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# INTELLIGENT TRANSPORTATION AN Contacts:

Gregory A. Fuller, PE-Intelligent Transport Timothy J. Williams, PE-Western Regi George C. Brown, PE-Signal Equipmen

		<b>R</b> –5605	Sig. 1.0
$\frac{\text{END T.I.}}{\text{IL-PO}}$	P PROJECT R- T STA 51+72	-5605 .36	See Orawings
	NCDOT" dated "Standard Spec and Structures"	d January 201 cifications for 1 dated Januar	2 and Roads ry 2012.
ND SIGNALS UNIT station System Engineer	Prepared DIVISION TRANSPORTATION DI	in the Office of: OF HIGHWAYS MOBILITY AND VISION DIIITY ON NORTH CARGON	SAFETY
on Signals Engineer nt Design Engineer	750 N. Greenfield P	d Signals arkway, Garner, 1	NC 27529



OASIS 2070 LOOP & DETECTOR INSTALLATION CHAP									AR	Т		
INDUCTIVE LOOPS DETECTOR PROGRAMMING												
LOOP	SIZE (FT)	DISTANCE FROM STOPBAR (FT)	TURNS	NEW LOOP	PHASE	CALLING	EXTENSION	FULL TIME DELAY	STRETCH TIME	DELAY TIME	SYSTEM LOOP	NEW CARD
1 ۸	6740	0	2-4-2	v	1	Y	Y	-	-	15	-	Y
IA	0740	0	2-4-2		6	Y	Y	Y	-	3	-	Y
1B	6X40	0	2-4-2	Y	1	Y	Y	-	-	15	-	Y
2A/S01	6X6	300	5	Y	2	Y	Y	-	-	-	Y	Y
2B/S02	6X6	300	5	Y	2	Y	Y	-	-	-	Y	Y
2C	6X40	0	2-4-2	Y	2	Y	Y	Y	-	3	-	Y
6A/SO3	6X6	300	5	Y	6	Y	Y	_	-	_	Y	Y
6B/S04	6X6	300	5	Y	6	Y	Y	_	_	_	Y	Y
8A	6X40	0	2-4-2	Y	8	Y	Y	-	-	-	-	Y

### 3 Phase Fully Actuated US 64 / 276 Brevard CLS

# NOTES

- 1. Refer to "Roadway Standard Drawings NCDOT" dated January 2012 and "Standard Specifications for Roads and Structures" dated January 2012.
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Phase 1 may be lagged.
- 4. Set all detector units to presence mode.
- 5. Locate new cabinet so as not to obstruct sight distance of vehicles turning right on red.
- 6. Maximum times shown in timing chart are for free-run operation only. Coordinated signal system timing values supersede these values.
- 7. Closed loop system data: Controller Asset #1287

# <u>LEGEND</u>



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Prepare	ed in the Offices o	of:	US 6	4-276	(Ash	eville	Highw	/ay)	SEAL	
	at Davidson River Village								CARO OFESSION	
		•			Conn	ector				<b>P</b>
S.			Division	14 Trans	sylvania	a County	Near B	revard	024393	
Sn	OF TRADECTIO		PLAN DATE:	Septembe	r 2015	REVIEWED BY:	T. Willi	iams	E TO SACINE FR	SN
.Greenfie	eld Pkwy.Garner.No	C 27529	PREPARED BY:	M. Maht	ooba	REVIEWED BY:			HI HY WIN	
$\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{$	SCALE			REVISIONS			INIT.	DATE		,ı''
	0	40							J. J. Williams	9/29/2015
									97AD792E8E934CA	DATE
	1 '' = 4 0 '								SIG. INVENTORY NO.	14-1287



LOOP NO.	LOOP TERMINAL	INPUT FILE POS.	PIN NO.	INPUT ASSIGNMENT NO.	DETECTOR NO.	NEMA PHASE	CALL	EXTEND	FULL TIME DELAY	STRETCH TIME	DELAY TIME
101	TB2-1,2	I1U	56	18	1	1	Y	Y			15
IH	-	J4U	48	10	26	6	Y	Y	Y		3
1B	TB5-11,12	J6L	46	8	18	1	Y	Y			15
2A/SØ1	TB2-5 <b>,</b> 6	I2U	39	1	2	2/SYS	Y	Y			
2B/SØ2	TB2-7 <b>,</b> 8	I2L	43	5	12	2/SYS	Y	Y			
2C	TB2-9,10	I3U	63	25	32	2	Y	Y	Y		3
6A/SØ3	TB3-5 <b>,</b> 6	J2U	40	2	6	6/SYS	Y	Y			
6B/SØ4	TB3-7 <b>,</b> 8	J2L	44	6	16	6/SYS	Y	Y			
8A	TB5-9,10	JGU	42	4	8	8	Y	Y			

PROJECT REFERENCE NO.	SHEET NO
R-5605	Sig 2.

