

**This electronic collection of documents is provided
for the convenience of the user
and is Not a Certified Document –**

**The documents contained herein were originally issued
and sealed by the individuals whose names and license
numbers appear on each page, on the dates appearing
with their signature on that page.**

**This file or an individual page
shall not be considered a certified document.**

TIP PROJECT: U-2707

CONTRACT: C203725

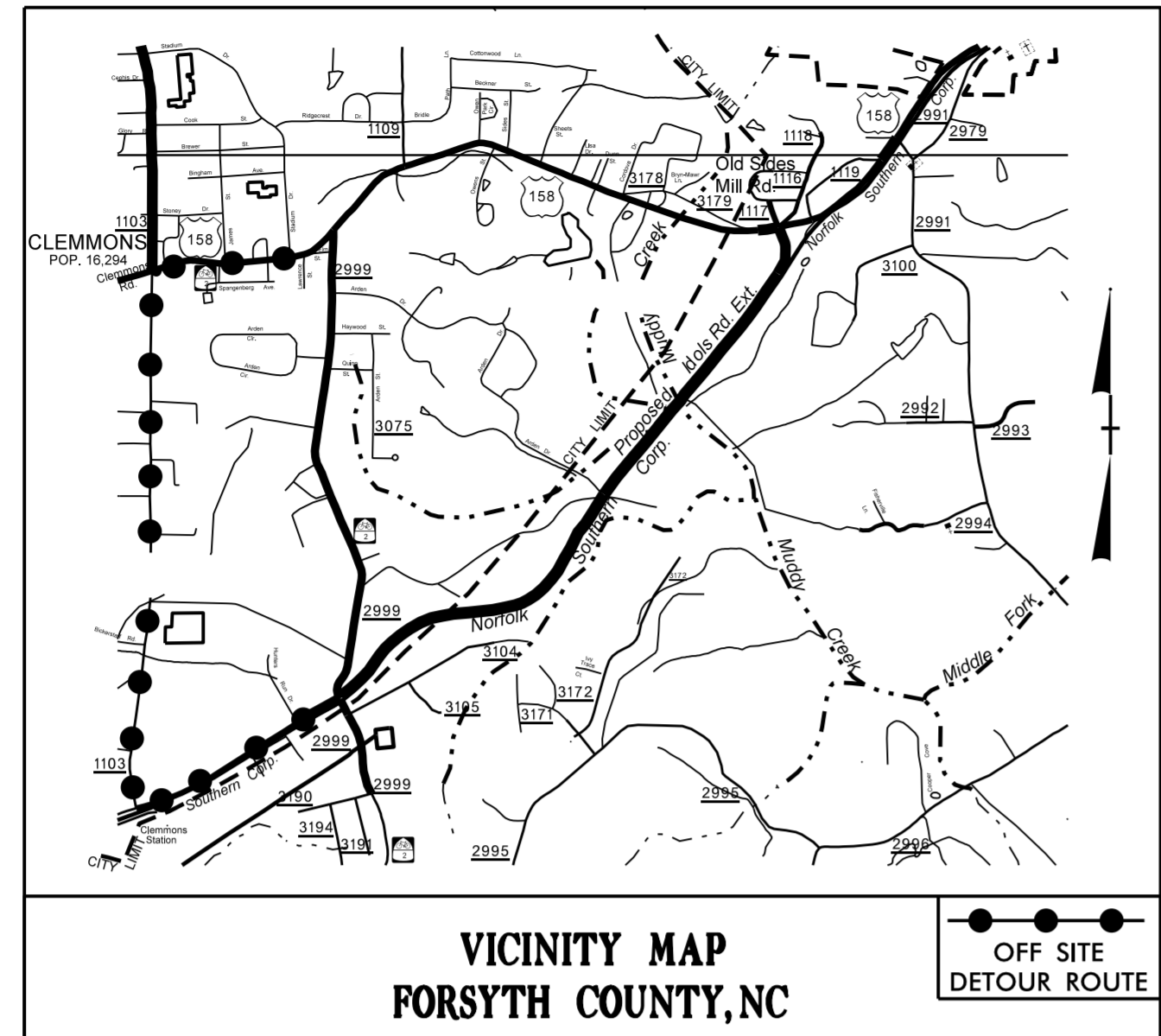
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

FORSYTH COUNTY

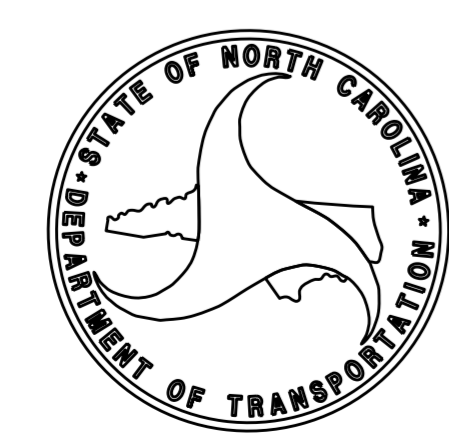
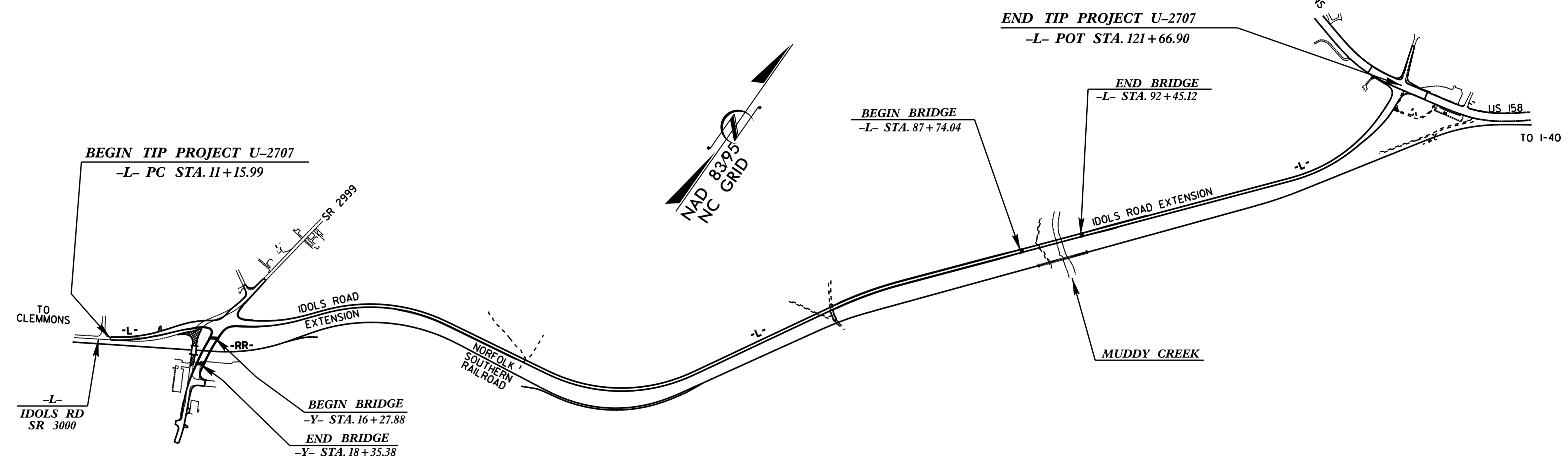
LOCATION: CLEMMONS - SR 3000 (IDOLS ROAD), FROM SR 2999 (HAMPTON ROAD) TO US 158 (CLEMMONS ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNALS, AND STRUCTURES

| STATE | STATE PROJECT REFERENCE NO. | SHEET NO. | TOTAL SHEETS |
|-----------------|-----------------------------|-------------|--------------|
| N.C. | U-2707 | | |
| STATE PROJ. NO. | F.A. PROJ. NO. | DESCRIPTION | |
| 34845.1.1 | STP-3000(1) | P.E. | |
| 34845.2.2 | STP-3000(1) | R/W & UTIL. | |
| 34845.3.3 | STP-3000(5) | CONST. | |
| | | | |
| | | | |
| | | | |



VICINITY MAP
FORSYTH COUNTY, NC



DESIGN DATA

| | |
|-------------------|-----------------|
| ADT 2016 = | 11,140 |
| ADT 2036 = | 17,840 |
| K = | 12 % |
| D = | 60 % |
| T = | 4 % * |
| V = | 50 MPH |
| * TTST = | 1% DUAL 3% |
| FUNC CLASS = | URBAN COLLECTOR |
| SUB REGIONAL TIER | |

PROJECT LENGTH

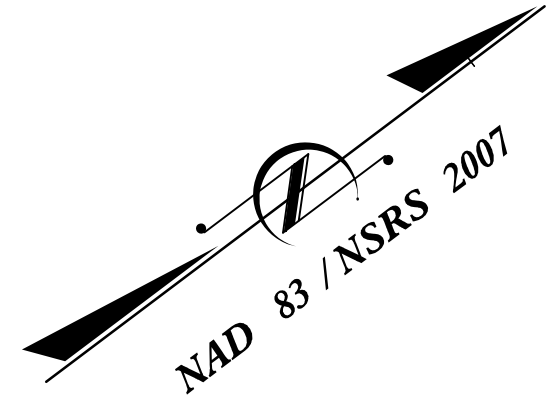
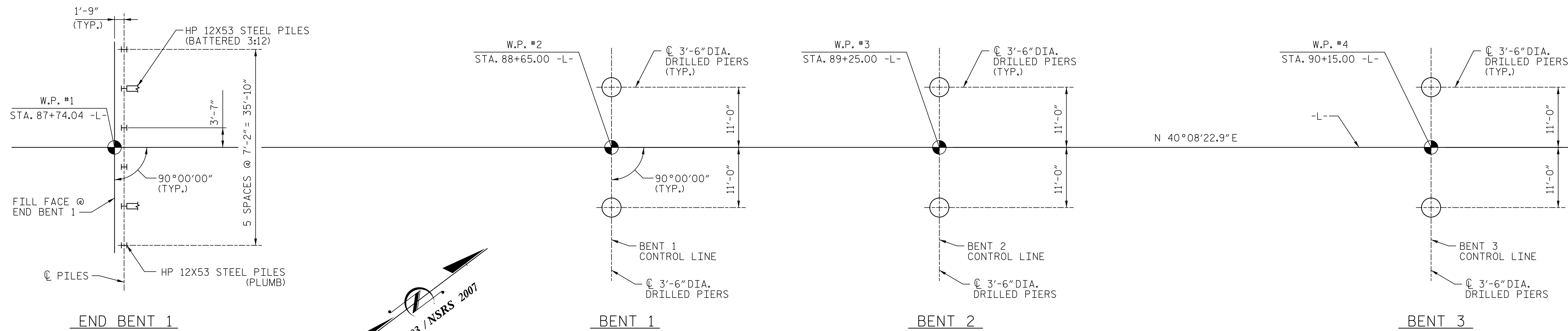
| | |
|---------------------------------------|----------|
| LENGTH ROADWAY TIP PROJECT U-2707 = | 2.004 MI |
| LENGTH STRUCTURE TIP PROJECT U-2707 = | 0.089 MI |
| TOTAL LENGTH TIP PROJECT U-2707 = | 2.093 MI |

Prepared for the Office of:

2012 STANDARD SPECIFICATIONS

LETTING DATE:
APRIL 19, 2016

STRUCTURES



NOTES:

FOR PILES, SEE PROJECT SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE. DRIVE PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.

PILES AT END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE. DRIVE PILES AT END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 175 TONS PER PILE.

FOR DRILLED PIERS, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 420 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 25 TSF.

INSTALL DRILLED PIERS AT BENT 1 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 659 FT. (LEFT) AND 656 FT. (RIGHT) WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 430 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 40 TSF.

INSTALL DRILLED PIERS AT BENT 2 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 659.5 FT. (LEFT) AND 657.5 FT. (RIGHT) WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT 3 ARE DESIGNED FOR A FACTORED RESISTANCE OF 490 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 40 TSF.

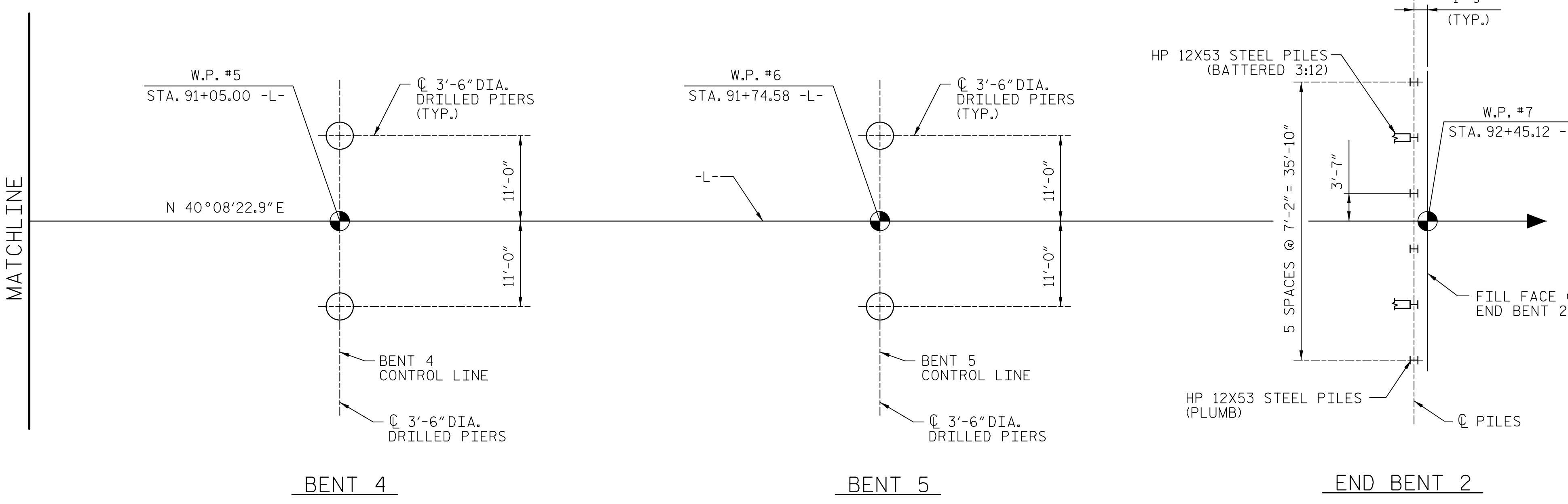
INSTALL DRILLED PIERS AT BENT 3 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 657 FT. (LEFT) AND 654 FT. (RIGHT) WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT 4 ARE DESIGNED FOR A FACTORED RESISTANCE OF 450 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 95 TSF (LEFT) AND 25 TSF (RIGHT).

INSTALL DRILLED PIERS AT BENT 4 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 661 FT. (LEFT) AND 667 FT. (RIGHT) WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT 5 ARE DESIGNED FOR A FACTORED RESISTANCE OF 400 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 40 TSF.

INSTALL DRILLED PIERS AT BENT 5 THAT EXTEND TO AN ELEVATION NO HIGHER THAN 665 FT. (LEFT) AND 668 FT. (RIGHT) WITH THE REQUIRED TIP RESISTANCE.



FOUNDATION LAYOUT PLAN

(DIMENSIONS LOCATING PILES ARE SHOWN TO THE PILE CENTERLINE)

NOTES (CONT'D.):

PERMANENT STEEL CASINGS MAY BE REQUIRED FOR DRILLED PIERS AT BENT 3. IF REQUIRED, DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 677 FT. WITHOUT PRIOR APPROVAL FROM THE ENGINEER. THE ENGINEER WILL DETERMINE THE NEED FOR PERMANENT CASINGS.

PERMANENT STEEL CASINGS MAY BE REQUIRED FOR DRILLED PIERS AT BENT 4. IF REQUIRED, DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 682 FT. WITHOUT PRIOR APPROVAL FROM THE ENGINEER. THE ENGINEER WILL DETERMINE THE NEED FOR PERMANENT CASINGS.

THE SCOUR CRITICAL ELEVATION FOR BENTS 1, 2 AND 3 IS ELEVATION 670 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SCOUR CRITICAL ELEVATION FOR BENT 4 IS ELEVATION 675 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

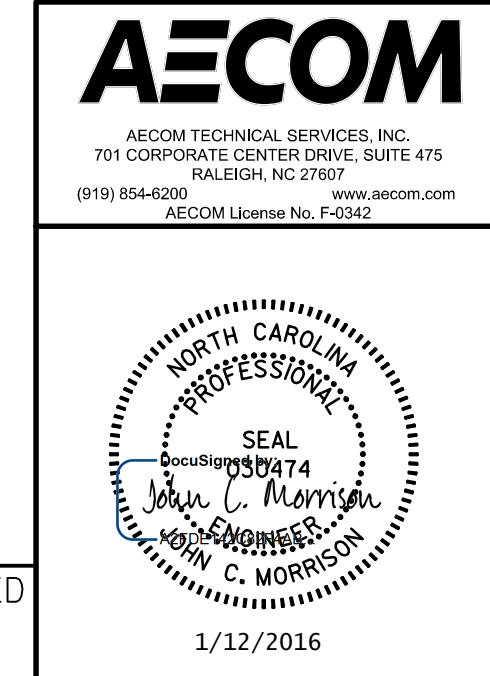
THE SCOUR CRITICAL ELEVATION FOR BENT 5 IS ELEVATION 680 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

SPT MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SPT. FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 2 OF 3 BRIDGE NO. 656

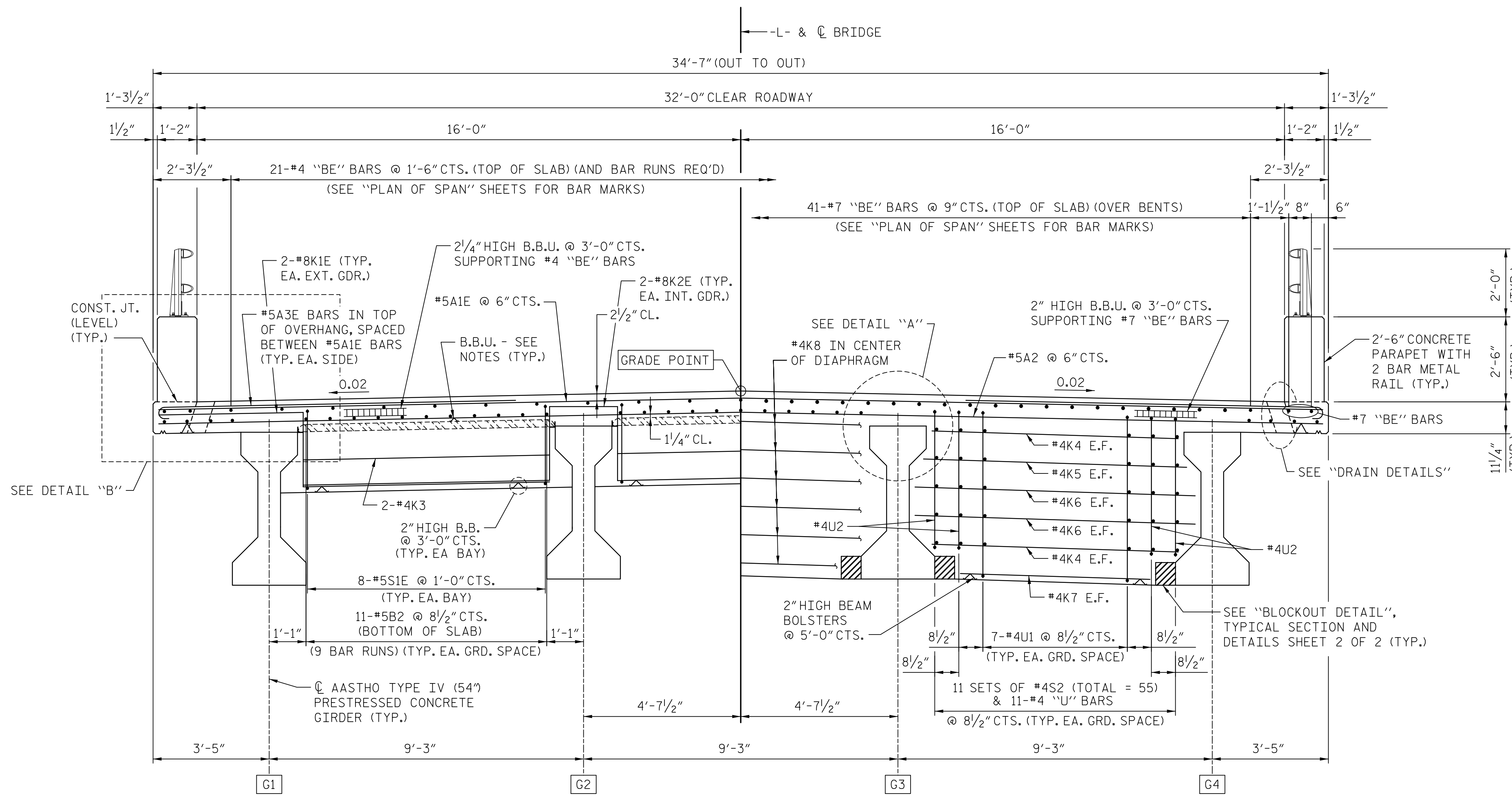


| | | | | | |
|----------------------------------------------------------------------------------|-----|-------|-----|-----|-----------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| GENERAL DRAWING | | | | | |
| BRIDGE ON SR 3000 (IDOLS ROAD) OVER MUDDY CREEK BETWEEN SR 2999 AND US 158 | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. S-02 | | | | | TOTAL SHEETS 86 |

DRAWN BY : KHC DATE : 10/2015
 CHECKED BY : JCM DATE : 10/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

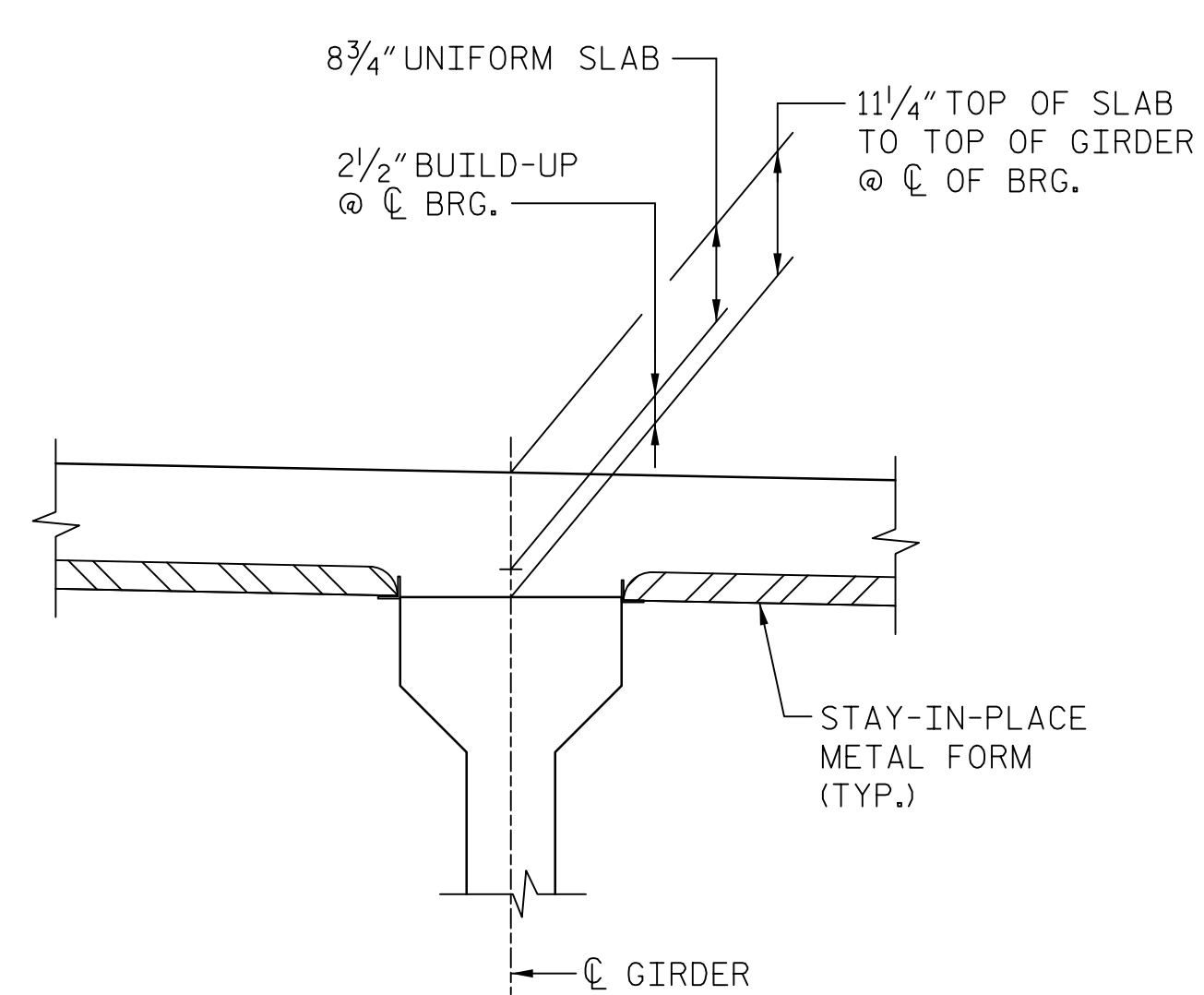
DATE: 1/12/2016 TIME: 9:25:54 AM USER: jmc_morris DON: R:\Bridges\656\Card\40_003_U2707_SML_S02.dwg



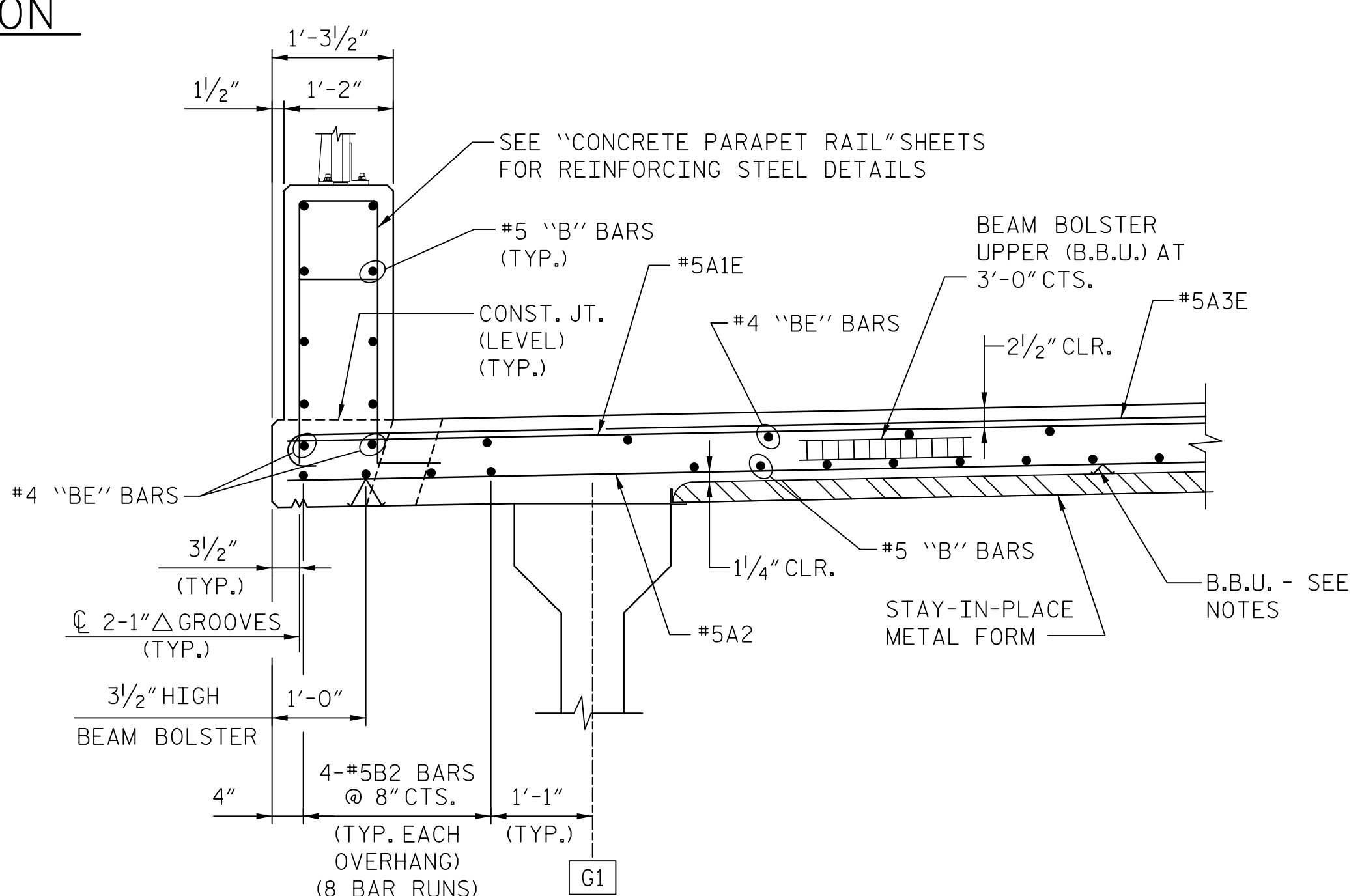
TYPICAL HALF SECTION
(SHOWING END BENT DIAPHRAGMS)

TYPICAL HALF SECTION
(SHOWING CONTINUOUS FOR LIVE LOAD DIAPHRAGMS)

TYPICAL SECTION



DETAIL "A"



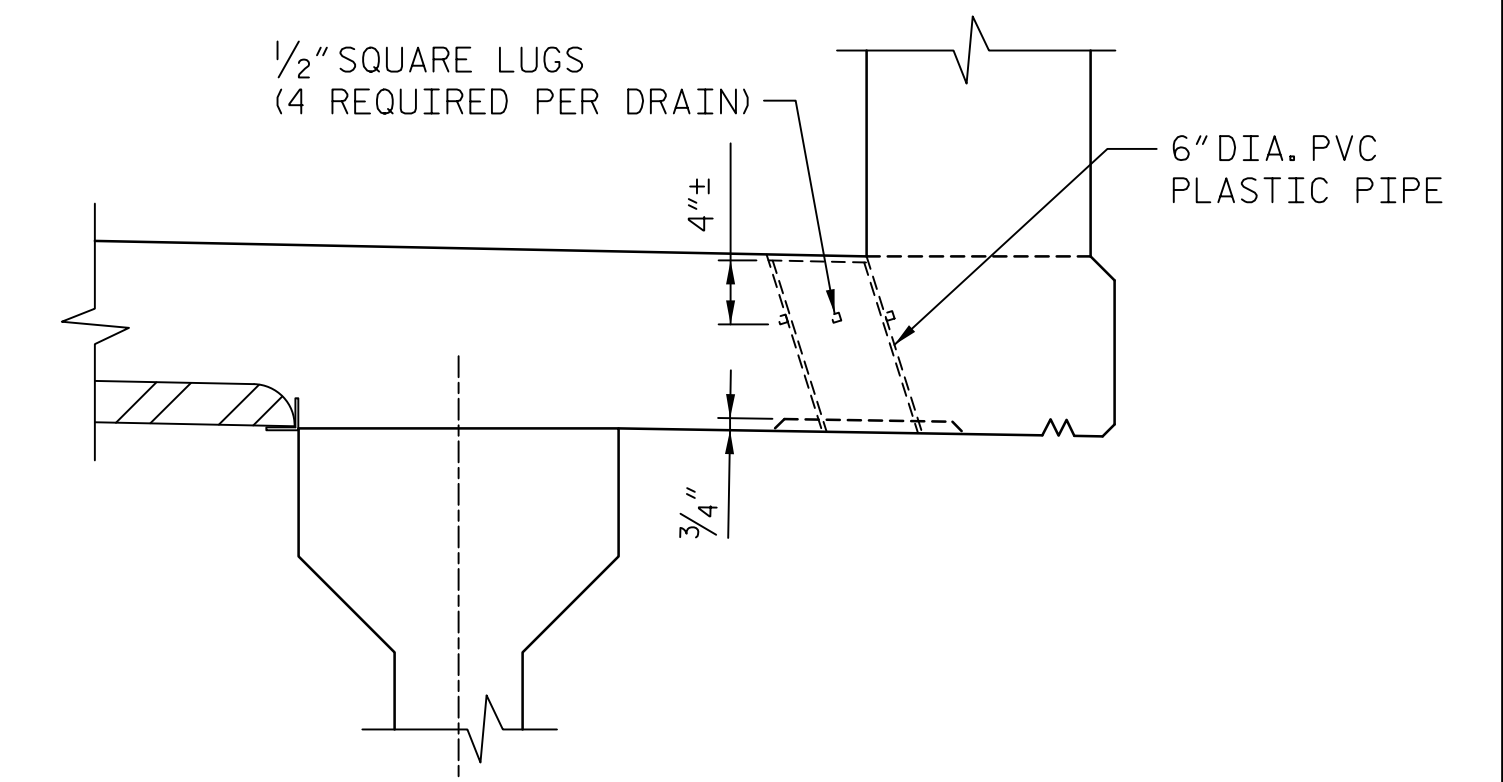
DETAIL "B"

NOTES:

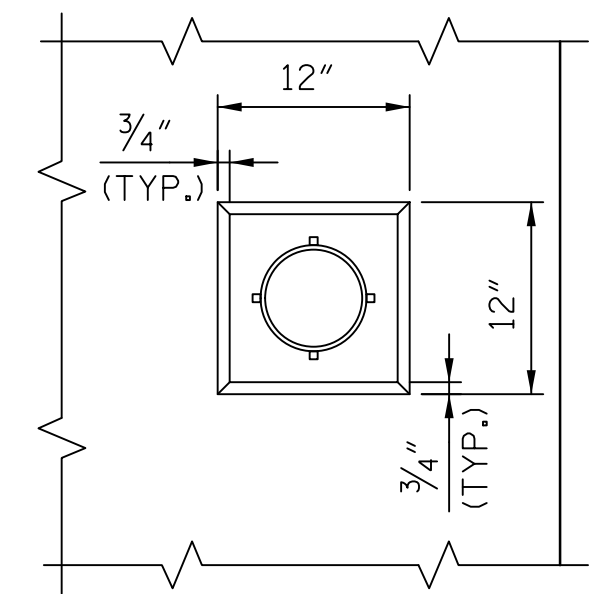
PROVIDE 1/4" HIGH BEAM BOLSTERS UPPER AT 4'-0" CTS. ATOP THE STAY-IN-PLACE METAL FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) AT 4'-0" CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 2 1/2" ABOVE THE TOP OF THE REMOVABLE FORM.

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

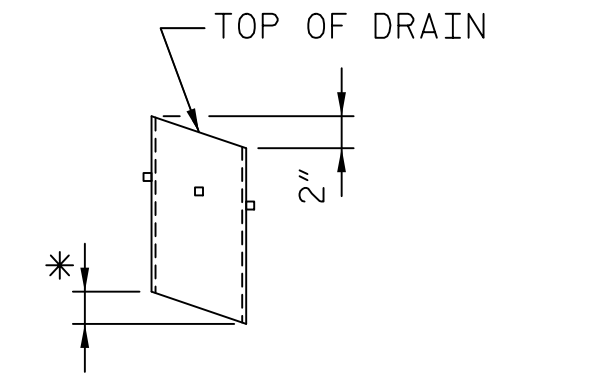
FOR SECTION AT INTERMEDIATE DIAPHRAGMS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE II, III, & IV PRESTRESSED CONCRETE GIRDERS".



ELEVATION



PLAN OF RECESS



PIPE DETAIL

DRAIN NOTES:

TOP OF FLOOR DRAIN TO BE SET 3/8" BELOW SURFACE OF SLAB.

4- 1/2" SQUARE LUGS TO BE GLUED TO THE PVC PLASTIC PIPE AT EQUAL SPACES AROUND THE PIPE DRAIN APPROXIMATELY 4" FROM THE TOP OF THE PIPE.

THE 6" DIA. PVC PLASTIC PIPE AND FITTINGS SHALL BE SCHEDULE 40 AND CONFORM TO ASTM D1785.

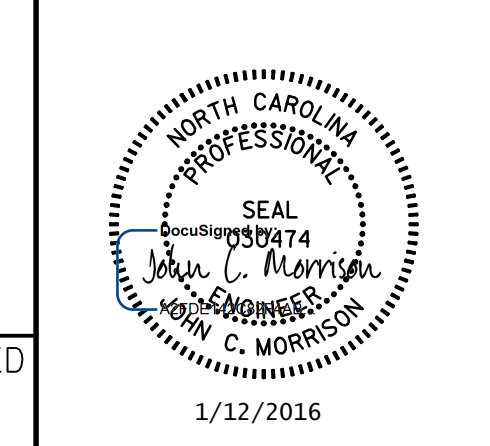
DRAIN DETAILS

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 1 OF 2 BRIDGE NO. 656

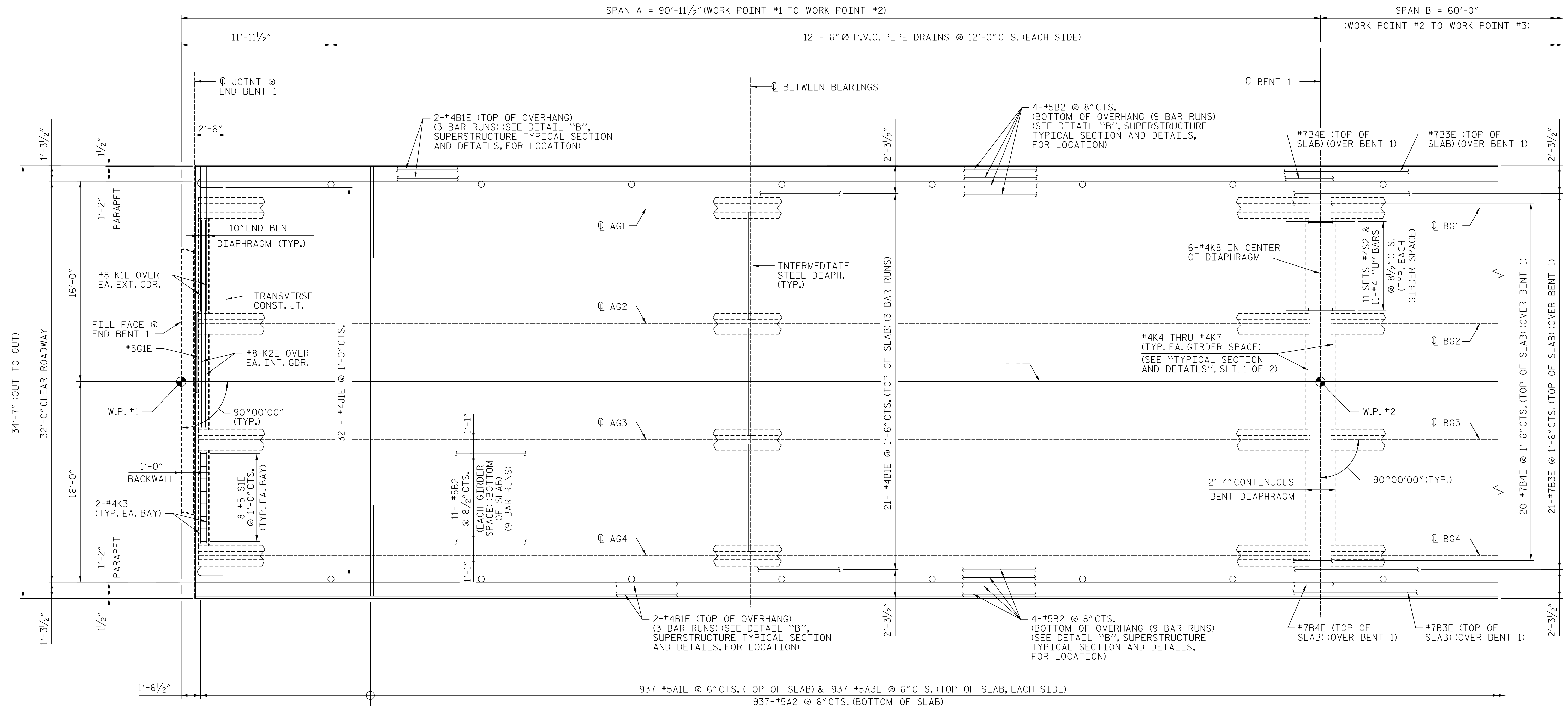


| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUPERSTRUCTURE TYPICAL SECTION AND DETAILS | | | | | |
| SHEET NO. S-05 | | | | | |
| TOTAL SHEETS 86 | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: MKT DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

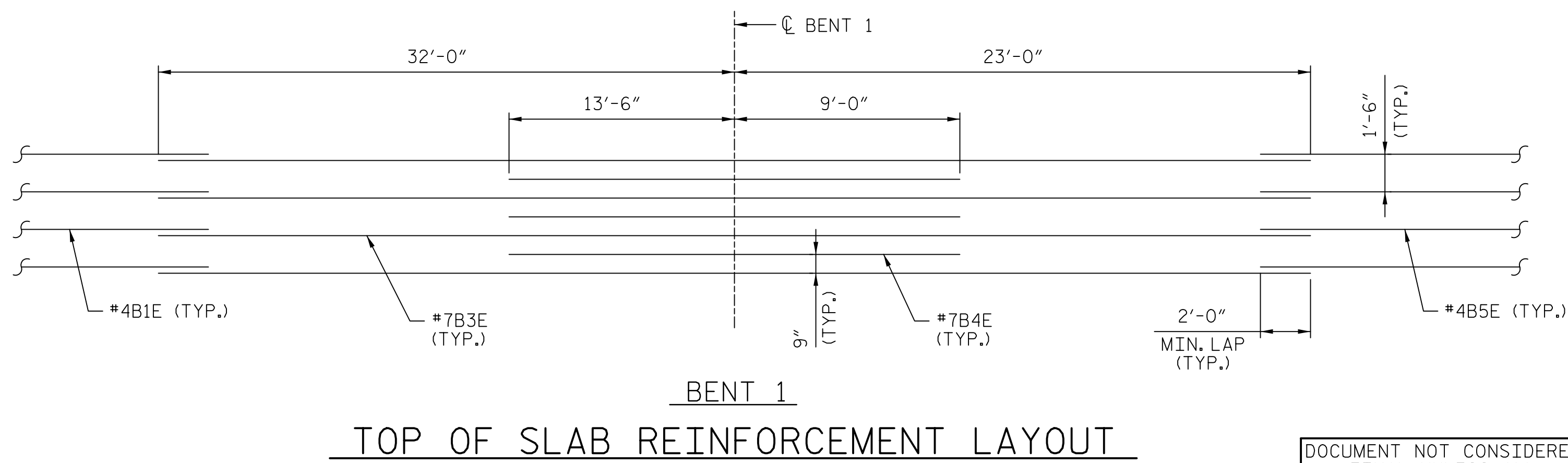
DATE: 11/12/2015 TIME: 9:54:05 AM
USER: jcm\jcm.dgn
DGN: R:\Bridge\656\Card\40_009_U2707_SML_S05_T50.dgn



PLAN OF SPAN A AND PARTIAL PLAN OF SPAN B

NOTES:

- FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.
- FOR SECTIONS "A-A" AND "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.
- ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.
- FOR PLACEMENT OF #4 J1 BARS, SEE "EXPANSION JOINT SEAL DETAILS" SHEET.



TOP OF SLAB REINFORCEMENT LAYOUT

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 1 OF 6 BRIDGE NO. 656

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 JOHN C. MORRIS
 PROFESSIONAL ENGINEER
 1/12/2016

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN A
 AND PARTIAL PLAN
 OF SPAN B

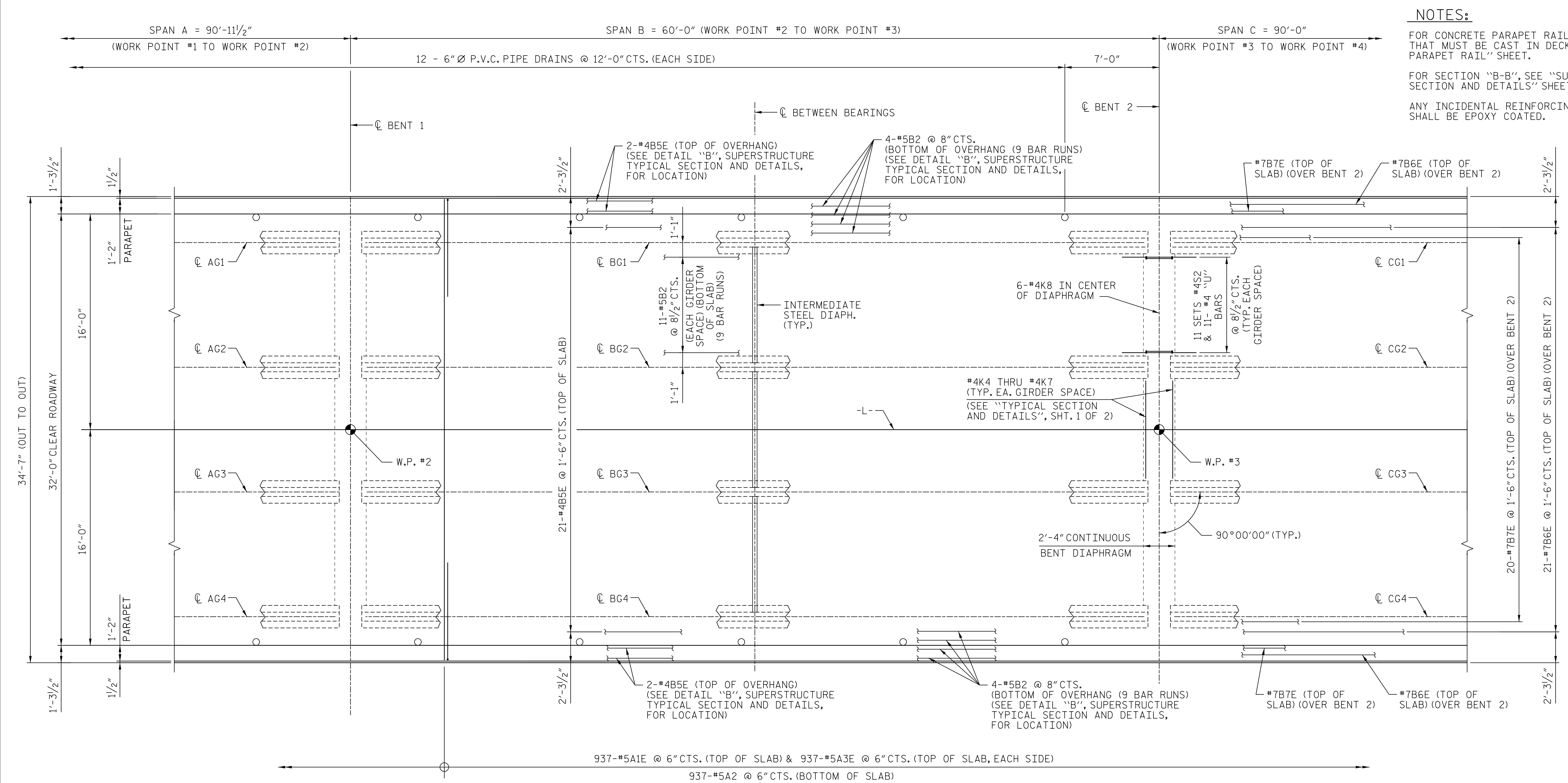
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-07**
 TOTAL SHEETS **86**

DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

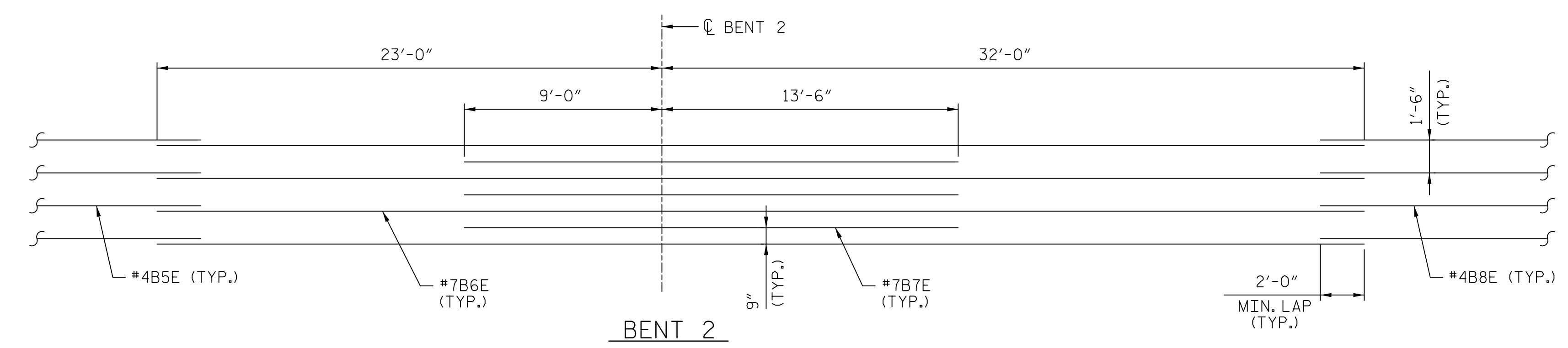
USER: P:\info\p040401\03_112707_SML_507_P50.dgn
 DATE: 1/12/2016
 TIME: 9:25:46 AM



NOTES:
 FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.
 FOR SECTION "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.
 ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.

PLAN OF SPAN B AND PARTIAL PLAN OF SPANS A & C

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 2 OF 6 BRIDGE NO. 656



TOP OF SLAB REINFORCEMENT LAYOUT

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 JOHN C. MORRIS
 ENGINEER
 1/12/2016

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN B
 AND PARTIAL PLAN
 OF SPANS A & C

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-08**
 TOTAL SHEETS **86**

DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

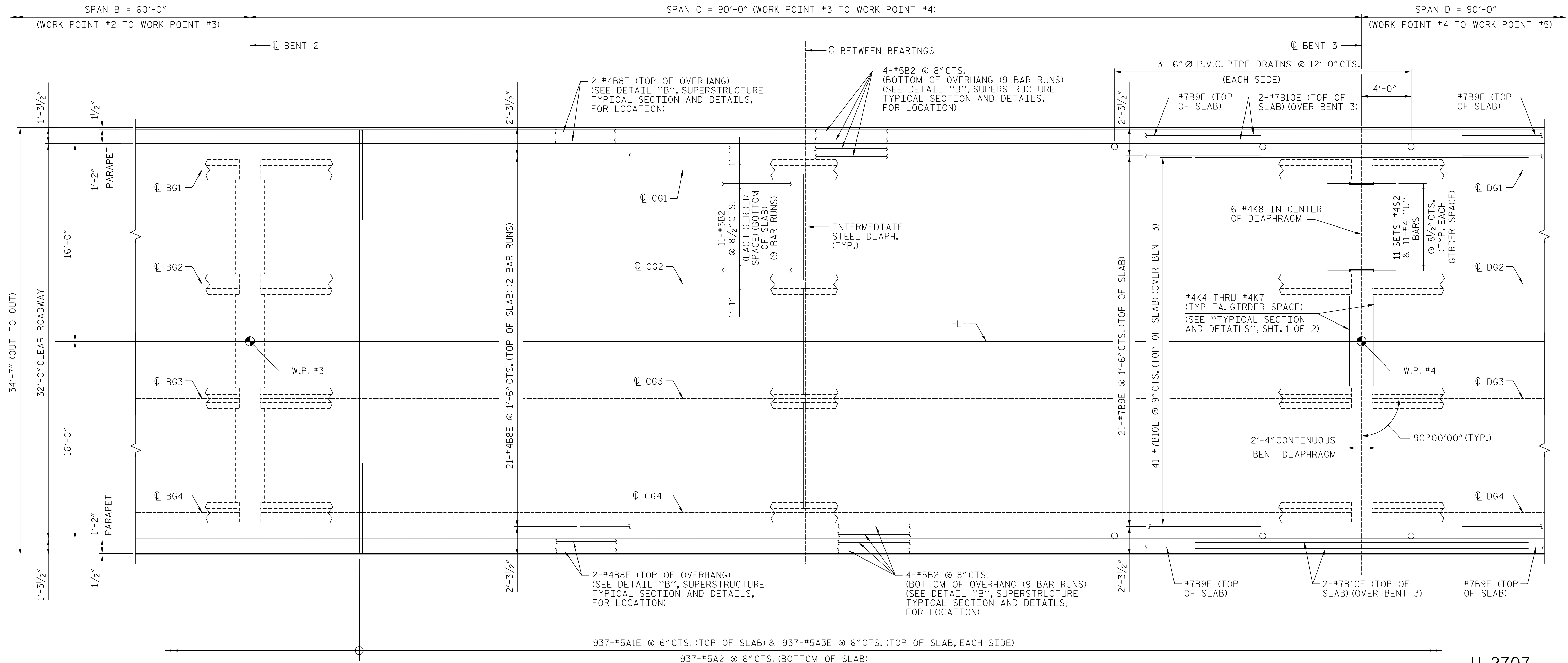
DATE: 1/12/2016 TIME: 9:54:48 AM
 USER: jcm\jcm DON: R:\Bridges\656\Card\401015\U2707_SML_S08_P502.dgn

NOTES:

FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.

FOR SECTION "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.

ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.

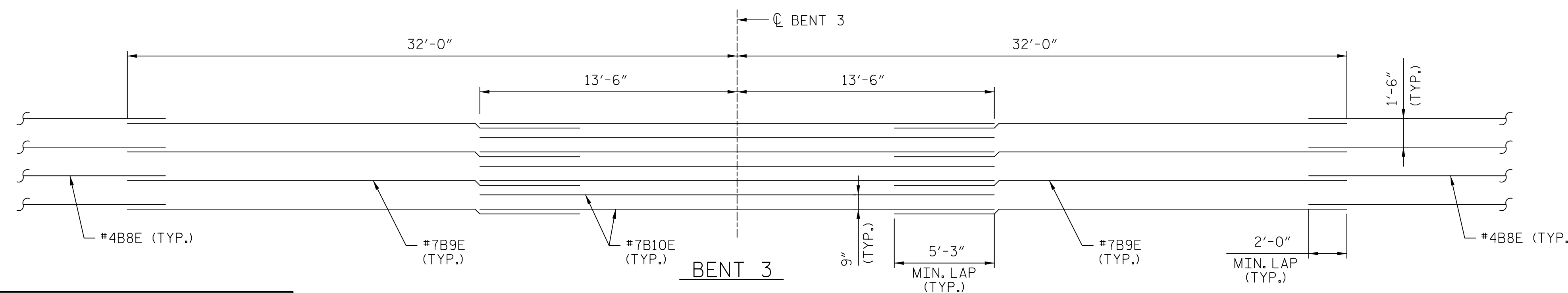


PLAN OF SPAN C AND PARTIAL PLAN OF SPANS B & D

PROJECT NO. U-2707
FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 3 OF 6 BRIDGE NO. 656



TOP OF SLAB REINFORCEMENT LAYOUT

DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 NORTH CAROLINA PROFESSIONAL ENGINEER
 JOHN C. MORRIS
 11/12/2016

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE PLAN OF SPAN C AND PARTIAL PLAN OF SPANS B & D

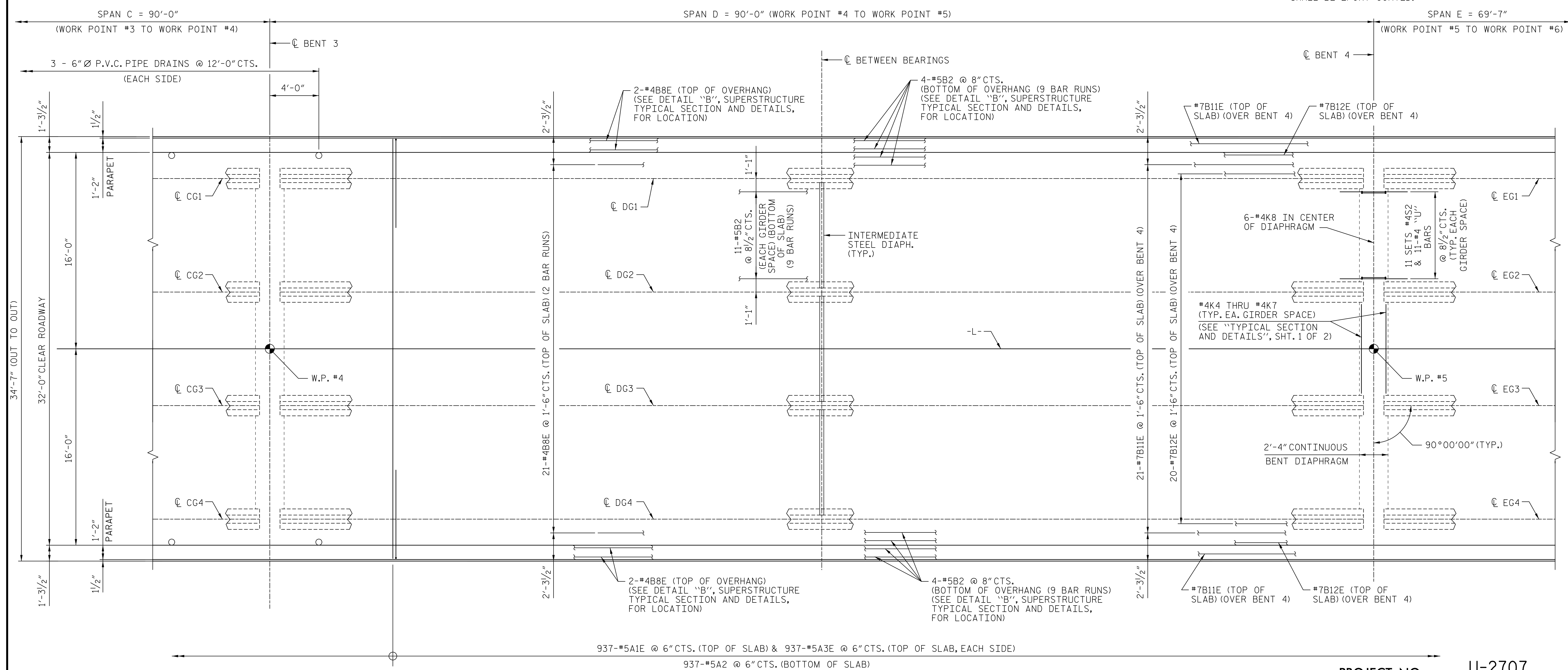
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-09**
 TOTAL SHEETS **86**

USER: P:\12\2015\11\27\15\112707_U2707_SML_509_P303.dgn
 DATE: 11/12/2015
 TIME: 9:54:23 AM

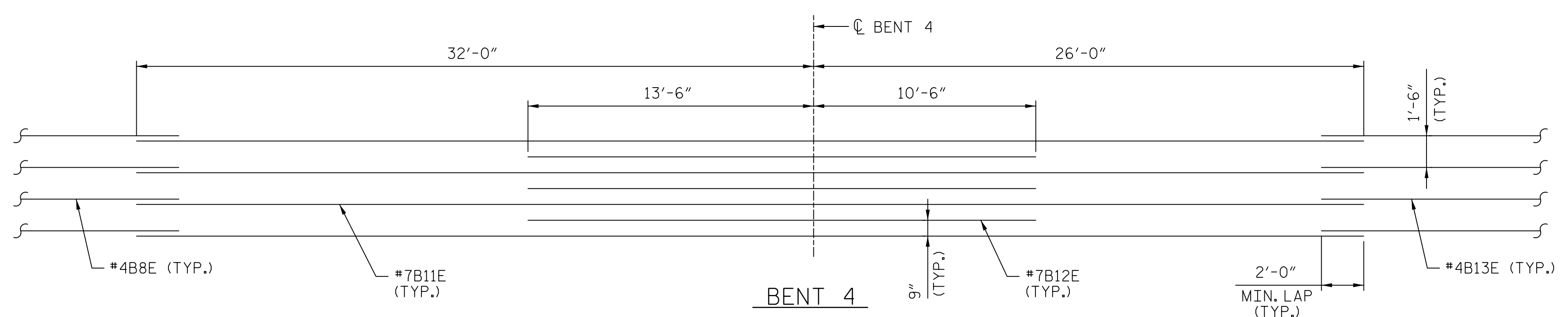
NOTES:

FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.
FOR SECTION "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.
ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.



PLAN OF SPAN D AND PARTIAL PLAN OF SPANS C & E

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 90+25.00 -L-
SHEET 4 OF 6 BRIDGE NO. 656



TOP OF SLAB REINFORCEMENT LAYOUT

DRAWN BY: MKT DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



AECOM logo and project information. Includes AECOM logo, company address (701 CORPORATE CENTER DRIVE, SUITE 475, RALEIGH, NC 27607), phone (919) 854-6200, website www.aecom.com, and license No. F-0342. Below is the title 'SUPERSTRUCTURE PLAN OF SPAN D AND PARTIAL PLAN OF SPANS C & E'. A table for REVISIONS is provided with columns for NO., BY, DATE, NO., BY, DATE. The sheet number is S-10 and the total sheets are 86.

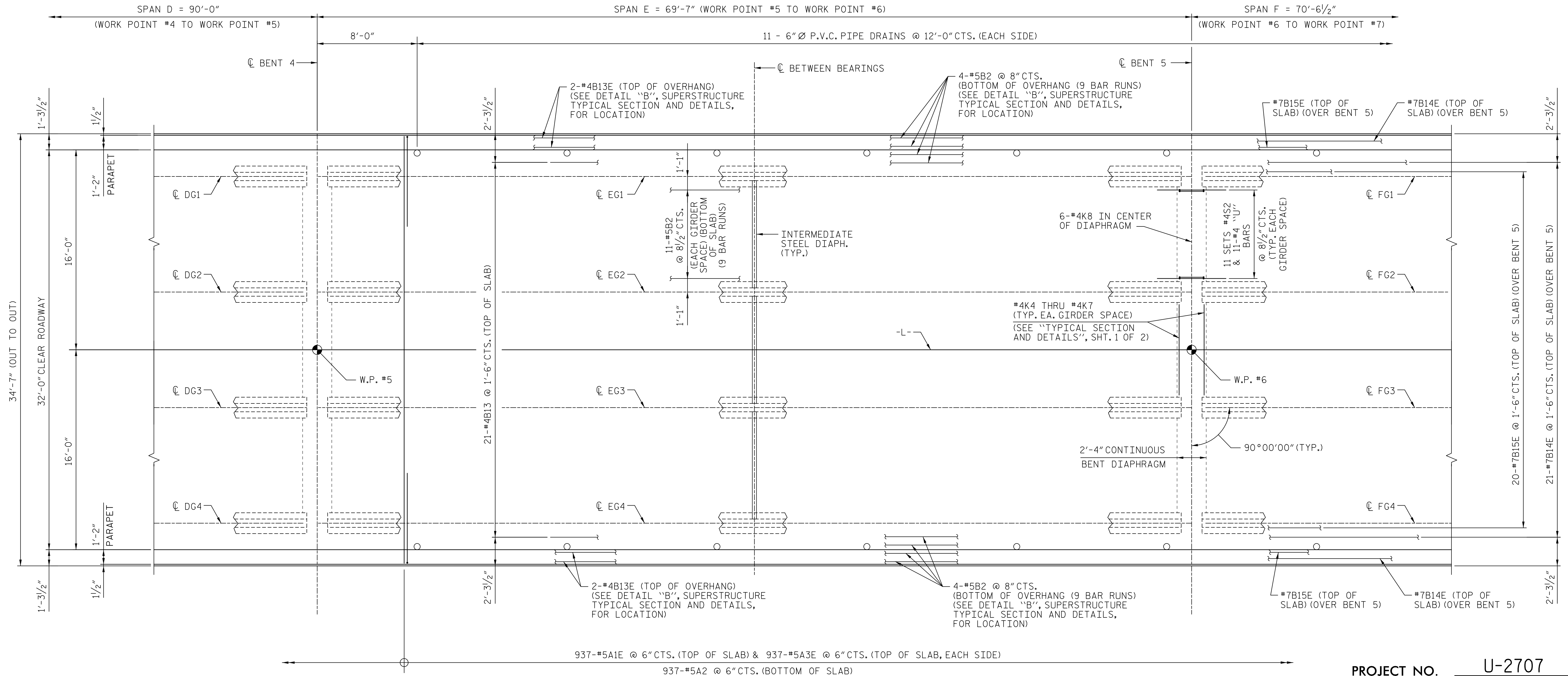
DATE: 11/12/2015 TIME: 9:54:24 AM
USER: jcm\jcm.dgn DGN: R:\Bridges\656\Card\40_019_U2707_SML_S10_P504.dgn

NOTES:

FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.

FOR SECTION "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.

ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.



PLAN OF SPAN E AND PARTIAL PLAN OF SPANS D & F

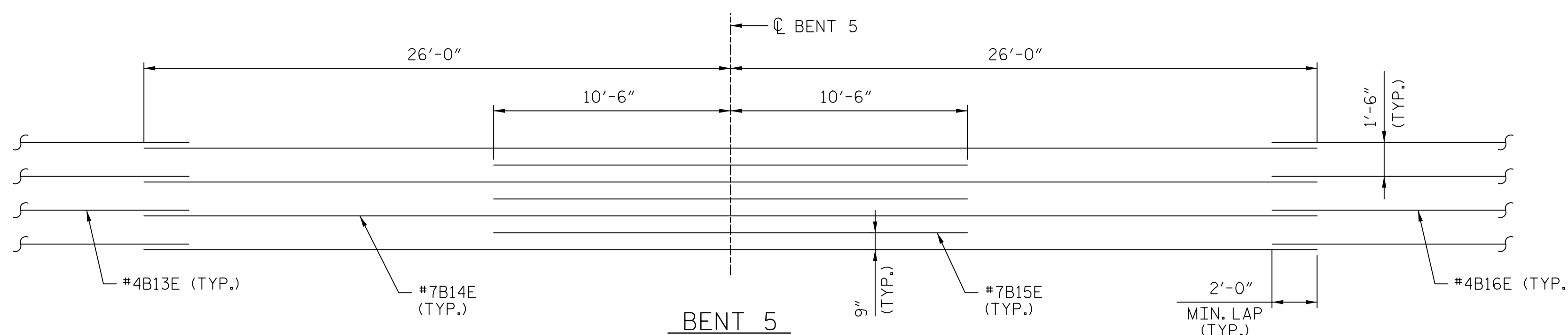
PROJECT NO. U-2707

FORSYTH COUNTY

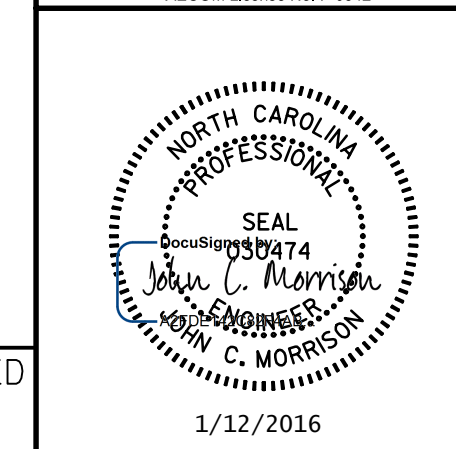
STATION: 90+25.00 -L-

SHEET 5 OF 6

BRIDGE NO. 656



TOP OF SLAB REINFORCEMENT LAYOUT



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

**SUPERSTRUCTURE
PLAN OF SPAN E
AND PARTIAL PLAN
OF SPANS D & F**

| REVISIONS | | | | | | SHEET NO. S-11 |
|-----------|-----|-------|-----|-----|-------|---------------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | |
| | | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: MKT DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

DATE: 11/12/2015
TIME: 9:54:26 AM

USER: \\gis\corpora...
DIR: R:\Bridges\656\Card\40_L02\L02707_SML_S11_P505.dgn

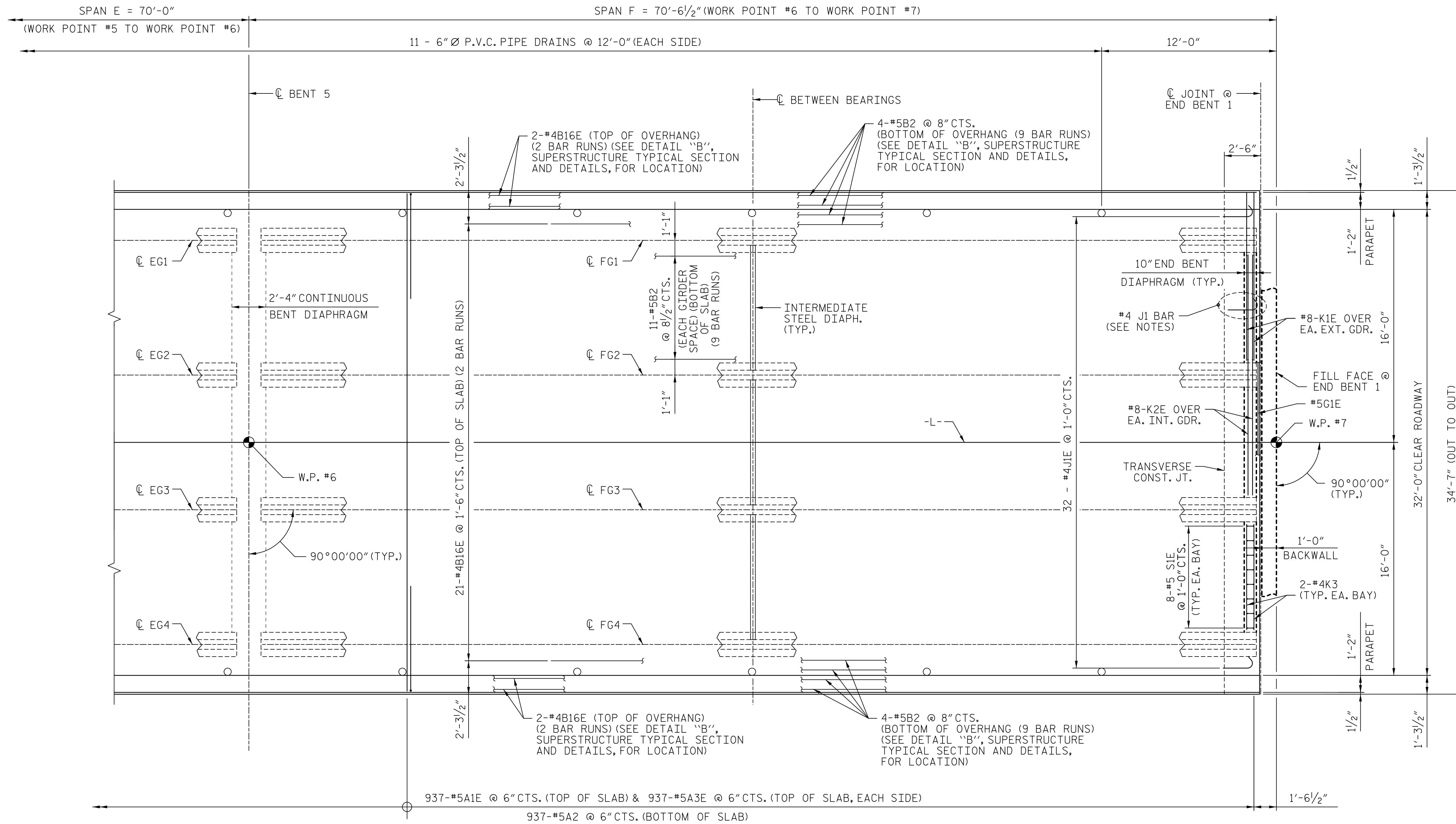
NOTES:

FOR CONCRETE PARAPET RAIL REINFORCING STEEL THAT MUST BE CAST IN DECK SLAB, SEE "CONCRETE PARAPET RAIL" SHEET.

FOR SECTIONS "A-A" AND "B-B", SEE "SUPERSTRUCTURE TYPICAL SECTION AND DETAILS" SHEET.

ANY INCIDENTAL REINFORCING USED IN THE DECK SHALL BE EPOXY COATED.

FOR PLACEMENT OF #4 J1 BARS, SEE "EXPANSION JOINT SEAL DETAILS" SHEET.



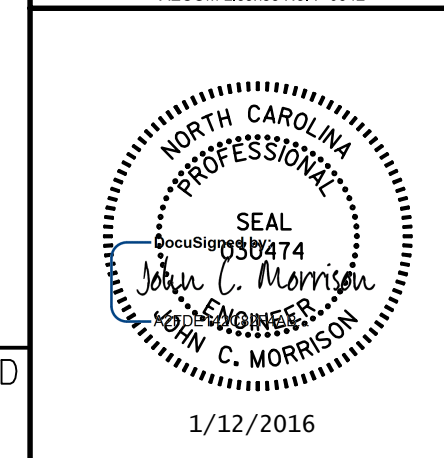
PLAN OF SPAN F AND PARTIAL PLAN OF SPAN E

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 6 OF 6 BRIDGE NO. 656



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
PLAN OF SPAN F
AND PARTIAL PLAN
OF SPAN E

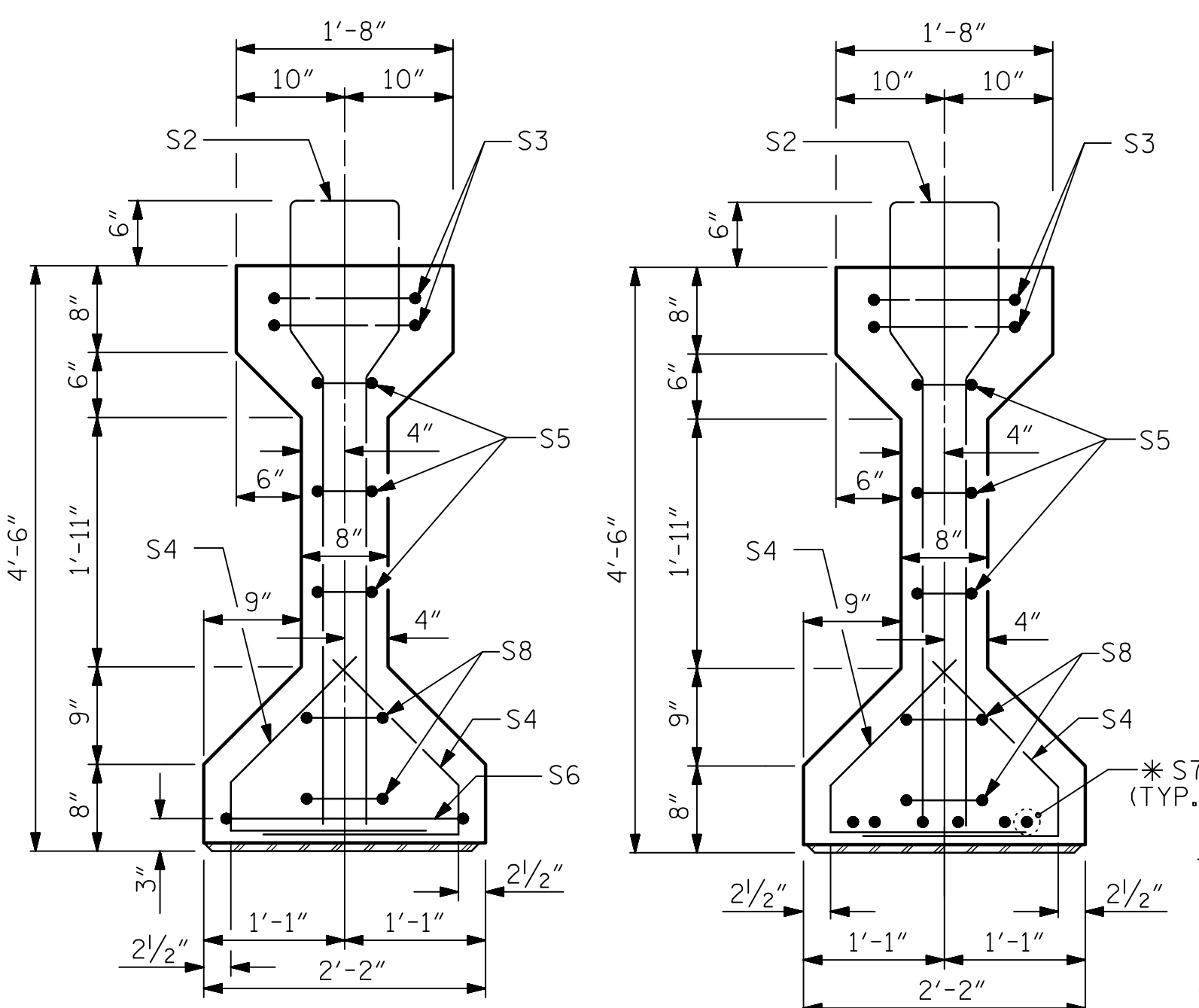
| REVISIONS | | | | | | SHEET NO. S-12 TOTAL SHEETS 86 |
|-----------|-----|-------|-----|-----|-------|-----------------------------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | |
| | | | | | | |

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

DRAWN BY : MKT DATE : 11/2015
CHECKED BY : JCM DATE : 11/2015

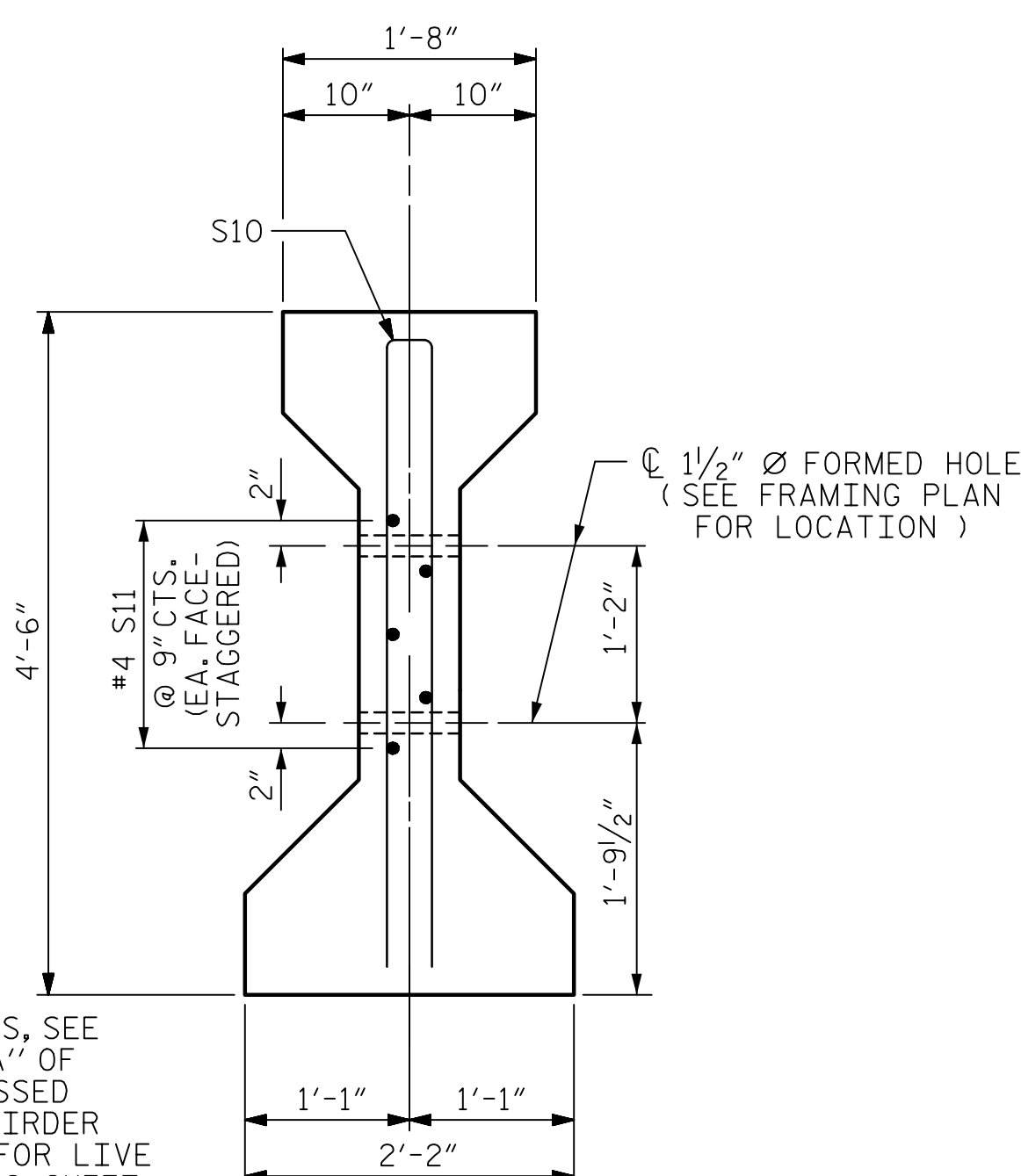
DATE: 11/12/2015
TIME: 9:54:33 AM

USER: \\gis\corpora...
DIR: R:\Bridges\656\Coord\40...
FILE: R:\Bridges\656\Coord\40..._L2707_SML_S12_P506.dgn



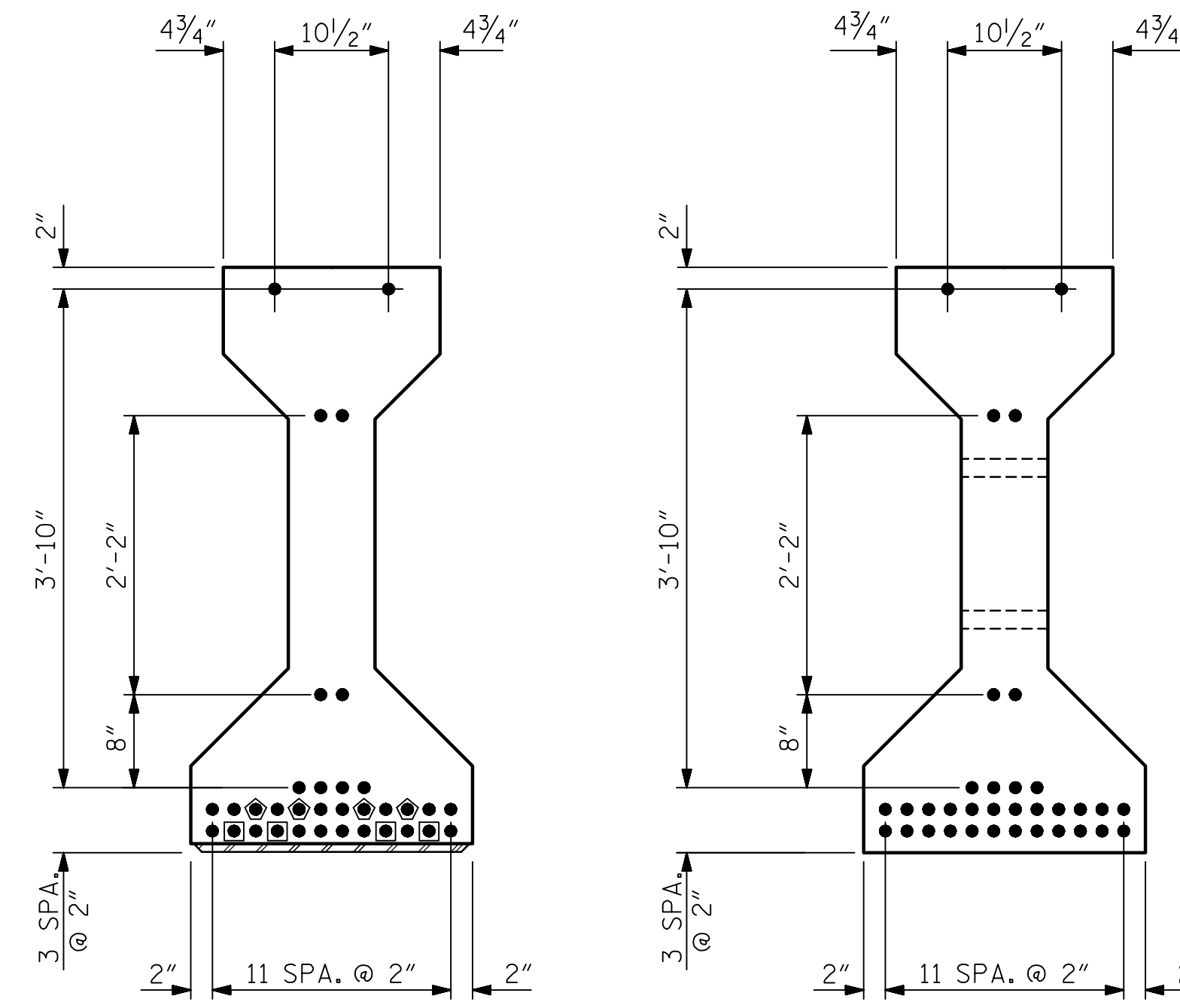
SECTION A-A

SECTION B-B



SECTION C-C
(S1 BARS NOT SHOWN)

* FOR S7 BARS, SEE
DETAIL "A" OF
PRESTRESSED
CONCRETE GIRDER
CONTINUOUS FOR LIVE
LOAD DETAILS SHEET



AT END OF GIRDER

AT C OF GIRDER

0.6" Ø LOW RELAXATION STRAND LAYOUT

- FULLY BONDED STRANDS
- STRANDS DEBONDED FOR 10'-0" FROM END OF GIRDER
- STRANDS DEBONDED FOR 8'-0" FROM END OF GIRDER

0.6" Ø L. R. GRADE 270 STRANDS

| AREA (SQUARE INCHES) | ULTIMATE STRENGTH (LBS. PER STRAND) | APPLIED PRESTRESS (LBS. PER STRAND) |
|-------------------------|-------------------------------------------|-------------------------------------------|
| 0.217 | 58,600 | 43,950 |

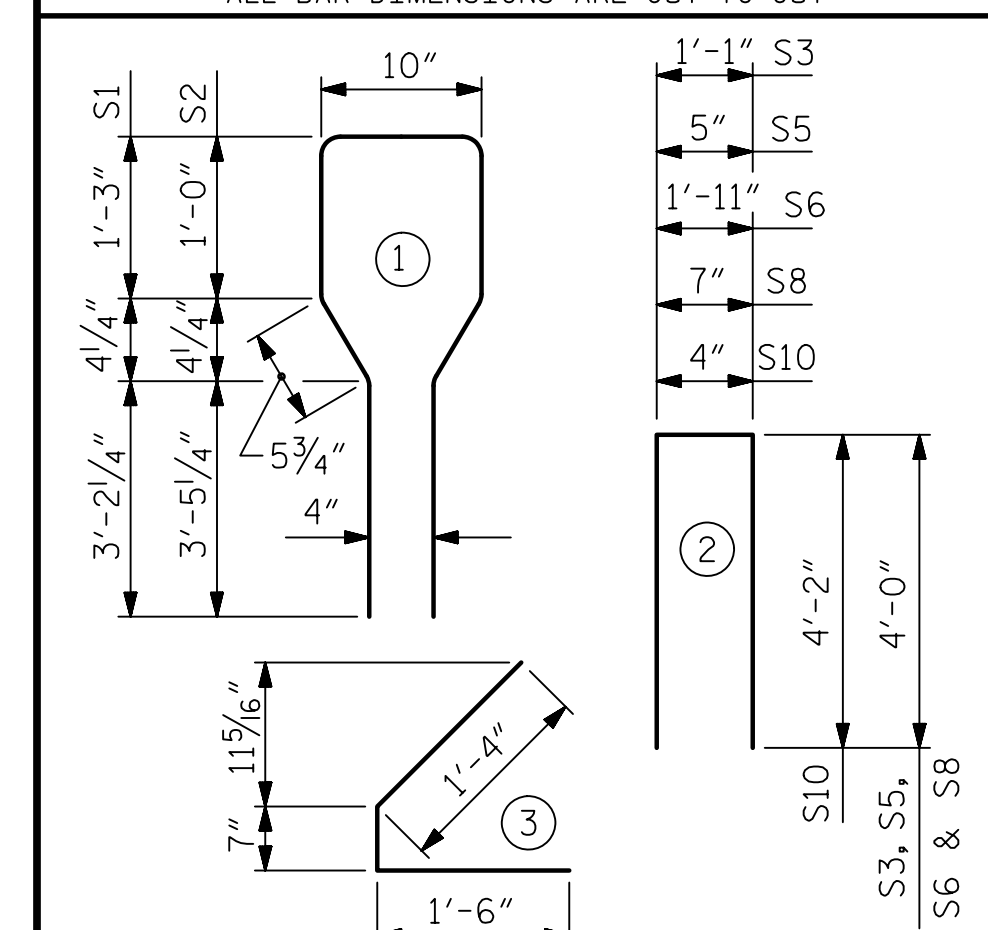
REINFORCING STEEL FOR ONE GIRDER

| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT |
|------|--------|------|------|--------|--------|
| S1 | 70 | #4 | 1 | 10'-8" | 499 |
| S2 | 12 | #6 | 1 | 10'-8" | 192 |
| S3 | 4 | #4 | 2 | 9'-1" | 24 |
| S4 | 64 | #4 | 3 | 3'-5" | 146 |
| S5 | 6 | #4 | 2 | 8'-5" | 34 |
| S6 | 1 | #4 | 2 | 9'-11" | 7 |
| * S7 | 6 | #5 | STR | 3'-8" | 23 |
| S8 | 4 | #4 | 2 | 8'-7" | 23 |
| S9 | 1 | #3 | STR | 1'-10" | 1 |
| S10 | 2 | #5 | 2 | 8'-8" | 18 |
| S11 | 5 | #4 | STR | 7'-0" | 23 |

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

BAR TYPES

ALL BAR DIMENSIONS ARE OUT-TO-OUT

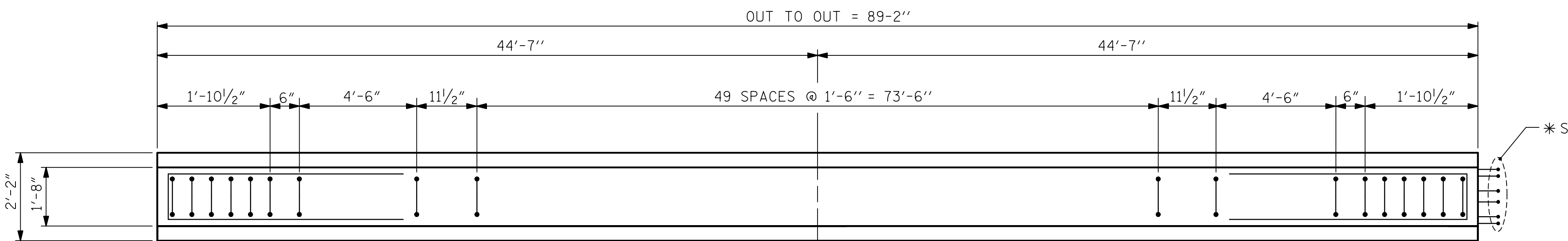


QUANTITIES FOR ONE GIRDER

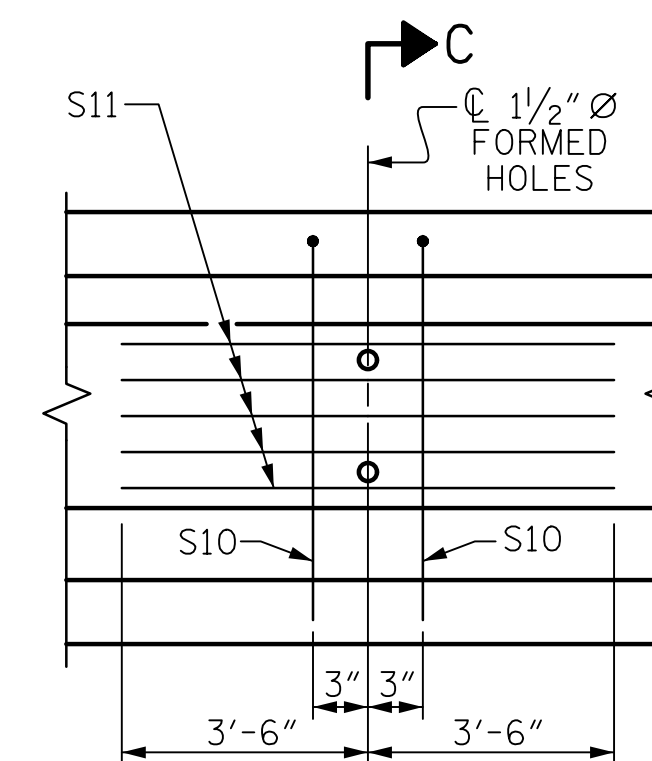
| EXT. & INT. GDERS | REINFORCING STEEL | 7000 PSI CONCRETE | 0.6" Ø L. R. STRANDS |
|-------------------|-------------------|-------------------|----------------------|
| | LB. | C.Y. | No. |
| | 990 | 18.1 | 34 |

GIRDERS REQUIRED

| NUMBER | LENGTH | TOTAL LENGTH |
|--------|--------|--------------|
| 4 | 89'-2" | 356'-8" |

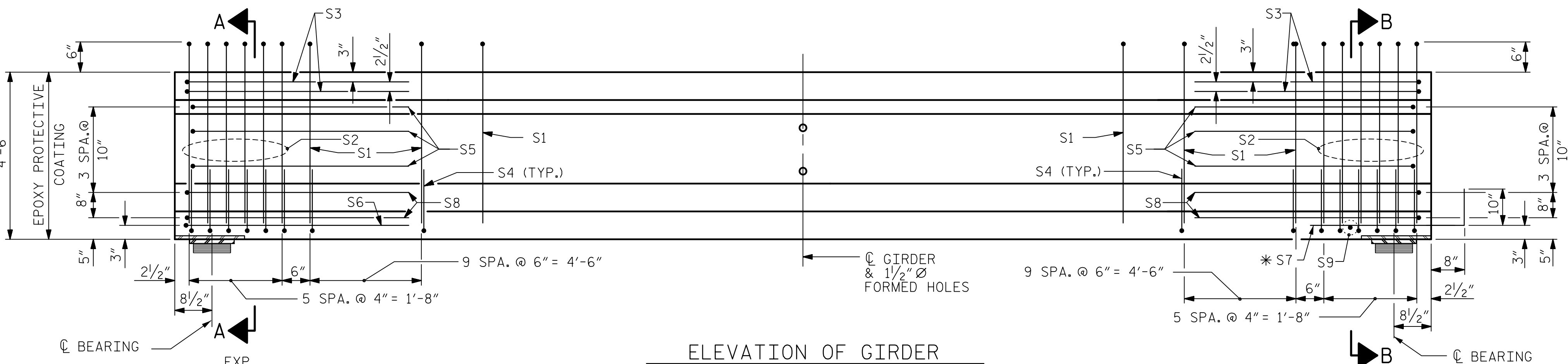


PLAN OF GIRDER



PARTIAL ELEVATION

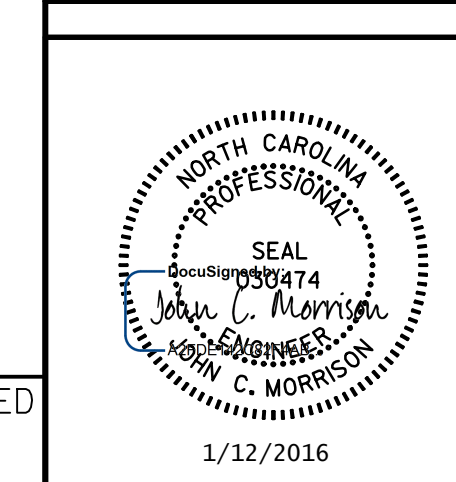
SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. 1,2,3 & 4



ELEVATION OF GIRDER

(SEE PARTIAL ELEVATION FOR ADDITIONAL "S" BARS)

| | |
|-----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 10/2015 |
| CHECKED BY : JCM | DATE : 10/2015 |
| DRAWN BY : ELR 8/91 | REV. 5/1/06R TLA/GM |
| CHECKED BY : GRP 8/91 | REV. 10/1/11 MAA/GM |
| | REV. 1/15 MAA/TMG |

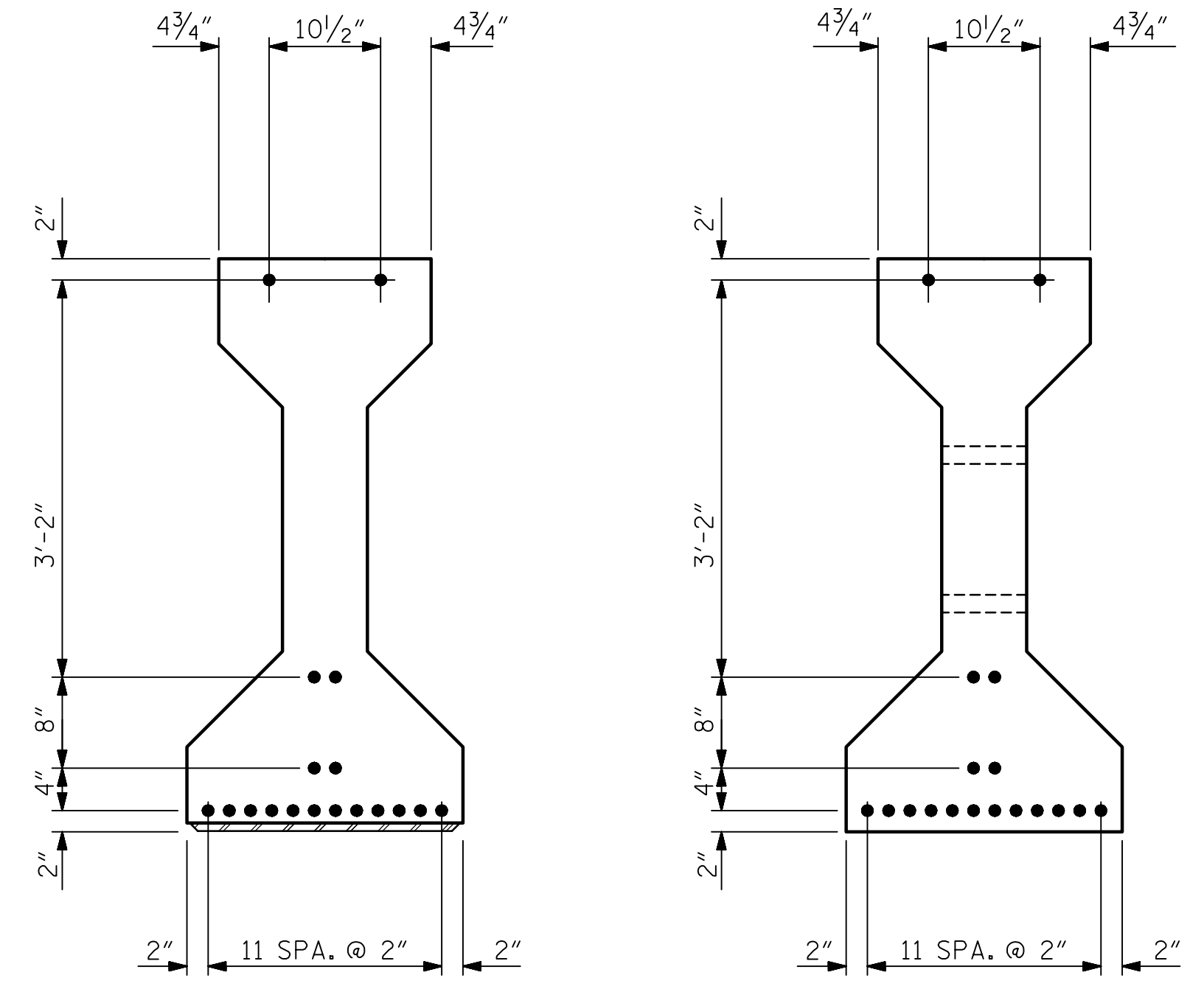
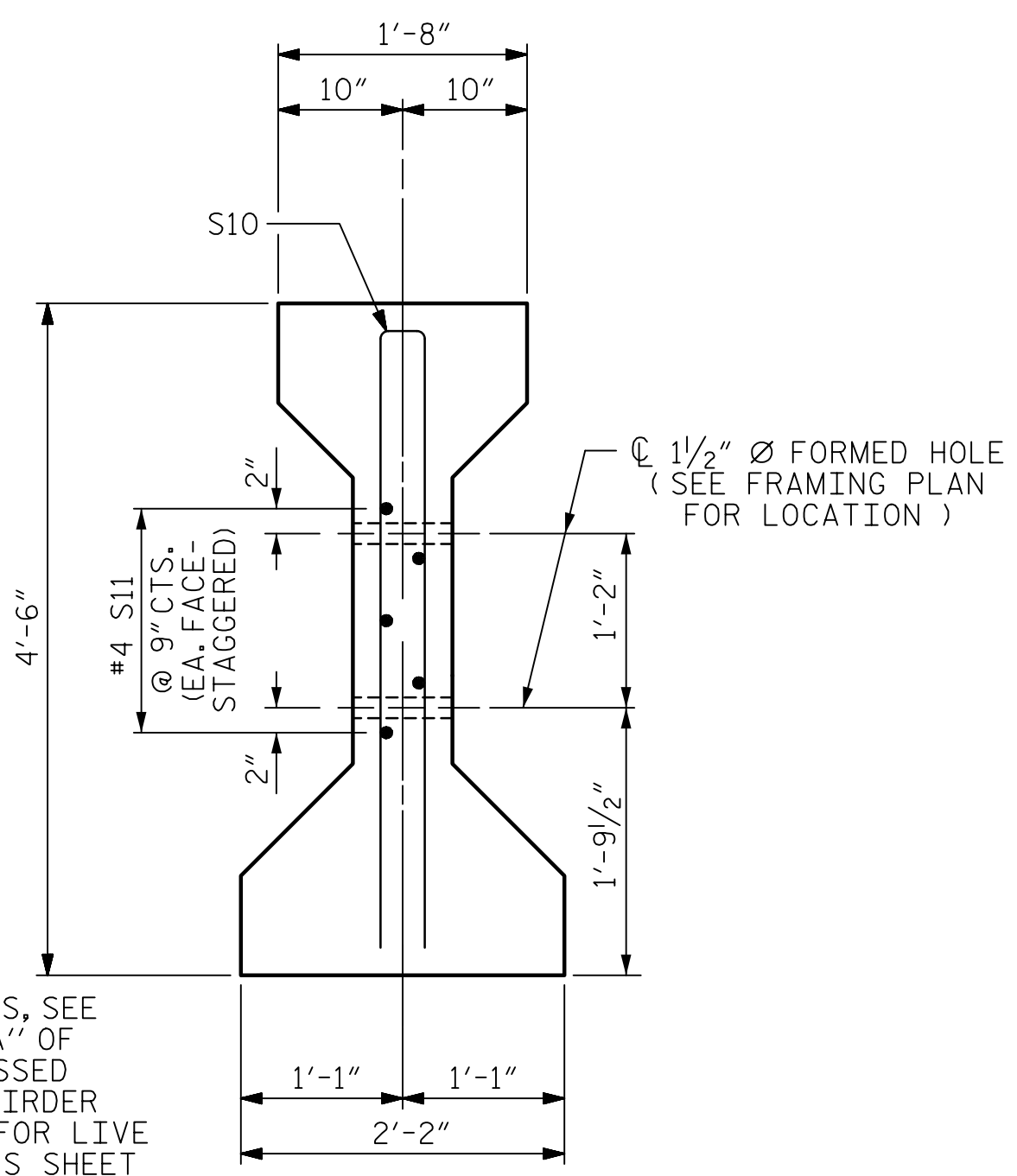
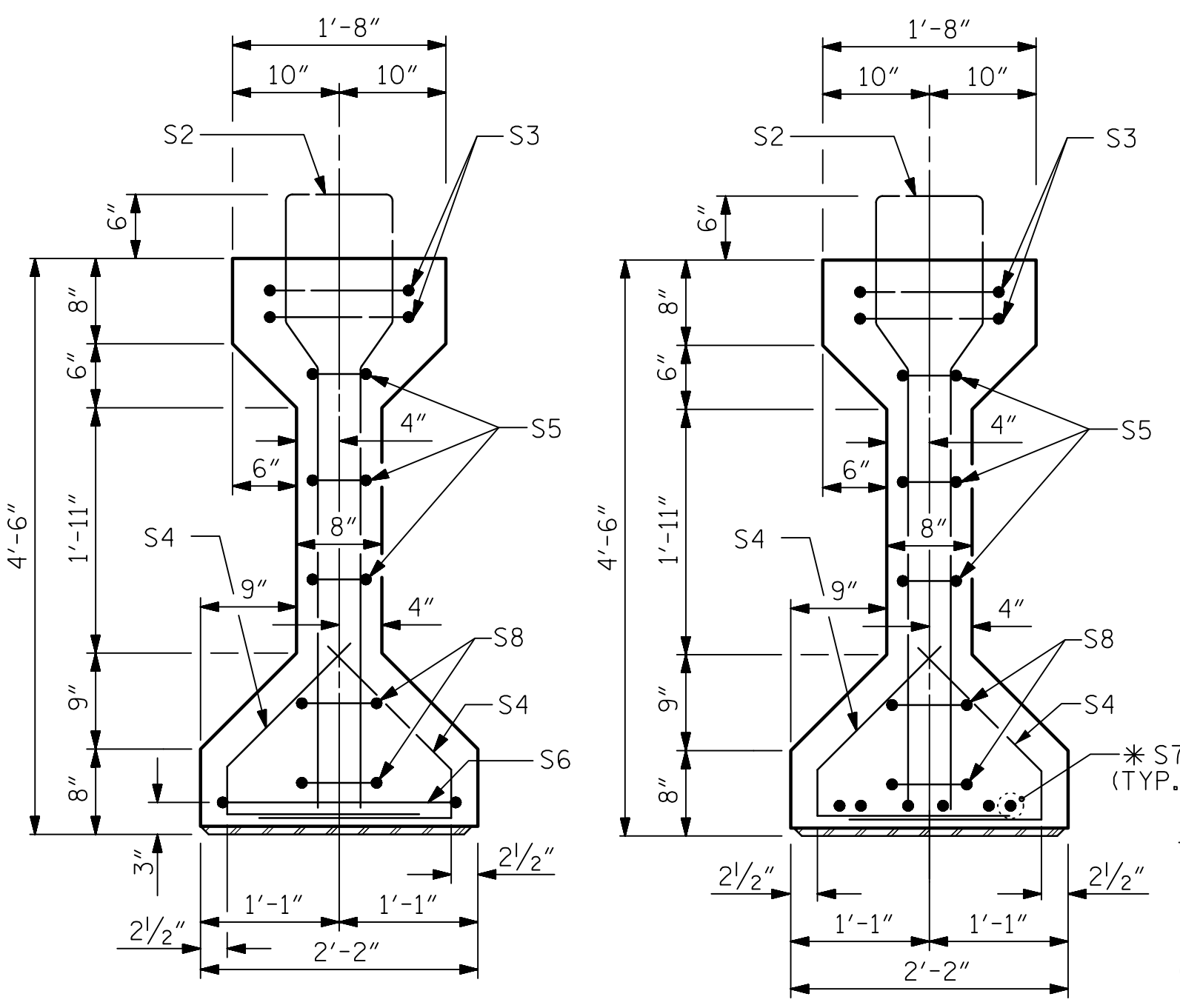


PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 1 OF 7 BRIDGE NO. 656

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 AASHTO TYPE IV
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 SPAN A

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-14 |
| 1 | | | 3 | | | TOTAL SHEETS |
| 2 | | | 4 | | | 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

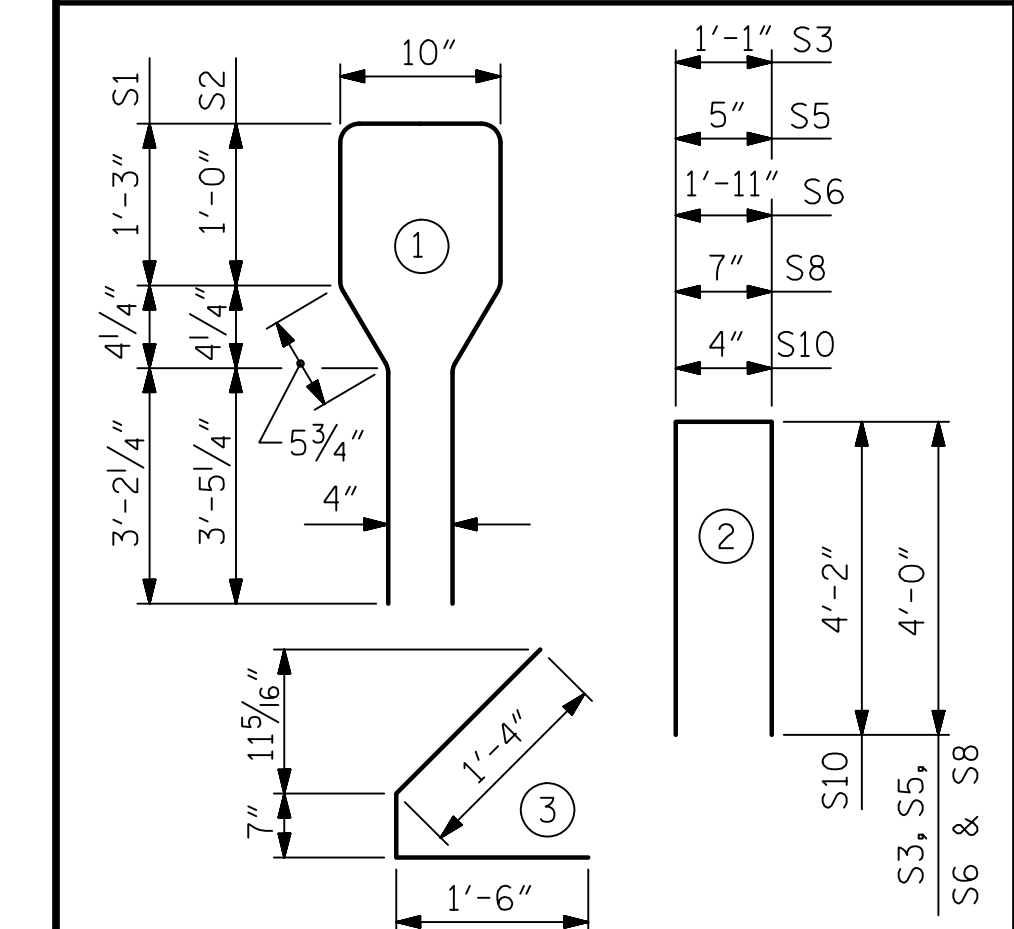


| 0.6" Ø L. R. GRADE 270 STRANDS | | |
|--------------------------------|----------------------------------------|----------------------------------------|
| AREA (SQUARE INCHES) | ULTIMATE STRENGTH (LBS. PER STRAND) | APPLIED PRESTRESS (LBS. PER STRAND) |
| 0.217 | 58,600 | 43,950 |

| REINFORCING STEEL FOR ONE GIRDER | | | | | |
|----------------------------------|--------|------|------|--------|--------|
| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT |
| S1 | 56 | #4 | 1 | 10'-8" | 399 |
| S2 | 12 | #6 | 1 | 10'-8" | 192 |
| S3 | 4 | #4 | 2 | 9'-1" | 24 |
| S4 | 64 | #4 | 3 | 3'-5" | 146 |
| S5 | 6 | #4 | 2 | 8'-5" | 34 |
| S6 | 1 | #4 | 2 | 9'-11" | 7 |
| * S7 | 6 | #5 | STR | 3'-8" | 23 |
| S8 | 4 | #4 | 2 | 8'-7" | 23 |
| S9 | 1 | #3 | STR | 1'-10" | 1 |
| S10 | 2 | #5 | 2 | 8'-8" | 18 |
| S11 | 5 | #4 | STR | 7'-0" | 23 |

