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DIMENSIONS SHOWN ARE TO THE CENTERLINE PILE AT THE BOTTOM OF THE CAP

▲ DENOTES DIRECTION OF 3:12 BATTER

NOTES:

FOR PILES, SEE PILES (LFD-ASD) SPECIAL PROVISIONS.

DRIVE PILES AT ABUTMENT NO.2 TO A REQUIRED BEARING CAPACITY OF 110 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT ABUTMENT NO. 2 IS 55 TONS PER PILE.

FOR DRILLED PIERS, SEE LFD DRILLED PIERS SPECIAL PROVISIONS.

DRILLED PIERS AT ABUTMENT NO.1 ARE DESIGNED FOR BOTH SKIN FRICTION AND END BEARING, CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 20 TSF,

DRILLED PIERS AT PIER NO.1, PIER NO.2 AND PIER NO.3 ARE DESIGNED FOR BOTH SKIN FRICTION AND END BEARING. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY OF 20 TSF.

DRILLED PIERS AT ABUTMENT NO.1 ARE DESIGNED FOR AN APPLIED LOAD OF 220 TONS EACH AT THE TOP OF THE COLUMN.

DRILLED PIERS AT PIER NO.1, PIER NO.2 AND PIER NO.3 ARE DESIGNED FOR AN APPLIED LOAD OF 365 TONS EACH AT THE TOP OF THE COLUMN.

INSTALL DRILLED PIERS AT ABUTMENT NO.1 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 382 FT AND SATISFY THE REQUIRED END BEARING CAPACITY.

INSTALL DRILLED PIERS AT PIER NO.1 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 378 FT AND SATISFY THE REQUIRED END BEARING CAPACITY.

INSTALL DRILLED PIERS AT PIER NO. 2 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 372 FT AND SATISFY THE REQUIRED END BEARING CAPACITY.

INSTALL DRILLED PIERS AT PIER NO. 3 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 367 FT (LT, CT) AND 371 FT (RT) AND SATISFY THE REQUIRED END BEARING CAPACITY,

DRILLED PIER EXCAVATIONS AT ABUTMENT NO.1 AND PIER NO.1 THROUGH PIER NO.3 WILL EXTEND INTO MATERIAL THAT DETERIORATES WHEN EXPOSED TO THE ELEMENTS. CHECK FIELD CONDITIONS FOR THE REQUIRED END BEARING CAPACITY AND PLACE CONCRETE IMMEDIATELY AFTER THE EXCAVATION IS COMPLETED.

SPT TESTING IS REQUIRED FOR DRILLED PIERS AT ABUTMENT NO.1 AND PIER NO.3.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS.

CSL TUBES AND TESTING ARE REQUIRED FOR DRILLED PIERS AT ABUTMENT NO.1 AND PIER NO.1 THROUGH PIER NO.3.

PROJECT NO. U-3308 DURHAM COUNTY

STATION: 24+55.20 -LALT-13+22.18 -CSXN-

SHEET 2 OF 5

__STA.14+04.42 -CSXN-

W.P. #5

STA. 14+05.68 -CSXN-

-82°-19′-36″

— € VERTICAL PILES

FILL FACE @

ABUTMENT 2

⊢© SURVEY -CSXN-

25911 WGINEER

Dwain Hathaway

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

FOR BRIDGE ON CSXT RAILWAY OVER ALSTON AVE. BETWEEN NC 147 AND ANGIER AVE.

REVISIONS Michael Baker Engineering NO. BY: Michael Baker 8000 Regency Parkway, Suite 600 No. BY: DATE: DATE: Cary, North Carolina 276518 NC License No F-1084 INTERNATIONAL

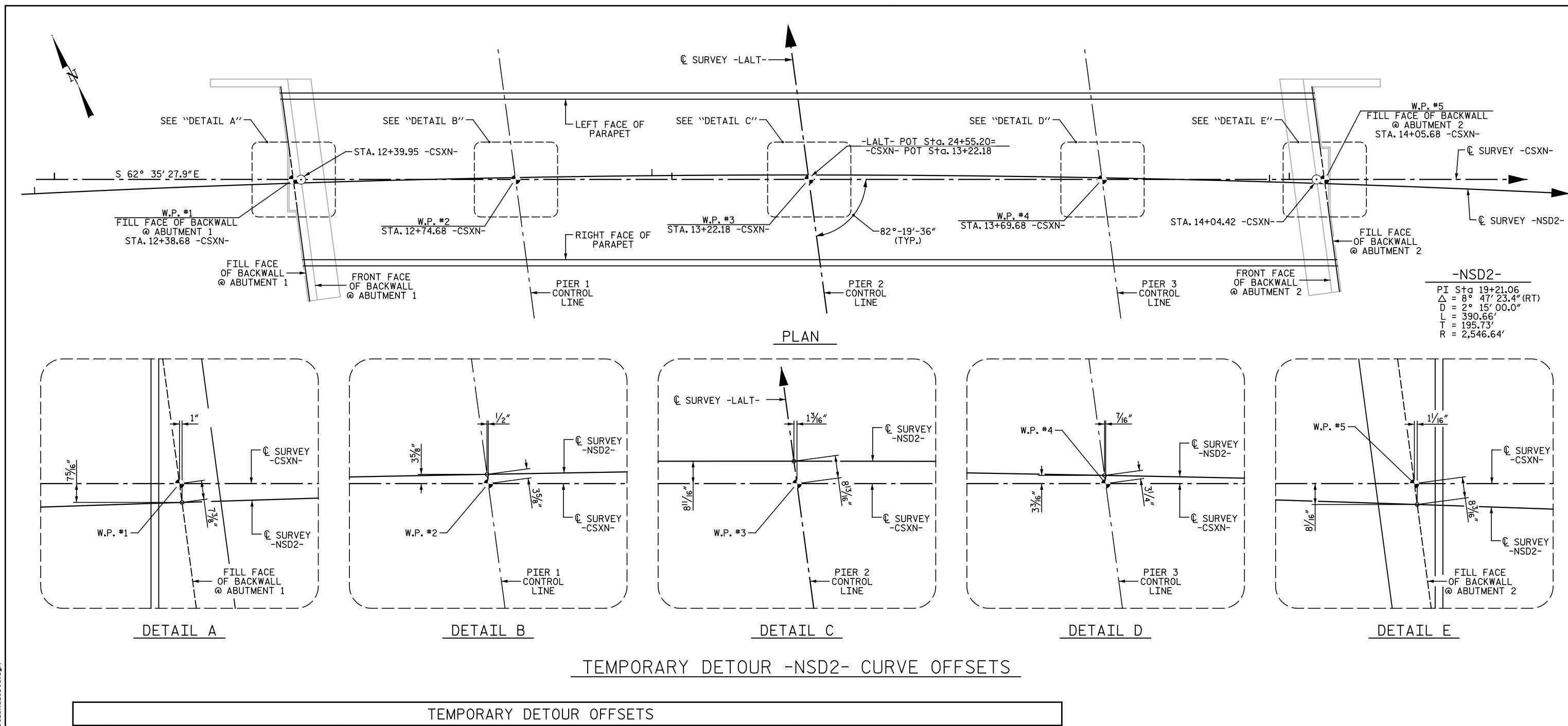
DRAWN BY : _____M. D. MAYHEW ____ DATE : 12-31-13 CHECKED BY: S. A. DENNEY DATE: 2-24-14

68 SITE 4

SHEET NO.

S4-2

TOTAL SHEETS



		TEMPORARY DETOUR OFFSETS																	
			SI	PAN A			SF	PAN B			SF	AN C		SPAN D					1
		-CSXN- STATION	OFFSET C -CSXN- TO -NSD2- *	OFFSET FROM ℚ -NSD2- TO LEFT FACE OF PARAPET **	OFFSET FROM ℚ -NSD2- TO RIGHT FACE OF PARAPET **	-CSXN- STATION	OFFSET	OFFSET FROM	OFFSET FROM ♠ -NSD2- TO RIGHT FACE OF PARAPET **	-CSXN- STATION	OFFSET € -CSXN- TO -NSD2- *	OFFSET FROM ℚ -NSD2- TO LEFT FACE OF PARAPET **	OFFSET FROM ℚ -NSD2- TO RIGHT FACE OF PARAPET **	-CSXN- STATION	OFFSET	OFFSET FROM ℚ -NSD2- TO LEFT FACE OF PARAPET **	OFFSET FROM ℚ -NSD2- TO RIGHT FACE OF PARAPET **		* * (R (L
	0.0	12+38.68	0.612(RT)	13 . 612	12.388	12+74.68	0.301(LT)	12,699	13.301	13+22.18	0.726(LT)	12,274	13.726	13+69.68	0.265(LT)	12.735	13.265	0.0	
	0.1	12+42.28	0.498(RT)	13.498	12 . 502	12+79.43	0.383(LT)	12.617	13.383	13+26.93	0.720(LT)	12.280	13.720	13+73.28	0.194(LT)	12.806	13.194	0.1	
	0.2	12+45.88	0.389(RT)	13.389	12.611	12+84.18	0.457(LT)	12.543	13 . 457	13+31.68	0.705(LT)	12.295	13.705	13+76.88	0.118(LT)	12.882	13.118	0.2	<u>}</u>
TNTO	0.3	12+49.48	0.285(RT)	13.285	12.715	12+88.93	0.521(LT)	12.479	13.521	13+36.43	0 . 681(LT)	12.319	13.681	13+80.48	0.037(LT)	12,963	13.037	0.2	٦ ح
=	0.4	12+53.08	0 . 186(RT)	13 . 186	12.814	12+93.68	0.577(LT)	12,423	13 . 577	13+41.18	0.648(LT)	12.352	13.648	13+84.08	0.049(RT)	13.049	12.951	0.4	- -
E	0.5	12+56.68	0.092(RT)	13.092	12,908	12+98.43	0.624(LT)	12.376	13.624	13+45.93	0.607(LT)	12.394	13.607	13+87.68	0 . 141(RT)	13.141	12.859	0.4 0.5 0.6	- - -
	0.6	12+60.28	0.003(RT)	13.003	12,997	13+03.18	0.662(LT)	12.338	13.662	13+50.68	0.556(LT)	12.444	13 . 556	13+91.28	0.237(RT)	13 . 237	12.763	0.6 F	-
	0.7	12+63.88	0.080(LT)	12.920	13,080	13+07.93	0.692(LT)	12.309	13.692	13+55.43	0.497(LT)	12,503	13 . 497	13+94.88	0.339(RT)	13 . 339	12.661	0.7	
	0.8	12+67.48	0 . 159(LT)	12.841	13 . 159	13+12.68	0.712(LT)	12.288	13.712	13+60.18	0.428(LT)	12 . 572	13.428	13+98.48	0.446(RT)	13,446	12.554	0.8	
	0.9	12+71.08	0 . 232(LT)	12,768	13,232	13+17.43	0.723(LT)	12.277	13.723	13+64.93	0.351(LT)	12.649	13.351	14+02.08	0.557(RT)	13.557	12.443	0.9	
	1.0	12+74.68	0.301(LT)	12.699	13.301	13+22.18	0.726(LT)	12.274	13.726	13+69.68	0.265(LT)	12.735	13.265	14+05.68	0.674(RT)	13.674	12.326	1.0	

* - NORMAL TO € -CSXN-** - NORMAL TO FACE OF PARAPET (RT) - DENOTES RIGHT OF SURVEY -CSXN-(LT) - DENOTES LEFT OF SURVEY -CSXN-

PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-

13+22.18 -CSXN-SHEET 3 OF 5

PICYGINEER Dwain Hathaway

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

FOR BRIDGE ON CSXT RAILWAY OVER ALSTON AVE. BETWEEN NC 147 AND ANGIER AVE.

REVISIONS SHEET NO. Michael Baker Engineering 8000 Regency Parkway, Suite 600 NO. BY: S4-3 DATE: NO. BY: Michael Baker DATE: Cary, North Carolina 276518 NC License No.: F-1084 TOTAL SHEETS INTERNATIONAL

DRAWN BY: J.N. AUSTIN DATE: 4-24-13 CHECKED BY: D.G. HATHAWAY DATE: 4-25-13

ASSUMED LIVE LOAD = COOPER E-80 W/ IMPACT OR ALTERNATE LIVE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT A.R.E.M.A. MANUAL, NORFOLK SOUTHERN RAILWAY GUIDELINES FOR DESIGN OF GRADE SEPARATION STRUCTURES, AND CSXT CRITERIA FOR OPEN DECK RAILROAD BRIDGES. IN CASE OF DISCREPANCY THE MORE STRINGENT CITERIA WILL GOVERN.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY, 2012 NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS), EXCEPT AS NOTED HEREIN, ELSEWHERE ON PLANS, OR IN THE SPECIAL PROVISIONS". (STRUCTURAL STEEL IN ACCORDANCE WITH CURRENT A.R.E.M.A. SPECIFICATIONS).

CONCRETE SHALL BE 4,500 PSI (SUBSTRUCTURE) OR 5,000 PSI (SUPERSTRUCTURE) CLASS AA CONCRETE WITH NO.57 COARSE AGGREGATE AND SHALL BE AIR-ENTRAINED, MINIMUM CEMENT CONTENT PER CUBIC YARD OF CONCRETE SHALL BE 6.0 BAGS/CY. NO SUBSTITUTION OF FLY ASH, BLAST FURNACE SLAG OR OTHER MATERIAL WILL BE PERMITTED IN MEETING THIS MINIMUM CEMENT REQUIREMENT. NO RUBBED SURFACE FINISH IS REQUIRED, CHAMFER ALL EXPOSED EDGES AND CORNERS 'INCH EXCEPT AS NOTED. THE USE OF GROUND GRANULATED BLAST FURNACE SLAG IS NOT PERMITTED IN THE STRUCTURE. SEE SPECIAL PROVISIONS FOR CAST-IN-PLACE CONCRETE.

REINFORCING STEEL SHALL BE ASTM DESIGNATION A615, GRADE 60. ALL REINFORCING BARS IN THE CONCRETE DECK AND CURBS SHALL BE EPOXY-COATED IN CONFORMANCE WITH ASTM A775 "STANDARD SPECIFICATION FOR EPOXY-COATED REINFORCING BARS", COMPATIBLE EPOXY-COATED STEEL TIE WIRES SHALL BE USED WITH THE EPOXY-COATED BARS, FABRICATION TO BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE" A.C.I. 315-80.

EXPANSION JOINT MATERIAL SHALL BE EITHER RUBBER OR CORK CONFORMING WITH AASHTO SPECIFICATIONS M-153-84 EXCEPT AS SHOWN ON THE PLANS OR IN THE SPECIAL PROVISIONS, CELLULAR AND BULB TYPE WATERSTOPS AND RUBBER JOINT COMPOUNDS SHALL BE AS SHOWN ON THE PLANS AND IN THE SPECIAL PROVISIONS.

STRUCTURE DRAINAGE SYSTEM: METAL DRAINS BEHIND ABUTMENTS AND DUCTILE IRON PIPE COLLECTOR SYSTEM, SHALL BE AS SHOWN ON THE PLANS AND OUTLINED IN THE SPECIAL PROVISIONS, DETAILS OF THE DRAINAGE SYSTEM SHALL BE SUBMITTED TO THE CHIEF ENGINEER-BRIDGES AND STRUCTURES, CSXT, JACKSONVILLE, FL. FOR APPROVAL.

CONTROL OF WORK: ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILWAY STRUCTURE SHALL BE PERFORMED SATISFACTORY TO THE ENGINEER AND/OR CSXT. ALL METHODS OF HANDLING WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY ENGINEER BEFORE PROCEEDING WITH THAT PORTION OF THE WORK. RAIL TRAFFIC SHALL, AT ALL TIMES, BE MAINTAINED AND PROTECTED, THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH RAIL OPERATIONS.

ALL CONSTRUCTION JOINTS SHOWN ON THESE PLANS SHALL BE REQUIRED UNLESS SHOWN OPTIONAL. CONSTRUCTION JOINTS SHALL NOT BE PERMITTED EXCEPT AS SHOWN ON THE PLANS, OR WHERE WRITTEN APPROVAL HAS BEEN OBTAINED.

DAMPPROOFING: PIER COLUMNS UP TO GROUND LINE. BACK OF BACKWALLS AND ABUTMENT SEATS, AND BACK OF WINGS SHALL BE

WATERPROOFING: ALL CONSTRUCTION JOINTS AND ANY SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL SHALL BE WATERPROOFED WITH A TWO PART WATERPROOFING SYSTEM CONSISTING OF A MEMBRANE LAYER AND A PROTECTION COURSE, STRIPS OF WATERPROOFING NO LESS THAN 2 FEET WIDE SHALL BE PLACED SYMMETRICALLY OVER JOINTS OR CRACKS. ADDITIONALLY, THE HORIZONAL SURFACES OF THE BALLAST TROUGH (EXCLUDING THE END DAM) SHALL BE WATERPROOFED WITH A TWO PART WATERPROOFING SYSTEM CONSISTING OF A MEMBRANE LAYER AND A 1 INCH THICK ASPHALT PLANKING OR OTHER RAILWAY APPROVED PROTECTION MATERIAL, ALL WATERPROOFING MATERIALS SHALL CONFORM TO THE RECOMMENDED PRACTICES IN THE A.R.E.M.A. MANUAL OF RAILWAY ENGINEERING CHAPTER 29.

BACKFILLING AROUND STRUCTURE: SEE SPECIAL PROVISIONS FOR BACKFILL BEHIND ABUTMENTS AND OTHER BACKFILL AROUND THE STRUCTURE.

FOR FOUNDATION RECOMMENDATION NOTES, SEE SHEET 2 OF 5.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE USED. THE BARS FROM THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS, PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

A 3/32" THICK BUTYL RUBBER MEMBRANE CONFORMING TO A.R.E.M.A. CHAPTER 8 SECTION 29.9.5 WILL BE INSTALLED ON THE BRIDGE DECK BALLAST RETAINERS, COST OF BUTYL RUBBER MEMBRANE MATERIAL AND INSTALLATION TO BE INCLUDED WITH THE COST OF "WATERPROOFING (RAILROAD STRUCTURES)", FOR "WATERPROOFING (RAILROAD STRUCTURES", SEE SPECIAL PROVISIONS.

FOR STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

FOR PAINTING STEEL STRUCTURES, SEE SPECIAL PROVISIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR METAL HANDRAIL, SEE SPECIAL PROVISIONS.

FOR GROUT, SEE A.R.E.M.A. CHAPTER 8 ARTICLE 14.4.8 AND ARTICLE 14.5.5. AND SPECIAL PROVISIONS.

FOR SUPERSTRUCTURE CONCRETE, SUBSTRUCTURE CONCRETE, REINFORCING STEEL, EPOXY COATED REINFORCING STEEL, SPIRAL COLUMN REINFORCING STEEL, AND DAMPPROOFING (RAILROAD STRUCTURES), SEE SPECIAL PROVISIONS FOR CAST-IN-PLACE CONCRETE.

THE EXISTING STRUCTURE CONSISTING OF 3 SPAN STRUCTURAL STEEL GIRDERS WITH TIMBER FLOOR ON STEEL TRESTLE BENTS AND LOCATED AT PROPOSED STRUCTURE SITE SHALL BE REMOVED.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD. THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS, ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+22.18 -CSXN-."

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 14 FT (LEFT) AND 16 FT (RIGHT) OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER, THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY SPAN FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY SPAN. SEE TEMPORARY SPAN SPECIAL PROVISIONS.

FOR TEMPORARY RAILROAD SHORING SPECIAL PROVISION AND PAY ITEM, SEE STR. #3. FOR ELASTOMERIC FLASHING, SEE SPECIAL PROVISIONS.

FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR'S ATTENTION IS DIRECTED TO THE EXISTING ABANDONED BRIDGE SOUTH OF THE PROPOSED -CSXN- BRIDGE. ANY REQUIREMENTS PERTAINING TO THE EXISTING STRUCTURE REMOVAL ARE ALSO APPLICABLE TO THE EXISTING ABANDONED BRIDGE.

DESIGN DATA:

SPECIFICATIONS:

CURRENT A.R.E.M.A., CSX TRANSPORTATION,

AND NORFOLK SOUTHERN DESIGN CRITERIA FOR GRADE SEPARATION PROJECTS.

AS PER A.R.E.M.A. SPECIFICATIONS.

LIVE LOAD: COOPERS E-80 + ALTERNATE WITH IMPACT

STRUCTURAL STEEL:

ASTM A709 GRADE 50 (Fy = 50 KSI)

REINFORCED CONCRETE:

SUPERSTRUCTURE: f'c = 5000 PSI SUBSTRUCTURE: f'c = 4500 PSI

REINFORCING STEEL: ASTM A615 GRADE 60

> PROJECT NO. <u>U-3308</u> DURHAM COUNTY

24+55.20 -LALT-STATION: 13+22.18 -CSXN-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

Dwain Hathaway 5/3/2016

FOR BRIDGE ON CSXT RAILWAY OVER ALSTON AVE. BETWEEN

Michael Baker INTERNATIONAL

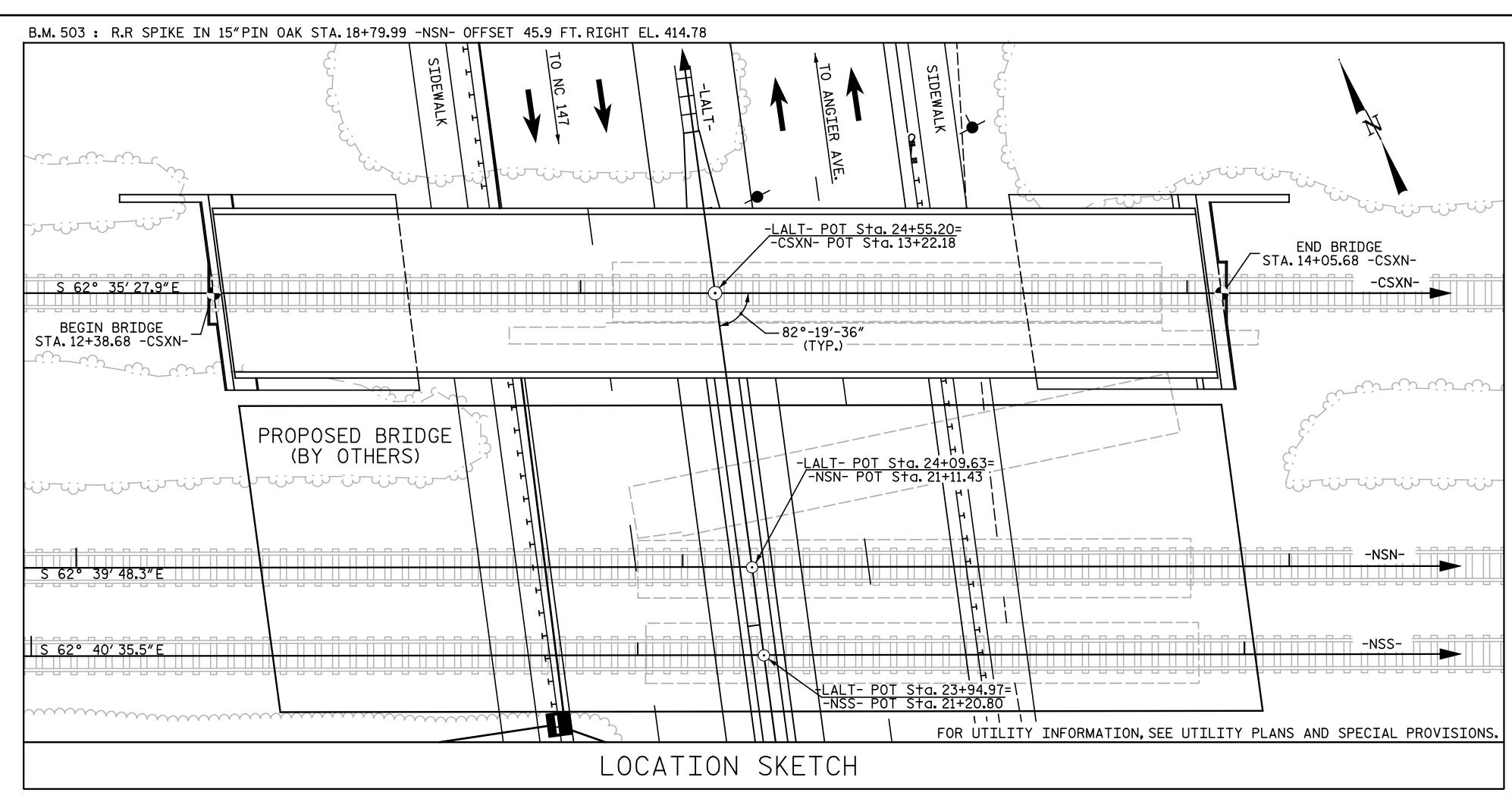
259II

WGINEER

Michael Baker Engineering 3000 Regency Parkway, Suite 600 NO. Cary, North Carolina 276518 NC License No. F-1084

NC 147 AND ANGIER AVE. SHEET NO. REVISIONS S4-4 NO. BY: DATE: DATE: BY: TOTAL SHEETS

DRAWN BY : J. N. AUSTIN DATE : 1-7-14 CHECKED BY: S. A. DENNEY DATE: 2-24-14



	TOTAL BILL OF MATERIAL															
	REMOVAL OF EXISTING STRUCTURE AT STA.13+22.18 -CSXN-	4'-0"DIA. DRILLED PIERS IN SOIL (LFD)	IPIERS NOT	5'-6"DIA. DRILLED PIERS IN SOIL (LFD)	5'-6"DIA. DRILLED PIERS NOT IN SOIL (LFD)	SID INSPECTIONS (LFD)	SPT TESTING (LFD)	(LFD)	UNCLASSIFIED STRUCTURE EXCAVATION AT STA.13+22.18 -CSXN-	REINFORCED CONCRETE DECK SLAB	CLASS AA CONCRETE AS MODIFIED BY RAILROAD	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	EPOXY COATED SPIRAL COLUMN REINFORCING STEEL	APPROX. 564,994 LBS. STRUCTURAL STEEL
	LUMP SUM	LIN. FT.	LIN.FT.	LIN. FT.	LIN.FT.	EA.	EA.	EA.	LUMP SUM	SQ. FT.	CU. YDS.	LBS.	LBS.	LBS.	LBS.	LUMP SUM
SUPERSTRUCTURE										4,601						
ABUTMENT 1		59 . 5	30.0				4	4			45.7	30,400		4,032		
PIER 1				7	35			3			57.1	5,088	16,449		3,909	
PIER 2				24	36			3			57.4	5,088	18,776		5,010	
PIER 3				45	26		3	3			57.2	5,088	20,147		5,665	
ABUTMENT 2											48.7	9,924				
TOTAL	LUMP SUM	59 . 5	30.0	76	97	13	7	13	LUMP SUM	4,601	266.1	55,588	55,372	4,032	14,584	LUMP SUM

	TOTAL BILL OF MATERIAL												
	PAINTING FOR STRUCTURAL STEEL	PROOFING (RAILROAD	DAMP- PROOFING (RAILROAD STRUCTURES)	STE	12×53 EL PILES (LFD)	METHOD A WATERPROOFING	METAL HANDRAIL	1'-0" CONCRETE PARAPET	4"SLOPE PROTECTION	SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES	STRUCTURE DRAINAGE SYSTEM AT STA.13+22.18 -CSXN-	CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY SPAN	ASBESTOS ASSESSMENT
	LUMP SUM	SQ. YDS.	SQ. YDS.	NO.	LIN. FT.	SQ. YDS.	LIN.FT.	LIN.FT.	SQ.YDS.	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM
SUPERSTRUCTURE		478					330.0	328.3					
ABUTMENT 1			52.7			10.3	20.1		145				
PIER 1			5.2			11.0							
PIER 2			5.2			11.0							
PIER 3			5.2			11.0							
ABUTMENT 2			52 . 1	18	540	10.0	19.7		143				
TOTAL	LUMP SUM	478	120.4	18	540	53. 3	369.8	328.3	288	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM

DRAWN BY: J. N. A. / M. D. M. DATE: 12-31-13 CHECKED BY: S. A. DENNEY DATE: 2-24-14

PROJECT NO. U-3308

DURHAM COUNTY
STATION: 24+55.20 -LALT-

| 13+22.18 -CSXN-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

Docusigned by:

Dwain Stathaway

283786071DA0460...

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 276518 NC License No. : F-1084

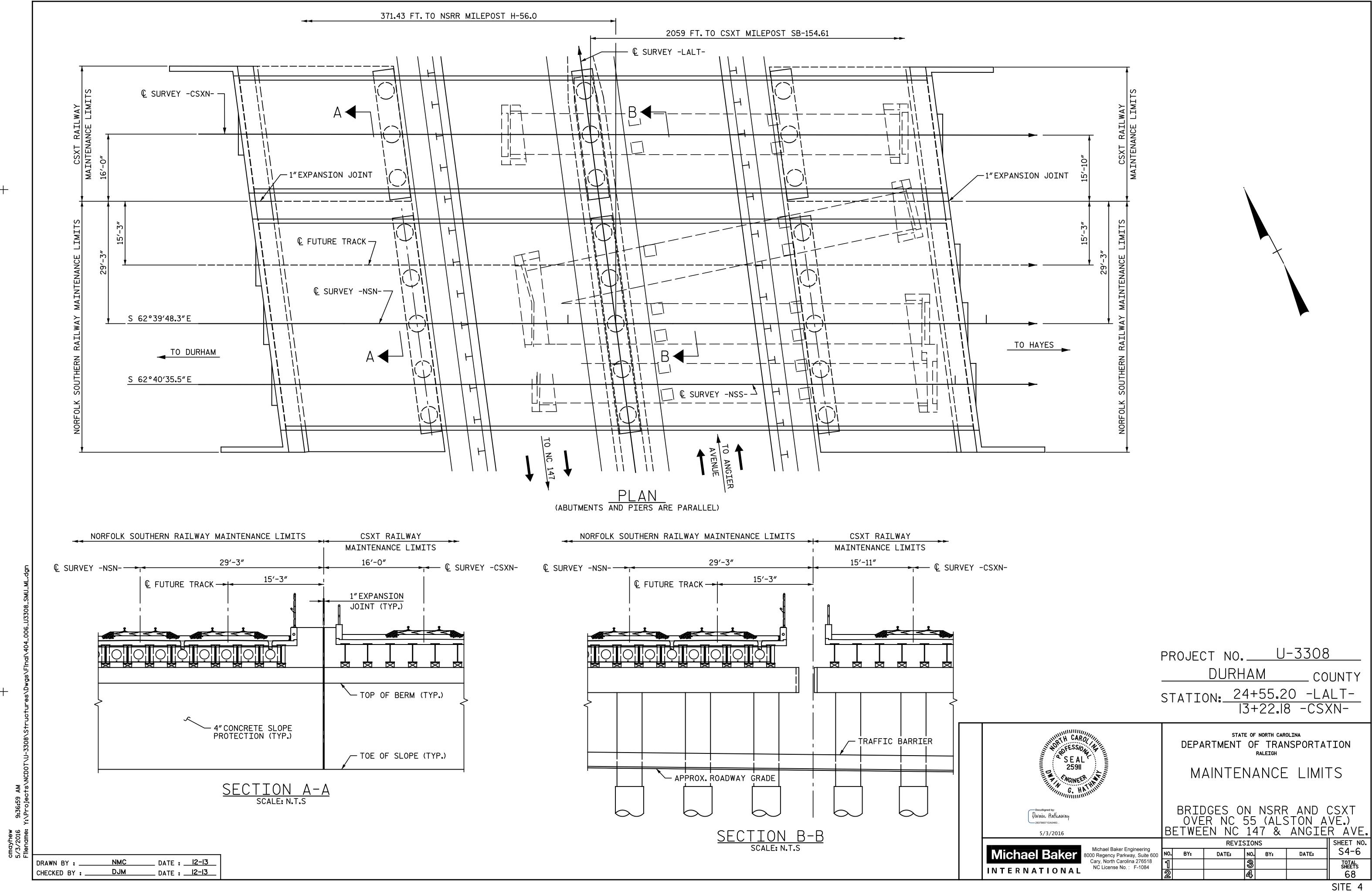
FOR BRIDGE ON CSXT RAILWAY
OVER ALSTON AVE. BETWEEN
NC 147 AND ANGIER AVE.

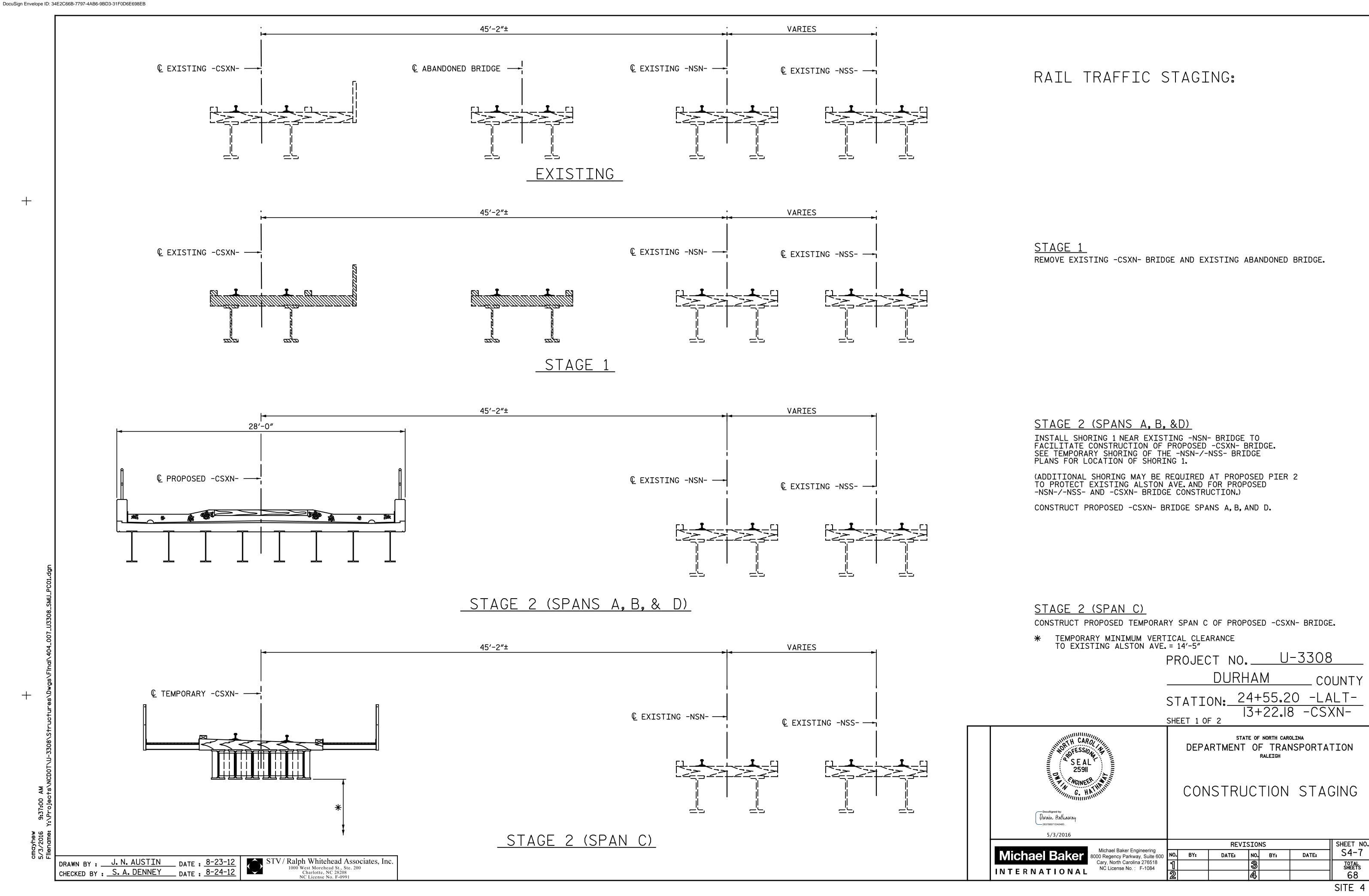
REVISIONS
SHEET NO.
S4-5

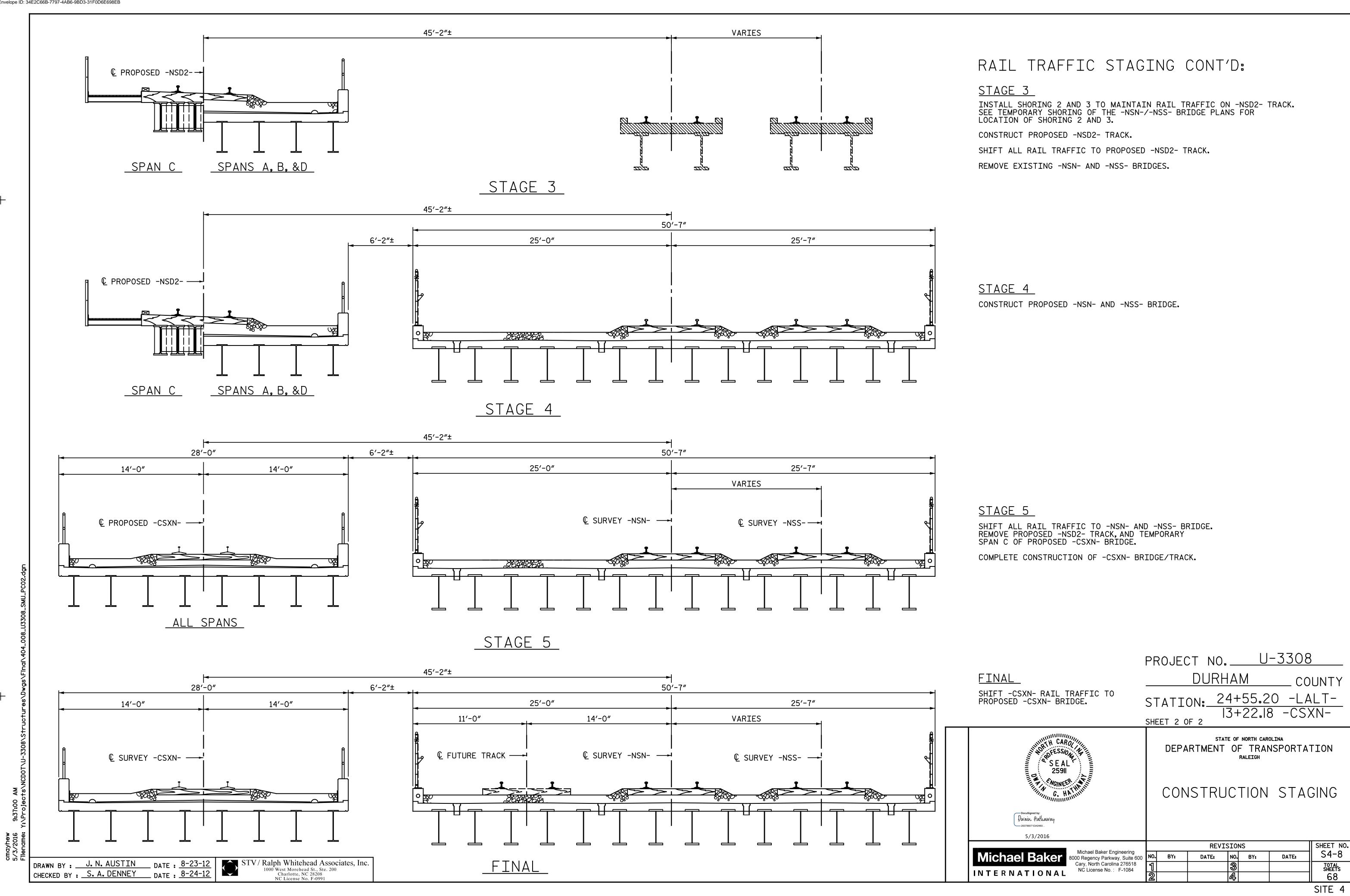
REVISIONS

BY: DATE: NO. BY: DATE: S4-5

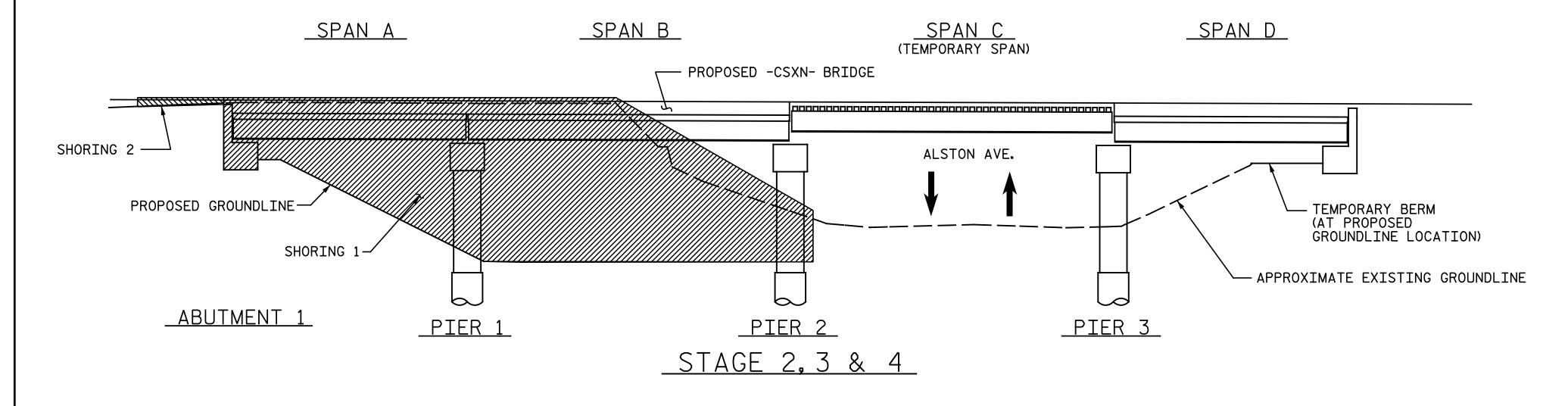
3 TOTAL SHEETS
68

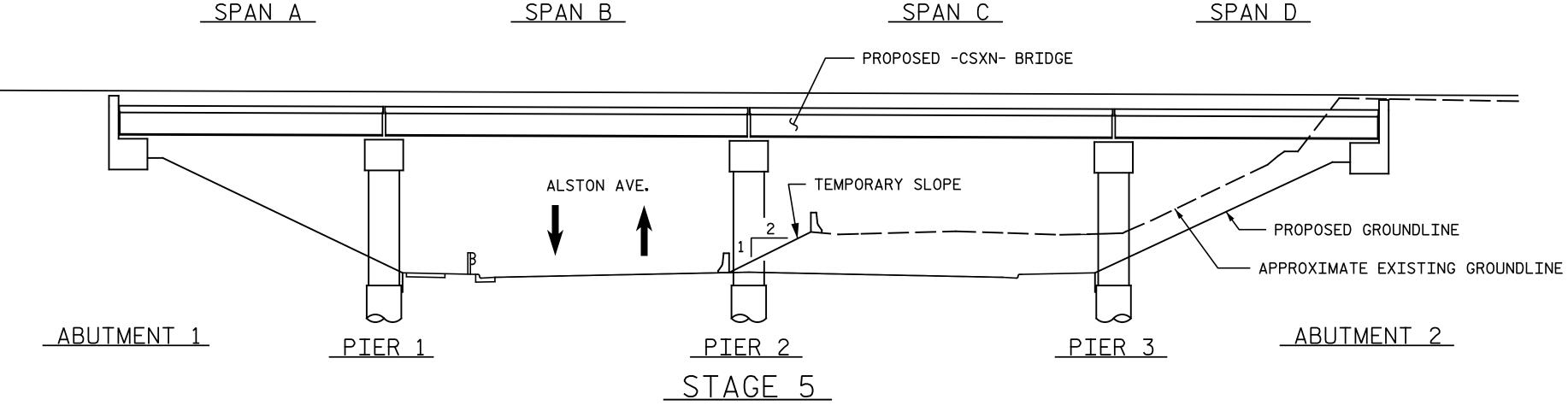


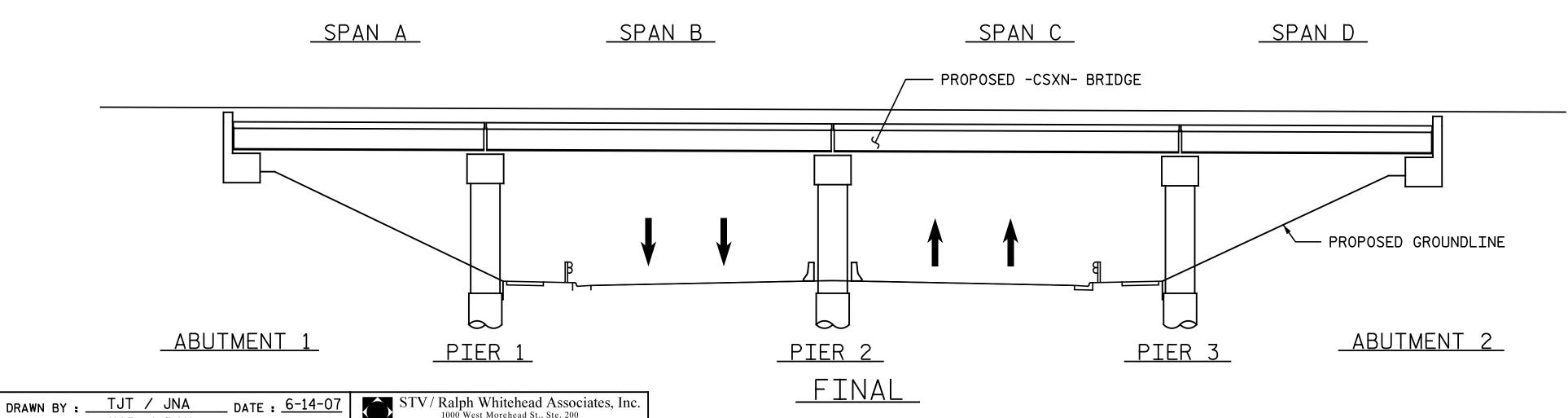




STAGE 1 (SECTION ALONG EXISTING -CSXN- BRIDGE)







1000 West Morehead St., Ste. 200 Charlotte, NC 28208 NC License No. F-0991

TRAFFIC STAGING:

NOTES: SEE RAIL TRAFFIC STAGING ON "CONSTRUCTION STAGING" SHEETS FOR ADDITIONAL INFORMATION.

TRAFFIC STAGING BELOW SHALL BE IN CONJUNCTION WITH TRAFFIC STAGING SHOWN ON PLANS FOR BRIDGE ON NSRR.

REFER TO TRACKWORK PLANS FOR TEMPORARY DETOUR PROFILE.

STAGE 1

ALSTON AVE. TRAFFIC TO REMAIN IN ORIGINAL POSITION.

REMOVE EXISTING -CSXN- BRIDGE AND EXISTING ABANDONED BRIDGE.

INSTALL SHORING 1 BETWEEN EXISTING -CSXN- BRIDGE AND EXISTING -NSN- BRIDGE. TO FACILITATE CONSTRUCTION OF PROPOSED -CSXN- BRIDGE, SEE TEMPORARY SHORING OF THE -NSN-/-NSS- BRIDGE PLANS FOR SHORING 1 LOCATION.

STAGE 2, 3 & 4

EXCAVATE IN SHORING 1 AREA AND CONSTRUCT PROPOSED -CSXN- BRIDGE SPANS A, B AND D. CONSTRUCT TEMPORARY SPAN C.

A TEMPORARY BERM WILL BE REQUIRED AT ABUTMENT 2 TO CONNECT PROPOSED GROUNDLINE TO EXISTING GROUNDLINE.

INSTALL SHORING 2 AND 3 BETWEEN SHORING 1 AND ABUTMENT 1 OF PROPOSED -CSXN-BRIDGE, PRIOR TO SHIFTING RAIL TRAFFIC TO -NSD2- TRACK, TO MAINTAIN RAIL TRAFFIC. SEE TEMPORARY SHORING OF THE -NSN-/-NSS- BRIDGE PLANS FOR SHORING 2 AND 3 LOCATION.

STAGE 5

FINAL

FINAL POSITIONS.

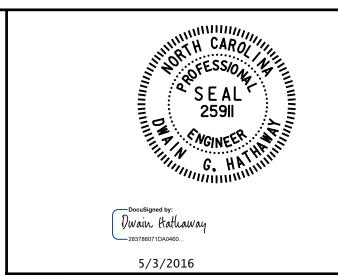
SHIFT ALSTON AVE. TRAFFIC TO

SHIFT ALSTON AVE. TRAFFIC TO CONSTRUCTED ROADWAY UNDER SPAN B OF CONSTRUCTED -CSXN- BRIDGE.

AFTER COMPLETION OF -NSN-/-NSS- BRIDGE AND RAIL TRAFFIC HAS BEEN SHIFTED ONTO COMPLETED -NSN-/-NSS- BRIDGE, REMOVE SHORING 2 AND 3 AND TEMPORARY SPAN C.