	SIGNAL HEAD HOOK-UP CHART																	
	S2	S3	S4	S5	S6	S7	S8	59	S1Ø	S	11	S12	AUX S1	AUX S2	AUX S3	AUX S4	AUX S5	AUX S6
	2	13	З	4	14	5	6	15	7	8	3	16	Q,	10	17	11	12	18
	2	2 PED	С	4	4 PED	5	6	6 PED	7	8	3	8 PED	OLA	OLB	SPARE	OLC	OLD	SPARE
82	22,23	NU	NU	NU	NU	NU	61,62	NU	NU	23	81,82	NU	<b>★</b>	NU	NU	★ 21	NU	NU
*	128						134				107							
	129						135				108							
	130						136				109							
													A121			A114		
126										108			A122			A115		
													A123			A116		
127										109								

ectrical Detail -	Sheet 1 of 2				
TRICAL AND PROGRAMMING DETAILS FOR:	US 64-276 (Ash	eville Hi	ighw	ay)	SEAL
Prepared in the Offices of: Nobility on-	a Davidson Ri Conn	t ver Villa ector	age		SEAL
Division MAR ROLL	Division 14 Transylva PLAN DATE: September 2015	nia County REVIEWED BY:	near E GCB	Brevard	E CONCINEER ON ST
	PREPARED BY: C. Strickland	REVIEWED BY:			CE C. Division
Management Sector	INIT.	DATE	Docusigned by: Jeorge C. Brown 9/30/2015		
N.Greenfield Pkwy.Garner.NC 27529					F12601ED0BEB434 DATE
					SIG. INVENTORY NO. 14-1287



RENCE	sc	HEDULE
Overlap	Α	Red
Overlap	Α	Yellow
Overlas	٨	Croop





SPECIAL NOTE The contractor is responsible for verifying that the mast arm attachment height (H1) willprovide the "Design Height" clearance from the roadway before submitting final shop drawings for approval. Verify elevation data below which was obtained by field measurement or from available project survey data. Elevation Data for Mast Arm Attachment (H1) Elevation Differences for: Pole 1 Baseline reference point at  $\bigcirc$ 0.0 ft. © Foundation @ ground level Elevation difference at High point of roadway surface +1.7 ft. Elevation difference at Edge of travelway or face of curb +1.2 ft.



WETAL FULE NU. I	METAL	POLE	No.	1
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PROJECT REFERENCE NO. SHEET NO. R-5605 Sig.2.3

	MAST ARM LOADING SC	HEDU	LE	
loading symbol	DESCRIPTION	AREA	SIZE	WEIGHT
	RIGID MOUNTED SIGNAL HEAD 12"-3 SECTION-WITH BACKPLATE	9.3 S.F.	25.5″W X 52.5″L	60 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-4 SECTION-WITH BACKPLATE	11.5 S.F.	25.5″W X 66.0″L	74 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-5 SECTION-WITH BACKPLATE	16.3 S.F.	42.0″W X 56.0″L	103 LBS
Street Name	STREET NAME SIGN RIGID MOUNTED	12.0 S.F.	18.0″W X 96.0″L	27 LBS

## <u>NOTES</u>

### DESIGN REFERENCE MATERIAL

1. Design the traffic signal structure and foundation in accordance with: • The 5th Edition 2009 AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, including all of the latest interim revisions. • The 2012 NCDOT "Standard Specifications for Roads and Structures." The latest addenda to the specifications can be found in the traffic signal project special provisions. • The 2012 NCDOT Roadway Standard Drawings.

• The traffic signal project plans and special provisions.

• The NCDOT "MetalPole Standards" located at the following NCDOT website:

https://connect.ncdot.gov/resources/safety/Pages/ITS-Design-Resources.aspx

### DESIGN REQUIREMENTS

2. Design the traffic signal structure using the loading conditions shown in the elevation views. These are anticipated worst case "design loads" and may not represent the actual loads that will be applied at the time of the installation. The contractor should refer to the traffic signal plans for the actual loads that will be applied at the time of the installation. 3. Design all signal supports using stress ratios that do not exceed 0.9.

4. The camber design for the mast arm deflection should provide an appearance of a low pitched arch where the tip or the free end of the mast arm does not deflect below horizontal when fully loaded.

5. A clamp-type bolted mast arm-to-pole connection may be used instead of the welded ring stiffened box connection shown as long as the connection meets all of the design requirements. This requires staggering the connections. Use elevation data for each arm to determine appropriate arm connection points.

6. Design base plate with 8 anchor bolt holes. Provide 2 inch x 60 inch anchor bolts. 7. The mast arm attachment height (H1) shown is based on the following design assumptions: a. Mast arm slope and deflection are not considered in determining the arm attachment height as they are assumed to offset each other.

b. Signalheads are rigidly mounted and vertically centered on the mast arm.

c. The roadway clearance height for design is as shown in the elevation views.

d. The top of the pole base plate is 0.75 feet above the ground elevation.

e. Refer to the Elevation Data Chart for the elevation differences between the proposed foundation ground leveland the high point of the roadway.

8. The pole manufacturer will determine the total height (H2) of each pole using the greater of the following:

• Mast arm attachment height (H1) plus 2 feet, or

• H1 plus 1/2 of the total height of the mast arm attachment assembly plus 1 foot. 9. If pole location adjustments are required, the contractor must gain approval from the Engineer as this may affect the mast arm lengths and arm attachment heights. The

contractor may contact the SignalDesign Section Senior StructuralEngineer for assistance at (919)773-2800.

10. The contractor is responsible for verifying that the mast arm length shown will allow proper positioning of the signalheads over the roadway.

11. The contractor is responsible for providing soil penetration testing data (SPT) to the pole manufacturer so site specific foundations can be designed.

