



	PROJECT	NOU-33 DURHAM	08 ₋ COUNTY
	STATION:	POT 24+09.63 POT 21+11.43	3 -LALT- -NSN-
DocuSigned by: Paul Kelly, Jr	DEPARTM	STATE OF NORTH CAROLINA ENT OF TRANSPC RALEIGH	ORTATION
26ADE85DEC6A498	GENE Fount	ERAL DRAV	VING AYOUT
SEAL 19765 E. KEL 3/31/2015	FOR OVER N BETWEEN	BRIDGE ON I NC 55 (ALSTO NC 147 & AN	NSRR N AVE.) GIER AVE.
Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	NO. BY: DA 1 2	REVISIONS TE: NO. BY: DA 3 4	TE: SHEET NO. S3-2 TOTAL SHEETS 44





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	PROJEC	T NO. DUR DN: <u>POT</u>	U CHAM 24+09	-3308 -3308 C0 9.63 -L	UNTY
TONC	SHEET 3 C	PUI IF 4 RTMENT	E OF NORTH CAR OF TRAI	M.P. M.P. OLINA NSPORTA	H-56.10 TION
STON2.	G	ENER/ DCAT	AL DF	RAWIN Sketc	IG CH
	F OVE BETWE	OR BR R NC S EN NC	IDGE 0 55 (ALS 147 &	N NSRI STON A ANGIE	R AVE. Sheet NO.
/ Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	NO. BY: 1 2	DATE:	NO. ВҮ: З 4	DATE:	S3-3 TOTAL SHEETS 44

	TOTAL BILL OF MATERIALS																			
	TEMPORARY RAILROAD SHORING	REMOVAL OF EXISTING STRUCTURES	4'-6"DIA. DRILLED PIER IN SOIL (LFD)	4'-0"DIA. DRILLED PIER IN SOIL (LFD)	4'-6"DIA. DRILLED PIER NOT IN SOIL (LFD)	4'-0"DIA. DRILLED PIER NOT IN SOIL (LFD)	SID INSPECTIONS (LFD)	SPT TESTING (LFD)	CSL TESTING (LFD)	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	CONCRETE	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	APPROX. 755,000 LBS. STRUCTURAL STEEL	PAINTING OF STRUCTURAL STEEL	HP12X53 STEEL PILES (LFD)	1'-0" × 2'-0" CONCRETE BARRIER RAIL	4″ SLOPE PROTECTION	STRUCTURE DRAINAGE SYSTEM
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	EACH	LUMP SUM	SQ.FT.	CU.YDS.	LBS.	LBS.	LUMP SUM	LUMP SUM	NO. LIN. FT.	LIN.FT.	SQ.YDS.	LUMP SUM
SUPERSTRUCTURE											8,278				LUMP SUM	LUMP SUM		328.3		LUMP SUM
ABUTMENT 1				86.5		35		5	5			83.0	30,164	5,549					233	
PIER 1			18.5		48				5			75.8	22,468	6,094						
PIER 2			70.5		45			5	5			75.5	26,633	8,386						
PIER 3			57.5		40				5			75.2	25,103	7,494						
ABUTMENT 2												90.8	14,367				28 840		233	
TOTAL	LUMP SUM	LUMP SUM	146.5	86.5	133	35	20	10	20	LUMP SUM	8,278	400.3	118,735	27,523	LUMP SUM	LUMP SUM	28 840	328.3	466	LUMP SUM

1. ASSUMED LIVE LOAD: AREMA E80 OR ALTERNATE LIVE LOAD.

2. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

- 3. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF AREMA'S "MANUAL FOR RAILWAY ENGINEERING, VOL. 2, STRUCTURES", AND NORFOLK SOUTHERN CORPORATION'S "GUIDELINES FOR DESIGN OF HIGHWAY SEPARATION STRUCTURES UNDER RAILROAD (UNDERPASS GRADE SEPARATION)".
- 4. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC ZONE 1.
- 5. REINFORCING STEEL SHALL BE ASTM 615, GRADE 60. ALL DIMENSIONS RELATING TO BAR SPACING ARE TO BAR CENTERS UNLESS NOTED OTHERWISE. FABRICATION IS TO BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE", ACI 315-80. ALL REINFORCING IN THE CONCRETE DECK SLAB AND PARAPETS SHALL BE EPOXY COATED.
- 6. EXPANSION JOINT MATERIAL SHALL BE EITHER RUBBER OR CORK CONFORMING WITH AASHTO SPECIFICATIONS M-153-84 EXCEPT AS SHOWN ON THE PLANS OR IN THE SPECIAL PROVISIONS. CELLULAR AND BULB TYPE WATERSTOPS AND RUBBER JOINT COMPOUNDS SHALL BE SHOWN ON THE PLANS AND IN THE SPECIAL PROVISIONS.
- 7. STRUCTURE DRAINAGE SYSTEM: METAL DRAINS BEHIND ABUTMENTS AND DUCTILE IRON PIPE COLLECTOR SYSTEM SHALL BE AS SHOWN ON THE PLANS AND OUTLINED IN THE SPECIAL PROVISIONS. DETAILS OF THE DRAINAGE SYSTEM SHALL BE SUBMITTED TO THE CHIEF ENGINEER, BRIDGES AND STRUCTURES, NORFOLK SOUTHERN CORPORATION, ATLANTA, GA FOR APPROVAL.
- 8. DAMPPROOFING: PIER COLUMNS UP TO THE GROUND LINE, BACK OF BACKWALLS, ABUTMENT SEATS AND BACK OF WINGS SHALL BE DAMPPROOFED, IN ACCORDANCE WITH AREMA CHAPTER 8, PART 29. SEE SPECIAL PROVISIONS FOR NS SPECIFICATIONS FOR CAST-IN-PLACE CONCRETE.
- 9. WATERPROOFING: ALL CONSTRUCTION JOINTS AND ANY SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL, SHALL BE WATERPROOFED WITH A TWO PART WATERPROOFING SYSTEM, CONSISTING OF A MEMBRANE LAYER AND A PROTECTIVE COURSE. STRIPS OF WATERPROOFING NO LESS THAN 2 FEET WIDE SHALL BE PLACED SYMMETRICALLY OVER JOINTS. THE ENTIRE BRIDGE DECK SHALL BE WATERPROOFED WITH A TWO PART WATERPROOFING SYSTEM. CONSISTING OF A MEMBRANE LAYER AND A 1 INCH THICK ASPHALT PLANKING OR OTHER RAILWAY APPROVED PROTECTION MATERIAL.ALL WATERPROOFING MATERIALS SHALL CONFORM TO THE RECOMMENDED PRACTICES IN THE AREMA MANUAL OF RAILWAY ENGINEERING CHAPTER 8. PART 29.
- 10. FOR WATERPROOFING, SEE SPECIAL PROVISIONS.
- 11. FOR WATERSTOPS, SEE SPECIAL PROVISIONS.
- 12. THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.
- 13. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY 2012. NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS), EXCEPT AS NOTED HEREIN, ELSEWHERE ON PLANS. OR IN THE SPECIAL PROVISIONS. STRUCTURAL STEEL IN ACCORDANCE WITH CURRENT AREMA SPECIFICATIONS AND NORFOLK SOUTHERN'S "SPECIFICATIONS FOR STEEL".

DRAWN BY :	NMC	DATE :	06-14
CHECKED BY :	DJM	DATE :	06-14

GENERAL NOTES

- RAIL OPERATIONS.
- SPECIAL PROVISIONS.
- 18. FOR CONDUITS IN PARAPETS. SEE SPECIAL PROVISIONS.
- 19. FOR PORTLAND CEMENT. SEE SPECIAL PROVISIONS.

- 27. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- 28. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- CAST-IN-PLACE CONCRETE" SPECIAL PROVISION.
- MEASURES INSTALLED AND ACCEPTED.
- STRUCTURES".
- STEEL".
- 47. AFTER SERVING AS TEMPORARY STRUCTURES, THE THREE EXISTING STRUCTURES CONSISTING OF 3 SPANS WITH TWO STEEL BEAMS AND 34. FOR ELASTOMERIC FLASHING, SEE SPECIAL PROVISIONS. TIMBER RAIL TIES AND A BUILT-UP STEEL LATTICE FRAME SUBSTRUCTURE AND LOCATED APPROXIMATELY AT THE LOCATION OF 35. FOR PROTECTION OF RAILWAY INTERESTS, SEE SPECIAL PROVISIONS. THE NEW BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL 36. FOR RAILROAD ROADBED, SEE RAILROAD ROADBED SPECIAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF PROVISIONS. THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.
- 37. FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- 38. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

14. ALL CONCRETE USED IN THE SUPERSTRUCTURE (DECK AND CURBS) SHALL BE MIN. 5.000 PSI CONCRETE AND ALL CONCRETE USED IN THE SUBSTRUCTURE SHALL BE MIN. 4,000 PSI CONCRETE, WITH NO.57 OR 67 COARSE AGGREGATE AND SHALL BE AIR-ENTRAINED. MINIMUM CEMENT PER CUBIC YARD OF CONCRETE SHALL BE 6.5 BAGS. NO SUBSTITUTION OF FLY ASH, BLAST FURNACE SLAG OR OTHER MATERIAL WILL BE PERMITTED IN MEETING THIS MINIMUM CEMENT REQUIREMENT. CHAMFER ALL EXPOSED EDGES AND CORNERS $\frac{3}{4}$ "EXCEPT AS NOTED ON THE PLANS. THE USE OF GROUND GRANULATED BLAST FURNACE SLAG IS NOT PERMITTED IN THIS STRUCTURE.

