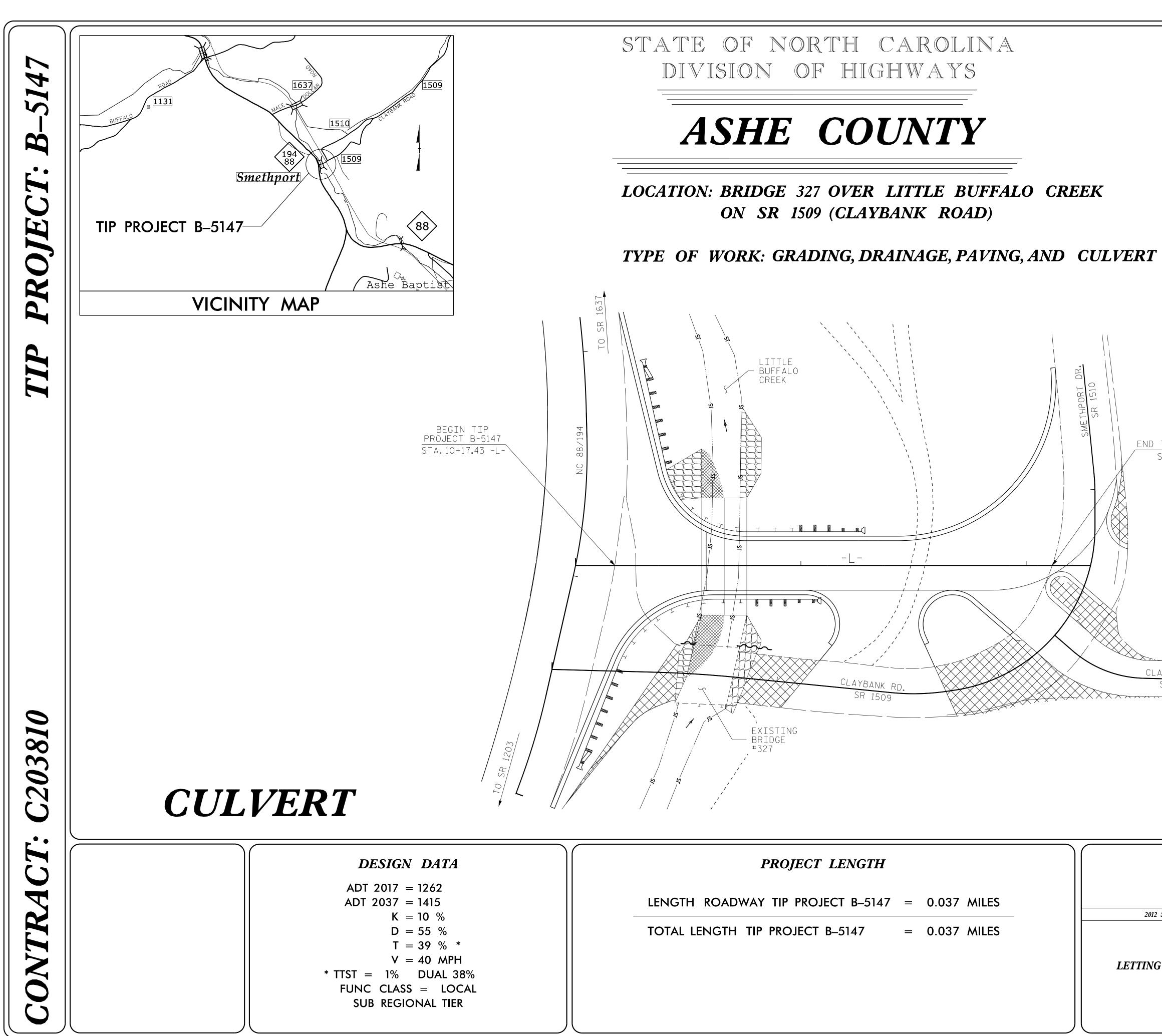
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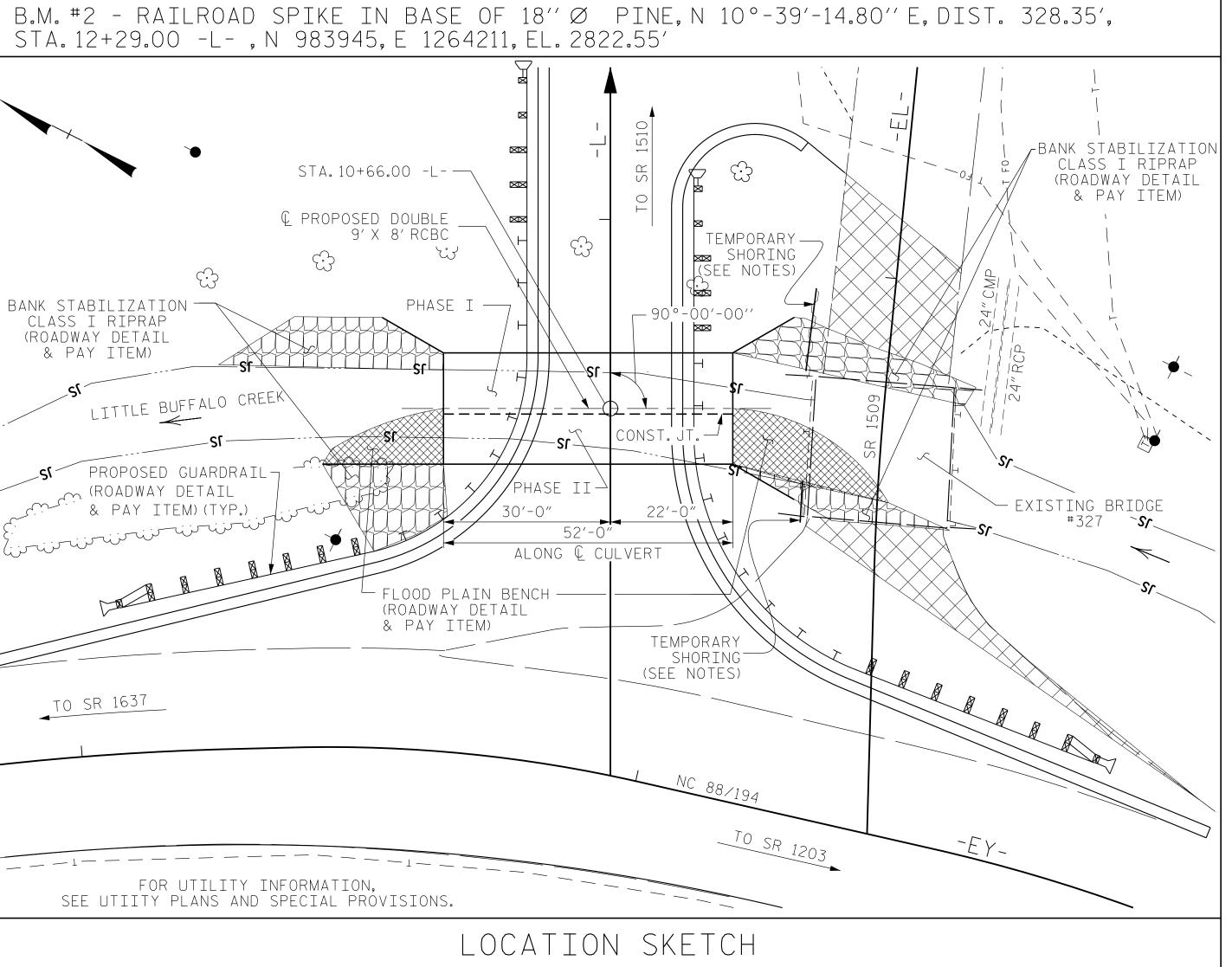
PROJECT LENGTH	Prepared in the Office of: DIVISION OF HIGHWAYS STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR.				
$ \qquad \text{LENGTH ROADWAY TIP PROJECT B}_{5147} = 0.037 \text{ MILES}$	RALEIGH, 2012 STANDARD SPECIFICATIONS	N.C. 27610			
TOTAL LENGTH TIP PROJECT B-5147 = 0.037 MILES	LETTING DATE : MAY 17, 2016	D. R. CALHOUN, P.E. PROJECT ENGINEER W. S. ARAFAT, P.E. PROJECT DESIGN ENGINEER			