NCDOT	Wind	Zone	4	(90	mph)	
		6		0.4		1

Prepared in the Offices of:	US 64-276 (Ashe	ville Highway	) SEAL					
Mobility and	at Davidson Biver Village							
		Connector						
F S S S S S								
	Division 14 Transylvani	a County Near Bre	vard = 1 024595					
Drai Design Sect	PLAN DATE: September 2015	REVIEWED BY: T. William	NS FROM NEER . A					
750 N.Greenfield Pkwy,Garner,NC 27529	PREPARED BY: M. Mahbooba	REVIEWED BY:	J. WILLIN					
SCALE	REVISIONS	INIT.	DATEDocuSigned by:					
0 N/A			J. J. Williams 10/1/2015					
N / A			SIG. INVENTORY NO. 14-1287					



METAL	POLE	No.	2

PROJECT REFERENCE NO. SHEET NO. Sig 2 4 R-5605

	MAST ARM LOADING SC	HEDU	LE	
loading symbol	DESCRIPTION	AREA	SIZE	WEIGHT
	RIGID MOUNTED SIGNAL HEAD 12"-3 SECTION-WITH BACKPLATE	9.3 S.F.	25.5″₩ X 52.5″L	60 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-5 SECTION-WITH BACKPLATE	16.3 S.F.	42.0″W X 56.0″L	103 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-4 SECTION-WITH BACKPLATE	11.5 S.F.	25.5″W X 66.0″L	74 LBS
1	SIGN RIGID MOUNTED	5.0 S.F.	24.0″W X 30.0″L	11 LBS
Street Name	STREET NAME SIGN RIGID MOUNTED	12.0 S.F.	18.0″W X 96.0″L	27 LBS

# <u>NOTES</u>

### DESIGN REFERENCE MATERIAL

1. Design the traffic signal structure and foundation in accordance with: • The 5th Edition 2009 AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, including all of the latest interim revisions. • The 2012 NCDOT "Standard Specifications for Roads and Structures." The latest addenda to the specifications can be found in the traffic signalproject specialprovisions. • The 2012 NCDOT Roadway Standard Drawings.

• The traffic signal project plans and special provisions.

• The NCDOT "MetalPole Standards" located at the following NCDOT website:

https://connect.ncdot.gov/resources/safety/Pages/ITS-Design-Resources.aspx

2. Design the traffic signal structure using the loading conditions shown in the elevation views. These are anticipated worst case "design loads" and may not represent the actual loads that will be applied at the time of the installation. The contractor should refer to the traffic signal plans for the actual loads that will be applied at the time of the installation. 3. Design all signal supports using stress ratios that do not exceed 0.9.

4. The camber design for the mast arm deflection should provide an appearance of a low pitched arch where the tip or the free end of the mast arm does not deflect below horizontal when fully loaded.

5. A clamp-type bolted mast arm-to-pole connection may be used instead of the welded ring stiffened box connection shown as long as the connection meets all of the design requirements. This requires staggering the connections. Use elevation data for each arm to determine appropriate arm connection points.

6. Design base plate with 8 anchor bolt holes. Provide 2 inch x 60 inch anchor bolts. 7. The mast arm attachment height (H1) shown is based on the following design assumptions: a. Mast arm slope and deflection are not considered in determining the arm attachment height as they are assumed to offset each other.

b. Signalheads are rigidly mounted and vertically centered on the mast arm.

c. The roadway clearance height for design is as shown in the elevation views.

d. The top of the pole base plate is 0.75 feet above the ground elevation.

e. Refer to the Elevation Data Chart for the elevation differences between the proposed foundation ground leveland the high point of the roadway.

8. The pole manufacturer will determine the total height (H2) of each pole using the greater of

• Mast arm attachment height (H1) plus 2 feet, or

• H1 plus 1/2 of the totalheight of the mast arm attachment assembly plus 1 foot. 9. If pole location adjustments are required, the contractor must gain approval from the Engineer as this may affect the mast arm lengths and arm attachment heights. The

contractor may contact the SignalDesign Section Senior StructuralEngineer for assistance at (919) 773-2800.

10. The contractor is responsible for verifying that the mast arm length shown will allow proper positioning of the signalheads over the roadway.

11. The contractor is responsible for providing soil penetration testing data (SPT) to the pole manufacturer so site specific foundations can be designed.

0T	Wind	Zone	4	(90	mph)	
Prenare	d in the Offi	ces of.		<u>^</u>	070	/ /

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Prepared in the Offices of:	US 64-276 (Ashe	ville Highv	vay)	SEAL	
NODITY CAR	H CARO				
Divis.	POFESSION				
FE STATE	Conn	ector		E SEAL	i i
STORE STORE	Division 14  Transylvania	024393	NS I		
noi Design Sect	PLAN DATE: September 2015	REVIEWED BY: T. Wi	lliams	5, FNGINEEK	
Greenfield Pkwy.Garner.NC 27529	PREPARED BY: M. Mahbooba	REVIEWED BY:		J.W.	
SCALE	REVISIONS	INIT.	DATE	DocuSigned by:	
0 N/A				J. J. Williams	10/1/2015
		97AD792E8E934CA	DATE		
N / A				SIG. INVENTORY NO.	14-1287





OASIS	OASIS 2070 LOOP & DETECTOR INSTALLATION CHART												
INDUCTIVE LOOPS DETECTOR PROGRAMMING													
LOOP	SIZE (FT)	DISTANCE FROM STOPBAR (FT)	TURNS	NEW LOOP	PHASE	CALLING	EXTENSION	FULL TIME DELAY	STRETCH TIME	DELAY TIME	SYSTEM LOOP	NEW CARD	
1 A	C V 40	0	2 4 2		1	Y	Y	-	-	15	-	Y	
IA	0740		2-4-2			6	Y	Y	Y	-	3	-	Y
1B	6X30	+5	2-4-2	Y	1	Y	Y	-	-	15	-	Y	
2A	6X6	355	6	Y	2	Y	Y	-	-	-	-	Y	
2B	6X40	0	2-4-2	Y	2	Y	Y	Y	-	3	-	Y	
4A	6X40	0	2-4-2	Y	4	Y	Y	-	-	3	-	Y	
4B	6X40	0	2-4-2	Y	4	Y	Y	-	-	10	-	Y	
4C	6X6	0	3	Y	4	Y	Y	-	-	15	-	Y	
6A	6X6	355	6	Y	6	Y	Y	-	-	-	-	Y	
84	6X20	+5	2-4-2	Y	8	Y	Y	-	-	_	-	Y	

PROJECT REFERENCE NO.	SHEET NO.
R-5605	Sig 3.0

# 3 Phase Fully Actuated Isolated

# NOTES

- 1. Refer to "Roadway Standard Drawings NCDOT" dated January 2012 and "Standard Specifications for Roads and Structures" dated January 2012.
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Phase 1 may be lagged.
- 4. Set all detector units to presence mode.
- 5. Locate new cabinet so as not to obstruct sight distance of vehicles turning right on red.

