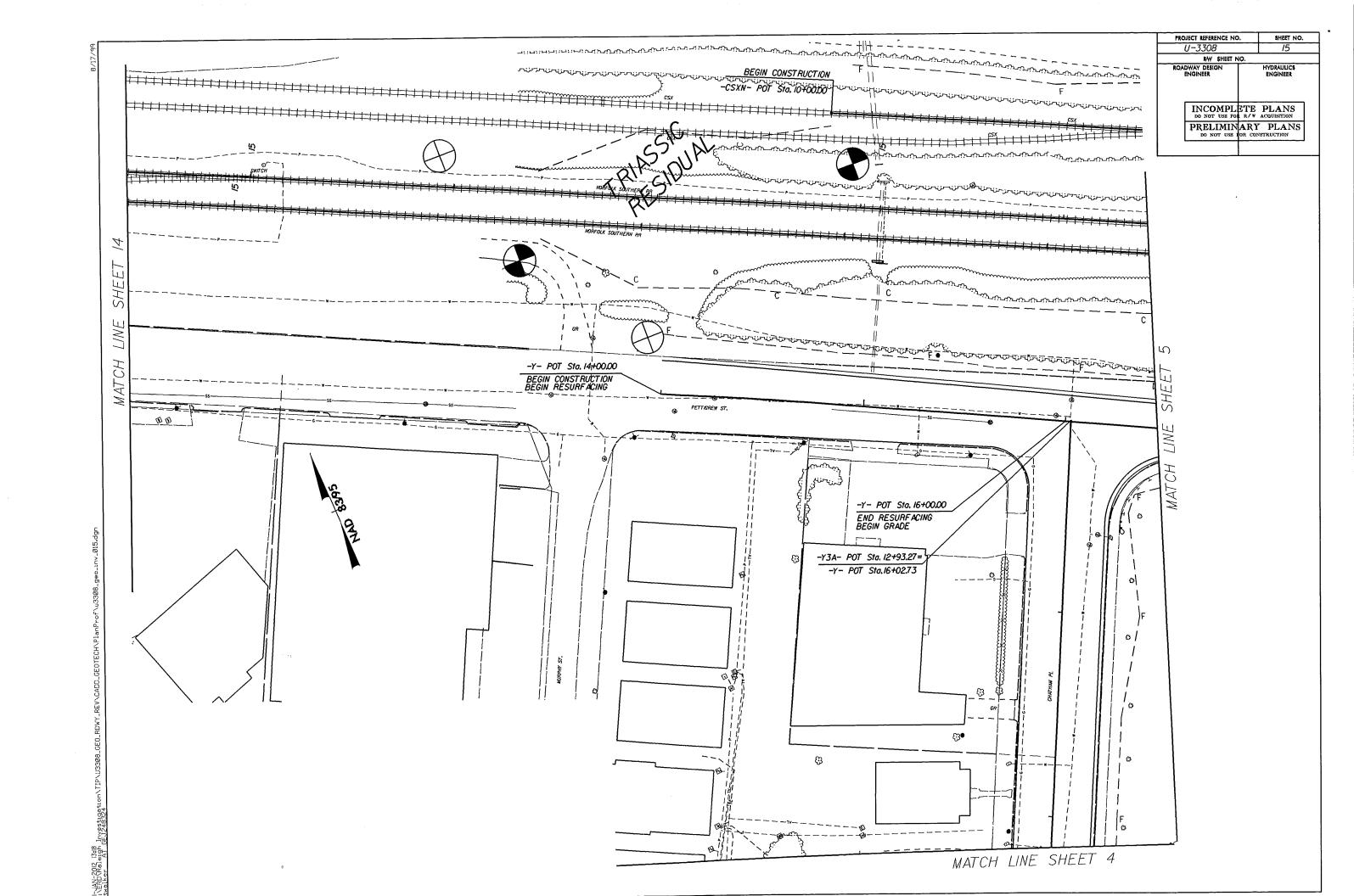


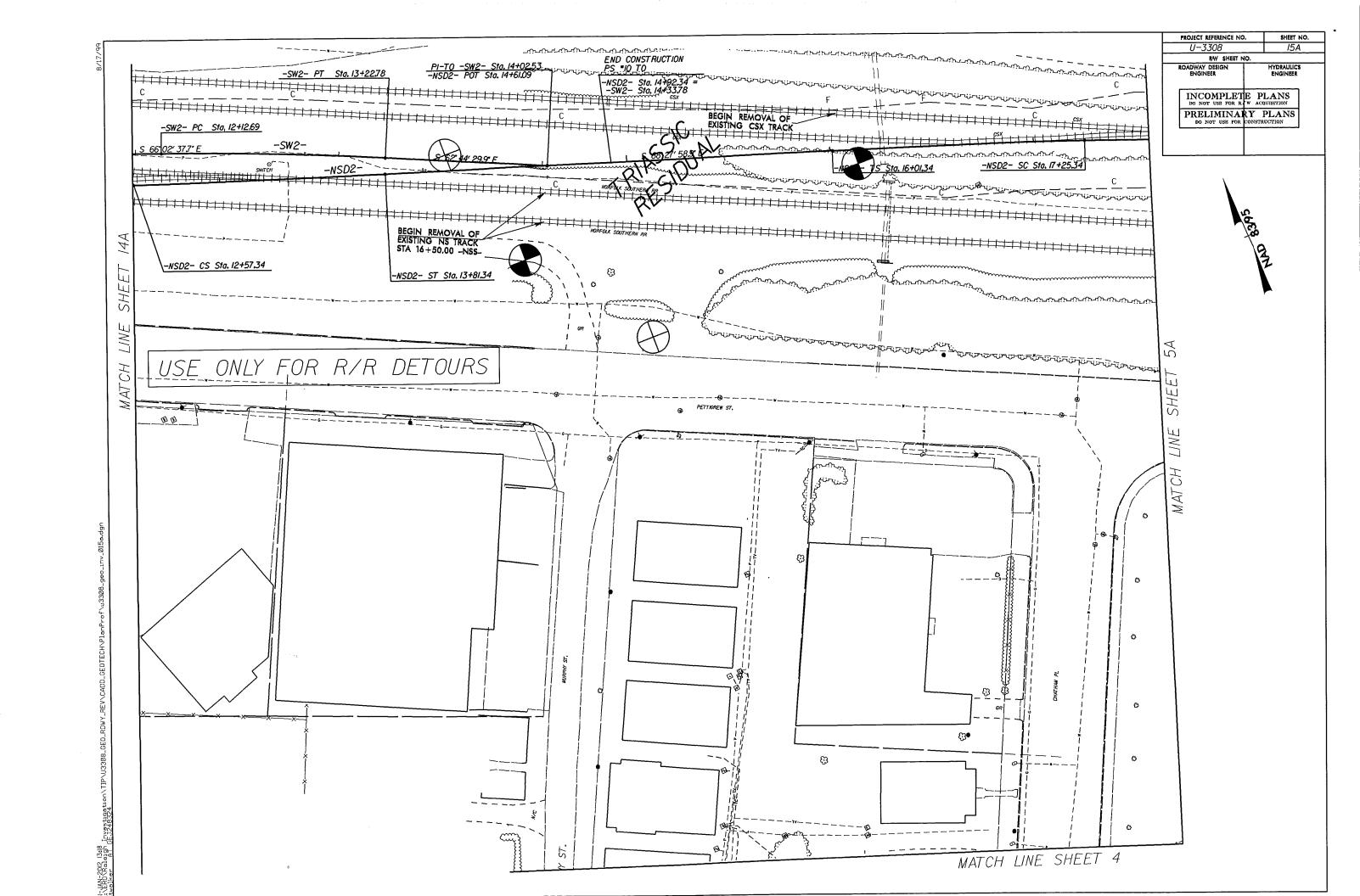


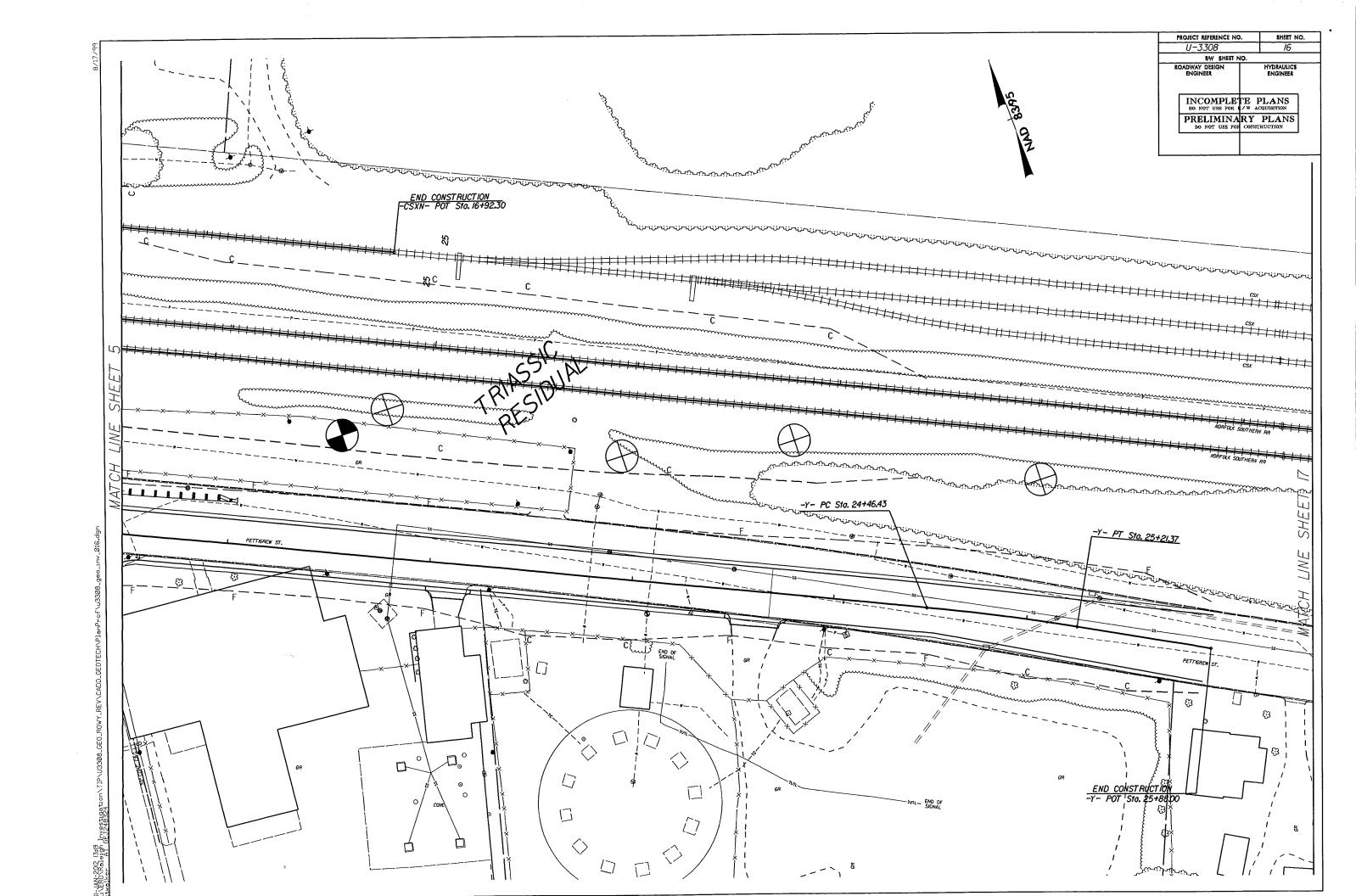


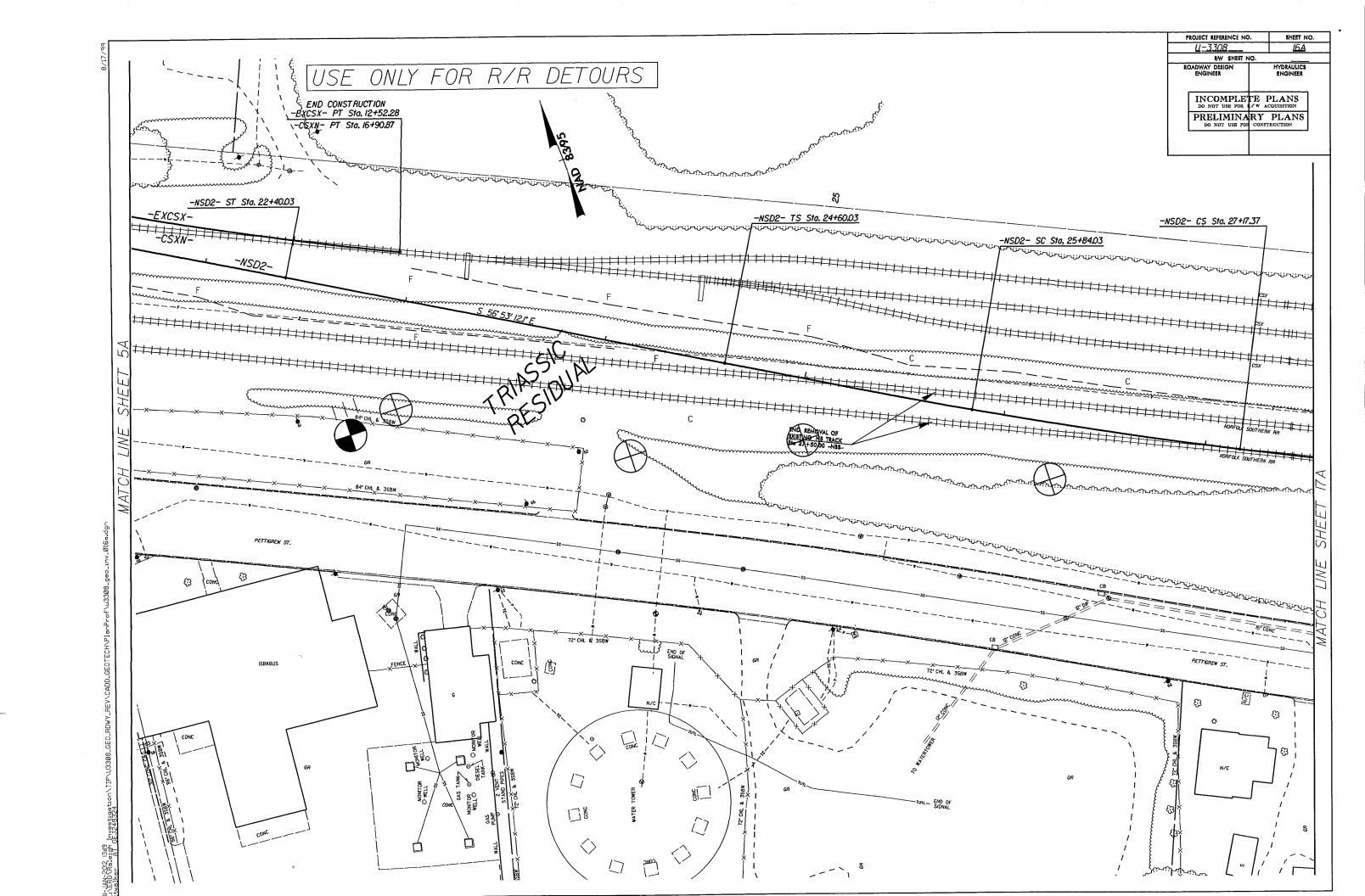


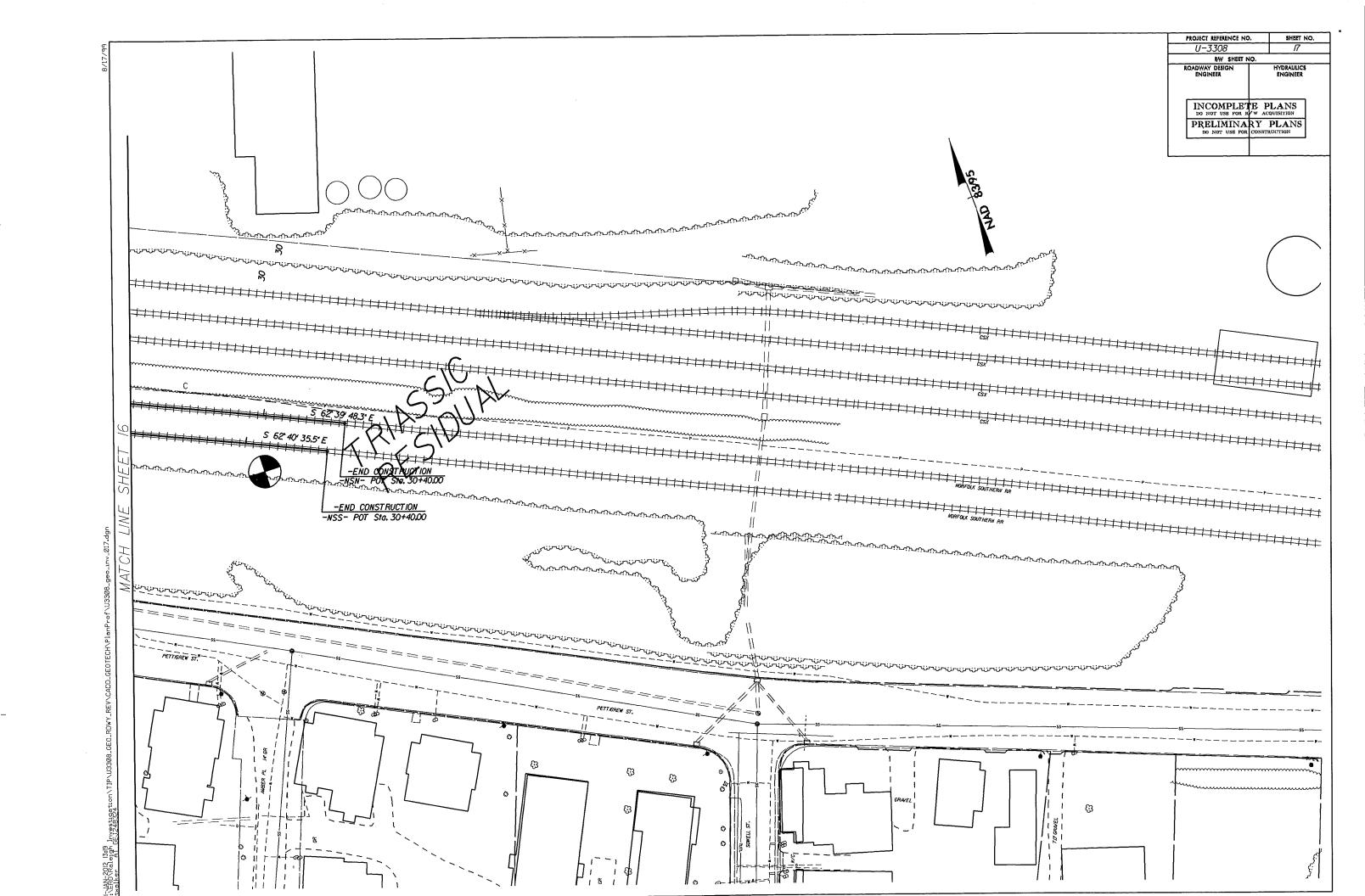


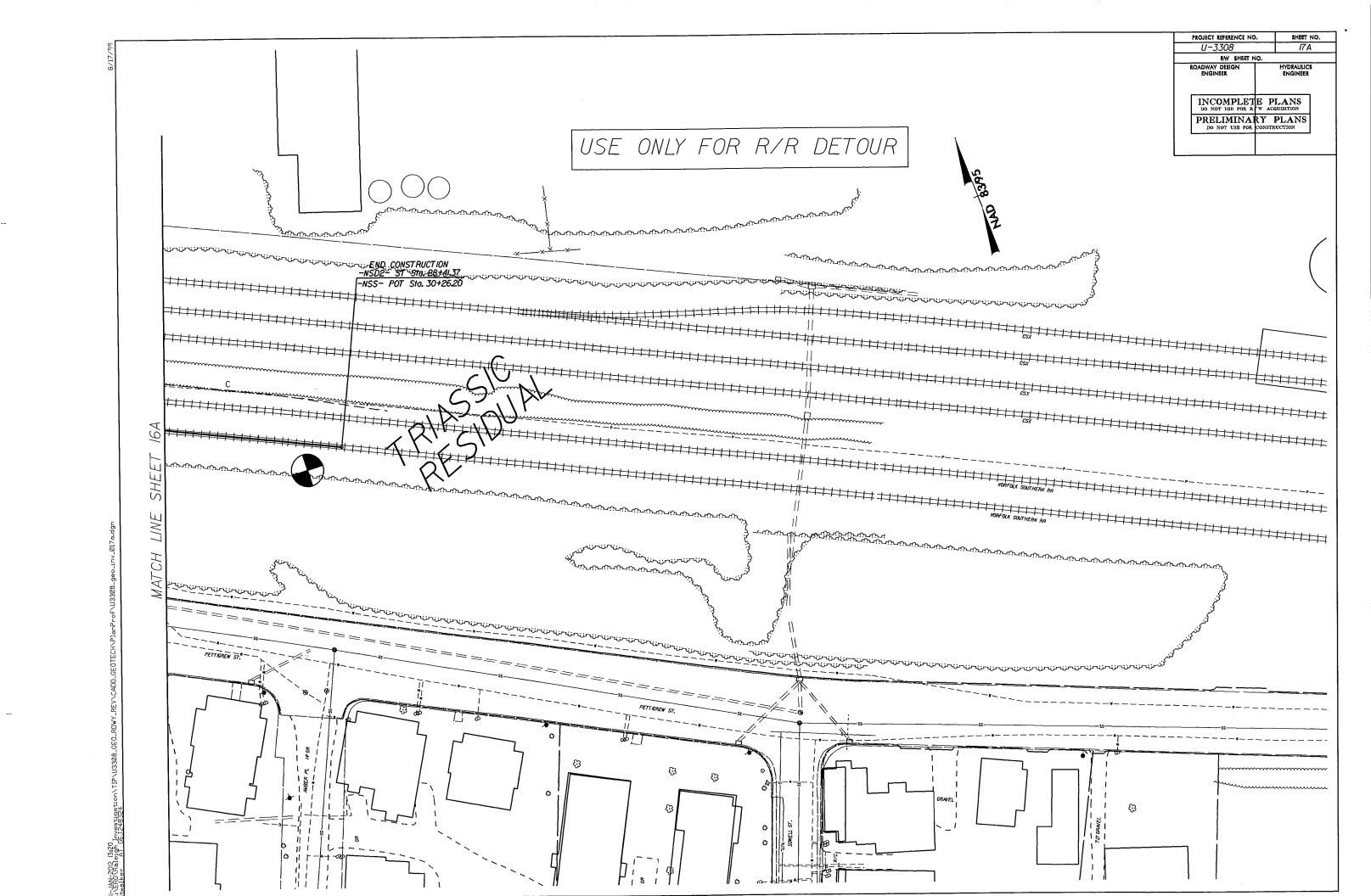


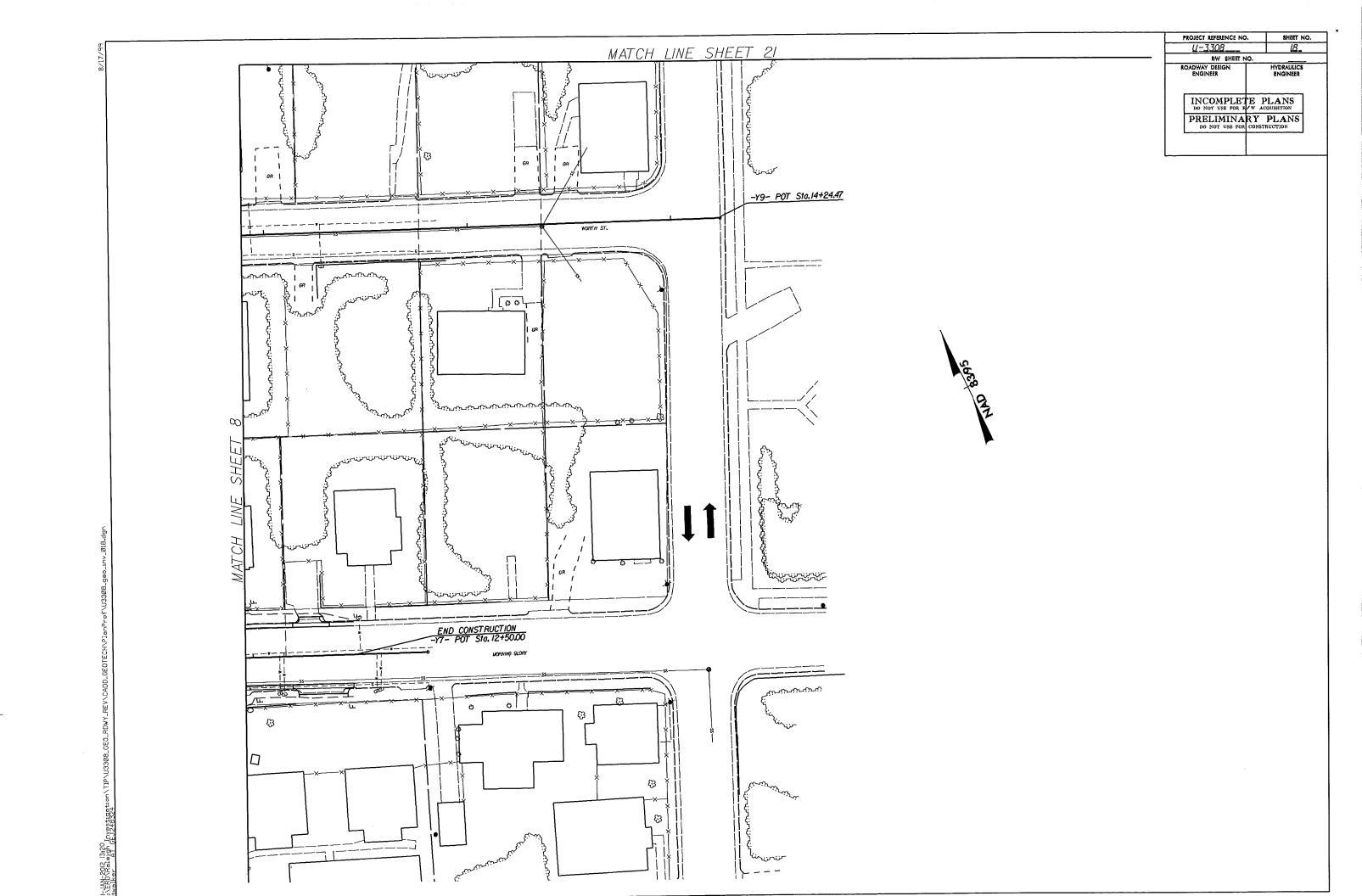


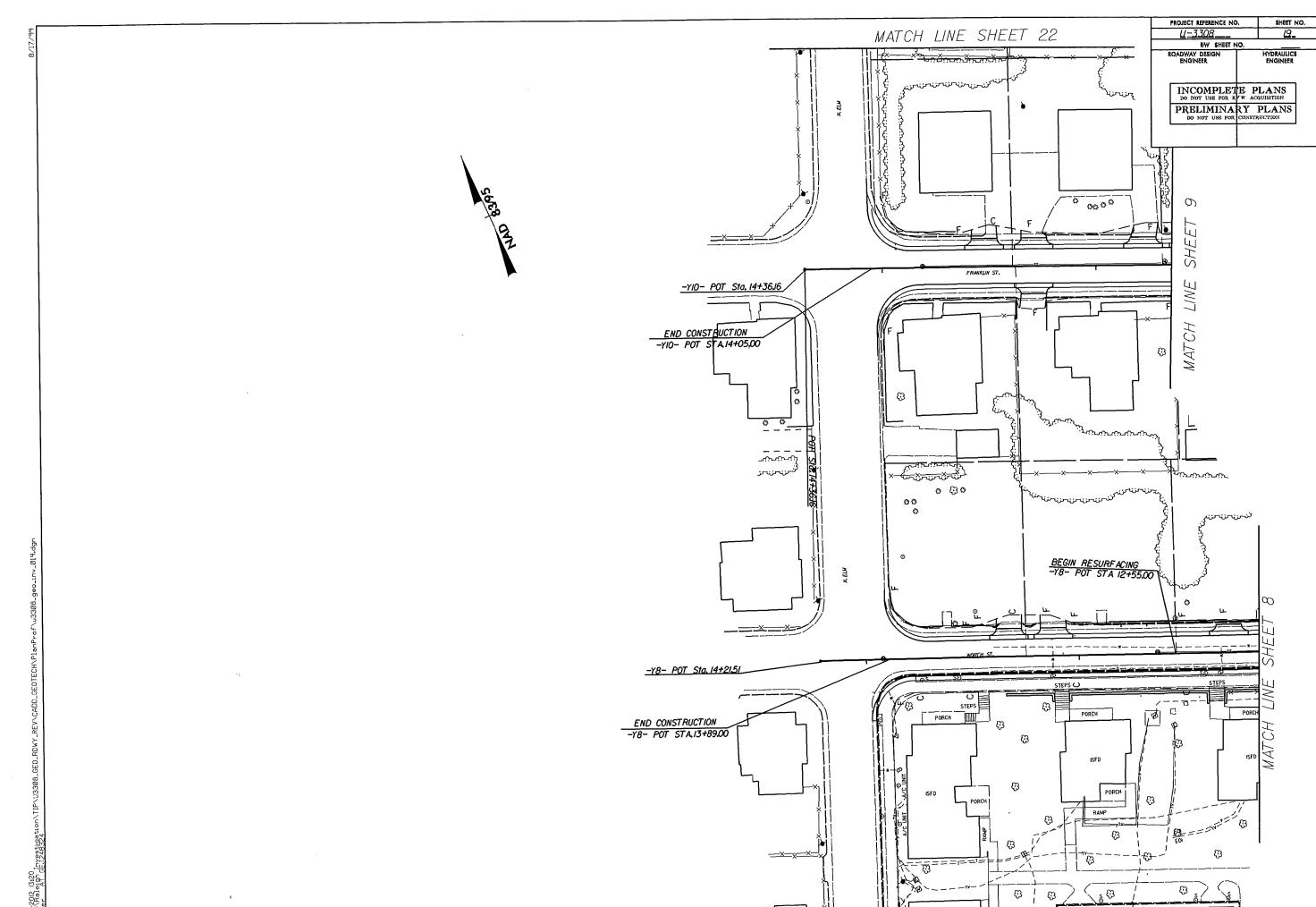