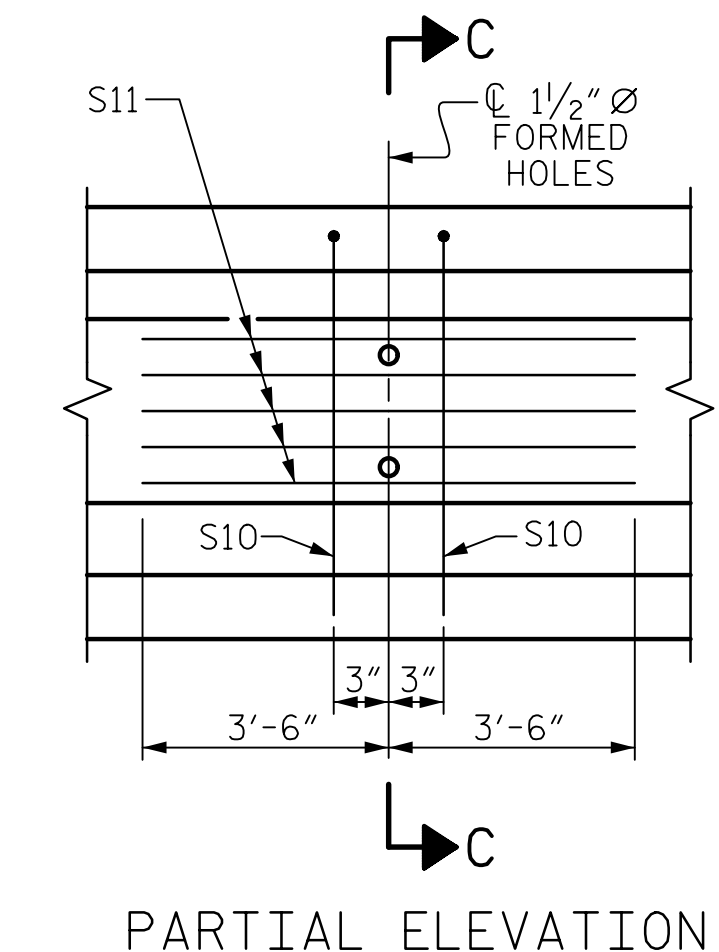
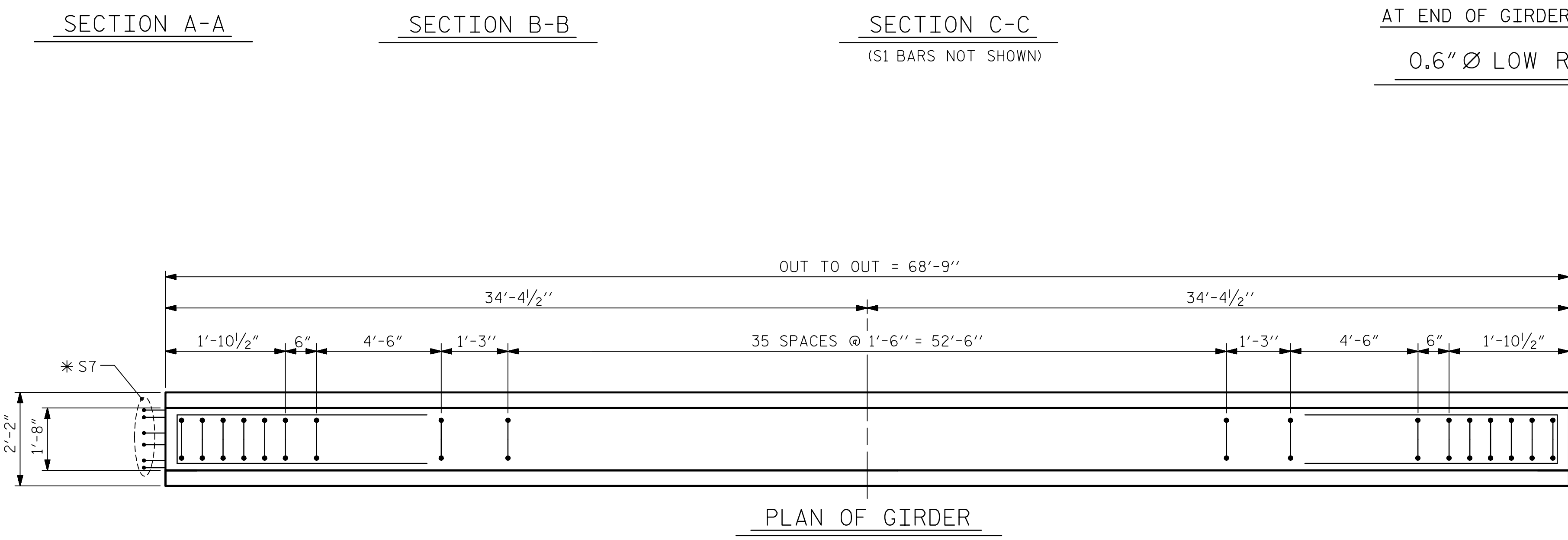
* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

BAR TYPES
ALL BAR DIMENSIONS ARE OUT-TO-OUT

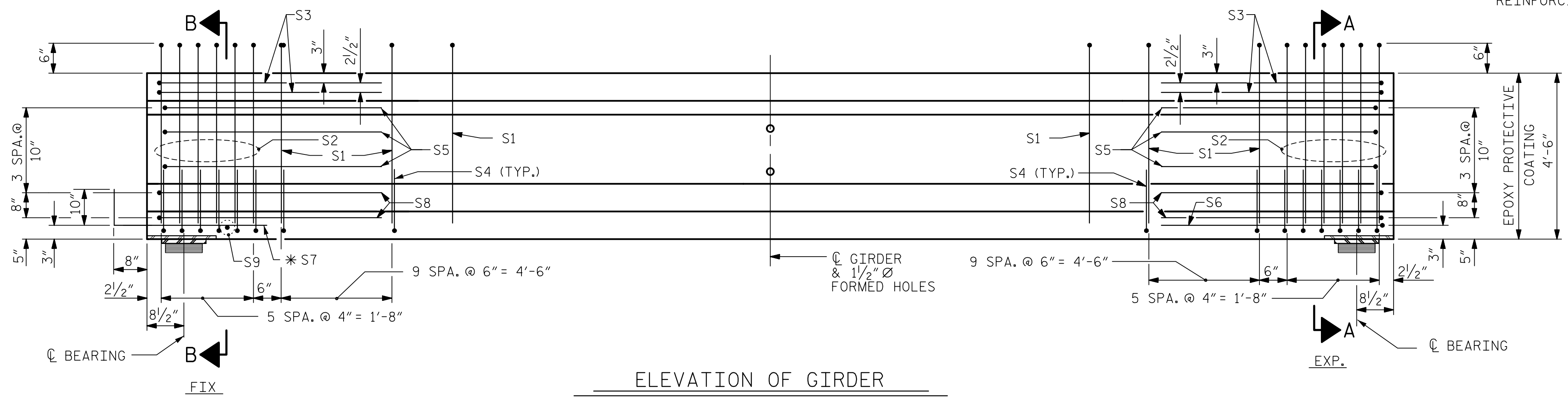


| QUANTITIES FOR ONE GIRDER | | | |
|---------------------------|--------------------------|---------------------------|-----------------------------|
| | REINFORCING STEEL LB. | 6000 PSI CONCRETE C.Y. | 0.6" Ø L. R. STRANDS No. |
| EXT. & INT. GDERS | 890 | 14.0 | 18 |

| GIRDERS REQUIRED | | |
|------------------|--------|--------------|
| NUMBER | LENGTH | TOTAL LENGTH |
| 4 | 68'-9" | 275'-0" |



SHOWING INTERMEDIATE DIAPHRAGM REINFORCING STEEL FOR GIRDER Nos. 1,2,3 & 4



(SEE PARTIAL ELEVATION FOR ADDITIONAL "S" BARS)

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 5 OF 7 BRIDGE NO. 656

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-4200 www.aecom.com
 AECOM License No. F-0352

SEAL
 JOHN C. MORRISON
 1/12/2016

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 AASHTO TYPE IV
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 SPAN F

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. **S-18**
 TOTAL SHEETS **86**

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016 TIME: 9:54:49 AM
 USER: N:\a\compton DON: R:\Bridge\656\Card\40_035_U2707_S18_P005.dgn

| | |
|-----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 10/2015 |
| CHECKED BY : JCM | DATE : 10/2015 |
| DRAWN BY : ELR 8/91 | REV. 5/1/06R TLA/GM |
| CHECKED BY : GRP 8/91 | REV. 10/1/11 MAA/GM |
| | REV. 1/15 MAA/TMG |

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

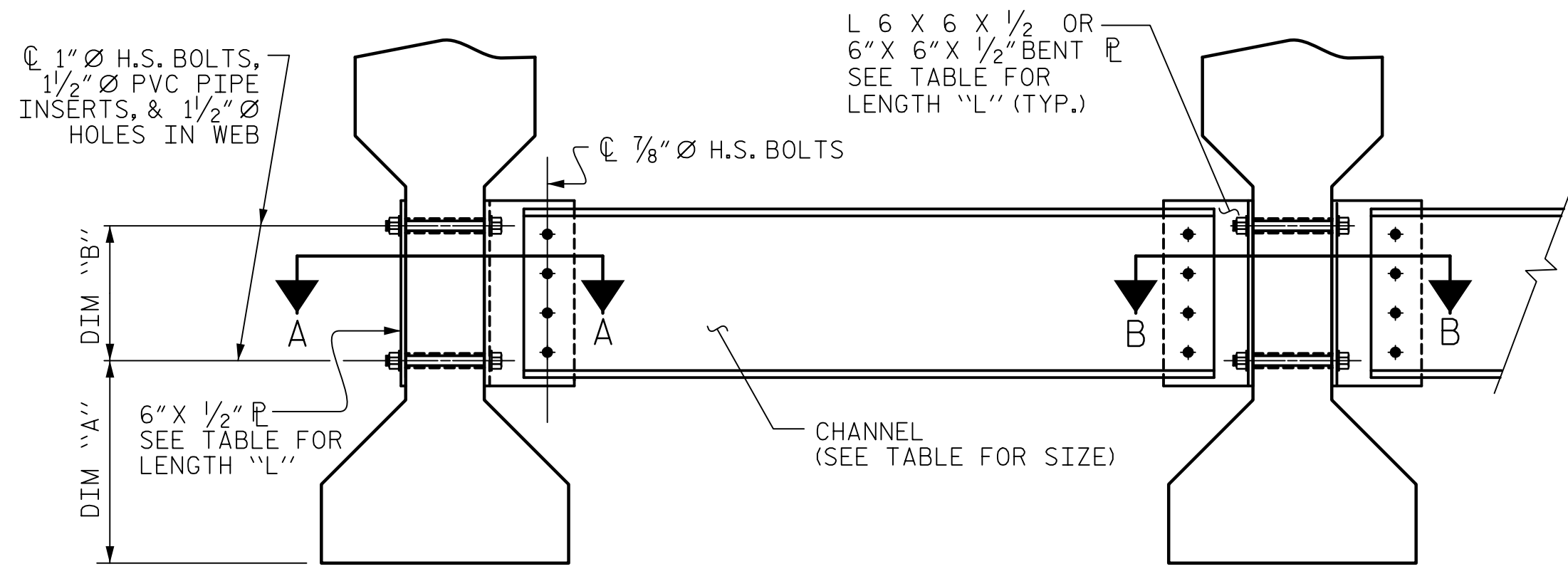
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

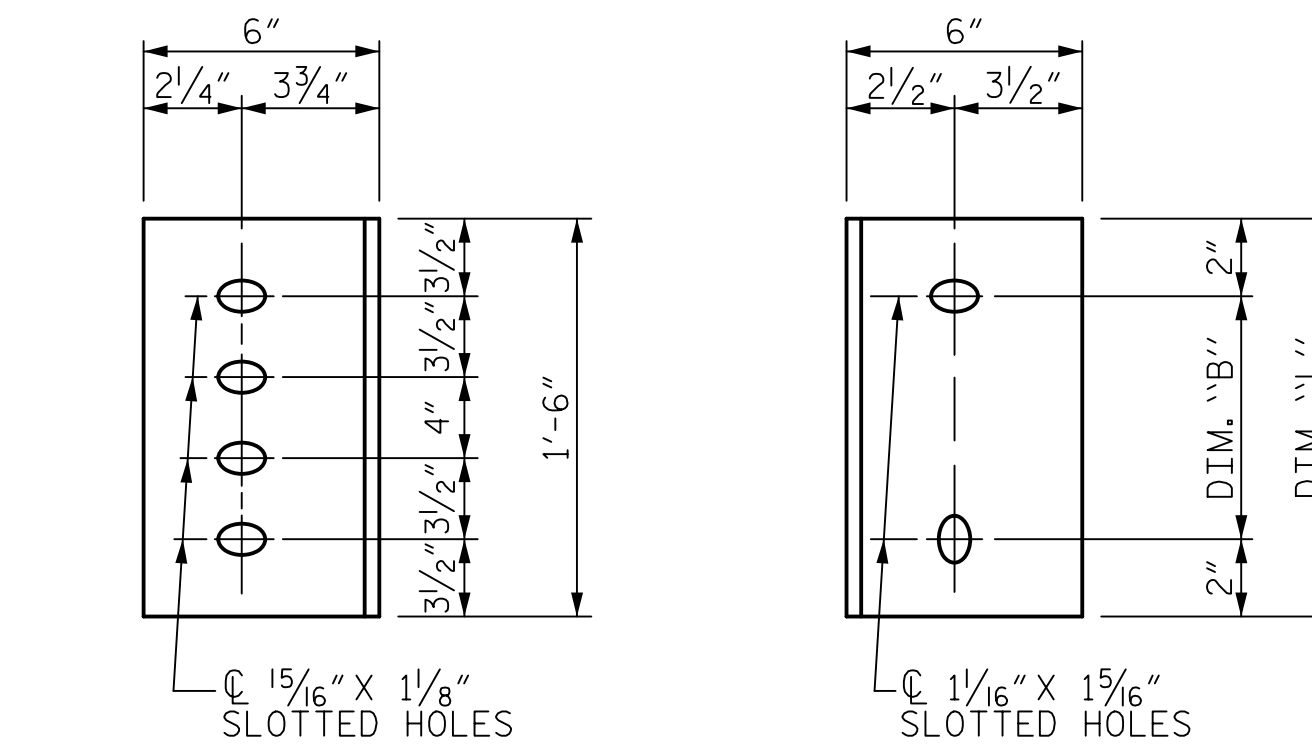
SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



EXTERIOR GIRDER **INTERIOR GIRDER**
PART SECTION AT INTERMEDIATE DIAPHRAGM
 (TYPE IV GIRDER SHOWN)



DIAPHRAGM FACE
 (TYPE III OR TYPE IV GDR.)
CONNECTOR PLATE DETAILS
WEB FACE

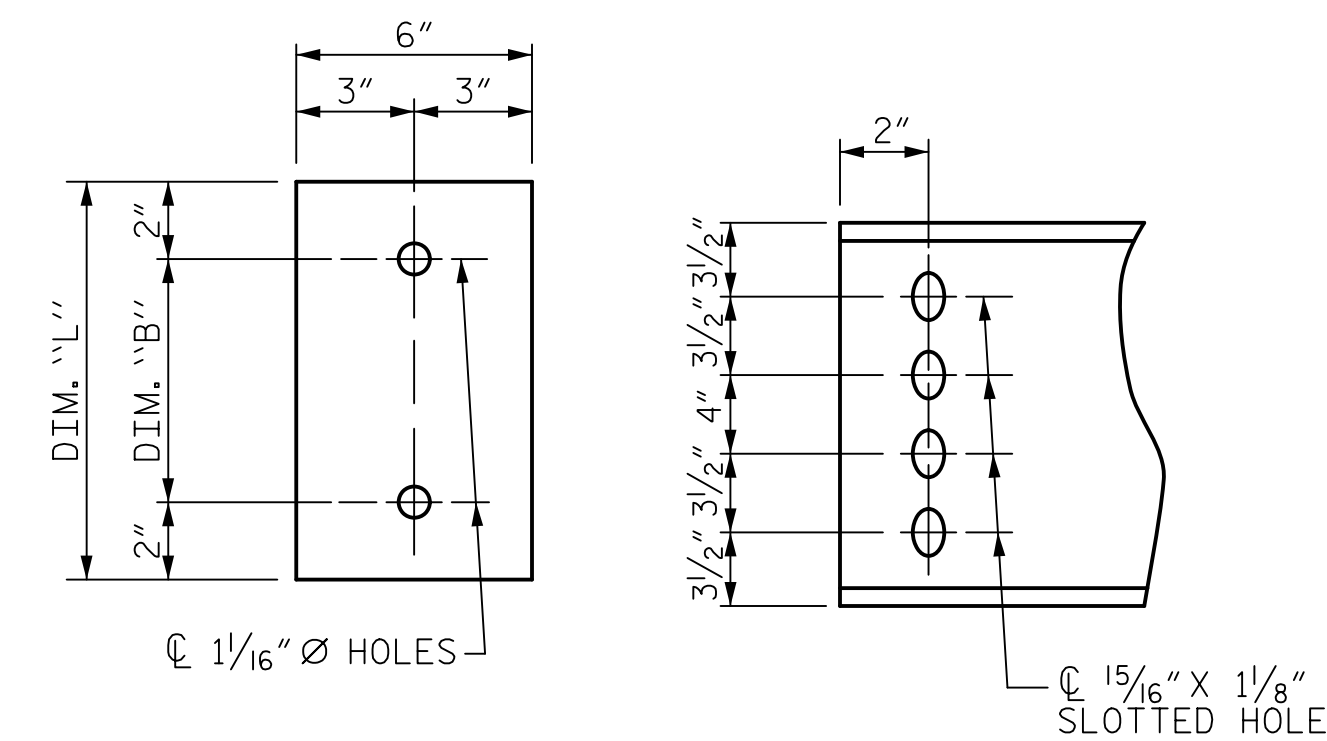
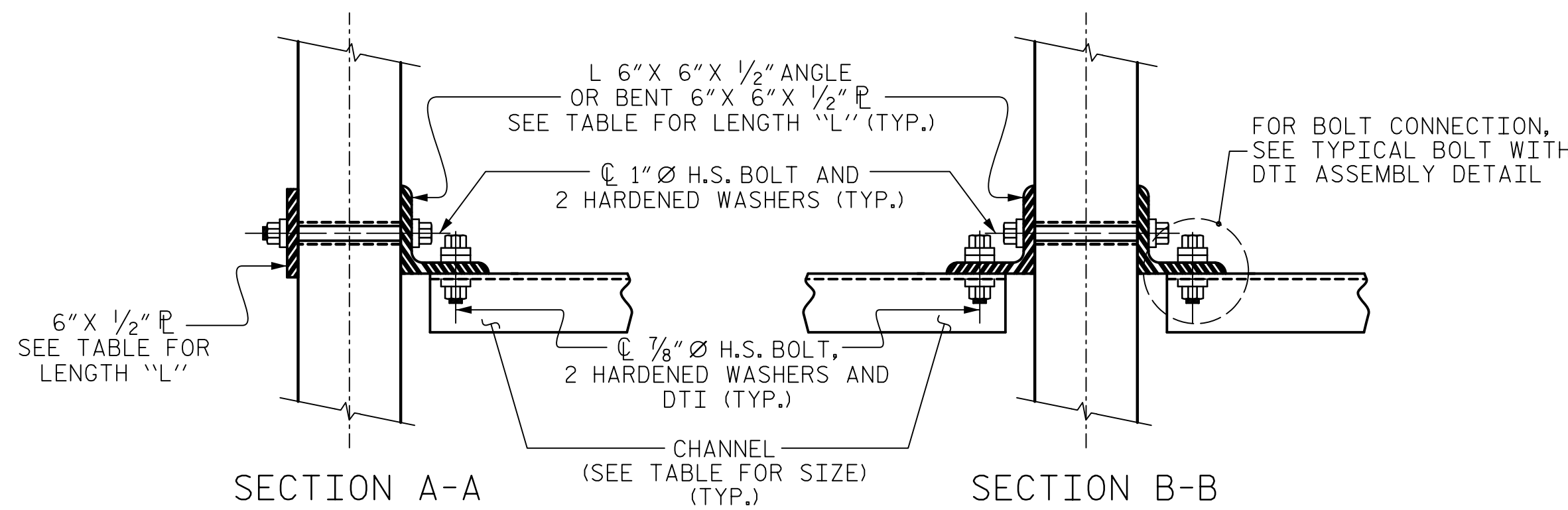
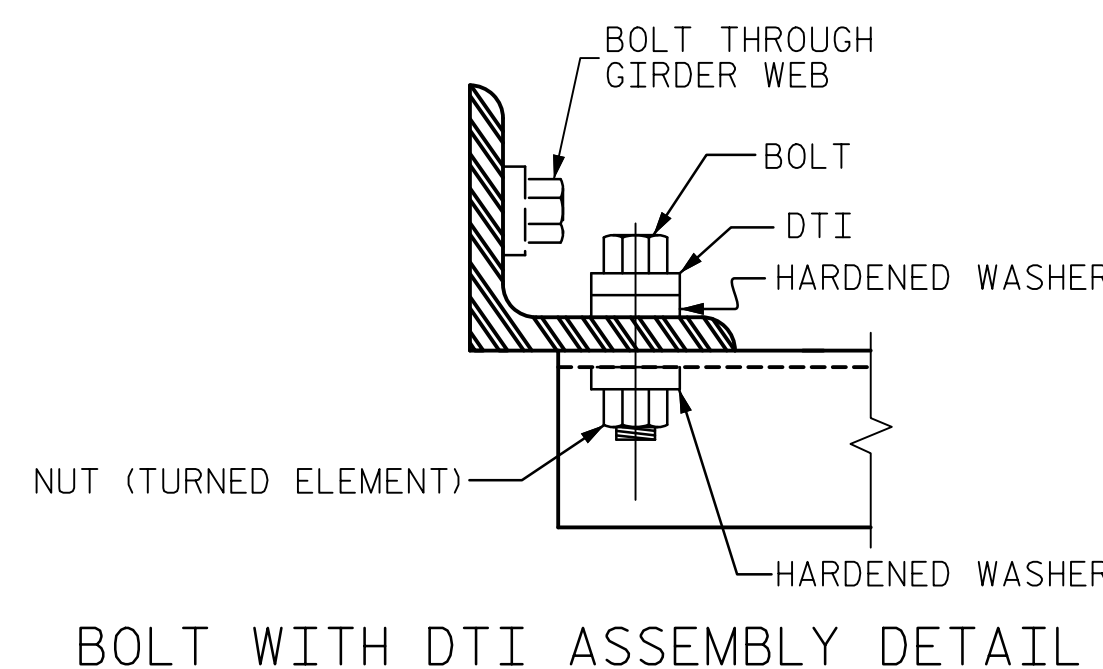


PLATE DETAILS **CHANNEL END**
 (TYPE IV GDR.)



SECTION A-A **SECTION B-B**
CONNECTION DETAILS
 (FOR SKEW = 90°)



BOLT WITH DTI ASSEMBLY DETAIL

TABLE

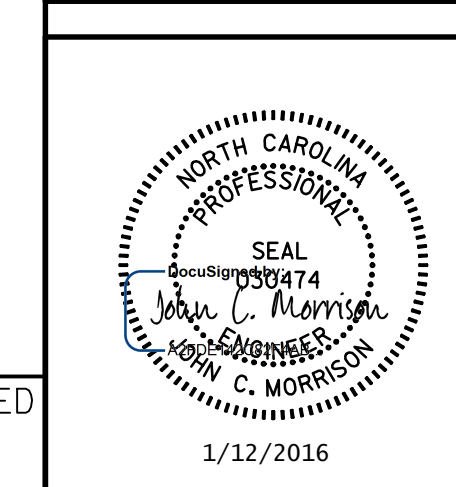
| GIRDER TYPE | CHANNEL SIZE | DIM "A" | DIM "B" | DIM "L" |
|-------------|--------------|-----------|---------|---------|
| IV | MC 18 x 42.7 | 1'-9 1/2" | 1'-2" | 1'-6" |

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 7 OF 7 BRIDGE NO. 656



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
STANDARD
INTERMEDIATE
STEEL DIAPHRAGMS
FOR TYPE II, III, & IV
PRESTRESSED CONCRETE
GIRDERS

REVISIONS

| NO. | BY: | DATE: | NO. | BY: | DATE: |
|-----|-----|-------|-----|-----|-------|
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO.
S-20
 TOTAL SHEETS
86

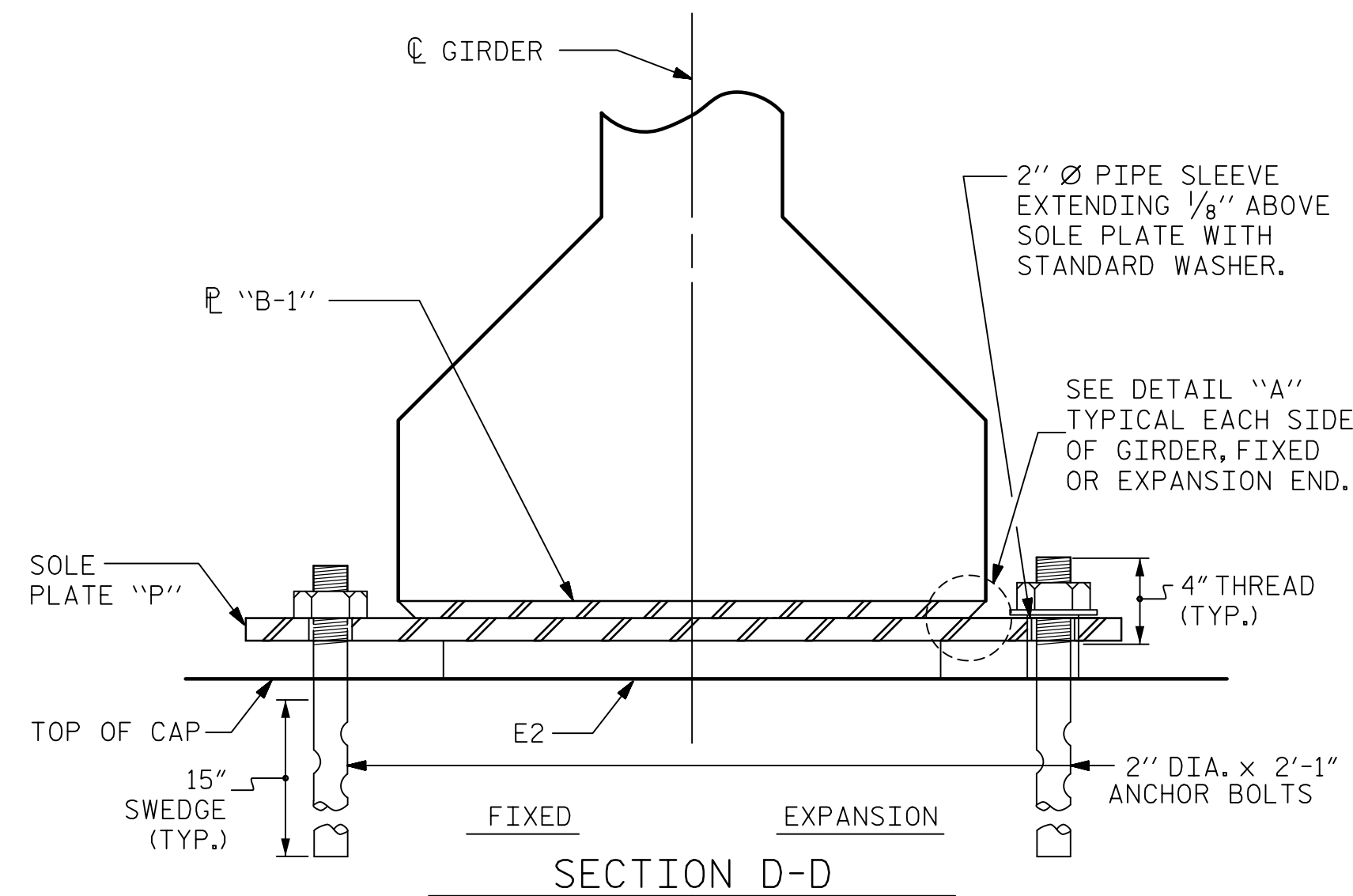
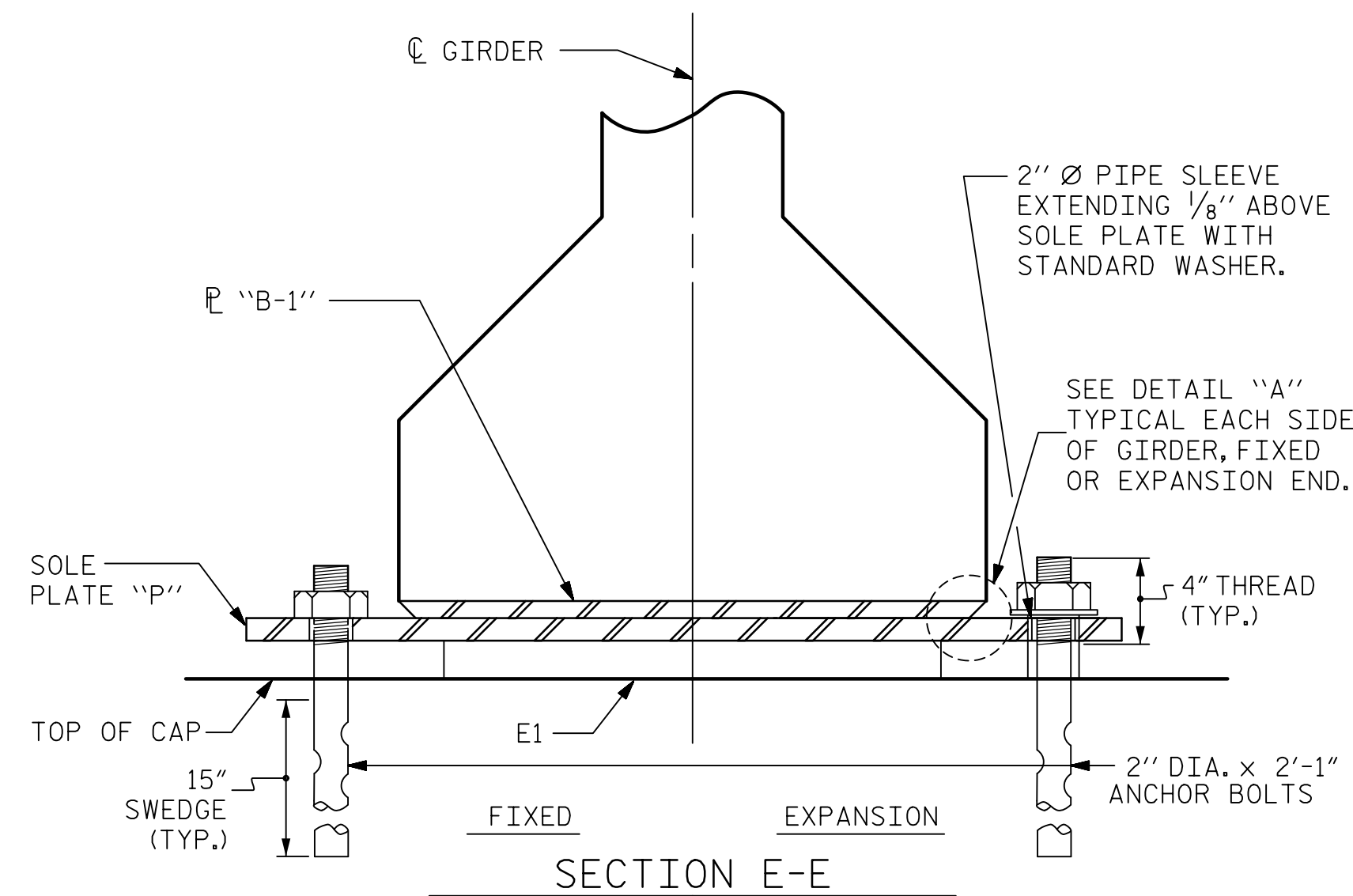
DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

1/12/2016

DATE: 1/12/2016
 TIME: 9:54:55 AM

USER: \\s01pcg10\656\cond\40_039_112707_SML_S20_P0007.dgn
 DGN: R:\Bridg\656\cond\40_039_112707_SML_S20_P0007.dgn

| | |
|----------------------|-----------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : TLA 6/05 | ADDED 10/21/05 |
| CHECKED BY : VC 6/05 | REV. 5/1/06RRR KMM/GM |
| | REV. 10/1/11 MAA/GM |



NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2"Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

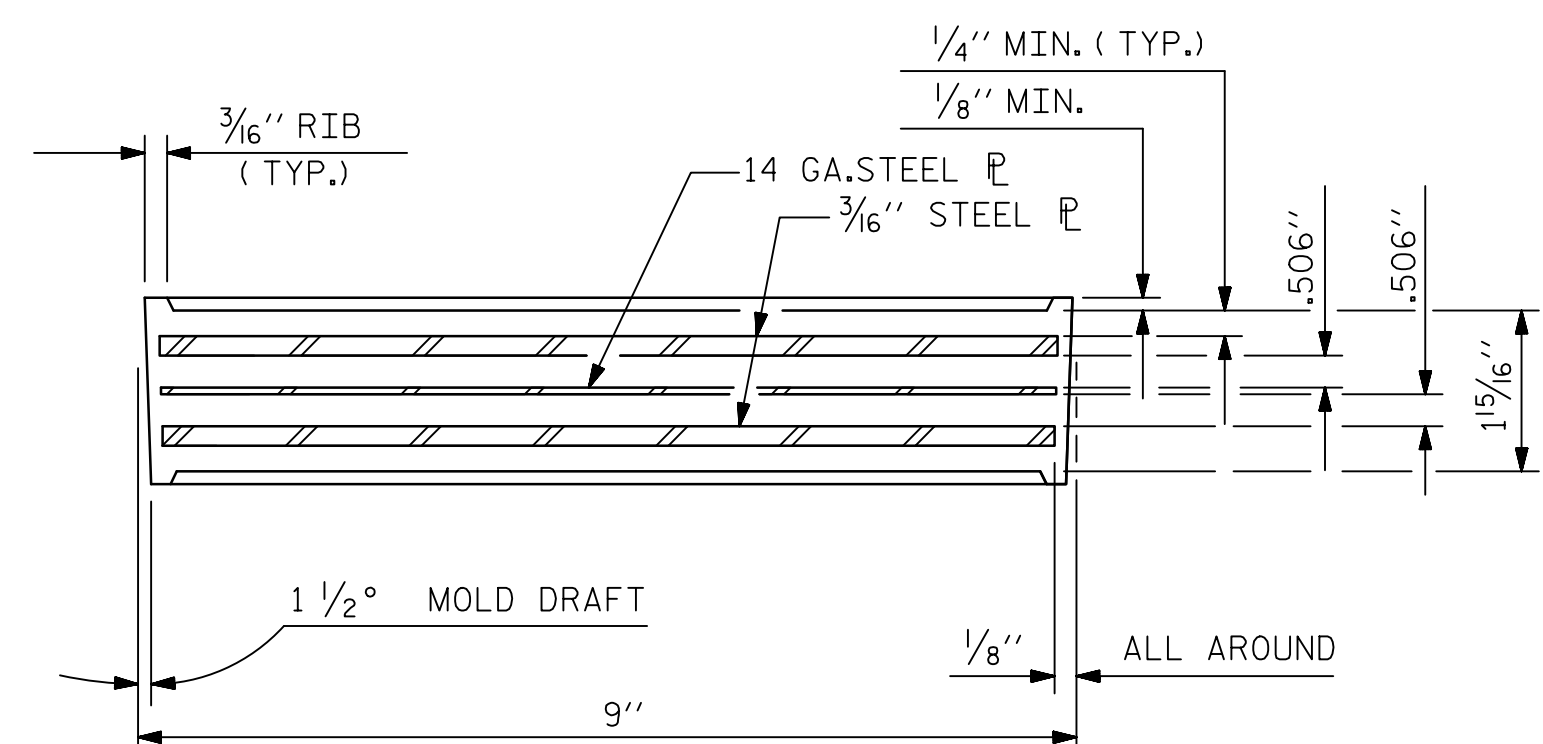
SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

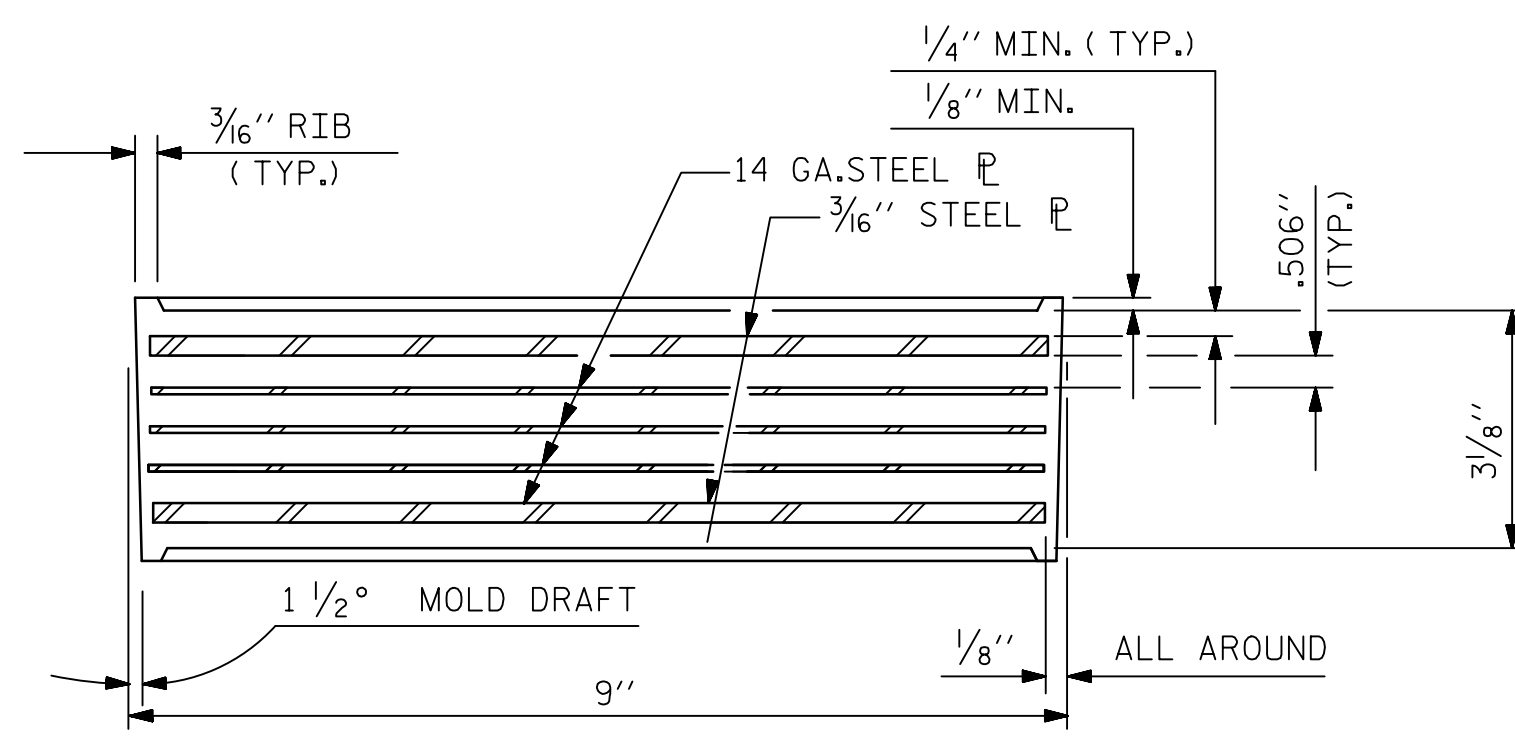
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

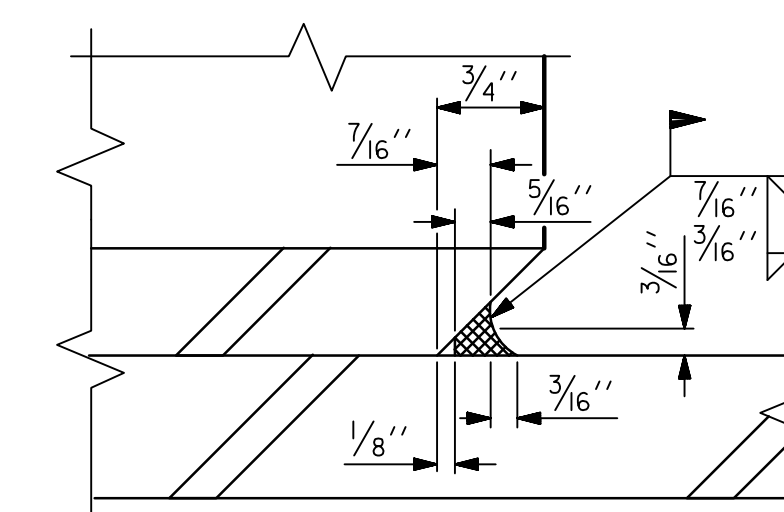
FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.



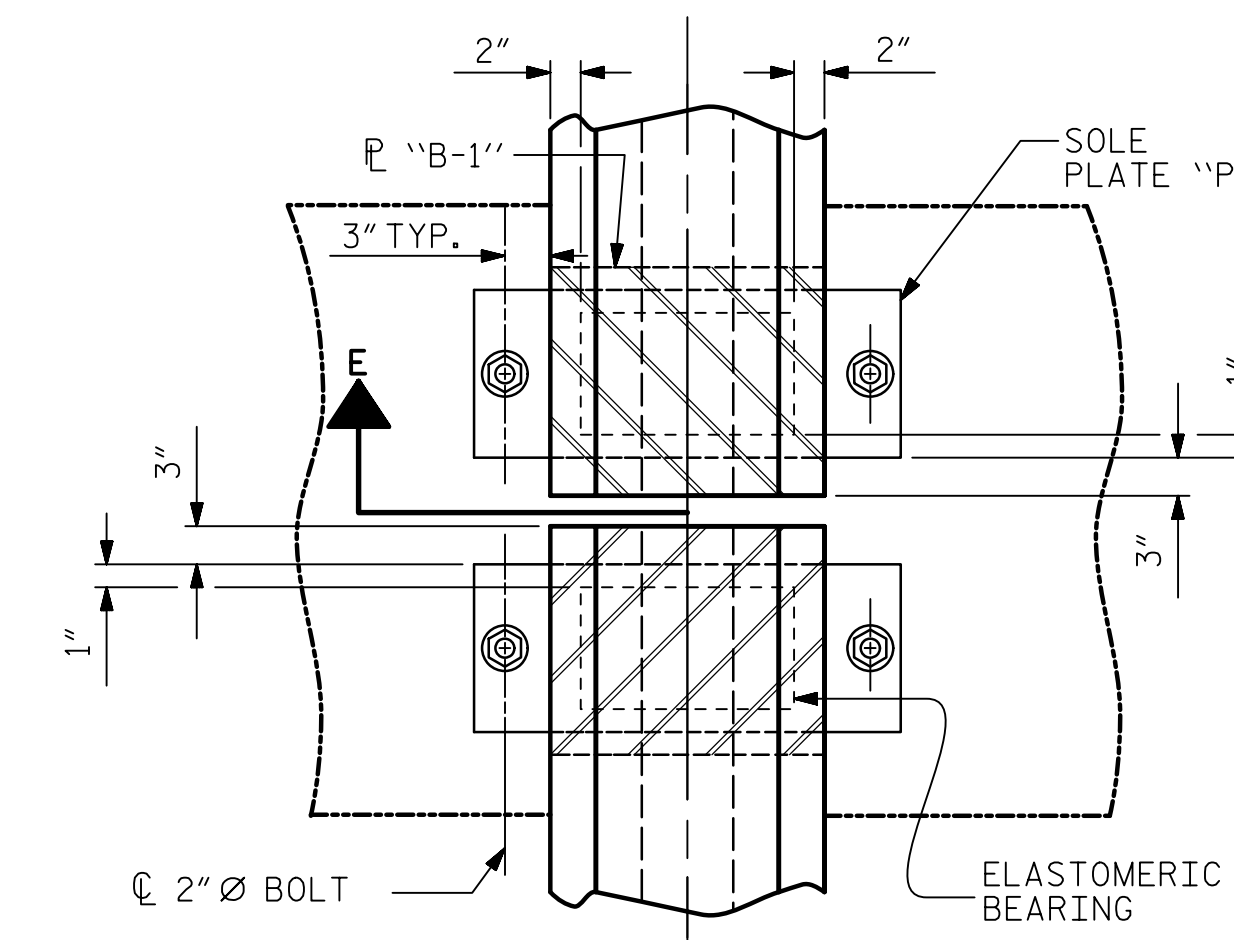
TYPICAL SECTION OF ELASTOMERIC BEARINGS



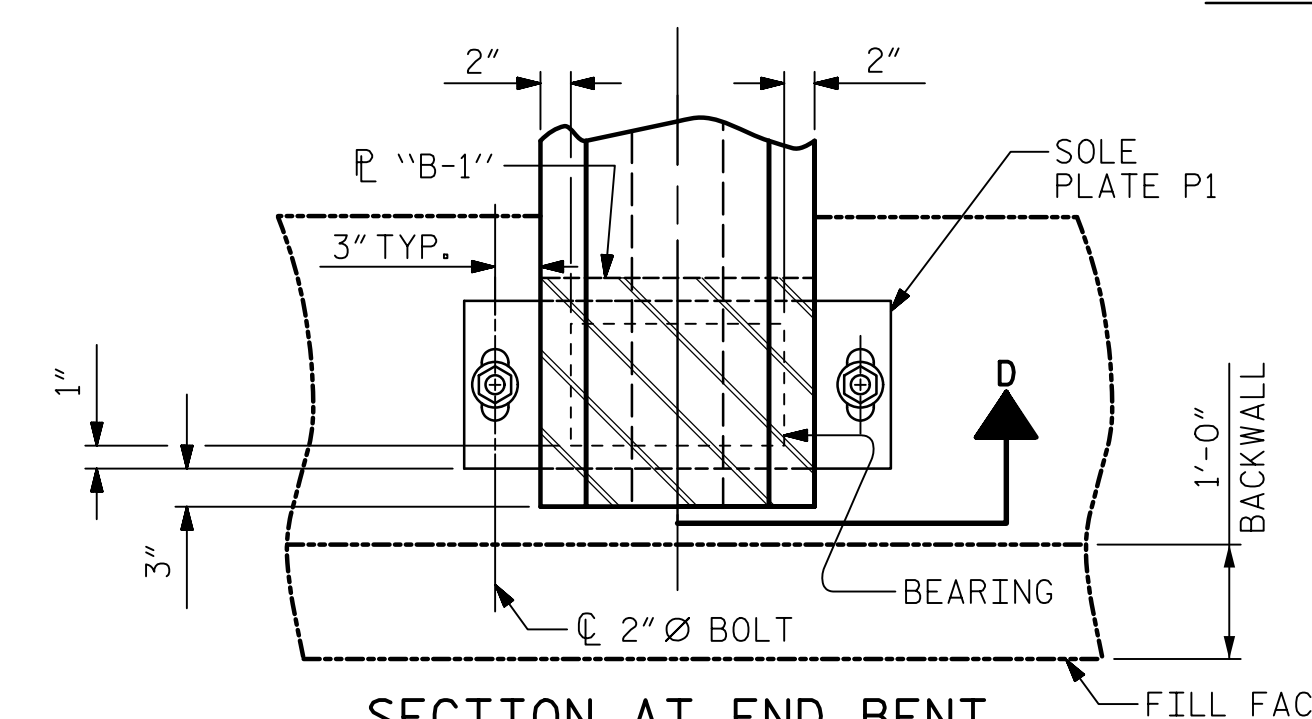
TYPICAL SECTION OF ELASTOMERIC BEARINGS



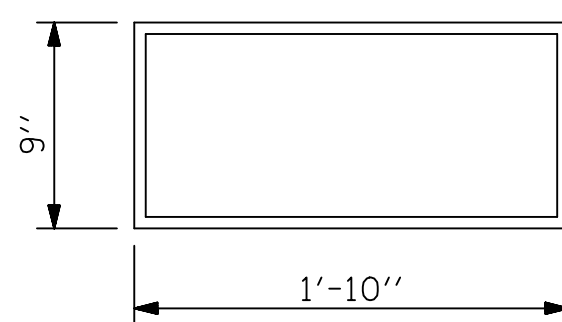
DETAIL "A"



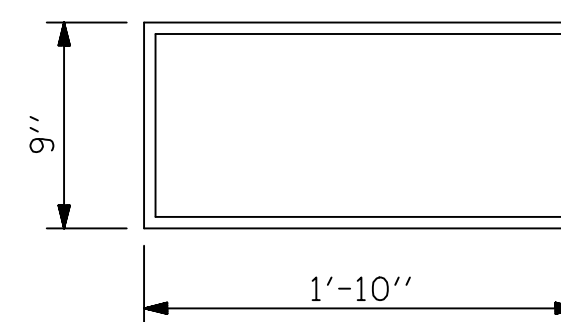
SECTION AT BENT (FIXED)



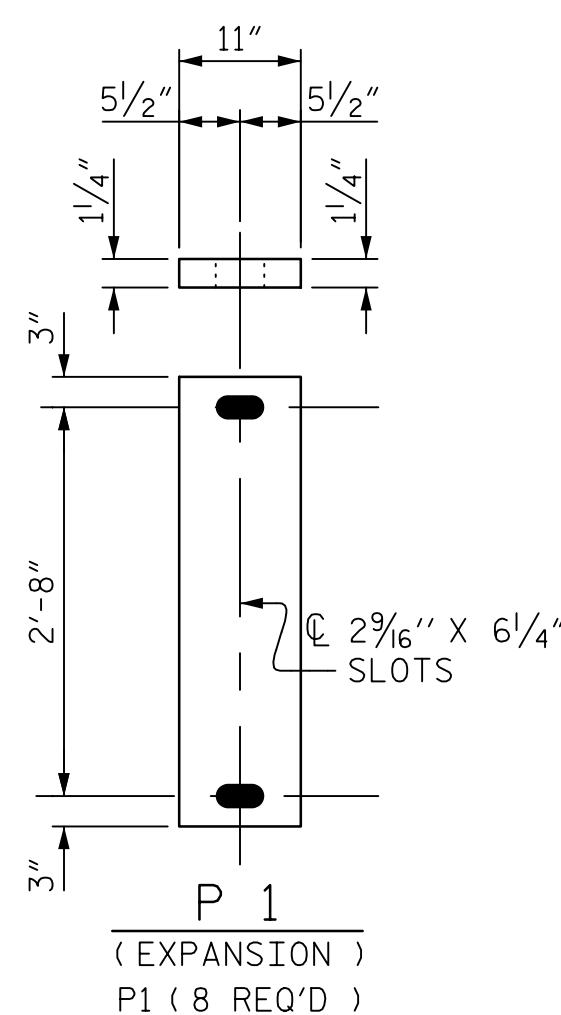
SECTION AT END BENT (EXPANSION)



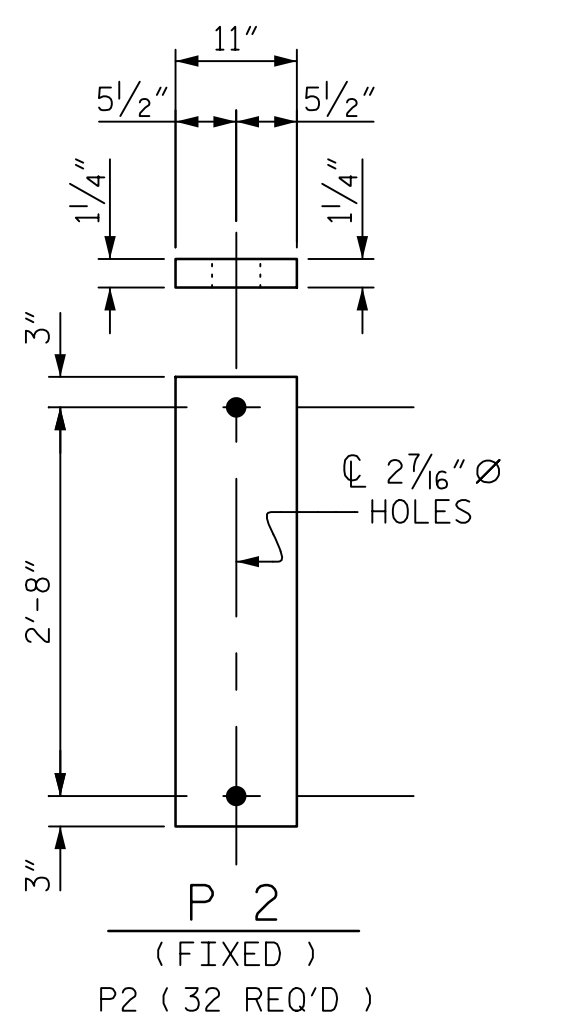
E1 (40 REQ'D)
PLAN VIEW OF ELASTOMERIC BEARING
TYPE IV



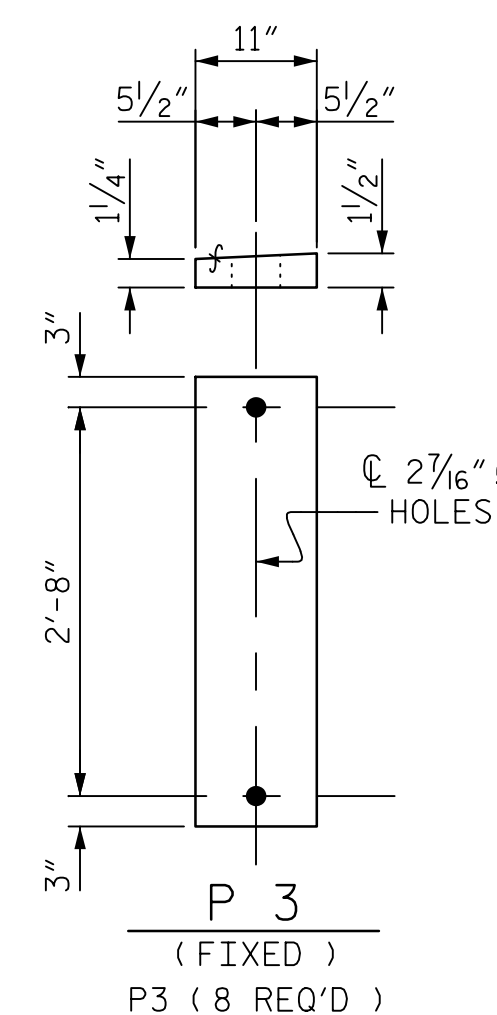
E2 (8 REQ'D)
PLAN VIEW OF ELASTOMERIC BEARING
MODIFIED TYPE IV



P 1 (EXPANSION)
P1 (8 REQ'D)



P 2 (FIXED)
P2 (32 REQ'D)



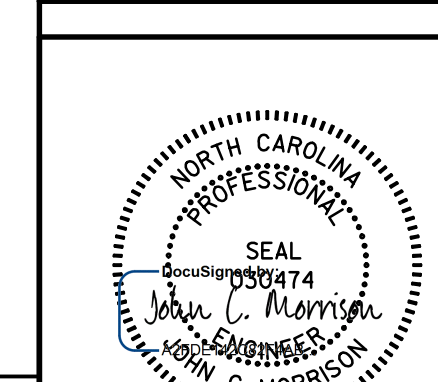
P 3 (FIXED)
P3 (8 REQ'D)

UP-STATION
SOLE PLATE DETAILS ("P")

| MAXIMUM ALLOWABLE SERVICE LOADS | |
|---------------------------------|-------|
| D.L.+L.L. (NO IMPACT) | |
| TYPE IV | 225 k |
| MODIFIED TYPE IV | 300 k |

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 90+25.00 -L-

BRIDGE NO. 656



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
ELASTOMERIC BEARING
DETAILS
PRESTRESSED CONCRETE GIRDER
SUPERSTRUCTURE

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. S-21
TOTAL SHEETS 86

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016
TIME: 9:54:58 AM

USER: \\s01\eng\p\656\card\40_04_L12707_SML_S21_LBR001.dgn

| | |
|----------------------|---------------------|
| ASSEMBLED BY: KHC | DATE: 11/2015 |
| CHECKED BY: JCM | DATE: 11/2015 |
| DRAWN BY: WJH 8/89 | REV. 10/1/11 MAA/GM |
| CHECKED BY: CRK 8/89 | REV. 6/13 AAC/MAA |
| | REV. 1/15 MAA/TMG |

NOTES:

THE CONCRETE PARAPET IN EACH SPAN SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT SLAB HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

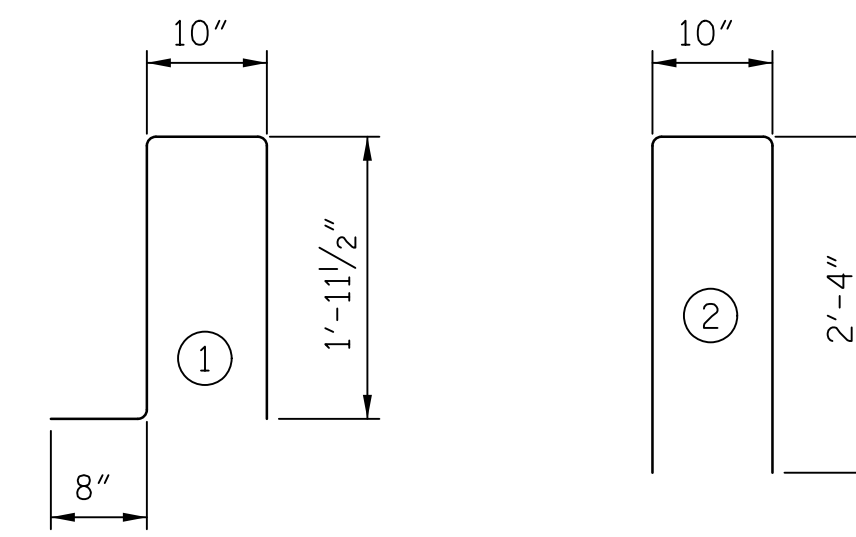
ALL REINFORCING STEEL IN PARAPET RAILS SHALL BE EPOXY COATED.

VERTICAL GROOVED CONTRACTION JOINTS 1/2" IN DEPTH SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS, THE CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FOR PLAN AND DETAILS OF PARAPET AND END POSTS, SEE "CONCRETE PARAPET AND END POST DETAILS" SHEET.

FOR CONCRETE PARAPET AND END POST DETAILS ON APPROACH SLAB, SEE "BRIDGE APPROACH SLAB DETAILS" SHEET 2 OF 2.

BAR TYPES

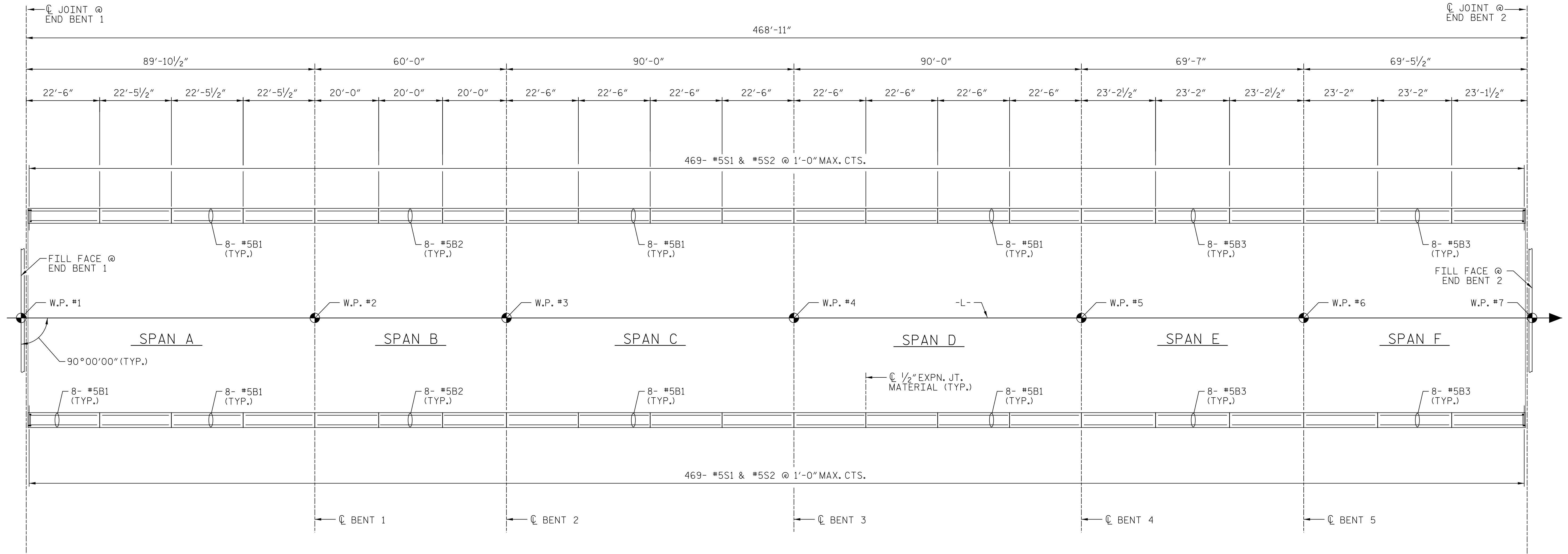


ALL BAR DIMENSIONS ARE OUT TO OUT.

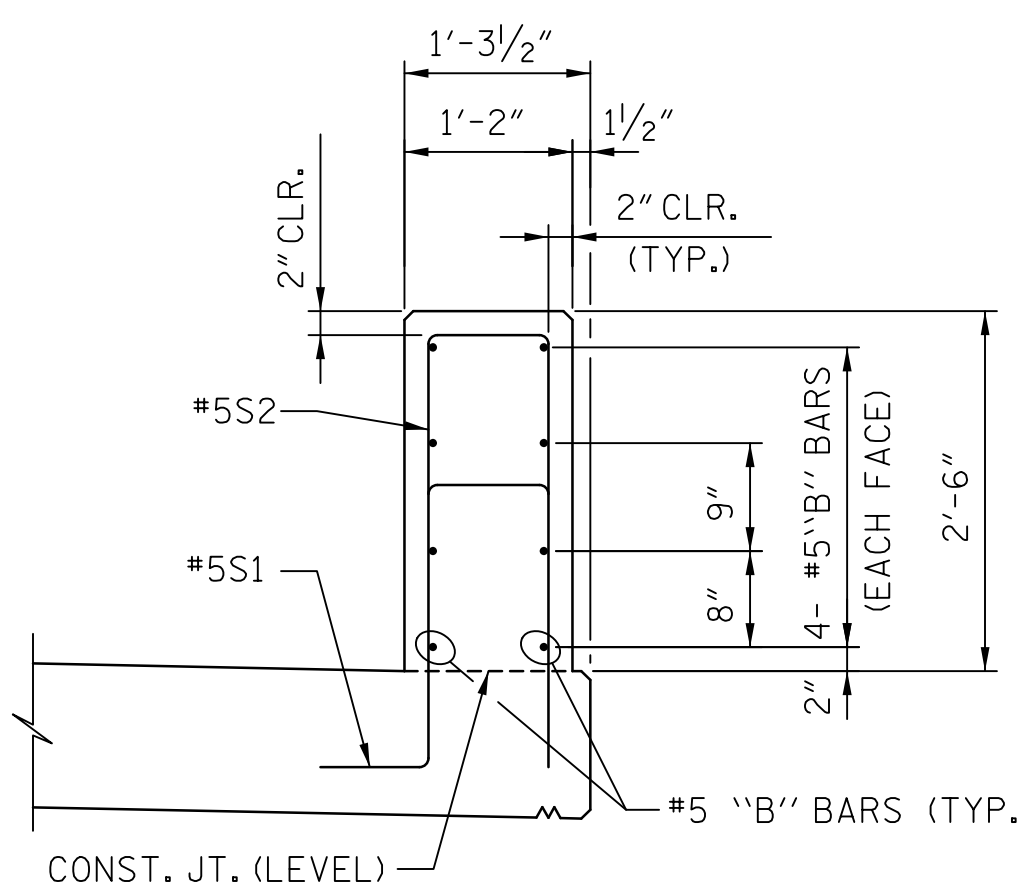
TWO BAR METAL RAIL

BILL OF MATERIAL

| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
|----------------------------------|-----|------|------|---------|--------|
| * B1 | 192 | #5 | STR | 22'-2" | 4439 |
| * B2 | 48 | #5 | STR | 19'-8" | 985 |
| * B3 | 96 | #5 | STR | 22'-10" | 2286 |
| * S1 | 938 | #5 | ① | 5'-5" | 5299 |
| * S2 | 938 | #5 | ② | 5'-6" | 5381 |
| * EPOXY COATED REINFORCING STEEL | | | | LBS. | 18390 |
| CLASS AA CONCRETE | | | | C.Y. | 101.3 |
| 1'-2" x 2'-6" CONCRETE PARAPET | | | | L.F. | 937.8 |



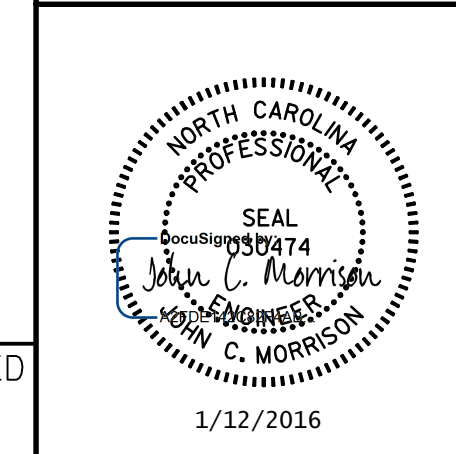
PLAN OF PARAPET REINFORCING



SECTION THRU PARAPET

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-

BRIDGE NO. 656



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

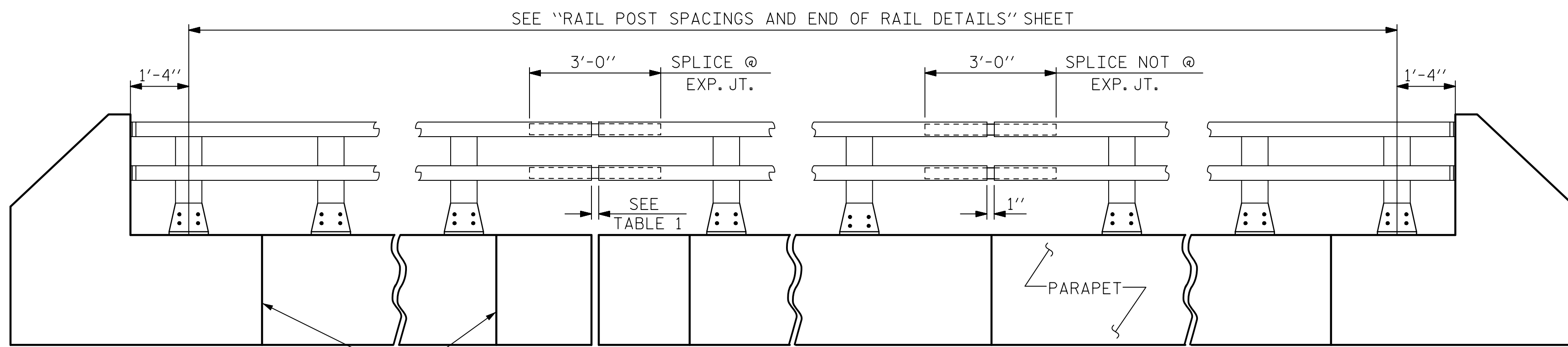
**SUPERSTRUCTURE
 CONCRETE PARAPET RAIL**

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-23 |
| 1 | | | 3 | | | TOTAL SHEETS |
| 2 | | | 4 | | | 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: KHC DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DATE: 1/12/2016 TIME: 9:25:04 AM
 USER: \\gis\compton DON: R:\Bridges\656\Coord\40_045_U2707_SML_S23_CP.dgn



ELEVATION
NOTE : FOR ATTACHMENT OF METAL RAIL TO END POST, SEE STANDARD NO. BMR2.

NOTES

AT THE CONTRACTOR'S OPTION, METAL RAIL MAY BE EITHER ALUMINUM OR GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE ALTERNATE MATERIALS; HOWEVER, THE CONTRACTOR WILL BE REQUIRED TO USE THE SAME RAIL MATERIAL ON ALL STRUCTURES ON THE PROJECT FOR WHICH METAL RAIL IS DESIGNATED.

UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE OPTION TO USE AN ALTERNATE TO THE 2 BAR METAL RAIL. THE ALTERNATE RAIL SHALL MEET THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MUST BE LISTED ON THE DEPARTMENT'S APPROVED PRODUCTS LIST (APL) UNDER "2 BAR METAL RAIL ALTERNATE". ADJUSTMENTS TO THE CONCRETE PARAPET WILL NOT BE ALLOWED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GALVANIZED STEEL RAILS

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

POST, POST BASES, RAILS, EXPANSION BARS AND CLAMP BARS: AASHTO M270 GRADE 36 STRUCTURAL STEEL - GALVANIZED TO AASHTO M111.

RIVETS: RIVETS SHALL MEET THE REQUIREMENTS OF ASTM A502 FOR GRADE 1 RIVETS.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL SPECIFICATIONS TT-P-641.

SHIMS: SHIMS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

RAIL CAPS: RAIL CAPS SHALL MEET THE REQUIREMENTS OF ASTM A570 FOR GRADE 33 OR A611 FOR GRADE C AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL, WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

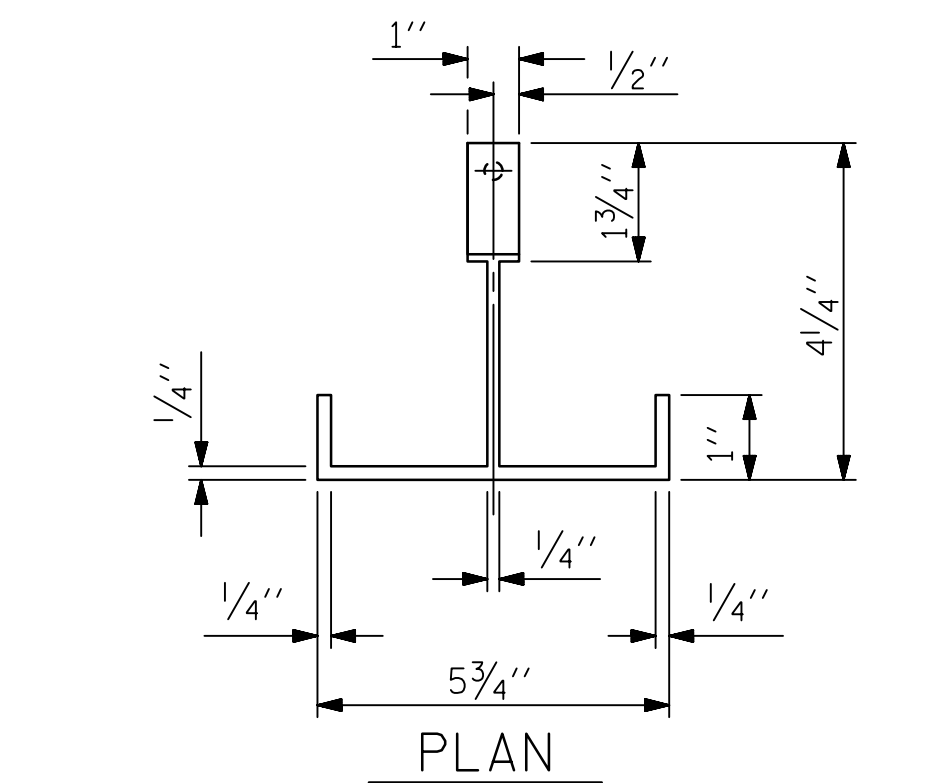
TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

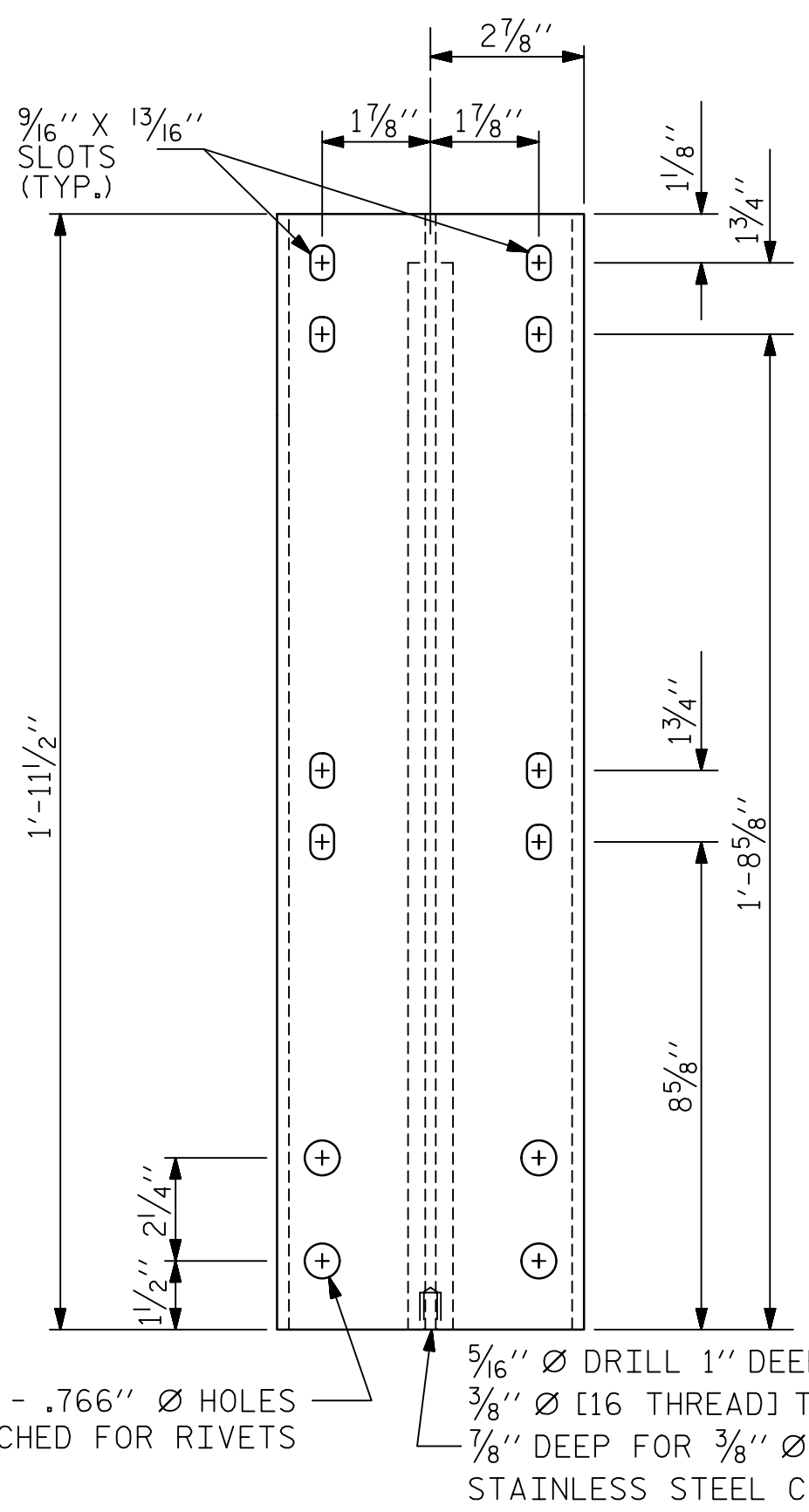
ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

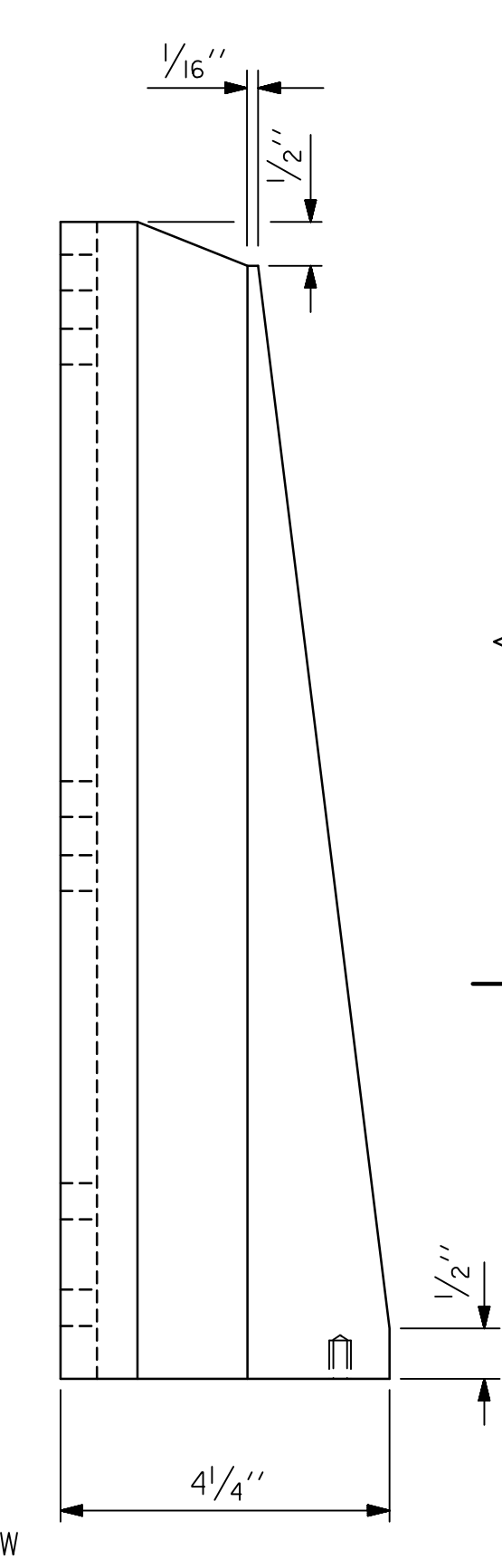
GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



PLAN



FRONT ELEVATION

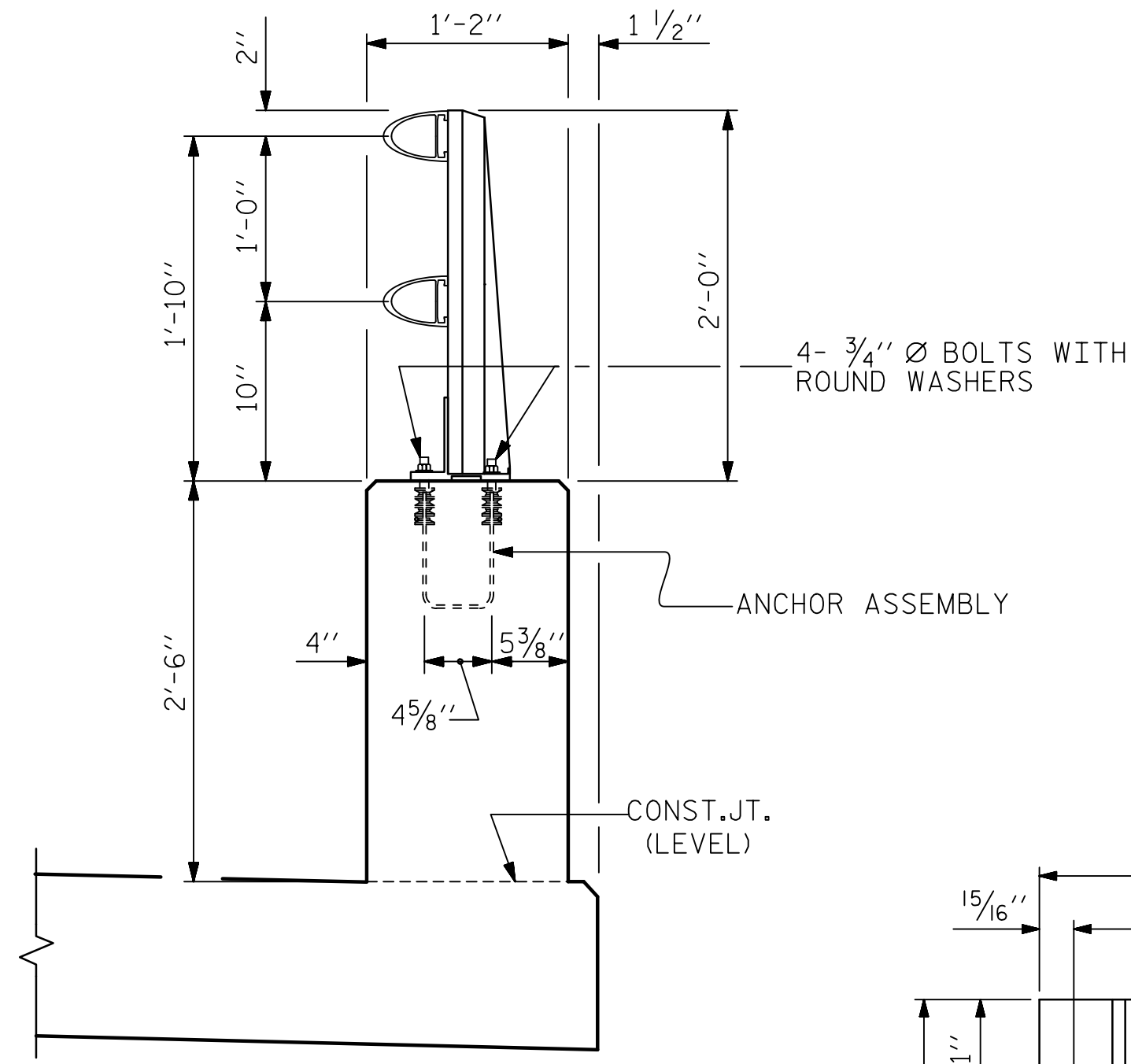


SIDE ELEVATION

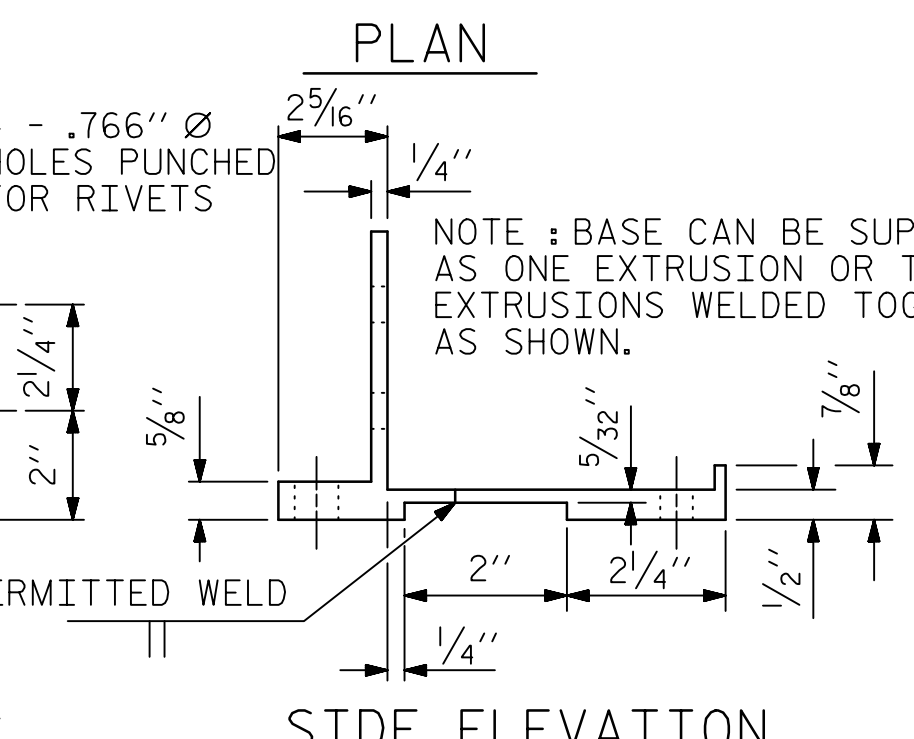
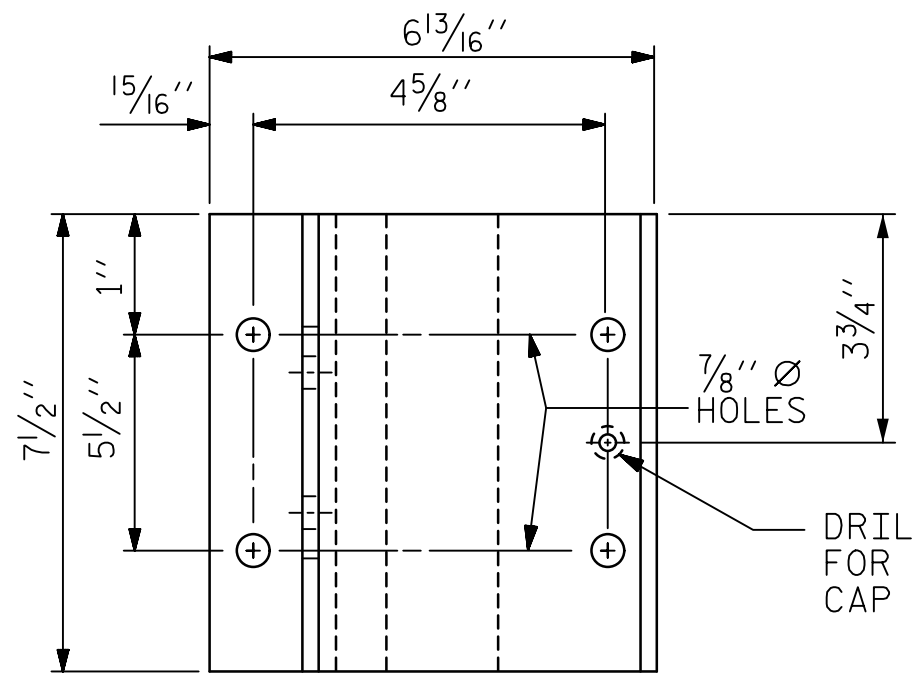
DETAILS OF POST

| | | |
|-----------------------|----------------|--------|
| ASSEMBLED BY : KHC | DATE : 11/2015 | TLA/GM |
| CHECKED BY : JCM | DATE : 11/2015 | MAA/GM |
| DRAWN BY : EEM 6/94 | REV. 5/1/06 | MAA/GM |
| CHECKED BY : RGW 6/94 | REV. 10/1/11 | MAA/GM |
| | REV. 6/13 | MAA/GM |

| EXP. JT. @ | RAIL OPENING |
|------------|--------------|
| END BENT 1 | 2" |
| END BENT 2 | 2" |



SECTION THRU PARAPET AND RAIL

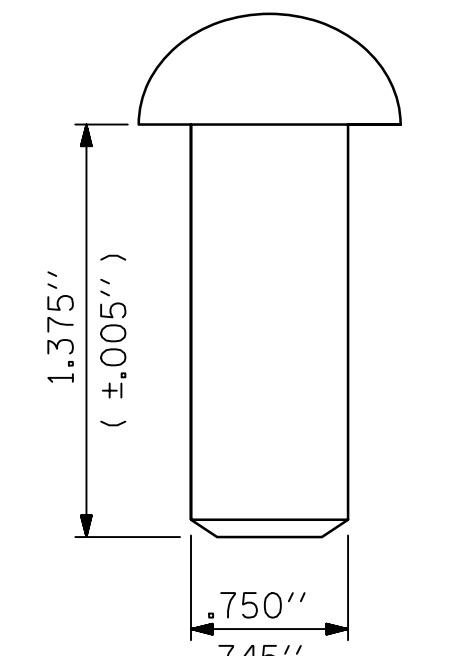


FRONT ELEVATION

SIDE ELEVATION

POST BASE DETAILS

PAY LENGTH = 962.8 LIN. FT.



RIVET DETAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 90+25.00 -L-
SHEET 2 OF 3 BRIDGE NO. 656

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-4200 www.aecom.com
AECOM License No. F-0352

SEAL
NORTH CAROLINA PROFESSIONAL ENGINEER
John C. Morrison
1/12/2016

| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|------------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| STANDARD 2 BAR METAL RAIL | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. S-25 | | | | | TOTAL SHEETS 86 |
| STD. NO. BMR3 | | | | | |

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

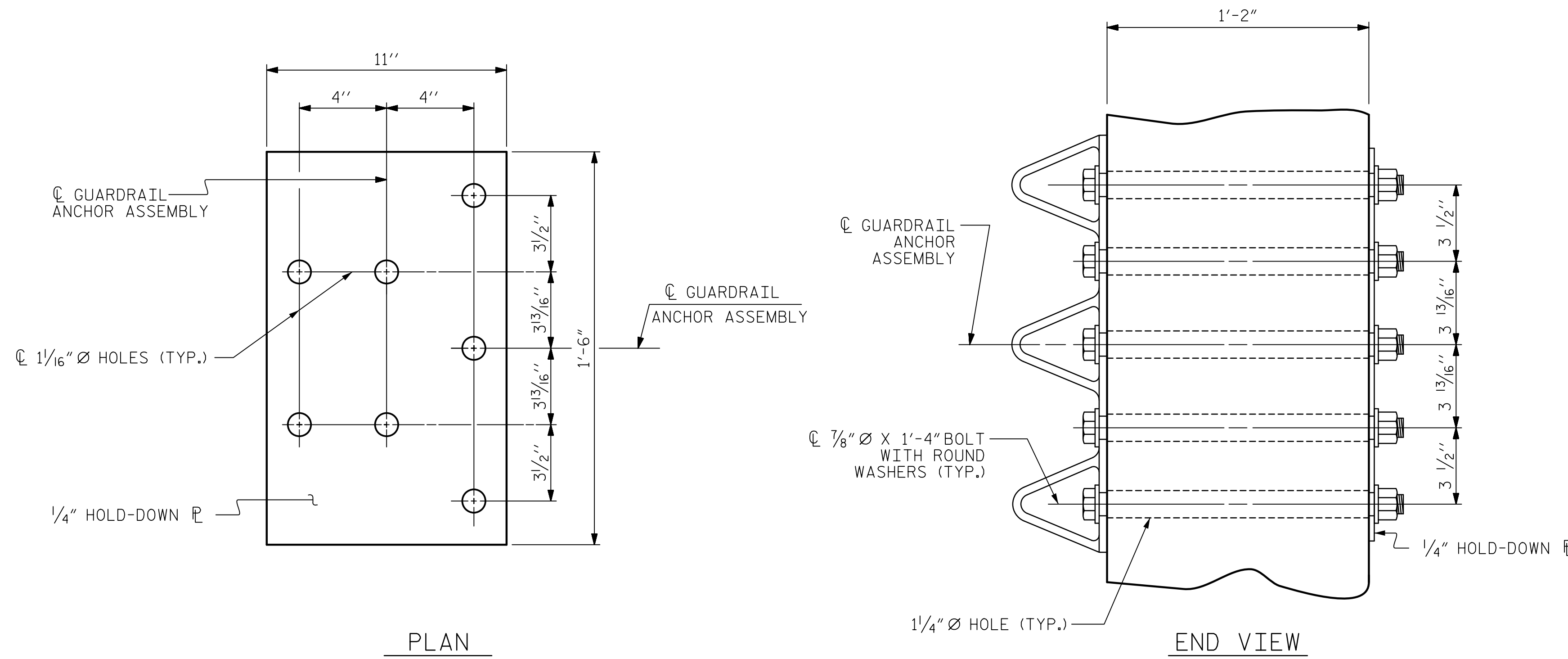
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

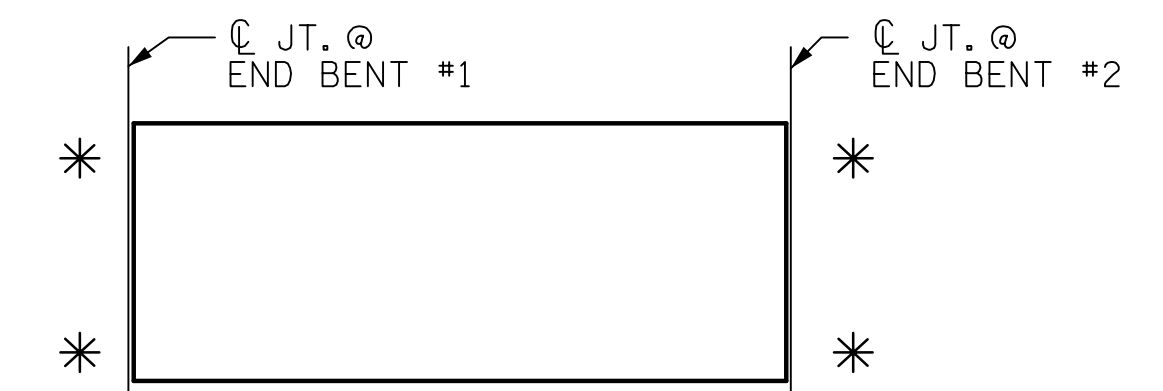
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

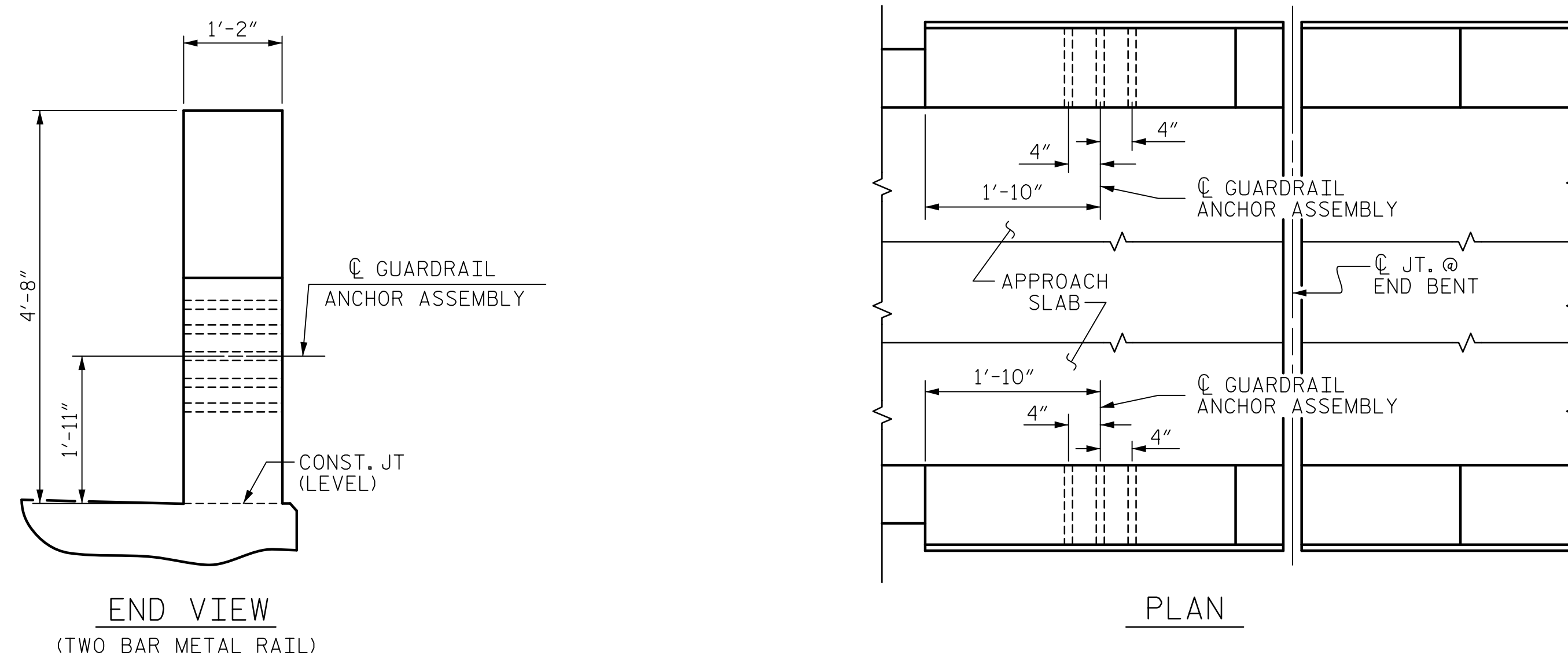


GUARDRAIL ANCHOR ASSEMBLY DETAILS



SKETCH SHOWING POINTS OF ATTACHMENT

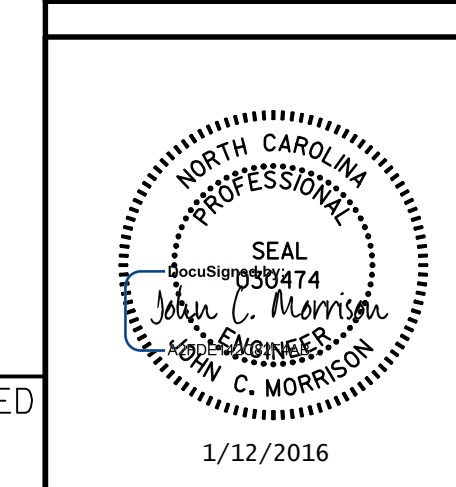
* LOCATION OF GUARDRAIL ATTACHMENT



LOCATION OF GUARDRAIL ANCHOR AT END POST

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-

BRIDGE NO. 656



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 GUARDRAIL ANCHORAGE
 DETAILS
 FOR METAL RAILS

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | TOTAL SHEETS |
| 1 | | | 3 | | | 86 |
| 2 | | | 4 | | | |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016

| | |
|----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : MAA 5/10 | REV. 12/5/11 MAA/GM |
| CHECKED BY : GM 5/10 | REV. 6/13 MAA/GM |
| | REV. 1/15 MAA/TMG |

DATE: 1/12/2016
 TIME: 9:25:17 AM

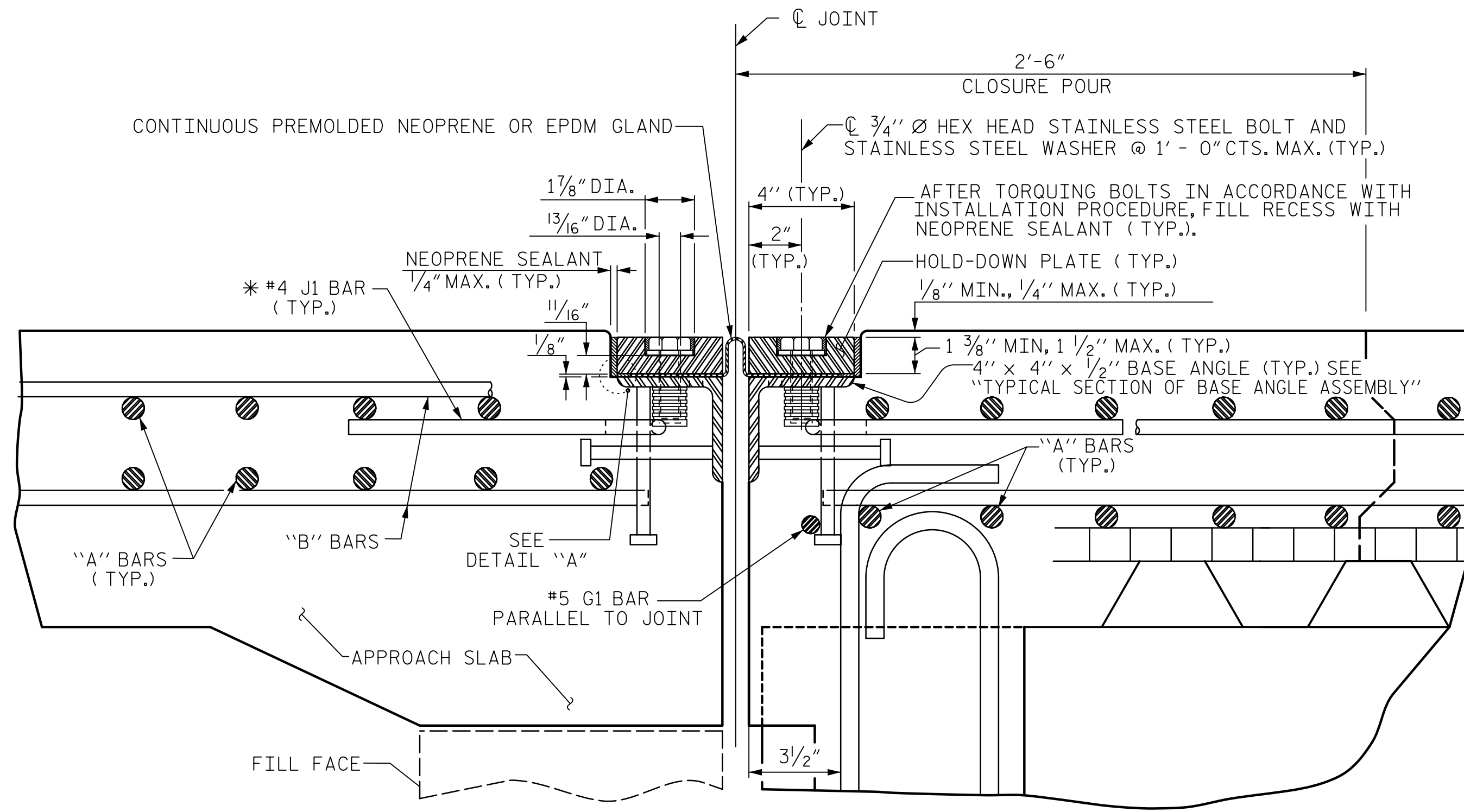
USER: \\s:\eng\p\p\656\cond\401_053_112707_SML_SZ_GRA3.dgn

INSTALLATION PROCEDURE

1. A TEMPLATE OR OTHER SUITABLE DEVICE SHALL BE USED TO FORM THE TOP OF THE EXPANSION JOINT SEAL BLOCKOUT TO THE PROPER DEPTH AND WIDTH. THE TEMPLATE SHALL BE 4/8" TO 4 1/4" WIDE AND OF SUCH THICKNESS AS TO PROVIDE FOR CORRECT FINAL ELEVATION OF TOP OF HOLD-DOWN PLATES. THE TEMPLATE SHALL BE ATTACHED TO THE BASE ANGLE ASSEMBLY WITH THE 3/4" Ø HEX HEAD BOLTS PROVIDED FOR THE HOLD-DOWN PLATES. A 1" Ø HOLE SHALL BE PROVIDED IN THE TEMPLATE CENTERED OVER EACH WEEP HOLE IN THE 4" X 4" X 1/2" BASE ANGLE. OTHER METHODS OF INSURING DRAINAGE THROUGH WEEP HOLES MAY BE EMPLOYED SUBJECT TO ENGINEER'S APPROVAL.
2. AFTER THE CONCRETE HAS BEEN CAST ON BOTH SIDES OF THE JOINT, REMOVE THE TEMPLATE. THOROUGHLY CLEAN THE BOLT HOLES AND THE ANGLE PLATE. REMOVE ANY EXCESS CONCRETE THAT COMES OUT OF THE WEEP HOLES. ANY DAMAGED STEEL SHALL BE COATED WITH A MINIMUM THICKNESS OF 4 DRY MILS OF ZINC-RICH PAINT IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
3. LAY THE GLAND ON THE BASE ANGLE AND FIELD MARK THE GLAND FOR THE BOLT HOLES. HOLES IN THE GLAND SHALL BE PUNCHED 1/8" IN DIAMETER WITH A HAND PUNCH.
4. IN ORDER TO CHECK FOR PROPER ALIGNMENT, PLACE THE GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. DO NOT APPLY NEOPRENE SEALANT. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE BUT DO NOT TIGHTEN. THE ENGINEER SHALL INSPECT THE JOINT SEAL DEVICE FOR PROPER ALIGNMENT.
5. AFTER INSPECTION, REMOVE THE HOLD-DOWN PLATES AND GLAND. APPLY NEOPRENE SEALANT TO THE BASE ANGLE IN ACCORDANCE WITH THE "INSTALLATION SKETCH". PLACE GLAND AND HOLD-DOWN PLATES ON THE BASE ANGLE. BOLT THE HOLD-DOWN PLATES TO THE BASE ANGLE ASSEMBLY AND TORQUE THE BOLTS TO 88 FT-LBS WITH A TORQUE WRENCH. CHECK THE TORQUE AFTER THREE (3) HOURS AND, IF NECESSARY, RETIGHTEN TO 88 FT-LBS. A FINAL CHECK SHALL BE MADE AT SEVEN (7) DAYS. TORQUE SHALL NOT BE LESS THAN 80 FT-LBS AFTER SEVEN (7) DAYS.
6. AFTER PROPER TORQUING, CLEAN THE BOLT HOLE RECESSES AND THE RECESS BETWEEN THE JOINT SEAL DEVICE AND CONCRETE, COMPLETELY FILL THESE RECESSES WITH NEOPRENE SEALANT.