15. CONTROL OF WORK: ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILWAY STRUCTURE SHALL BE PERFORMED SATISFACTORY TO THE ENGINEER AND/OR NORFOLK SOUTHERN RAILWAY COMPANY.ALL METHODS OF HANDLING THE WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY COMPANY BEFORE PROCEEDING WITH THAT PORTION OF THE WORK.RAIL TRAFFIC SHALL, AT ALL TIMES, BE MAINTAINED AND PROTECTED. THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH

16. NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

17. FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES. SEE

20. FOR FINE AND COARSE AGGREGATE, SEE SPECIAL PROVISIONS.

21. SEE "STRUCTURAL STEEL NOTES" SHEET FOR ADDITIONAL NOTES.

22. FOR RUBBER JOINT COMPOUNDS, SEE SPECIAL PROVISIONS.

23. FOR STRUCTURE DRAINAGE SYSTEM. SEE SPECIAL PROVISIONS.

24. FOR RAILROAD TRACKWORK. SEE RAILROAD TRACKWORK PLANS.

25. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

26. FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

29. FOR CAST-IN-PLACE CONCRETE, SEE "NS SPECIFICATIONS FOR

30. WORK SHALL NOT BEGIN ON THIS BRIDGE UNTIL THE TEMPORARY SHORING HAS BEEN INSTALLED AND APPROVED, THE SITE EXCAVATED TO THE APPROVED TEMPORARY GRADE ELEVATIONS, AND THE NECESSARY TEMPORARY EROSION CONTROL AND SEDIMENT CONTROL

31. FOR TEMPORARY RAILROAD SHORING, SEE SPECIAL PROVISIONS.

32. FOR BACKFILL BEHIND ABUTMENTS AND OTHER BACKFILL AROUND THE STRUCTURE, SEE SPECIAL PROVISION "BACKFILLING AROUND

33. FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISIONS FOR 'NS SPECIFICATIONS FOR PAINTING SHOP FABRICATED BRIDGE

	TOTAL BILL OF MATERIALS CONT'D.										
	DAMPPROOFING (RAILROAD STRUCTURES)	TWO PART MEMBRANE WATER PROOFING SYSTEM	MEMBRANE LAYER WATER PROOFING SYSTEM FOR DECK	1″ASPHALT PLANKING PROTECTIVE COURSE FOR DECK	HANDRAIL AND FENCE	SELF-LUBRICATING EXPANSION BEARING ASSEMBLES	CONDUIT IN PARAPET	ASBESTOS ASSESSMENT			
	SQ. YDS.	SQ. YDS.	SQ. YDS.	SQ. YDS.	LIN.FT.	LUMP SUM	LUMP SUM	LUMP SUM			
SUPERSTRUCTURE			913	913	326.0	LUMP SUM	LUMP SUM				
ABUTMENT 1	84	18.8			22.1						
PIER 1	21										
PIER 2	12										
PIER 3	21										
ABUTMENT 2	84	18.8			22.1						
TOTAL	222	37.6	913	913	370.2	LUMP SUM	LUMP SUM	LUMP SUM			

39. ALL CONSTRUCTION JOINTS SHOWN ON THESE PLANS SHALL BE REQUIRED UNLESS SHOWN OPTIONAL. CONSTRUCTION JOINTS SHALL NOT BE PERMITTED EXCEPT AS SHOWN ON THE PLANS, OR WHERE WRITTEN APPROVAL HAS BEEN OBTAINED.

- 40. BENCHMARK: SEE LOCATION SKETCH.
- 41. DIRECT TENSION INDICATORS (DTI) WILL NOT BE PERMITTED. USE THE TURN-OF-NUT METHOD FOR INSTALLING AND TIGHTENING HIGH STRENGTH BOLTS.
- 42. THE RAILROAD TRACK TOP OF RAIL ELEVATIONS ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION. VERIFY THE TOP OF RAIL ELEVATIONS AND REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.
- 43. FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.
- 44. THE CONTRACTOR IS REMINDED THAT WORK ON THIS PROJECT REQUIRES WORKING NEAR EXISTING STRUCTURES. EVERY EFFORT HAS BEEN MADE TO IDENTIFY DISCREPANCIES AND ENSURE THAT THE DETAILS ARE DEPICTED CORRECTLY. HOWEVER, SINCE THE PROJECT INVOLVES WORKING NEAR EXISTING STRUCTURES, THE CONTRACTOR CAN EXPECT AND SHOULD PLAN ON ENCOUNTERING VARIANCES AND DEVIATIONS BETWEEN THE INFORMATION FOUND IN THESE DRAWINGS AND THE EXISTING CONDITIONS. ACTUAL FIELD CONDITIONS MAY REQUIRE MODIFICATIONS IN CONSTRUCTION DETAILS AND QUANTITIES. THE CONTRACTOR IS RESPONSIBLE TO FIELD VERIFY ALL DETAILS INCLUDING GEOMETRY AND ELEVATIONS PRIOR TO THE INSTALLATION OF ANY MATERIAL. THE CONTRACTOR SHALL SUBMIT TO NCDOT AND NORFOLK SOUTHERN COPIES OF FIELD SURVEYS AND VERIFICATIONS FOR INCLUSION INTO THE CONSTRUCTION RECORDS FOR THE PROJECT.
- 45. FOR REINFORCED CONCRETE DECK SLAB, SEE SPECIAL PROVISION FOR NS SPECIFICATIONS FOR CAST-IN-PLACE CONCRETE.FOR MEASUREMENT AND PAYMENT OF REINFORCED CONCRETE DECK SLAB, SEE THE STANDARD SPECIFICATIONS.
- 46. FOR STRUCTURAL STEEL, SEE SPECIAL PROVISIONS FOR NS SPECIFICATIONS FOR STRUCTURAL STEEL.

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48. THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 31'-O"TO THE RIGHT OF -NSN- AND 29'-2" TO THE LEFT OF -NSN- AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

49. THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE

DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

50. FOR HANDRAIL AND FENCE, SEE SPECIAL PROVISIONS.

51. INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR

FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR REMOVAL OF EXISTING STRUCTURE STA. 24+09.63 -LALT-.

52. THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND ELEVATION. THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT BELOW THE GROUND LINE.

53. FOR ASBESTOS ASSESSMENT FOR	PROJEC	T NO.	<u>U</u>	-3308	
BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.		DUR	HAM	CO	UNTY
	STATI	ON: POT	24+0	9.63 -L	ALT-
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900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	ป 2		<u> </u>		SHEETS 44







RAIL TRAFFIC STAGING:

<u>STAGE 1</u> REMOVE EXISTING -CSXN- BRIDGE AND EXISTING ABANDONED BRIDGE.

STAGE 2 (SPANS A, B, &D)

INSTALL SHORING 1 NEAR EXISTING -NSN- BRIDGE TO FACILITATE CONSTRUCTION OF PROPOSED -CSXN- BRIDGE. SEE TEMPORARY SHORING SHEET 1 OF 5 FOR LOCATION OF SHORING 1.

(ADDITIONAL SHORING MAY BE REQUIRED AT PROPOSED PIER 2 TO PROTECT EXISTING ALSTON AVE.AND FOR PROPOSED -NSN-/-NSS- AND -CSXN- BRIDGE CONSTRUCTION.)

CONSTRUCT PROPOSED TEMPORARY SPAN C OF PROPOSED -CSXN- BRIDGE.

PROJECT NO._

SHEET 1 OF 2

NO. BY:

DURHAM

STATION: POT 24+09.63 -LALT-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RALEIGH

CONSTRUCTION STAGING

BY:

REVISIONS

NO.

DATE:

U-3308

POT 21+11.43 -NSN-

COUNTY

SHEET NO. S3-5

total sheets 44

DATE:

CONSTRUCT PROPOSED -CSXN- BRIDGE SPANS A, B, AND D.