1				ALIDDA	TOTAL			
STATE STATE PROJECT REPERENCE NO. NO. SHEETS								
N.C.		B-5147						
	PROJ. NO.	F. A. PROJ. NO.		DESCRIPT				
42308.1.1 42308.2.1		BRZ1509(8)		P.E. ROW/U				
		Soos						
	MA D83 CRID							
	NA DOS GRID							

END TIP PROJECT B-5147 STA.12+11.55 -L-

CLAYBANK RD.		
SR 1509	TO SR 1510	
< / · · · · · · · · · · · · · · · · · ·		

B.M. #2 - RAILROAD SPIKE IN BASE OF 18''Ø STA.12+29.00 -L- ,N 983945,E 1264211,EL.28	
STA. 10+66.00 -L- © PROPOSED DOUBLE S'X 8'RCBC ROADWAY DETAIL & PAY ITEM ST LITTLE BUFFALO CREEK (ROADWAY DETAIL & PAY ITEM (ROADWAY DETAIL & PAY ITEM (ROADWAY DETAIL & PAY ITEM (ROADWAY DETAIL & PAY ITEM (ROADWAY DETAIL & PAY ITEM TO SR 1637	CONST. JT
FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS. LOCATIO	N SKETCH <u>HYDRAUL</u> DESIGN DISCHARGE FREQUENCY OF DESIGN F DESIGN HIGH WATER ELEV DRAINAGE AREA BASE DISCHARGE (0100) BASE HIGH WATER ELEV <u>OVERTOPPING DISCHARG</u> FREQUENCY OF OVERTOPI OVERTOPPING FLOOD ELE
	GRADE POINT ELEV. @ S BED ELEV. @ STATION 1 ROADWAY SLOPES = 2:1



IC DATA

_____ = 590 C.F.S FLOOD____= 5+ YEARS LEVATION = 2818.0 FT. _____ = 3.96 SQ.MI.))_____ = 1500 C.F.S. VATION = 2820.92 FT.

NG FLOOD DATA

RGE ----- = 613 C.F.S. PPING FLOOD ----- = 5+ YEARS ELEVATION = 2818.11 FT.

'AY DATA

STATION 10+66.00 -L- = 2822.48 FT. 10+66.00 -L- = 2811.20 FT.

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

DESIGN FILL _____ 2.43 FT. (MIN.), 3.73 FT. (MAX.)

FOR OTHER DESIGN DATA AND NOTES, SEE STANDARD NOTE SHEET.

3'' Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN PHASE I CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1. PHASE I WING FOOTINGS, FLOOR SLAB AND CURTAIN WALL TO THE CONSTRUCTION JOINT INCLUDING 4" OF PHASE I VERTICAL WALLS.
- 2. THE REMAINING PORTION OF PHASE I WALLS AND PHASE I WINGS FULL HEIGHT.
- 3. PHASE I SILLS

CONCRETE IN PHASE II CULVERT TO BE POURED IN THE FOLLOWING ORDER:

- 1. PHASE II WING FOOTINGS, FLOOR SLAB AND CURTAIN WALL TO THE CONSTRUCTION JOINT INCLUDING 4" OF PHASE II VERTICAL WALLS.
- 2. THE REMAINING PORTION OF PHASE II WALLS AND PHASE II WINGS FULL HEIGHT.
- 3. PHASE II SILLS.

4. ROOF SLAB AND HEADWALLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEETS.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACES OF THE EXTERIOR WALLS AND BOTH FACES OF INTERIOR WALL ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION SEQUENCE, SEE EROSION CONTROL PLANS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

TOTAL STRUCTURE QUANT CLASS A CONCRETE PHASE I ----- 49.8 C.Y PHASE II ----- 86.2 C.Y TOTAL ----- 136.0 C.Y REINFORCING STEEL PHASE T ----- 6018 LBS PHASE II ----- 8187 LBS TOTAL ----- 14205 LBS CULVERT EXCAVATION------ LUMP SUM OUNDATION COND.MAT'L PHASE I ----- 49 TONS PHASE II -----39 TONS TOTAL -----88 TONS REMOVAL OF EXISTING STRUCTURE ------ LUMP SUN ASBESTOS ASSESSMENT------ LUMP SUN

FOR CULVERT DIVERSION DETAILS AND PAY ITEM. SEE EROSION CONTROL PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF 2 SPANS: 1 @ 12'-10", 1 @ 12'-9" TIMBER FLOOR ON 11 LINES OF 12" STEEL I-BEAMS @ 2'-5¹/4" CTS.; WITH A CLEAR ROADWAY WIDTH OF 25'-1" ON END BENTS AND INTERIOR BENT WITH TIMBER CAPS, TIMBER PILES POST AND SILLS AND LOCATED APPROXIMATELY 50' UPSTREAM FROM PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. SEE SPECIAL PROVISIONS.

FOR SALVAGE AND DELIVERY OF EXISTING 12"I-BEAMS, BEARING PLATES, TIMBER FLOORING, CRUTCH BENT, CAPS AND NEW POSTS, SEE SPECIAL PROVISIONS.

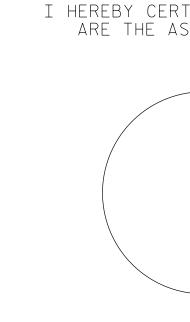
INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE."

THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES. SEE SPECIAL PROVISIONS

