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BEGIN STATE PROJECT B-5404

# STR NORTH CARE VLSADEPER TRANSPOR

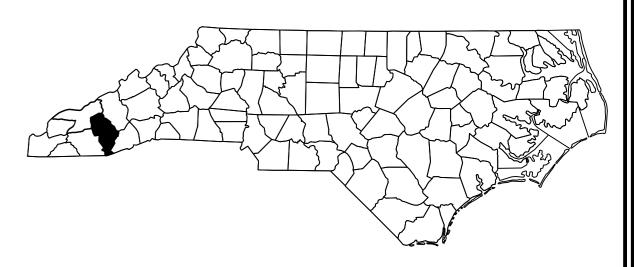


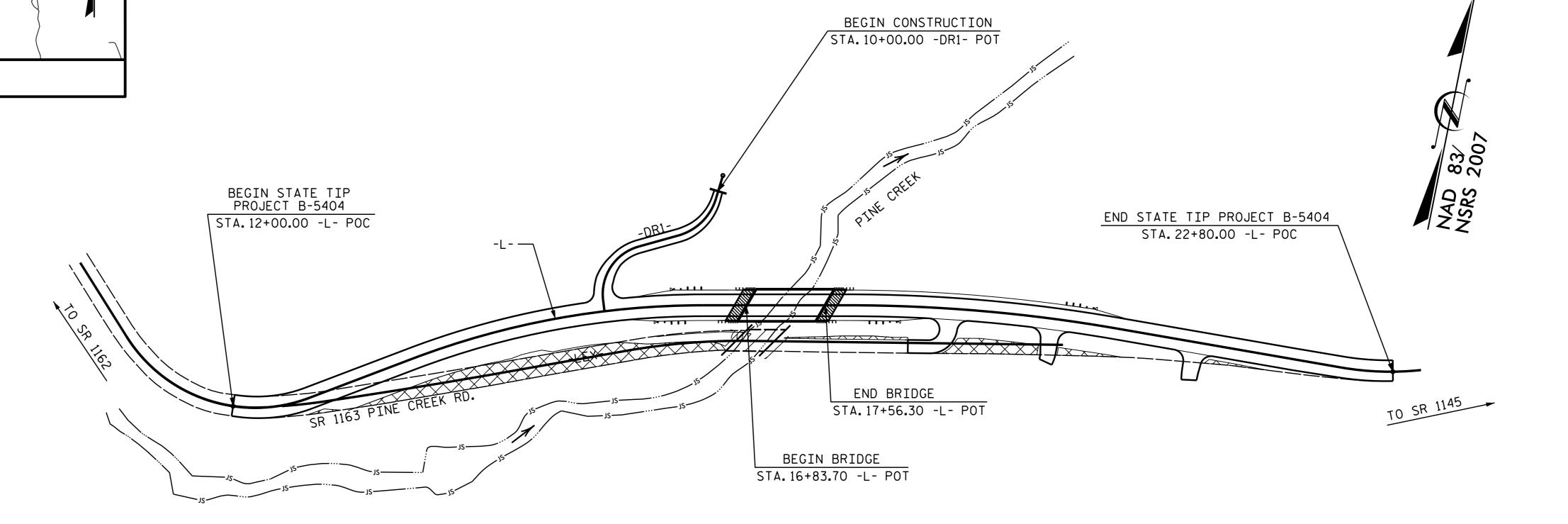
# JACKSON COUNTY

LOCATION: BRIDGE NO. 136 OVER PINE CREEK
ON SR 1163 (PINE CREEK ROAD)

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE

STATE	STATE	PROJECT REPERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	B-4	5404		
STAT	B PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	NOI
46	119.1.1	BRZ-1163 (10)	P.E	
46	119.2.FD1	BRZ-1163 (10)	ROV	√, UTIL
46	119.3.FD1	BRZ-1163 (10)	СО	NST.