* TEMPORARY MINIMUM VERTICAL CLEARANCE TO EXISTING ALSTON AVE. = 14'-5"

DocuSigned k

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SEAL 19765

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900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991

Paul Kelly Jr

26ADE85DEC6A498

3/31/2015

STAGE 2 (SPAN C)



RAIL TRAFFIC STAGING CONT'D:

<u>Stage 3</u> INSTALL SHORING 2 AND 3 TO MAINTAIN RAIL TRAFFIC ON -NSD2- TRACK.SEE SEE TEMPORARY SHORING SHEET 1 OF 5 FOR LOCATION OF SHORING 2 AND 3. CONSTRUCT PROPOSED -NSD2- TRACK. SHIFT ALL RAIL TRAFFIC TO PROPOSED -NSD2- TRACK. REMOVE EXISTING -NSN- AND -NSS- BRIDGES.

<u>STAGE 4</u> CONSTRUCT PROPOSED -NSN- AND -NSS- BRIDGE.

STAGE 5 SHIFT ALL RAIL TRAFFIC TO -NSN- AND -NSS- BRIDGE. REMOVE PROPOSED -NSD2- TRACK,AND TEMPORARY SPAN C OF PROPOSED -CSXN- BRIDGE. COMPLETE CONSTRUCTION OF -CSXN- BRIDGE/TRACK.

FINAL SHIFT -CSXN- RAIL TRAFFIC TO PROPOSED -CSXN- BRIDGE.

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Charlotte, NC 28202 NC License No. F-0991	2		4		SHEETS



TRAFFIC STAGING:

NOTES: SEE RAIL TRAFFIC STAGING ON "CONSTRUCTION STAGING" SHEETS FOR ADDITIONAL INFORMATION. SEE PLANS FOR BRIDGE ON CSX FOR TRAFFIC STAGING STAGES 1 AND 2. TRAFFIC STAGING BELOW SHALL BE IN CONJUNCTION WITH TRAFFIC STAGING SHOWN ON PLANS FOR BRIDGE ON CSX. TEMPORARY SHORING 2 AND 3 TO BE INSTALLED DURING CONSTRUCTION OF

ALSTON AVE. TRAFFIC TO REMAIN IN EXISTING POSITION. REMOVE EXISTING -NSN- AND -NSS- BRIDGES. EXCAVATE ADJACENT TO TEMPORARY SHORING 1 AREA.

CONSTRUCT PROPOSED -NSN-/-NSS- BRIDGE, SPANS A AND B.

SHIFT ALSTON AVE. TRAFFIC TO CONSTRUCTED ROADWAY UNDER SPAN B AS SHOWN. EXCAVATE REMAINDER OF EXISTING GROUND. REMOVE TEMPORARY SHORING 2. (REMOVE ADDITIONAL SHORING AT PIER 2 IF REQUIRED.) CONSTRUCT SPANS C AND D OF PROPOSED -NSN-/-NSS- BRIDGE.CONSTRUCT REMAINDER OF ROADWAY FOR ALSTON AVE.

E. TRAFFIC TO	PROJEC	T NO. DUR DN: <u>POT</u> POT	U HAM 24+09 21+11	-3308 CO 9.63 -L .43 -NS	UNTY _ALT- SN-	
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/ Ralph Whitehead Associates, Inc.	NO. BY:	DATE:	NO. BY:	DATE:	S3-7	
900 W Trade Street, Suite 715 Charlotte, NC 28202	1		3		TOTAL SHEETS	
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- 1. FOR OPEN JOINT IN CURB DETAILS, SEE "DECK JOINT DETAILS" SHEET.
- 2. FOR DECK DRAIN LOCATIONS, SEE "STRUCTURE DRAINAGE SYSTEM" SHEET.
- 3. FOR SECTION B-B, SEE "DECK JOINT DETAILS" SHEET.

	PROJEC	T NO.	U	-3308				
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alph Whitehead Associates, Inc. 900 W Trade Street, Suite 715	<u>1</u>		<u>3</u>	DATE	TOTAL			
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- 1. FOR OPEN JOINT IN CURB DETAILS, SEE "DECK JOINT DETAILS" SHEET.
- 2. FOR DECK DRAIN LOCATIONS, SEE "STRUCTURE DRAINAGE SYSTEM" SHEET.
- 3. FOR SECTION B-B, SEE "DECK JOINT DETAILS" SHEET.

	PROJEC	T NO.	U	-3308					
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900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	ገ 2		<u> </u>		SHEETS 44				





STIMATED QUA	ANTI	TIES
	UNIT	QTY.
	EACH	56
	EACH	8
	EACH	48
	EACH	56
	EACH	8
	EACH	8
	EACH	16
	EACH	24
	FT	400'-8"
	FT	67′-7"
	FT	20'-0"
	EACH	4
	EACH	8
	EACH	8
	EACH	8
	EACH	4
	FT	82′-6"
	FT	2'-0"
	EACH	26
	EACH	6
	EACH	40
	EACH	24







STRUCTURAL STEEL NOTES

STRUCTURAL STEEL: ALL STRUCTURAL STEEL SHAPES, PLATES AND BARS SHALL BE ASTM A709, GRADE 50 OR 50W, UNLESS NOTED OTHERWISE. FRACTURE CRITICAL MEMBERS SHALL BE ASTM A709, GRADE 50F2 OR 50WF2 (SUPPLEMENTAL REQUIREMENT S84 AND S29 SHALL APPLY). NON-FRACTURE CRITICAL MEMBERS SHALL BE ASTM A709, GRADE 50T2 OR 50WT2 (SUPPLEMENTAL REQUIREMENTS S83 SHALL APPLY). ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND THE FOLLOWING REQUIREMENTS:

- 1. THE MATERIAL SUPPLIED SHALL BE OTHER THAN RIMMED OR CAPPED STEEL.
- 2. THE MATERIAL SUPPLIED SHALL BE SILICONE KILLED, FINE GRAIN PRACTICE.
- 3. CERTAIN ELEMENTS OF THE STRUCTURE ARE NOTED AS "FRACTURE CRITICAL MEMBERS" (FCM) AND SHALL MEET THE REQUIREMENTS FOR "FRACTURE CONTROL PLAN FOR FRACTURE CRITICAL MEMBERS" (AREMA CHAPTER 15, SECTION 1.14.) THE IMPACT REQUIREMENTS FOR FRACTURE CRITICAL MEMBERS SHALL BE AS REQUIRED FOR ZONE 2 SERVICE TEMPERATURE. TEST RESULTS SHALL BE FURNISHED TO THE ENGINEER OR AUTHORIZED REPRESENTATIVE.
- 4. ALL NON-FRACTURE CRITICAL MEMBERS OF THE STRUCTURE SHALL CONFORM TO THE SUPPLEMENTAL REQUIREMENTS FOR NON-FRACTURE CRITICAL IMPACT TEST NOTED IN AREMA CHAPTER 15, SECTION 1.2.1, TABLE 15-1-2 FOR ZONE 2 SERVICE TEMPERATURE. TEST RESULTS SHALL BE FURNISHED TO ENGINEER OR AUTHORIZED REPRESENTATIVE.

ALL STEEL MATERIAL SHALL BE STRAIGHT AND FREE FROM SHARP KINKS AND BENDS. ANY STEEL MATERIAL EXHIBITING SUCH DEFICIENCIES SHALL BE CAUSE FOR THE REJECTION OF THE MATERIAL.STRAIGHTENING OF THE MATERIAL SHALL NOT BE ACCEPTABLE.

MATERIAL AND WORKMANSHIP: EXCEPT AS MAY OTHERWISE BE SPECIFIED ON THE PROJECT PLANS OR SPECIFICATIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) MANUAL FOR RAILWAY ENGINEERING. PROJECT SHALL ALSO ADHERE TO STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (NCDOT) STANDARD SPECIFICATIONS, 2012 REVISION. IN THE EVENT OF CONFLICTS THE MORE STRINGENT SHALL APPLY.

SPECIFICATIONS: CURRENT EDITION, AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) MANUAL FOR RAILWAY ENGINEERING, NORFOLK SOUTHERN UNDERPASS GRADE SEPARATION DESIGN CRITERIA, STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (NCDOT) STANDARD SPECIFICATIONS, 2012 REVISION.

ALL W-SHAPE BEAMS SHALL BE FABRICATED WITH THE NATURAL MILL CAMBER OF THE BEAM "UP".

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED.

MILL TEST REPORTS: NORFOLK SOUTHERN RAILWAY COMPANY SHALL BE FURNISHED COPIES OF MILL TEST REPORTS FOR ALL MATERIALS EXCEPT MISCELLANEOUS PLATES AND SHAPES.REPORTS SHALL INDICATE COMPLIANCE WITH ALL SPECIFIED REQUIREMENTS.

