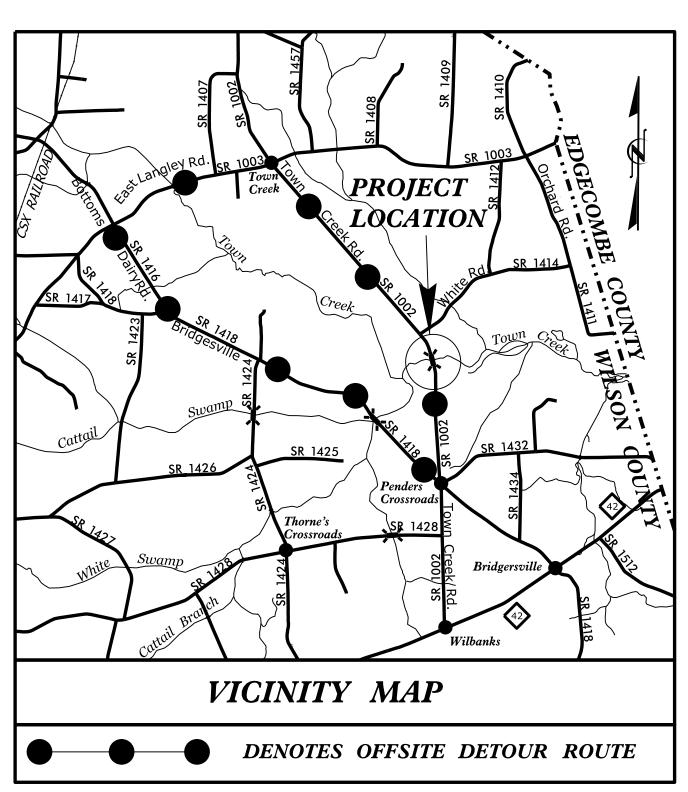
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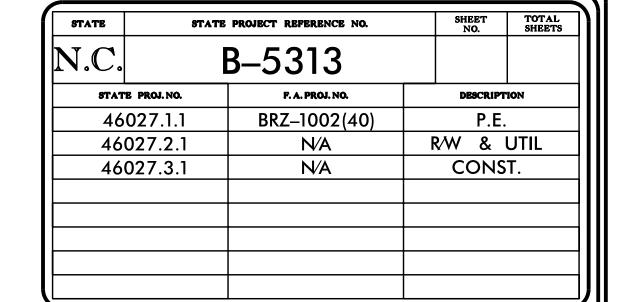


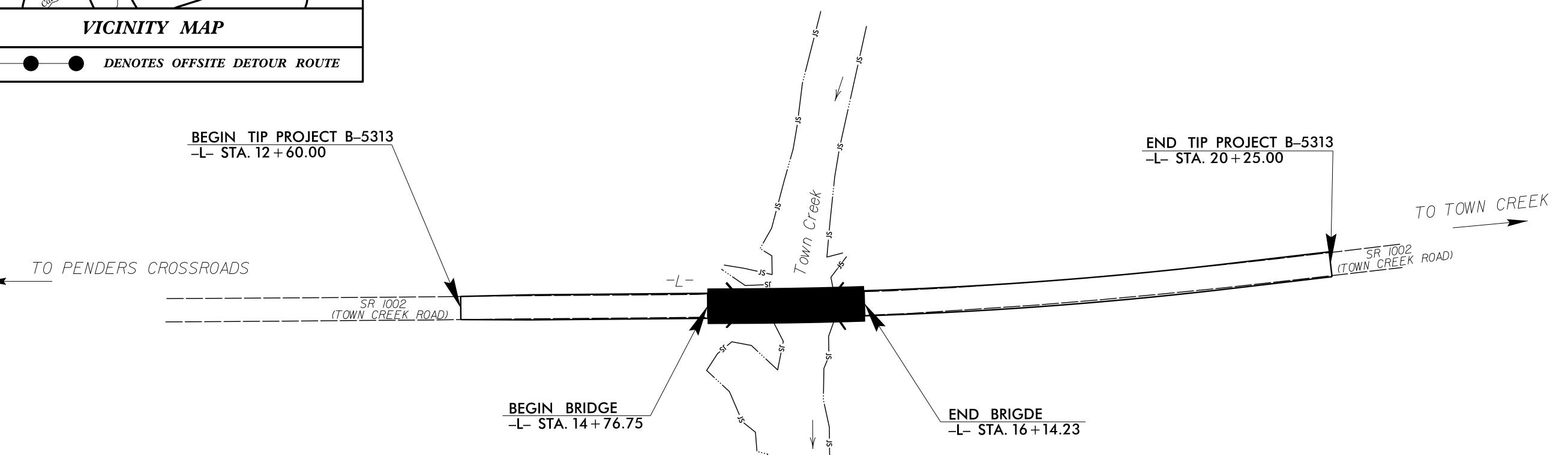
# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

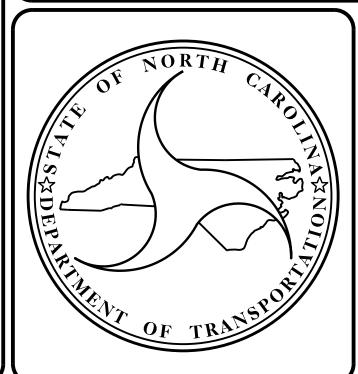
## WILSON COUNTY

LOCATION: BRIDGE NO. 109 OVER TOWN CREEK ON SR 1002 (TOWN CREEK ROAD)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE







STRUCTURE

#### DESIGN DATA

ADT (2016) = 653ADT (2036) = 913

K = 11 %

D = 65 %

T = 10 % \*\*

\* V = 60 MPH \*\* (TTST 2 %, DUAL 8 %)

FUNC CLASS=MINOR COLLECTOR

SUBREGIONAL TIER

#### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5313 = 0.119 MILES LENGTH STRUCTURE TIP PROJECT B-5313 = 0.026 MILES

TOTAL LENGTH TIP PROJECT B-5313 = 0.145 MILES

### Prepared in the Office of: **DIVISION OF HIGHWAYS**

STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.
RALEIGH, N.C. 27610

2012 STANDARD SPECIFICATIONS

LETTING DATE:

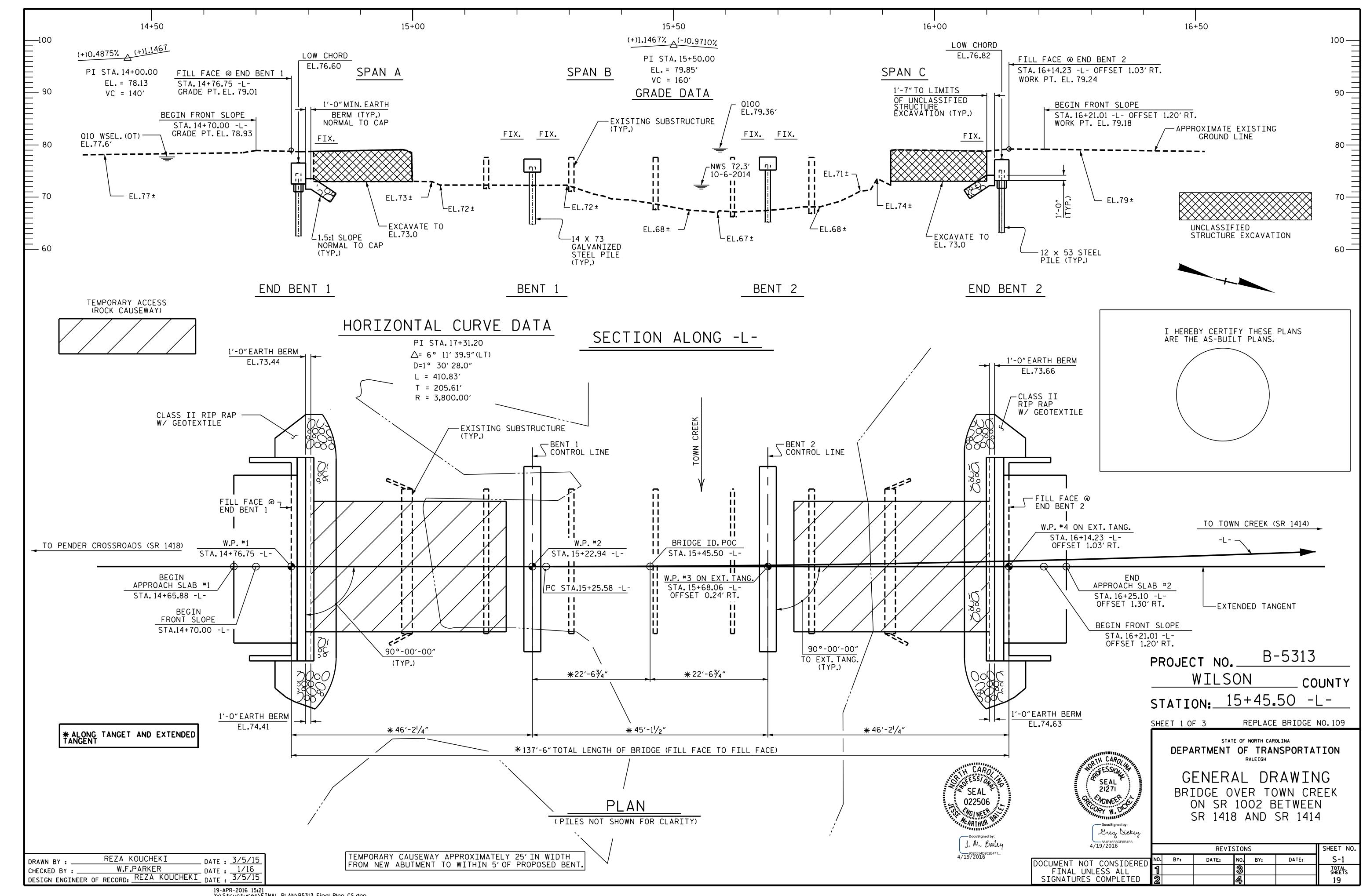
JUNE 21, 2016

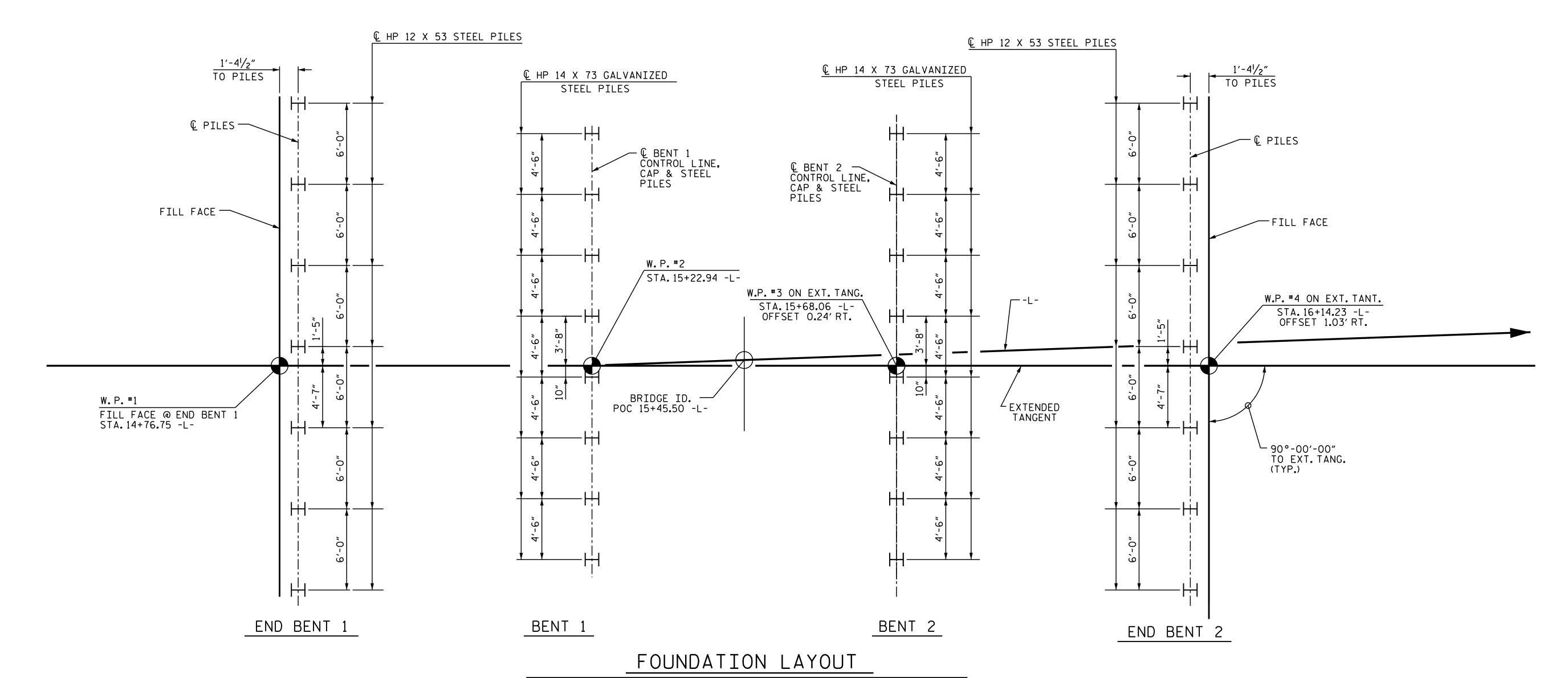
J.M. BAILEY, P.E.

PROJECT ENGINEER

G.W. DICKEY, P.E.

PROJECT DESIGN ENGINEER





DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE.

#### NOTES:

FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENTS 1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 65 TONS PER PILE.

DRIVE PILES AT END BENTS 1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 110 TONS PER PILE.

PILES AT BENTS 1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.

DRIVE PILES AT BENTS 1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 215 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR SCOUR.

INSTALL PILES AT BENTS 1 AND 2 TO A TIP ELEVATION NO HIGHER THAN 32 FT.

THE SCOUR CRITICAL ELEVATION FOR BENTS 1 AND 2 IS ELEVATION 59.0 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TESTING PILES WITH PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATION.