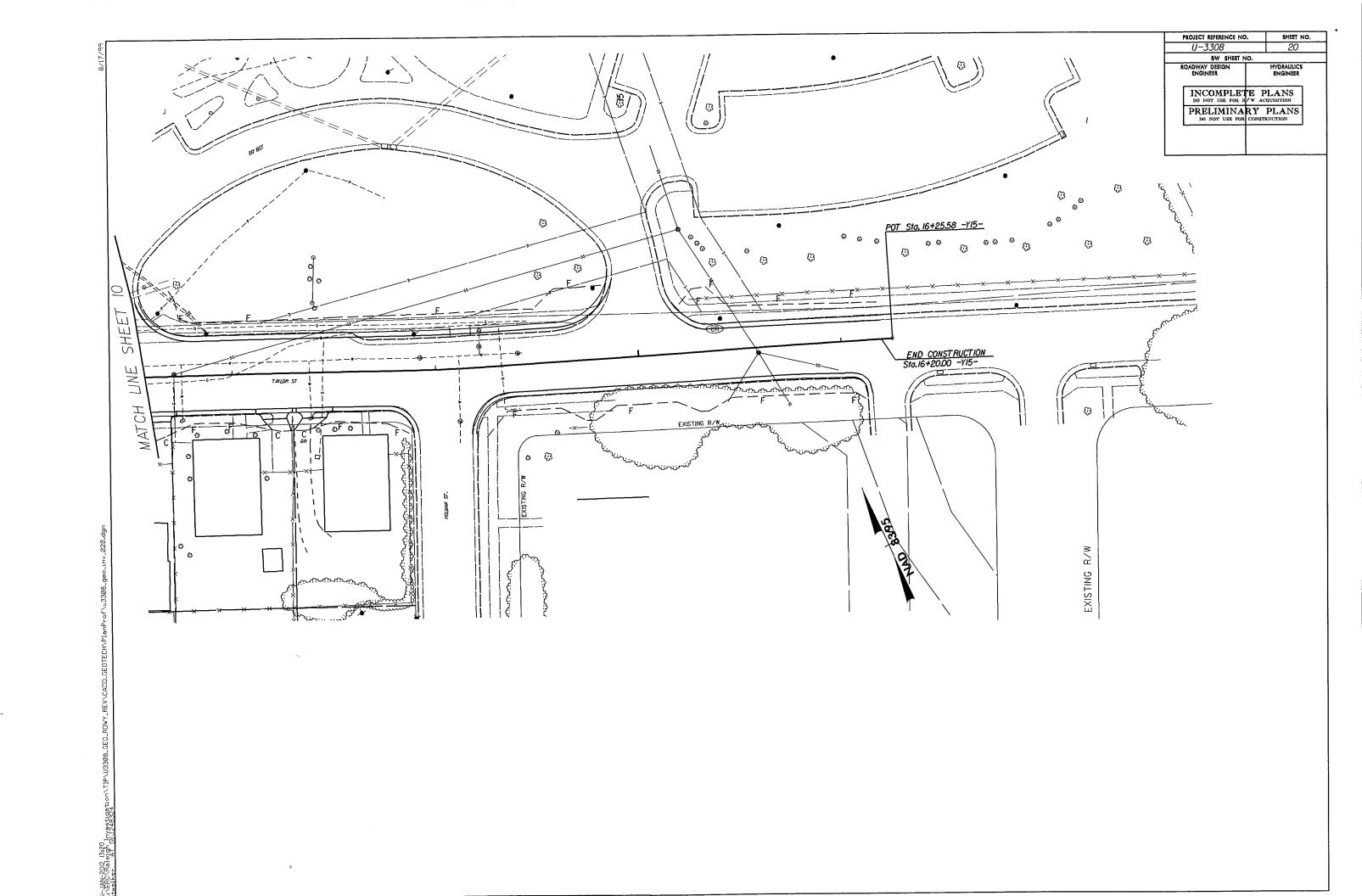


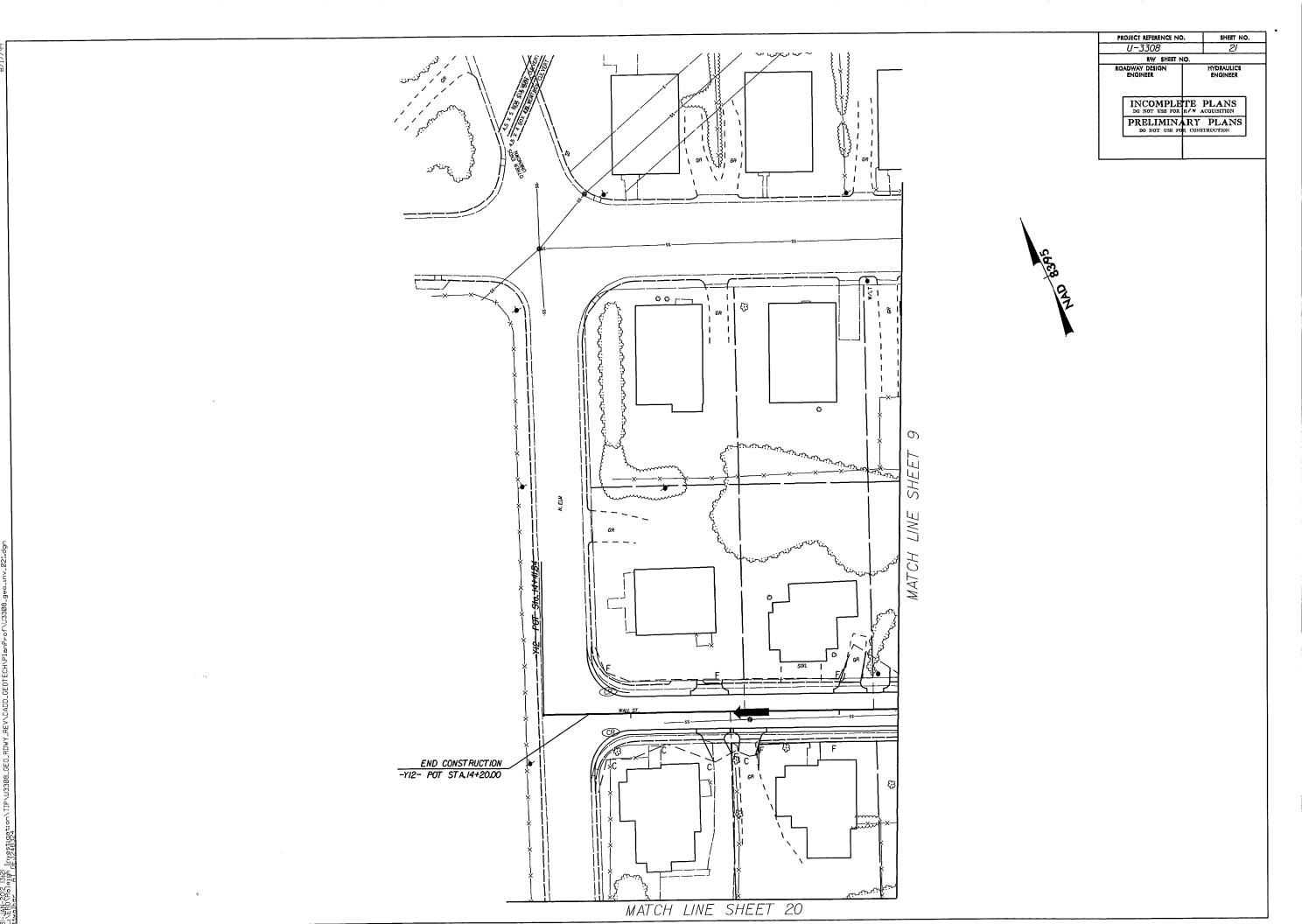


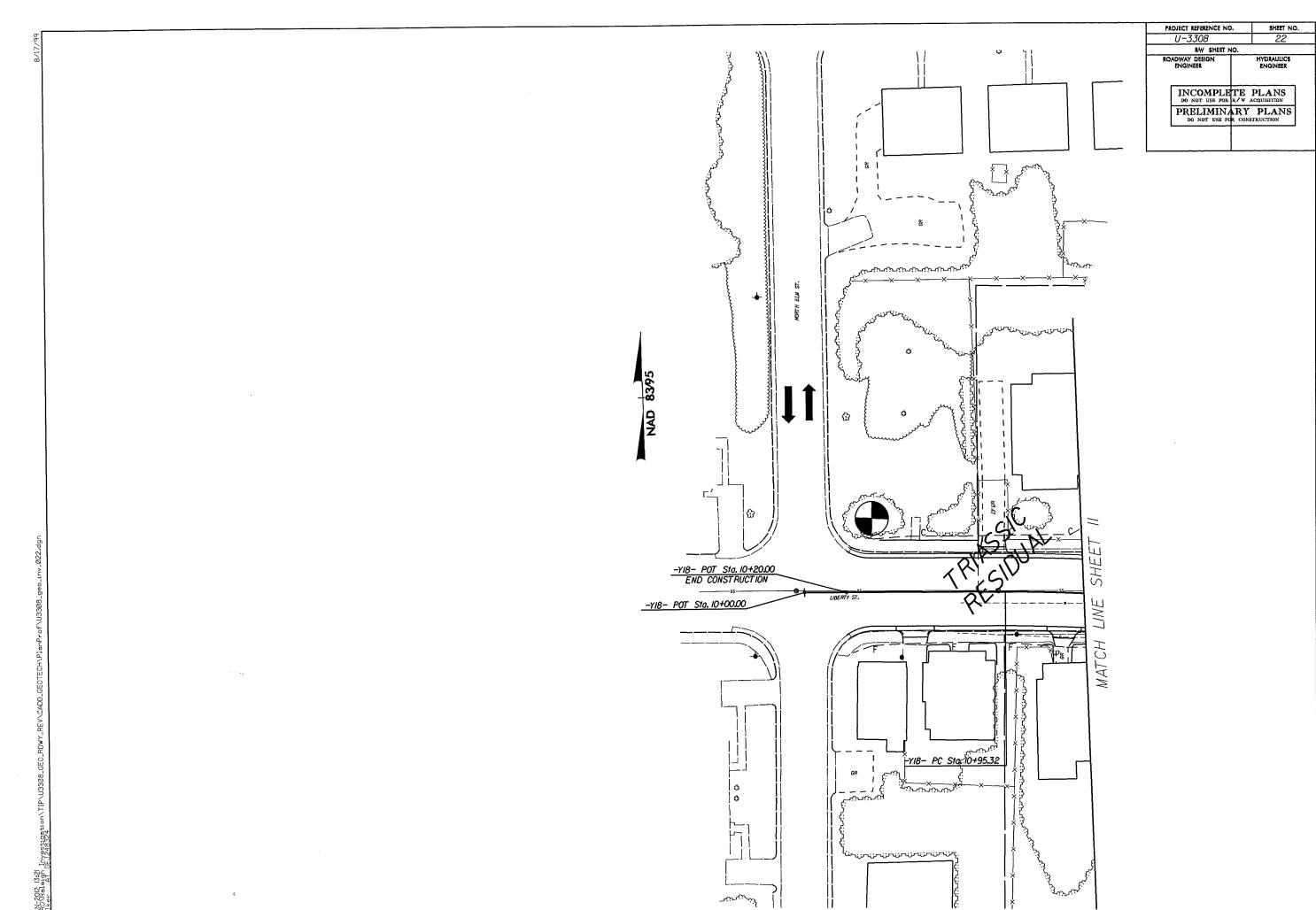


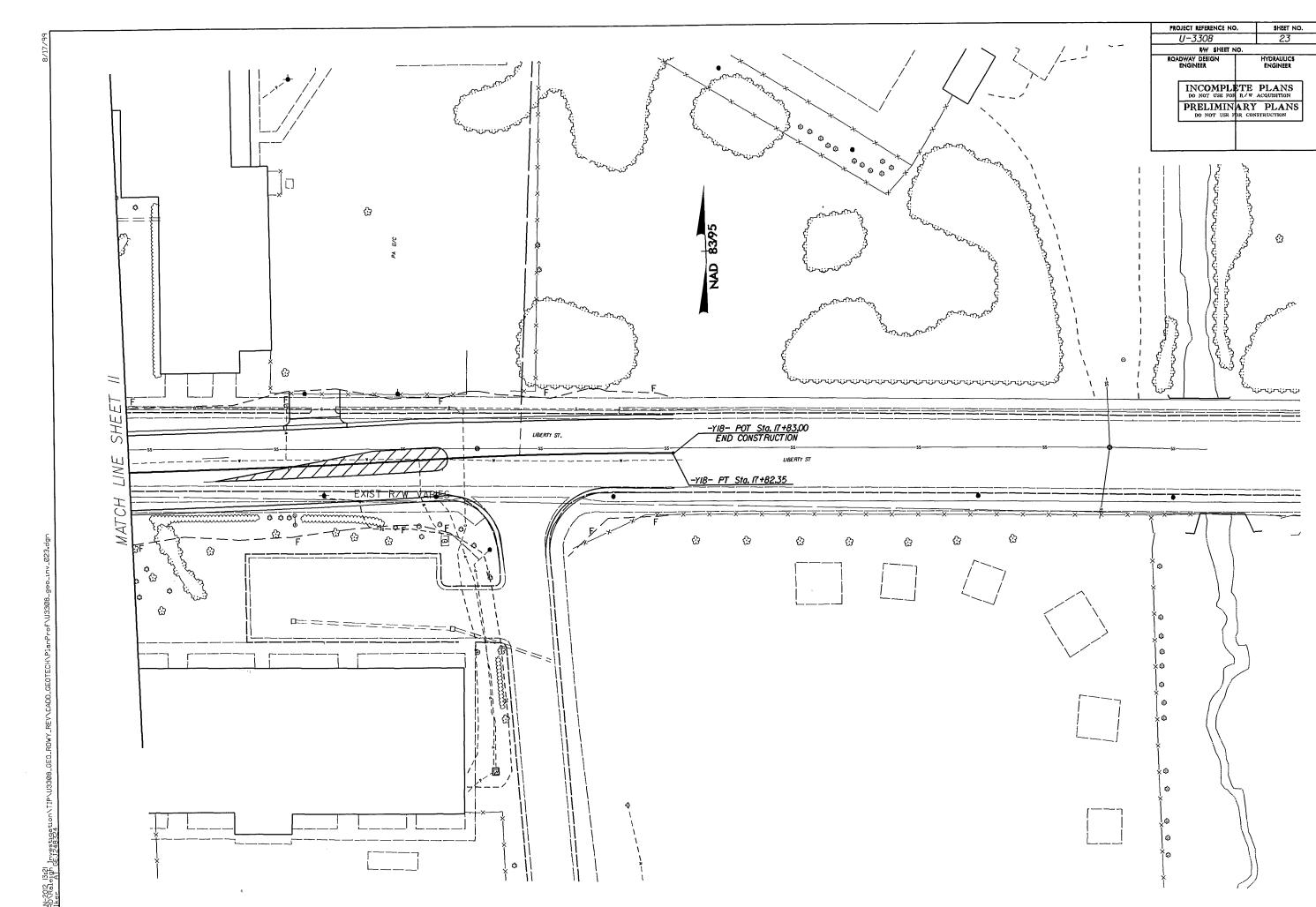


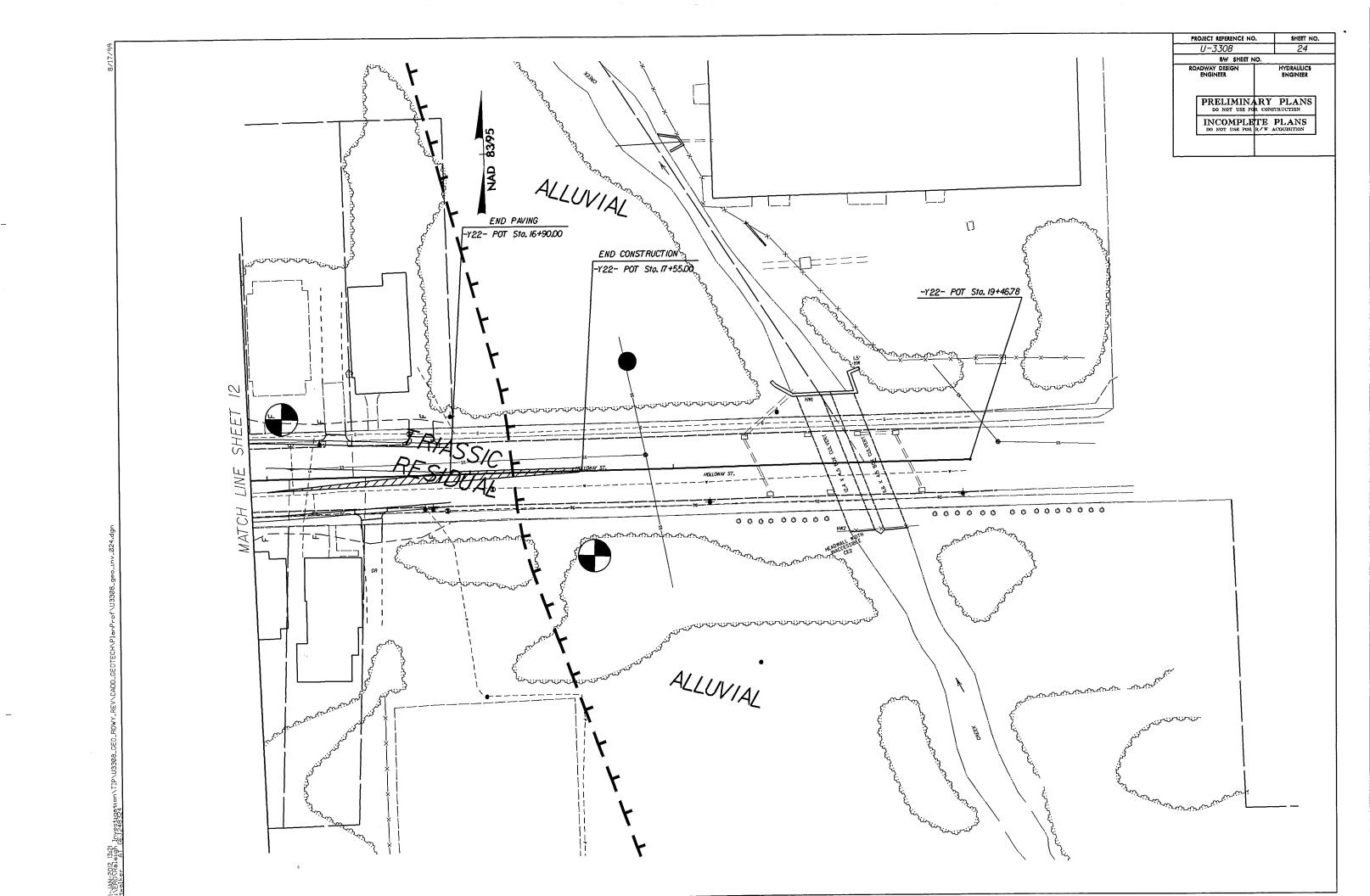


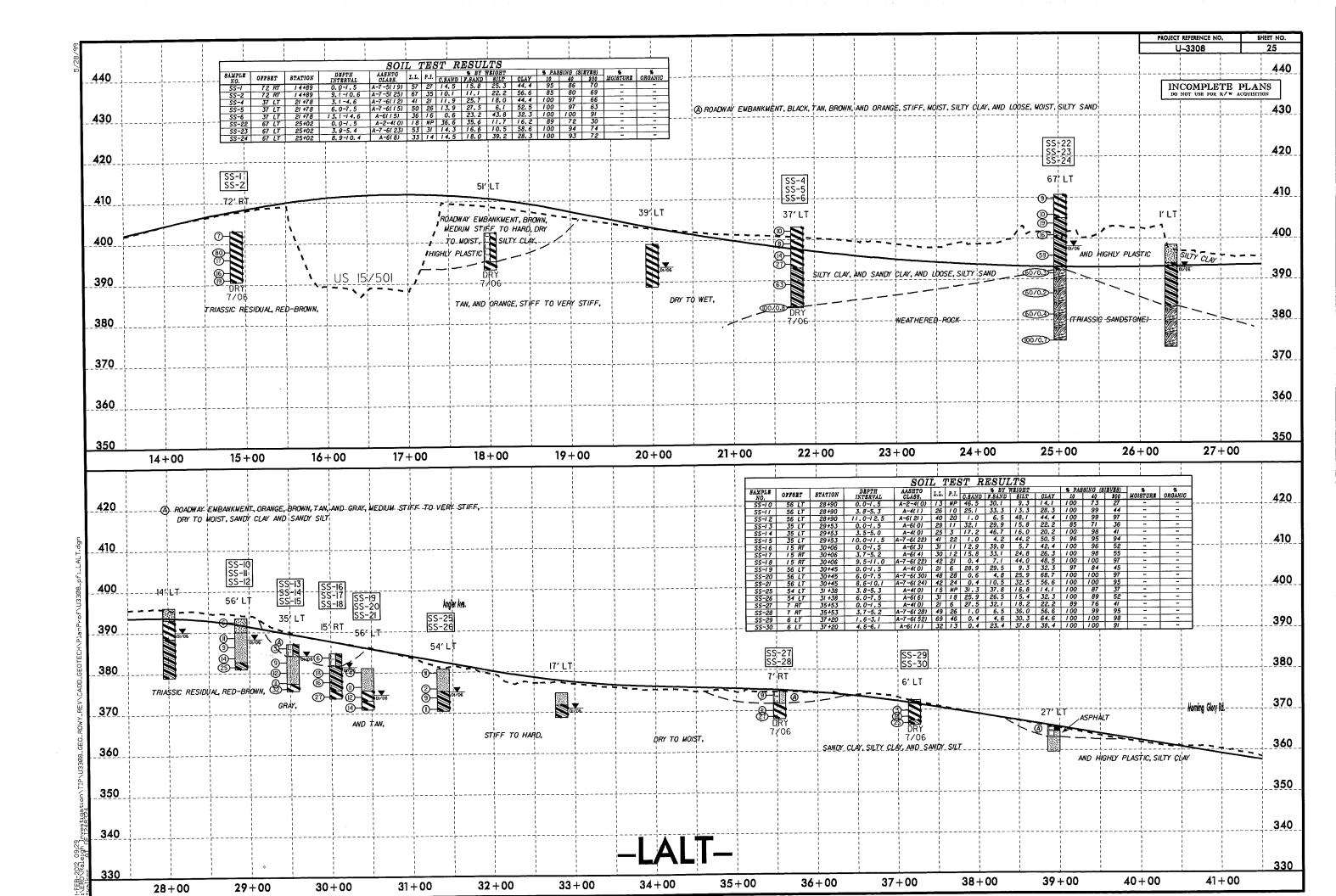


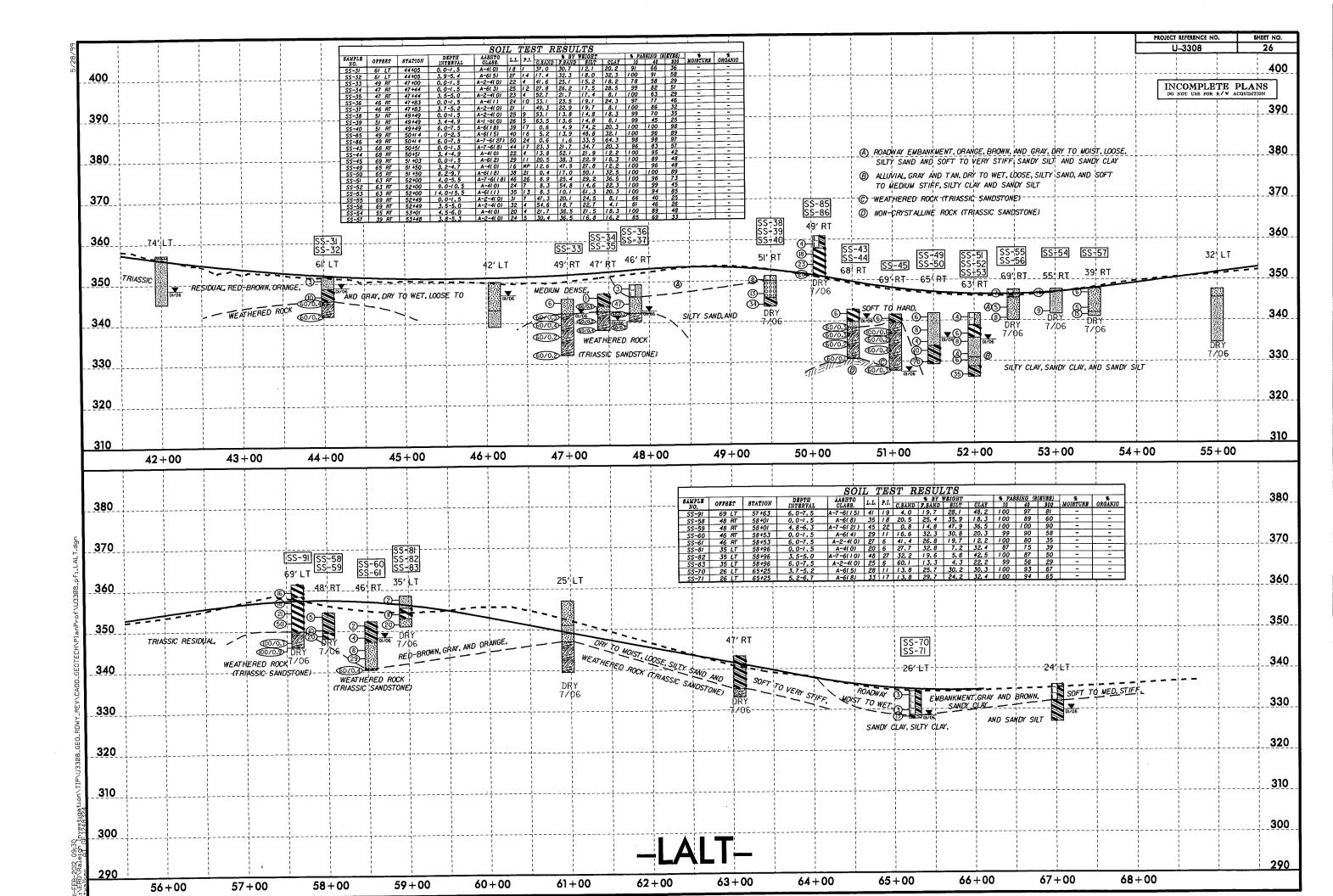


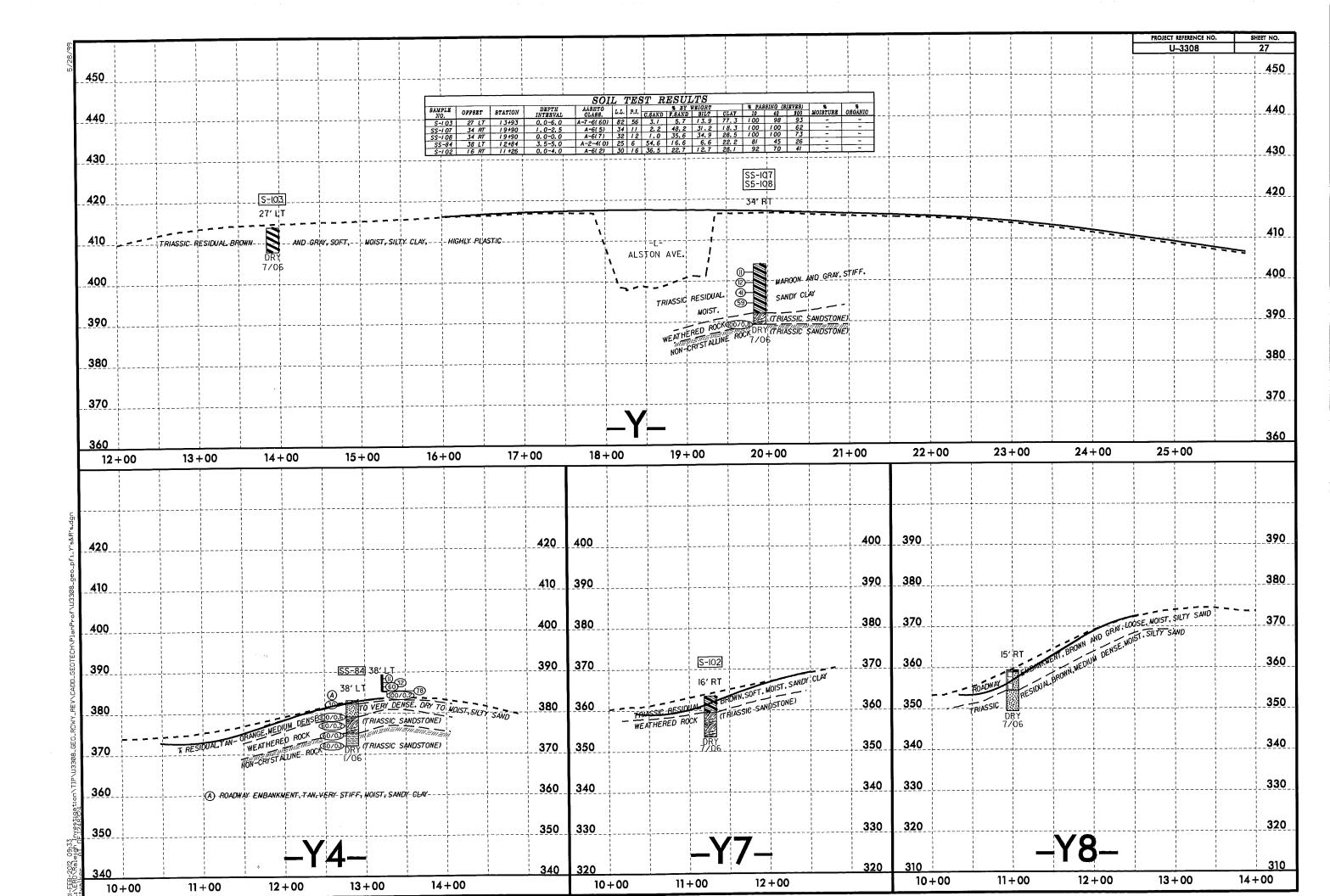