EXCAVATE REMAINDER OF EXISTING GROUND AND CONSTRUCT PROPOSED SPANS C OF -CSXN- BRIDGE. CONSTRUCT REMAINDER OF ROADWAY FOR ALSTON AVE.

PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-



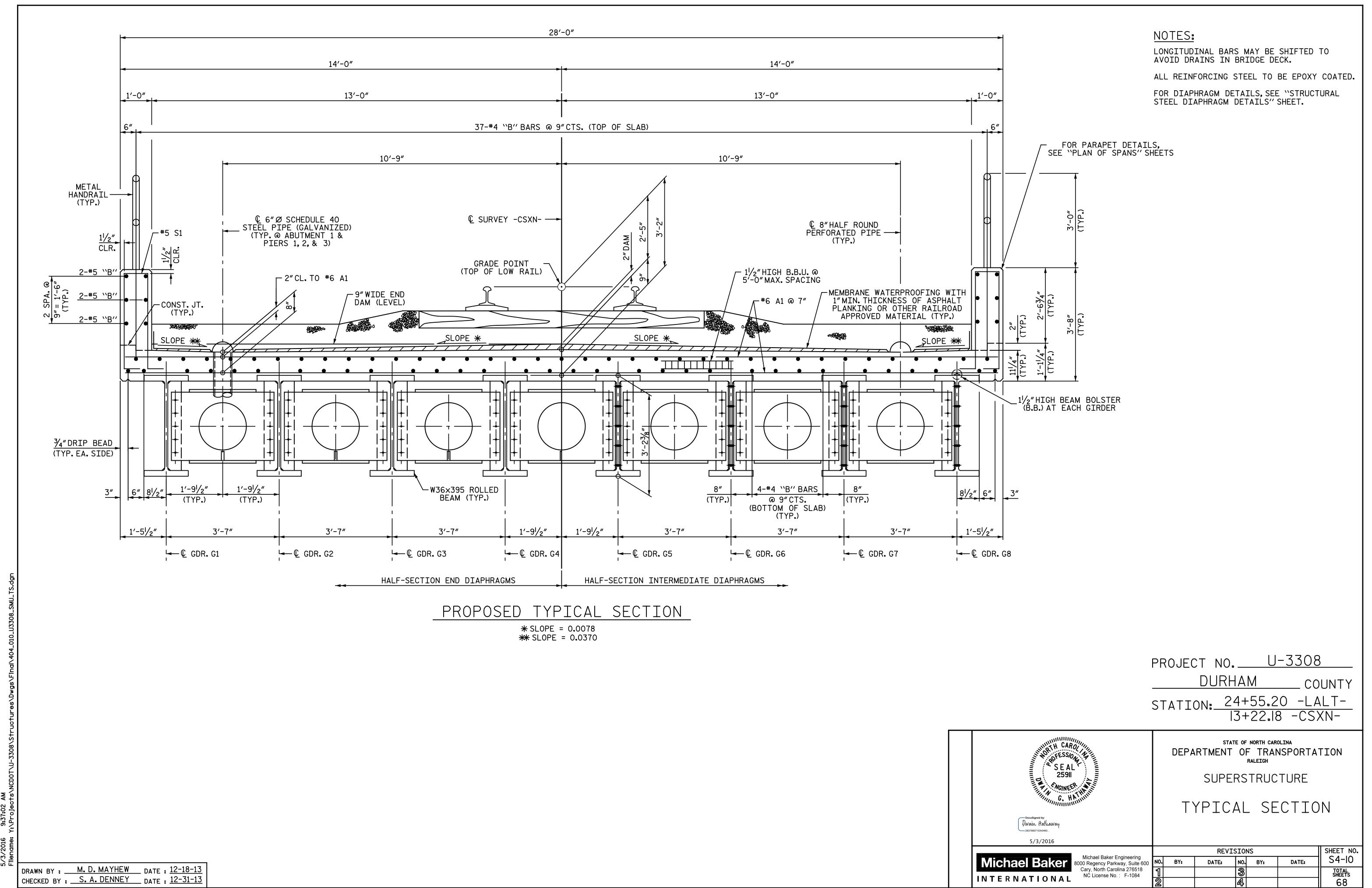
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

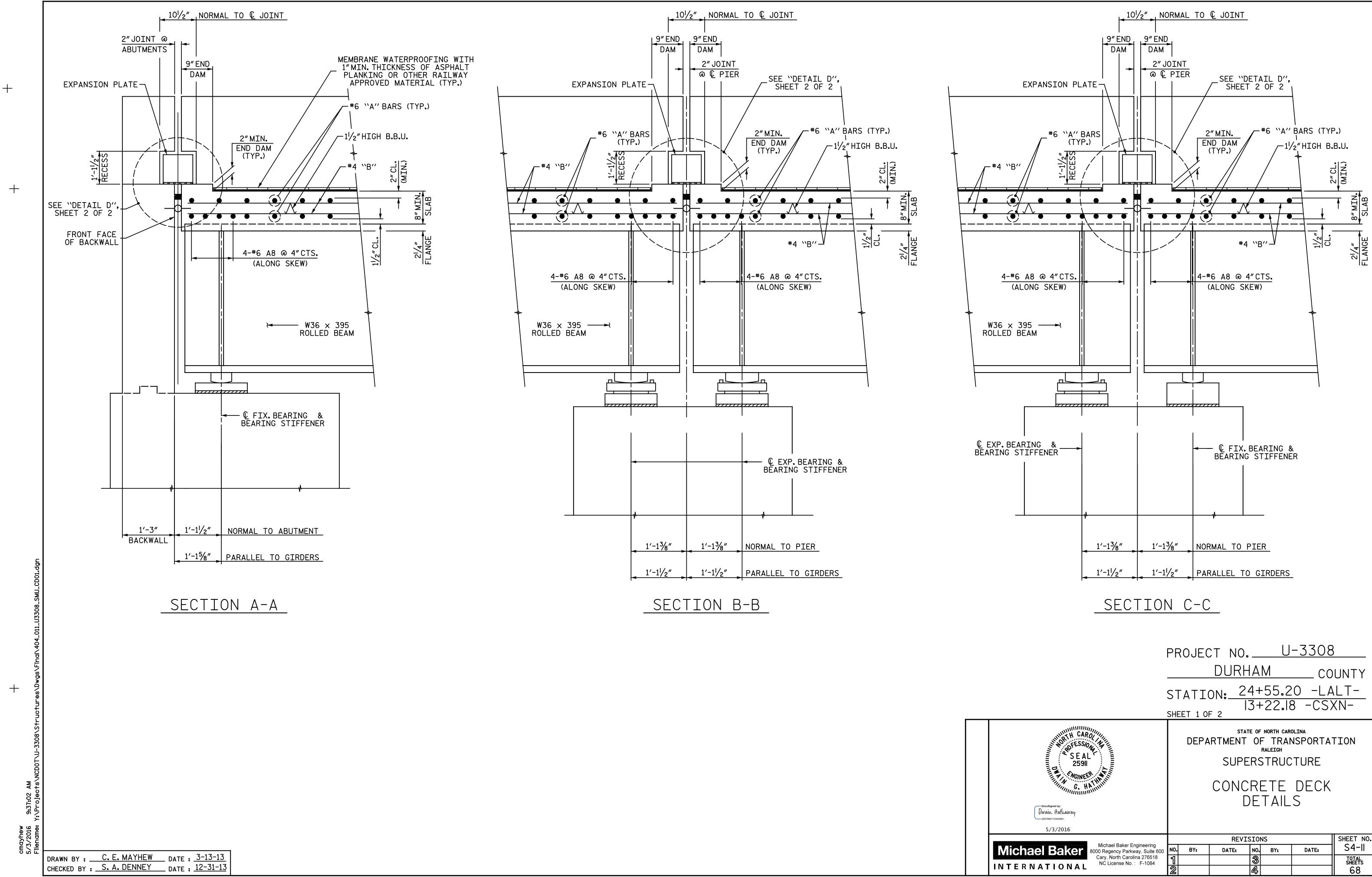
STAGING

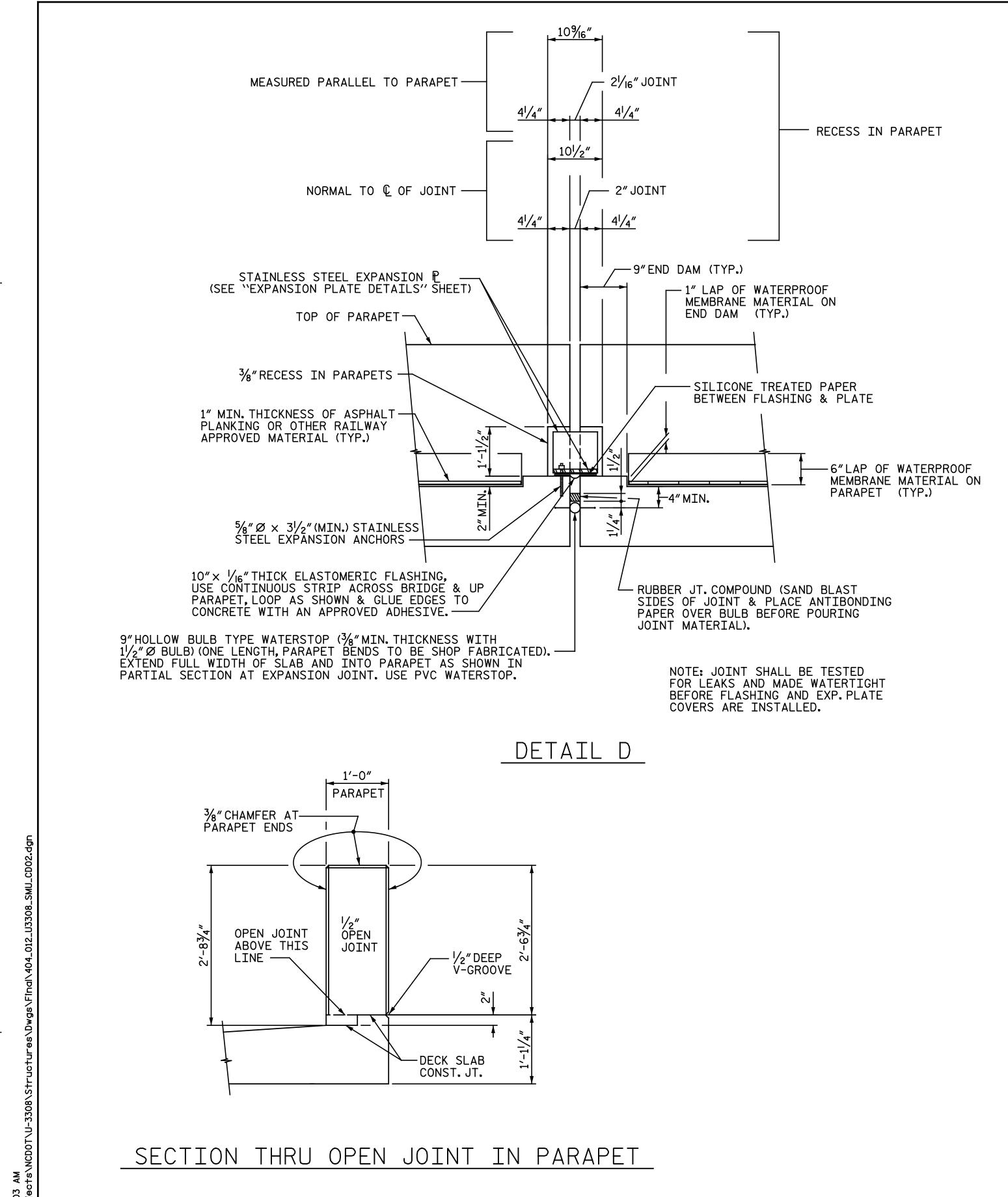
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Michael Baker BY: Cary, North Carolina 276518 NC License No.: F-1084 INTERNATIONAL

SHEET NO. **REVISIONS** S4-9 NO. BY: DATE: DATE: TOTAL SHEETS

CHECKED BY: KGB / DGH DATE: 6-14-07







EXPANSION PLATE

EXPANSION PLATE

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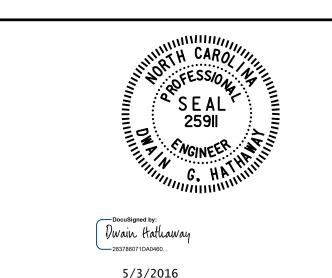
PART SECTION AT EXPANSION JOINT

PROJECT NO. <u>U-3308</u>

<u>DURHAM</u> COUNTY

STATION: 24+55.20 -LALT-| 13+22.18 -CSXN-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

CONCRETE DECK DETAILS

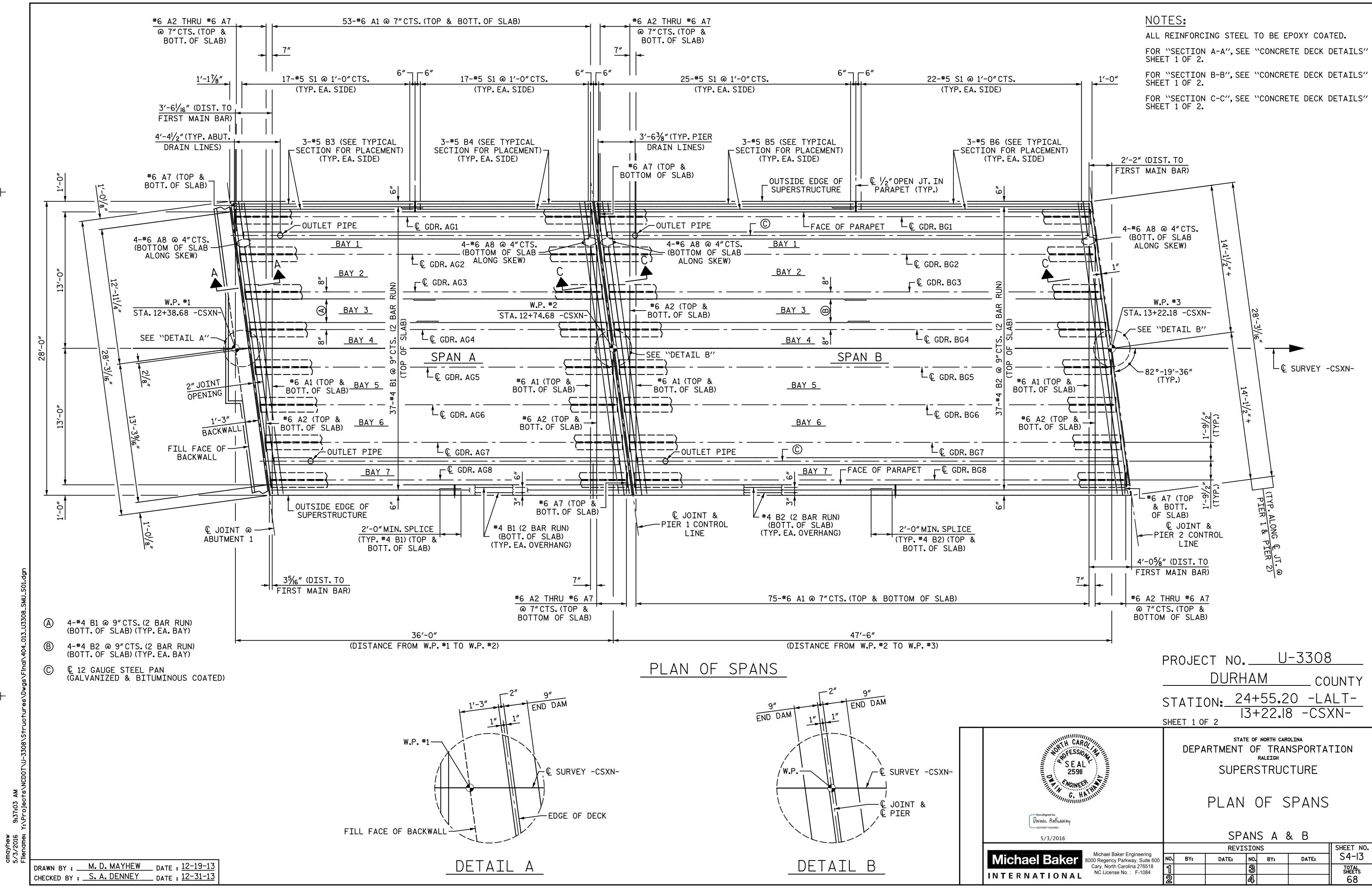
Michael Baker

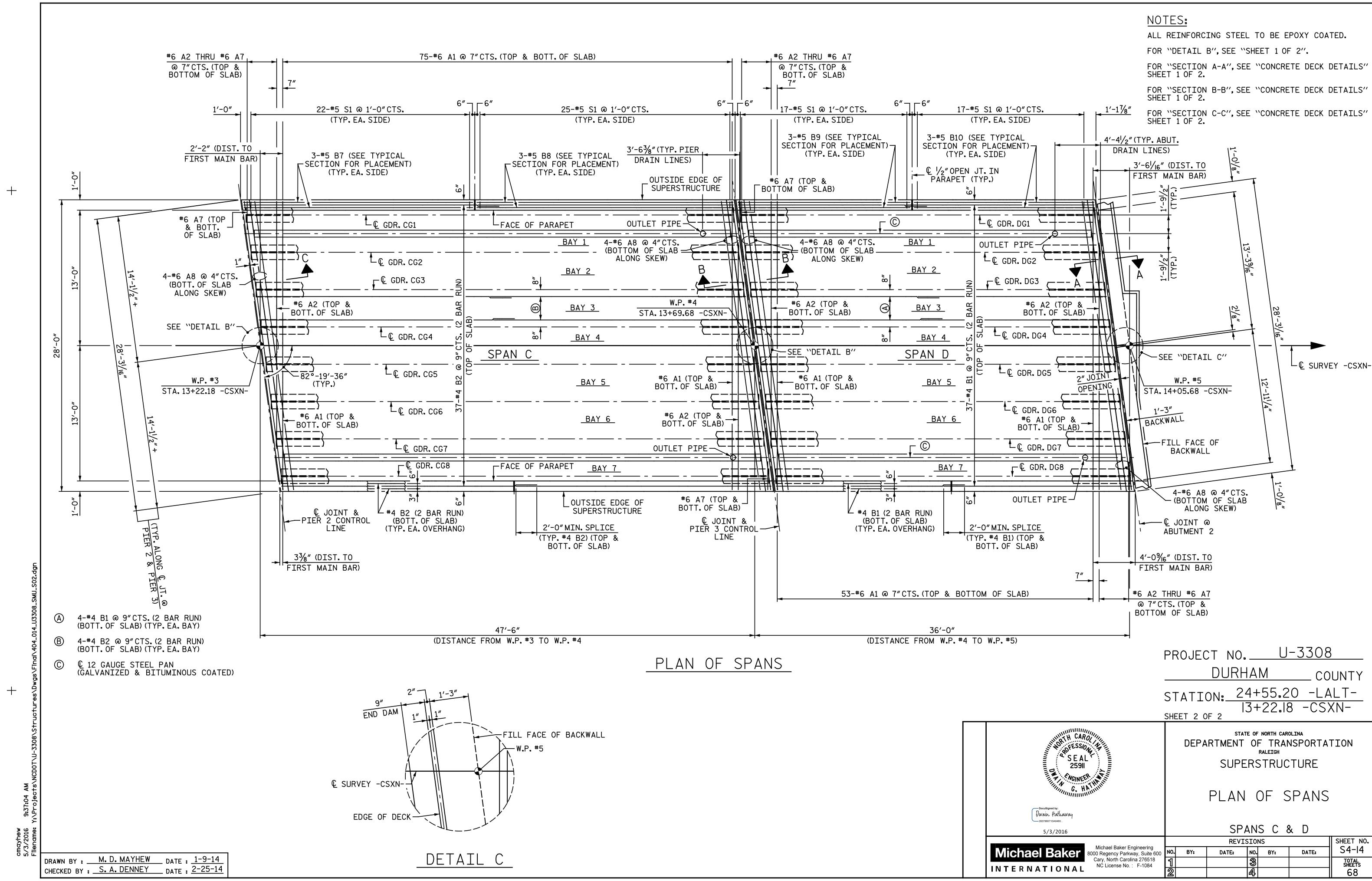
Michael Baker

8000 Regence
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NC Licer

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

DRAWN BY: C.E. MAYHEW DATE: 3-13-13
CHECKED BY: S. A. DENNEY DATE: 12-31-13

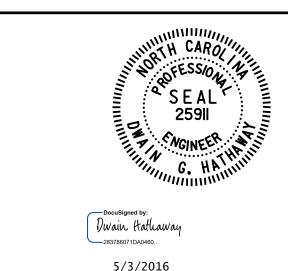




STRUCTURAL STEEL NOTES

- 1. SPECIFICATIONS: AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (A.R.E.M.A.) MANUAL FOR RAILWAY ENGINEERING, 2014 EDITION.
 - CURRENT CSX TRANSPORTATION CRITERIA FOR OPEN DECK RAILROAD BRIDGES.
 - CURRENT NORFOLK SOUTHERN RAILWAY GUIDELINES FOR DESIGN OF GRADE SEPARATION STRUCTURES.
- 2. LIVE LOADS: COOPER E-80 WITHOUT HAMMER BLOW WITH FULL DESIGN IMPACT AS PER A.R.E.M.A.
- 3. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CSX TRANSPORTATION "SPECIFICATIONS FOR STRUCTURAL STEEL".
- 4. STRUCTURAL STEEL: ALL STRUCTURAL STEEL FOR FRACTURE CRITICAL MEMBERS (FCM) SHALL BE ASTM A709, GRADE 50F2, Fy = 50,000 PSI. NON-FRACTURE CRITICAL MEMBERS SHALL BE ASTM A709, GRADE 50T2. BEARING ASSEMBLIES SHALL BE ASTM A709 GRADE 50T2, ALL STRUCTURAL STEEL SHALL MEET THE REQUIREMENTS OF THE PROJECT SPECIAL PROVISION FOR STRUCTURAL STEEL FOR RAILROAD BRIDGES AND THE FOLLOWING REQUIREMENTS:
 - 1. THE MATERIAL SUPPLIED SHALL BE OTHER THAN RIMMED OR CAPPED STEEL
 - 2. THE MATERIAL SUPPLIED SHALL BE SILCON-KILLED, FINE GRAIN PRACTICE.
 - 3. CERTAIN ELEMENTS OF THE STRUCTURE ARE NOTED AS "FRACTURE CRITICAL MEMBERS" (FCM) AND SHALL MEET THE REQUIREMENTS FOR "FRACTURE CONTROL PLAN FOR FRACTURE CRITICAL MEMBERS" OF SECTION 1.14, CHAPTER 15 OF THE A.R.E.M.A. MANUAL. THE IMPACT REQUIREMENTS FOR FRACTURE CRITICAL MEMBERS SHALL BE REQUIRED FOR ZONE 2 SERVICE TEMPERATURE, TEST RESULTS SHALL BE FURNISHED TO THE ENGINEER AND RAILROAD FOR THEIR USE.
 - 4. ANCHOR BOLTS FOR BEARING DEVICES SHALL CONFORM TO ASTM F1554 GRADE 55. ANCHOR BOLTS, NUT, AND PLATE WASHERS SHALL BE MECHANICALLY GALVANIZED IN ACCORDANCE WITH AASHTO M298.
- 5. ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED.
- 6. STRUCTURAL STEEL SHALL BE PAINTED IN ACCORDANCE WITH CSX TRANSPORTATION "SPECIFICATIONS FOR PAINTING STRUCTURAL STEEL".
- 7. BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.
- 8. FABRICATION: THE FABRICATOR SHALL BE CERTIFIED IN THE SIMPLE SPAN STEEL BRIDGE CATEGORY, UNDER THE AISC QUALITY CERTIFICATION PROGRAM.
- 9. FABRICATION OF ALL STEEL MEMBERS SHALL BE ACCORDING TO THE A.R.E.M.A. MANUAL FOR RAILWAY ENGINEERING, CHAPTER 15, PART 3 - FABRICATION.
- 10. MILL TEST REPORTS: CSX TRANSPORTATION AND NORFOLK SOUTHERN RAILWAY SHALL BE FURNISHED COPIES OF MILL TEST REPORTS FOR ALL MATERIALS EXCEPT MISCELLANEOUS PLATES AND SHAPES, REPORTS SHALL INDICATE COMPLIANCE WITH ALL SPECIFIED REQUIREMENTS.
- 11. INSPECTION: SHOP INSPECTION BY CSX TRANSPORTATION OR ITS AUTHORIZED AGENT.
- 12. SHOP DRAWINGS: SHOP DRAWINGS SHALL BE APPROVED BY THE CHIEF-ENGINEER BRIDGES, CSX TRANSPORTATION, JACKSONVILLE, FL.
- 13. HOLES: OPEN HOLES AS NOTED.
- 14. BOLTED CONNECTIONS SHALL BE MADE WITH $\frac{7}{8}$ " DIAMETER, ASTM A325, TYPE 1 HIGH STRENGTH BOLTS WITH HARDENED WASHERS IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS USING THE TURN OF THE NUT METHOD. DIRECT TENSION INDICATORS SHALL NOT BE USED.
- 15. HIGH STRENGTH BOLTS, NUTS & WASHERS: ASTM DESIGNATION A325. ALL HIGH STRENGTH BOLTS, NUTS & WASHERS SHALL BE MECHANICALLY GALVANIZED. HOT-DIPPED GALVANIZED BOLTS WILL NOT BE ALLOWED.
- 16. ANCHOR BOLTS: SHALL BE $1^{1}/4^{\prime\prime}$ Ø IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS AND SHALL BE GROUTED IN FORMED HOLES AFTER GIRDERS ARE ERECTÉD.
- 17. BEARING PADS SHALL BE USED WHENEVER STEEL MASONRY PLATE, OR OTHER STEEL BEARING PLATE, BEARS ON CONCRETE. PADS SHALL BE PREFORMED FABRIC BEARING PADS, $\frac{1}{2}$ THICK, 31 PLY. PREFORMED FABRIC BEARING PADS SHALL BE SHOCK PAD (MIL-C-882C SPECIFICATIONS) STYLE NO. 15175 AS MANUFACTURED BY ALERT MANUFACTURING AND SUPPLY COMPANY. CHICAGO, ILLINOIS, OR FABREEKA PADS AS MANUFACTURED BY FABREEKA PRODUCTS COMPANY, BOSTON, MASSACHUSETTS, OR SORBTEX PADS AS MANUFACTURED BY VOSS ENGINEERING, INC., CHICAGO, ILLINOIS, OR APPROVED EQUAL.
- 18. NO CAMBER FOR GIRDERS REQUIRED, TURN NATURAL MILL CAMBER UP.

PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

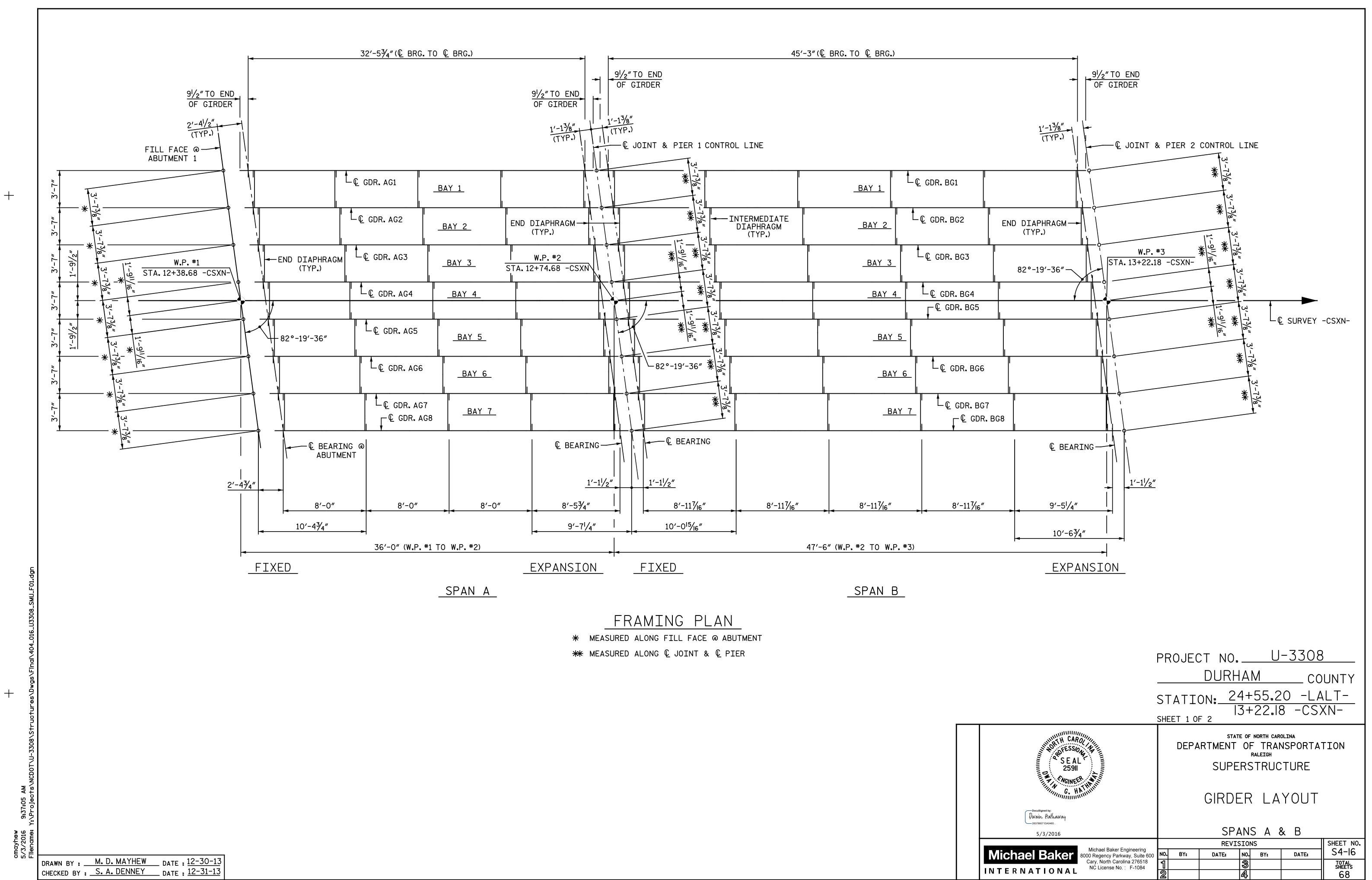
STRUCTURAL STEEL NOTES

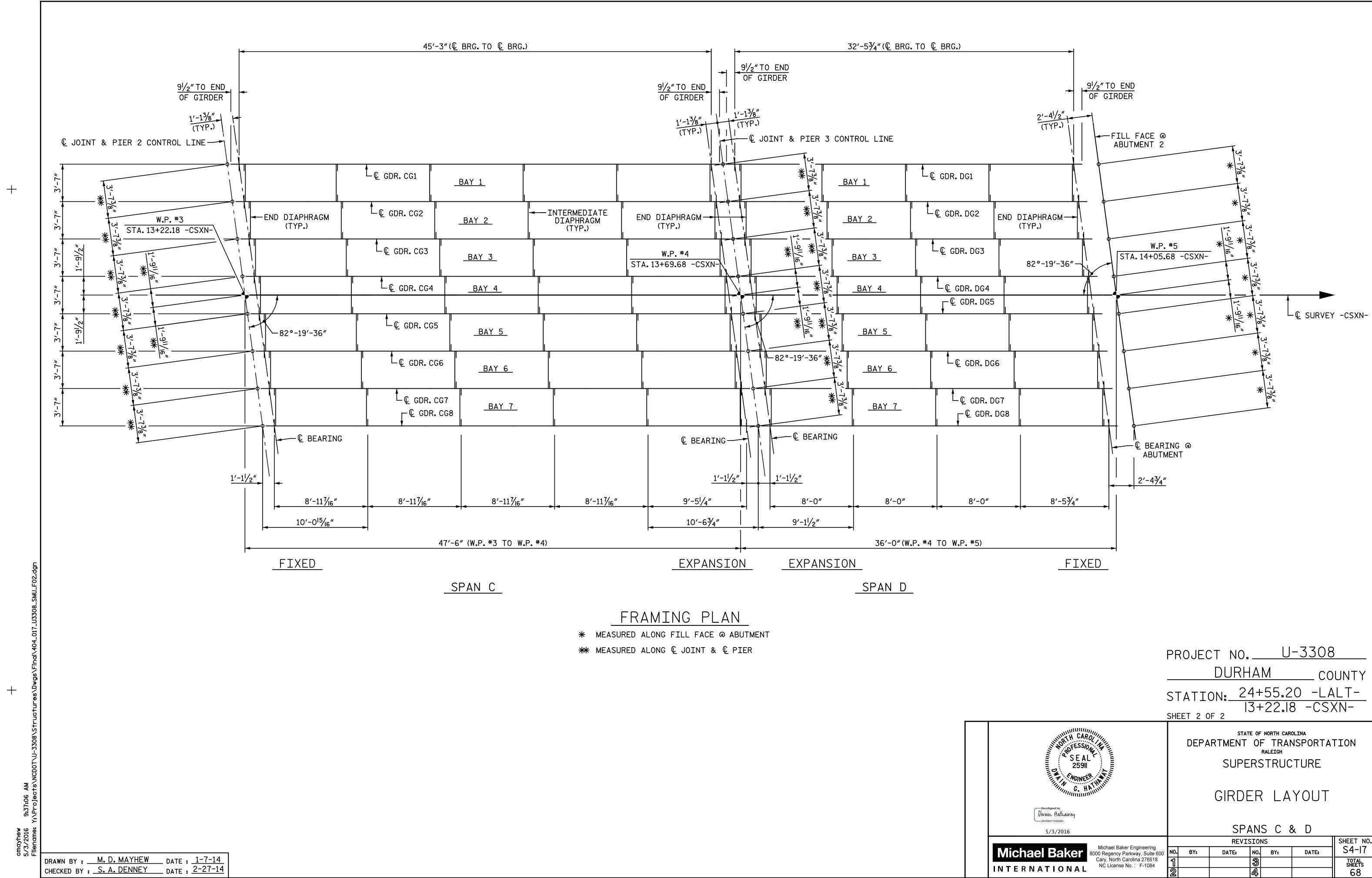
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INTERNATIONAL	NO

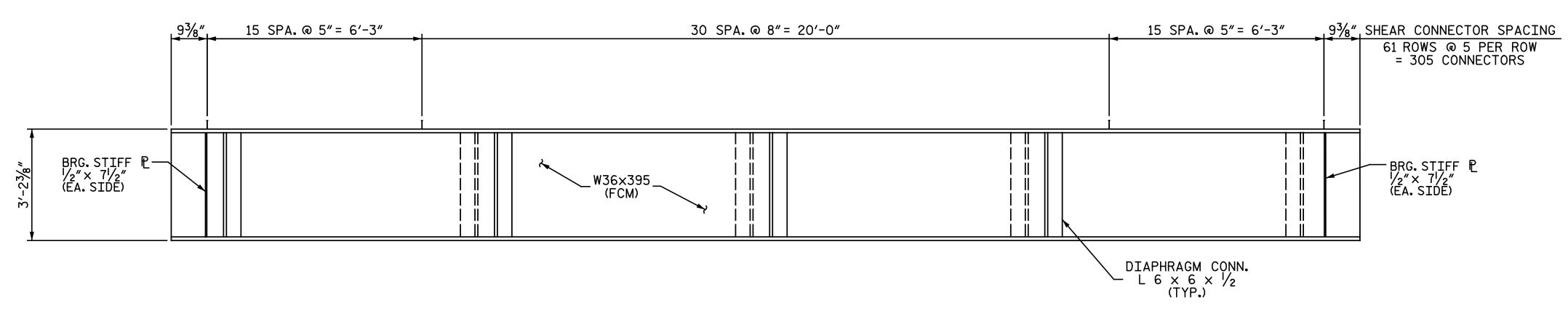
ichael Baker Engineering Regency Parkway, Suite 600 ary, North Carolina 276518 NC License No. : F-1084

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	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S4-I5
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CHECKED BY: S. A. DENNEY DATE: 2-26-14

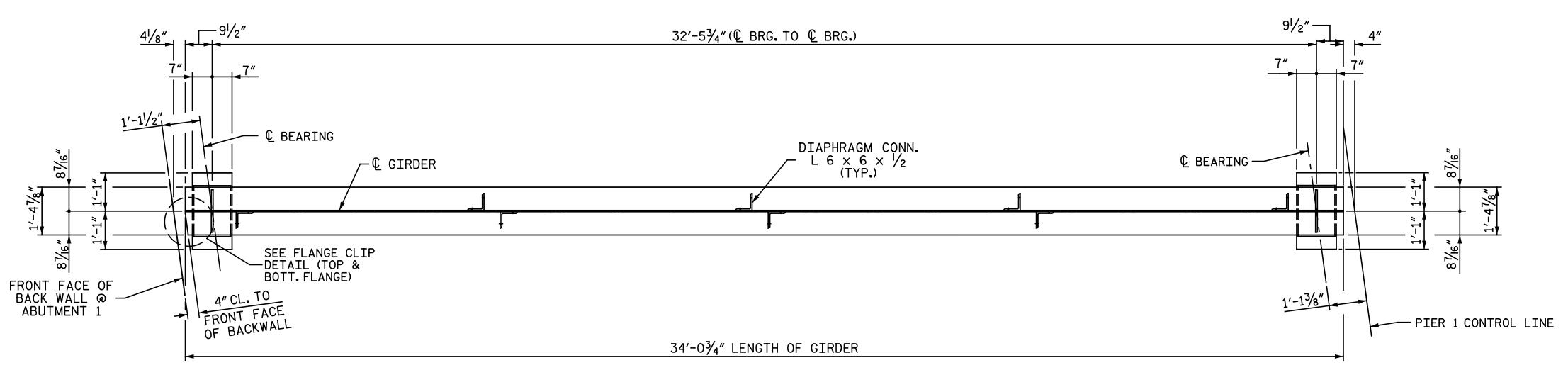






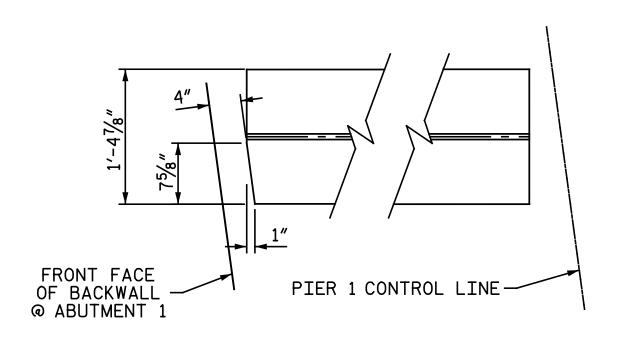
GIRDER ELEVATION - SPAN A

(INTERIOR GIRDER SHOWN, OTHERS SIMILAR)



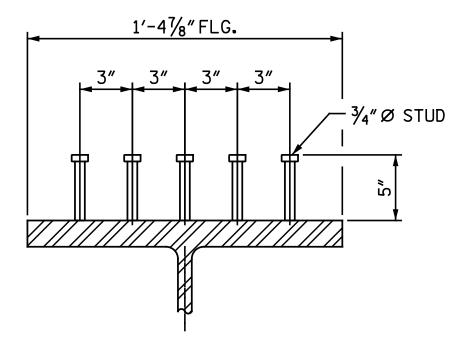
BOTTOM FLANGE DETAIL - SPAN A

(INTERIOR GIRDER SHOWN, OTHERS SIMILAR)
DIAPHRAGM CONNECTORS SHALL NOT BE PLACED ON
OUTSIDE OF EXTERIOR GIRDERS.



FLANGE CLIP DETAIL - SPAN A

NOTE: CLIP TOP & BOTTOM FLANGES AT EACH END OF GIRDER.



SHEAR STUD DETAIL

PROJECT NO. U-3308

DURHAM COUNTY

STATION: 24+55.20 -LALT-

| 13+22.18 -CSXN-

SHEET 1 OF 4



DEPARTMENT OF TRANSPORTATION RALEIGH
SUPERSTRUCTURE

STATE OF NORTH CAROLINA

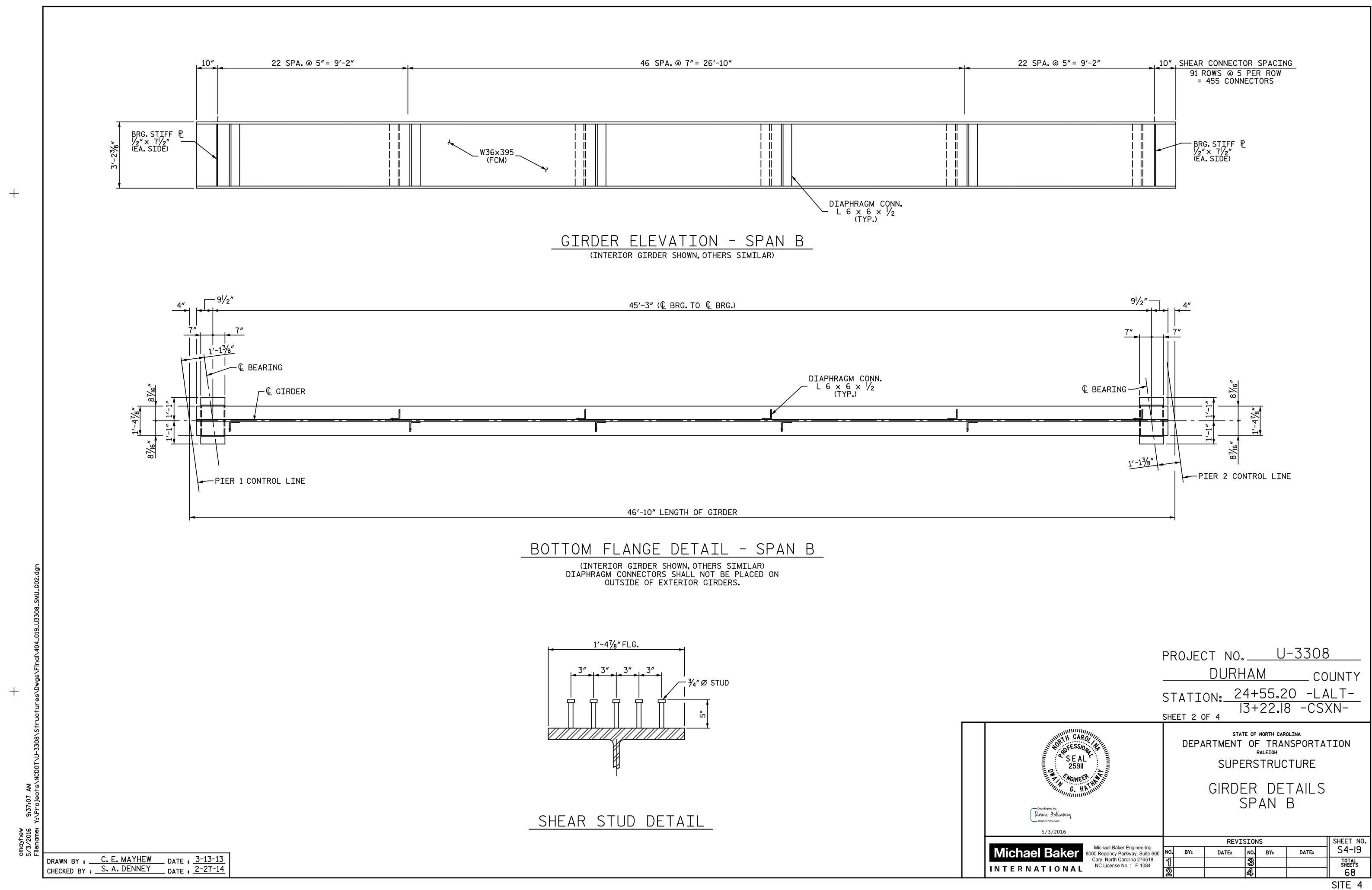
GIRDER DETAILS SPAN A

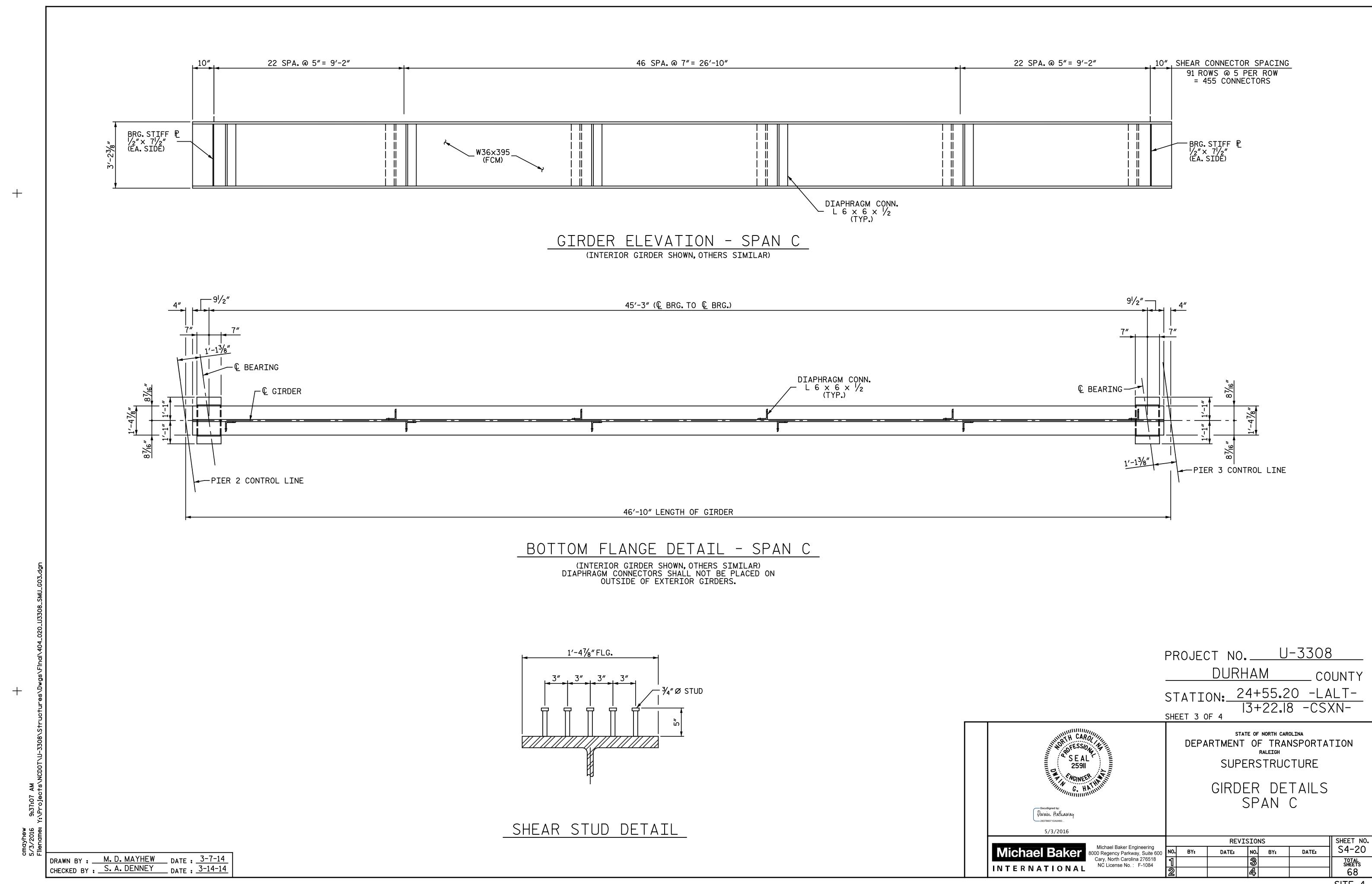
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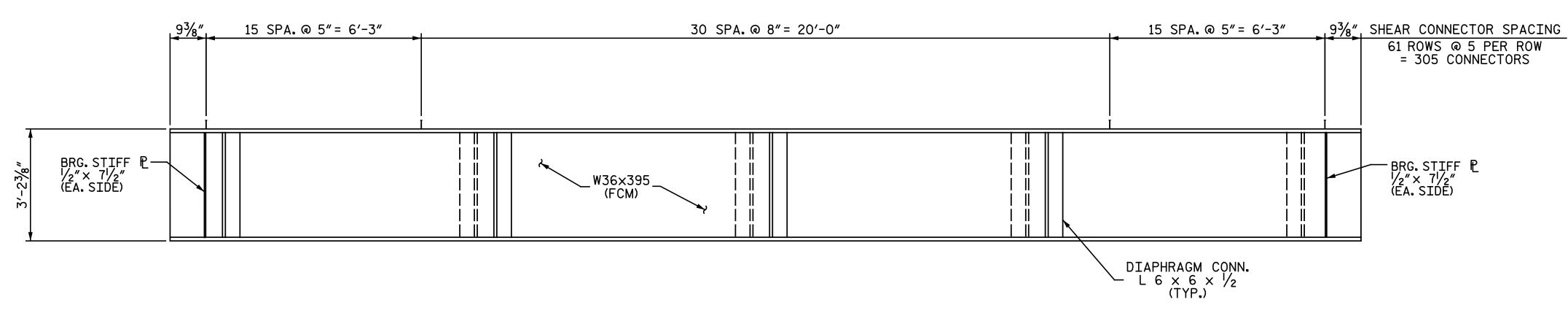
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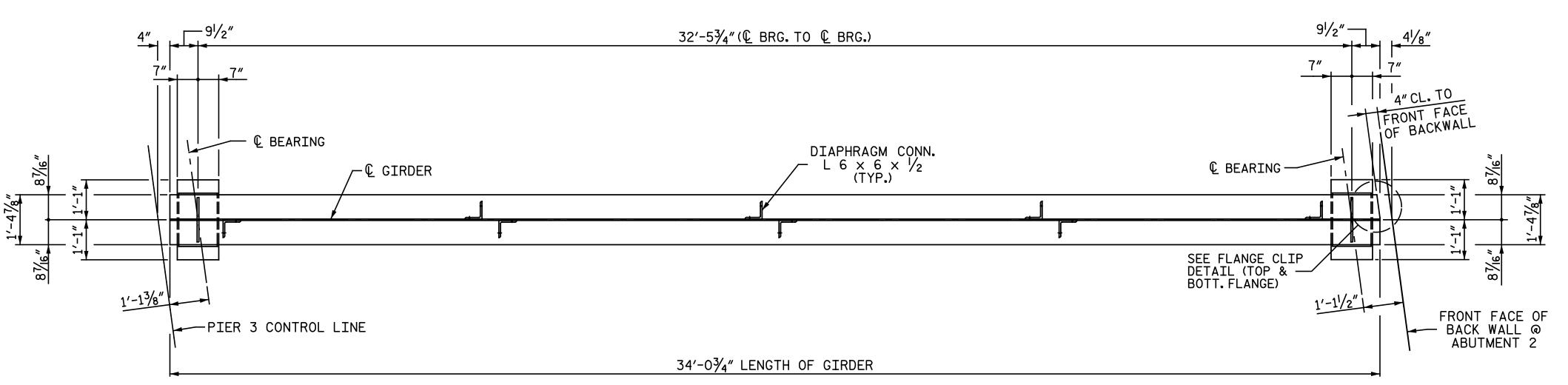






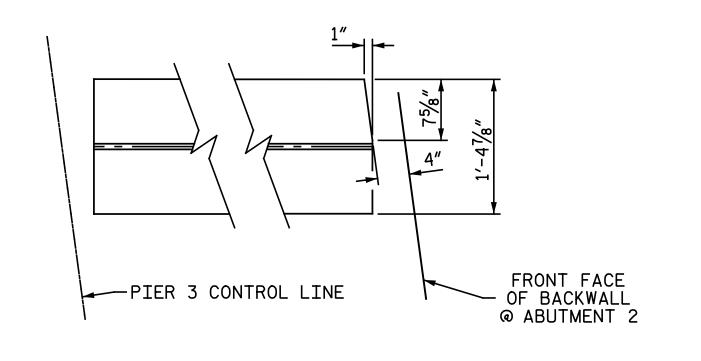
GIRDER ELEVATION - SPAN D

(INTERIOR GIRDER SHOWN, OTHERS SIMILAR)

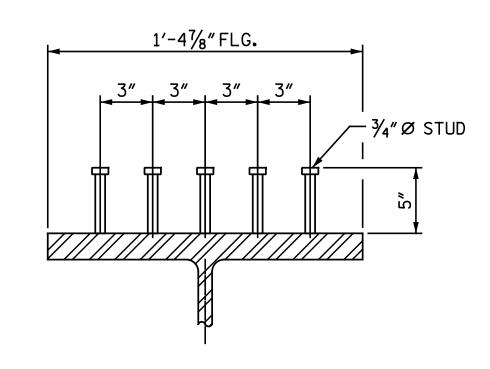


BOTTOM FLANGE DETAIL - SPAN D

(INTERIOR GIRDER SHOWN, OTHERS SIMILAR)
DIAPHRAGM CONNECTORS SHALL NOT BE PLACED ON
OUTSIDE OF EXTERIOR GIRDERS.