GENERAL NOTES

1. FOR EXPANSION JOINT SEALS, SEE SPECIAL PROVISIONS.
2. ALL PLATES AND ANGLES SHALL CONFORM TO AASHTO M270 GRADE 36 STEEL OR APPROVED EQUAL. ALL HOLD-DOWN BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL CONFORM TO ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. ALL STUD ANCHORS SHALL CONFORM TO AASHTO M169, GRADES 1010 THRU 1020 OR APPROVED EQUAL. ALL CONCRETE INSERTS SHALL BE CLOSED END AND SHALL CONFORM TO AASHTO M169, GRADE 12L14. TENSILE CAPACITY SHALL BE 3000 LBS. MIN.
3. A PREMOLDED CORRUGATED OR NON-CORRUGATED GLAND SHALL BE USED FOR JOINTS SKEWED BETWEEN 50° THRU 130°. FOR JOINTS SKEWED LESS THAN 50° OR MORE THAN 130°, ONLY A CORRUGATED GLAND SHALL BE USED.
4. CLOSED END FERRULES AND STUD ANCHORS SHALL BE SHOP WELDED AND ALL HOLES SHALL BE SHOP DRILLED AS SHOWN ON PLANS. STUD ANCHORS SHALL BE ELECTRIC ARC END WELDED WITH COMPLETE FUSION.
5. SURFACES COMING IN CONTACT WITH NEOPRENE SHALL BE GROUND SMOOTH PRIOR TO METALLIZING.
6. UPON COMPLETION OF SHOP FABRICATION, THE HOLD DOWN PLATE AND BASE ANGLE ASSEMBLY, AS SHOWN IN THE "TYPICAL SECTION OF BASE ANGLE ASSEMBLY", SHALL BE METALLIZED. SEE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).
7. BASE ANGLE ASSEMBLY SHALL BE CONTINUOUS FOR THE LENGTH OF THE JOINT. AT CROWN BREAKS, THE ENDS OF THE BASE ANGLE ASSEMBLY SHALL BE CUT PARALLEL TO THE BRIDGE CENTERLINE FOR SKEWS LESS THAN 80° AND GREATER THAN 100°. FINISHED WELD SHALL BE GROUND SMOOTH AND COATED WITH A MINIMUM THICKNESS OF 4 DRY MILS OF ZINC-RICH PAINT IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
8. FIELD SPLICES OF HOLD-DOWN PLATES SHALL BE KEPT TO A MINIMUM. CONTRACTOR SHALL FURNISH DETAILED PLANS SHOWING PROPOSED SPLICE LOCATIONS FOR APPROVAL. HOLD-DOWN PLATES SHALL NOT EXCEED 20' LENGTHS UNLESS APPROVED BY THE ENGINEER.
9. NO ALTERNATE JOINT DETAILS SHALL BE PERMITTED IN LIEU OF THOSE SHOWN ON THESE PLANS.
10. THE CONTRACTOR MAY, AT HIS OPTION, USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF CONCRETE INSERTS FOR COVER PLATES. THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



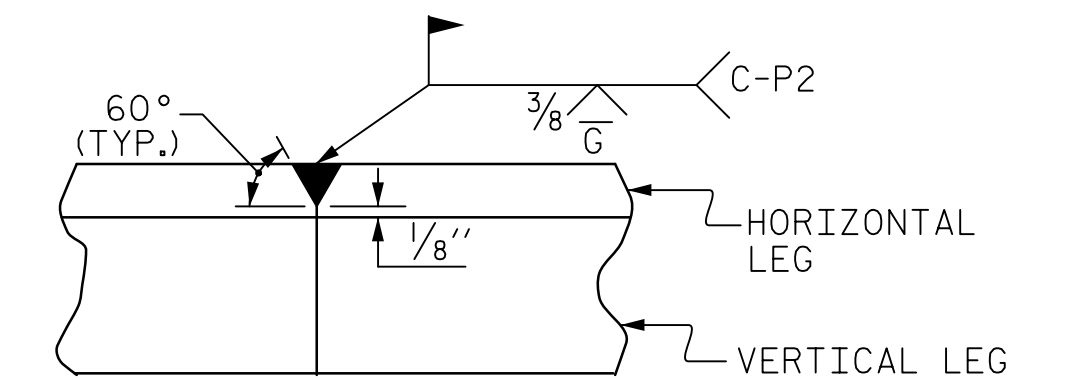
EXPANSION JOINT DETAILS

SECTION NORMAL TO JOINT -- PRESTRESSED GIRDER SUPERSTRUCTURE

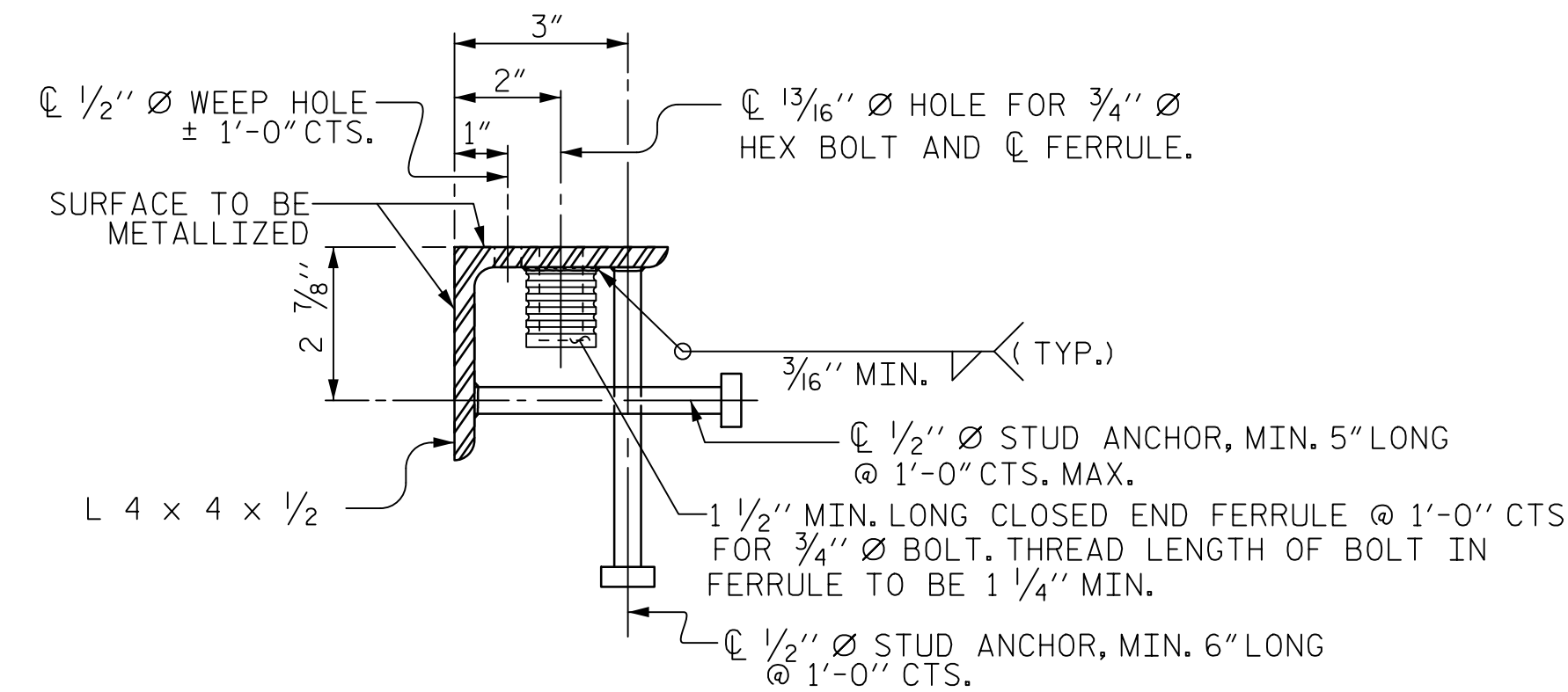
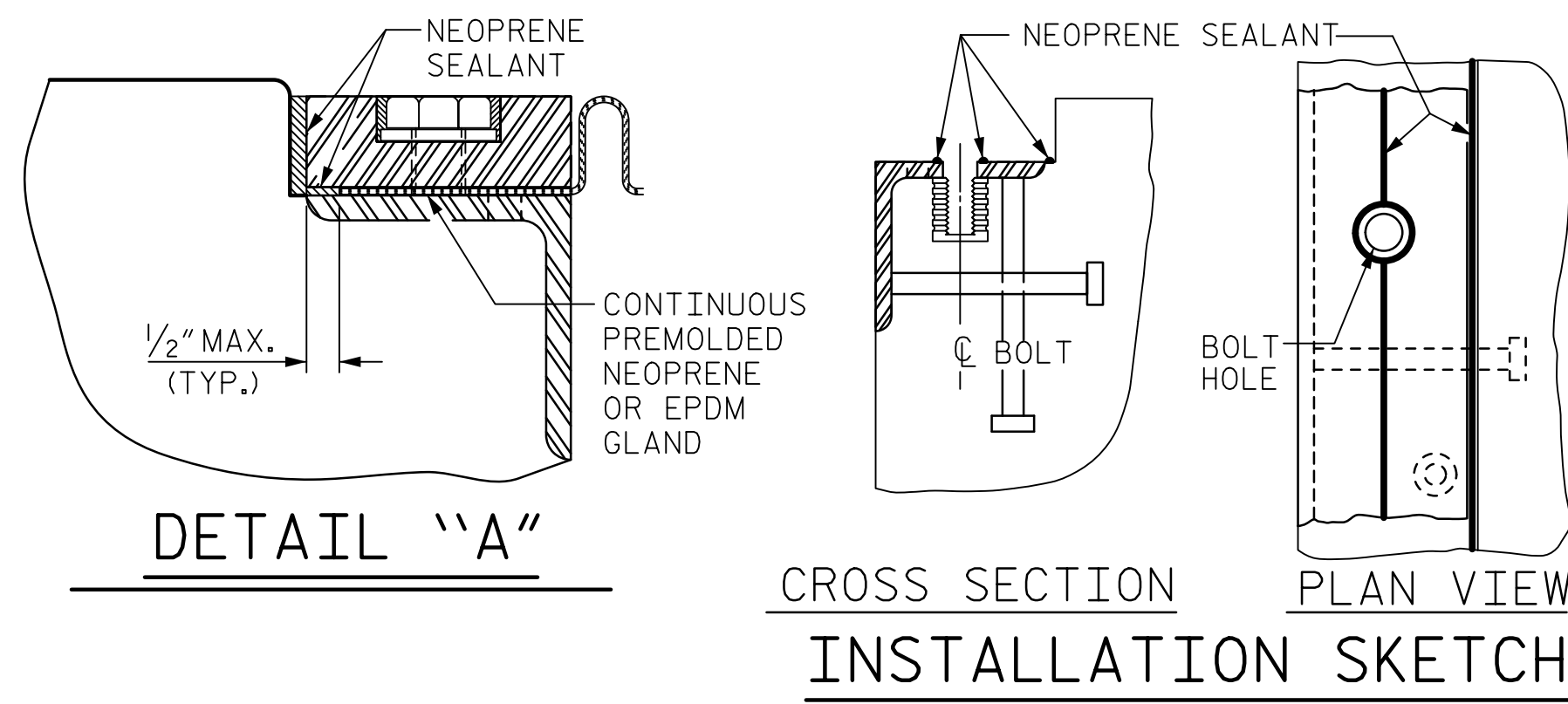
* THE QUANTITY OF #4 J1 BARS ON THE BILL OF MATERIAL IS BASED ON 1'-0" CENTERS. J1 BARS SHALL BE PLACED AT EACH VERTICAL STUD ANCHOR BOLT. IN THE EVENT THAT THE NUMBER OF VERTICAL STUD ANCHORS EXCEEDS THE NUMBER OF J1 BARS SPECIFIED, ADDITIONAL J1 BARS WILL NOT BE REQUIRED.

DATE: 1/12/2015
TIME: 9:25:30 AM

| MOVEMENT AND SETTING AT JOINT | | | | | |
|-------------------------------|------------|--------------------------------|--------------------------------------|--------------------------------------|--------------------------------------|
| END BENT NO. | SKEW ANGLE | TOTAL MOVEMENT (ALONG CL RDWY) | PERPENDICULAR JOINT OPENING AT 45° F | PERPENDICULAR JOINT OPENING AT 60° F | PERPENDICULAR JOINT OPENING AT 90° F |
| 1 | 90°00'00" | 1 7/16" | 2 1/16" | 1 3/16" | 1 1/4" |
| 2 | 90°00'00" | 1 7/16" | 2 1/16" | 1 3/16" | 1 1/4" |



DETAIL- FIELD WELD SPLICE OF BASE ANGLE



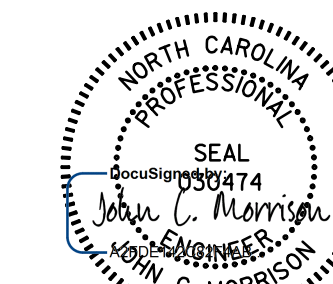
TYPICAL SECTION OF BASE ANGLE ASSEMBLY

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 1 OF 2 BRIDGE NO. 656



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD
EXPANSION JOINT
SEAL DETAILS

REVISIONS

| NO. | BY: | DATE: | NO. | BY: | DATE: |
|-----|-----|-------|-----|-----|-------|
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO.
S-28
TOTAL SHEETS
86

| | |
|------------------------|----------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : REK 9/87 | REV. 5/7/03R RWW/JTE |
| CHECKED BY : CRK 10/87 | REV. 5/1/06R TLA/GM |
| | REV. 10/1/11 MAA/GM |

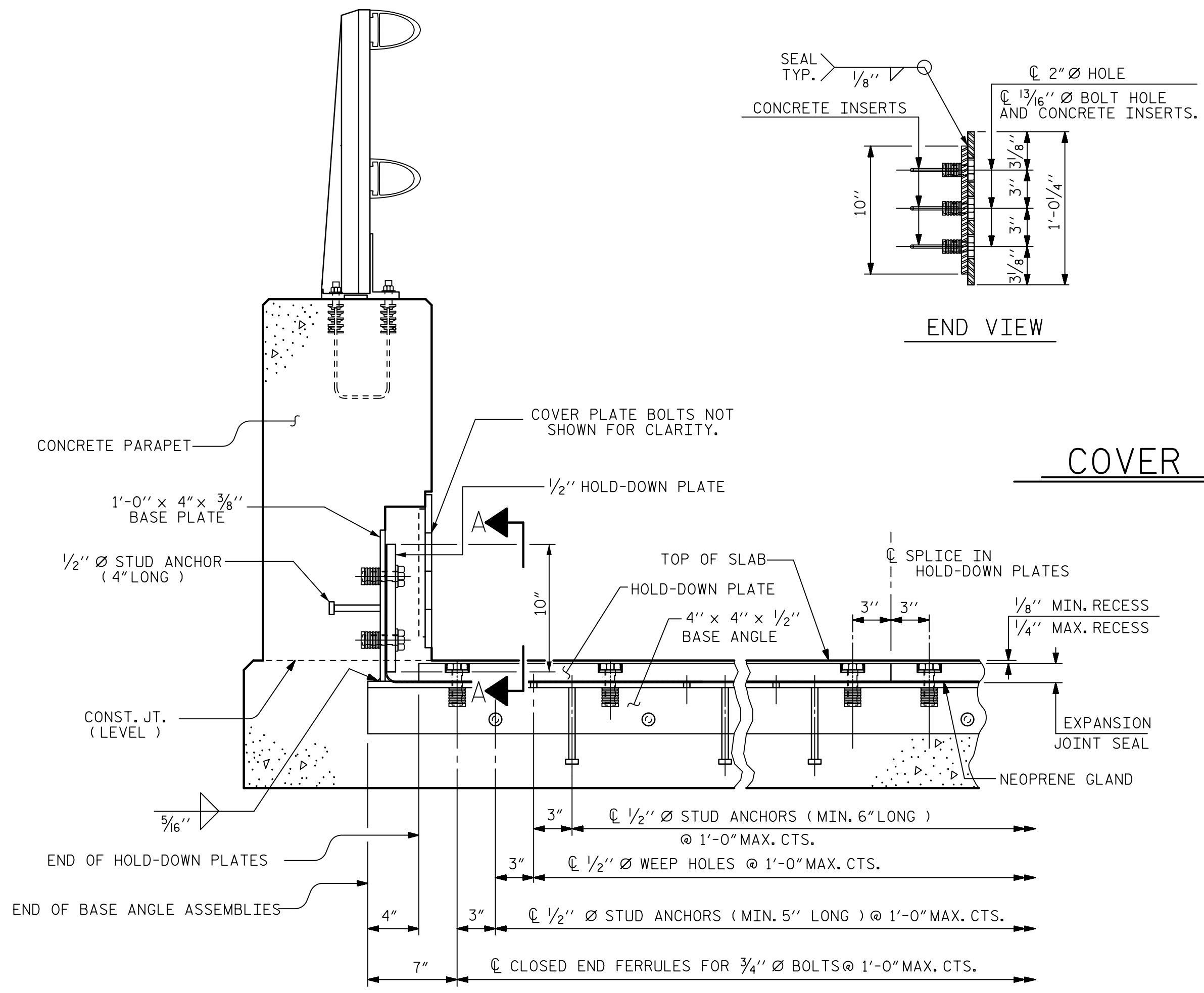
DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

1/12/2016

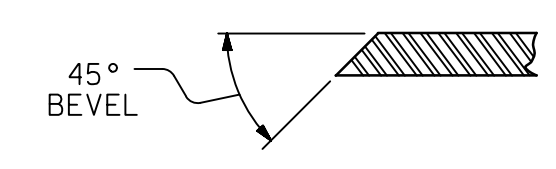
USER: \\s01\engr\p\B56\Cond\401_055_U2707_SML_S28_EJ50.dgn

DATE: 1/12/2015
TIME: 9:25:54 AM

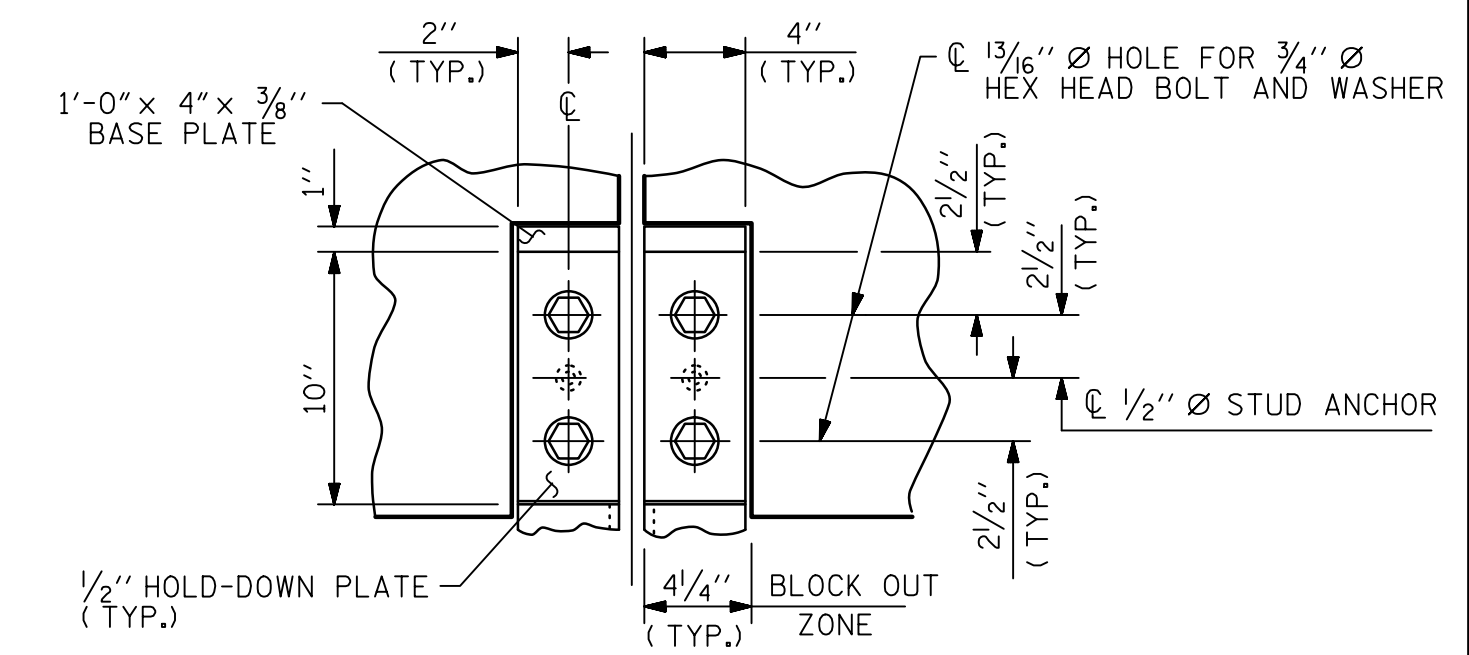
USER: kmc...
JOB: R:\Bridge\656\Comp\401_057_U2707_SML_S29_EJS02.dgn



COVER PLATE DETAILS

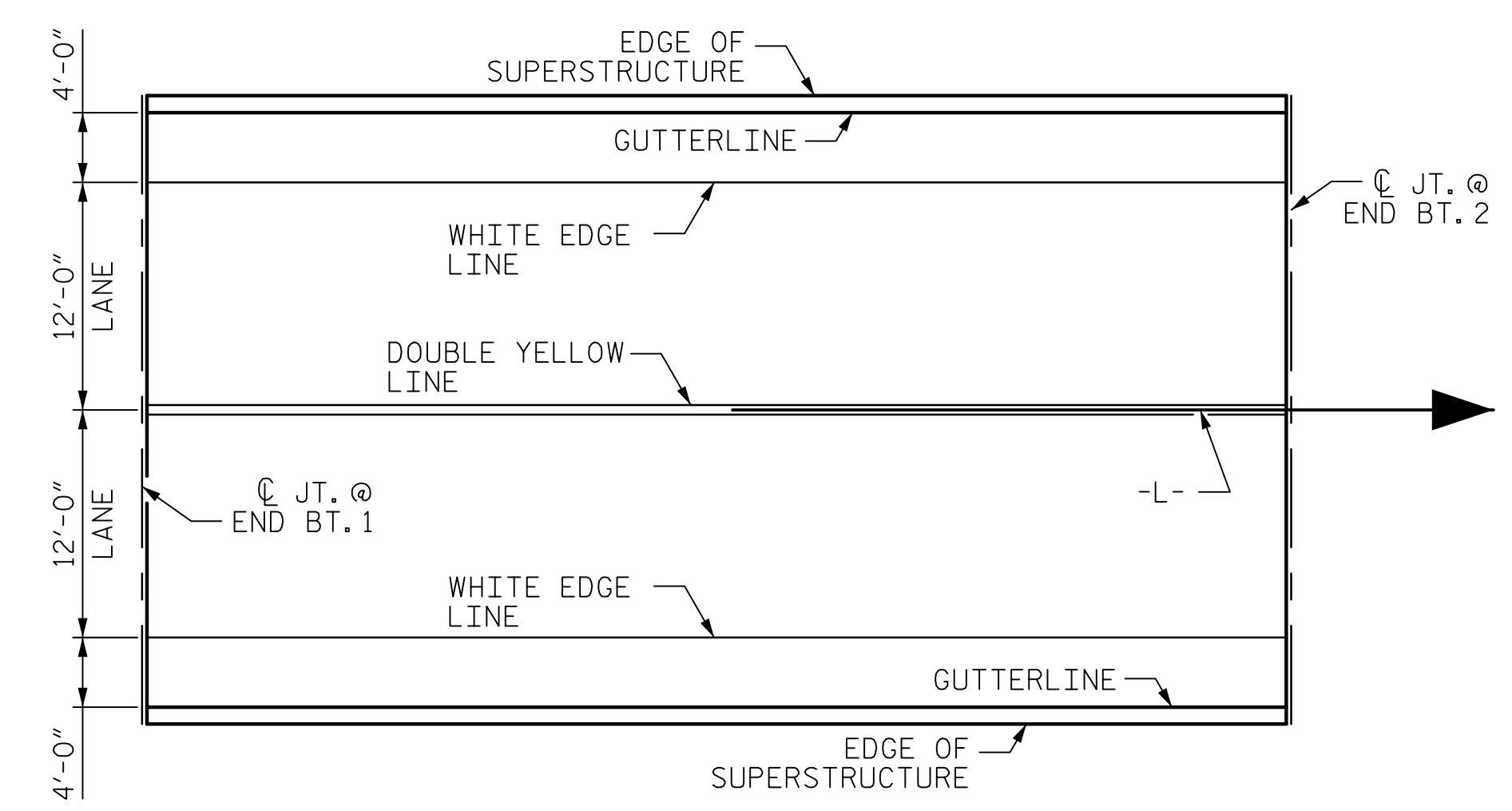


SECTION B - B

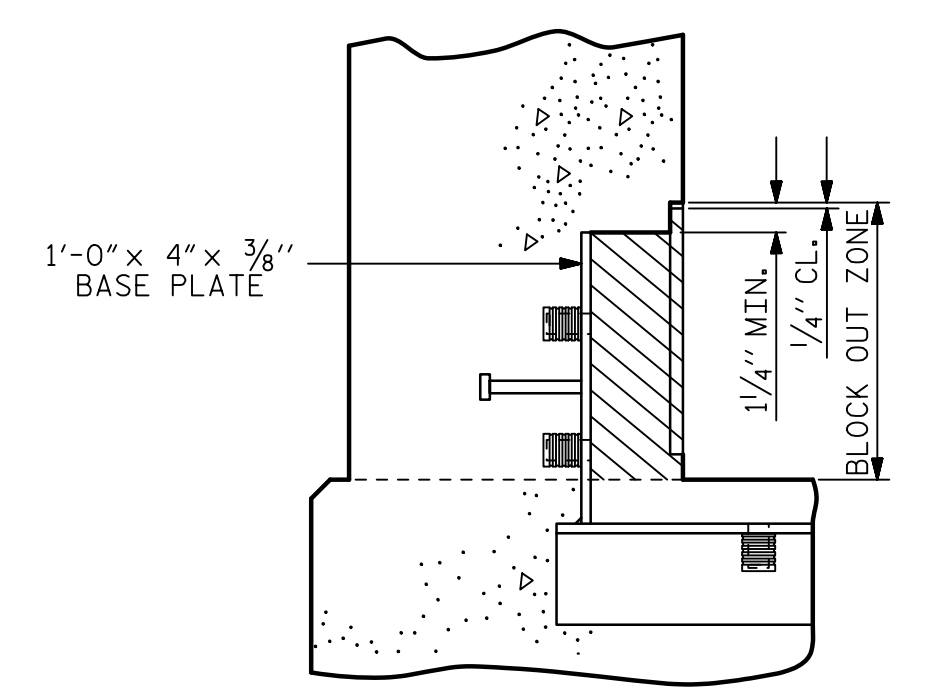


SECTION A - A

SECTION THRU RAIL NORMAL TO JOINT

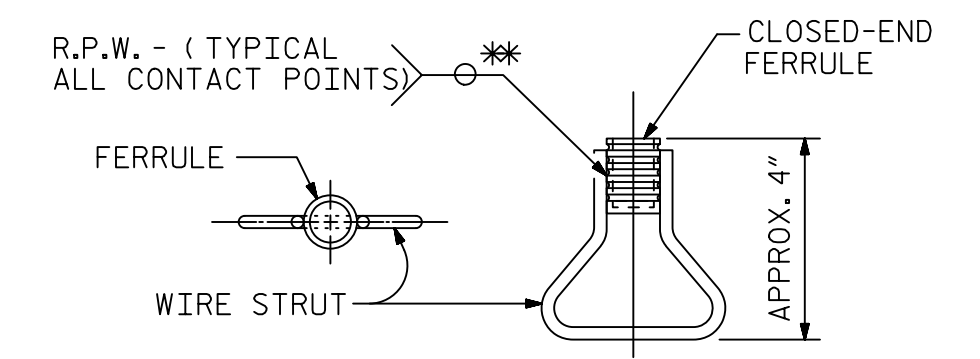


PAVEMENT MARKING ALIGNMENT



BLOCK OUT DETAIL

SEE "SECTION A - A" FOR OTHER DETAILS.



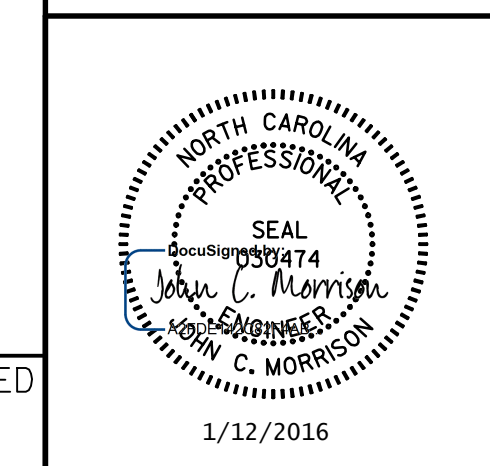
CONCRETE INSERT

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. U-2707
FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 2 OF 2 BRIDGE NO. 656



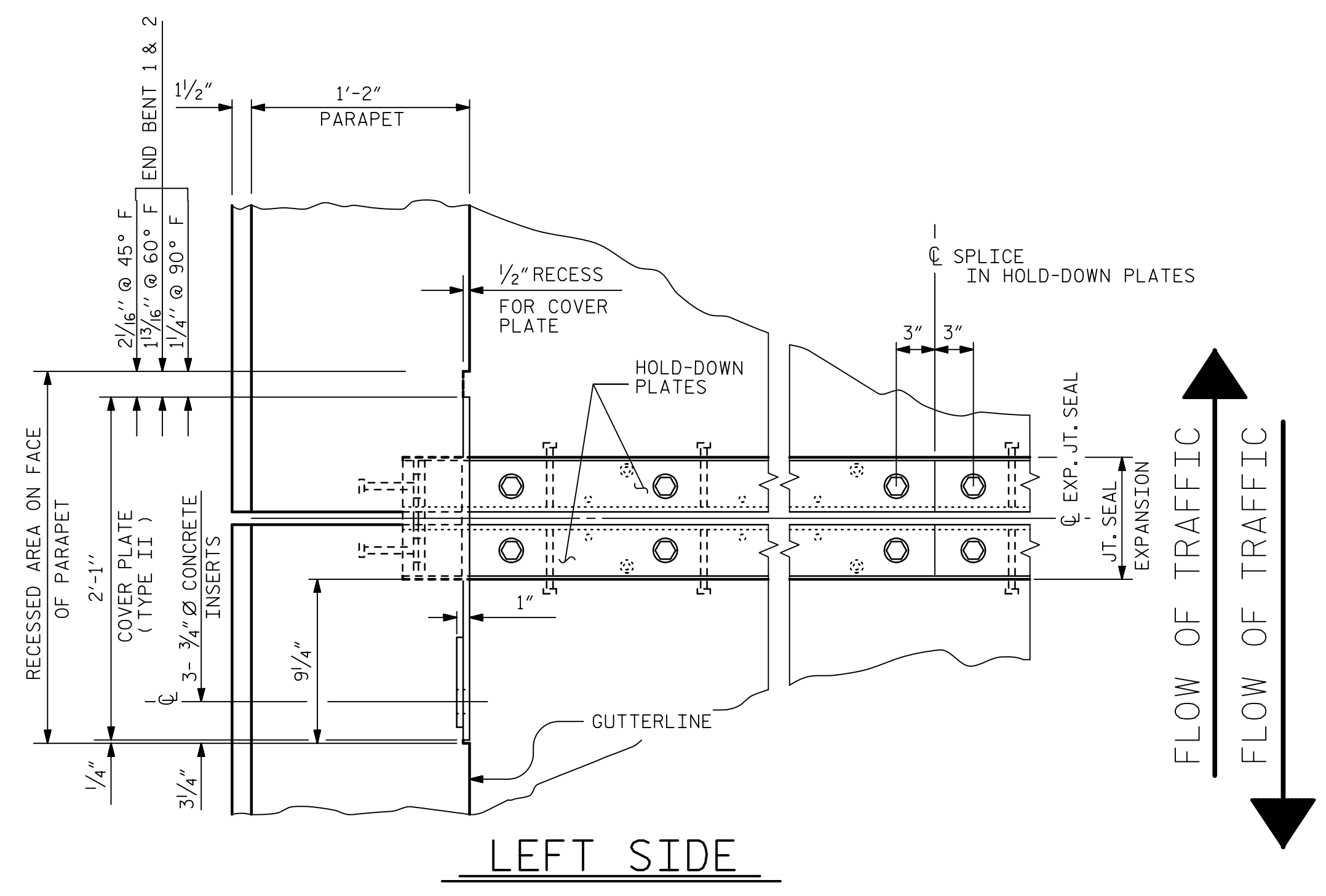
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-------|
| EXPANSION JOINT SEAL DETAILS FOR CONCRETE PARAPET RAIL | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. **S-29**
TOTAL SHEETS **86**

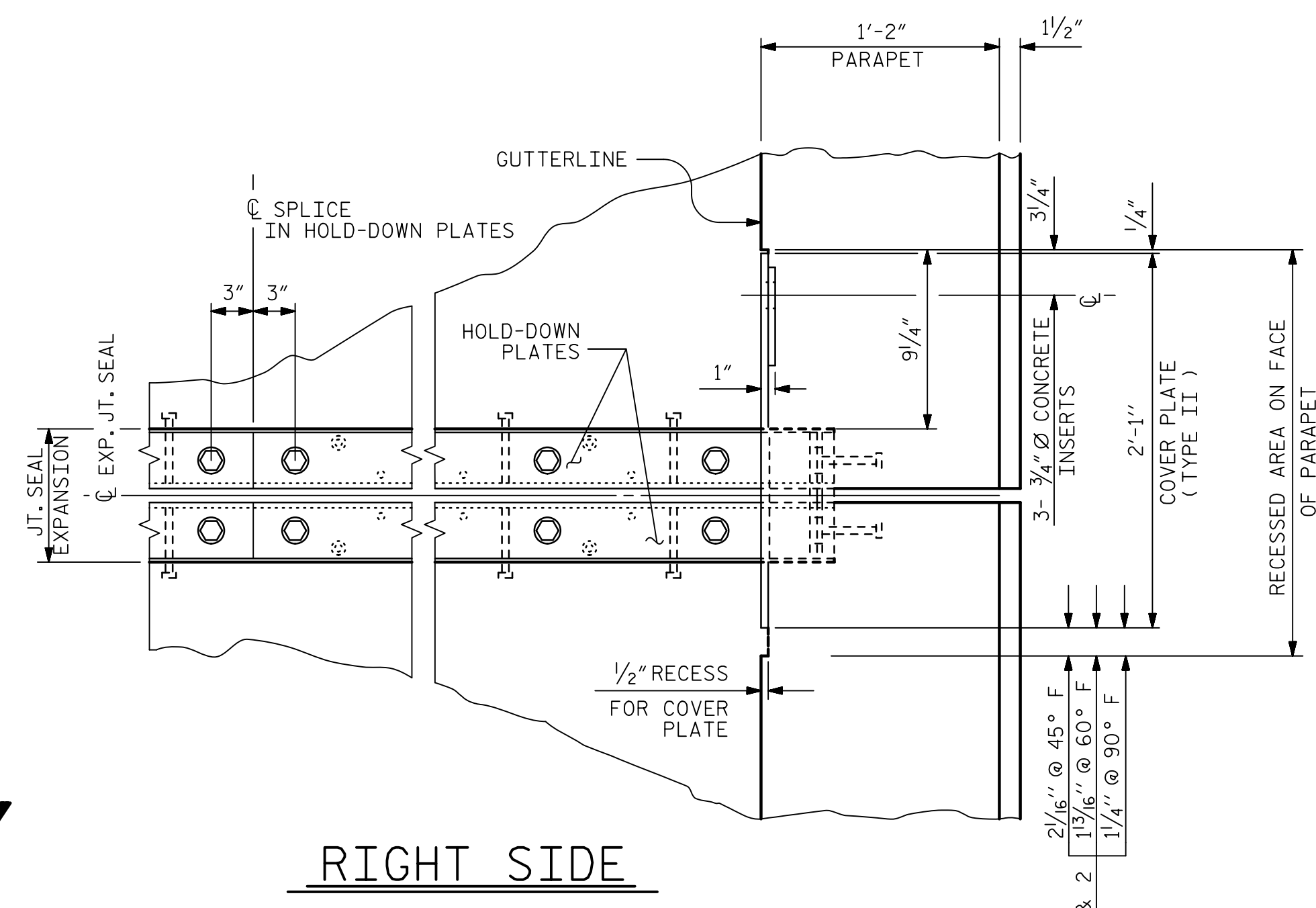
DRAWN BY: KHC DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

1/12/2015



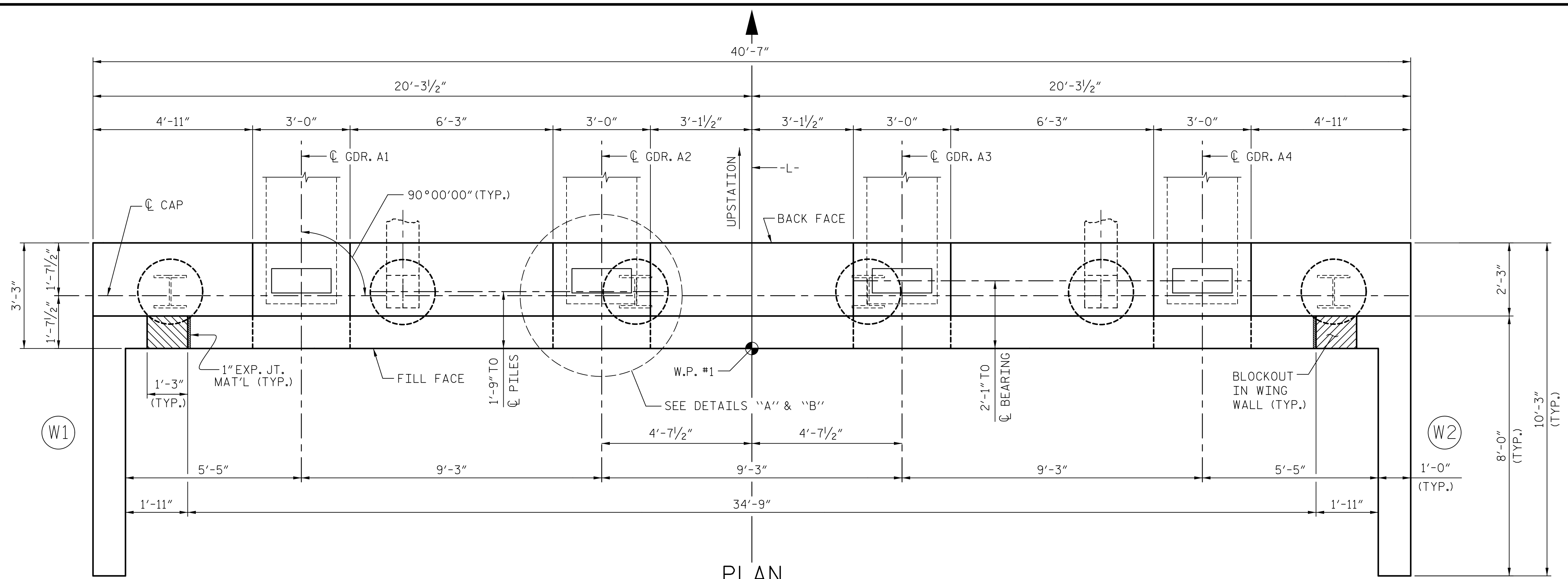
LEFT SIDE



RIGHT SIDE

PLAN OF EXPANSION JOINT SEAL

↑ FLOW OF TRAFFIC
↓ FLOW OF TRAFFIC



NOTES:

FOR PILE SPLICE DETAILS, SEE END BENT 1, SHEET 3 OF 3.

FOR SECTION A-A, SEE END BENT 1, SHEET 3 OF 3.

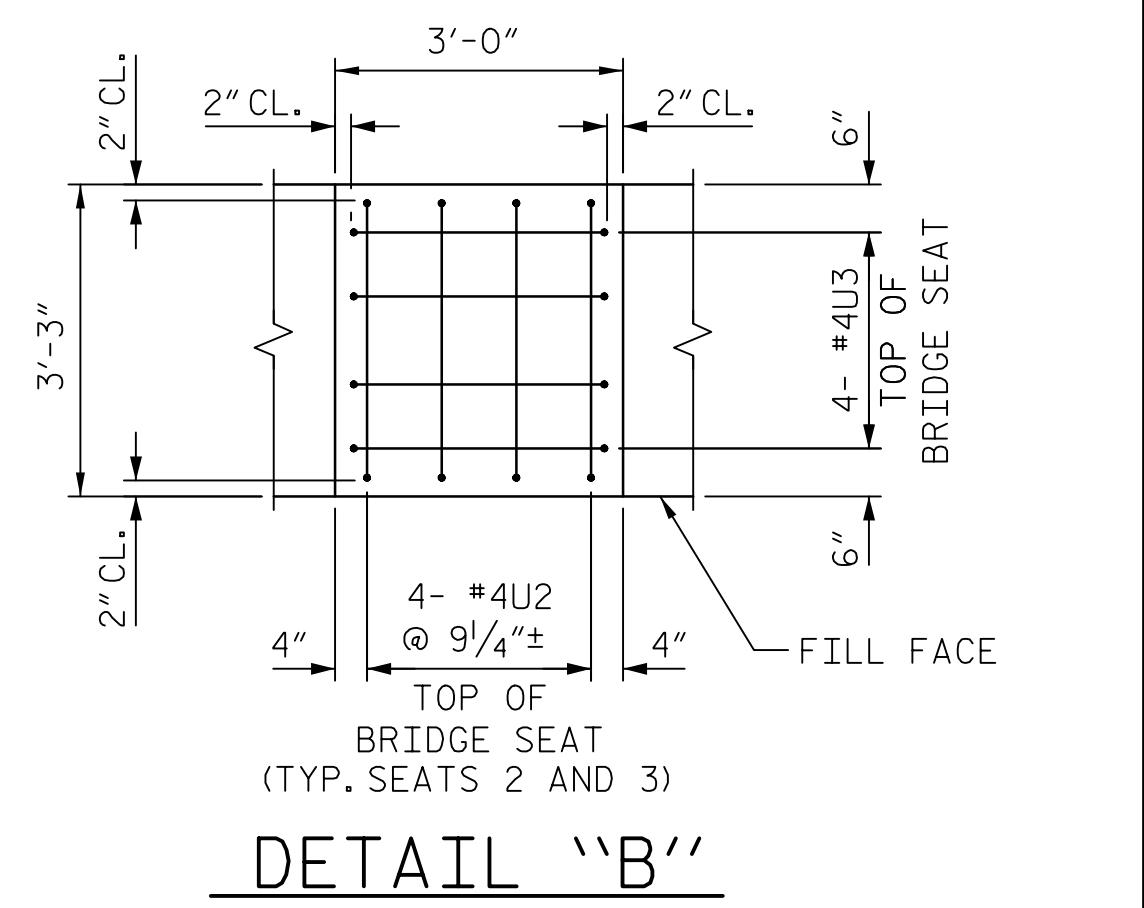
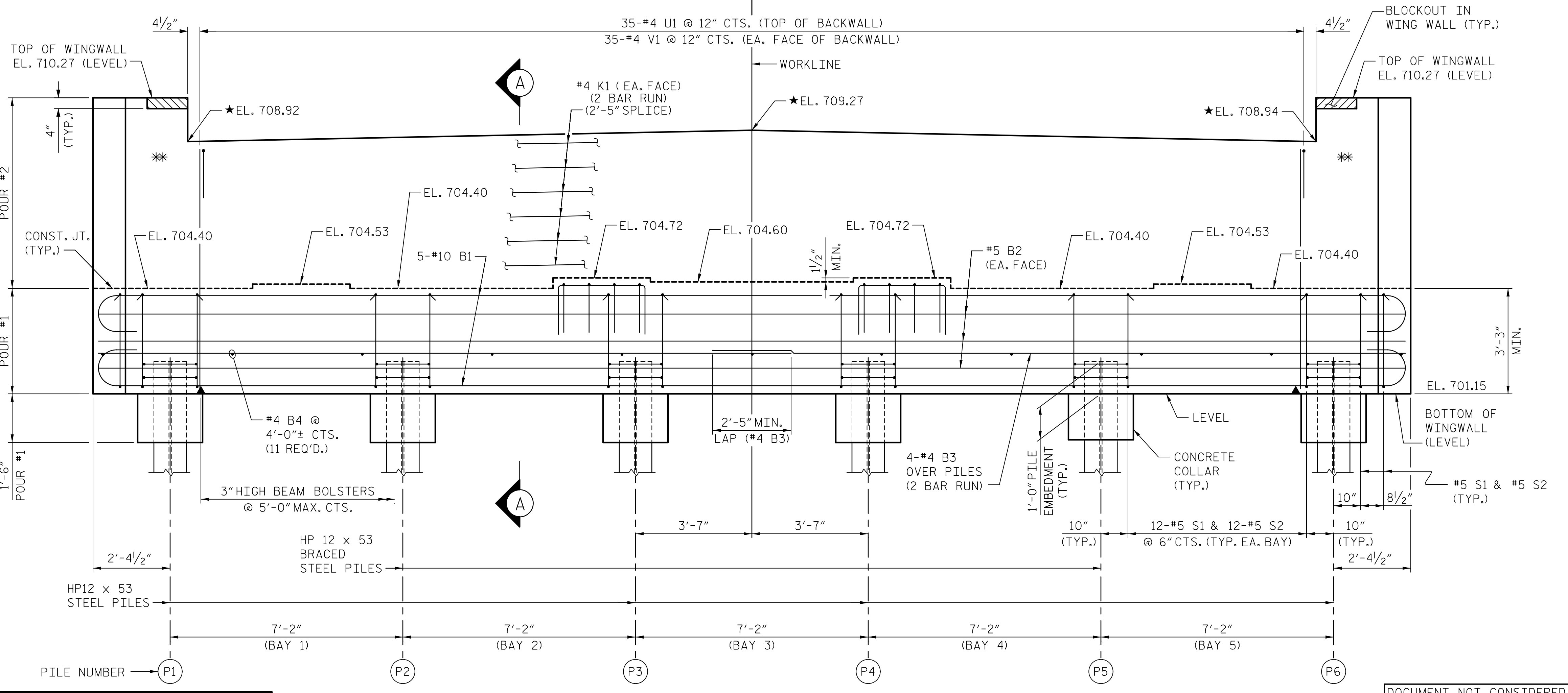
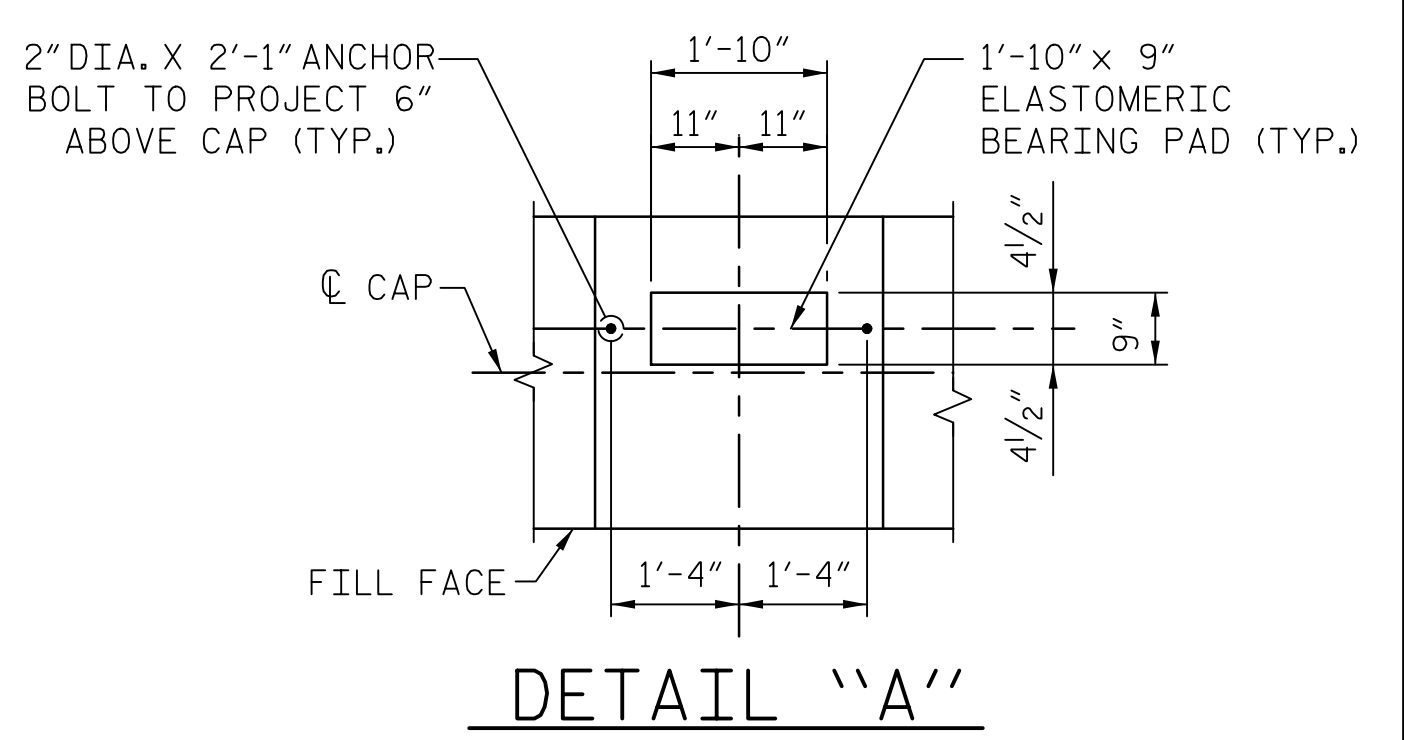
FOR DETAILS OF LOWER WINGWALLS AND REINFORCING STEEL, SEE END BENT 1, SHEET 2 OF 3.

FOR TEMPORARY DRAINAGE DETAILS, SEE END BENT 1, SHEET 3 OF 3.

FOR BLOCKOUT IN WINGWALL DETAIL, SEE END BENT 1, SHEET 3 OF 3.

★ THIS ELEVATION TAKEN ON FILL FACE OF THE BACKWALL

** REINFORCING IN WING NOT SHOWN FOR CLARITY. FOR DETAIL, SEE END BENT 1, SHEET 2 OF 3.



PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 90+25.00 -L-

SHEET 1 OF 3 BRIDGE NO. 656

AECOM

AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-6200 www.aecom.com
AECOM License No. F-0342

SEAL
NORTH CAROLINA PROFESSIONAL ENGINEER
John C. Morrison
12/12/2016

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

**SUBSTRUCTURE
END BENT 1
PLAN AND ELEVATION**

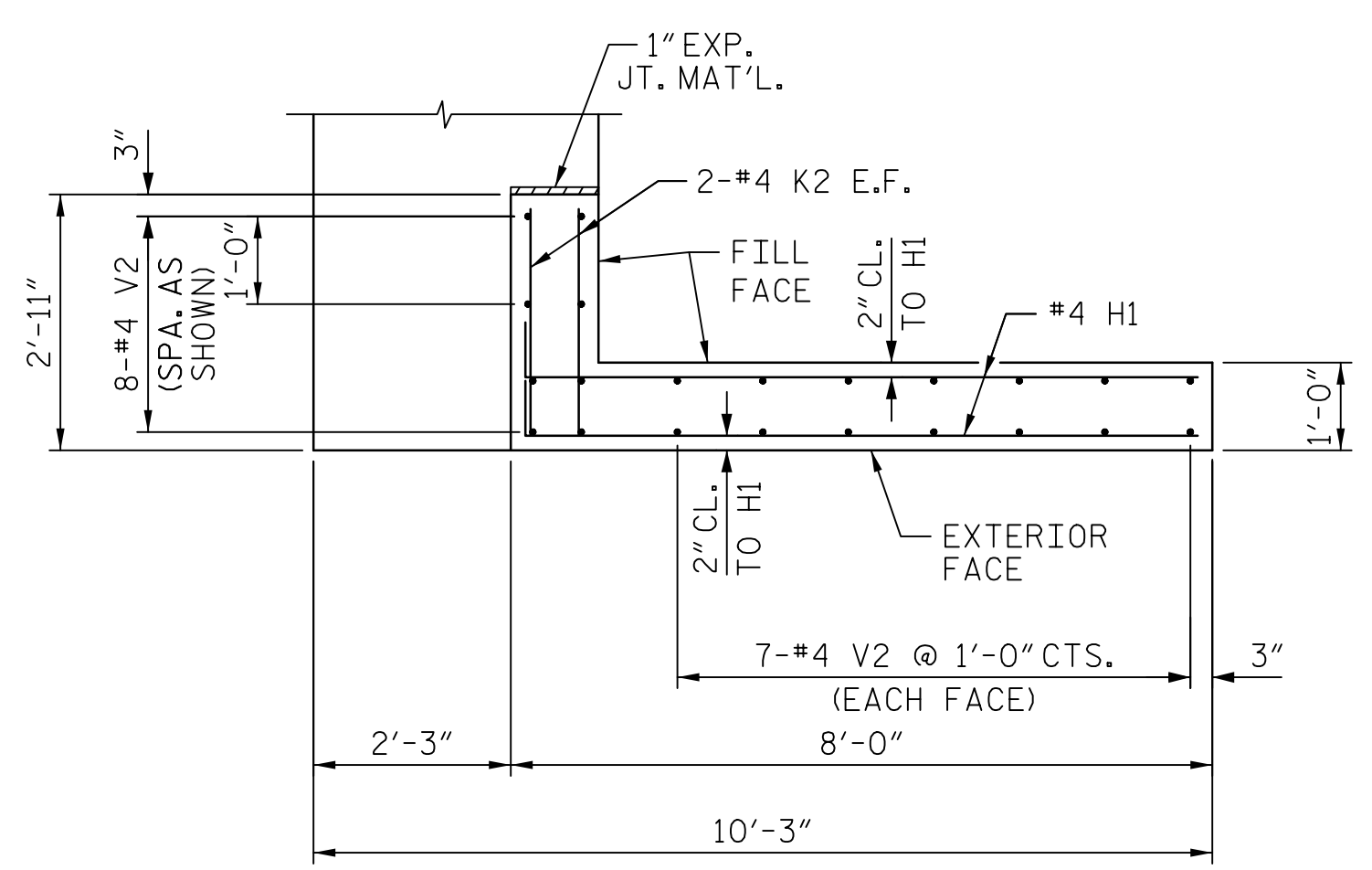
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-31**
TOTAL SHEETS **86**

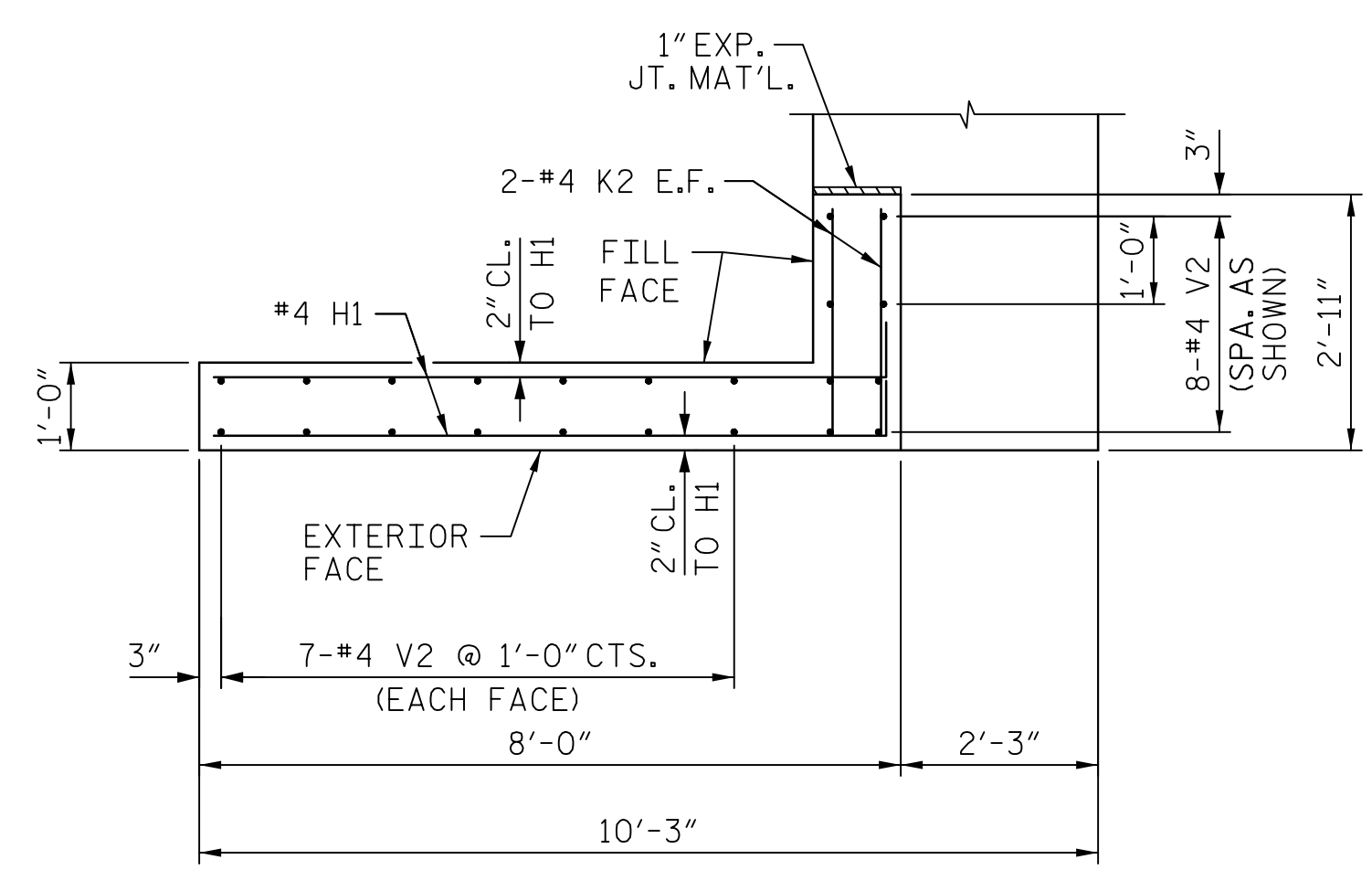
DRAWN BY: MKT DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

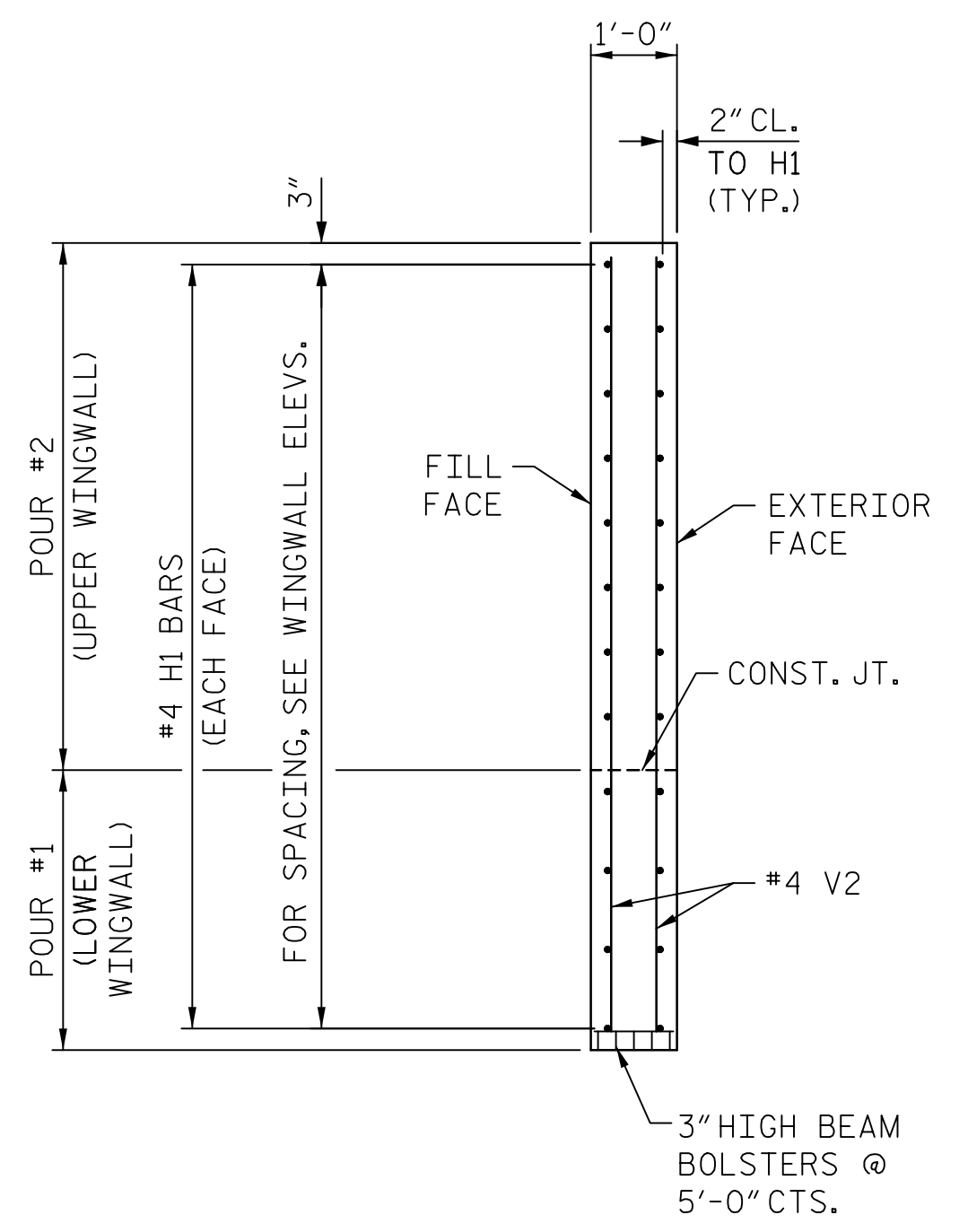
DATE: 11/12/2015 TIME: 9:25:32 AM
USER: A:\Engineering\656\Card\40_L06_L12707_SML_531_EBL.dwg



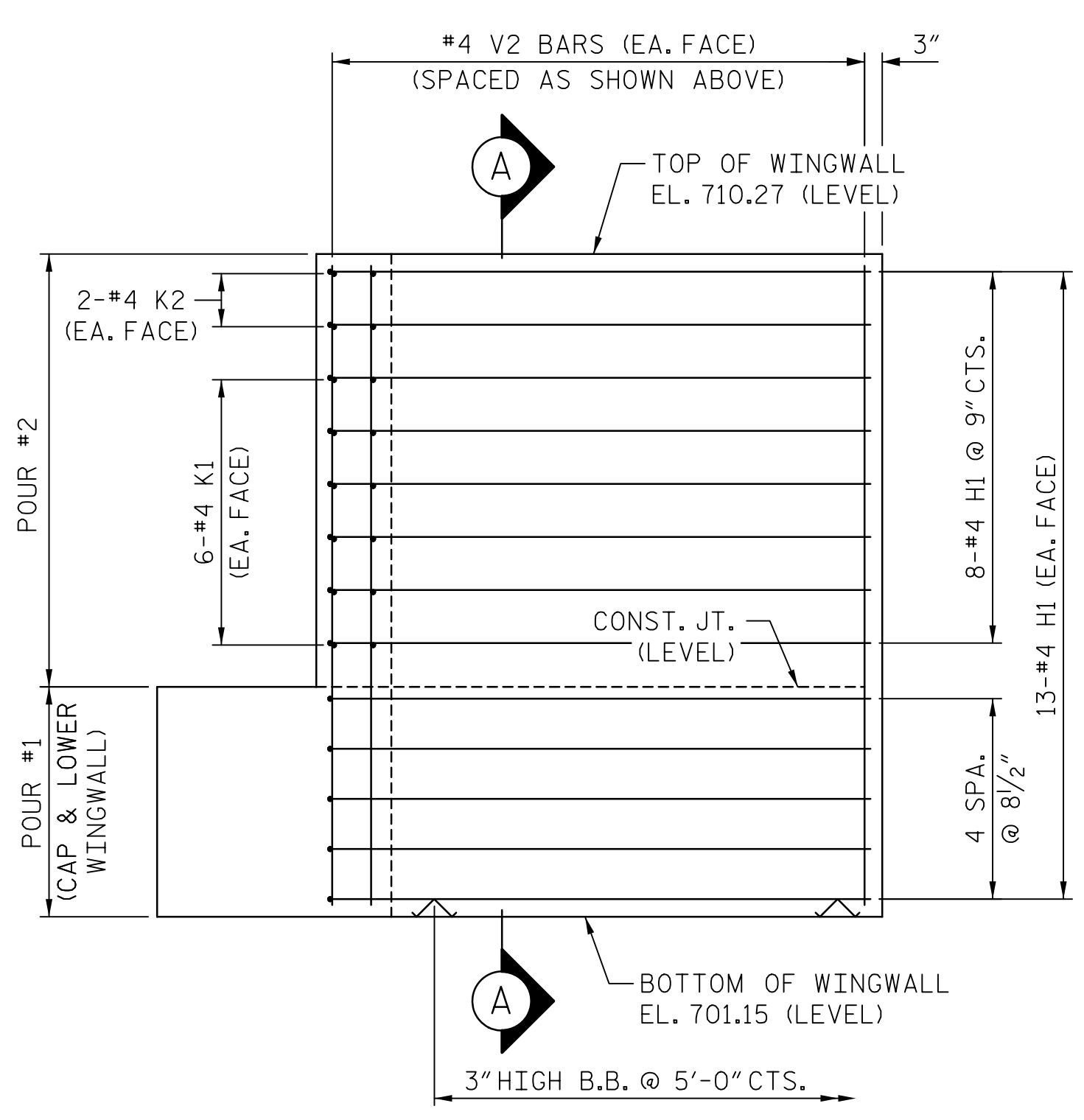
PLAN - WINGWALL 1



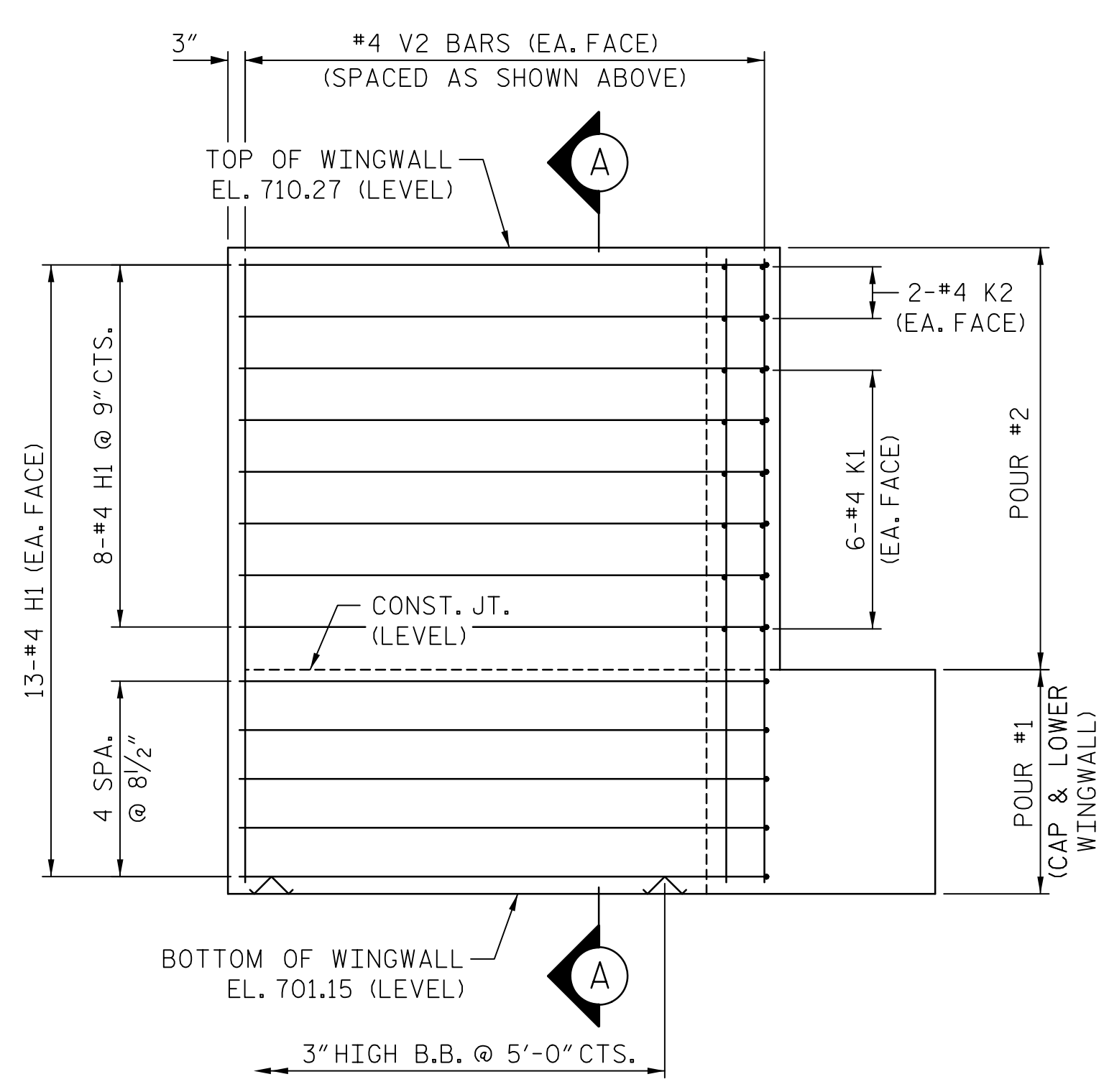
PLAN - WINGWALL 2



SECTION "A-A"



ELEVATION - WINGWALL 1



ELEVATION - WINGWALL 2

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 2 OF 3 BRIDGE NO. 656



| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|--------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUBSTRUCTURE END BENT 1 DETAILS | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |
| SHEET NO. S-32 | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

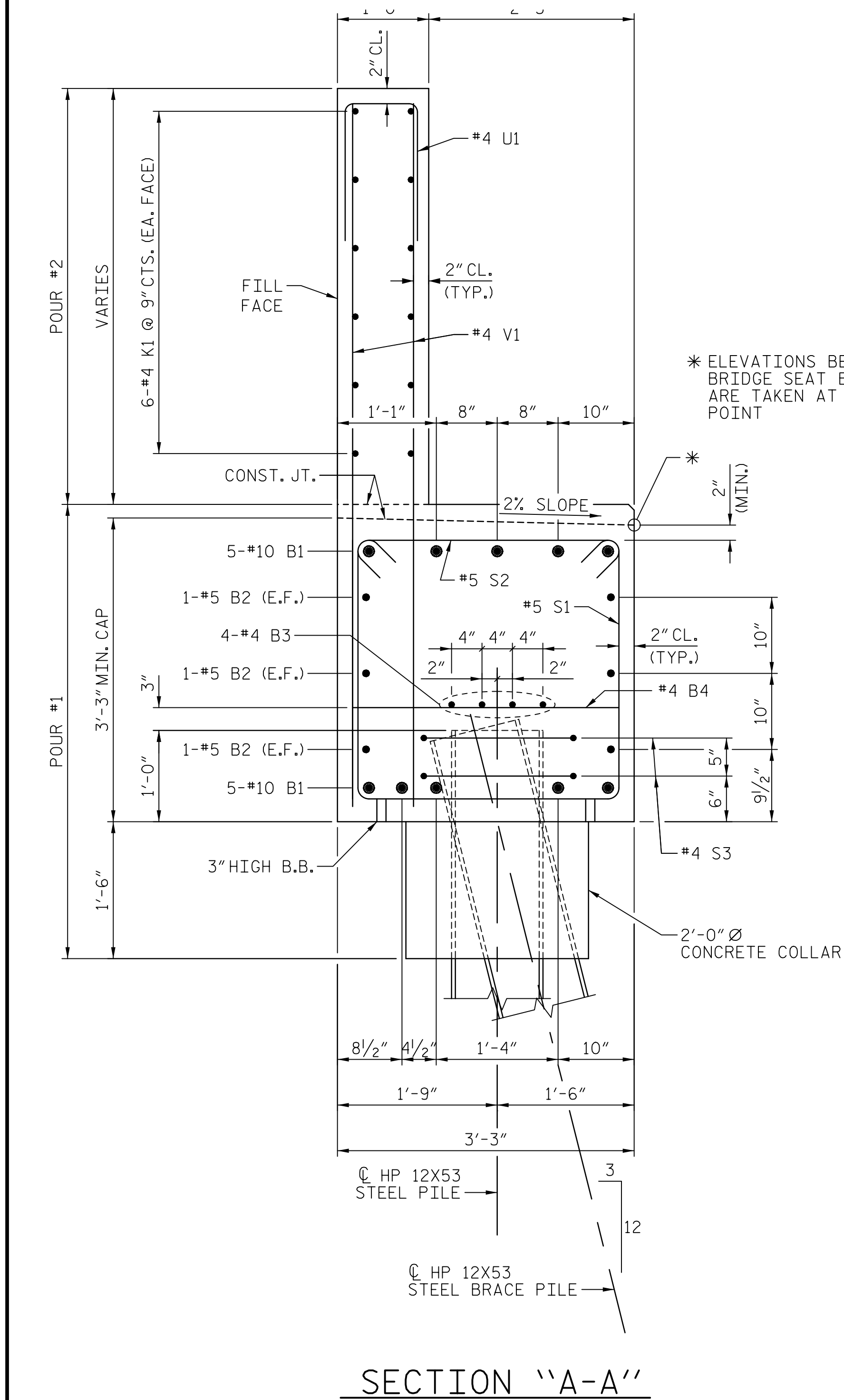
DATE: 1/12/2016
 TIME: 9:25:38 AM

USER: R:\a\c\morrison
 DN: R:\a\c\morrison\656\Card\40_L063_U2707_SML_S32_EBI_02.dgn

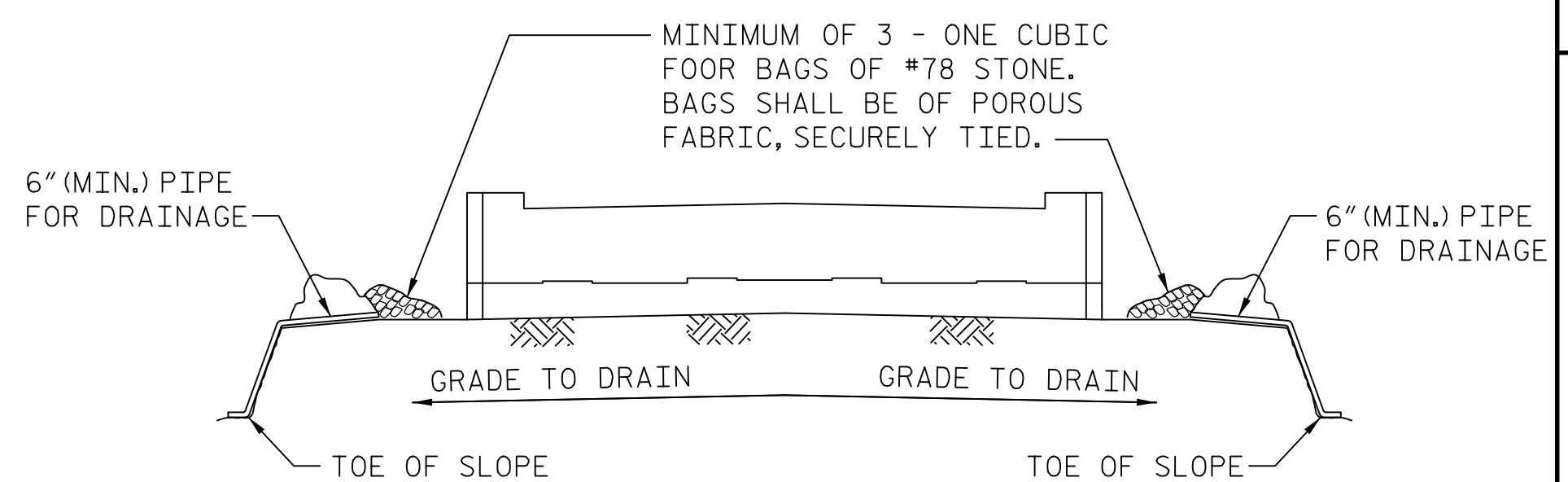
DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DATE: 1/12/2016 TIME: 9:25:45 AM

USER: \\s:\corporate\AECOM\Drawings\2015\12\2707_SML\533_EBI_03.dgn



SECTION "A-A"

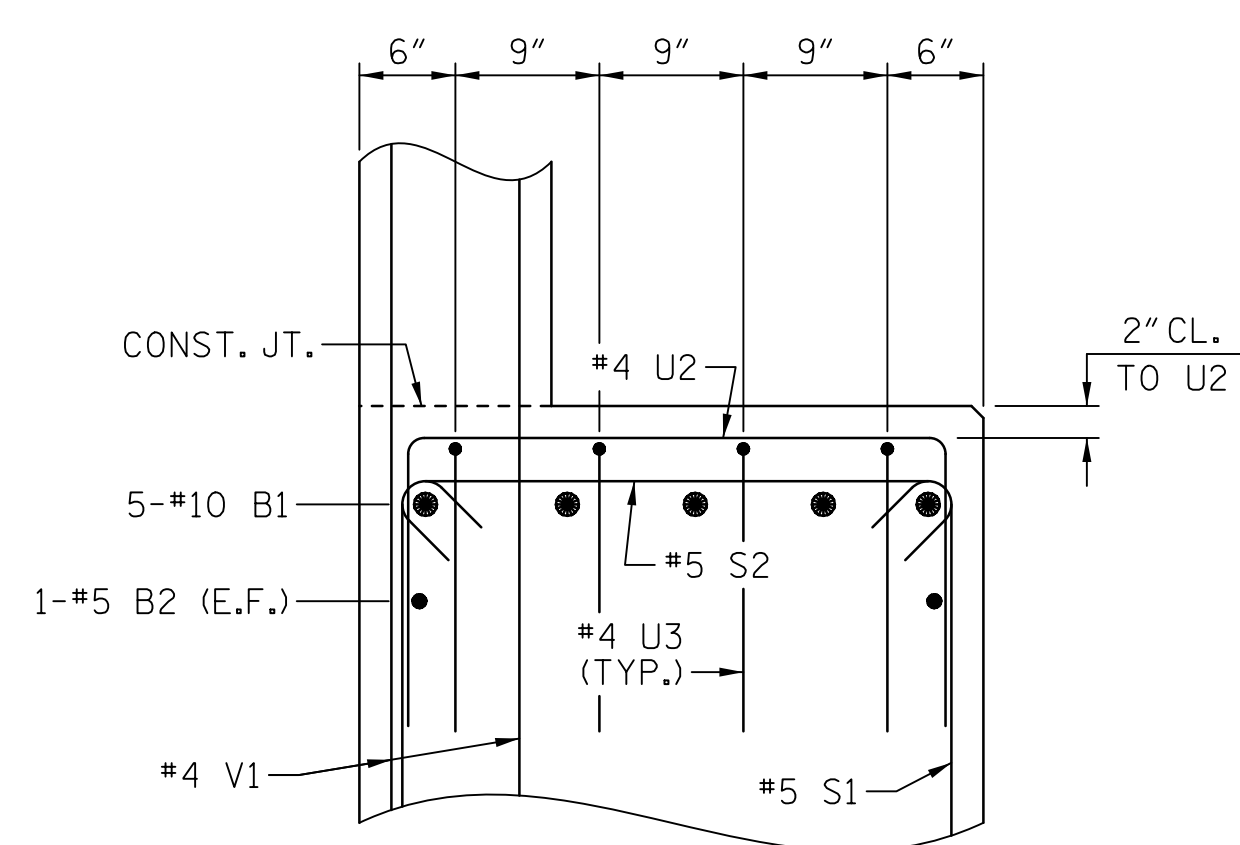


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

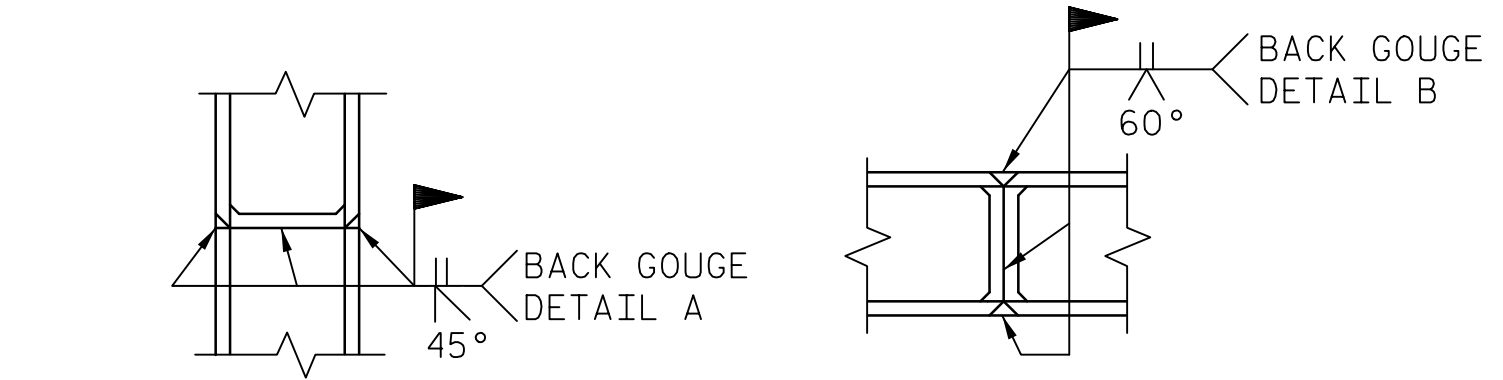
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

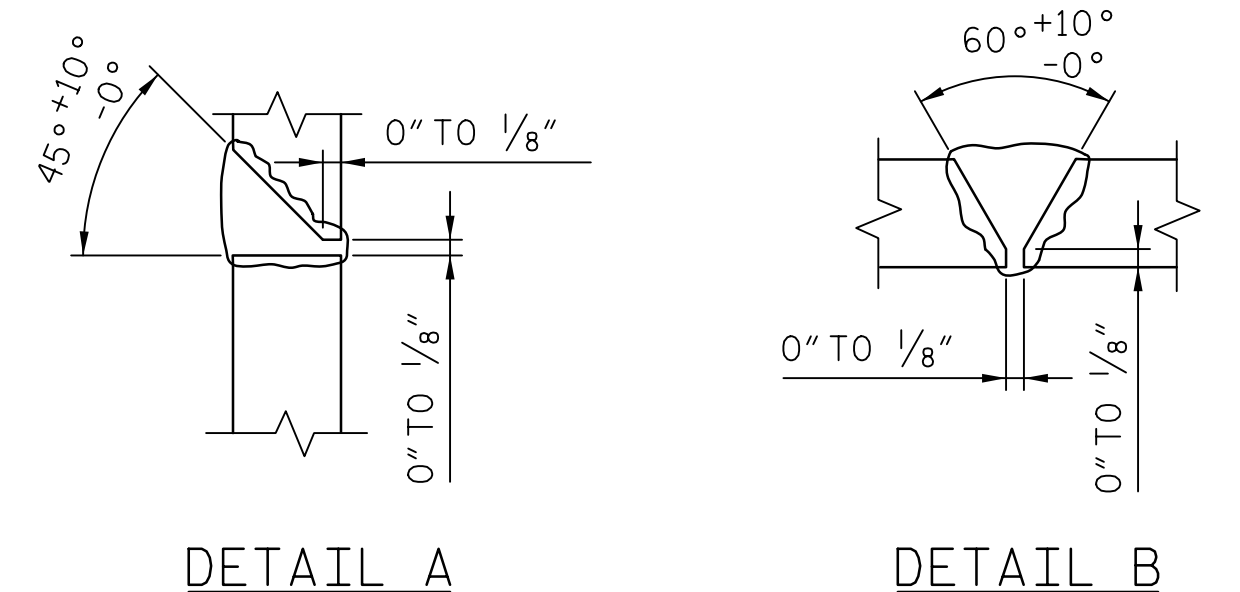
TEMPORARY DRAINAGE AT END BENT



BRIDGE SEAT SECTION



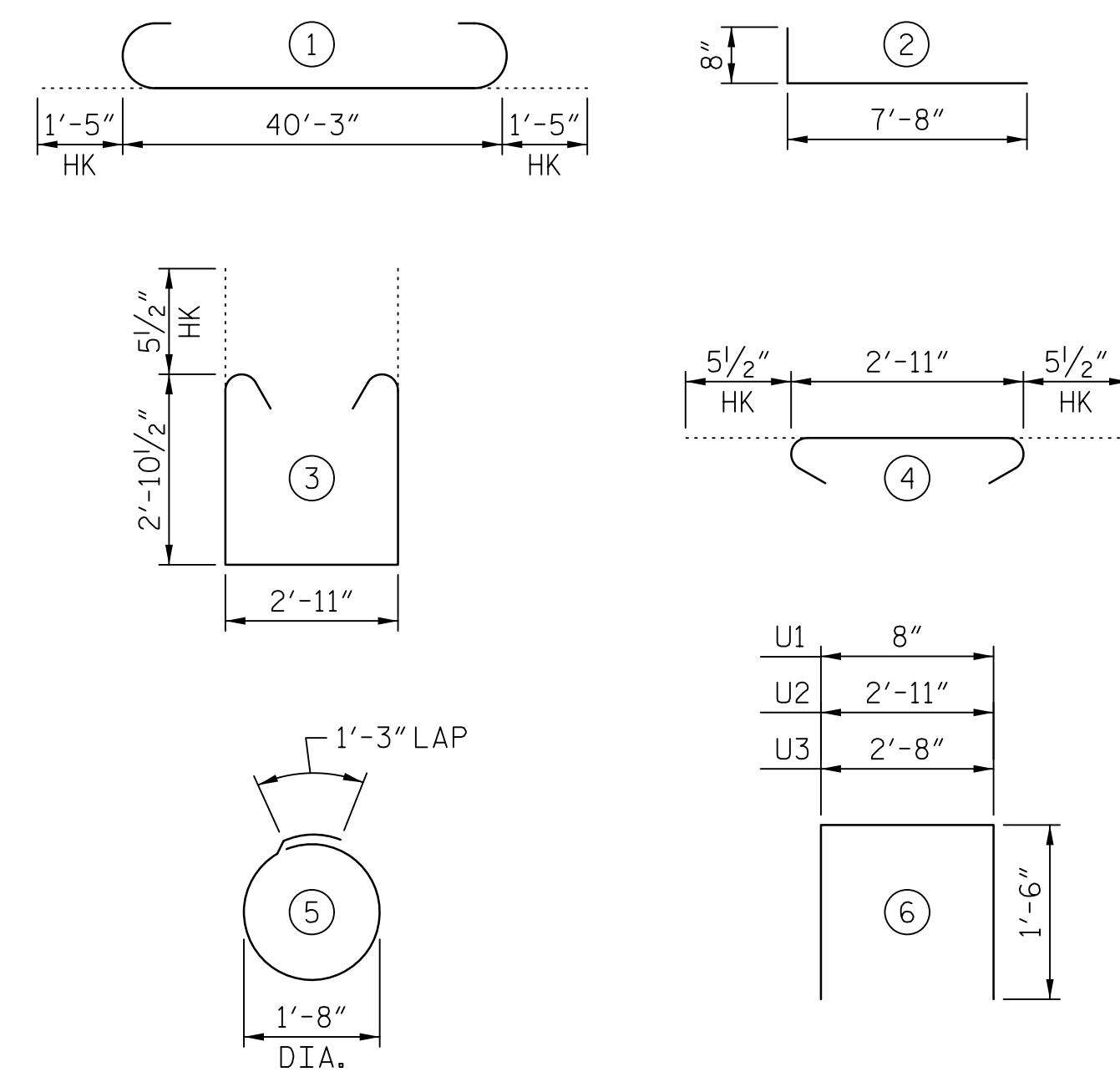
*PILE VERTICAL *PILE HORIZONTAL OR VERTICAL



*POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BAR TYPE



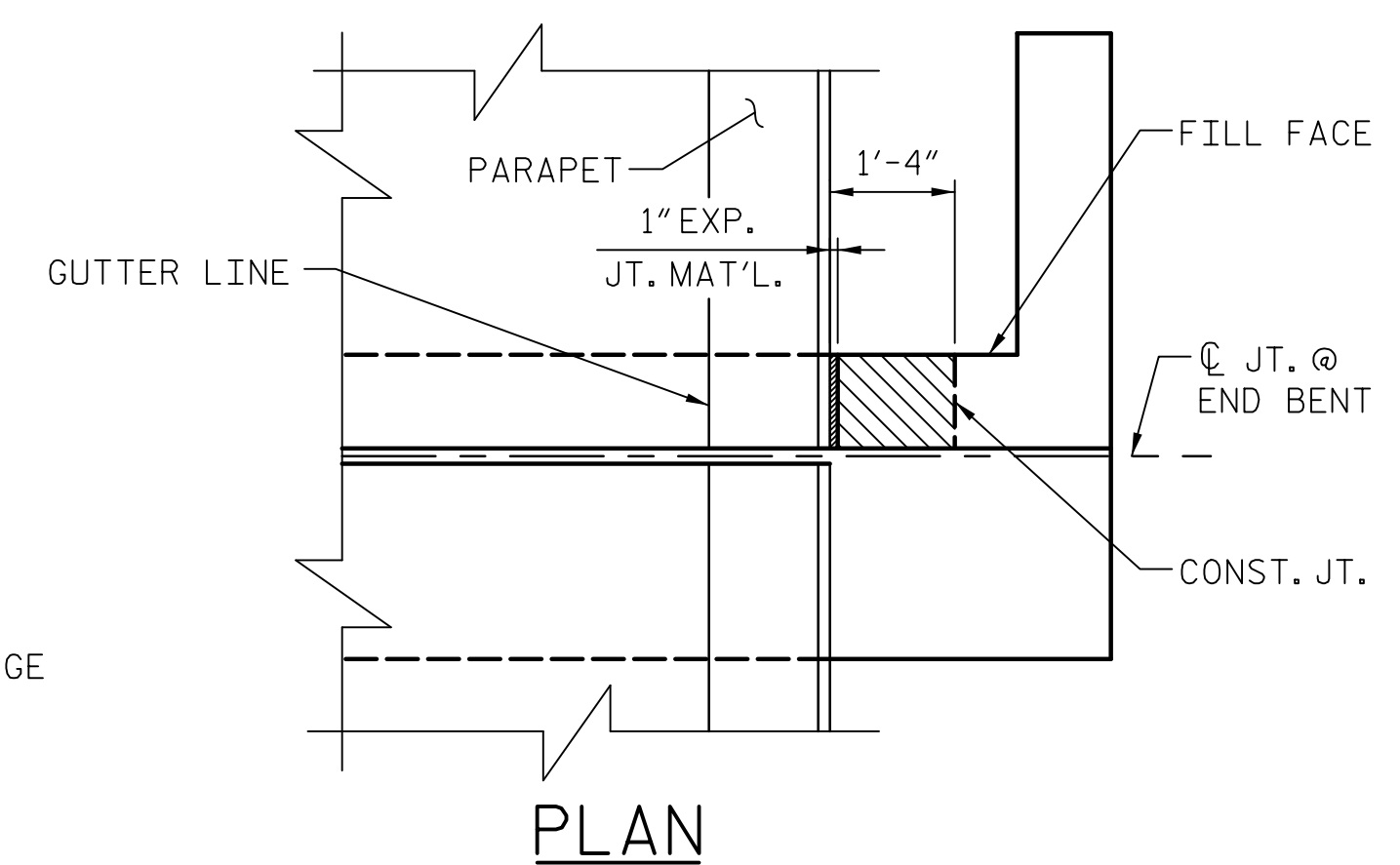
ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

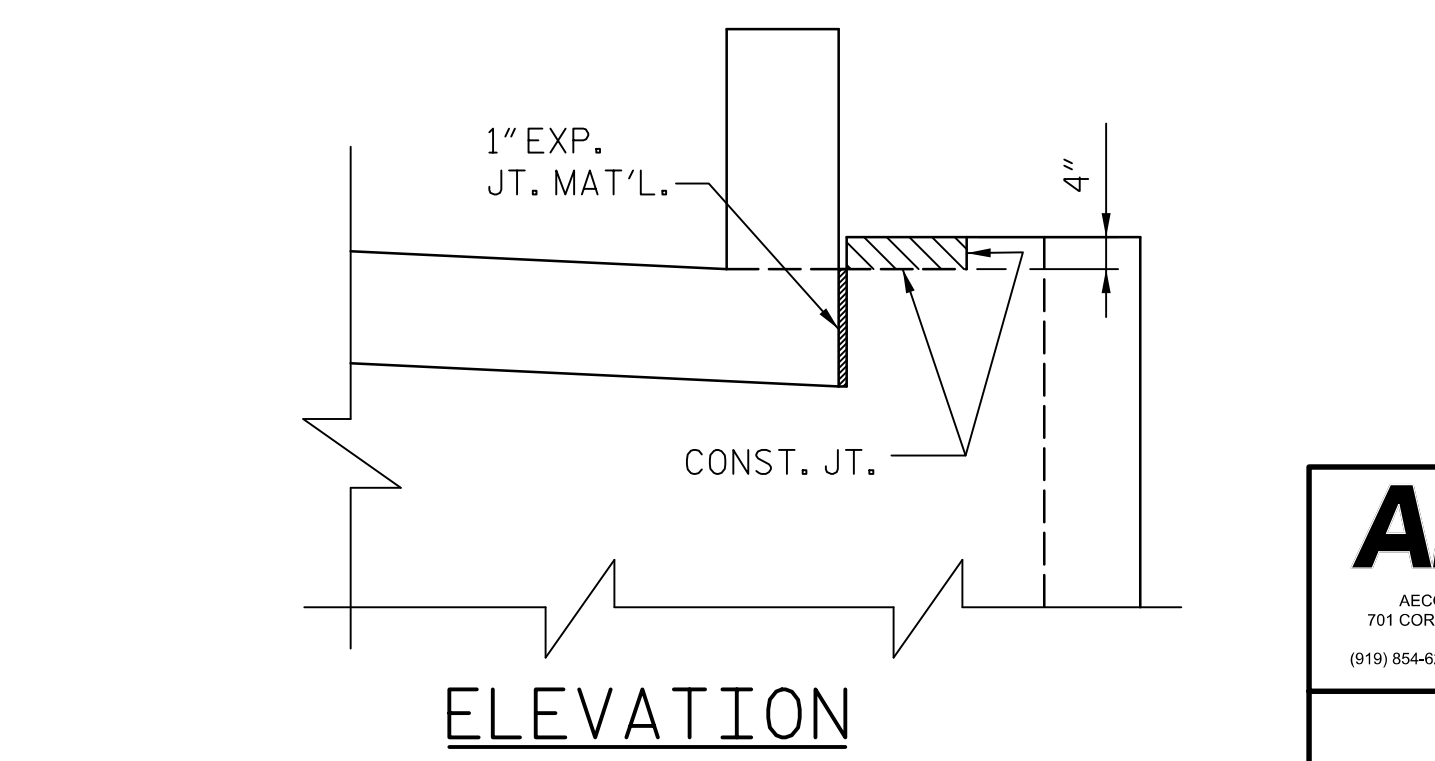
| END BENT 1 | | | | | |
|------------------------------------------|------|------|--------|--------|----------------|
| BAR NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| B1 | 10 | #10 | ① | 43'-1" | 1854 |
| B2 | 6 | #5 | STR | 40'-3" | 252 |
| B3 | 8 | #4 | STR | 21'-4" | 114 |
| B4 | 11 | #4 | STR | 2'-11" | 21 |
| | | | | | |
| H1 | 52 | #4 | ② | 8'-4" | 289 |
| | | | | | |
| K1 | 24 | #4 | STR | 21'-4" | 342 |
| K2 | 8 | #4 | STR | 2'-7" | 14 |
| | | | | | |
| S1 | 64 | #5 | ③ | 9'-7" | 640 |
| S2 | 64 | #5 | ④ | 3'-10" | 256 |
| S3 | 12 | #4 | ⑤ | 6'-6" | 52 |
| | | | | | |
| U1 | 35 | #4 | ⑥ | 3'-8" | 86 |
| U2 | 8 | #4 | ⑥ | 5'-11" | 32 |
| U3 | 8 | #4 | ⑥ | 5'-8" | 30 |
| | | | | | |
| V1 | 70 | #4 | STR | 7'-5" | 347 |
| V2 | 44 | #4 | STR | 8'-9" | 257 |
| TOTAL REINFORCING STEEL | | | | | 4586 LBS. |
| CLASS A CONCRETE | | | | | |
| POUR #1 (CAP, COLLARS & LOWER WINGWALLS) | | | | | 19.1 C.Y. |
| POUR #2 (BACKWALL & UPPER WINGWALL) | | | | | 10.2 C.Y. |
| TOTAL = | | | | | 29.3 C.Y. |
| HP 12x53 STEEL PILES: | | | | | |
| NO. = 6 | | | | | LIN. FT. = 210 |

NOTES:

- FOR OTHER NOTES, SEE "FOUNDATION LAYOUT" SHEET AND "LOCATION SKETCH" SHEET.
- THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET AND END POST ARE CAST IF SLIP FORMING IS USED.
- STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
- BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.
- THE TOP SURFACE OF THE END BENT CAP, EXCEPT THE BRIDGE SEAT BUILDUPS, SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.
- THE TOP SURFACE AREAS OF THE END BENTS CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.
- INSTALL THE 4" Ø DRAIN PIPE THROUGH THE WINGWALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS, REINFORCING STEEL IN THE WINGWALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



PLAN



ELEVATION

BLOCKOUT IN WINGWALL DETAILS

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 3 OF 3 BRIDGE NO. 656

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

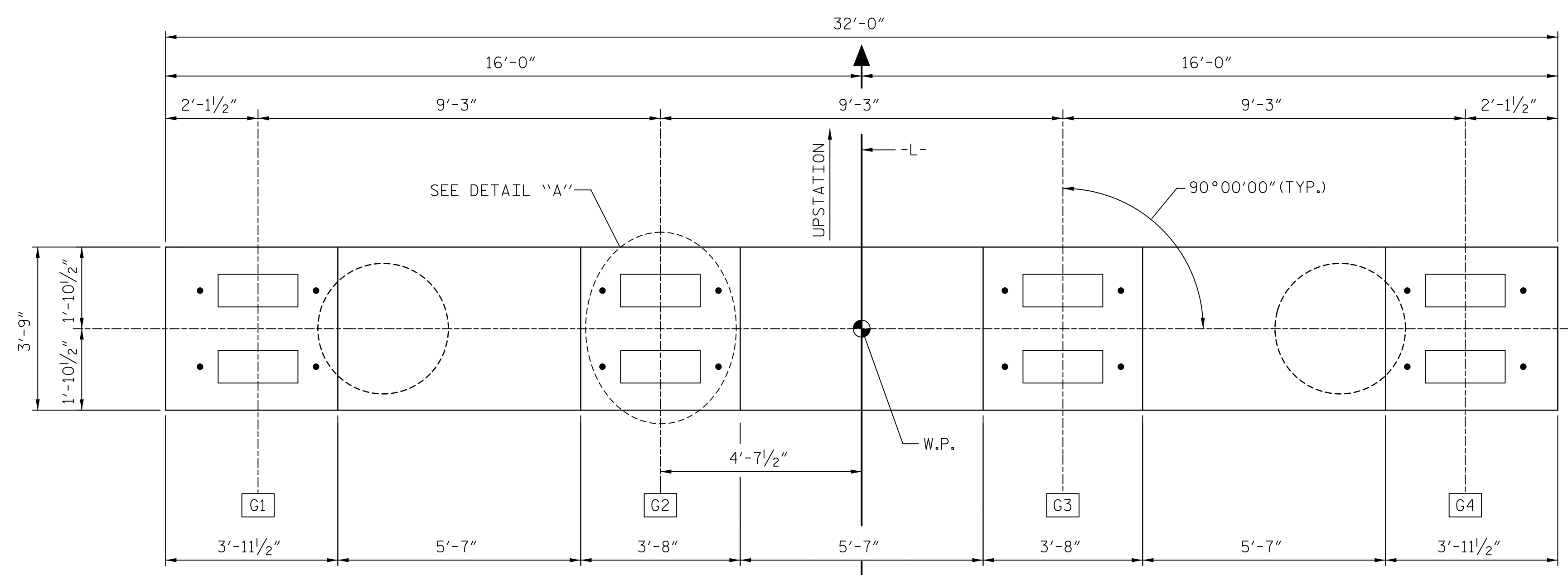
SEAL
 JOHN C. MORRISON
 ENGINEER
 1/12/2016

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-33 |
| | | | | | | TOTAL SHEETS |
| | | | | | | 86 |

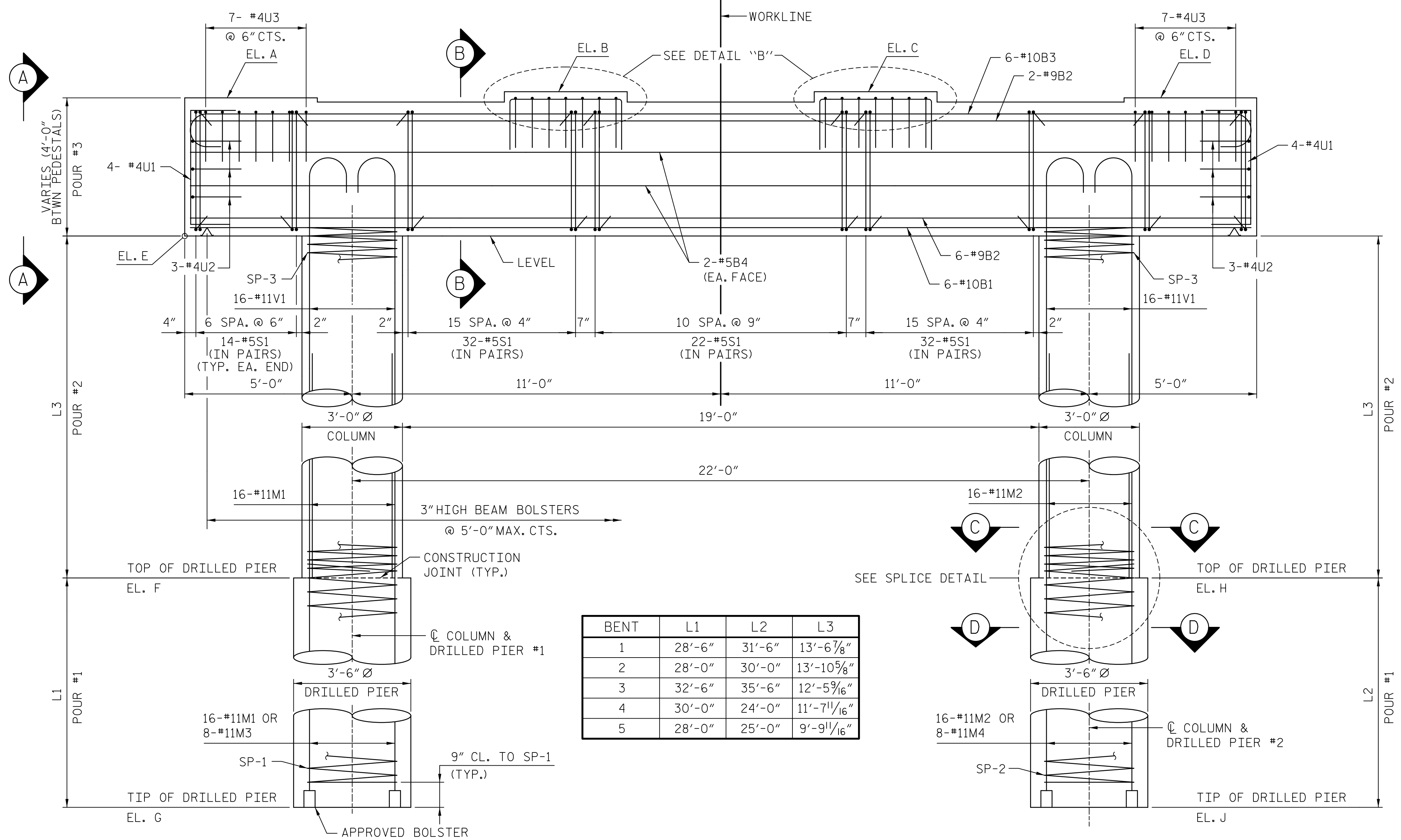
DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES:
 STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.
 HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.
 ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCEMENT STEEL".
 THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.
 SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIERS WILL NOT BE PERMITTED.
 THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT ONE FOOT BELOW THE GROUND LINE.