# STRUCTURE

VICINITY MAP

# DESIGN DATA

END STATE PROJECT B-5404//

> ADT 2016 = 1,020 ADT 2036 = 1,500

> > K = 12 %

D = 70 %T = 7 %

V = 40 MPH \* TTST = 1% DUAL 6%

FUNC CLASS = RURAL LOCAL SUB REGIONAL TIER

# PROJECT LENGTH

LENGTH ROADWAY F.A. PROJECT B-5404 = LENGTH STRUCTURE F.A. PROJECT B-5404 = TOTAL LENGTH F.A. PROJECT B-5404 =

-5404 = 0.014 MILES 4 = 0.205 MILES

0.191 MILES

#### Prepared in the Office of:

# **DIVISION OF HIGHWAYS**

STRUCTURES MANAGEMENT UNIT
1000 BIRCH RIDGE DR.
RALEIGH, N.C. 27610

2012 STANDARD SPECIFICATIONS

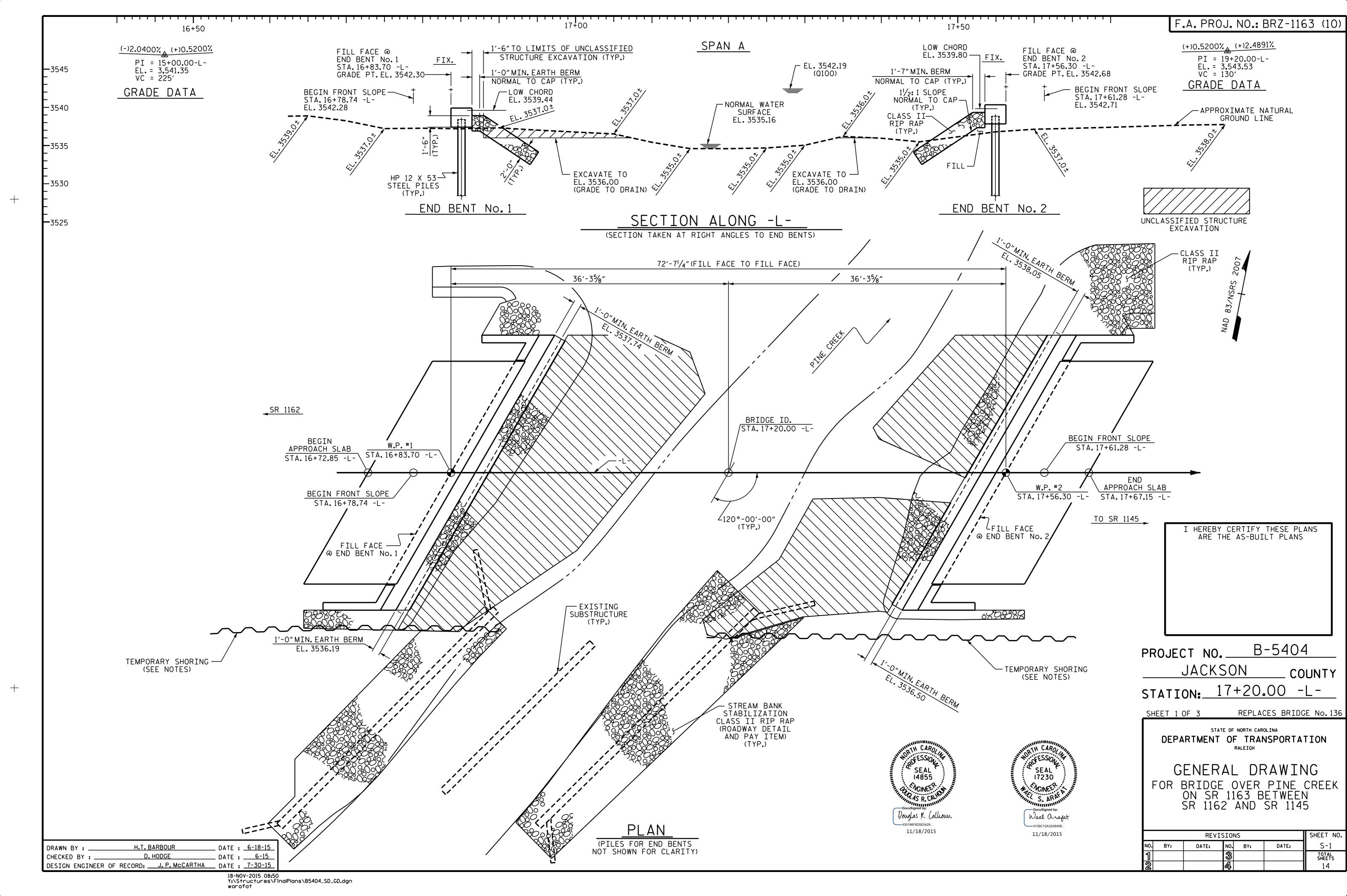
LETTING DATE: JANUARY 19, 2016

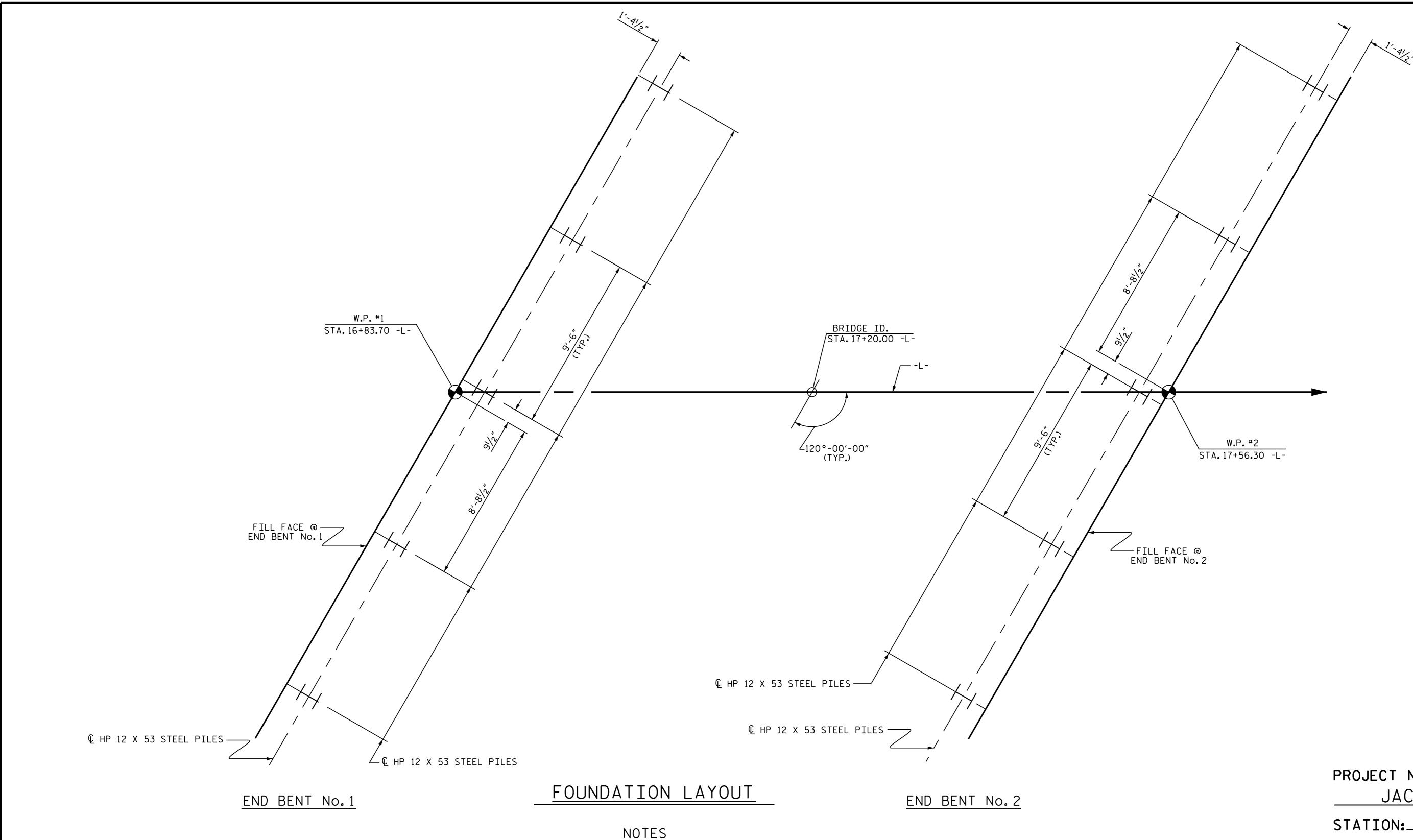
D. R. CALHOUN, P.E.