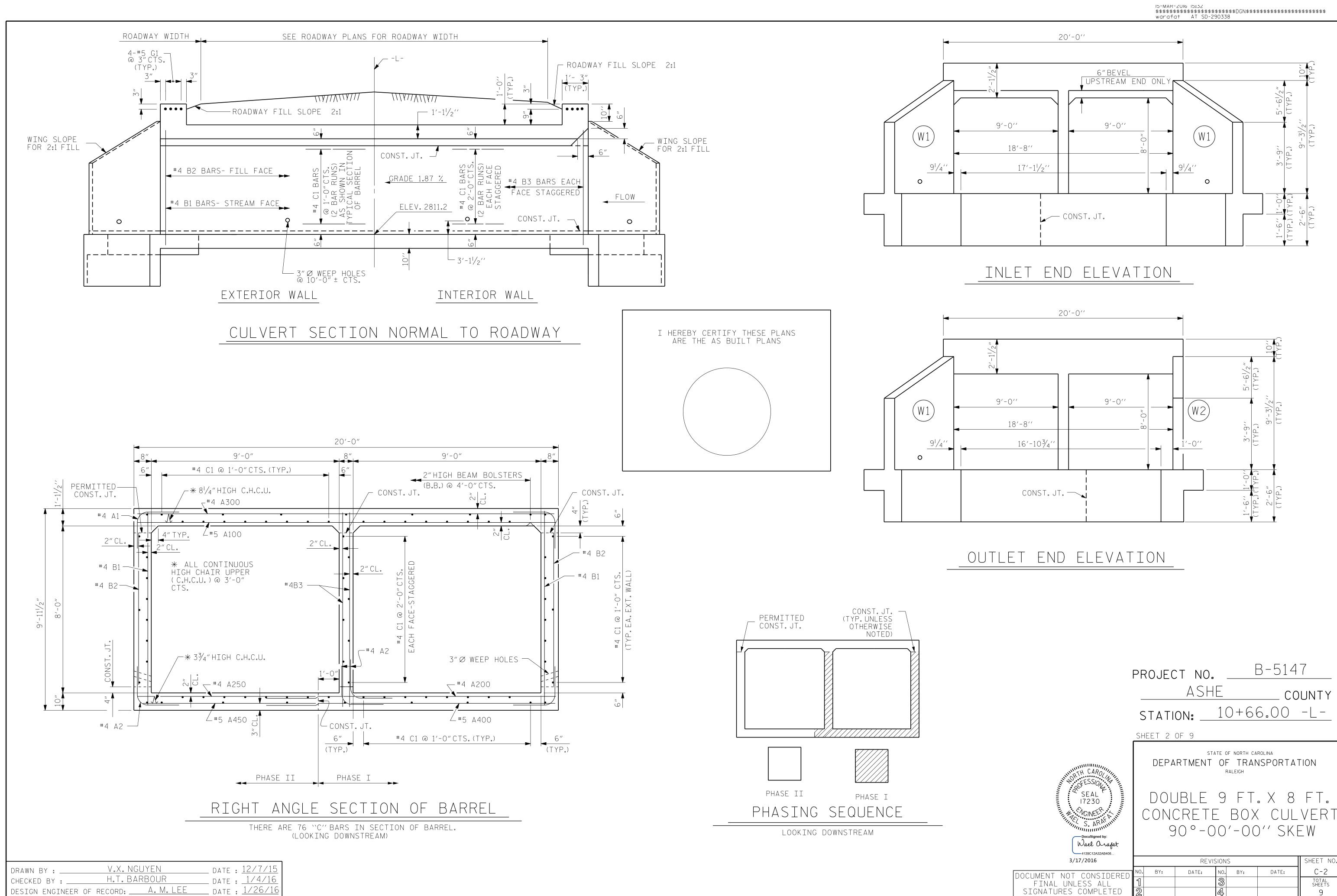




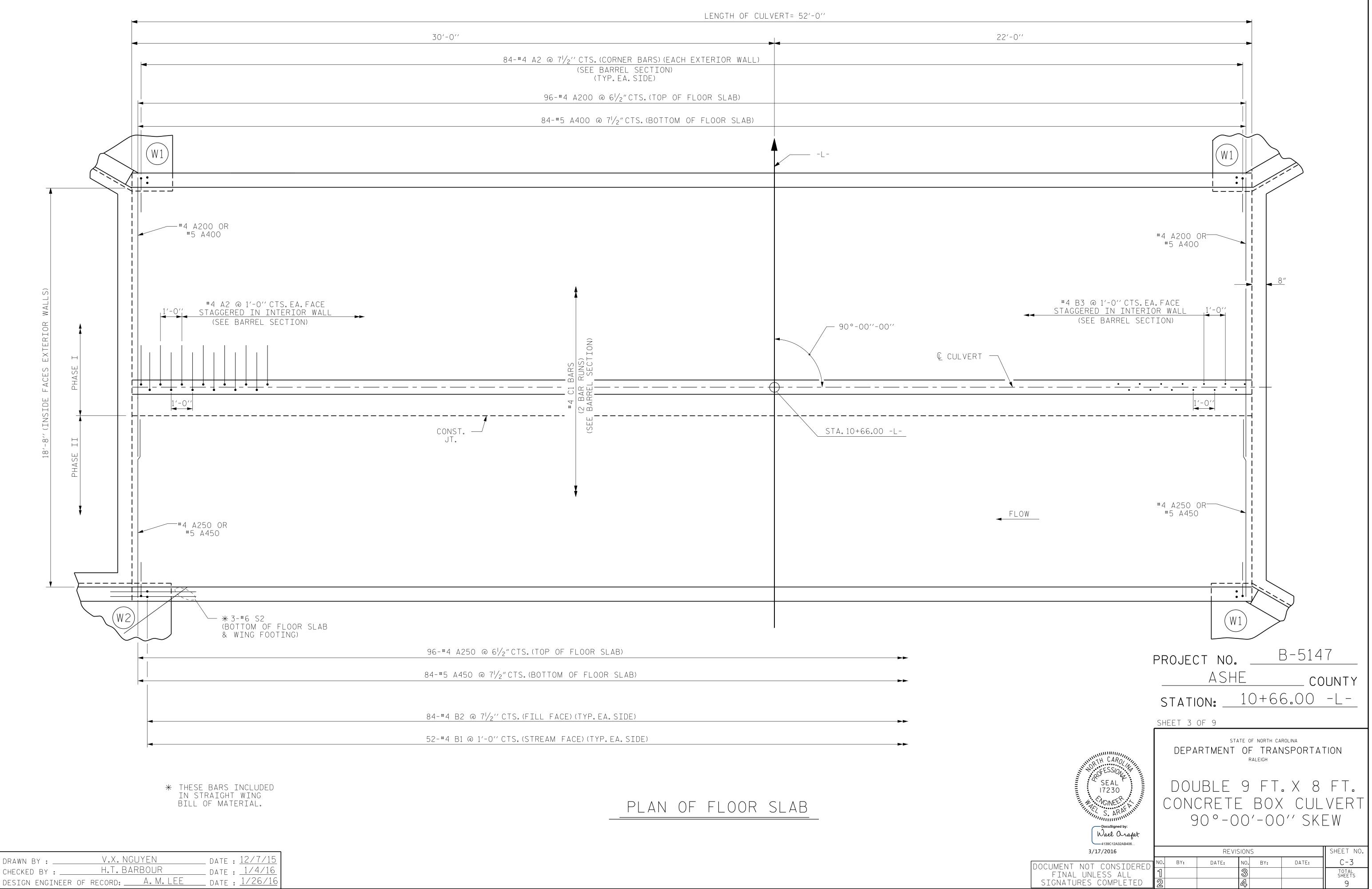
FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

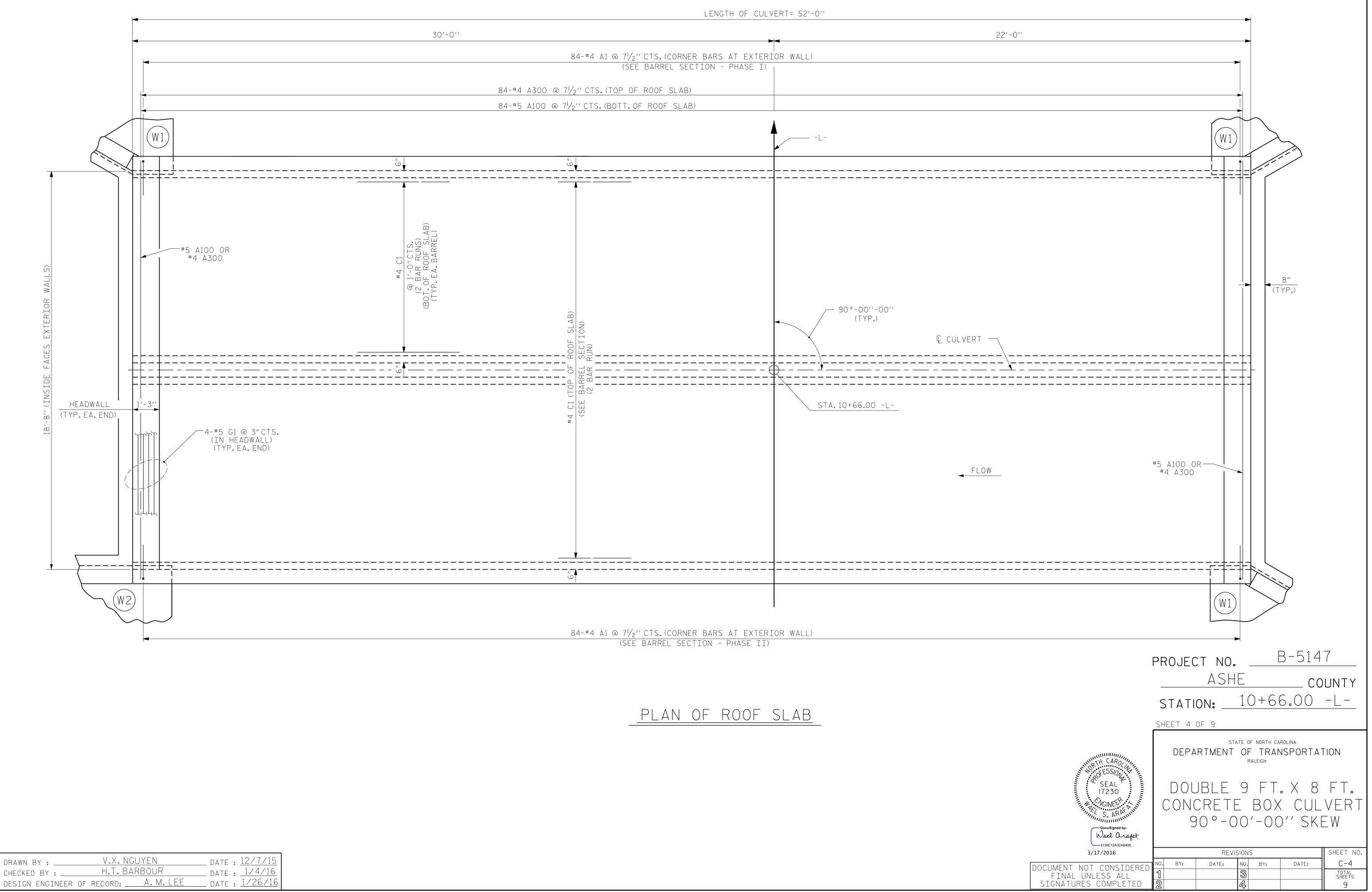
FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