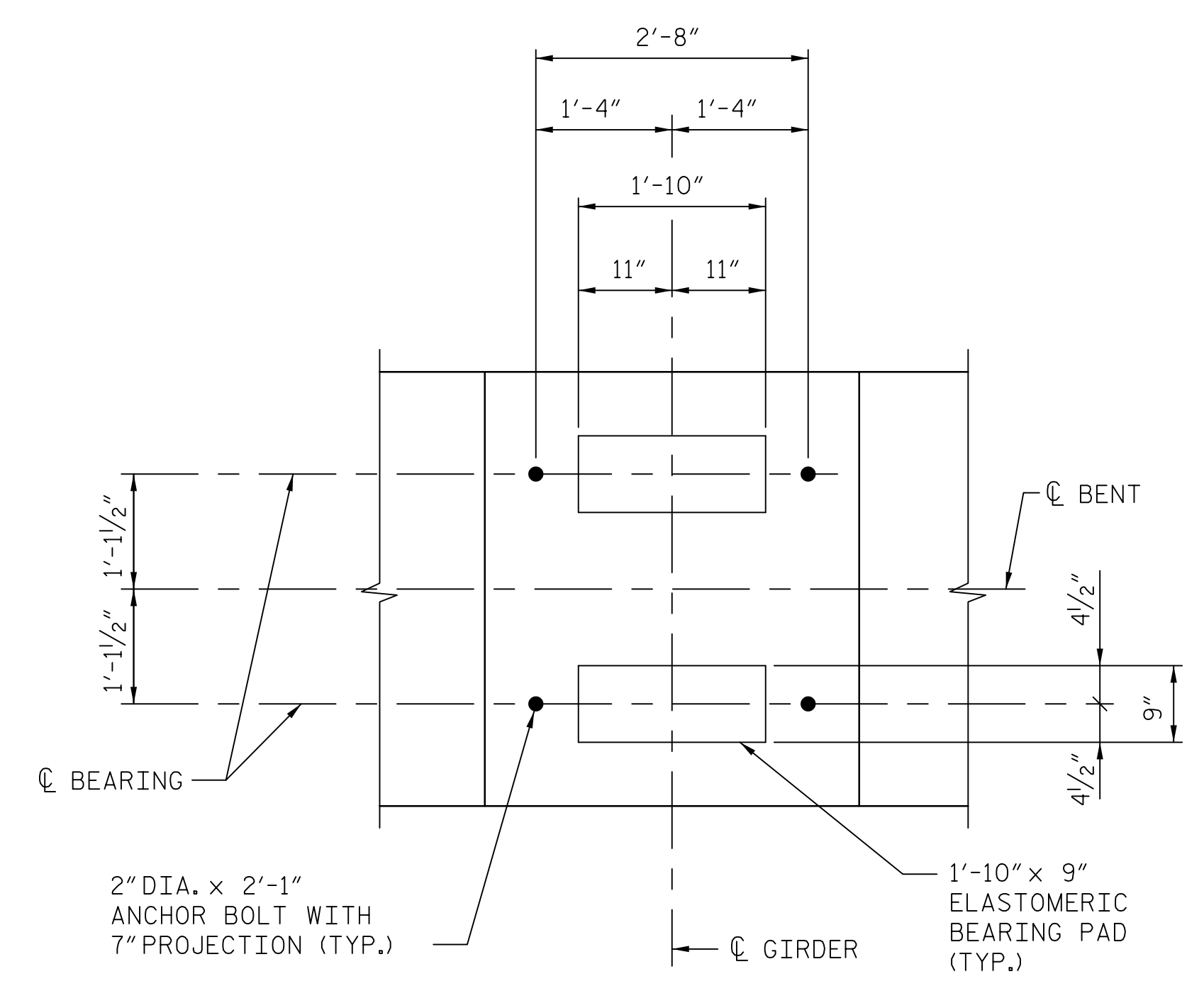


PLAN



| BENT | L1 | L2 | L3 |
|------|--------|--------|--------------|
| 1 | 28'-6" | 31'-6" | 13'-6 7/8" |
| 2 | 28'-0" | 30'-0" | 13'-10 5/8" |
| 3 | 32'-6" | 35'-6" | 12'-5 9/16" |
| 4 | 30'-0" | 24'-0" | 11'-7 11/16" |
| 5 | 28'-0" | 25'-0" | 9'-9 11/16" |

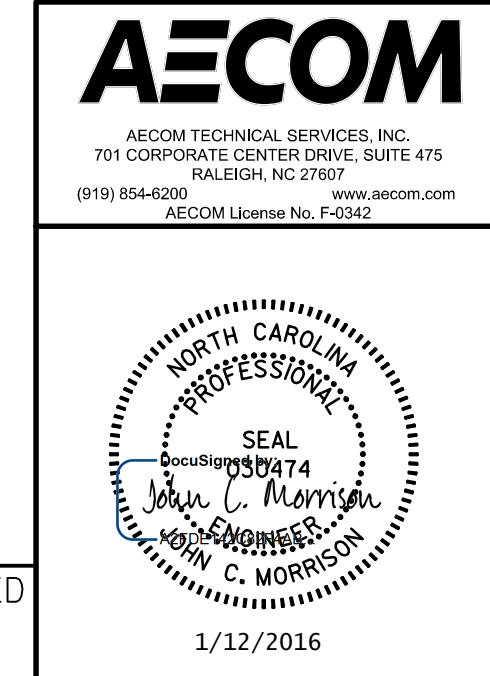
ELEVATION



DETAIL "A"

| ELEVATIONS | | | | | | | | | |
|------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| BENT | EL. A | EL. B | EL. C | EL. D | EL. E | EL. F | EL. G | EL. H | EL. J |
| 1 | 705.10 | 705.28 | 705.28 | 705.10 | 700.97 | 687.40 | 658.90 | 687.40 | 655.90 |
| 2 | 705.41 | 705.60 | 705.60 | 705.41 | 701.29 | 687.40 | 659.40 | 687.40 | 657.40 |
| 3 | 705.89 | 706.07 | 706.07 | 705.89 | 701.76 | 689.30 | 656.80 | 689.30 | 653.80 |
| 4 | 706.36 | 706.55 | 706.55 | 706.36 | 702.24 | 690.60 | 660.60 | 690.60 | 666.60 |
| 5 | 706.73 | 706.92 | 706.92 | 706.73 | 702.61 | 692.80 | 664.80 | 692.80 | 667.80 |

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 1 OF 3 BRIDGE NO. 656



| REVISIONS | | | | | | SHEET NO. | |
|-----------|-----|-------|-----|-----|-------|--------------|--|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-34 | |
| | | | | | | TOTAL SHEETS | |
| | | | | | | 86 | |

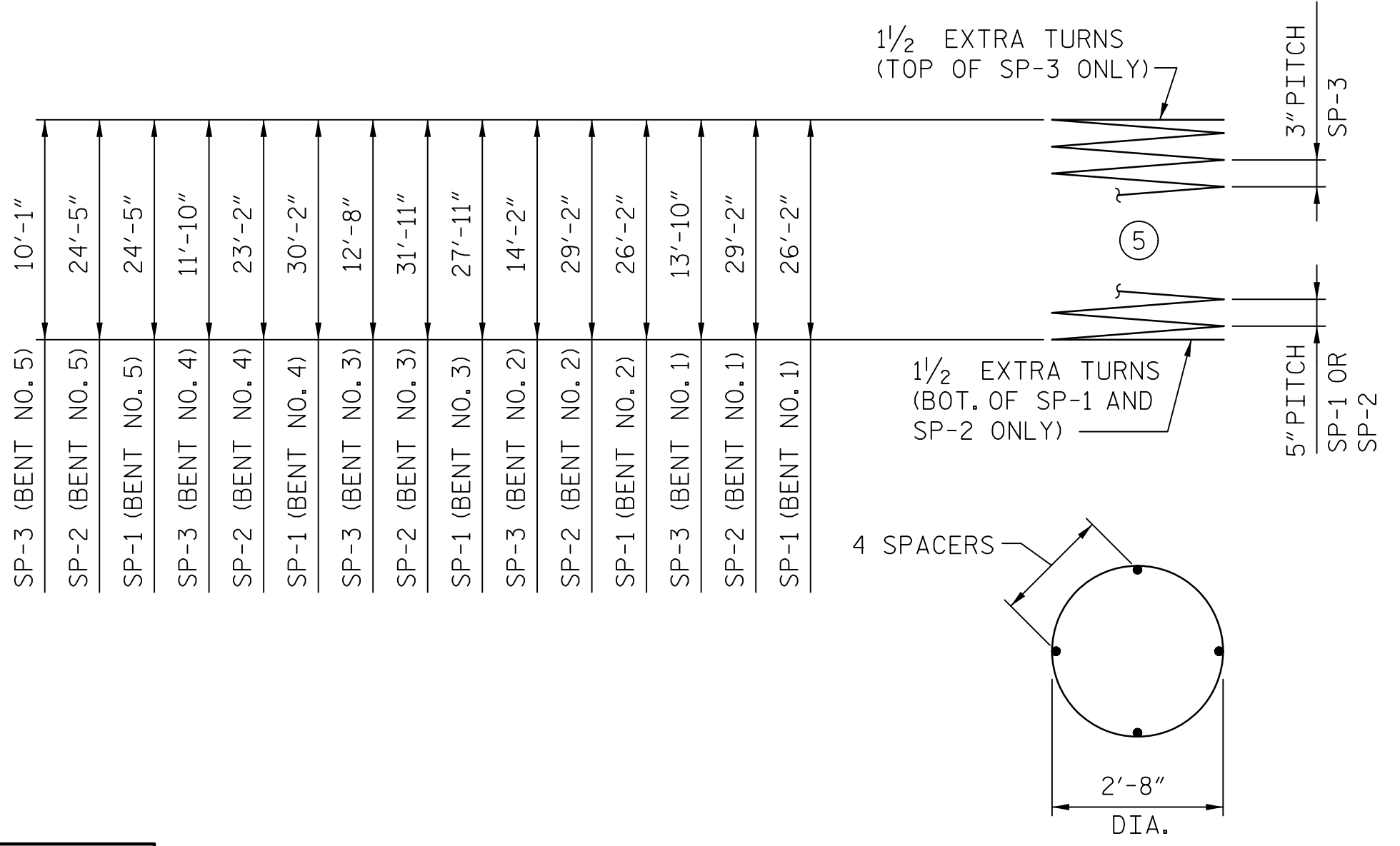
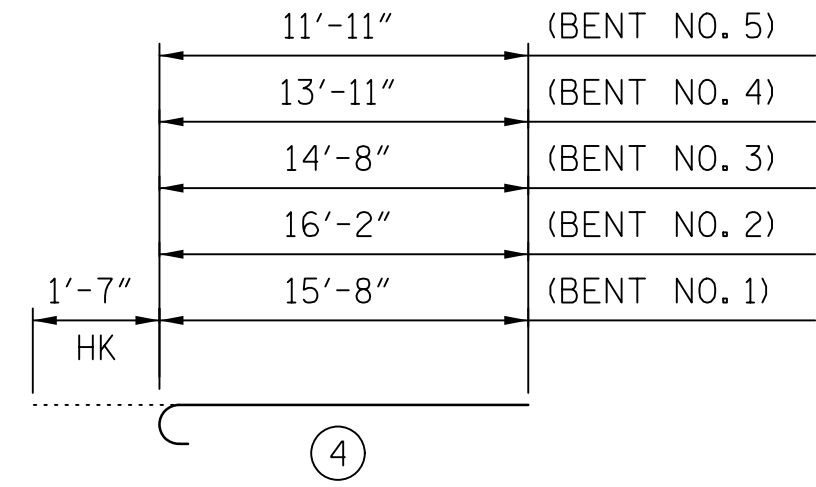
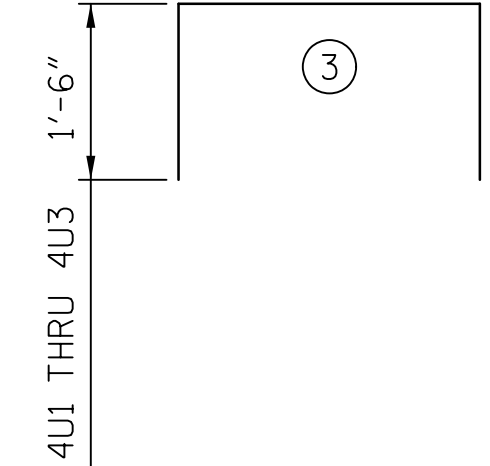
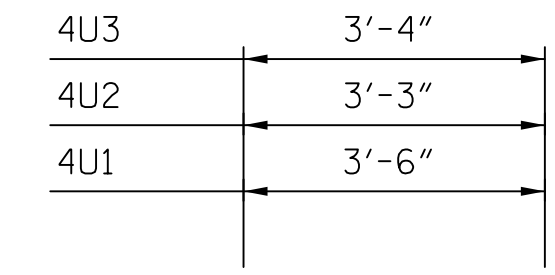
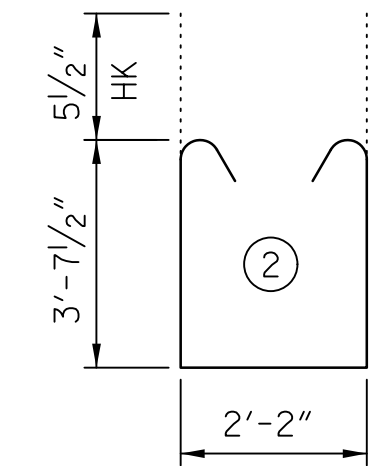
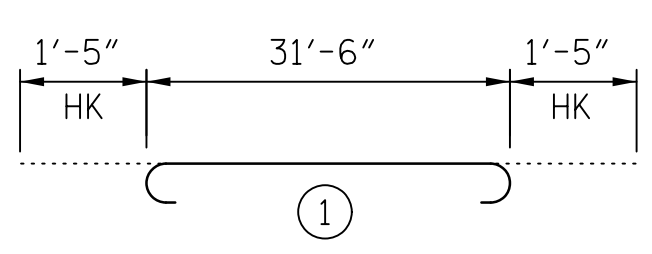
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016
 TIME: 9:25:50 AM

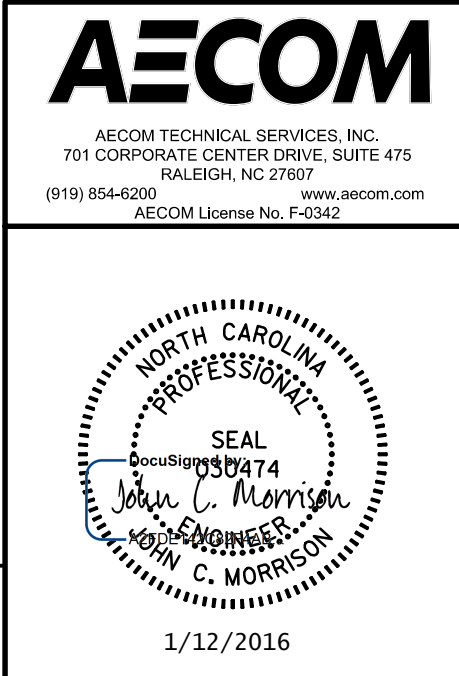
USER: kkc
 DGN: R:\Bridge\656\Card\40_L07_U2707_SML_S34_BTO.dgn

DRAWN BY: KHC DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

| BAR TYPES | | | | | | BILL OF MATERIAL | | | | | | BILL OF MATERIAL | | | | | | BILL OF MATERIAL | | | | | | BILL OF MATERIAL | | | | | | | | | | | |
|---------------------------------------------------------------------------------------------------------|-----|------|------|---------|--------|---------------------------------------------------------------------------------------------------------|-----|------|------|---------|--------|---------------------------------------------------------------------------------------------------------|-----|------|------|---------|--------|---------------------------------------------------------------------------------------------------------|-----|------|------|---------|--------|---------------------------------------------------------------------------------------------------------|-----|------|------|---------|--------|--------|--|--|--|--|--|
| ALL BAR DIMENSIONS ARE OUT TO OUT. | | | | | | BENT 1 | | | | | | BENT 2 | | | | | | BENT 3 | | | | | | BENT 4 | | | | | | BENT 5 | | | | | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | | | | | | |
| B1 | 6 | #10 | STR | 31'-6" | 814 | B1 | 6 | #10 | STR | 31'-6" | 814 | B1 | 6 | #10 | STR | 31'-6" | 814 | B1 | 6 | #10 | STR | 31'-6" | 814 | B1 | 6 | #10 | STR | 31'-6" | 814 | | | | | | |
| B2 | 8 | #9 | STR | 31'-6" | 857 | B2 | 8 | #9 | STR | 31'-6" | 857 | B2 | 8 | #9 | STR | 31'-6" | 857 | B2 | 8 | #9 | STR | 31'-6" | 857 | B2 | 8 | #9 | STR | 31'-6" | 857 | | | | | | |
| B3 | 6 | #10 | ① | 34'-4" | 886 | B3 | 6 | #10 | ① | 34'-4" | 886 | B3 | 6 | #10 | ① | 34'-4" | 886 | B3 | 6 | #10 | ① | 34'-4" | 886 | B3 | 6 | #10 | ① | 34'-4" | 886 | | | | | | |
| B4 | 4 | #5 | STR | 31'-6" | 131 | B4 | 4 | #5 | STR | 31'-6" | 131 | B4 | 4 | #5 | STR | 31'-6" | 131 | B4 | 4 | #5 | STR | 31'-6" | 181 | B4 | 4 | #5 | STR | 31'-6" | 131 | | | | | | |
| M1 | 16 | #11 | STR | 39'-0" | 3315 | M1 | 16 | #11 | STR | 39'-0" | 3315 | M1 | 16 | #11 | STR | 43'-0" | 3655 | M1 | 16 | #11 | STR | 40'-6" | 3443 | M1 | 16 | #11 | STR | 38'-6" | 3273 | | | | | | |
| M2 | 16 | #11 | STR | 42'-0" | 3570 | M2 | 16 | #11 | STR | 40'-6" | 3443 | M2 | 16 | #11 | STR | 46'-0" | 3910 | M2 | 16 | #11 | STR | 34'-6" | 2933 | M2 | 16 | #11 | STR | 35'-6" | 3018 | | | | | | |
| M3 | 8 | #11 | STR | 28'-0" | 1190 | M3 | 8 | #11 | STR | 27'-6" | 1169 | M3 | 8 | #11 | STR | 32'-0" | 1360 | M3 | 8 | #11 | STR | 29'-6" | 1254 | M3 | 8 | #11 | STR | 27'-6" | 1169 | | | | | | |
| M4 | 8 | #11 | STR | 31'-0" | 1318 | M4 | 8 | #11 | STR | 29'-6" | 1254 | M4 | 8 | #11 | STR | 35'-0" | 1488 | M4 | 8 | #11 | STR | 23'-6" | 999 | M4 | 8 | #11 | STR | 24'-6" | 1041 | | | | | | |
| S1 | 114 | #5 | ② | 10'-4" | 1229 | S1 | 114 | #5 | ② | 10'-4" | 1229 | S1 | 114 | #5 | ② | 10'-4" | 1229 | S1 | 114 | #5 | ② | 10'-4" | 1229 | S1 | 114 | #5 | ② | 10'-4" | 1229 | | | | | | |
| U1 | 8 | #4 | ③ | 7'-0" | 37 | U1 | 8 | #4 | ③ | 7'-0" | 37 | U1 | 8 | #4 | ③ | 7'-0" | 37 | U1 | 8 | #4 | ③ | 7'-0" | 37 | U1 | 8 | #4 | ③ | 7'-0" | 37 | | | | | | |
| U2 | 6 | #4 | ③ | 6'-3" | 25 | U2 | 6 | #4 | ③ | 6'-3" | 25 | U2 | 6 | #4 | ③ | 6'-3" | 25 | U2 | 6 | #4 | ③ | 6'-3" | 25 | U2 | 6 | #4 | ③ | 6'-3" | 25 | | | | | | |
| U3 | 40 | #4 | ③ | 6'-4" | 169 | U3 | 40 | #4 | ③ | 6'-4" | 169 | U3 | 40 | #4 | ③ | 6'-4" | 169 | U3 | 40 | #4 | ③ | 6'-4" | 169 | U3 | 40 | #4 | ③ | 6'-4" | 169 | | | | | | |
| V1 | 32 | #11 | ④ | 17'-3" | 2933 | V1 | 32 | #11 | ④ | 17'-9" | 3018 | V1 | 32 | #11 | ④ | 16'-3" | 2763 | V1 | 32 | #11 | ④ | 15'-6" | 2635 | V1 | 32 | #11 | ④ | 13'-6" | 2295 | | | | | | |
| REINFORCING STEEL LBS. 16474 | | | | | | REINFORCING STEEL LBS. 16347 | | | | | | REINFORCING STEEL LBS. 17324 | | | | | | REINFORCING STEEL LBS. 15462 | | | | | | REINFORCING STEEL LBS. 14944 | | | | | | | | | | | |
| SP-1 | 1 | ** | ⑤ | 553'-6" | 577 | SP-1 | 1 | ** | ⑤ | 545'-0" | 568 | SP-1 | 1 | ** | ⑤ | 636'-0" | 663 | SP-1 | 1 | ** | ⑤ | 586'-6" | 612 | SP-1 | 1 | ** | ⑤ | 545'-0" | 568 | | | | | | |
| SP-2 | 1 | ** | ⑤ | 611'-0" | 637 | SP-2 | 1 | ** | ⑤ | 586'-6" | 612 | SP-2 | 1 | ** | ⑤ | 694'-0" | 724 | SP-2 | 1 | ** | ⑤ | 462'-6" | 482 | SP-2 | 1 | ** | ⑤ | 487'-6" | 508 | | | | | | |
| SP-3 | 2 | *** | ⑤ | 487'-0" | 651 | SP-3 | 2 | *** | ⑤ | 495'-0" | 661 | SP-3 | 2 | *** | ⑤ | 446'-0" | 596 | SP-3 | 2 | *** | ⑤ | 421'-0" | 562 | SP-3 | 2 | *** | ⑤ | 363'-0" | 485 | | | | | | |
| SPIRAL REINFORCING STEEL LBS. 1865 | | | | | | SPIRAL REINFORCING STEEL LBS. 1841 | | | | | | SPIRAL REINFORCING STEEL LBS. 1983 | | | | | | SPIRAL REINFORCING STEEL LBS. 1656 | | | | | | SPIRAL REINFORCING STEEL LBS. 1561 | | | | | | | | | | | |
| *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR #5 PLAIN OR DEFORMED BAR. | | | | | | | | | | | |
| *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | | *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | | | | | | | |
| BENT 1 TOTAL QUANTITIES | | | | | | BENT 2 TOTAL QUANTITIES | | | | | | BENT 3 TOTAL QUANTITIES | | | | | | BENT 4 TOTAL QUANTITIES | | | | | | BENT 5 TOTAL QUANTITIES | | | | | | | | | | | |
| CLASS A CONCRETE | | | | | | CLASS A CONCRETE | | | | | | CLASS A CONCRETE | | | | | | CLASS A CONCRETE | | | | | | CLASS A CONCRETE | | | | | | | | | | | |
| POUR 2 (COLUMNS) C.Y. 7.1 | | | | | | POUR 2 (COLUMNS) C.Y. 7.3 | | | | | | POUR 2 (COLUMNS) C.Y. 6.5 | | | | | | POUR 2 (COLUMNS) C.Y. 6.1 | | | | | | POUR 2 (COLUMNS) C.Y. 5.1 | | | | | | | | | | | |
| POUR 3 (CAP) C.Y. 18.2 | | | | | | POUR 3 (CAP) C.Y. 18.2 | | | | | | POUR 3 (CAP) C.Y. 18.2 | | | | | | POUR 3 (CAP) C.Y. 18.2 | | | | | | POUR 3 (CAP) C.Y. 18.2 | | | | | | | | | | | |
| TOTAL CLASS A CONCRETE C.Y. 25.3 | | | | | | TOTAL CLASS A CONCRETE C.Y. 25.5 | | | | | | TOTAL CLASS A CONCRETE C.Y. 24.7 | | | | | | TOTAL CLASS A CONCRETE C.Y. 24.3 | | | | | | TOTAL CLASS A CONCRETE C.Y. 23.3 | | | | | | | | | | | |
| DRILLED PIERS, CONCRETE | | | | | | DRILLED PIERS, CONCRETE | | | | | | DRILLED PIERS, CONCRETE | | | | | | DRILLED PIERS, CONCRETE | | | | | | DRILLED PIERS, CONCRETE | | | | | | | | | | | |
| POUR 1 C.Y. 21.4 | | | | | | POUR 1 C.Y. 20.7 | | | | | | POUR 1 C.Y. 24.2 | | | | | | POUR 1 C.Y. 19.2 | | | | | | POUR 1 C.Y. 18.9 | | | | | | | | | | | |
| 3'-6" Ø DRILLED PIERS IN SOIL LIN. FT. 46.0 | | | | | | 3'-6" Ø DRILLED PIERS IN SOIL LIN. FT. 45.0 | | | | | | 3'-6" Ø DRILLED PIERS IN SOIL LIN. FT. 52.0 | | | | | | 3'-6" Ø DRILLED PIERS IN SOIL LIN. FT. 41.0 | | | | | | 3'-6" Ø DRILLED PIERS IN SOIL LIN. FT. 42.0 | | | | | | | | | | | |
| 3'-6" Ø DRILLED PIERS NOT IN SOIL LIN. FT. 14.0 | | | | | | 3'-6" Ø DRILLED PIERS NOT IN SOIL LIN. FT. 13.0 | | | | | | 3'-6" Ø DRILLED PIERS NOT IN SOIL LIN. FT. 16.0 | | | | | | 3'-6" Ø DRILLED PIERS NOT IN SOIL LIN. FT. 13.0 | | | | | | 3'-6" Ø DRILLED PIERS NOT IN SOIL LIN. FT. 11.0 | | | | | | | | | | | |
| CSL TUBES LIN. FT. 252.0 | | | | | | CSL TUBES LIN. FT. 244.0 | | | | | | CSL TUBES LIN. FT. 284.0 | | | | | | CSL TUBES LIN. FT. 228.0 | | | | | | CSL TUBES LIN. FT. 224.0 | | | | | | | | | | | |
| PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIERS LIN. FT. 0.0 | | | | | | PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIERS LIN. FT. 0.0 | | | | | | PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIERS LIN. FT. 26.6 | | | | | | PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIERS LIN. FT. 19.2 | | | | | | PERMANENT STEEL CASING FOR 3'-6" Ø DRILLED PIERS LIN. FT. 0.0 | | | | | | | | | | | |



PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 3 OF 3 BRIDGE NO. 656



| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|------------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUBSTRUCTURE BENTS 1 THRU 5 | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |
| SHEET NO. S-36 | | | | | TOTAL SHEETS 86 |

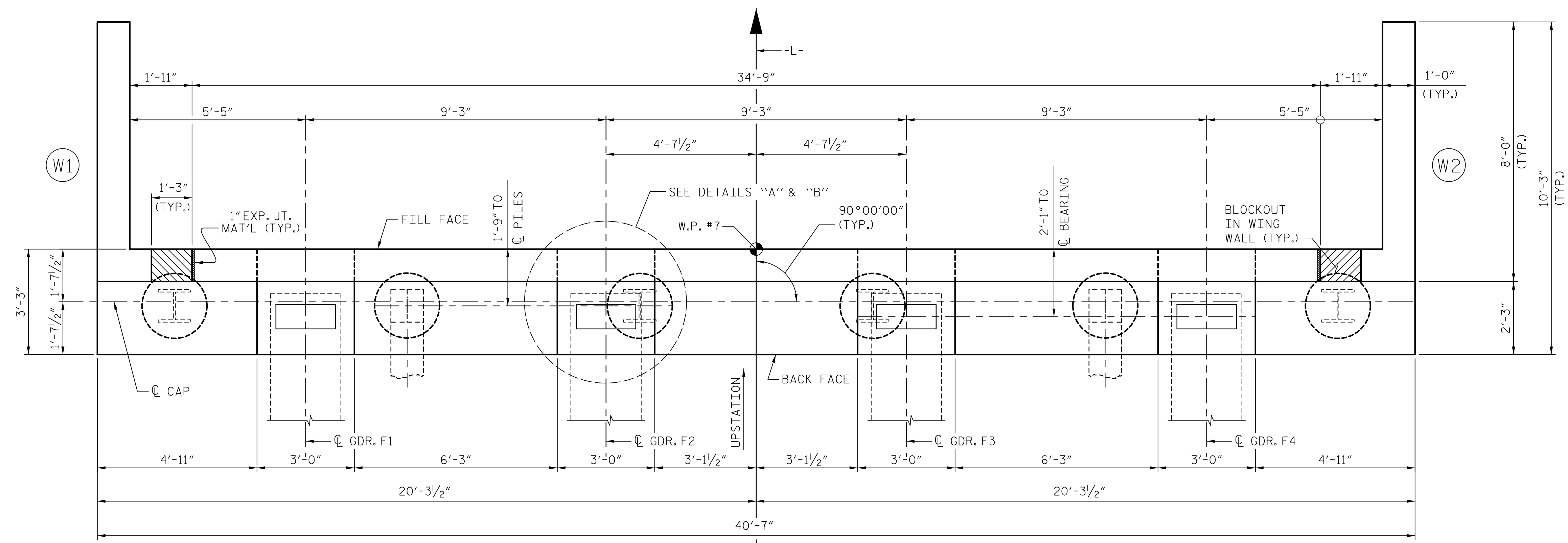
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016 TIME: 9:25:59 AM
 USER: jcm\jcm.dgn
 DGN: R:\Bridges\656\Card\401_L12707_S36_B703.dgn

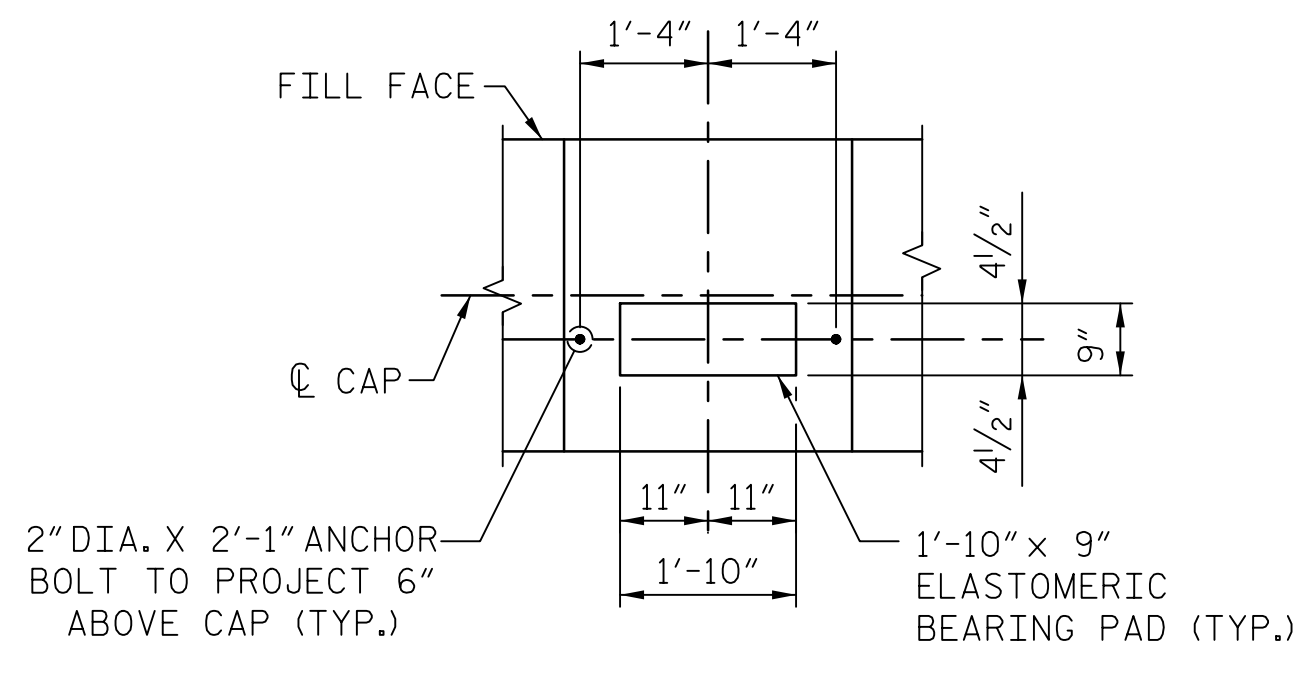
DRAWN BY: KHC DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

NOTES:

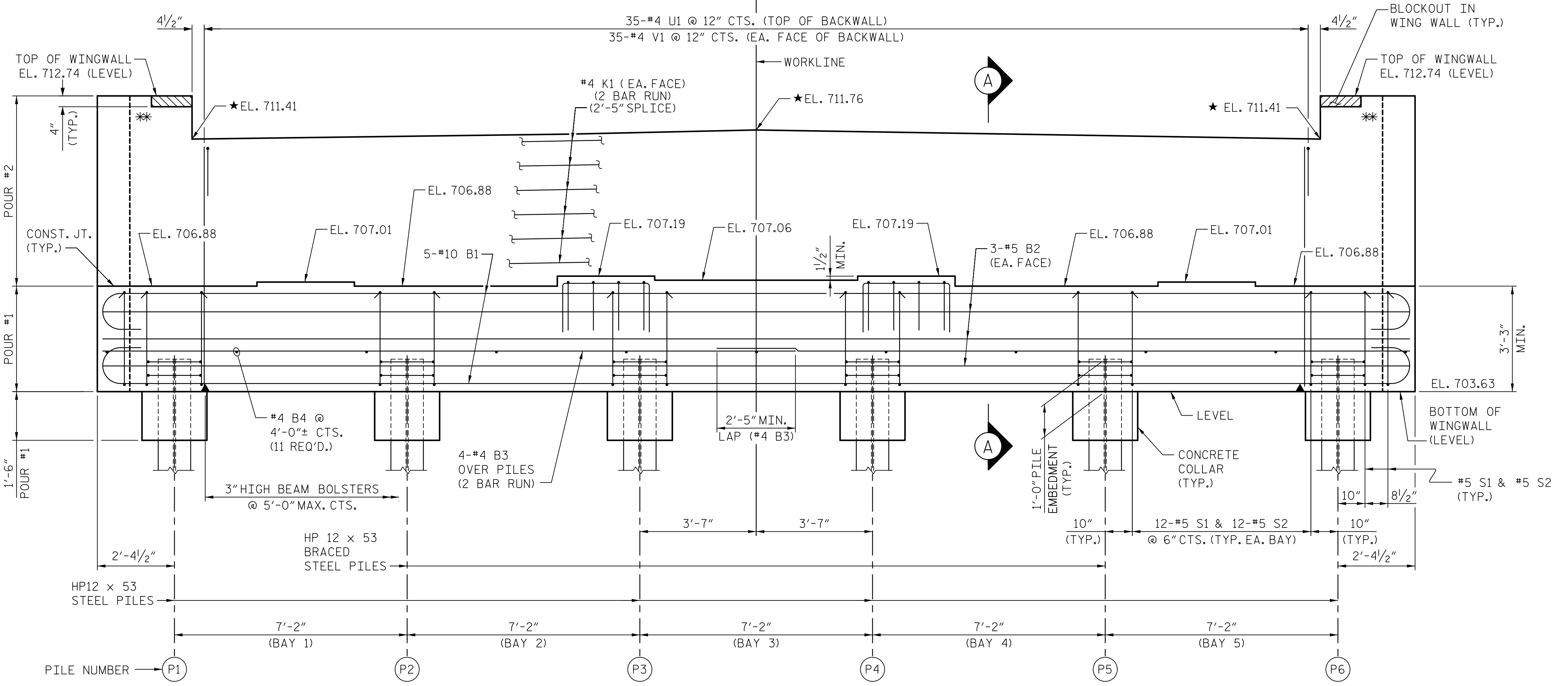
- FOR PILE SPLICE DETAILS, SEE END BENT 2, SHEET 3 OF 3.
- FOR SECTION A-A, SEE END BENT 2, SHEET 3 OF 3.
- FOR DETAILS OF LOWER WINGWALLS AND REINFORCING STEEL, SEE END BENT 2, SHEET 2 OF 3.
- FOR TEMPORARY DRAINAGE DETAILS, SEE END BENT 2, SHEET 3 OF 3.
- ★ THIS ELEVATION TAKEN ON FILL FACE OF THE BACKWALL.
- ✱ REINFORCING IN WING NOT SHOWN FOR CLARITY, FOR DETAIL, SEE END BENT 2, SHEET 2.



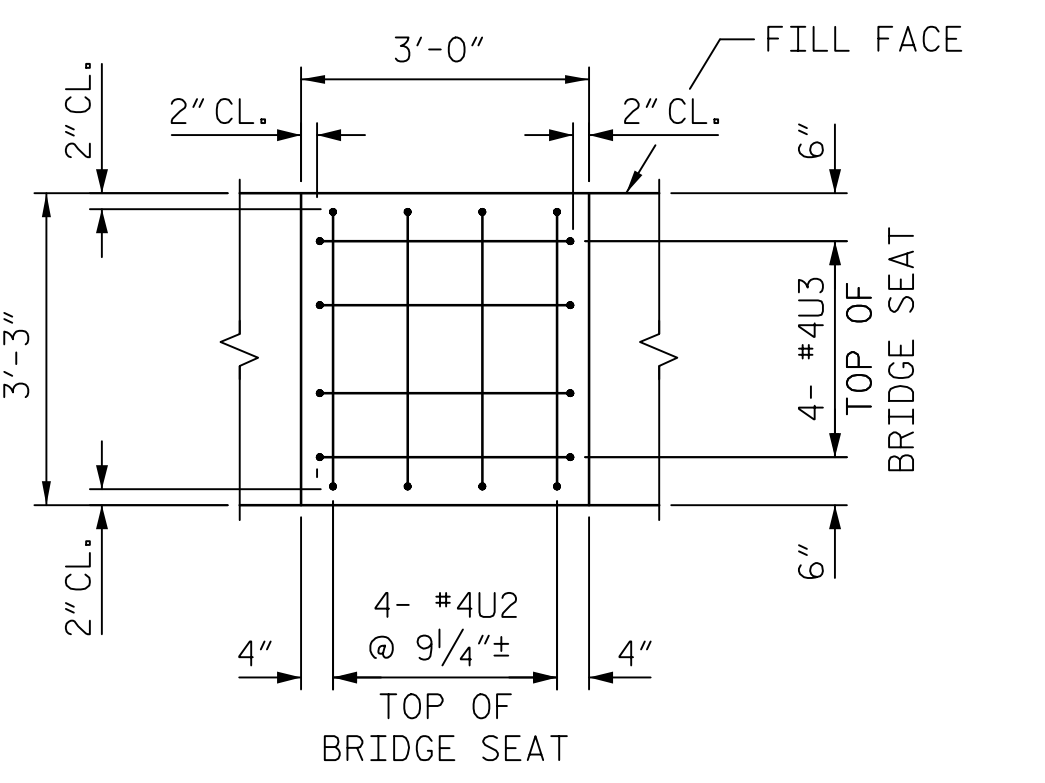
PLAN



DETAIL "A"



ELEVATION



DETAIL "B"

(TYP. SEATS 2 AND 3)

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 1 OF 3 BRIDGE NO. 656

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 NORTH CAROLINA PROFESSIONAL ENGINEER
 JOHN C. MORRISON
 11/12/2016

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

**SUBSTRUCTURE
 END BENT 2
 PLAN AND ELEVATION**

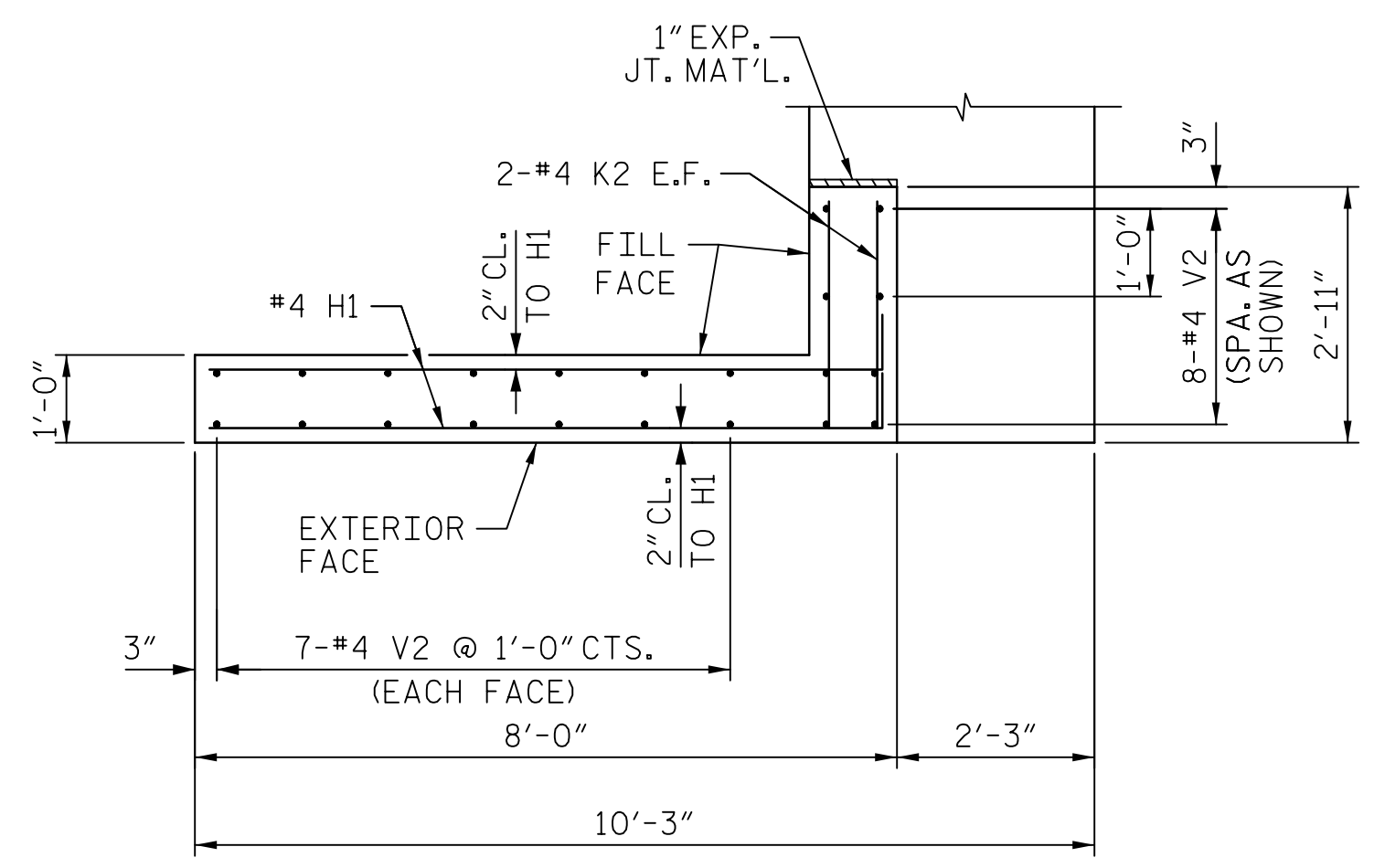
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-37**
 TOTAL SHEETS **86**

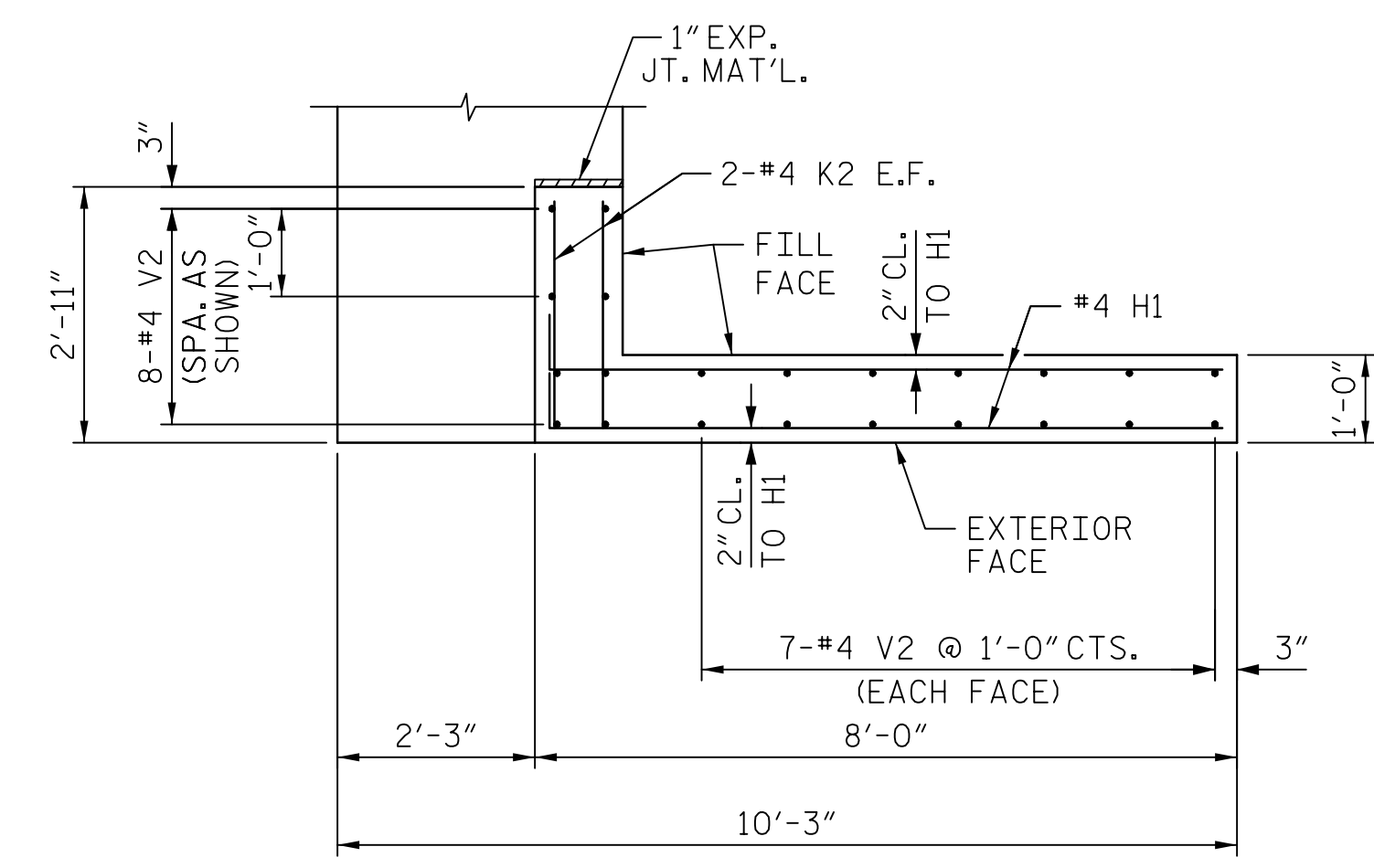
DRAWN BY : MKT DATE : 11/2015
 CHECKED BY : JCM DATE : 11/2015

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

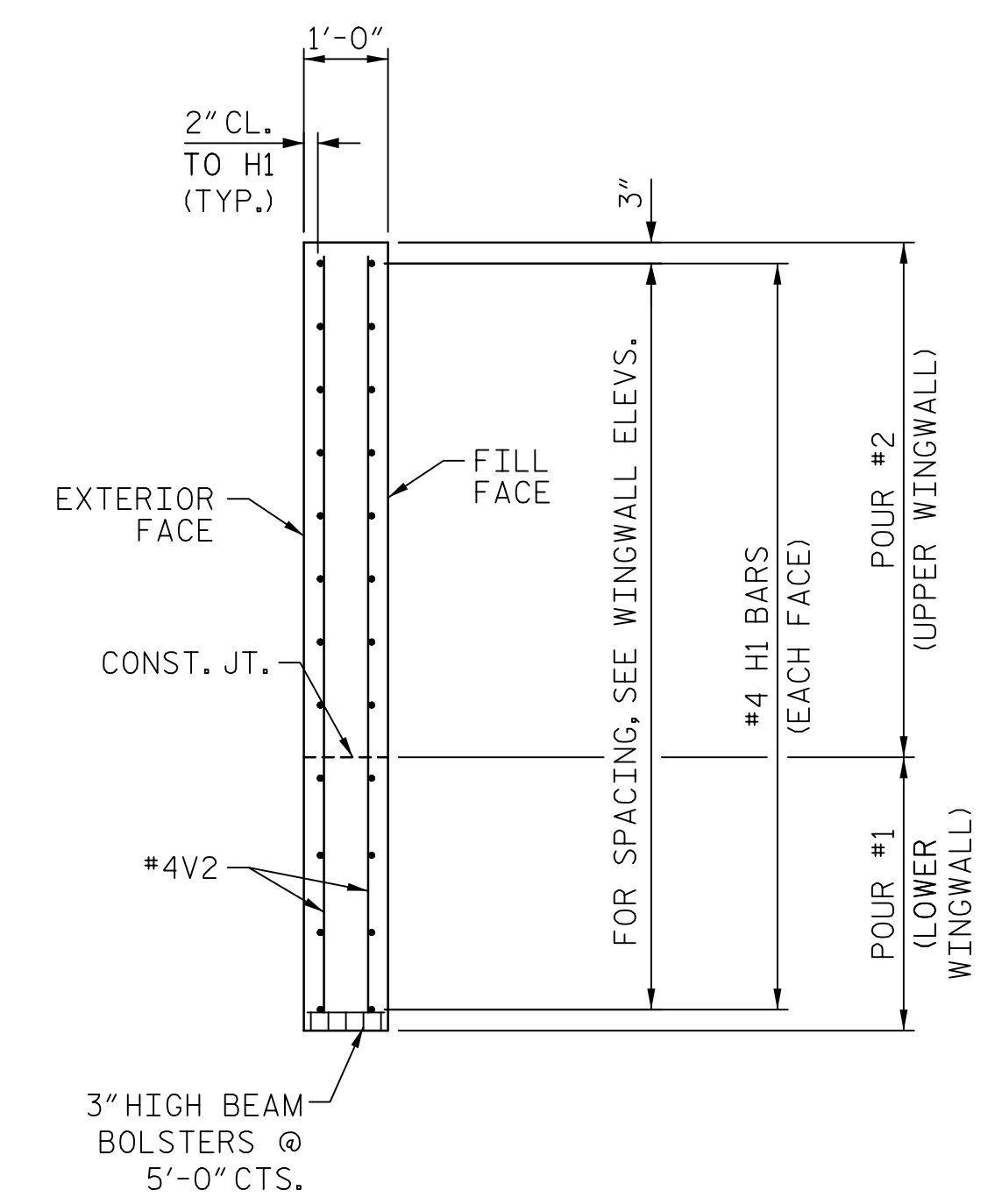
DATE: 11/12/2016 TIME: 9:25:04 AM
 USER: jcm\jcm.dgn
 DGN: R:\Projects\656\656.dgn\40_L073_U2707_SML_S37_EB2.dgn



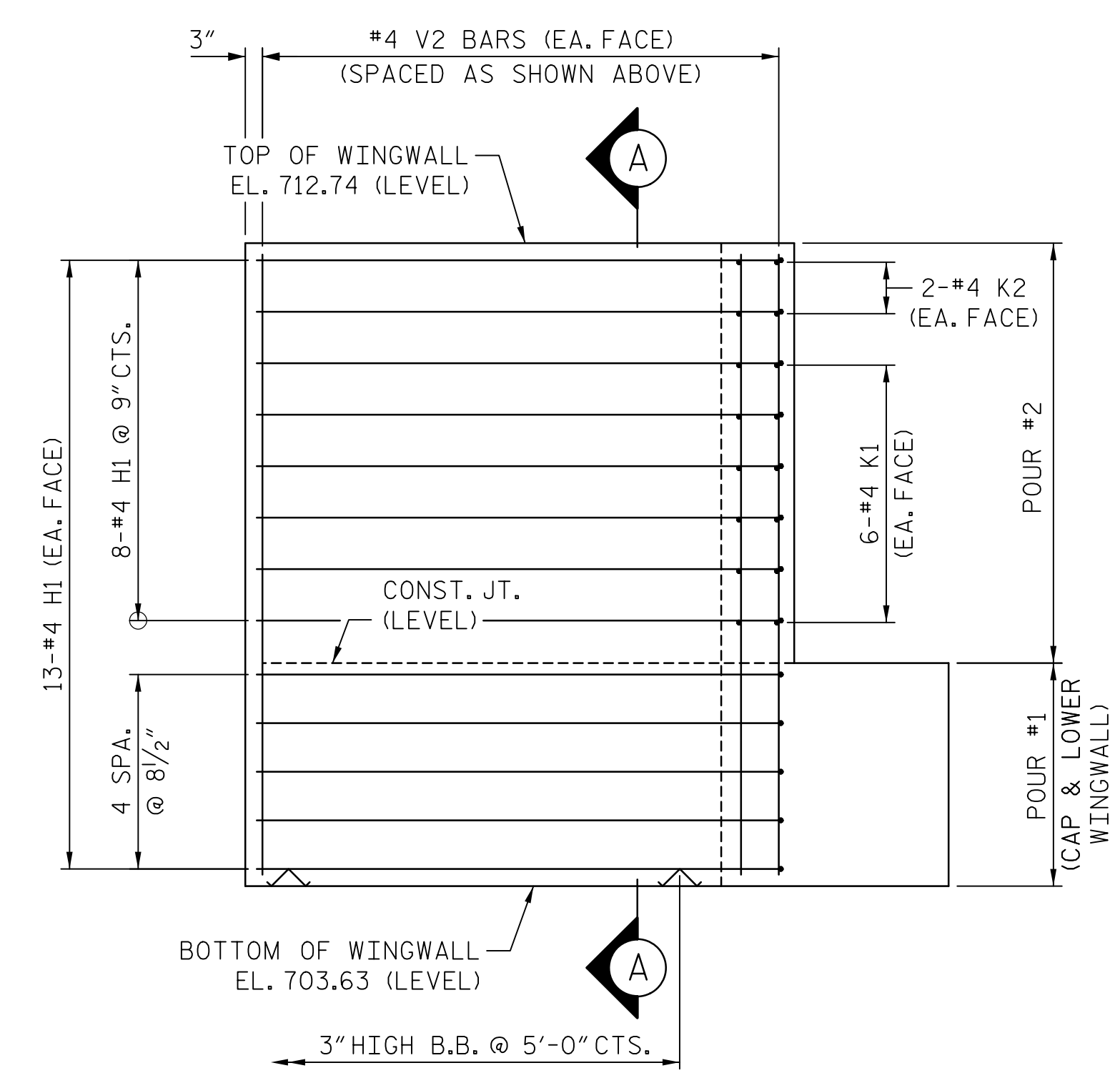
PLAN - WINGWALL 1



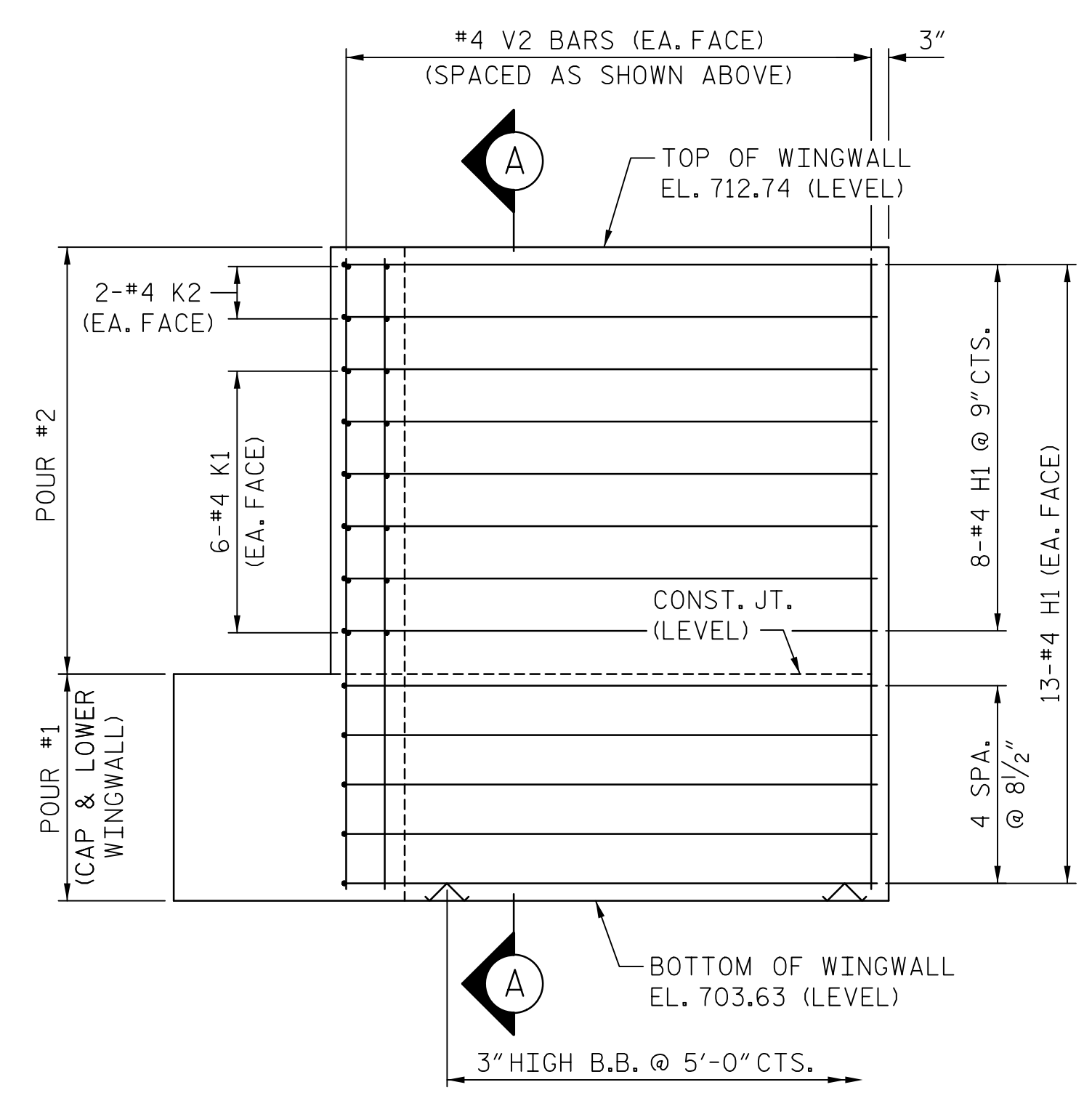
PLAN - WINGWALL 2



SECTION "A-A"



ELEVATION - WINGWALL 1



ELEVATION - WINGWALL 2

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 90+25.00 -L-
 SHEET 2 OF 3 BRIDGE NO. 656

AECOM
AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 NORTH CAROLINA PROFESSIONAL ENGINEER
 JOHN C. MORRISON
 1/12/2016

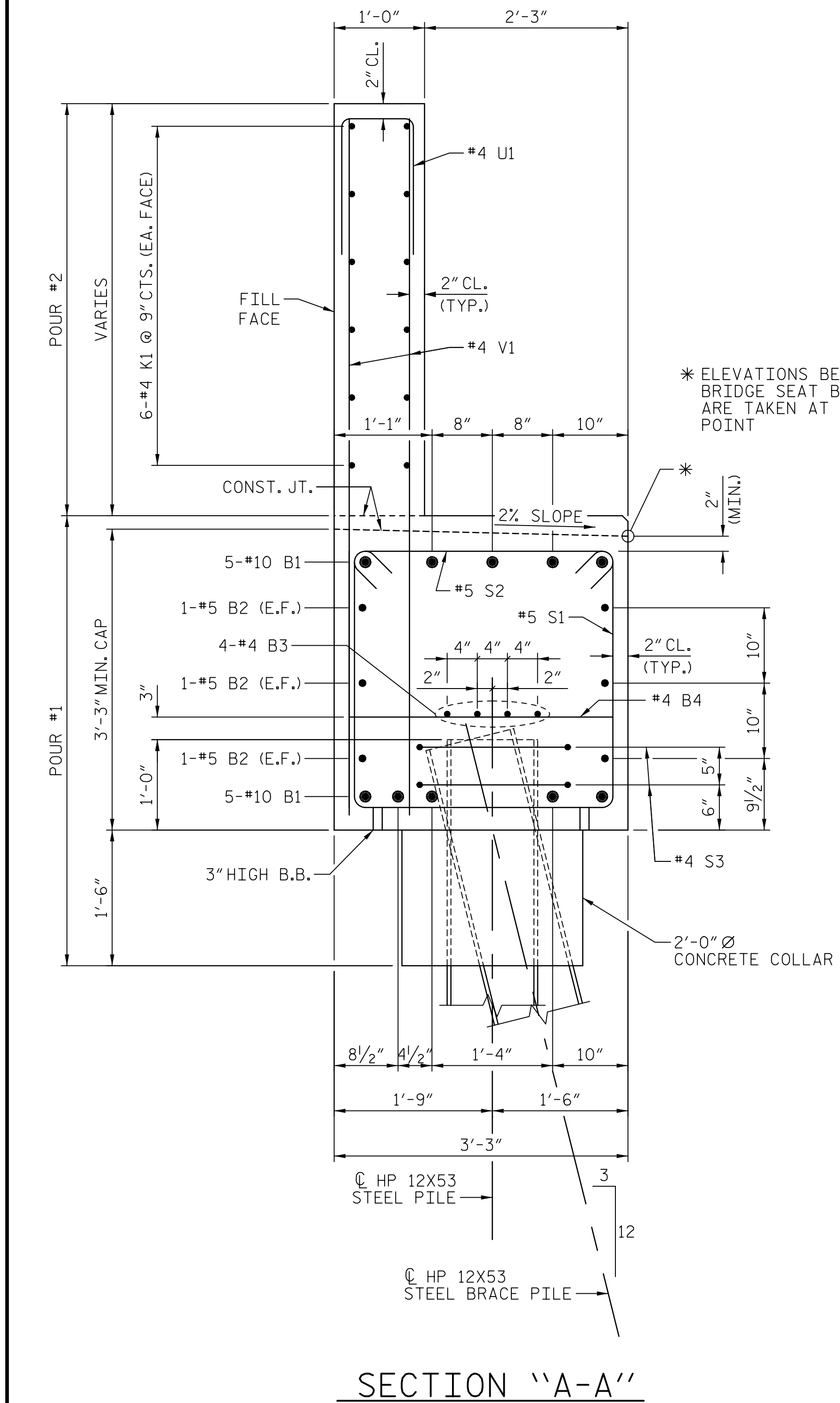
| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|---------------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUBSTRUCTURE END BENT 2 DETAILS | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |
| SHEET NO. S-38 | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

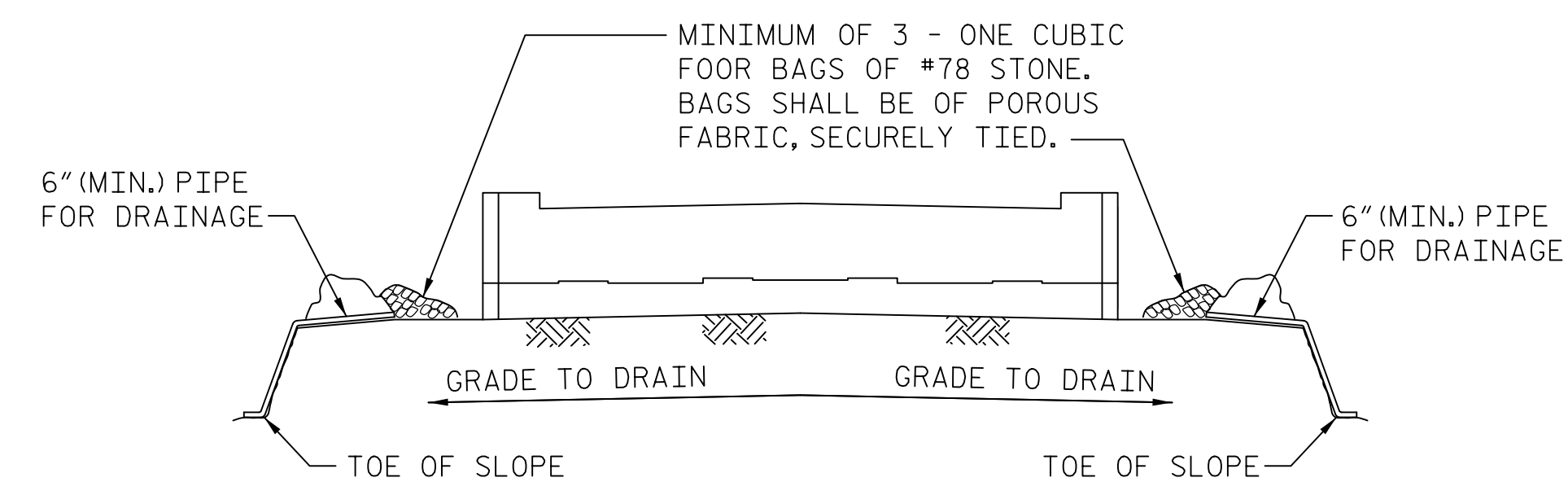
USER: R:\w\c\proj\656\Comp\401_075_U2707_SML_538_EB2_02.dgn
 DATE: 1/12/2016 9:25:09 AM
 DRAWN BY: MKT DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DATE: 11/12/2015
TIME: 9:25:05 AM

USER: \\ms01\proj\p\A\Bridges\656\Cont\401_DTT_U2707_SML_S39_EB2_03.dgn



SECTION "A-A"

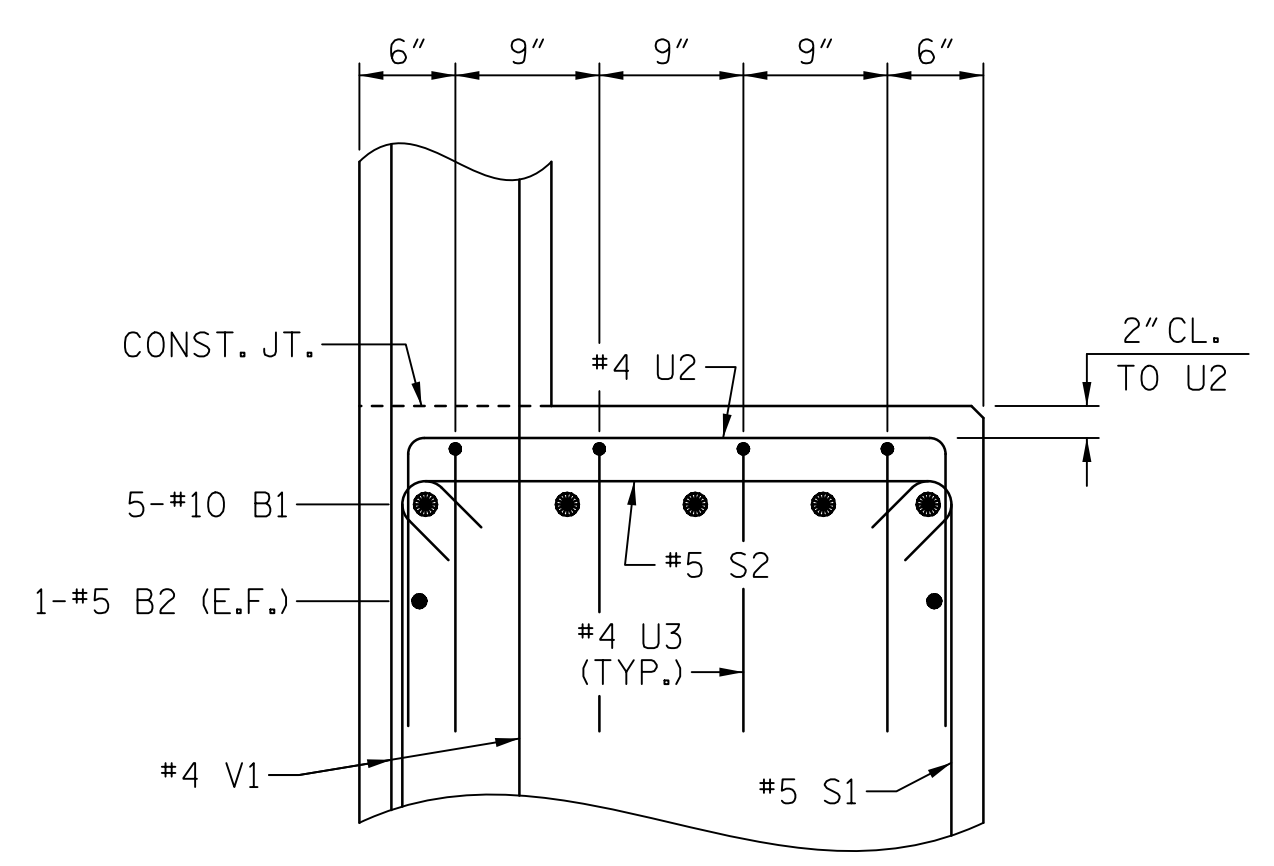


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

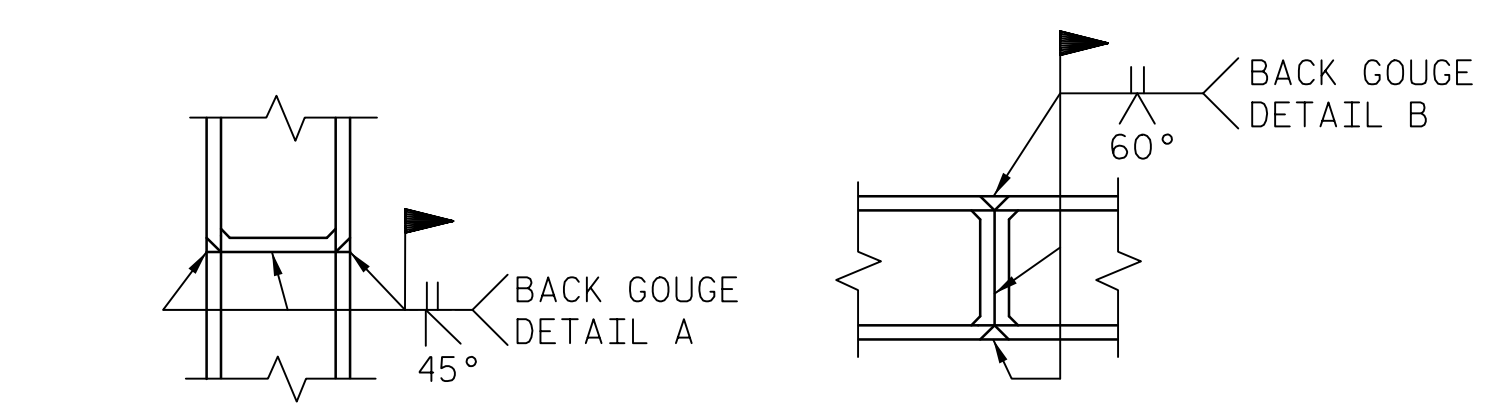
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS.

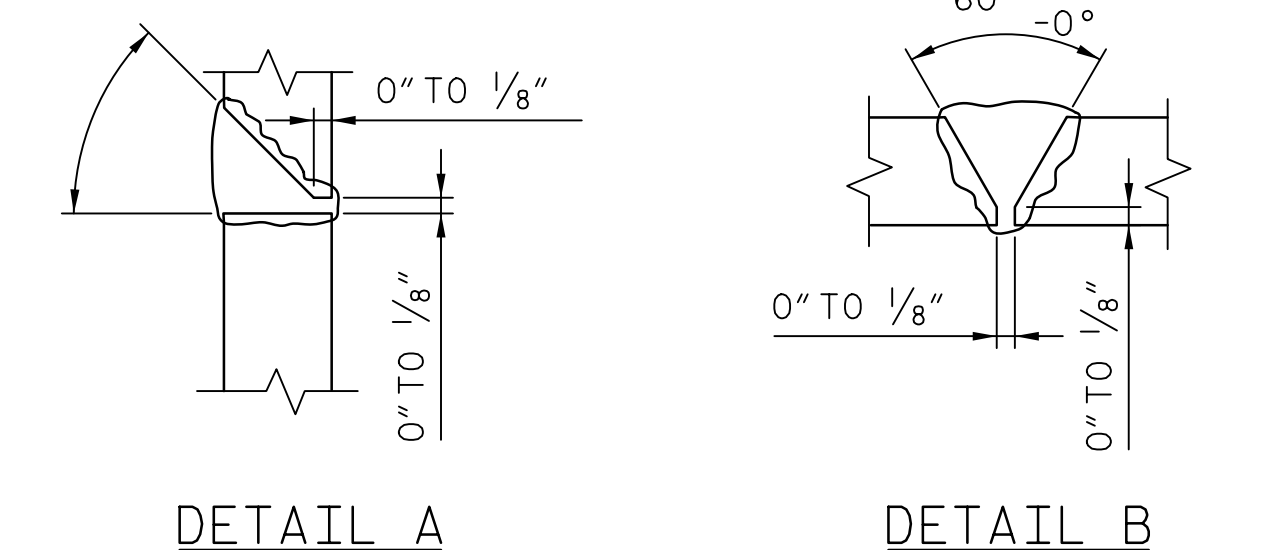
TEMPORARY DRAINAGE AT END BENT



BRIDGE SEAT SECTION



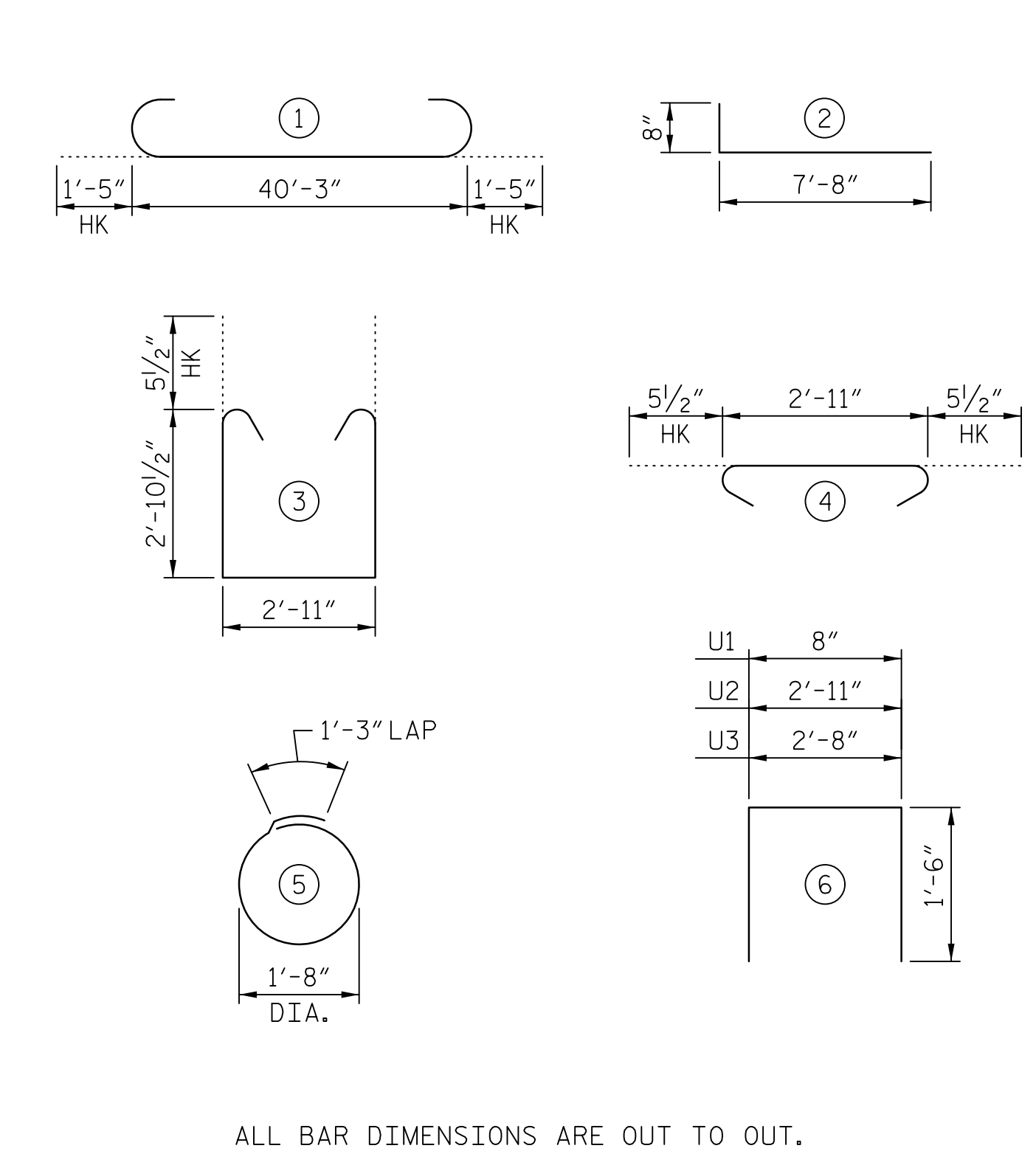
**PILE VERTICAL **PILE HORIZONTAL OR VERTICAL



**POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL

| END BENT 2 | | | | | |
|------------------------------------------|------|------|--------|--------|----------------|
| BAR NO. | SIZE | TYPE | LENGTH | WEIGHT | |
| B1 | 10 | #10 | ① | 43'-1" | 1854 |
| B2 | 6 | #5 | STR | 40'-3" | 252 |
| B3 | 8 | #4 | STR | 21'-4" | 114 |
| B4 | 11 | #4 | STR | 2'-11" | 21 |
| | | | | | |
| H1 | 52 | #4 | ② | 8'-4" | 289 |
| | | | | | |
| K1 | 24 | #4 | STR | 21'-4" | 342 |
| K2 | 8 | #4 | STR | 2'-7" | 14 |
| | | | | | |
| S1 | 64 | #5 | ③ | 9'-7" | 640 |
| S2 | 64 | #5 | ④ | 3'-10" | 256 |
| S3 | 12 | #4 | ⑤ | 6'-6" | 52 |
| | | | | | |
| U1 | 35 | #4 | ⑥ | 3'-8" | 86 |
| U2 | 8 | #4 | ⑥ | 5'-11" | 32 |
| U3 | 8 | #4 | ⑥ | 5'-8" | 30 |
| | | | | | |
| V1 | 70 | #4 | STR | 7'-5" | 347 |
| V2 | 44 | #4 | STR | 8'-9" | 257 |
| TOTAL REINFORCING STEEL | | | | | 4586 LBS. |
| CLASS A CONCRETE | | | | | |
| POUR #1 (CAP, COLLARS & LOWER WINGWALLS) | | | | | 19.1 C.Y. |
| POUR #2 (BACKWALL & UPPER WINGWALL) | | | | | 10.1 C.Y. |
| TOTAL = | | | | | 29.2 C.Y. |
| HP 12x53 STEEL PILES | | | | | |
| NO. = 6 | | | | | LIN. FT. = 330 |

NOTES:

FOR OTHER NOTES, SEE "FOUNDATION LAYOUT" SHEET AND "LOCATION SKETCH" SHEET.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE PARAPET AND END POST ARE CAST IF SLIP FORMING IS USED.

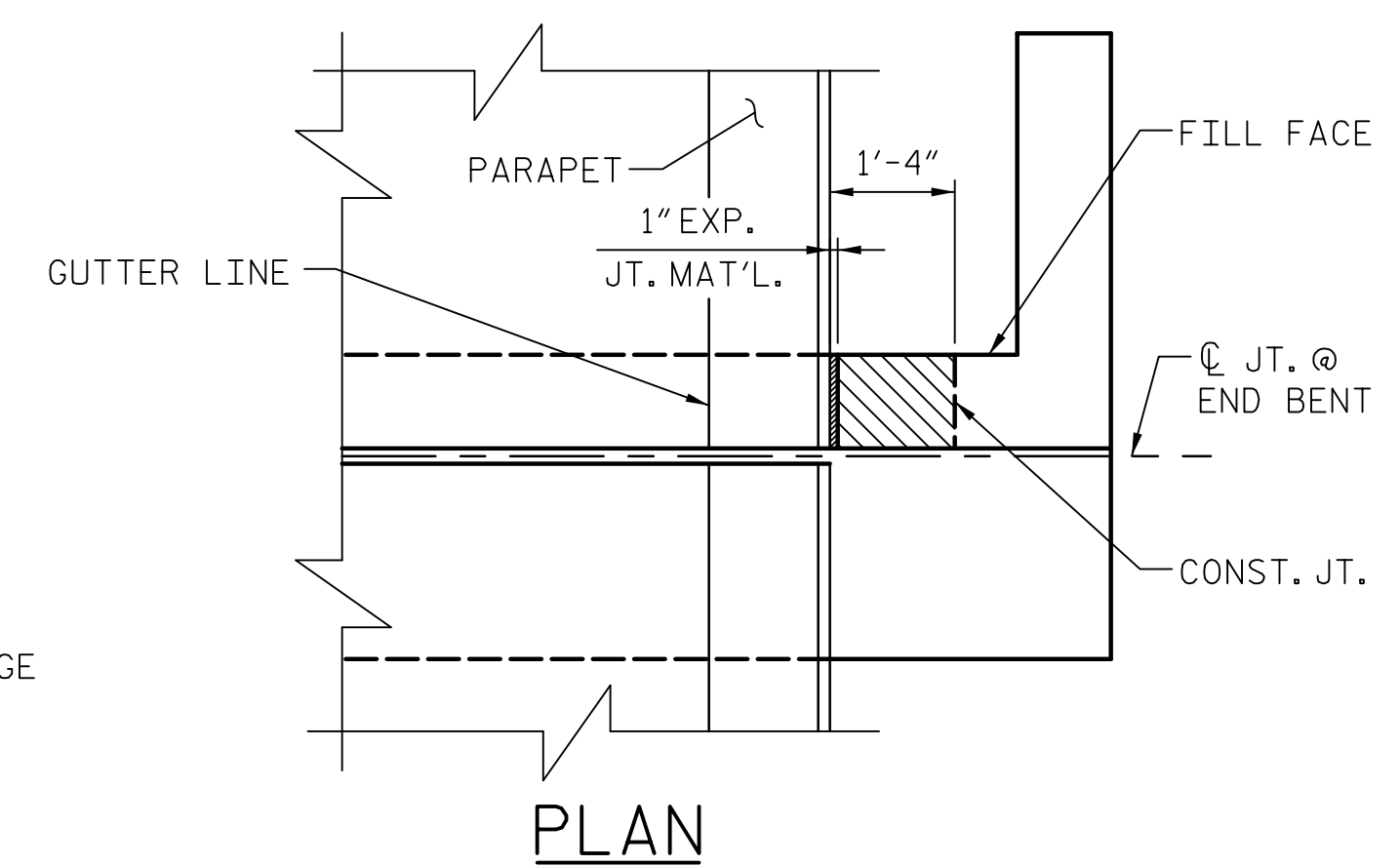
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.

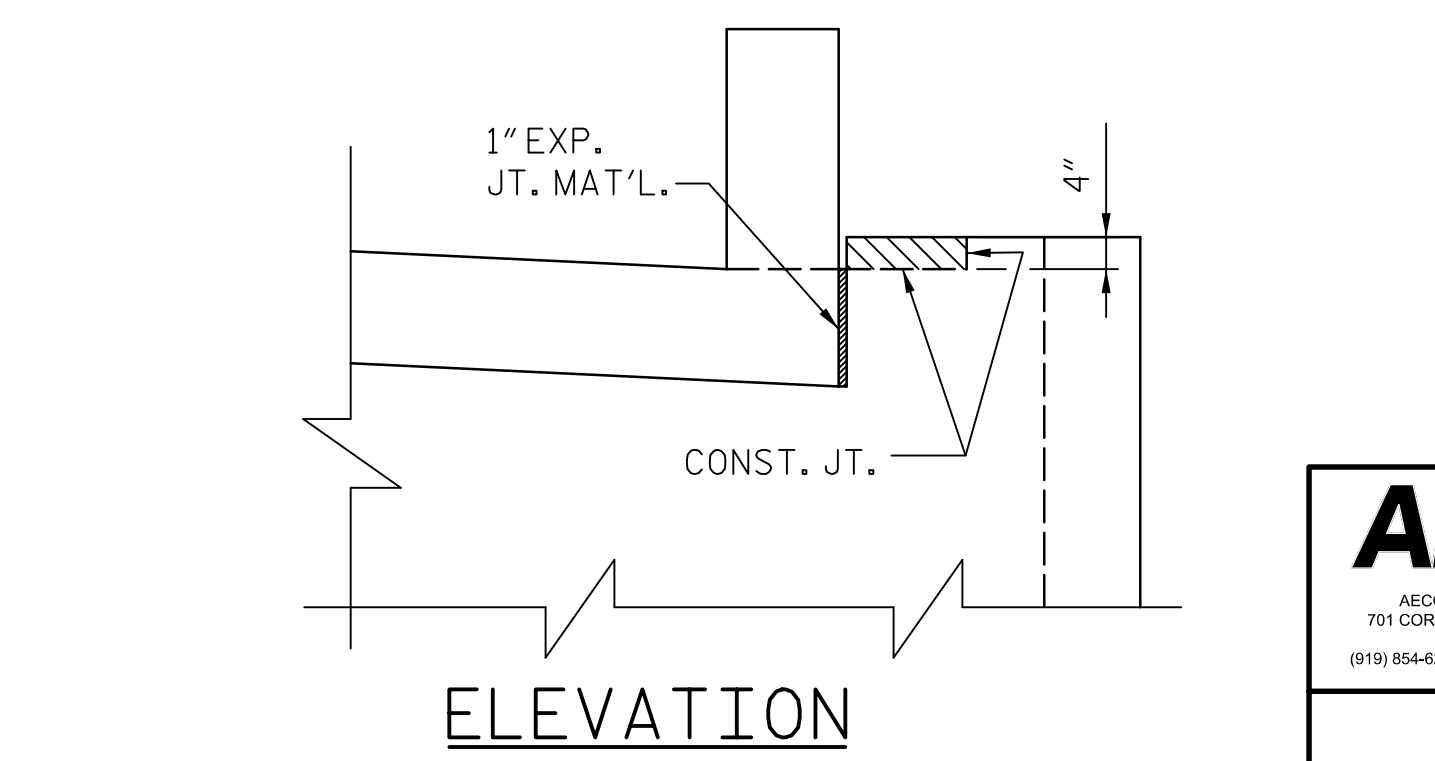
THE TOP SURFACE OF THE END BENT CAP, EXCEPT THE BRIDGE SEAT BUILDUPS, SHALL BE SLOPED TRANSVERSELY FROM THE FILL FACE TO THE BACK FACE AT THE RATE OF 2%.

THE TOP SURFACE AREAS OF THE END BENTS CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, EXCEPT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

INSTALL THE 4" Ø DRAIN PIPE THROUGH THE WINGWALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WINGWALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



PLAN



ELEVATION

BLOCKOUT IN WINGWALL DETAILS

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 90+25.00 -L-
SHEET 3 OF 3 BRIDGE NO. 656

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-6200 www.aecom.com
AECOM License No. F-0342

SEAL
NORTH CAROLINA PROFESSIONAL ENGINEER
John C. Morrison
1/12/2016

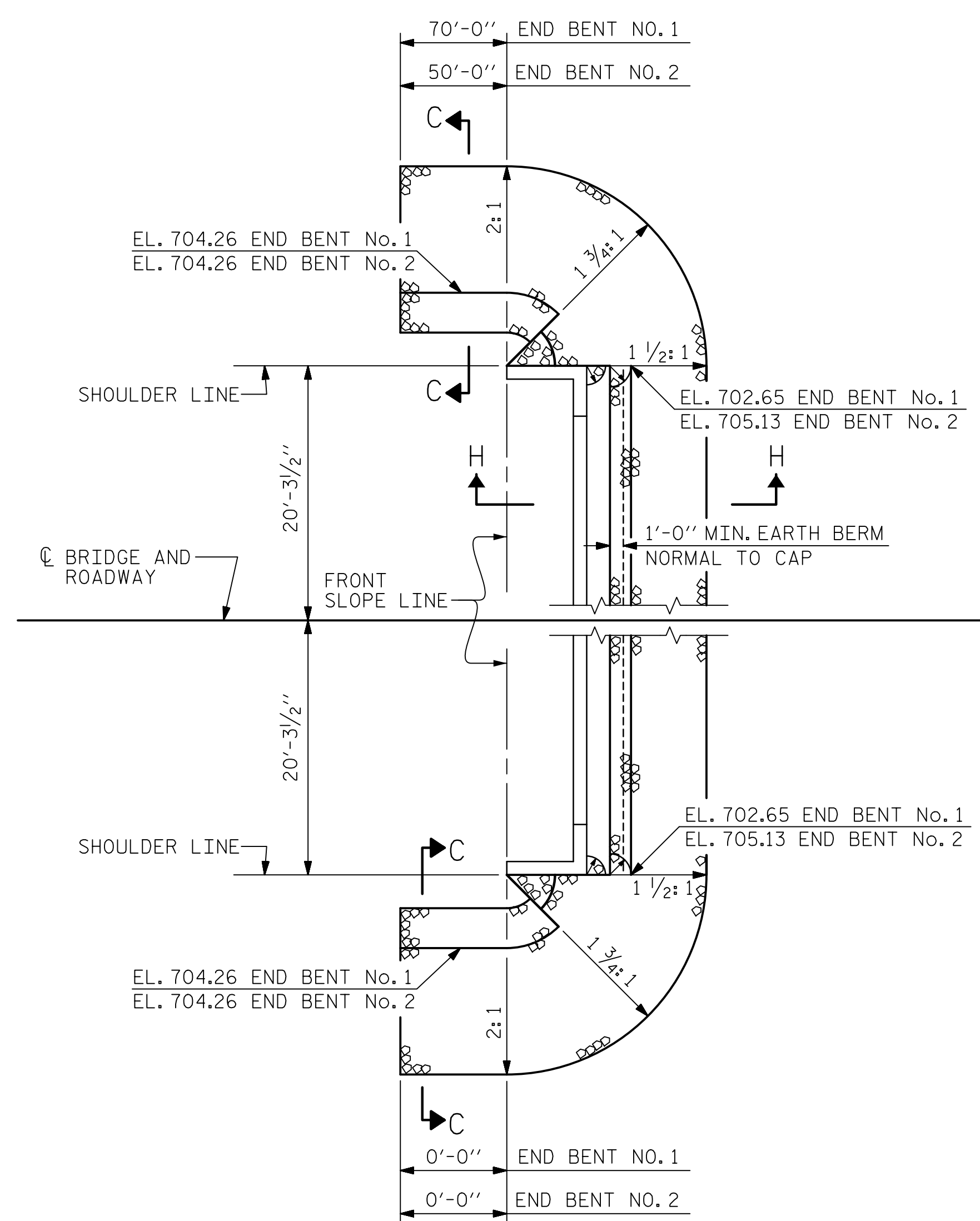
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-----------------|
| SUBSTRUCTURE END BENT 2 SECTIONS AND DETAILS | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |
| SHEET NO. S-39 | | | | | TOTAL SHEETS 86 |

DRAWN BY : MKT DATE : 11/2015
CHECKED BY : JCM DATE : 11/2015

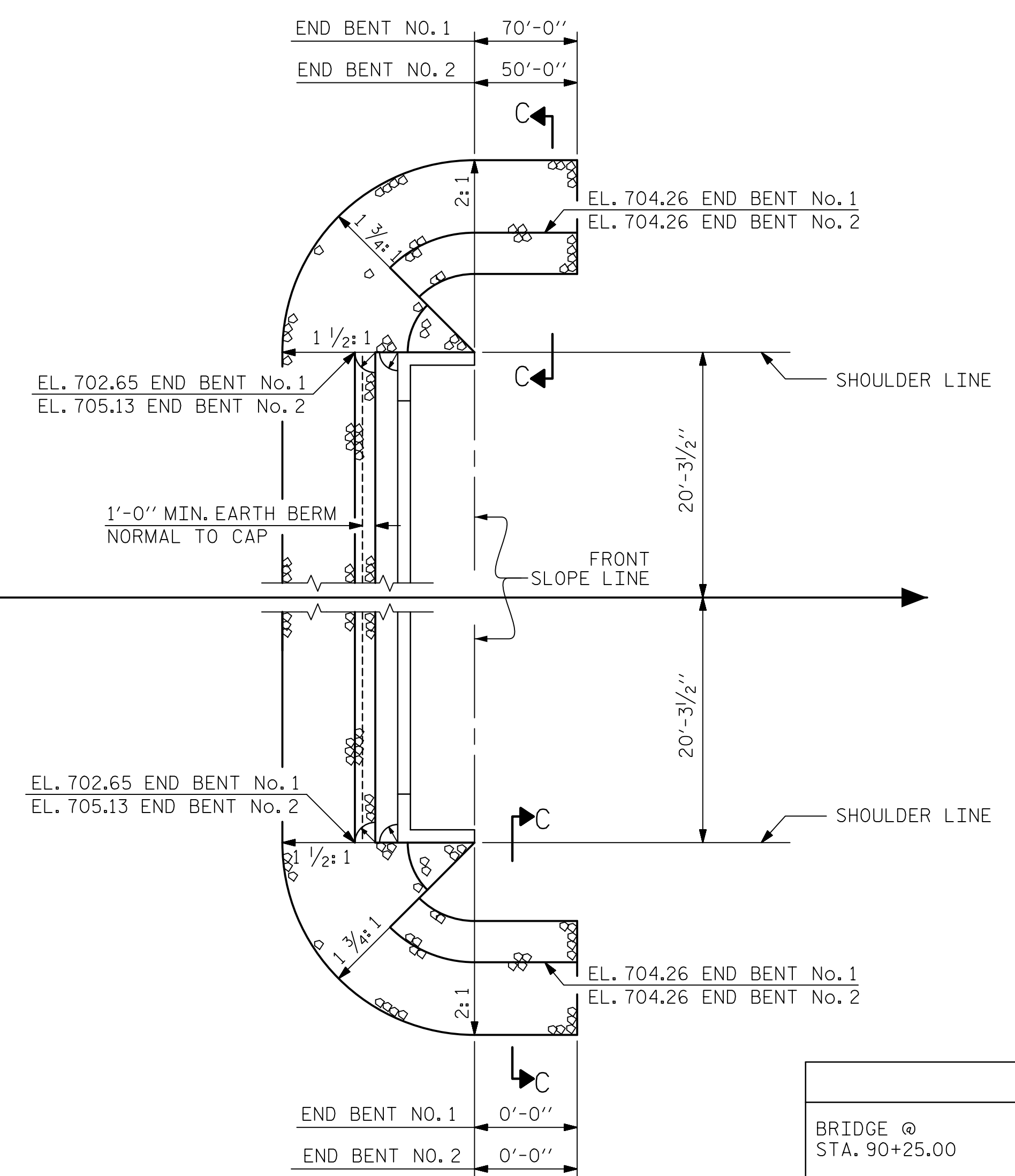
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016
TIME: 9:25:05 AM

USER: \\s01\project\p\B56\Card\401_079_U2707_SML_S40_RR.dgn
DGN: RR1RR1.dgn



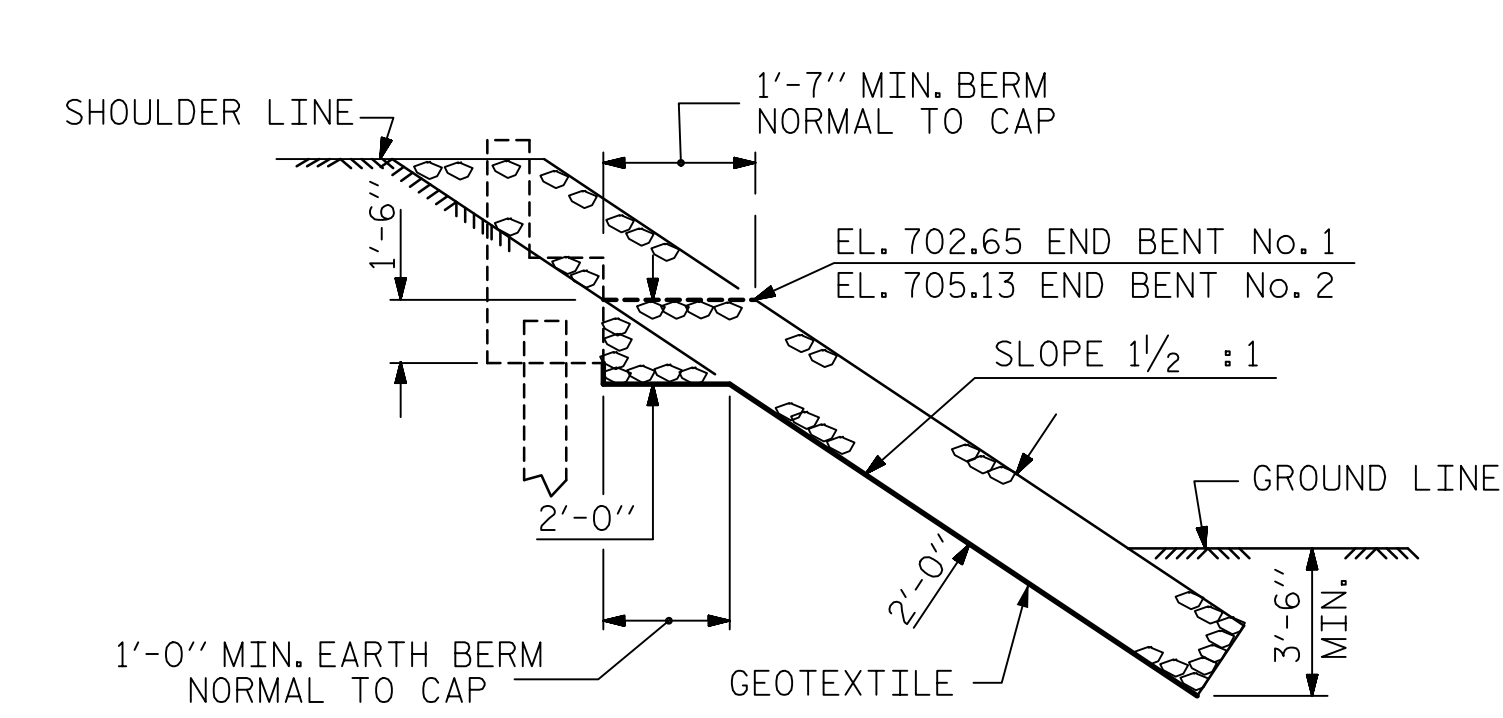
SHOULDER RIP RAP IS HIGHER THAN BERM RIP RAP



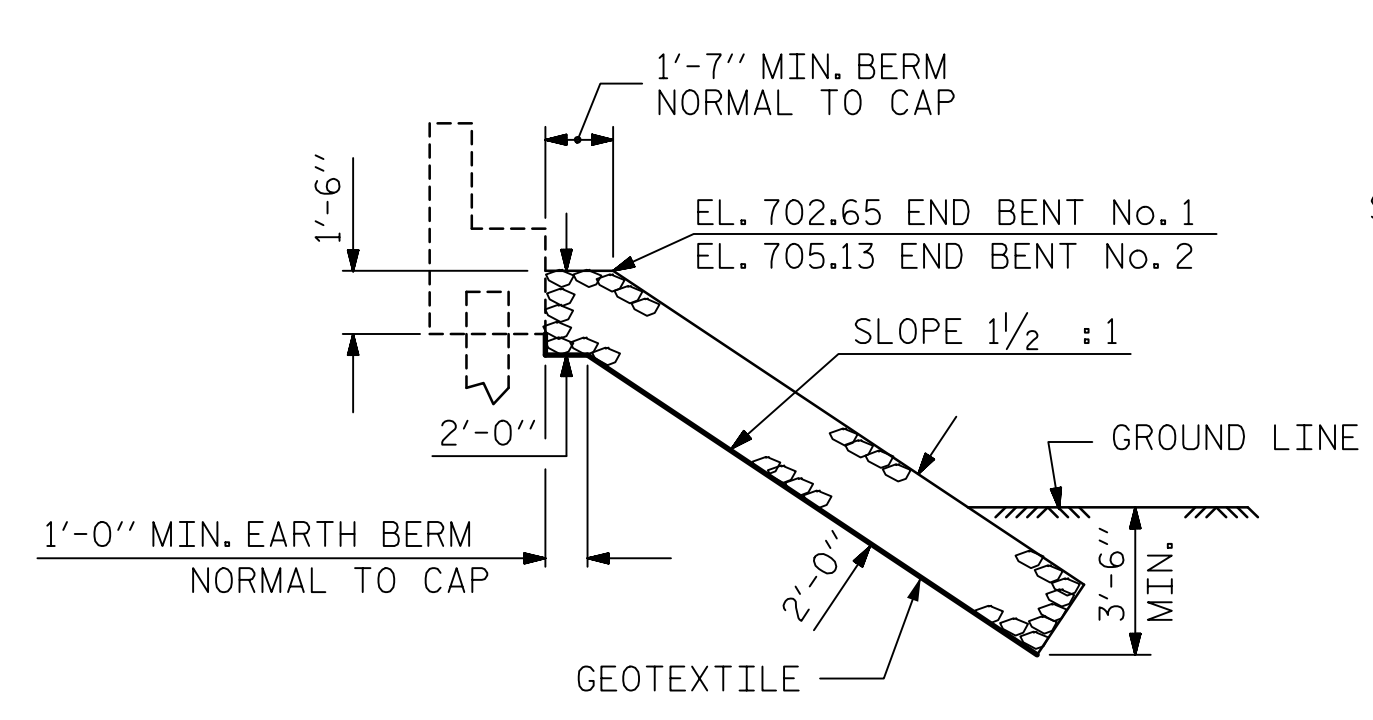
BERM RIP RAPPED

NOTES :
FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

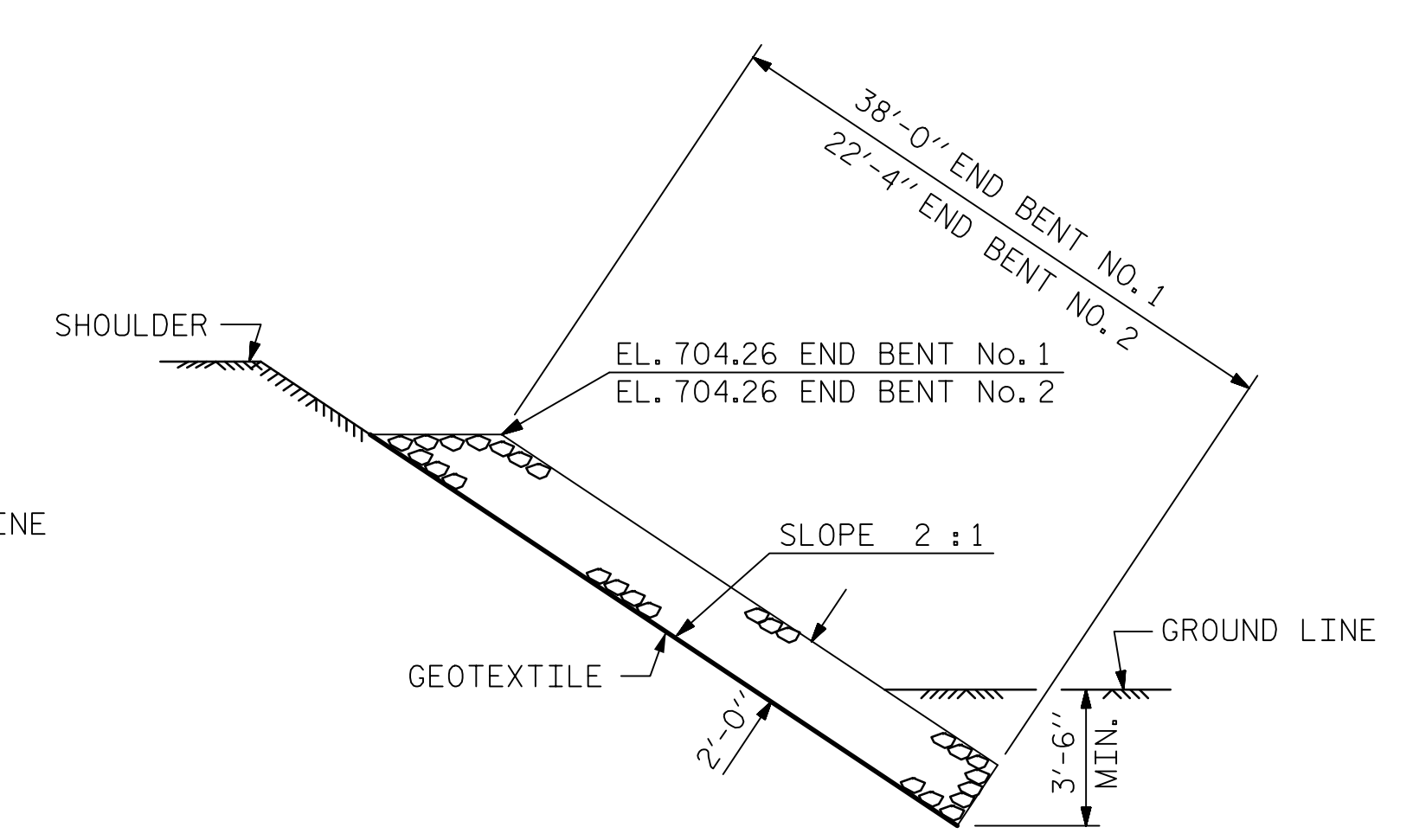
| ESTIMATED QUANTITIES | | |
|---------------------------|--------------------------------------|----------------------------|
| BRIDGE @ STA. 90+25.00 | RIP RAP CLASS II (2'-0" THICK) | GEOTEXTILE FOR DRAINAGE |
| | TONS | SQUARE YARDS |
| END BENT 1 | 952 | 1058 |
| END BENT 2 | 565 | 627 |



SECTION H-H



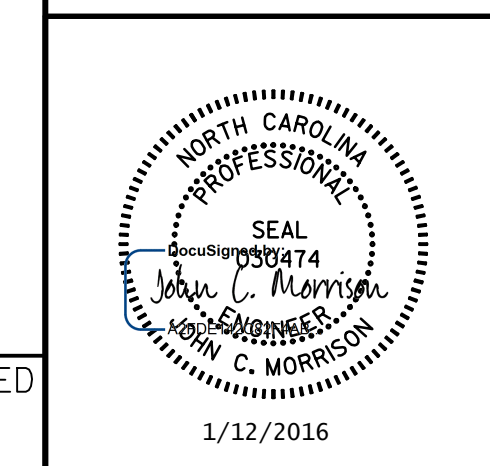
SECTION C-C
BERM RIP RAPPED



SECTION C-C

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 90+25.00 -L-

BRIDGE NO. 656

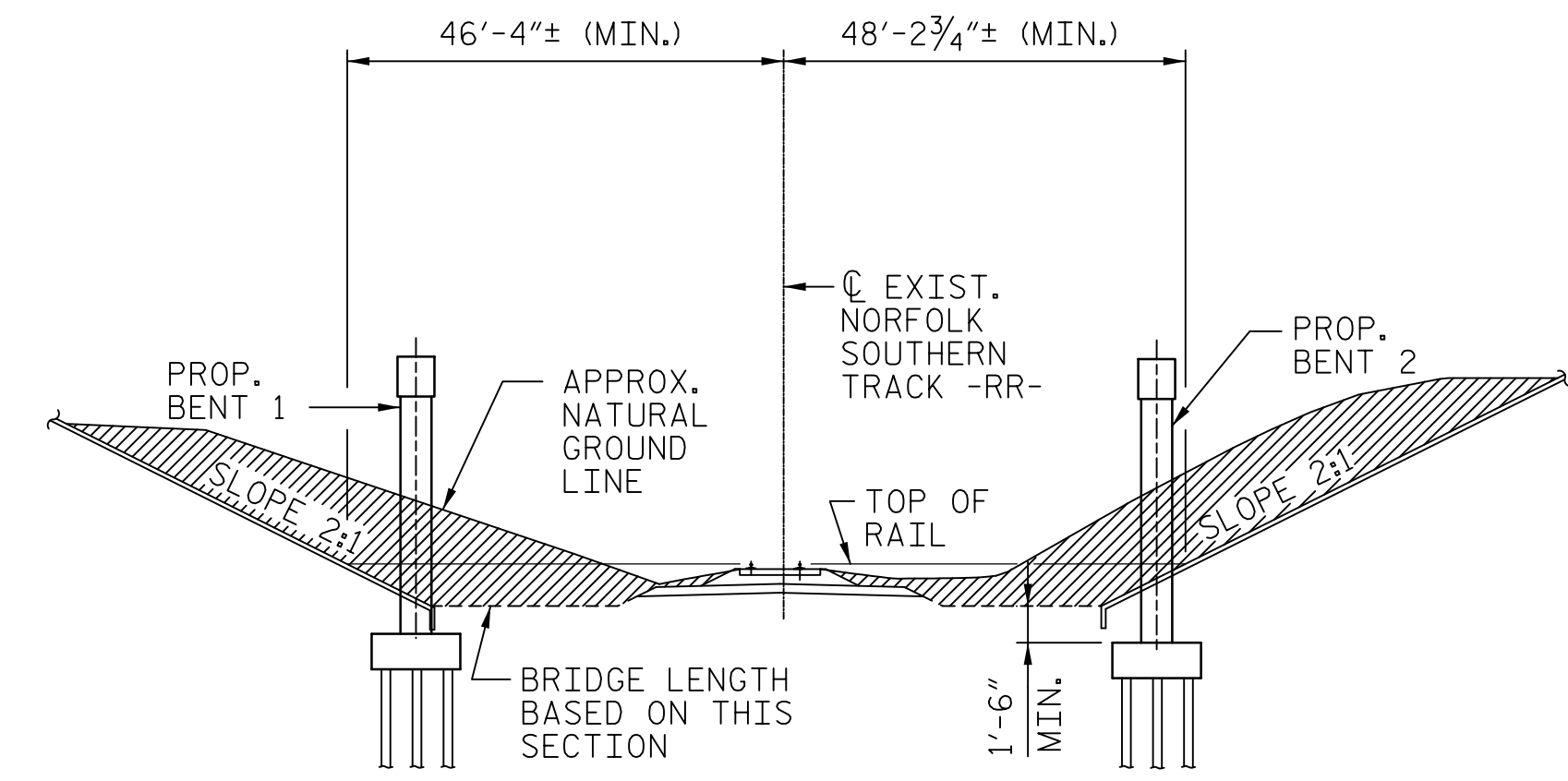
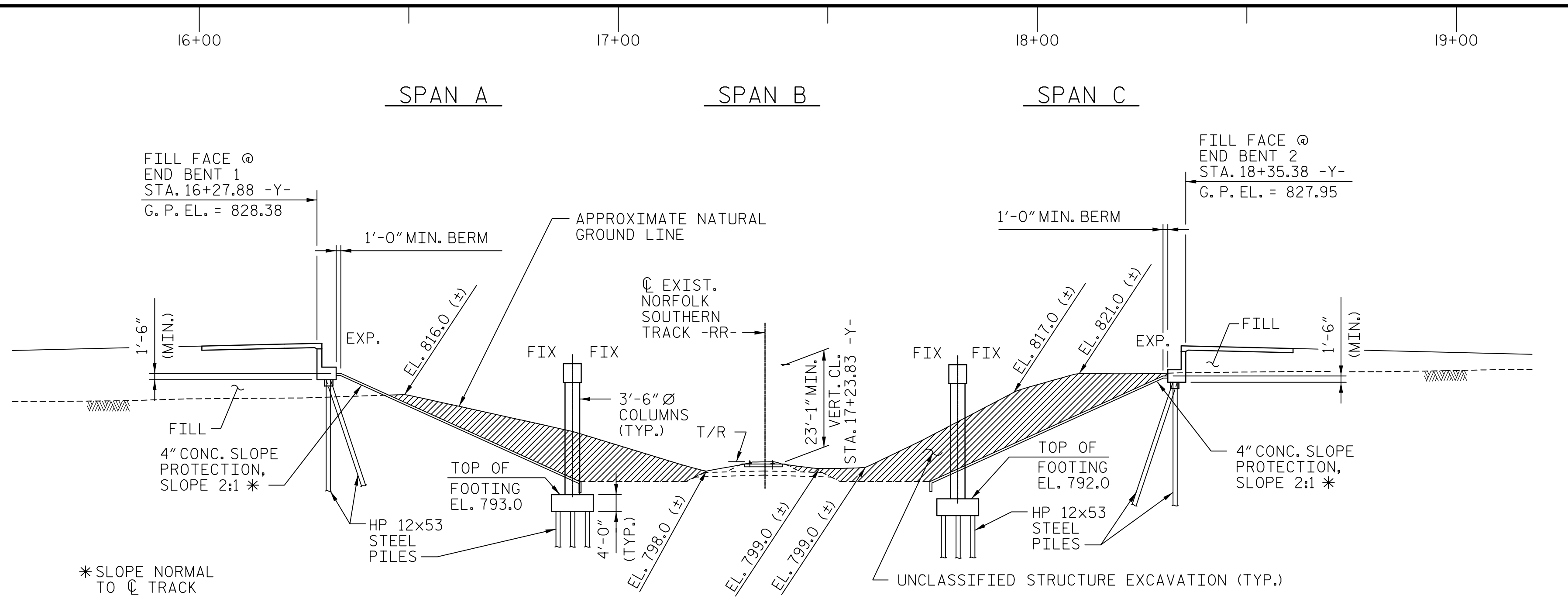


| | | | | | |
|-------------------------------------------------------------------------------------------------------|-----|-------|-----|-----|------------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD = RIP RAP DETAILS = | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. S-40 | | | | | TOTAL SHEETS 86 |

| | |
|-----------------------|----------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : REK 1/84 | REV. 5/1/06R TLA/GM |
| CHECKED BY : RDU 1/84 | REV. 10/1/11 MAA/GM |
| | REV. 12/21/11 MAA/GM |

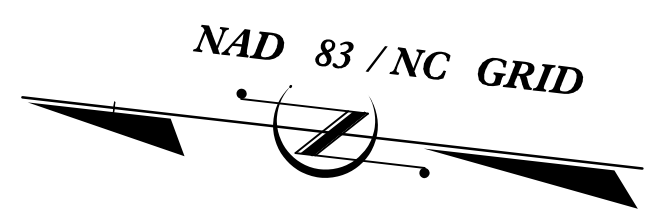
DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

1/12/2016



GRADE DATA -Y-
(+2.3400% (-2.4533%)

PI STA. 17+25.00 -Y-
ELEV. = 830.660
K = 44
V.C. = 210.000'



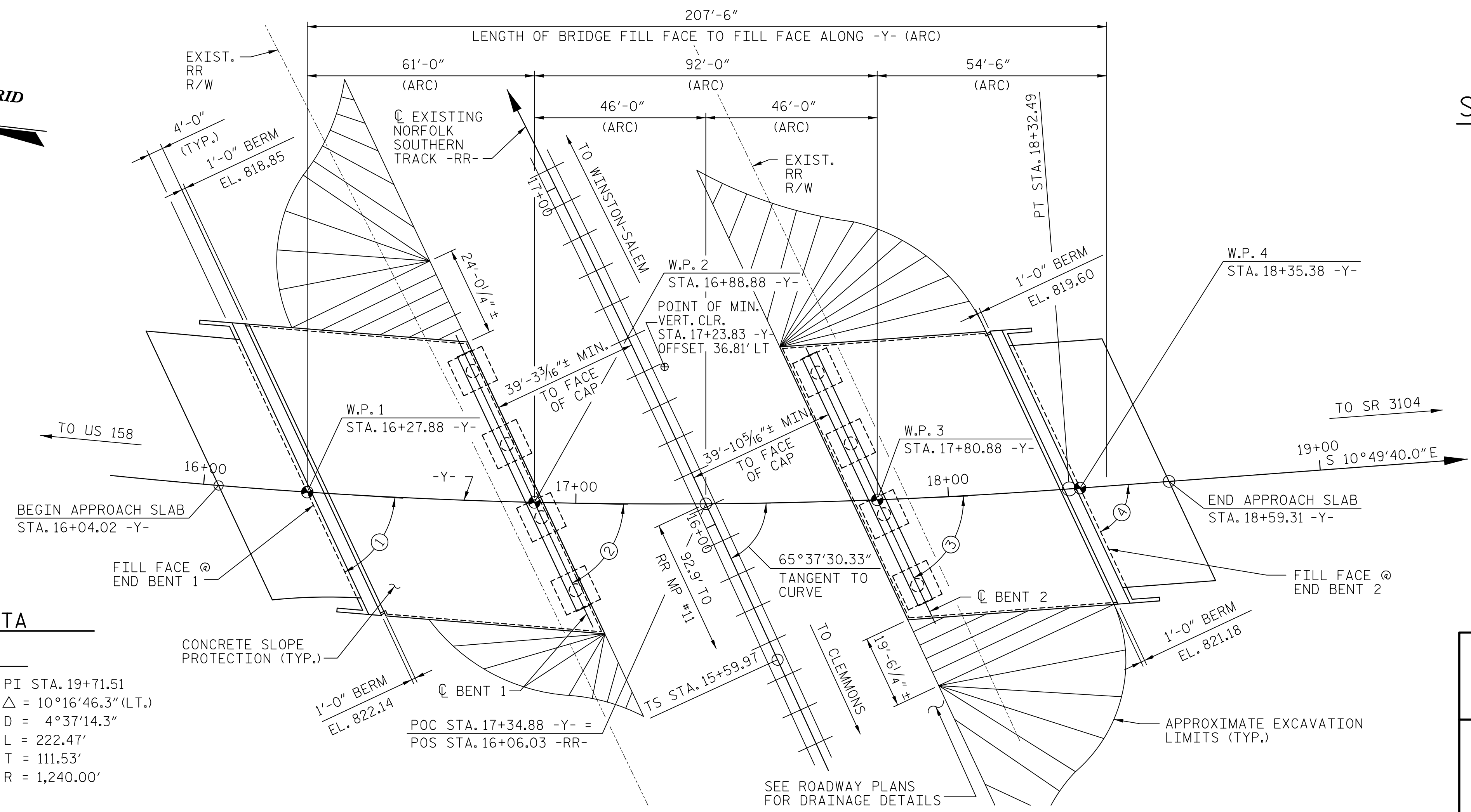
| TABLE OF ANGLES | |
|-----------------|-----------|
| No. | ANGLE |
| 1 | 61°03'44" |
| 2 | 63°28'25" |
| 3 | 67°06'31" |
| 4 | 69°08'51" |

ANGLES ARE TANGENT TO CURVE

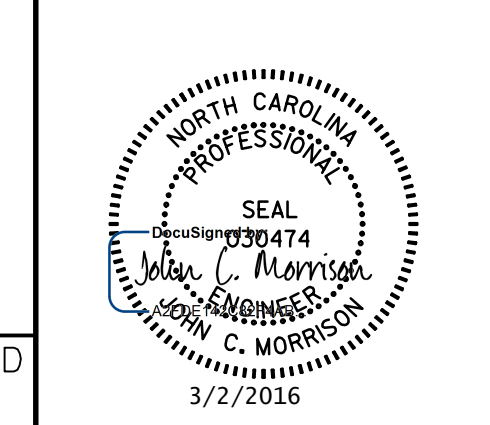
| TOP OF RAIL ELEVATIONS EXISTING NORFOLK SOUTHERN TRACK | |
|--------------------------------------------------------|-------------------|
| TRACK STATION | LEFT & RIGHT RAIL |
| STA. 15+50.00 | 800.849 |
| STA. 16+00.00 | 800.173 |
| STA. 16+50.00 | 799.538 |

HORIZONTAL CURVE DATA

| -Y- | -RR- | -RR- |
|-----------------------|-------------------|-----------------------|
| PI STA. 15+84.89 | PIs STA. 17+60.13 | PI STA. 19+71.51 |
| Δ = 19°45'58.9" (LT.) | θs = 6°55'51.4" | Δ = 10°16'46.3" (LT.) |
| D = 3°57'05.2" | Ls = 300.00' | D = 4°37'14.3" |
| L = 500.23' | LT = 200.15' | L = 222.47' |
| T = 252.63' | ST = 100.14' | T = 111.53' |
| R = 1,450.00' | | R = 1,240.00' |
| SE = 0.03 | | |



PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 1 OF 4 BRIDGE NO. 109 MILEPOST #10.98

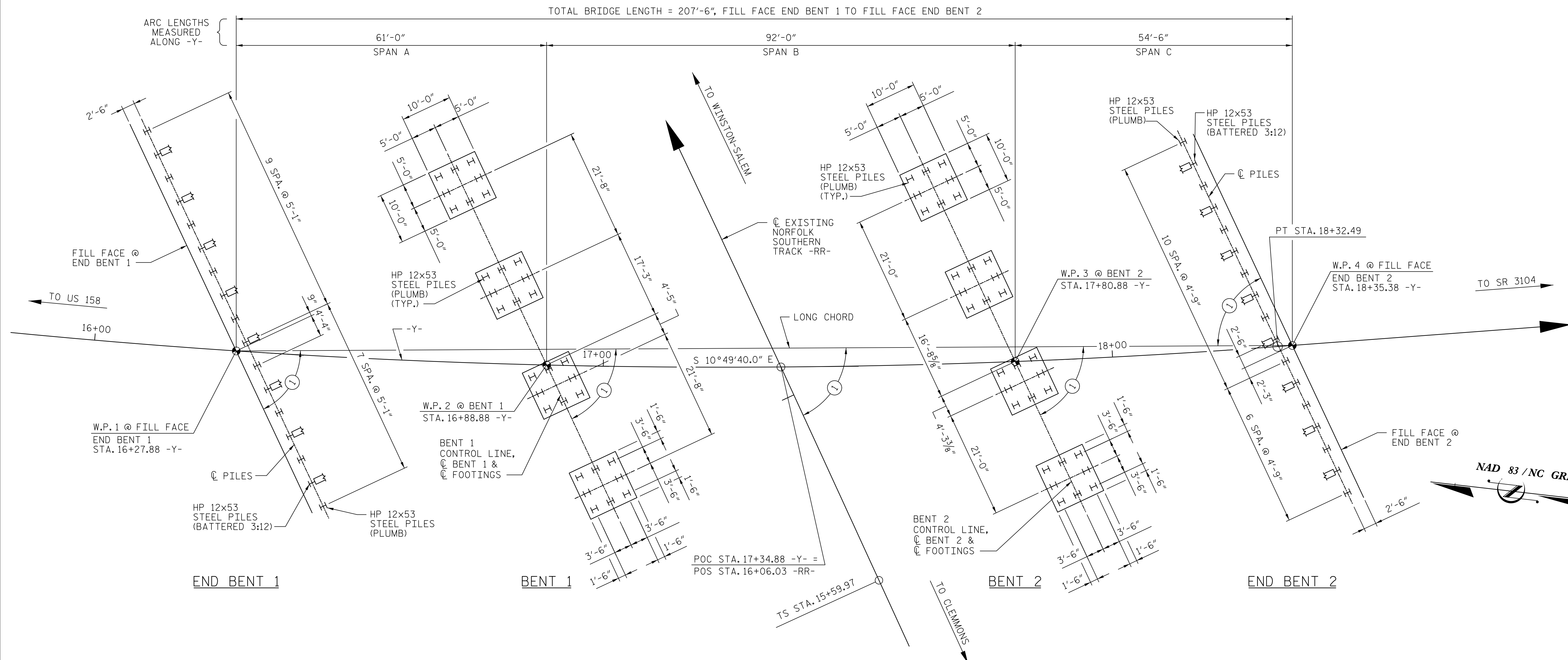


| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
|----------------------------------------------------------------------------------------------------------------|-----|-------|-----|-----|-----------------|
| GENERAL DRAWING FOR BRIDGE ON SR 2999 (HAMPTON ROAD) OVER NORFOLK SOUTHERN RAILROAD BETWEEN US 158 AND SR 3104 | | | | | |
| REVISIONS | | | | | SHEET NO. |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| | | | | | S-43 |
| | | | | | TOTAL SHEETS 86 |

DRAWN BY: KHC DATE: 10/2015
CHECKED BY: JCM DATE: 10/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

USER: \\sfr\p000\ncasomes\com\Data\Proj\leas\604462\16400_Technical\400_NC DOT_Shd_Tech_Design_Files\Structures\Bridg 109\Cad\402_001_U2707_SML_S43_GDD.dgn
 DATE: 3/2/2016
 TIME: 6:53:52 AM



FOUNDATION LAYOUT

① 65°09'40" TO LONG CHORD

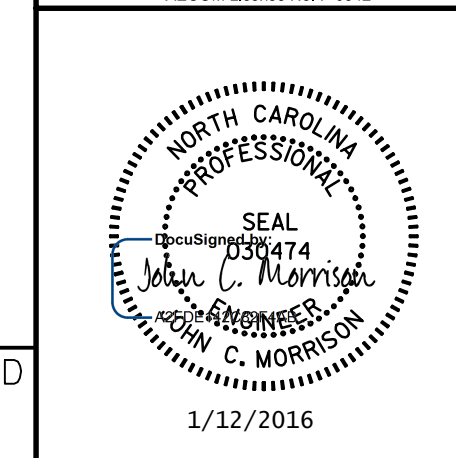
NOTES:

- FOR PILES, SEE PROJECT SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 70 TONS PER PILE.
- DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS.
- PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 65 TONS PER PILE.
- DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 110 TONS PER PILE.
- PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.
- DRIVE PILES AT BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 170 TONS PER PILE.
- PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 779 FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT BENT NO.1.
- STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT BENT NO.1. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- PILES AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 95 TONS PER PILE.
- DRIVE PILES AT BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 160 TONS PER PILE.