PROJECT NO. B-5313

WILSON COUNTY

STATION: 15+45.50 -L-

SHEET 2 OF 3

SEAL 21271

CONVERTED ON THE PROPERTY OF THE P

DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

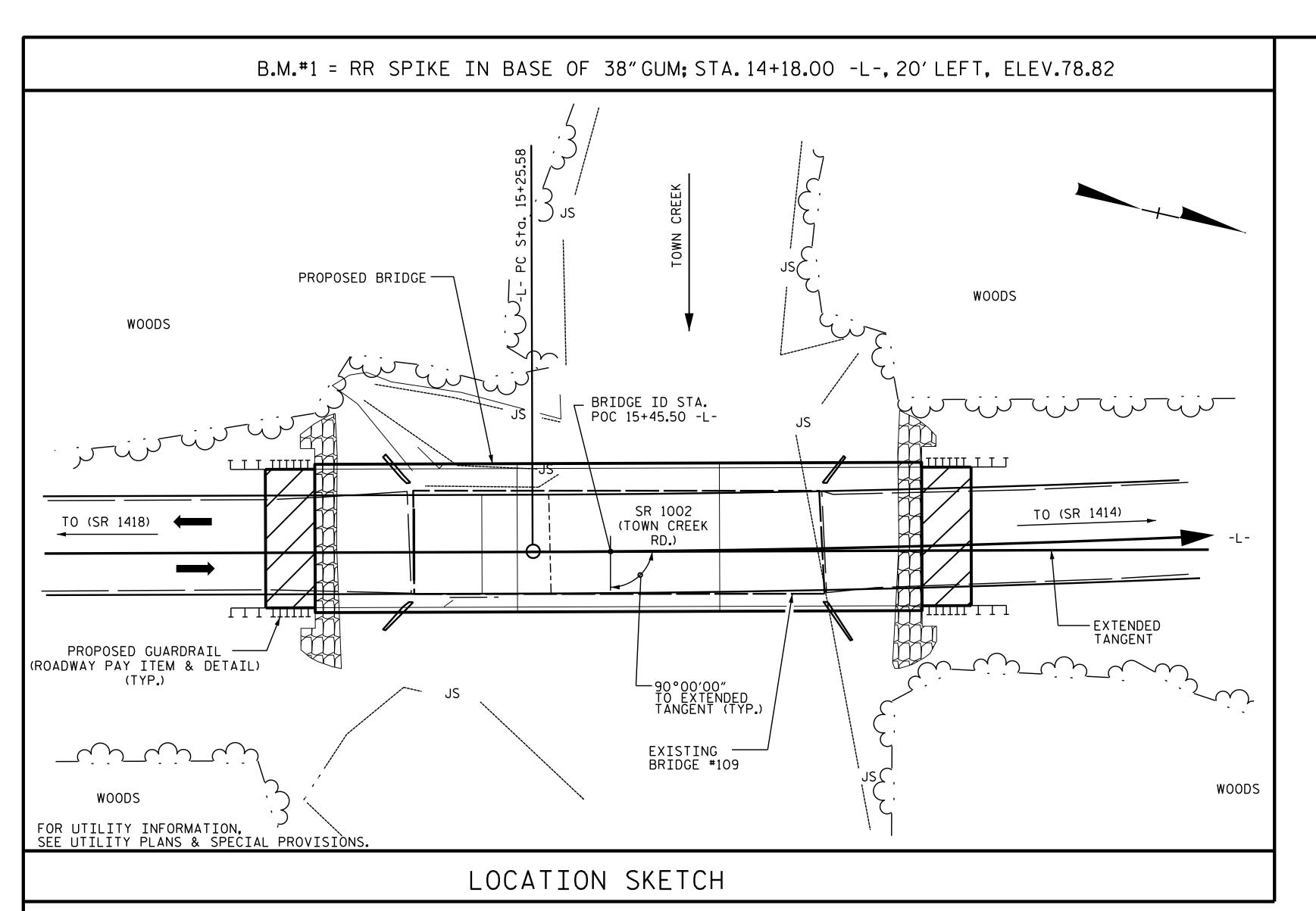
BRIDGE OVER TOWN CREEK

STATE OF NORTH CAROLINA

BRIDGE OVER TOWN CREE ON SR 1002 BETWEEN SR 1418 AND SR 1414

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 4 19

DRAWN BY: REZA KOUCHEKI
CHECKED BY: W.F. PARKER
DESIGN ENGINEER OF RECORD: REZA KOUCHEKI
DATE: 8/26/15
DATE: 8/26/15



#### NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION STATION 15+45.50 -L-ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIY ON ROADWAY PLANS.

#### HYDRAULIC DATA

DESIGN DISCHARGE = 2191 CFS
FREQUENCY OF DESIGN FLOOD = 10 YRS
DESIGN HIGH WATER ELEVATION = 77.60
DRAINAGE AREA = 52.9 SQ. MI.
BASE DISCHARGE (Q100) = 4485 CFS
BASE HIGH WATER ELEVATION = 79.36

#### OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 2191 CFS FREQUENCY OF OVERTOPPING FLOOD = 10 ± YRS OVERTOPPING FLOOD ELEVATION = 77.5

(APPROX. STA. 12+90.0 -L-)

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 15'-8", 1 SPAN @ 15'-0", 1 SPAN @ 15'-2", 1 SPAN @ 14'-10", 1 SPAN @ 15'-0", AND 1 SPAN @ 15'-8"; TIMBER CAPS & PILES; REINFORCED CONCRETE DECK ON TIMBER JOISTS WITH 3" AWS AND CLEAR ROADWAY WIDTH OF 23'-0", SHALL BE REMOVED.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 32 FT LEFT AND RIGHT OF CENTERLINE ROADWAY, AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR INTERIOR BENTS 1 AND 2, ONLY PARTIAL GALVANIZING OF THE PILES IS REQUIRED. SEE INTERIOR BENTS SHEETS FOR REQUIRED GALVANIZING LENGTHS. PAYMENT FOR PARTIALLY GALVANIZED PILES WILL BE MADE UNDER THE CONTRACT UNIT PRICE FOR GALVANIZED STEEL PILES.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

							- TO	TAL	BILL	OF	MAT	ERIAL							
	CONSTRUCTION, MAINTENANCE, AND REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL		12 X 53 EL PILES	HP 14 GALVAN STEEL	X 73 IIZED PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE:	O'' X 1'-9'' STRESSED CRETE ED SLABS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ. YD.	LUMP SUM	NO.	LIN.FT.	LUMP SUM
SUPERSTRUCTURE	-					LUMP SUM							270.75			LUMP SUM	33	1485.00	
END BENT 1				LUMP SUM	21.6		2636	7	455			3		60	66				
BENT 1					10.7		2136			8	640	4							
BENT 2					10.7		2136			8	640	4							
END BENT 2				LUMP SUM	21.6		2636	7	420			3		65	72				
TOTAL	LUMP SUM	LUMP SUM	2	LUMP SUM	64.6	LUMP SUM	9544	14	875	16	1280	14	270.75	125	138	LUMP SUM	33	1485.00	LUMP SUM

PROJECT NO. B-5313

WILSON COUNTY

STATION: 15+45.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

BRIDGE OVER TOWN CREEK ON SR 1002 BETWEEN SR 1418 AND SR 1414

REVISIONS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

2

REVISIONS

SHEET NO.

BY: DATE: NO. BY: DATE: S-3

TOTAL SHEETS

19

DRAWN BY: GHOLAMREZA KOUCHEKI DATE: 8/26/15
CHECKED BY: W.F. PARKER DATE: 1/16
DESIGN ENGINEER OF RECORD: G.KOUCHEKI DATE: 8/26/15

#### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) ROLLING RATING MINIMUN RATING (RF) GIRDER CONT DIST, LEFT SPAN DIST, LEFT SPAN STI CT( DI FA $_{ m CI}$ 1.088 1.75 0.277 0.539 1.23 1.09 EL 22 45′ 2.2 45′ 22 HL-93(Inv)N/A1.34 45′ 0.80 0.277 1.590 0.539 1.59 HL-93(Opr)1.35 0.277 1.74 45′ EL 22 45′ 2.2 N/A N/A EL DESIGN LOAD 36.000 48.104 0.539 1.45 1.34 45′ 45′ 22 HS-20(Inv) 2 1.336 1.75 0.277 1.65 45′ EL 22 2.2 0.80 0.277 RATING 0.539 2.2 HS-20(0pr) 36.000 1.882 67.763 1.35 0.277 2.14 45′ EL 22 1.88 45′ N/A EL 13.500 2.611 35.252 0.277 4.02 0.539 4.01 0.277 2.61 45′ EL 45′ 45′ 22 SNSH 22 EL 2.2 0.80 EL 0.539 20.000 2.108 42.166 0.277 3.25 2.94 45′ 0.277 45′ SNGARBS2 45′ EL 22 EL 2.2 0.80 2.11 22 17.6 0.539 22.000 2.067 45.466 0.277 3.15 2.77 0.277 2.07 22 SNAGRIS2 45′ EL 45′ 2.2 0.80 45′ EL 0.539 27.250 35.527 0.277 22 2.01 2.2 0.277 1.30 45′ 22 SNCOTTS3 1.304 2.01 45′ EL 0.80 1.4 EL 34.925 1.150 40.181 0.277 45′ 0.539 1.74 45′ 0.80 0.277 1.15 45′ 22 SNAGGRS4 1.77 EL 22 2.2 EL 35.550 0.277 0.539 1.79 1.121 EL 22 45′ 2.2 45′ 22 SNS5A 39.841 1.73 45′ EL 0.80 0.277 1.12 39.950 1.056 42.175 0.277 1.63 0.539 1.67 0.277 1.06 SNS6A 45′ EL 22 45′ 2.2 45′ 22 EL 0.80 0.539 22 SNS7B 42.000 42.268 0.277 1.55 45′ EL 22 1.68 45′ 2.2 0.80 0.277 1.01 45′ 1.006 EL LEGAL LOAD 0.539 1.96 33.000 42.759 45′ 1.30 TNAGRIT3 1.296 0.277 45′ EL 22 EL 2.2 0.80 0.277 45′ 22 RATING 0.539 1.88 0.277 1.31 TNT4A 33.075 1.309 43.305 0.277 2.02 45′ EL 22 45′ EL 2.2 0.80 45′ EL 22 TNT6A 41.600 1.099 45.712 0.277 1.69 45′ EL 22 0.539 1.83 45′ 2.2 0.80 0.277 1.10 45′ EL 22 1.4 EL 42.000 0.277 45′ EL 0.539 1.69 45′ 2.2 0.277 1.12 45′ 22 TNT7A 1.120 47.043 1.73 22 0.80 EL 0.539 1.61 42.000 1.166 48.975 0.277 1.8 45′ 45′ 2.2 0.80 0.277 1.17 45′ 22 TNT7B 1.4 EL 22 EL 0.539 1.55 43.000 1.111 47.757 0.277 45' 0.277 45′ 22 TNAGRIT4 1.71 45′ EL 22 2.2 1.4 0.80 1.11 EL 1.033 46.505 0.277 0.539 1.59 1.03 45.000 1.59 EL 22 2.2 0.80 0.277 45′ 22 TNAGT5A 45′ EL

LOAD FACTORS:

	DESIGN LOAD RATING	LIMIT STATE	<sup>9</sup> DC	g <sub>D M</sub>
		STRENGTH I	1.25	1.50
	FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

••

4

22

45′

1.01

2.2

0.80 0.277

Controlling Load Rating

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

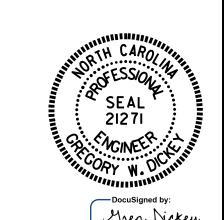
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5313

WILSON COUNTY

STATION: 15+45.50 -L-



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

LRFR SUMMARY FOR

45' CORED SLAB UNIT

90° SKEW

(NON-INTERSTATE TRAFFIC)

REVISIONS SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 2 4 19

1 2 3

1.4 0.277

1.56

45′

22 0.539

1.47

FOR SPAN 'A', 'B', & 'C'

ASSEMBLED BY: REZA KOUCHEKIDATE: 11/15
CHECKED BY: W.F.PARKER DATE: 11/15
DRAWN BY: CVC 6/10

CHECKED BY : DNS 6/10

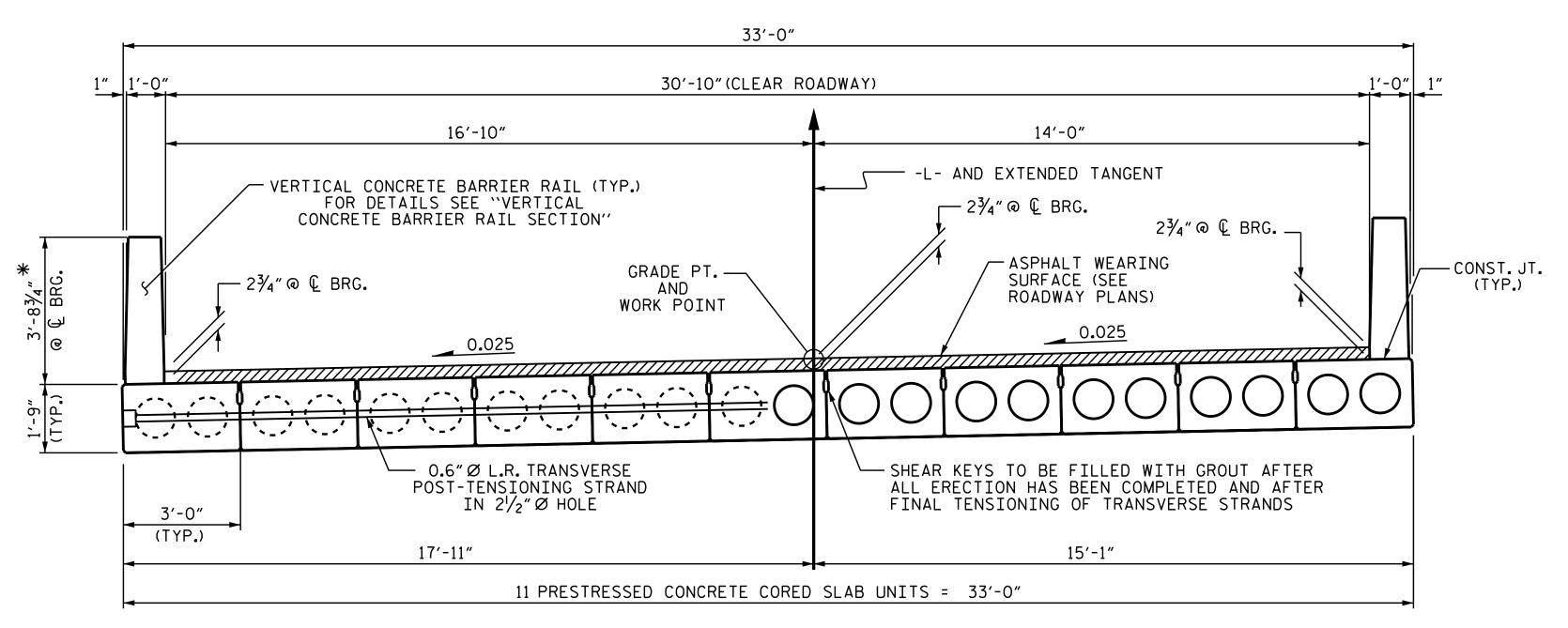
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1.009