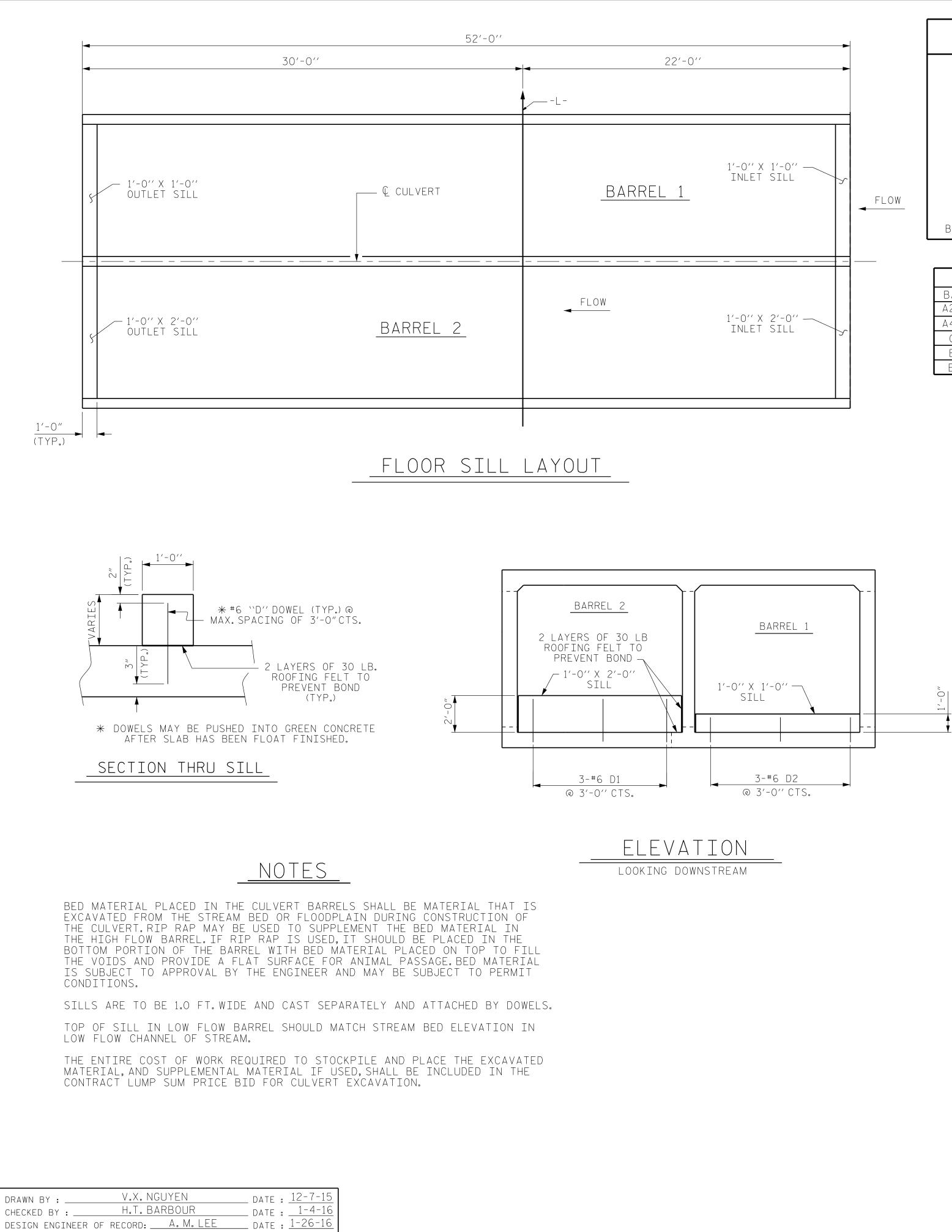
TIFY THESE PLANS 5 BUILT PLANS						
	PR	OJEC	CT NO.	[3-514	7
		0020	ASH		CO	UNTY
	ST	ΑΤΙΟ	N:	10+6		
	SH	eet 1 ()F 9	REPLA	CES BRIDG	E #327
Marine Stranger		DEPA		OF NORTH CA OF TRAN raleigh	^{rolina} NSPORTAT	ΓΙΟΝ
RTH CAROLINA SEAL 17230		DOL	JBLE	9 FT	.X 8	FT.
Deltasighed by:			CRETE	E BOX	(CUL	VERT
nown Wael Orafa	F	C)	0 - 0 0	d'' SKE	ΞW
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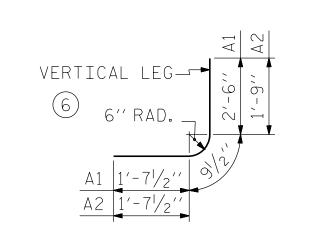






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BAR TYPE



BAR DIMENSIONS ARE OUT TO OUT

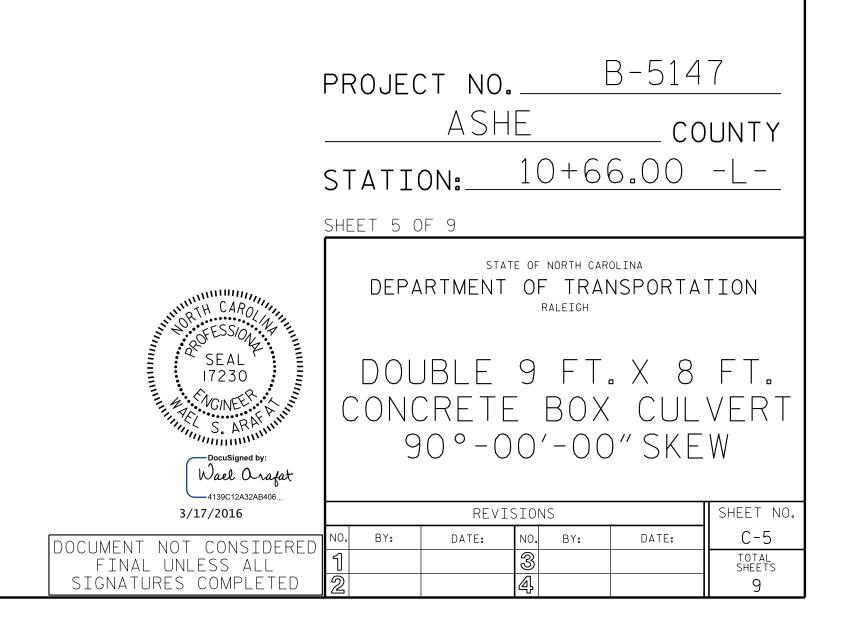
SPLICE CHART							
BAR SIZE SPLICE LENGTH							
A200	#4	1'-5"					
A400	#5	1'-9"					
C1	#4	1'-11"					
B1	#4	1'-5"					
B3	#4	1'-5"					