PROJECT ENGINEER

W. S. ARAFAT, P.E.

PROJECT DESIGN ENGINEER





FOR PILES, SEE SPECIAL PROVISIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 167 TONS PER PILE.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 167 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING, FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFCATIONS.

IF NECESSARY, PREDRILL PILE LOCATIONS ON THE CENTERLINE AND LEFT OF CENTERLINE AT BOTH END BENTS WITH EQUIPMENT THAT WILL RESULT IN A MAXIMUM PREDRILLING DIAMETER OF 12% FOR PREDRILLING FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

SEAL 17230 Wael Orafat

4139C12A32AB406... 11/18/2015

PROJECT NO. B-5404 JACKSON COUNTY

17+20.00 -L-

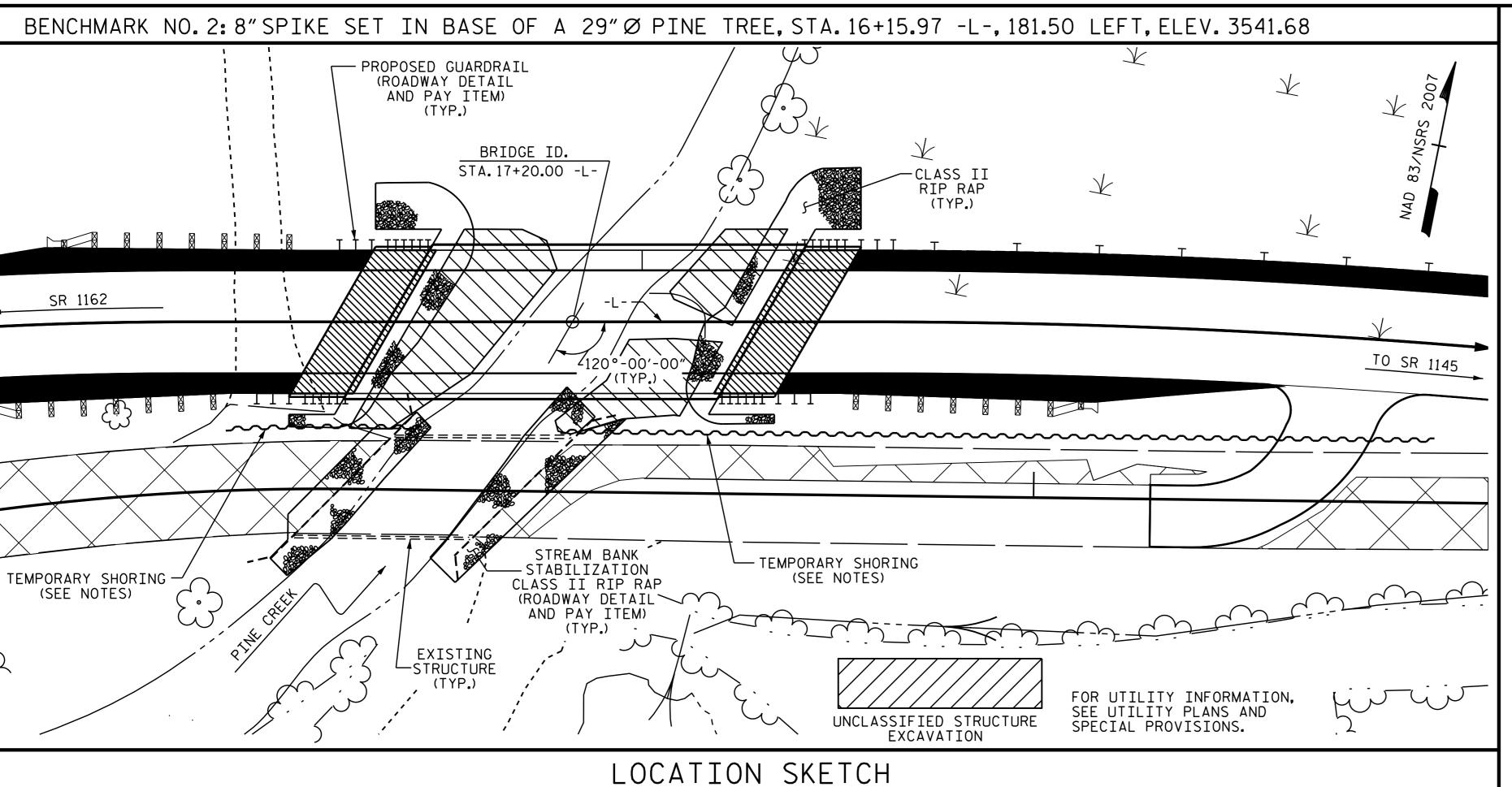
SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE OVER PINE CREEK ON SR 1163 BETWEEN SR 1162 AND SR 1145

SHEET NO. REVISIONS S-2 NO. BY: DATE: BY: DATE: 14

H. T. BARBOUR \_ DATE : <u>6-17-15</u> DRAWN BY : \_\_ DATE : <u>6-15</u> D. HODGE CHECKED BY : \_\_  STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENTS. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.



ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 17+20.00 -L-."