45.408

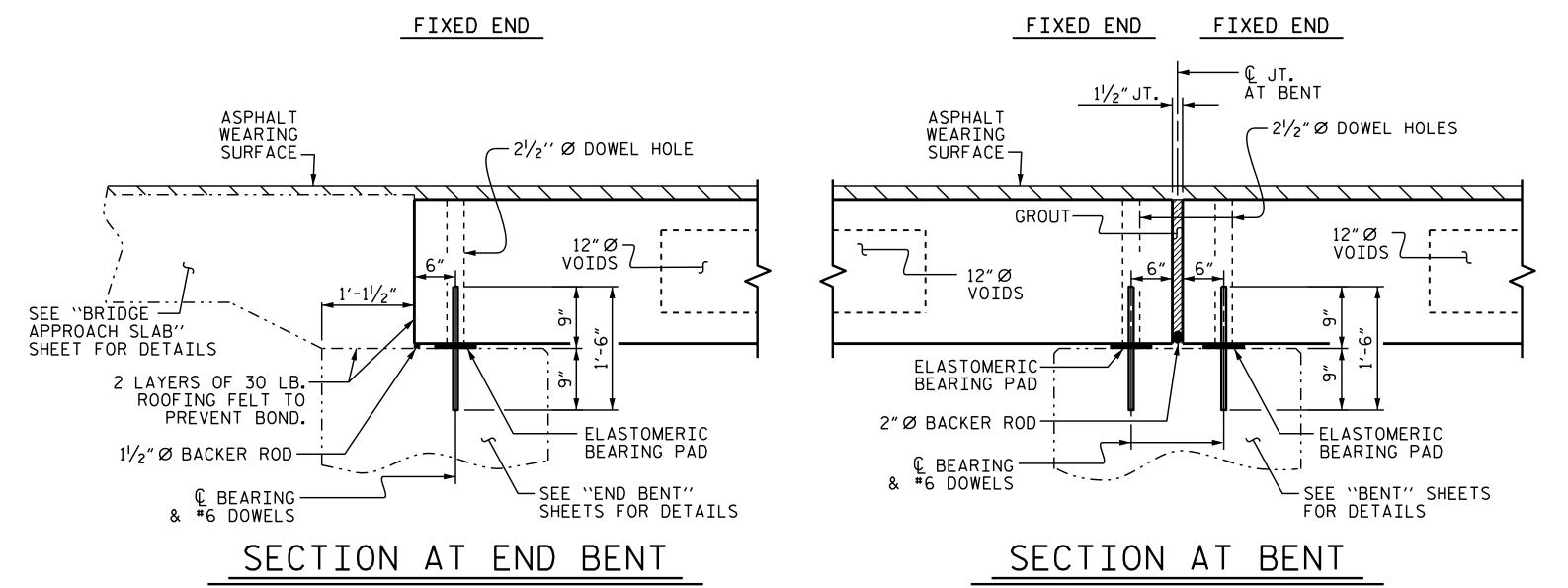
45.000

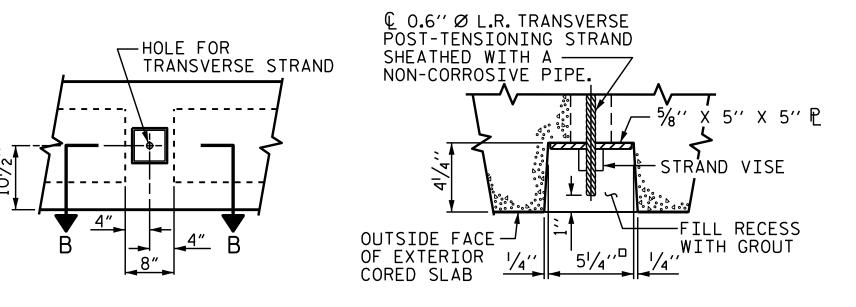
TNAGT5B



#### TYPICAL SECTION

\*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

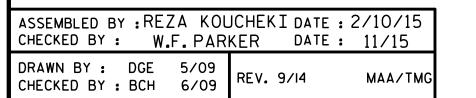


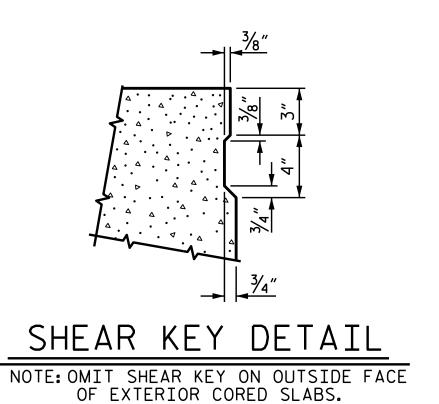


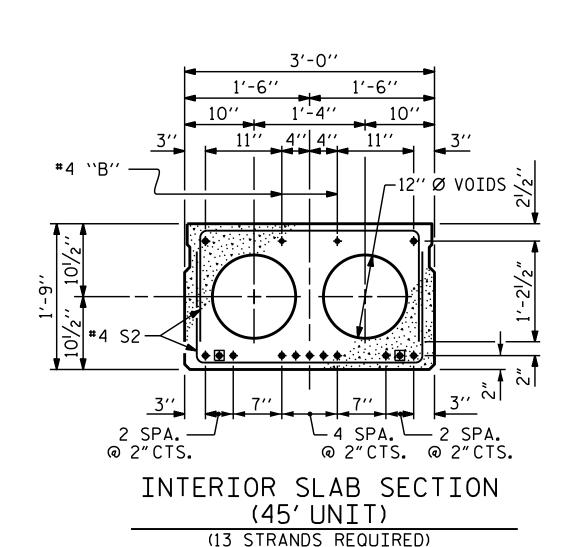
ELEVATION VIEW

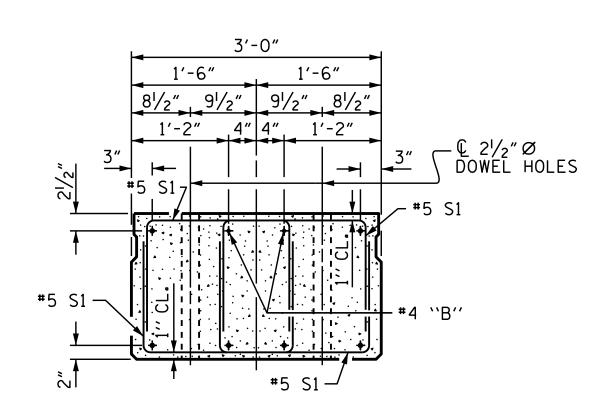
SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS



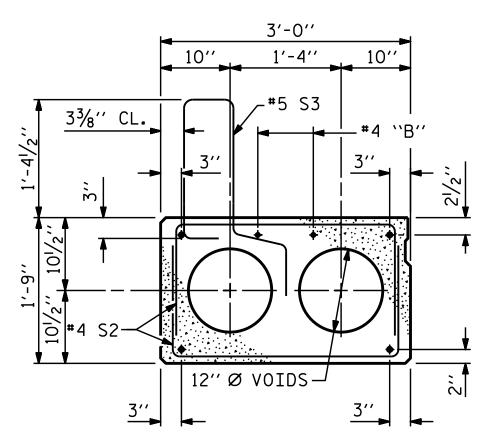






## END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

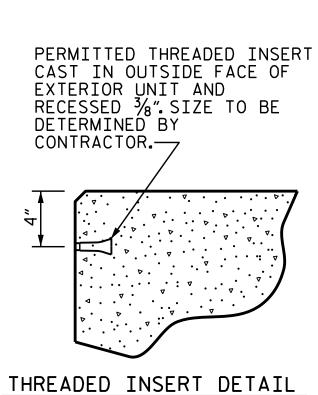


EXT. SLAB SECTION (FOR PRESTRESSED STRAND LAYOUT, SEE

INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNI SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

#### DEBONDING LEGEND



PROJECT NO. B-5313 WILSON COUNTY STATION: 15+45.50 -L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

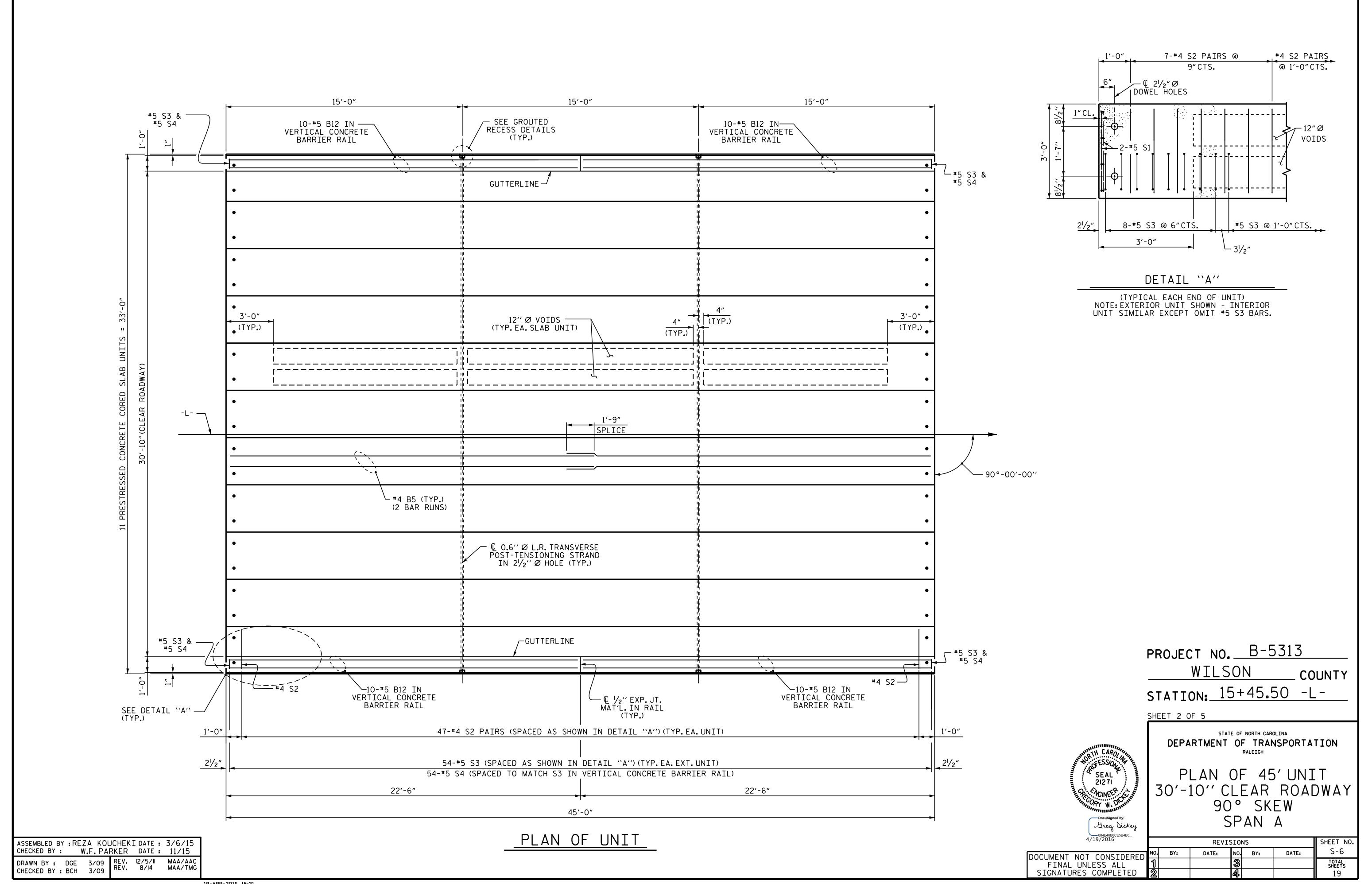
3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