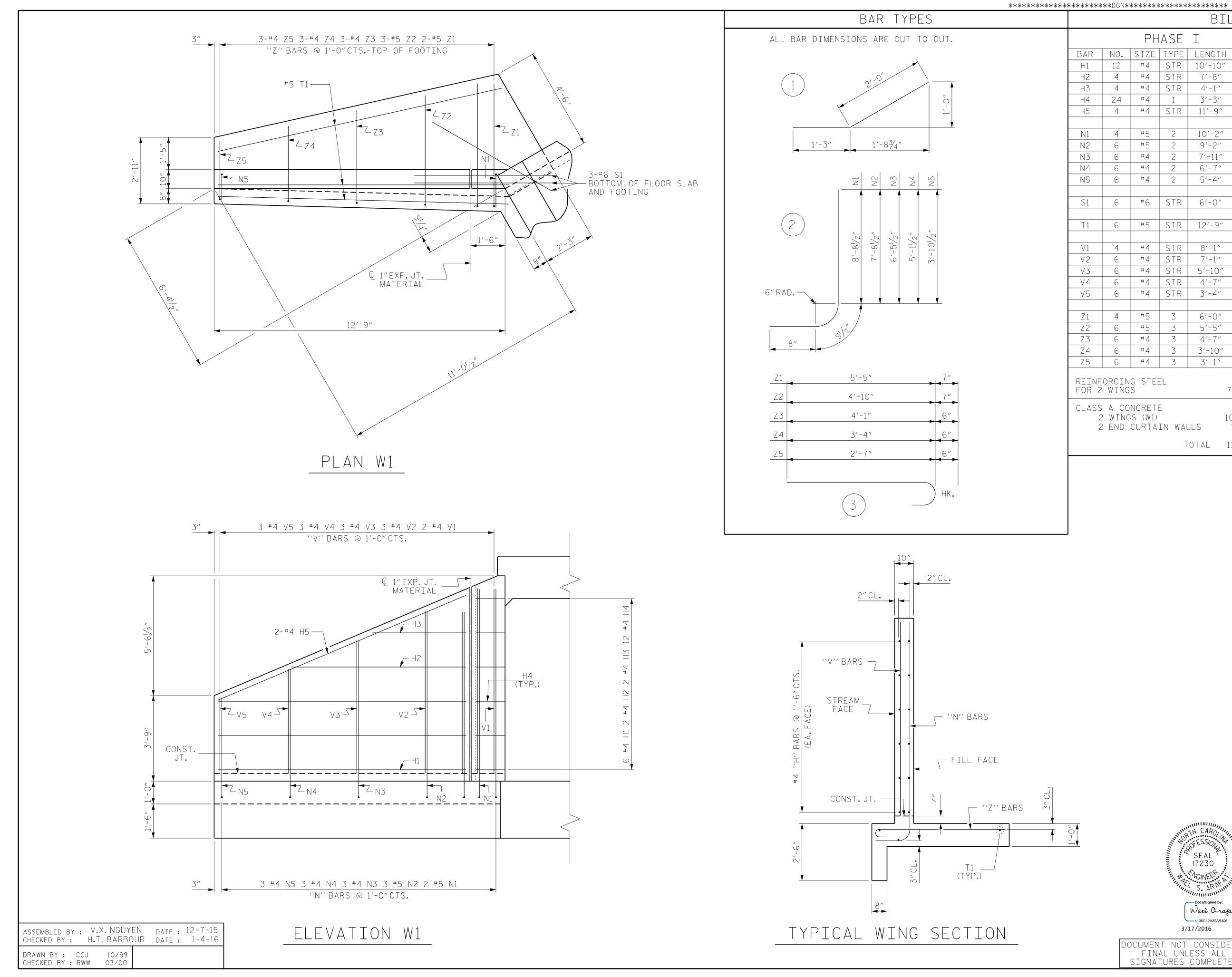
			REIN	IFORC	ING	BAR (SCHE	DULE			
	PHASE I							PHAS	EII		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	84	#4	1	4'-11"	276	A1	84	#4	1	4'-11"	276
A2	188	#4	1	4'-2"	523	A2	84	#4	1	4'-2"	234
A200	96	# 4		10/ 0//	010	A 1 O O	84	+ -	CID	10/ 0//	1707
A200	96	#4	STR.	12'-9"	818	A100	84	#5	STR.	19'-8"	1723
A400	84	#5	STR.	13'-1"	1146	A250	96	#4	STR.	8'-4"	534
B1	52	#4	STR.	9'-7"	333	A300	84	#4	STR.	19'-8"	1104
B2	84	#4	STR.	7′-4″	411						
Β3	104	#4	STR.	9'-7"	666	A450	84	#5	STR.	8'-4"	730
C1	62	# <u>4</u>	STR.	26'-10"	1111	B1	52	#4	STR.	9'-7"	333
						B2	84	#4	STR.	7′-4″	411
D2	6	#6	STR.	1'-5"	13						
	REINFOR	CING STE	EL	52	97 LBS.	C1	90	#4	STR.	26'-10"	1613
						G1	8	#5	STR.	19'-8"	164
						D1	6	#6	STR.	2'-5"	22
							REINFO	RCING ST	EEL	71	44 LBS.

PHASE I STRUCTURE QUANT	Ι
CLASS A CONCRETE BARREL WINGS ETC TOTAL	
REINFORCING STEEL BARREL WINGS ETC TOTAL	
CULVERT EXCAVATION	_
FOUNDATION COND.MAT'L.	

TIES	
_37.9 C.Y.	
11.9 C.Y.	
_49.8 C.Y.	
5297 LBS.	
721 LBS.	
6018 LBS.	
LUMP SUM	
49 TONS	

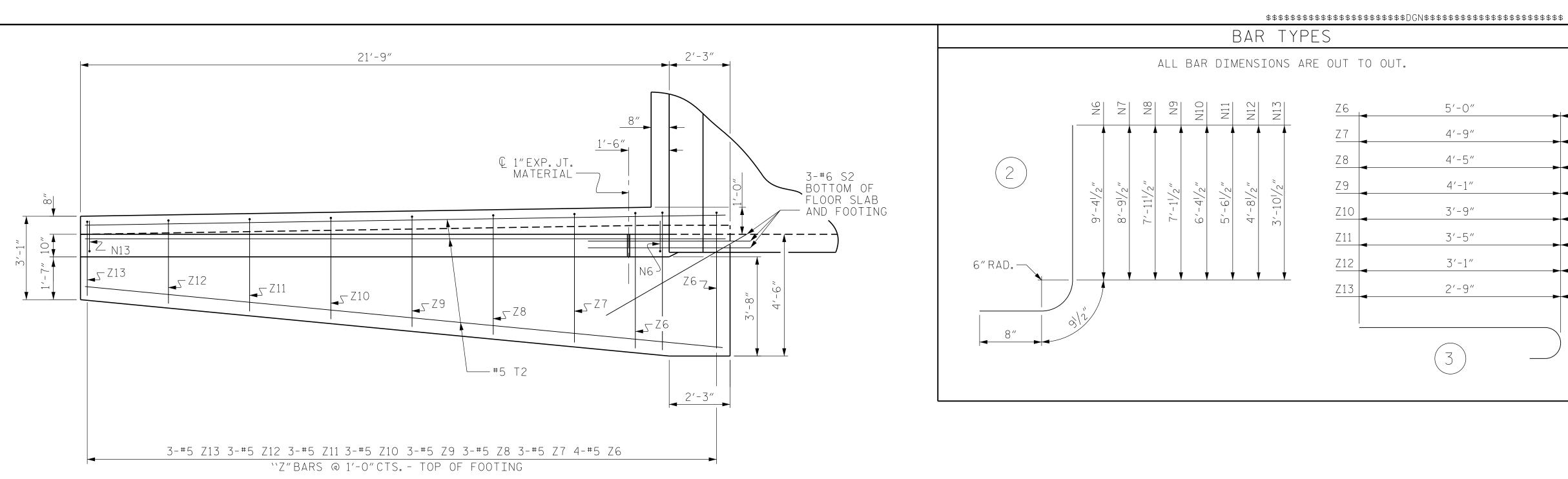
PHASE II STRUCTURE QUAN	TITIES
CLASS A CONCRETE	
BARREL	68.8 C.Y.
WINGS ETC	17.4 C.Y.
TOTAL	86.2 C.Y.
REINFORCING STEEL	
REINFORCING STEEL BARREL	7144 LBS.
	7144 LBS. 1043 LBS.
BARREL	
BARRELWINGS ETC	1043 LBS.