R/W

 $\square 6A$ 

LEGEND



Inctallation

w installation				
Prepared in the Offices of:	US 64 (Hendersonville High	nway)	SEAL	
Nobility and Section	at Davidson River Village Connector / Store Entra Division 14 Transylvania County Near PLAN DATE: September 2015 REVIEWED BY: T. Wi	e NCC Brevard Lliams	SEAL 024393	A SMA
I.Greenfield Pkwy,Garner,NC 27529	PREPARED BY: M. Mahbooba REVIEWED BY:		J. W	Tin .
SCALE	REVISIONS INIT.	DATE	DocuSigned by:	
0 40			J. J. Williams	9/29/2015
			97AD792E8E934CA	DATE
1 "=40'			SIG. INVENTORY NO.	14-1288



LOOP NO.	LOOP TERMINAL	INPUT FILE POS.	PIN NO.	INPUT ASSIGNMENT NO.	DETECTOR NO.	NEMA PHASE	CALL	EXTEND	FULL TIME DELAY	STRETCH TIME	DELAY
101	TB2-1,2	I1U	56	18	1	1	Y	Y			15
	-	J4U	48	10	26	6	Y	Y	Y		3
1B	TB5-11,12	J6L	46	8	18	1	Y	Y			15
2A	TB2-5,6	I2U	39	1	2	2	Y	Y			
2B	TB2-7,8	I2L	43	5	12	2	Y	Y	Y		3
4A	TB4-9,10	I6U	41	3	4	4	Y	Y			3
4B	TB4-11,12	I6L	45	7	14	4	Y	Y			10
4C	TB6-1 <b>,</b> 2	I7U	65	27	34	4	Y	Y			15
6A	TB3-5 <b>,</b> 6	J2U	40	2	6	6	Y	Y			
8A	TB5-9,10	J6U	42	4	8	8	Y	Y			

PROJECT REFERENCE NO.	SHEET NO.
R - 5605	Sig. 3.1

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ç	51	S2	S3	S4	S5	S6	S7	S8	59	S10	S11	S12	AUX S1	AUX S2	AUX S3	AUX S4	AUX S5	AUX S6
	1	2	13	3	4	14	5	6	15	7	8	16	g	10	17	11	12	18
	1	2	2 PED	3	4	4 PED	5	6	6 PED	7	8	8 PED	OLA	OLB	SPARE	OLC	OLD	SPARE
<b>★</b>	82	22,23	NU	NU	41,42	NU	NU	61,62	NU	NU	81,82	NU	11	NU	NU	<b>★</b> 21	NU	NU
	*	128			101			134			107							
		129			102			135			108							
		130			103			136			109							
													A121			A114		
	126												A122			A115		
													A123			A116		
27	127																	

lectrical Detail -	Sheet 1 of 2			
CTRICAL AND PROGRAMMING DETAILS FOR:	US 64 (Henderso a	nville Highv t	way)	SEAL CARO
COLOR HODILITY ONCOLOR DIVIS	Davidson Ri Connector / S Division 14 Transylva	SEAL 022013		
HILL MARCANEL	PREPARED BY: C. Strickland REVISIONS	REVIEWED BY: GCB	DATE	DocuSigned by:
N.Greenfield Pkwy.Garner.NC 27529			· · · · · · · · · · · · · · · · · · ·	Seconde         Seconde         9/30/2015           F12601ED0BEB434         DATE           SIG. INVENTORY NO.         14-1288



RENCE	SC	CHEDULE
Overlap	Α	Red
Overlap	Α	Yellow
Overlap	Α	Green





PROJECT REFERENCE NO. SHEET NO. Sig 3.3 R-5605

	MAST ARM LOADING SC	HEDU	LE	
loading symbol	DESCRIPTION	AREA	SIZE	WEIGHT
	RIGID MOUNTED SIGNAL HEAD 12"-3 SECTION-WITH BACKPLATE	9.3 S.F.	25.5″₩ X 52.5″L	60 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-4 SECTION-WITH BACKPLATE	11.5 S.F.	25.5″W X 66.0″L	74 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-5 SECTION-WITH BACKPLATE	16.3 S.F.	42.0″W X 56.0″L	103 LBS
Street Name	STREET NAME SIGN RIGID MOUNTED	16.0 S.F.	24.0″W X 96.0″L	36 LBS

### <u>NOTES</u>

### DESIGN REFERENCE MATERIAL

1. Design the traffic signalstructure and foundation in accordance with: • The 5th Edition 2009 AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, including all of the latest interim revisions. • The 2012 NCDOT "Standard Specifications for Roads and Structures." The latest addenda to the specifications can be found in the traffic signalproject specialprovisions. • The 2012 NCDOT Roadway Standard Drawings.

• The traffic signal project plans and special provisions.