PROJECT NO. U-2707
 FORSYTH COUNTY

STATION: 17+34.88 -Y-

SHEET 2 OF 4 BRIDGE NO. 109



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE ON SR 2999
 (HAMPTON ROAD) OVER
 NORFOLK SOUTHERN RAILROAD
 BETWEEN US 158 AND SR 3104

| REVISIONS | | | | | | SHEET NO. | |
|-----------|-----|-------|-----|-----|-------|--------------|--|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-44 | |
| 1 | | | 3 | | | TOTAL SHEETS | |
| 2 | | | 4 | | | 86 | |

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

DATE: 1/12/2016
 TIME: 8:55:22 AM

USER: jmc@aecom.com
 DGN: R:\2999\109\109-001\109-001.dgn

DRAWN BY: KHC DATE: 10/2015
 CHECKED BY: JCM DATE: 10/2015

GENERAL NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THE EXISTING STRUCTURE CONSISTING OF A 3 SPAN CONTINUOUS REINFORCED CONCRETE SLAB SUPERSTRUCTURE WITH SPANS OF 24'-0", 22'-0" AND 24'-0"; CLEAR ROADWAY WIDTH OF 28'-0" AND REINFORCED CONCRETE END BENTS AND BENTS SUPPORTED ON PRECAST PRESTRESSED CONCRETE PILES; AND HAVING PRECAST CONCRETE BULKHEADS; AND LOCATED 93 FT WEST OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

RAILROAD TRACKS AND OTHER RAILROAD PROPERTY MUST BE PROTECTED FROM DAMAGE DURING REMOVAL OF THE EXISTING STRUCTURE. THE CONTRACTOR SHALL SUBMIT A DETAILED PROCEDURE FOR DEMOLITION OF THE EXISTING STRUCTURE IN ACCORDANCE WITH THE REQUIREMENTS OUTLINED BY NORFOLK SOUTHERN'S GUIDELINES FOR DESIGN OF HIGHWAY SEPARATION STRUCTURES OVER RAILROAD (OVERHEAD GRADE SEPARATION). DEMOLITION PLANS AND PROCEDURE SHALL BE SUBMITTED TO THE CHIEF ENGINEER BRIDGES & STRUCTURES FOR REVIEW AND APPROVAL PRIOR TO BEGINNING DEMOLITION.

THE SUBSTRUCTURE TYPE INDICATED FOR THE EXISTING BRIDGE IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS IDENTIFIED FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE IDENTIFIED AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE RAILROAD TRACK TOP OF RAIL ELEVATIONS SHOWN ON THE PLANS ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE TOP OF RAIL ELEVATIONS AND REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

REMOVABLE FORMS MAY BE USED IN LIEU OF STAY-IN-PLACE METAL FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

PRESTRESSED CONCRETE DECK PANELS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS. THE SKEWED END CONDITION AT EACH END BENT AND BENT IS SUCH THAT THE USE OF 4' WIDE PRESTRESSED CONCRETE DECK PANELS IS NOT POSSIBLE; USE OF 8' WIDE PRESTRESSED CONCRETE DECK PANELS IS NECESSARY.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

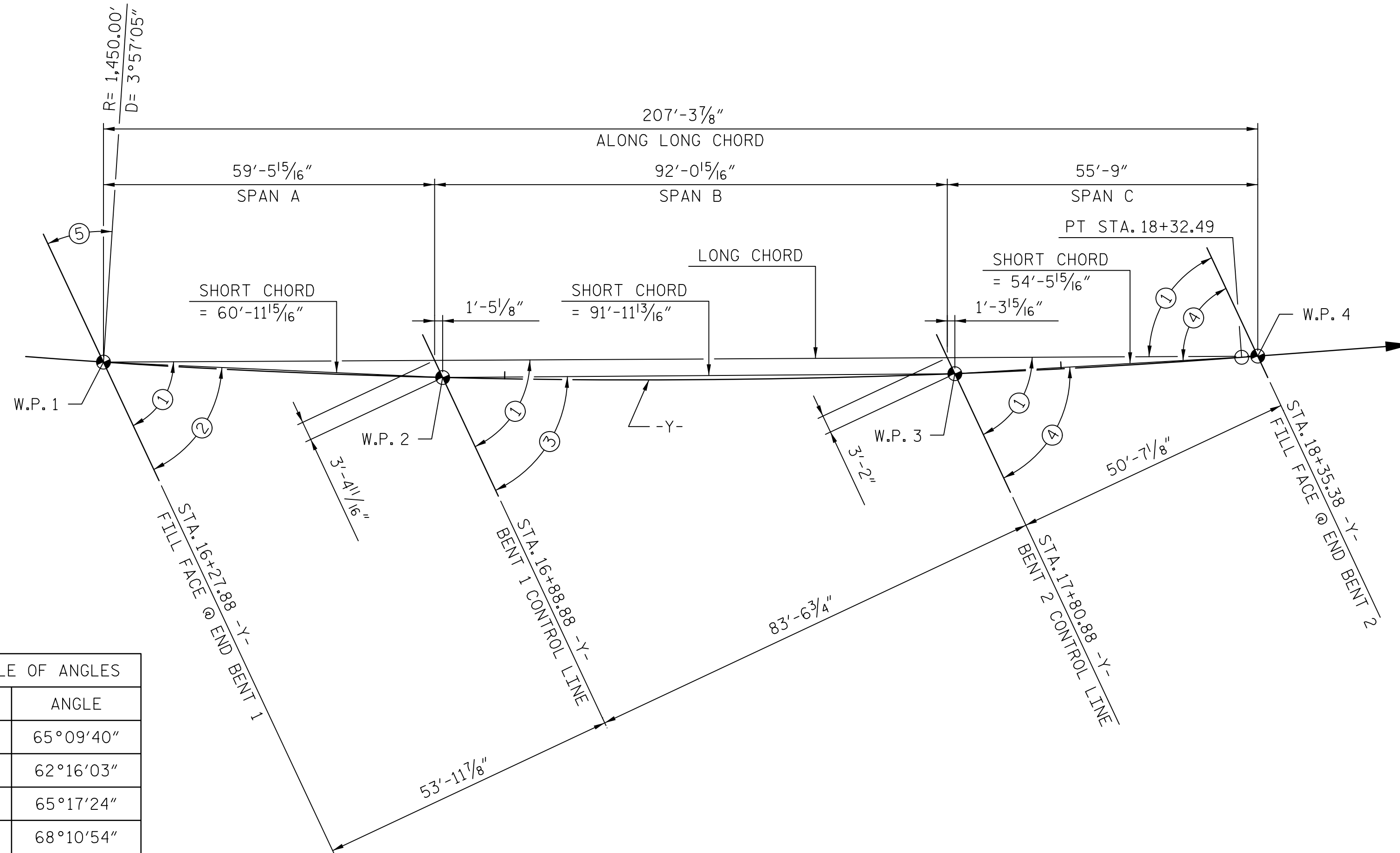
FOR PLACING LOAD ON STRUCTURE MEMBERS, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET 1 OF 4 SHALL BE EXCAVATED FOR A DISTANCE OF 90± FEET EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

WORK SHALL NOT BE STARTED ON THIS BRIDGE UNTIL EXCAVATION IS COMPLETE.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.



LONG CHORD LAYOUT
ALL BENTS AND END BENTS ARE PARALLEL

| TABLE OF ANGLES | |
|-----------------|-----------|
| No. | ANGLE |
| 1 | 65°09'40" |
| 2 | 62°16'03" |
| 3 | 65°17'24" |
| 4 | 68°10'54" |
| 5 | 28°56'16" |

USFR: \\sfr\p00\ncases\mef.com\Data\Proj\Bents\604462\16400_Technical\400_NCDOT_Shd_Tech_Design_Files\Structure\Bent\109\Coord\02_005_U2707_SML_545_G00.dgn TIME: 6:56:00 AM
 DATE: 3/2/2016
 DN: \\sfr\p00\ncases\mef.com\Data\Proj\Bents\604462\16400_Technical\400_NCDOT_Shd_Tech_Design_Files\Structure\Bent\109\Coord\02_005_U2707_SML_545_G00.dgn TIME: 6:56:00 AM

DRAWN BY : KHC DATE : 10/2015
 CHECKED BY : JCM DATE : 10/2015

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 3 OF 4 BRIDGE NO. 109

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 030474
 W. C. MORRIS
 ENGINEER
 3/2/2016

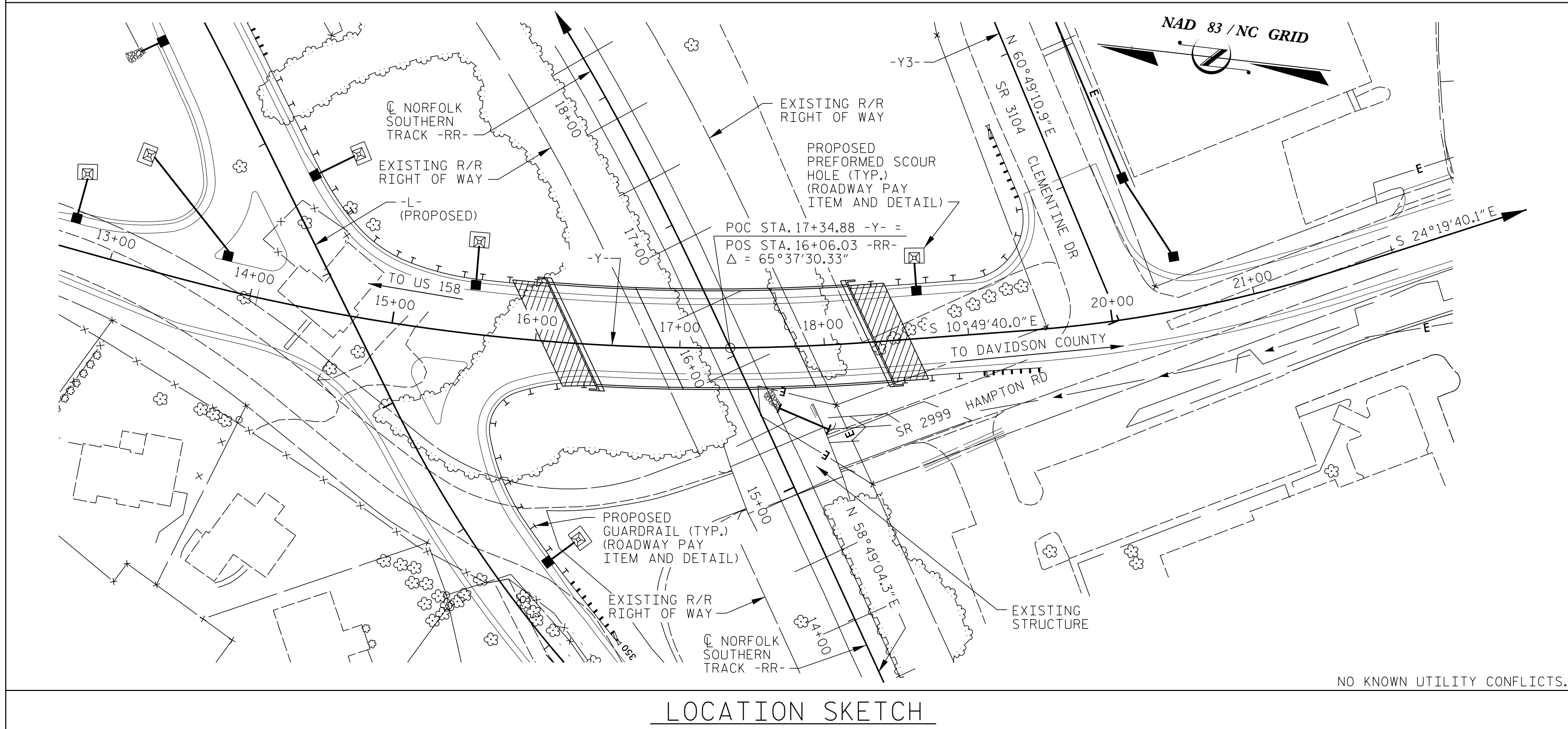
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

GENERAL DRAWING
 FOR BRIDGE ON SR 2999
 (HAMPTON ROAD) OVER
 NORFOLK SOUTHERN RAILROAD
 BETWEEN US 158 AND SR 3104

| REVISIONS | | | | | | SHEET NO. S-45 |
|-----------|-----|-------|-----|-----|-------|--------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | |
| 1 | | | 3 | | | TOTAL SHEETS 86 |
| 2 | | | 4 | | | |

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

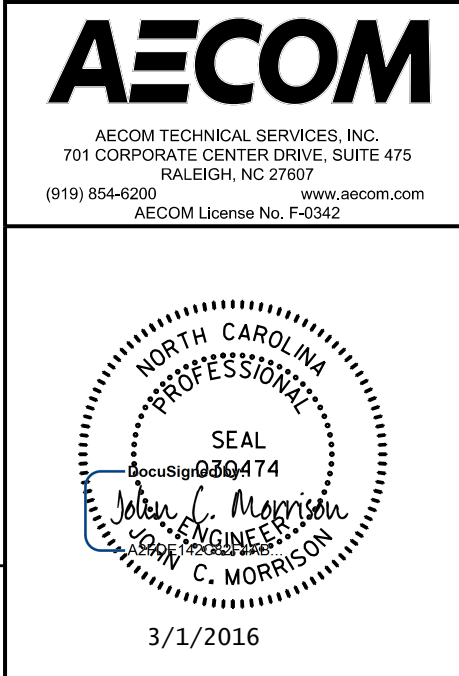
BENCH MARK: BM-3 RAILROAD SPIKE IN BASE OF 18" WHITE OAK,
 139.00' RIGHT OF STA. 9+80.00 -Y-
 COORDINATES: N 824600 E 1594553
 ELEV. = 811.83



TOTAL BILL OF MATERIAL

| | REMOVAL OF EXISTING STRUCTURE AT STA. 17+34.88 -Y- | PILE EXCAVATION | | FOUNDATION EXCAVATION FOR BENT | UNCLASSIFIED STRUCTURE EXCAVATION | REINFORCED CONCRETE DECK SLAB | GROOVING BRIDGE FLOORS | CLASS A CONCRETE | BRIDGE APPROACH SLABS | REINFORCING STEEL | SPIRAL COLUMN REINFORCING STEEL | 54" PRESTRESSED CONCRETE GIRDERS | | HP 12x53 STEEL PILES | STEEL PILE POINTS | TWO BAR METAL RAIL | 1'-2" x 2'-6" CONCRETE PARAPET | 72" CHAIN LINK FENCE | 4" SLOPE PROTECTION | ELASTOMERIC BEARINGS | FOAM JOINT SEALS | ASBESTOS ASSESSMENT | |
|----------------|----------------------------------------------------|-----------------|-------------|--------------------------------|-----------------------------------|-------------------------------|------------------------|------------------|-----------------------|-------------------|---------------------------------|----------------------------------|----------|----------------------|-------------------|--------------------|--------------------------------|----------------------|---------------------|----------------------|------------------|---------------------|----------|
| | | IN SOIL | NOT IN SOIL | | | | | | | | | NO. | LIN. FT. | | | | | | | | | | |
| | LUMP SUM | LIN. FT. | LIN. FT. | LUMP SUM | LUMP SUM | SQ. FT. | SQ. FT. | CU. YDS. | LUMP SUM | LBS. | LBS. | NO. | LIN. FT. | NO. | LIN. FT. | EACH | LIN. FT. | LIN. FT. | LIN. FT. | SQ. YDS. | LUMP SUM | LUMP SUM | LUMP SUM |
| SUPERSTRUCTURE | | | | | | 14285 | 13492 | | LUMP SUM | | | 24 | 1615.23 | | | | 394.2 | 409.2 | 406.7 | | LUMP SUM | LUMP SUM | |
| END BENT No. 1 | | | | | LUMP SUM | | | 65.7 | | 10234 | | | | 17 | 935 | | | | | 598 | | | |
| BENT No. 1 | | 276 | 44 | LUMP SUM | | | | 151.2 | | 29282 | 2790 | | | 32 | 640 | 32 | | | | | | | |
| BENT No. 2 | | | | LUMP SUM | | | | 150.2 | | 29391 | 2884 | | | 32 | 1440 | | | | | | | | |
| END BENT No. 2 | | | | LUMP SUM | LUMP SUM | | | 59.6 | | 8897 | | | | 17 | 1105 | | | | | 533 | | | |
| TOTAL | LUMP SUM | 276 | 44 | LUMP SUM | LUMP SUM | 14285 | 13492 | 426.7 | LUMP SUM | 77804 | 5674 | 24 | 1615.23 | 96 | 4120 | 32 | 394.2 | 409.2 | 406.7 | 1131 | LUMP SUM | LUMP SUM | LUMP SUM |

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 4 OF 4 BRIDGE NO. 109



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

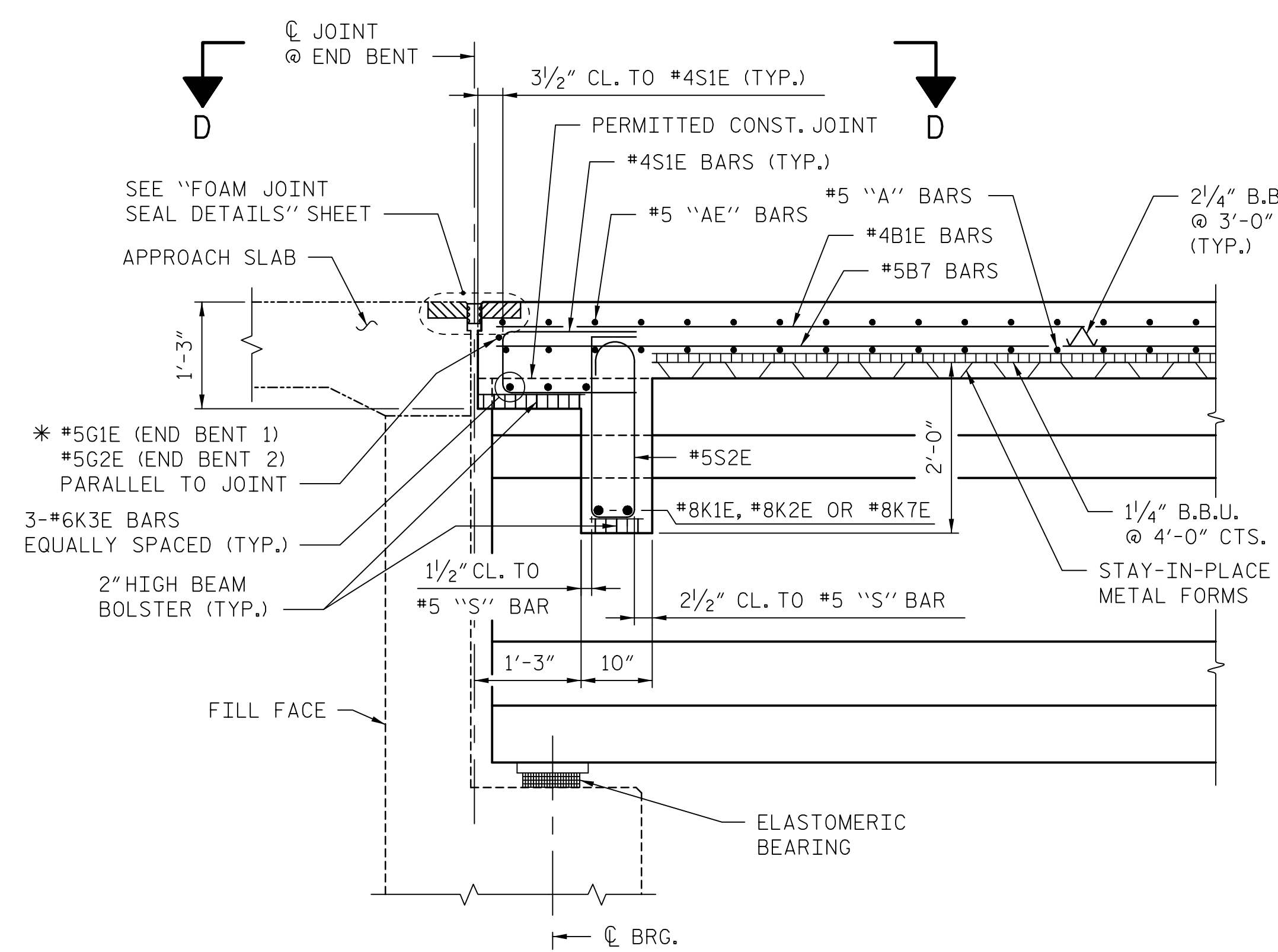
GENERAL DRAWING
 FOR BRIDGE ON SR 2999
 (HAMPTON ROAD) OVER
 NORFOLK SOUTHERN RAILROAD
 BETWEEN US 158 AND SR 3104

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-46 |
| 1 | | | 3 | | | TOTAL SHEETS |
| 2 | | | 4 | | | 86 |

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

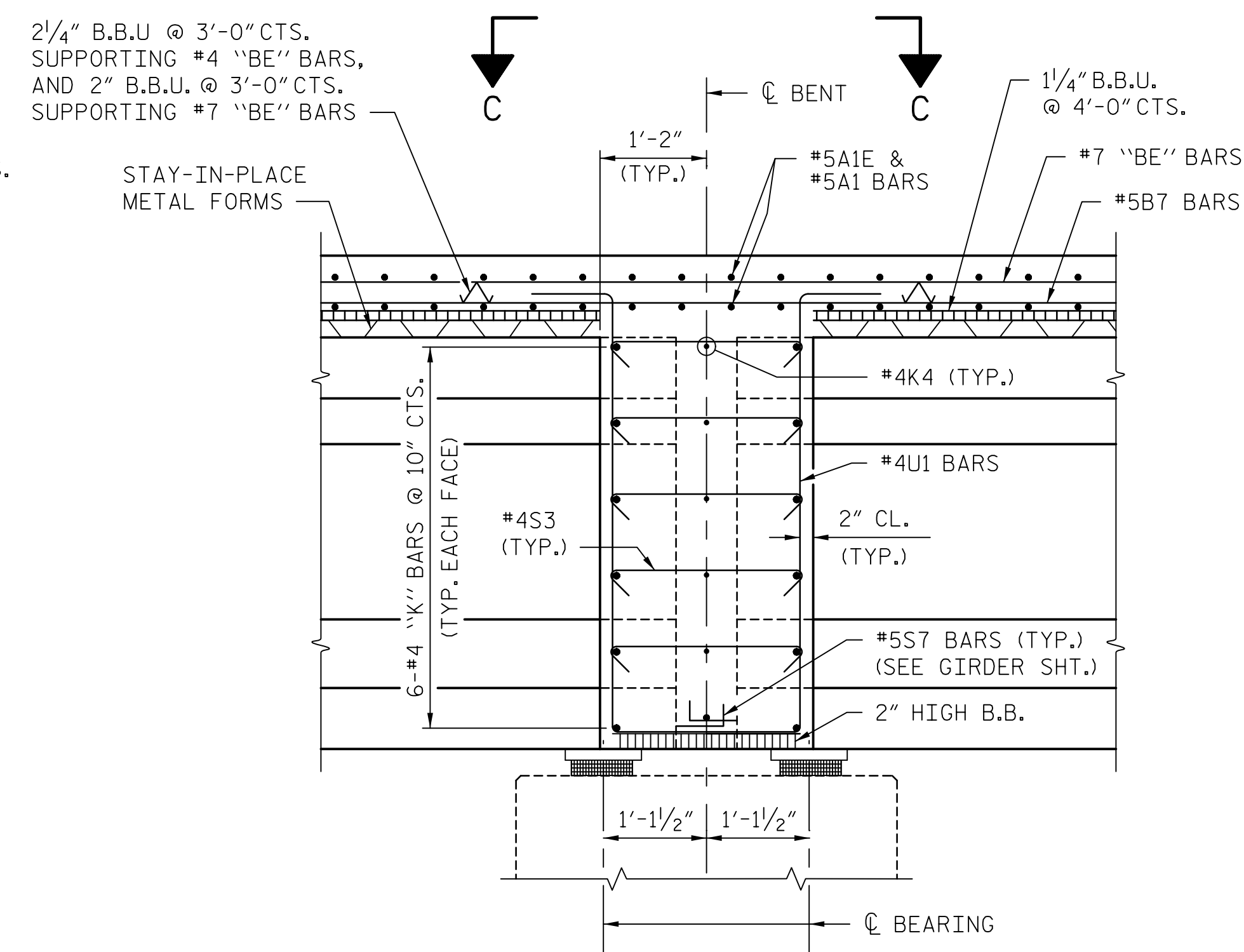
DATE: 3/1/2015 TIME: 2:55:28 PM
 USER: MCF50C DGN: R:\64426\8400_Tech\Design\Files\Structures\Bridges\109\Coord\A02_D07_U2707_SML_S46_G004.dgn

DRAWN BY: KHC DATE: 10/2015
 CHECKED BY: JCM DATE: 10/2015

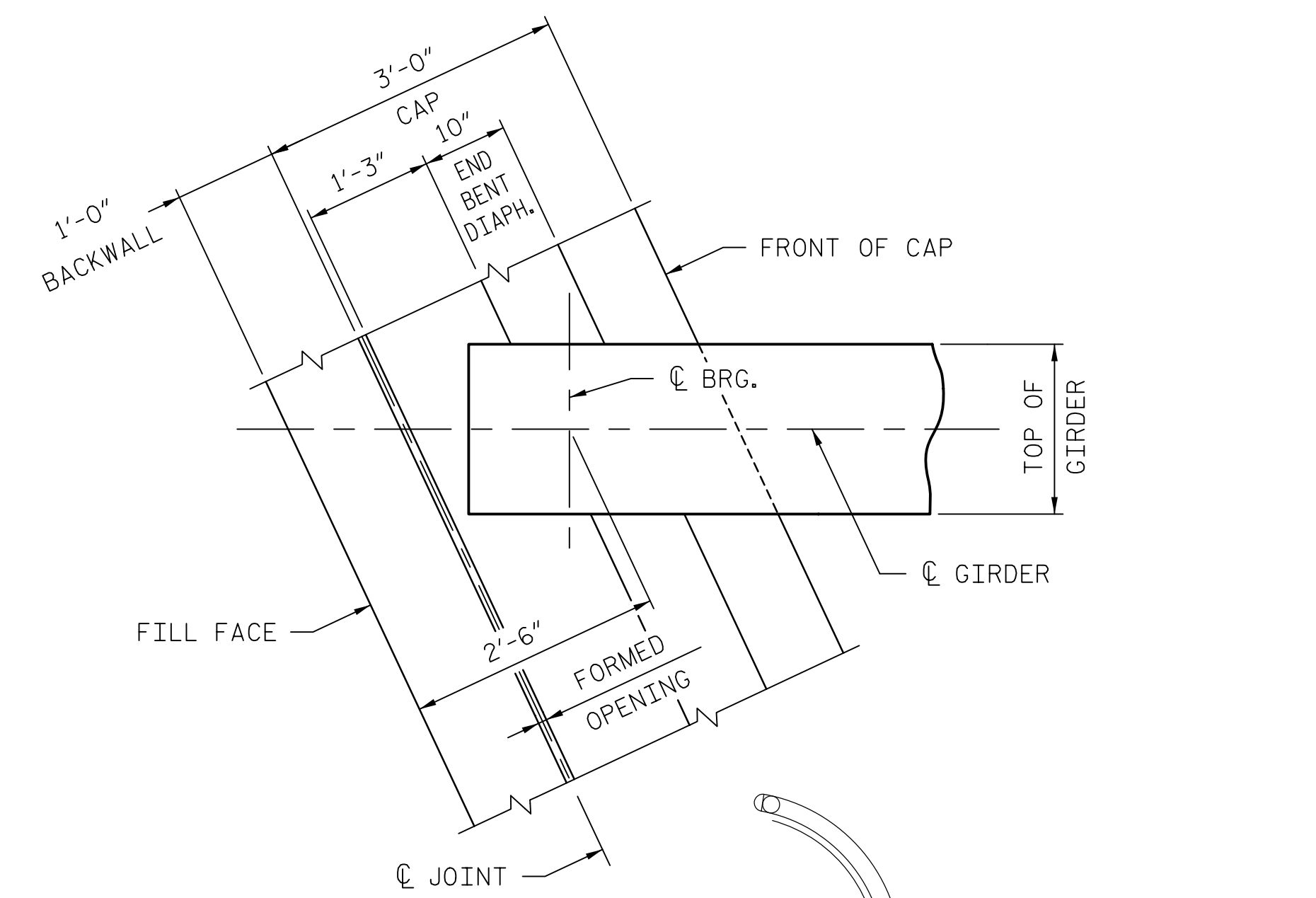


SECTION A-A
(END BENT 1 SHOWN, END BENT 2 SIMILAR)

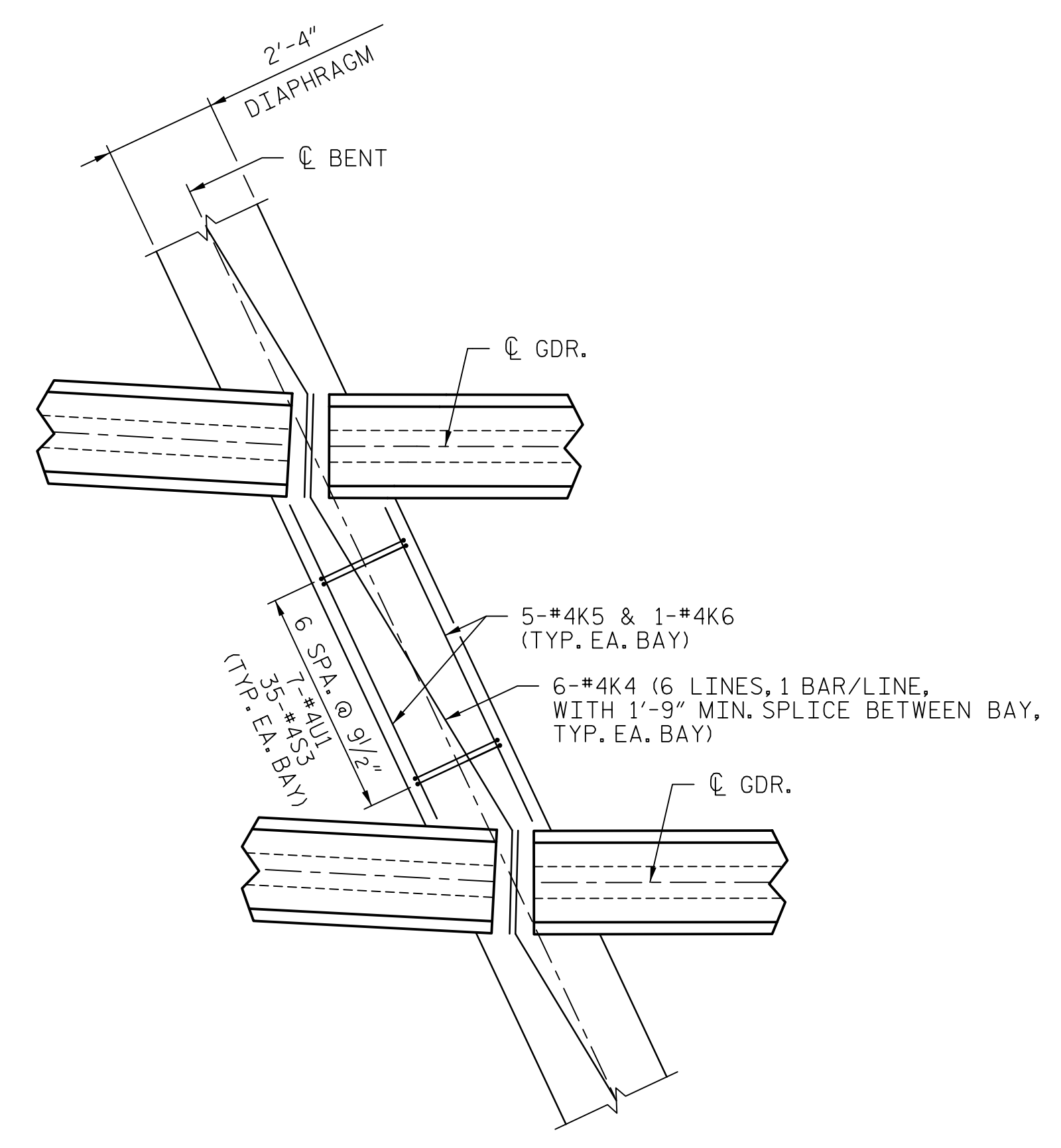
* #5 "G" BARS MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO CLEAR REINFORCING STEEL AND STIRRUPS.



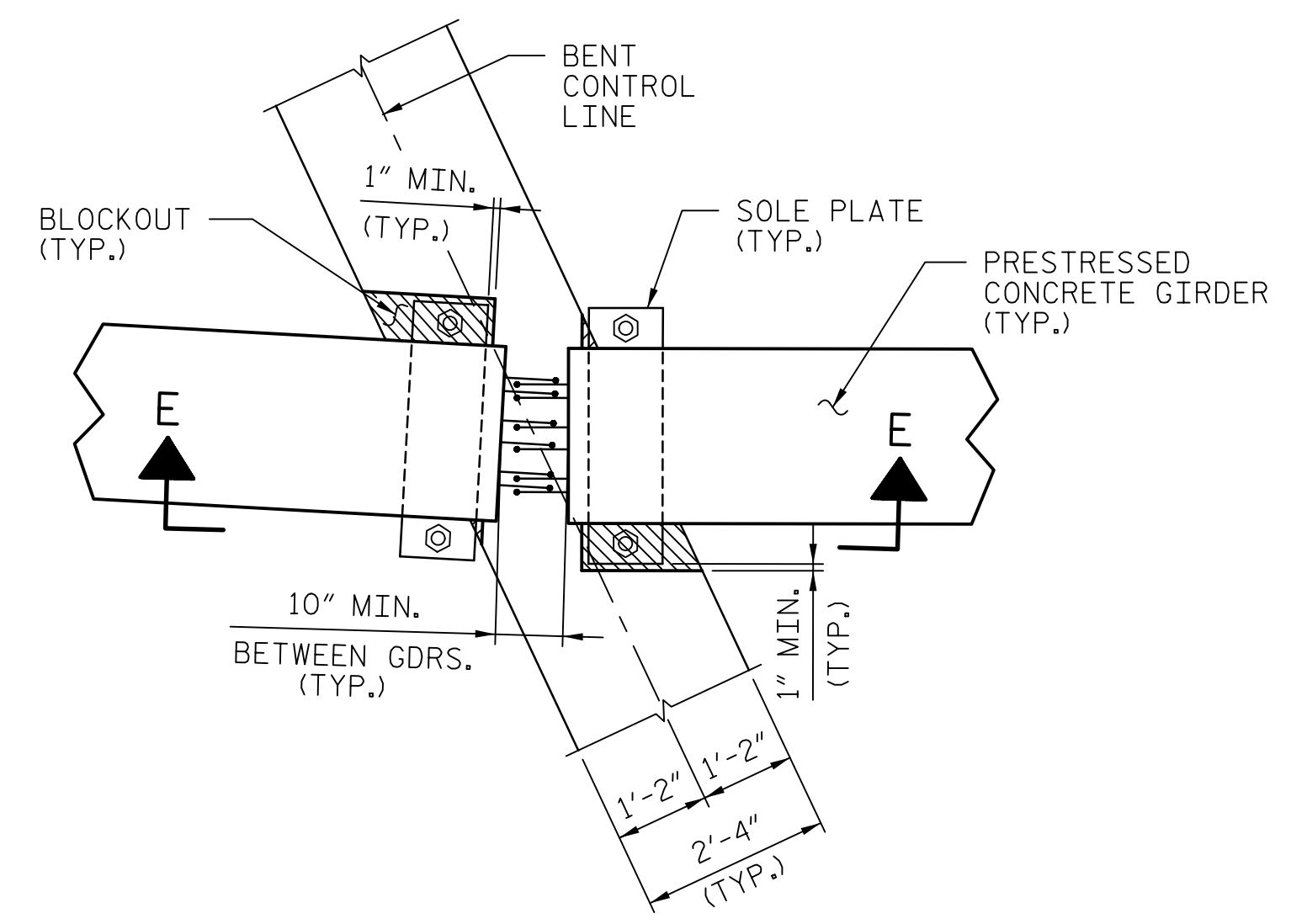
SECTION B-B



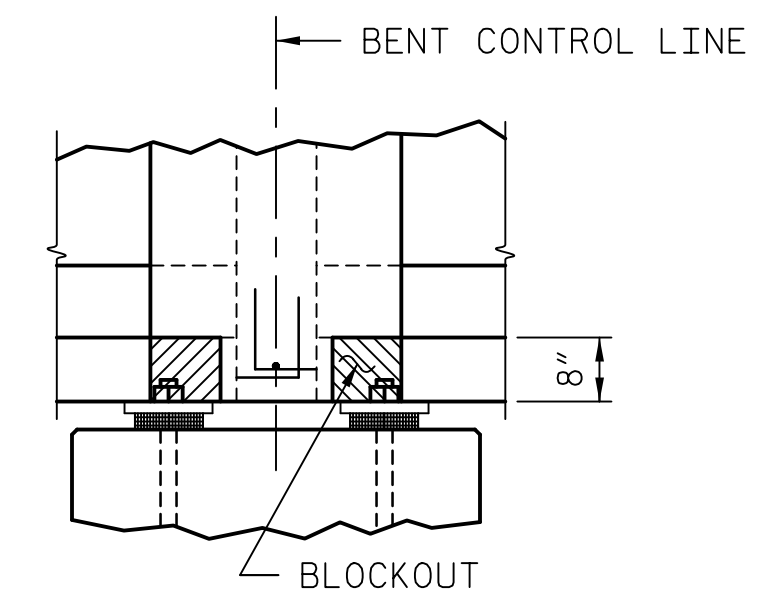
VIEW D-D



VIEW C-C

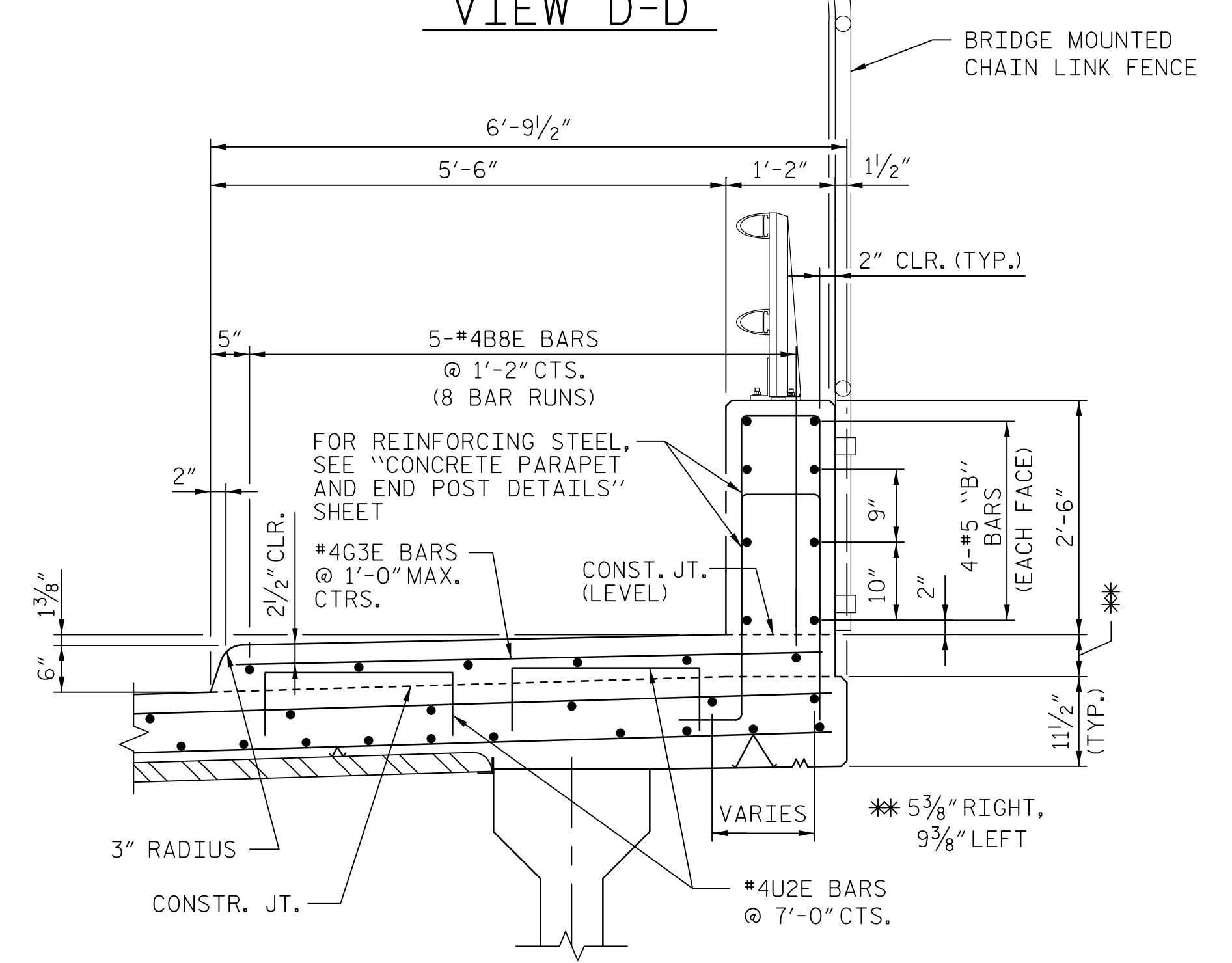


PLAN VIEW



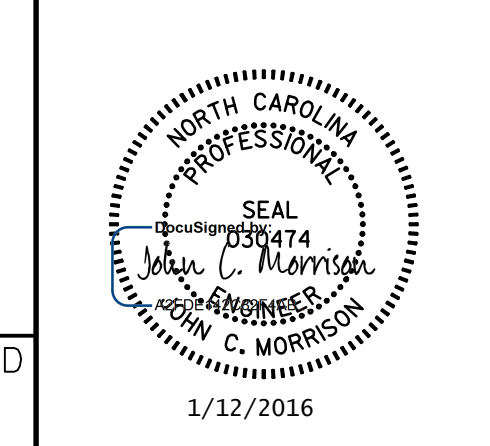
SECTION E-E

BENT DIAPHRAGM BLOCKOUT DETAIL



SECTION THRU SIDEWALK
(RIGHT SIDE SHOWN, LEFT SIDE SIMILAR)

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 2 OF 2 BRIDGE NO. 109



| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUPERSTRUCTURE TYPICAL SECTION DETAILS | | | | | |
| SHEET NO. S-49 | | | | | |
| TOTAL SHEETS 86 | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

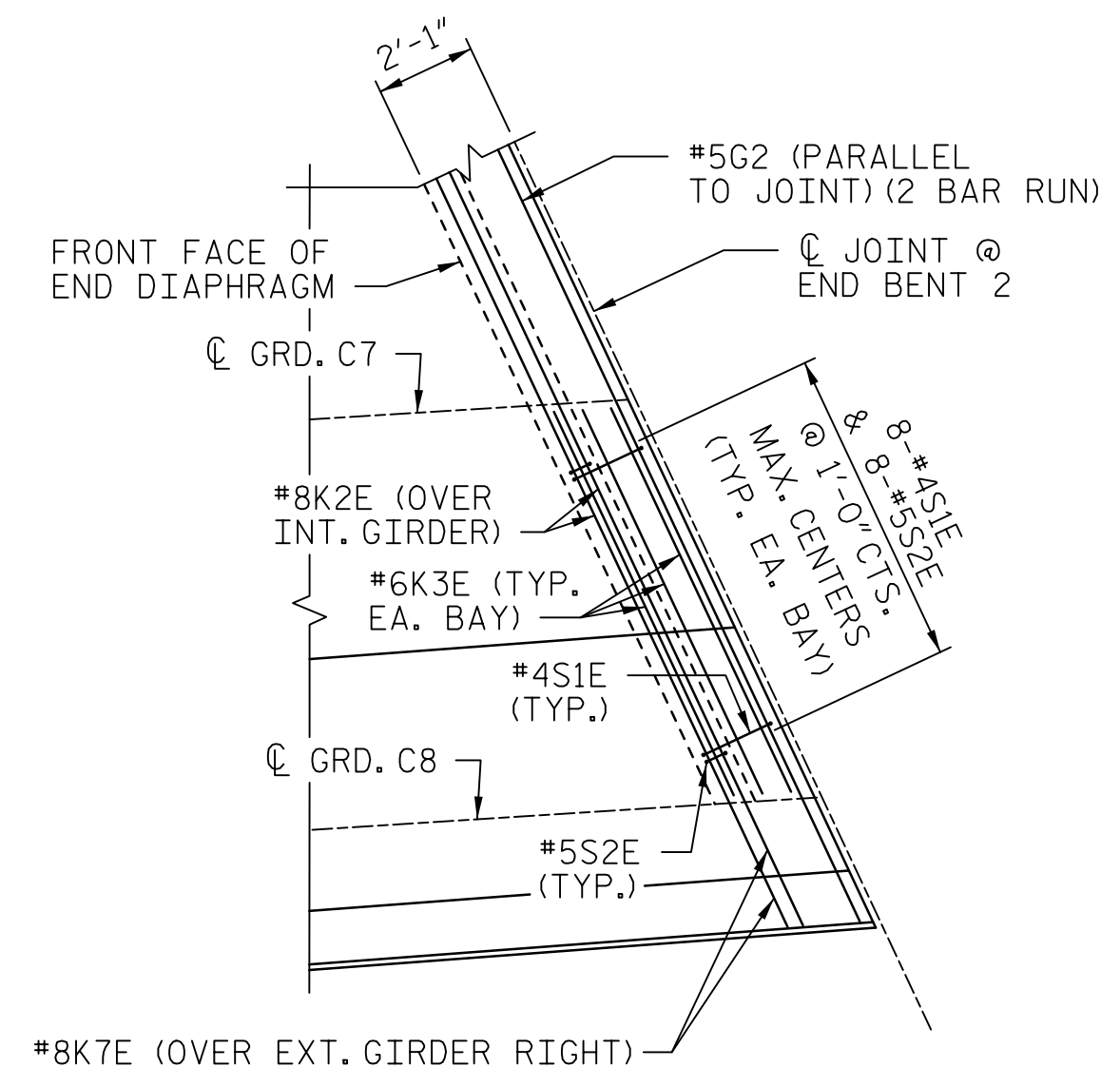
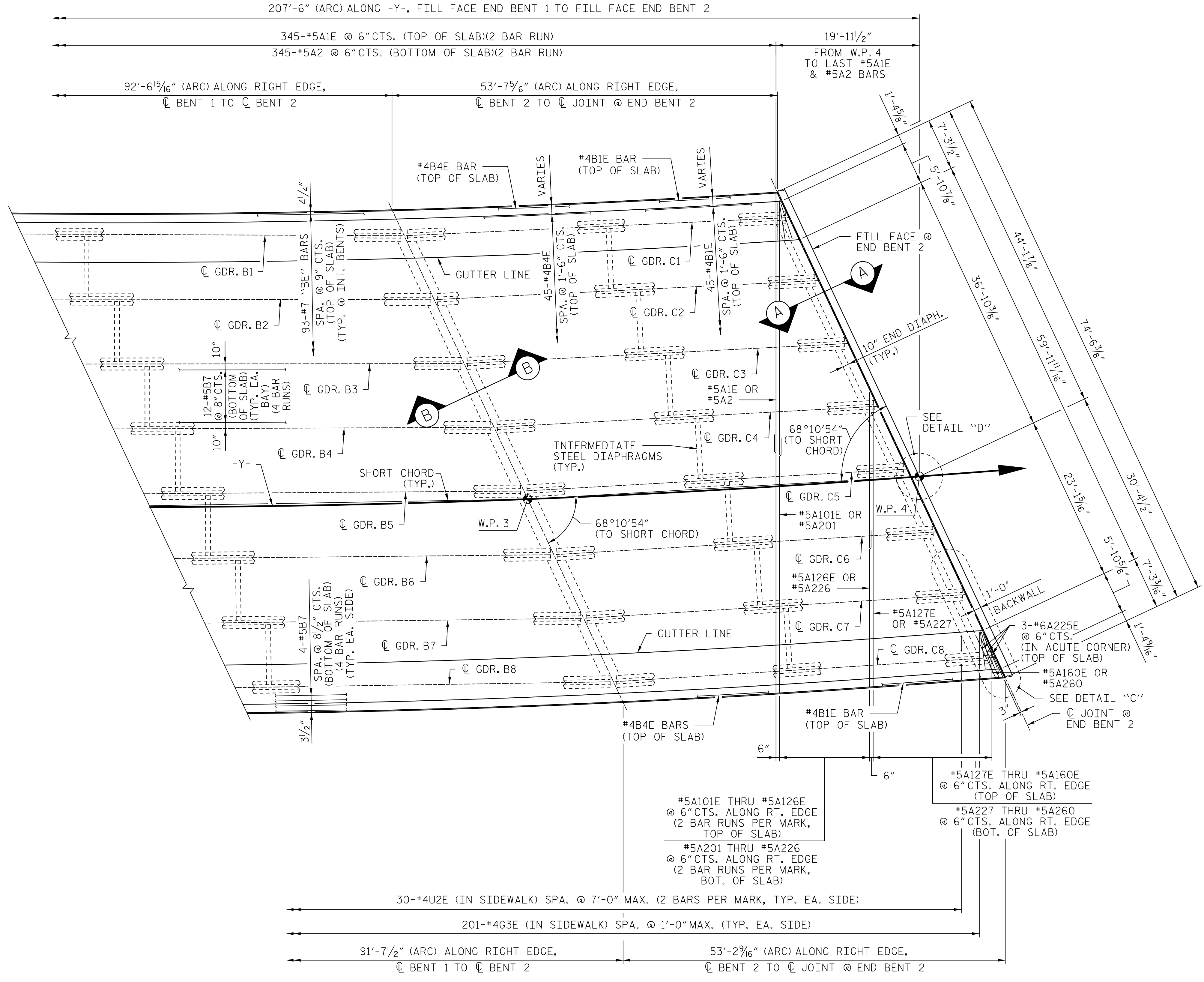
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 1/12/2016 TIME: 8:55:40 AM

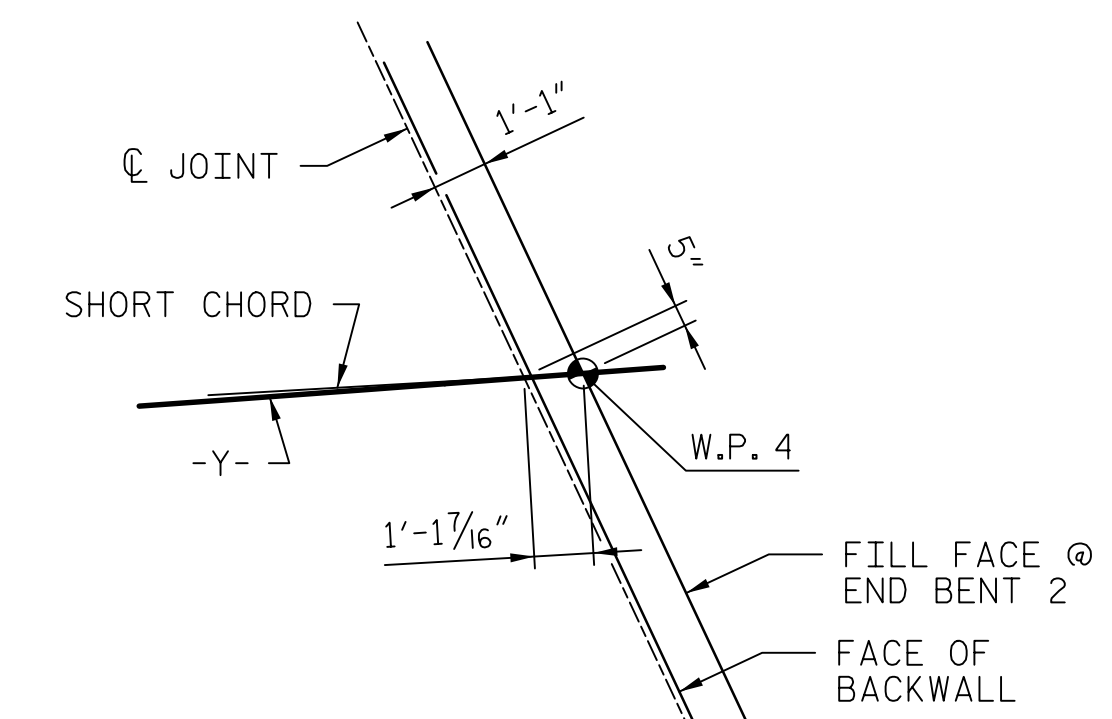
USER: \\s01c01p01\proj\109\109-100\109-100-103-102707_SML-549-T-502.dgn

DRAWN BY: DDL DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

FOR NOTES, SEE SHEET 1 OF 3.



DETAIL "C" "A" & "B" BARS NOT SHOWN



DETAIL "D"

SPAN B

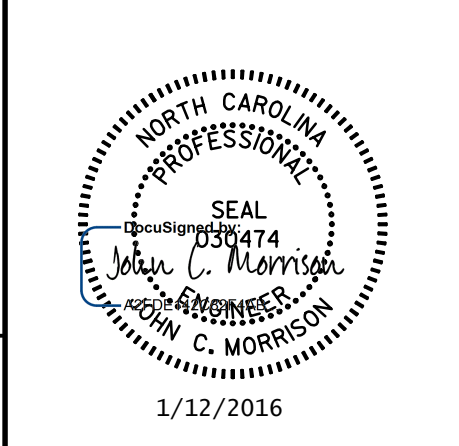
SPAN C

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 17+34.88 -Y-

SHEET 2 OF 3 BRIDGE NO. 109



| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-----------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUPERSTRUCTURE PARTIAL PLAN OF SPAN B AND PLAN OF SPAN C | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. S-51 | | | | | TOTAL SHEETS 86 |

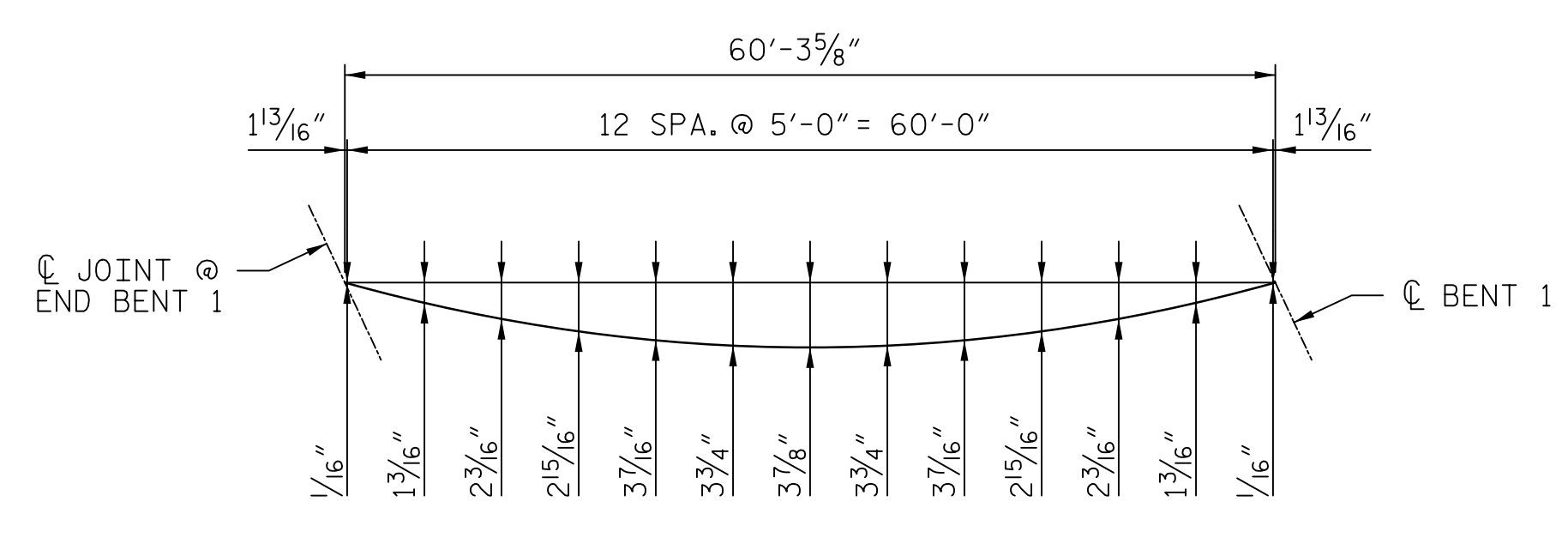
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: DDL DATE: 4/2015

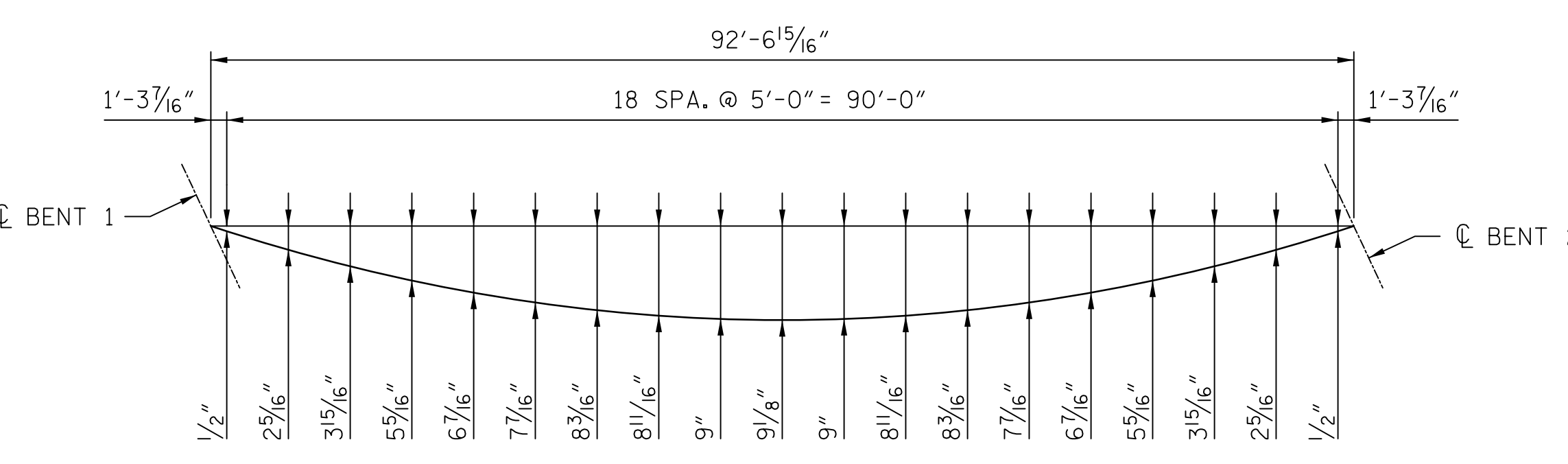
CHECKED BY: JCM DATE: 4/2015

DATE: 1/12/2016 TIME: 8:55:48 AM

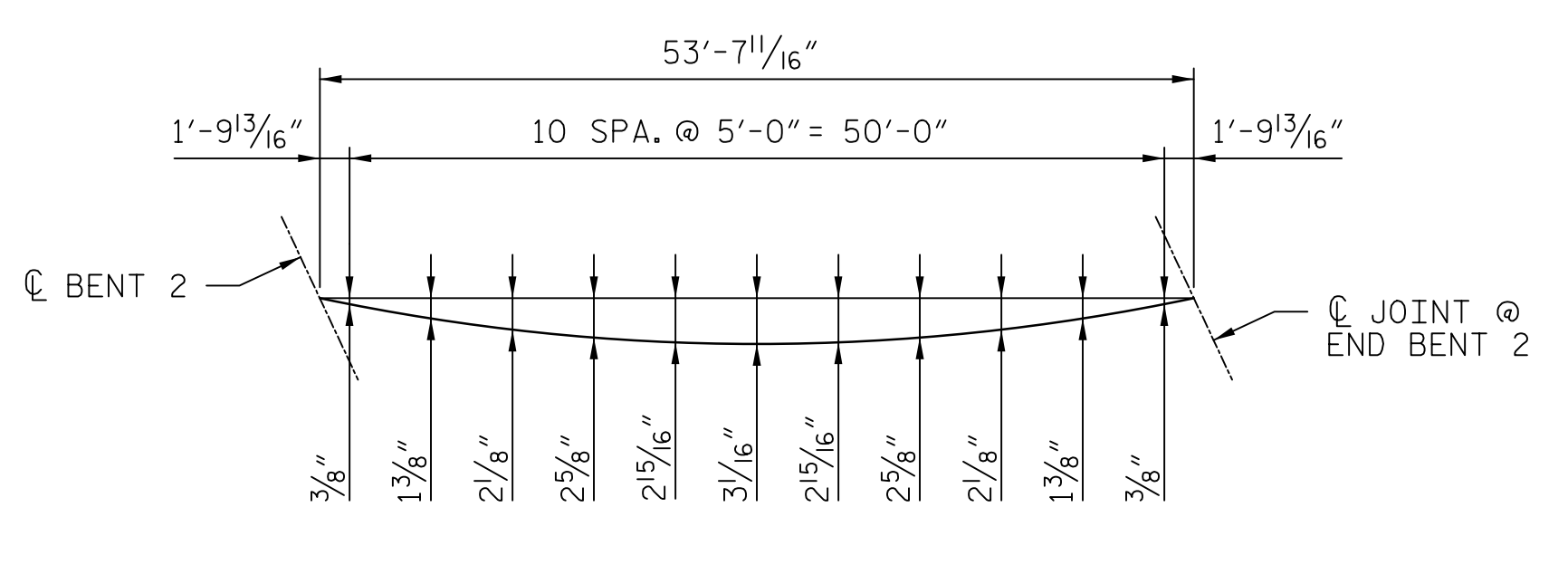
USER: jcm\jcm.dgn



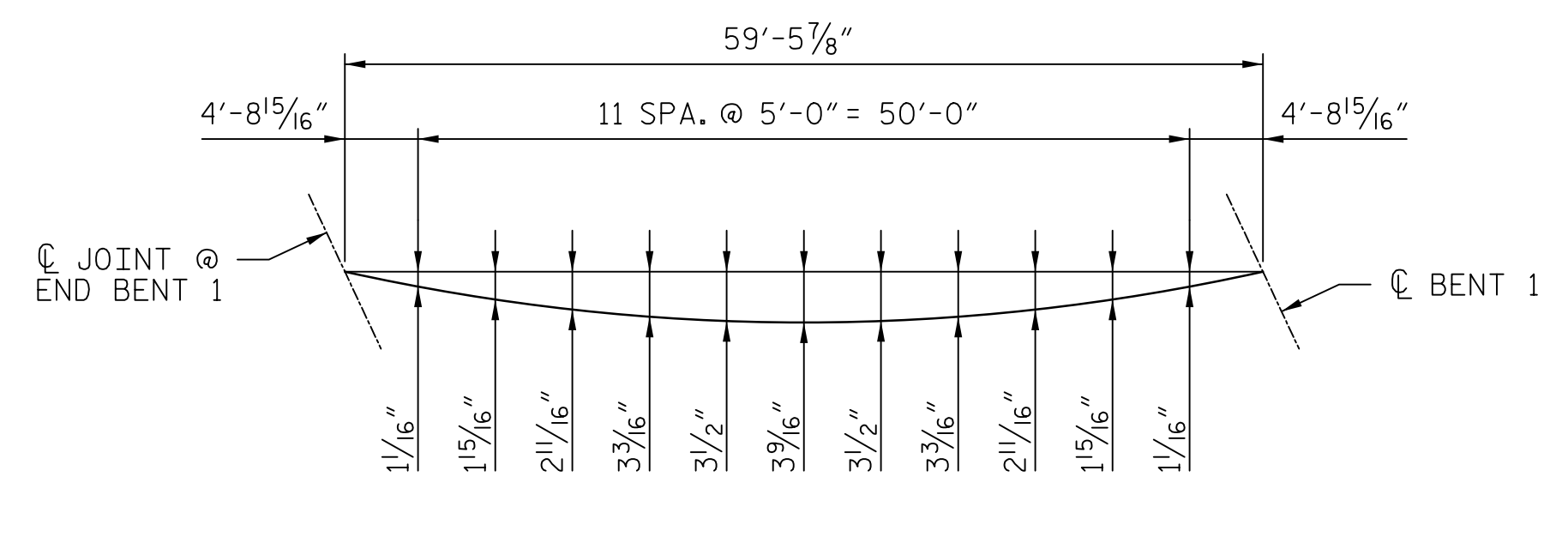
LEFT SIDE - EDGE OF SLAB



LEFT SIDE - EDGE OF SLAB

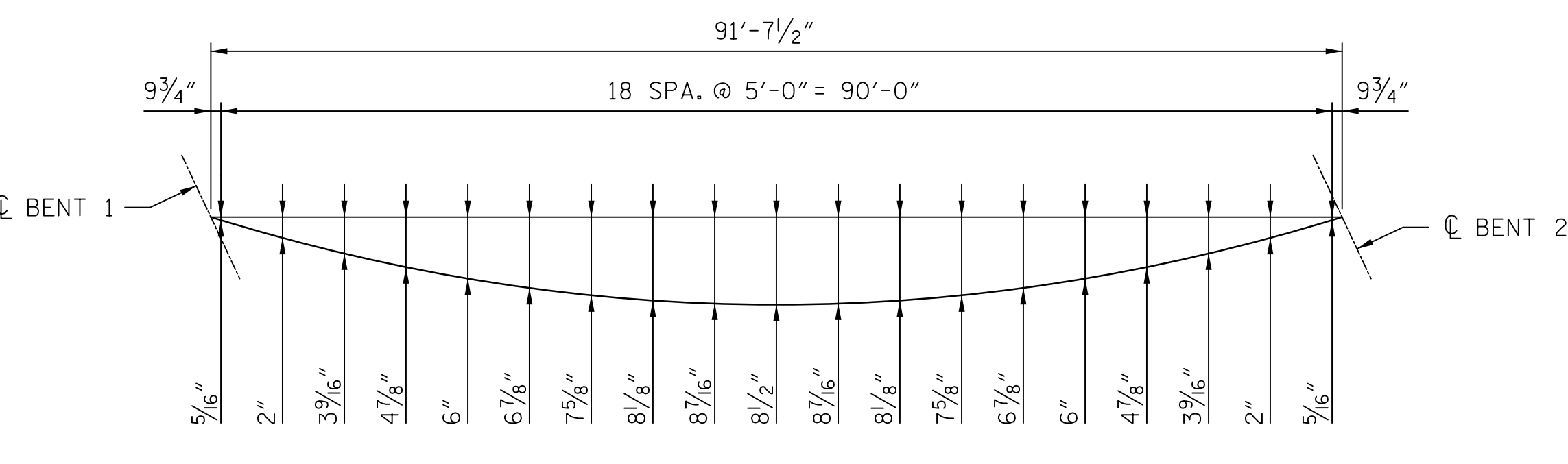


LEFT SIDE - EDGE OF SLAB



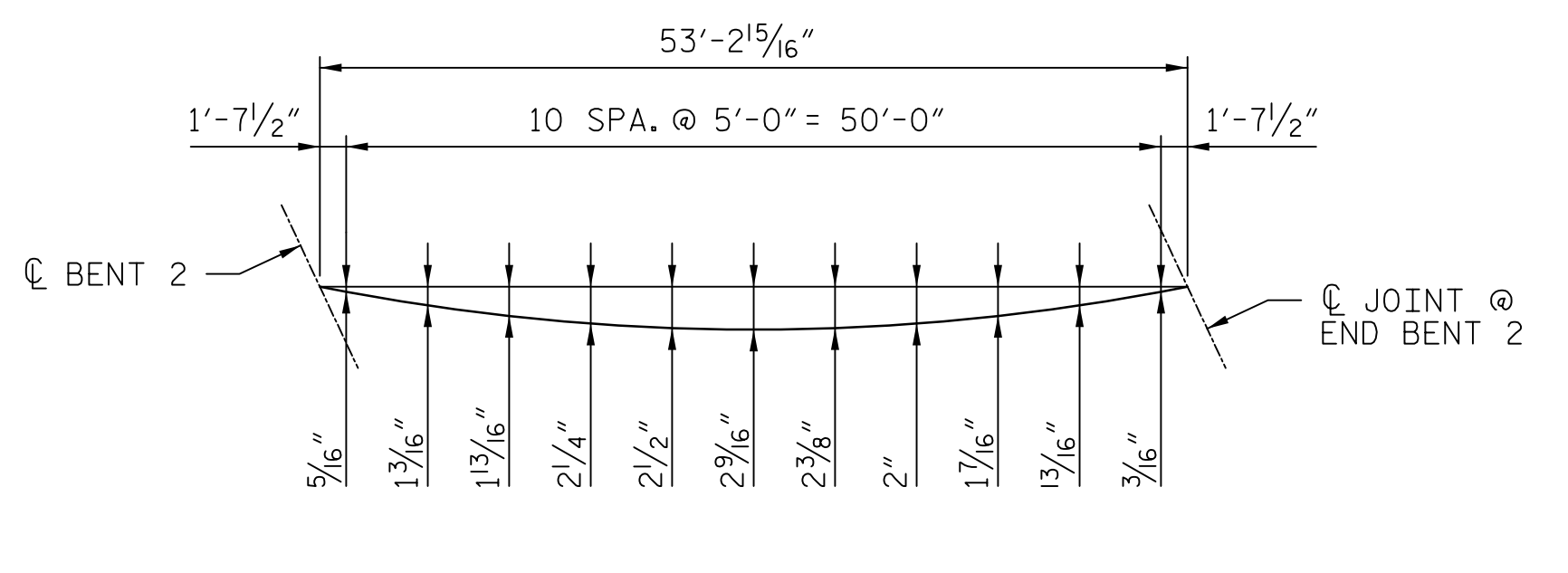
RIGHT SIDE - EDGE OF SLAB

ARC OFFSETS - SPAN A



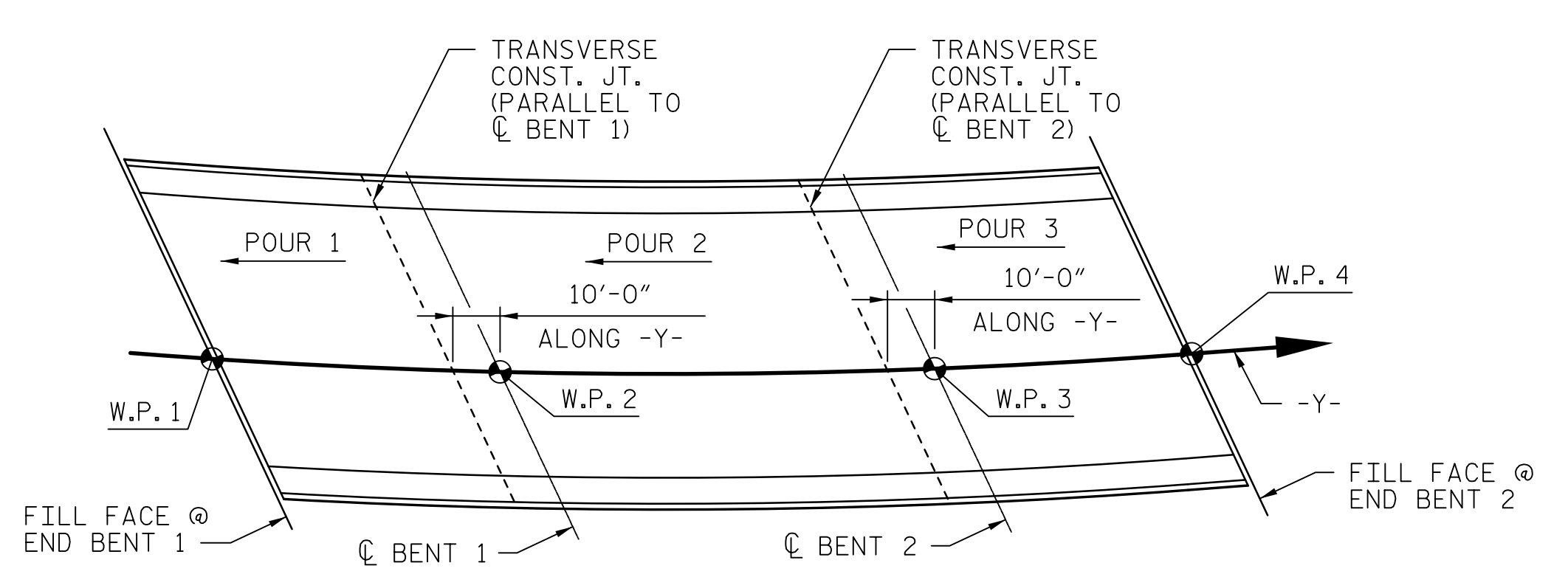
RIGHT SIDE - EDGE OF SLAB

ARC OFFSETS - SPAN B

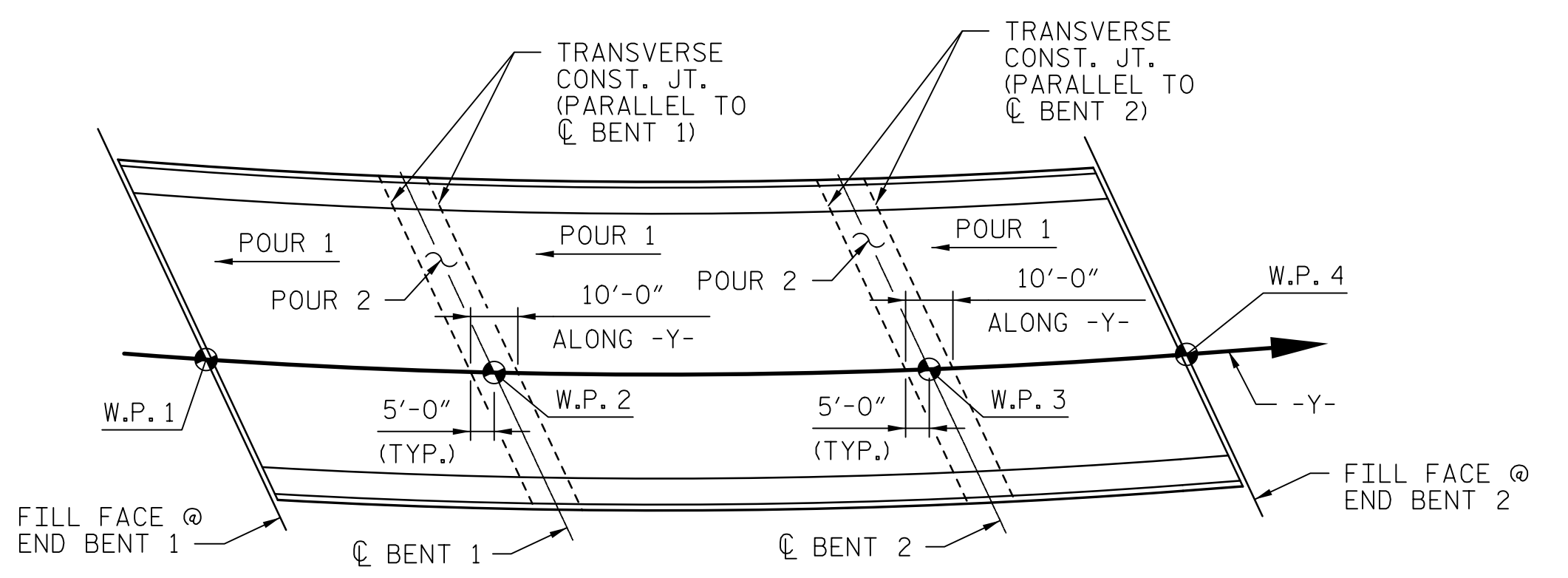


RIGHT SIDE - EDGE OF SLAB

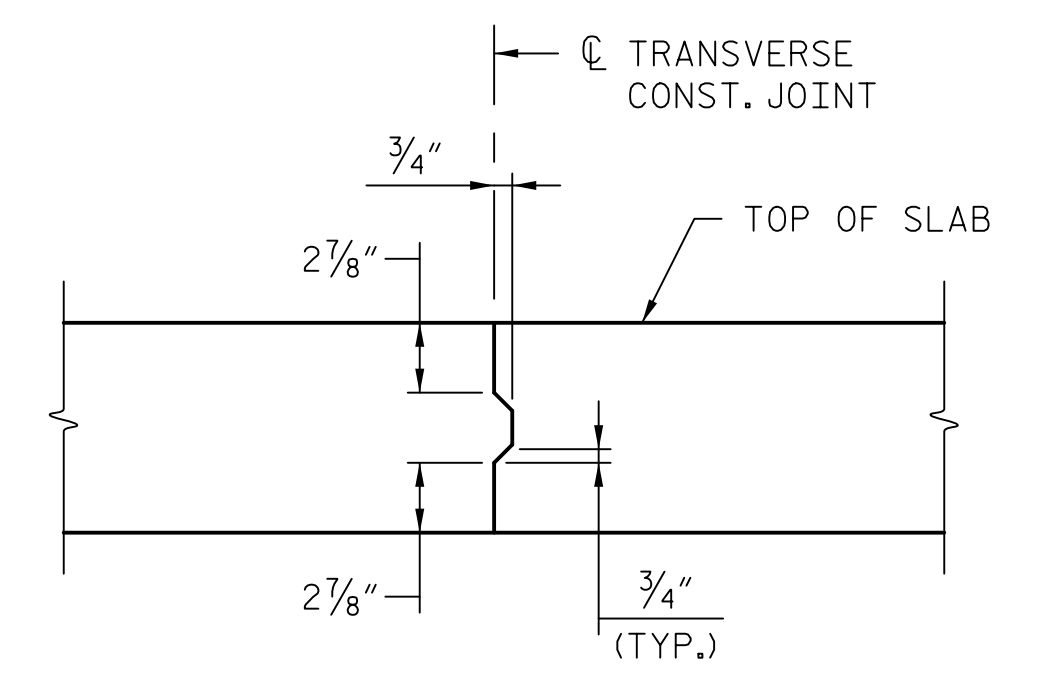
ARC OFFSETS - SPAN C



POURING DIAGRAM

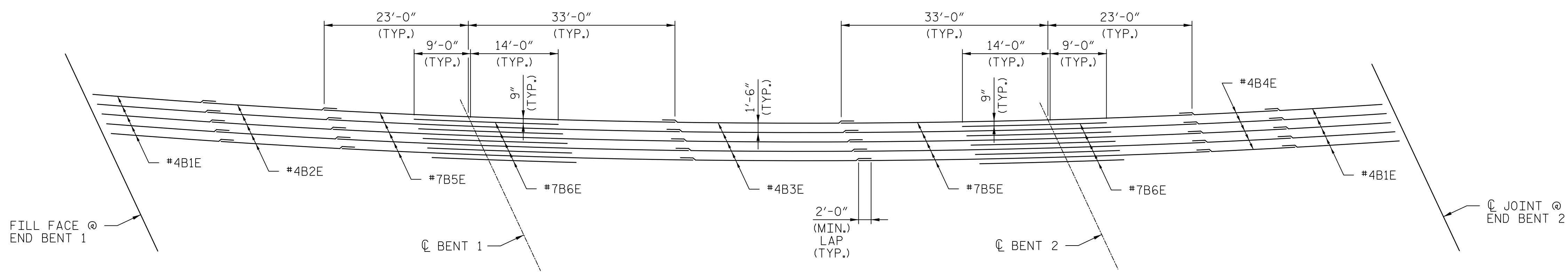


OPTIONAL POURING DIAGRAM



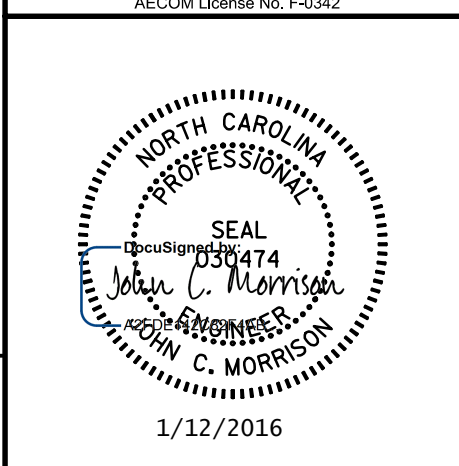
TRANSVERSE CONSTRUCTION JOINT DETAIL

REINFORCING STEEL IN SLAB NOT SHOWN.
LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT.



TOP "B" BAR REINFORCING LAYOUT

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 3 OF 3 BRIDGE NO. 109



| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|--------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUPERSTRUCTURE PLAN OF SPAN DETAILS | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. S-52 | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

1/12/2016

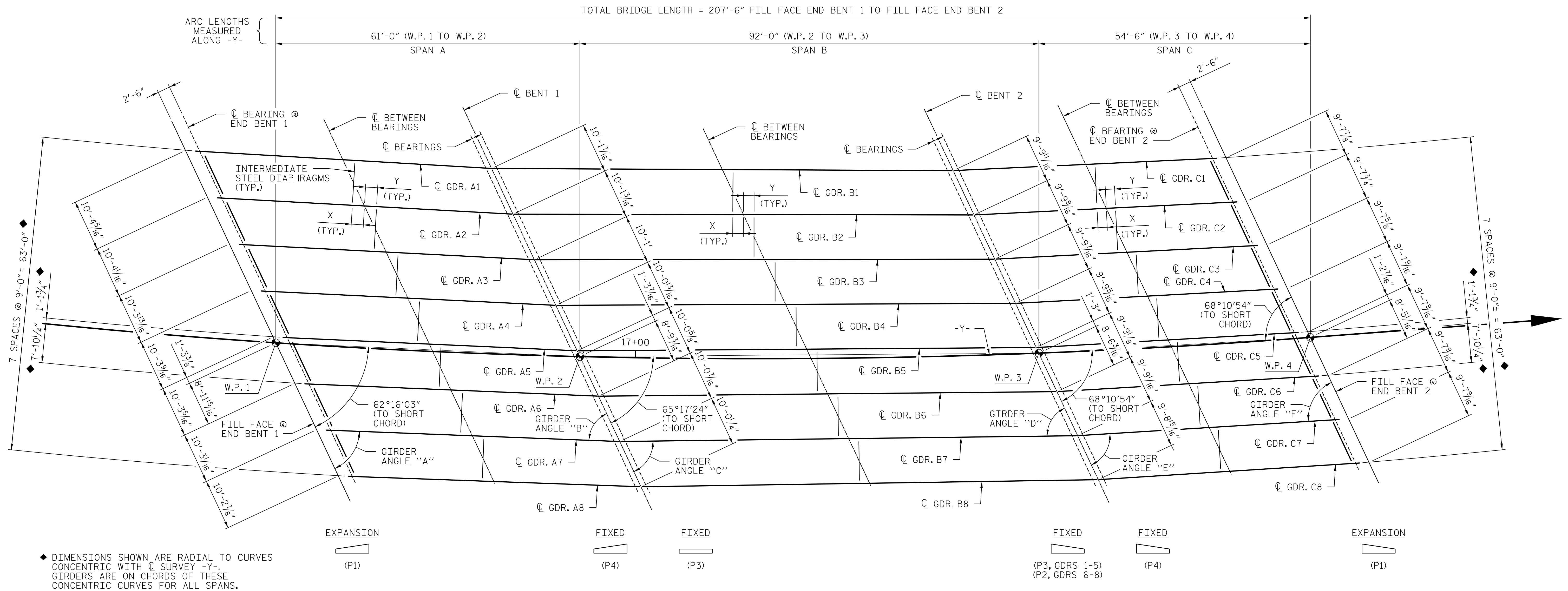
DATE: 1/12/2016
TIME: 8:45:53 AM

USER: jmc@ecorp.com
DGN: R:\Bridges\09\04\02\09_U2707_SML_552_P303.dgn

DRAWN BY: DBL DATE: #/2015
CHECKED BY: J&M DATE: #/2015

NOTES:

- FOR BEARING AND SOLE PLATE SIZES, SEE "ELASTOMERIC BEARING DETAILS" SHEET.



FRAMING PLAN

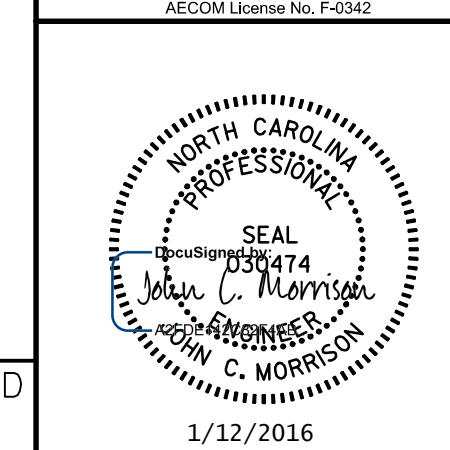
| | SPAN A | | SPAN B | | SPAN C | |
|----|--------------------------------------|--------------------------------------|-------------------------------------|-------------------------------------|--------------------------------------|--------------------------------------|
| | X | Y | X | Y | X | Y |
| G1 | - | 2'-5 ⁵ / ₁₆ " | - | 2'-1 ⁵ / ₈ " | - | 1'-10 ⁵ / ₁₆ " |
| G2 | 2'-5 ⁵ / ₁₆ " | 2'-5 ¹ / ₁₆ " | 2'-1 ⁵ / ₈ " | 2'-1 ¹ / ₂ " | 1'-10 ⁵ / ₁₆ " | 1'-10 ¹ / ₈ " |
| G3 | 2'-5 ¹ / ₁₆ " | 2'-4 ³ / ₁₆ " | 2'-1 ⁷ / ₁₆ " | 2'-1 ¹ / ₄ " | 1'-10 ¹ / ₈ " | 1'-10" |
| G4 | 2'-4 ³ / ₁₆ " | 2'-4 ⁹ / ₁₆ " | 2'-1 ¹ / ₄ " | 2'-1 ¹ / ₁₆ " | 1'-10" | 1'-9 ³ / ₁₆ " |
| G5 | 2'-4 ⁵ / ₈ " | 2'-4 ³ / ₈ " | 2'-1 ¹ / ₁₆ " | 2'-0 ⁷ / ₈ " | 1'-9 ³ / ₁₆ " | 1'-9 ¹ / ₁₆ " |
| G6 | 2'-4 ³ / ₈ " | 2'-4 ³ / ₁₆ " | 2'-0 ⁷ / ₈ " | 2'-0 ¹ / ₁₆ " | 1'-9 ¹ / ₁₆ " | 1'-9 ¹ / ₂ " |
| G7 | 2'-4 ¹ / ₈ " | 2'-3 ¹⁵ / ₁₆ " | 2'-0 ¹ / ₁₆ " | 2'-0 ¹ / ₂ " | 1'-9 ¹ / ₂ " | 1'-9 ³ / ₈ " |
| G8 | 2'-3 ¹⁵ / ₁₆ " | - | 2'-0 ¹ / ₂ " | - | 1'-9 ³ / ₈ " | - |

NOTE: DIMENSION X AND Y ARE MEASURED FROM CL GIRDERS.

| GIRDER | ANGLE "A" | ANGLE "B" | ANGLE "C" | ANGLE "D" | ANGLE "E" | ANGLE "F" |
|--------|-----------|-----------|-----------|-----------|-----------|-----------|
| 1 | 61°29'44" | 61°29'47" | 64°35'43" | 64°35'42" | 67°33'22" | 67°33'19" |
| 2 | 61°41'32" | 61°41'36" | 64°46'03" | 64°46'03" | 67°42'21" | 67°42'19" |
| 3 | 61°53'11" | 61°53'15" | 64°56'15" | 64°56'14" | 67°51'13" | 67°51'11" |
| 4 | 62°04'40" | 62°04'43" | 65°06'18" | 65°06'18" | 67°59'58" | 67°59'56" |
| 5 | 62°15'58" | 62°16'02" | 65°16'13" | 65°16'13" | 68°08'35" | 68°08'32" |
| 6 | 62°27'08" | 62°27'11" | 65°26'00" | 65°25'59" | 68°16'38" | 68°16'36" |
| 7 | 62°38'08" | 62°38'11" | 65°35'39" | 65°35'38" | 68°24'05" | 68°24'03" |
| 8 | 62°48'58" | 62°49'02" | 65°45'10" | 65°45'09" | 68°30'55" | 68°30'53" |

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-

BRIDGE NO. 109



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

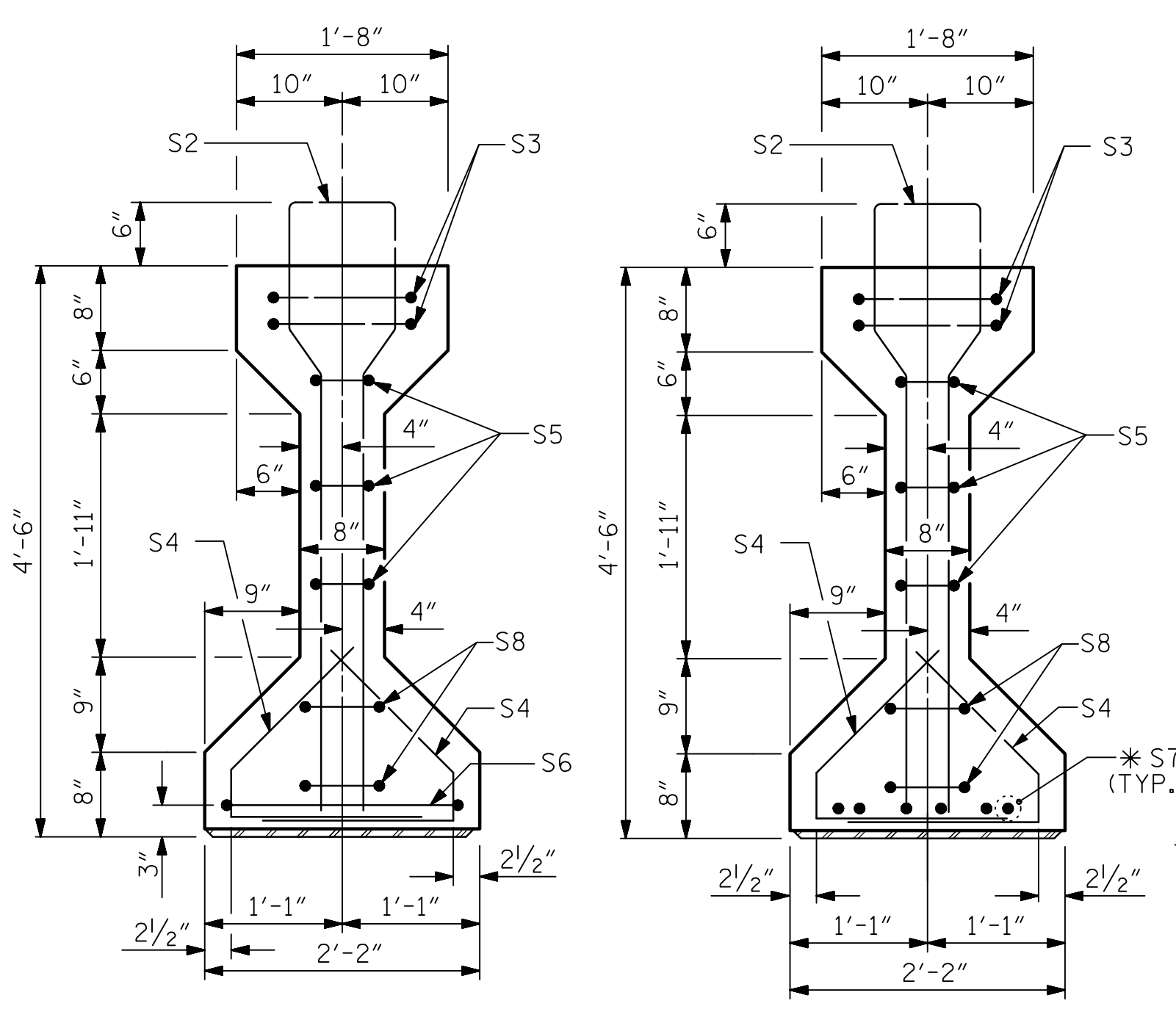
**SUPERSTRUCTURE
FRAMING PLAN**

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|----------------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-53 TOTAL SHEETS 86 |
| 1 | | | 3 | | | |
| 2 | | | 4 | | | |

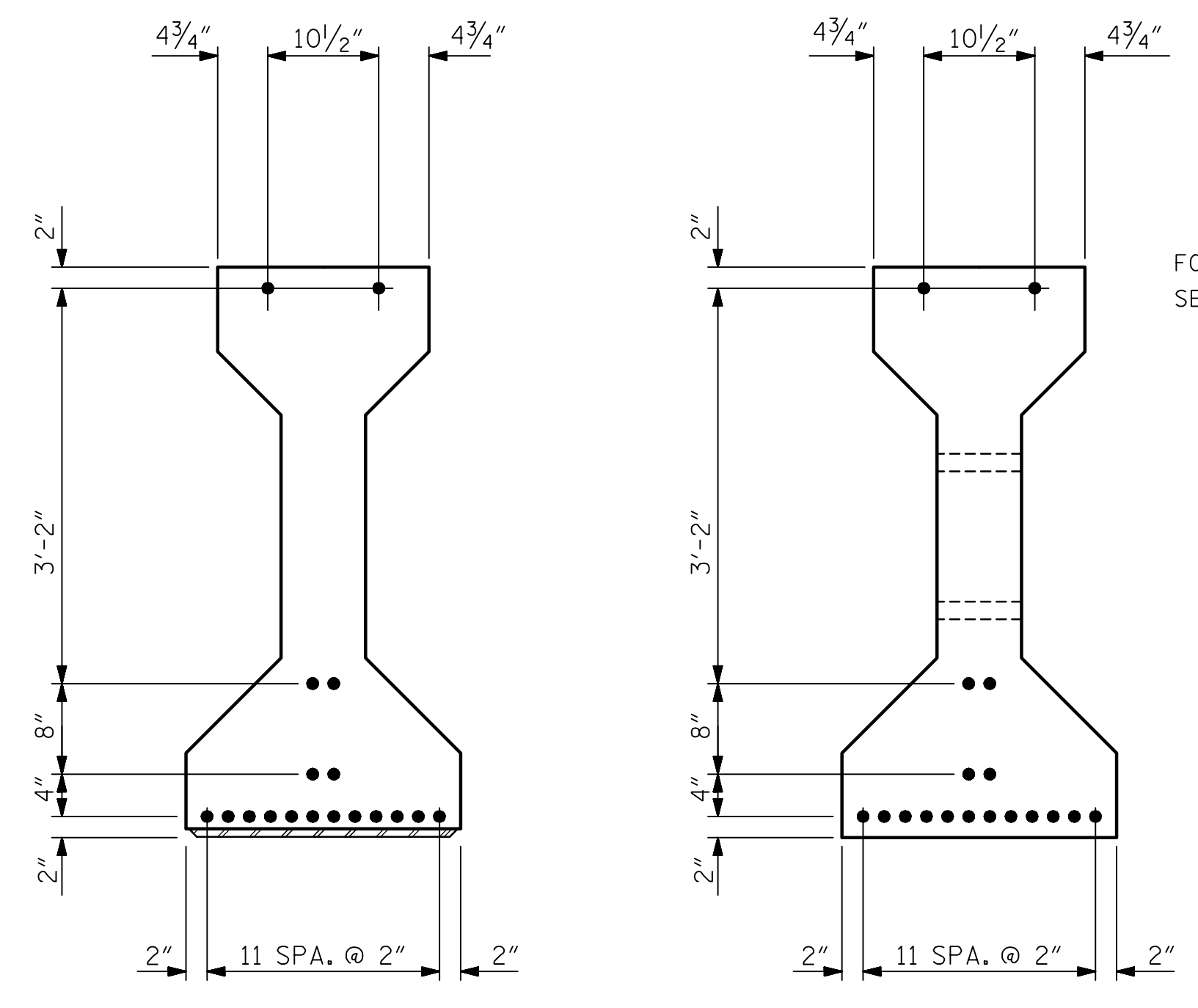
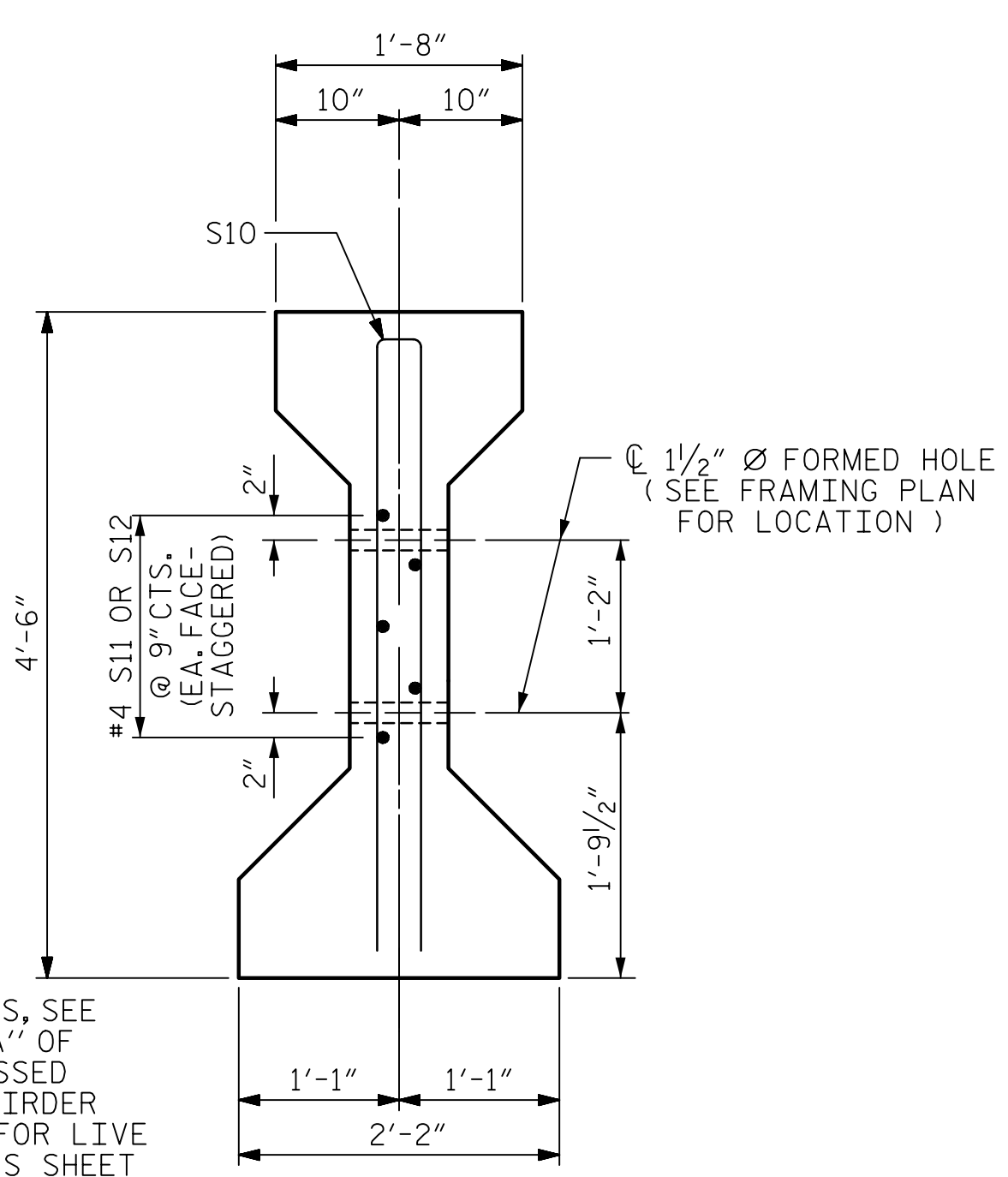
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016

DRAWN BY: DDL DATE: 4/2015
 CHECKED BY: JCM DATE: 4/2015



* FOR S7 BARS, SEE DETAIL "A" OF PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS SHEET



FOR NOTES, SEE SHEET 4 OF 5.