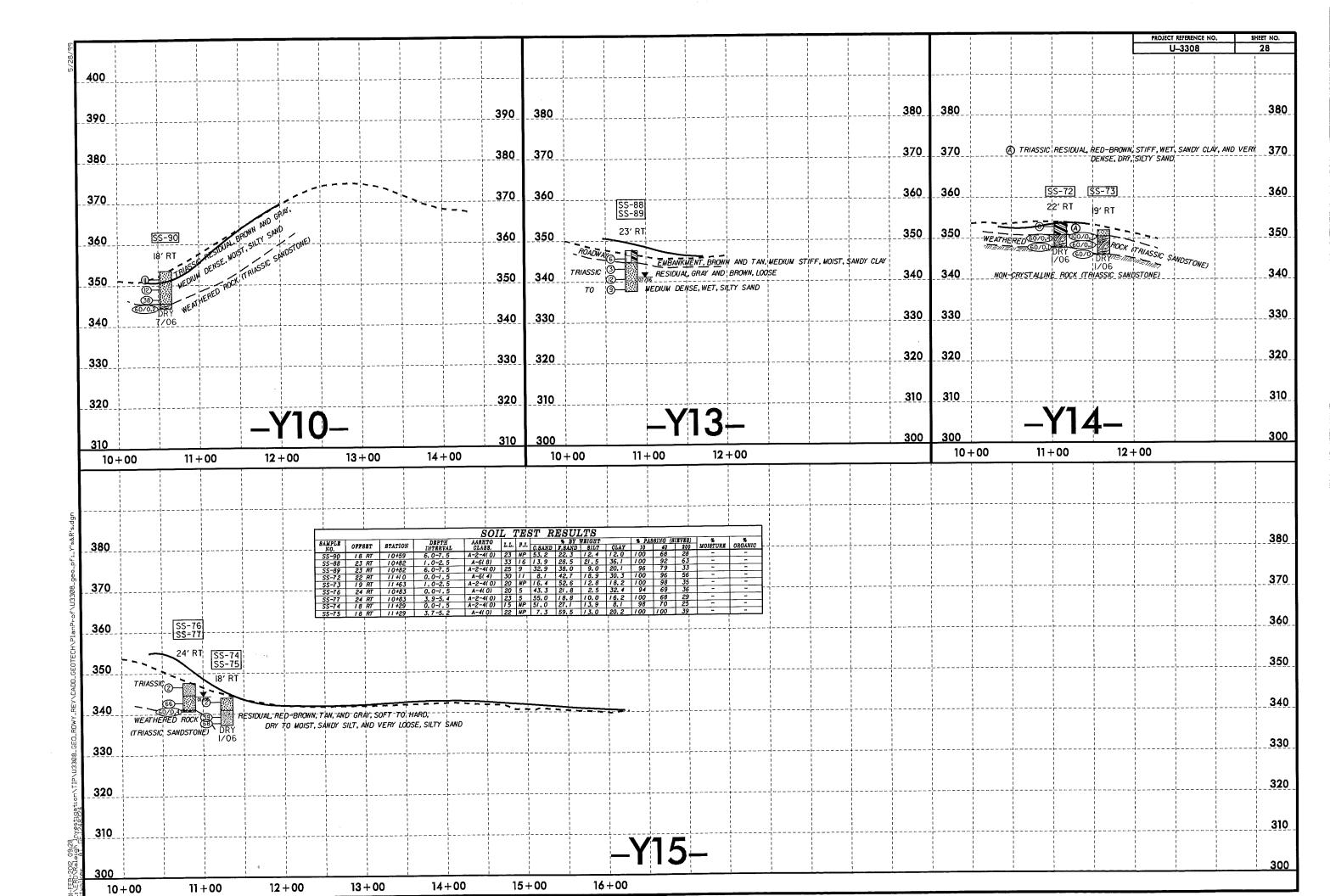


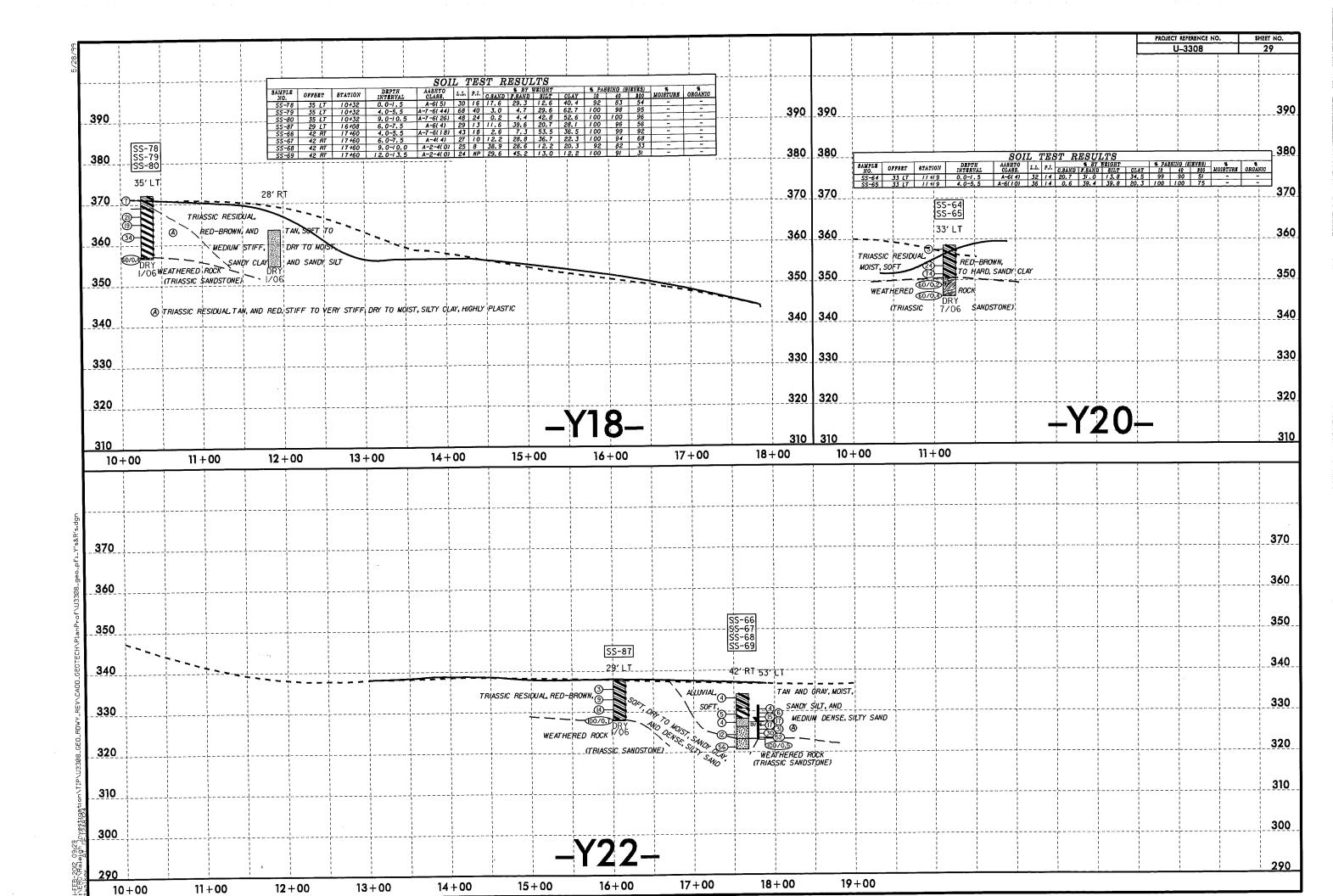




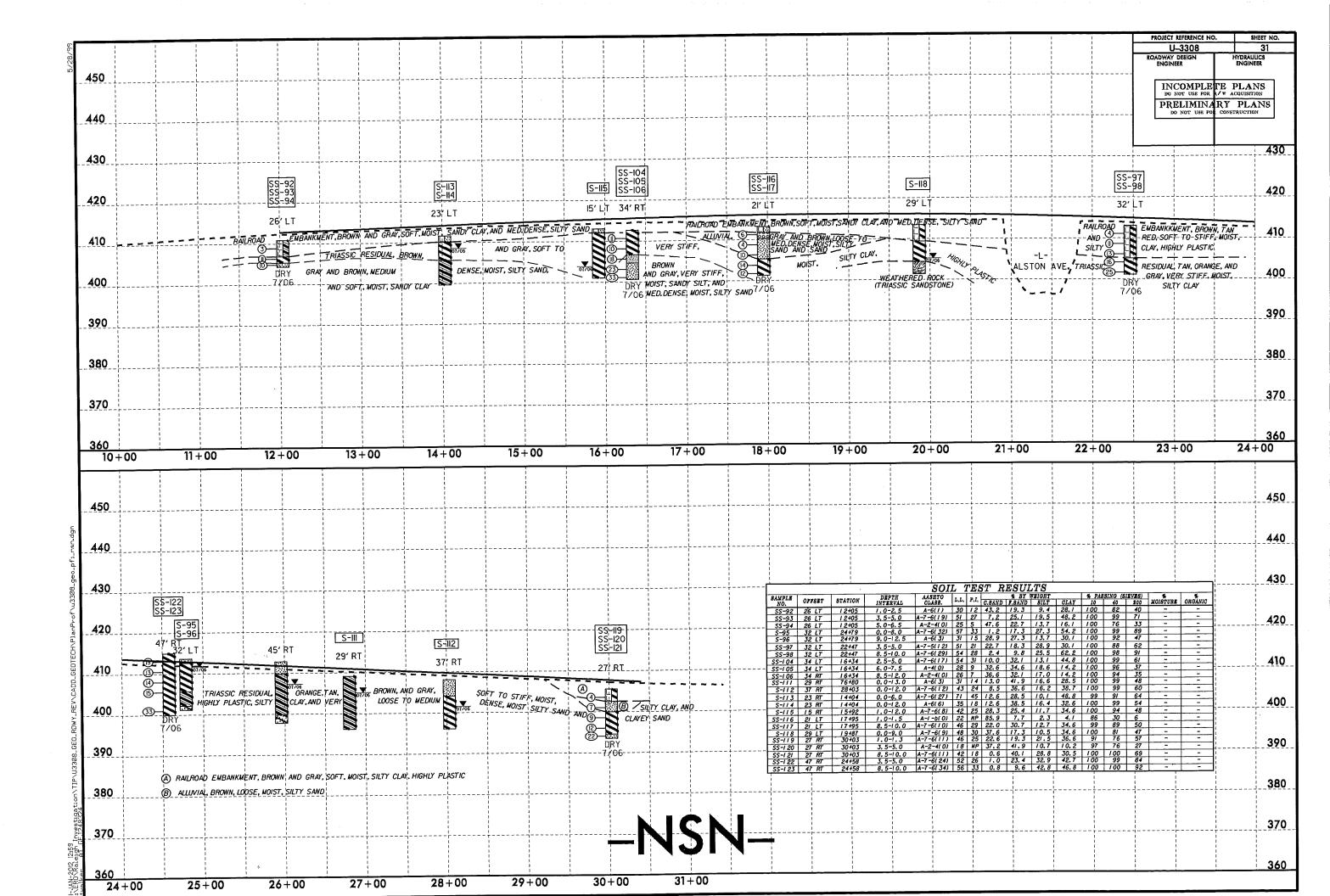


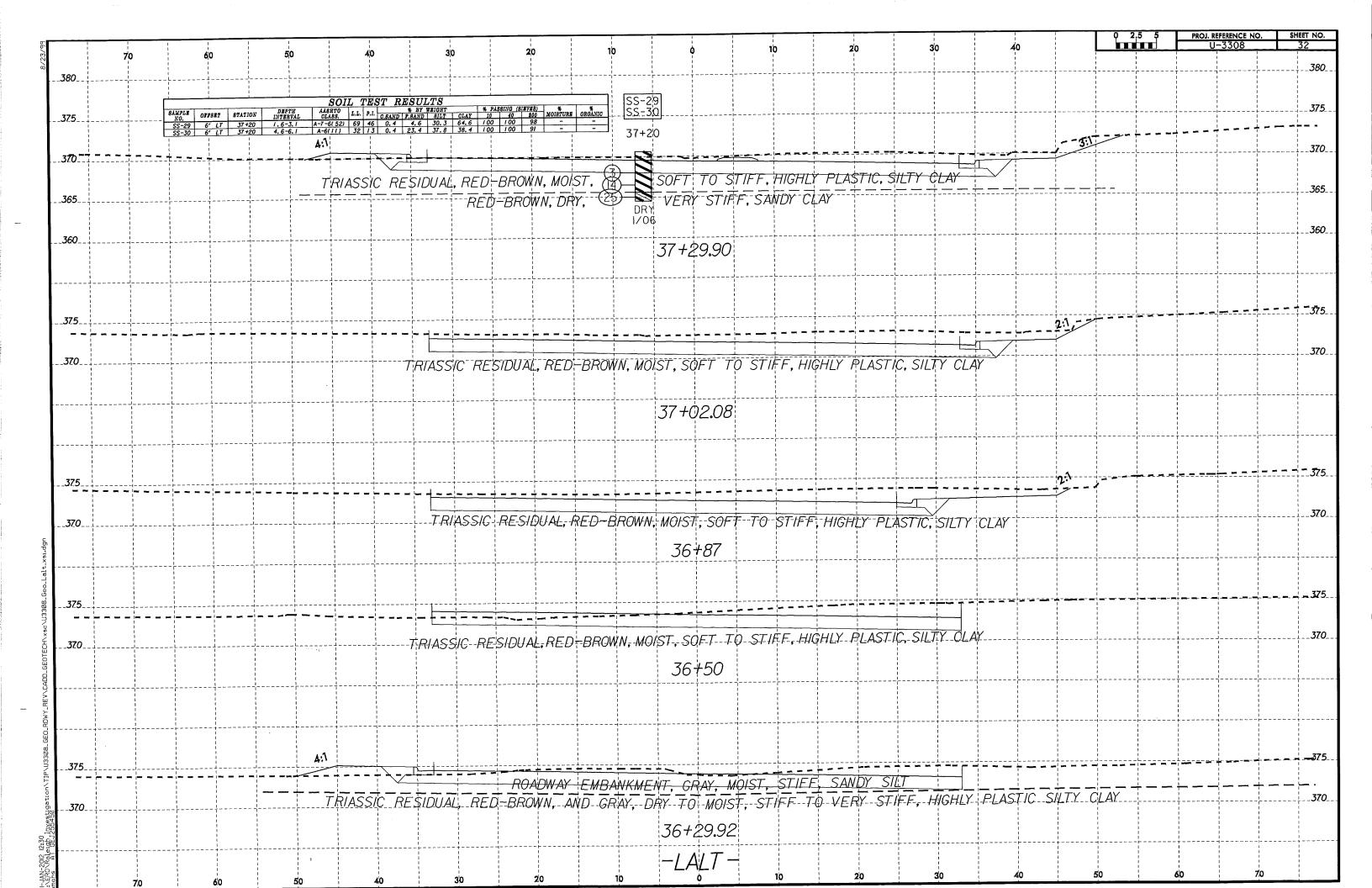


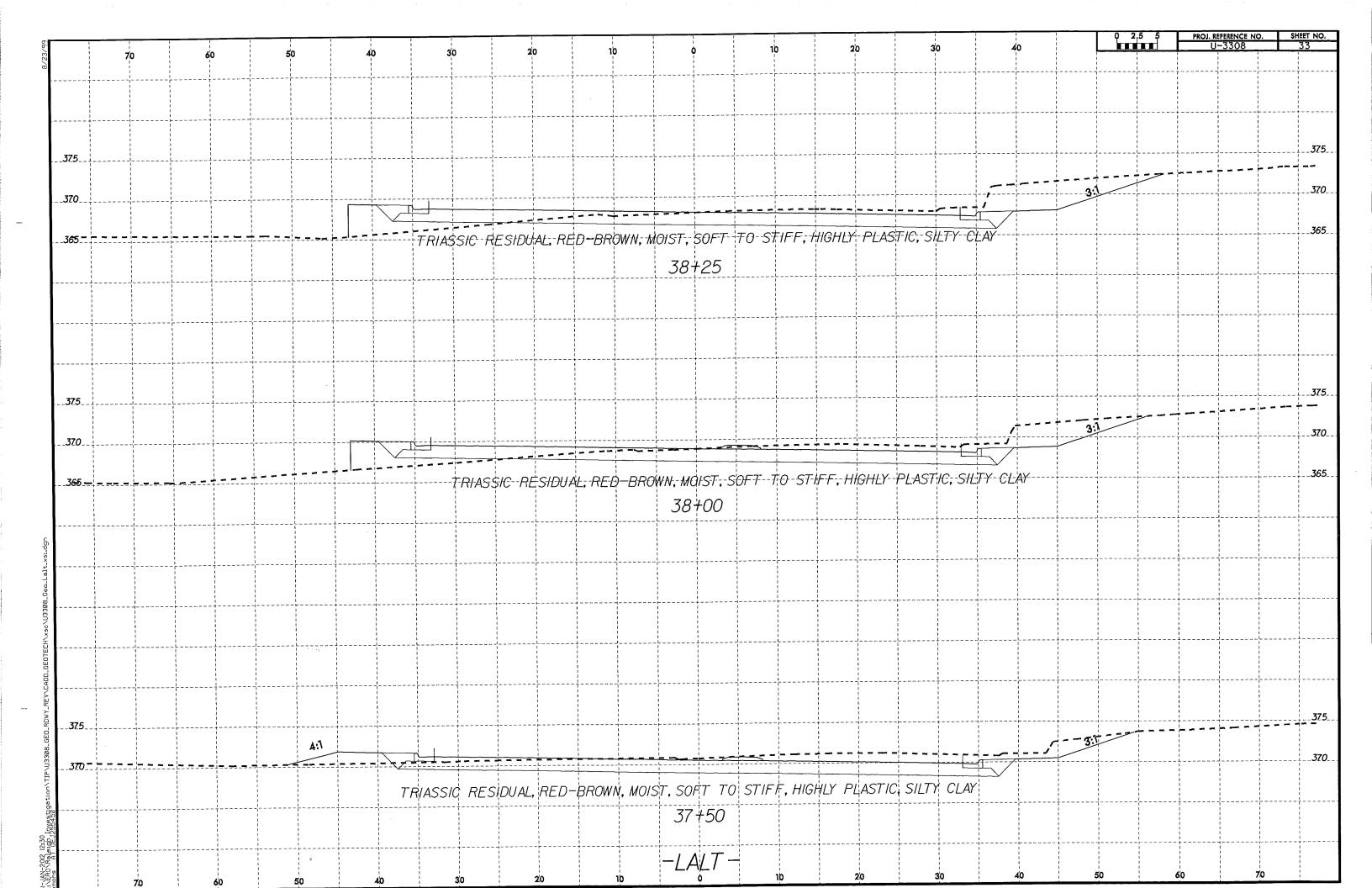


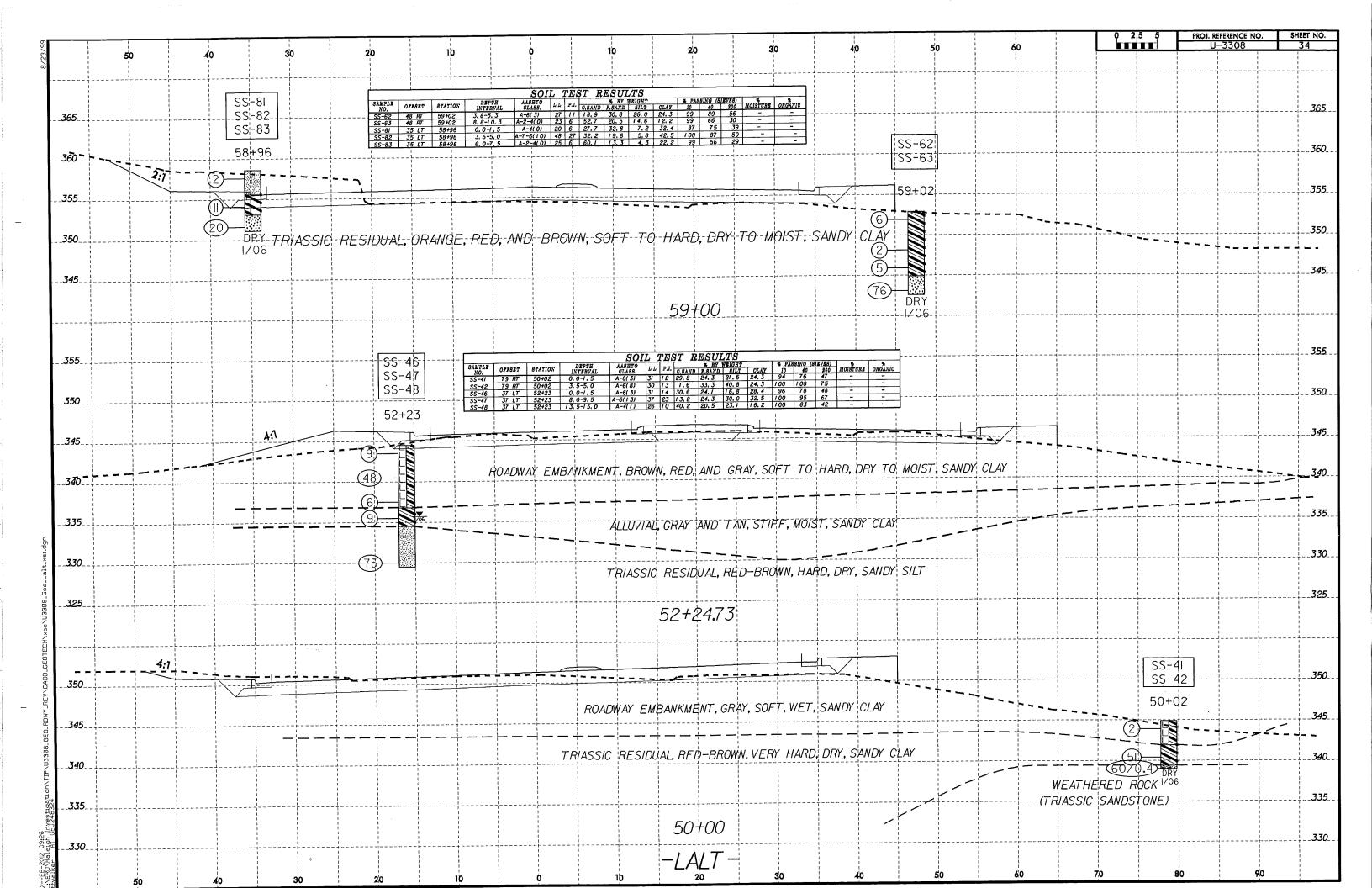


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| STATE | STATE PROJECT REPERENCE NO. | SHEET NO. | TOTAL SHEETS |