• The NCDOT "MetalPole Standards" located at the following NCDOT website:

https://connect.ncdot.gov/resources/safety/Pages/ITS-Design-Resources.aspx

2. Design the traffic signal structure using the loading conditions shown in the elevation views. These are anticipated worst case "design loads" and may not represent the actual loads that will be applied at the time of the installation. The contractor should refer to the traffic signal plans for the actual loads that will be applied at the time of the installation. 3. Design all signal supports using stress ratios that do not exceed 0.9.

4. The camber design for the mast arm deflection should provide an appearance of a low pitched arch where the tip or the free end of the mast arm does not deflect below horizontal when fully loaded.

5. A clamp-type bolted mast arm-to-pole connection may be used instead of the welded ring stiffened box connection shown as long as the connection meets all of the design

6. Design base plate with 8 anchor bolt holes. Provide 2 inch x 60 inch anchor bolts. 7. The mast arm attachment height (H1) shown is based on the following design assumptions: a. Mast arm slope and deflection are not considered in determining the arm attachment height as they are assumed to offset each other.

b. Signalheads are rigidly mounted and vertically centered on the mast arm.

c. The roadway clearance height for design is as shown in the elevation views.

d. The top of the pole base plate is 0.75 feet above the ground elevation.

e. Refer to the Elevation Data Chart for the elevation differences between the proposed foundation ground leveland the high point of the roadway.

8. The pole manufacturer will determine the total height (H2) of each pole using the greater of

• Mast arm attachment height (H1) plus 2 feet, or

• H1 plus 1/2 of the total height of the mast arm attachment assembly plus 1 foot. 9. If pole location adjustments are required, the contractor must gain approval from the Engineer as this may affect the mast arm lengths and arm attachment heights. The contractor may contact the SignalDesign Section Senior StructuralEngineer for assistance at (919) 773-2800.

10.The contractor is responsible for verifying that the mast arm length shown willallow proper positioning of the signalheads over the roadway.

11. The contractor is responsible for providing soilpenetration testing data (SPT) to the pole manufacturer so site specific foundations can be designed.

DOT Wind Zone	4 (90 mph)
Prepared in the Offices of:	US 64 (Hendersonville Highway) SEAL
NODILITY ON STA	at
O VICE STREET	Davidson River Village
	Connector / Store Entrance
	Division 14 Transylvania County Near Brevard
Drai Design Section	PLAN DATE: September 2015 REVIEWED BY: T. Williams
Greenfield Pkwy.Garner.NC 27529	PREPARED BY: M. Mahbooba REVIEWED BY:
SCALE	REVISIONS INIT. DATE DocuSigned by ///////////////////////////////////
0 N/A	J. J. Williams 10/1/2015
N / A	SIG. INVENTORY NO. 4-1288



PROJECT REFERENCE NO. SHEET NO. Sig 3.4 R-5605

	MAST ARM LOADING SC	HEDU	LE	
loading symbol	DESCRIPTION	AREA	SIZE	WEIGHT
	RIGID MOUNTED SIGNAL HEAD 12"-3 SECTION-WITH BACKPLATE	9.3 S.F.	25.5″₩ X 52.5″L	60 LBS
0000	RIGID MOUNTED SIGNAL HEAD 12"-4 SECTION-WITH BACKPLATE	11.5 S.F.	25.5″W X 66.0″L	74 LBS
	RIGID MOUNTED SIGNAL HEAD 12"-5 SECTION-WITH BACKPLATE	16.3 S.F.	42.0″W X 56.0″L	103 LBS
Street Name	STREET NAME SIGN RIGID MOUNTED	16.0 S.F.	24.0″W X 96.0″L	36 LBS

### <u>NOTES</u>

### DESIGN REFERENCE MATERIAL

1. Design the traffic signalstructure and foundation in accordance with: • The 5th Edition 2009 AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals, including all of the latest interim revisions. • The 2012 NCDOT "Standard Specifications for Roads and Structures." The latest addenda to the specifications can be found in the traffic signal project special provisions. • The 2012 NCDOT Roadway Standard Drawings.

• The traffic signal project plans and special provisions.

• The NCDOT "MetalPole Standards" located at the following NCDOT website:

https://connect.ncdot.gov/resources/safety/Pages/ITS-Design-Resources.aspx

2. Design the traffic signal structure using the loading conditions shown in the elevation views. These are anticipated worst case "design loads" and may not represent the actual loads that will be applied at the time of the installation. The contractor should refer to the traffic signal plans for the actual loads that will be applied at the time of the installation. 3. Design all signal supports using stress ratios that do not exceed 0.9.

4. The camber design for the mast arm deflection should provide an appearance of a low pitched arch where the tip or the free end of the mast arm does not deflect below horizontal when fully loaded.

5. A clamp-type bolted mast arm-to-pole connection may be used instead of the welded ring stiffened box connection shown as long as the connection meets all of the design

6. Design base plate with 8 anchor bolt holes. Provide 2 inch x 60 inch anchor bolts. 7. The mast arm attachment height (H1) shown is based on the following design assumptions: a. Mast arm slope and deflection are not considered in determining the arm attachment height as they are assumed to offset each other.

b. Signalheads are rigidly mounted and vertically centered on the mast arm.

c. The roadway clearance height for design is as shown in the elevation views.

d. The top of the pole base plate is 0.75 feet above the ground elevation.

e. Refer to the Elevation Data Chart for the elevation differences between the proposed foundation ground leveland the high point of the roadway.