INSPECTION: SHOP INSPECTION BY NORFOLK SOUTHERN RAILWAY COMPANY OR ITS AUTHORIZED AGENT, SEE "NS SPECIFICATIONS FOR STRUCTURAL STEEL" SPECIAL PROVISION FOR ADDITIONAL WELDING INSPECTION OF FLANGE PLATE TO WEB PLATES WELDS.

SHOP AND FIELD PAINT: ALL NEW STRUCTURAL STEEL MEMBERS, EXCEPT FAYING SURFACES, SHALL BE PAINTED IN ACCORDANCE WITH 'NS SPECIFICATIONS FOR PAINTING SHOP FABRICATED BRIDGE STEEL" SPECIAL PROVISION. THE SYSTEM TO BE USED SHALL BE AN INORGANIC ZINC-ACRYLIC SYSTEM. TOP COAT SHALL BE APPLIED IN THE FIELD.

SHOP DRAWINGS: SHOP DRAWINGS SHALL BE APPROVED BY THE CHIEF ENGINEER BRIDGES & STRUCTURES, NORFOLK SOUTHERN CORPORATION, ATLANTA, GEORGIA. MATERIAL SHALL NOT BE FABRICATED UNTIL DRAWINGS HAVE BEEN APPROVED. COPIES OF APPROVED SHOP DRAWINGS ARE TO BE FURNISHED TO THE ENGINEER. SHOP DRAWINGS SHALL BE LABELED "NORFOLK SOUTHERN M.P. H-56.10".

HOLES: OPEN HOLES AS NOTED.



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ANCHOR BOLTS SHALL BE GROUTED IN FORMED HOLES AFTER GIRDERS ARE ERECTED.

FOR PROTECTION OF PAINTED STEEL, SEE "NS SPECIFICATIONS FOR PAINTING SHOP FABRICATED BRIDGE STEEL" SPECIAL PROVISION.

FOR STRUCTURAL STEEL, SEE SPECIAL PROVISION "NS SPECIFICATIONS FOR STRUCTURAL STEEL".

WELDING: WELDING SHALL BE IN ACCORDANCE WITH AASHTO/ AWS-D1.5:2012 AND AMERICAN NATIONAL STANDARD, INCLUDING INTERIMS, AS MODIFIED OR SUPPLEMENTED BY THE AREMA MANUAL FOR RAILWAY ENGINEERING.

ALL WELDS SHALL BE MADE WITH E7018 ELECTRODES.WELDING SHALL BE PERFORMED WITH THE SUBMERGED ARC WELDING (SAW) OR SHIELDED METAL ARC WELDING (SMAW) PROCESS.FRACTURE-CRITICAL MEMBER FLANGE TO WEB WELDS SHALL BE MADE BY THE SUBMERGED ARC WELDING (SAW) PROCESS.

ALL WELDS ARE TO BE SHOP WELDS, UNLESS NOTED OTHERWISE. WELDING PROCEDURE AND SIZES SHALL BE AS SHOWN IN THE PROJECT PLANS.

THERE SHALL BE THOROUGH FUSION BETWEEN WELD METAL AND BASE METAL AND BETWEEN SUCCESSIVE PASSES OF THE WELD. ALL CRATERS SHALL BE FILLED TO THE FULL CROSS SECTION OF THE WELD.

PRIOR TO WELDING, EACH WELDER SHALL HAVE BEEN CERTIFIED IN ACCORDANCE WITH AWS REQUIREMENTS DURING A PERIOD OF ONE (1) YEAR PRIOR TO WORK ON THE BRIDGE. THE FABRICATOR SHALL FURNISH THE ENGINEER OR AUTHORIZED REPRESENTATIVE WITH AN AWS CERTIFICATE FOR EACH WELDER, COVERING THEIR ABILITY TO MAKE A COMPLETE AND SATISFACTORY WELD OF EACH KIND TO BE USED ON THE PROJECT.

SURFACES AND EDGES TO BE WELDED SHALL BE SMOOTH, UNIFORM AND FREE FROM FINS, TEARS CRACKS, OR OTHER DEFICIENCIES WHICH WOULD ADVERSELY AFFECT THE QUALITY OR STRENGTH OF THE WELD.SURFACES TO BE WELDED AND SURFACES ADJACENT TO A WELD SHALL ALSO BE FREE OF ANY SCALE, SLAG, RUST, MOISTURE, GREASE OR OTHER FOREIGN MATERIAL THAT WILL INHIBIT PROPER WELDING.

NON-DESTRUCTIVE TESTING OF THE FRACTURE CRITICAL MEMBERS IS TO BE PERFORMED BY AN INDEPENDENT TESTING COMPANY APPROVED BY THE ENGINEER AND CONTRACTED BY THE FABRICATOR. PERSONAL QUALIFICATIONS AND CERTIFICATION ARE TO BE IN ACCORDANCE WITH THE CURRENT AREMA MANUAL CHAPTER 15 FOR FRACTURE CRITICAL MEMBERS. COPIES OF THE TEST ARE TO BE FURNISHED TO THE ENGINEER OR AUTHORIZED REPRESENTATIVE FOR INCLUSION IN THEIR PROJECT FILE.

BOLTS: ALL BOLTED CONNECTIONS SHALL BE MADE WITH 7/8"DIA. ASTM A325, TYPE 3 BOLTS UNLESS NOTED OTHERWISE. NUTS AND WASHER SHALL BE A563, GRADE C3, AND F436, TYPE 3 RESPECTIVELY. ALL BOLTS, NUTS, AND WASHERS SHALL BE MECHANICALLY GALVANIZED UNLESS NOTED OTHERWISE. OPEN HOLES SHALL BE ¹⁵/₁₆"DIA. UNLESS NOTED OTHERWISE. ALL BOLTS, NUTS, AND WASHERS WILL BE SUPPLIED FROM A SINGLE SOURCE WITH DOCUMENTATION OF THEIR SOURCE AND QUALITY CERTIFICATION. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-NUT METHOD" IN ACCORDANCE WITH AREMA MANUAL CHAPTER 15, SECTION 3.2.3 - INSTALLATION OF HIGH STRENGTH BOLTS. ANY BOLTS THAT REQUIRE REMOVAL AFTER BEING TIGHTENED SHALL BE DISCARDED AND A NEW BOLT INSTALLED, UNLESS OTHERWISE NOTED.

ALL BOLT HOLES SHALL BE SUB-DRILLED AND REAMED OR DRILLED FROM THE SOLID. AT NO TIME ARE HOLES TO BE SUB-PUNCHED AND REAMED OR PUNCHED FULL SIZE.

BOLTS SHALL BE INSTALLED WITH THE BOLT HEADS EXPOSED TO THE WEATHER. THE SPECIFIED WASHERS SHALL BE INSTALLED BENEATH THE TURNING ELEMENT. VERTICALLY POSITIONED BOLTS WHICH HAVE BOTH THE HEAD AND NUT EXPOSED TO WEATHER SHALL HAVE THE HEAD PLACED ABOVE THE NUT. HORIZONTAL POSITIONED NUTS SHALL HAVE THE HEADS ON THE VISIBLE SIDE OF THE CONNECTION.



	PROJEC ⁻	T NO	U·	-3308			
		DURH	IAM	CO	UNTY		
	STATIO	N: <u>POT</u> POT	24+09 21+11	9.63 -L .43 -NS	<u>ALT-</u> 5N-		
DocuSigned by: Paul Kelly, Jr 26ADE85DEC6A498	DEPAR	state (RTMENT (OF NORTH CARC DF TRAN RALEIGH	ISPORTA	TION		
SEAL 19765	STRUCTURAL STEEL NOTES						
3/31/2015		REVISI	ONS		SHEET NO.		
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900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	1		\$ }		TOTAL SHEETS 44		
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- 1. ALL DIMENSIONS ON THIS SHEET ARE HORIZONTAL.
- 2. FOR PIPE SUPPORT ANGLE (PSA) DETAILS, SEE "STRUCTURE DRAINAGE SYSTEM" SHEET 2 OF 4.
- 3.FOR DIAPHRAGM SUPPORT ANGLE (DSA) DETAILS, SEE "STRUCTURE DRAINAGE SYSTEM" SHEET 2 OF 4 AND "STRUCTURAL STEEL DETAILS" SHEET.
- 4. FOR SHEAR CONNECTOR DETAIL, SEE SHEET 1

	PROJEC	T NO. DUR	U HAM	-3308 C0	UNTY			
	STATION: POT 24+09.63 -LALT POT 21+11.43 -NSN- SHEET 2 OF 3							
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- 4. FOR SHEAR CONNECTOR DETAIL, SEE SHEET 1

	PROJECT NO. U-33 DURHAM	308 _ COUNTY						
	STATION: POT 24+09.63 -LALT- POT 21+11.43 -NSN- SHEET 3 OF 3							
Paul Kelly Jr 26ADE85DEC6A498	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPO RALEIGH	ORTATION						
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CHECKED BY	. DJM	DATE :	06-14