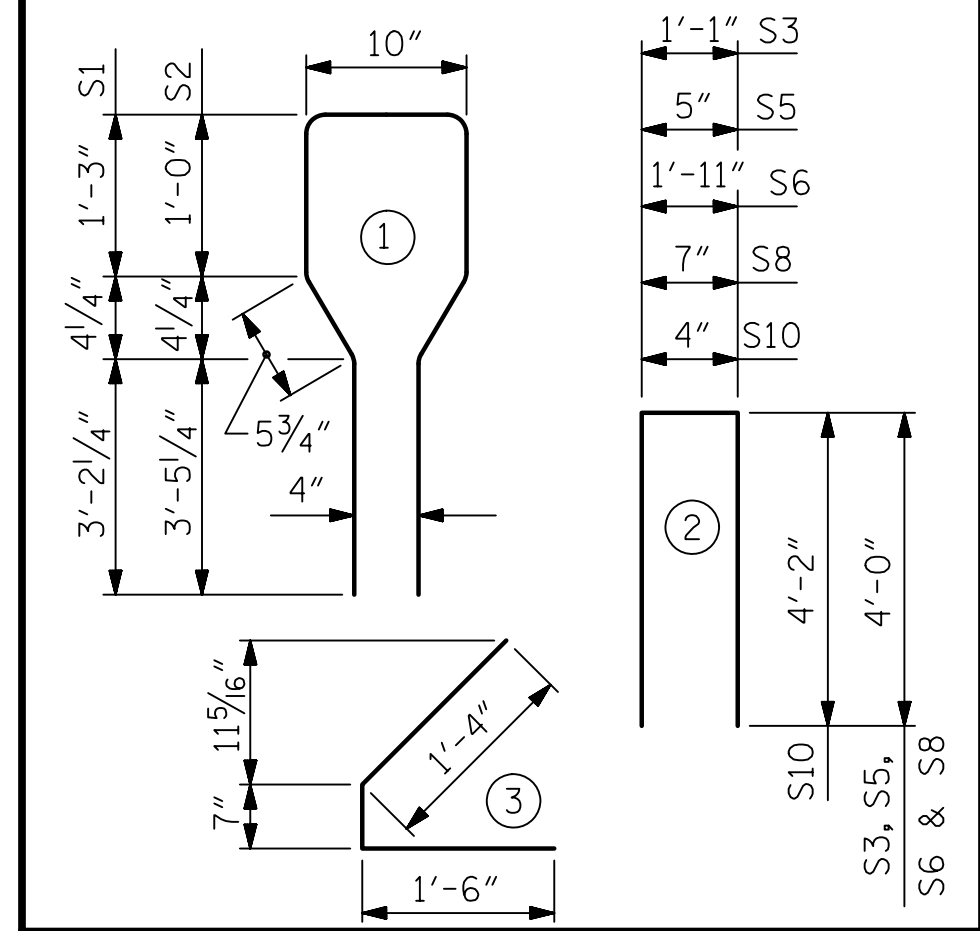
| 0.6" Ø L. R. GRADE 270 STRANDS | | |
|--------------------------------|-------------------------------------|-------------------------------------|
| AREA (SQUARE INCHES) | ULTIMATE STRENGTH (LBS. PER STRAND) | APPLIED PRESTRESS (LBS. PER STRAND) |
| 0.217 | 58,600 | 43,950 |

| REINFORCING STEEL FOR ONE GIRDER | | | | | |
|----------------------------------|--------|------|------|--------|--------|
| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT |
| S1 | 49 | #4 | 1 | 10'-8" | 349 |
| S2 | 12 | #6 | 1 | 10'-8" | 192 |
| S3 | 4 | #4 | 2 | 9'-1" | 24 |
| S4 | 64 | #4 | 3 | 3'-5" | 146 |
| S5 | 6 | #4 | 2 | 8'-5" | 34 |
| S6 | 1 | #4 | 2 | 9'-11" | 7 |
| * S7 | 6 | #5 | STR | 3'-8" | 23 |
| S8 | 4 | #4 | 2 | 8'-7" | 23 |
| S9 | 1 | #3 | STR | 1'-10" | 1 |
| S10 | 2 | #5 | 2 | 8'-8" | 18 |
| S10 | 4 | #5 | 2 | 8'-8" | 36 |
| S11 | 5 | #4 | STR | 7'-0" | 23 |
| S12 | 5 | #4 | STR | 12'-0" | 40 |

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

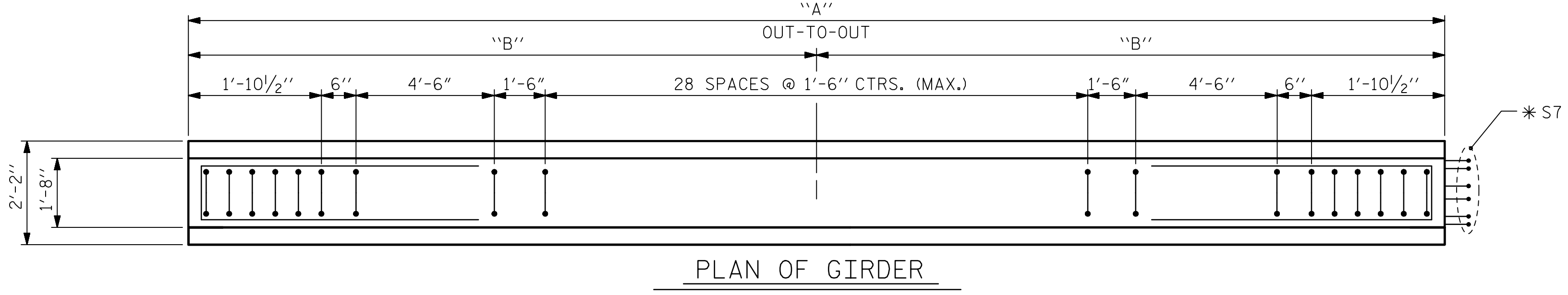
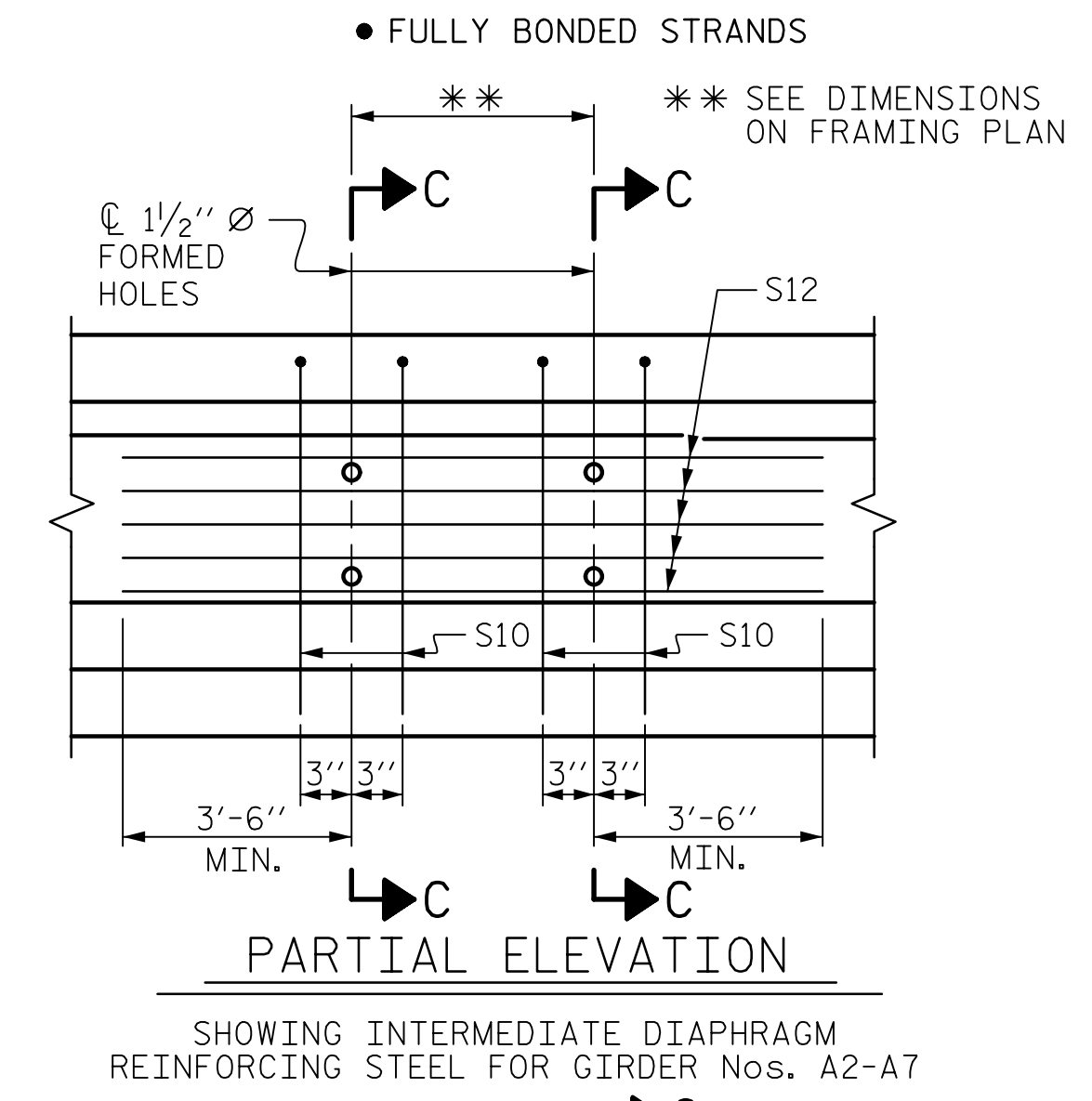
BAR TYPES

ALL BAR DIMENSIONS ARE OUT-TO-OUT

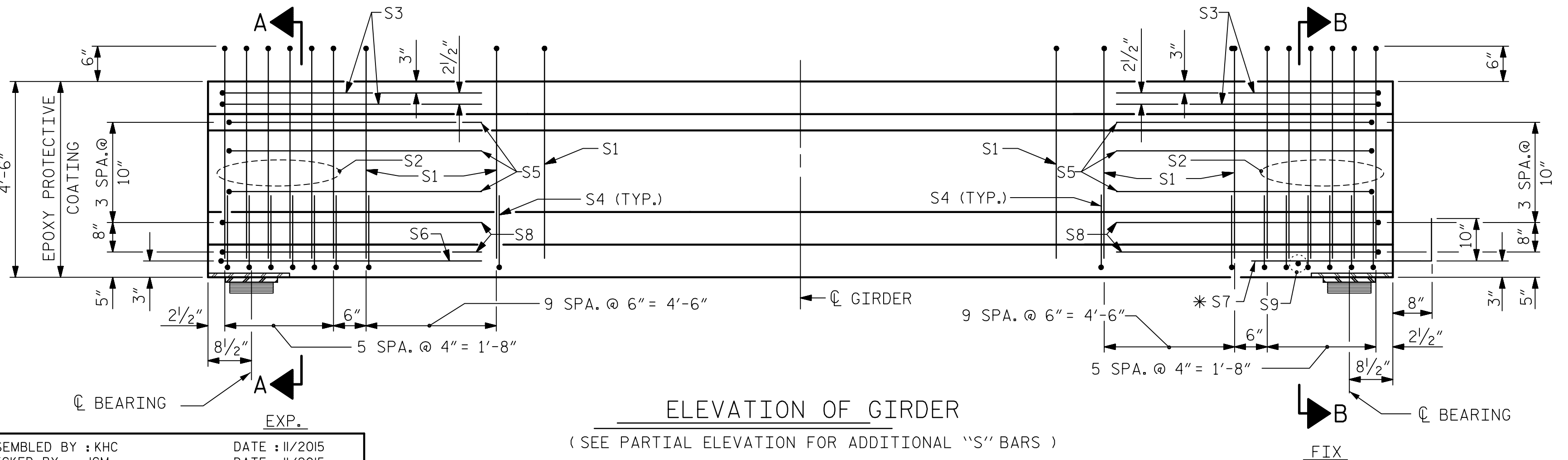


| GDR. | "A" | "B" |
|------|-------------|------------|
| A1 | 58'-10 1/2" | 29'-5 1/4" |
| A2 | 58'-9 1/4" | 29'-4 5/8" |
| A3 | 58'-8" | 29'-4" |
| A4 | 58'-6 3/4" | 29'-3 3/8" |
| A5 | 58'-5 1/2" | 29'-2 3/4" |
| A6 | 58'-4 1/2" | 29'-2 1/4" |
| A7 | 58'-3 1/4" | 29'-1 5/8" |
| A8 | 58'-2" | 29'-1" |

0.6" Ø LOW RELAXATION STRAND LAYOUT



PLAN OF GIRDER



ELEVATION OF GIRDER

(SEE PARTIAL ELEVATION FOR ADDITIONAL "S" BARS)

| | |
|-----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : ELR 8/91 | REV. 5/1/06R TLA/GM |
| CHECKED BY : GRP 8/91 | REV. 10/1/11 MAA/GM |
| | REV. 1/15 MAA/TMG |



PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 1 OF 5 BRIDGE NO. 109

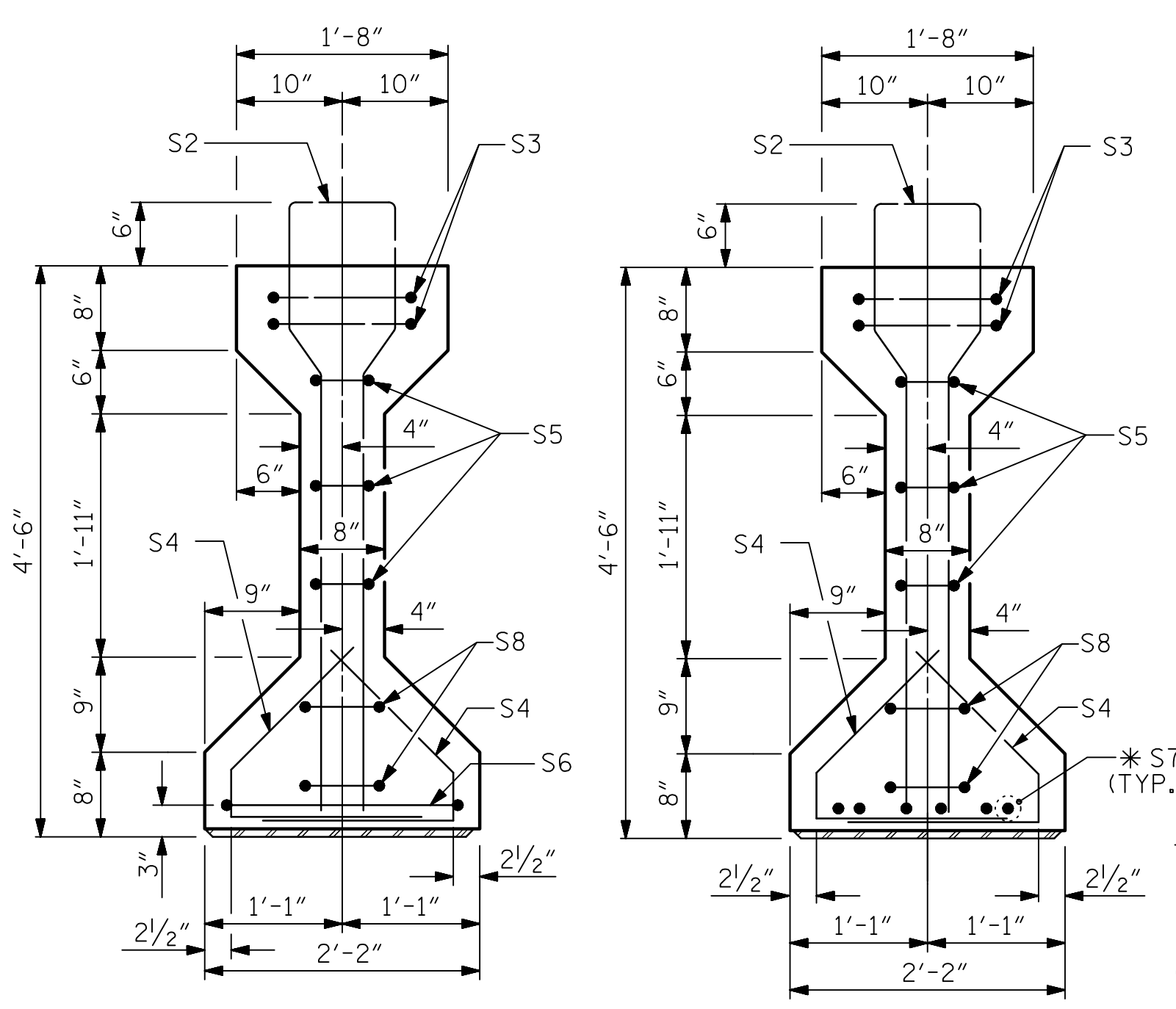
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 AASHTO TYPE IV
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 SPAN A

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

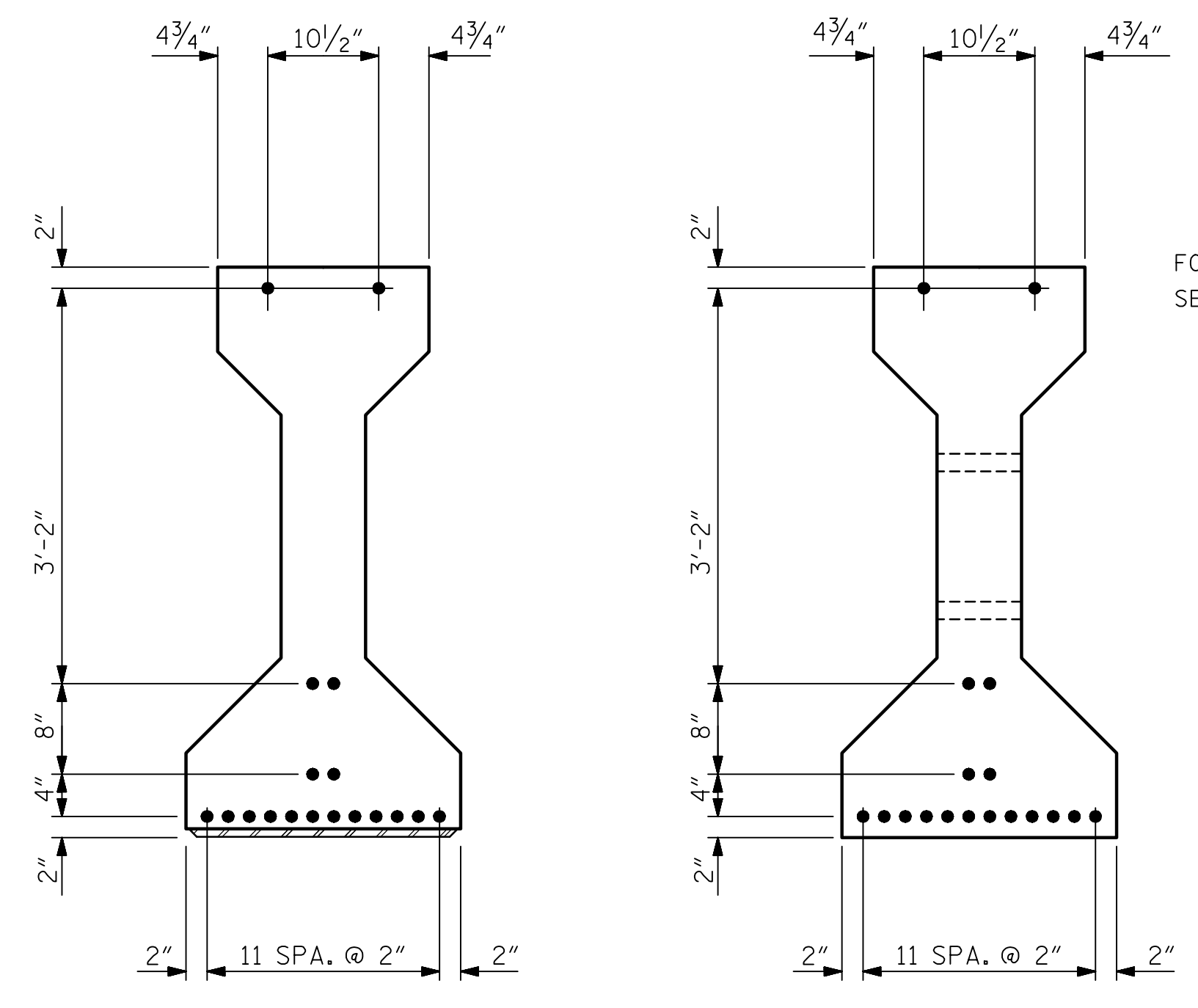
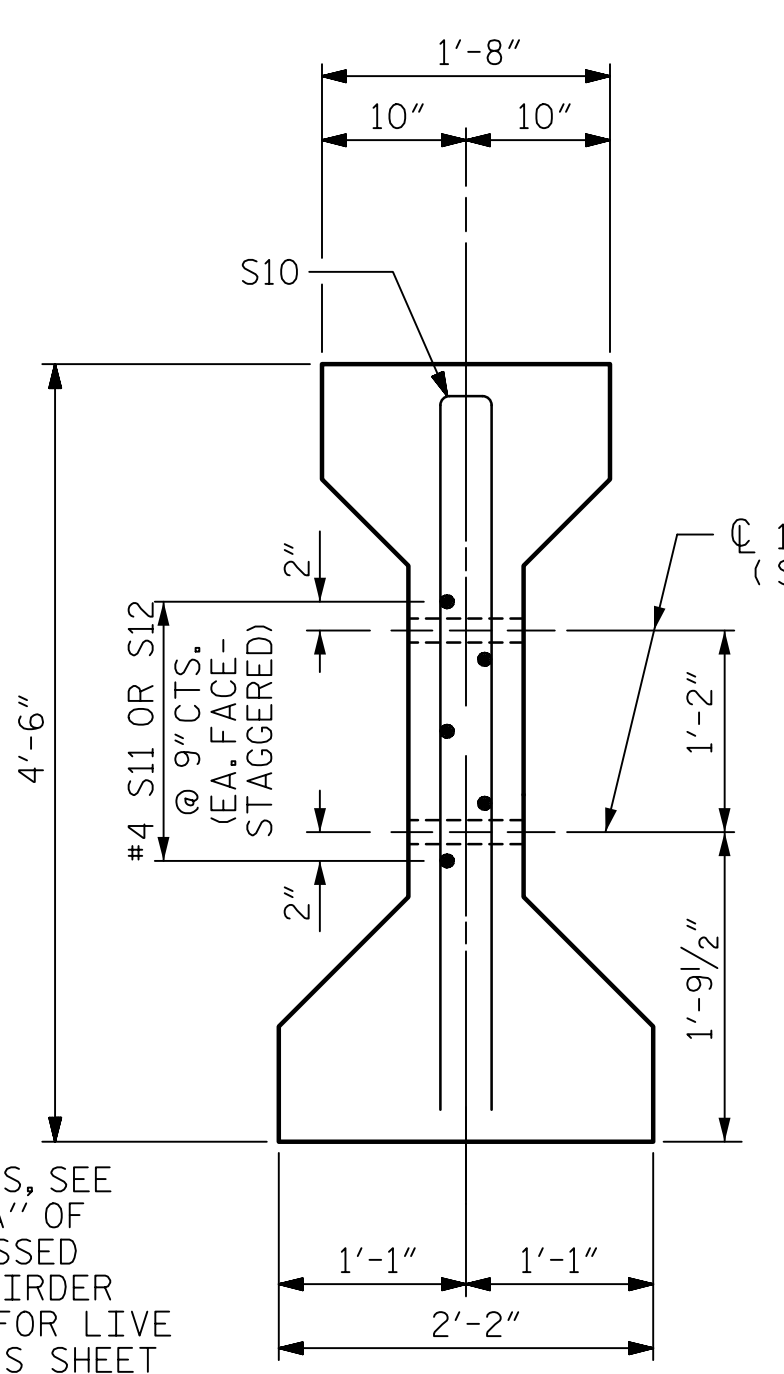
SHEET NO. **S-54**
 TOTAL SHEETS **86**

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016



* FOR S7 BARS, SEE DETAIL "A" OF PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS SHEET



FOR NOTES, SEE SHEET 4 OF 5.

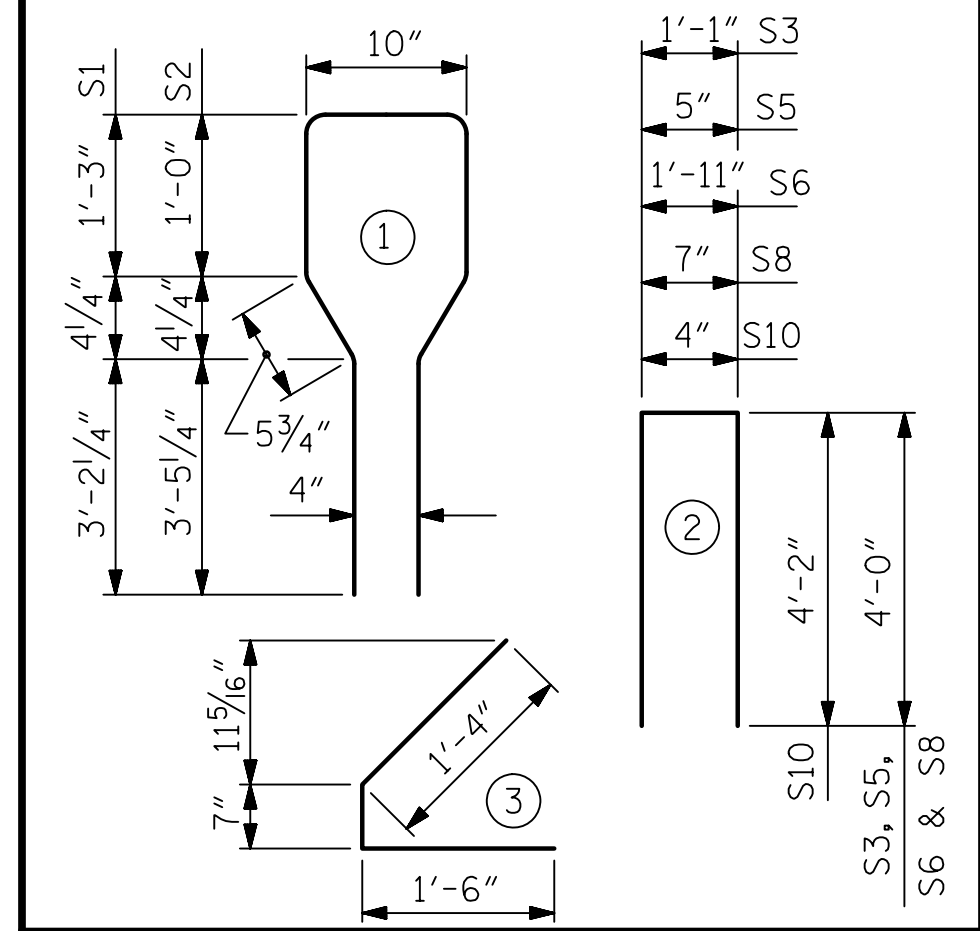
| 0.6" Ø L. R. GRADE 270 STRANDS | | |
|--------------------------------|-------------------------------------|-------------------------------------|
| AREA (SQ. INCHES) | ULTIMATE STRENGTH (LBS. PER STRAND) | APPLIED PRESTRESS (LBS. PER STRAND) |
| 0.217 | 58,600 | 43,950 |

| REINFORCING STEEL FOR ONE GIRDER | | | | | |
|----------------------------------|--------|------|------|--------|--------|
| BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT |
| S1 | 45 | #4 | 1 | 10'-8" | 321 |
| S2 | 12 | #6 | 1 | 10'-8" | 192 |
| S3 | 4 | #4 | 2 | 9'-1" | 24 |
| S4 | 64 | #4 | 3 | 3'-5" | 146 |
| S5 | 6 | #4 | 2 | 8'-5" | 34 |
| S6 | 1 | #4 | 2 | 9'-11" | 7 |
| * S7 | 6 | #5 | STR | 3'-8" | 23 |
| S8 | 4 | #4 | 2 | 8'-7" | 23 |
| S9 | 1 | #3 | STR | 1'-10" | 1 |
| S10 | 2 | #5 | 2 | 8'-8" | 18 |
| S10 | 4 | #5 | 2 | 8'-8" | 36 |
| S11 | 5 | #4 | STR | 7'-0" | 23 |
| S12 | 5 | #4 | STR | 12'-0" | 40 |

* NOTE: S7 BARS SHALL BE BENT BEFORE SHIPMENT. HEAT BENDING SHALL NOT BE ALLOWED.

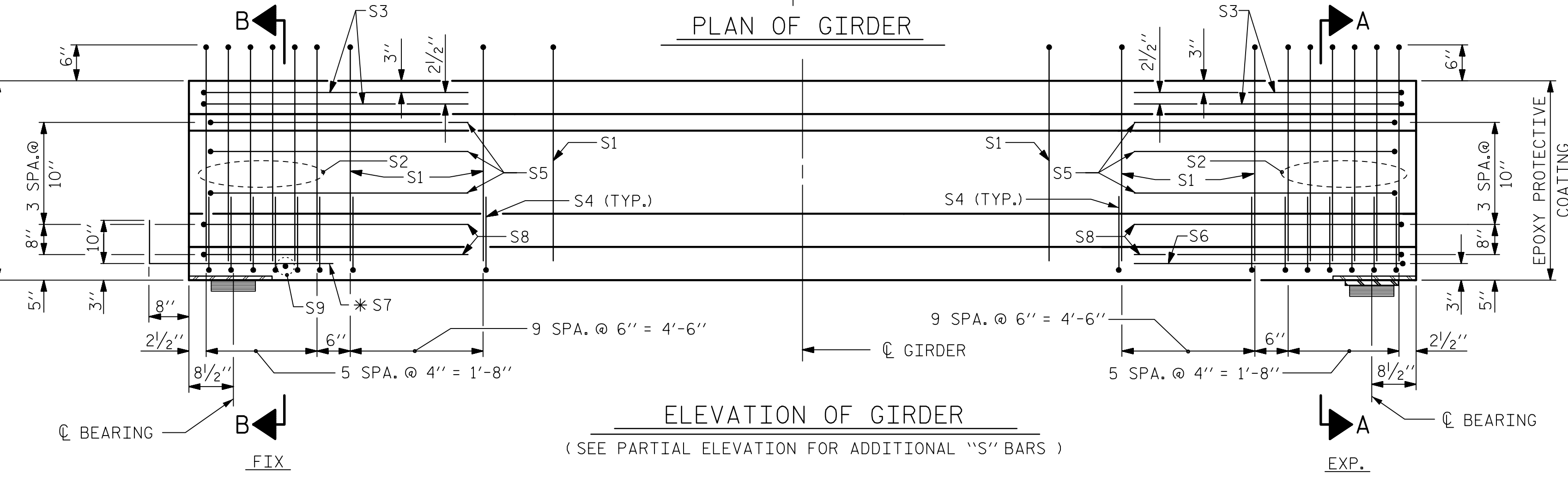
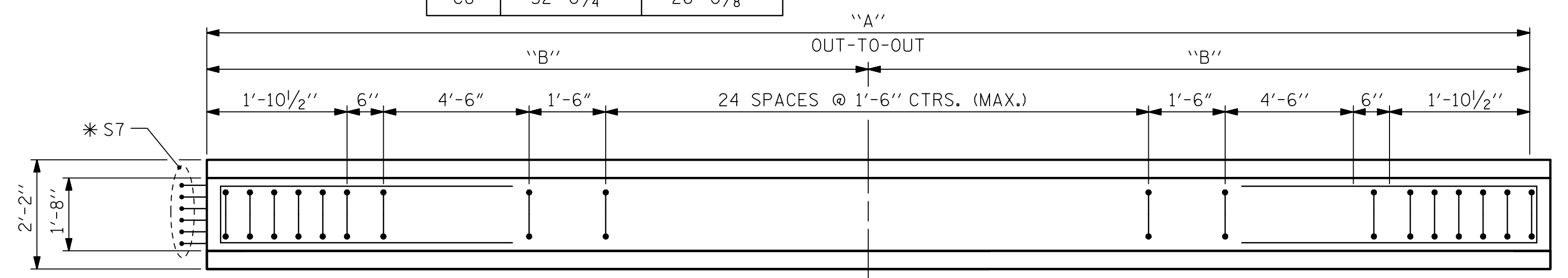
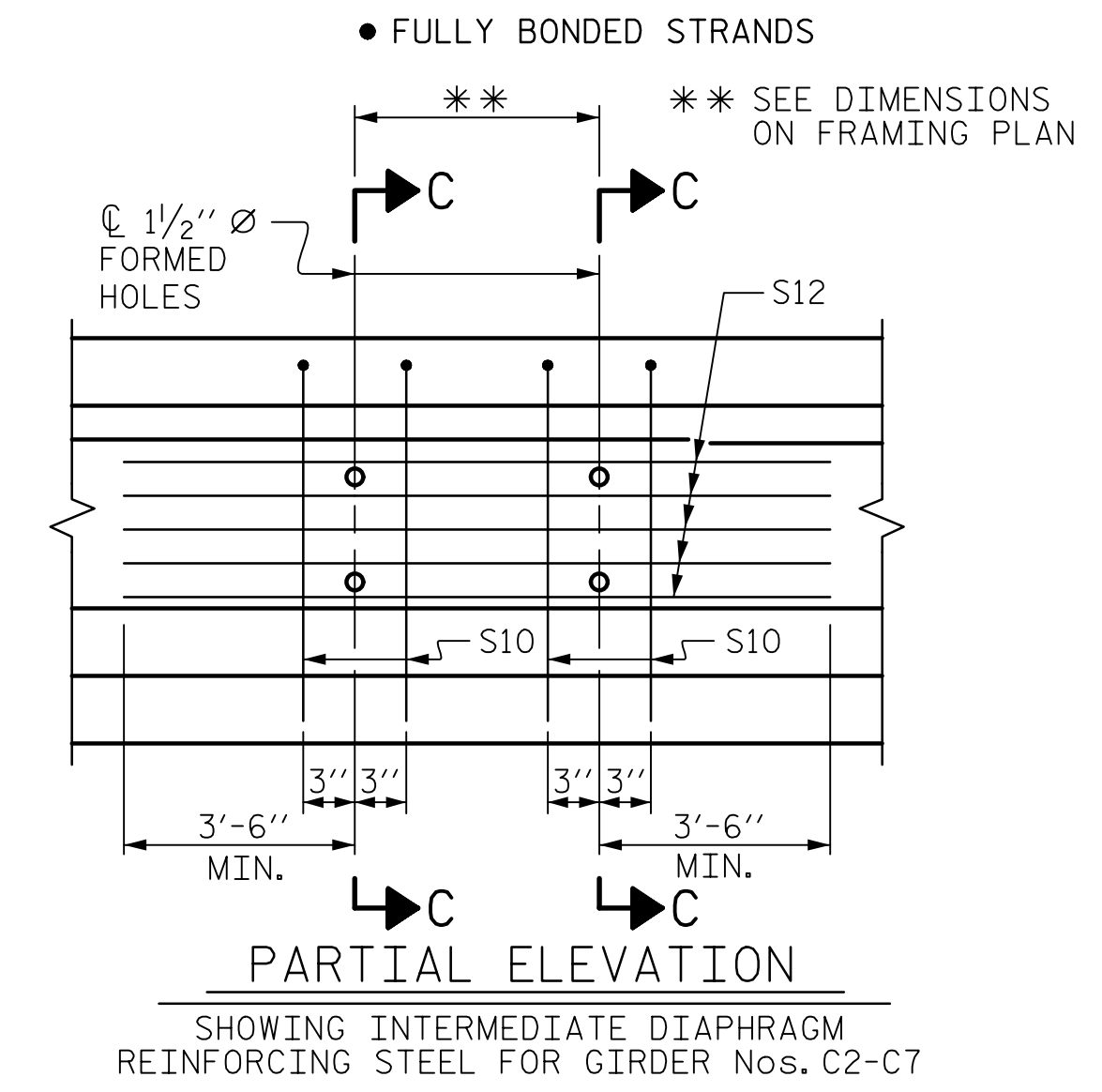
BAR TYPES

ALL BAR DIMENSIONS ARE OUT-TO-OUT



| GDR. | "A" | "B" |
|------|------------|------------|
| C1 | 52'-4" | 26'-2" |
| C2 | 52'-3 1/4" | 26'-1 5/8" |
| C3 | 52'-2 1/2" | 26'-1 1/4" |
| C4 | 52'-2" | 26'-1" |
| C5 | 52'-1 1/4" | 26'-0 5/8" |
| C6 | 52'-0 3/4" | 26'-0 3/8" |
| C7 | 52'-0 1/4" | 26'-0 1/8" |
| C8 | 52'-0 1/4" | 26'-0 1/8" |

0.6" Ø LOW RELAXATION STRAND LAYOUT



| QUANTITIES FOR ONE GIRDER | | | |
|---------------------------|-------------------------|--------------------------|----------------------------|
| | REINFORCING STEEL (LB.) | 6000 PSI CONCRETE (C.Y.) | 0.6" Ø L. R. STRANDS (No.) |
| EXTERIOR GIRDER | 812 | 10.6 (AVG.) | 18 |
| INTERIOR GIRDER | 847 | 10.6 (AVG.) | 18 |

| GIRDERS REQUIRED | | |
|------------------|-----------|--------------|
| NUMBER | LENGTH | TOTAL LENGTH |
| 8 | SEE TABLE | 417'-2 1/4" |

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 3 OF 5 BRIDGE NO. 109



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
AASHTO TYPE IV
 PRESTRESSED CONCRETE GIRDER
 CONTINUOUS FOR LIVE LOAD
 SPAN C

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. **S-56**
 TOTAL SHEETS **86**

ASSEMBLED BY : KHC DATE : 11/2015
 CHECKED BY : JCM DATE : 11/2015
 DRAWN BY : ELR 8/91 REV. 5/1/06R TLA/GM
 CHECKED BY : GRP 8/91 REV. 10/1/11 MAA/GM
 REV. 1/15 MAA/TMG

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

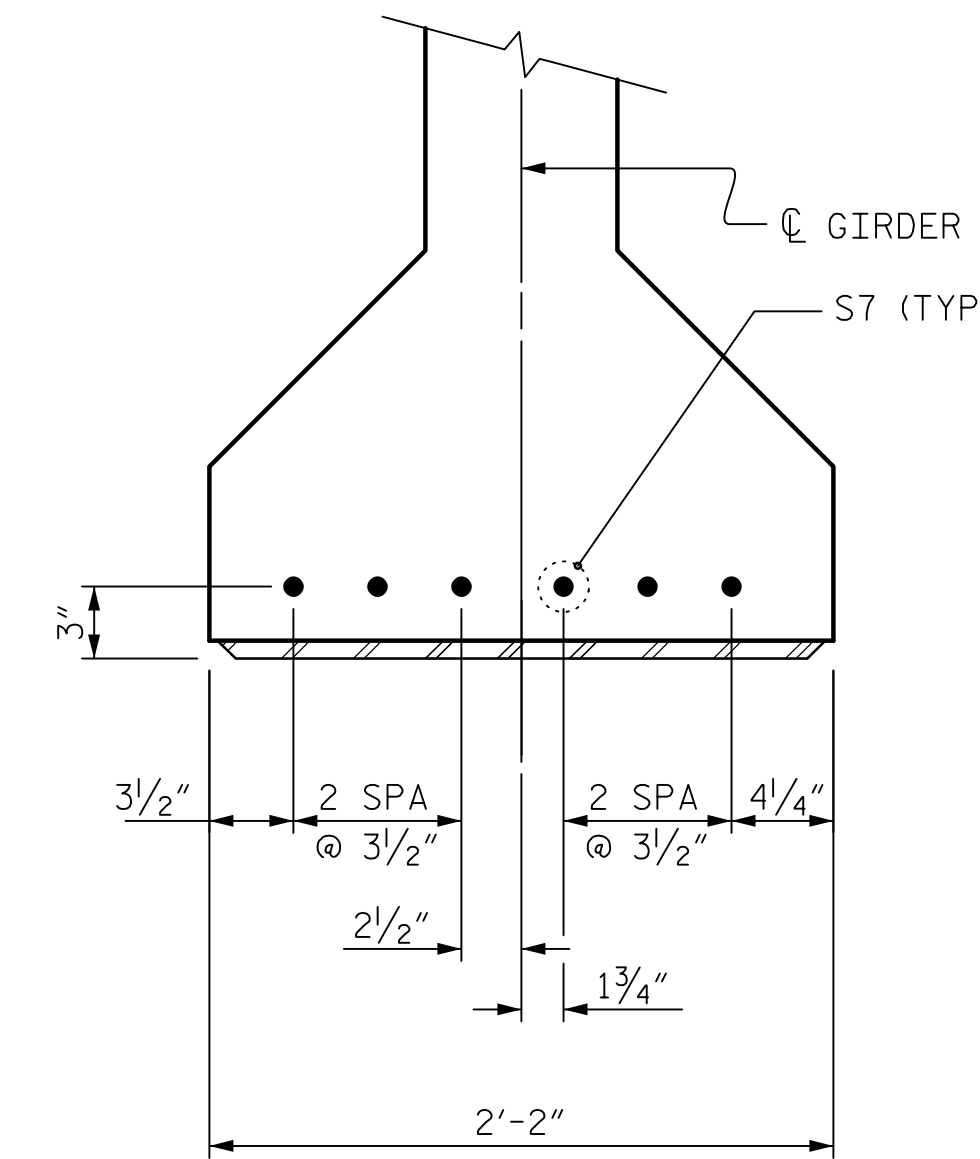
ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4500 PSI FOR SPAN A & C, AND 5500 PSI FOR SPAN B.

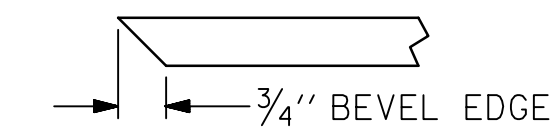
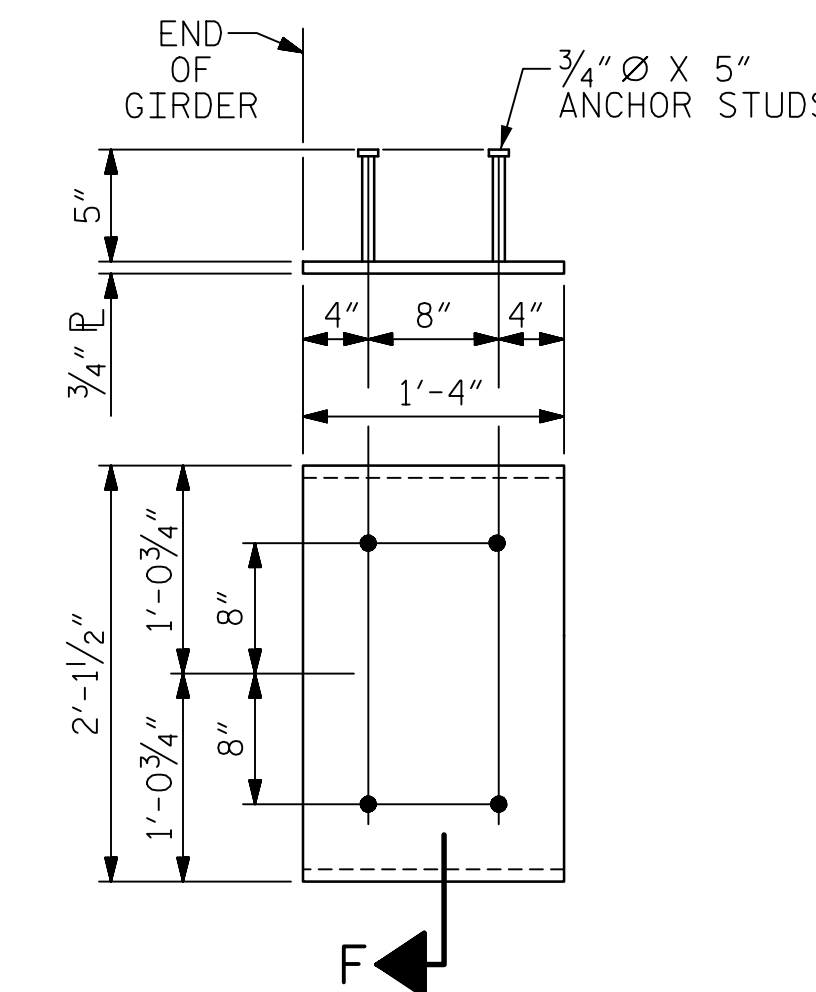
DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".



DETAIL "A"

(FOR AASHTO TYPE IV GIRDERS)



SECTION "F"

(SEE NOTES)

EMBEDDED PLATE "B-1" DETAILS FOR AASHTO TYPE IV GIRDER

(2 REQ'D PER GIRDER)

| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| SPAN A | | | | | | | | | | | |
| GIRDERS 1, 3 - 6, & 8 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.018 | 0.034 | 0.047 | 0.055 | 0.058 | 0.055 | 0.047 | 0.034 | 0.018 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.006 | 0.013 | 0.018 | 0.021 | 0.022 | 0.021 | 0.018 | 0.013 | 0.006 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 1/8" | 1/4" | 3/8" | 7/16" | 7/16" | 7/16" | 3/8" | 1/4" | 1/8" | 0" |

| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| SPAN A | | | | | | | | | | | |
| GIRDERS 2 & 7 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.018 | 0.034 | 0.047 | 0.055 | 0.058 | 0.055 | 0.047 | 0.034 | 0.018 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.006 | 0.013 | 0.018 | 0.022 | 0.023 | 0.022 | 0.018 | 0.013 | 0.006 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 1/8" | 1/4" | 3/8" | 3/8" | 7/16" | 3/8" | 3/8" | 1/4" | 1/8" | 0" |

| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| SPAN B | | | | | | | | | | | |
| GIRDERS 1, 2, 7, & 8 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.055 | 0.104 | 0.143 | 0.167 | 0.175 | 0.167 | 0.143 | 0.104 | 0.055 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.037 | 0.074 | 0.102 | 0.121 | 0.127 | 0.121 | 0.102 | 0.074 | 0.037 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 3/8" | 3/8" | 1/2" | 9/16" | 9/16" | 9/16" | 1/2" | 3/8" | 3/16" | 0" |

| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| SPANS B | | | | | | | | | | | |
| GIRDERS 3 & 6 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.055 | 0.104 | 0.143 | 0.167 | 0.175 | 0.167 | 0.143 | 0.104 | 0.055 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.037 | 0.072 | 0.101 | 0.118 | 0.125 | 0.118 | 0.101 | 0.072 | 0.037 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 1/4" | 3/8" | 1/2" | 9/16" | 5/8" | 9/16" | 1/2" | 3/8" | 1/4" | 0" |

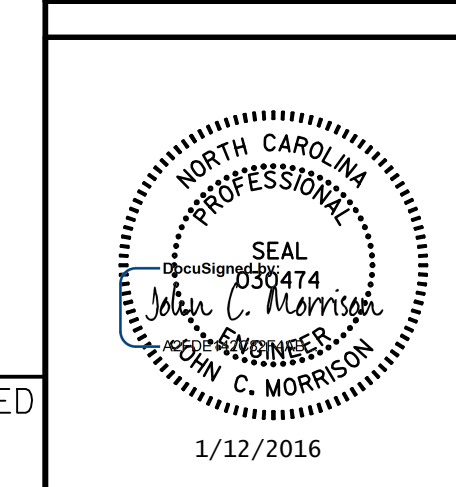
| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|--------|--------|--------|-------|-------|-------|-------|
| SPAN B | | | | | | | | | | | |
| GIRDERS 4 - 5 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.055 | 0.104 | 0.143 | 0.167 | 0.175 | 0.167 | 0.143 | 0.104 | 0.055 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.035 | 0.068 | 0.095 | 0.111 | 0.117 | 0.111 | 0.095 | 0.068 | 0.035 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 1/4" | 7/16" | 9/16" | 11/16" | 11/16" | 11/16" | 9/16" | 7/16" | 1/4" | 0" |

| DEAD LOAD DEFLECTION TABLE FOR GIRDERS | | | | | | | | | | | |
|----------------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| SPAN C | | | | | | | | | | | |
| GIRDERS 1 - 8 | | | | | | | | | | | |
| TENTH POINTS | BRG. | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | BRG. |
| CAMBER (GIRDER ALONE IN PLACE) ↑ | 0.000 | 0.016 | 0.030 | 0.041 | 0.048 | 0.050 | 0.048 | 0.041 | 0.030 | 0.016 | 0.000 |
| * DEFLECTION DUE TO SUPERIMPOSED DL ↓ | 0.000 | 0.004 | 0.008 | 0.011 | 0.013 | 0.014 | 0.013 | 0.011 | 0.008 | 0.004 | 0.000 |
| FINAL CAMBER (OR DEFLECTION) ↑ | 0" | 1/8" | 1/4" | 3/8" | 7/16" | 7/16" | 7/16" | 3/8" | 1/4" | 1/8" | 0" |

* INCLUDES FUTURE WEARING SURFACE
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-

SHEET 4 OF 5 BRIDGE NO. 109



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
PRESTRESSED CONCRETE GIRDER
CONTINUOUS FOR LIVE LOAD DETAILS
AND DEAD LOAD DEFLECTION TABLES

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-57 |
| 1 | | | 3 | | | TOTAL SHEETS |
| 2 | | | 4 | | | 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016

DATE: 1/12/2016
TIME: 8:55:00 AM

USER: \\s01\engr\p...
DIR: \\s01\engr\p...
FILE: 17+34.88 -Y-
SHEET: 4 OF 5

| | |
|------------------------|---------------------|
| ASSEMBLED BY : MKT | DATE : 12/2015 |
| CHECKED BY : JCM | DATE : 12/2015 |
| DRAWN BY : ELR 11/91 | REV. 10/1/11 MAA/GM |
| CHECKED BY : GRP 11/91 | REV. 1/15 MAA/TMG |
| | REV. 2/15 MAA/TMG |

STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL, CONNECTOR PLATES AND PLATE WASHERS SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

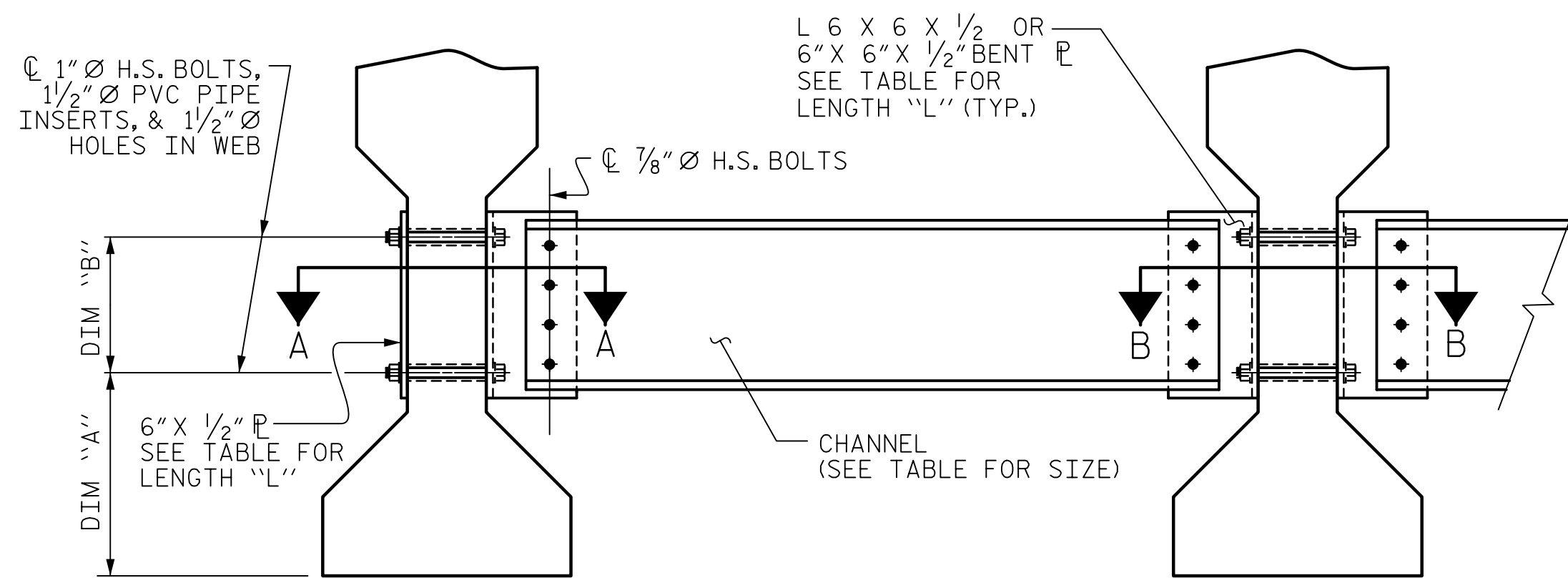
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST 1/4" PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

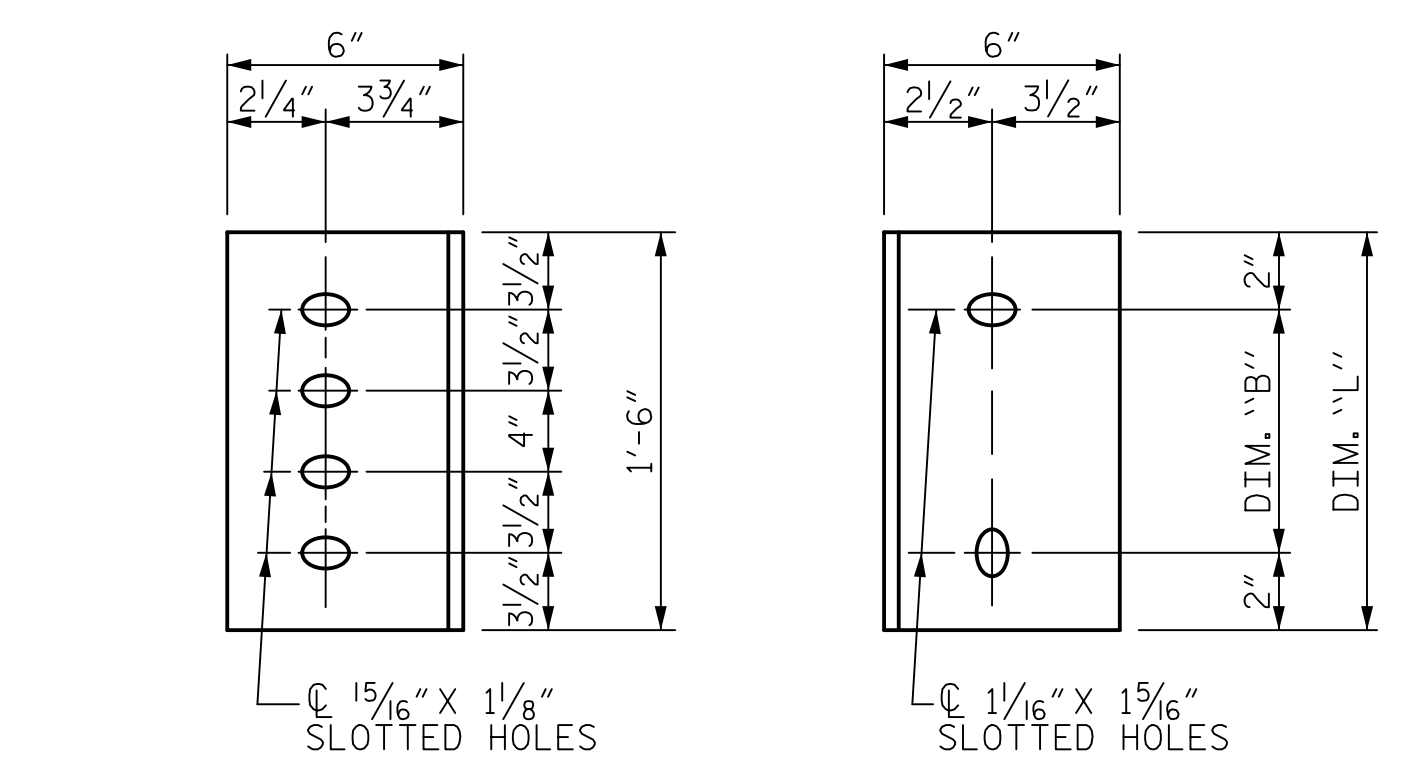
CONTRACTOR SHALL SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, TEMPORARY STRUTS SHALL BE PLACED BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED. ALL AASHTO M164 H.S. BOLTS SHALL BE FULLY TIGHTENED AFTER THE STRUTS HAVE BEEN REMOVED.

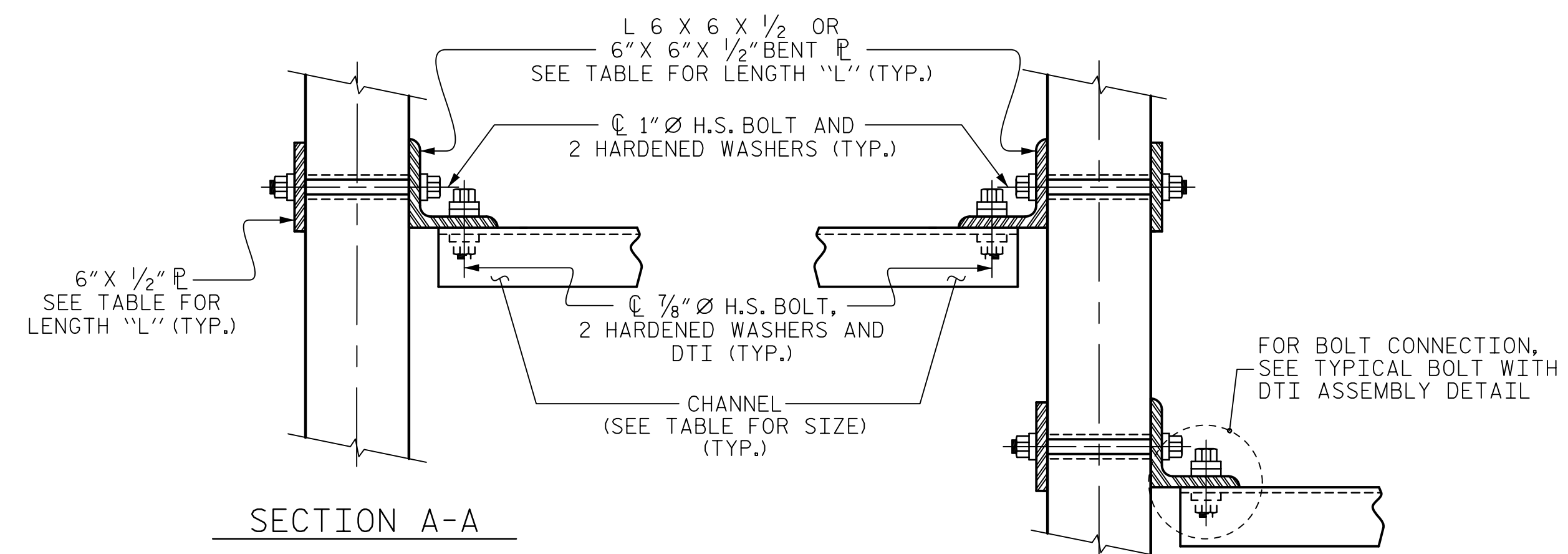
THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



PART SECTION AT INTERMEDIATE DIAPHRAGM
(TYPE IV GIRDER SHOWN)



CONNECTOR PLATE DETAILS



CONNECTION DETAILS
(FOR SKEW < 70° OR SKEW > 110°)

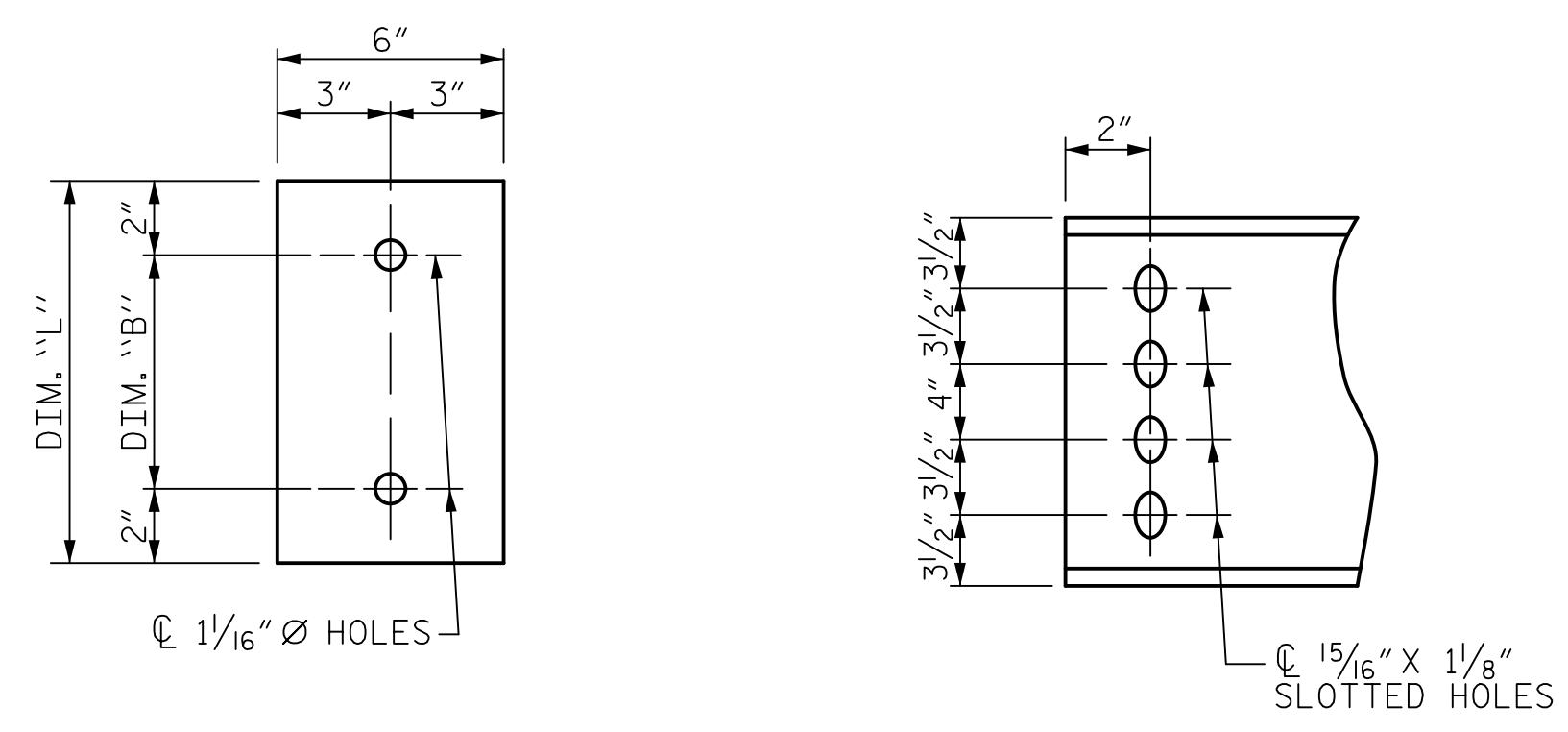
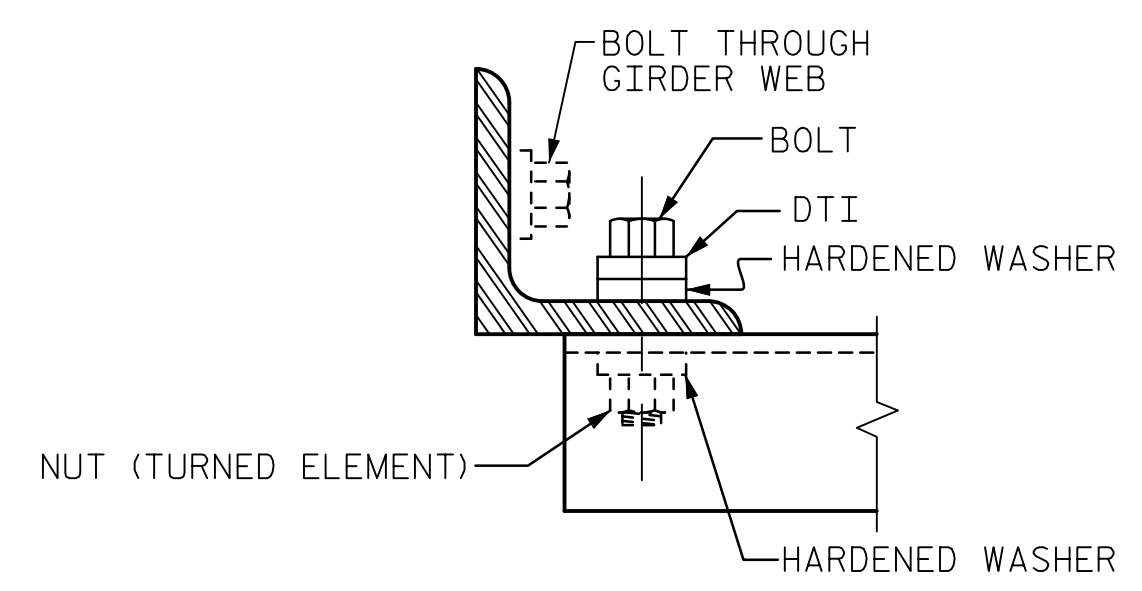


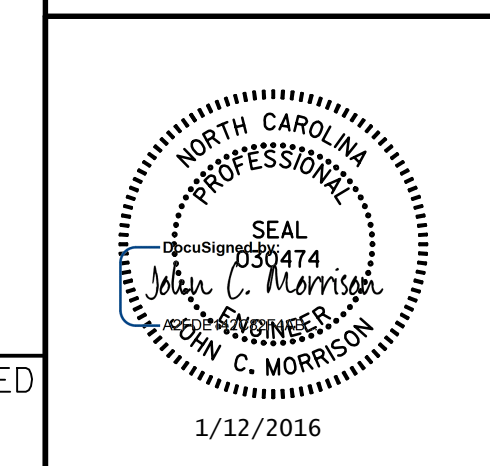
PLATE DETAILS and **CHANNEL END**
(TYPE IV GDR.)

| TABLE | | | | |
|-------------|--------------|-----------|---------|---------|
| GIRDER TYPE | CHANNEL SIZE | DIM "A" | DIM "B" | DIM "L" |
| IV | MC 18 x 42.7 | 1'-9 1/2" | 1'-2" | 1'-6" |



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 5 OF 5 BRIDGE NO. 109



| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
|---------------------------------------------------------------------------------------------|-----|-------|-----|-----|---------------------------|
| STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS | | | | | |
| REVISIONS | | | | | SHEET NO. |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| | | | | | S-58 |
| | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016

DATE: 1/12/2016 TIME: 8:55:03 AM USER: \\s01pc001\p001\c001\402_03_1\U2707_SML_558_P0005.dgn

| | |
|----------------------|-----------------------|
| ASSEMBLED BY : DDL | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : TLA 6/05 | ADDED 10/21/05 |
| CHECKED BY : VC 6/05 | REV. 5/1/06RRR KMM/GM |
| | REV. 10/1/11 MAA/GM |

NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

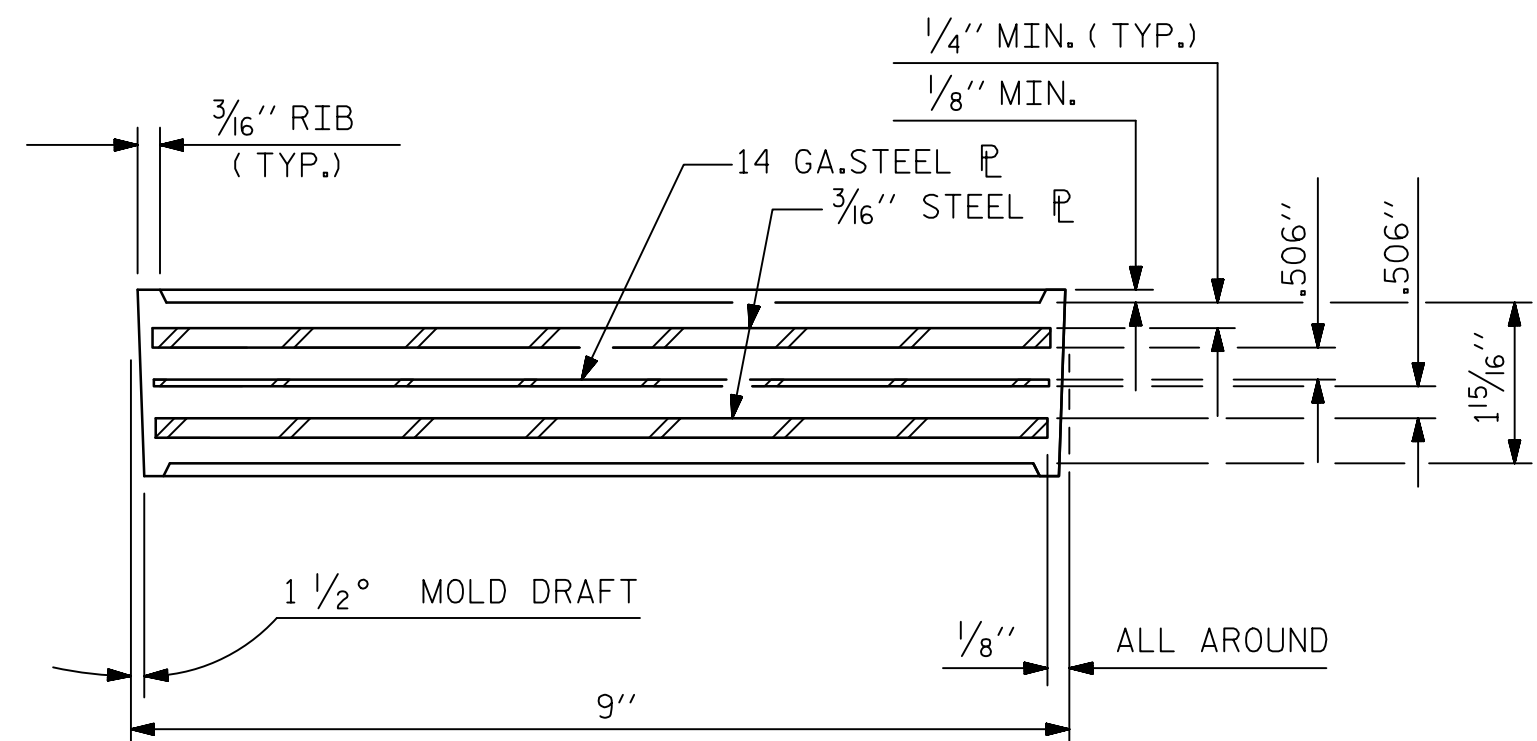
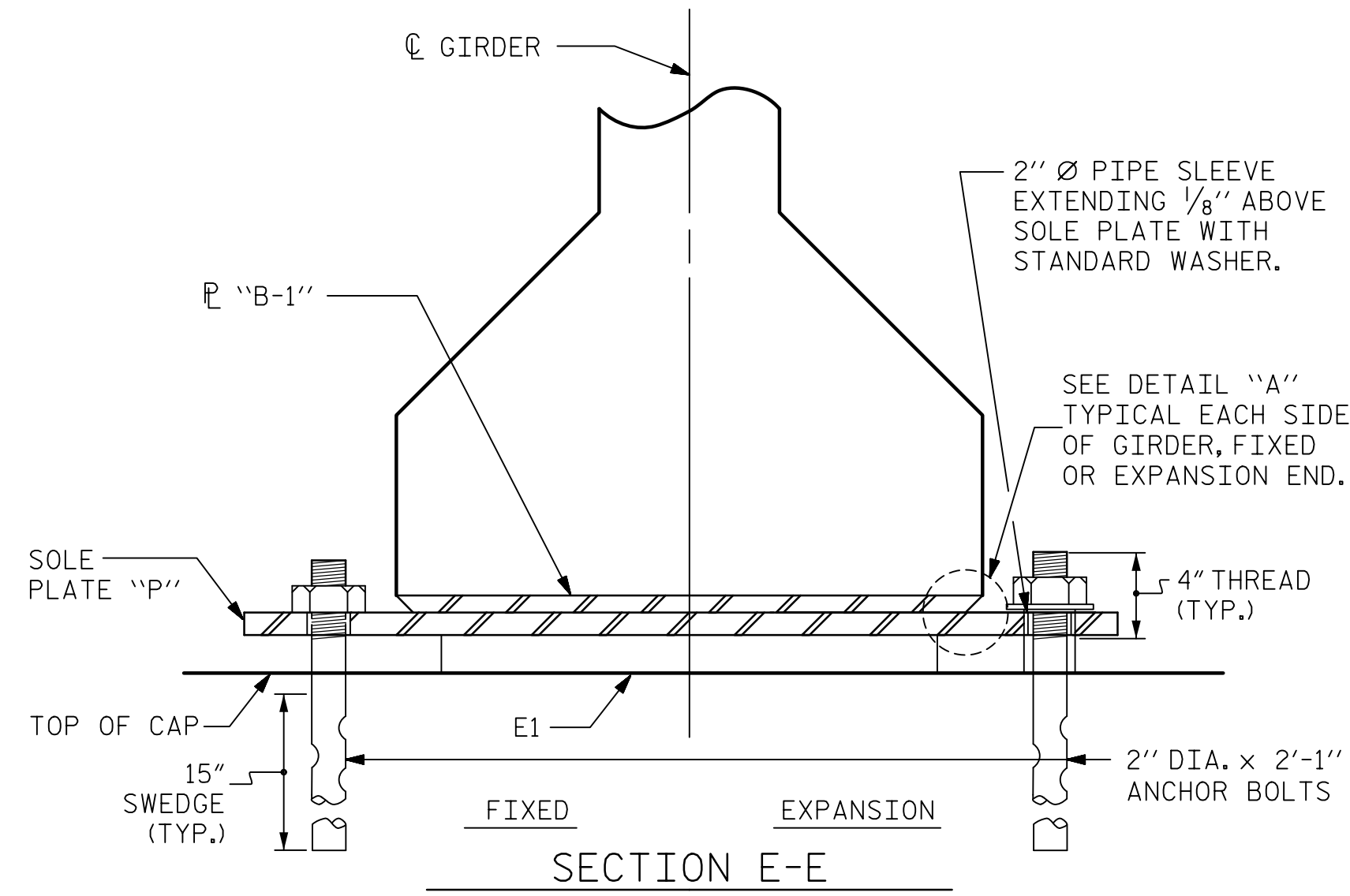
SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

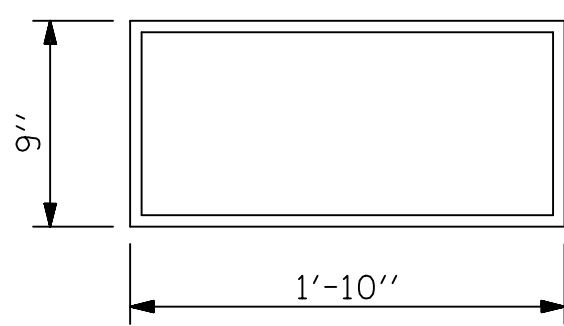
ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.



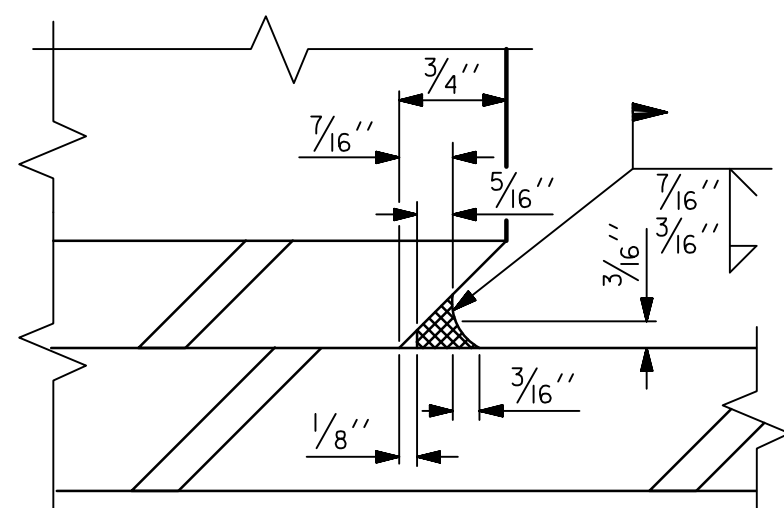
TYPICAL SECTION OF ELASTOMERIC BEARINGS



E1 (48 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

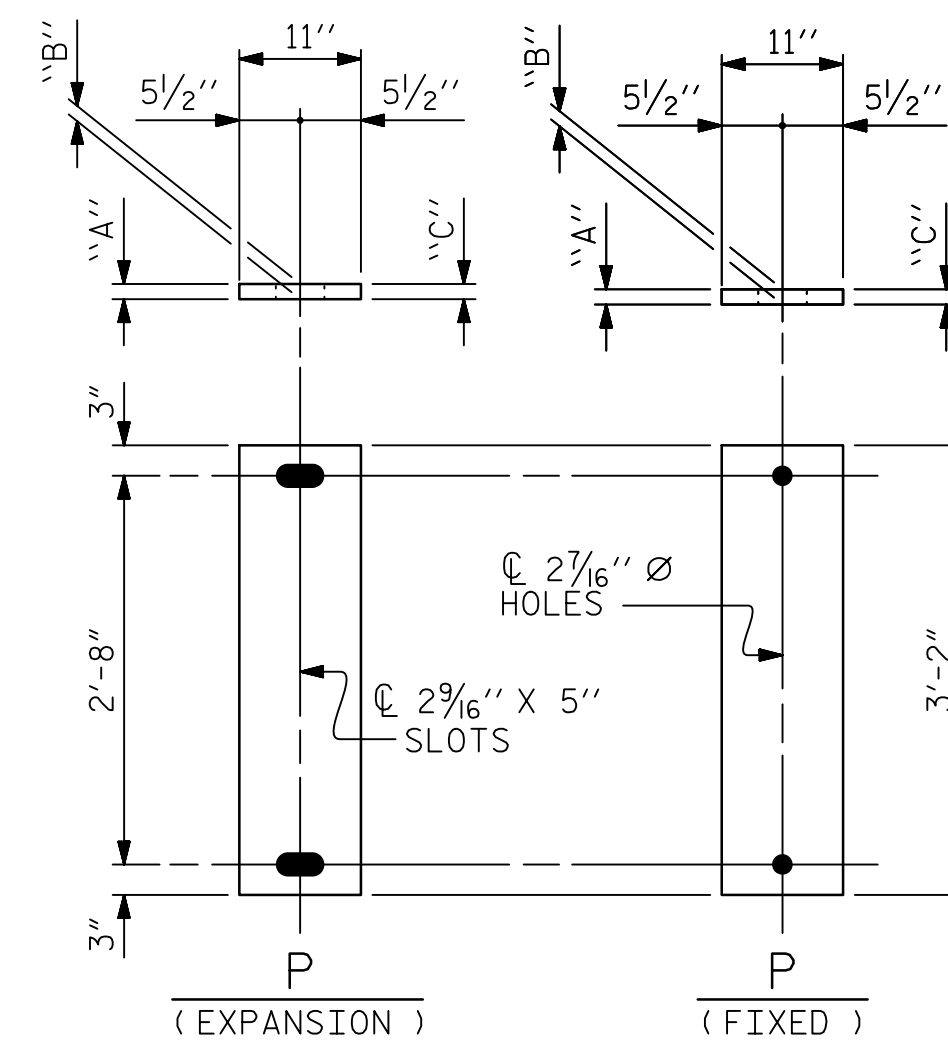
TYPE IV



DETAIL "A"

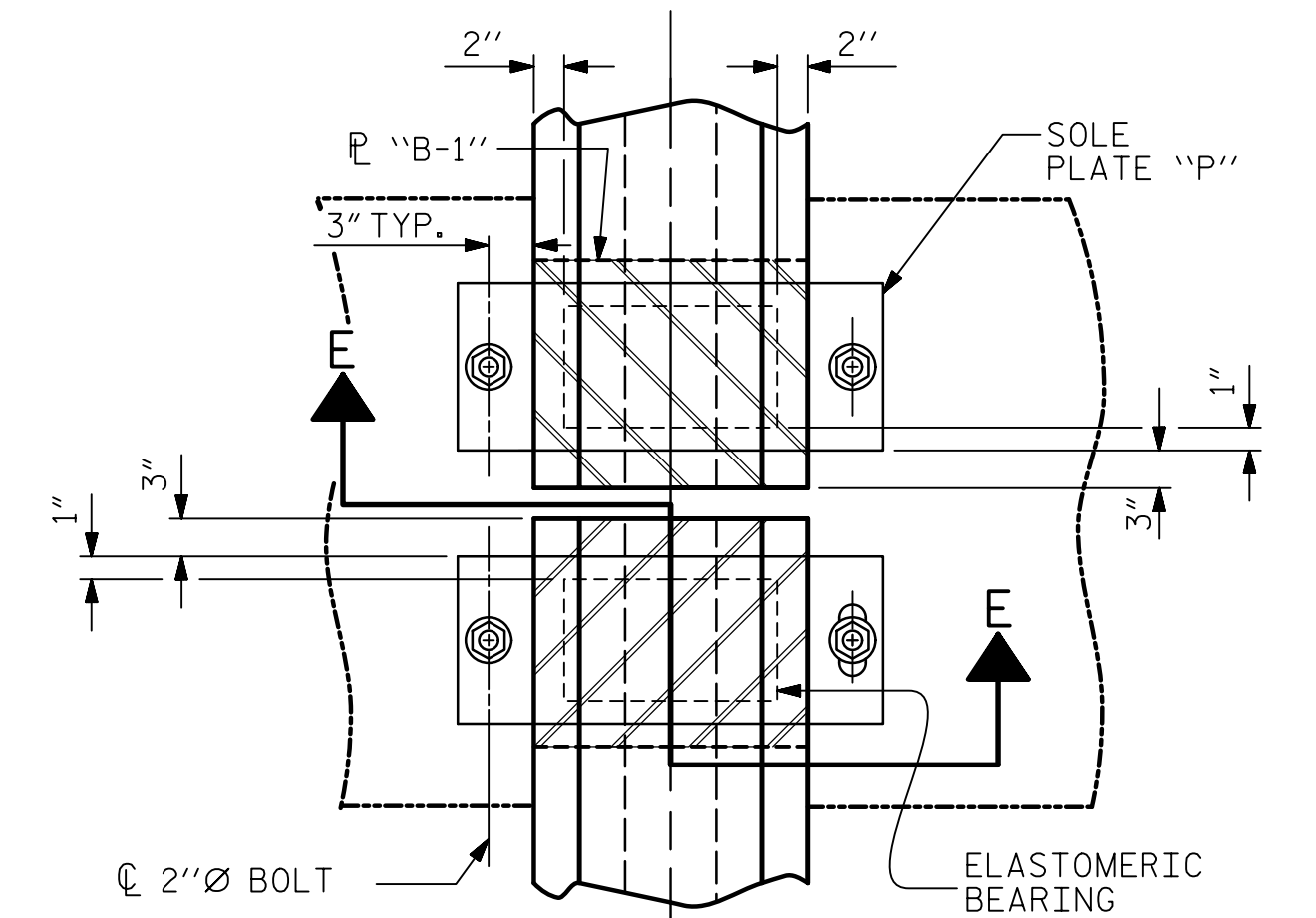
| TABLE OF SOLE PLATES | | | | | |
|----------------------|-------|----------|----------|----------|-----|
| TYPE | F/E * | "A" (in) | "B" (in) | "C" (in) | No. |
| P1 | E | 1/4 | 1 3/8 | 1 1/2 | 16 |
| P2 | F | 1 5/8 | 1 11/16 | 1 3/4 | 3 |
| P3 | F | 1 1/2 | 1 1/2 | 1 1/2 | 13 |
| P4 | F | 1/4 | 1 5/16 | 1 3/8 | 16 |

* DENOTES FIXED OR EXPANSION



SOLE PLATE DETAILS ("P")

(SEE "FRAMING PLAN" FOR SOLE PLATE "P" LOCATIONS AND PLATE ORIENTATION)



TYPICAL HALF-PLAN (SHOWING CONTINUOUS BENT) TYPICAL HALF-PLAN (SHOWING EXP. BRG. @ END BENT)

| | |
|---------------------------------|-------|
| MAXIMUM ALLOWABLE SERVICE LOADS | |
| D.L.+L.L. (NO IMPACT) | |
| TYPE IV | 225 k |

PROJECT NO. U-2707

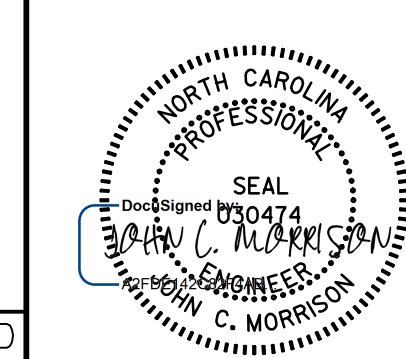
FORSYTH COUNTY

STATION: 17+34.88 -Y-

BRIDGE NO. 109



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
ELASTOMERIC BEARING
DETAILS
PRESTRESSED CONCRETE GIRDER
SUPERSTRUCTURE



| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. S-59
TOTAL SHEETS 86

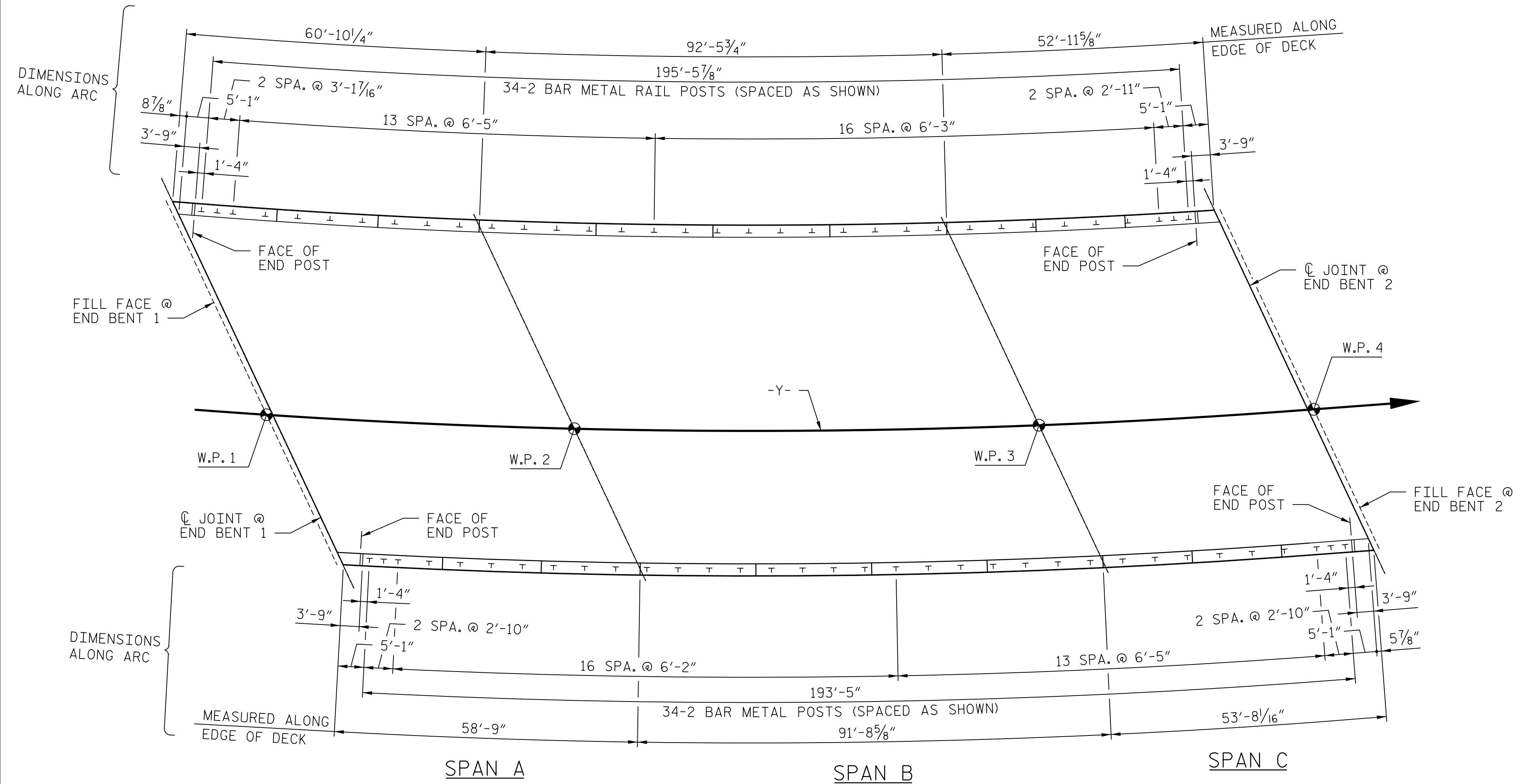
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/13/2016

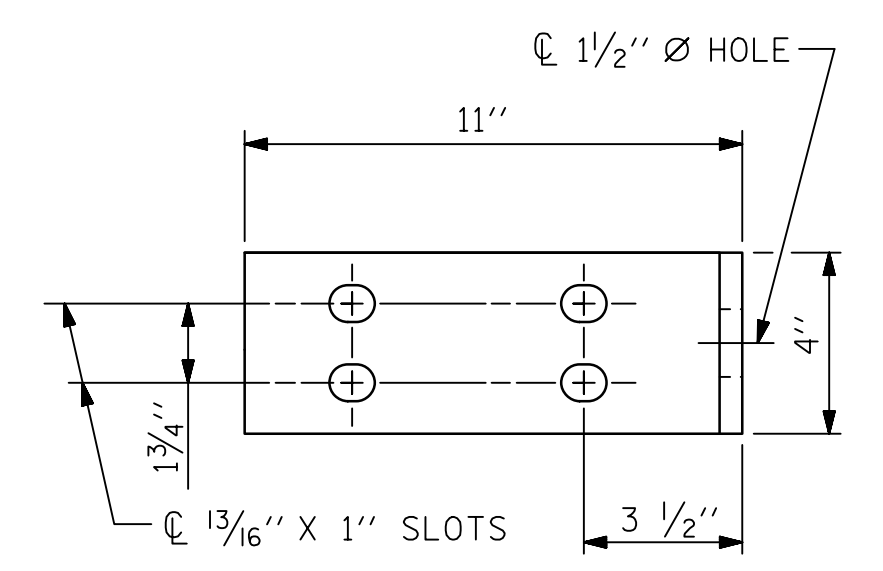
DATE: 1/13/2016
TIME: 2:53:44 PM

USER: \\us-east-1-nfs-001-corp-002-033-liz707_sml-559_brc.dgn
DGN: \\us-east-1-nfs-001-corp-002-033-liz707_sml-559_brc.dgn

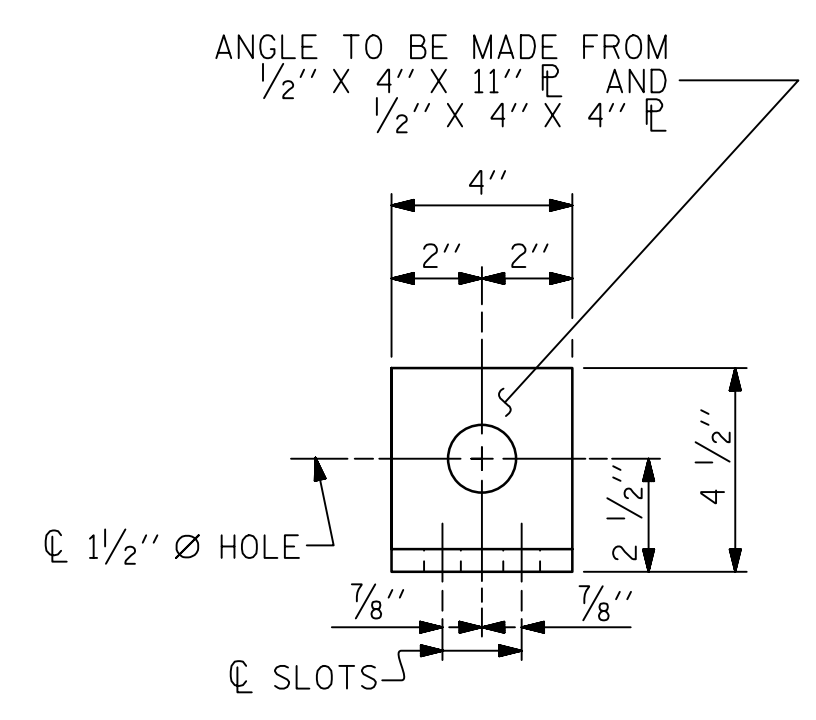
| | |
|-----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : WJH 8/89 | REV. 10/1/11 MAA/GM |
| CHECKED BY : CRK 8/89 | REV. 6/13 AAC/MAA |
| | REV. 1/15 MAA/TMG |



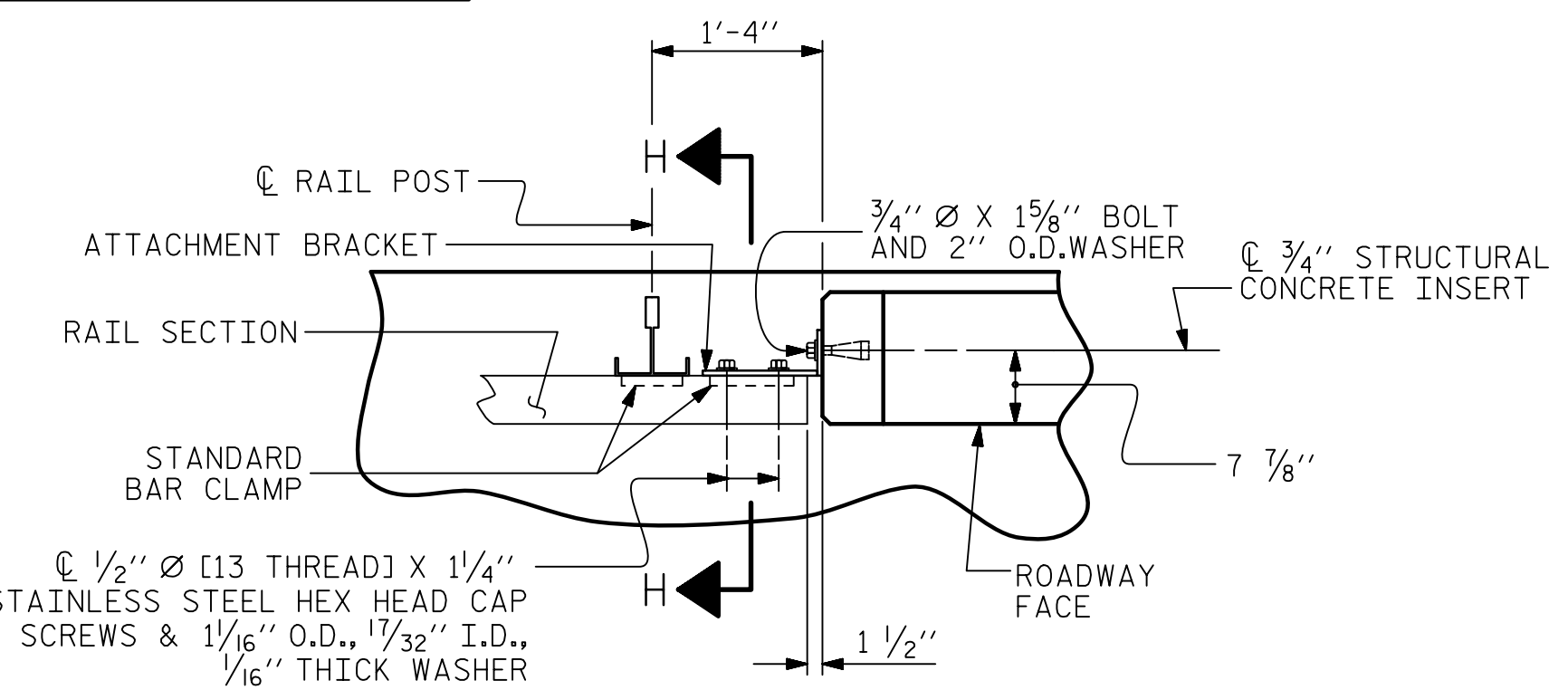
PLAN OF RAIL POST SPACINGS



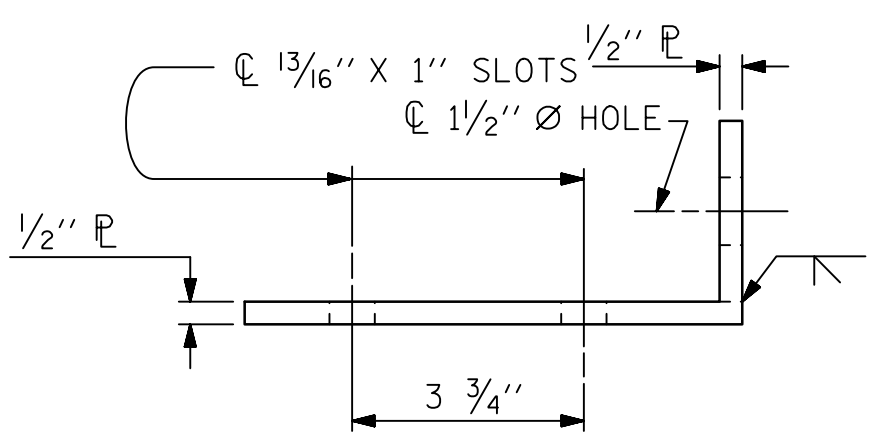
ELEVATION



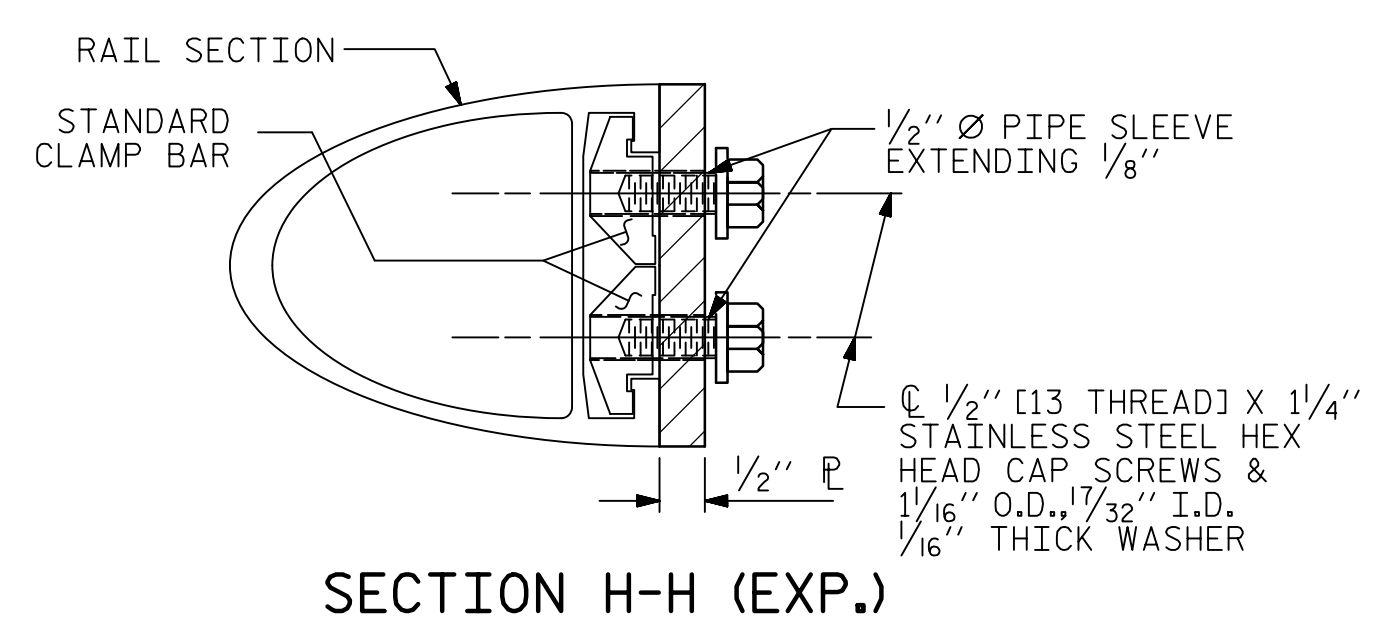
END VIEW (FIX AND EXP.)



PLAN - RAIL AND END POST



TOP VIEW



DETAILS FOR ATTACHING METAL RAIL TO END POST

NOTES

- STRUCTURAL CONCRETE INSERT
- THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 1 1/2".
 - 1 - 3/4" x 1 5/8" BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 3/4" x 1 5/8" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
 - WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 1/16" diameter WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

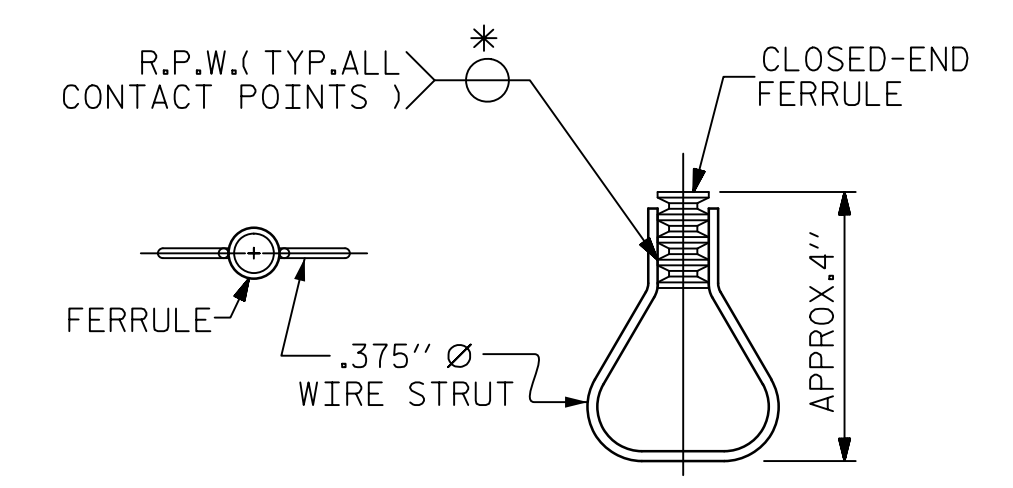
- METAL RAIL TO END POST CONNECTION
- THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
 - 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4" x 1 5/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/4" x 1 5/8" BOLT SHALL HAVE N.C. THREADS.
 - CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
 - STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
 - 1/2" diameter PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE 3/4" STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE 3/4" STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE 1/2" PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" x 1 5/8" BOLT WITH WASHER SHALL BE REPLACED WITH A 3/4" x 6 1/2" BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 3/4" x 1 5/8" BOLT SHALL APPLY TO THE 3/4" x 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



STRUCTURAL CONCRETE INSERT

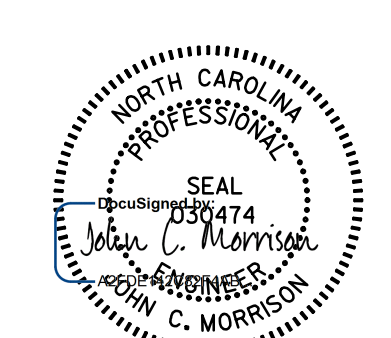
* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 17+34.88 -Y-

SHEET 1 OF 3 BRIDGE NO. 109



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
RAIL POST SPACINGS
AND
END OF RAIL DETAILS
FOR ONE OR TWO BAR METAL RAILS

| REVISIONS | | | | | | SHEET NO. |
|-----------|-----|-------|-----|-----|-------|--------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-62 |
| 1 | | | 3 | | | TOTAL SHEETS |
| 2 | | | 4 | | | 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016

DATE: 1/12/2016
TIME: 8:55:36 AM

USER: jk...
DIR: R:\Bridge\030474\02_039_12707_SML_S62_ZBMR01.dgn

| | |
|-----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : FCJ 1/88 | REV. 5/7/03 RWW/JTE |
| CHECKED BY : CRK 3/89 | REV. 5/1/06 TLA/GM |
| | REV. 10/1/11 MAA/GM |

NOTES

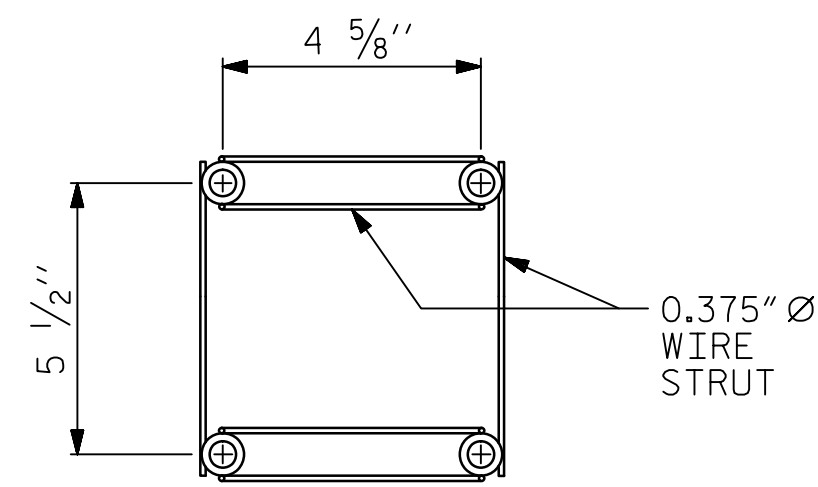
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS :

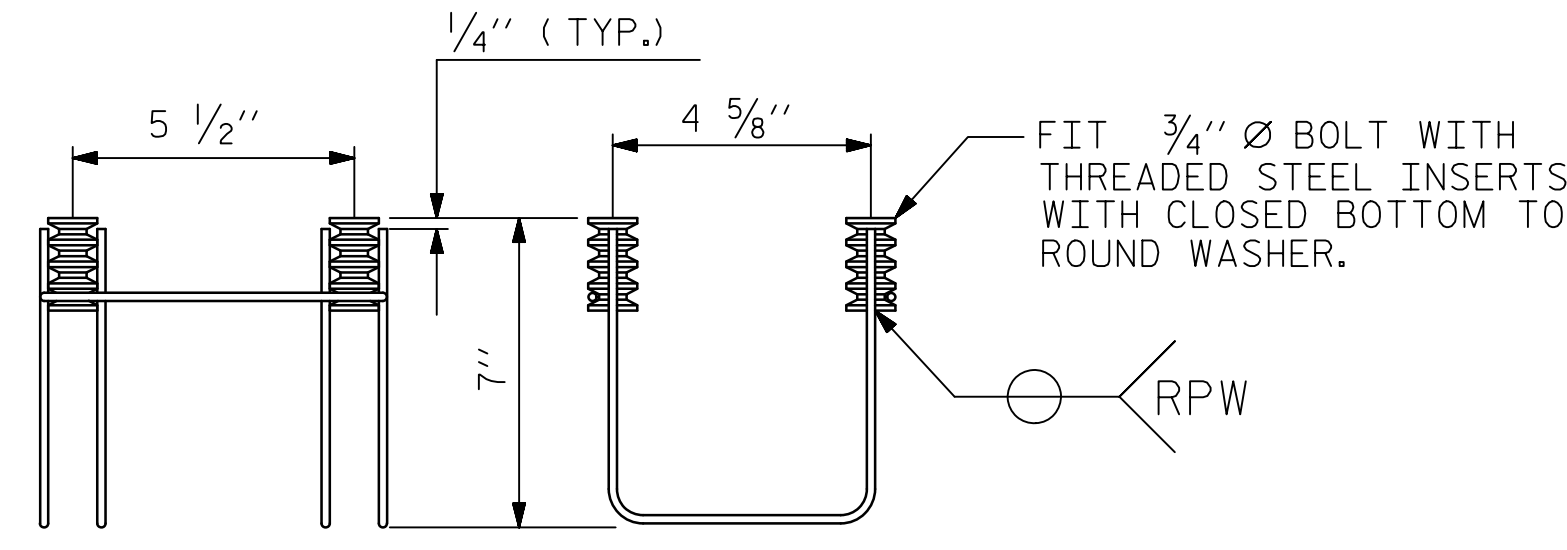
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 - 3/4" Ø X 2 1/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 2 1/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 1/6" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.