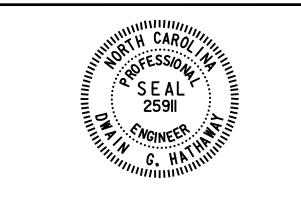


FLANGE CLIP DETAIL - SPAN D NOTE: CLIP TOP & BOTTOM FLANGES AT EACH END OF GIRDER.



SHEAR STUD DETAIL

PROJECT NO. U-3308 DURHAM . COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

SHEET 4 OF 4

GIRDER DETAILS SPAN D

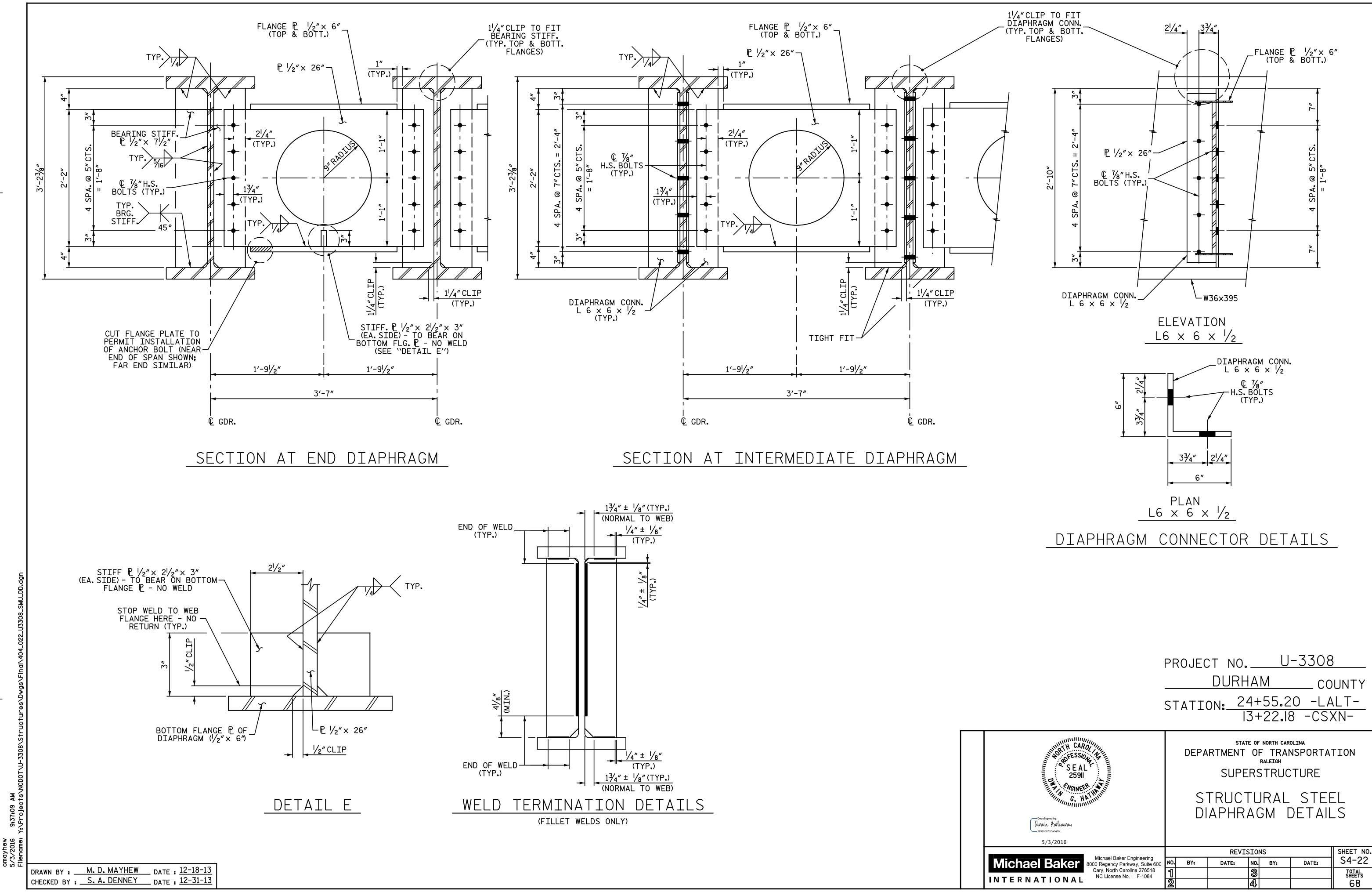
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Dwain Hathaway

Michael Baker Engineering
8000 Regency Parkway, Suite 600
No. BY: Cary, North Carolina 276518 NC License No.: F-1084

REVISIONS SHEET NO. S4-2I NO. BY: DATE: DATE: TOTAL SHEETS 68

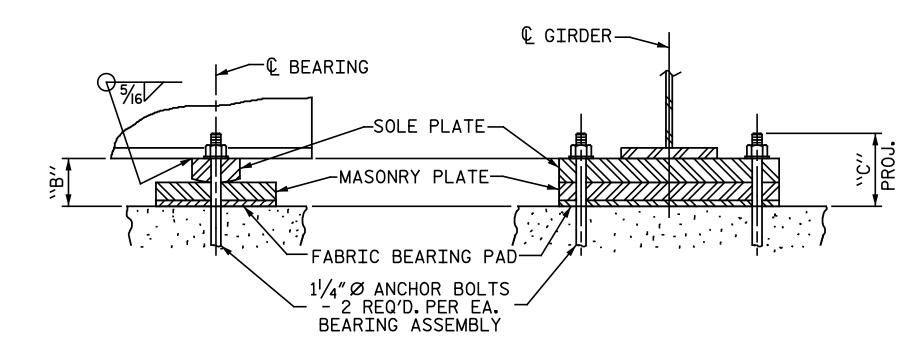
DRAWN BY: C.E. MAYHEW DATE: 3-13-13
CHECKED BY: S.A. DENNEY DATE: 3-14-14



BEARINGS - FIXED END

SOLE PLATE

MASONRY PLATE



BEARING ASSEMBLY - FIXED END

(32 REQ'D)

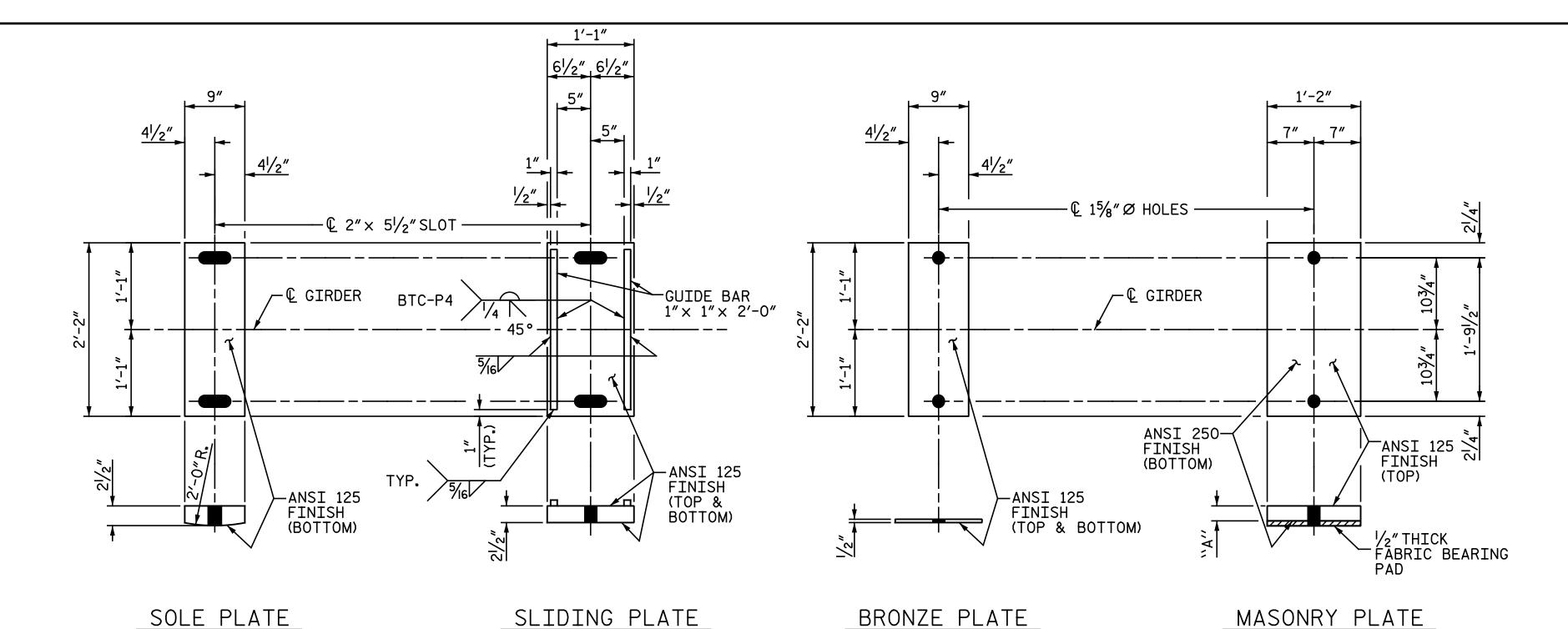
NOTES:

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND GIVEN AN ADDITIONAL 1/4 TURN, THE THREAD OF THE NUT AND BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

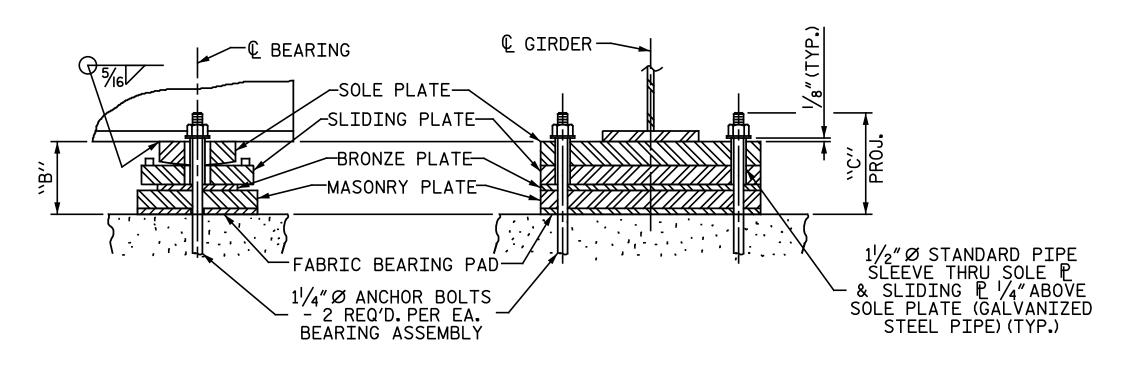
BEARING PADS SHALL BE PREFORMED FABRIC BEARING PADS, 1/2" THICK AND SHALL BE EITHER SHOCK PAD STYLE 15175, AS MANUFACTURED BY THE ALERT MANUFACTURING AND SUPPLY COMPANY, CHICAGO, IL; OR FABREEKA PADS AS MANUFACTURED BY FABREEKA PRODUCTS COMPANY, BOSTON, MA. OR SORBTEX PADS AS MANUFACTURED BY VOSS ENGINEERING, INC. CHICAGO, IL OR AN APPROVED EQUAL.

ALL BEARING PLATES AND PIPE SLEEVES ARE TO BE GALVANIZED. SEE STD. SPEC. 1072-13.

AT THE CONTRACTOR'S OPTION, A FILL PLATE MAY BE USED IN COMBINATION WITH MASONRY PLATE TO OBTAIN REQUIRED PLATE THICKNESS SHOWN IN TABLE. IF A FILL PLATE IS USED, MINIMUM MASONRY PLATE THICKNESS SHALL BE 21/4". MINIMUM FILL PLATE THICKNESS SHALL BE 11/4". FILL PLATES SHALL HAVE THE SAME LENGTH, WIDTH AND HOLE PATTERN AS THE MASONRY PLATE.



BEARINGS - EXPANSION END



BEARING ASSEMBLY - EXPANSION END

(32 REQ'D)

	MASONRY PLATE SCHEDULE (DIMENSIONS IN INCHES)												
	FIXED E	ND @ ABU	TMENT 1	EXPANS:	ION END	@ PIERS	FIXE	END @ F	PIERS	FIXED END @ ABUTMENT 2			
GIRDER	\`A''	\\B''	,,C,,	``A''	\\B''	''C''	\\A''	\\B''	,,C,,	``A''	\`B''	''C''	
1	21/4"	5 ¹ / ₄ "	71/2"	21/4"	8 ¹ /4"	11"	5 ¹ / ₄ "	8 ¹ /4"	11"	21/4"	5 ¹ / ₄ "	71/2"	
2	21/4"	5 ¹ / ₄ "	71/2"	21/4"	8 ¹ / ₄ "	11"	5 ¹ / ₄ "	8 ¹ / ₄ "	11"	21/4"	5 ¹ / ₄ "	71/2"	
3	21/4"	5 ¹ /4"	71/2"	21/4"	8 ¹ / ₄ "	11"	5 ¹ / ₄ "	8 ¹ /4"	11"	21/4"	5 ¹ /4"	71/2"	
4	2 ¹ / ₄ "	5 ^l / ₄ "	7 ¹ /2"	2 ¹ / ₄ "	8 ¹ / ₄ "	11"	5 ¹ / ₄ "	8 ¹ /4"	11"	21/4"	5 ¹ /4"	71/2"	
5	21/4"	5 ¹ / ₄ "	71/2"	21/4"	81/4"	11"	5 ¹ / ₄ "	8 ¹ / ₄ "	11"	21/4"	5 ¹ / ₄ "	71/2"	
6	21/4"	5 ¹ /4"	71/2"	21/4"	8 ¹ / ₄ "	11"	5 ¹ /4"	8 ¹ /4"	11"	21/4"	5 ¹ /4"	71/2"	
7	21/4"	5 ¹ / ₄ "	71/2"	21/4"	8 ¹ / ₄ "	11"	5 ¹ / ₄ "	8 ¹ / ₄ "	11"	21/4"	5 ¹ / ₄ "	71/2"	
8	21/4"	5 ¹ / ₄ "	71/2"	21/4"	8 ¹ / ₄ "	11"	5 ¹ / ₄ "	8 ¹ / ₄ "	11"	21/4"	5 ¹ / ₄ "	71/2"	

1'-10\\\2" AB-1
2'-2" AB-2

PIERS 1 THRU 3

3"\Omega \times \frac{3}{8}" THICK
WASHER W/1\(\frac{5}{16}\)"\Omega HOLE

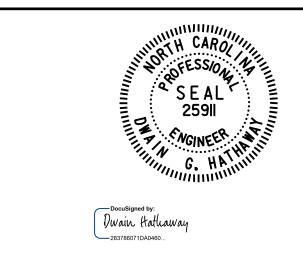
DETAIL OF ANCHOR BOLTS

NOTE: ANCHOR BOLTS SHALL HAVE
1'-3" MIN. EMBEDMENT

PROJECT NO. U-3308

DURHAM COUNTY

STATION: 24+55.20 -LALT-



DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE
BEARING DETAILS

SEARING DE

Michael Baker
INTERNATIONAL

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

REVISIONS

REVISIONS

NO. BY: DATE: NO. BY: DATE:

1084
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DRAWN BY: M.D. MAYHEW DATE: 1-7-14
CHECKED BY: S.A. DENNEY DATE: 3-3-14

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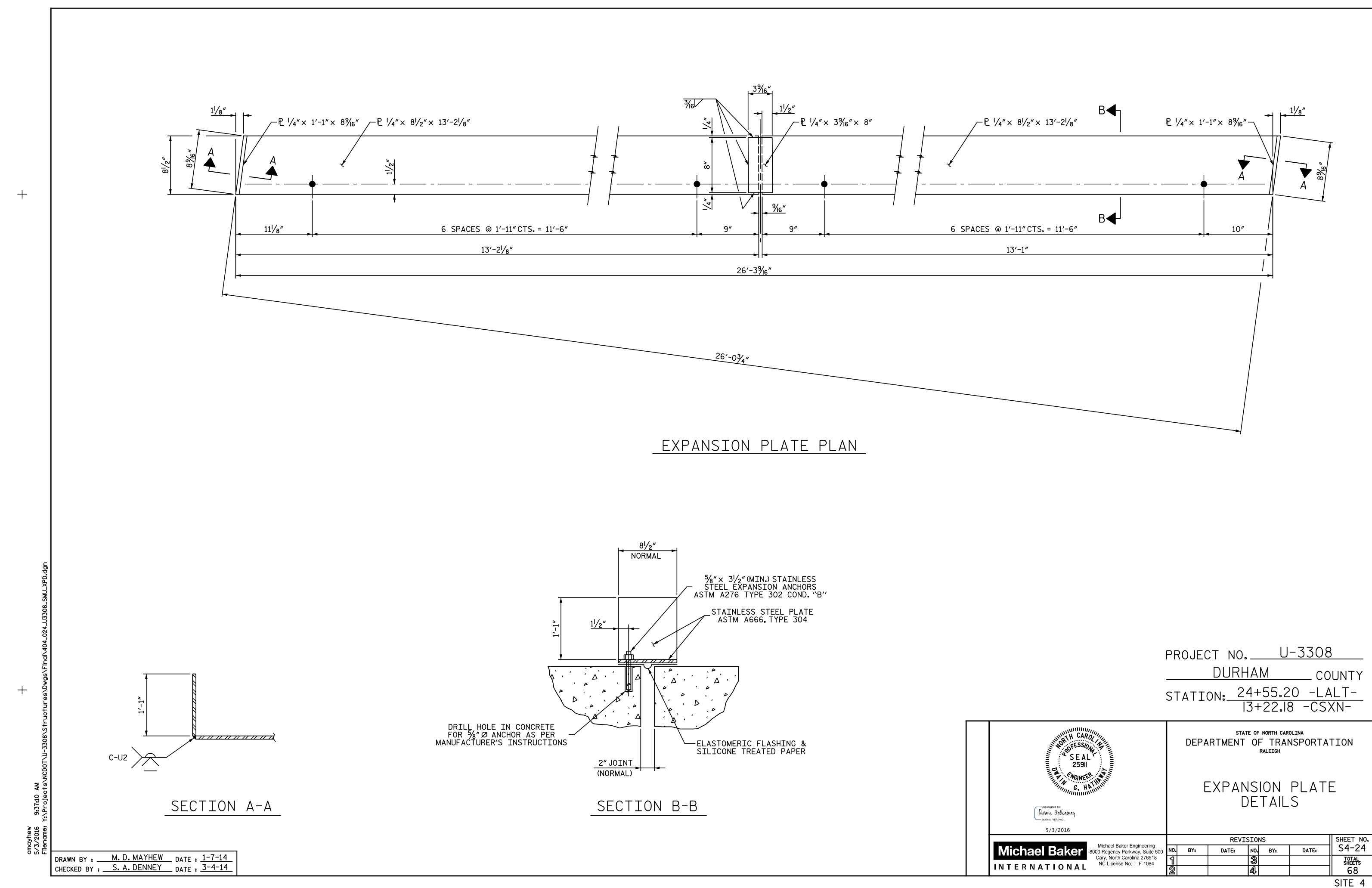
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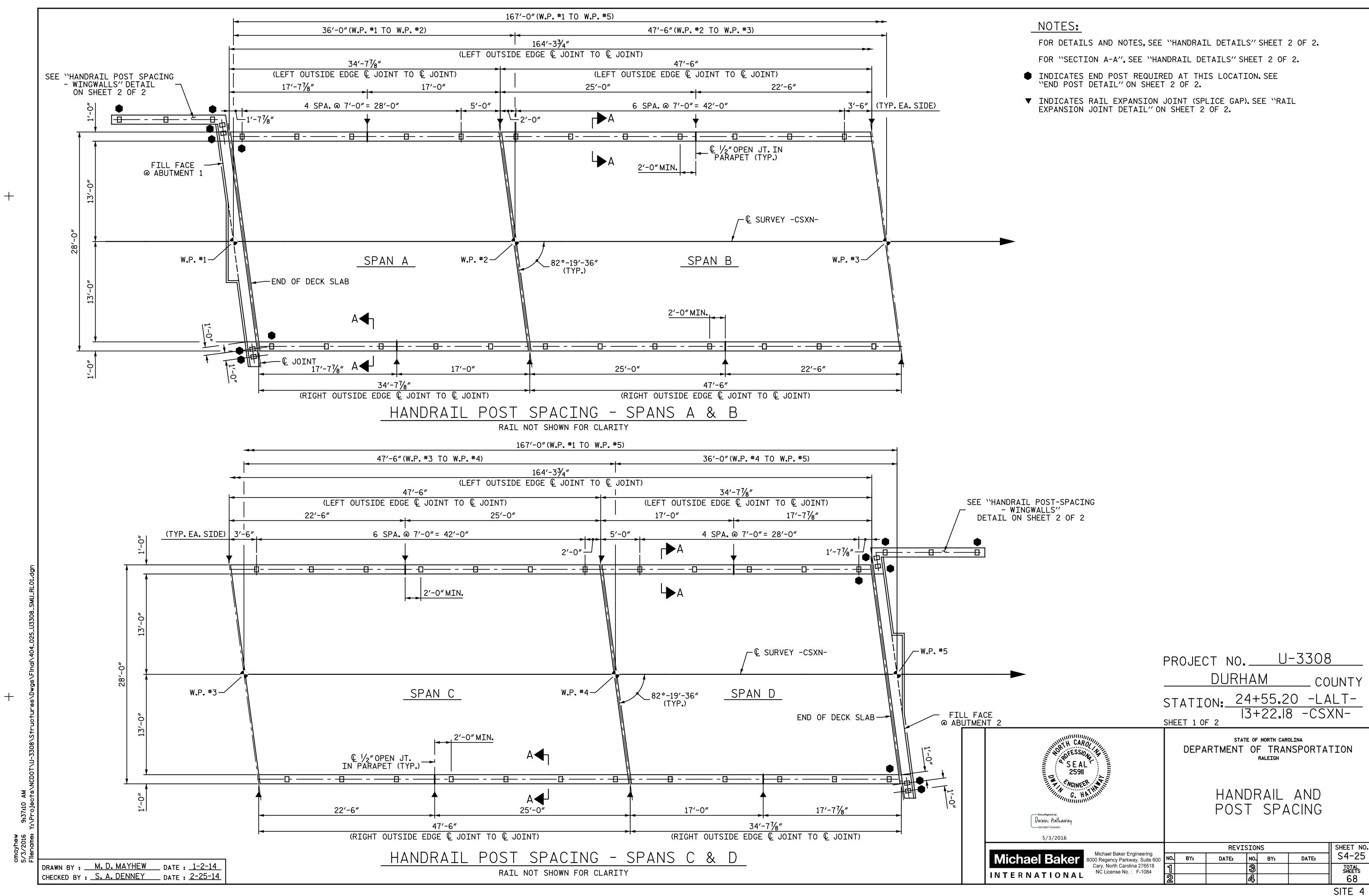
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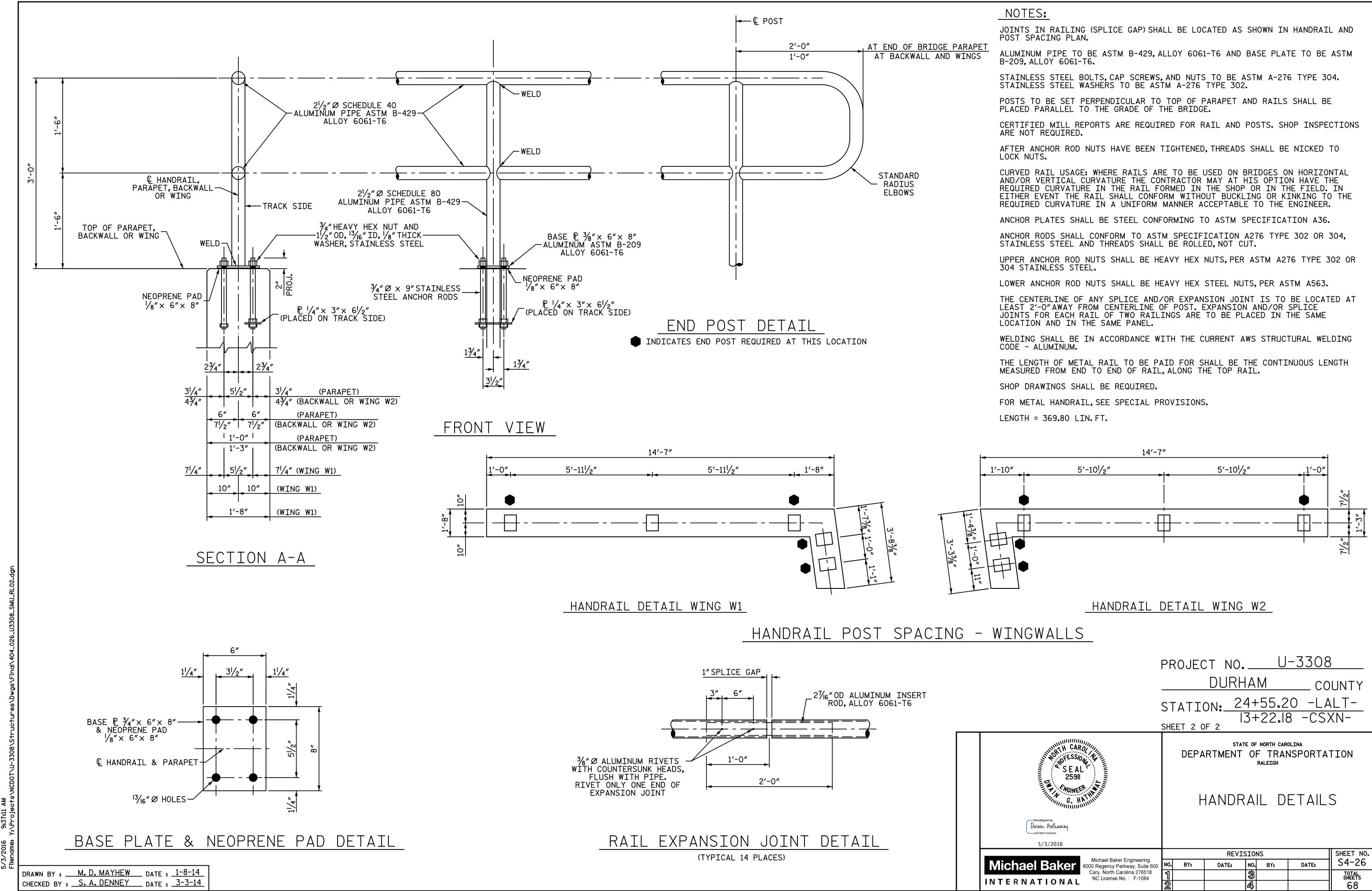
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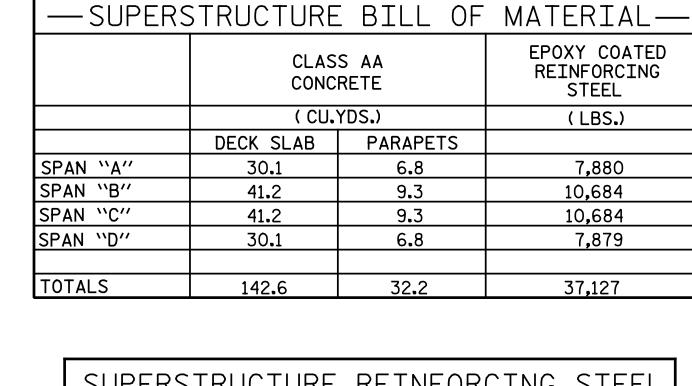
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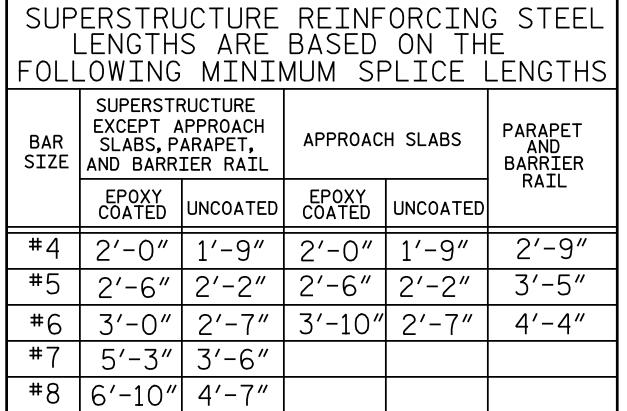


	REINFORCING STEEL SCHEDULE																						
		S	PAN	А		SPAN B						SPAN C					SPAN D						
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1 A2 A3 A4 A5 A6 A7	106 4 4 4 4 4	6666666	STR STR STR STR STR STR	27' - 8" 23' - 8" 19' - 4" 15' - 0" 10' - 8" 6' - 4" 2' - 7"	4,405 142 116 90 64 38 16	A1 A2 A3 A4 A5 A6 A7	150 4 4 4 4 4	6 6 6 6 6	STR STR STR STR STR STR	27' - 8" 23' - 8" 19' - 4" 15' - 0" 10' - 8" 6' - 4" 2' - 7"	6,233 142 116 90 64 38 16	A1 A2 A3 A4 A5 A6 A7	150 4 4 4 4 4	6 6 6 6 6 6	STR STR STR STR STR STR	27' - 8" 23' - 8" 19' - 4" 15' - 0" 10' - 8" 6' - 4" 2' - 7"	6,233 142 116 90 64 38 16	A1 A2 A3 A4 A5 A6 A7	106 4 4 4 4 4	6666666	STR STR STR STR STR STR	27' - 8" 23' - 8" 19' - 4" 15' - 0" 10' - 8" 6' - 4" 2' - 7"	4,405 142 116 90 64 38 16
B1 B3 B4 S1 EPOXY	8 138 12 12 68 COATEI	6 4 5 5 5 REINF	STR STR STR STR 1	27' - 11" 18' - 1" 17' - 1" 16' - 7" 8' - 3" LBS.	335 1,667 214 208 585 7,880	A8 B2 B5 B6 S1 EPOXY	8 138 12 12 94 COATED	6 4 5 5 5 REINF	STR STR STR STR 1	27' - 11" 24' - 6" 24' - 5" 22' - 1" 8' - 3" LBS.	335 2,259 306 276 809 10,684	B2 B7 B8 S1 EPOXY	8 138 12 12 94 COATED	6 4 5 5 5 REINF	STR STR STR STR 1	27' - 11" 24' - 6" 21' - 11" 24' - 7" 8' - 3" LBS.	335 2,259 274 308 809 10,684	B1 B9 B10 S1 EPOXY	8 138 12 12 68 COATEE	6 4 5 5 5 REINF	STR STR STR STR 1	27' - 11" 18' - 1" 16' - 5" 17' - 3" 8' - 3" LBS.	335 1,667 205 216 585 7,879

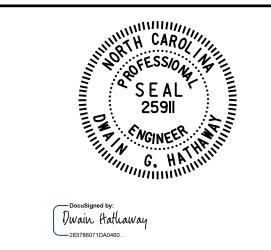


ALL BAR DIMENSIONS ARE OUT TO OUT

-BAR TYPES-



PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-|3+22.|8 -CSXN-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE

BILL OF MATERIAL

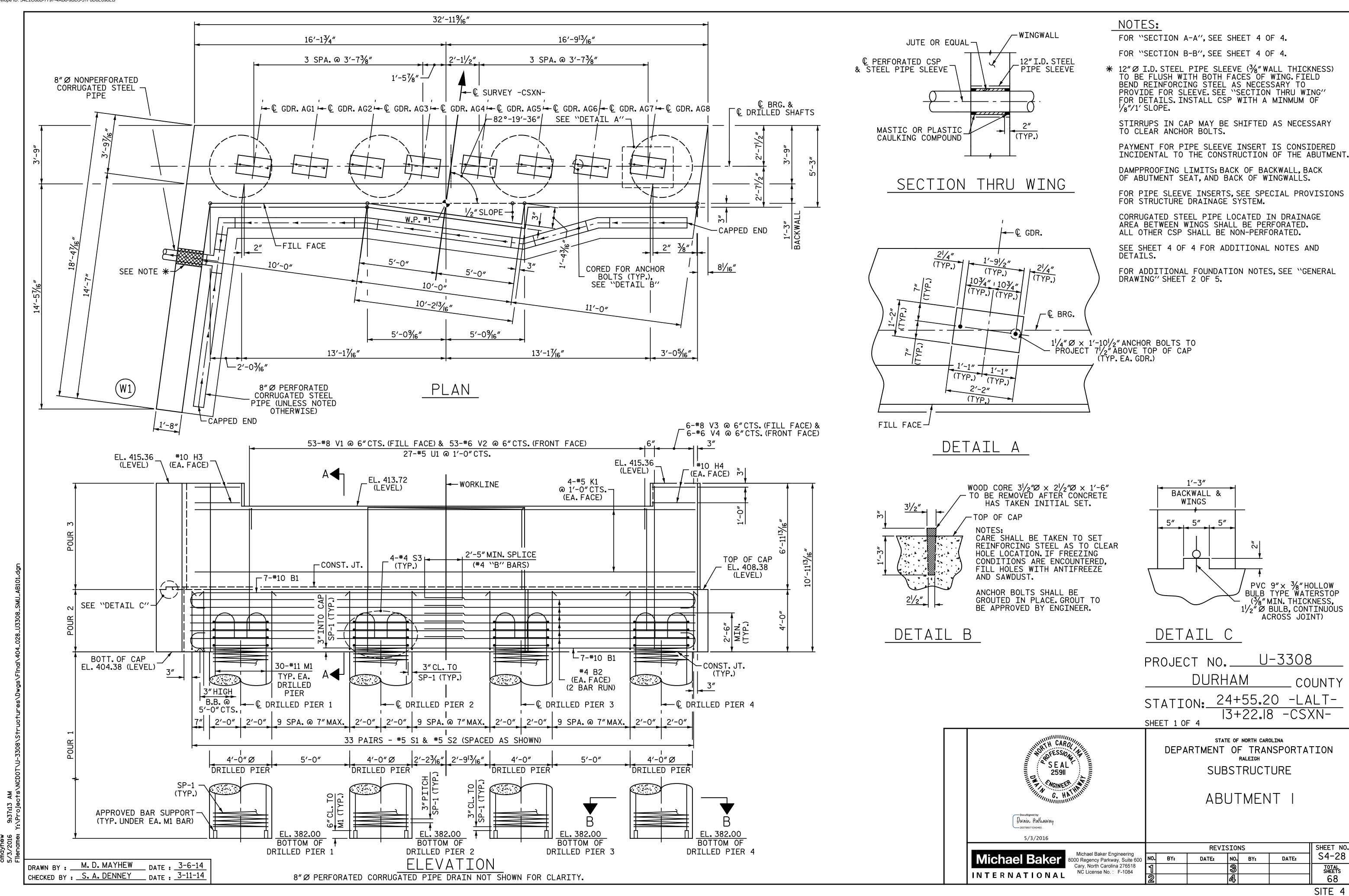
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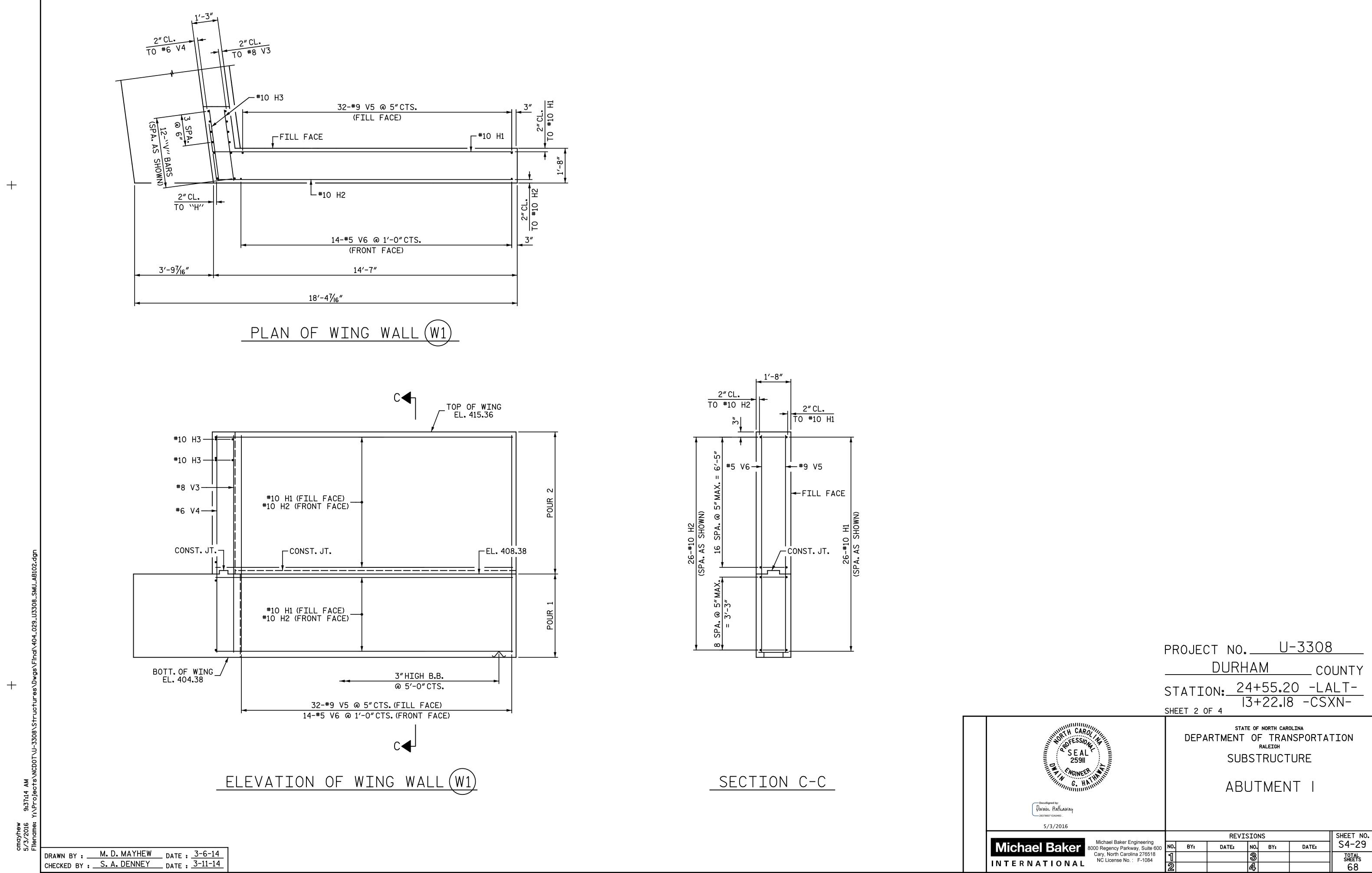
Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 276518 NC License No.: F-1084

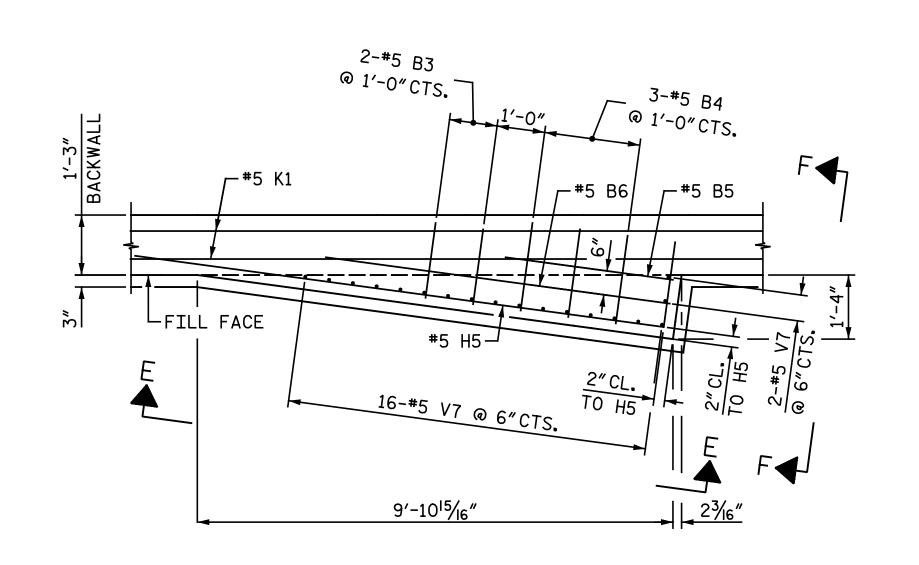
SHEET NO. S4-27 REVISIONS DATE: NO. BY: DATE: TOTAL SHEETS 68

_SMU_E	 -		164'-3 / ₁₆ " (©	C JT. TO C JT.)		≻	
13308		34′-7⅓″(€ JT. TO W.P. #2)	47'-6"(W.P. #2 TO W.P. #3)	47'-6"(W.P. #3 TO W.P. #4)	34′-7⅓″(W.P. #4 TO € JT.)		
104_027_1	1	SPAN A	SPAN B	SPAN C	SPAN D		
gs\Findl\4	FILL FACE @		PIER 1 CONTROL LINE & & EXP. JT.	PIER 2 CONTROL LINE & © EXP. JT.	PIER 3 CONTROL LINE & © EXP. JT.	FILL FACE @ ABUTMENT 2	
ctures\Dw			%; -0, -0, -0, -0, -0, -0, -0, -0, -0, -0,			W.P. #5 C SURVEY	-CSXN-
308\Struc	W.P. #1	W.P. #2-/\	82°-19′-36″ W.P. #3	82°-19′-36″ W.P. #4~	82°-19′-36″	82°-19′-36″	
0T\U-3	© JOINT →	-111	<u> </u>			U JOINT	
†s\NCD							
Projec			OF REINFORCE	OR COMPUTING AREA D_CONCRETE DECK SLAE	3 =====		
); 			(SQ.	FT. = 4,601)			

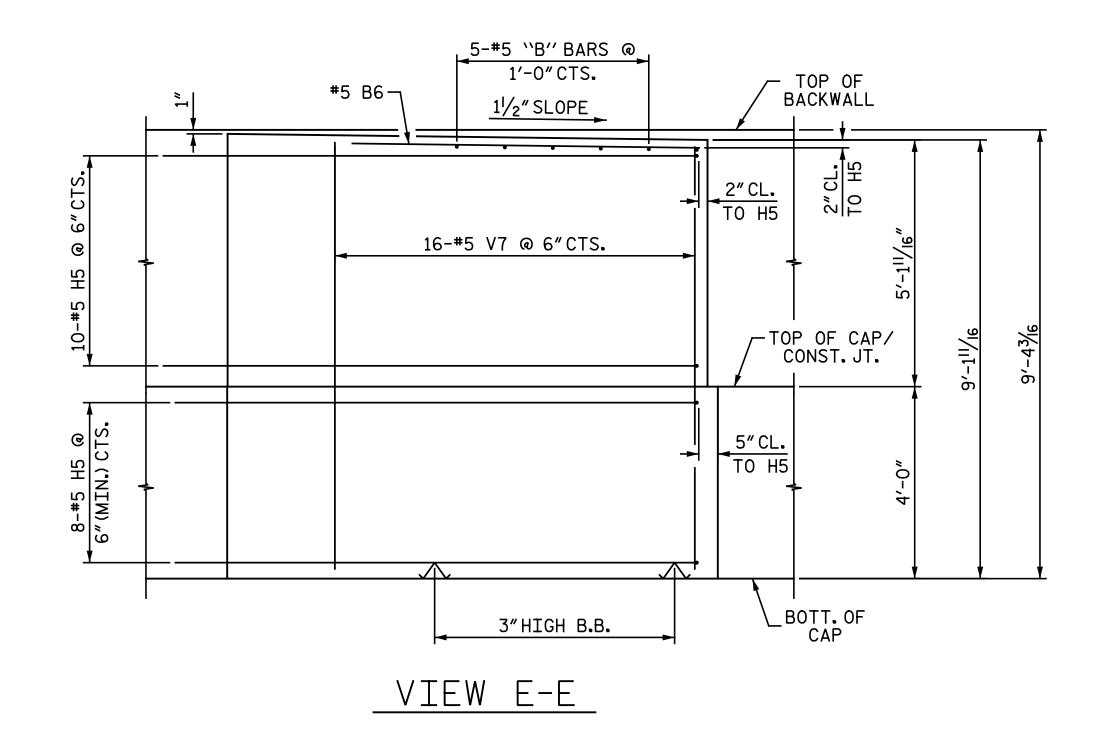
DRAWN BY: M.D. MAYHEW DATE: 3-4-14
CHECKED BY: S.A. DENNEY DATE: 3-17-14

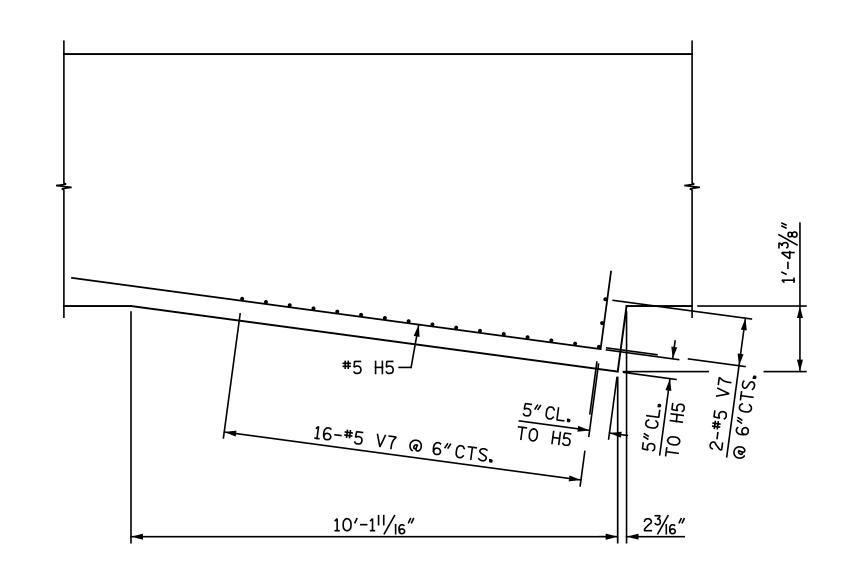






PLAN - BACKWALL EXTENSION DETAIL





PLAN - CAP EXTENSION DETAIL

NOTE: BACKWALL NOT SHOWN FOR CLARITY

- TOP OF BACKWALL

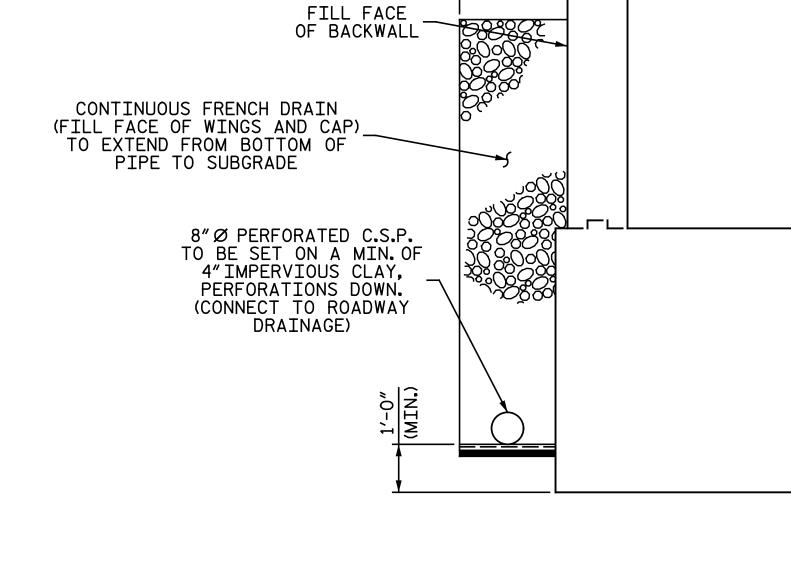
∟BOTT.OF CAP

VIEW F-F

 $1\frac{1}{2}$ " SLOPE \neg

2"CL. TO H5

TOP OF CAP/-CONST.JT.