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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY DURHAM

PROJECT DESCRIPTION NC 55- WIDENING AND IMPROVEMENTS FROM NC 147 (DURHAM FREEWAY) TO US 70/NC 98 (HOLLOWAY ST.) SITE DESCRIPTION TEMPORARY SHORING ALONG

-LALT-

CONTENTS

SHEET NO. DESCRIPTION TITLE SHEET 2, 2A LEGEND SITE PLAN 3 4, 5 BORE LOGS

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| CAUTION | NOTICE |

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CEMERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IMP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NDICATED IN THE SUBSURFACE OR INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION, THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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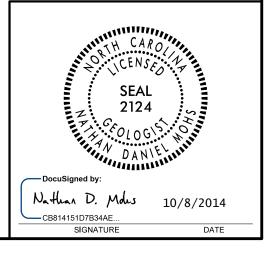
- NOTES:

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| TERRACON |
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| N.D. MOHS |
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| INVESTIGATED BY N.D. MOHS |
| DRAWN BY |
| CHECKED BY N.T. ROBERSON |
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| SUBMITTED BY N.T. ROBERSON |
| SEDTEMBED 2014 |

PERSONNEL



| PROJECT REFERENCE NO. | SHEET NO. |
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| U-3308 | 2 |

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 1 OF 2)