PLAN

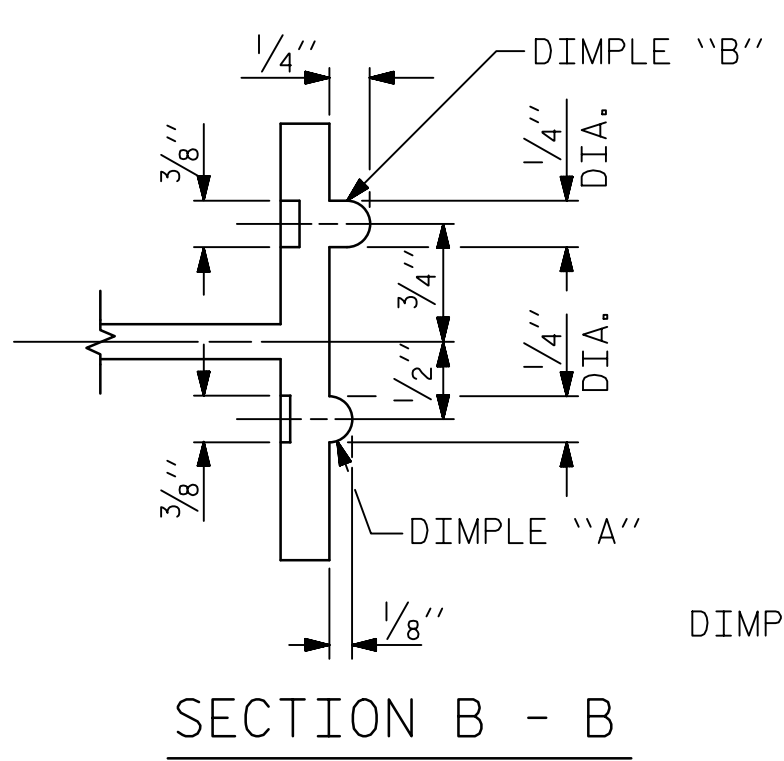


SIDE VIEW ELEVATION

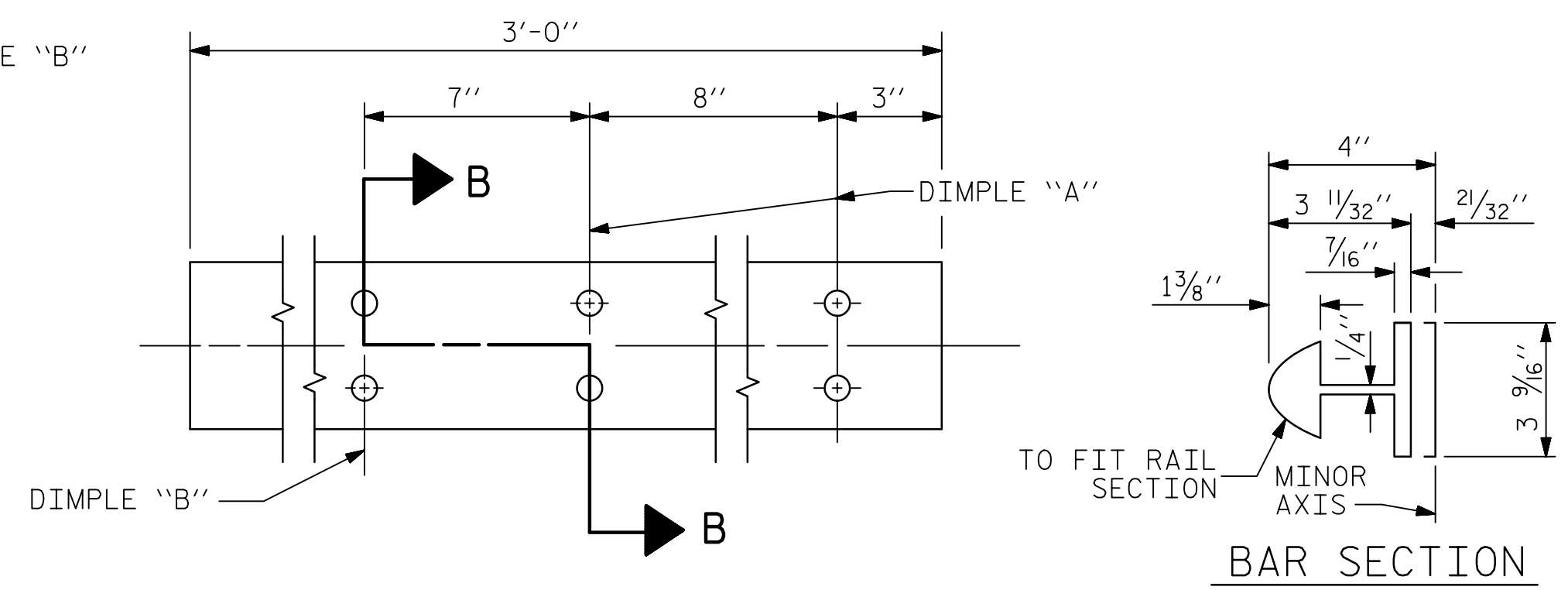
MINIMUM LENGTH OF THREADS IN INSERT (FERRULE) : 1 3/4"

4-BOLT METAL RAIL ANCHOR ASSEMBLY

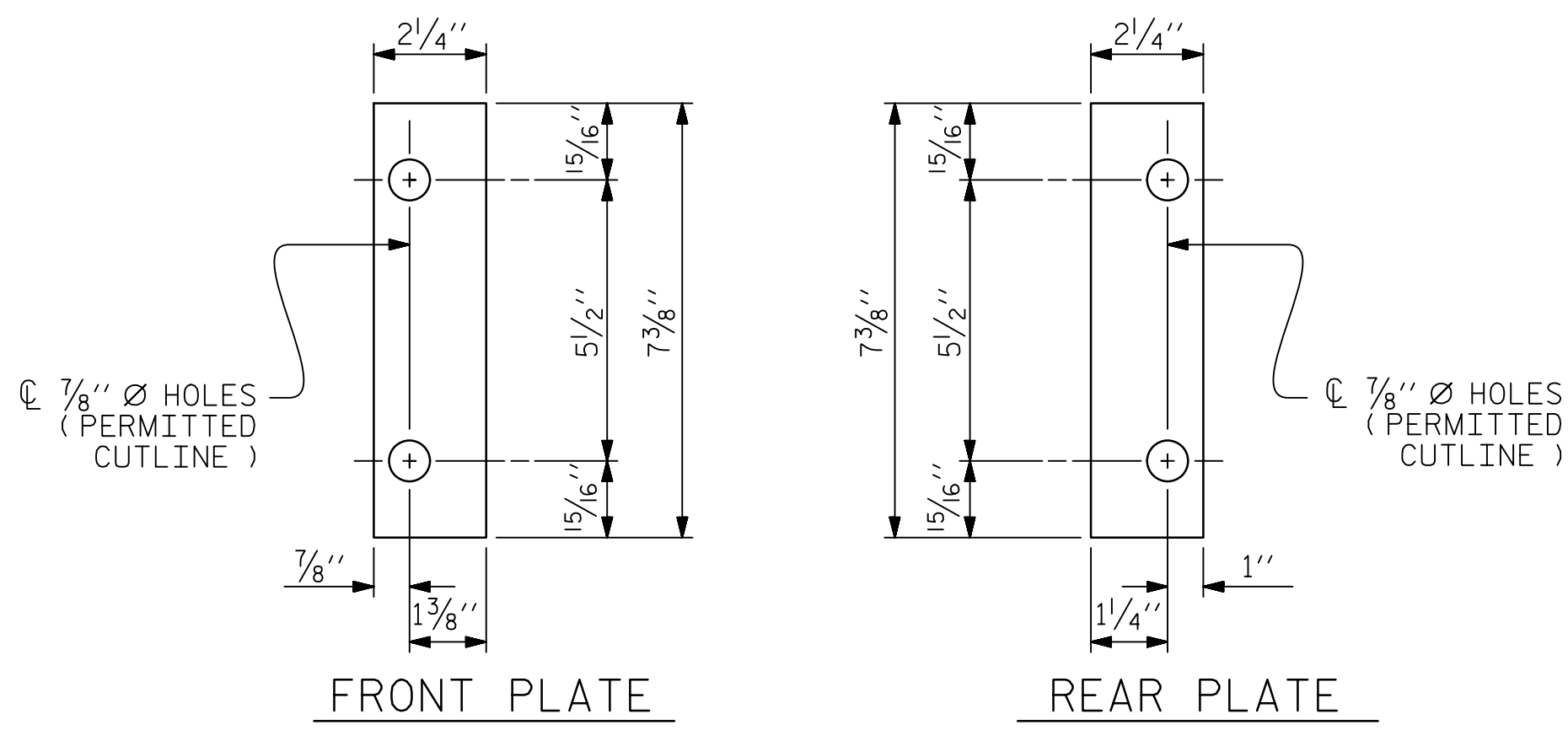
(68 ASSEMBLIES REQUIRED)



SECTION B - B



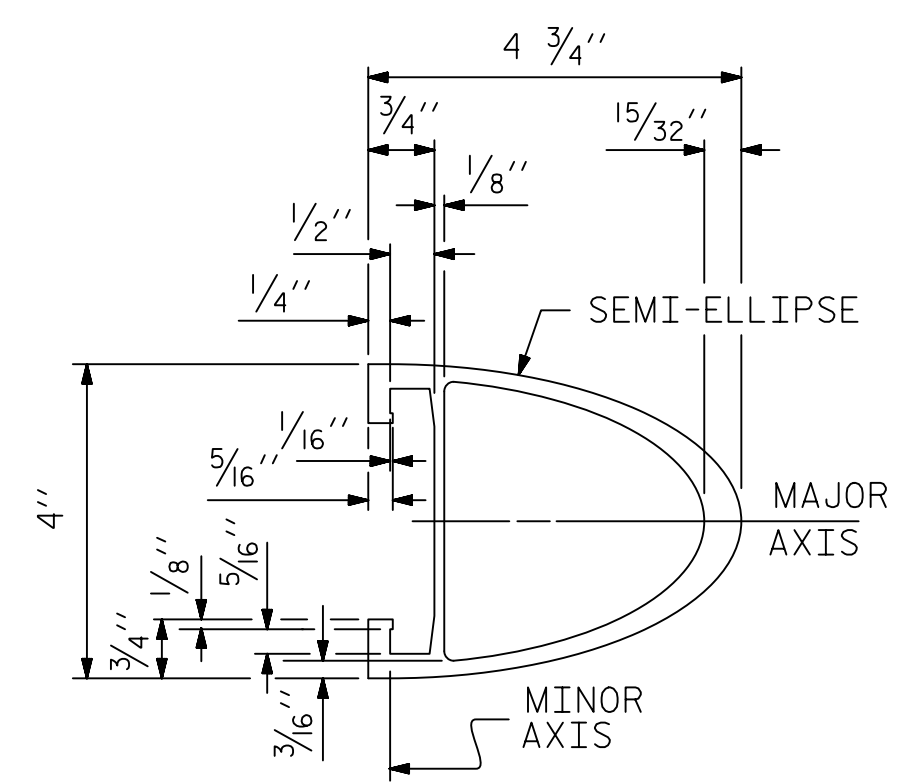
EXPANSION BAR DETAILS



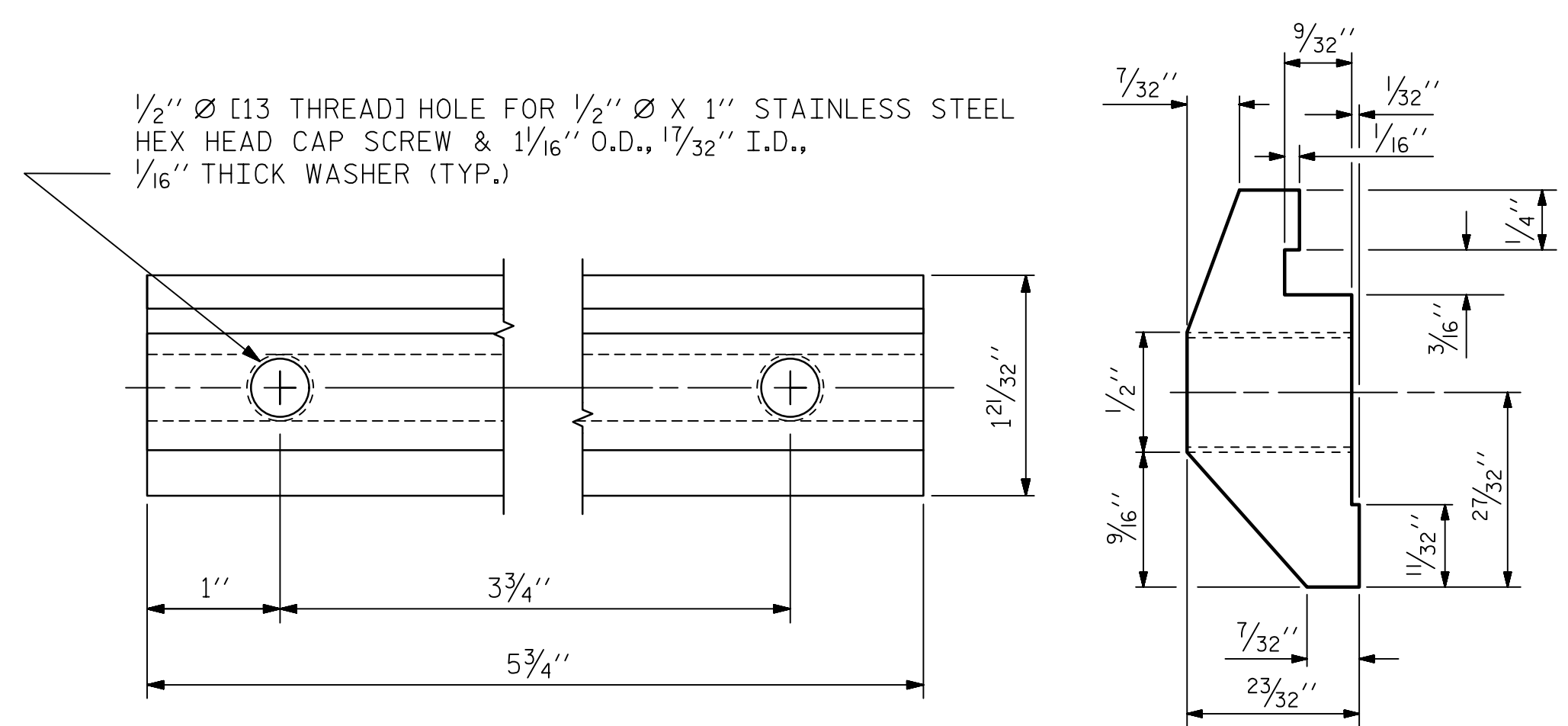
FRONT PLATE REAR PLATE

SHIM DETAILS

NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

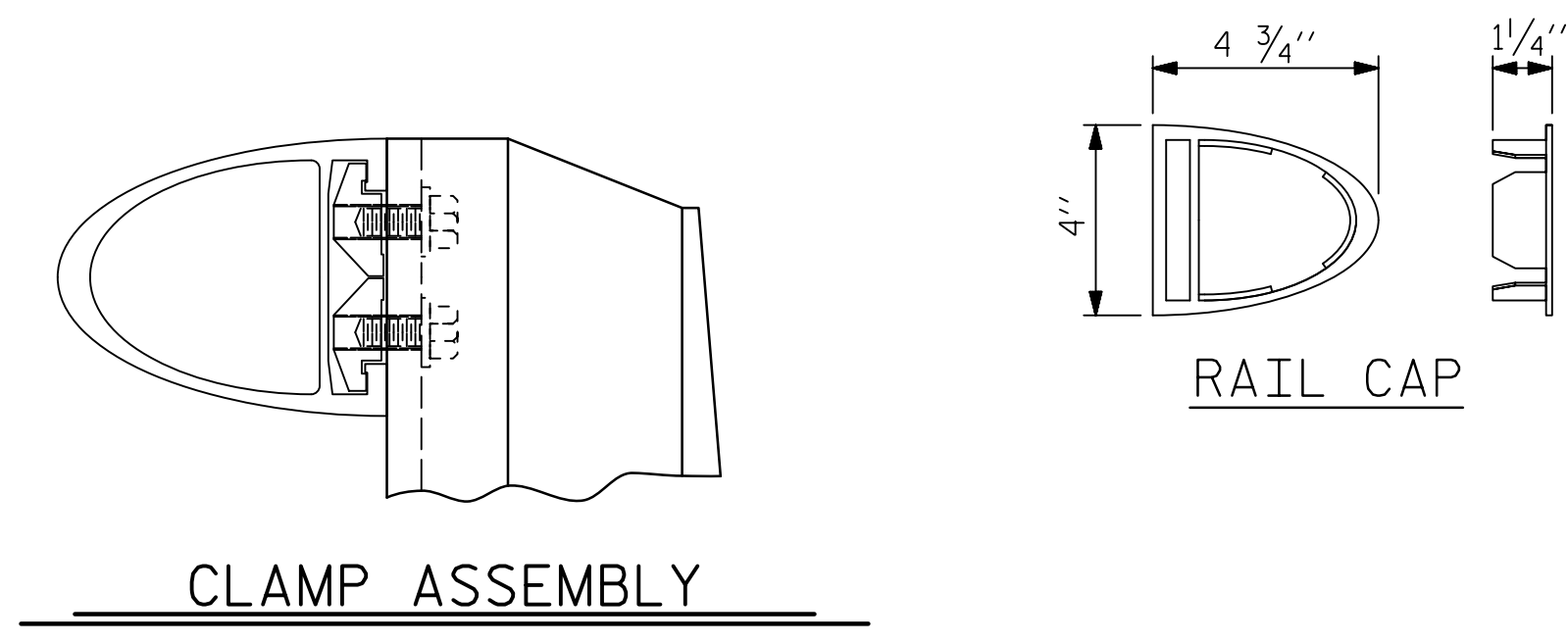


RAIL SECTION

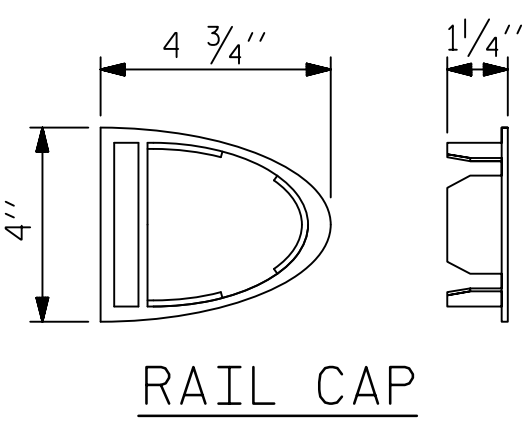


CLAMP BAR DETAIL

(4 REQUIRED PER POST)



CLAMP ASSEMBLY



RAIL CAP

PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 3 OF 3 BRIDGE NO. 109

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-4200 www.aecom.com
 AECOM License No. F-0342

SEAL
 JOHN C. MORRISON
 PROFESSIONAL ENGINEER
 12/12/2016

| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|--------------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| STANDARD 2 BAR METAL RAIL | | | | | |
| SHEET NO. S-64 | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 11/12/2015
 TIME: 8:55:32 AM

USER: \\s01\engr\p\109\cadd\402_043_U2707_SML_S64_2BMR03.dgn

| | |
|-----------------------|----------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : EEM 6/94 | REV. 8/16/99 MAB/LES |
| CHECKED BY : RGW 6/94 | REV. 5/1/06R KMM/GM |
| | REV. 10/1/11 MAA/GM |

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

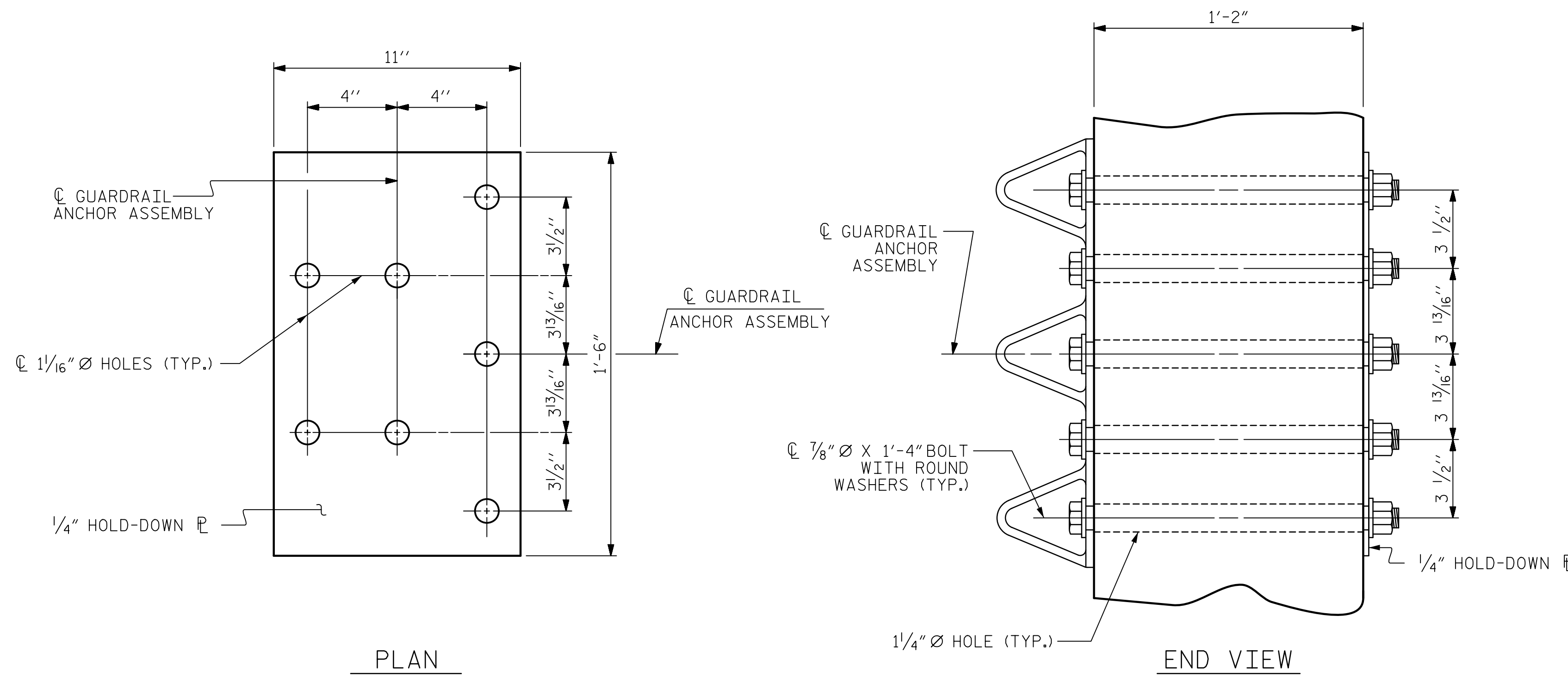
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

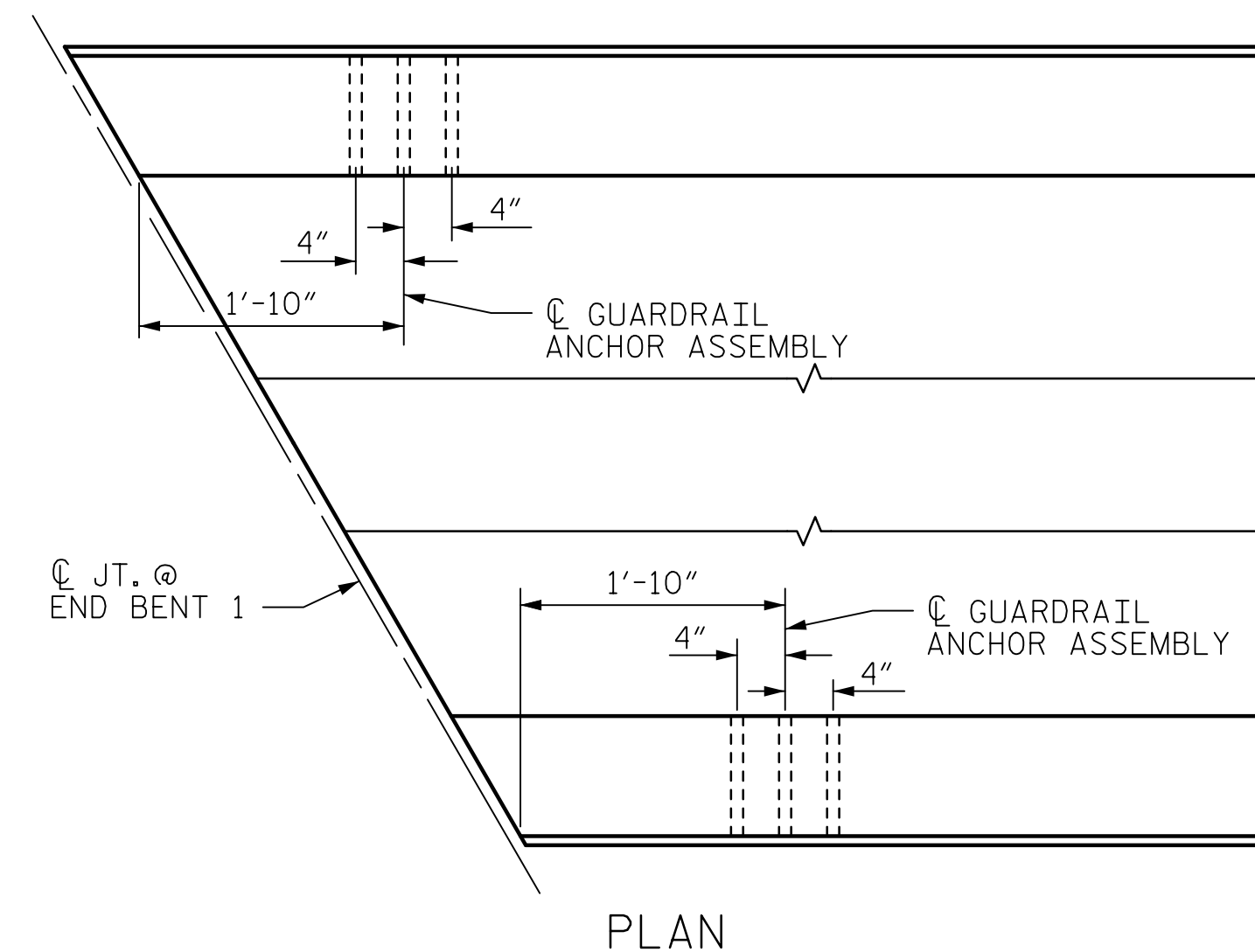
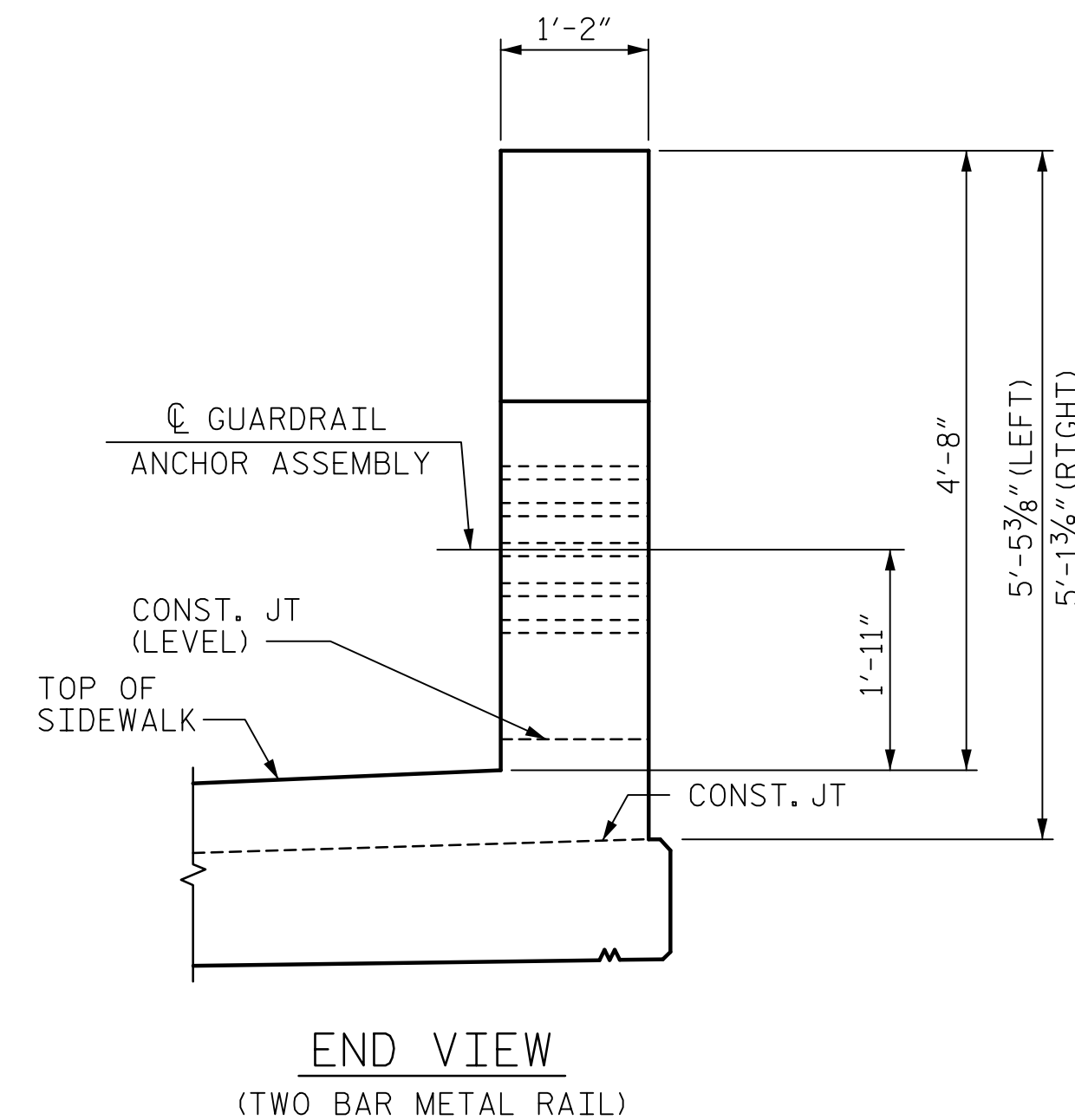
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

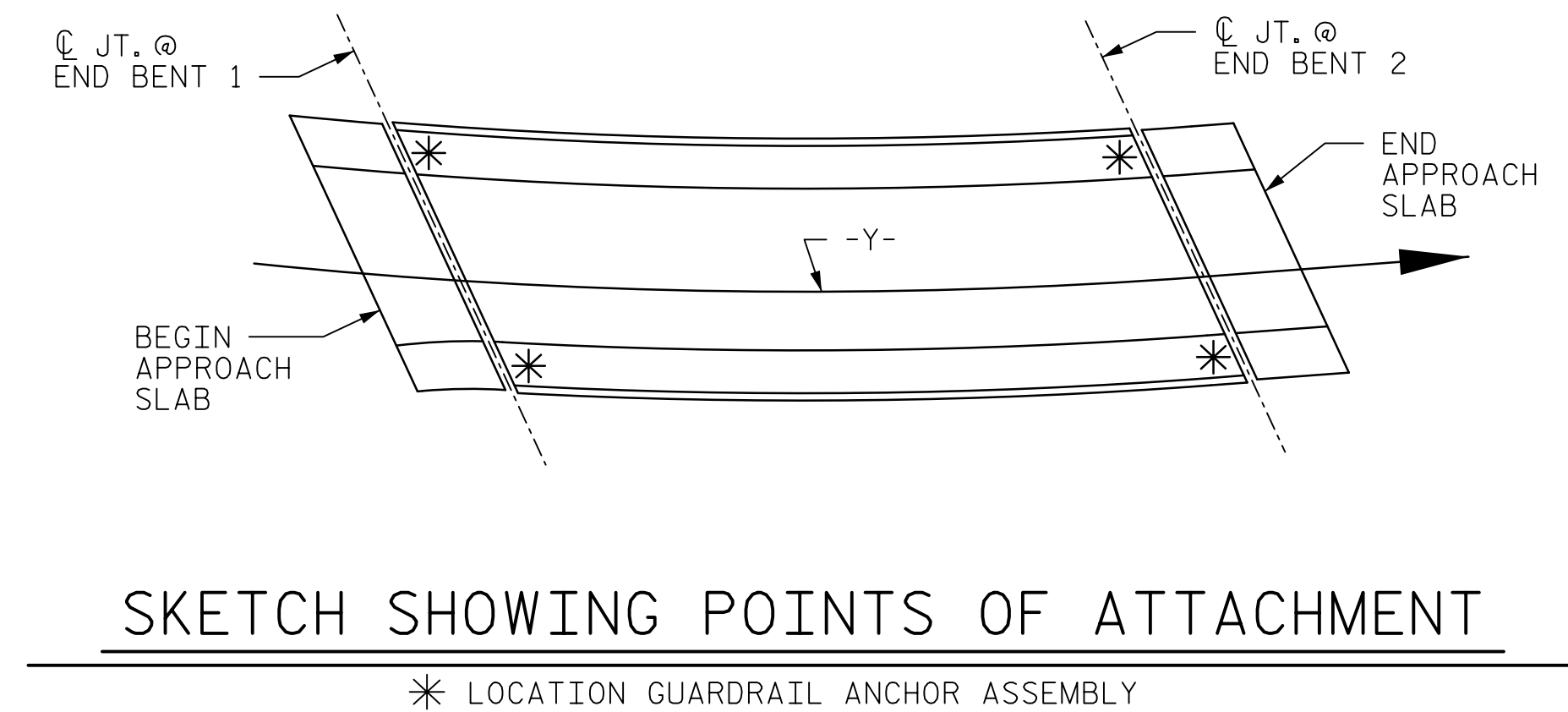


GUARDRAIL ANCHOR ASSEMBLY DETAILS



NOTE: GUARDRAIL ANCHOR ASSEMBLIES AT END BENT 1 SHOWN, DETAILS FOR END BENT 2 SIMILAR.

LOCATION OF GUARDRAIL ANCHOR AT END POST



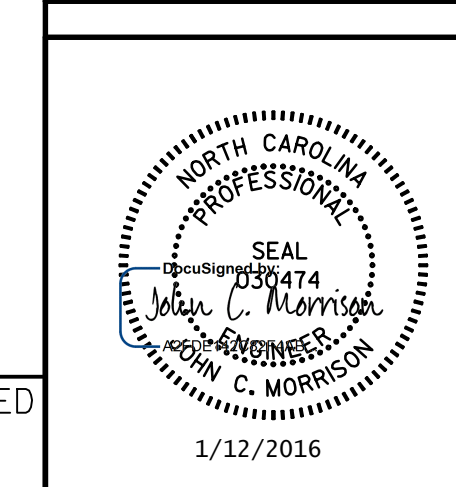
SKETCH SHOWING POINTS OF ATTACHMENT

* LOCATION GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-

BRIDGE NO. 109

| | |
|----------------------|---------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : MAA 5/10 | REV. 12/5/11 MAA/GM |
| CHECKED BY : GM 5/10 | REV. 6/13 MAA/GM |
| | REV. 1/15 MAA/TMC |



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD
 GUARDRAIL ANCHORAGE
 DETAILS
 FOR METAL RAILS

REVISIONS

| NO. | BY: | DATE: | NO. | BY: | DATE: |
|-----|-----|-------|-----|-----|-------|
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. S-65
 TOTAL SHEETS 86

DOCUMENT NOT CONSIDERED
 FINAL UNLESS ALL
 SIGNATURES COMPLETED

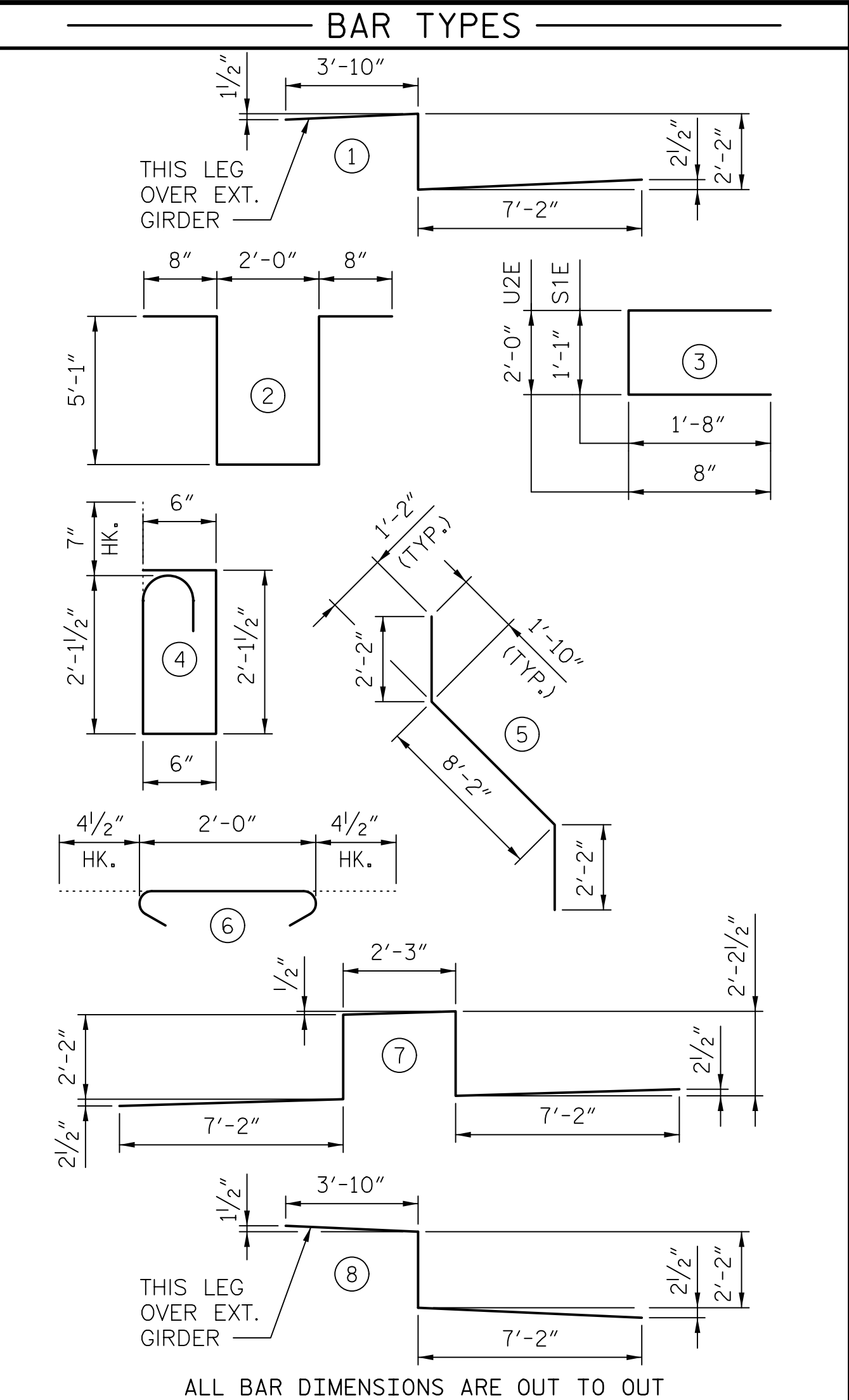
1/12/2016

DATE: 1/12/2016
 TIME: 8:55:35 AM

USER: \\s01\corporate\p03\cadd\02_045_U2707_SML_S65_GRA3.dgn

REINFORCING STEEL SCHEDULE

| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|-----|-----|------|------|---------|--------|-------|-----|------|------|--------|--------|-------|-----|------|------|---------|--------|------|-----|------|------|--------|--------|------|-----|------|------|--------|--------|----|-----|----|------|--------|--------|-------|---|----|------|-------|---|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|-------|---|----|------|-------|---|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|------|---|----|------|--------|----|------|---|----|------|---------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|------|---|----|------|-------|----|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|------|---|----|------|--------|---|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|------|---|----|------|--------|---|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|---|------|---|----|------|-------|---|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|-------|----|------|---|----|------|-------|---|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|-------|---|------|---|----|------|-------|---|-------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|-------|---|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|-------|---|-------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|-------|---|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|-------|---|-------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|-------|---|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|---------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|---------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|--------|----|-------|---|----|------|--------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|-------|----|-------|---|----|------|---------|----|------|---|----|------|---------|----|------|---|----|------|--------|----|-------|---|----|------|--------|---|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|-------|---|----|------|---------|----|------|---|----|------|--------|----|------|---|----|------|--------|----|
| A1E | 690 | #5 | STR. | 35'-11" | 25,848 | A157E | 1 | #5 | STR. | 6'-10" | 7 | A216E | 1 | #5 | STR. | 11'-10" | 12 | A250 | 1 | #5 | STR. | 14'-7" | 15 | A309 | 1 | #5 | STR. | 19'-1" | 20 | A2 | 690 | #5 | STR. | 35'-9" | 25,728 | A158E | 1 | #5 | STR. | 5'-9" | 6 | A217E | 1 | #5 | STR. | 10'-10" | 11 | A251 | 1 | #5 | STR. | 13'-5" | 14 | A310 | 1 | #5 | STR. | 18'-1" | 19 | A101E | 2 | #5 | STR. | 35'-6" | 74 | A159E | 1 | #5 | STR. | 4'-7" | 5 | A218E | 1 | #5 | STR. | 9'-10" | 10 | A252 | 1 | #5 | STR. | 12'-4" | 13 | A311 | 1 | #5 | STR. | 17'-0" | 18 | A102E | 2 | #5 | STR. | 35'-0" | 73 | A160E | 1 | #5 | STR. | 3'-6" | 4 | A219E | 1 | #5 | STR. | 8'-9" | 9 | A253 | 1 | #5 | STR. | 11'-3" | 12 | A312 | 1 | #5 | STR. | 16'-0" | 17 | A103E | 2 | #5 | STR. | 34'-5" | 72 | A161E | 2 | #5 | STR. | 35'-7" | 74 | A220E | 1 | #5 | STR. | 7'-9" | 8 | A254 | 1 | #5 | STR. | 10'-2" | 11 | A313 | 1 | #5 | STR. | 14'-11" | 16 | A104E | 2 | #5 | STR. | 33'-10" | 71 | A162E | 2 | #5 | STR. | 35'-1" | 73 | A221E | 1 | #5 | STR. | 6'-9" | 7 | A255 | 1 | #5 | STR. | 9'-0" | 10 | A314 | 1 | #5 | STR. | 13'-11" | 15 | A105E | 2 | #5 | STR. | 33'-4" | 70 | A163E | 2 | #5 | STR. | 34'-7" | 72 | A222E | 1 | #5 | STR. | 5'-9" | 6 | A256 | 1 | #5 | STR. | 7'-11" | 8 | A315 | 1 | #5 | STR. | 12'-11" | 14 | A106E | 2 | #5 | STR. | 32'-9" | 68 | A164E | 2 | #5 | STR. | 34'-0" | 71 | A223E | 1 | #5 | STR. | 4'-8" | 5 | A257 | 1 | #5 | STR. | 6'-10" | 7 | A316 | 1 | #5 | STR. | 11'-10" | 13 | A107E | 2 | #5 | STR. | 32'-3" | 67 | A165E | 2 | #5 | STR. | 33'-6" | 70 | A224E | 1 | #5 | STR. | 3'-8" | 4 | A258 | 1 | #5 | STR. | 5'-9" | 6 | A317 | 1 | #5 | STR. | 10'-10" | 11 | A108E | 2 | #5 | STR. | 31'-8" | 66 | A166E | 2 | #5 | STR. | 33'-0" | 69 | A225E | 6 | #6 | STR. | 6'-0" | 54 | A259 | 1 | #5 | STR. | 4'-7" | 5 | A318 | 1 | #5 | STR. | 9'-10" | 10 | A109E | 2 | #5 | STR. | 31'-1" | 65 | A167E | 2 | #5 | STR. | 32'-6" | 68 | A201 | 2 | #5 | STR. | 35'-3" | 74 | A260 | 1 | #5 | STR. | 3'-6" | 4 | A319 | 1 | #5 | STR. | 8'-9" | 9 | A110E | 2 | #5 | STR. | 30'-7" | 64 | A168E | 2 | #5 | STR. | 31'-11" | 67 | A202 | 2 | #5 | STR. | 34'-9" | 73 | A261 | 2 | #5 | STR. | 35'-4" | 74 | A320 | 1 | #5 | STR. | 7'-9" | 8 | A111E | 2 | #5 | STR. | 30'-0" | 63 | A169E | 2 | #5 | STR. | 31'-5" | 66 | A203 | 2 | #5 | STR. | 34'-2" | 71 | A262 | 2 | #5 | STR. | 34'-10" | 73 | A321 | 1 | #5 | STR. | 6'-9" | 7 | A112E | 2 | #5 | STR. | 29'-6" | 62 | A170E | 2 | #5 | STR. | 30'-11" | 65 | A204 | 2 | #5 | STR. | 33'-7" | 70 | A263 | 2 | #5 | STR. | 34'-4" | 72 | A322 | 1 | #5 | STR. | 5'-9" | 6 | A113E | 2 | #5 | STR. | 28'-11" | 60 | A171E | 2 | #5 | STR. | 30'-5" | 64 | A205 | 2 | #5 | STR. | 33'-1" | 69 | A264 | 2 | #5 | STR. | 33'-9" | 71 | A323 | 1 | #5 | STR. | 4'-8" | 5 | A114E | 2 | #5 | STR. | 28'-4" | 59 | A172E | 2 | #5 | STR. | 29'-11" | 63 | A206 | 2 | #5 | STR. | 32'-6" | 68 | A265 | 2 | #5 | STR. | 33'-3" | 70 | A324 | 1 | #5 | STR. | 3'-8" | 4 | A115E | 2 | #5 | STR. | 27'-10" | 58 | A173E | 2 | #5 | STR. | 29'-4" | 61 | A207 | 2 | #5 | STR. | 32'-0" | 67 | A266 | 2 | #5 | STR. | 32'-9" | 68 | A116E | 2 | #5 | STR. | 27'-3" | 57 | A174E | 2 | #5 | STR. | 28'-10" | 60 | A208 | 2 | #5 | STR. | 31'-5" | 66 | A267 | 2 | #5 | STR. | 32'-3" | 67 | A117E | 2 | #5 | STR. | 26'-9" | 56 | A175E | 2 | #5 | STR. | 28'-4" | 59 | A209 | 2 | #5 | STR. | 30'-10" | 64 | A268 | 2 | #5 | STR. | 31'-8" | 66 | A118E | 2 | #5 | STR. | 26'-2" | 55 | A176E | 2 | #5 | STR. | 27'-10" | 58 | A210 | 2 | #5 | STR. | 30'-4" | 63 | A269 | 2 | #5 | STR. | 31'-2" | 65 | A119E | 2 | #5 | STR. | 25'-7" | 54 | A177E | 2 | #5 | STR. | 27'-4" | 57 | A211 | 2 | #5 | STR. | 29'-9" | 62 | A270 | 2 | #5 | STR. | 30'-8" | 64 | A120E | 2 | #5 | STR. | 25'-1" | 52 | A178E | 2 | #5 | STR. | 26'-9" | 56 | A212 | 2 | #5 | STR. | 29'-3" | 61 | A271 | 2 | #5 | STR. | 30'-2" | 63 | A121E | 2 | #5 | STR. | 24'-6" | 51 | A179E | 2 | #5 | STR. | 26'-3" | 55 | A213 | 2 | #5 | STR. | 28'-8" | 60 | A272 | 2 | #5 | STR. | 29'-8" | 62 | A122E | 2 | #5 | STR. | 24'-0" | 50 | A180E | 2 | #5 | STR. | 25'-9" | 54 | A214 | 2 | #5 | STR. | 28'-1" | 59 | A273 | 2 | #5 | STR. | 29'-1" | 61 | A123E | 2 | #5 | STR. | 23'-5" | 49 | A181E | 2 | #5 | STR. | 25'-3" | 53 | A215 | 2 | #5 | STR. | 27'-7" | 58 | A274 | 2 | #5 | STR. | 28'-7" | 60 | A124E | 2 | #5 | STR. | 22'-10" | 48 | A182E | 2 | #5 | STR. | 24'-9" | 52 | A216 | 2 | #5 | STR. | 27'-0" | 56 | A275 | 2 | #5 | STR. | 28'-1" | 59 | A125E | 2 | #5 | STR. | 22'-4" | 47 | A183E | 2 | #5 | STR. | 24'-2" | 50 | A217 | 2 | #5 | STR. | 26'-6" | 55 | A276 | 2 | #5 | STR. | 27'-7" | 58 | A126E | 2 | #5 | STR. | 21'-9" | 46 | A184E | 2 | #5 | STR. | 23'-8" | 49 | A218 | 2 | #5 | STR. | 25'-11" | 54 | A277 | 2 | #5 | STR. | 27'-1" | 57 | A127E | 1 | #5 | STR. | 39'-11" | 42 | A185E | 2 | #5 | STR. | 23'-2" | 48 | A219 | 2 | #5 | STR. | 25'-4" | 53 | A278 | 2 | #5 | STR. | 26'-6" | 55 | A128E | 1 | #5 | STR. | 38'-10" | 41 | A186E | 2 | #5 | STR. | 22'-8" | 47 | A220 | 2 | #5 | STR. | 24'-10" | 52 | A279 | 2 | #5 | STR. | 26'-0" | 54 | A129E | 1 | #5 | STR. | 37'-8" | 40 | A187E | 2 | #5 | STR. | 22'-2" | 46 | A221 | 2 | #5 | STR. | 24'-3" | 51 | A280 | 2 | #5 | STR. | 25'-6" | 53 | A130E | 1 | #5 | STR. | 36'-7" | 38 | A188E | 2 | #5 | STR. | 21'-7" | 45 | A222 | 2 | #5 | STR. | 23'-9" | 50 | A281 | 2 | #5 | STR. | 25'-0" | 52 | A131E | 1 | #5 | STR. | 35'-6" | 37 | A189E | 1 | #5 | STR. | 39'-8" | 41 | A223 | 2 | #5 | STR. | 23'-2" | 48 | A282 | 2 | #5 | STR. | 24'-6" | 51 | A132E | 1 | #5 | STR. | 34'-5" | 36 | A190E | 1 | #5 | STR. | 38'-8" | 40 | A224 | 2 | #5 | STR. | 22'-7" | 47 | A283 | 2 | #5 | STR. | 23'-11" | 50 | A133E | 1 | #5 | STR. | 33'-4" | 35 | A191E | 1 | #5 | STR. | 37'-7" | 39 | A225 | 2 | #5 | STR. | 22'-1" | 46 | A284 | 2 | #5 | STR. | 23'-5" | 49 | A134E | 1 | #5 | STR. | 32'-2" | 34 | A192E | 1 | #5 | STR. | 36'-7" | 38 | A226 | 2 | #5 | STR. | 21'-6" | 45 | A285 | 2 | #5 | STR. | 22'-11" | 48 | A135E | 1 | #5 | STR. | 31'-1" | 33 | A193E | 1 | #5 | STR. | 35'-7" | 37 | A227 | 1 | #5 | STR. | 39'-11" | 42 | A286 | 2 | #5 | STR. | 22'-5" | 47 | A136E | 1 | #5 | STR. | 30'-0" | 31 | A194E | 1 | #5 | STR. | 34'-6" | 36 | A228 | 1 | #5 | STR. | 38'-10" | 41 | A287 | 2 | #5 | STR. | 21'-11" | 46 | A137E | 1 | #5 | STR. | 28'-11" | 30 | A195E | 1 | #5 | STR. | 33'-6" | 35 | A229 | 1 | #5 | STR. | 37'-8" | 39 | A288 | 2 | #5 | STR. | 21'-4" | 45 | A138E | 1 | #5 | STR. | 27'-9" | 29 | A196E | 1 | #5 | STR. | 32'-5" | 34 | A230 | 1 | #5 | STR. | 36'-7" | 38 | A289 | 1 | #5 | STR. | 39'-8" | 42 | A139E | 1 | #5 | STR. | 26'-8" | 28 | A197E | 1 | #5 | STR. | 31'-5" | 33 | A231 | 1 | #5 | STR. | 35'-6" | 37 | A290 | 1 | #5 | STR. | 38'-8" | 40 | A140E | 1 | #5 | STR. | 25'-7" | 27 | A198E | 1 | #5 | STR. | 30'-5" | 32 | A232 | 1 | #5 | STR. | 34'-5" | 36 | A291 | 1 | #5 | STR. | 37'-7" | 39 | A141E | 1 | #5 | STR. | 24'-6" | 26 | A199E | 1 | #5 | STR. | 29'-4" | 31 | A233 | 1 | #5 | STR. | 33'-4" | 35 | A292 | 1 | #5 | STR. | 36'-7" | 38 | A142E | 1 | #5 | STR. | 23'-4" | 24 | A200E | 1 | #5 | STR. | 28'-4" | 30 | A234 | 1 | #5 | STR. | 32'-2" | 34 | A293 | 1 | #5 | STR. | 35'-7" | 37 | A143E | 1 | #5 | STR. | 22'-3" | 23 | A201E | 1 | #5 | STR. | 27'-3" | 29 | A235 | 1 | #5 | STR. | 31'-1" | 33 | A294 | 1 | #5 | STR. | 34'-6" | 36 | A144E | 1 | #5 | STR. | 21'-2" | 22 | A202E | 1 | #5 | STR. | 26'-3" | 28 | A236 | 1 | #5 | STR. | 30'-0" | 31 | A295 | 1 | #5 | STR. | 33'-6" | 35 | A145E | 1 | #5 | STR. | 20'-1" | 21 | A203E | 1 | #5 | STR. | 25'-3" | 26 | A237 | 1 | #5 | STR. | 28'-11" | 30 | A296 | 1 | #5 | STR. | 32'-5" | 34 | A146E | 1 | #5 | STR. | 18'-11" | 20 | A204E | 1 | #5 | STR. | 24'-3" | 25 | A238 | 1 | #5 | STR. | 27'-9" | 29 | A297 | 1 | #5 | STR. | 31'-5" | 33 | A147E | 1 | #5 | STR. | 17'-10" | 19 | A205E | 1 | #5 | STR. | 23'-2" | 24 | A239 | 1 | #5 | STR. | 26'-8" | 28 | A298 | 1 | #5 | STR. | 30'-5" | 32 | A148E | 1 | #5 | STR. | 16'-9" | 18 | A206E | 1 | #5 | STR. | 22'-2" | 23 | A240 | 1 | #5 | STR. | 25'-7" | 27 | A299 | 1 | #5 | STR. | 29'-4" | 31 | A149E | 1 | #5 | STR. | 15'-8" | 16 | A207E | 1 | #5 | STR. | 21'-2" | 22 | A241 | 1 | #5 | STR. | 24'-6" | 26 | A300 | 1 | #5 | STR. | 28'-4" | 30 | A150E | 1 | #5 | STR. | 14'-7" | 15 | A208E | 1 | #5 | STR. | 20'-1" | 21 | A242 | 1 | #5 | STR. | 23'-4" | 24 | A301 | 1 | #5 | STR. | 27'-3" | 29 | A151E | 1 | #5 | STR. | 13'-5" | 14 | A209E | 1 | #5 | STR. | 19'-1" | 20 | A243 | 1 | #5 | STR. | 22'-3" | 23 | A302 | 1 | #5 | STR. | 26'-3" | 28 | A152E | 1 | #5 | STR. | 12'-4" | 13 | A210E | 1 | #5 | STR. | 18'-1" | 19 | A244 | 1 | #5 | STR. | 21'-2" | 22 | A303 | 1 | #5 | STR. | 25'-3" | 26 | A153E | 1 | #5 | STR. | 11'-3" | 12 | A211E | 1 | #5 | STR. | 17'-0" | 18 | A245 | 1 | #5 | STR. | 20'-1" | 21 | A304 | 1 | #5 | STR. | 24'-3" | 25 | A154E | 1 | #5 | STR. | 10'-2" | 11 | A212E | 1 | #5 | STR. | 16'-0" | 17 | A246 | 1 | #5 | STR. | 18'-11" | 20 | A305 | 1 | #5 | STR. | 23'-2" | 24 | A155E | 1 | #5 | STR. | 9'-0" | 10 | A213E | 1 | #5 | STR. | 14'-11" | 16 | A247 | 1 | #5 | STR. | 17'-10" | 19 | A306 | 1 | #5 | STR. | 22'-2" | 23 | A156E | 1 | #5 | STR. | 7'-11" | 8 | A214E | 1 | #5 | STR. | 13'-11" | 15 | A248 | 1 | #5 | STR. | 16'-9" | 18 | A307 | 1 | #5 | STR. | 21'-2" | 22 | A215E | 1 | #5 | STR. | 12'-11" | 14 | A249 | 1 | #5 | STR. | 15'-8" | 16 | A308 | 1 | #5 | STR. | 20'-1" | 21 |



REINFORCING STEEL 54,252

EPOXY COATED REINFORCING STEEL 56,190

"E" DENOTES EPOXY COATED REINFORCING STEEL.

SUPERSTRUCTURE BILL OF MATERIAL

| SPANS | CLASS AA CONCRETE | REINFORCING STEEL | EPOXY COATED REINFORCING STEEL |
|----------------|-------------------|-------------------|--------------------------------|
| | (CU. YDS.) | (LBS.) | (LBS.) |
| "A", "B" & "C" | | 54,252 | 56,190 |
| POUR 1 | 114.2 | | |
| POUR 2 | 227.7 | | |
| POUR 3 | 169.2 | | |
| SIDEWALKS | 45.6 | | |
| **TOTALS | 556.7 | 54,252 | 56,190 |

**QUANTITIES FOR END POSTS & PARAPETS ARE INCLUDED ON "RAIL POST SPACING AND END RAIL DETAILS" SHEET.

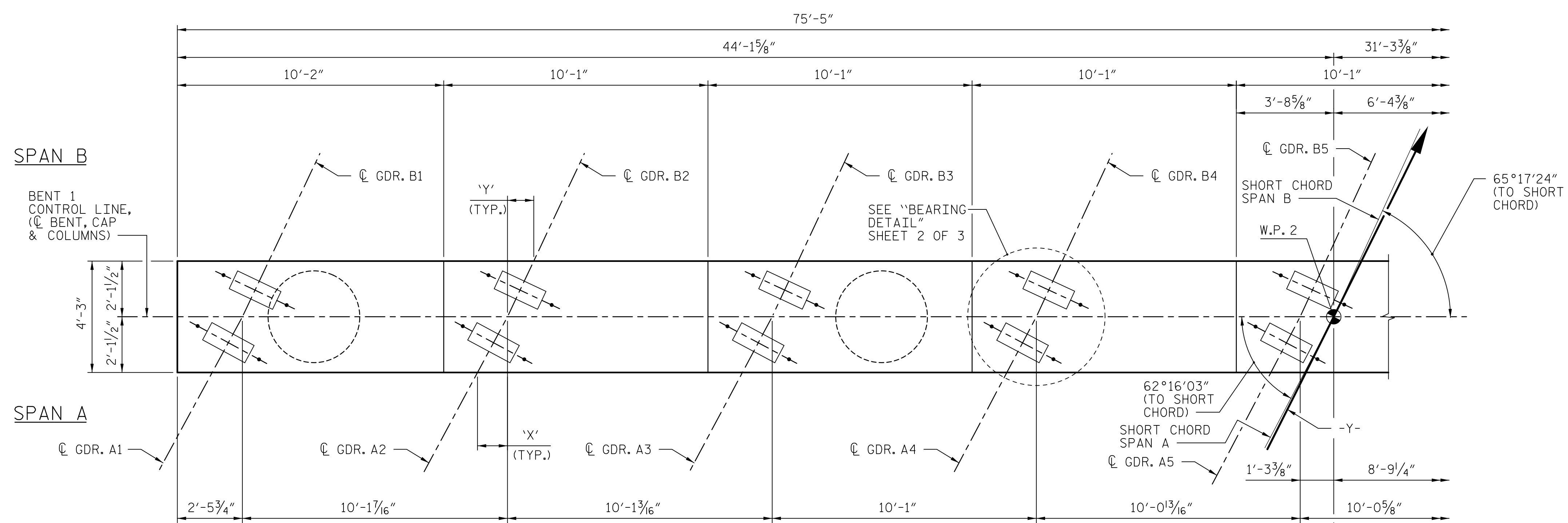
QUANTITIES FOR APPROACH SLABS ARE INCLUDED ON "BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT" SHEET.

PROJECT NO. U-2707

FORSYTH COUNTY

STATION: 17+34.88 -Y-

BRIDGE NO. 109



NOTES:

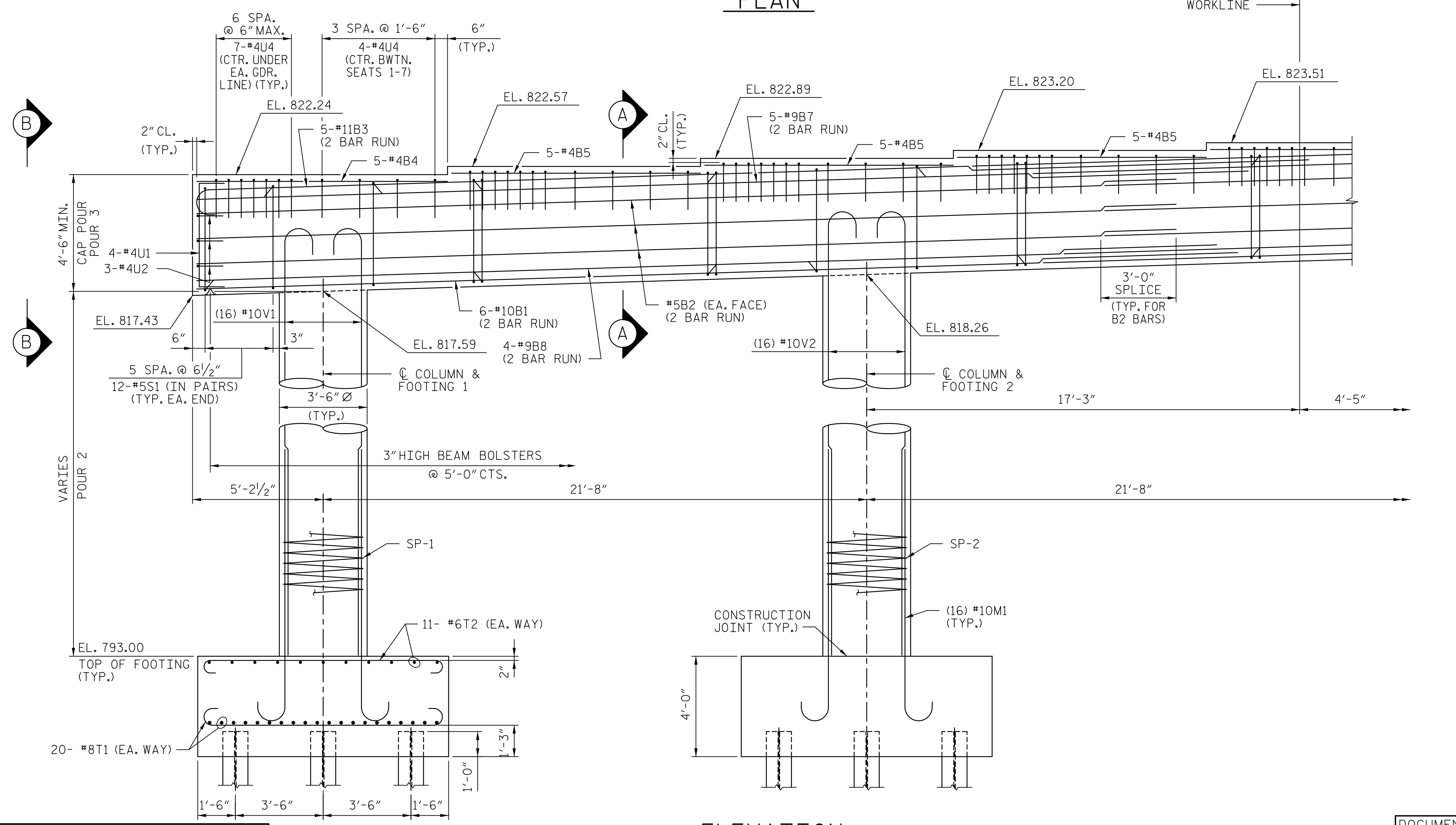
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS.

HOOKS ON "V" BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR OTHER NOTES, SEE "FOUNDATION LAYOUT" SHEET AND "LONG CHORD LAYOUT" SHEET.

FOR PILE SPLICE DETAILS, SEE "END BENT" SHEETS.

| GIRDER | 'X' | 'Y' |
|--------|------------|-----------|
| 1 | 1'-1 7/8" | 1'-0 1/8" |
| 2 | 1'-1 3/4" | 1'-0" |
| 3 | 1'-1 5/8" | 11 5/16" |
| 4 | 1'-1 1/2" | 11 3/16" |
| 5 | 1'-1 7/16" | 11 3/4" |
| 6 | 1'-1 5/16" | 11 1/16" |
| 7 | 1'-1 3/16" | 11 9/16" |
| 8 | 1'-1 1/8" | 11 1/2" |



DATE: 1/12/2016 TIME: 8:55:02 AM

USER: N:\a\compton DATE: 1/12/2016 TIME: 8:55:02 AM

DRAWN BY: DDL DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

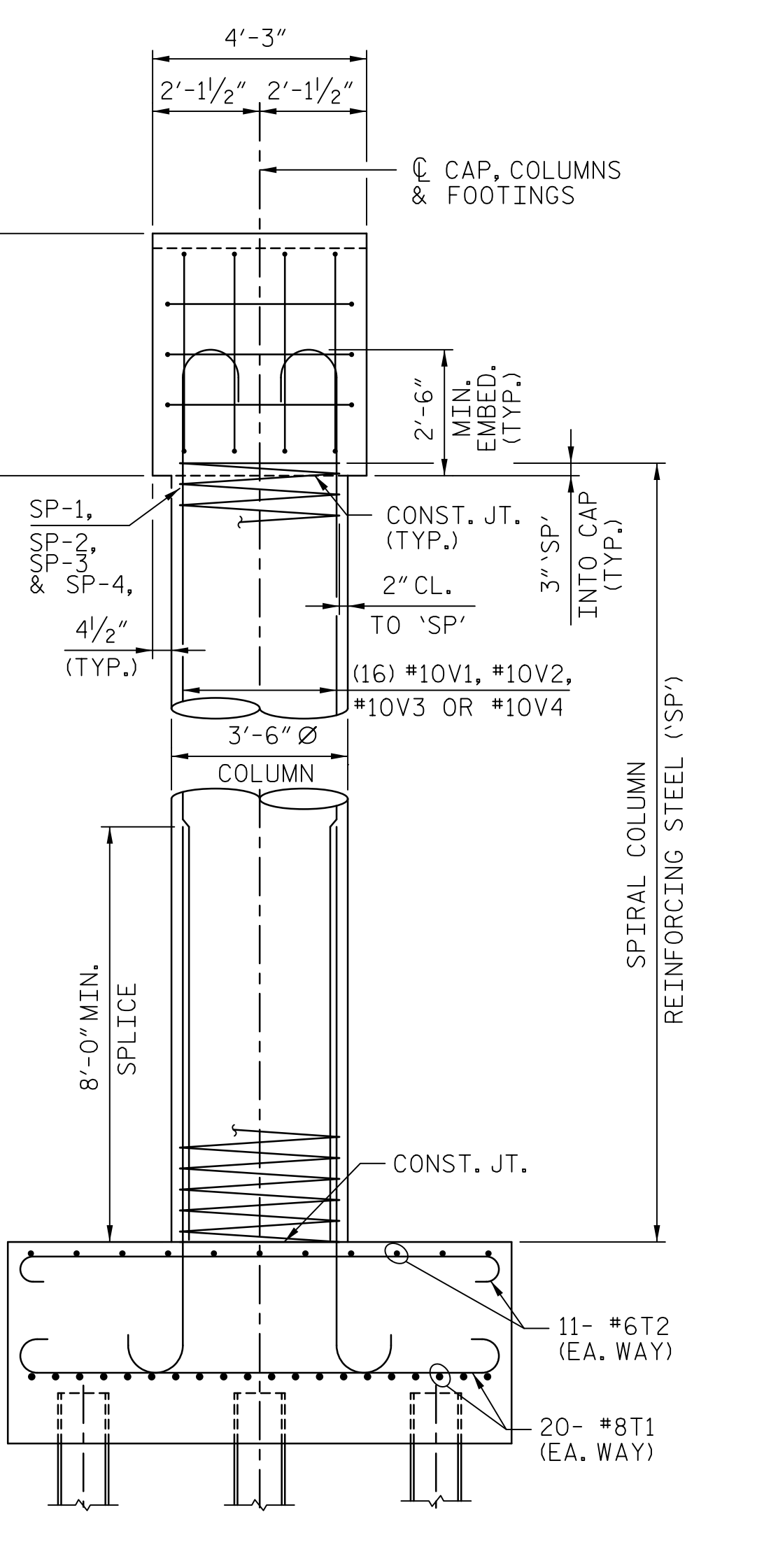
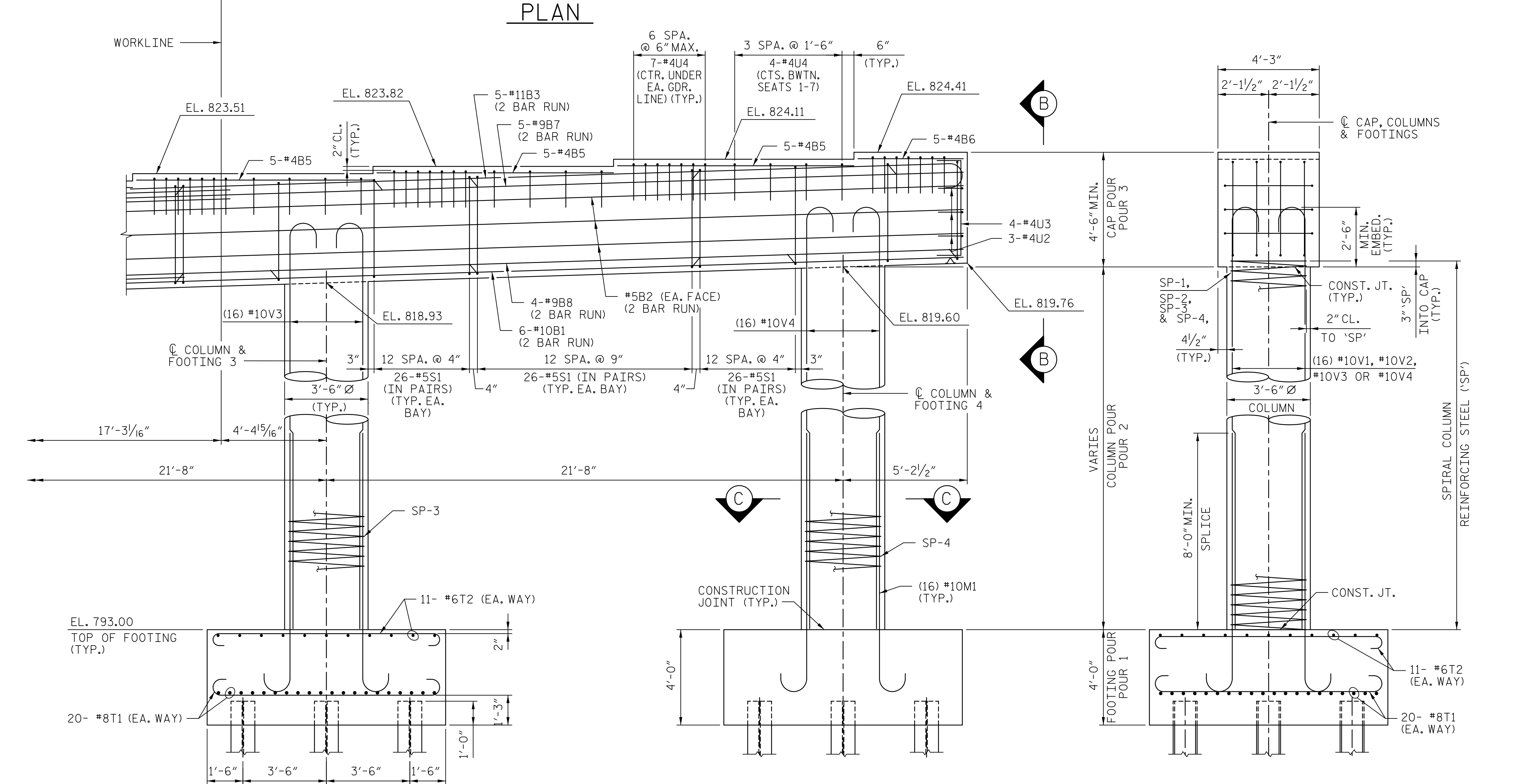
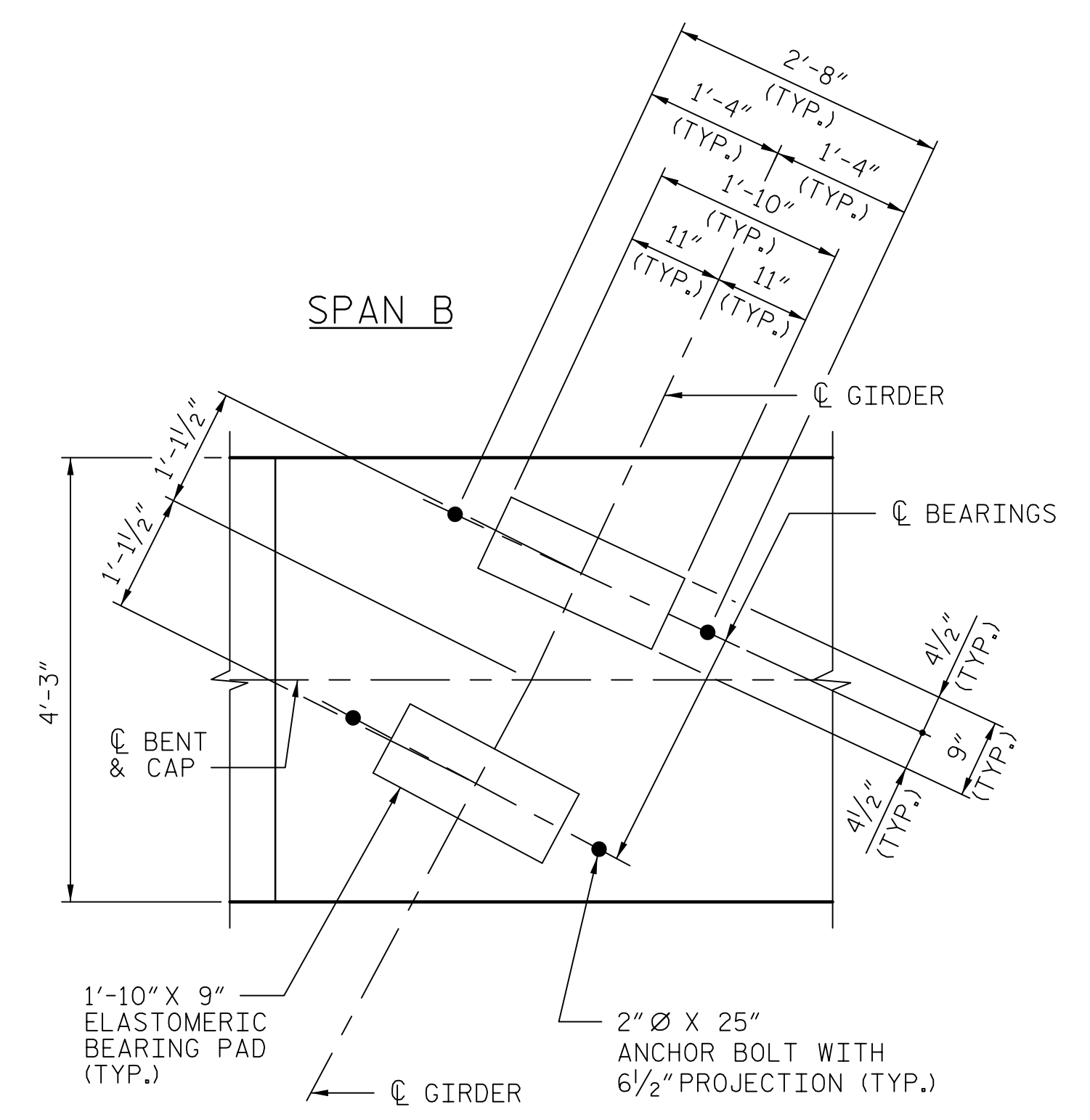
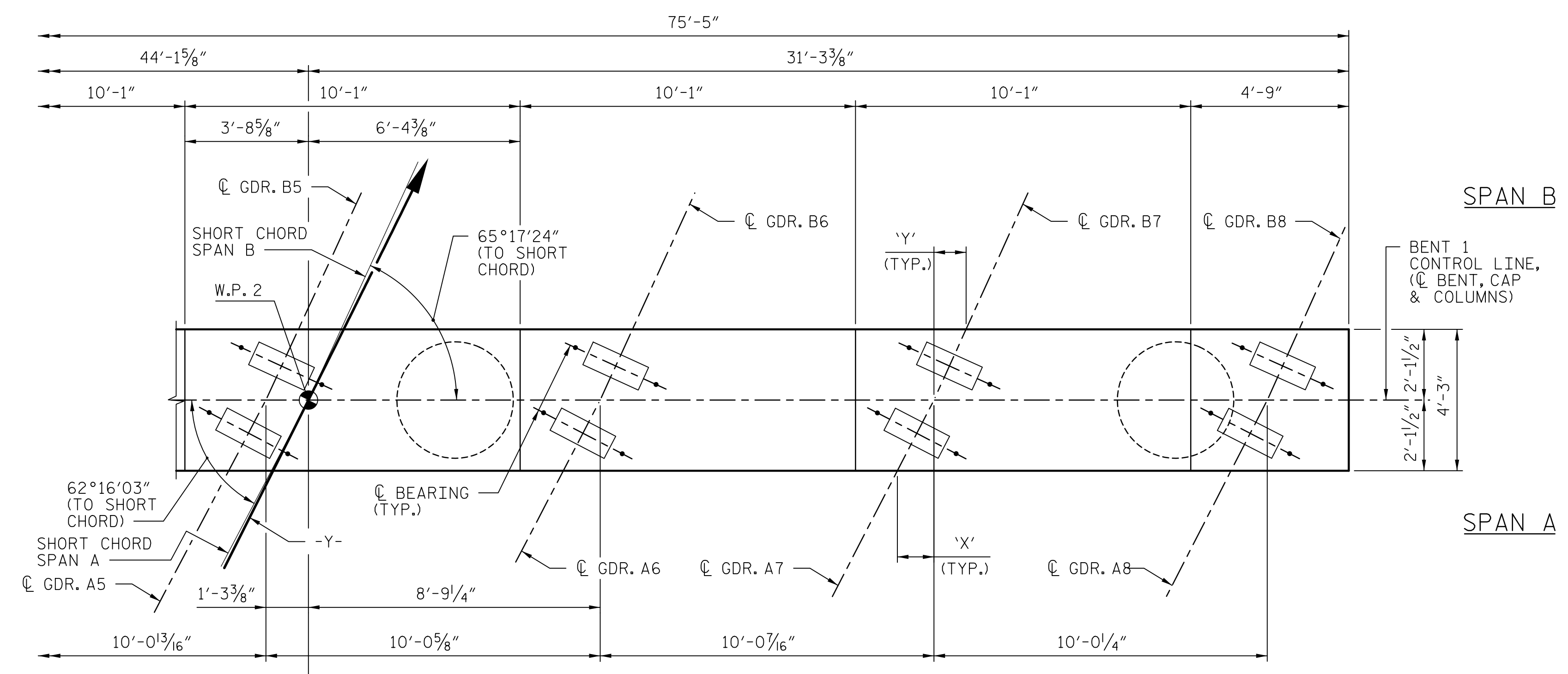


PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 1 OF 3 BRIDGE NO. 109

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 BENT 1
 PLAN AND ELEVATION

| REVISIONS | | | | | | SHEET NO. S-72 |
|-----------|-----|-------|-----|-----|-------|--------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | |
| | | | | | | TOTAL SHEETS 86 |



DATE: 1/12/2016
TIME: 8:55:05 AM

USER: A:\aecom\p\1010402\06_L12707_SML_573_B1_02.dgn
DGN: A:\aecom\p\1010402\06_L12707_SML_573_B1_02.dgn

DRAWN BY: DDL DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 2 OF 3 BRIDGE NO. 109

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-6200 www.aecom.com
AECOM License No. F-0342

SEAL
1010402
1/12/2016
JOHN C. MORRIS
REGISTERED PROFESSIONAL ENGINEER

1/12/2016

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
BENT 1
PLAN AND ELEVATION

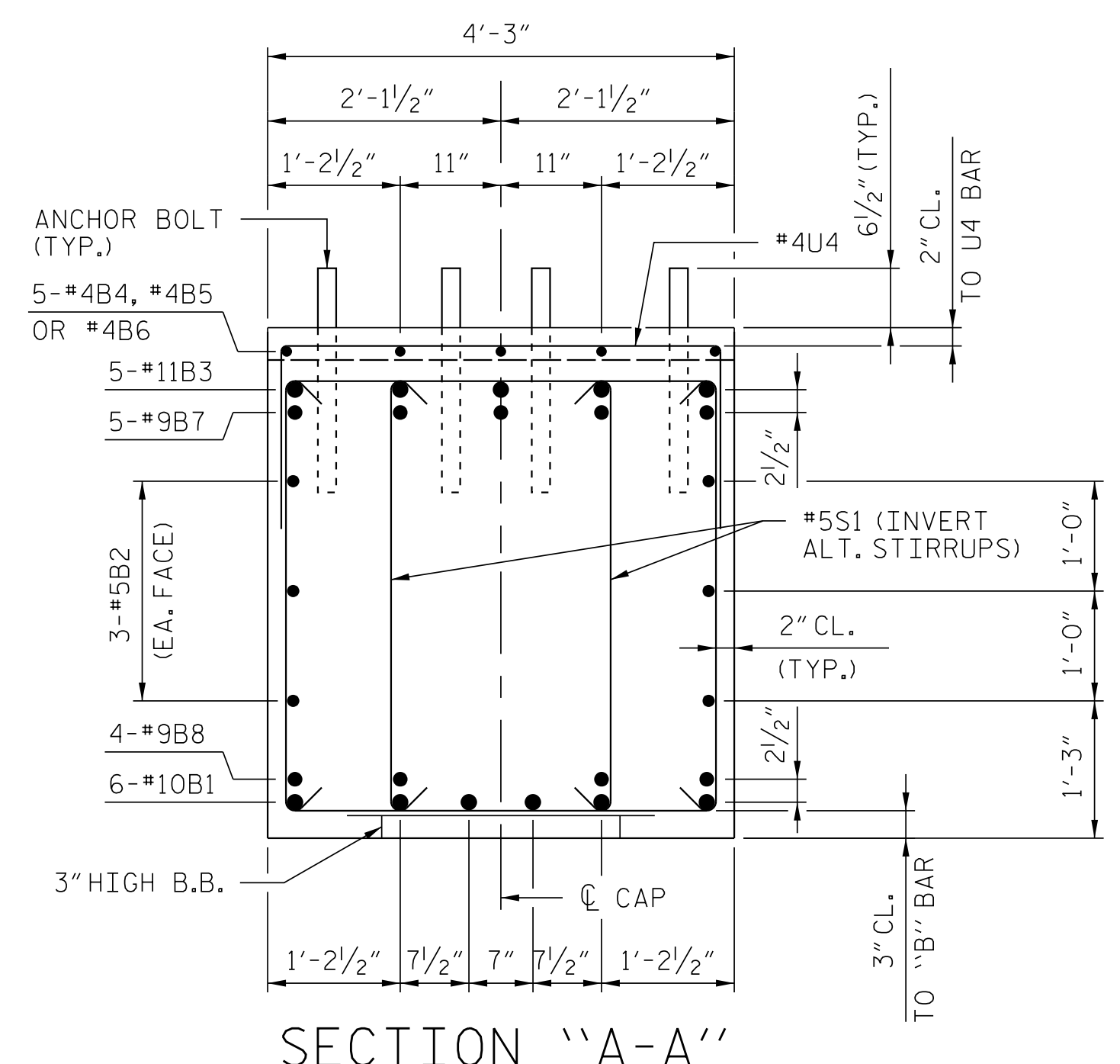
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. S-73
TOTAL SHEETS 86

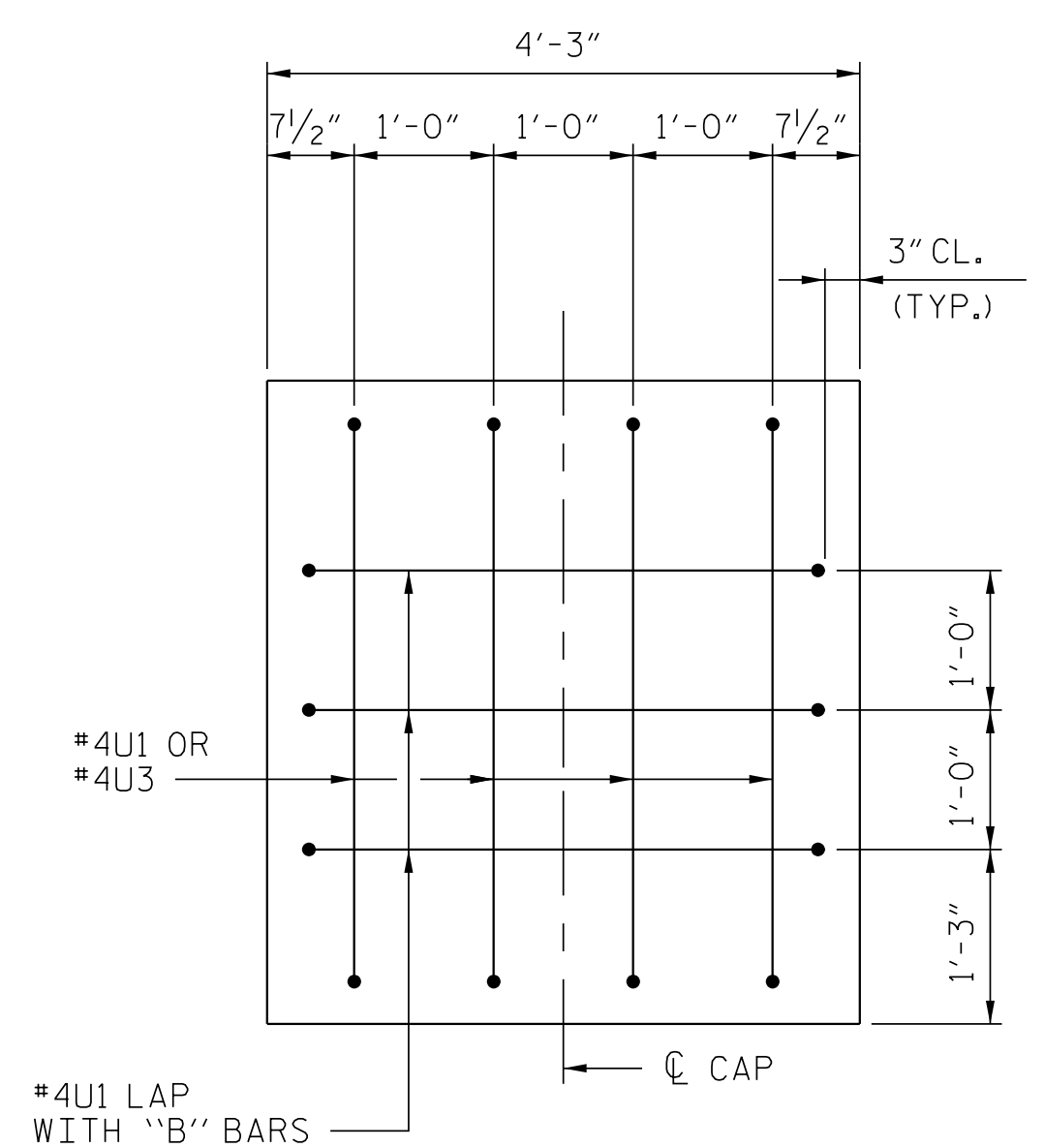
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DATE: 3/1/2016
TIME: 2:40:57 PM

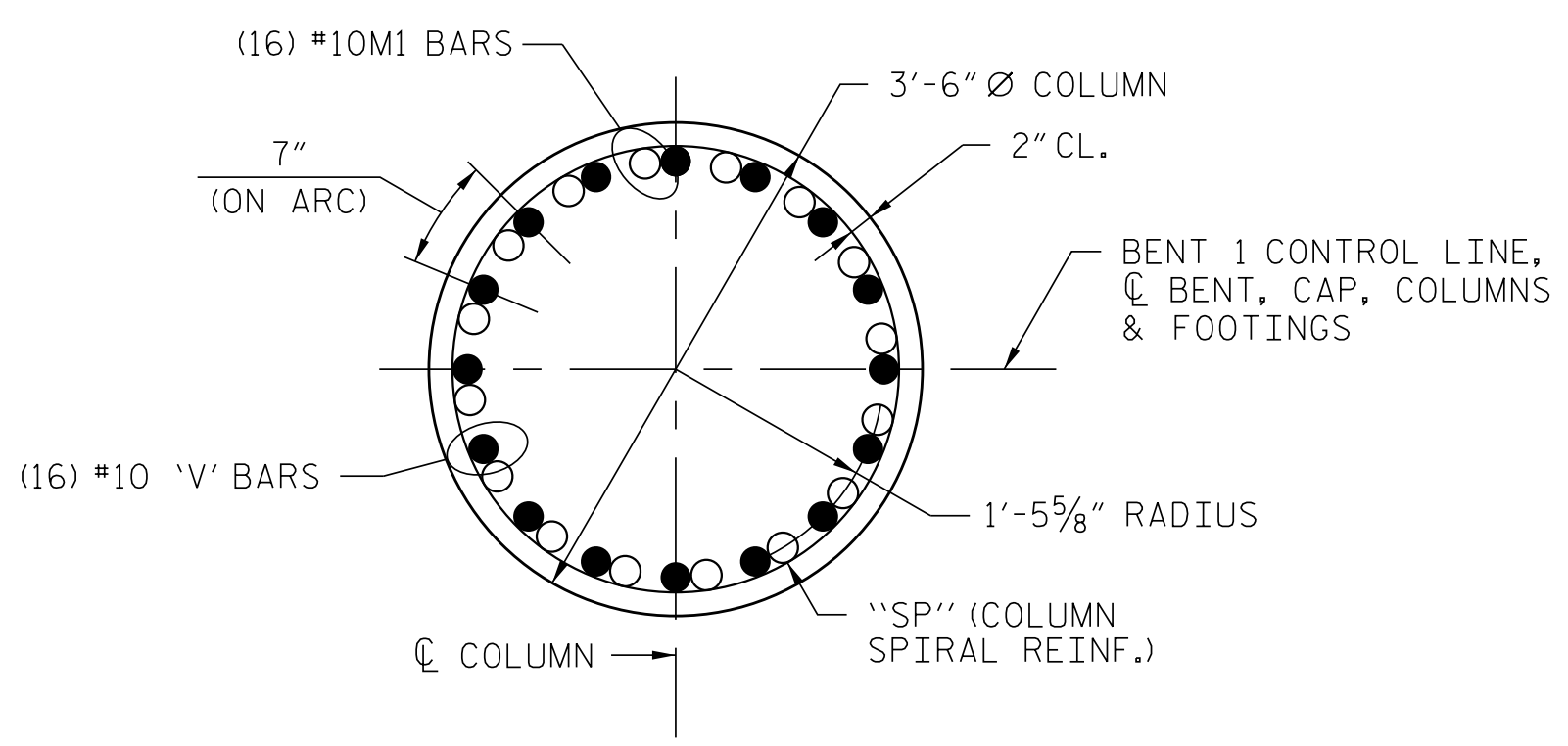
USER: MCF150C
DGN: R:\644626\400_Technical\450_MCDOT_S14_Tech_Design_Files\Structures\Bridges\09\Coord\A02_063_U2707_SML_S14_B1_03.dgn



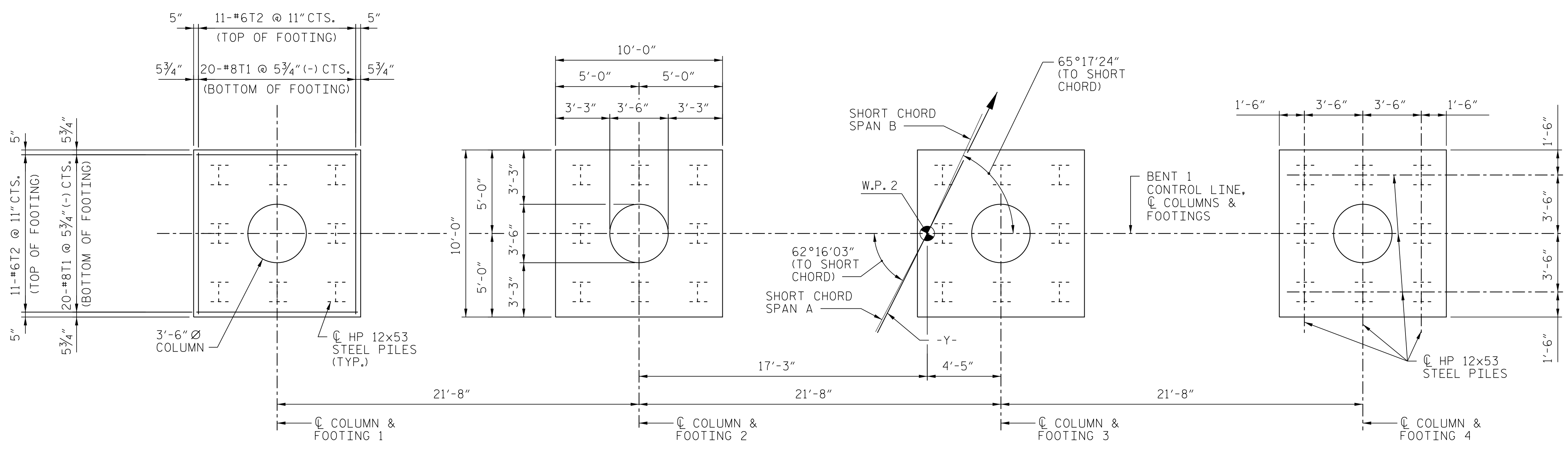
SECTION "A-A"



END VIEW "B-B"
(RIGHT END SHOWN, LEFT END SIMILAR)



SECTION "C-C"



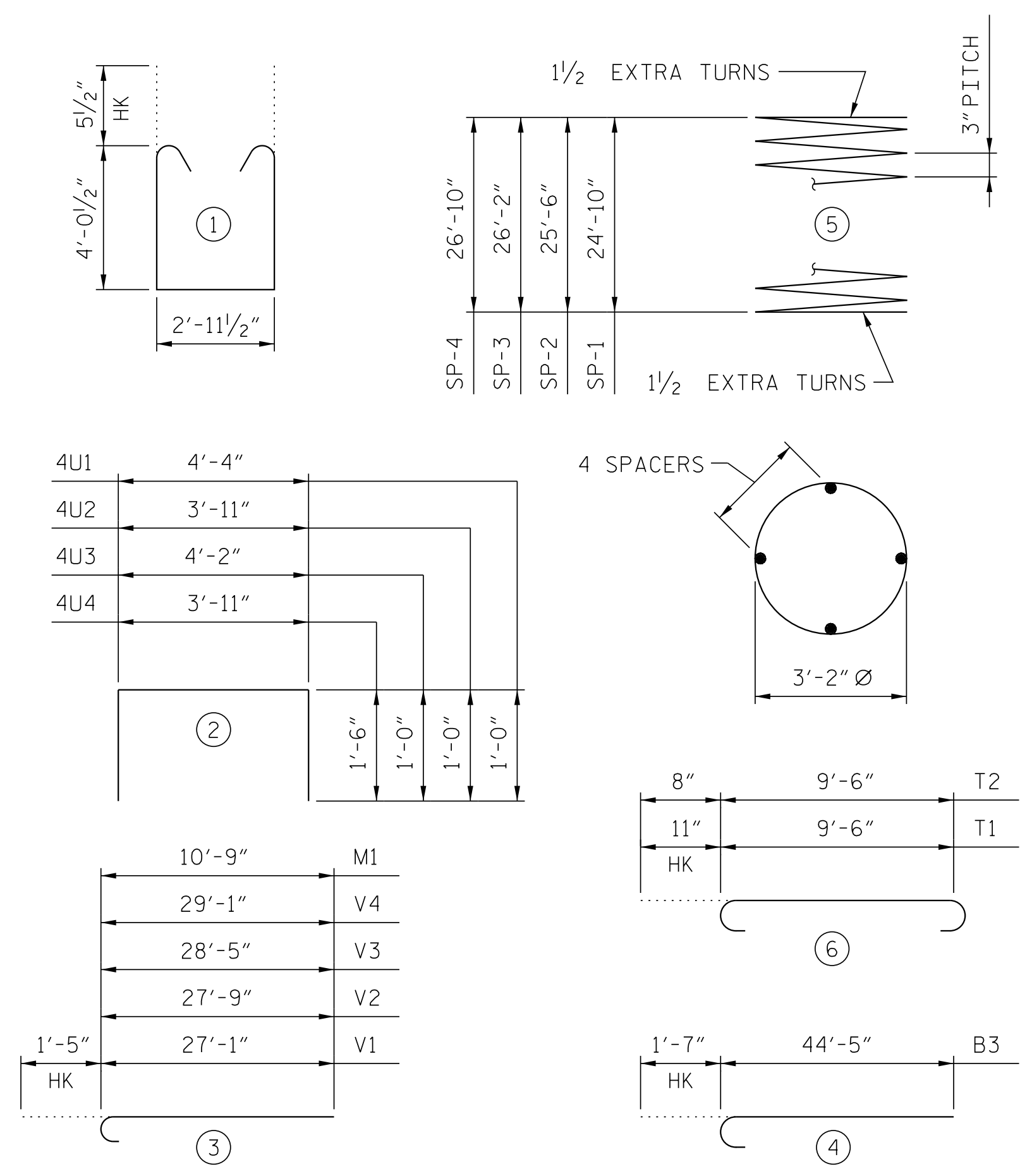
PLAN OF FOOTINGS

| SPLICE LENGTHS | | |
|----------------|---------|--------|
| BAR SIZE | TOP BAR | BASIC |
| #9 | 8'-9" | 6'-3" |
| #10 | 11'-1" | 7'-11" |
| #11 | 13'-7" | 9'-9" |

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

BAR TYPES

ALL BAR DIMENSIONS ARE OUT TO OUT.



BILL OF MATERIAL

BENT 1

| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
|-----|-----|------|------|---------|--------|
| B1 | 12 | #10 | STR | 41'-7" | 2147 |
| B2 | 12 | #5 | STR | 39'-2" | 489 |
| B3 | 10 | #11 | (4) | 46'-0" | 2445 |
| B4 | 5 | #4 | STR | 10'-0" | 33 |
| B5 | 30 | #4 | STR | 9'-11" | 199 |
| B6 | 5 | #4 | STR | 4'-5" | 15 |
| B7 | 10 | #9 | STR | 41'-11" | 1425 |
| B8 | 8 | #9 | STR | 40'-8" | 1106 |
| M1 | 64 | #10 | (3) | 12'-2" | 3351 |
| S1 | 258 | #5 | (1) | 12'-0" | 3229 |
| T1 | 160 | #8 | (6) | 11'-4" | 4842 |
| T2 | 88 | #6 | (6) | 10'-10" | 1432 |
| U1 | 4 | #4 | (2) | 6'-4" | 16 |
| U2 | 6 | #4 | (2) | 5'-11" | 24 |
| U3 | 4 | #4 | (2) | 6'-2" | 17 |
| U4 | 84 | #4 | (2) | 6'-11" | 388 |
| V1 | 16 | #10 | (3) | 28'-6" | 1962 |
| V2 | 16 | #10 | (3) | 29'-2" | 2008 |
| V3 | 16 | #10 | (3) | 29'-10" | 2054 |
| V4 | 16 | #10 | (3) | 30'-6" | 2100 |

| REINFORCING STEEL | | LBS. | | 29282 | |
|-------------------|---|------|-----|-----------|-----|
| SP-1 | 1 | *** | (5) | 1004'-11" | 671 |
| SP-2 | 1 | *** | (5) | 1031'-2" | 689 |
| SP-3 | 1 | *** | (5) | 1057'-5" | 706 |
| SP-4 | 1 | *** | (5) | 1083'-6" | 724 |

| SPIRAL REINFORCING STEEL | | LBS. | | 2790 | |
|---------------------------------------------------------------------------------------------------------|--|------|--|------|--|
| *** THE "SP" SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR. | | | | | |

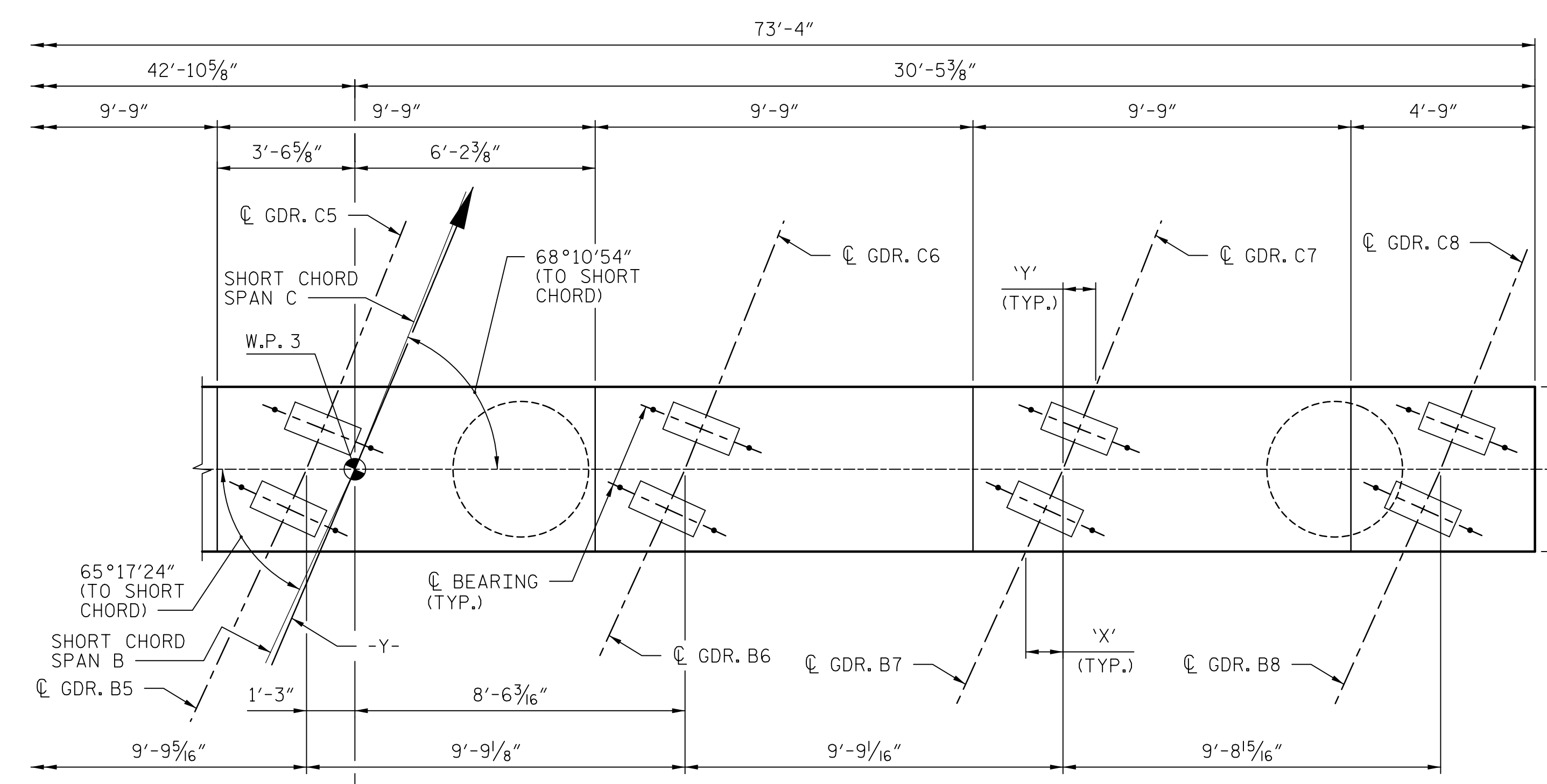
BENT 1 TOTAL QUANTITIES

| CLASS A CONCRETE | | |
|-----------------------------------|----------|------|
| POUR 1 (FOOTINGS) | C.Y. | 59.2 |
| POUR 2 (COLUMNS) | C.Y. | 36.5 |
| POUR 3 (CAP) | C.Y. | 55.5 |
| TOTAL CLASS A CONCRETE C.Y. 151.2 | | |
| HP 12 x 53 STEEL PILES NO. 32 | LIN. FT. | 640 |
| STEEL PILE POINTS | EA. | 32 |

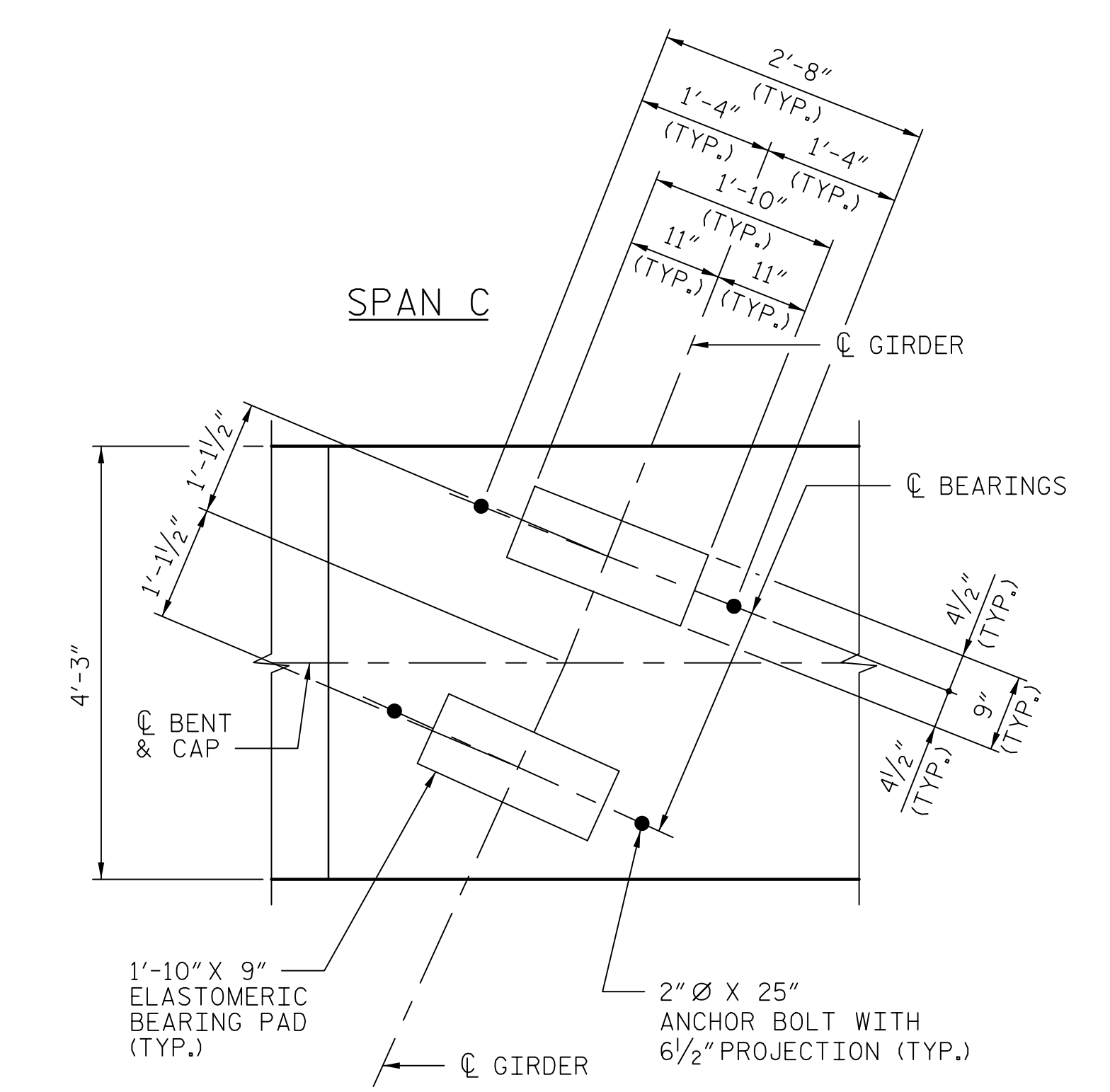
PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 3 OF 3 BRIDGE NO. 109

| REVISIONS | | | | | | SHEET NO. | |
|-----------|-----|-------|-----|-----|-------|-----------------|--|
| NO. | BY: | DATE: | NO. | BY: | DATE: | S-74 | |
| | | | | | | TOTAL SHEETS 86 | |

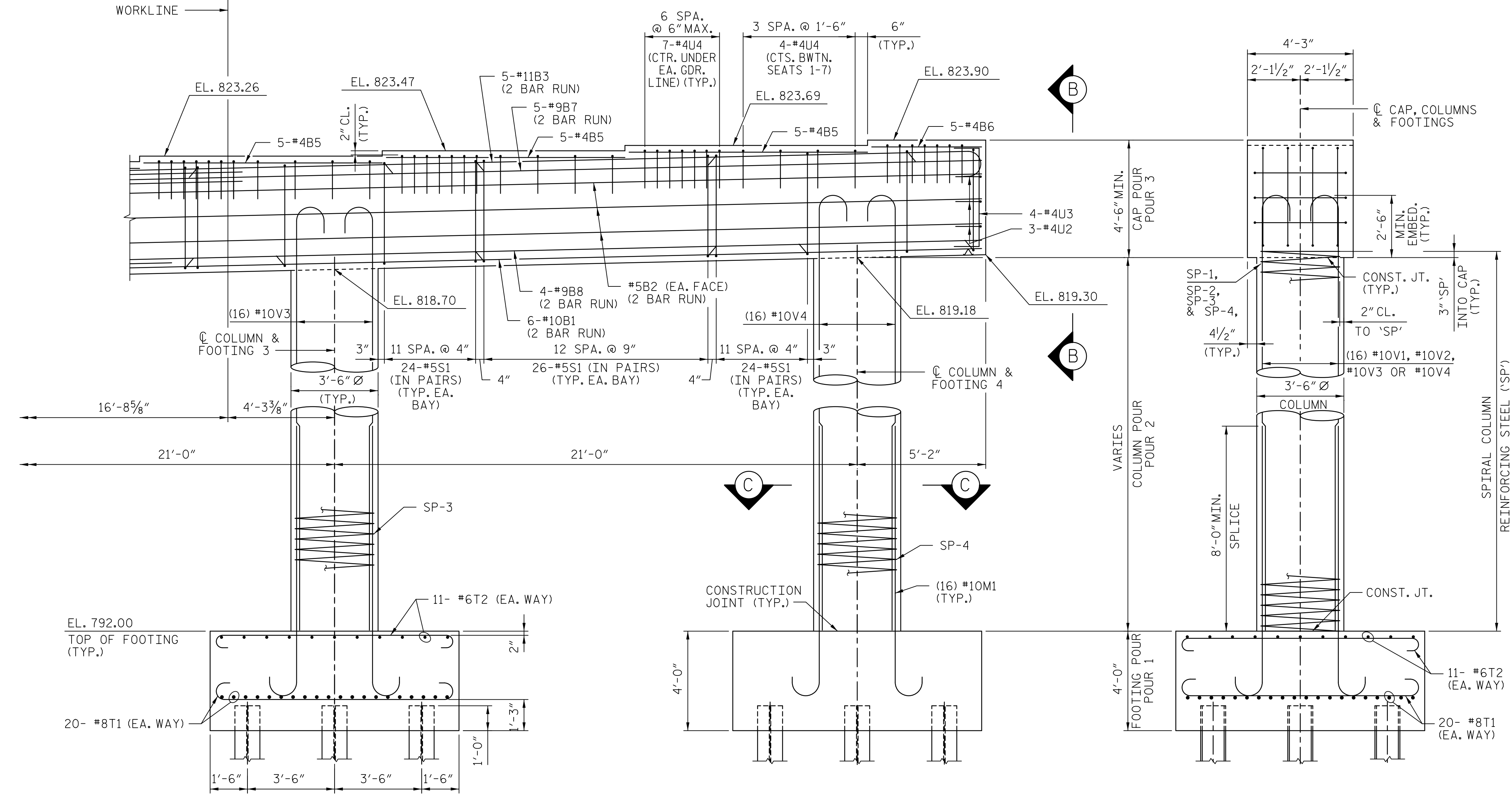
DRAWN BY: DDL DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015



PLAN



BEARING DETAIL



ELEVATION

END VIEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 2 OF 3 BRIDGE NO. 109

AECOM
 AECOM TECHNICAL SERVICES, INC.
 701 CORPORATE CENTER DRIVE, SUITE 475
 RALEIGH, NC 27607
 (919) 854-6200 www.aecom.com
 AECOM License No. F-0342

SEAL
 NORTH CAROLINA PROFESSIONAL ENGINEER
 JOHN C. MORRIS
 1/12/2016

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 BENT 2
 PLAN AND ELEVATION

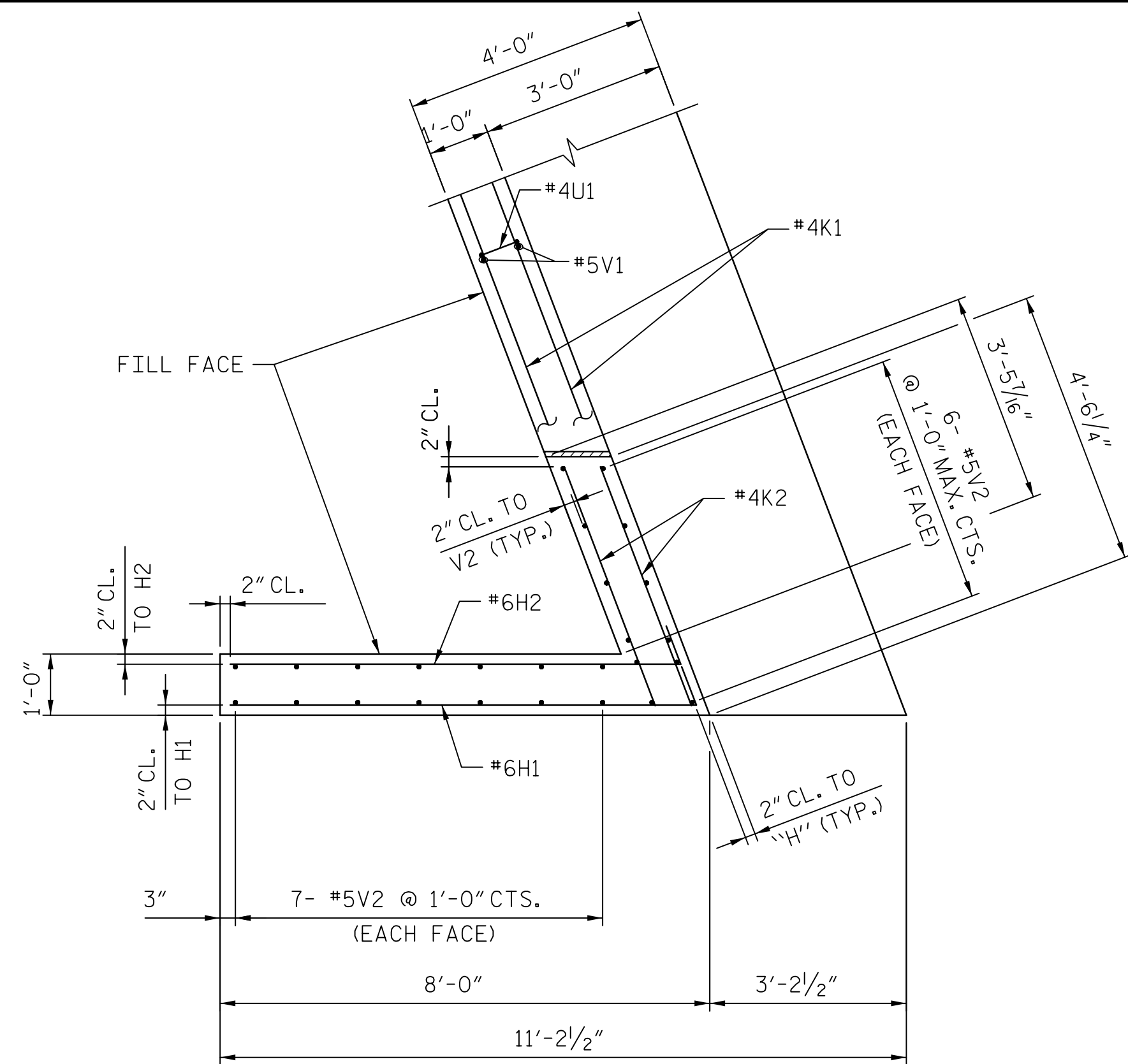
| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |