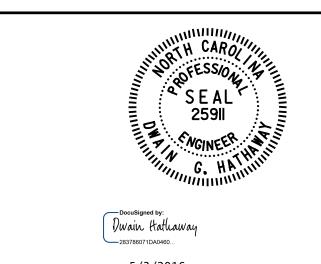
VARIES (2'-0" TO

2′-3″)

DRAINAGE AT ABUTMENT 1

SHEET 3 OF 4

PROJECT NO. U-3308 DURHAM _ COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE

ABUTMENT I

Michael Baker INTERNATIONAL

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

SHEET NO. S4-30 REVISIONS DATE: NO. BY: DATE: TOTAL SHEETS 68

DRAWN BY: M. D. MAYHEW DATE: 3-6-14
CHECKED BY: S. A. DENNEY DATE: 3-11-14

GRADE TO DRAIN GRADE TO DRAIN -TOE OF SLOPE TOE OF SLOPE -

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT ABUTMENT

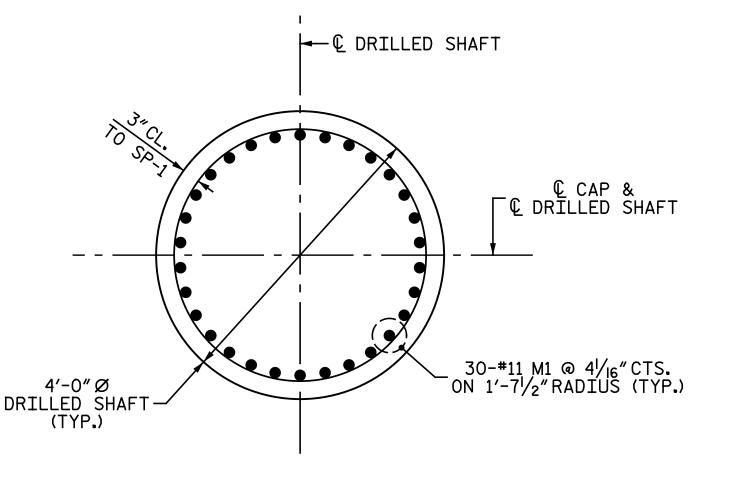
NOTES:

BACKWALL SHALL BE PLACED BEFORE APPLYING EPOXY PROTECTIVE COATING.

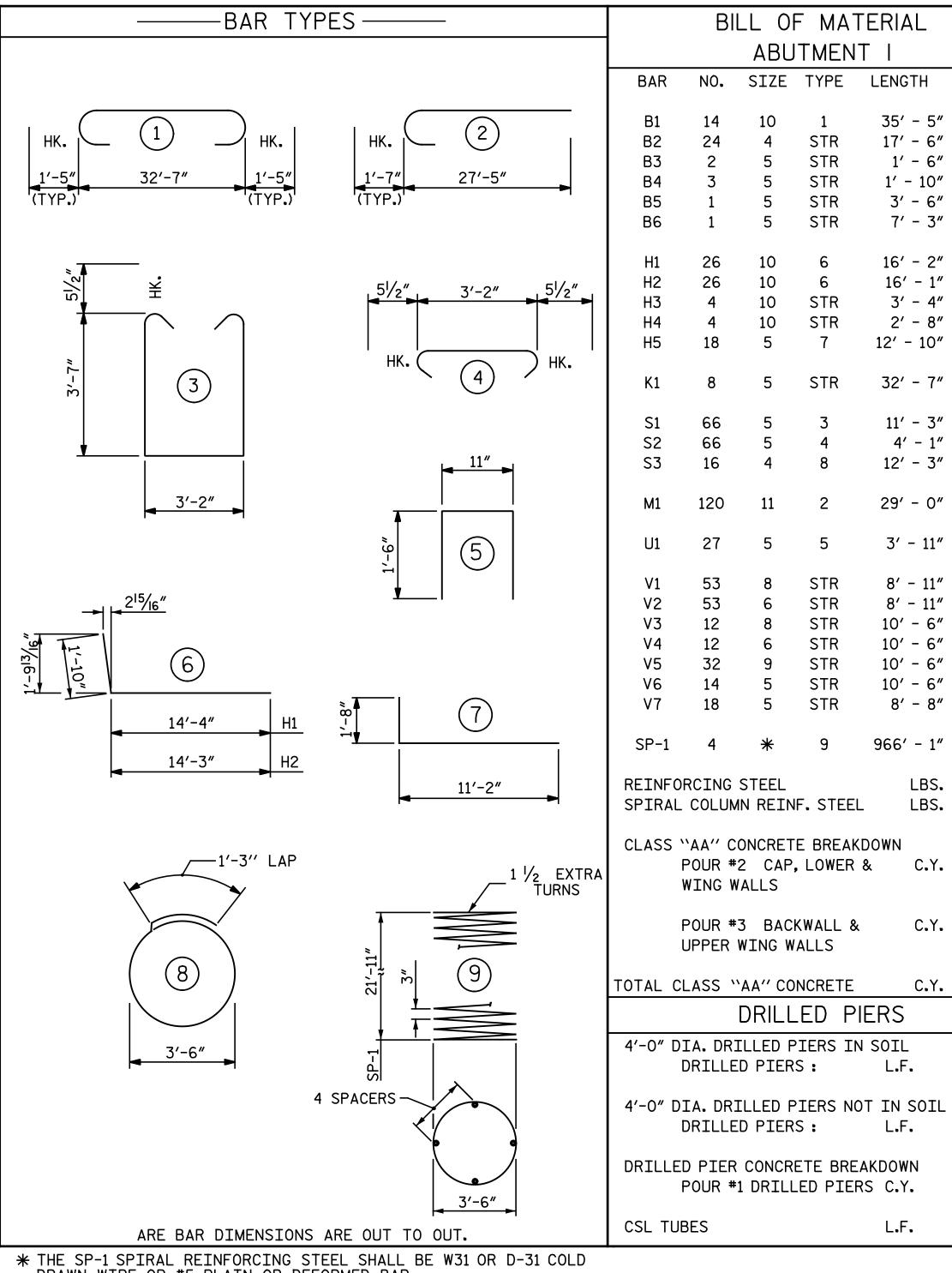
THE TOP SURFACE AREAS OF THE ABUTMENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".



SECTION B-B



DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

PROJECT NO. U-3308 DURHAM COUNTY 24+55.20 -LALT-STATION:_

DRILLED PIERS

BILL OF MATERIAL

ABUTMENT

WEIGHT

2,134

1,809 1,799

57

1,262

1,142

4,032

4,032

30.1

15**.**6

59.5

30.0

41.7

382.08

LBS. 30,400

1' - 6"

1' - 10"

3' - 6" 7' - 3"

16' - 2"

12' - 10"

32' - 7"

11' - 3"

12' - 3"

3' - 11"

10' - 6"

966' - 1"

LBS.

C.Y.

C.Y.

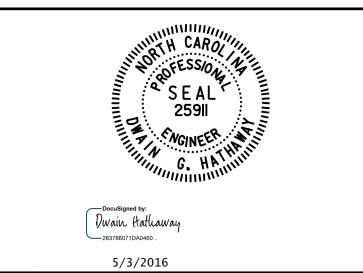
L.F.

L.F.

C.Y. 45.7

29' - 0" 18,489

SHEET 4 OF 4



DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

ABUTMENT

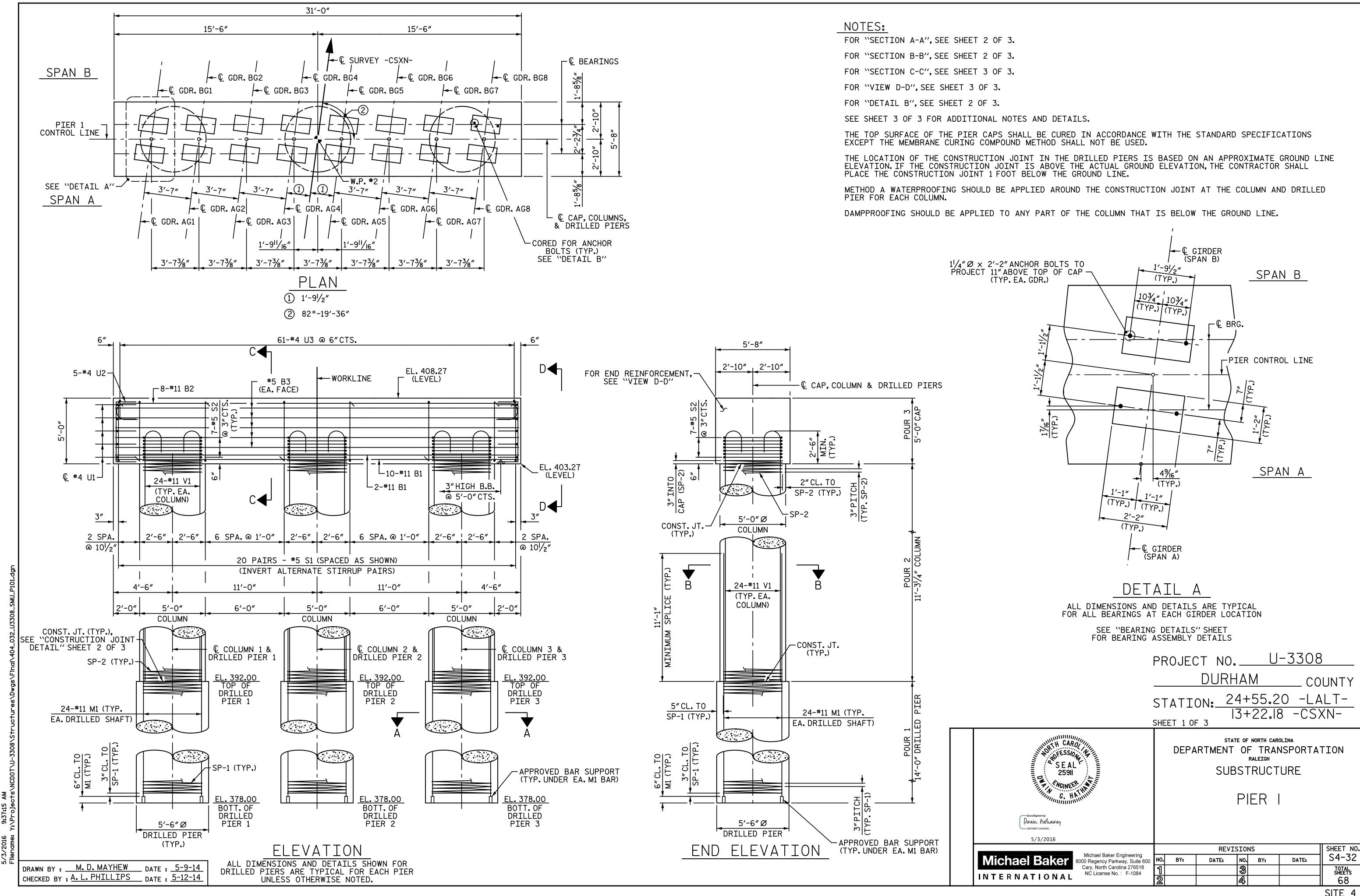
STATE OF NORTH CAROLINA

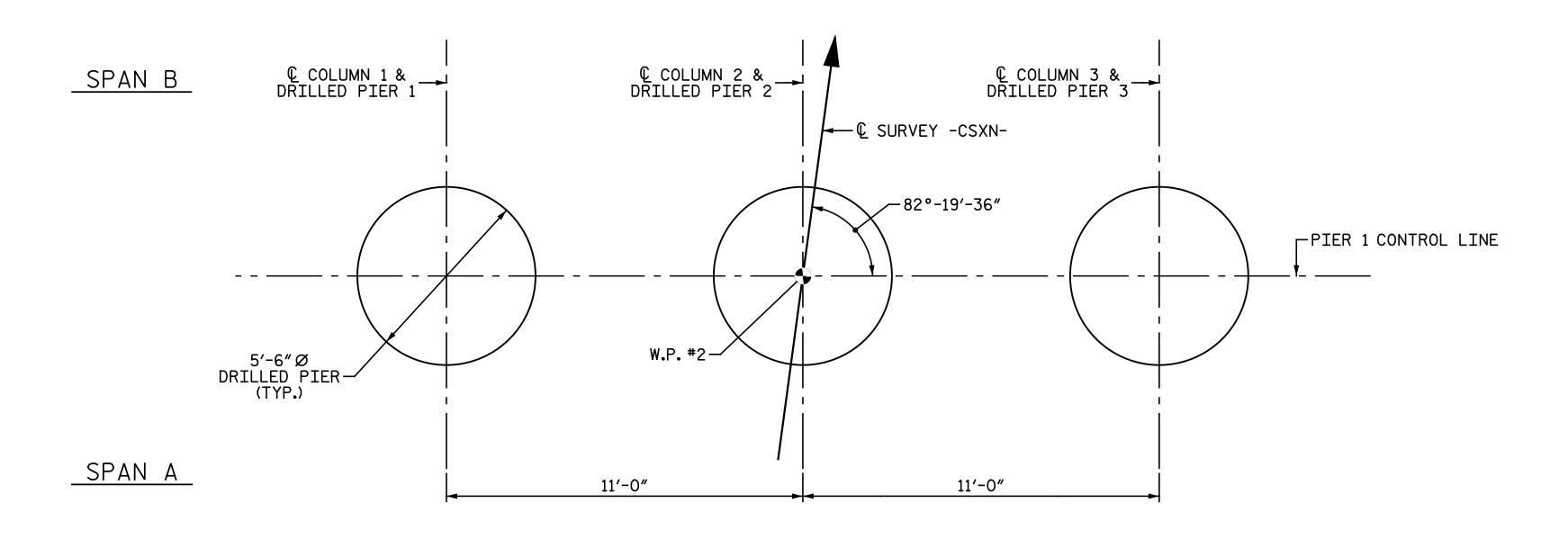
Michael Baker INTERNATIONAL

Michael Baker Engineering 8000 Regency Parkway, Suite 600 NO. Cary, North Carolina 276518 NC License No. F-1084

REVISIONS SHEET NO. S4-3I NO. BY: DATE: DATE: BY: TOTAL SHEETS 68

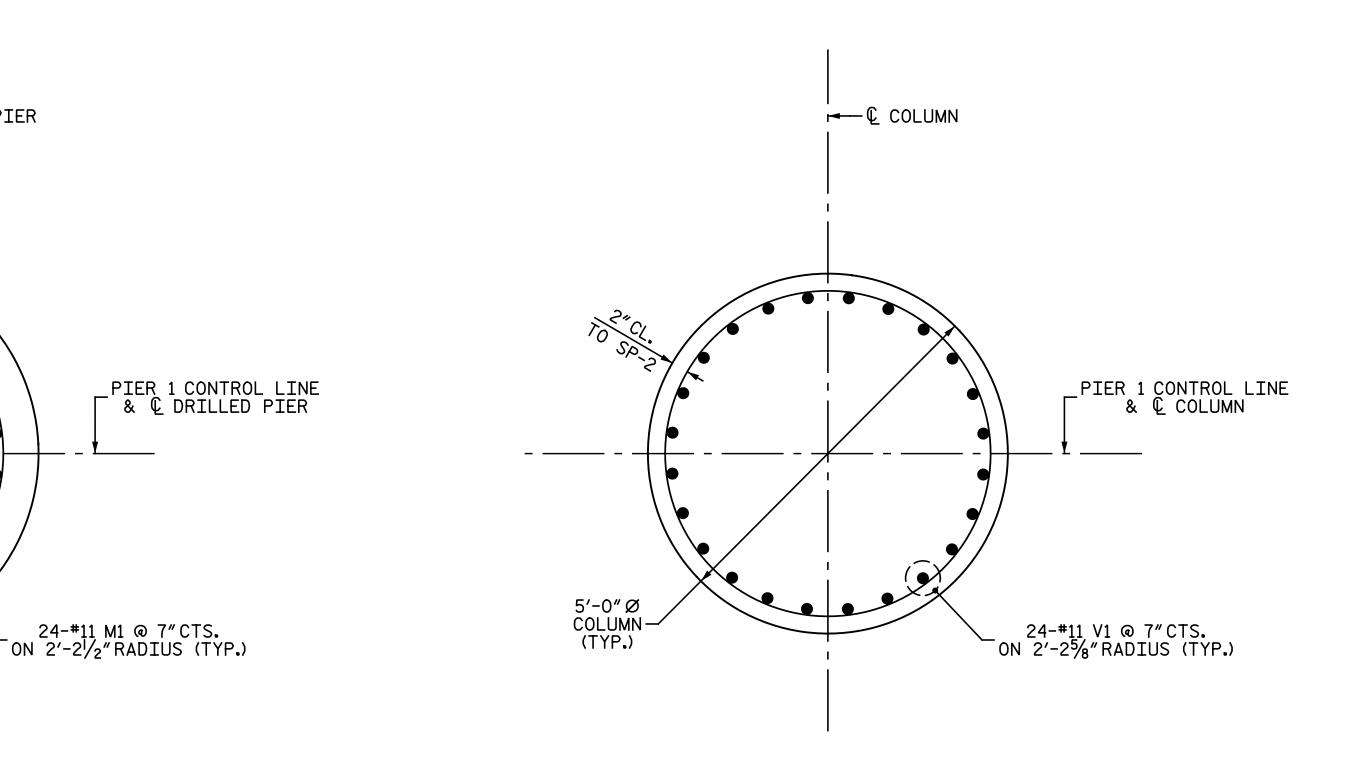
CHECKED BY: S. A. DENNEY DATE: 3-11-14





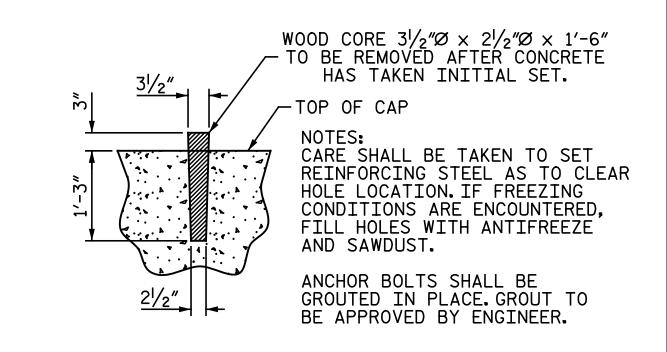
PLAN OF DRILLED PIERS ALL DIMENSIONS AND DETAILS SHOWN FOR DRILLED PIERS ARE TYPICAL FOR EACH PIER UNLESS OTHERWISE NOTED.

r ← C DRILLED PIER



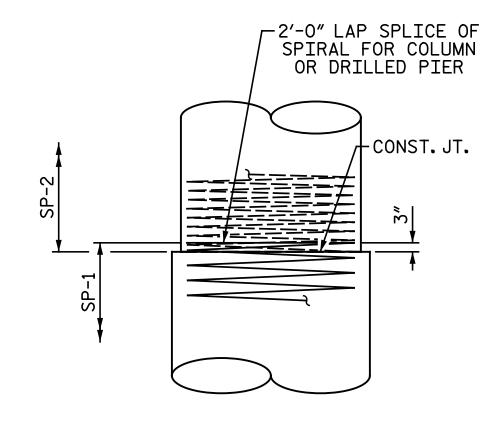
SECTION A-A

SECTION B-B



DETAIL B

FOR LOCATION OF "DETAIL B", SEE SHEET 1 OF 3.

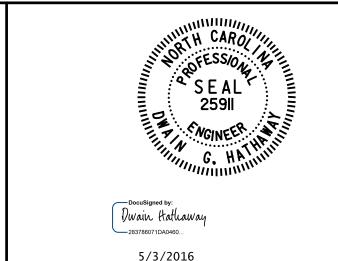


CONSTRUCTION JOINT DETAIL

PROJECT NO. <u>U-3308</u>

<u>DURHAM</u> COUNTY

STATION: 24+55.20 -LALT
I3+22.I8 -CSXN
SHEET 2 OF 3



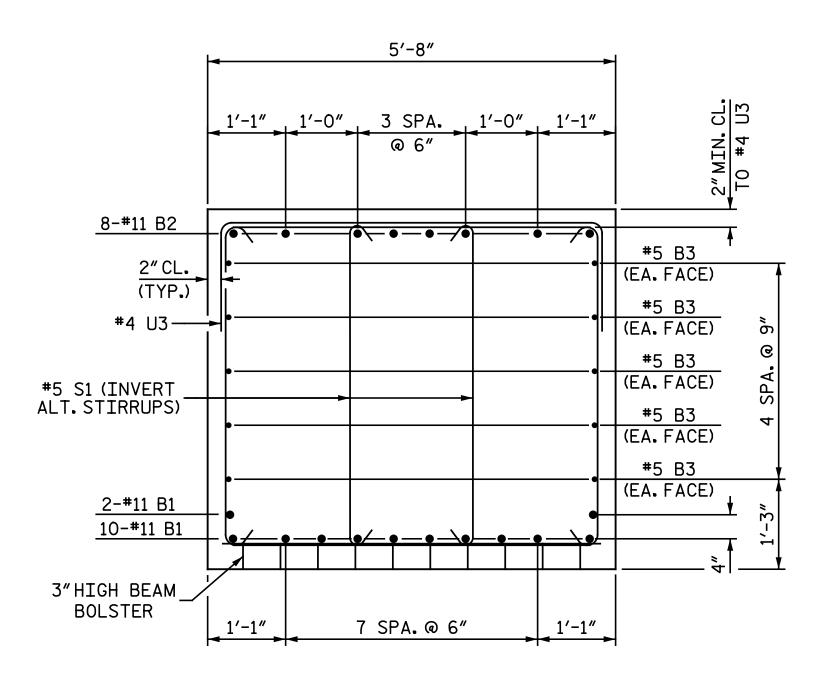
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

PIER I DETAILS

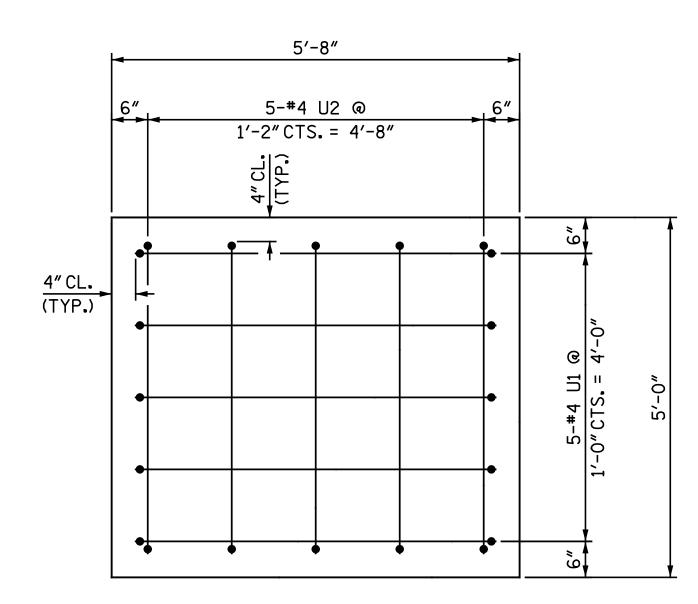
5/3/2016	
REVISIONS	SHEET NO.
Michael Baker Engineering 8000 Regency Parkway, Suite 600 NO. BY: DATE: NO. BY: DATE:	S4-33
Cary, North Carolina 276518	TOTAL SHEETS
ITERNATIONAL No license No. 1 F-1084 2 2	68

DRAWN BY: M.D. MAYHEW DATE: 5-9-14
CHECKED BY: A.L. PHILLIPS DATE: 5-12-14

5'-6"Ø DRILLED PIER-/ (TYP.)



SECTION C-C



VIEW D-D

CHECKED BY: A. L. PHILLIPS DATE: 5-12-14

NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

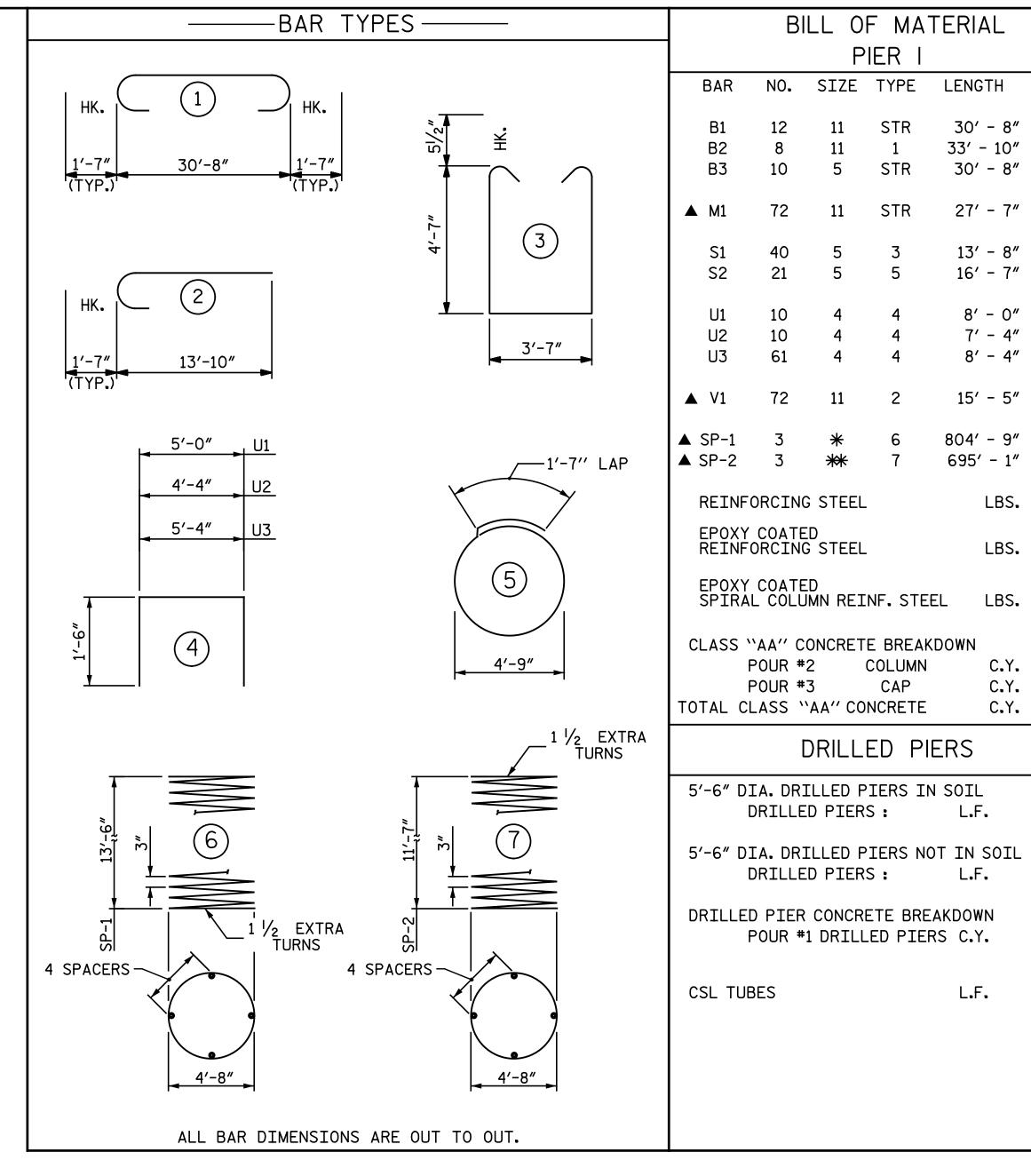
HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "EPOXY COATED REINFORCING STEEL" AND "EPOXY COATED SPIRAL COLUMN REINFORCING STEEL".

THE CONTRACTORS ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

FOR ADDITIONAL FOUNDATION NOTES, SEE "GENERAL DRAWING" SHEET 2 OF 5.



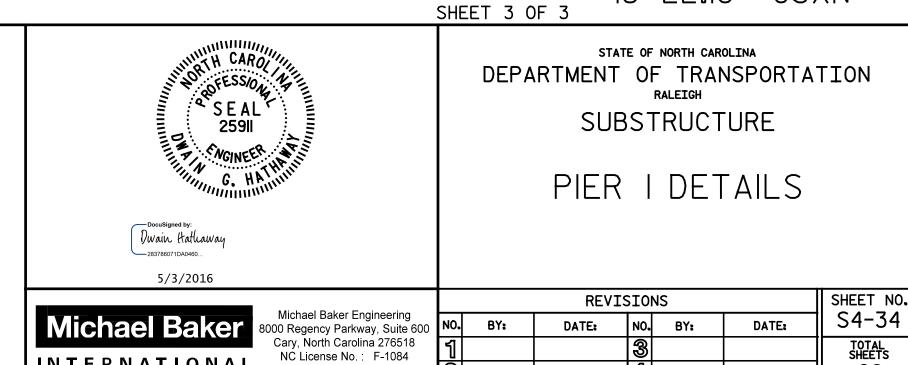
* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

INTERNATIONAL

▲ DENOTES EPOXY COATED REINFORCING STEEL

PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-



68 SITE 4

WEIGH7

1,438

320

10,552

570

363

53

49

340

5,897

2,517

1,392

LBS. 5,088

LBS. 16,449

LBS. 3,909

C.Y. 24.6

C.Y.

C.Y.

32.5

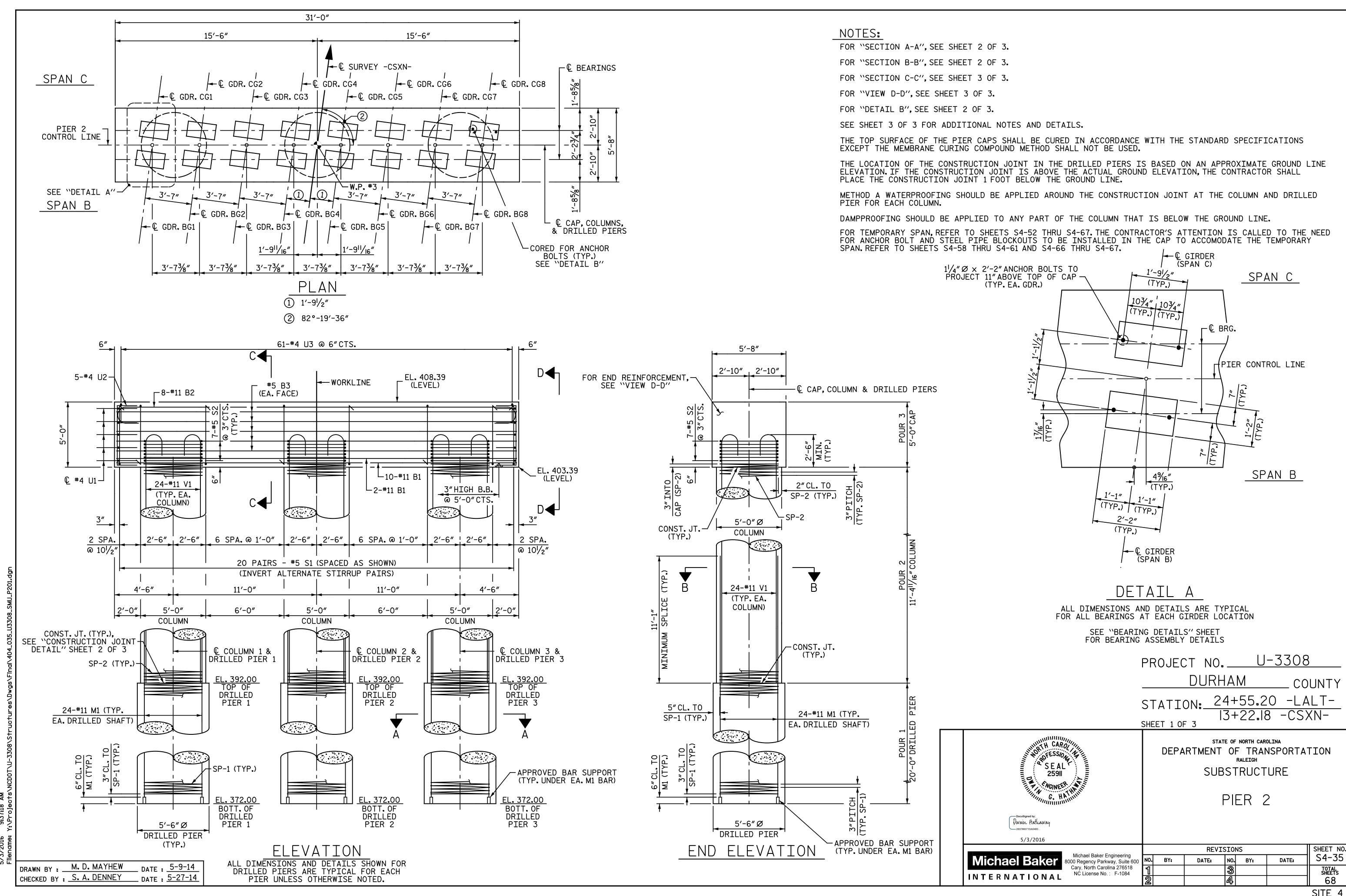
57**.**1

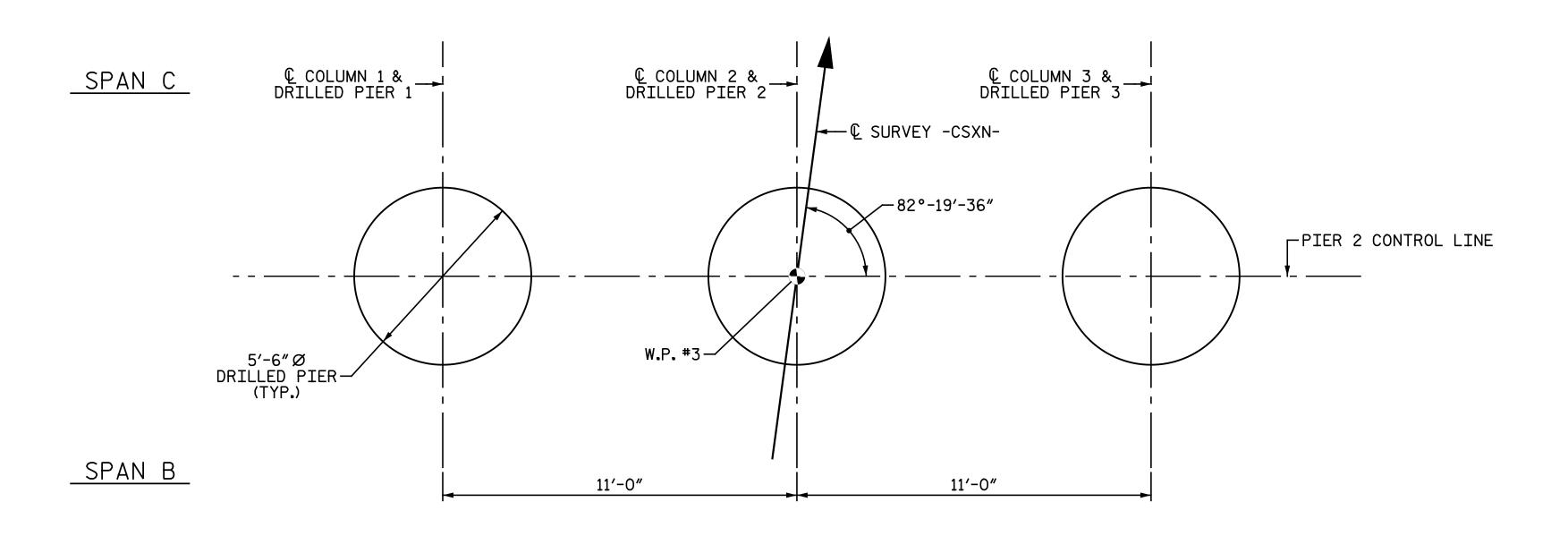
7.0

35.0

37.0

232.5





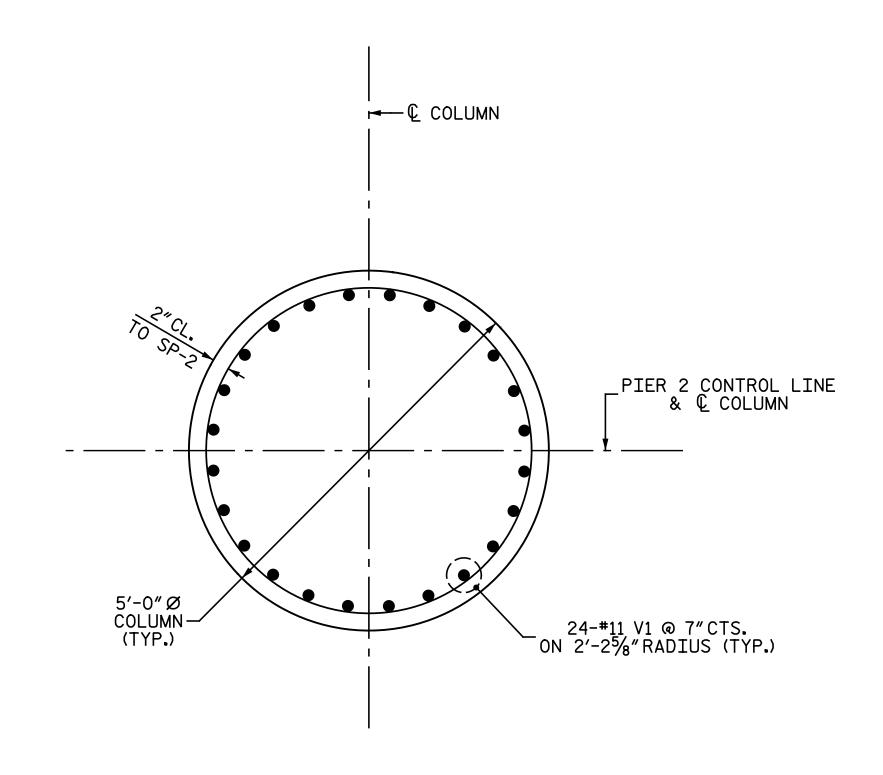
PLAN OF DRILLED PIERS ALL DIMENSIONS AND DETAILS SHOWN FOR DRILLED PIERS ARE TYPICAL FOR EACH PIER UNLESS OTHERWISE NOTED.

_PIER 2 CONTROL LINE | & @ DRILLED PIER

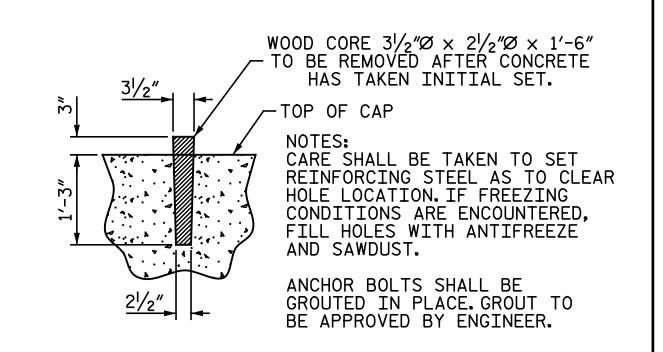
24-#11 M1 @ 7"CTS. ON 2'-2¹/₂"RADIUS (TYP.)

r ← C DRILLED PIER

SECTION A-A

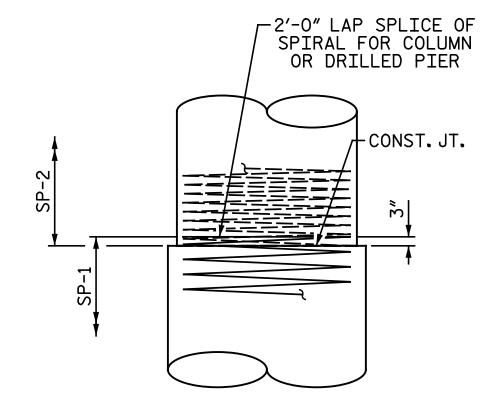


SECTION B-B



DETAIL B

FOR LOCATION OF "DETAIL B", SEE SHEET 1 OF 3.

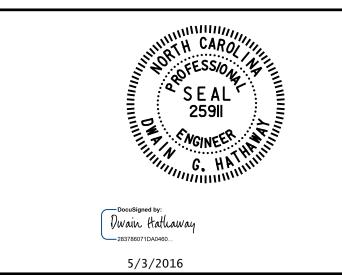


CONSTRUCTION JOINT DETAIL

PROJECT NO. <u>U-3308</u>

<u>DURHAM</u> COUNTY

STATION: 24+55.20 -LALT
I3+22.I8 -CSXN
SHEET 2 OF 3



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

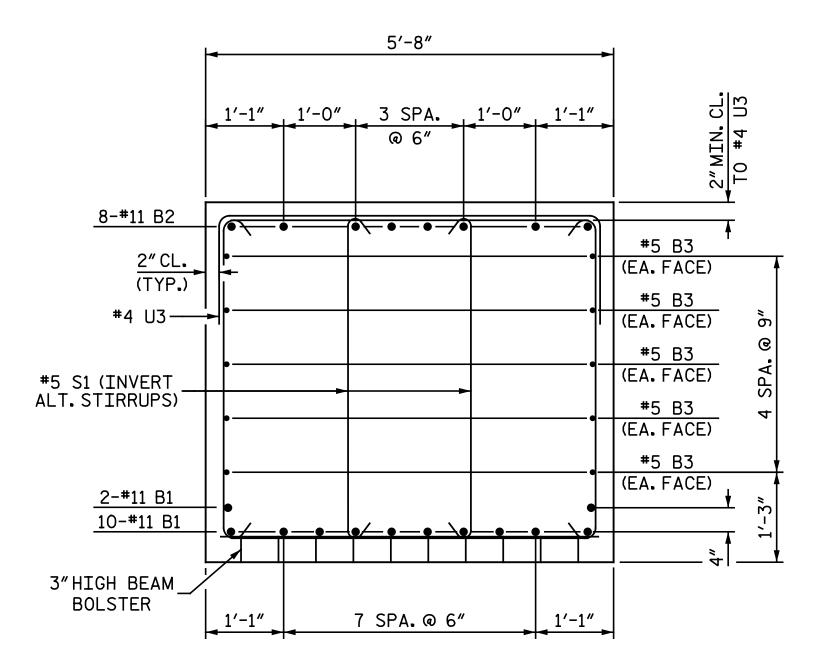
PIER 2 DETAILS

283786071DA0460							
5/3/2016							
		REVISIONS					SHEET NO
Michael Baker Engineering 8000 Regency Parkway, Suite 600	NO.	BY:	DATE	NO.	BY:	DATE:	S4-36
Cary, North Carolina 276518	1			3			TOTAL SHEETS
TERNATIONAL NO LICENSE NO. 1 P-1004	2			4			68

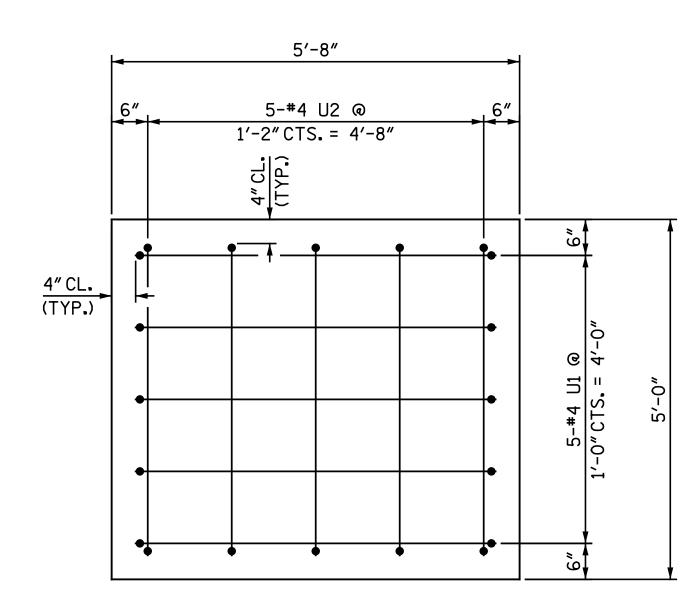
9:37:19 AM Y:\Projects\NCDOT\U-3308\Structures\D

DRAWN BY: M. D. MAYHEW DATE: 5-9-14
CHECKED BY: S. A. DENNEY DATE: 5-27-14

5'-6"Ø DRILLED PIER-/ (TYP.)