8. The pole manufacturer will determine the total height (H2) of each pole using the greater of

• Mast arm attachment height (H1) plus 2 feet, or

• H1 plus 1/2 of the total height of the mast arm attachment assembly plus 1 foot. 9. If pole location adjustments are required, the contractor must gain approval from the Engineer as this may affect the mast arm lengths and arm attachment heights. The contractor may contact the SignalDesign Section Senior StructuralEngineer for assistance at (919) 773-2800.

10.The contractor is responsible for verifying that the mast arm length shown willallow proper positioning of the signalheads over the roadway.

11. The contractor is responsible for providing soilpenetration testing data (SPT) to the pole manufacturer so site specific foundations can be designed.

DOT Wind Zone	4 (90 mph)
Prepared in the Offices of:	US 64 (Hendersonville Highway) SEAL
NODILITY ONA	at
	Davidson River Village
Liston	Connector / Store Entrance
State and the state of the stat	Division 14 Transylvania County Near Brevard E 📜 <sup>024393</sup> 🤅 E
On OF TRADECINO	PLAN DATE: September 2015 REVIEWED BY: T. Williams
Greenfield Pkwy,Garner,NC 27529	PREPARED BY: M. Mahbooba REVIEWED BY:
SCALE	REVISIONS INIT. DATE DocuSigned by:///////////
O N/A	J. J. Williams 10/1/2015
N / A	SIG. INVENTORY NO. 4-288









![](_page_16_Figure_0.jpeg)

-44E8E32E147E4C4.

SIG. INVENTORY NO.

NONE

![](_page_16_Figure_1.jpeg)

![](_page_17_Figure_0.jpeg)

NONE

![](_page_17_Figure_5.jpeg)

DATE

-44E8E32E147E4C4.

SIG. INVENTORY NO.

![](_page_18_Figure_0.jpeg)

26-AUG-2014 08:44 S:\*ITS&SU\*ITS Signals\*Signal Design Section\*Eastern Region\*M Sheets\*2012\_M7\_Con Details Foundation

nchor Bo	1t	Details	PROJECT REFERENCE NO. R-5605	SHEET NO. Sig. M7
Shown for Clar	<u>'</u> ity)		•	
Foundation	2"- A Hea wit Top Anc (Sa	Pole Base Plate I" Chamfer (Typ) 5" Foundation Projection Above Ground Level Above Ground Level Anchor Bolts (Typ) vy Hex Nut h Flat Washer and Bottom (Typ) hor Bolt Lock Plate me as Base Plate Template)		s – Foundations
ion		Notes		
S		1. The number of C-bars i	s based on	e e
		foundation depth and/o For standard foundatio sheets M 8 and M 9 for	r as required. ns, see details.	
		<ol> <li>Circular tie reinforci be vertically adjusted at a depth between 2'-0 to facilitate the inst electrical conduit ent cage.</li> </ol>	ng rings may by +/- 3" O" and 3'-0" allation of ering in the	tion
2'-6"		3. The length of V1-bars foundation depth. For foundations, see sheet	is based on standard s M 8 and M 9	Ú Ú
2-0		for details. Vertical bars (V1) may be horiz adjusted by +/-3"to fa	reinforcing ontally cilitate the	tr
-2″ Nonmetallic onduit (Stub an	d	installation of electr entering into the cage	ical conduit	U S
ap unused condu or future use)	it	4. Provide Vertical reinf as required per design M 8 and M9 for details	orcement . See sheets	S
Prepared in the Offices of:			SEAL	
Nobility one solution of the solution of the solution of the solution of the solution of the solution of the solution of the solution of the solution of the solution of the s	PLAN DATE:	Construction Details Foundations AUGUST 2013 DESIGNED BY: K.C. DUR	IGON	ROVAL NA CHINA
Greenfield Pkwy,Garner,NC 27529 SCALE	PREPARED BY	N. BITTING REVIEWED BY: D.C. SAR	AKAR DATE DocuSigned by:	8/26/2014
	·			0/20/2014 DATE