| Secretary and a common of the Protest and the Secretary of the Secretary | (PAGE 1 OF 2) | | | | | | | | | | | | | | | | |
|--|---|---------------|--|---------------|-----------------|---------------------|--|---------------------------|-----------------------|---|--|--|--|--|--|--|--|
| Solid Conference contents and contents performed in the contents of the cont | SOIL DESCRIPTION | | | | | | | | | | | | GRADATION | | | | |
| STEP LEGEN DAY ASSIST OF LAST FLOW STEP STEP STATE LEGEN DAY ASSIST OF LAST FLOW STATE LEGEN DAY ASSIST OF LAST FLOW DAY ASSIS | SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO I 206, ASTM DI586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: | | | | | | | ER AND ITO T IONS (| YIEL 206. GENER | _D LESS ASTM D BALLY I | 5 THAN 100 1586). SOIL NCLUDE TH | <u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. | | | | | |
| ### STILL LEGEND AND AGAPTIC CLASSIFICATION STILL LEGEND AND AGAPTIC CLASSIFICATION | CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, | | | | | | | | E, PL | ASTICIT | Y. ETC. FOR | | | | | | |
| Constant primers Constant pr | VERY STIFF,GRAY,SILTY CLAY,MOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 | | | | | | | | | | | | | | | | |
| The content of the | CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS | | | | | | | | | IALS | | | | | | | |
| COMPRESSIBILITY | | | | | | | (> 35% PASSING *200) | | | | | | | | | | |
| SHOPP SHOP | CLASS. | A-1-a A-1-b | A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7 | | | | | | | | | | | | | | |
| 1 | SYMBOL | | | | 33 | S | | 77.7 | | | | | | | | | |
| 19 | | 30000000000 | ***** | 1020100110201 | 276 | 754.75 | | | | | | SII T- | *********** | HIGHLY COMPRESSIBLE LL > 50 | | | |
| MATCH MATC | | | 51 MN | | | | | | | | | CLAY | | | | | |
| MINISTRATE 19 MINISTRATE | | 15 MX 25 MX 1 | Ø MX | 35 MX 35 MX | 35 MX | 35 MX | 36 MN | 36 MN | 36 MN | 36 MN | | SUILS | | | | | |
| | | | | | | | | | | | con c | WITU | | LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% | | | |
| CONSISTENCY ORGANIC | | _ 6 MX | | | | | | | | | LITTL | .E OR | HIGHI Y | | | | |
| SIGNAL PROPERTY STATE CAPET STATE CAPET STATE CAPET STATE CAPET STATE | | | - | 0 | _ | _ | - | _ | _ | _ | | | ORGANIC | GROUND WATER | | | |
| TEXTURE OR GRAIN SOLE STORMS FOR APPLICATION FOR APPLI | | | FINE | SILTY O | R CLAYE | Y | SIL | TY | CLA | AYEY | | | SUILS | WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING | | | |
| PIGH AFF SARROW S. LL - 30 DENSENDES S. LL - 30 DENSE S. SUBJECT S. S | | GRAVEL, AND | | | | | | | | | | _ | | \blacksquare STATIC WATER LEVEL AFTER $\underline{24}$ HOURS | | | |
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| PRIMARY SOIL TYPE | | r. | I UF H | | | | | | | | > LL - 30 | | | · · | | | |
| MATHERIAL MEDIUM DENSE 10 TO 38 | PRIMARY S | SOIL TYPE | С | | | | | RATION | RESI | | | RESSIVE S | TRENGTH | III HONDANI ENDRIKNEN WEZ DI & DI DINECTION | | | |
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| VERY SOFT CORE BORING SOUNDITION SOU | | | | | | | | | | | | | | | | | |
| SIT-CLAY MEDIUM STIFF 8 TO 15 | | | | | | | | < | 2 | | | — INFERRED SOIL BOUNDARY — CORE BORING ■ SOUNDING ROD | | | | | |
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| U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 8 0U.DER (COBALE (COBAL | | | | | | | | > | 30 | | | | INSTALLATION | | | | |
| DOPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.093 BOULDER (COBBLE (BUT) COBBLE (GRAVEL) COARSE (GRAVEL) COA | | | | | | | | | SI. | ZE | | | | | | | |
| BOULDER (COBBLE (RAVEL SAND) SAND SAND SAND SAND SAND SAND SAND SAND | | | | | | | | | | | | | IXX EXCAVATION IX UNSUITABLE WASTE IXX ACCEPTABLE, BUT NOT TO BE ■ | | | | |
| GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE DESCRIPTION OF TERMS ON JOHN PLASTIC LIMIT SET SALE FIELD MOISTURE DESCRIPTION OF TERMS ON JOHN PLASTIC LIMIT SET SALE FIELD MOISTURE DESCRIPTION OF TERMS AR - AUGER REFUSAL MED MICCA MICCACEOUS WEA WEATHER YOUNG TO MODE ATTER TO A DITTURE TEST OF TOWN MODE AND ADDITION OF TERMS ON JOHN PLASTIC LIMIT SET SALE FIRED SALE SALE SALE SALE SALE SALE SALE SALE | BOULDER COBBLE GRAV | | | | /EL COARSE FINE | | | | | SAND | SILT CLAY | | | UNDERCUT UNDERCUT EMBANKMENT OR BACKFILL GRADABLE ROCK EMBANKMENT OR BACKFILL | | | |
| SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS C CLAY MOD MODERATELY CPT - CONE PENETRATION TEST NP - NON PLASTIC CT - CONE PENETRATION TEST NON PLASTIC CT - CONE PENETRATION TEST NON PLASTIC CT - CONE PENETRATION TEST NON PLASTIC CT - | GRAIN MM | 1 305 | | 75 | | | 1002. | | 3.25 | " 30 | | 0.005 | | | | | |
| SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION OF TOLLATOMETER TEST PHT - PRESSUREMETER TEST PHT - PRESSUREMETER TEST PHT - PRESSUREMETER TEST SAMPLE ABBE OFF - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - SAULK S - SPLIT SPO S - SPLIT SPO S - SPLIT SPO S - SPLIT SPO S - FINE FINE FOSS FOSSILIFEROUS SLI SLICHTLY RS - ROCK FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORN RATIO OPTIMUM MOISTURE SHRINKAGE LIMIT OPTIMUM MOISTURE S - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE OPTIMUM MOISTURE S - SPLIT SPO S - SPLIT SPO S - SPLIT SPO S - SPLIT SPO S - FINE S - FINE FRAGS FRAGMENTS W - MOISTURE CONTENT CBR - CALIFORN ATTAIN OPTIMUM MOISTURE DRILL UNITS: MOWANCING TOOLS: TOME-45C CME-45C CME-45C TOME-45C TOME-550 X 8' HOLLOW AUGERS - B - NON - NON - NEAR DESCRIPTION TUNG,-CARBIDE INSERTS CORS SAMPLE ABBE SAMPLE ABBE SAMPLE ABBE SAMPLE ABBE SAMPLE ABBE SAMPLE ABBE SPLIT SPO S - VERY LOW DESCRIPTION ORG ORGANIC S - SPLIT SID S - SPLIT SPO S - VERY LOW DESCRIPTION ORG ORGANIC ORG. | SIZE IN | | | 3 | | | | | | | | | | | | | |
| OMT - OILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBE DT - DYNAMIC PERTRATION TEST SAP SARROLITIC S - BULK SAP SAROLY SAROL | | | | | | | | LĄT | ION | OF | TERMS | | | CPT - CONE PENETRATION TEST NP - NON PLASTIC $\dot{\gamma}_{ m d}$ - DRY UNIT WEIGHT | | | |
| - SATURATED - USUALLY LIQUID, VERY WET, USUALLY PLASTIC RANGE (PI) PL PLASTIC LIMIT OPTIMUM MOISTURE SHINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE PLASTICITY NON PLASTIC OPLASTICITY PLASTICITY PLASTICITY - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE FRAC FRACTURED, FRACTURES FRAC FRACTURED, FRACTURES TOR - TRICONE REFUSAL RT - RECOMPACT FRACS FRACMENTS W - MOISTURE CONTENT RATIO DRILL UNITS: OFILL UNITS: OFILL UNITS: OFILL UNITS: OFILL UNITS: OFILL UNITS: OFICATION OF CONTINUOUS FLIGHT AUGER CORE SIZE: W - VOID RATIO SS SAND, SAND, SANDY SS - SPLIT SPO TO SIL - | | | | | | | | | GUIDE | FOR | FIELD MOI | STURE DES | SCRIPTION | | | | |
| LL LIQUID LIMIT PLASTIC RANGE (P1) PL PLASTIC LIMIT OPTIMUM MOISTURE SL OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE SU OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - PLASTICITY NON PLASTIC - PLASTICITY INDEX (P1) OPT STRENGTH NON PLASTIC - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE FRAC FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRAC FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRAC FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURED FRACS FRAGMENTS W - MOISTURE CONTENT RATIO - MOIST - (M) SUBJECT - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED FRACS FRAGMENTS W - MOISTURE CONTENT RATIO - MOIST - (M) SUBJECT - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT FRACTURED, FRACTURES TCR - TRICONR REFUSAL RT - RECOMPACT RT - | | | | | | | ED - | | | | | | | e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON | | | |
| PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE FRAC FRACTURED, FRACTURES FRACS FRACTURED, FRACTURES - WOIST - (W) - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES DRYING TO CME-45C - CH2+ BITS - CME-550 X 8* HOLLOW AUGERS - B - B - B - N - N - N - N - N | | LIQUID I | LIMIT | | | | | | | | | WATE | r IABLE | | | | |
| PLASTIC LIMIT OM OPTIMUM MOISTURE SHRINKAGE LIMIT OM OPTIMUM MOISTURE SHRINKAGE LIMIT - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO A | RANGE | | | | - WE | T - (W | | | | | | |) | FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL | | | |
| OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT ORILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS X AUTOMATIC CORE SIZE: CME-55 CME-55 X B* HOLLOW AUGERS DEASTICITY NON PLASTIC ORIL UNITS: ADVANCING TOOLS: HAMMER TYPE: X AUTOMATIC X AUTOMATIC CORE SIZE: X B* HOLLOW AUGERS DEASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC OF VERY LOW OF TUMG,-CARBIDE INSERTS | (PI) _{PL} L | + PLASTIC | LIM | ш | | | HITHIN OFTENDED MOTOTORE | | | | | | | | | | |
| CME-45C | | | | - MOI | IST - | - (M) SOLID; AT OR | | | | R NEAR OF | PTIMUM MO | ISTURE | DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: | | | | |
| CME-55 X 8* HOLLOW AUGERS -8 -8 -8 -8 -8 -8 -8 - | | | | | | Y - (D | | | | | |) | 6' CONTINUOUS FLIGHT AUGER CORE CIZE | | | | |
| CME-550 X HARD FACED FINGER BITS -N | | | | | | | | | | | | CME-55 | | | | | |
| NON PLASTIC 0-5 VERY LOW | | | | | | | DF | CME-550 | | | | | | | | | |
| | NON PLASTIC 0-5 VERY LOW | | | | <u>51</u> | TUNGCARBIDE INSERTS | | | | | | | | | | | |
| MODERATELY PLASTIC 16-25 MEDIUM CASING W/ ADVANCER PRIST HOLF DIV | | | | | | MEDIUM | CASING W/ ADVANCER HAND TOULS: POST HOLE DIGGER | | | | | | | | | | |
| HIGH PLASTIC 26 OR MORE HIGH PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER | | | | | | | | | | | | PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER | | | | | |
| COLOR TRICONE - TUNGCARB. SOUNDING ROD | LULUK | | | | | | | | | | | | X D-50 | | | | |
| DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). | DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. | | | | | | | | | | | CORE BIT VANE SHEAR TEST | | | | | |
| | MODIFIERS SOUR AS EIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE AFFERMANCE. | | | | | | | nng | UJEL | יט טי | | | | | | | |

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS (PAGE 2 OF 2)

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN D.IF FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.

ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

| WEATHERED ROCK (WR) | |
|---|--|
| CRYSTALLINE ROCK (CR) | |
| NON-CRYSTALLINE ROCK (NCR) | |
| COASTAL PLAIN SEDIMENTARY ROCK (CP) | |

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES 3 $100~\mathrm{BLOWS}$ PER FOOT IF TESTED.

FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT

FINE TO COARSE GRAIN IONEQUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.

FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.

COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

ERESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER

VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS II OF A CRYSTALLINE NATURE. (V SLI.)

ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO SLIGHT

1 INCH, OPEN JOINTS MAY CONTAIN CLAY, IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN MODERATE

GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH MODERATELY

SEVERE (MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK, IF TESTED, WOULD YIELD SPT REFUSAL ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT SEVERE

REDUCED IN STRENOTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. (SEV.) IF TESTED. WOULD YIELD SPT N VALUES > 100 BPF

ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VERY SEVERE (V SEV.) VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF COMPLETE

ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS

ROCK HARDNESS

VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.

CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED HARD TO DETACH HAND SPECIMEN.

MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE

EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.

CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. MEDILIM CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE HARD POINT OF A GEOLOGIST'S PICK.

CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. SOFT

VERY CAN BE CARVED WITH KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY SOF1

FRACTURE SPACING

| TERM | SPACING | TERM | THICKNESS |
|----------------|---------------------|---------------------|-------------------|
| VERY WIDE | MORE THAN 10 FEET | VERY THICKLY BEDDED | 4 FEET |
| WIDE | 3 TO 10 FEET | THICKLY BEDDED | 1.5 - 4 FEET |
| MODERATELY CLO | SE 1 TO 3 FEET | THINLY BEDDED | 0.16 - 1.5 FEET |
| CLOSE | 0.16 TO 1 FOOT | VERY THINLY BEDDED | 0.03 - 0.16 FEET |
| VERY CLOSE | LESS THAN 0.16 FEET | THICKLY LAMINATED | 0.008 - 0.03 FEET |
| | | THINLY LAMINATED | < 0.008 FEET |

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

RUBBING WITH FINGER FREES NUMEROUS GRAINS. GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.

GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. MODERATELY INDURATED

GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; INDURATED DIFFICULT TO BREAK WITH HAMMER.

SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: EXTREMELY INDURATED

SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

 $\underline{\sf DIP}$ DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

 $\underline{\mathsf{FLOAT}}$ - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM, FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

 $\underline{\mathsf{LEOGE}}$ - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVINIS STRATIM AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

<u>SAPROLITE (SAP.)</u> - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - I - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

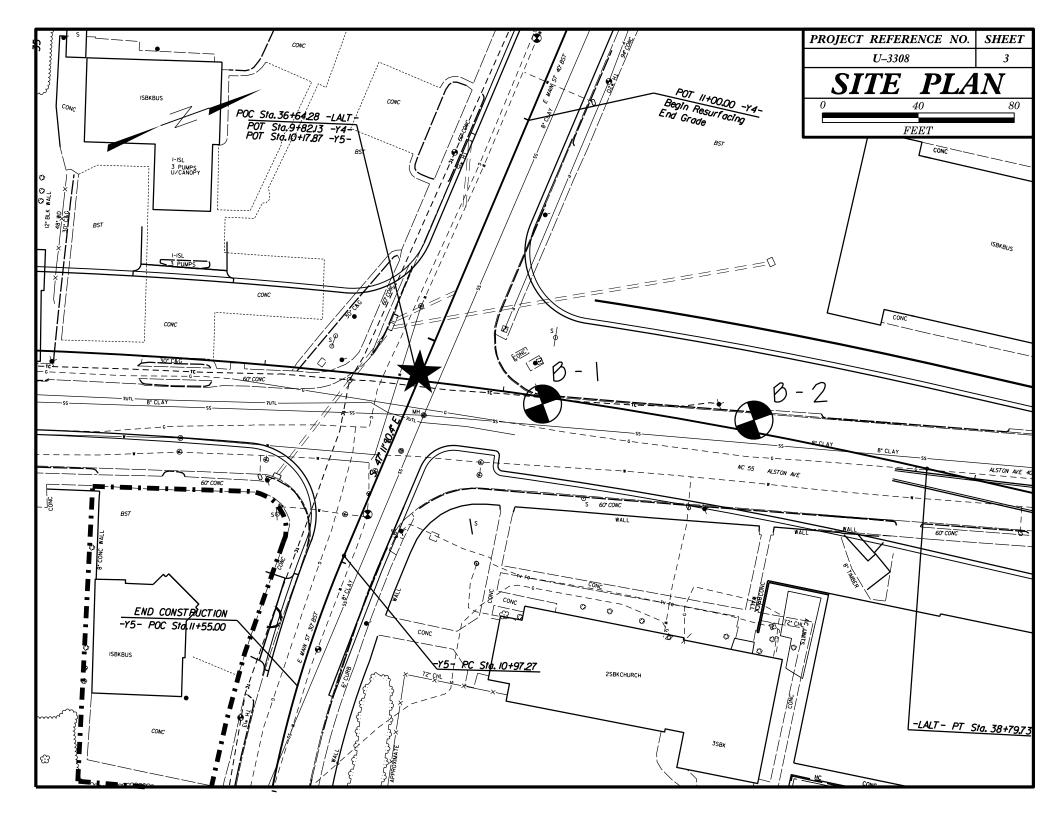
TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

ELEVATION: FEET

NOTES:

DATE: 8-15-14



10/8/14

ICDOT BORE SINGLE U3308