SECTION C-C



VIEW D-D

DRAWN BY: M. D. MAYHEW DATE: 5-9-14
CHECKED BY: S. A. DENNEY DATE: 5-27-14

NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

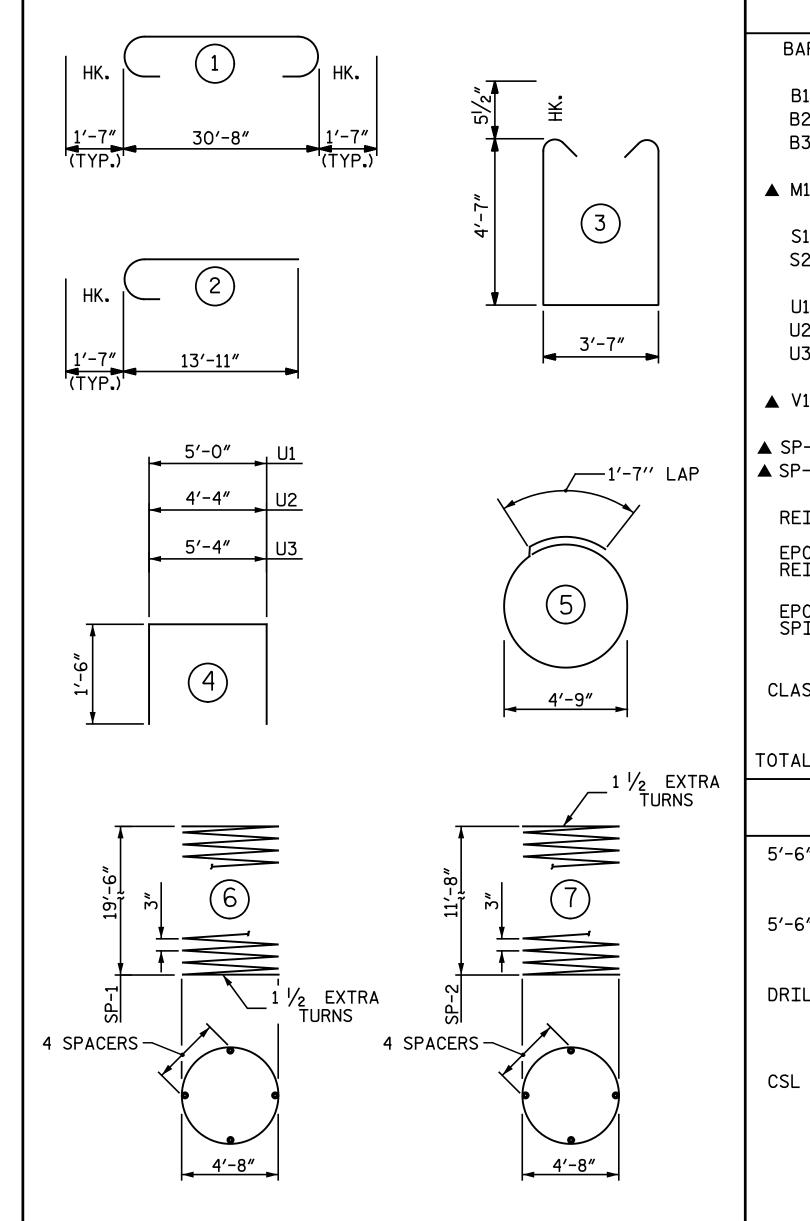
HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "EPOXY COATED REINFORCING STEEL" AND "EPOXY COATED SPIRAL COLUMN REINFORCING STEEL".

THE CONTRACTORS ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD

FOR ADDITIONAL FOUNDATION NOTES, SEE "GENERAL DRAWING" SHEET 2 OF 5.



-BAR TYPES ----

BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
B1 B2 B3	12 8 10	11 11 5	STR 1 STR	30′ - 8″ 33′ - 10″ 30′ - 8″	1,955 1,438 320
▲ M1	72	11	STR	33′ - 7″	12,847
S1 S2	40 21	5 5	3 5	13' - 8" 16' - 7"	570 363
U1 U2 U3	10 10 61	4 4 4	4 4 4	8' - 0" 7' - 4" 8' - 4"	53 49 340
▲ V1	72	11	2	15′ - 6″	5,929
▲ SP-1 ▲ SP-2	3 3	* **	6 7	1152' - 8" 700' - 0"	3,606 1,404
REINF	ORCIN	G STEEL		LBS.	5,088
	COATE ORCIN	ED G STEEL		LBS.	18,776
EPOXY SPIRA	COATE L COLL	D JMN REI	NF. STE	EL LBS.	5,010
f	POUR #	2 3	COLUMN CAP	C.Y.	32.5
	[DRILL	ED PI	ERS	
5′-6″ D:			PIERS I		24.0
				OT IN SOIL L.F.	36.0
				EAKDOWN RS C.Y.	52 . 8
CSL TUE	BES			L.F.	322.5

BILL OF MATERIAL

PIER 2

* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

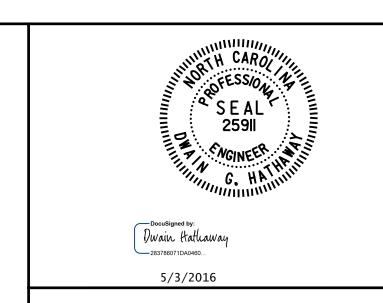
ALL BAR DIMENSIONS ARE OUT TO OUT.

- ** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.
- ▲ DENOTES EPOXY COATED REINFORCING STEEL

PROJECT NO. U-3308

DURHAM COUNTY

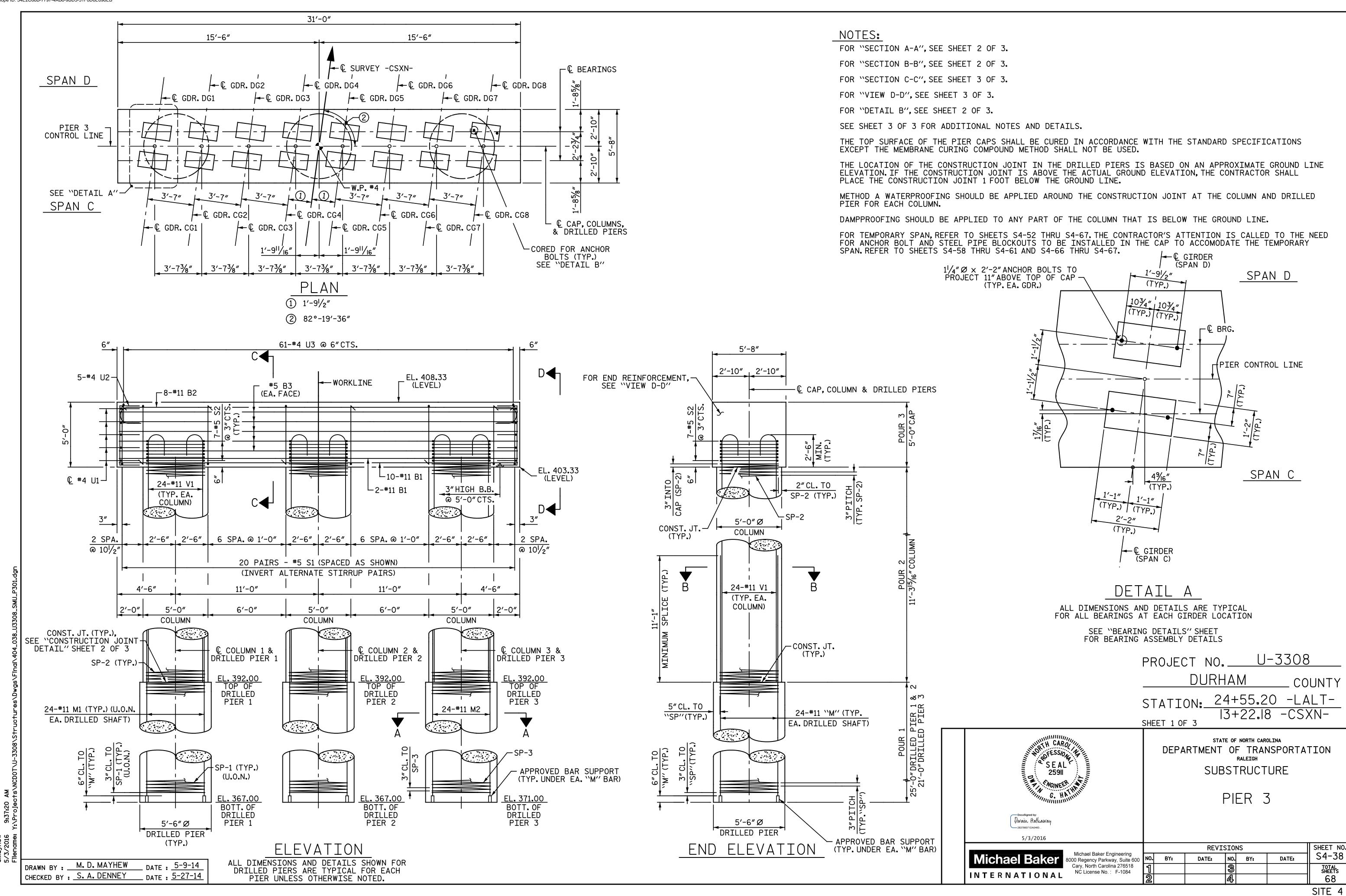
STATION: 24+55.20 -LALT
I3+22.18 -CSXN
SHEET 3 OF 3

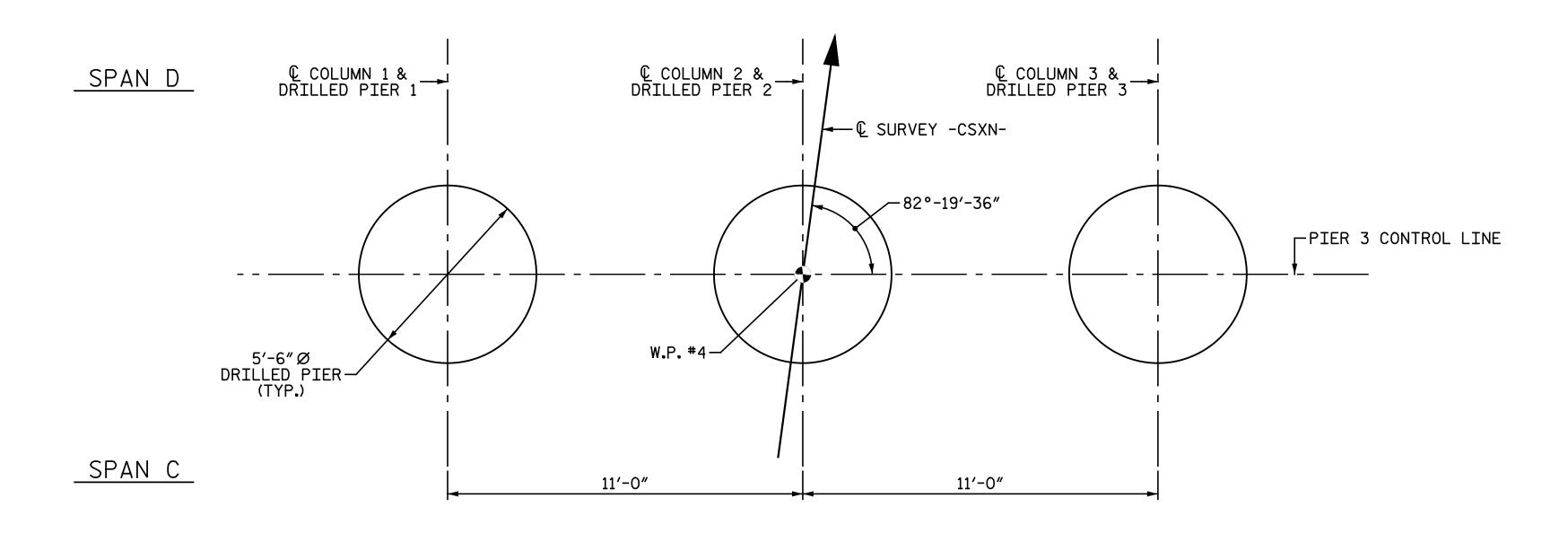


STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

PIER 2 DETAILS

SITE 4





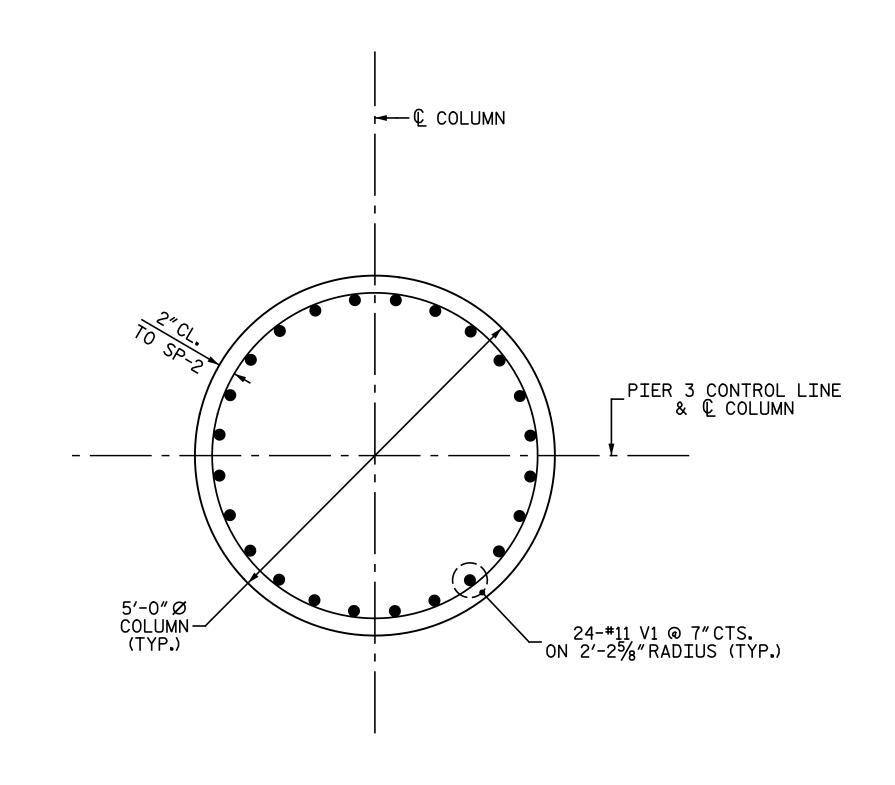
PLAN OF DRILLED PIERS ALL DIMENSIONS AND DETAILS SHOWN FOR DRILLED PIERS ARE TYPICAL FOR EACH PIER UNLESS OTHERWISE NOTED.

_PIER 3 CONTROL LINE | & @ DRILLED PIER

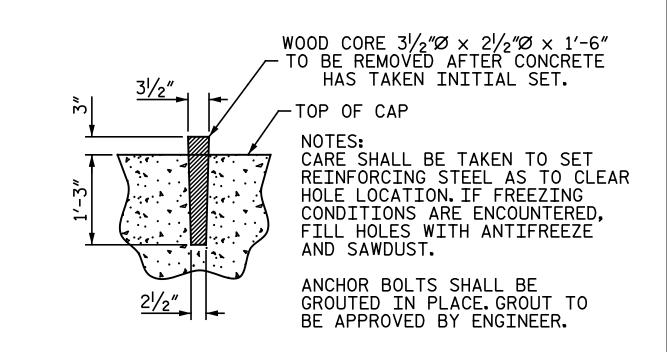
_ 24-#11 ``M'' @ 7" CTS. ON 2'-2¹/₂" RADIUS (TYP.)

→ C DRILLED PIER

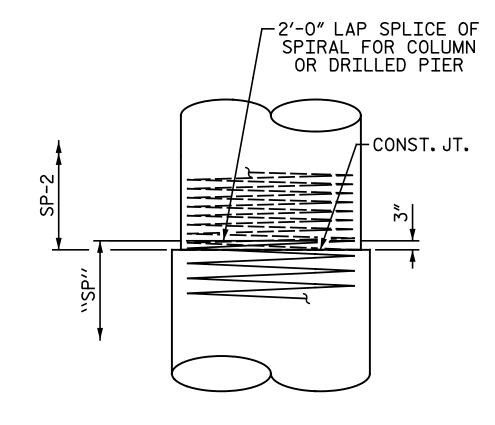
SECTION A-A



SECTION B-B

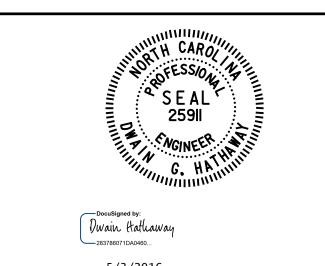


DETAIL B FOR LOCATION OF "DETAIL B", SEE SHEET 1 OF 3.



CONSTRUCTION JOINT DETAIL

PROJECT NO. U-3308 DURHAM _ COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-SHEET 2 OF 3



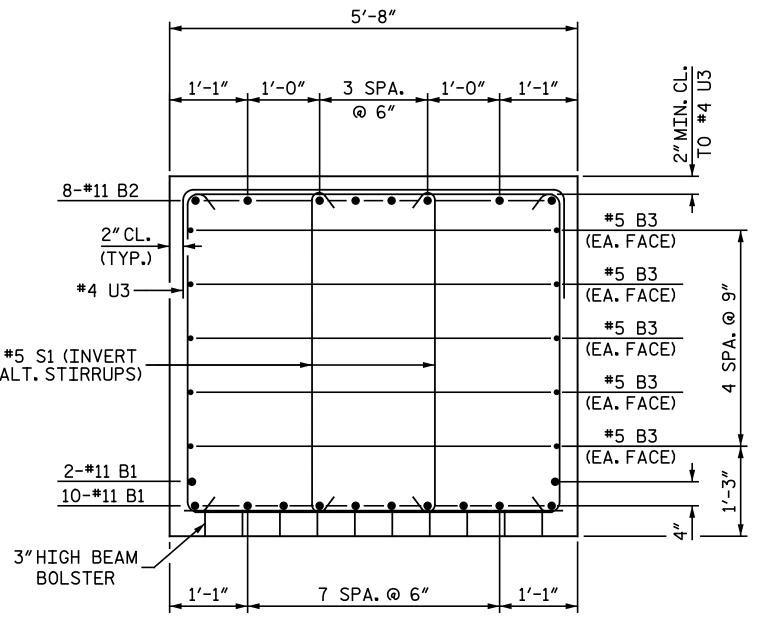
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE

PIER 3 DETAILS

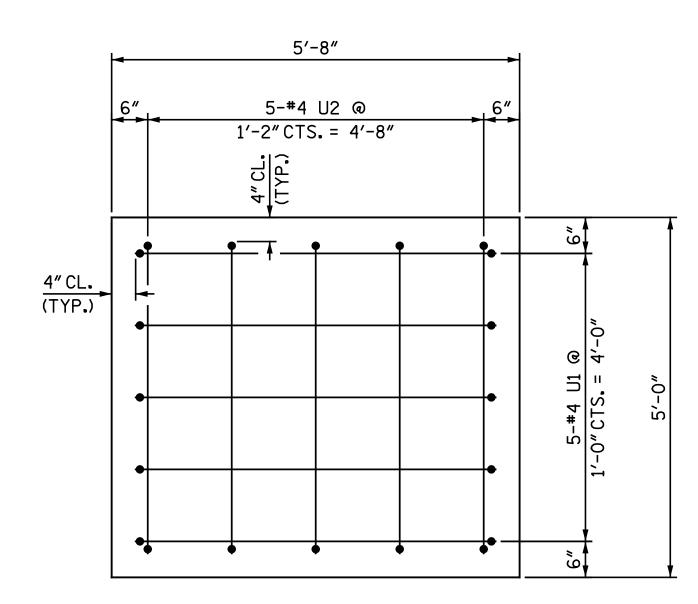
5/3/2016							
	10.1		REVI	SION	S		SHEET NO.
VICAACI BAKCI 8000 Reg	ael Baker Engineering gency Parkway, Suite 600	O. BY:	DATE:	NO.	BY:	DATE:	S4-39
Cary, N	North Carolina 276518 icense No.: F-1084			3			TOTAL SHEETS
ITERNATIONAL	2	2		4			68
	_			_			

DRAWN BY: M.D. MAYHEW DATE: 5-9-14
CHECKED BY: S.A. DENNEY DATE: 5-27-14

5'-6"Ø DRILLED PIER-/ (TYP.)



SECTION C-C



NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

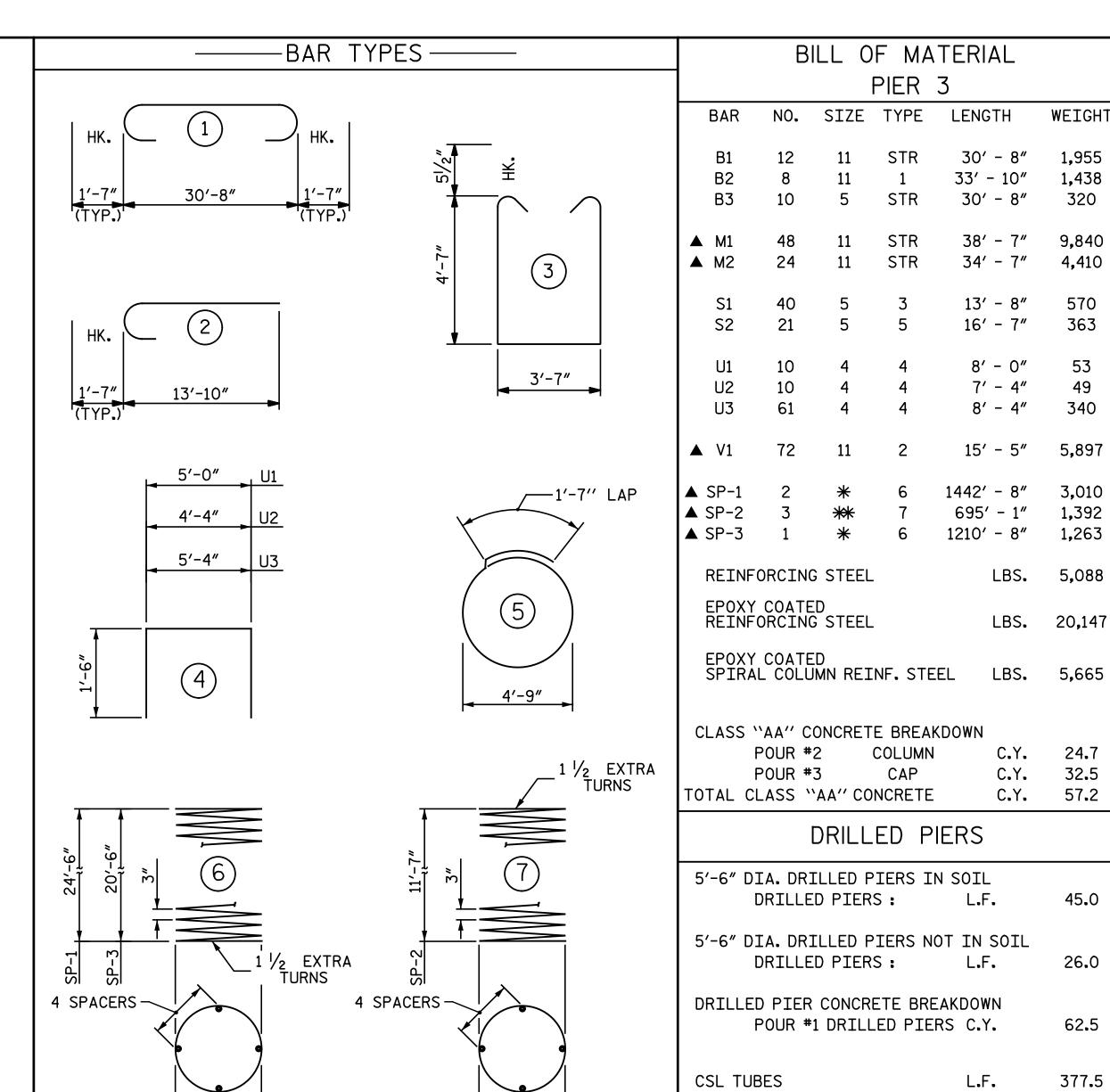
HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

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FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

FOR ADDITIONAL FOUNDATION NOTES, SEE "GENERAL DRAWING" SHEET 2 OF 5.

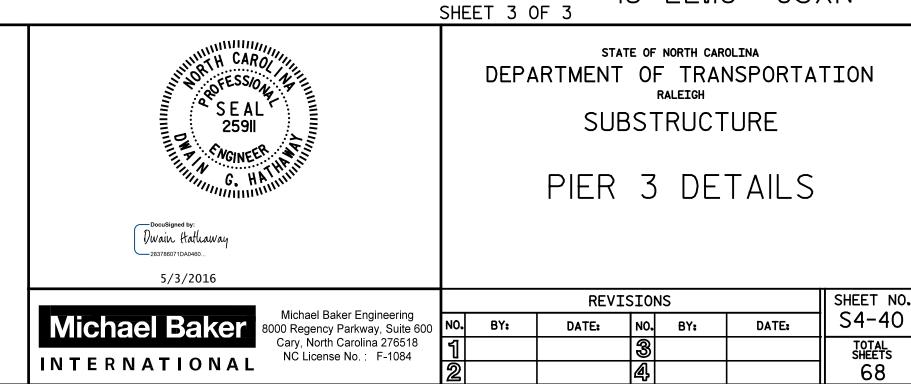


* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR.

ALL BAR DIMENSIONS ARE OUT TO OUT.

- ** THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.
- ▲ DENOTES EPOXY COATED REINFORCING STEEL

PROJECT NO. U-3308 DURHAM COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-





CHECKED BY: S. A. DENNEY DATE: 5-27-14 1,438

320

9,840

4,410

570

363

53

49

340

5,897

3,010 1,392

1,263

32.5

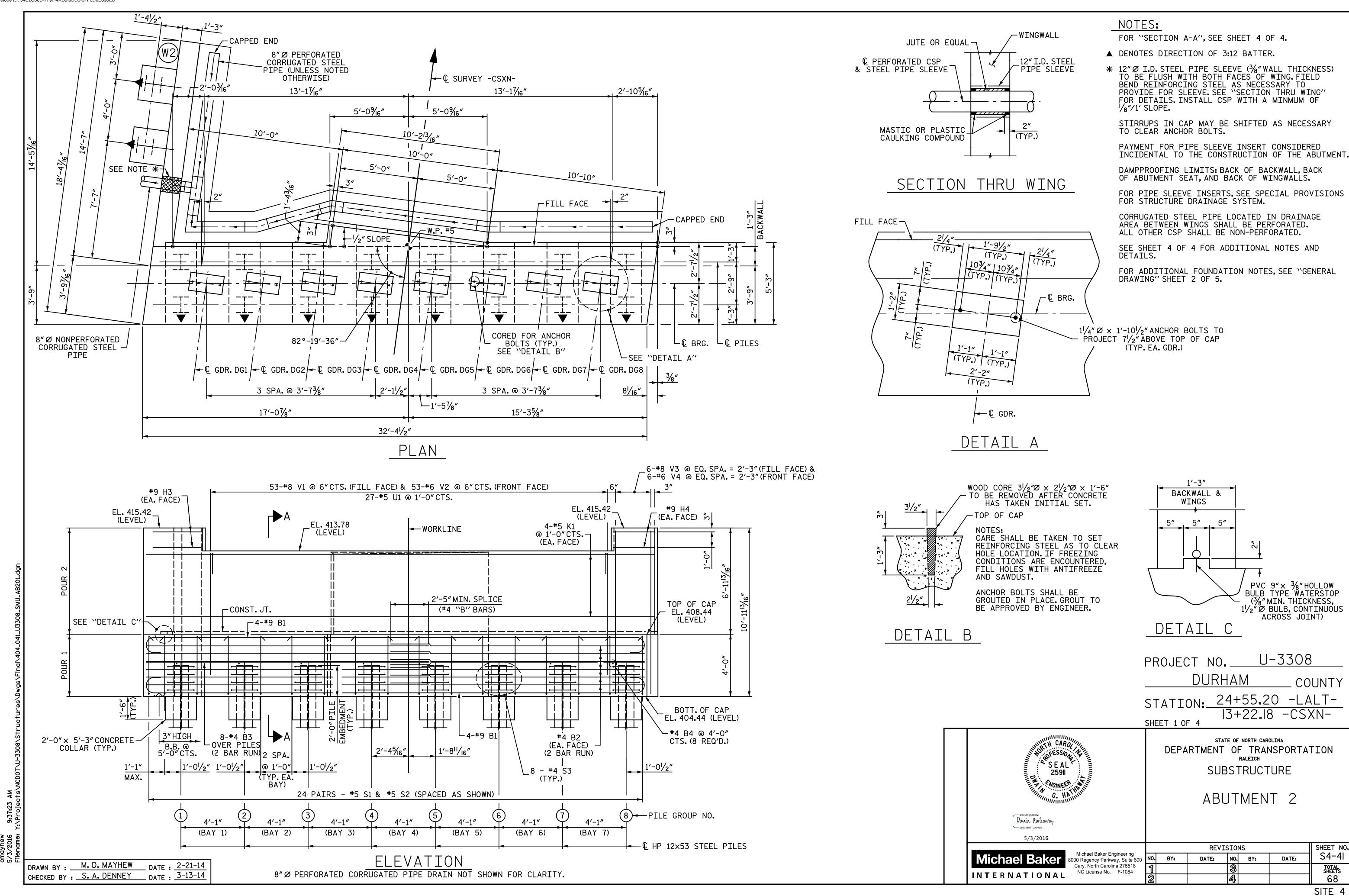
57.2

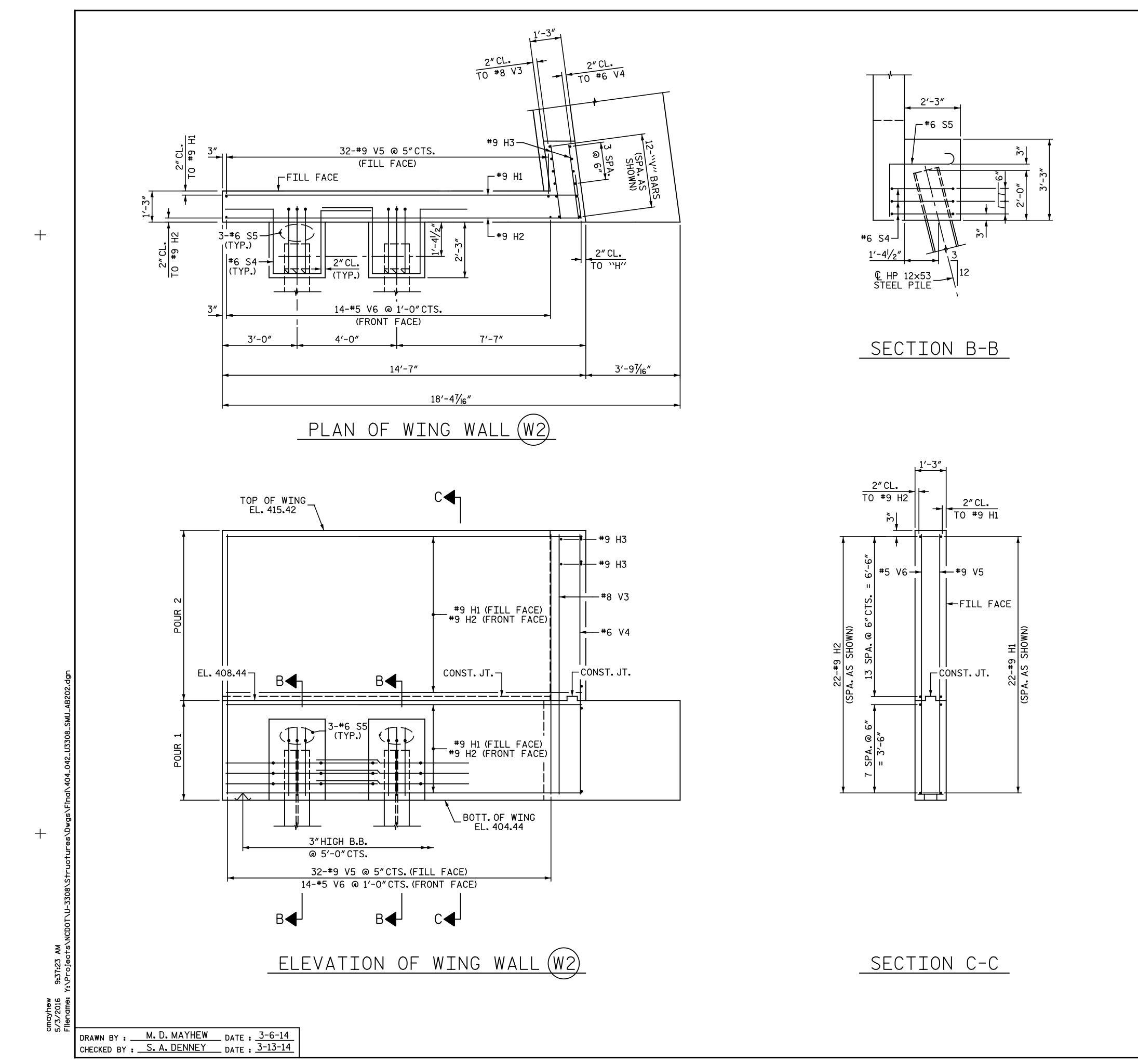
45.0

26.0

62.5

377.5





PROJECT NO. <u>U-3308</u>

<u>DURHAM</u> COUNTY

STATION: 24+55.20 -LALT-

Docusigned by:

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE

13+22.18 -CSXN-

ABUTMENT 2

283786071DA0460... 5/3/2016

Michael Baker Engineering
Michael Baker Engineering
8000 Regency Parkway, Suit
Cary, North Carolina 2765
NC License No.: F-108

Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

REVISIONS

NO. BY: DATE:

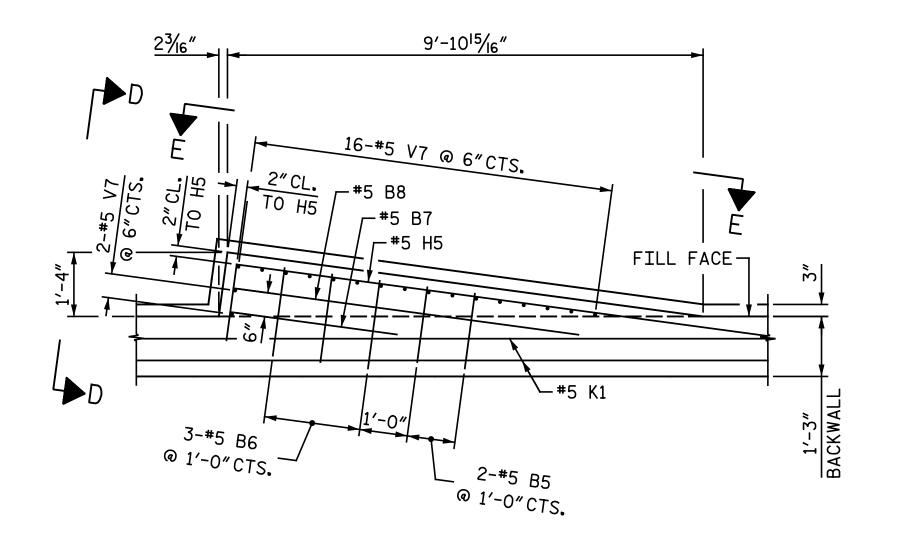
3
DATE:

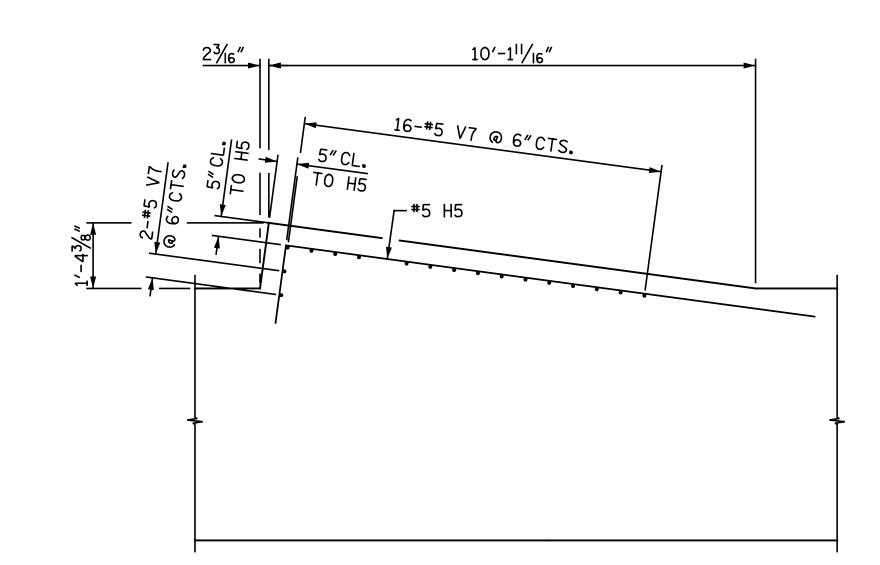
SHEET 2 OF 4

TOTAL SHEETS 68 SITE 4

SHEET NO.

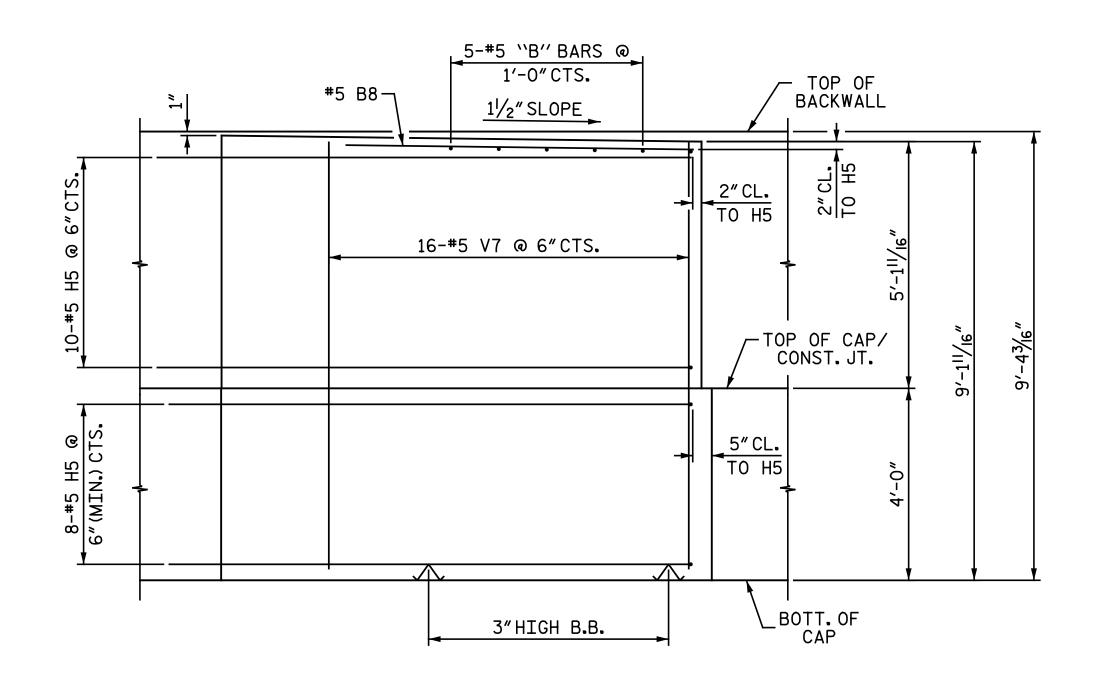
S4-42



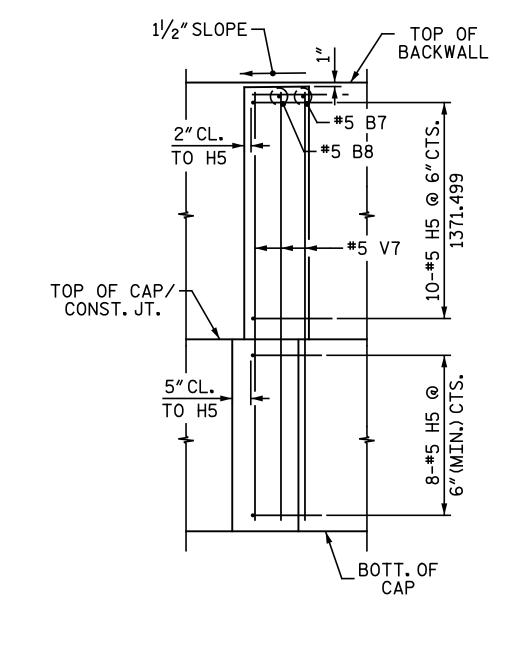


PLAN - BACKWALL EXTENSION DETAIL

PLAN - CAP EXTENSION DETAIL NOTE: BACKWALL NOT SHOWN FOR CLARITY



VIEW E-E

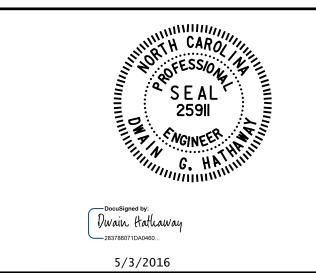


VIEW D-D

VARIES (2'-0" TO 2′-3″) FILL FACE OF BACKWALL CONTINUOUS FRENCH DRAIN
(FILL FACE OF WINGS AND CAP)_
TO EXTEND FROM BOTTOM OF
PIPE TO SUBGRADE 8"Ø PERFORATED C.S.P.
TO BE SET ON A MIN. OF
4"IMPERVIOUS CLAY,
PERFORATIONS DOWN.
(CONNECT TO ROADWAY
DRAINAGE) 1'-0" (MIN.)

DRAINAGE AT ABUTMENT 2

PROJECT NO. U-3308 DURHAM _ COUNTY STATION: 24+55.20 -LALT-13+22.18 -CSXN-SHEET 3 OF 4

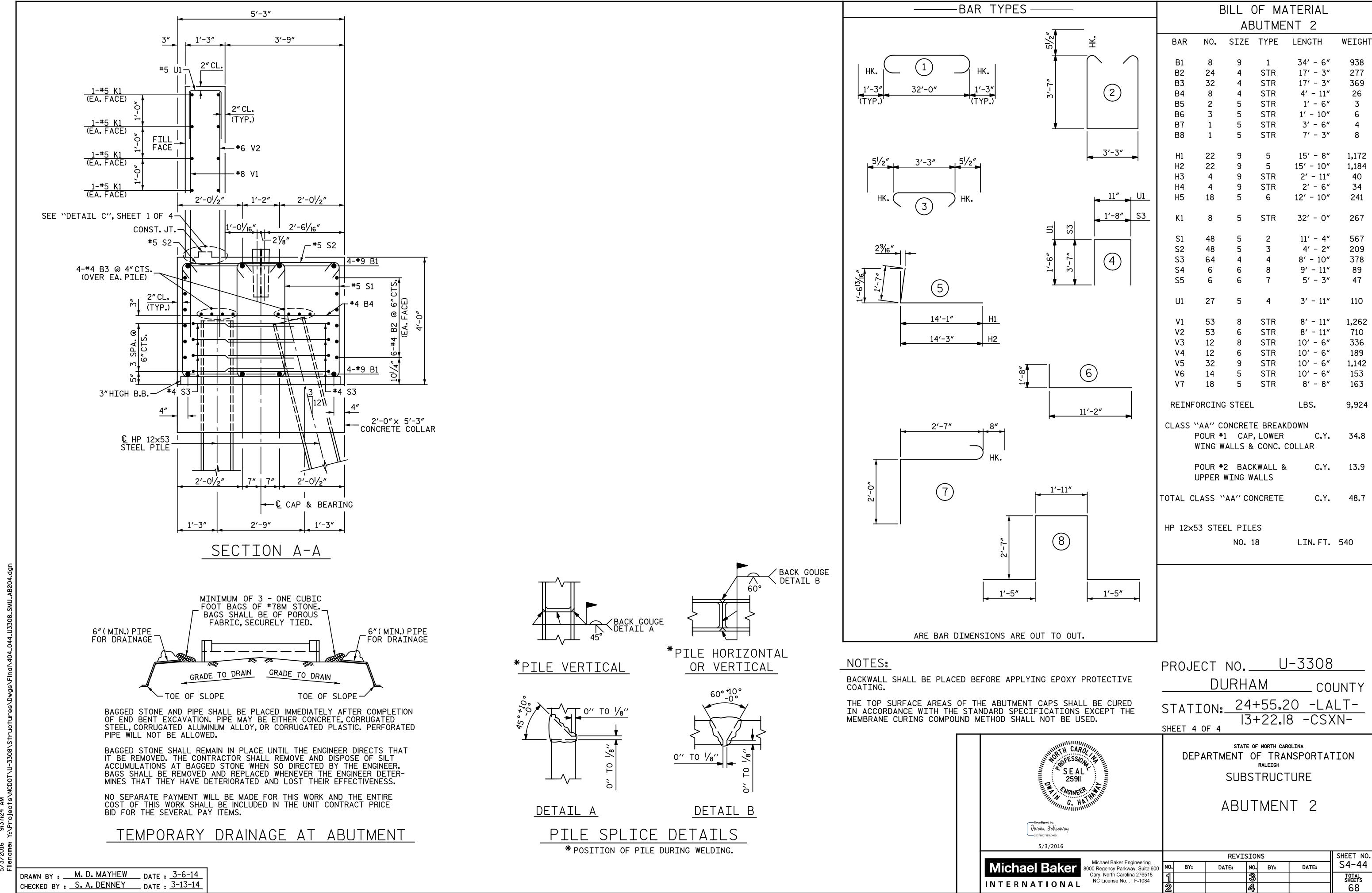


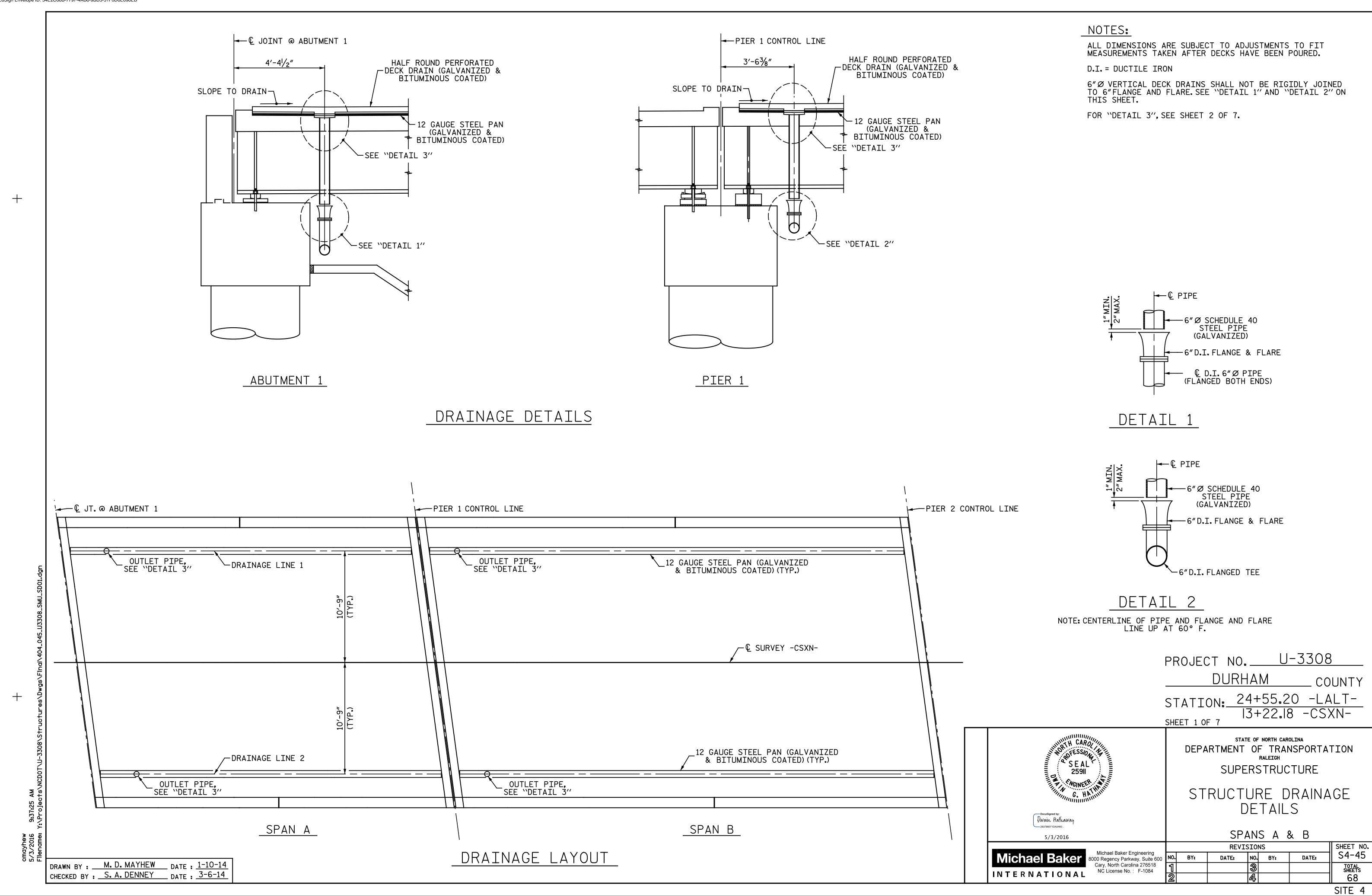
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE

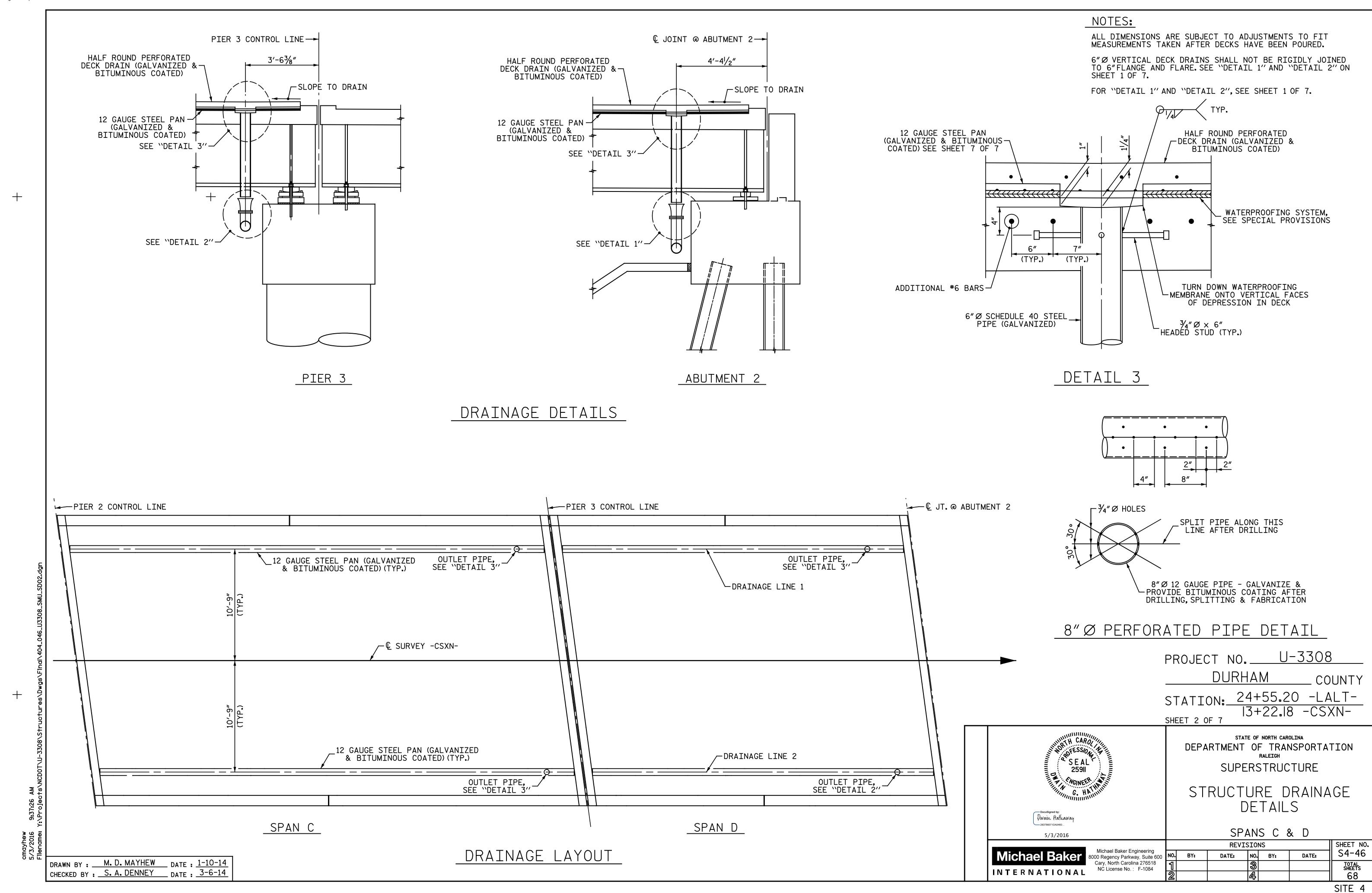
ABUTMENT 2

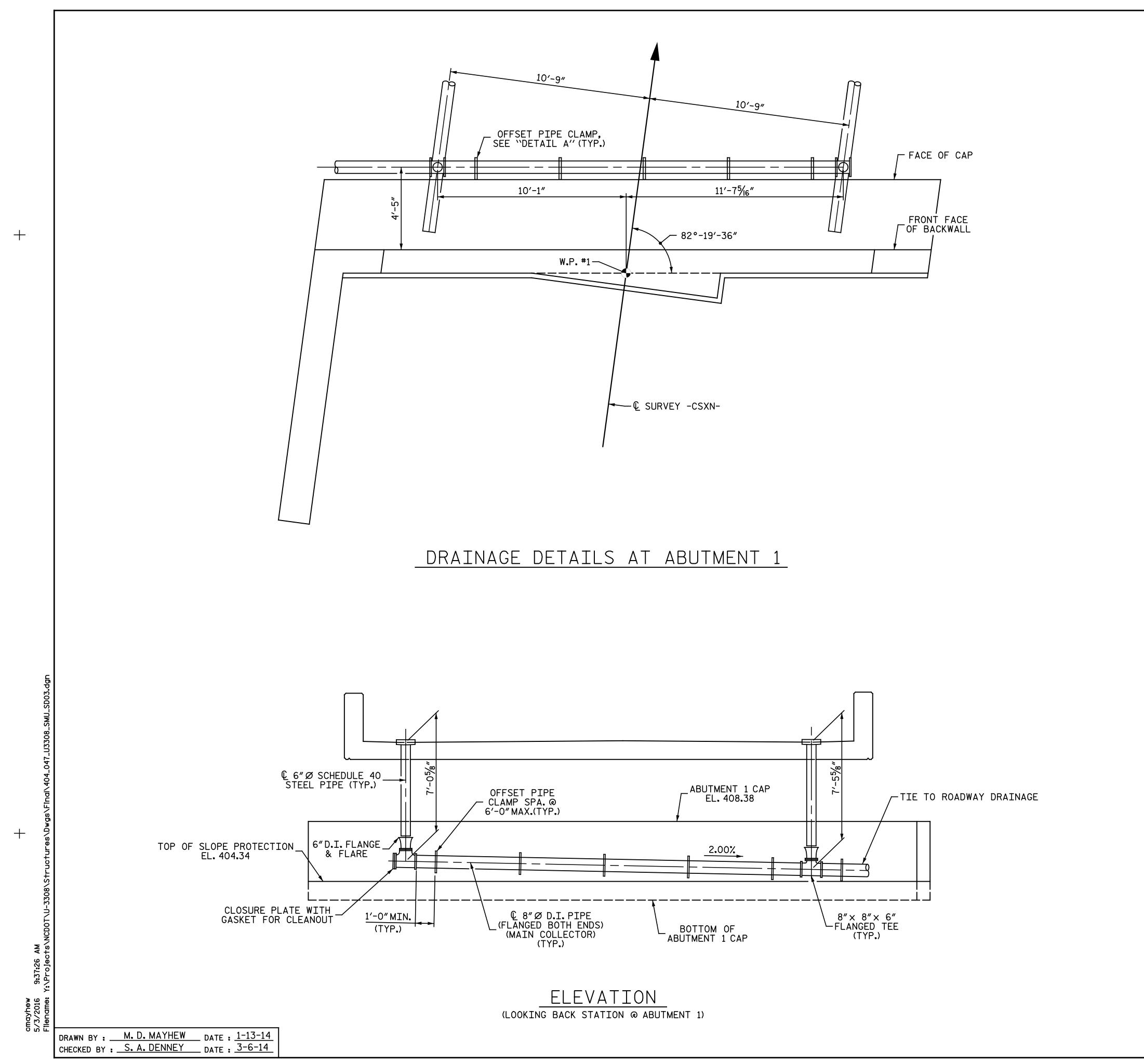
5/3/2016							
		REVISIONS					SHEET NO.
oooo regency rankway, outle ooo	NO.	BY:	DATE	NO.	BY:	DATE:	S4-43
Cary, North Carolina 276518 INTERNATIONAL Cary, North Carolina 276518 NC License No.: F-1084	1			3			TOTAL SHEETS
	2			4			68
							SITE 4

DRAWN BY: M. D. MAYHEW DATE: 3-6-14
CHECKED BY: S. A. DENNEY DATE: 3-13-14





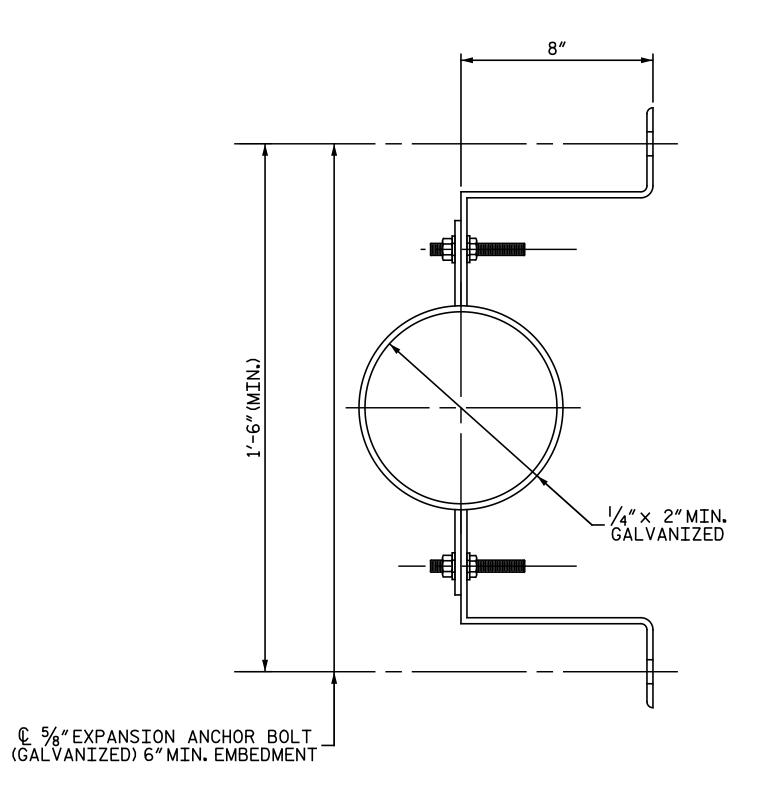




NOTES:

ALL DIMENSIONS ARE SUBJECT TO ADJUSTMENTS TO FIT MEASUREMENTS TAKEN AFTER DECKS HAVE BEEN POURED.

D.I. = DUCTILE IRON

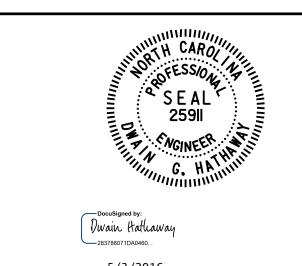


DETAIL A

PROJECT NO. U-3308

DURHAM COUNTY

STATION: 24+55.20 -LALT
I3+22.18 -CSXN
SHEET 3 OF 7



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

STRUCTURE DRAINAGE DETAILS

5/3/2016 ABUTMENT I REVISIONS

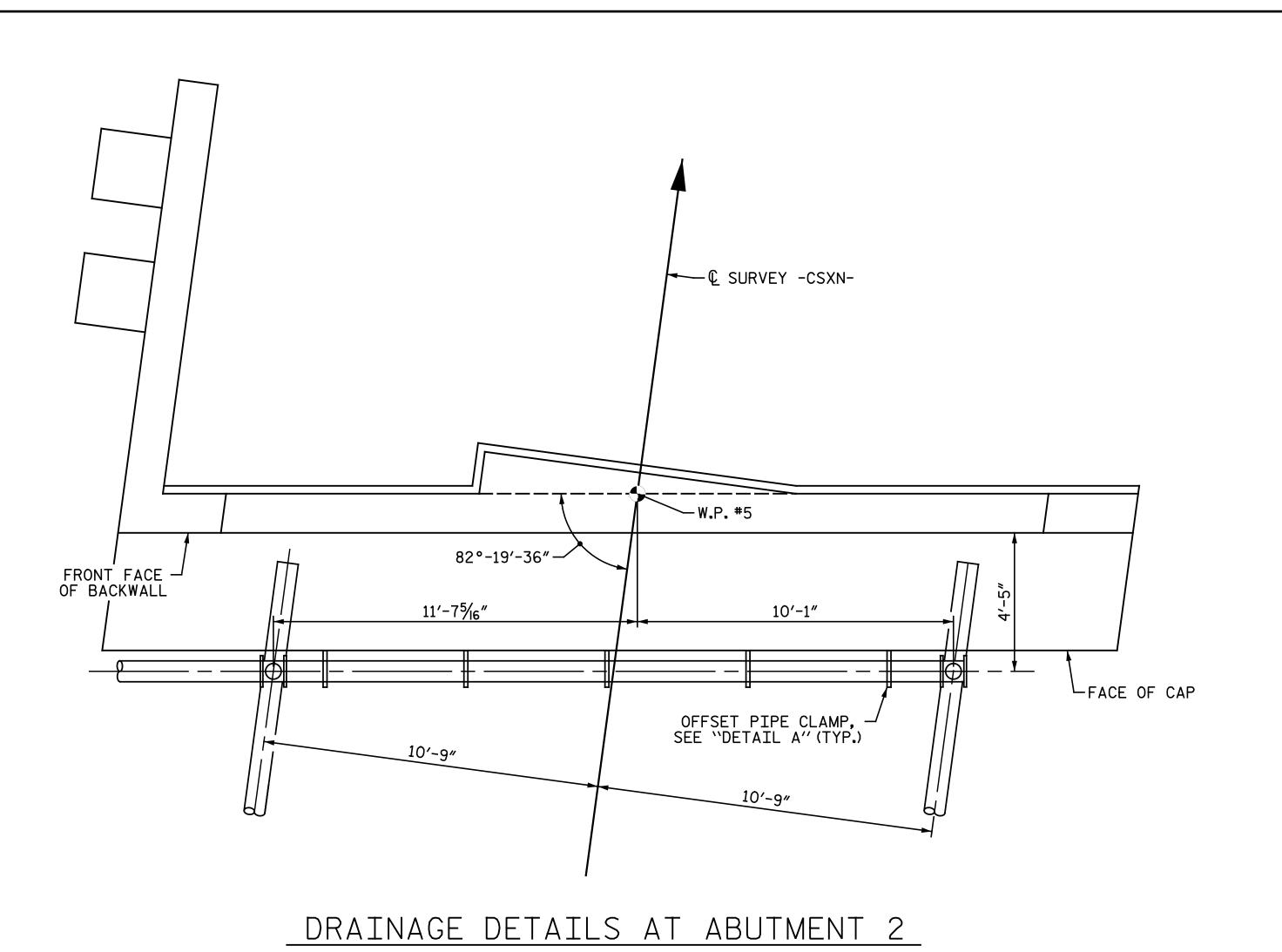
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

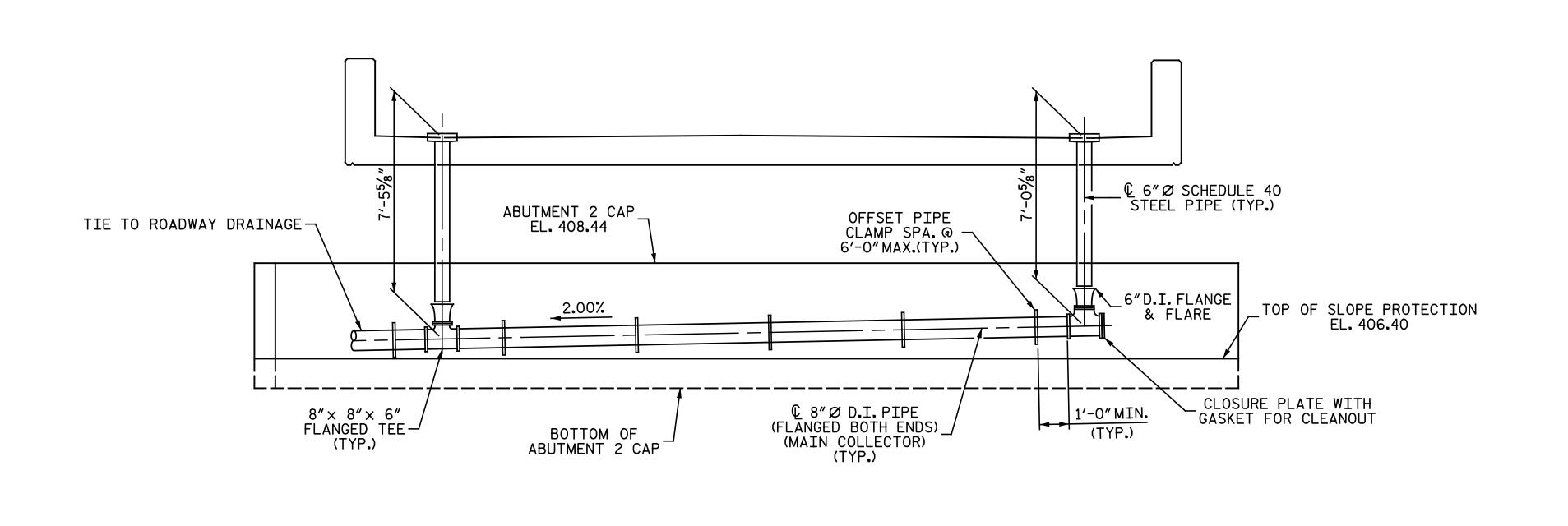
REVISIONS

BY: DATE: NO. BY: DATE: SHEET NO. S4-47

3 TOTAL SHEETS
68

DRAWN BY: M. D. MAYHEW DATE: 1-13-14
CHECKED BY: S. A. DENNEY DATE: 3-6-14





ELEVATION

(LOOKING UP STATION @ ABUTMENT 2)

NOTES:

ALL DIMENSIONS ARE SUBJECT TO ADJUSTMENTS TO FIT MEASUREMENTS TAKEN AFTER DECKS HAVE BEEN POURED.

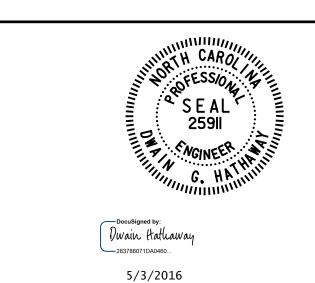
D.I. = DUCTILE IRON

FOR "DETAIL A", SEE SHEET 3 OF 7.

PROJECT NO. <u>U-3308</u>

<u>DURHAM</u> COUNTY

STATION: 24+55.20 -LALT
I3+22.I8 -CSXN
SHEET 4 OF 7



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

STRUCTURE DRAINAGE DETAILS

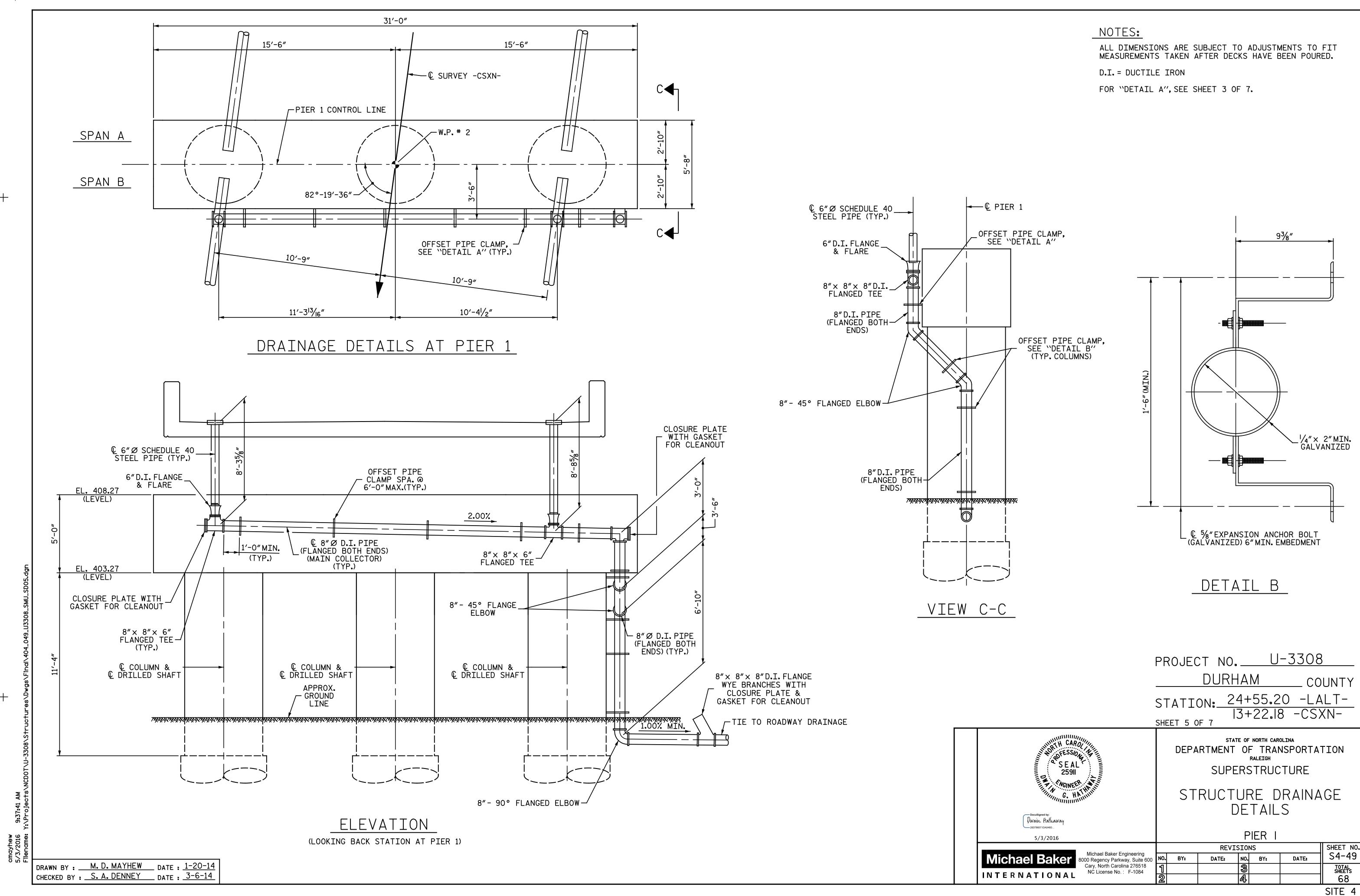
ABUTMENT 2

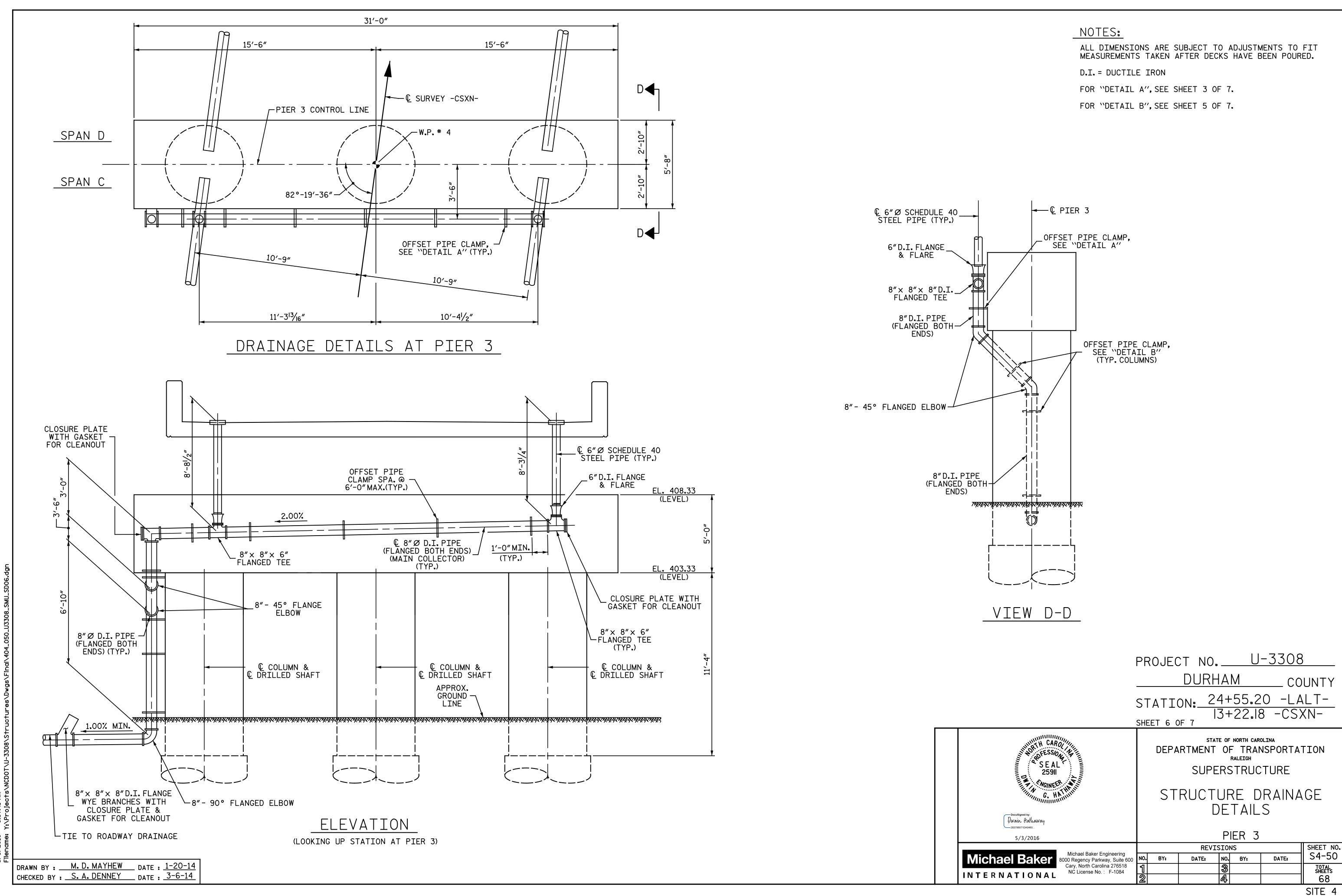
Michael Baker Engineering
8000 Regency Parkway, Suite 600
Cary, North Carolina 276518
NC License No.: F-1084

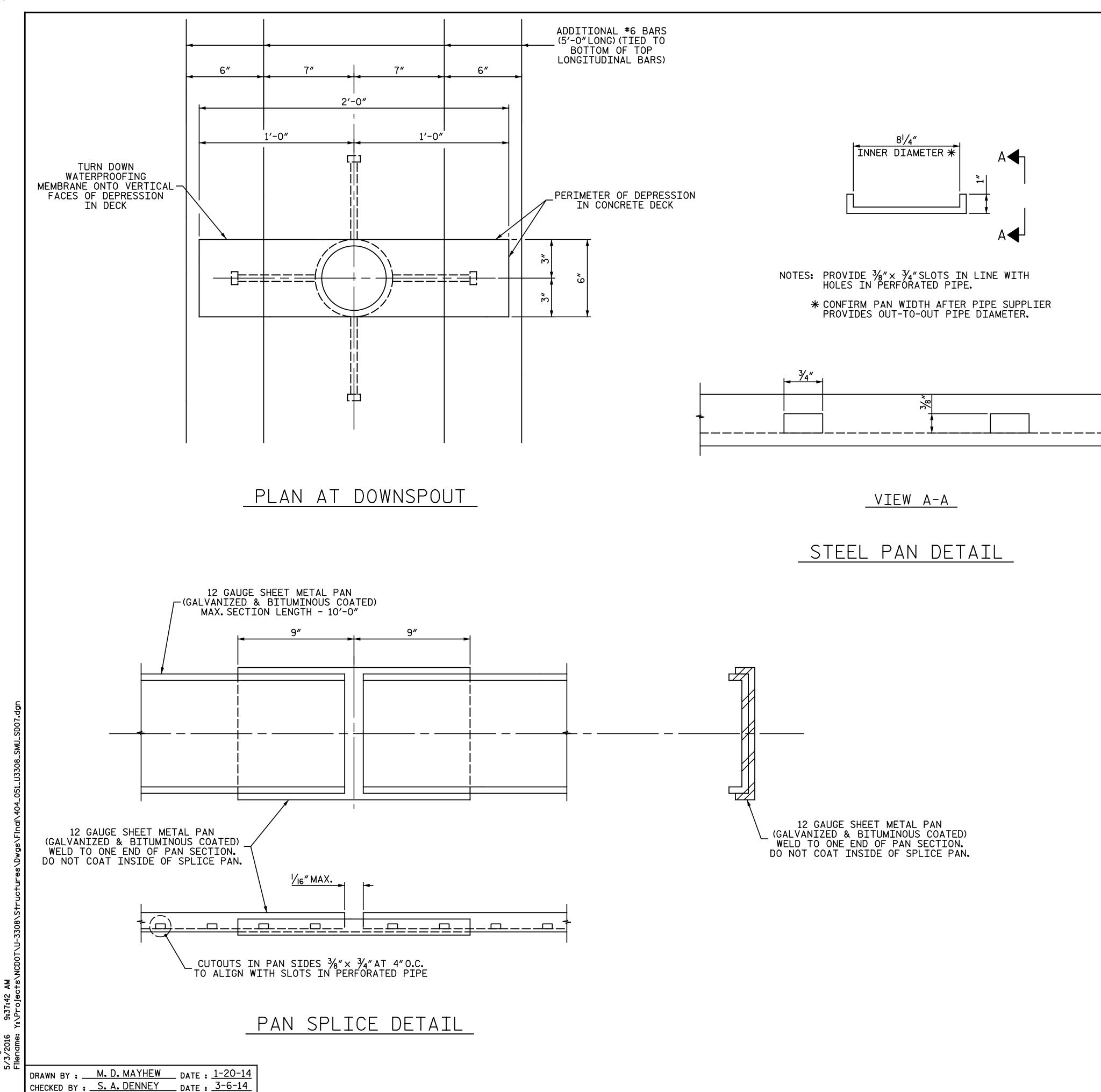
REVISIONS

NO. BY: DATE: NO. BY: DATE:

1 3 5 TOTAL
SHEET SHEETS
68







NOTES:

ALL PIPES, FLANGES AND FITTINGS SHALL BE CLASS 53 DUCTILE IRON, UNLESS NOTED OTHERWISE.

ALL BENDS TO BE SHORT RADIUS, INCLUDING FLANGE AND FLARE BENDS, UNLESS OTHERWISE NOTED.

MAKE FINAL PIPE ALIGNMENT AND TIGHTEN U-BOLTS AFTER RAILROAD TRACK HAS BEEN LAID ACROSS THE BRIDGE.

PAYMENT FOR ALL MATERIALS, FABRICATION, INSTALLATION AND ADJUSTMENTS RELATED TO STRUCTURE DRAINAGE SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR "STRUCTURE DRAINAGE SYSTEM". NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE FOR ANY COMPONENT OF THE STRUCTURE DRAINAGE SYSTEM INCLUDING, BUT NOT LIMITED TO:

- DUCTILE IRON PIPE AND FITTINGS
- STANDARD PIPE
- GASKET AND PVC OR NEOPRENE COATED STRIPS
- STEEL SUPPORT ANGLES AND PLATES
- U-BOLTS AND H.S. BOLTS, WASHERS, AND NUTS - OFFSET PIPE CLAMPS AND EXPANSION ANCHOR BOLTS

PROVIDE PVC OR NEOPRENE-COATED STRIPS, EPOXY-CEMENTED TO THE U-BOLT OR PIPE FOR STRAY CURRENT PROTECTION.

THE OUTSIDE COATING FOR D.I. PIPE MAY BE PAINTED WITH A SHOP PRIME COAT OF INORGANIC ZINC PRIMER AND A FINISH (FIELD) COATING OF VINYL PAINT AS SPECIFIED FOR THE STRUCTURAL STEEL.

FOR STRUCTURE DRAINAGE SYSTEM, SEE SPECIAL PROVISIONS.

D.I. = DUCTILE IRON

FOR WATERPROOFING SYSTEM, SEE SPECIAL PROVISION "WATERPROOFING (RAILROAD STRUCTURES)".

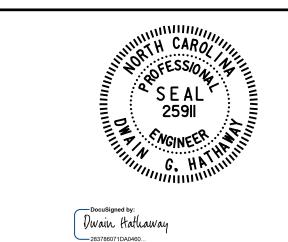
STRUCTURE DRAINAGE SYSTEM ESTIMATED QUANTITIES					
ITEM	QUANTITY	UNIT			
8"I.D. DUCTILE IRON PIPE, CLASS 53 (FLANGED BOTH ENDS)	137 . 5	FT.			
6"Ø STD PIPE (GALVANIZED, WITH $rac{3}{4}$ "Ø $ imes$ 6"HEADED STUDS)	57	FT.			
6" & 8"I.D. DUCTILE IRON PIPE FLANGED FITTINGS, 250 P.S.I. P.R.	2760	LBS.			
8"I.D. DUCTILE IRON BLIND FLANGES	420	LBS.			
8"I.D. PERFORATED HALF ROUND PIPE, (GALVANIZED AND BITUMINOUS COATED)	314.5	FT.			
STEEL PAN BELOW HALF ROUND PIPE, (GALVANIZED AND BITUMINOUS COATED)	314.5	FT.			
8"I.D. OFFSET PIPE CLAMPS	240	LBS.			

PROJECT NO. U-3308

DURHAM COUNTY

STATION: 24+55.20 -LALT-

SHEET 7 OF 7



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

STRUCTURE DRAINAGE DETAILS

Michael Baker

NOTERNATIONAL

Michael Baker

Cary, Note Line

NOTE Line

Michael Baker

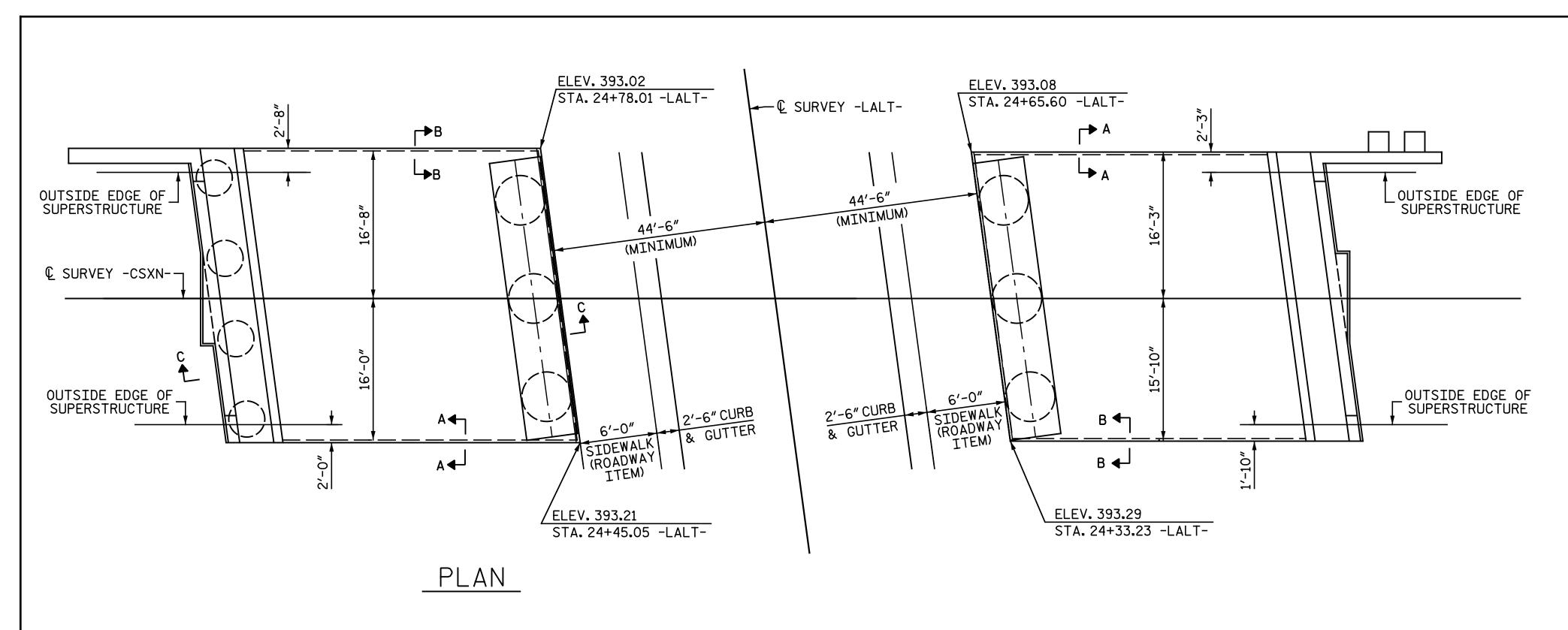
NOTE Line

Michael Baker Engineering 8000 Regency Parkway, Suite 600 Cary, North Carolina 276518 NC License No.: F-1084

REVISIONS

NO. BY: DATE: NO. BY: DATE: S4-51

1 3 5 5 68



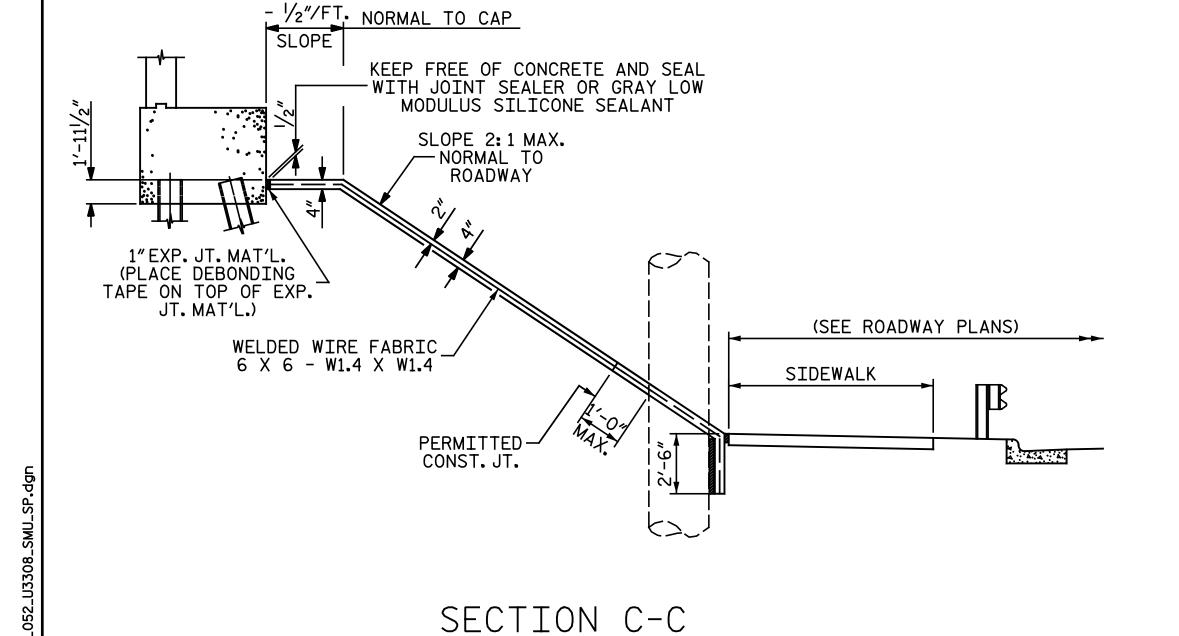
GENERAL NOTES:

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS, MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING, SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5'STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

BRIDGE @ STA.13+22.18 -CSXN-	4 INCH SLOPE PROTECTION	* WELDED WIRE FABRIC 60 INCHES WIDE					
	SQUARE YARDS	APPROX. L.F.					
ABUTMENT 1	145	290					
ABUTMENT 2	143	286					

* QUANTITY SHOWN IS BASED ON 5' POURS.



1"EXP. JT. MAT'L. (PLACE — DEBONDING TAPE ON TOP OF EXP. JT. MAT'L.)

PLAN WHERE CONCRETE

BE PLACED AROUND

A PIER COLUMN

SEAL WITH GRAY LOW-

MODULUS SILICONE SEALANT, 1/2" DEEP (MIN.)

2'-0"LONG #4 BARS SPA. @ 1'-6"CTS. MAX. 5'-0" CONST.JT.TO BE NORMAL TO END BENT CAP OR HORIZONTAL STRIP WIDTHS MAY VARY IN CURVED PORTION.

POURING DETAIL

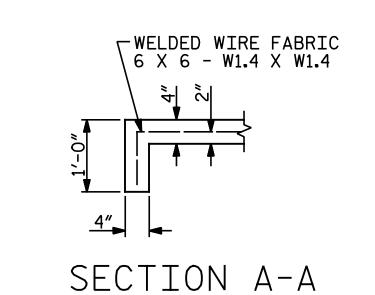
POUR A 4'-O" STRIP FIRST. STRIP WIDTHS MAY VARY IN CURVED PORTION.

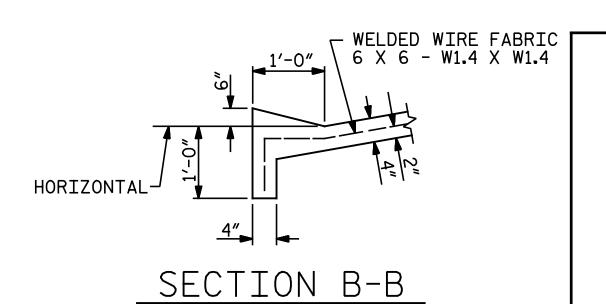
OPTIONAL POURING DETAIL

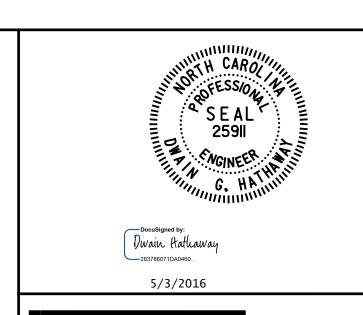
-CONST.JT.TO BE NORMAL TO END BENT CAP OR HORIZONTAL

PROJECT NO. <u>U-3308</u> DURHAM COUNTY 24+55.20 -LALT-STATION:_

13+22.18 -CSXN-







SLOPE PROTECTION

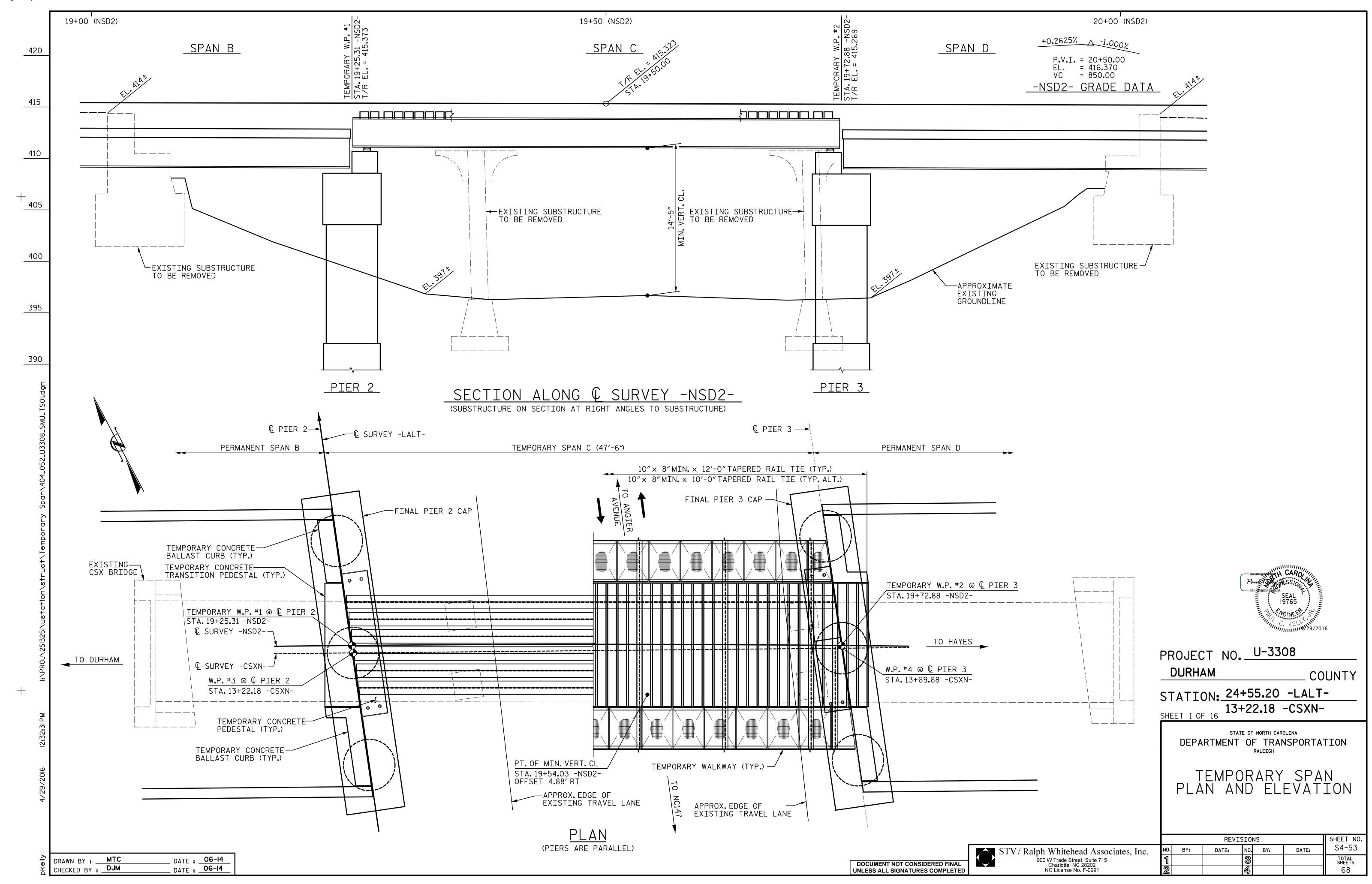
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

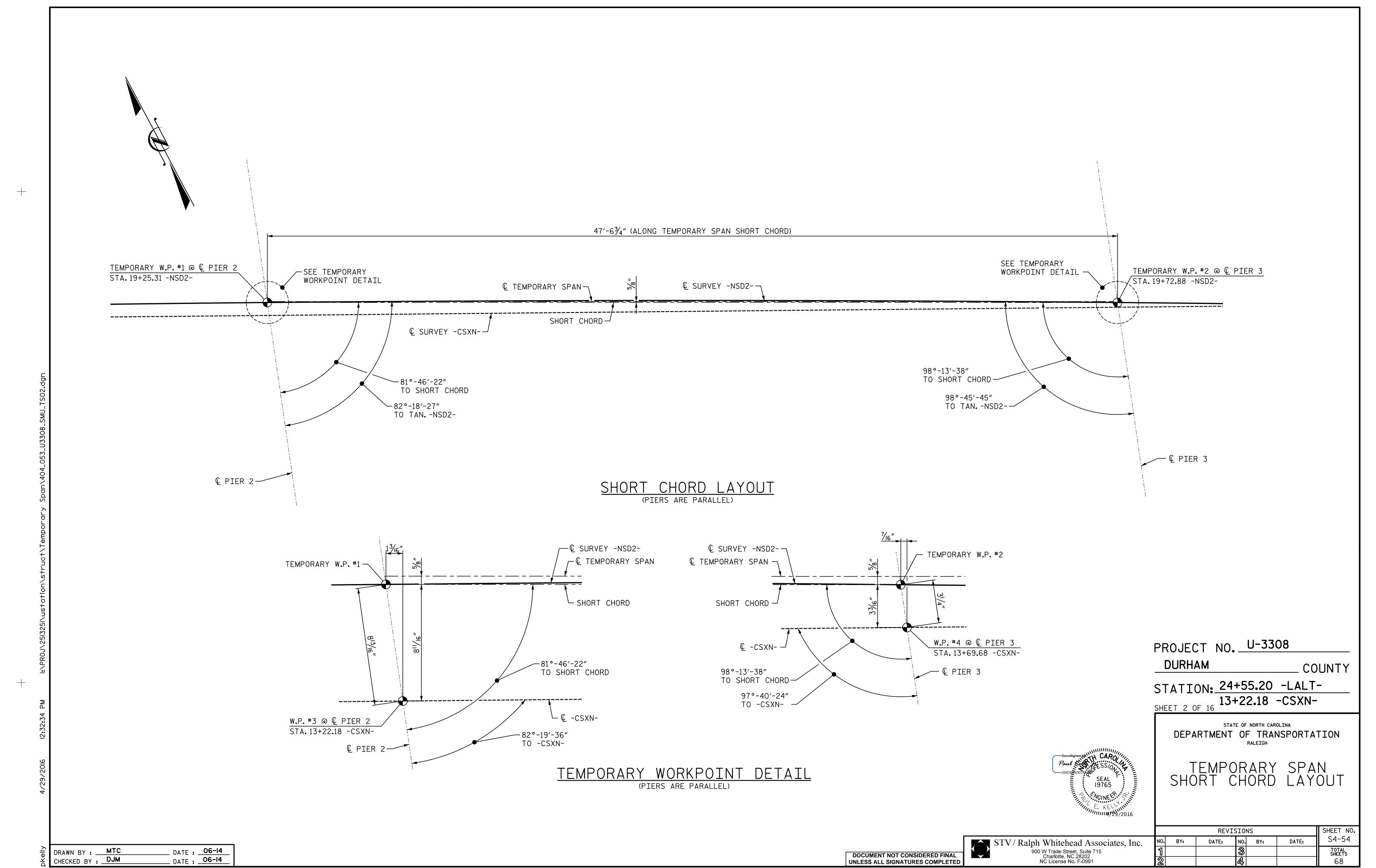
DETAILS

		REVISIONS						SHEET NO
/lichael Baker	Michael Baker Engineering 8000 Regency Parkway, Suite 600	NO.	BY:	DATE:	NO.	BY:	DATE:	S4-52
	Cary, North Carolina 276518 NC License No.: F-1084	1			3			TOTAL SHEETS
TERNATIONAL		2			4			68
					-			CITE 1

CHECKED BY: S. A. DENNEY DATE: 3-6-14

SITE 4





TEMPORARY SPAN GENERAL NOTES

- 1. THE TEMPORARY SPAN HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF AREMA'S "MANUAL FOR RAILWAY ENGINEERING, VOL. 2, STRUCTURES", AND NORFOLK SOUTHERN CORPORATION'S "UNDERPASS GRADE SEPARATION CRITERIA".
- 2. ASSUMED LIVE LOAD: COOPER E80 OR ALTERNATE LIVE LOAD.
- 3. FOR ADDITIONAL NOTES AND DESIGN DATA SEE SHEET "GENERAL DRAWING SHEET 4 OF 5% IN THE EVENT ANY NOTES CONFLICT, THE NOTES ON THIS SHEET SHALL CONTROL FOR THE TEMPORARY SPAN.
- 4. THE TEMPORARY SPAN HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES FOR SEISMIC ZONE 1.
- 5. REINFORCING STEEL SHALL BE ASTM 615, GRADE 60. ALL DIMENSIONS RELATING TO BAR SPACING ARE TO BAR CENTERS UNLESS NOTED OTHERWISE, FABRICATION IS TO BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE", ACI 315 (CURRENT EDITION). ALL REINFORCING IN THE CONCRETE DECK SLAB AND PARAPETS SHALL BE EPOXY COATED.
- 6. THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.
- 7. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY 2012, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS), EXCEPT AS NOTED HEREIN, ELSEWHERE ON PLANS, OR IN THE SPECIAL PROVISIONS. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CURRENT AREMA SPECIFICATIONS AND NORFOLK SOUTHERN'S "SPECIFICATIONS FOR
- 8. ALL CONCRETE USED FOR THE TEMPORARY SPAN (DECK AND CURBS) SHALL BE MIN. 5,000 PSI CONCRETE, WITH NO.57 OR 67 COARSE AGGREGATE AND SHALL BE AIR-ENTRAINED. MINIMUM CEMENT PER CUBIC YARD OF CONCRETE SHALL BE 6.5 BAGS. NO SUBSTITUTION OF FLY ASH, BLAST FURNACE SLAG OR OTHER MATERIAL WILL BE PERMITTED IN MEETING THIS MINIMUM CEMENT REQUIREMENT. CHAMFER ALL EXPOSED EDGES AND CORNERS 3/4" EXCEPT AS NOTED ON THE PLANS. THE USE OF GROUND GRANULATED BLAST FURNACE SLAG IS NOT PERMITTED IN THIS STRUCTURE.
- 9. CONTROL OF WORK: ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILWAY STRUCTURE SHALL BE PERFORMED SATISFACTORY TO THE ENGINEER AND/OR NORFOLK SOUTHERN RAILWAY COMPANY. ALL METHODS OF HANDLING THE WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY COMPANY BEFORE PROCEEDING WITH THAT PORTION OF THE WORK, RAIL TRAFFIC SHALL, AT ALL TIMES, BE MAINTAINED AND PROTECTED. THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH RAIL OPERATIONS.
- 10. NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.
- 11. FOR PORTLAND CEMENT, SEE SPECIAL PROVISIONS.
- 12. FOR FINE AND COARSE AGGREGATE. SEE SPECIAL PROVISIONS.
- 13. SEE "TEMPORARY SPAN STRUCTURAL STEEL NOTES" SHEET FOR ADDITIONAL NOTES.