THE MATERIAL SHOWN IN THE HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF 2 SPANS @ 17'-8" WITH 4" ASPHALT WEARING SURFACE ON 4"X 8"TIMBERS ON 8 LINES OF 16"CONTINUOUS I-BEAMS @ 2'-7"CTS. AND A CLEAR ROADWAY WIDTH OF 19.208 FT., ON TIMBER CAPS AND TIMBER POST AND SILLS AT THE END BENTS AND CRUTCH BENT LOCATED UP STREAM FROM THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE POSTED LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

# HYDRAULIC DATA

DESIGN DISCHARGE 1100 CFS
FREQUENCY OF DESIGN FLOOD 25 YEARS
DESIGN HIGH WATER ELEVATION 3540.10
DRAINAGE AREA 4.07 SQ. MI.
BASE DISCHARGE (Q100) 1790 CFS
BASE HIGH WATER ELEVATION 3542.19

# OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE\_\_\_\_\_\_ 1835 CFS FREQUENCY OF OVERTOPPING FLOOD\_\_ 100 (+) YR. OVERTOPPING FLOOD ELEVATION\_\_\_\_\_ 3542.40 (@ STA. 15+66.80 -L-)

SHEET

Wall Orafat

11/18/2015

PROJECT NO. B-5404

JACKSON COUNTY

STATION: 17+20.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING
FOR BRIDGE OVER PINE CREEK
ON SR 1163 BETWEEN
SR 1162 AND SR 1145

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-3

TOTAL SHEETS

14

	——————————————————————————————————————															
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	NFORCING HP 12 X 53 STEEL PILES PO		STEEL PILE POINTS	PREDRILLING FOR PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	I FUR	ELASTOMERIC BEARINGS	PRES CO	" X 2'-0" STRESSED NCRETE ED SLABS
	LUMP SUM	EA.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EA.	LIN.FT.	LIN.FT.	TONS	SQ. YARDS	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE					LUMP SUM						140.00			LUMP SUM	10	700
END BENT NO. 1			LUMP SUM	14.8		2206	5	115	5	52		90	100			
END BENT NO. 2			LUMP SUM	14.8		2206	5	115	5	50		95	105			
TOTAL	LUMP SUM	1	LUMP SUM	29.6	LUMP SUM	4412	10	230	10	102	140.00	185	205	LUMP SUM	10	700

# LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIM	IT S	ГАТЕ				SE	RVICE	III	LIMI	T STA	TE	
						-				MOMENT					SHEAR						MOMENT			•
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A		1.06		1.75	0.248	1.14	70′	EL	34.423	0.655	1.06	70′	EL	6.885	0.80	0.248	1.11	70′	EL	34.423	
DESIGN		HL-93(0pr)	N/A		1.374		1.35	0.248	1.48	70′	EL	34.423	0.655	1.37	70′	EL	6.885	N/A						
LOAD RATING		HS-20(Inv)	36.000	<b>(2)</b>	1.32	47.508	1.75	0.248	1.48	70′	EL	34.423	0.655	1.32	70′	EL	6.885	0.80	0.248	1.44	70′	EL	34.423	
IVATINO		HS-20(0pr)	36.000		1.711	61.585	1.35	0.248	1.91	70′	EL	34.423	0.655	1.71	70′	EL	6.885	N/A						
		SNSH	13.500		3.204	43.258	1.4	0.248	4.12	70′	EL	34.423	0.655	3.9	70′	EL	6.885	0.80	0.248	3.20	70′	EL	34.423	
		SNGARBS2	20.000		2.403	48.063	1.4	0.248	3.09	70′	EL	34.423	0.655	2.78	70′	EL	6.885	0.80	0.248	2.40	70′	EL	34.423	
		SNAGRIS2	22.000		2.282	50 <b>.</b> 21	1.4	0.248	2.94	70′	EL	34.423	0.655	2.58	70′	EL	6 <b>.</b> 885	0.80	0.248	2.28	70′	EL	34.423	
		SNCOTTS3	27.250		1.595	43.463	1.4	0.248	2.05	70′	EL	34.423	0.655	1.95	70′	EL	6.885	0.80	0.248	1.59	70′	EL	34.423	
	S S	SNAGGRS4	34.925		1.339	46.755	1.4	0.248	1.72	70′	EL	34.423	0.655	1.62	70′	EL	6 <b>.</b> 885	0.80	0.248	1.34	70′	EL	34.423	
		SNS5A	35.550		1.309	46.526	1.4	0.248	1.68	70′	EL	34.423	0.655	1.65	70′	EL	6.885	0.80	0.248	1.31	70′	EL	34.423	
		SNS6A	39.950		1.203	48.069	1.4	0.248	1.55	70′	EL	34.423	0.655	1.5	70′	EL	6 <b>.</b> 885	0.80	0.248	1.20	70′	EL	34.423	
LEGAL		SNS7B	42.000		1.146	48.129	1.4	0.248	1.47	70′	EL	34.423	0.655	1.48	70′	EL	6 <b>.</b> 885	0.80	0.248	1.15	70′	EL	34.423	
LOAD RATING		TNAGRIT3	33.000		1.468	48.444	1.4	0.248	1.89	70′	EL	34.423	0.655	1.79	70′	EL	6 <b>.</b> 885	0.80	0.248	1.47	70′	EL	34.423	
NATINO		TNT4A	33.075		1.475	48.79	1.4	0.248	1.9	70′	EL	34.423	0.655	1.74	70′	EL	6 <b>.</b> 885	0.80	0.248	1.48	70′	EL	34.423	
		TNT6A	41.600		1.208	50.272	1.4	0.248	1.55	70′	EL	34.423	0.655	1.58	70′	EL	6.885	0.80	0.248	1.21	70′	EL	34.423	
	TST	TNT7A	42.000		1.216	51.061	1.4	0.248	1.56	70′	EL	34.423	0.655	1 <b>.</b> 55	70′	EL	6.885	0.80	0.248	1.22	70′	EL	34.423	
		TNT7B	42.000		1.261	52 <b>.</b> 955	1.4	0.248	1.62	70′	EL	34.423	0.655	1.44	70′	EL	6.885	0.80	0.248	1.26	70′	EL	34.423	
		TNAGRIT4	43.000		1.197	51.476	1.4	0.248	1.54	70′	EL	34.423	0.655	1.4	70′	EL	6.885	0.80	0.248	1.20	70′	EL	34.423	
		TNAGT5A	45.000		1.128	50.745	1.4	0.248	1.45	70′	EL	34.423	0.655	1.39	70′	EL	6.885	0.80	0.248	1.13	70′	EL	34.423	
		TNAGT5B	45.000	<b>(3)</b>	1.113	50.088	1.4	0.248	1.43	70′	EL	34.423	0.655	1.33	70′	EL	6.885	0.80	0.248	1.11	70′	EL	34.423	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