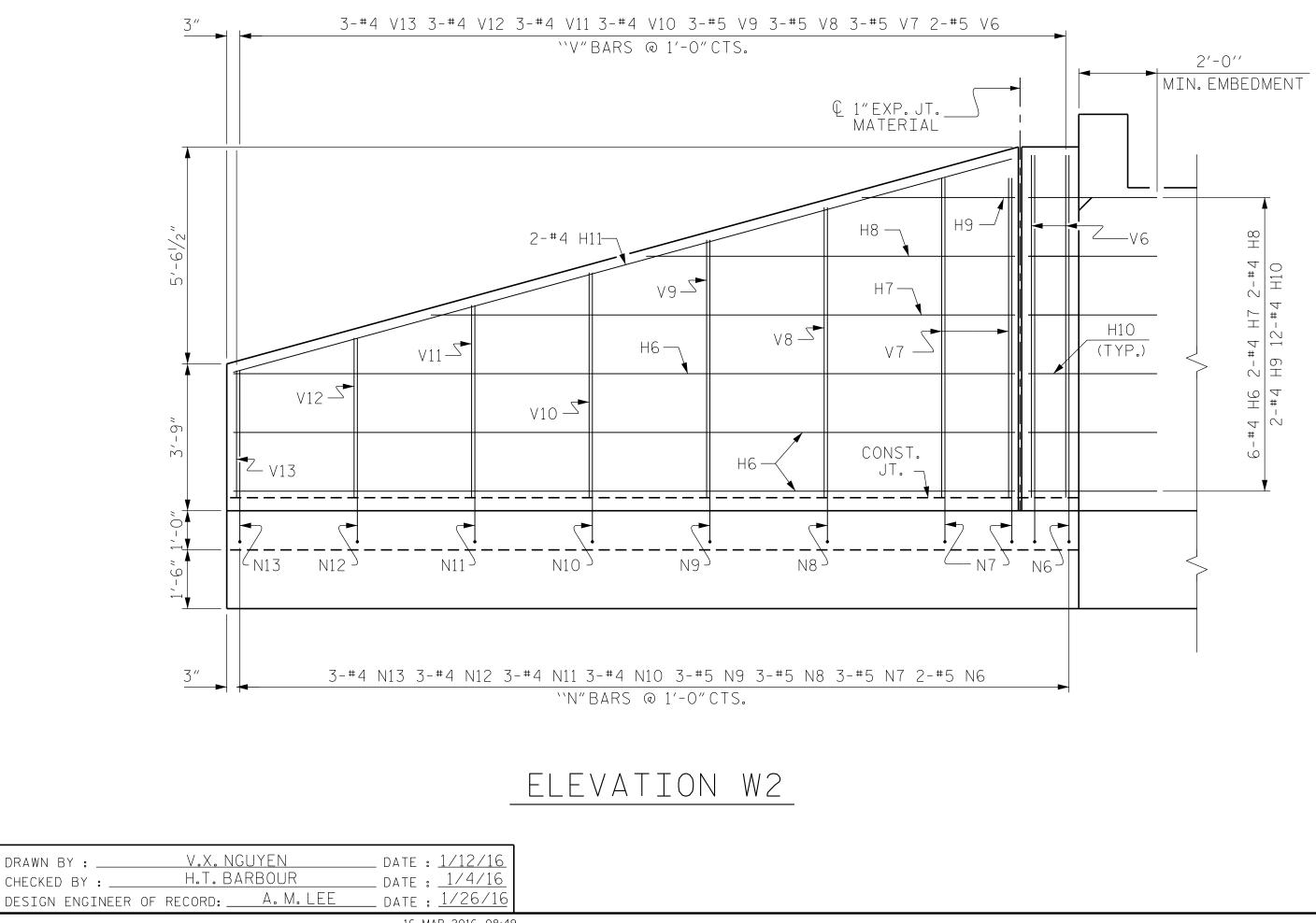
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			BIL	L OF	MATE	RIAL	_			
	PH	ASE	I		PHASE II					
NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
12	#4	STR	10'-10"	87	H1	6	#4	STR	10'-10"	43
4	#4	STR	7′-8″	20	H2	2	#4	STR	7'-8"	10
4	#4	STR	4'-1"	11	H3	2	#4	STR	4'-1"	5
24	#4	1	3'-3"	52	H4	12	#4	1	3'-3"	26
4	#4	STR	11'-9"	31	Н5	2	#4	STR	11'-9"	16
4	#5	2	10'-2"	42	N1	2	#5	2	10'-2"	21
6	#5	2	9'-2"	57	N2	3	#5	2	9'-2"	29
6	#4	2	7'-11"	32	NZ N3	3	#4	2	7'-11"	16
6	#4	2	6'-7"	26	N4	3	#4	2	6'-7"	10
6	#4	2	5'-4"	20	N5	3	#4	2	5'-4"	11
0	1	۷			TNO					
6	#6	STR	6'-0"	54	S1	3	#6	STR	6'-0"	27
6	#5	STR	12'-9"	80	Τ1	3	#5	STR	12'-9"	40
4	#4	STR	8'-1"	22	V1	2	#4	STR	8'-1"	11
6	#4	STR	7'-1"	28	V2	3	#4	STR	7'-1"	14
6	#4	STR	5′-10″	23	٧3	3	#4	STR	5′-10″	12
6	#4	STR	4'-7"	18	V4	3	#4	STR	4'-7"	9
6	#4	STR	3'-4"	13	V5	3	#4	STR	3'-4"	7
	#5	3	6'-0"		71		+			17
4	#5	3	6 -0 5'-5"	25 34	Z1 Z2	2	#5 #5	3	6'-0" 5'-5"	13 17
6	#4	3	4'-7"	18	ZZ Z3	3	#4	3	4'-7"	9
6	#4	3	3'-10"	15	Z 3 Z 4	3	#4	3	3'-10"	8
6	#4	3	3'-1"	12	Z4 Z5	3	#4	3	3'-1"	6
RCIN WING	IG STEE ;s	ĒL	7	21 LBS.	REINF For 1		G STEE	ĒL	3	63 LBS.
WINC	NCRETE GS (W1) CURTA	IN WAL	_LS í).7 C.Y. 1.2 C.Y. 1.9 C.Y.	1 2	WING HEAD		IN WAL	LS 0	.4 C.Y. .9 C.Y. .9 C.Y. .2 C.Y.

	PROJECT <u>A</u> STATION:	<u>SHE</u> 10+6		7 UNTY -L-
SEAL IT230 Docusigned by: Wael Q-rafat	STA CONCRE h = 8'-0'		NSPORTA WING CUL' SLOPE	S
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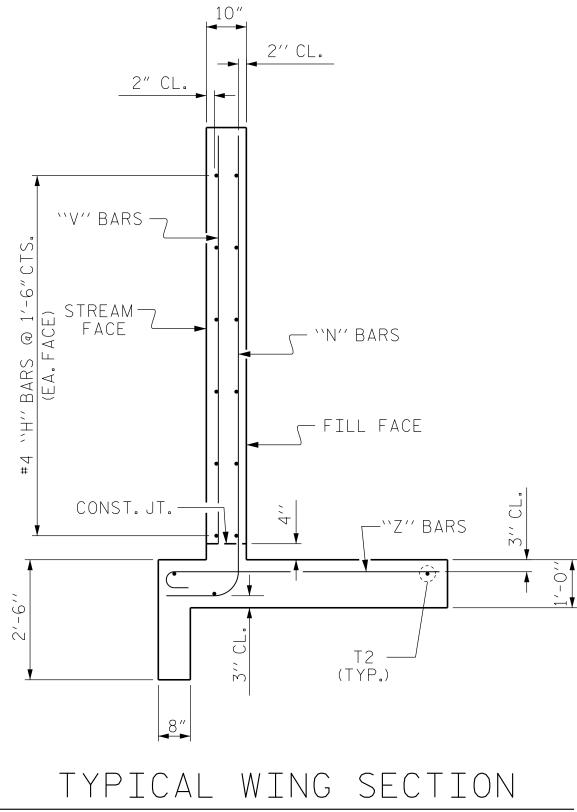