DOCU SIG

21271 : MOINEER

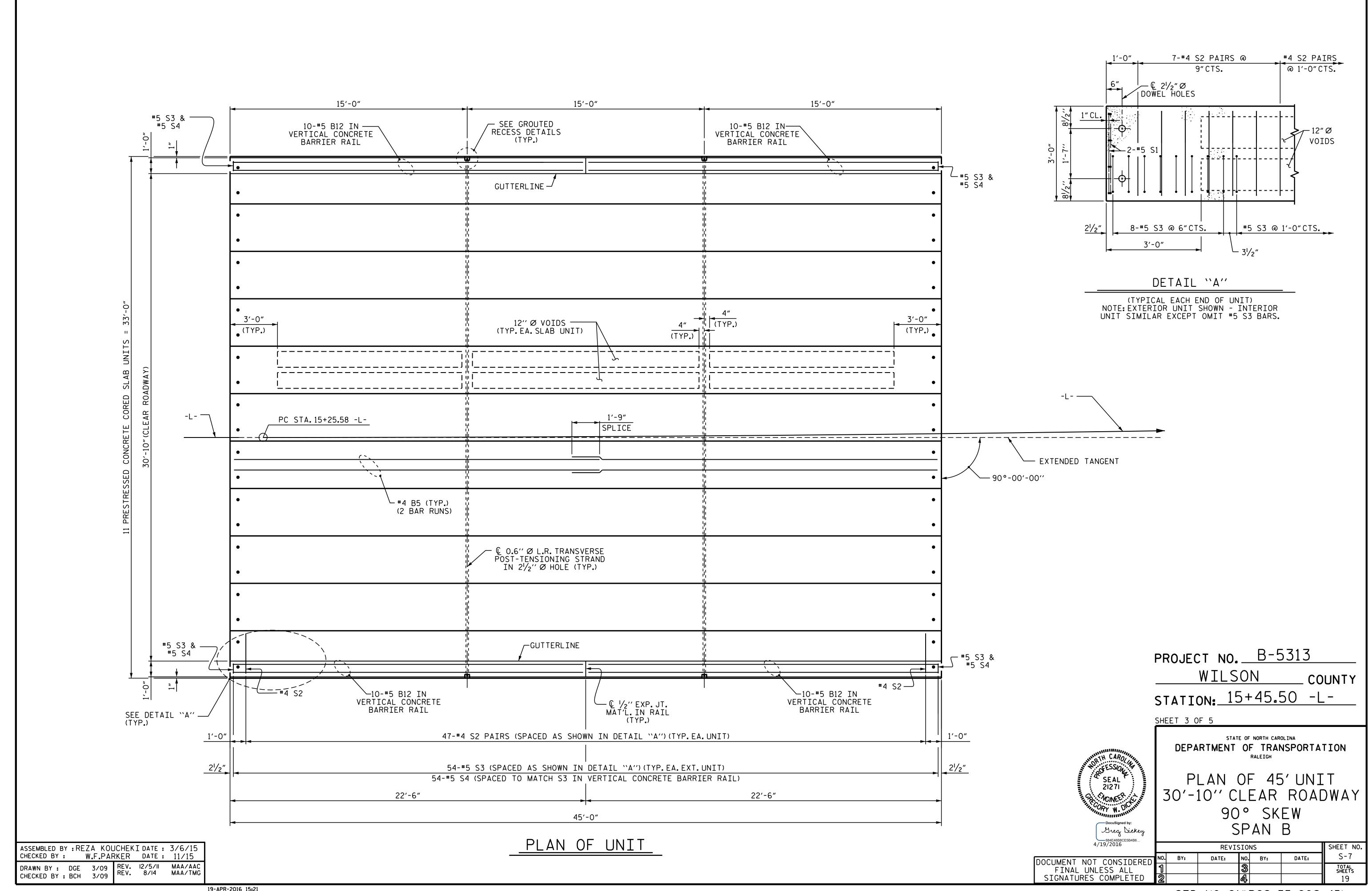
Greg Dickey

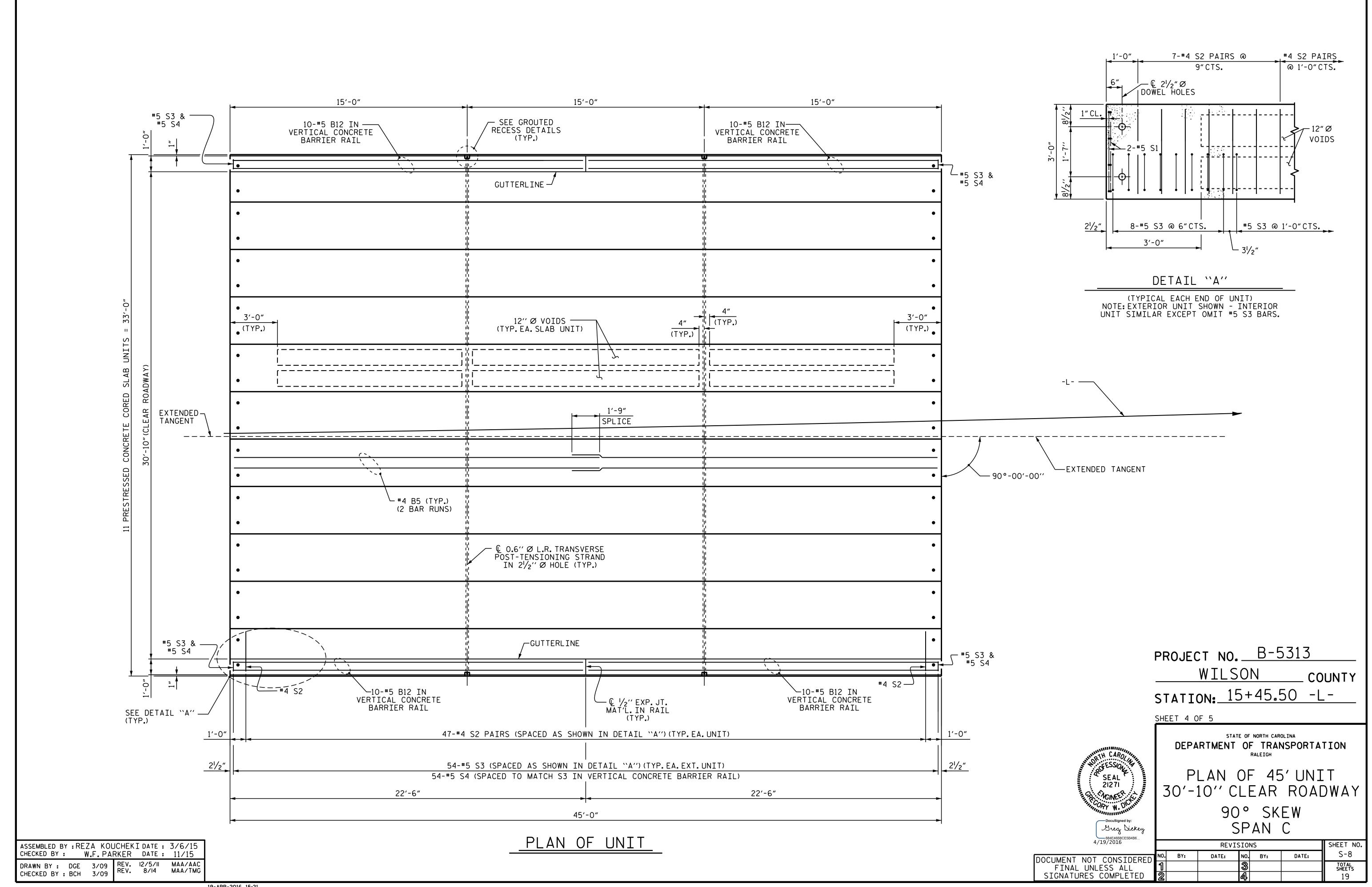
884E46B8CE5B4B6									
4/19/2016		REVISIONS							
JMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-5		
FINAL UNLESS ALL	1			3			TOTAL SHEETS		
GNATURES COMPLETED	2			4			19		

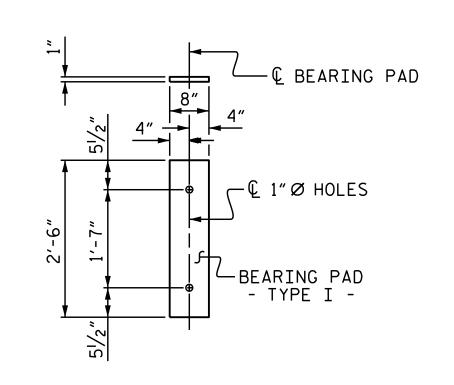


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STD. NO. 21" PCS\_33\_90S\_45L







FIXED END (TYPE I - 66 REQ'D)

#### ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

10"

-#5 S4

(TYP.)

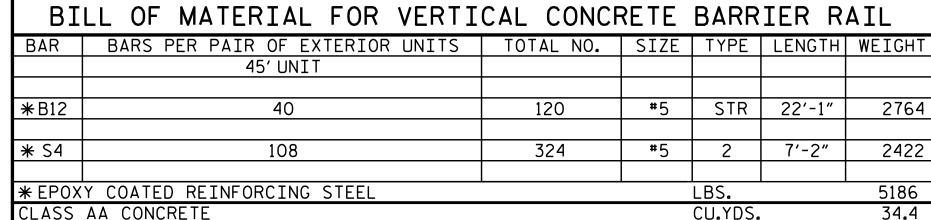
 $2\frac{3}{8}$ " CL.

-#5 S3 X

VERTICAL DIM. VARIE

VERTICAL CONCRETE BARRIER RAIL SECTION

2"CL.MIN.



BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT									
EXTERIOR UNIT   INTERIOR UNIT							OR UNIT		
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
B5	4	#4	STR	23'-3"	62	23'-3"	62		
S1	8	#5	3	4'-3"	35	4'-3"	35		
S2	94	#4	3	5′-4″	335	5′-4″	335		
* S3	54	#5	1	5′-7″	314				
REINFO	ORCING :	STEEL	LB:	5.	432		432		
	Y COATE								
REIN	IFORCING	STEEL	LB:	S <b>.</b>	314				
5000 F	P.S.I.CO	NCRETE	CU. YDS	ò.	6 <b>.</b> 5		6 <b>.</b> 5		
0.6"Ø	L.R. STR	ANDS	No	) <b>.</b>	13		13		

LN.FT.

CORED	SLABS REQUIRED						
	NUMBER	LENGTH	TOTAL LENGTH				
45' UNIT							
EXTERIOR C.S.	6	45'-0"	270'-0"				
INTERIOR C.S.	27	45'-0"	1215'-0"				
ΤΟΤΔΙ	33	45'-0"	1485'-0"				

## 2 270.75 73/4" 2'-8'' ALL BAR DIMENSIONS ARE OUT TO OUT

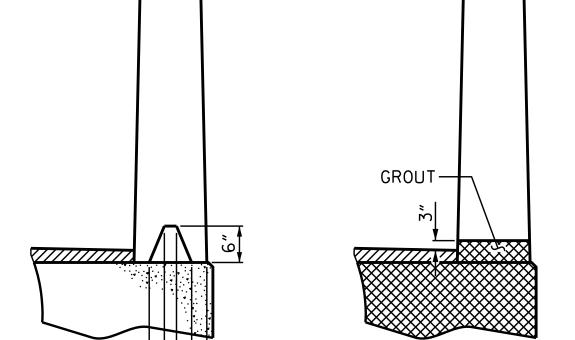
BAR TYPES

DEAD LOAD DEFLECTION AND	ND CAMBER
	3'-0" × 1'-9"
45' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	7⁄8″ ∮
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	<sup>1</sup> ∕8″ <b>†</b>
FINAL CAMBER	3⁄4″ ∤

\*\* INCLUDES FUTURE WEARING SURFACE

ASPHALT OVERLAY THICKNESS

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT



TOTAL VERTICAL CONCRETE BARRIER RAIL

SECTION T-T

AT OPEN JOINT AT BENT (THIS IS TO BE USED WHERE FOAM JOINT IS NOT USED) SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED) WHEN SLIP FORM IS USED) © OPEN JT.IN →,
RAIL @ BENT →, HAMFE CHAMFER ¾″ CHAMFER CHAMFER #5 S3 (SEE "PLAN OF UNIT" FOR SPACING) ELEVATION AT EXPANSION JOINTS

	ī.		
RANDS			
0.6"Ø L.R.		CONCRETE	RE
0.217			

#### @ MID-SPAN @ MID-SPAN 3′-8″

RAIL HEIGHT

CONCRETE RELEA	ASE STRENGTH
UNIT	PSI
45' UNITS	4000

#### NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

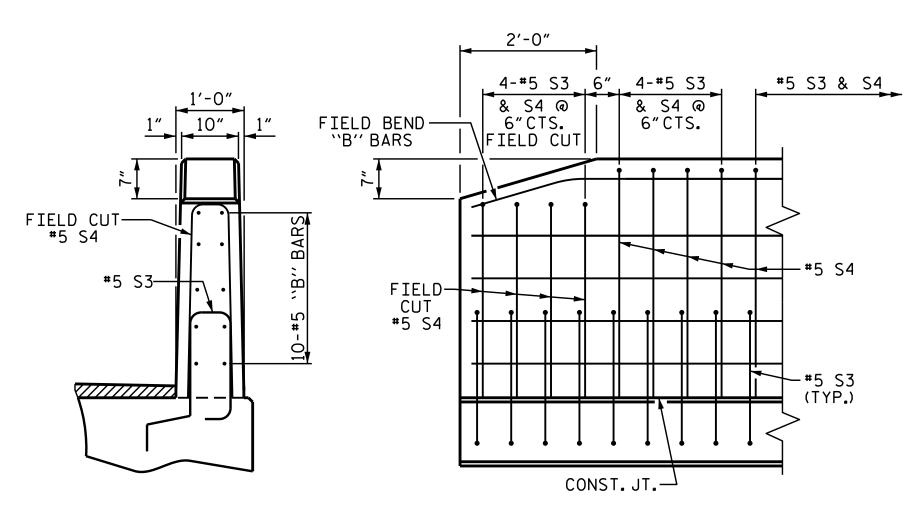
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.



END VIEW

45' UNITS

58,600

43,950

GRADE 270 STRANDS

AREA

(SQUARE INCHES)

ULTIMATE STRENGTH (LBS.PER STRAND)

APPLIED PRESTRESS

(LBS.PER STRAND

SIDE VIEW

END OF RAIL DETAILS

B-5313 PROJECT NO. \_ WILSON COUNTY STATION: 15+45.50 -L-

SHEET 5 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0'' X 1'-9'' PRESTRESSÉD CONCRETE CORED SLAB UNIT 90° SKEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

FESSION,

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NOINEER

Greg Dickey

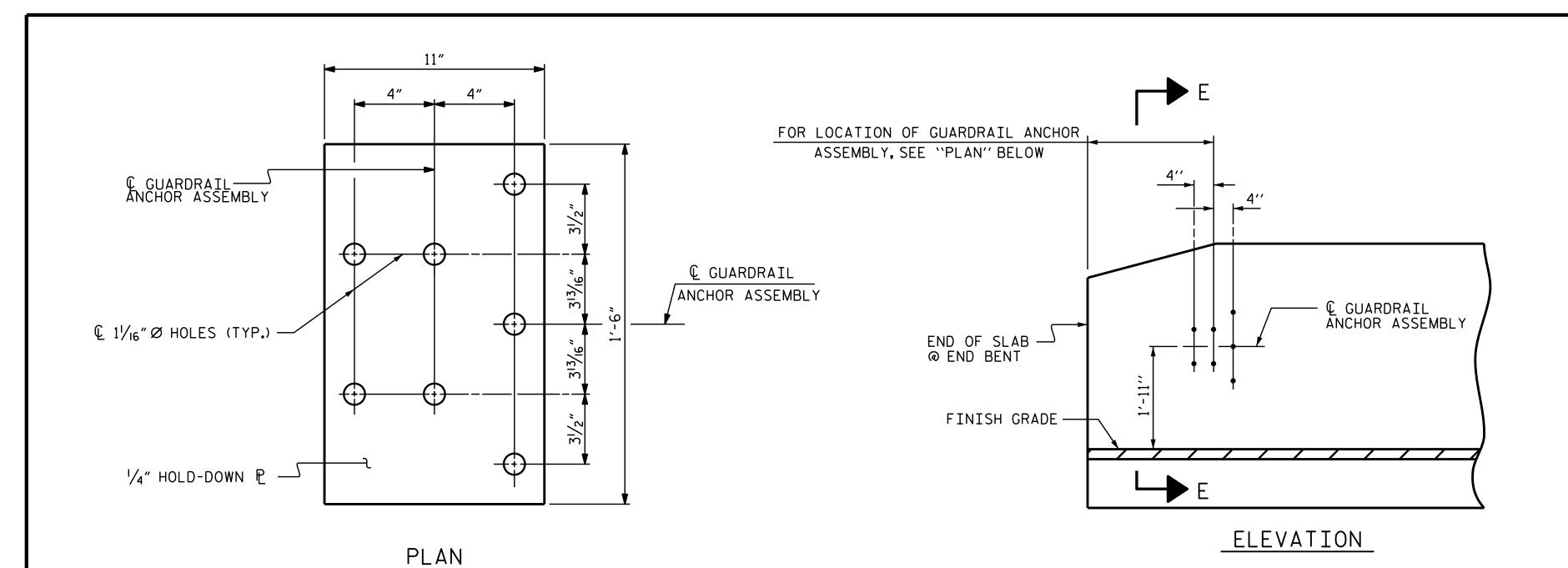
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ASSEMBLED BY : REZA KOUCHEKI DATE : 2/10/15 CHECKED BY: W.F. PARKER DATE : 11/15 DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09
REV. II/14