- 14. FOR RAILROAD TRACKWORK, SEE RAILROAD TRACKWORK PLANS.
- 15. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- 16. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- 17. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- 18. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- 19. FOR CAST-IN-PLACE CONCRETE, SEE SPECIAL PROVISION FOR NORFOLK SOUTHERN SPECIFICATIONS FOR CAST-IN-PLACE CONCRETE.
- 20. FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISION FOR NORFOLK SOUTHERN SPECIFICATIONS FOR PAINTING SHOP FABRICATED BRIDGE STEEL.
- 21. FOR PROTECTION OF RAILWAY INTERESTS, SEE SPECIAL PROVISIONS.
- 22. FOR RAILROAD ROADBED, SEE RAILROAD ROADBED SPECIAL PROVISIONS.
- 23. FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE, SEE SPECIAL PROVISIONS.
- 24. SHOCK PADS SHALL BE PREFORMED FABRIC BEARING PADS, 1/2" THICK, AND SHALL BE EITHER SHOCK PAD STYLE 15175, AS MANUFACTURED BY THE ALERT MANUFACTURING AND SUPPLY COMPANY, CHICAGO, IL; OR FABREEKA PADS, AS MANUFACTURED BY THE FABREEKA PRODÚCTS COMPANY, BOSTON, MA; OR SORBTEX PADS, AS MANUFACTURED BY VOSS ENGINEERING, INC., CHICAGO, IL; OR AN APPROVED EQUAL.
- 25. ALL CONSTRUCTION JOINTS SHOWN ON THESE PLANS SHALL BE REQUIRED UNLESS SHOWN OPTIONAL. CONSTRUCTION JOINTS SHALL NOT BE PERMITTED EXCEPT AS SHOWN ON THE PLANS, OR WHERE WRITTEN APPROVAL HAS BEEN OBTAINED.
- 26. BENCHMARK: SEE LOCATION SKETCH.
- 27. DIRECT TENSION INDICATORS (DTI) WILL NOT BE PERMITTED. USE THE TURN-OF-NUT METHOD FOR INSTALLING AND TIGHTENING HIGH STRENGTH BOLTS.
- 28. THE RAILROAD TRACK TOP OF RAIL ELEVATIONS ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE, PRIOR TO BEGINNING BRIDGE CONSTRUCTION. VERIFY THE TOP OF RAIL ELEVATIONS AND REPORT ANY VARIATIONS TO THE ENGINEER, ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.
- 29. THE CONTRACTOR IS REMINDED THAT WORK ON THIS PROJECT REQUIRES WORKING NEAR EXISTING STRUCTURES. EVERY EFFORT HAS BEEN MADE TO IDENTIFY DISCREPANCIES AND ENSURE THAT THE DETAILS ARE DEPICTED CORRECTLY, HOWEVER, SINCE THE PROJECT INVOLVES WORKING NEAR EXISTING STRUCTURES, THE CONTRACTOR CAN EXPECT AND SHOULD PLAN ON ENCOUNTERING VARIANCES AND DEVIATIONS BETWEEN THE INFORMATION FOUND IN THESE DRAWINGS AND THE EXISTING CONDITIONS. ACTUAL FIELD CONDITIONS MAY REQUIRE MODIFICATIONS IN CONSTRUCTION DETAILS AND QUANTITIES. THE CONTRACTOR IS RESPONSIBLE TO FIELD VERIFY ALL DETAILS INCLUDING GEOMETRY AND ELEVATIONS PRIOR TO THE INSTALLATION OF ANY MATERIAL. THE CONTRACTOR SHALL SUBMIT TO NCDOT AND NORFOLK SOUTHERN COPIES OF FIELD SURVEYS AND VERIFICATIONS FOR INCLUSION INTO THE CONSTRUCTION RECORDS FOR THE PROJECT.
- 30. ALL COSTS ASSOCIATED WITH THE TEMPORARY SPAN. INCLUDING BUT NOT LIMITED TO ERECTION, DEMOLITION/REMOVAL, PEDESTALS AND BALLAST CURBS. ALL MATERIALS (INCLUDING BUT NOT LIMITED TO STRUCTURAL STEEL, CONCRETE, REINFORCING STEEL, DECK GRATING, TRACK. TIES. MECHANICAL ANCHORS AND ANCHOR BOLTS. ETC.) AND LABOR SHALL BE INCLUDED IN THE LUMP SUM BID PRICE FOR THE "CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY SPAN". NO ADDITIONAL PAYMENT WILL BE MADE.

PROJECT NO. U-3308

DURHAM

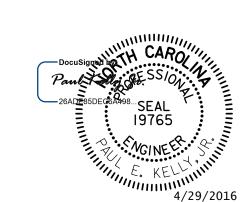
COUNTY

STATION: 24+55.20 -LALT-13+22.18 -CSXN-

SHEET 3 OF 16

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TEMPORARY SPAN GENERAL NOTES



DATE: IO. BY: TV / Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-099

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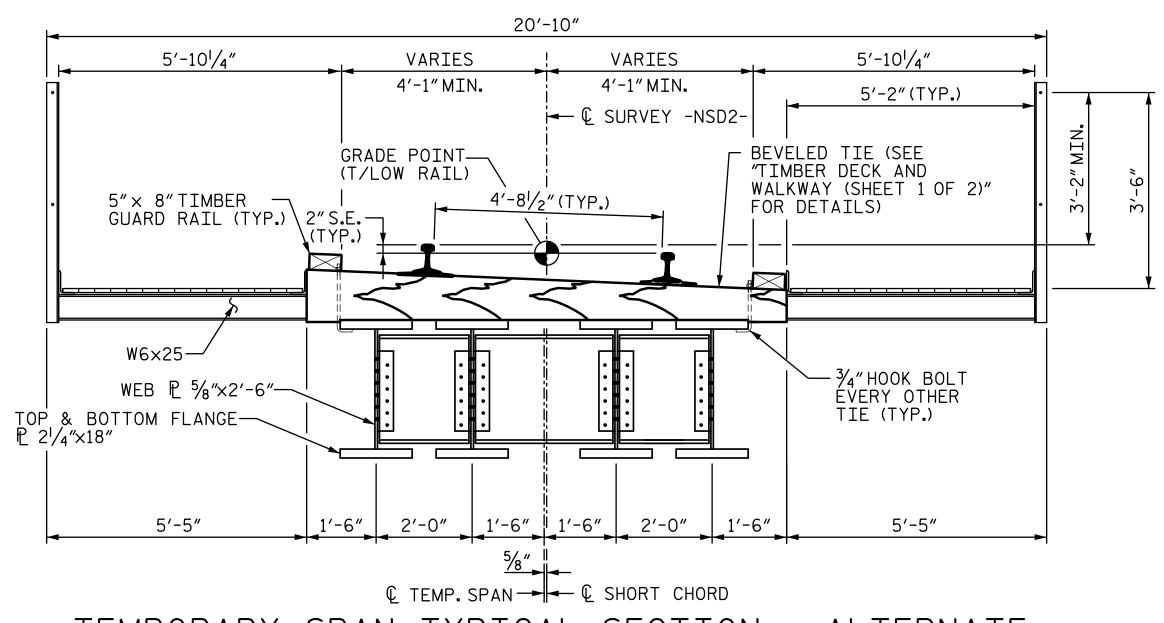
UNLESS ALL SIGNATURES COMPLETED

REVISIONS SHEET NO S4-55 DATE: BY: TOTAL SHEETS

DATE : 06-14 MTC DRAWN BY : CHECKED BY : DJM DATE : 06-14 20'-10"

TEMPORARY SPAN TYPICAL SECTION

(FOR TEMPORARY SPAN ALONG -NSD2- TRACK) (SIMPLE SPAN NON-COMPOSITE ASTM A-709 GRADE 50 W33x263 ROLLED BEAMS)



TEMPORARY SPAN TYPICAL SECTION - ALTERNATE

(FOR TEMPORARY SPAN ALONG -NSD2- TRACK) (SIMPLE SPAN NON-COMPOSITE ASTM A-709 GRADE 50 PLATE GIRDERS)

NOTE: THE ALTERNATE CONFIGURATION HAS BEEN PROVIDED AS AN ACCEPTABLE ALTERNATIVE TO THE TEMPORARY SPAN AS DETAILED IN THESE PLANS. SHOULD THE CONTRACTOR CHOOSE TO USE THE ALTERNATE, THE APPLICABLE TEMPORARY SPAN PLAN SHEETS SHALL BE UPDATED, SIGNED AND SEALED BY A LICENSED AND REGISTERED ENGINEER AND SUBMITTED FOR APPROVAL TO THE DEPARTMENT AND RAILROADS PRIOR TO BEGINNING CONSTRUCTION. ALL COSTS ASSOCIATED WITH THE ALTERNATE DESIGN AND PLAN SUBMITTAL SHALL BE INCLUDED IN THE LUMP SUM BID PRICE FOR THE TEMPORARY SPAN, NO ADDITIONAL MEASUREMENT OR PAYMENT WILL BE MADE.

SEAL 19765

/////^{29/2016}

PROJECT NO. U-3308

DURHAM

STATION: 24+55.20 -LALT-

COUNTY

13+22.18 -CSXN-

SHEET 4 OF 16

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

TEMPORARY SPAN TYPICAL SECTIONS

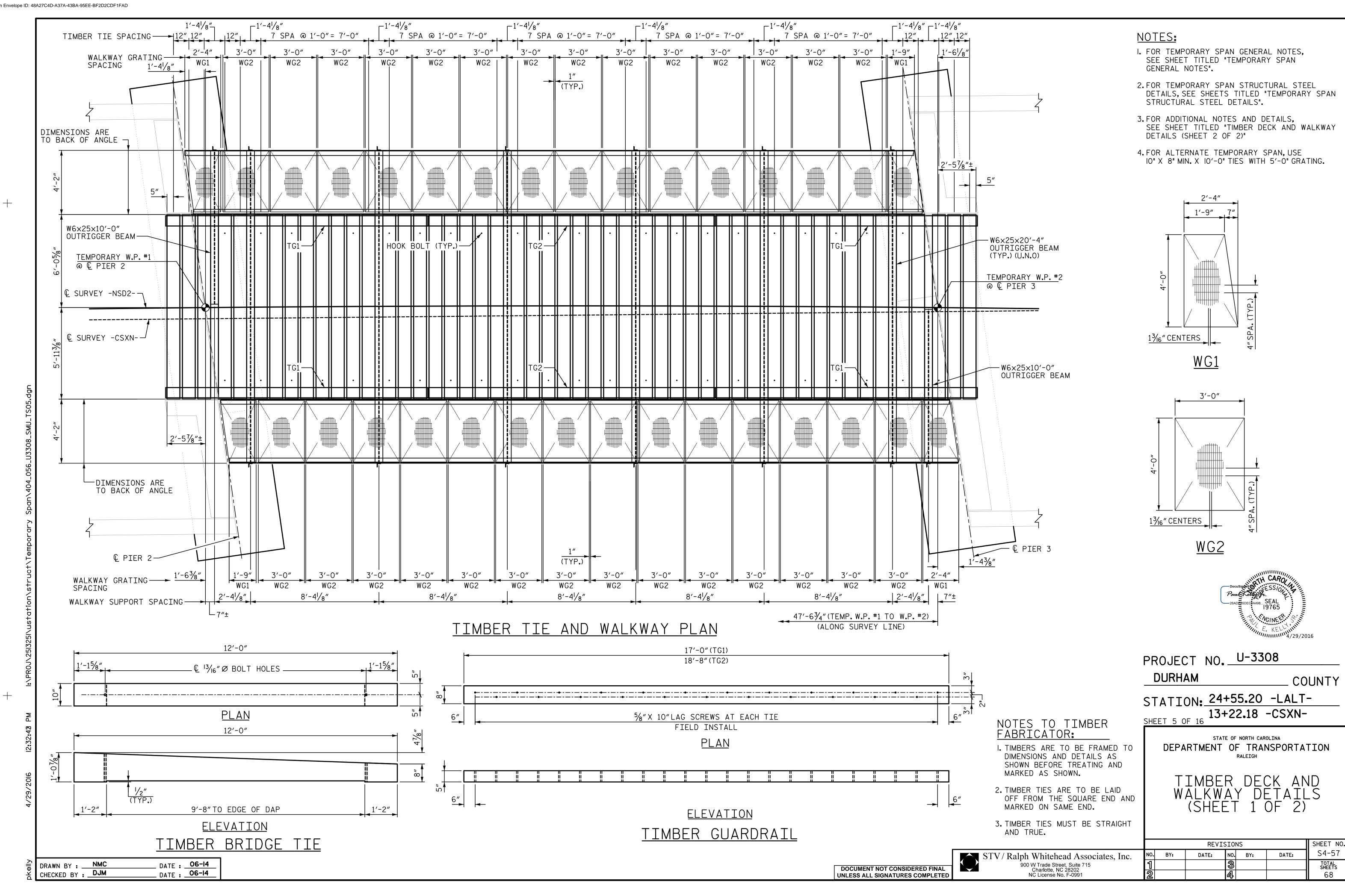
STV/Ralph Whitehead Associates, Inc. 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991

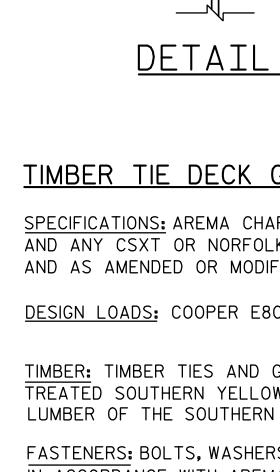
REVISIONS SHEET NO. S4-56 DATE: DATE: NO. BY: BY: TOTAL SHEETS

_ DATE : 06-14 MTC DRAWN BY: CHECKED BY : DJM _ DATE : 06-14

UNLESS ALL SIGNATURES COMPLETED

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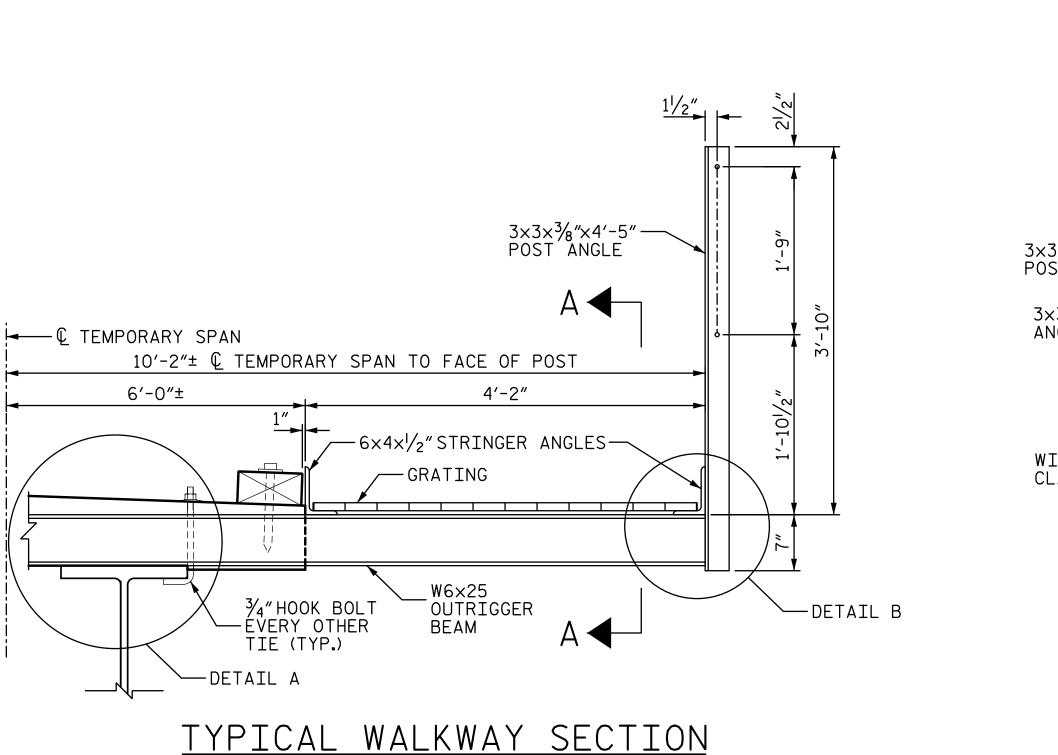


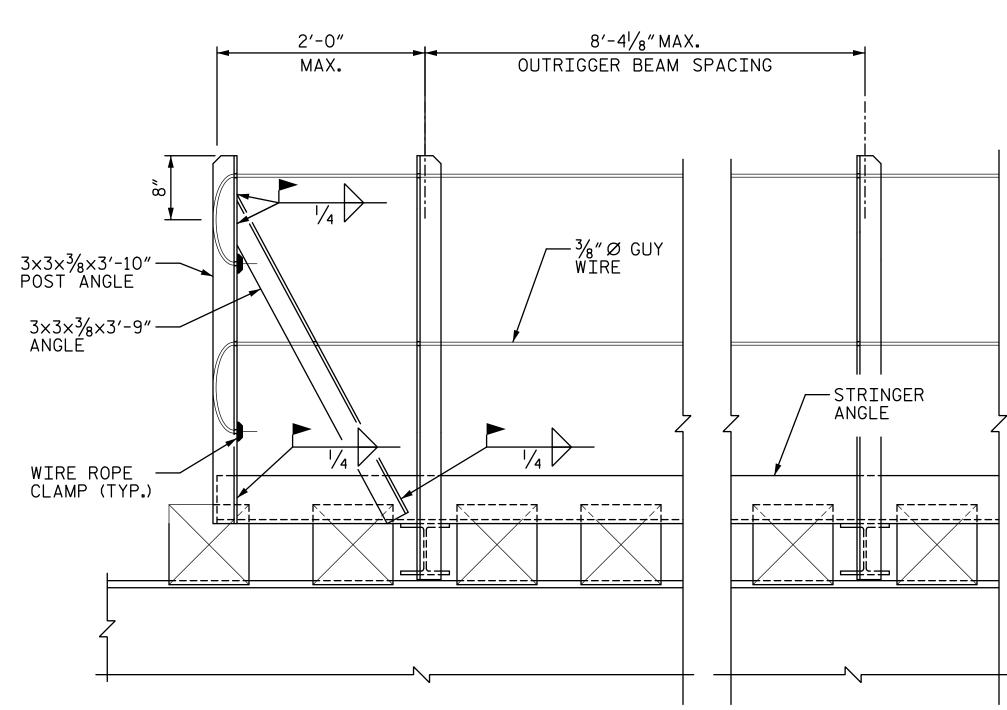


NMC

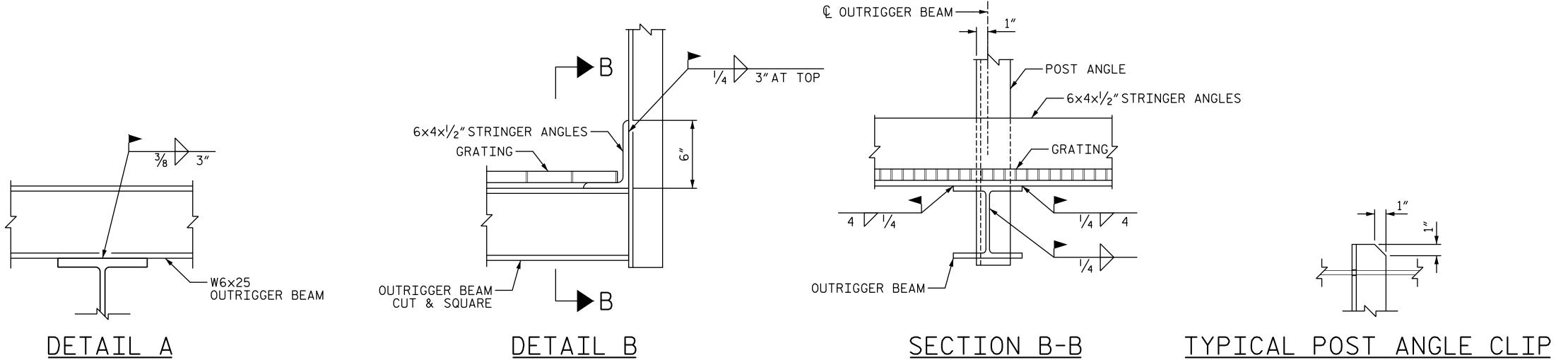
DRAWN BY :

CHECKED BY : DJM





ELEVATION AT TEMPORARY SPAN ENDS



TIMBER TIE DECK GENERAL NOTES:

SPECIFICATIONS: AREMA CHAPTER 7, "TIMBER STRUCTURES" AND ANY CSXT OR NORFOLK SOUTHERN STANDARD PROCEDURES, AND AS AMENDED OR MODIFIED ON THESE PLANS.

DESIGN LOADS: COOPER E80 LIVE LOAD

TIMBER: TIMBER TIES AND GUARDRAIL SHALL BE CREOSOTE TREATED SOUTHERN YELLOW PINE, DENSE STRUCTURAL 65, LUMBER OF THE SOUTHERN PINE INSPECTION BUREAU.

FASTENERS: BOLTS, WASHERS, LAG SCREWS, ETC. SHALL BE IN ACCORDANCE WITH AREMA 7.1.6 "SPECIFICATION OF FASTENERS FOR TIMBER TRESTLES".

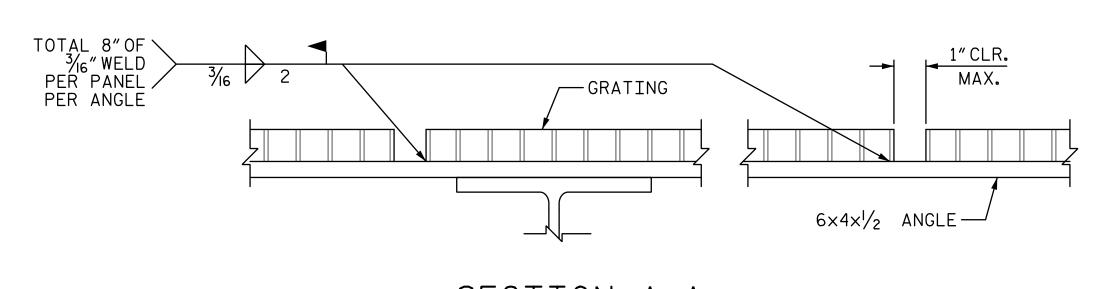
TIMBER GUARDRAIL: GUARDRAILS SHALL BE FASTENED TO EACH TIMBER BRIDGE TIE.

. DATE : 06-14

_ DATE : 06-14

TIE ANCHORAGE: THE TIMBER TIES SHALL BE FASTENED TO THE BEAM FLANGES WITH 3/4" DIA. HOOK BOLTS, OR EQUAL AT EVERY OTHER TIE. TIMBER TIES SHALL BE DAPPED TO FIT TOP FLANGES OF BEAM AS SHOWN ON SHEET TITLED "TIMBER DECK AND WALKWAY DETAILS (I OF 2)".

WALKWAY: WALKWAY GRATING SHALL BE RECTANGULAR WELDED GRATING MADE OF ASTM A569 STEEL, HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 AFTER FABRICATION. BEARING BARS ARE TO BE I" X 1/8" AT 13/6" CENTERS, AND TWISTED SQ. BARS AT 4" CENTERS. HANDRAIL POST SHALL BE MADE OF ASTM A36 STEEL, HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 AFTER FABRICATION. GUY WIRE SHALL BE 3/8" DIA. GALVANIZED WIRE ROPE ANCHORED WITH GALVANIZED WIRE ROPE CLIPS (GALV. MALLEABLE IRON HEAVY DUTY CROSBY CLIPS AND ROPE THIMBLE).



SECTION A-A

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TIMBER DECK AND WALKWAY QUANTITIES

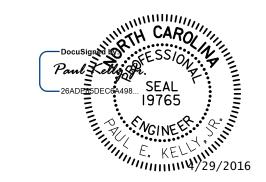
ITEMS	UNIT	QUANTITY
W6X25 OUTRIGGER BEAM	LBS.	3550
L6X4X ¹ / ₂ STRINGER	LBS.	3176
L3X3X3/8 POSTS AND END BRACE	LBS.	664
WALKWAY GRATING	LBS.	2775
8"MIN.X 10"X 12'-0"TIE	EA.	27
8"MIN.X 10"X 12'-0"TIE W/ HOOK BOLT	EA.	23
5"X 8"TIMBER GUARD RAIL	LF.	105.3

TIMBER DECK AND WALKWAY QUANTITIES - ALTERNATE

ITEMS	UNIT	QUANTITY
		QUAITITI
W6X25 OUTRIGGER BEAM	LBS.	3550
L6X4X1/2 STRINGER	LBS.	3176
$L3X3X\frac{3}{8}$ POSTS AND END BRACE	LBS.	664
WALKWAY GRATING	LBS.	3440
8"MIN. X 10"X 10'-0"TIE	EA.	27
8"MIN. X 10"X 12'-0"TIE W/ HOOK BOLT	EA.	23
5"X 8"TIMBER GUARD RAIL	LF.	105.3

NOTES:

- I. FOR TEMPORARY SPAN GENERAL NOTES, SEE SHEET TITLED "TEMPORARY SPAN GENERAL NOTES".
- 2. FOR TEMPORARY SPAN STRUCTURAL STEEL DETAILS, SEE SHEETS TITLED "TEMPORARY SPAN STRUCTURAL STEEL DETAILS".
- 3. FOR ADDITIONAL NOTES AND DETAILS, SEE SHEET TITLED "TIMBER DECK AND WALKWAY DETAILS (I OF 2)".
- 4. FOR ALTERNATE TEMPORARY SPAN, SEE SHEET TITLED "TEMPORARY SPAN TYPICAL SECTION"



PROJECT NO. U-3308

DURHAM COUNTY

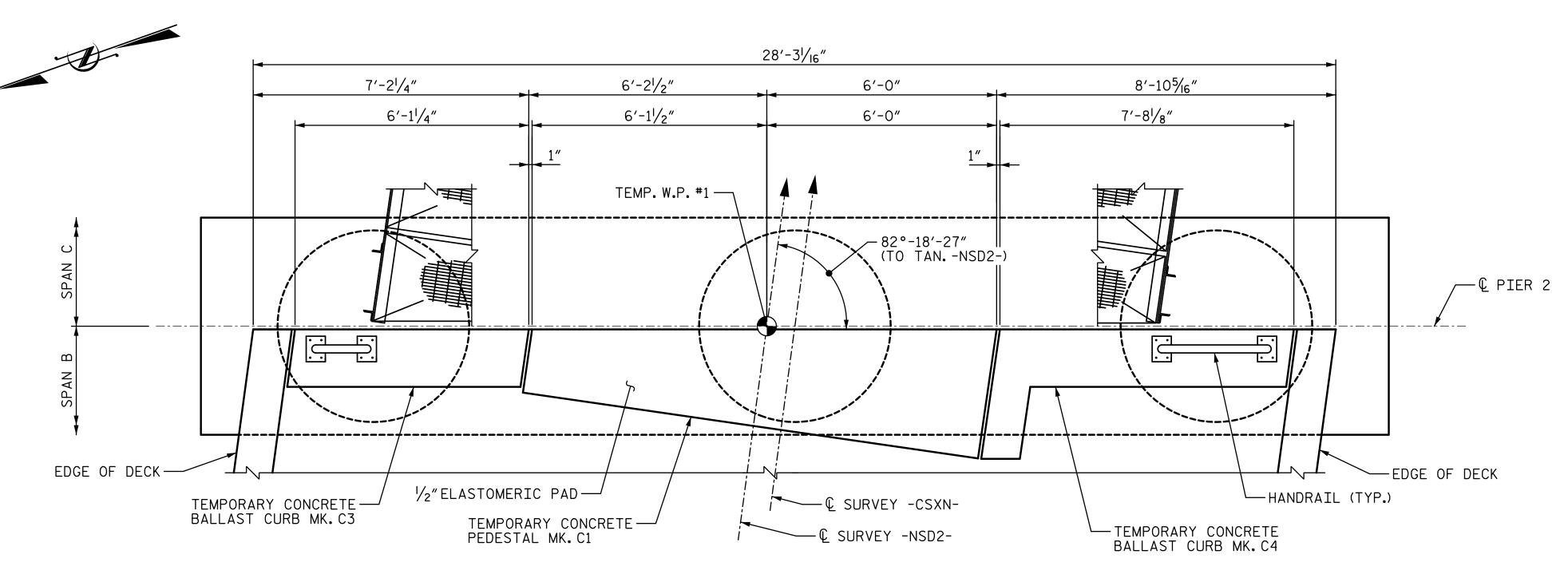
STATION: 24+55.20 -LALT-

13+22.18 -CSXN-

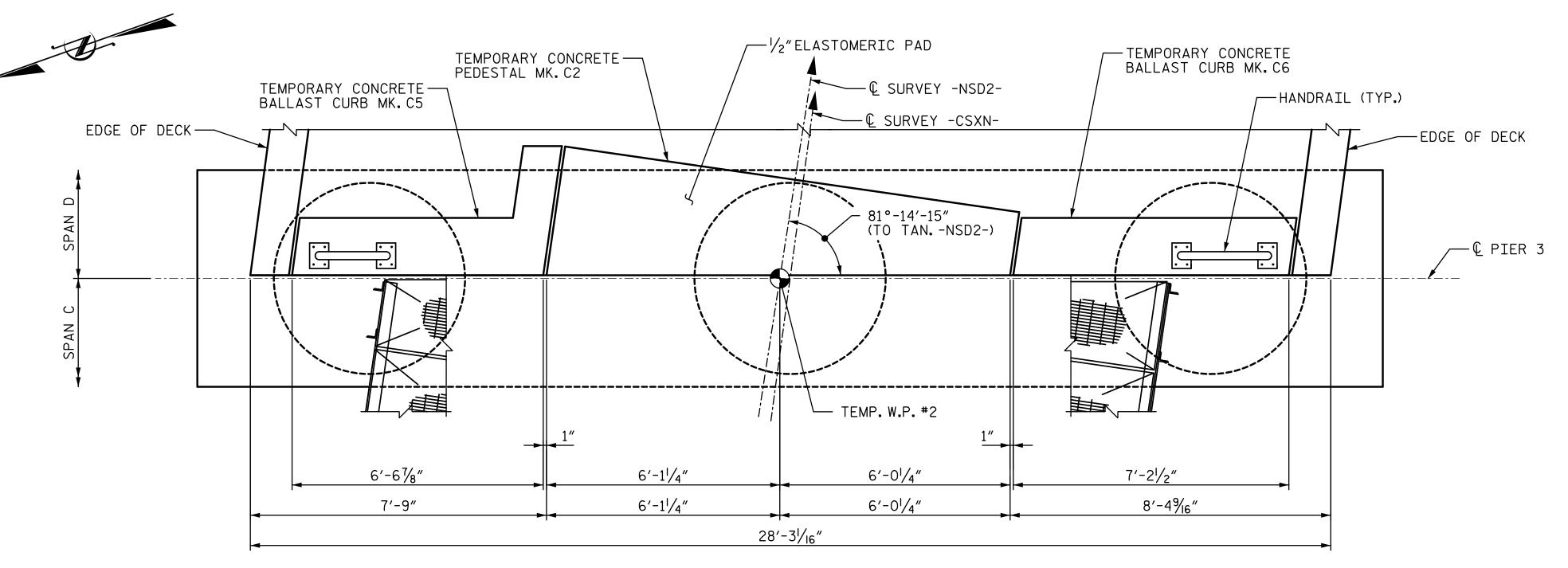
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TIMBER DECK AND WALKWAY DETAILS (SHEET 2 OF 2)

REVISIONS SHEET NO. S4-58 DATE: NO. DATE: NO. BY: BY: TOTAL SHEETS



<u>PLAN VIEW - TEMPORARY CONCRETE PEDESTAL</u> AND BALLAST RETAINER AT PIER 2



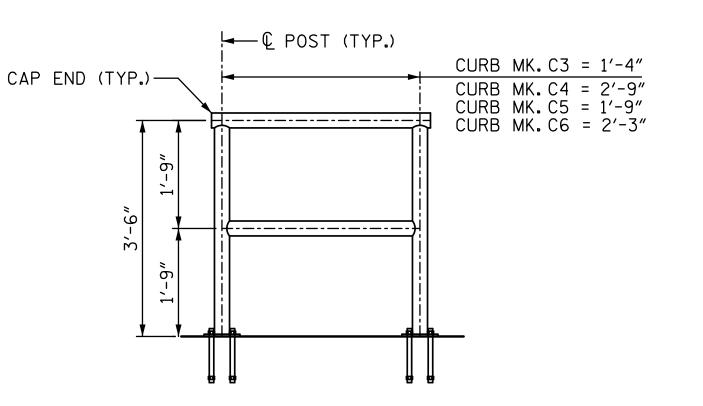
PLAN VIEW - TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER AT PIER 3

NOTES:

- 1. FOR ADDITIONAL HANDRAIL DETAILS, SEE SHEET TITLED "HANDRAIL
- 2. FOR LOCATION OF HANDRAIL, SEE SHEETS TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER".
- 3. 1/2" ELASTOMERIC PAD SHALL BE SIZED TO THE FULL PLAN DIMENSIONS OF THE TEMPORARY CONCRETE PEDESTAL (MK. C1 AND C2).

CIP CONCRETE PEDESTAL & BALLAST CURB INSTALLATION SEQUENCE

- 1. PLACE FELT FABRIC ON EXISTING PIER CAP.
- 2. INSTALL MECHANICAL ANCHORS AND ANCHOR BLOCKOUT, SEE ANCHOR BOLT DETAIL ON SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (4 OF 4)"FOR ANCHOR INFORMATION.
- 3. FORM CIP CONCRETE PEDESTAL, BALLAST CURB, BLOCKOUTS, ETC.
- 4. PLACE CONCRETE AND FINISH TO SPECIFIED ELEVATIONS AND INSTALL ANCHOR BOLTS.
- 5. REMOVE FORMS.
- 6. INSTALL HANDRAILS ON TEMPORARY CONCRETE BALLAST CURBS.
- 7. PLACE $\frac{1}{2}$ " ELASTOMERIC PAD ON TEMPORARY CONCRETE PEDESTAL.
- 8. INSTALL TEMPORARY SPAN AND TRACK.
- 9. AFTER COMPLETION OF NSRR BRIDGE, REMOVE TEMPORARY SPAN AND
- 10. REMOVE ANCHOR BOLTS, CIP COMPONENTS AND FELT FABRIC.



TEMPORARY BALLAST CURB HANDRAIL DETAIL

PROJECT NO. U-3308

DURHAM

COUNTY

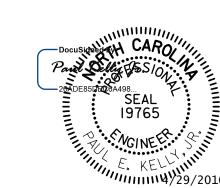
TOTAL SHEETS

STATION: 24+55.20 -LALT-13+22.18 -CSXN-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)



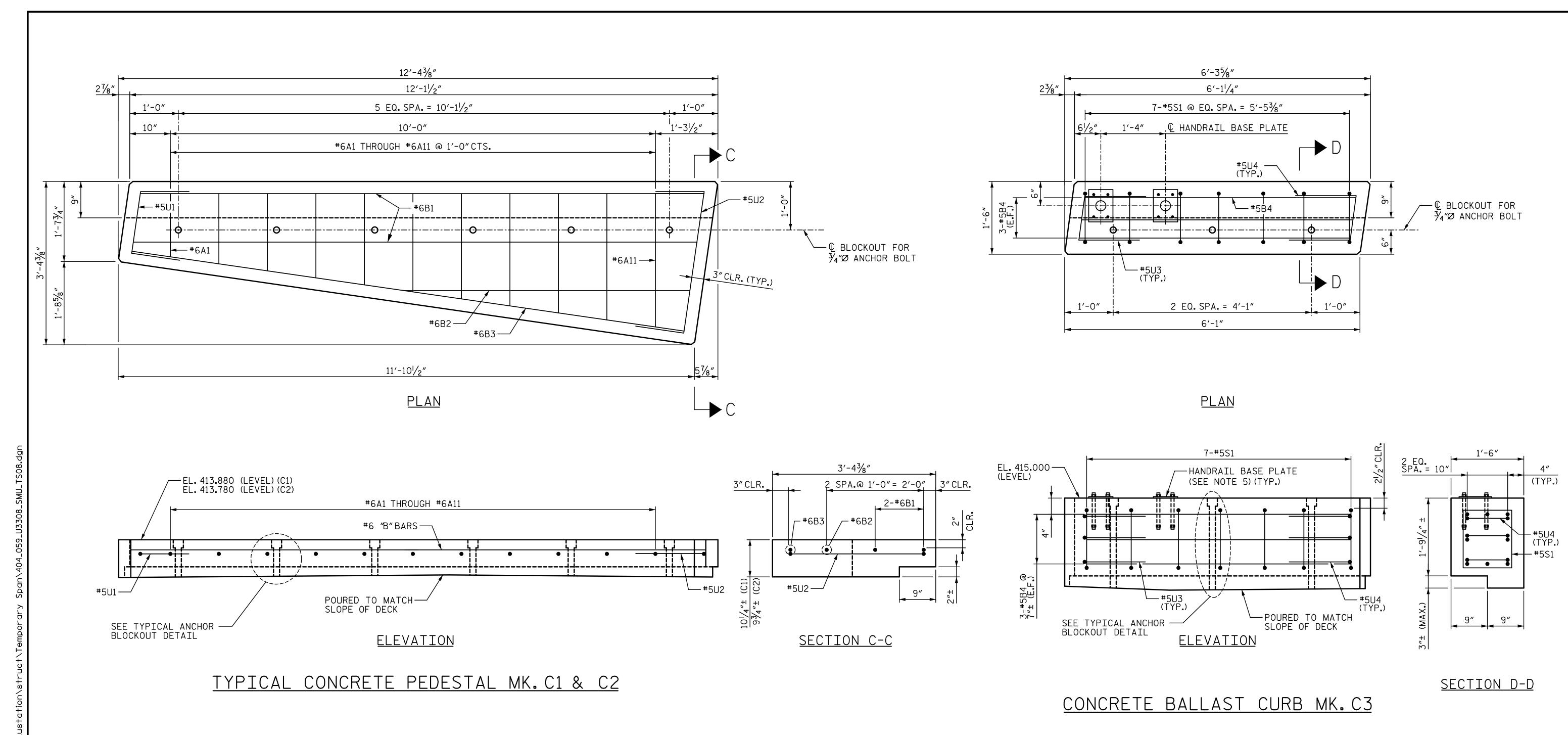
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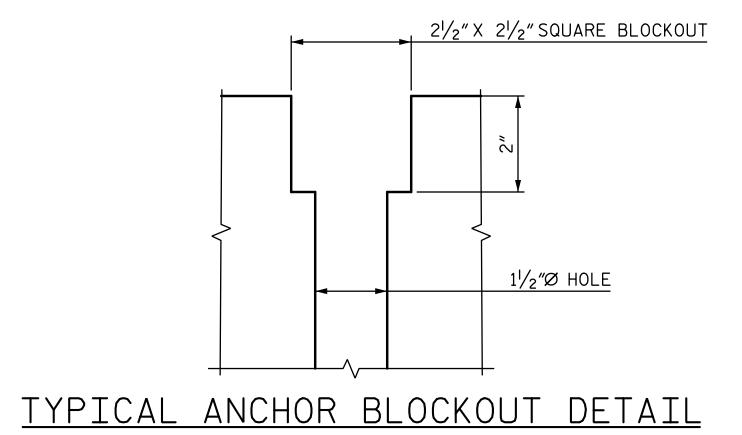
REVISIONS SHEET NO. S4-59 NO. BY: DATE:

DATE : 06-14
DATE : 06-14 MTC DRAWN BY : CHECKED BY : DJM

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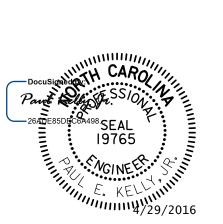
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NOTES:

- 1. ALL BLOCKOUT HOLES PASS THROUGH FULL DEPTH OF PEDESTAL OR BALLAST CURB.
- 2. PREFORMED $1^{\prime}\!/_2{''}\varnothing$ HOLES FOR ANCHOR BOLTS SHALL BE FORMED WITH PERMANENT CORRUGATED METAL DUCTS MADE FROM 26 GAUGE STRIP STEEL CONFORMING TO ASTM A 65M, G90 COATING WEIGHT, OR APPROVED EQUAL.
- 3. SEE SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (4 OF 4) FOR ANCHOR BOLT
- 4. CONTRACTOR MAY ADJUST REINFORCEMENT SLIGHTLY AS NECESSARY TO AVOID BLOCKOUTS AND HANDRAIL BASE PLATE/ANCHOR BOLTS.
- 5. ANCHOR RODS FOR BASE PLATE SHALL BE CAST-IN-PLACE WITH THE BALLAST CURB. FOR BASE PLATE AND ANCHOR ROD DETAILS, SEE SHEET TITLED "HANDRAIL DETAILS".
- 6. FOR HANDRAIL DETAILS, SEE SHEETS TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)" AND "HANDRAIL DETAILS".
- 7. CHAMFER EDGES $\frac{3}{4}$ " X $\frac{3}{4}$ ".
- 8. 3"COVER U.N.O.
- 9. FOR ADDITIONAL NOTES, SEE SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)".



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PROJECT NO. U-3308 DURHAM COUNTY

STATION: 24+55.20 -LALT-13+22.18 -CSXN-SHEET 8 OF 16

STATE OF NORTH CAROLINA

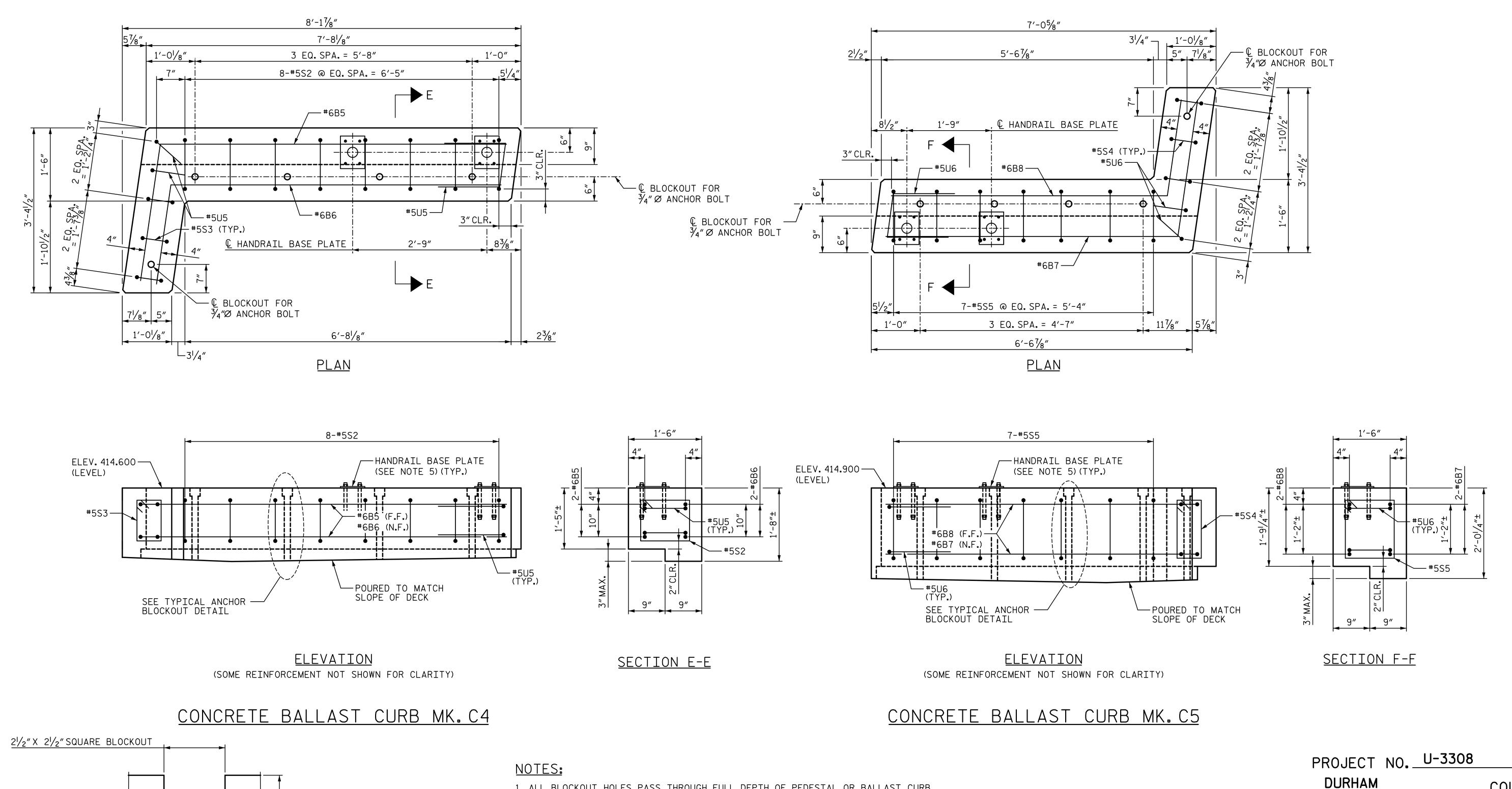
DEPARTMENT OF TRANSPORTATION

PEDESTAL AND BALLAST RETAINER (2 OF 4)

REVISIONS SHEET NO. S4-60 STV/Ralph Whitehead Associates, Inc. DATE: DATE: BY: NO. BY: TOTAL SHEETS

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MTC DRAWN BY : _ DATE : 06-14 CHECKED BY : DJM



- 1. ALL BLOCKOUT HOLES PASS THROUGH FULL DEPTH OF PEDESTAL OR BALLAST CURB.
- 3. SEE SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (4 OF 4) FOR ANCHOR BOLT
- 4. CONTRACTOR MAY ADJUST REINFORCEMENT SLIGHTLY AS NECESSARY TO AVOID BLOCKOUTS AND HANDRAIL BASE PLATE/ANCHOR BOLTS.
- 5. ANCHOR RODS FOR BASE PLATE SHALL BE CAST-IN-PLACE WITH THE BALLAST CURB. FOR BASE PLATE AND ANCHOR RODS DETAILS, SEE SHEET TITLED "HANDRAIL DETAILS".
- 6. FOR HANDRAIL DETAILS, SEE SHEETS TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)" AND "HANDRAIL DETAILS".
- 7. CHAMFER EDGES $\frac{3}{4}$ " X $\frac{3}{4}$ ".
- 8. 3" COVER U.N.O.
- 9. FOR ADDITIONAL NOTES, SEE SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)".