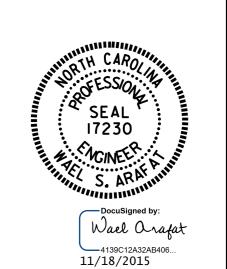
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-5404

JACKSON COUNTY

STATION: 17+20.00-L-



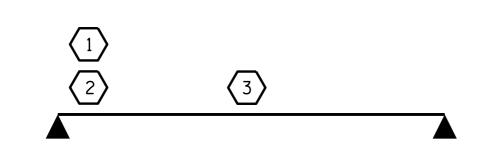
STATE OF NORTH CAROLINA

STANDARD

LRFR SUMMARY FOR
70' CORED SLAB UNIT
120° SKEW

(NON-INTERSTATE TRAFFIC)

	REVIS	SIO	NS		SHEET N
BY:	DATE:	NO.	BY:	DATE:	S-4
		3			TOTAL SHEETS
		$\boldsymbol{A}$			1./

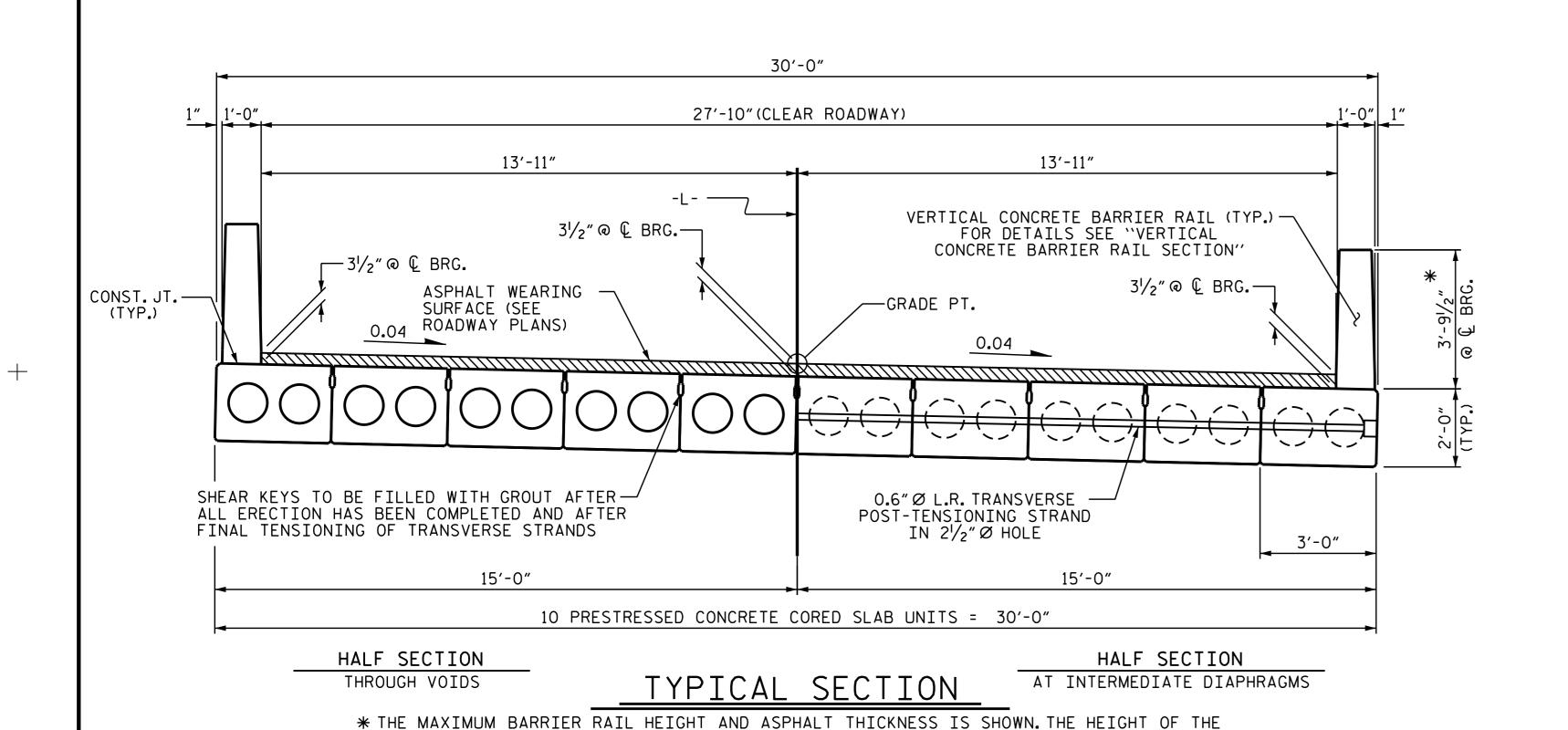


LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: H. T. BARBOUR DATE: 4-14-15
CHECKED BY: V. X. NGUYEN DATE: 6-15

DRAWN BY: CVC 6/10
CHECKED BY: DNS 6/10



BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE

12" Ø \_ \_\_\_\_

ELASTOMERIC BEARING PAD

SEE "END BENT"
SHEETS FOR DETAILS

"VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL, SHEET 3 OF 3.

 $\sim 2^{1/2}$  Ø DOWEL HOLE

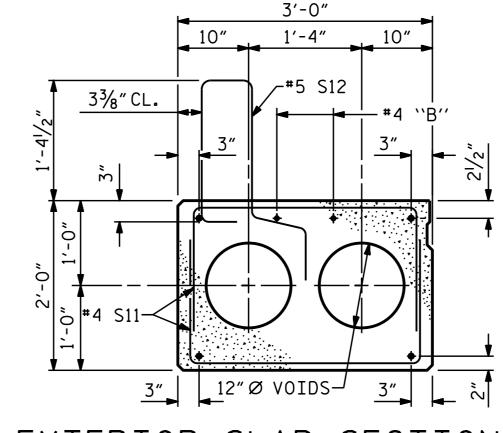
FIXED END

SECTION AT END BENT

1'-11/2"