	SATURATED SOIL CONDITION																	
	STANDARD STRAIN POLES								S 48″	TANDAR Diameter D	D FOU	NDATIO Length (L)	NS – Feet			Reinfor	cement	
				Base	Reactior	ns at the	Pole Base		C	ay			Sand		Longi	tudinal	Stirr	ups
		Case No.	Pole Height (Ft.)	Plate BC (In.)	Axial (kip)	Shear (kip)	Moment (ft–kip)	Medium N–Value 4–8	Stiff N–Value 9–15	Very Stiff N–Value 16–30	Hard N–Value >30	Loose N–Value 4–10	Medium N–Value 11–30	Dense N–Value >30	Bar Size (#)	Quantity	Bar Size (#)	Spacing (in.)
W	L	S26L3	26	25	2	11	270	19	13	9	8	17	14.5	12.5	8	13	4	12
N D	Ġ H	S30L3	30	25	2	11	300	20	13.5	9	8	17.5	15	13	8	14	4	12
Z	T	S35L3	35	25	3	11	320	20	13.5	9.5	8	17.5	15	13	8	15	4	12
N   E	H E A	S30H3	30	29	3	16	450	24.5	17	13	11	21	17.5	15	8	18	4	12
1	V Y	S35H3	35	29	4	16	515	26	17.5	12	8.5	22	18.5	16	8	20	4	12
W T	L	S26L2	26	23	2	10	245	18	12.5	8.5	8	16.5	14	12	8	13	4	12
N D	Ġ H	S30L2	30	23	2	10	270	19	12.5	9	8	16.5	14	12.5	8	13	4	12
Z	Ť	S35L2	35	23	3	10	300	19.5	13	9	8	17	14.5	13	8	14	4	12
N E	H E A	S30H2	30	29	3	15	415	25.5	15.5	11	8	20	17	14.5	8	17	4	12
2	V Y	S35H2	35	29	4	15	475	25	16.5	11.5	8	21	17.5	15.5	8	19	4	12
W	L T	S26L2	26	23	2	10	245	18	12.5	8.5	8	16.5	14	12	8	13	4	12
N D	Ġ H	S30L2	30	23	2	10	270	19	12.5	9	8	16.5	14	12.5	8	13	4	12
Z	T	S35L2	35	23	3	10	300	19.5	13	9	8	17	14.5	13	8	14	4	12
N E	H E A	S30H2	30	29	3	15	415	25.5	15.5	11	8	20	17	14.5	8	17	4	12
3	V Y	S35H2	35	29	4	15	475	25	16.5	11.5	8	21	17.5	15.5	8	19	4	12
W T	L	S26L1	26	22	2	8	190	16	11	8	8	15	12.5	11	8	12	4	12
N D	Ĝ H	S30L1	30	22	2	8	205	16.5	11.5	8	8	15	13	11.5	8	12	4	12
Z	Т	S35L1	35	22	3	8	230	17	12	8	8	15.5	13.5	11.5	8	12	4	12
N E	H E A	S30H1	30	25	3	12	320	20.5	14	9.5	8	18	15	13.5	8	15	4	12
4	V Y	S35H1	35	25	4	12	350	21	14.5	10	8	18.5	15.5	13.5	8	16	4	12
WI	L	S26L2	26	23	2	10	245	18	12.5	8.5	8	16.5	14	12	8	13	4	12
N   D	Ġ H	S30L2	30	23	2	10	270	19	12.5	9	8	16.5	14	12.5	8	13	4	12
Z	Ť	S35L2	35	23	3	10	300	19.5	13	9	8	17	14.5	13	8	14	4	12
N   E	H E A	S30H2	30	29	3	15	415	25.5	15.5	11	8	20	17	14.5	8	17	4	12
5	V Y	S35H2	35	29	4	15	475	25	16.5	11.5	8	21	17.5	15.5	8	19	4	12

48" Dia. Foundations Concrete Volume (cubic yards) = (0.465) x Foundation Depth

			PROJECT REF	ERENCE NO.	SHEET NO.
Fabrication D	)esian Notes	) 5 <b>:</b>	N-30L		51g. M8
Values shown in t column represent allowed for desig Min. base plate t <b>Dundation Sele</b> Perform a standar foundation site t Select the approp	the "Reactions a the minimum acc on using a design thickness (T) is <b>CtiON:</b> The penetration of the observation of the observat	at the Po ceptable gn CSR of s 2.0 inc test at e ″ value. e from M	le Base" capacity 1.00. hes. ach prop 1 drawin	osed g.	ondition
Select the soil t describes the soi Get the appropria plans or from the Select the approp	ype (Clay or Sa l characterist ate standard po Engineer oriate column in Value Select	and) that ics. le case n n the cha	best umber fr rt based	om the on	Soil C
Soll type and N based on the pole The foundation de and the row inter Reference Drilled Design Methods, F S30H1 - Hard Clay-S S30H2 - Hard Clay-S S30H3 - Hard Clay-S - Dense Sand- S35H1 - Hard Clay - S35H2 - Very Stiff - Hard Clay- - Dense Sand- S35H3 - Very Stiff - Dense Sand-	value. Select e load case opth is the value sect. d Shafts: Constr FHWA -IF-99-025 Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing Stirrup Spacing	the appr ue where ruction P : 6 in. c : 6 in. c g: 6 in. og: 6 in. oacing: 6 g: 6 in. oacing: 6 g: 6 in. oacing: 6 g: 6 in.	opriate the colu rocedure /c /c /c c/c c/c c/c c/c in. c/c c/c c/c	nn s and	Foundation-Saturated
					Pole
					Strain
					Standard
Prepared in the Offices of: Nobility and Control of the offices of: Nobility and Nobility and N	Standard St Foundation f Soil Con PLAN DATE:SEPTEMBER 2013 PREPARED BY: N. BITTING REVISIONS	rain Pol or Satura dition DESIGNED BY: C. REVIEWED BY: D	e ated B COGDELL . SARKAR INIT. DATE	Decusioned by	EAL SSIONAR EAL BO94 C. SARTINI
				Debesh (. S	arkar 8/26/2014