CONST. JT. —

3′-8¾″ 'CUTTERLINE / RAIL HEIGHT'

VARIES THICKNE



#### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 1/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $7/8^{\prime\prime}$  Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

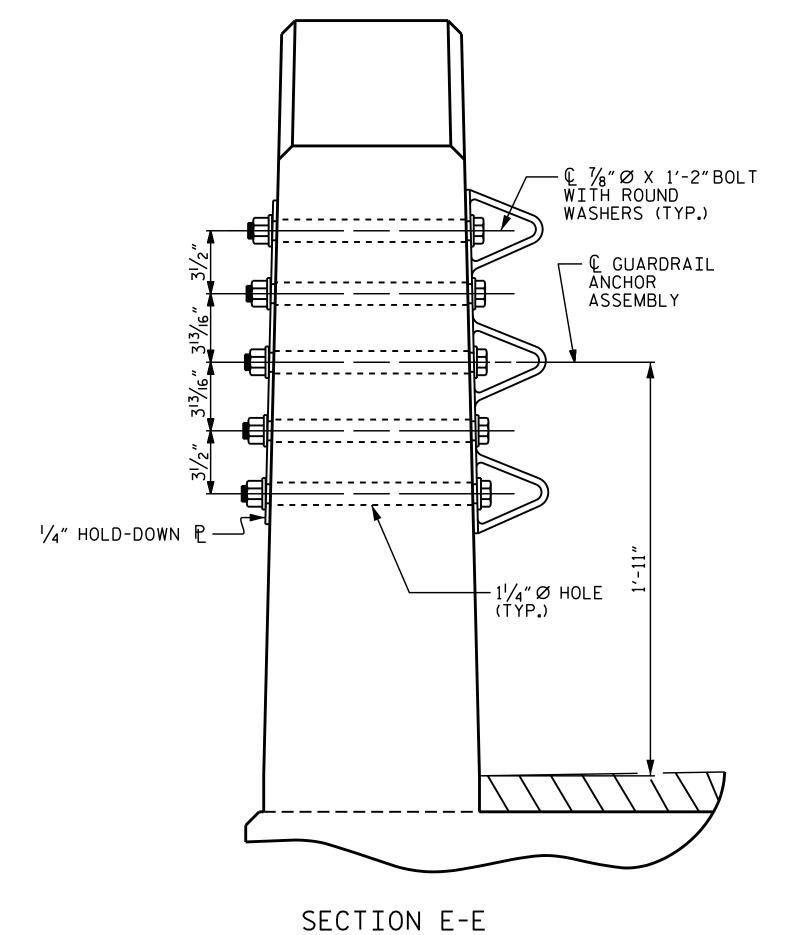
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

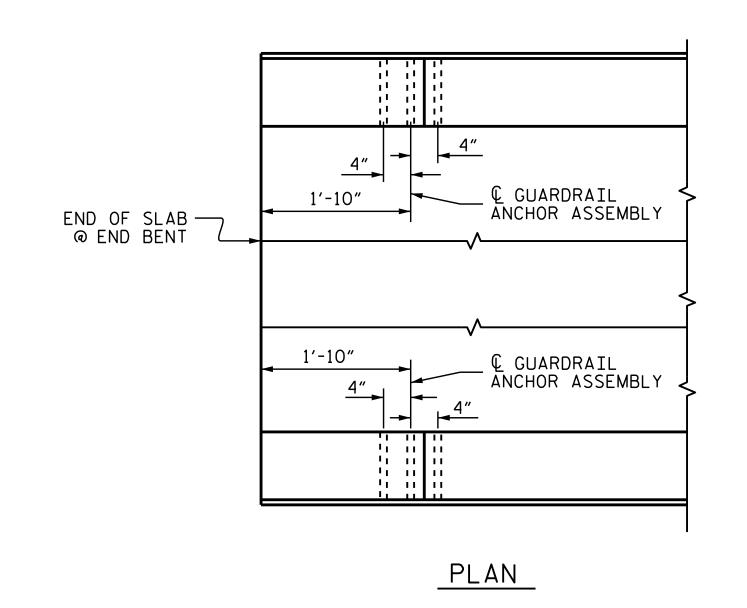


GUARDRAIL ANCHOR ASSEMBLY DETAILS

ASSEMBLED BY : REZA KOUCHEKI DATE : 2/10/15 CHECKED BY : W.F.PARKER DATE : 11/15

REV. 1/15

DRAWN BY : MAA 5/10 CHECKED BY : GM 5/10 MAA/GM MAA/GM MAA/TMG



LOCATION OF ANCHORS FOR GUARDRAIL

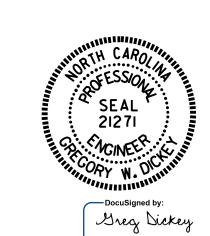
END BENT #1 SHOWN, END BENT #2 SIMILAR.



#### SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

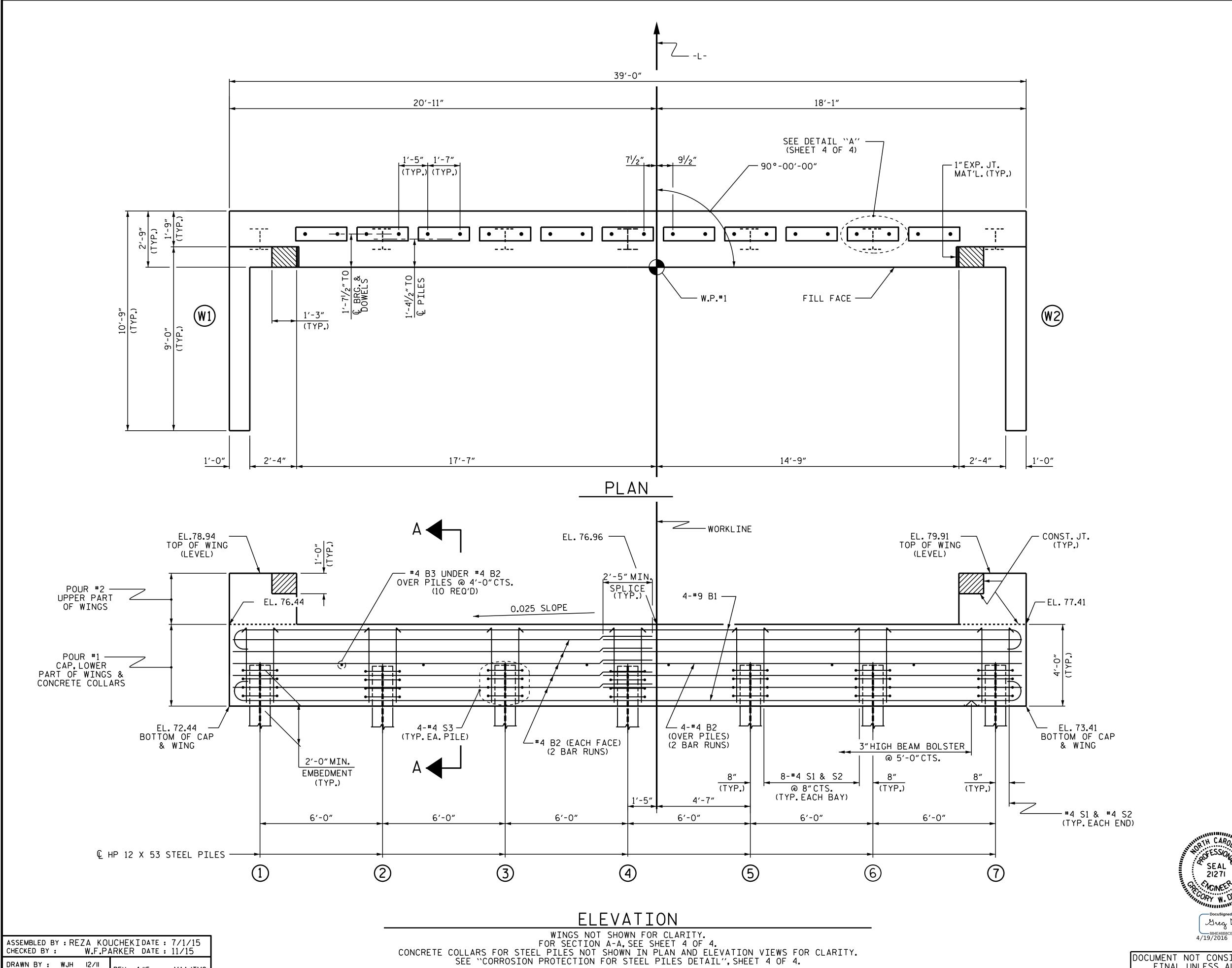
B-5313 PROJECT NO. WILSON \_ COUNTY 15+45.50 -L-STATION:\_



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP ELE	OF PILE VATIONS
1	74.48
2	74.63
3	74 <b>.</b> 78
4	74.93
5	75 <b>.</b> 07
6	75 <b>.</b> 22
7	75 <b>.</b> 37

B-5313 PROJECT NO.\_ WILSON \_ COUNTY

STATION: 15+45.50 -L-

SHEET 1 OF 4

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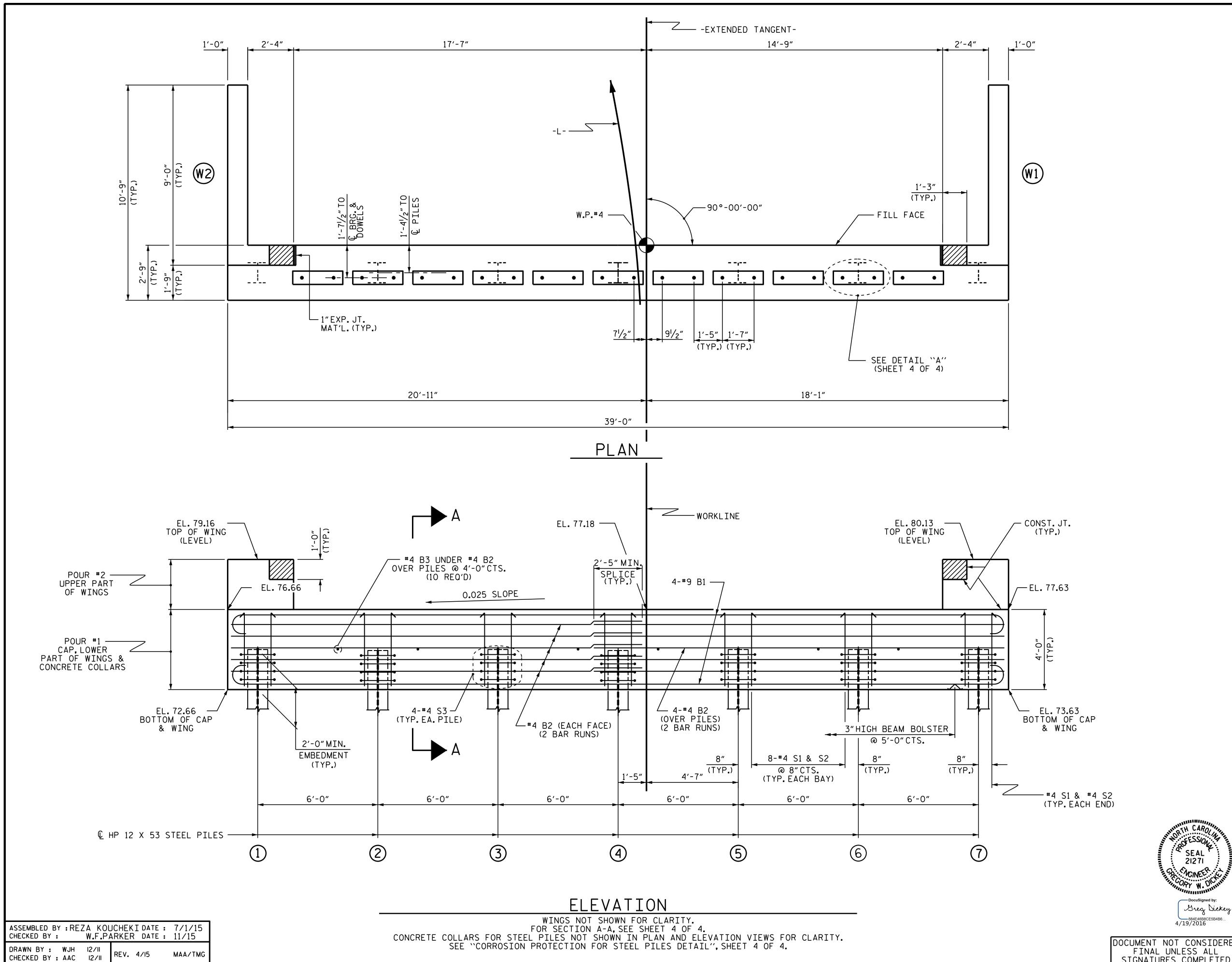
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

END BENT No. 1

Greg Dickey 884E46B8CE5B4B6... 4/19/2016 SHEET NO. **REVISIONS** S-11 DATE: DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 19