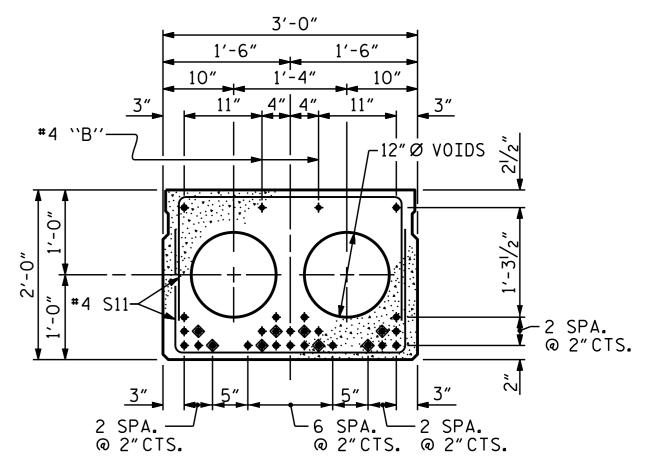
ASPHALT -

WEARING SURFACE



EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

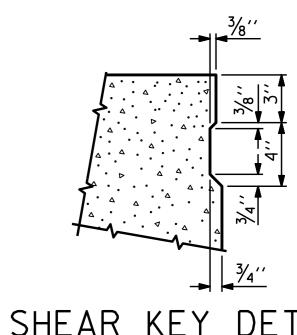


INTERIOR SLAB SECTION (70'UNIT) (28 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

SEAL 17230

\* NGINEEP

Wael Orafat

4139C12A32AB406 11/18/2015

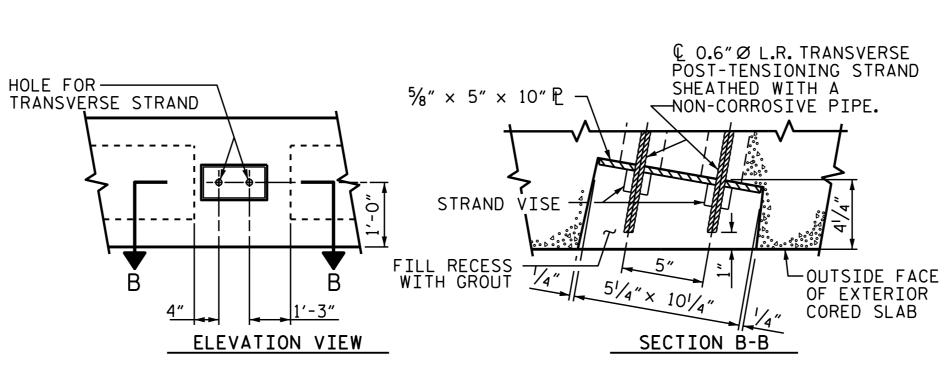
B-5404 PROJECT NO.\_ JACKSON COUNTY STATION: 17+20.00-L-

SHEET 1 OF 3

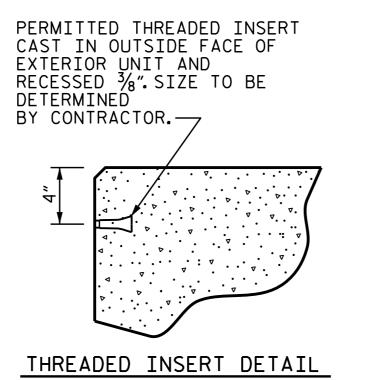
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

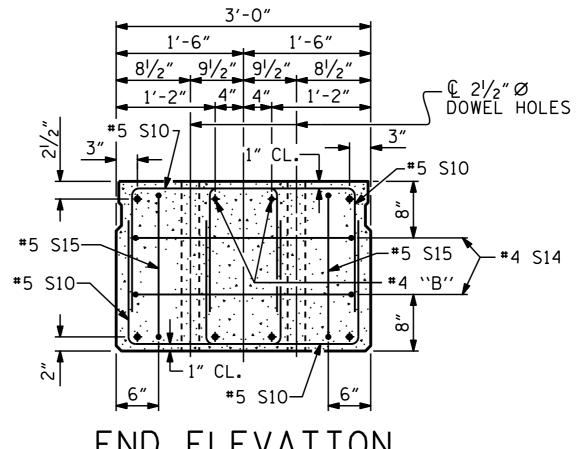
3'-0'' X 2'-0'' PRESTRESSED CONCRETE CORED SLAB UNIT

SHEET NO. REVISIONS S-5 NO. BY: DATE: DATE: BY:



GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS





END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

ASSEMBLED BY: H. T. BARBOUR DATE: 4-14-15 CHECKED BY: V. X. NGUYEN DATE : 6-15 DRAWN BY : MAA 6/10 CHECKED BY : MKT 7/10 REV. 8/14 MAA/TMG

SEE "BRIDGE"