PLAN W2

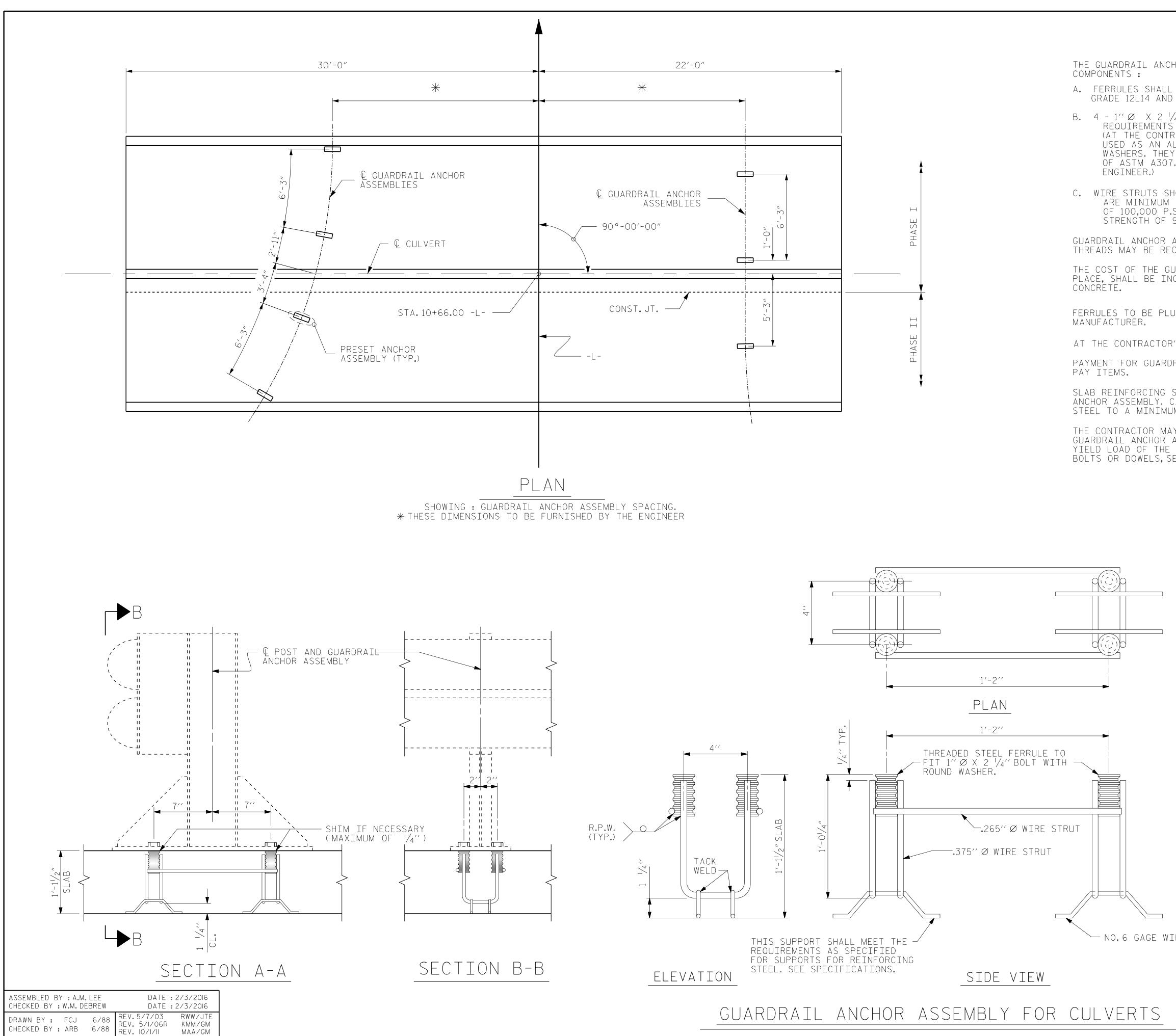
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16-MAR-2016 08:49 \$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$ warafat AT SD-290338



\$DGN\$\$\$\$\$\$\$\$\$\$\$\$\$\$\$	16-MAR-2	016 08:4	49	1D W	rafat A	T SD-290338
		BIL	L OF	MA	TERIA	L
- -			PHA	ASE :	II	
	BAR	NO.	SIZE #1	TYPE	LENGTH	WEIGHT
5'-0"	H6 H7	6 2	#4 #4	STR STR	19'-11" 14'-11"	80
4'-9" 7"	Н8	2	#4	STR	9′-5″	13
	H9 H10	2 12	#4 #4	STR STR	3'-11" 3'-3"	5 26
4'-5"	H10 H11	2	#4 #4	STR	20'-7"	26
4'-1"						
3'-9" 7"	N6 N7	2 3	#5 #5	2	10'-10" 10'-3"	23
3'-5" 7"	N8	3	#5	2	9'-5"	29
► ►	N9	3	#5	2	8'-7"	27
3'-1"	N10 N11	3 3	#4 #4	2	7'-10" 7'-0"	16
2'-9"	N12	3	#4	2	6'-2"	19
	N13	3	#4	2	5′-4″	11
НК.	S2	3	#6	STR	6'-0"	27
(3)					0 0	
	T2	3	#5	STR	23'-8"	74
	V6	2	#5	STR	8′-9″	18
	V 6 V 7	<u> </u>	#5 #5	STR	8'-9" 8'-2"	26
	V8	3	#5	STR	7'-5″	23
	V9 V10	3 3	#5 #4	STR	6'-7" 5'-9"	21
	V10 V11	3 3	#4 #4	STR STR	5'-9" 4'-11"	12
	V12	3	#4	STR	4'-1"	8
	V13	3	#4	STR	3'-3"	7
	Z6	4	#5	3	5'-7"	23
	Ζ7	3	#5	3	5'-4"	17
	Z8 Z9	3 3	#5 #5	3	5'-0" 4'-8"	16 15
	Z 9 Z10	3	#5	3	4'-4"	13
	Z11	3	#5	3	4'-0"	13
	Z12 713	3 3	#5 #5	3	3′-8″ 3′-4″	11
	Z13		_	_	ے ا	10
	REINF FOR 1		G STEE	EL	68	30 LBS
			NCRETE			
					W2)	9.2 C.Y.
				Т	TOTAL S	0.2 C.Y.
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SEAL 17230	S	TRA		HT DR	WING)
Docusigned by: Wael Orafat	ONCF	RET [ΞB	ОХ	CULV	/ERT
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GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS ``A"

FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO A MINIMUM.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1"Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".

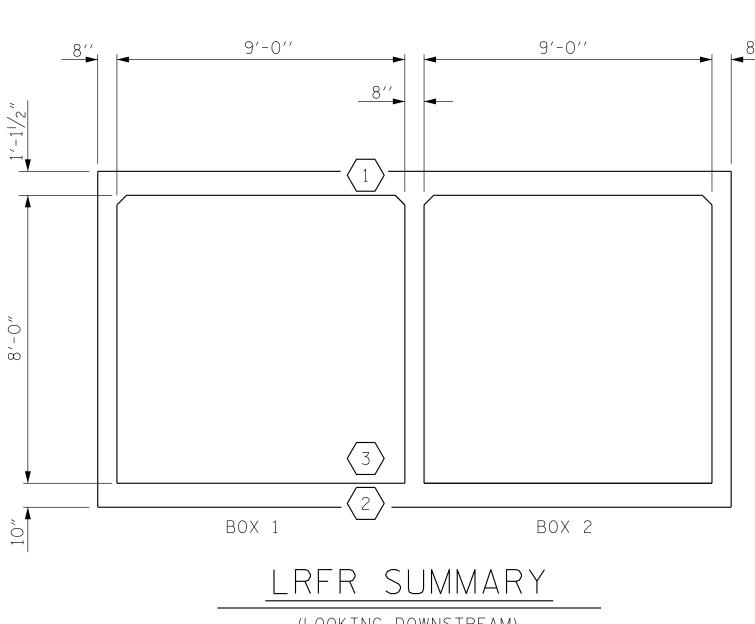
B. 4 - 1" \varnothing X 2 $\frac{1}{4}$ " BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1'' \varnothing X 2¹/₄'' GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE

C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A $7_{16}{}^{\prime\prime}$ Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

	PROJEC STATIC	ASH DN: <u>1</u>			, UNTY <u>L</u> –
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SID. NO. GRAI

		()R RATIN Rete bo>			RTS.			
										STRENGTH	I LIM	IT ST	ATE			
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING #	MINIMUM Rating factors (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE TYPE	DISTANCE FROM Left end of element (f+)	
DESIGN LOAD RATING		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.02		1.75	1.46	1	TOP SLAB	3.87	1.02	1	TOP SLAB	8.49	I
		HL-93 (OPERATING)	NZA		1.33		1.35	1.90	1	TOP SLAB	3.87	1.33	1	TOP SLAB	8.49	
		HS-20 (INVENTORY)	36.00	2	1.13	40.51	1.75	1.47	1	TOP SLAB	4.11	1.13	1	BOTTOM SLAB	8.73	
		HS-20 (OPERATING)	36.00		1.46	52.51	1.35	1.91	1	TOP SLAB	4.11	1.46	1	BOTTOM SLAB	8.73	
		SNSH	13.50		2.16	29.15	1.40	2.69	1	TOP SLAB	4.11	2.16	1	TOP SLAB	8.49	
		SNGARBS2	20.00		2.02	40.42	1.40	2.51	1	TOP SLAB	4.11	2.02	1	TOP SLAB	8.49	
	ICL	SNAGRIS2	22.00		2.05	45.00	1.40	2.69	1	TOP SLAB	4.11	2.05	1	BOTTOM SLAB	8.73	
		SNCOTTS3	27.25		1.29	35.08	1.40	1.83	1	TOP SLAB	3.87	1.29	1	TOP SLAB	8.49	
	SLE (S	SNAGGRS4	34.93		1.31	45.72	1.40	1.88	1	BOTTOM SLAB	8.94	1.31	1	BOTTOM SLAB	8.73	
	SINGLE	SNS5A	35.55		1.45	51.54	1.40	2.06	1	BOTTOM SLAB	8.94	1.45	1	BOTTOM SLAB	8.73	
		SNS6A	39.95		1.28	51.07	1.40	1.95	1	BOTTOM SLAB	8.94	1.28	1	BOTTOM SLAB	8.73	
LEGAL LOAD		SNS7B	42.00		1.34	56.35	1.40	1.95	1	BOTTOM SLAB	8.94	1.34	1	BOTTOM SLAB	8.73	
RATING	ER	TNAGRIT3	33.00		1.62	53.36	1.40	2.41	1	BOTTOM SLAB	8.94	1.62	1	BOTTOM SLAB	8.73	
	RAIL	TNT4A	33.08		1.54	50.87	1.40	2.18	1	TOP SLAB	3.87	1.54	1	TOP SLAB	8.49	Γ
TOR SEMI-T		TNT6A	41.60		1.45	60.31	1.40	2.05	1	BOTTOM SLAB	8.94	1.45	1	BOTTOM SLAB	8.73	
	SEN ST)	TNT7A	42.00		1.42	59.52	1.40	2.17	1	BOTTOM SLAB	8.94	1.42	1	BOTTOM SLAB	8.73	
	TOR (TT)	TNT7B	42.00		1.53	64.34	1.40	2.16	1	BOTTOM SLAB	8.94	1.53	1	BOTTOM SLAB	8.73	
	TRAC	TNAGRIT4	43.00		1.39	59.60	1.40	1.99	1	BOTTOM SLAB	8.94	1.39	1	BOTTOM SLAB	8.73	
	TRUCK	TNAGT5A	45.00		1.19	53.37	1.40	1.74	1	BOTTOM SLAB	8.94	1.19	1	BOTTOM SLAB	8.73	
	TRL	TNAGT5B	45.00	3	1.09	48.93	1.40	1.62	1	BOTTOM SLAB	8.94	1.09	1	BOTTOM SLAB	8.73	



(LOOKING	DOWNST

ASSEMBLED BY : CHECKED BY :		:12-10-15 : 1-4-16
DRAWN BY : WMC Checked by : GM	REV.10/1/11	MAA/GM

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RATING (LRFR) ETE BOX CULVERTS

STRENGTH I LIMIT STATE MENT Shear FACTOR FR(0F (+) DISTANCE LEFT END ELEMENT (-ANCE END ELEMENT TYPE ° NO RATING ELEMEN⁻ Type $- \stackrel{\square}{\geq}$ BOX DIS 3.87 TOP SLAB 1.02 TOP SLAB 8.49 1 TOP SLAB 3.87 1.33 TOP SLAB 8.49 1 TOP SLAB 4.11 1.13 BOTTOM SLAB 8.73 1 4.11 BOTTOM SLAB 8.73 1.46 TOP SLAB TOP SLAB 4.11 2.16 TOP SLAB 8.49 1 4.11 TOP SLAB 2.02 TOP SLAB 8.49 1 4.11 BOTTOM SLAB TOP SLAB 2.05 8.73 1 3.87 1.29 TOP SLAB TOP SLAB 8.49 1 8.94 8.73 BOTTOM SLAB BOTTOM SLAB 1.31 1 BOTTOM SLAB 8.94 1.45 BOTTOM SLAB 8.73 1 8.94 BOTTOM SLAB 1.28 BOTTOM SLAB 8.73 1 8.94 1.34 BOTTOM SLAB 8.73 BOTTOM SLAB BOTTOM SLAB 8.94 1.62 BOTTOM SLAB 8.73 3.87 1.54 TOP SLAB TOP SLAB 8.49 1 BOTTOM SLAB 8.94 BOTTOM SLAB 1.45 8.73 1 BOTTOM SLAB 8.94 1.42 BOTTOM SLAB 8.73 1 BOTTOM SLAB 8.94 1.53 BOTTOM SLAB 8.73

COMMENT

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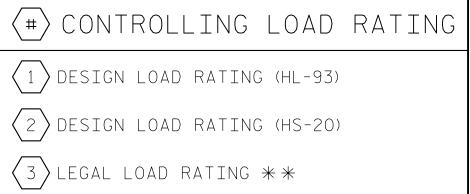
LOAD FACTORS:

DESIGN LOAD	RATING	FACTORS
LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
ΕV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

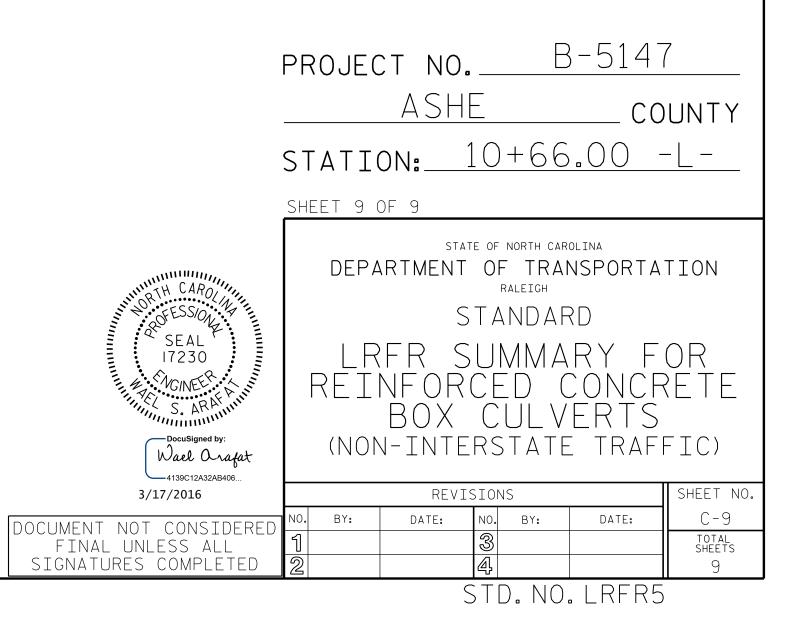
DESTON LOAD DATING EACTORS

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.



** SEE CHART FOR VEHICLE TYPE



DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRESS IN EXTREME TIDER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE

AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

STD. NO. SN