	E	BEARIN	NG DIN	MENSI	7 – NC	Г				
BEAM 4	BEAM 5	BEAM 6	BEAM 7	BEAM 8	BEAM 9	BEAM 10	BEAM 11	BEAM 12	BEAM 13	BEAM 1
DIM.T	DIM.T	DIM.T	DIM.T	DIM.T						
3 /8″	3 ¹ /8″	3 ¹ /8″	3 /8″	3″	3″	3″	3″	3″	3″	3″
3 /8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3″	3″	3″	3″	3″	3″	3″
3 /8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3″	3″	3″	3″	3″	3″	3″
3 /8″	3 ¹ /8″	3 ¹ /8″	3 /8″	3″	3″	3″	3″	3″	3″	3″
31/4″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3″	3″	3″	3″	3″	3″	3″
31/4″	3 ¹ /8″	3 ¹ /8″	3 /8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3″	3″	3″	3″
31/4″	31/8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	31/8″	3″	3″	3″	3″
3 ¹ /4″	31/8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	3 ¹ /8″	31/8″	3″	3″	3″	3″

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900 W Trade Street, Suite 715	1			3			TOTAL SHEETS
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	NOTES FOR HANDRAIL AND FENCING	
	1. ALL HANDRAIL PIPE, SLEEVES, AND EXPANSION JOINTS TO BE SMOOTH AND FREE OF SHARP EDGES.	
	2. ALUMINUM PIPE TO BE ASTM B-241.ALLOY 6061-T6,ALUMINUM BASE PLATE.CLOSURE PLATE.AND HANDRAIL PLATE TO BE ASTM B-209.ALLOY 6061-T6.	
	3. FENCE FABRIC TO BE TYPE III ALUMINUM ALLOY WIRE ASTM B-211, ALLOY 6061-T89 OR T94.	€ HANDRAIL
	4. BRACE RAIL AND BRACE ENDS, POST TOPS, TURNBUCKLES, TRUSS RODS, GATE HINGES AND LATCHES, STRETCHER BARS, AND BAR BANDS IN ACCORDANCE WITH AASHTO M181.	Ţ
	5. STAINLESS STEEL BOLTS, NUTS, AND ANCHOR RODS TO BE ASTM A-276, TYPE 304 STAINLESS STEEL WASHERS TO BE ASTM A-276, TYPE 302. ANCHOR ROD THREADS SHALL BE ROLLED. NOT CUT.	TOP OF B TO
	6. POST TO BE SET PERPENDICULAR TO TOP OF CURB AND RAILS SHALL BE PLACED PARALLEL TO THE GRADE OF THE BRIDGE.	- <u>- 0</u> - 10 - 18
	7. BOTTOM OF BASE PLATE SHALL BE THOROUGHLY COATED WITH ALUMINUM IMPREGNATED CAULKING COMPOUND OR APPROVED QUALITY.	
	8. CERTIFIED MILL REPORTS ARE REQUIRED FOR POST, RAIL, AND FENCE FABRIC. SHOP INSPECTION IS NOT REQUIRED.	
	9. AFTER ANCHOR BOLT AND OTHER BOLT NUTS HAVE BEEN TIGHTENED, THREADS SHALL BE NICKED TO LOCK NUTS.	
	10. THE ALUMINUM BRACE BANDS USED TO SECURE HANDRAIL SLEEVE SHALL BE OF SUCH SIZE NECESSARY TO CLAMP TIGHTLY TO FENCE POST.	
	11. WELDING SHALL BE IN ACCORDANCE WITH THE CURRENT AWS STRUCTURAL WELDING CODE - ALUMINUM.	
	12. ANCHOR PLATE SHALL BE STEEL CONFORMING TO ASTM SPECIFICATION A36.	
	13. UPPER ANCHOR ROD NUTS SHALL BE HEAVY HEX NUTS,PER ASTM A276 TYPE 302 OR 304 STAINLESS STEEL.	
	14. LOWER ANCHOR ROD NUTS SHALL BE HEAVY HEX NUTS, PER ASTM A563.	
	15. FOR HANDRAIL AND FENCE, SEE SPECIAL PROVISIONS.	
	16. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND APPROVAL.SEE SUBMITTAL OF WORKING DRAWINGS SPECIAL PROVISIONS FOR DETAILS.	
02.dgn		
J_HRFD	1 ¹ /4"Ø SCHEDULE 40 ALUM. PIPE ASTM	FABRIO RAIL / 9 GAUG
.024_U3308_SMI		R BANDS
truct/Final/403_	HANDRAIL TO BE ATTACHED TO ONLY ONE FENCE POST AT JOINTS WITHOUT GATE. PROVIDE HANDRAIL EXPANSION JOINT BETWEEN SPANS STRETCHER BAR	STRETCHER BAF
station\s1	3/8″Ø ALUMINUM TRUSS ROD WITH TURNBUCKLE IN END PANEL OF EACH RUN EA.RUN.(SAME AS TOP RAIL)	ALUM.

BOTTOM TENSION WIRE OF 7 GA.

ALUM. COATED STEEL SPRING WIRE FASTENED AT 2'-0" INTERVALS

WITH 11 GA. GALV. STEEL HOG RINGS

DATE : 06-14

_ DATE : 06-14

KPL

DRAWN BY :

CHECKED BY : DJM

7″±

2″JT.

8″ ¦ 8″

16″

DETAIL AT DECK JOINT

DETAILS FOR CHAIN LINK FENCE

NOT TO SCALE

	REINFORCING BAR SCHEDULE																						
		SPA	ΝΑ					SPA	ΝB					SPA	N C					SPA	N D		
MARK	NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGHT (LB.)	MARK	NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGHT (LB.)	MARK	NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGHT (LB.)	MARK	NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGHT (LB.)
A1	52	#5	STR.	50′-3″	2,725	A1	78	#5	STR.	50'-3"	4,088	A1	78	#5	STR.	50'-3″	4,088	A1	52	#5	STR.	50'-3″	2,725
A2	16	#5	STR.	50'-8″	846	A2	16	#5	STR.	50′-8″	846	A2	16	#5	STR.	50′-8″	846	A2	16	#5	STR.	50′-8″	846
Α3	4	#5	STR.	43′-5″	181	Α3	4	#5	STR.	43′-5″	181	A3	4	#5	STR.	43′-5″	181	A3	4	#5	STR.	43′-5″	181
Α4	4	#5	STR.	35′-11″	150	Α4	4	#5	STR.	35′-11″	150	Α4	4	#5	STR.	35′-11″	150	A4	4	#5	STR.	35′-11″	150
A5	4	#5	STR.	28'-5″	119	A5	4	#5	STR.	28'-5″	119	A5	4	#5	STR.	28′-5″	119	A5	4	#5	STR.	28′-5″	119
A6	4	#5	STR.	20'-11″	87	A6	4	#5	STR.	20'-11"	87	A6	4	#5	STR.	20'-11″	87	A6	4	#5	STR.	20'-11"	87
Α7	4	#5	STR.	13′-5″	56	Α7	4	#5	STR.	13′-5″	56	Α7	4	#5	STR.	13′-5″	56	Α7	4	#5	STR.	13′-5″	56
A8	4	# 5	STR.	5'-11"	25	A8	4	#5	STR.	5'-11″	25	A8	4	#5	STR.	5'-11″	25	A8	4	#5	STR.	5'-11″	25
B1	102	#5	STR.	34'-1"	3,626	B1	102	#5	STR.	47'-0"	5,000	B1	102	#5	STR.	47'-0"	5,000	B1	102	#5	STR.	34'-1"	3,626
B2	36	# 5	STR.	11'-0"	413	B2	48	#5	STR.	11'-5″	572	B2	48	#5	STR.	11'-5″	572	B2	36	#5	STR.	11'-0"	413
S1	72	#5	(1)	6′-5″	482	S1	104	#5	(1)	6'-5″	696	S1	104	#5	(1)	6′-5″	696	S1	72	#5		6'-5″	482

SPA SPA SPA SPA T01

	BA	١R	τγρε	S						
	ALL BAR	DIMENS	SIONS AF	RE OUT TO O	UT.					
SU	PERSTRUC	TURE	BILL	OF MAT	ERIAL					
	CAST-IN 5000 CONC	N-PLAC PSI RETE	E	EPOX REII	Y COATED NFORCING STEEL					
	(C.	Y.)		(LB)						
AN "A" AN "B"	DECK SLAB 59.3 81.3	C	URB 5.1 7.0		8,710 11,820					
AN "C"	81.3		(.0 F 1		11,820					
TALS	281.2	2			<u>0,110</u> 41,060					
SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS										
BAR ST7F	SUPERSTRUCT EXCEPT APPR SLABS, PARAF	URE DACH PET,	APPR04	ACH SLABS	PARAPET AND BARRIER					