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COUNTY

STATION: 24+55.20 -LALT-13+22.18 -CSXN-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PEDESTAL AND BALLAST RETAINER (3 OF 4)

REVISIONS SHEET NO. S4-61 DATE: DATE: NO. BY: BY: TOTAL SHEETS

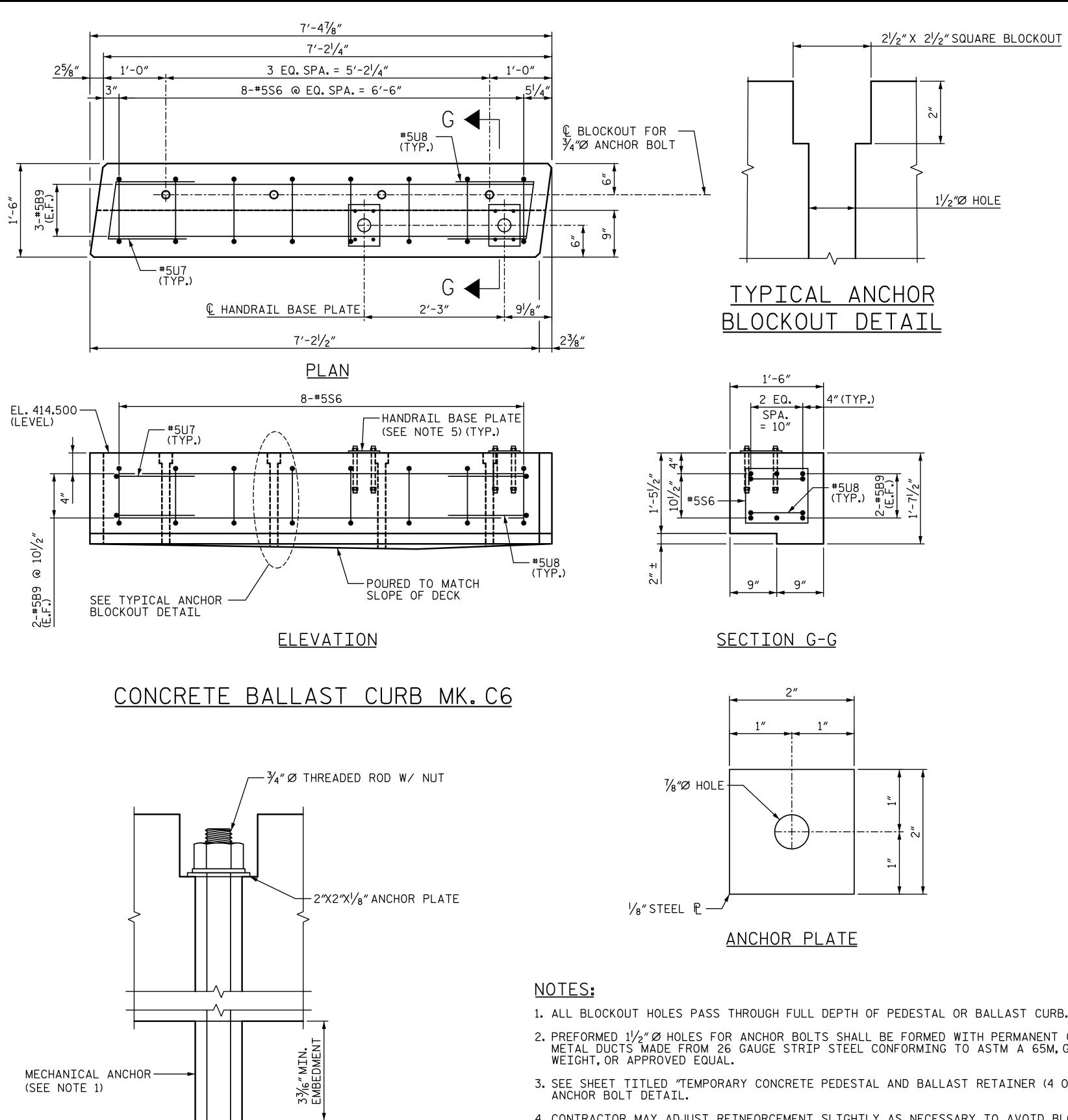
DATE : 06-14 MTC DRAWN BY : _ DATE : 06-14 CHECKED BY : DJM

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TYPICAL ANCHOR BLOCKOUT DETAIL

 $1\frac{1}{2}$ "Ø HOLE

+



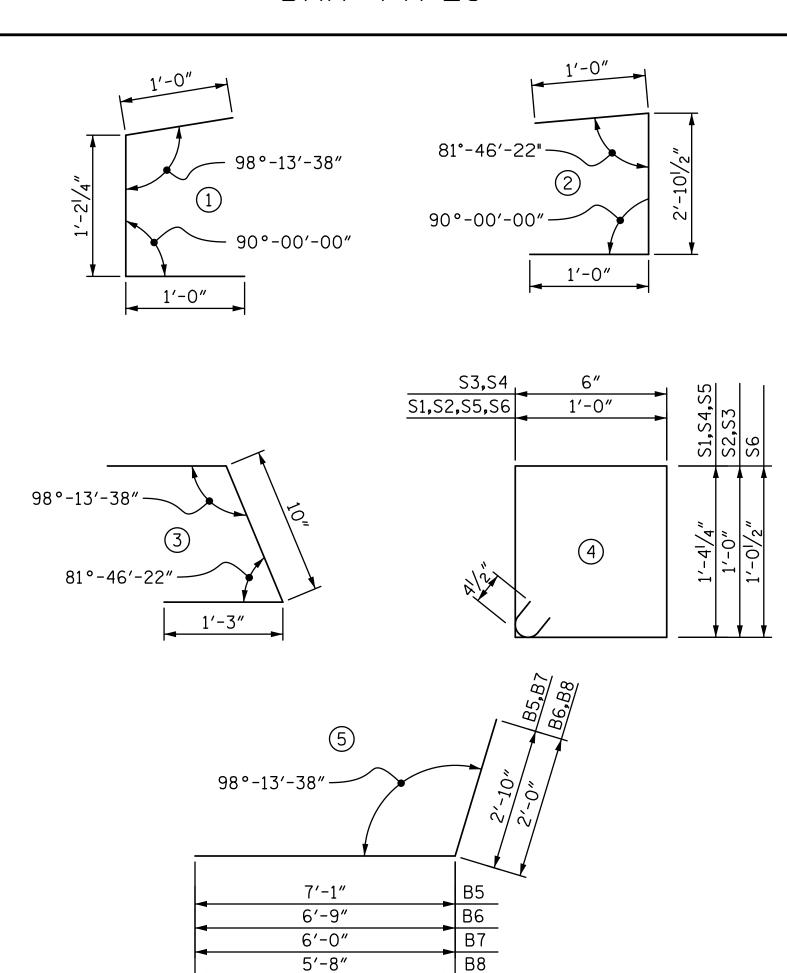
REINFORCING BAR SCHEDULE

TEM	PORAR	Y BAL	LAST	RETAI	:NER
5		0.7.7.5	T.V.D.E	LENGTH	WEIGHT
MARK	NO.	SIZE	TYPE	(ft. in.)	(ID.)
A1	2	6	STR.	1'-4"	4
A2	2	6	STR.	1′-5″	4
АЗ	2	6	STR.	1'-7"	5
Α4	2	6	STR.	1'-9"	5
A5	2	6	STR.	1'-11"	6
А6	2	6	STR.	2'-0"	6
Α7	2	6	STR.	2'-2"	7
A8	2	6	STR.	2'-3"	7
Α9	2	6	STR.	2′-6″	7
A10	2	6	STR.	2'-7"	8
A11	2	6	STR.	2'-9"	8
B1	4	6	STR.	11'-8"	71
B2	2	6	STR.	5′-11″	18
В3	2	6	STR.	11'-6"	35
B4	8	5	STR.	5′-7″	47
B5	2	6	5	9'-11"	30
В6	2	6	5	8′-9″	26
В7	2	6	5	8'-10"	27
B8	2	6	5	7′-8″	23
В9	6	5	STR.	6′-8″	42
S1	7	5	4	5′-6″	40
S2	8	5	4	4'-9"	40
S3	3	5	4	3′-9″	12
S4	3	5	4	4′-6″	14
S5	7	5	4	5′-6″	40
S6	8	5	4	4′-10″	40
U1	2	5	1	3′-2″	7
U2	2	5	2	4'-11"	10
U3	3	5	3	3′-4″	10
U4	3	5	3	3'-4"	10
U5	2	5	3	3′-4″	7
U6	2	5	3	3′-4″	7
U7	2	5	3	3′-4″	7
U8	2	5	3	3'-4"	7

	BILL OF MA	TERIAL
COMPONENT MK.	CIP CONCRETE	REINFORCING STEEL
	(C.Y.)	(LBS.)
CI	0.9	104
C2	0.9	104
С3	0.7	107

	(C.Y.)	(LBS.)
CI	0.9	104
C2	0.9	104
C3	0.7	107
C4	0.8	115
C5	0.8	111
C6	0.7	96
TOTAL	4.8	637

BAR TYPES



TOTAL QUANTITIES				
ITEMS	UNIT	QUANTITY		
CIP CONCRETE (5,000 PSI)	C.Y.	4.8		
REINFORCING STEEL (60,000 PSI)	LBS.	637		
1/8"STEEL PLATE	EA.	29		
¾"∅ THREADED ROD AND	Γ,	29		
MECHANICAL ANCHOR ASSEMBLY	EA.			
HANDRAIL	LIN. FT.	8.3		
HANDRAIL BASE PLATE ASSEMBLY	EA.	8		
1/2"ELASTOMERIC PAD	SQ.FT.	60.8		

ALL BAR DIMENSIONS ARE OUT TO OUT.

| SQ. FT. | 60.8 PROJECT NO. U-3308 DURHAM COUNTY

STATION: 24+55.20 -LALT-

SHEET 10 OF 16 13+22.18 -CSXN-

DEPARTMENT OF TRANSPORTATION

PEDESTAL AND BALLAST RETAINER (4 OF 4)

SHEET NO. S4-62

TOTAL SHEETS

DATE:

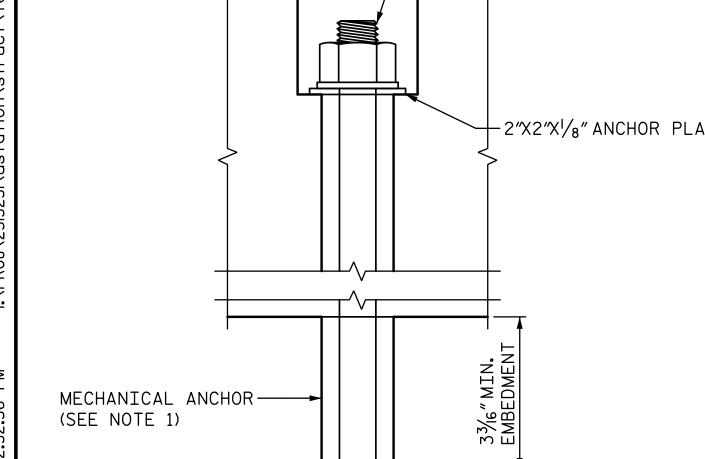
STATE OF NORTH CAROLINA

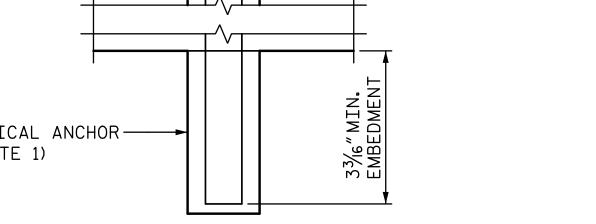
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ANCHOR BOLT DETAIL

- 2. PREFORMED $1^{l}\!/_{2}{''}\varnothing$ HOLES FOR ANCHOR BOLTS SHALL BE FORMED WITH PERMANENT CORRUGATED METAL DUCTS MADE FROM 26 GAUGE STRIP STEEL CONFORMING TO ASTM A 65M, G90 COATING WEIGHT, OR APPROVED EQUAL.
- 3. SEE SHEET TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (4 OF 4) FOR
- 4. CONTRACTOR MAY ADJUST REINFORCEMENT SLIGHTLY AS NECESSARY TO AVOID BLOCKOUTS AND HANDRAIL BASE PLATE/ANCHOR BOLTS.
- 5. ANCHOR RODS FOR BASE PLATE SHALL BE CAST-IN-PLACE WITH THE BALLAST CURB. FOR BASE PLATE AND ANCHOR RODS DETAILS, SEE SHEET TITLED "HANDRAIL DETAILS".
- 6. FOR HANDRAIL DETAILS, SEE SHEETS TITLED "TEMPORARY CONCRETE PEDESTAL AND BALLAST RETAINER (1 OF 4)" AND "HANDRAIL DETAILS".
- 7. CHAMFER EDGES $\frac{3}{4}$ " X $\frac{3}{4}$ ".
- 8. 3" COVER U.N.O.
- 9. FOR MECHANICAL ANCHOR, USE $\frac{3}{4}$ " Ø, RED HEAD MULTI-STEP II DROP-IN ANCHOR, OR APPROVED EQUAL. MECHANICAL ANCHORS SHALL BE INSTALLED PER MANUFACTURER RECOMMENDATIONS.

TEMPORARY SPAN STRUCTURAL STEEL NOTES

FOR ADDITIONAL NOTES, SEE SHEET TITLED "TEMPORARY SPAN GENERAL NOTES"

STRUCTURAL STEEL: ALL STRUCTURAL STEEL SHAPES, PLATES AND BARS SHALL BE ASTM A709, GRADE 50 OR 50W, UNLESS NOTED OTHERWISE. FRACTURE CRITICAL MEMBERS SHALL BE ASTM A709, GRADE 50F2 OR 50WF2 (SUPPLEMENTAL REQUIREMENT S84 AND S29 SHALL APPLY). NON-FRACTURE CRITICAL MEMBERS SHALL BE ASTM A709, GRADE 50T2 OR 50WT2 (SUPPLEMENTAL REQUIREMENTS S83 SHALL APPLY). ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND THE FOLLOWING REQUIREMENTS:

- 1. THE MATERIAL SUPPLIED SHALL BE OTHER THAN RIMMED OR CAPPED STEEL.
- 2. THE MATERIAL SUPPLIED SHALL BE SILICONE KILLED, FINE GRAIN PRACTICE.
- 3. CERTAIN ELEMENTS OF THE STRUCTURE ARE NOTED AS "FRACTURE CRITICAL MEMBERS" (FCM) AND SHALL MEET THE REQUIREMENTS FOR "FRACTURE CONTROL PLAN FOR FRACTURE CRITICAL MEMBERS" (AREMA CHAPTER 15, SECTION 1.14.) THE IMPACT REQUIREMENTS FOR FRACTURE CRITICAL MEMBERS SHALL BE AS REQUIRED FOR ZONE 2 SERVICE TEMPERATURE. TEST RESULTS SHALL BE FURNISHED TO THE ENGINEER OR AUTHORIZED REPRESENTATIVE.
- 4. ALL NON-FRACTURE CRITICAL MEMBERS OF THE STRUCTURE SHALL CONFORM TO THE SUPPLEMENTAL REQUIREMENTS FOR NON-FRACTURE CRITICAL IMPACT TEST NOTED IN AREMA CHAPTER 15, SECTION 1.2.1, TABLE 15-1-2 FOR ZONE 2 SERVICE TEMPERATURE. TEST RESULTS SHALL BE FURNISHED TO ENGINEER OR AUTHORIZED REPRESENTATIVE.

ALL STEEL MATERIAL SHALL BE STRAIGHT AND FREE FROM SHARP KINKS AND BENDS. ANY STEEL MATERIAL EXHIBITING SUCH DEFICIENCIES SHALL BE CAUSE FOR THE REJECTION OF THE MATERIAL. STRAIGHTENING OF THE MATERIAL SHALL NOT BE ACCEPTABLE.

MATERIAL AND WORKMANSHIP: EXCEPT AS MAY OTHERWISE BE SPECIFIED ON THE PROJECT PLANS OR SPECIFICATIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) MANUAL FOR RAILWAY ENGINEERING. PROJECT SHALL ALSO ADHERE TO STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (NCDOT) STRUCTURE DESIGN UNIT DESIGN MANUAL, 2007 REVISION. IN THE EVENT OF CONFLICTS THE MORE STRINGENT SHALL APPLY.

SPECIFICATIONS: AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) MANUAL FOR RAILWAY ENGINEERING, 2014 EDITION, NORFOLK SOUTHERN UNDERPASS GRADE SEPARATION DESIGN CRITERIA, STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STRUCTURE DESIGN UNIT DESIGN MANUAL, STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES.

ALL W-SHAPE BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER OF THE BEAM

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED.

MILL TEST REPORTS: NORFOLK SOUTHERN RAILWAY COMPANY SHALL BE FURNISHED COPIES OF MILL TEST REPORTS FOR ALL MATERIALS EXCEPT MISCELLANEOUS PLATES AND SHAPES. REPORTS SHALL INDICATE COMPLIANCE WITH ALL SPECIFIED REQUIREMENTS.

INSPECTION: SHOP INSPECTION BY NORFOLK SOUTHERN RAILWAY COMPANY OR ITS AUTHORIZED AGENT, SEE SPECIAL PROVISIONS FOR ADDITIONAL WELDING INSPECTION OF FLANGE PLATE TO WEB PLATES WELDS.

SHOP AND FIELD PAINT: ALL NEW STRUCTURAL STEEL MEMBERS, EXCEPT FAYING SURFACES, SHALL BE PAINTED IN ACCORDANCE WITH THE CURRENT NORFOLK SOUTHERN CORPORATION SPECIFICATIONS. THE SYSTEM TO BE USED SHALL BE AN INORGANIC ZINC-ACRYLIC SYSTEM. TOP COAT SHALL BE APPLIED IN THE FIELD.

SHOP DRAWINGS: SHOP DRAWINGS SHALL BE APPROVED BY THE CHIEF ENGINEER BRIDGES & STRUCTURES, NORFOLK SOUTHERN CORPORATION, ATLANTA, GEORGIA. MATERIAL SHALL NOT BE FABRICATED UNTIL DRAWINGS HAVE BEEN APPROVED. COPIES OF APPROVED SHOP DRAWINGS ARE TO BE FURNISHED TO NCDOT. SHOP DRAWINGS SHALL BE LABELED "NORFOLK SOUTHERN M.P. H-56.10".

HOLES: OPEN HOLES AS NOTED.

ANCHOR BOLTS SHALL BE GROUTED IN FORMED HOLES AFTER GIRDERS ARE ERECTED.

BEARING PADS SHALL BE USED WHENEVER STEEL MASONRY PLATE, OR OTHER STEEL BEARING PLATE, BEARS ON CONCRETE. PADS SHALL BE PREFORMED FABRIC BEARING PADS, 1/2" THICK. PREFORMED BEARING PADS SHALL BE SHOCK PAD STYLE 15175, AS MANUFACTURED BY THE ALERT MANUFACTURING AND SUPPLY COMPANY, CHICAGO, IL; OR FABREEKA PADS, AS MANUFACTURED BY THE FABREEKA PRODUCTS COMPANY, BOSTON, MA; OR SORBTEX PADS, AS MANUFACTURED BY VOSS ENGINEERING, INC., CHICAGO, IL; OR AN APPROVED EQUAL.

FOR PROTECTION OF PAINTED STEEL, SEE STANDARD SPECIFICATIONS FOR PAINTING STEEL STRUCTURES.

FOR STRUCTURAL STEEL, SEE SPECIAL PROVISION "NORFOLK SOUTHERN SPECIFICATIONS FOR STRUCTURAL STEEL".

WELDING: WELDING SHALL BE IN ACCORDANCE WITH AASHTO/ AWS-D1.5M:2012 AND AMERICAN NATIONAL STANDARD, INCLUDING INTERIMS, AS MODIFIED OR SUPPLEMENTED BY THE AREMA MANUAL FOR RAILWAY ENGINEERING.

ALL WELDS SHALL BE MADE WITH E7018 ELECTRODES. WELDING SHALL BE PERFORMED WITH THE SUBMERGED ARC WELDING (SAW) OR SHIELDED METAL ARC WELDING (SMAW) PROCESS. FRACTURE-CRITICAL MEMBER FLANGE TO WEB WELDS SHALL BE MADE BY THE SUBMERGED ARC WELDING (SAW) PROCESS.

ALL WELDS ARE TO BE SHOP WELDS, UNLESS NOTED OTHERWISE. WELDING PROCEDURE AND SIZES SHALL BE AS SHOWN IN THE PROJECT PLANS.

THERE SHALL BE THOROUGH FUSION BETWEEN WELD METAL AND BASE METAL AND BETWEEN SUCCESSIVE PASSES OF THE WELD. ALL CRATERS SHALL BE FILLED TO THE FULL CROSS SECTION OF THE WELD.

PRIOR TO WELDING, EACH WELDER SHALL HAVE BEEN CERTIFIED IN ACCORDANCE WITH AWS REQUIREMENTS DURING A PERIOD OF ONE (1) YEAR PRIOR TO WORK ON THE BRIDGE. THE FABRICATOR SHALL FURNISH THE ENGINEER OR AUTHORIZED REPRESENTATIVE WITH AN AWS CERTIFICATE FOR EACH WELDER, COVERING THEIR ABILITY TO MAKE A COMPLETE AND SATISFACTORY WELD OF EACH KIND TO BE USED ON THE PROJECT.

SURFACES AND EDGES TO BE WELDED SHALL BE SMOOTH, UNIFORM AND FREE FROM FINS, TEARS CRACKS, OR OTHER DEFICIENCIES WHICH WOULD ADVERSELY AFFECT THE QUALITY OR STRENGTH OF THE WELD. SURFACES TO BE WELDED AND SURFACES ADJACENT TO A WELD SHALL ALSO BE FREE OF ANY SCALE, SLAG, RUST, MOISTURE, GREASE OR OTHER FOREIGN MATERIAL THAT WILL INHIBIT PROPER WELDING.

NON-DESTRUCTIVE TESTING OF THE FRACTURE CRITICAL MEMBERS IS TO BE PERFORMED BY AN INDEPENDENT TESTING COMPANY APPROVED BY THE ENGINEER AND CONTRACTED BY THE FABRICATOR. PERSONAL QUALIFICATIONS AND CERTIFICATION ARE TO BE IN ACCORDANCE WITH THE CURRENT AREMA MANUAL CHAPTER 15 FOR FRACTURE CRITICAL MEMBERS. COPIES OF THE TEST ARE TO BE FURNISHED TO THE ENGINEER OR AUTHORIZED REPRESENTATIVE FOR INCLUSION IN THEIR PROJECT

THERE SHALL BE NO FIELD WELDING ON THIS PROJECT OTHER THAN AS DIRECTLY DETAILED IN THESE PLANS, UNLESS APPROVED BY THE ENGINEER OR AUTHORIZED REPRESENTATIVE IN WRITING.

BOLTS: ALL BOLTED CONNECTIONS SHALL BE MADE WITH 7/8"DIA. ASTM A325, TYPE 3 BOLTS UNLESS NOTED OTHERWISE. NUTS AND WASHER SHALL BE A563, GRADE C3, AND F436, TYPE 3 RESPECTIVELY. ALL BOLTS, NUTS, AND WASHERS SHALL BE MECHANICALLY GALVANIZED UNLESS NOTED OTHERWISE. OPEN HOLES SHALL BE "5/16" DIA. UNLESS NOTED OTHERWISE. ALL BOLTS, NUTS, AND WASHERS WILL BE SUPPLIED FROM A SINGLE SOURCE WITH DOCUMENTATION OF THEIR SOURCE AND QUALITY CERTIFICATION. ALL HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-NUT METHOD" IN ACCORDANCE WITH AREMA MANUAL CHAPTER 15, SECTION 3.2.3 - INSTALLATION OF HIGH STRENGTH BOLTS. ANY BOLTS THAT REQUIRE REMOVAL AFTER BEING TIGHTENED SHALL BE DISCARDED AND A NEW BOLT INSTALLED, UNLESS OTHERWISE NOTED.

ALL BOLT HOLES SHALL BE SUB-DRILLED AND REAMED OR DRILLED FROM THE SOLID. AT NO TIME ARE HOLES TO BE SUB-PUNCHED AND REAMED OR PUNCHED FULL SIZE.

ALL HOLES SHALL BE $^{\prime}_{16}$ LARGER THAN THE SPECIFIED BOLT SIZE UNLESS OTHERWISE NOTED IN THE PROJECT PLANS.

BOLTS SHALL BE INSTALLED WITH THE BOLT HEADS EXPOSED TO THE WEATHER. THE SPECIFIED WASHERS SHALL BE INSTALLED BENEATH THE TURNING ELEMENT. VERTICALLY POSITIONED BOLTS WHICH HAVE BOTH THE HEAD AND NUT EXPOSED TO WEATHER SHALL HAVE THE HEAD PLACED ABOVE THE NUT. HORIZONTAL POSITIONED NUTS SHALL HAVE THE HEADS ON THE VISIBLE SIDE OF THE CONNECTION.

SWEDGED ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 55, AS DESIGNATED IN THE PROJECT PLANS. SWEDGED ANCHOR BOLTS SHALL NOT BE PAINTED. ANCHOR BOLT NUTS AND WASHER SHALL CONFORM TO ASTM A563, GRADE C3 HEAVY HEX WITH NYLON INSERT AND ASTM F436, TYPE 3 CIRCULAR WASHERS, RESPECTIVELY. ANCHOR BOLTS AND ALL ASSOCIATED HARDWARE SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153. CLASS C.

NO SALVAGED MATERIALS WILL BE ALLOWED FOR THE CONSTRUCTION OF THE TEMPORARY SPAN.

APPROX. STRUCTURAL STEEL QUANTITIES

ITEMS	UNIT	QUANTITY
TEMPORARY SPAN APPROX. STRUCTURAL STEEL	LBS.	90,200
TEMPORARY SPAN APPROX. STRUCTURAL STEEL - ALTERNATE	LBS.	77,000

PROJECT NO. U-3308

DURHAM

__ COUNTY

STATION: 24+55.20 -LALT-

13+22**.**18 -CSXN-

SHEET 11 OF 16

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

TEMPORARY SPAN STRUCTURAL STEEL NOTES

SIEEL NOIE

STV / Ralph Whitehead Associates, Inc.

900 W Trade Street, Suite 715
Charlotte, NC 28202
NC License No. F-0991

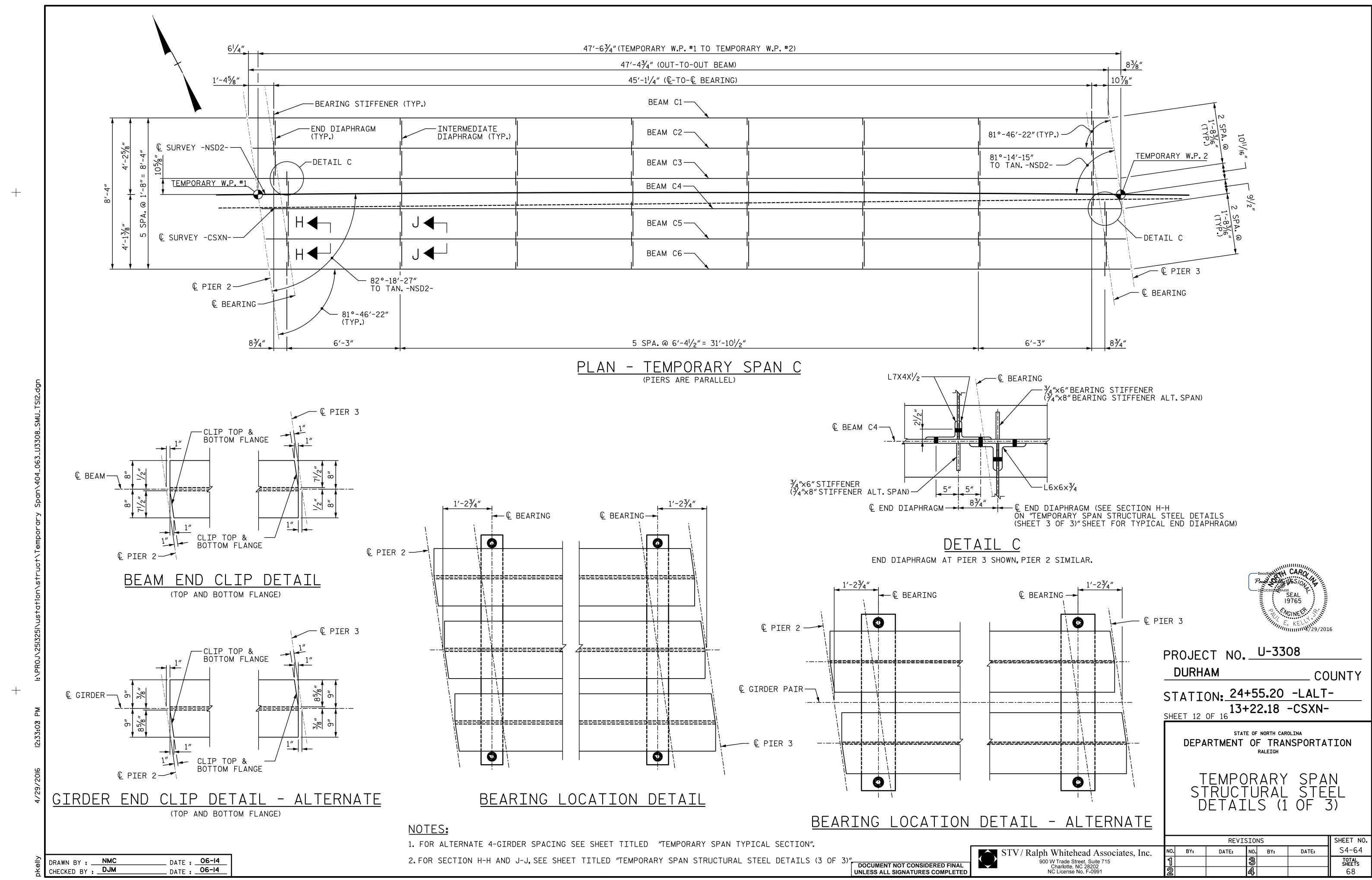
REVISIONS SHEET NO.

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DRAWN BY: NMC DATE: 06-14
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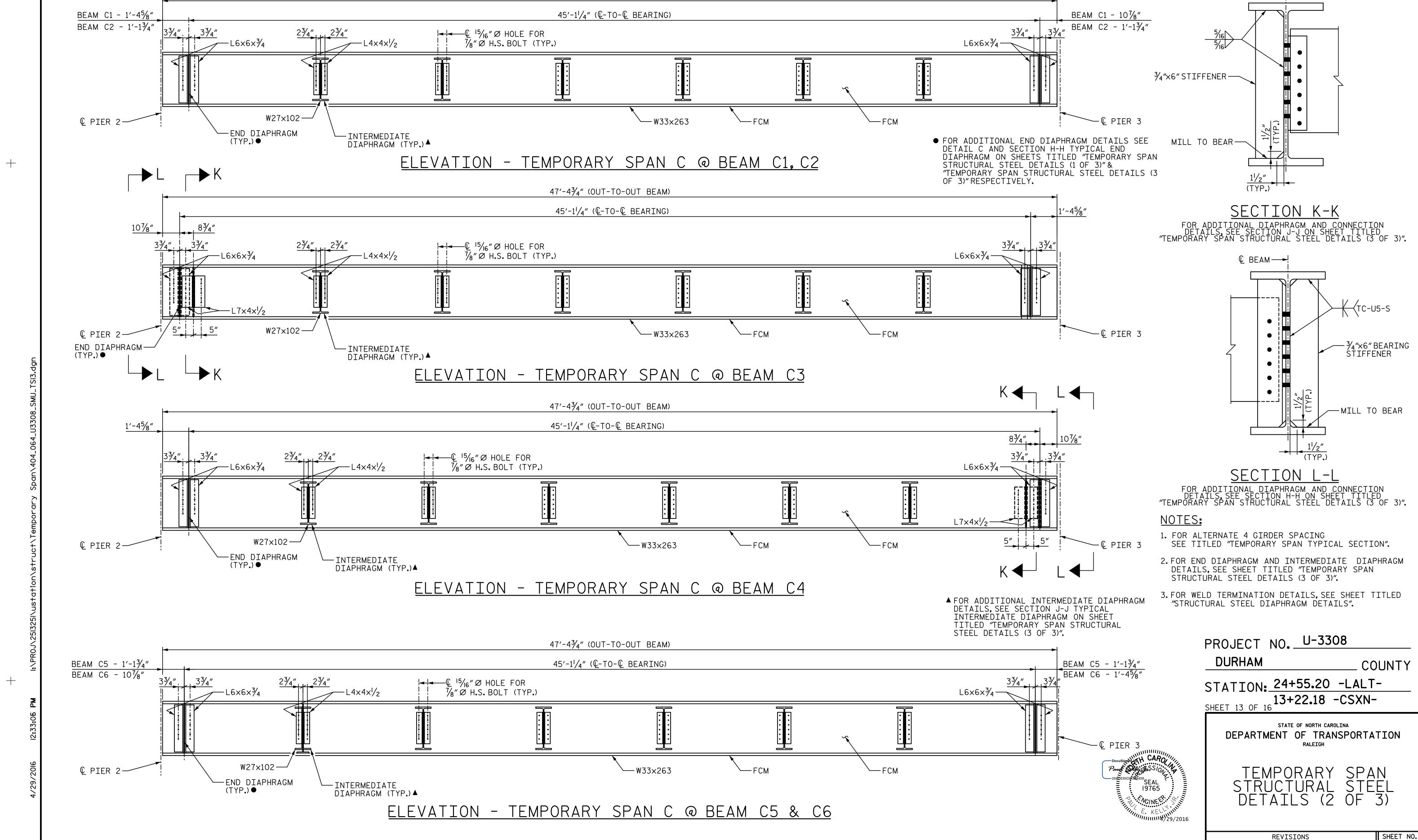
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TOTAL SHEETS

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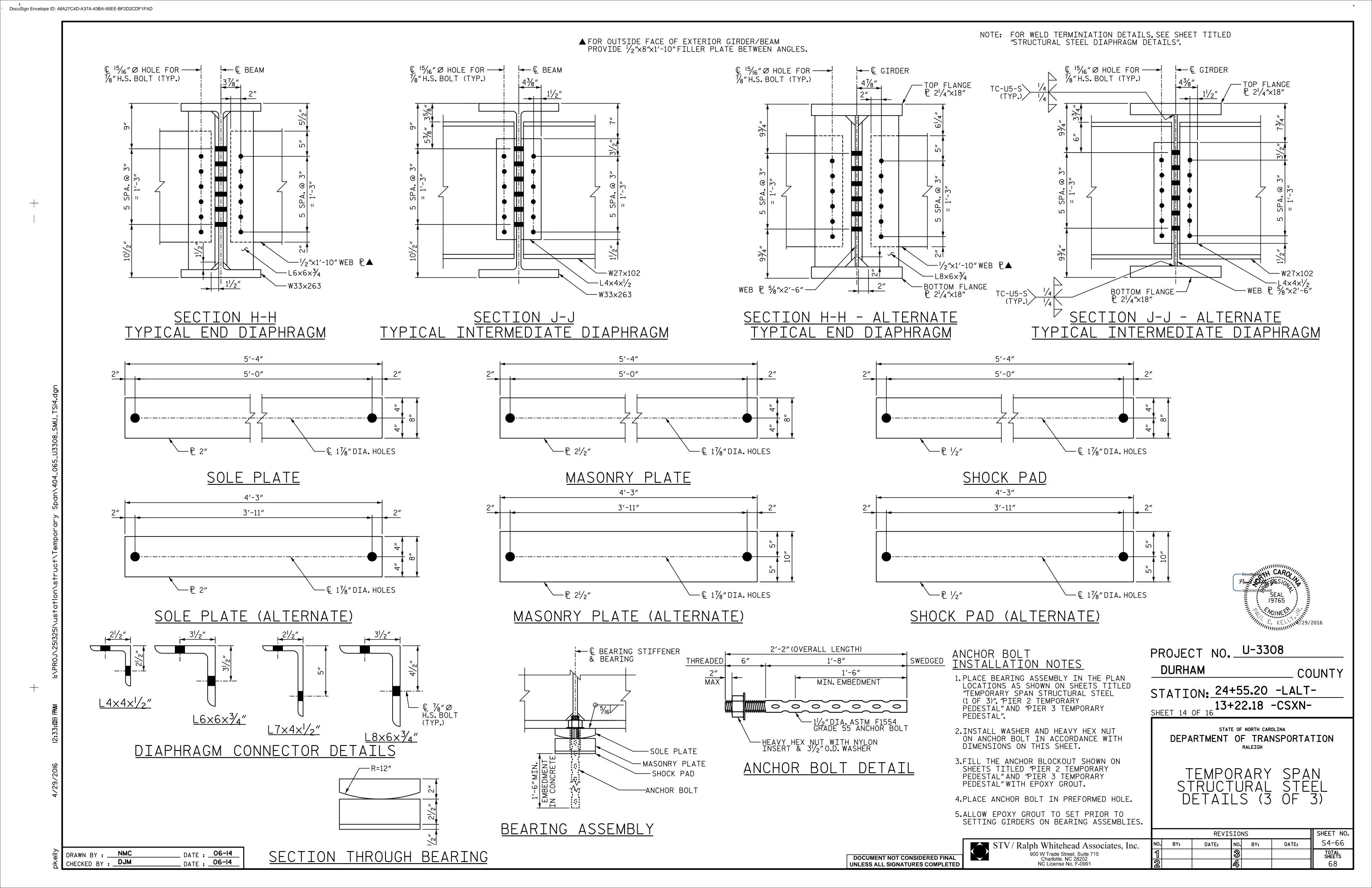
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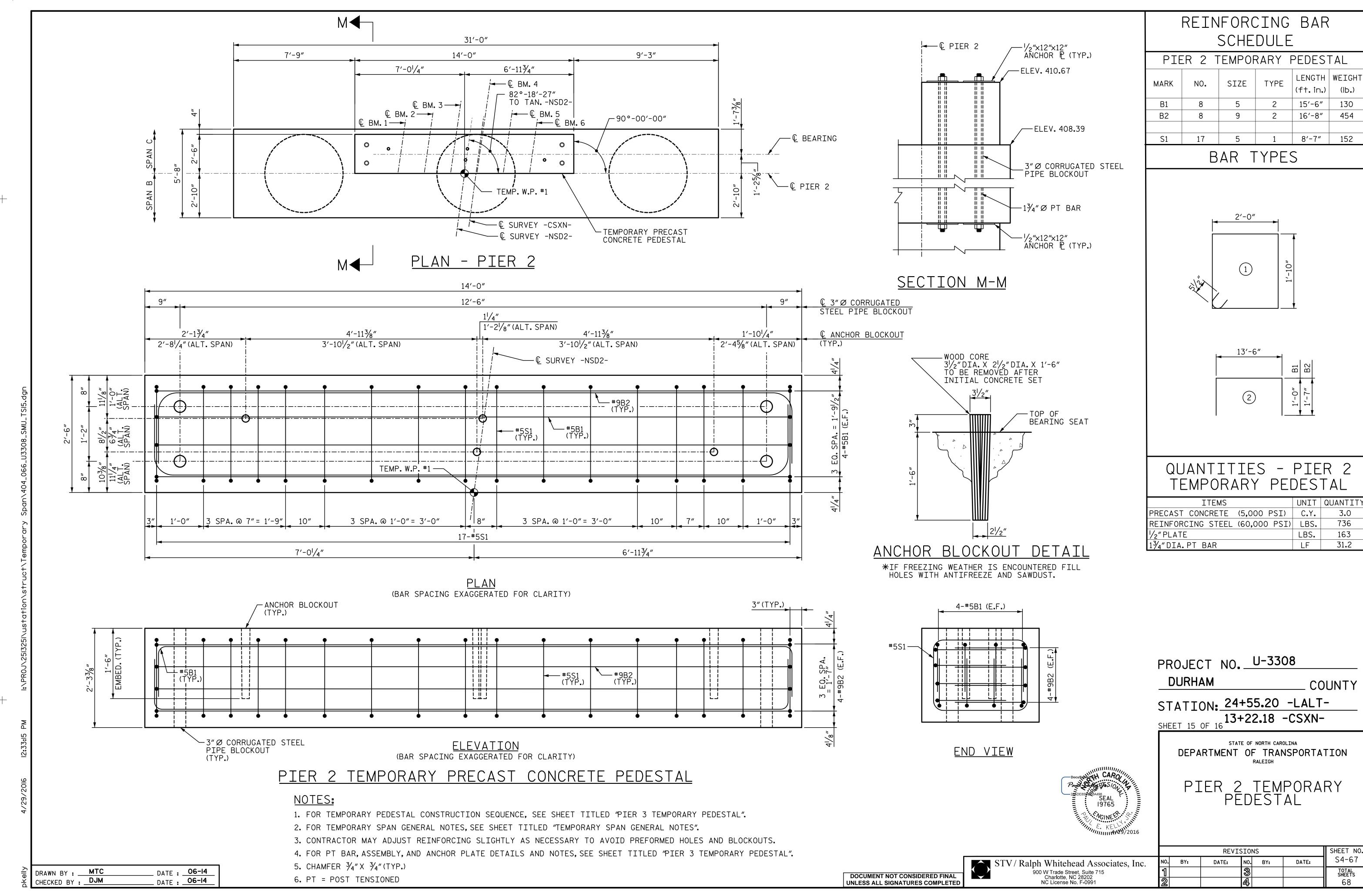
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3. INSTALL BEARINGS AND ANCHORS FOR TEMPORARY SPAN.

DATE : 06-14

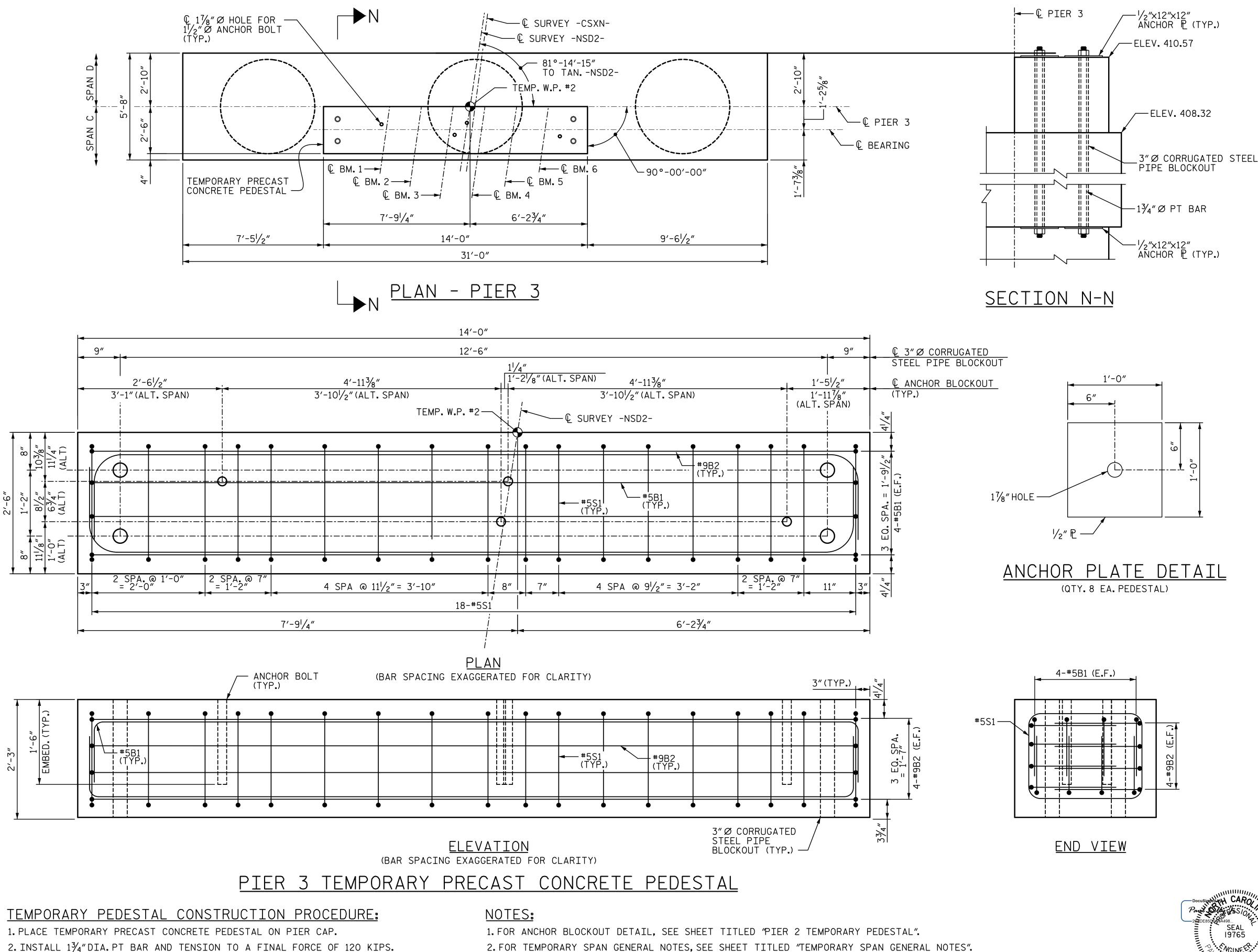
_ DATE : 06-14

MTC

DRAWN BY :

CHECKED BY : DJM

4. UPON REMOVAL OF TEMPORARY SPAN, REMOVE TEMPORARY PEDESTAL AND PT BARS. GROUT 3"DIA. CORRUGATED BLOCKOUT IN PIER CAP USING NON-SHRINK GROUT.

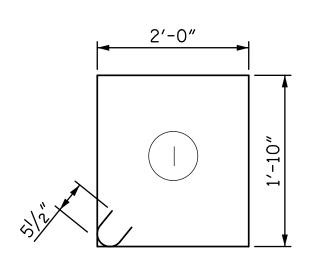


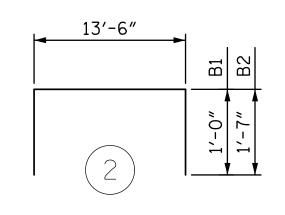
REINFORCING BAR SCHEDULE

PIER 3 TEMPORARY PEDESTAL LENGTH | WEIGHT SIZE TYPE MARK NO. (ft.in.)

15′-6″ 16'-8" 454 2 18 8'-7"

BAR TYPES





QUANTITIES - PIER 3 TEMPORARY PEDESTAL

ITEMS	UNIT	QUANTITY
PRECAST CONCRETE (5,000 PSI)	C.Y.	2.9
REINFORCING STEEL (60,000 PSI)	LBS.	745
√2"PLATE	LBS.	163
1¾"DIA.PT BAR	LF	31.2

PROJECT NO. U-3308

DURHAM

COUNTY

STATION: 24+55.20 -LALT-

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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PIER 3 TEMPORARY PEDESTAL

REVISIONS SHEET NO. S4-68 STV/Ralph Whitehead Associates, Inc. DATE: NO. BY: DATE: BY: 900 W Trade Street, Suite 715 Charlotte, NC 28202 NC License No. F-0991 TOTAL SHEETS

2. FOR TEMPORARY SPAN GENERAL NOTES, SEE SHEET TITLED "TEMPORARY SPAN GENERAL NOTES".

3. CONTRACTOR MAY ADJUST REINFORCING SLIGHTLY AS NECESSARY TO AVOID PREFORMED HOLES AND BLOCKOUTS.

4. FOR PT BAR, USE WILLIAMS FORM ENGINEERING CORP., "150-KSI ALL-THREAD-BAR" OR APPROVED EQUAL.

5. PT BAR ASSEMBLY AND PROTECTION SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

6. CHAMFER $\frac{3}{4}$ " X $\frac{3}{4}$ " (TYP.)

7. PT = POST TENSIONED

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