SHEET NO. **S-76**
 TOTAL SHEETS **86**

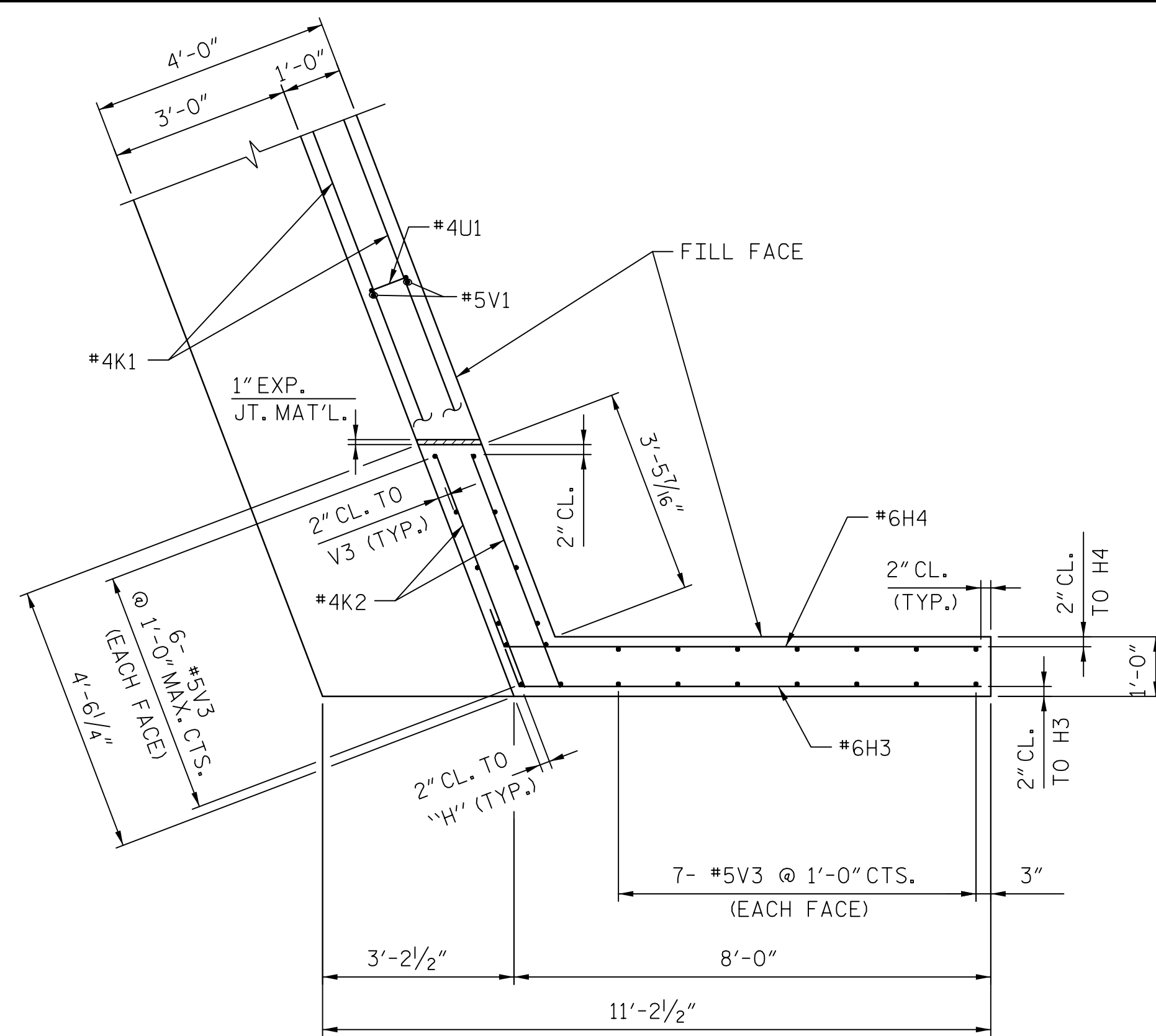
DATE: 1/12/2016
 TIME: 8:55:46 AM

USER: jcm
 DGN: R:\Bent\109\109-CAD\BENT_172707_SML-576_B2_02.dgn

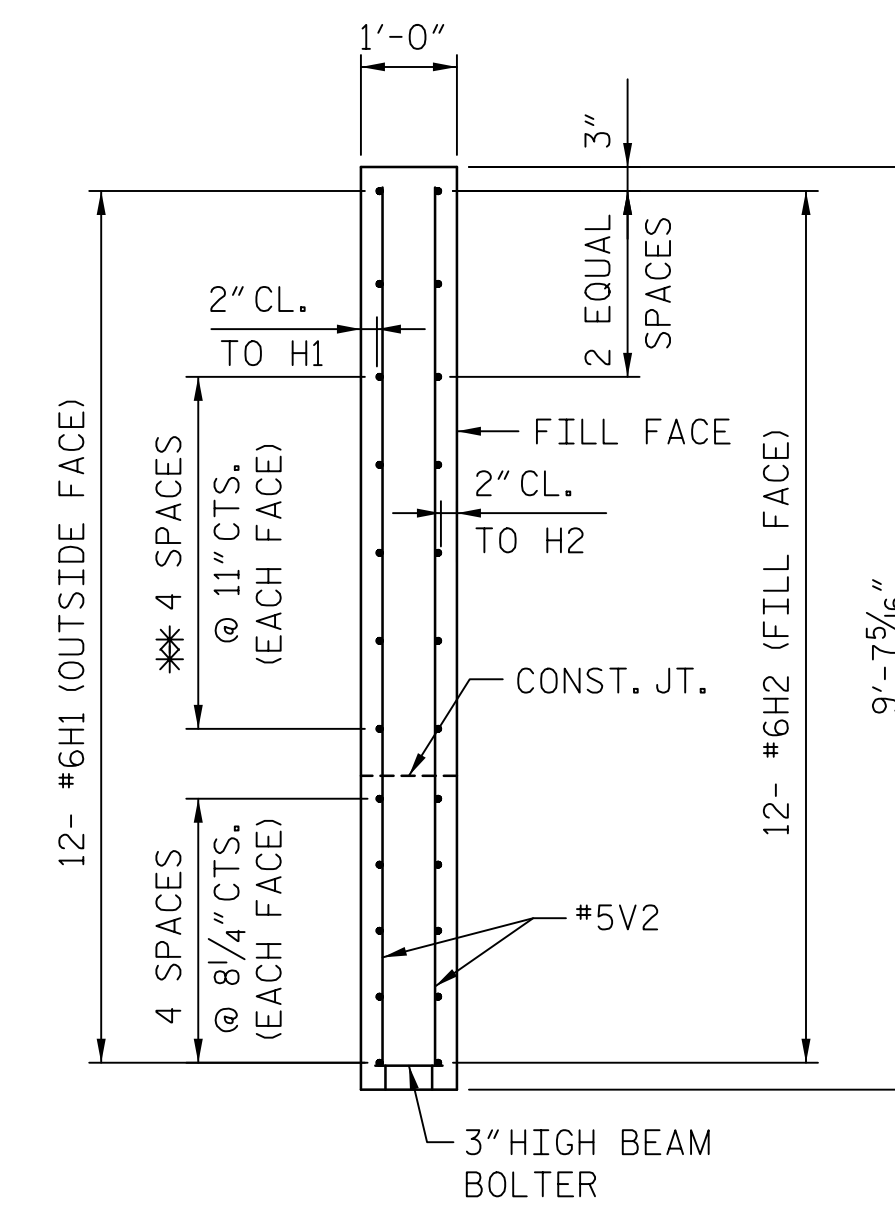
DRAWN BY: DDL DATE: 11/2015
 CHECKED BY: JCM DATE: 11/2015



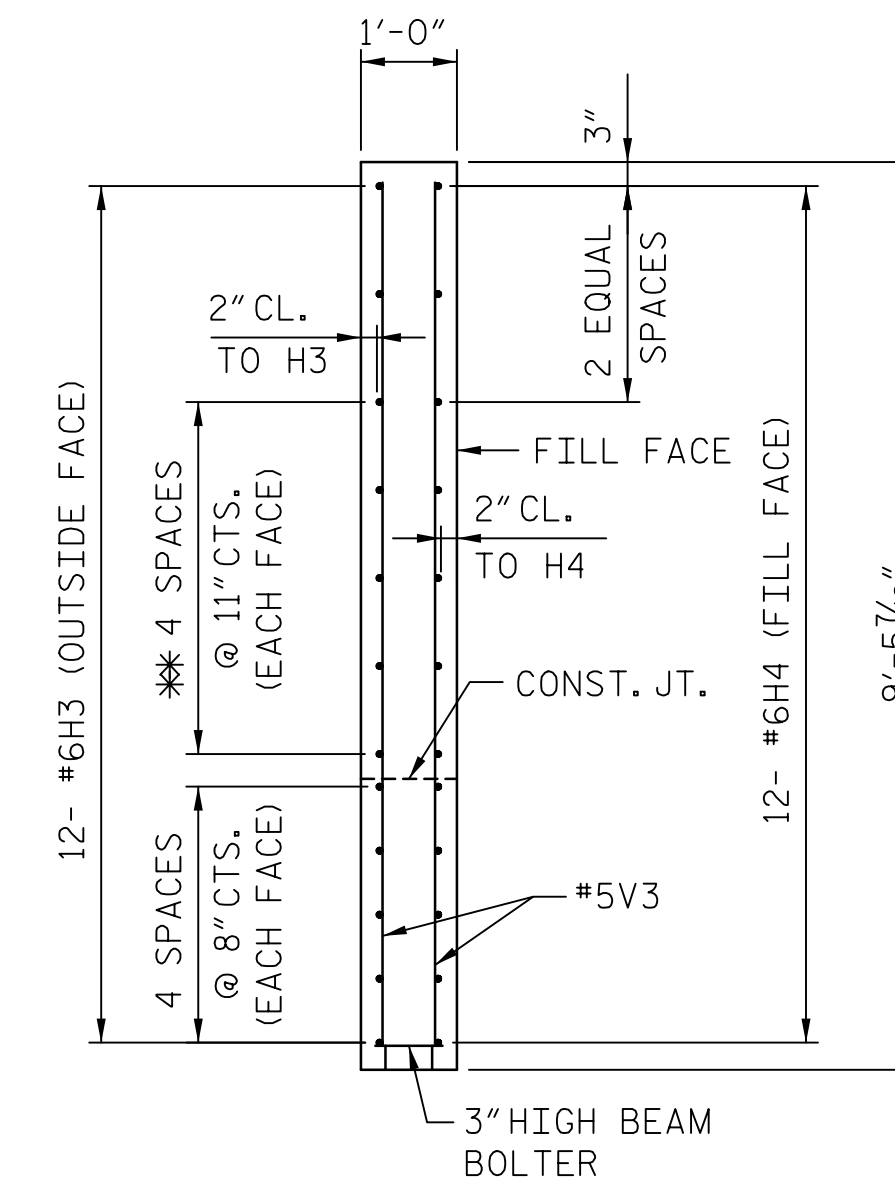
PLAN - WINGWALL 1



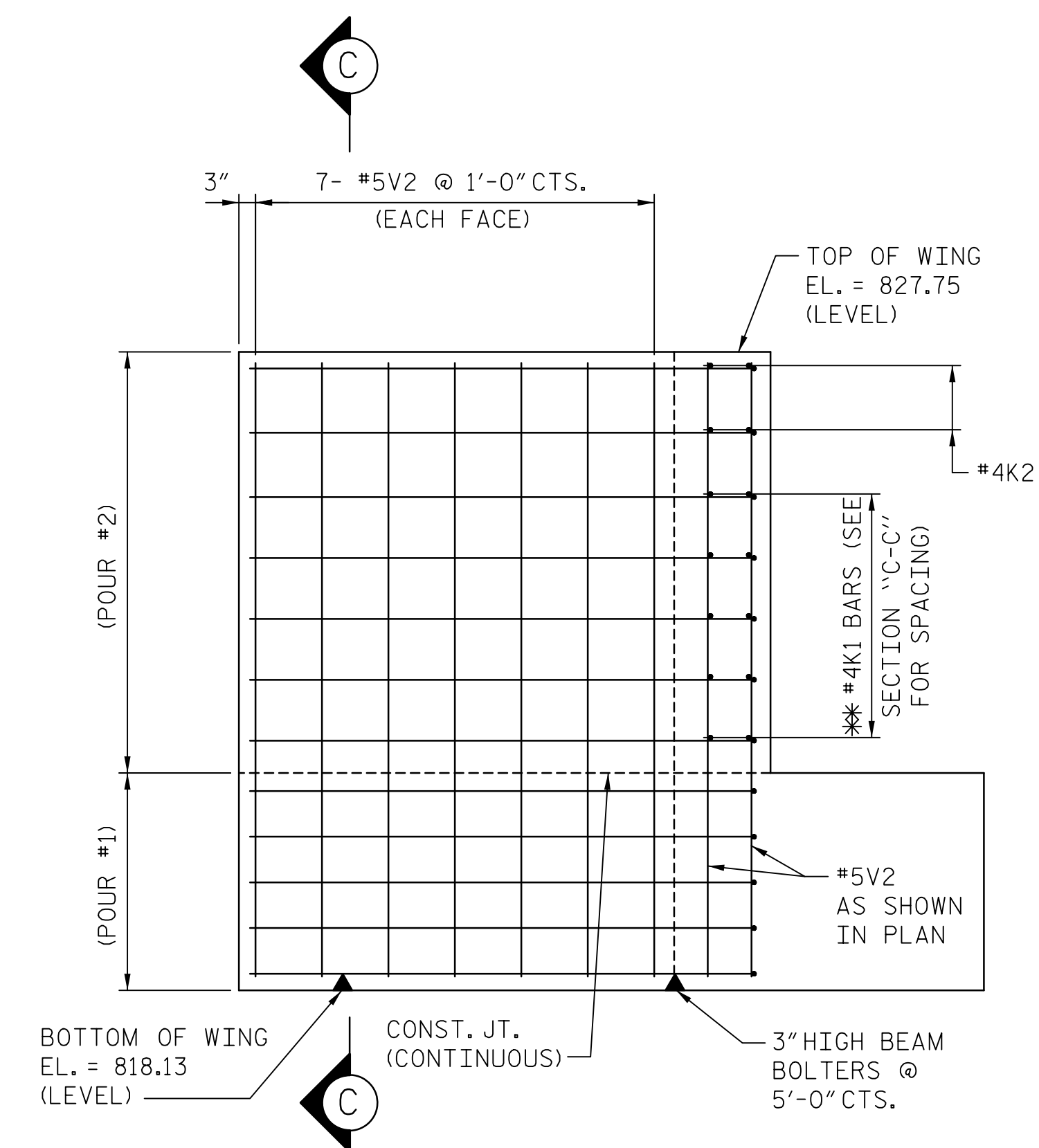
PLAN - WINGWALL 2



SECTION "C-C"

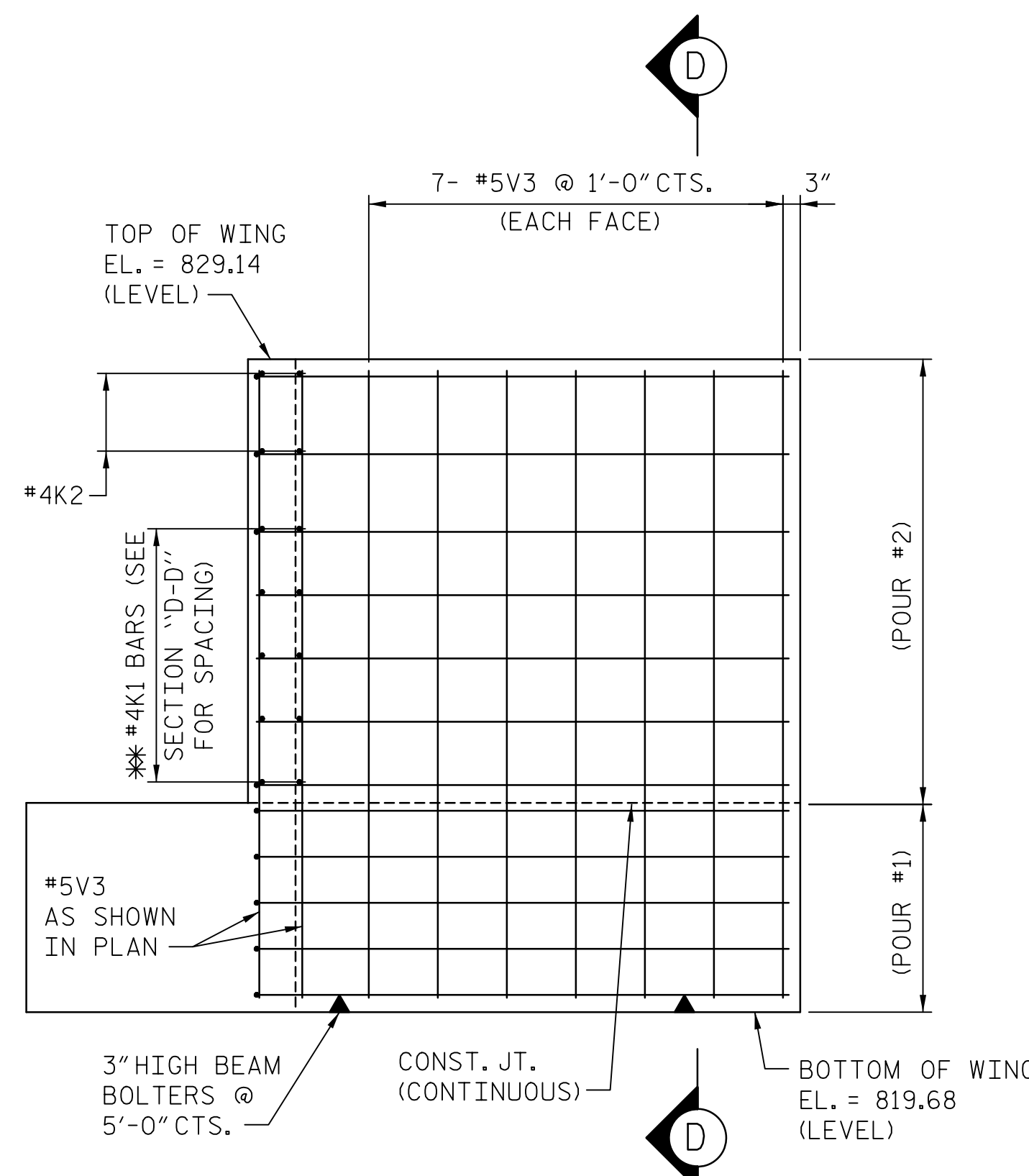


SECTION "D-D"



ELEVATION - WINGWALL 1

** PLACE #6 'H' BARS TO MATCH #4K1 BARS IN BACKWALL



ELEVATION - WINGWALL 2

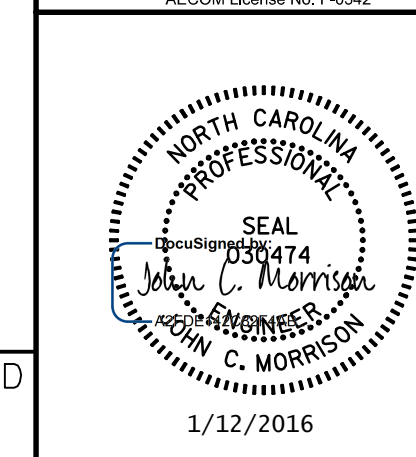
** PLACE #6 'H' BARS TO MATCH #4K1 BARS IN BACKWALL

NOTES:
SEE SHEET 4 OF 4 FOR NOTES.

DATE: 1/12/2016
TIME: 8:55:35 AM

USER: A:\a\compton
DGN: A:\Bridges\009\040402\075_U2707_SML_S90_EB2_03.dgn

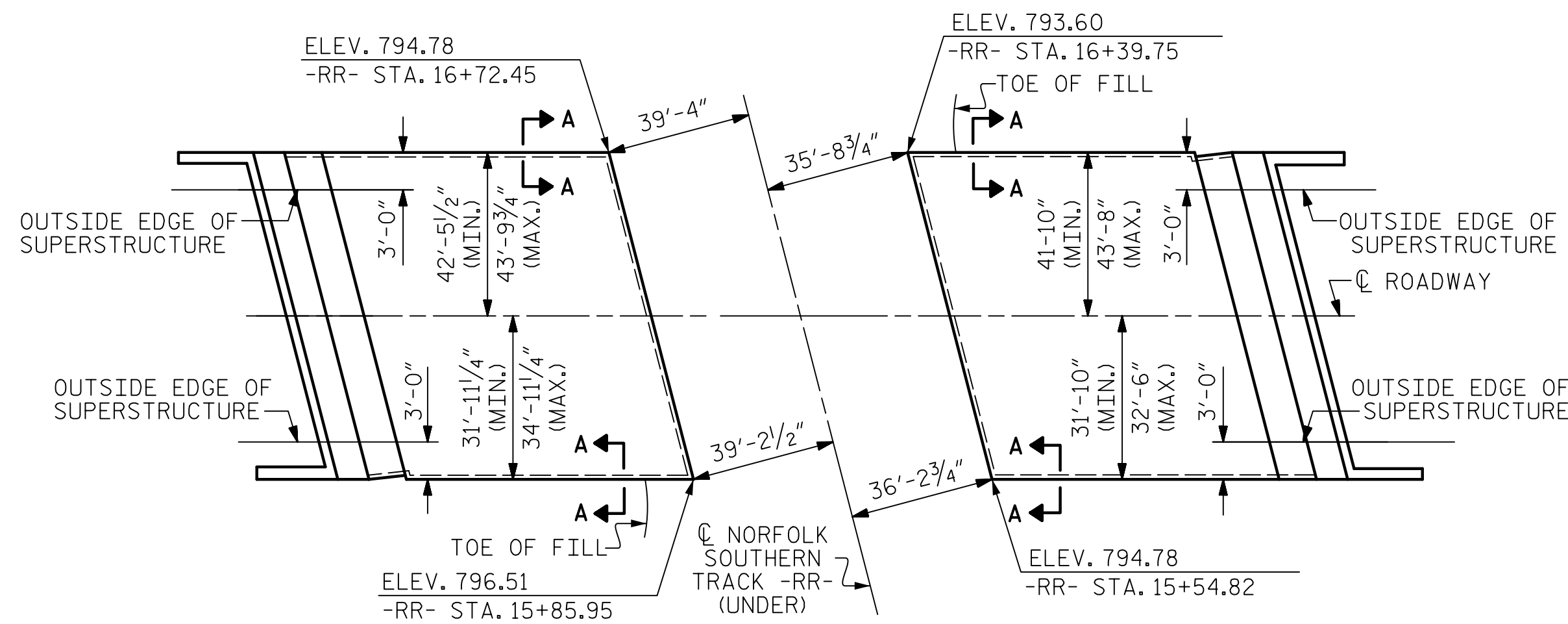
PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 3 OF 4 BRIDGE NO. 109



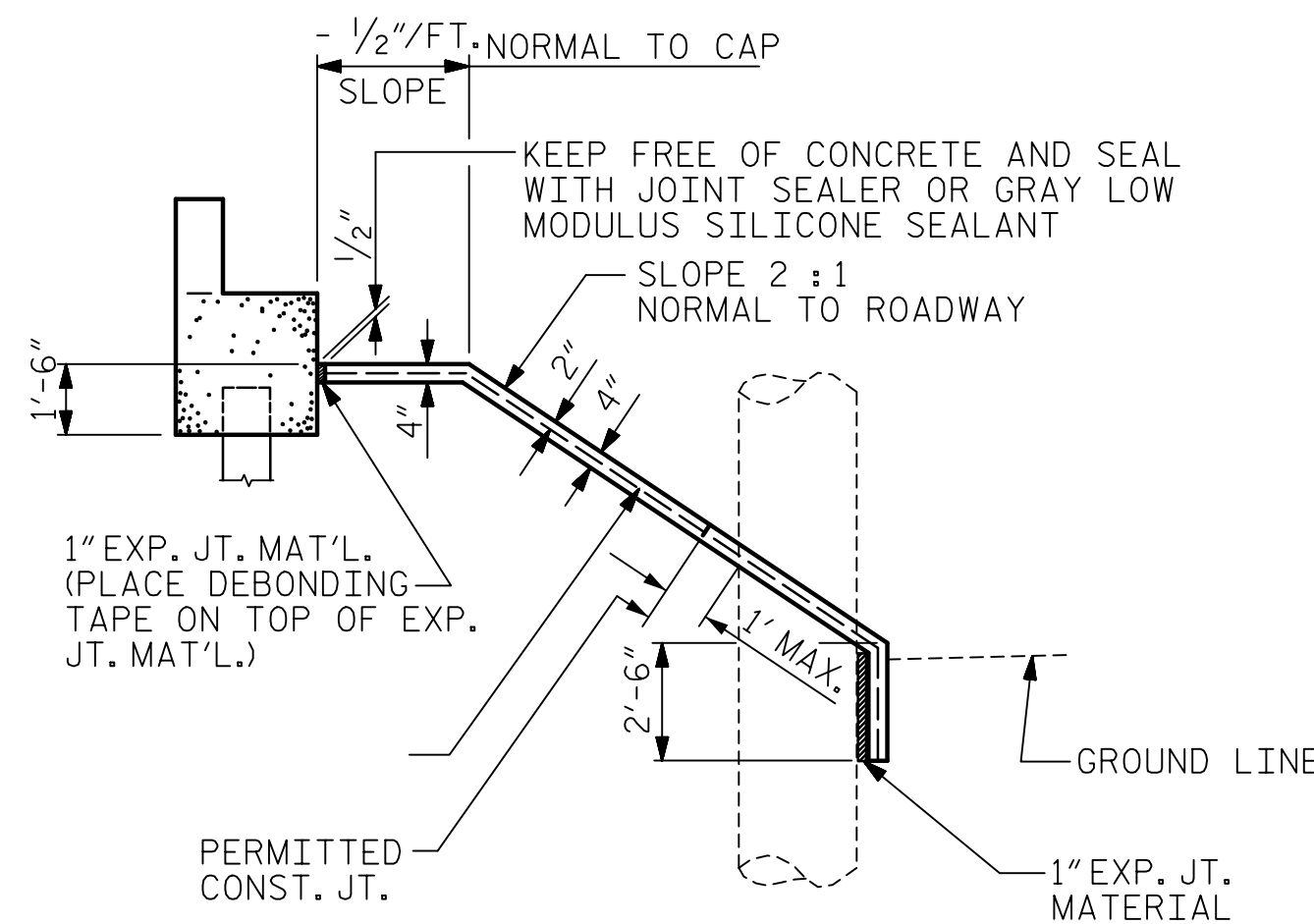
| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-----------------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| SUBSTRUCTURE END BENT 2 SECTIONS AND DETAILS | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| | | | | | |
| SHEET NO. S-80 | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1/12/2016



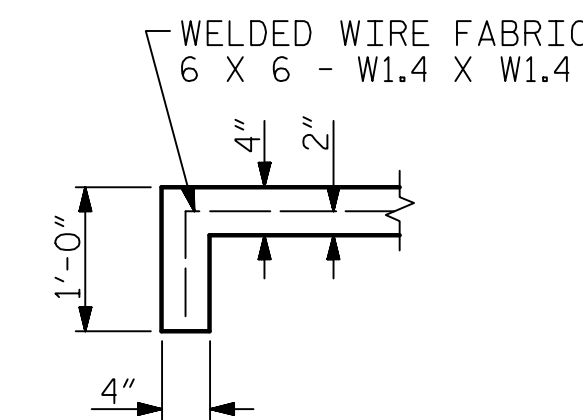
PLAN



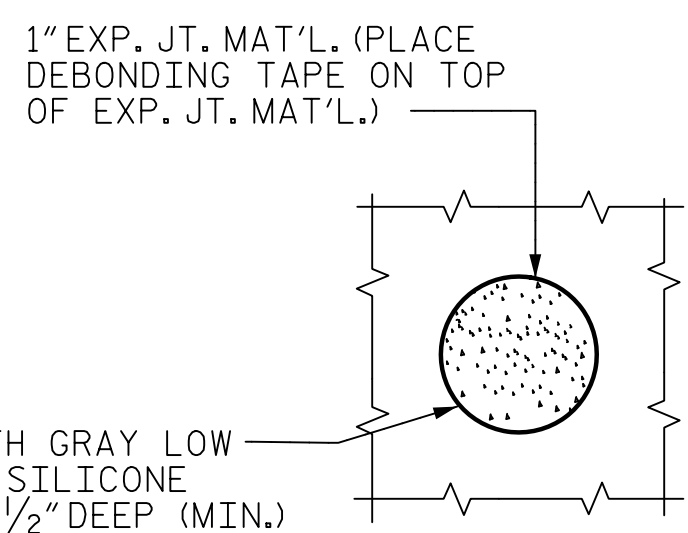
SECTION ALONG ROADWAY WITH SHOULDER PIER

| BRIDGE @ STA. 17+34.88 -Y- | 4 INCH SLOPE PROTECTION | * WELDED WIRE FABRIC 60 INCHES WIDE |
|-------------------------------|----------------------------|-------------------------------------------|
| | SQUARE YARDS | APPROX. L.F. |
| END BENT 1 | 598 | 1077 |
| END BENT 2 | 533 | 960 |

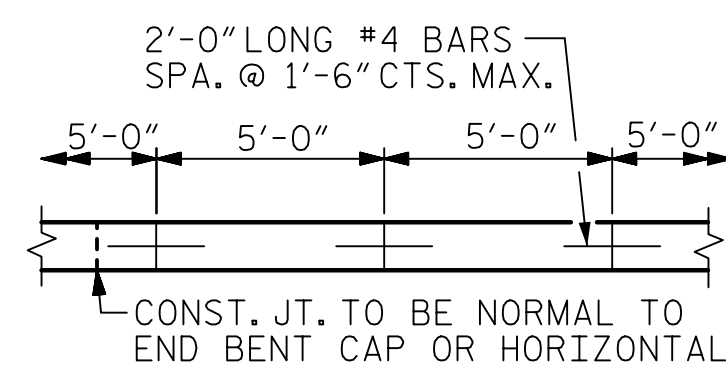
* QUANTITY SHOWN IS BASED ON 5' POURS.



SECTION A-A

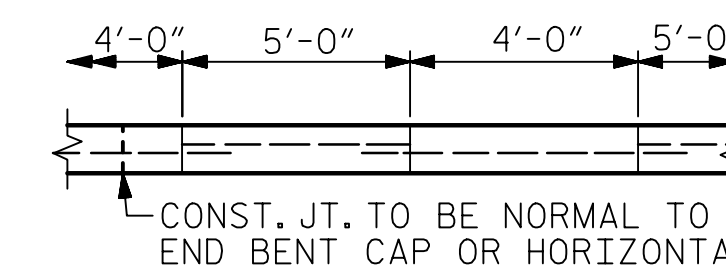


PLAN WHERE CONCRETE SLOPE PROTECTION MUST BE PLACED AROUND A BENT COLUMN



CONST. JT. TO BE NORMAL TO END BENT CAP OR HORIZONTAL
STRIP WIDTHS MAY VARY IN CURVED PORTION.

POURING DETAIL



POUR A 4'-0" STRIP FIRST. STRIP WIDTHS MAY VARY IN CURVED PORTION.

OPTIONAL POURING DETAIL

NOTES:

ALTERNATE "A" SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5' STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0" LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4' AND 5' STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

DATE: 1/12/2016
TIME: 8:55:42 AM

USER: \\s01\apps\009\cadd\02_079_112707_SML-SBE_SPO\app

| | |
|-----------------------|----------------------|
| ASSEMBLED BY : KHC | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : ELR 5/92 | REV. 5/1/06 TLA/GM |
| CHECKED BY : GRP 6/92 | REV. 10/1/11 MAA/GM |
| | REV. 12/21/11 MAA/GM |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-8200 www.aecom.com
AECOM License No. F-0342

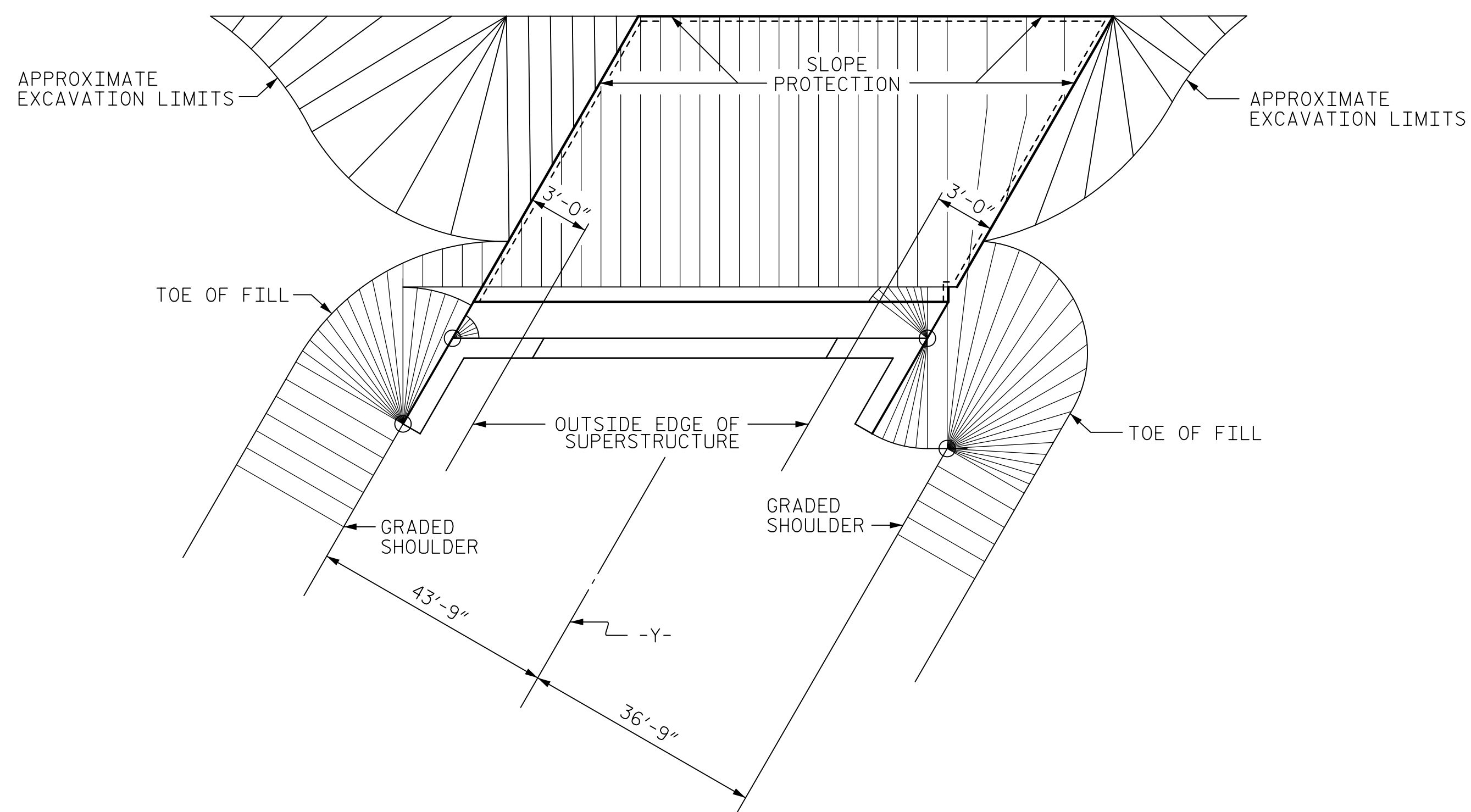
SEAL
DocuSign
330474
John C. Morrison
ENGINEER
JOHN C. MORRISON
1/12/2016

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 1 OF 2 BRIDGE NO. 109

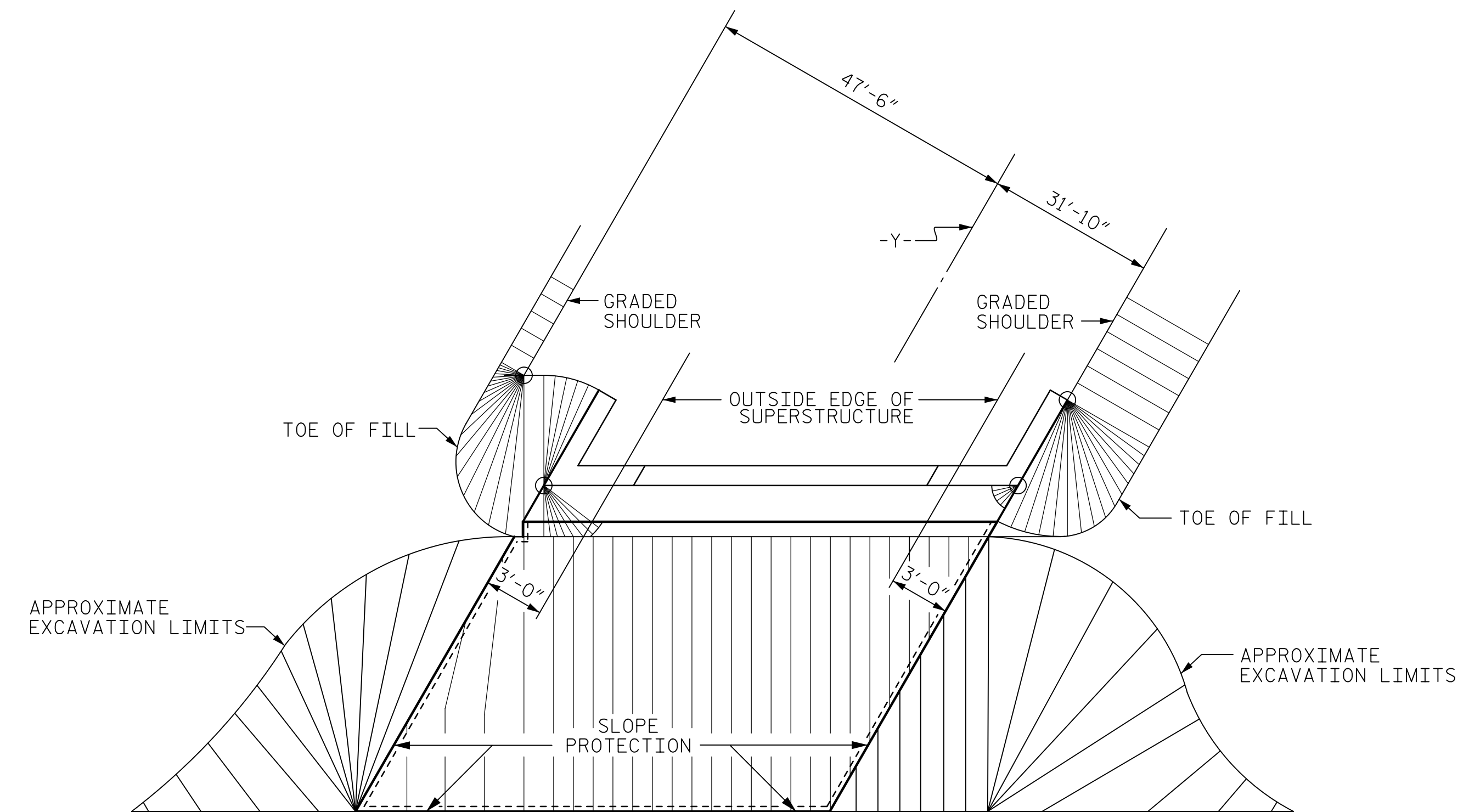
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
SLOPE PROTECTION
DETAILS

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. **S-82**
TOTAL SHEETS **86**



PLAN - END BENT 1
(2:1 SLOPE)



PLAN - END BENT 2
(2:1 SLOPE)

DATE: 1/12/2016
TIME: 8:55:45 AM

USER: A:\aecom\proj\172707_SML\S83_SPO2.dgn
DGN: A:\aecom\proj\172707_SML\S83_SPO2.dgn

DRAWN BY : DDL DATE : 11/2015
CHECKED BY : JCM DATE : 11/2015

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 2 OF 2 BRIDGE NO. 109

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-6200 www.aecom.com
AECOM License No. F-6342

SEAL
1130474
John C. Morris
REGISTERED PROFESSIONAL ENGINEER
JOHN C. MORRIS
1/12/2016

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

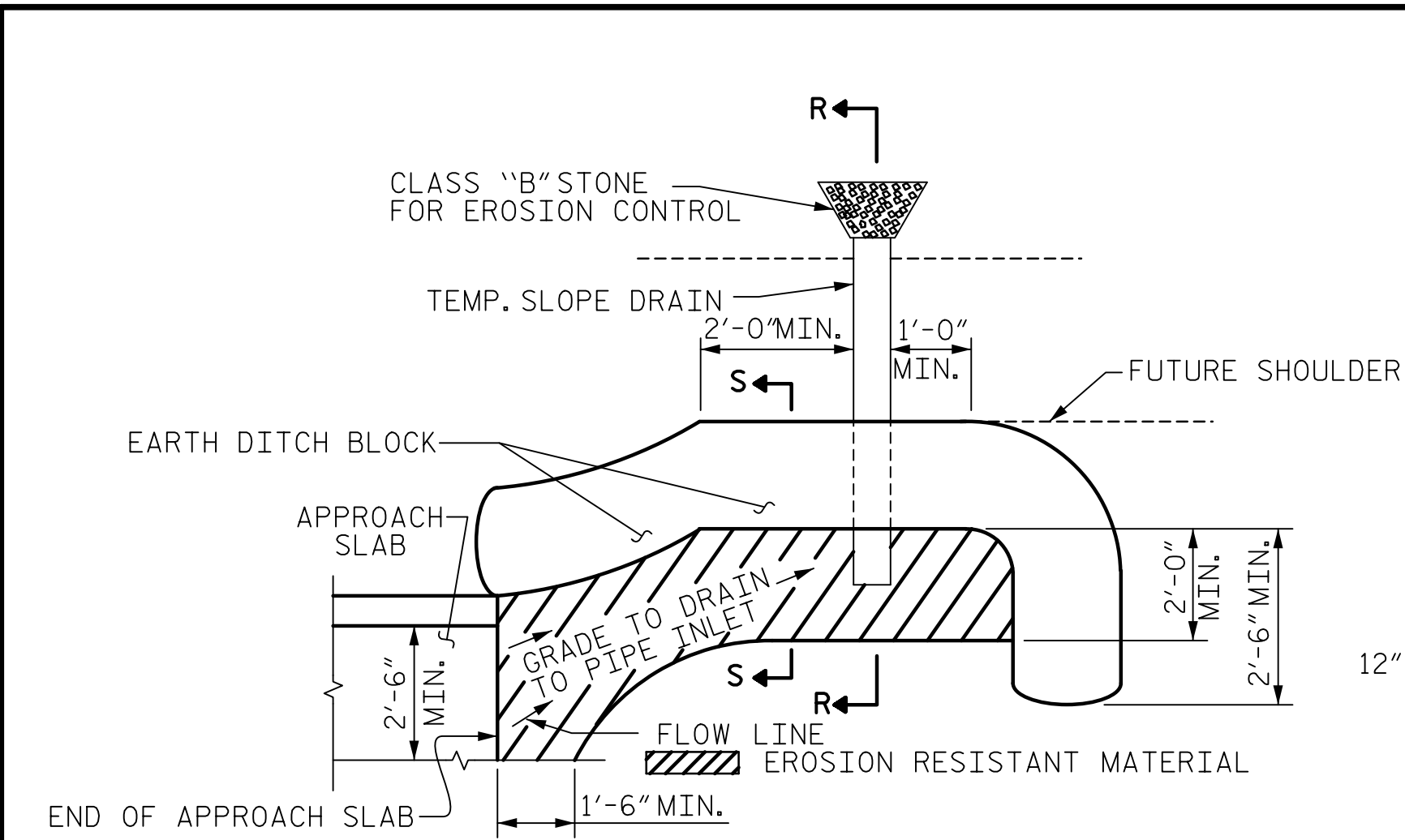
**SLOPE PROTECTION
DETAILS**

| REVISIONS | | | | | | SHEET NO. S-83 |
|-----------|-----|-------|-----|-----|-------|---------------------------|
| NO. | BY: | DATE: | NO. | BY: | DATE: | |
| | | | | | | TOTAL SHEETS 86 |

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

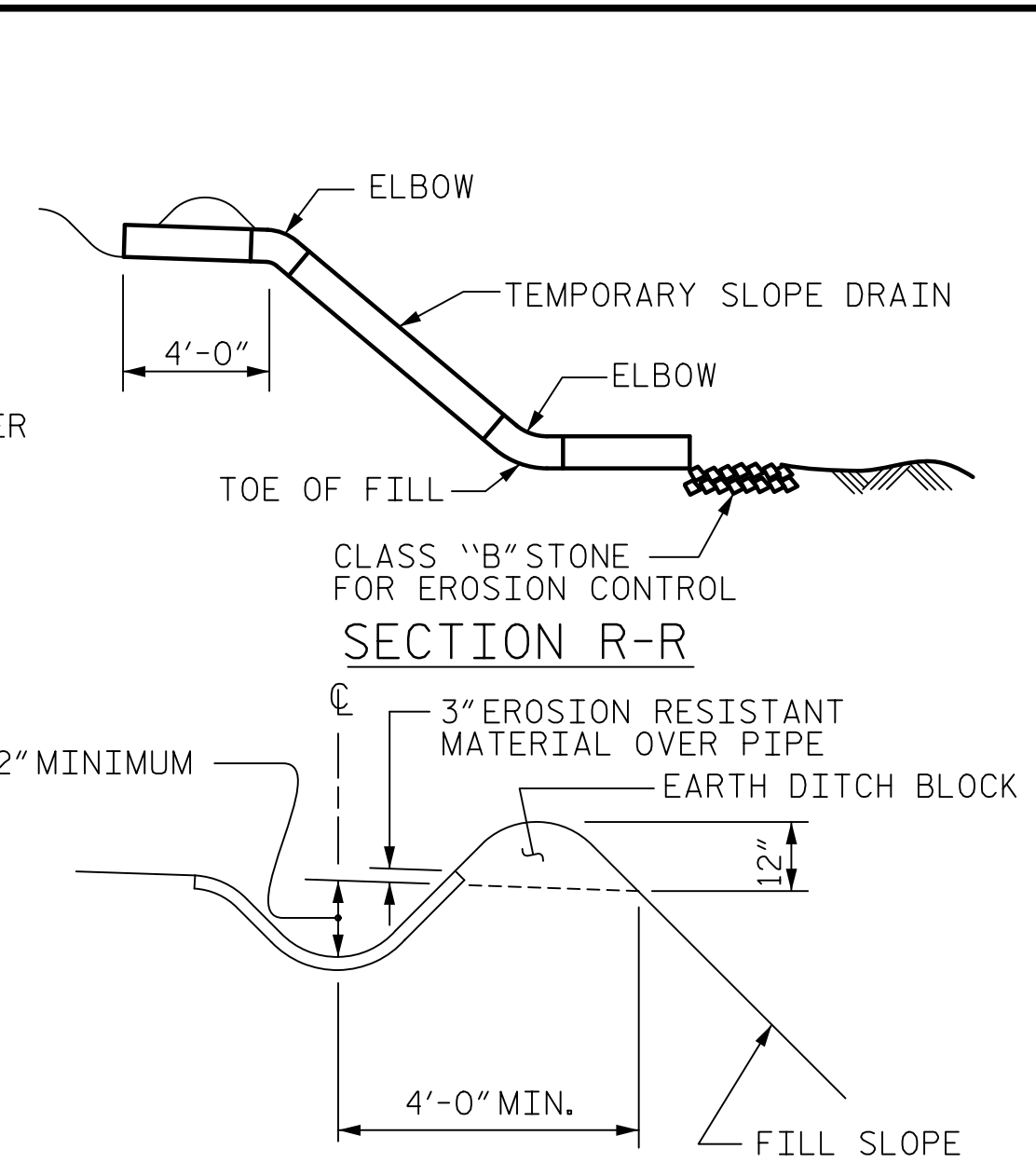
DATE: 1/12/2016
TIME: 8:55:45 AM

USER: \\s:\aecom\p\009\cadd\02_083_112707_S1U1_S84_ASO1.dgn
DGN: R:\Bridges\009\cadd\02_083_112707_S1U1_S84_ASO1.dgn



NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

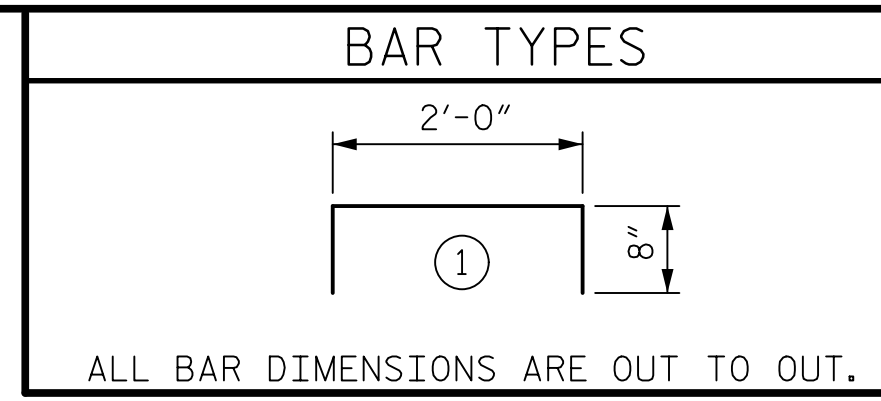
PLAN VIEW



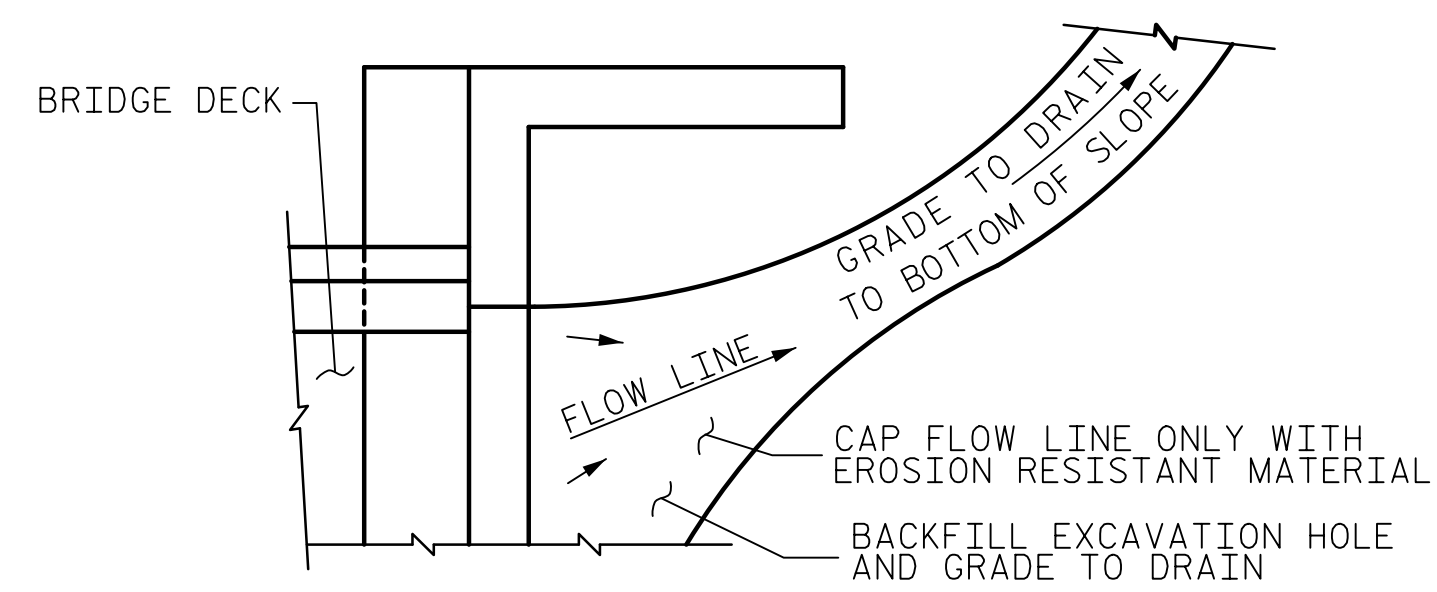
SECTION S-S

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



ALL BAR DIMENSIONS ARE OUT TO OUT.



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL

| BILL OF MATERIAL | | | | | |
|---------------------------------|-----|------|------|---------|--------|
| APPROACH SLAB AT EB #1 | | | | | |
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| *A1 | 75 | #4 | STR | 27'-10" | 1394 |
| A2 | 78 | #4 | STR | 27'-8" | 1442 |
| *B1 | 133 | #5 | STR | 23'-11" | 3318 |
| *B2 | 1 | #5 | STR | 21'-2" | 22 |
| *B3 | 1 | #5 | STR | 14'-6" | 15 |
| *B4 | 1 | #5 | STR | 9'-10" | 10 |
| *B5 | 1 | #5 | STR | 6'-0" | 6 |
| *B6 | 1 | #5 | STR | 2'-8" | 3 |
| *B7 | 1 | #5 | STR | 23'-6" | 25 |
| B8 | 133 | #6 | STR | 24'-10" | 4961 |
| B9 | 1 | #6 | STR | 21'-2" | 32 |
| B10 | 1 | #6 | STR | 14'-6" | 22 |
| B11 | 1 | #6 | STR | 9'-10" | 15 |
| B12 | 1 | #6 | STR | 6'-0" | 9 |
| B13 | 1 | #6 | STR | 2'-8" | 4 |
| B14 | 1 | #6 | STR | 23'-6" | 35 |
| *B15 | 10 | #4 | STR | 24'-9" | 165 |
| *G1 | 52 | #4 | STR | 5'-1" | 177 |
| *U1 | 20 | #4 | ① | 3'-4" | 45 |
| REINFORCING STEEL | | | | LBS. | 6520 |
| *EPOXY COATED REINFORCING STEEL | | | | LBS. | 5180 |
| CLASS AA CONCRETE | | | | | |
| POUR 1 SLAB | | | | C. Y. | 73.3 |
| POUR 2 SIDEWALK | | | | C. Y. | 6.3 |
| CLASS AA CONCRETE | | | | C. Y. | 79.6 |

NOTES

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

THE COSTS OF THE SIDEWALK, TEMPORARY DRAINAGE AND TEMPORARY BERM AND SLOPE DRAINS WILL BE PAID FOR UNDER THE LUMP SUM PRICE FOR BRIDGE APPROACH SLAB.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

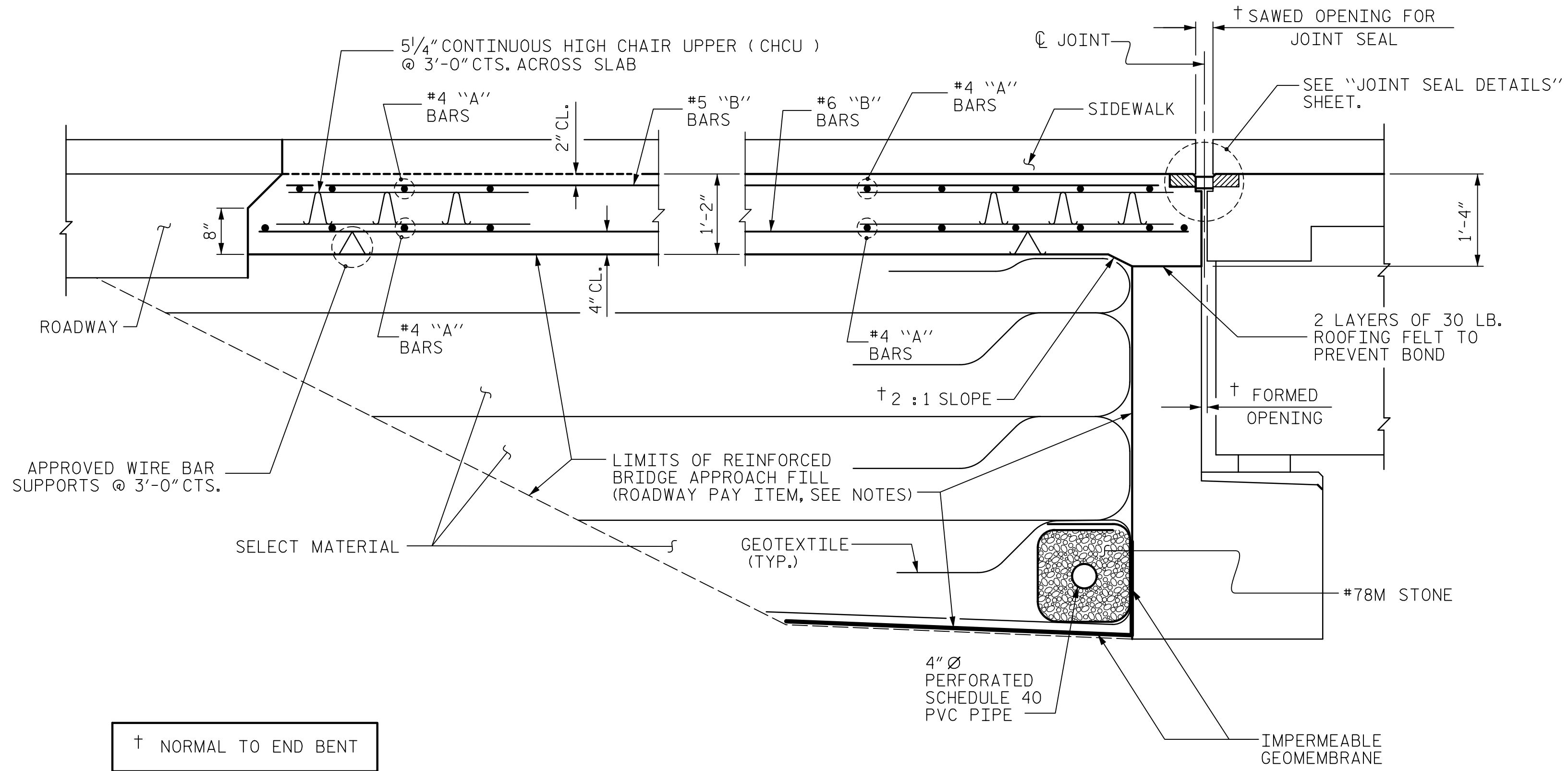
THE JOINT SHALL BE SAWED PRIOR TO THE CASTING OF THE PARAPET AND END POST, AND SIDEWALK.

FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE FOAM JOINT SEAL SHALL BE 3 1/16".

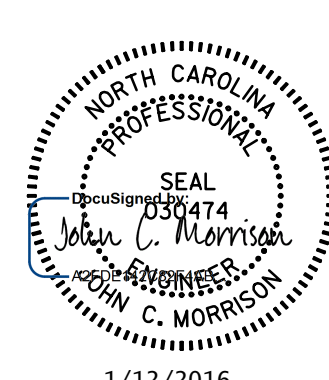
FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.

| APPROACH SLAB AT EB #2 | | | | | |
|---------------------------------|-----|------|------|--------|--------|
| BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT |
| *A1 | 75 | #4 | STR | 25'-2" | 1261 |
| A2 | 78 | #4 | STR | 25'-0" | 1303 |
| *B1 | 134 | #5 | STR | 23'-8" | 3308 |
| B2 | 134 | #6 | STR | 24'-7" | 4948 |
| *B3 | 10 | #4 | STR | 24'-7" | 164 |
| *G1 | 52 | #4 | STR | 5'-1" | 177 |
| *U1 | 20 | #4 | ① | 3'-4" | 45 |
| REINFORCING STEEL | | | | LBS. | 6251 |
| *EPOXY COATED REINFORCING STEEL | | | | LBS. | 4955 |
| CLASS AA CONCRETE | | | | | |
| POUR 1 SLAB | | | | C. Y. | 72.5 |
| POUR 2 SIDEWALK | | | | C. Y. | 6.4 |
| CLASS AA CONCRETE | | | | C. Y. | 78.9 |



SECTION THRU SLAB

| SPLICE LENGTHS | | |
|----------------|--------------|----------|
| BAR SIZE | EPOXY COATED | UNCOATED |
| #4 | 2'-0" | 1'-9" |
| #5 | 2'-6" | 2'-2" |
| #6 | 3'-10" | 2'-7" |



PROJECT NO. U-2707
FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 1 OF 3 BRIDGE NO. 109

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE APPROACH SLAB
 FOR FLEXIBLE PAVEMENT

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO.
S-84
 TOTAL SHEETS
86

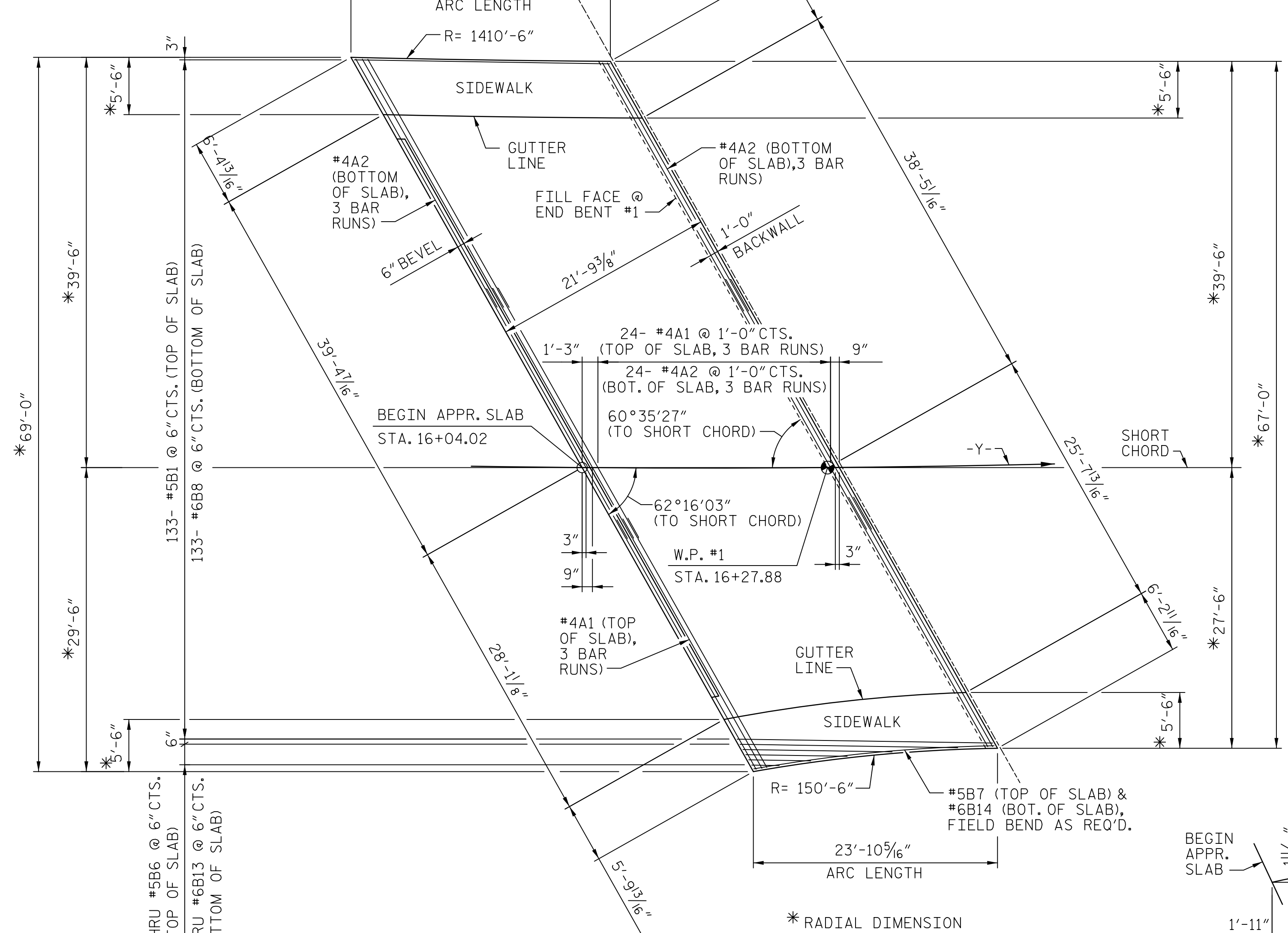
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

| | |
|-----------------------|----------------------|
| ASSEMBLED BY : MTB | DATE : 11/2015 |
| CHECKED BY : JCM | DATE : 11/2015 |
| DRAWN BY : EEM 3/95 | REV. 5/7/03R RWW/JTE |
| CHECKED BY : VAP 3/95 | REV. 5/1/06RR KMM/GM |
| | REV. 10/1/11 MAA/GM |

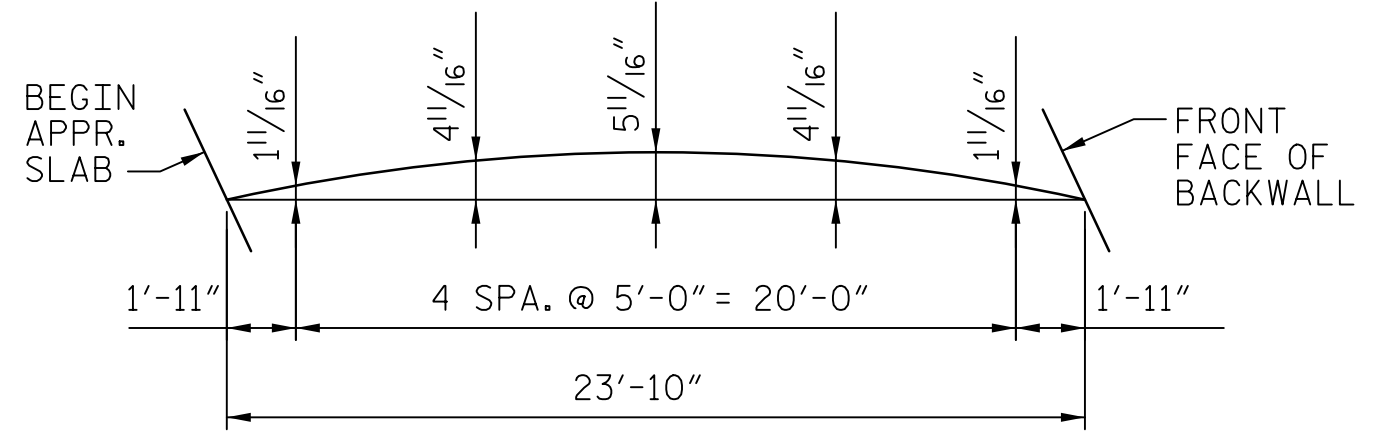
DATE: 1/12/2016
TIME: 8:55:59 AM

USER: A:\s\c\p\p\109\c\c\402\dbs_112707_SML_S95_A502.dgn
DGN: A:\s\c\p\p\109\c\c\402\dbs_112707_SML_S95_A502.dgn

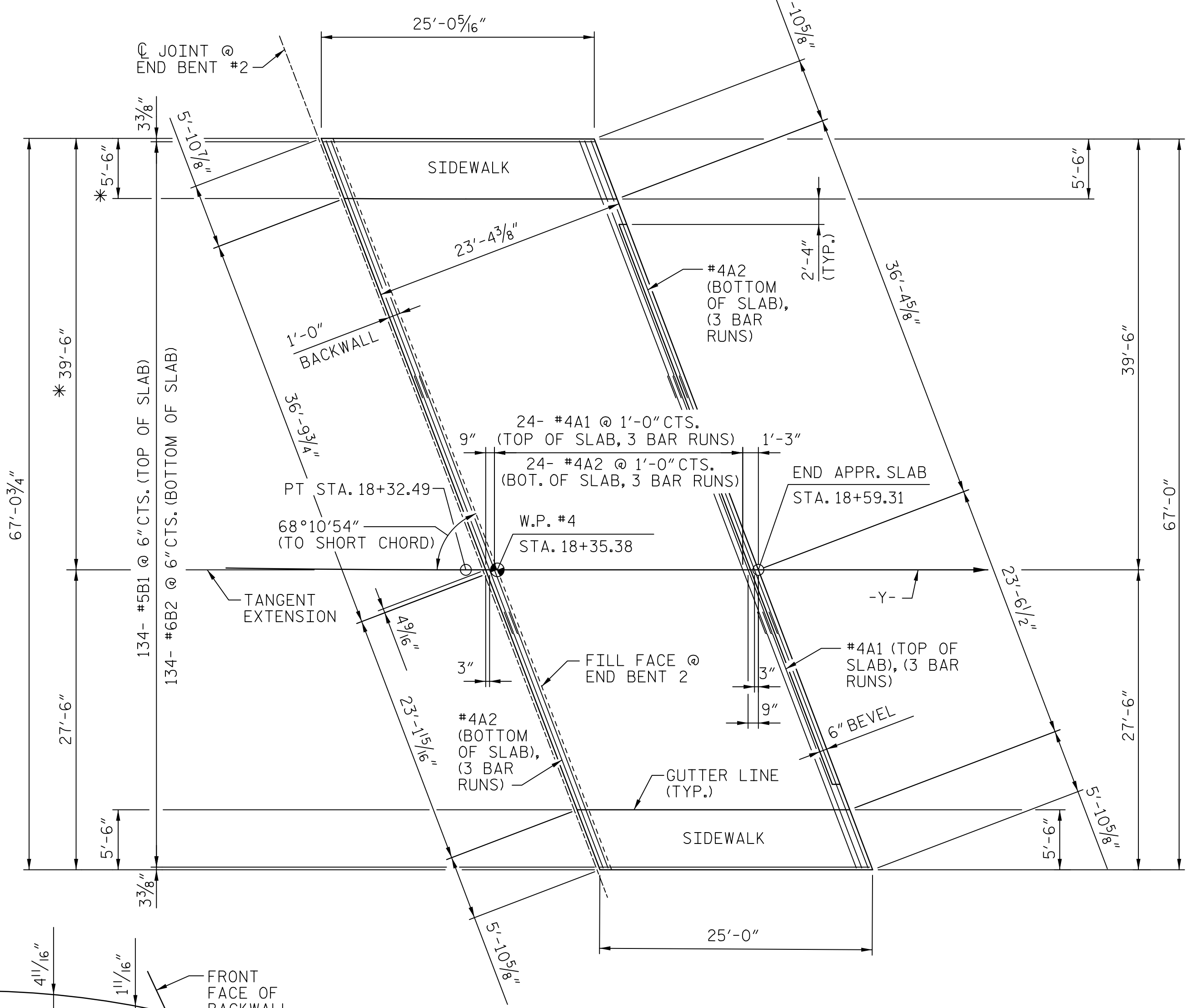
SHORT CHORD IS THE LINE BETWEEN BEGIN BRIDGE APPR. SLAB AND W.P. #1 @ END BENT #1



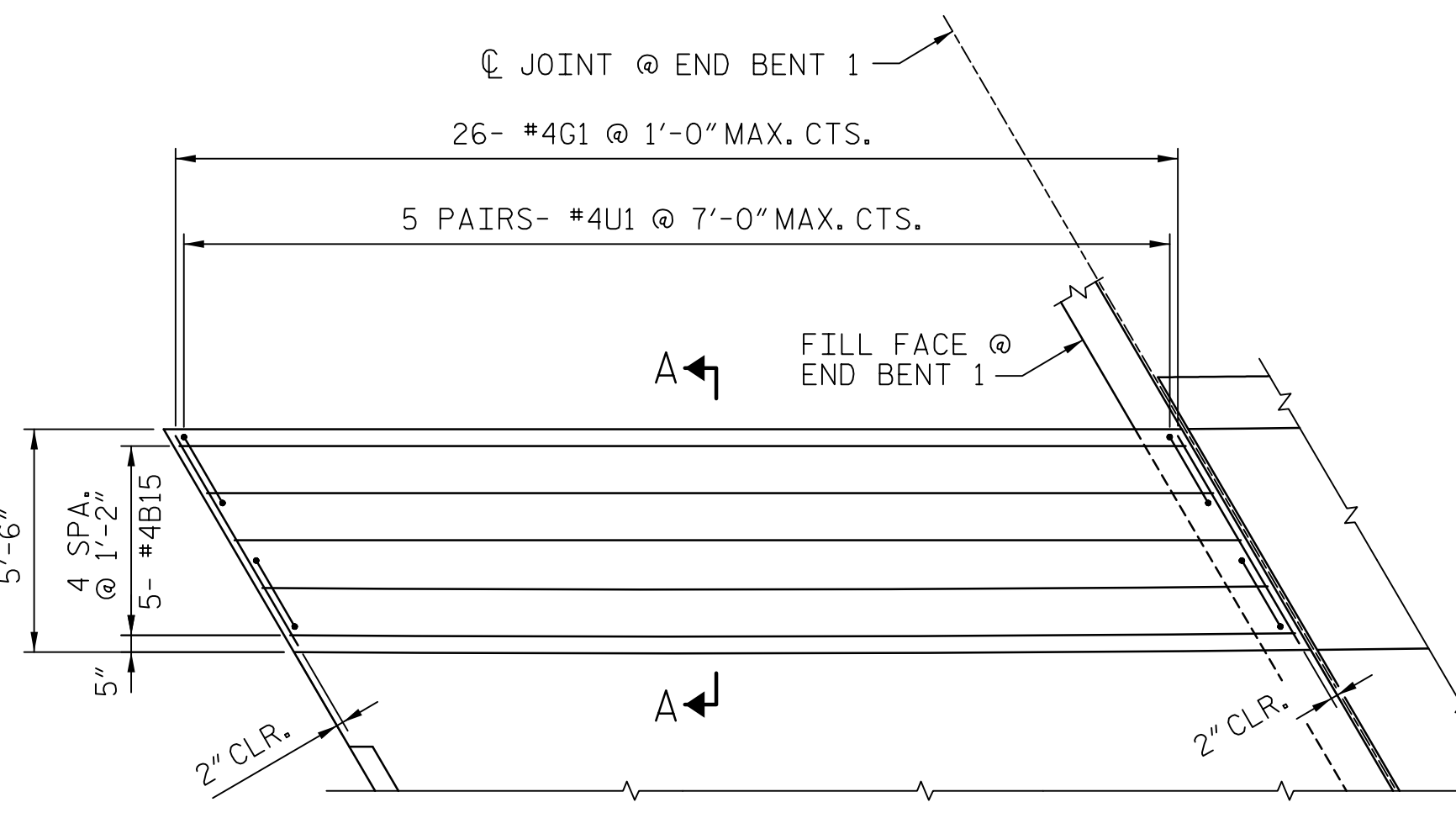
PLAN @ END BENT #1



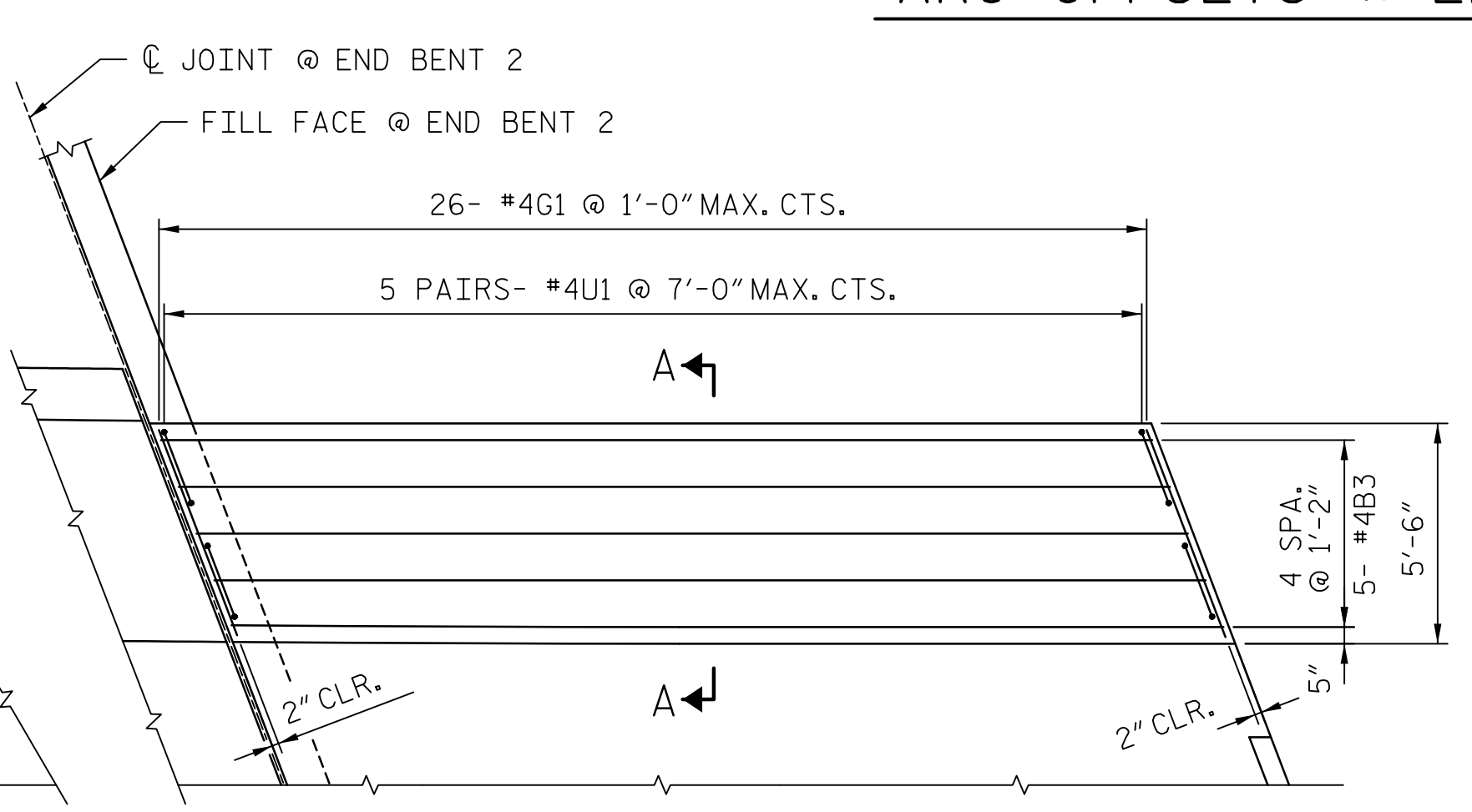
RIGHT SIDE - EDGE OF SLAB
ARC OFFSETS @ END BENT #1



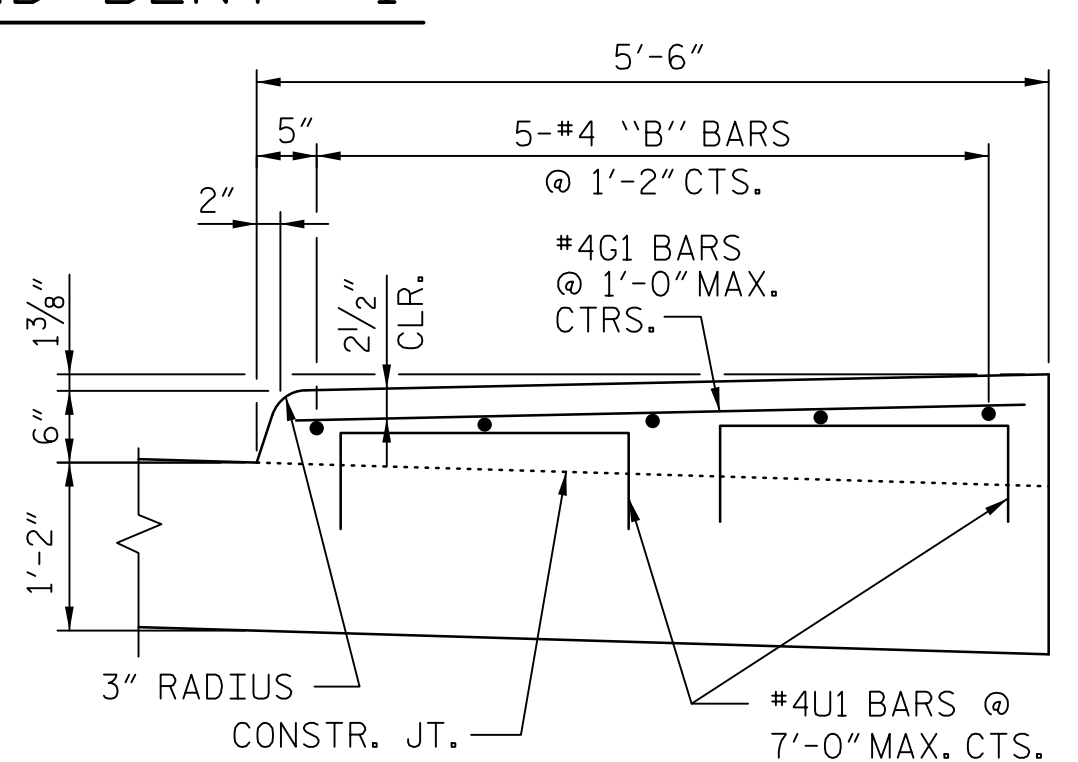
PLAN @ END BENT #2



SIDEWALK PLAN
(END BENT #1 LEFT SHOWN,
END BENT #1 RIGHT SIMILAR)



SIDEWALK PLAN
(END BENT #2 LEFT SHOWN,
END BENT #2 RIGHT SIMILAR)



SECTION A-A
SIDEWALK DETAILS

NOTE:
GROOVED CONTRACTION JOINT, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE SIDEWALK IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATED AT A SPACING OF 8 FT. TO 10 FT. BETWEEN EXPANSION JOINTS. NO CONTRACTION JOINTS WILL BE REQUIRED FOR SEGMENTS LESS THAN 10 FEET IN LENGTH.

DOCUMENT NOT CONSIDERED
FINAL UNLESS ALL
SIGNATURES COMPLETED

PROJECT NO. U-2707
FORSYTH COUNTY
STATION: 17+34.88 -Y-
SHEET 2 OF 3 BRIDGE NO. 109

AECOM
AECOM TECHNICAL SERVICES, INC.
701 CORPORATE CENTER DRIVE, SUITE 475
RALEIGH, NC 27607
(919) 854-6200 www.aecom.com
AECOM License No. F-0342

SEAL
1330474
John C. Morris
REGISTERED PROFESSIONAL ENGINEER
JOHN C. MORRIS

1/12/2016

| | | | | | |
|--------------------------------------------------------------------|-----|-------|-----|-----|-------|
| STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH | | | | | |
| BRIDGE APPROACH SLAB FOR FLEXIBLE PAVEMENT | | | | | |
| REVISIONS | | | | | |
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |
| SHEET NO. | | | | | S-85 |
| TOTAL SHEETS | | | | | 86 |

DRAWN BY: MTB DATE: 11/2015
CHECKED BY: JCM DATE: 11/2015

NOTES

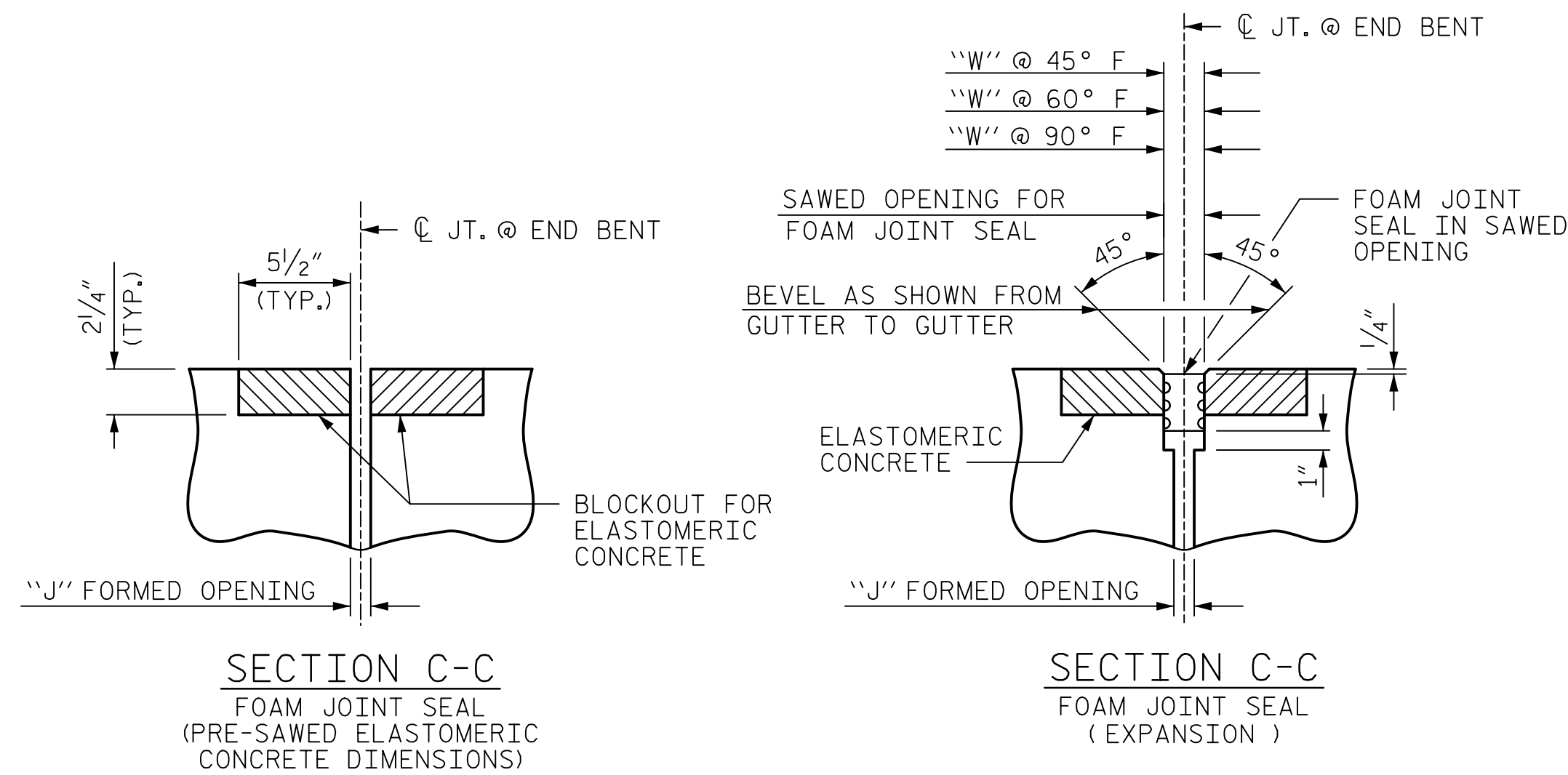
THE STEEL PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 OR APPROVED EQUAL. AFTER FABRICATION, THE PLATES SHALL BE COMMERCIALY BLAST CLEANED AND EITHER COATED WITH A MINIMUM THICKNESS OF 4 MILS (DRY) OF ZINC-RICH PAINT, GALVANIZED OR METALLIZED TO A MINIMUM THICKNESS OF 6 MILS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

THE 3/4" DIAMETER HEX HEAD BOLTS SHALL CONFORM TO ASTM F593 ALLOY 304 STAINLESS STEEL.

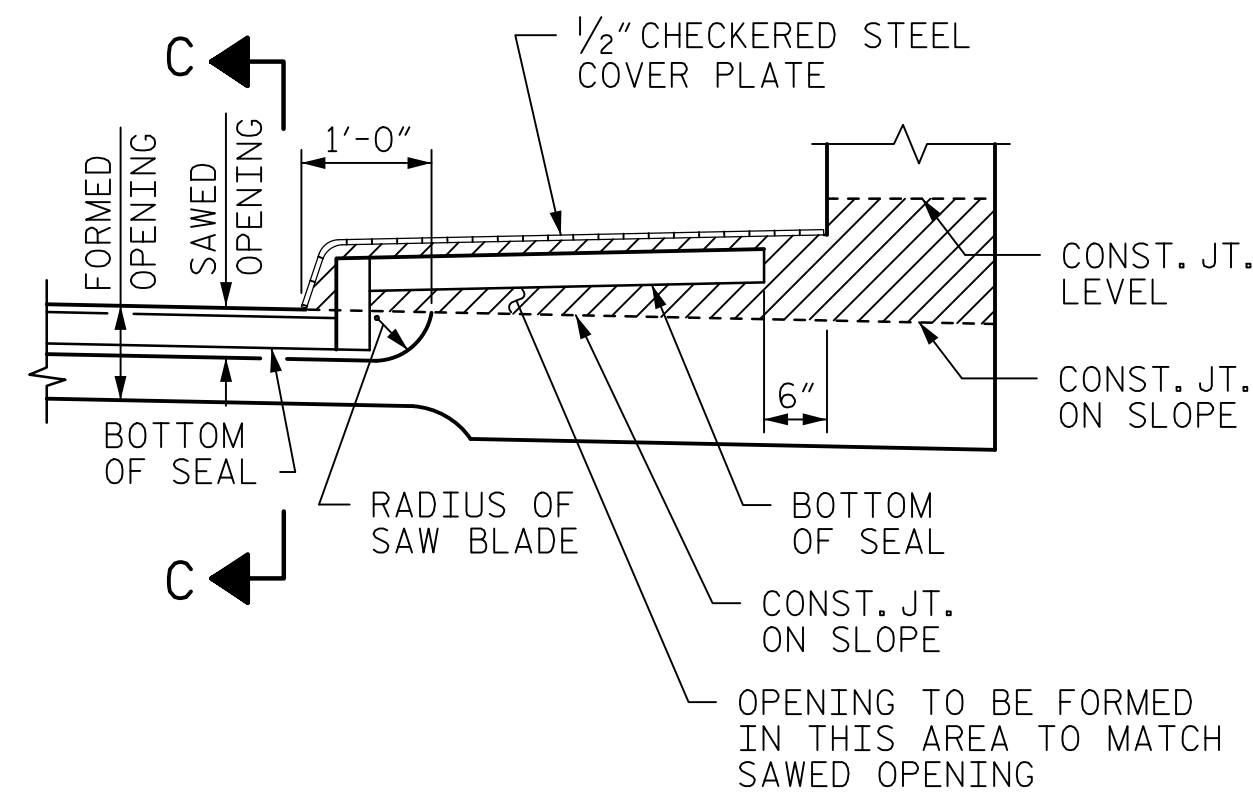
NO SEPARATE PAYMENT WILL BE MADE FOR FURNISHING AND INSTALLING THE COVER PLATE. THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE LUMP SUM PRICE FOR "FOAM JOINT SEALS".

FOR FOAM JOINT SEALS, SEE SPECIAL PROVISIONS.

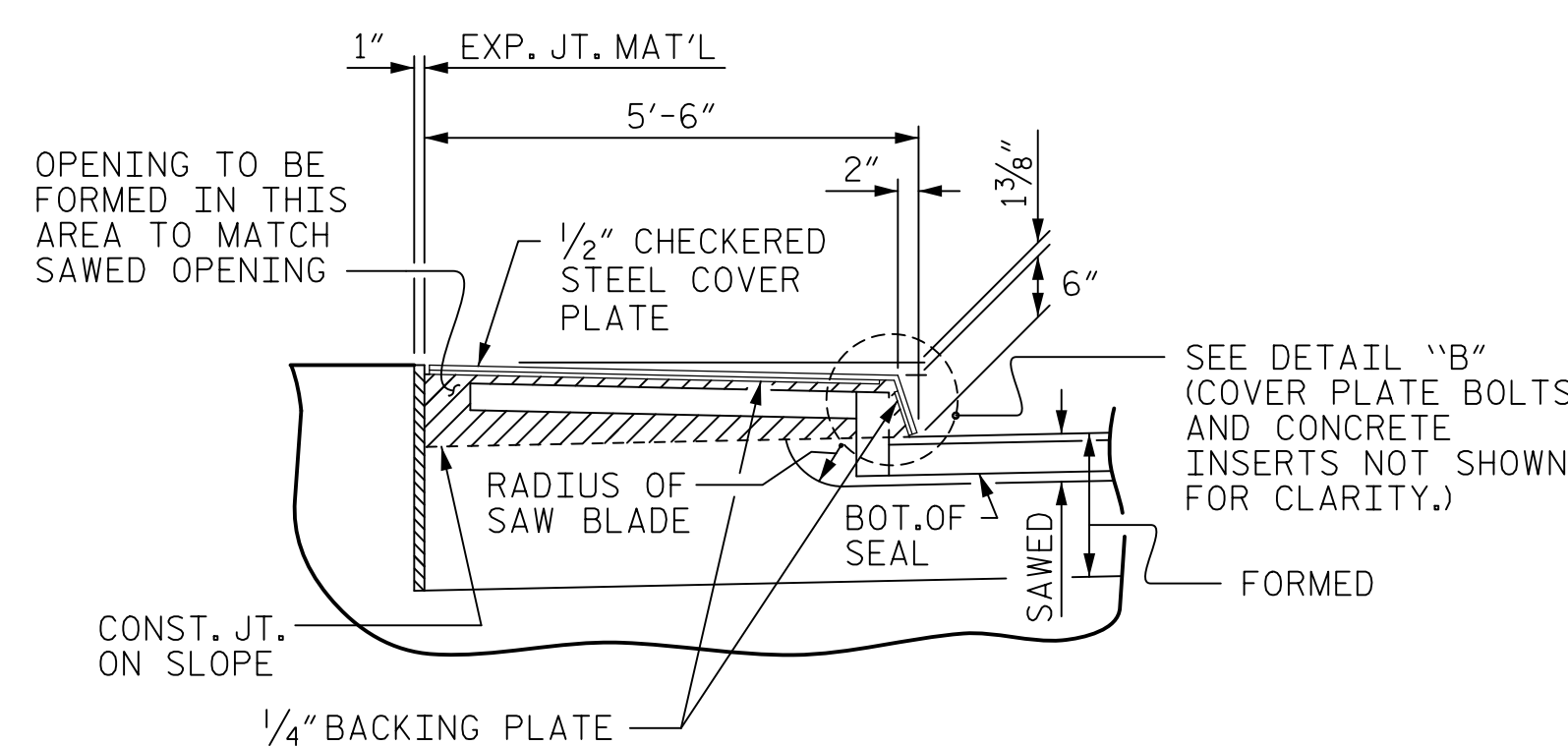
FOR ELASTOMERIC CONCRETE, SEE SPECIAL PROVISIONS.



TYPICAL FOAM JOINT SEAL DETAIL



SECTION A-A



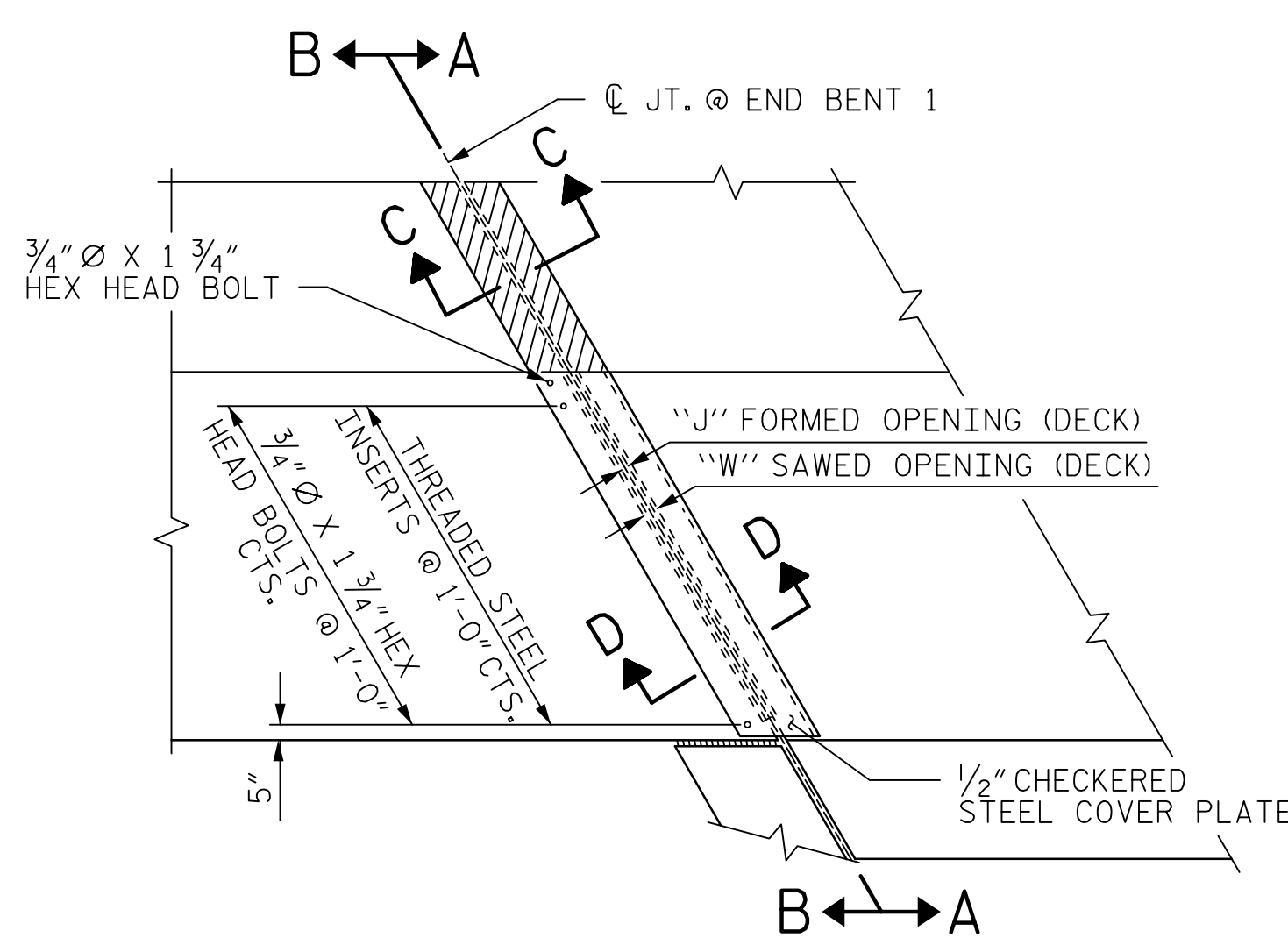
SECTION B-B

| MOVEMENT AND SETTING AT FOAM JOINT | | | | | | | |
|------------------------------------|----------------------|---------------------------------|-------------------------------|--------------------|------------------------------------------|------------------------------------------|------------------------------------------|
| END BENT NO. | SKEW ANGLE (TO TAN.) | NOMINAL UNCOMPRESSED SEAL WIDTH | TOTAL MOVEMENT (ALONG C RDWY) | "J" FORMED OPENING | "W" PERPENDICULAR JOINT OPENING AT 30° F | "W" PERPENDICULAR JOINT OPENING AT 60° F | "W" PERPENDICULAR JOINT OPENING AT 90° F |
| EB1 | 61°03'44" | 2 1/2" | 1 1/8" | 1 1/4" | 2 1/8" | 1 5/16" | 1 9/16" |
| EB2 | 69°08'51" | 2 1/2" | 1 1/8" | 1 1/4" | 2 1/8" | 1 5/16" | 1 9/16" |

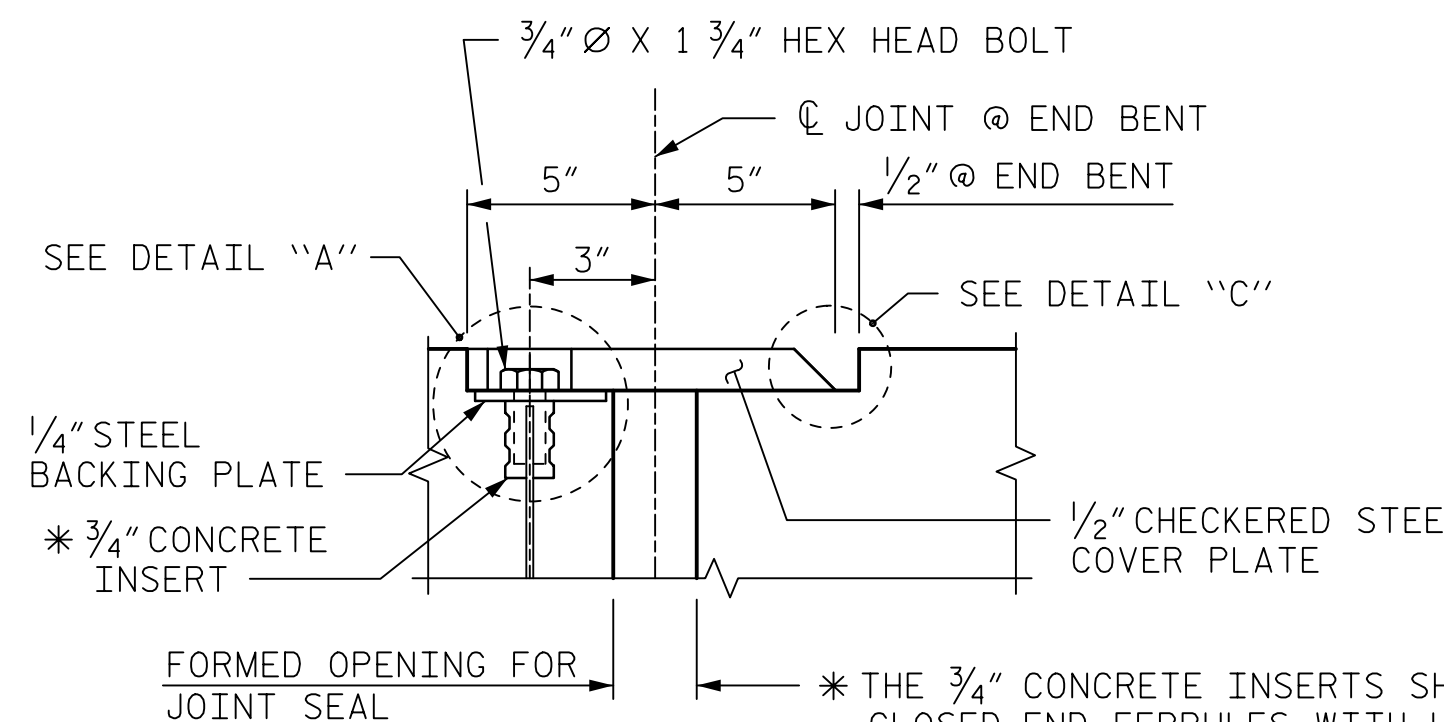
TOTAL MOVEMENT IS CALCULATED ALONG THE CENTERLINE OF ROADWAY. JOINT OPENINGS ARE MEASURED PERPENDICULAR TO THE JOINT.

| BILL OF MATERIAL | |
|------------------|----------------------------------|
| END BENT NO. | ELASTOMERIC CONCRETE * (CU. FT.) |
| 1 | 11.34 |
| 2 | 10.64 |
| TOTAL | 21.98 |

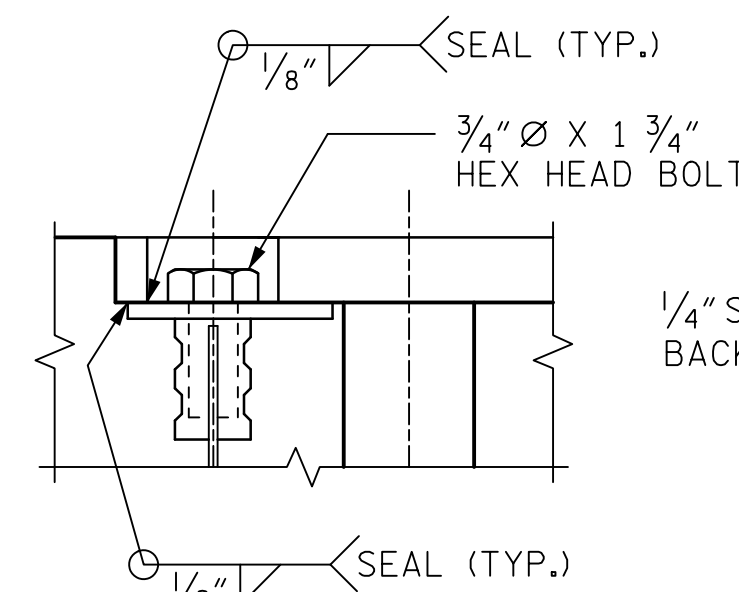
*BASED ON THE MINIMUM BLOCKOUT SHOWN.



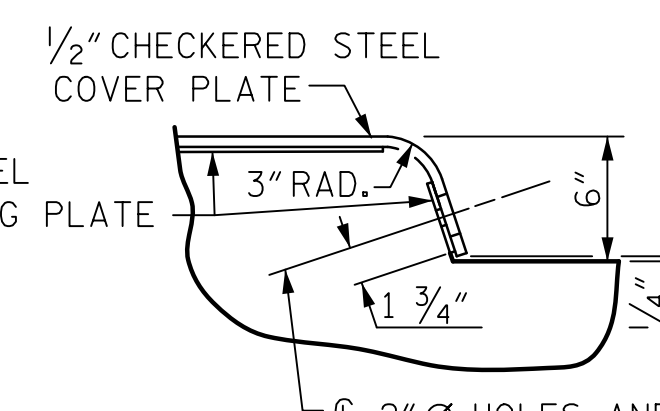
PLAN VIEW OF FOAM JOINT SEAL WITH COVER PLATE @ END BENT FOR SIDEWALK



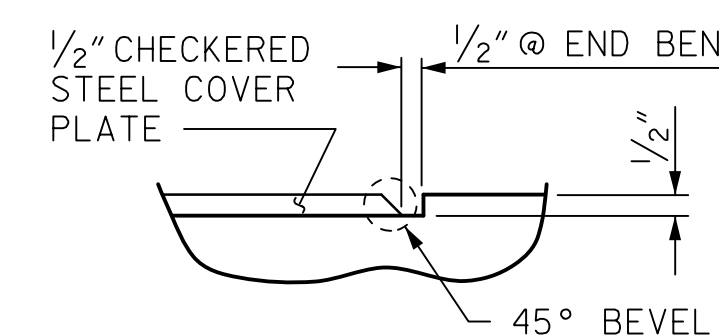
SECTION D-D



DETAIL "A"



DETAIL "B"



DETAIL "C"

JOINT SEAL DETAILS @ END BENT

| | | | |
|----------------|-----------|---------------|---------|
| ASSEMBLED BY : | DDL | DATE : | 11/2015 |
| CHECKED BY : | JCM | DATE : | 11/2015 |
| DRAWN BY : | FCJ 11/88 | REV. 10/11/11 | MAA/GM |
| CHECKED BY : | ARB 11/88 | REV. 7/12 | MAA/GM |
| | | REV. 6/13 | MAA/GM |

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



PROJECT NO. U-2707
 FORSYTH COUNTY
 STATION: 17+34.88 -Y-
 SHEET 3 OF 3 BRIDGE NO. 109

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 STANDARD
 BRIDGE APPROACH
 SLAB DETAILS

| REVISIONS | | | | | |
|-----------|-----|-------|-----|-----|-------|
| NO. | BY: | DATE: | NO. | BY: | DATE: |
| 1 | | | 3 | | |
| 2 | | | 4 | | |

SHEET NO. S-86
 TOTAL SHEETS 86

STANDARD NOTES

DESIGN DATA:

| | | |
|-------------------------------------------------|----------|----------------------------|
| SPECIFICATIONS | ----- | A.A.S.H.T.O. (CURRENT) |
| LIVE LOAD | ----- | SEE PLANS |
| IMPACT ALLOWANCE | ----- | SEE A.A.S.H.T.O. |
| STRESS IN EXTREME FIBER OF | | |
| STRUCTURAL STEEL - AASHTO M270 GRADE 36 | - | 20,000 LBS. PER SQ. IN. |
| - AASHTO M270 GRADE 50W | - | 27,000 LBS. PER SQ. IN. |
| - AASHTO M270 GRADE 50 | - | 27,000 LBS. PER SQ. IN. |
| REINFORCING STEEL IN TENSION | | |
| | GRADE 60 | -- 24,000 LBS. PER SQ. IN. |
| CONCRETE IN COMPRESSION | ----- | 1,200 LBS. PER SQ. IN. |
| CONCRETE IN SHEAR | ----- | SEE A.A.S.H.T.O. |
| STRUCTURAL TIMBER - TREATED OR | | |
| UNTREATED - EXTREME FIBER STRESS | ----- | 1,800 LBS. PER SQ. IN. |
| COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER | ----- | 375 LBS. PER SQ. IN. |
| EQUIVALENT FLUID PRESSURE OF EARTH | ----- | 30 LBS. PER CU. FT. |
| | | (MINIMUM) |

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

DATE: 3/14/2014
TIME: 11:25:42 AM

USER: hmsrd
DIR: C:\6279356400_Technical_Information_Disc_Files\450_KCDDT\51of\tech\DesignFiles\Structures\Bridges\0\Card\U2525B_SD_DS1_M42_S1.dgn