BAR SIZE	SLABS, P AND BARR	ARAPET, IER RAIL	APPRUAC	H SLABS	AND BARRIER		
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL		
#4	2'-0"	1'-9"	2'-0"	1'-9"	2'-9"		
#5	2'-6"	2'-2"	2'-6"	2'-2"	3′-5″		
#6	3'-0"	2'-7"	3'-10"	2'-7"	4'-4"		
#7	5'-3"	3'-6"					
#8	6'-10"	4'-7"					

PROJECT NO. U-3308 DURHAM STATION: POT 24+09.63 -LALT-POT 21+11.43 -NSN-STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH DocuSigned by: Paul Kelly, Jr 26ADE85DEC6A498... SUPERSTRUCTURE BILL OF MATERIALS · CAROj" SEAL 19765 NGINEER 3/31/2015 SHEET NO. REVISIONS S3-25 STV / Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991 DATE: NO. BY: NO. BY: DATE: total sheets 44

- 1. FOR EPOXY PROTECTIVE COATING, SEE STANDARD SPECIFICATIONS.
- 2. BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.
- 3. THE TOP SURFACE AREAS OF THE CAP SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.
- 4. STIRRUPS AND "U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- 5. HOOKS ON BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
- 6. SEE SHEET 5 OF 5 FOR BEARING LAYOUT DETAIL AND SHEET 2 OF 5 FOR ANCHOR BLOCK OUT DETAIL.
- 7. SEE SHEET 3 OF 5 FOR BACKWALL EXTENSION DETAILS.
- 8. FOR SECTIONS AND VIEWS, SEE SHEETS 2 OF 5 AND 3 OF 5.
- 9. FOR WING WALL DETAILS, SEE SHEET 4 OF 5.
- 10. THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.
- 11. SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIERS WILL NOT BE PERMITTED.
- 12. FOR BEARINGS AND ANCHOR BOLTS, SEE THE "BEARING DETAILS" SHEET.
- 13. FOR OTHER NOTES, SEE "FOUNDATION LAYOUT" SHEET.
- 14. SEE DETAIL "A" ON SHEET TITLED "DECK JOINT DETAILS", FOR LOCATION OF WATERSTOP TO BE INSTALLED IN THE BACKWALL.
- 15. FOR DAMPPROOFING, SEE 'NS SPECIFICATION FOR CAST-IN-PLACE CONCRETE" SPECIAL PROVISION AND GENERAL NOTES ON GENERAL DRAWING SHEET 4 OF 4.

	PROJEC	CT NO. DUR		-3308		
	STATI	DN: <u>POT</u> POT	24+09 21+11	9.63 -L .43 -N	<u>_ALT-</u> SN-	
Paul Kelly fr	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH ABUTMENT 1					
26ADE85DEC6A498 CAROUTH CAROUTH SEAL 19765 SEAL 19765 SEAL 19765						
12/11/2015		REVIS	IONS		SHEET NO.	
Ralph Whitehead Associates, Inc.	NO. BY:	DATE:	NO. BY:	DATE:	S3-26	
900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	า 2		<u> </u>		SHEETS 44	

PLAN - TYPICAL BACKWALL EXTENSION DETAIL

1'-3"

MIN.

VARIES

0″ TO

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

-SP-1 (DRILLED PIERS 1&2)(TYP.) SP-2 (DRILLED PIERS 3-5)(TYP.) 5'-0" - #11M1 (DRILLED PIERS 1&2)(TYP.) #11M2 (DRILLED PIERS 3-5)(TYP.) 1'-3″ 3'-9″ /── € DRILLED PIER 2'-6" , 2'-6" CAP & DRILLED PIER POUR 3 BACKWALL FILL FACE — CONST. JT. (CONST. JT. DETAIL) 3" SPIRAL ERMINATION POUR CAP ഹി 0-) MIN 6″ - CONST.JT. 20-#11 "M" BARS -POUR 1 DRILLED PIER DRILLED PIER SPIRAL REINF. — 9% (, YP.) (, YP.) APPROVED BAR SUPPORT (TYP, UNDER EA. "M" BAR) ò 0 4'-0"Ø SECTION AT DRILLED PIER (CAP REINFORCEMENT NOT SHOWN FOR CLARITY) 41/2 81/2 01/2 PROJECT NO. U-3308 01/2 DURHAM COUNTY 0/2 STATION: POT 24+09.63 -LALT-10¹/2″ POT 21+11.43 -NSN-SHEET 3 OF 5 ໍ້ມີ STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DocuSigned by: Paul Kelly, Jr 26ADE85DEC6A498... RALEIGH TH CARO ABUTMENT 1 SEAL 19765 NGINEER

3/31/2015							
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/ Ralph Whitehead Associates, Inc.	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-28
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3/30/2015				
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QUANTITIES	
	ABUT.1
INFORCING STEEL LBS.	30,164
IRAL REINFORCING STEEL LBS.	5,549
I.P. CONCRETE : POUR 2 - CAP & LOWER WING WALL CU. YDS.	62.2
POUR 3 - BACKWALL & UPPER WING WALL CU.YDS.	20.8
TOTAL CU. YDS.	83.0
-O"Ø DRILLED PIERS IN SOIL L.F.	86.5
-O"Ø DRILLED PIERS NOT IN SOIL L.F.	35.0
DTAL 4'-O"Ø DRILLED PIERS L.F.	121.5
CU.YDS.	56.5
L TUBES L.F.	516.0
PT TESTING EA.	5

	PROJEC	T NO. DUR	U HAM	-3308		
	STATIO	DN: <u>POT</u> POT	24+0 21+11	9.63 -L .43 -N	<u>ALT-</u> SN-	
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DocuSigned by: Paul Kelly. Jr 26ADE85DEC6A498	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SEAL 19765	ABUTMENT 1					
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/ Ralph Whitehead Associates. Inc.	NO. BY:	DATE:	NO. BY:	DATE:	S3-30	
900 W Trade Street, Suite 715	1		3		TOTAL SHEETS	
Charlotte, NC 28202 NC License No. F-0991	2		A		44	

NOTES: 1. FOR EPOXY PROTECTIVE COATING, SEE STANDARD SPECIFICATIONS. SEE 'DETAIL OF WOOD 2. STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. 3. HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL. 4. THE TOP SURFACE AREAS OF THE PIER CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED. 5. SEE SHEET 3 OF 3 FOR BEARING DETAIL AND DETAIL OF WOOD CORE PIER, COLUMN FOR ANCHOR BOLT HOLES. 6. THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH. 7. SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIERS WILL NOT BE PERMITTED. 8. A TWO PART WEATHERPROOFING SYSTEM SHOULD BE APPLIED AROUND THE CONSTRUCTION JOINT AT THE COLUMN AND DRILLED PIER FOR EACH COLUMN. 9. METHOD B DAMPROOFING SHOULD BE APPLIED TO ANY PART OF THE COLUMN THAT IS BELOW THE GROUND LINE. 10. THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT BELOW THE GROUND LINE. TABLE A ELEVATIONS & € PIER, COLUMN & DRILLED PIER DIMENSIONS 5'-0" PIER 2 | PIER 3 PIER 1 2'-6" ! 2'-6" EL.″A″ 409.32 409.19 409.03 EL.*"*B" 404.19 404.03 404.32 EL.*"*C" 392.40 392.40 392.40 EL.″D″ 380.90 375.90 372.90 375.90 372.90 EL.*"*E" 380.90 EL.*"*F" 377.90 364.90 372.90 372.90 EL.*"*G″ 377.90 364.90 364.90 EL.*"*H" 372.90 377.90 3'-0' MIN 11′-9<mark>1/</mark>2″ DIM. "AA" 11'-11" | 11'-7¹/2" DIM."BB" 11'-6″ 16'-6″ 19′-6″ DIM "CC" 14'-6" 27'-6″ 19′-6″ 6″ TABLE B PLAN VIEW CONST. JT. INFORMATION В Α С D В 2 3 4 DIM CARO 4'-0"Ø SEAL 19765 Paul Kelly J COLUMN SPIRAL REINF.SP-3 -26ADE85DEC6A498 VGINEE 3/31/2015 U-3308 PROJECT NO. DURHAM COUNTY STATION: POT 24+09.63 -LALT-POT 21+11.43 -NSN-SHEET 1 OF 3 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION ____ RALEIGH 4'-6"Ø PIER 1, 2, & 3 SIDE VIEW DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED REVISIONS SHEET NO. S3-31 DATE: DATE: NO. NO. BY: BY: TV / Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991 TOTAL SHEETS 44