None

— DocuSigned by: Debeshe (.	Sarkars	8/26/2014
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	DRY SOIL CONDITION																	
			ST	stan Rain	NDARD N POL	) ES			S 48″	TANDAR Diameter D	NDARD FOUNDATIONS					Reinfor	cement	
				Base	Reaction	ns at the	Pole Base		C	ay			Sand		Longitudinal Stirr		rups	
		Case No.	Pole Height (Ft.)	Plate BC (In.)	Axial (kip)	Shear (kip)	Moment (ft–kip)	Medium N–Value 4–8	Stiff N–Value 9–15	Very Stiff N–Value 16–30	Hard N–Value >30	Loose N–Value 4–10	Medium N–Value 11–30	Dense N–Value > 30	Bar Size (#)	Quantity	Bar Size (#)	Spacing (in.)
W	Ļ	S26L3	26	25	2	11	270	18	12.5	9	8	14.5	11	10	8	13	4	12
N D	ц С Ц	S30L3	30	25	2	11	300	18.5	13	9	8	15	11.5	10	8	14	4	12
Z O	Ť	S35L3	35	25	3	11	320	19	13.5	9.5	8	15	11.5	10.5	8	15	4	12
N E	H E	S30H3	30	29	3	16	450	23	16	11	8	17.5	13.5	11.5	8	18	4	12
1	V Y	S35H3	35	29	4	16	515	24.5	16.5	12	8.5	18.5	14	12	8	20	4	12
W	Ļ	S26L2	26	23	2	10	245	17	12	8.5	8	14	11	9.5	8	13	4	12
л N D	G	S30L2	30	23	2	10	270	18	12.5	8.5	8	14.5	11	10	8	13	4	12
Z	Ť	S35L2	35	23	3	10	300	18.5	13	9	8	14.5	11.5	10	8	14	4	12
N E	H E A	S30H2	30	29	3	15	415	22	15	10.5	8	17	13	11.5	8	17	4	12
2	V Y	S35H2	35	29	4	15	475	23.5	16	11.5	8	18	13.5	12	8	19	4	12
W	L	S26L2	26	23	2	10	245	17	12	8.5	8	14	11	9.5	8	13	4	12
N D	Ġ H	S30L2	30	23	2	10	270	18	12.5	8.5	8	14.5	11	10	8	13	4	12
Z	T	S35L2	35	23	3	10	300	18.5	13	9	8	14.5	11.5	10	8	14	4	12
N E	H E A	S30H2	30	29	3	15	415	22	15	10.5	8	17	13	11.5	8	17	4	12
3	V Y	S35H2	35	29	4	15	475	23.5	16	11.5	8	18	13.5	12	8	19	4	12
W	LI	S26L1	26	22	2	8	190	15.5	10.5	8	8	13	10	9	8	12	4	12
Ň D	G   H	S30L1	30	22	2	8	205	15.5	11	8	8	13	10	9	8	12	4	12
Z O	T	S35L1	35	22	3	8	230	16.5	11.5	8	8	13.5	10.5	9	8	12	4	12
Ň E	H E A	S30H1	30	25	3	12	320	19.5	13.5	9.5	8	15	12	10.5	8	15	4	12
4	V Y	S35H1	35	25	4	12	350	20	14	10	8	15.5	12	10.5	8	15	4	12
W I	L	S26L2	26	23	2	10	245	17	12	8.5	8	14	11	9.5	8	13	4	12
N D	Ġ H	S30L2	30	23	2	10	270	18	12.5	8.5	8	14.5	11	10	8	13	4	12
Z O	<b>T</b>	S35L2	35	23	3	10	300	18.5	13	9	8	14.5	11.5	10	8	14	4	12
N E	H E A	S30H2	30	29	3	15	415	22	15	10.5	8	17	13	11.5	8	17	4	12
5	V Y	S35H2	35	29	4	15	475	23.5	16	11.5	8	18	13.5	12	8	19	4	12

26-AUG-2014 08:38 S:\*ITS&SU\*ITS Signals\*Signal Design Section\*Eastern Region\*M Sheets\*2012\_M9\_Standard Foundations [

48" Dia. Foundations Concrete Volume (cubic yards) = (0.465) x Foundation Depth

	PROJECT REFERENCE NO.	SHEET NO.
Fabrication Design Notes		s⊥y. M9
Values shown in the "Reactions at the Po column represent the minimum acceptable allowed for design using a design CSR o Min. base plate thickness (T) is 2.0 inc	le Base" capacity f 1.00. hes.	
oundation Selection:		
Perform a standard penetration test at e	ach proposed	<b>—</b>
foundation site to determine "N" value.		
Select the appropriate wind zone from M Select the soil type (Clay or Sand) that describes the soil characteristics	1 drawing. best	
Get the appropriate standard pole case n	umber from the	
plans or from the Engineer. Select the appropriate column in the cha	rt based on	
soil type and "N" value. Select the appr	opriate row	
based on the pole load case.	the column	Š
and the row intersect.		
Reference Drilled Shafts: Construction P	rocedures and	
Design Methods, FHWA -1F-99-025		
30H1 - Hard Clay-Stirrup Spacing: 6 in. c	/c	
- Dense Sand-Stirrup Spacing: 6 in.	c/c	Ż
30H2 - Very Stiff Clay: Stirrup Spacing: - Hard Clay: Stirrup Spacing: 6 in.	6 in. c/c c/c	0
- Medium Clay: Stirrup Spacing: 6 in	. c/c	
- Dense Sand: Stirrup Spacing: 6 in.	c/c 6 in c/c	
- Hard Clay: Stirrup Spacing: 6 in.	c/c	
- Medium Clay: Stirrup Spacing: 6 in	. c/c	
- Dense Sand: Stirrup Spacing: 6 in. 35H1 - Hard Clav: tirrup Spacing: 6 in. c	C/C /C	o l
- Dense Sand: Stirrup Spacing: 6 in.	c/c	
35H2 - Very Stiff Clay: Stirrup Spacing:	6 in. c/c	
- Medium Clay: Stirrup Spacing: 6 in	. c/c	
- Dense Sand: Stirrup Spacing: 6 in.	c/c	0
- Hard Clay: Stirrup Spacing: 6 in.	c/c	
- Medium Clay: Stirrup Spacing: 6 in	. c/c	
- Dense Sand: Stirrup Spacing: 6 in.	c/c	
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noi Design	Secti	PLAN DATE: SEPTEMBER 2013	DESIGNED BY:	C.B COG	DELL	INGINEER OF
750 N.Greenfield Pkwy.G	arner,NC 27529	PREPARED BY: N. BITTING	REVIEWED BY:	D. SAR	KAR	THE C. SAMIN
SCALE		REVISIONS		INIT.	DATE	
0	NA					Docusigned by:
						VC//CSU (. SUPER 8/26/2014
None					+	

![](_page_21_Figure_0.jpeg)