DRAWN BY: WJH 12/II
CHECKED BY: AAC 12/II
REV. 4/15



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP OF PILE ELEVATIONS

1 74.68
2 74.83
3 74.98
4 75.13
5 75.27
6 75.42
7 75.59

PROJECT NO. B-5313
WILSON COUNTY

STATION: 15+45.50 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

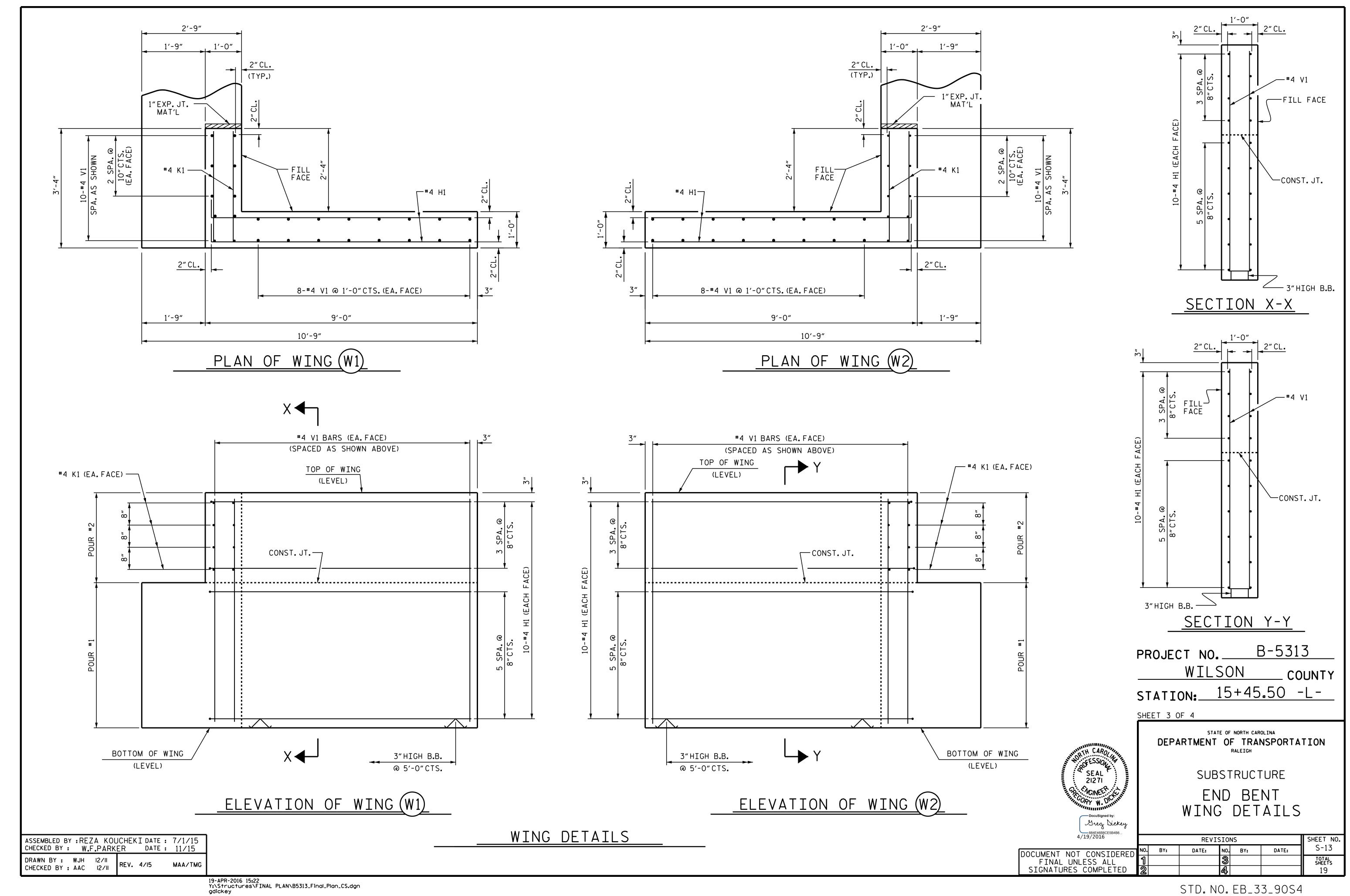
RALEIGH

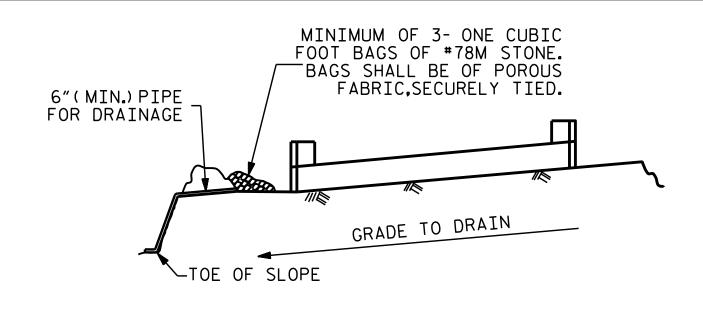
SUBSTRUCTURE

END BENT No. 2

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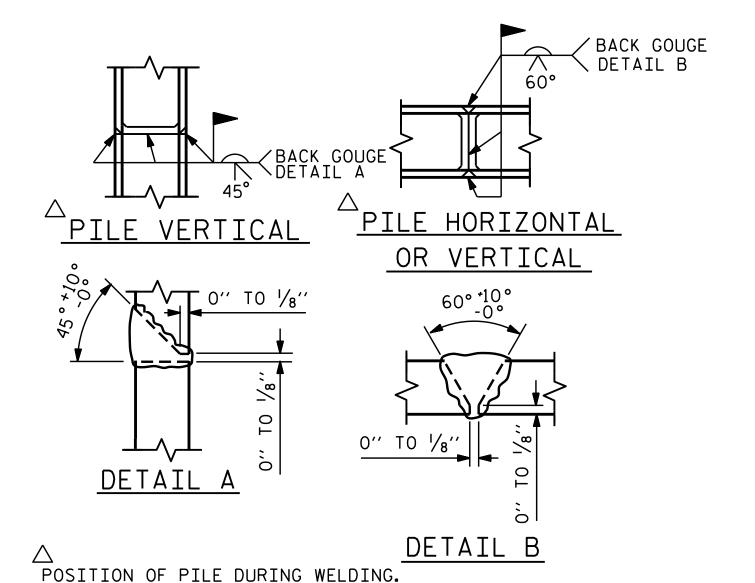


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

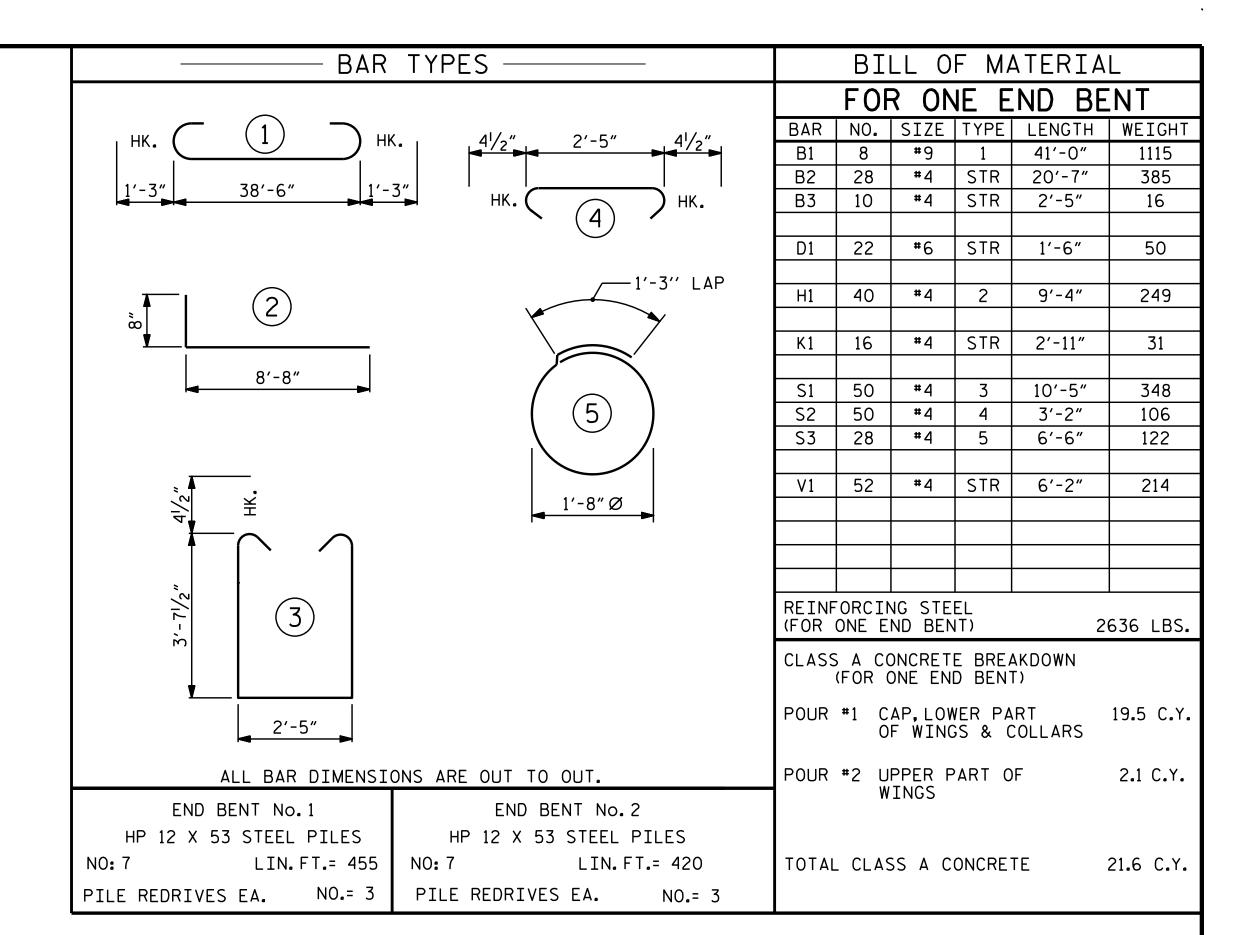
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

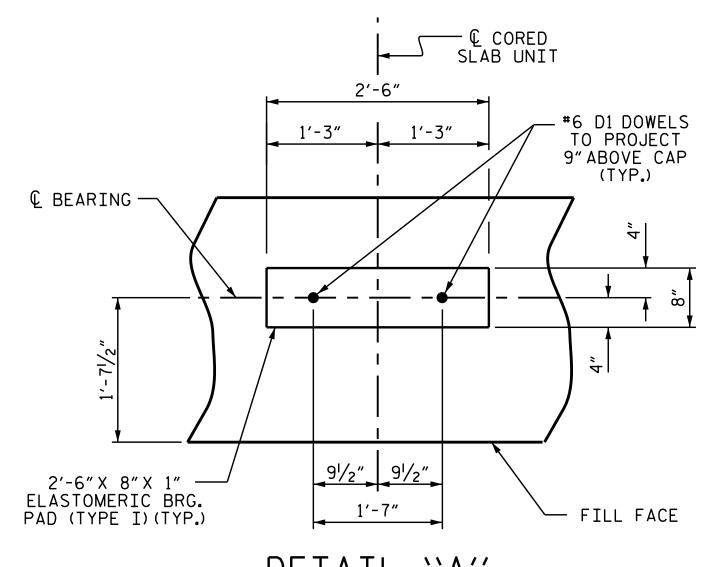
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