				F	REINF	FORC	ING	BAR	SCH	EDUl
	PIE	R 1					PIE	ER 2		
NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGHT (LBS.)	MARK	NO.	SIZE	TYPE	LENGTH (FT.IN.)	WEIGH [.] (LBS.)
16	#9	STR.	51′-6″	2,802	B1	16	#9	STR.	51′-6″	2,802
8	#9	1	54'-0"	1,469	B2	8	#9	1	54'-0"	1,469
6	#5	STR.	51′-6″	322	B3	6	#5	STR.	51′-6″	322
32	#11	STR.	20'-6"	3,485	M1	32	#11	STR.	25′-6″	4,335
48	#11	STR.	23'-6"	5,993	M2	48	#11	STR.	36'-6"	9,308
64	#5	(2)	13'-6"	901	S1	64	#5	2	13'-6"	901
20	#5	3	14'-11"	311	S2	20	#5	3	14'-11"	311
22	5	5	7'-6″	172	U1	22	5	5	7'-6″	172
80	11	4	16'-6″	7,013	V1	80	11	(4)	16'-6″	7,013
2	*	$\overline{(7)}$	516'-10"	1.078	SP1	2	*	(7)	744'-0"	1.552
3	*	(7)	653'-2"	2,044	SP2	3	*	(7)	1,243'-9"	3,892
5	*	6	569'-10"	2 , 972	SP3	5	*	<u> </u>	564'-2"	2,942
CING STE	 EEL		LBS.	22,468	REINFOR	 RCING STI	 EEL		LBS.	26,633
REINFORG	CING STEE	EL	LBS.	6,094	SPIRAL	REINFOR	CING STE	EL	LBS.	8,386
	NO. 16 8 6 32 48 64 20 22 80 22 80 22 10 5 CING STERENTOR	PIE NO. SIZE 16 #9 8 #9 6 #5 32 #11 48 #11 48 #11 48 #11 64 #5 20 #5 20 #5 20 #5 80 11 22 5 80 11 22 5 80 11 22 5 80 11 22 5 80 11	PIER 1 NO. SIZE TYPE 16 #9 STR. 8 #9 ① 6 #5 STR. 32 #11 STR. 48 #11 STR. 48 #11 STR. 64 #5 ② 20 #5 ③ 20 #5 ③ 80 11 ④ 22 5 ⑤ 80 11 ④ 2 * ⑦ 3 * ⑦ 5 * ⑥ 2ING STEEL STEEL	PIER 1 NO. SIZE TYPE LENGTH (FT. IN.) 16 #9 STR. 51'-6" 8 #9 ① 54'-0" 6 #5 STR. 51'-6" 32 #11 STR. 20'-6" 48 #11 STR. 20'-6" 48 #11 STR. 23'-6" 64 #5 ② 13'-6" 20 #5 ③ 14'-11" 20 #5 ⑤ 7'-6" 80 11 ④ 16'-6" 2 * ⑦ 516'-10" 3 * ⑦ 653'-2" 5 * ⑥ 569'-10" 2 LBS. LBS.	PIER 1 NO. SIZE TYPE LENGTH WEIGHT (FT. IN.) (LBS.) 16 *9 STR. 51'-6" 2,802 8 *9 ① 54'-0" 1,469 6 *5 STR. 51'-6" 322 32 *11 STR. 20'-6" 3,485 48 *11 STR. 23'-6" 5,993 64 *5 ② 13'-6" 901 20 *5 ③ 14'-11" 311 20 *5 ⑤ 7'-6" 172 80 11 ④ 16'-6" 7,013 2 * ⑦ 516'-10" 1,078 3 * ⑦ 653'-2" 2,044 5 * ⑥ 569'-10" 2,972 2 LBS. 22,468 22,468	PIER 1 MARK NO. SIZE TYPE LENGTH WEIGHT MARK 16 #9 STR. 51'-6" 2,802 B1 8 #9 ① 54'-0" 1,469 B2 6 #5 STR. 51'-6" 322 B3 32 #11 STR. 20'-6" 3,485 M1 48 #11 STR. 20'-6" 3,485 M1 64 #5 ② 13'-6" 901 S1 20 #5 ③ 14'-11" 311 S2 21 5 ⑤ 7'-6" 172 U1 80 11 ④ 16'-6" 7,013 V1 2 * ⑦ 516'-10" 1,078 SP1 3 * ⑦ 653'-2" 2,044 SP2 5 * ⑥ 569'-10" 2,972 SP3 CING STEEL LBS. 22,468 REINFOR	REINFORC PIER 1 MARK NO. NO. SIZE TYPE LENGTH WEIGHT MARK NO. 16 #9 STR. 51'-6" 2,802 B1 16 8 #9 ① 54'-0" 1,469 B2 8 6 #5 STR. 51'-6" 322 B3 6 32 #11 STR. 20'-6" 3,485 M1 32 48 #11 STR. 23'-6" 5,993 M2 48 64 #5 ② 13'-6" 901 S1 64 20 #5 ③ 14'-11" 311 S2 20 64 #5 ② 13'-6" 901 S1 64 20 #5 ⑤ 7'-6" 172 U1 22 80 11 ④ 16'-6" 7,013 V1 80 2 * ⑦ 516'-10" 1,078 SP1 2 3 * ⑦ 653'-2"	REINFORCING PIER 1 PIE NO. SIZE TYPE LENGTH WEIGHT MARK NO. SIZE 16 *9 STR. 51'-6" 2,802 B1 16 *9 8 *9 ① 54'-0" 1,469 B2 8 *9 6 *5 STR. 51'-6" 322 B3 6 *5 32 *11 STR. 20'-6" 3,485 M1 32 *11 48 *11 STR. 20'-6" 3,485 M1 32 *11 48 *11 STR. 20'-6" 3,485 M1 32 *11 48 *11 STR. 23'-6" 5,993 M2 48 *11 5 ③ 13'-6" 901 S1 64 *5 20 *5 3 14'-11" 311 S2 20 *5 80 11 ④ 16'-6" 7,013 V1 80 11 2 * ⑦	REINFORCING BAR PIER 1 PIER 2 NO. SIZE TYPE LENGTH WEIGHT MARK NO. SIZE TYPE 16 *9 STR. 51'-6" 2,802 B1 16 *9 STR. 8 *9 ① 54'-0" 1,469 B2 8 *9 ① 6 *5 STR. 51'-6" 322 B3 6 *5 STR. 32 *11 STR. 20'-6" 3,485 M1 32 *11 STR. 48 *11 STR. 20'-6" 5,993 M2 48 *11 STR. 20 *5 ③ 14'-11" 311 S2 20 *5 ③ 20 *5 ③ 14'-11" 311 S2 20 *5 ⑤ 22 5 ⑤ 7'-6" 172 U1 22 5 ⑤ 80 11 ④ 16'-6" 7,013 V1 80 11 ④	REINFORCING BAR SCHE PIER 1 PIER 2 NO. SIZE TYPE LENGTH WEIGHT MARK NO. SIZE TYPE LENGTH 16 *9 STR. 51'-6" 2,802 B1 16 *9 STR. 51'-6" 8 *9 ① 54'-0" 1,469 B2 8 *9 ① 54'-0" 6 *5 STR. 51'-6" 322 B3 6 *5 STR. 51'-6" 32 *111 STR. 20'-6" 3,485 M1 32 *111 STR. 25'-6" 48 *111 STR. 23'-6" 5,993 M2 48 *111 STR. 36'-6" 64 *5 ② 13'-6" 901 S1 64 *5 ② 13'-6" 22 5 ⑤ 7'-6" 172 U1 22 5 ⑤ 7'-6" 22 5 ⑤ </td

THE CONTRACTOR'S ATTENTION IS LONGITUDINAL REINFORCEMENT FOR

BE #5 PLAIN OR DEFORMED BAR

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Charlotte, NC 28202 NC License No. F-0991	2			4			SHEETS 44

PLAN - TYPICAL BACKWALL EXTENSION DETAIL

_ DATE : <u>06-14</u> _ DATE : <u>06-14</u>

ELEVATION (W1)

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PLAN (W1)

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

	PROJEC	CT NO.	U HAM	-3308 C0	UNTY	
	STATI	ON: <u>POT</u> POT	24+09	9.63 -L .43 -NS	<u>_ALT-</u> SN-	
Paul Kelly, Jr 26ADE85DEC6A498	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH ABUTMENT 2					
SEAL 19765 E. KELLINGINEER						
		REVISI		DATE	SHEET NO. S3-37	
Kalph Whitehead Associates, Inc. 900 W Trade Street, Suite 715	<u>но.</u> 1		3	DATE:	TOTAL	
Charlotte, NC 28202 NC License No. F-0991	2	l	4		44	