#### TEMPORARY DRAINAGE AT END BENT

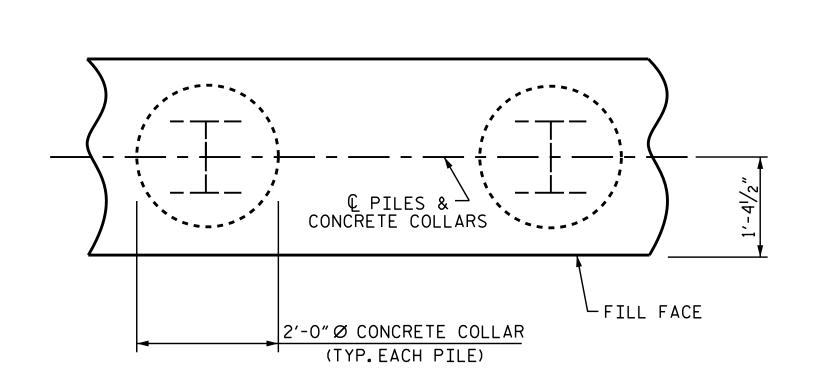


PILE SPLICE DETAILS





DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



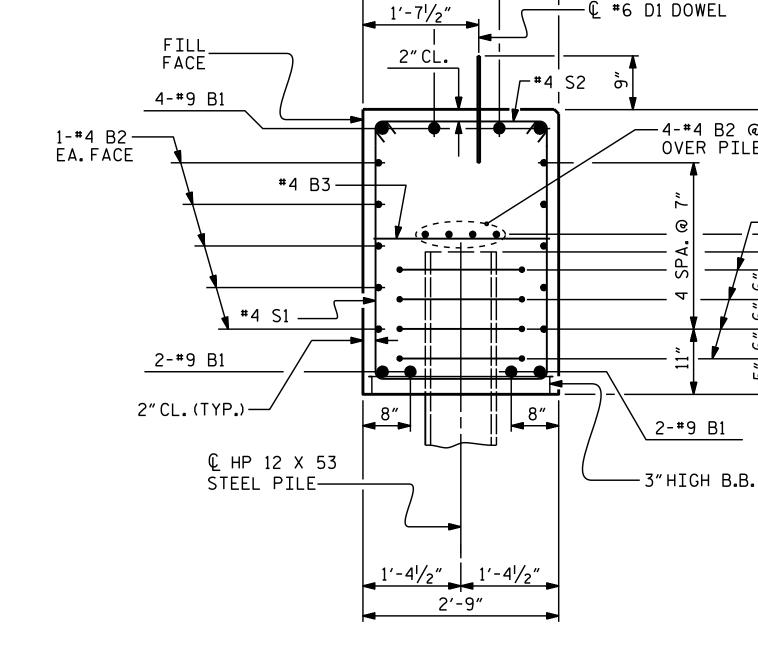
PLAN

| | | | | CONCRETE — COLLAR BOTTOM OF CAP © HP 12 X 53 STEEL PILE 2'-0" ELEVATION

CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

ASSEMBLED BY : REZA KOUCHEKI DATE : 7/1/15 CHECKED BY: W.F.PARKER DATE: 11/15 DRAWN BY: WJH 12/11 CHECKED BY : AAC 12/11



SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

B-5313 PROJECT NO.\_ WILSON \_ COUNTY <u>15+4</u>5.50 -L-STATION:\_ SHEET 4 OF 4 STATE OF NORTH CAROLINA

> RALEIGH SUBSTRUCTURE

END BENT No.1 & 2

DEPARTMENT OF TRANSPORTATION

DETAILS Greg Dickey 884E46B8CE5B4B6. 4/19/2016 REVISIONS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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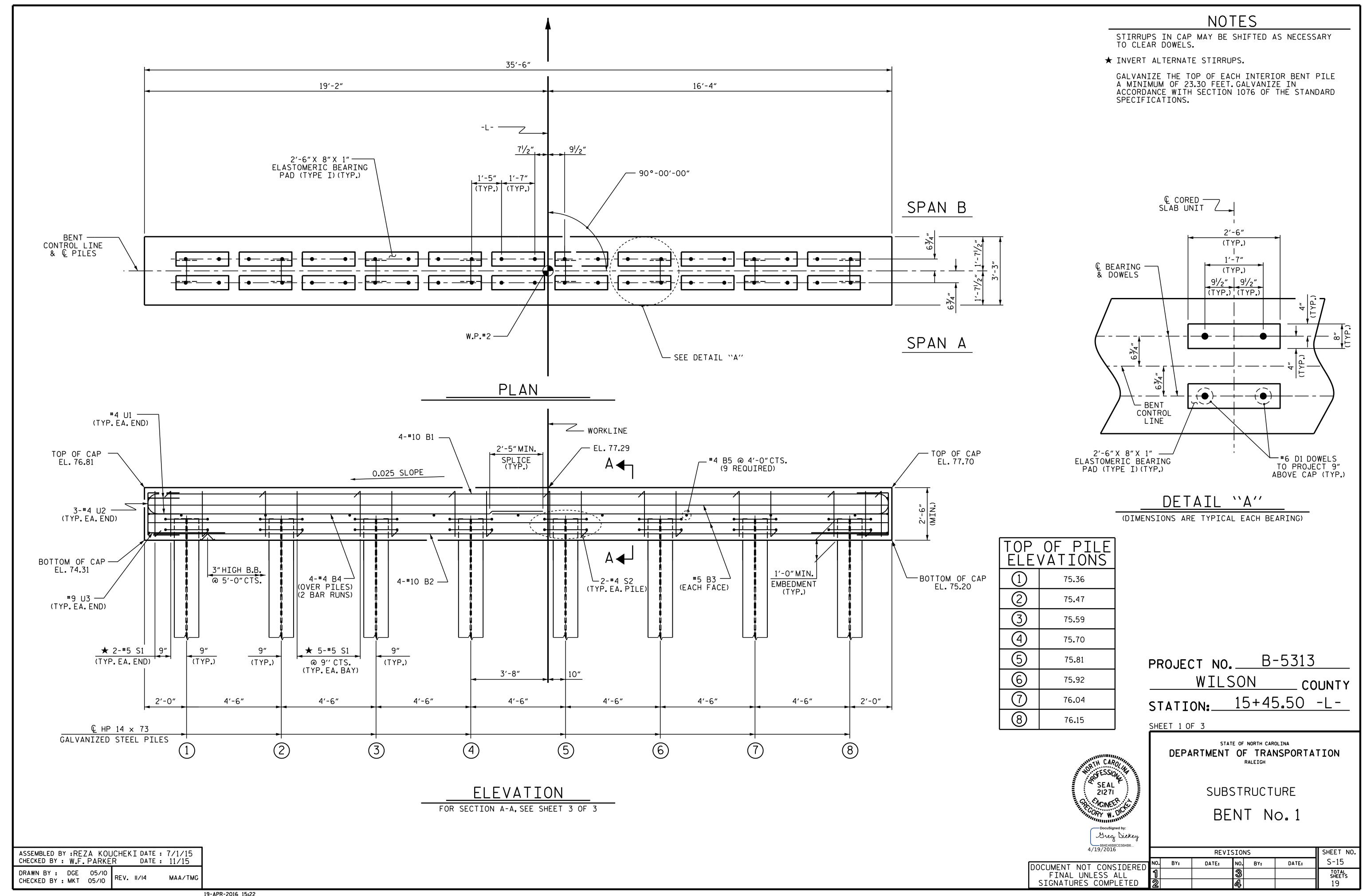
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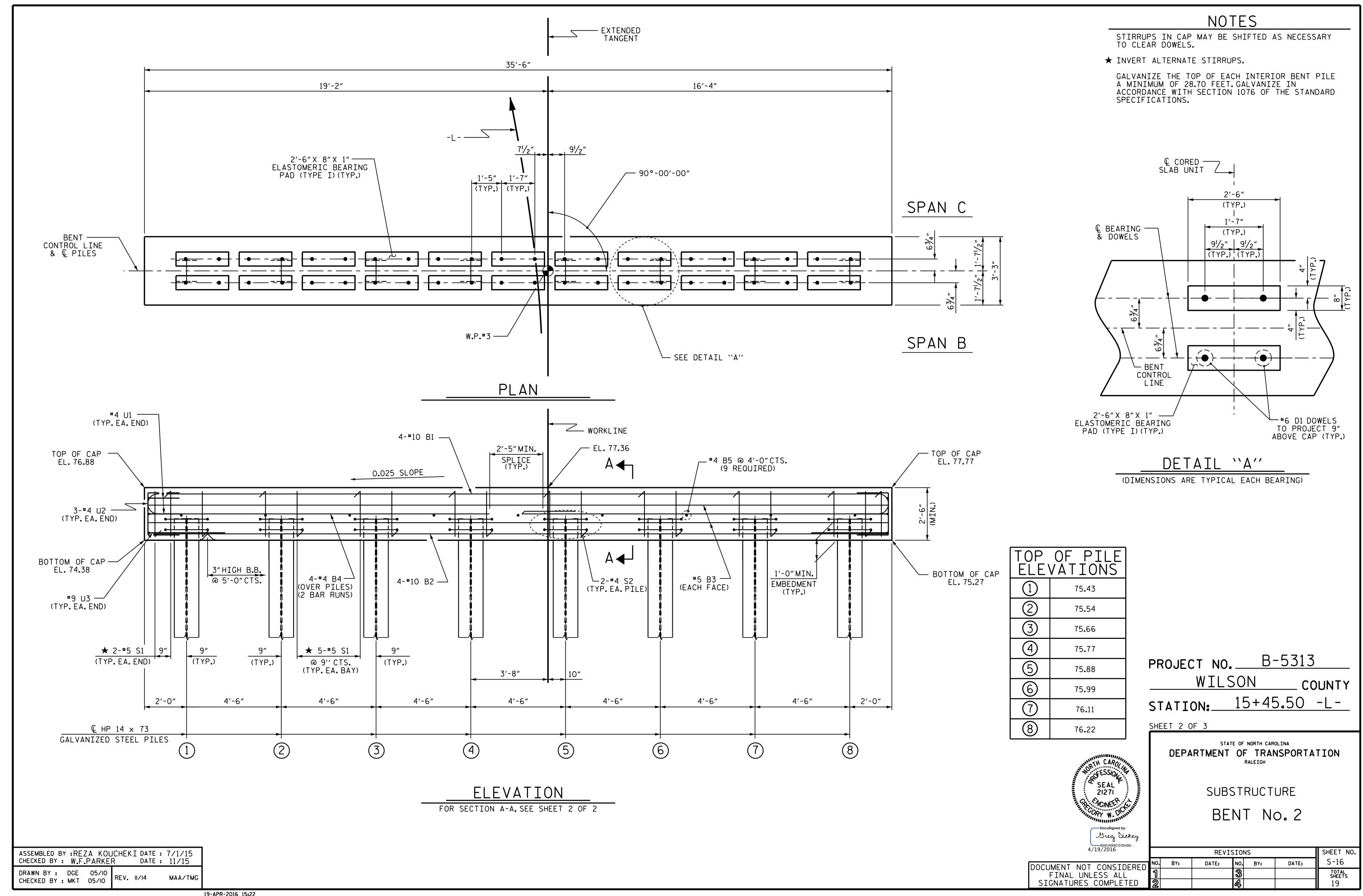
— 4-#4 B2 @ 4" CTS.