8 BAR
"X"
10'-9″
10'-0"

DRAWN BY : TJT/CLG CHECKED BY : DJM

___ DATE : <u>06-14</u> ___ DATE : <u>06-14</u>

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

QUANTITIES		
		ABUT.2
INFORCING STEEL	LBS.	14,367
I.P.CONCRETE : POUR 1 - CAP & LOWER WING WALL	CU.YDS.	70.0
POUR 2 - BACKWALL & UPPER WING WALL	CU.YDS.	20.8
TOTAL	CU.YDS.	90.8
9 12x53 STEEL PILES		
NO.	EA.	28
LENGTH	L.F.	840.0

BEARING LAYOUT DETAIL

	PROJEC	CT NO. DUR	U HAM	-3308 CC	UNTY	
	SHEET 5 0	DN: <u>POT</u> POT	24+09 21+11	9.63 -L .43 -NS	<u>LALT-</u> SN-	
Paul Kelly Jr 26ADE85DEC6A498	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SEAL 19765 5. KELLINGINEER SCIENCE 3/31/2015	ABUTMENT 2					
		REVIS	IONS		SHEET NO.	
Ralph Whitehead Associates, Inc.	NO. BY:	DATE:	NO. BY:	DATE:	S3-38	
900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	12		<u>अ</u> 4		TOTAL SHEETS 44	

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. STRAIGHT EDGING WILL NOT BE REQUIRED UNLESS, IN THE OPINION OF THE ENGINEER, VISUAL INSPECTION INDICATES A NEED FOR IT. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS.FOR BERM WIDTH.SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4.60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-O"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

1"EXPANSION JOINT MATERIAL AND ALL LABOR, MATERIALS AND INCIDENTALS REQUIRED FOR INSTALLATION SHALL BE CONSIDERED INCIDENTAL TO THE PAY ITEM, "4 INCH SLOPE PROTECTION".

BRIDGE @ STA.24+09.63 -LALT-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE
	SQUARE YARDS	APPROX.L.F.
ABUTMENT 1	233	560
ABUTMENT 2	233	560

* QUANTITY SHOWN IS BASED ON 5' POURS.

WELDED WIRE FABRIC 6 X 6 - W1.4 X W1.4

	PROJECT NO. <u>U-3308</u> <u>DURHAM</u> CO STATION: <u>POT 24+09.63 -L</u> POT 21+11.43 -NS						UNTY ALT- SN-
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Ralph Whitehead Associates, Inc.	NO.	BY:	DATE:	NO.	BY:	DATE:	S3-39
900 W Trade Street, Suite 715 Charlotte, NC 28202	1			3			TOTAL SHEETS
NC License No. F-0991	2			44			44

RETAINED SOIL PROPERTIES IN-SITU SOIL ASSUMED UNIT WEIGHT OF SOIL (Υ) = 120 lbs/ft' ANGLE OF INTERNAL FRICTION (Φ) = 24° COHESION (c) = 0 WALL FRICTION = 8° WEATHERED ROCK (EL. VARIES 388-393) ASSUMED UNIT WEIGHT OF SOIL (Υ) = 70 lbs/ft' ANGLE OF INTERNAL FRICTION (Φ) = 36° COHESION (c) = 0 WALL FRICTION = 12° SHOULD THE CONTRACTOR ENCOUNTER SOIL OTHER THAN DESCRIBED ABOVE, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY.

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<u>PLAN - STAGE 2</u>

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

- 1. FOR TEMPORARY RAILROAD SHORING, SEE SPECIAL PROVISIONS.
- 2. ALL STEEL HP PILING, STEEL W SHAPES, STEEL PLATES, AND ANGLES SHALL BE ASTM A709 GR. 50, IN GOOD CONDITION.
- 3. ALL TIMBER SHALL BE GRADE NO.2 SOUTHERN PINE, IN GOOD CONDITION.
- 4. FOR EXCAVATABLE FLOWABLE FILL, SEE SECTION 1000-6 OF THE STANDARD SPECIFICATIONS.
- 5. CONTRACTOR SHALL DETERMINE EXACT LOCATION OF SHORING AND LIMITS OF EXCAVATION IN THE FIELD. IF SHORING DEPTHS OR RAILROAD CLEARANCES VIOLATE THESE PLANS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER.
- 6. CONTRACTOR SHALL VERIFY REQUIRED PILE LENGTHS PRIOR TO INSTALLATION.
- 7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ANY AND ALL ADDITIONAL OSHA AND STATE SAFETY REQUIREMENTS PERTAINING TO THIS EXCAVATION.
- 8. FOR DETAILS, SEE SHEETS 3,4, AND 5 OF 5.
- 9. CONTRACTOR SHALL VERIFY LOCATION OF EXISTING UTILITIES PRIOR TO START OF CONSTRUCTION AND NOTIFY THE ENGINEER IF THERE ARE ANY CONFLICTS.
- 10. ALL WELDING SHALL BE IN ACCORDANCE WITH CURRENT AWS STANDARDS AND PERFORMED BY A CERTIFIED WELDER.
- 11. CONCRETE FOR SHAFTS SHALL BE CLASS A AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 psi.
- 12. CONTRACTOR MAY SUBMIT FOR APPROVAL ALTERNATE SHORING PLANS AND CALCULATIONS. SHORING PLANS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE APPROVED PRIOR TO BEGINNING CONSTRUCTION.
- 13. ALL TIE BACKS SHALL BE A CEMENT GROUTED ANCHOR EMBEDDED INTO BEDROCK AND SHALL BE DESIGNED FOR A SERVICE LOAD OF 70 KIPS.CONTRACTOR SHALL SUBMIT ANCHOR DESIGN FOR APPROVAL PRIOR TO BEGINNING CONSTRUCTION.ANCHOR DESIGN SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN NORTH CAROLINA.
- 14. DIRECT DRAINAGE AWAY FROM FACE OF SHORING.
- 15. INSTALLED PILES SHALL BE WITHIN 1/8"/FT FROM VERTICAL.
- 16. TEMPORARY RAILROAD SHORING HAS BEEN DESIGNED FOR THE RETAINED SOIL PROPERTIES SHOWN ON THESE PLANS AND IN ACCORDANCE WITH AREMA CHAPTER 8 AND CSXT AND NORFOLK SOUTHERN REQUIREMENTS.

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900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991	1 2			3 4			total sheets 44

1. FOR NOTES, SEE SHEET 1 OF 5.

2. FOR DETAILS, SEE SHEETS 3,4, AND 5 OF 5.

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NOTES:

1. FOR NOTES SEE SHEET 1 OF 5.

2. FOR DETAILS SEE SHEET 5 OF 5.

CONSTRUCTION SEQUENCE SHORING 2 AND 3:

- 1. WITH SHORING 1 STILL IN PLACE AND PRIOR TO BACKFILLING BEHIND ABUTMENTS, PREDRILL SHAFT HOLE TO THE REQUIRED EMBEDMENT DEPTH SHOWN ON PLANS.
- 2. INSERT W 21×201 INTO PREDRILLED HOLES AND FILL WITH CONCRETE TO THE DEPTH SHOWN.
- 3. INSTALL STEEL LAGGING TO WITHIN 3'-O" OF FINISHED GROUND SURFACE. WELD LAGGING AS SHOWN IN THE STEEL LAGGING DETAIL.
- 4. INSTALL BENT CONNECTION PLATE AS SHOWN IN THE BENT CONNECTION PLATE DETAIL.
- 5. BACKFILL BEHIND SHORING IN 1'-O" MAXIMUM LIFTS. COMPACT SOIL BEHIND WALL PER CONTRACT DOCUMENTS. USE HAND COMPACTION NEAR SHORING. BACKFILL TO WITHIN 3'-O" OF FINISHED GROUND ELEVATION.
- 6. INSTALL TIMBER LAGGING AND PROCEED WITH BACKFILL/COMPACTION UNTIL FINISHED GRADE IS ATTAINED.
- 7. AFTER BACKFILL IS COMPLETED, EXCAVATE IN FRONT OF SHORING AS SHOWN ON SHEET 2 OF 5.

REMOVAL SEQUENCE SHORING 2 AND 3:

- 1. EXCAVATE 3'-O"AND REMOVE/CUT TIMBER LAGGING, UPPER PORTTION OF THE BENT CONNECTOR PLATE, W 21×201 TO 3'-O"BELOW FINISHED GROUND LINE.
- 2. BACKFILL EXCAVATED AREA TO FINISHED GROUND ELEVATION AND HAND COMPACT SOIL IN 1'-0" MAXIMUM LIFTS.

	PROJECT NO. <u>U-3308</u> <u>DURHAM</u> COUNTY STATION: <u>POT 24+09.63 -LALT-</u> POT 21+11.43 -NSN-						
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1. FOR NOTES SEE SHEET 1 OF 5.

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E. KELL 3/31/2015						
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