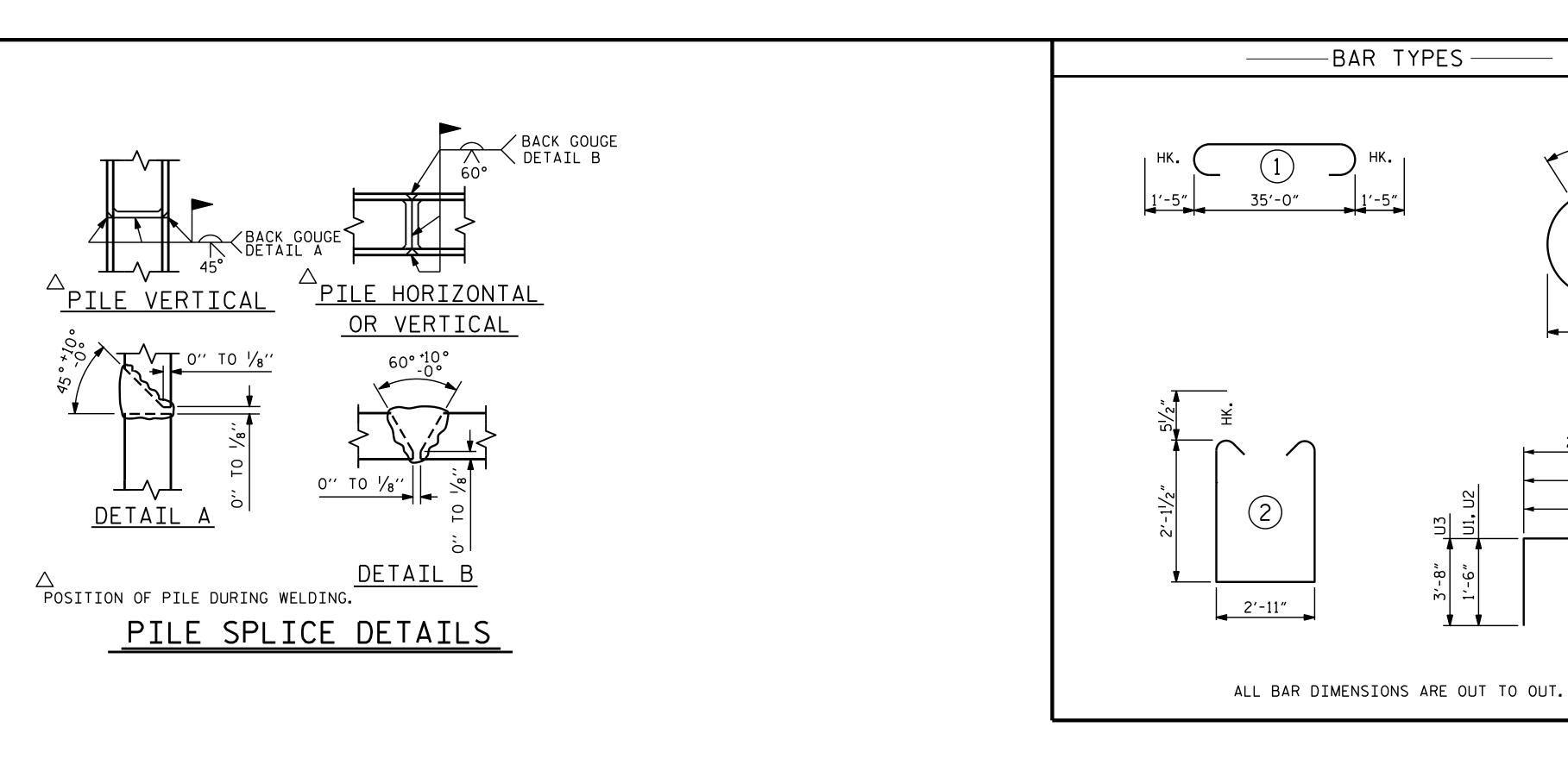
OVER PILES

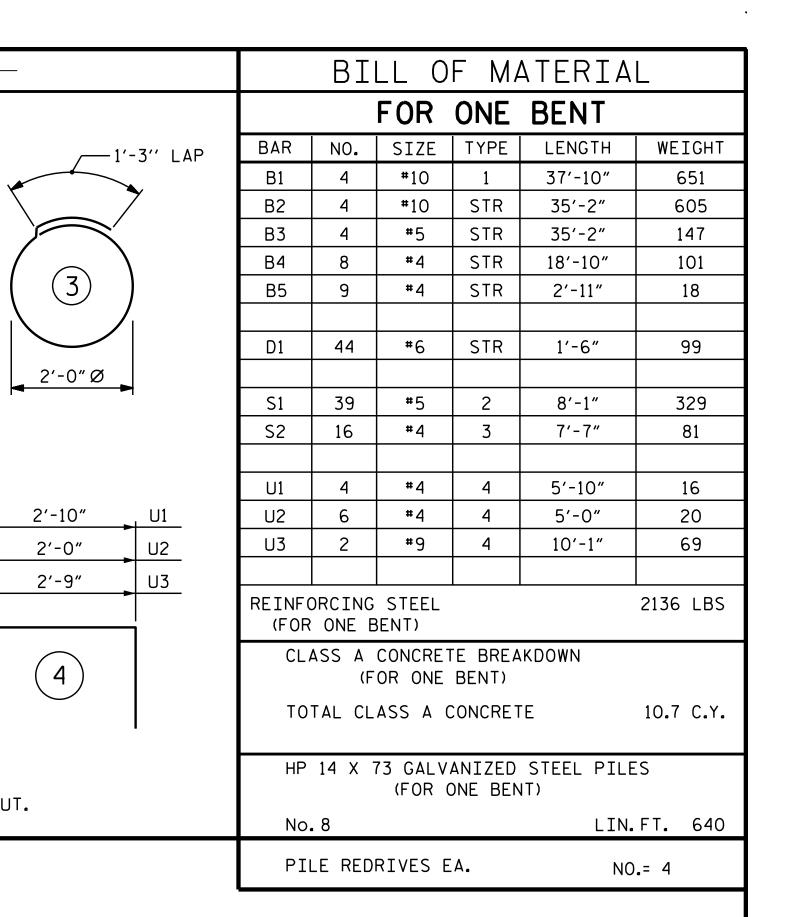
2-#9 B1

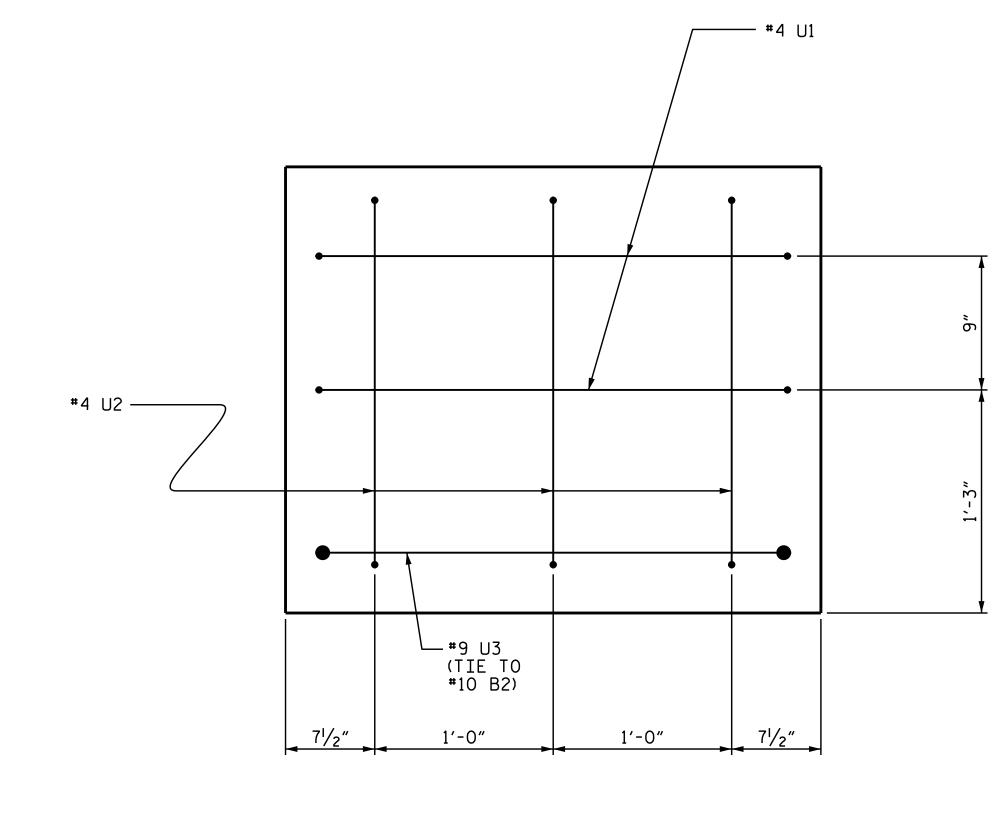
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#9 U3
(TIE TO \*10 B2)

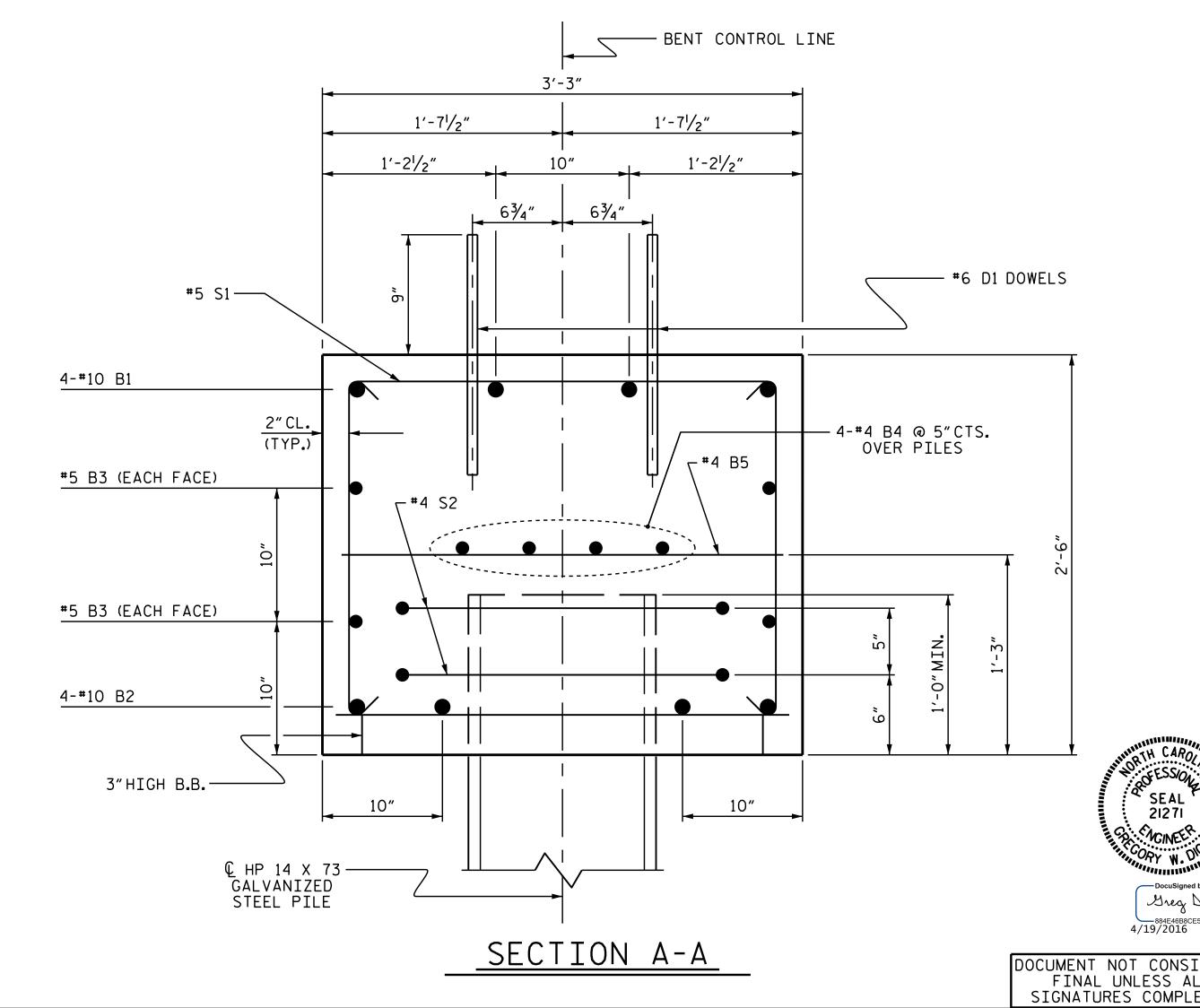
1'-0"

1'-0"

TV/2"

END OF CAP VIEW

(TYPICAL BOTH ENDS)



PROJECT NO. B-5313

WILSON COUNTY

STATION: 15+45.50 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

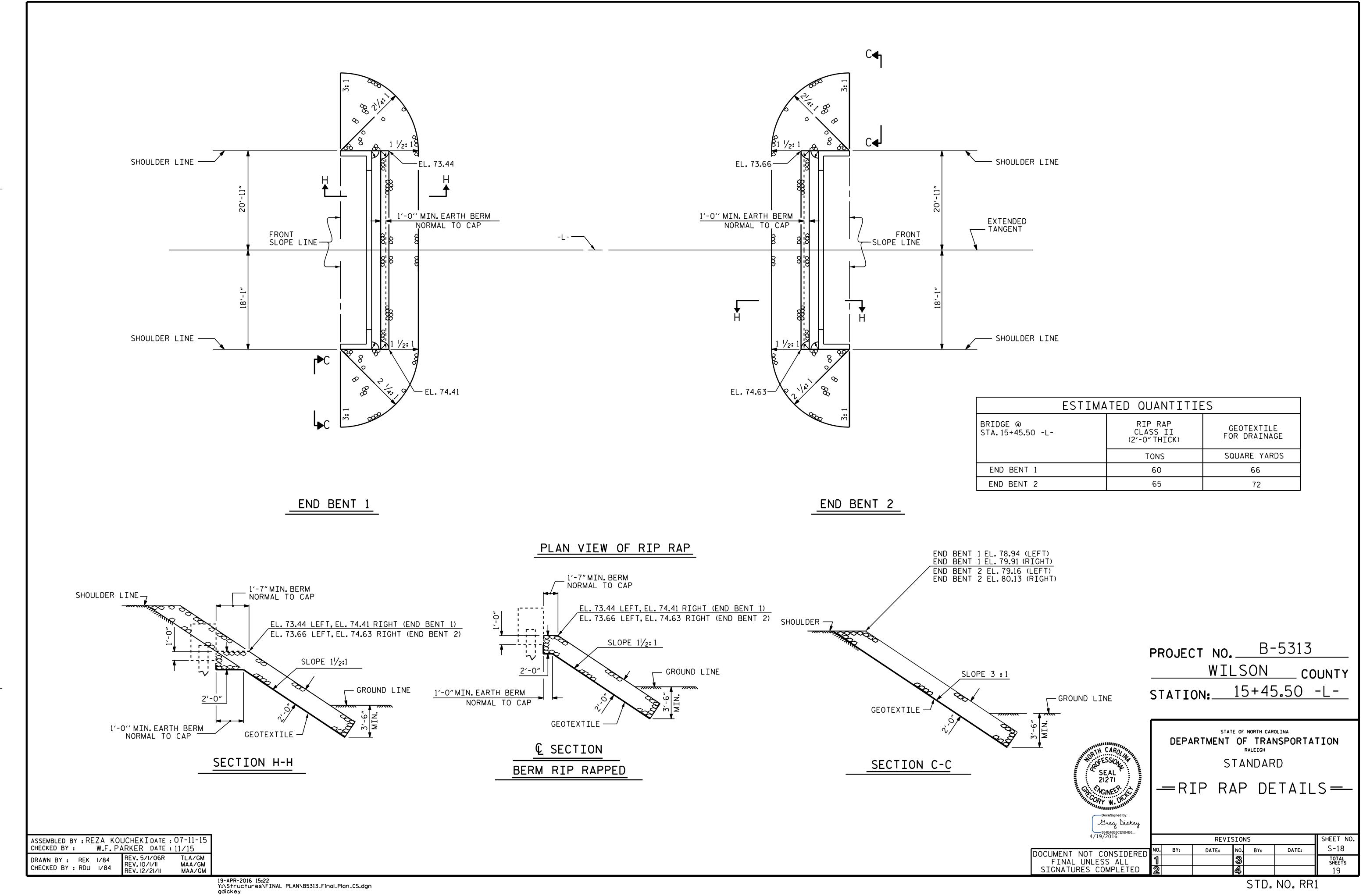
SUBSTRUCTURE

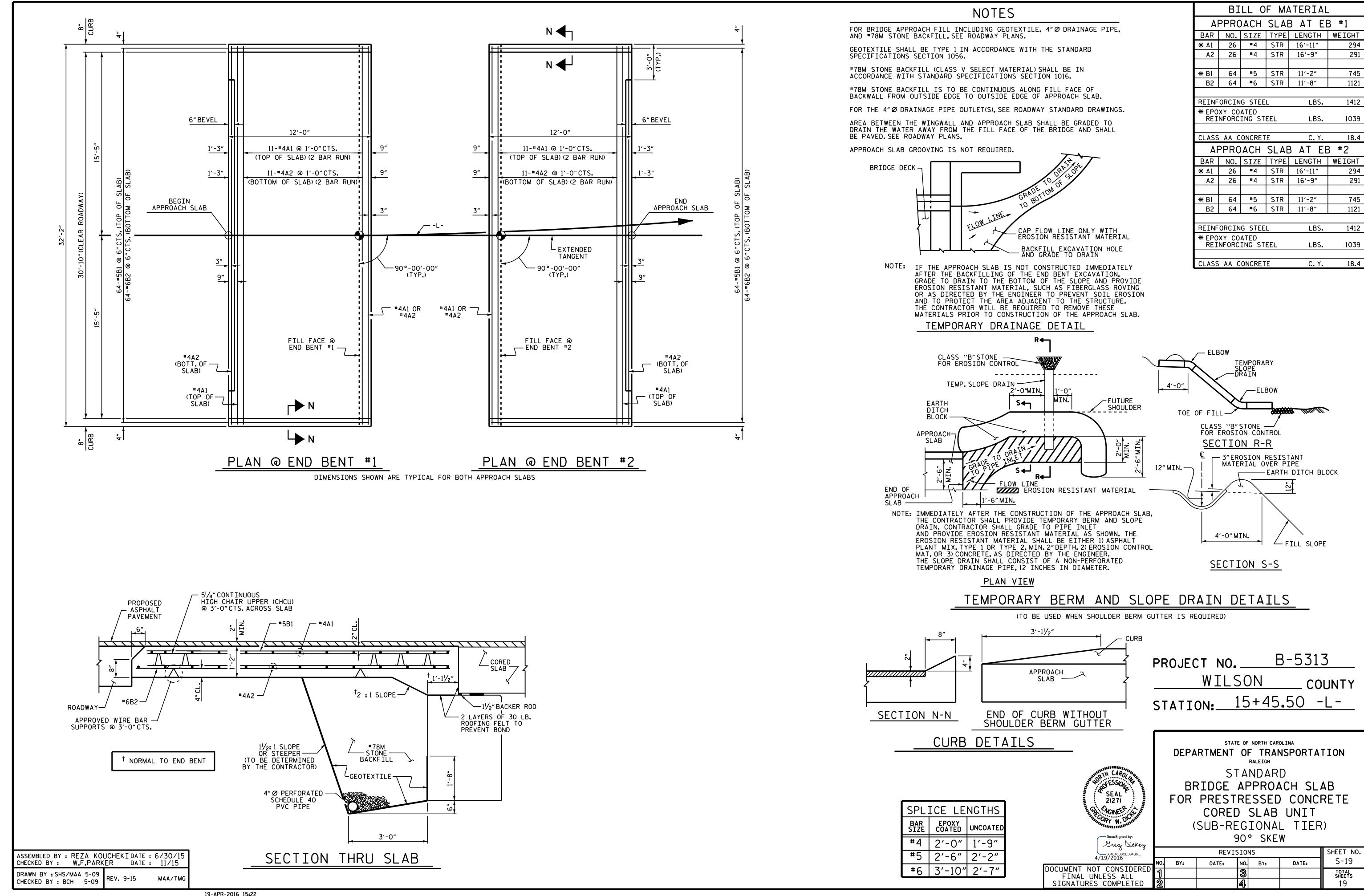
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DRAWN BY: REZA KOUCHEKI DATE: 7/1/15
CHECKED BY: W.F.PARKER DATE: 11/15

DRAWN BY: DGE 05/10 REV. II/14 CHECKED BY: MKT 05/10





#### STANDARD NOTES

#### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS. PER SO. IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

REV. 6-16-95 EEM (J) RGW REV. 5-7-03 RWW (J) JTE

REV. 8-16-99 RWW (x) LES REV. 5-1-06 TLA (x) GM

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

REV. 10-1-11 MAA (/) GM

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

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AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

STD. NO. SN