APPROACH SLAB"
SHEET FOR DETAILS

2 LAYERS OF 30 LB.-ROOFING FELT TO PREVENT BOND.

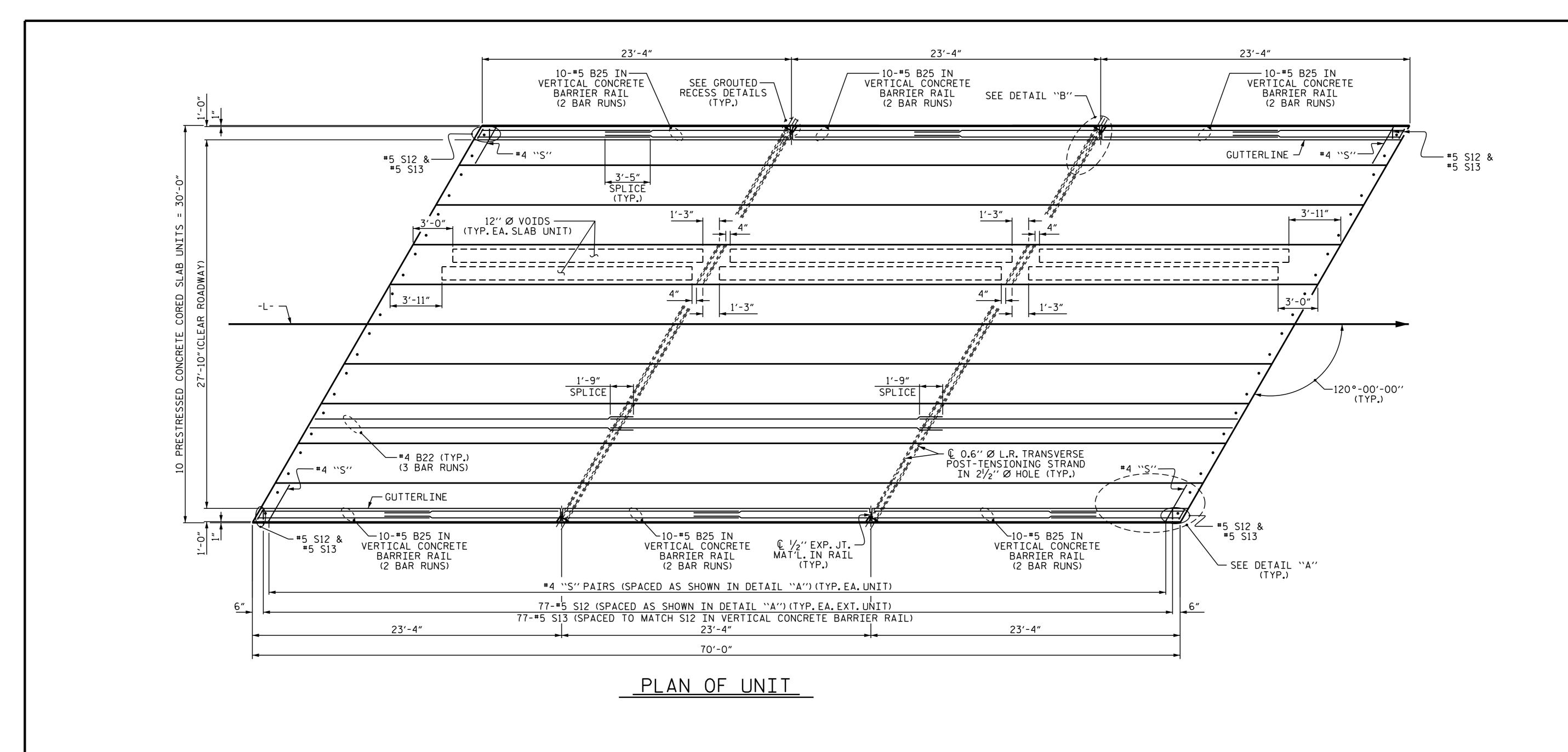
♠ BEARING & #6 DOWELS

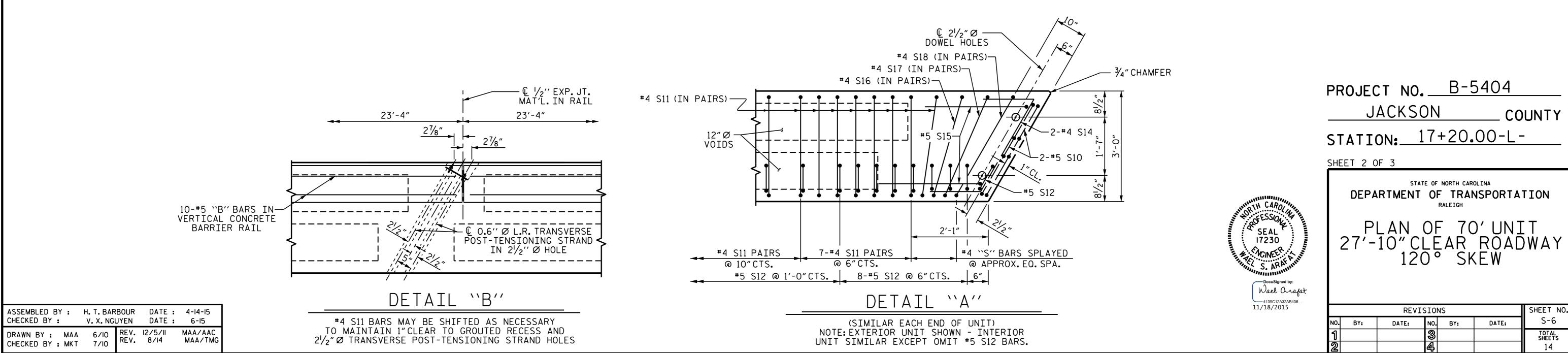
11/2" Ø BACKER ROD

Y:\Structures\FinalPlans\B5404\_SD\_TS.dgn warafat

STD. NO. 24PCS4\_30\_120S

18-NOV-2015 09:07





DATE:

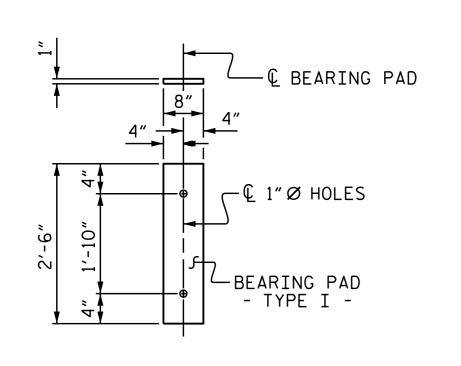
NO. BY:

COUNTY

SHEET NO.

S-6

TOTAL SHEETS



FIXED END (TYPE I - 20 REQ'D)

# ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

70' UNITS

-#5 S13

(TYP.)

\_2¾"CL.

VERTICAL DIM. VARIES

-- #5 S12 SEE "PLAN OF UNIT" FOR SPACING

1'-0"

10"

12"CL.

BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT											
EXTERIOR UNIT   INTERIOR UNIT											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT				
B22	6	#4	STR	24'-6"	98	24'-6"	98				
S10	8	<b>#</b> 5	3	5′-0″	42	5′-0″	42				
S11	170	#4	3	5′-10″	662	5′-10″	662				
<b>*</b> S12	79	#5	1	5′-7″	460						
S14	4	#4	4	5′-11″	16	5′-11″	16				
S15	4	<b>#</b> 5	3	7'-1"	30	7'-1"	30				
S16	4	#4	3	5′-11″	16	5′-11″	16				
S17	4	#4	3	6′-1″	16	6'-1"	16				
S18	4	#4	3	6′-3"	17	6′-3″	17				
REINF(	ORCING S	STEEL	LBS	S.	897		897				
	Y COATE IFORCINO		LB:	S <b>.</b>	460						
7000 P.S.I. CONCRETE CU. YDS. 12.0 12.0											
0.6"Ø	0.6" Ø L.R. STRANDS No. 28 28										

RAIL HEIGHT

@ MID-SPAN

3′-8″

21/2"

CONST. JT

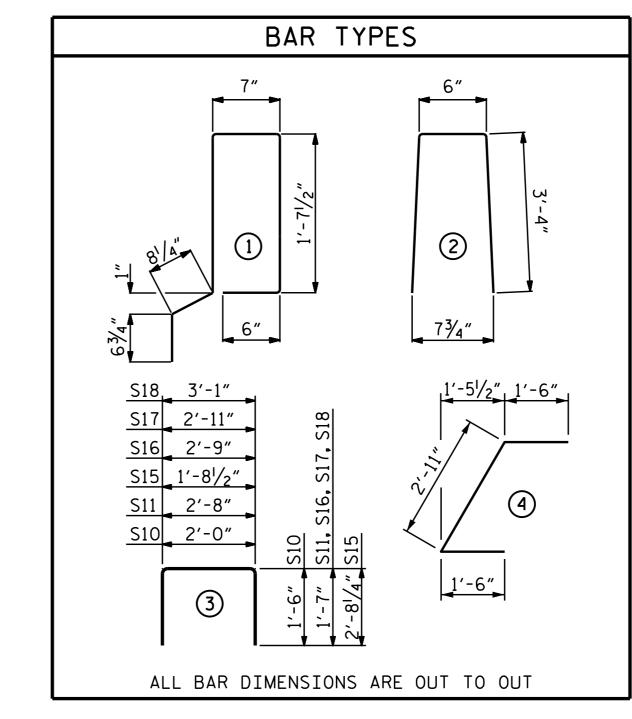
SECTION S-S

AT DAM IN OPEN JOINT

(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

© 1/2"EXP.JT.MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP.JT.MAT'L. WHEN SLIP FORM IS USED)

GRADE 270 STRANDS							
	0.6"Ø L.R.						
AREA (SQUARE INCHES)	0.217						
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600						
APPLIED PRESTRESS (LBS.PER STRAND)	43,950						



BI	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL											
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT						
	70' UNIT											
<b>∗</b> B25	120	120	#5	STR	13'-8"	1711						
<b>*</b> S13	158	158	#5	2	7′-2″	1181						
<b>∗</b> EP0X	Y COATED REINFORCING STEEL			LBS.		2892						
CLASS	CLASS AA CONCRETE CU.YDS. 18.1											
TOTAL												

CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
70'UNIT			
EXTERIOR C.S.	2	70'-0"	140'-0"
INTERIOR C.S.	8	70′-0″	560′-0″
TOTAL	10	-	700'-0"

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/4" 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3⁄4″ ♦
FINAL CAMBER	11/2"

\*\* INCLUDES FUTURE WEARING SURFACE

# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2\frac{1}{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

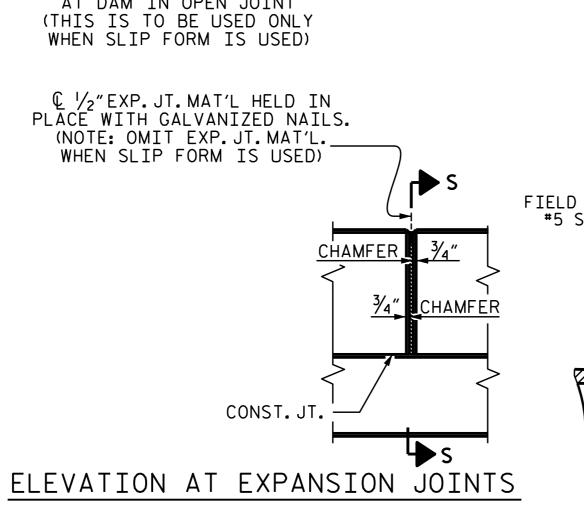
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

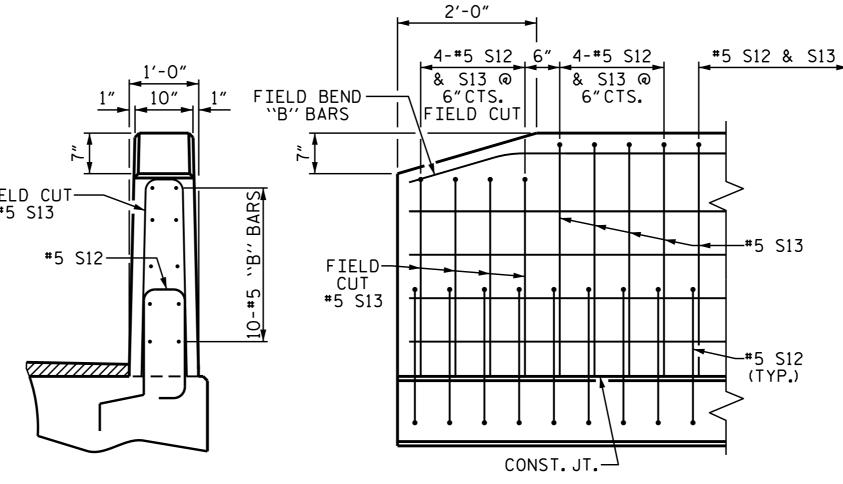
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

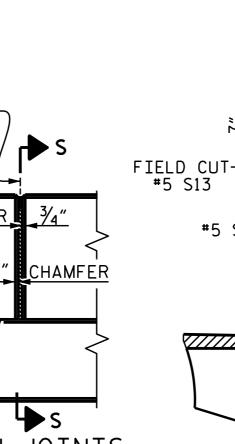
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.





SIDE VIEW



END VIEW

END OF RAIL DETAILS



4139C12A32AB406 11/18/2015

PSI

5500

CONCRETE RELEASE STRENGTH

UNIT

70' UNITS

PROJECT NO. \_

STATION: \_

SHEET 3 OF 3

JACKSON

STANDARD PRESTRESSED CONCRETE CORED SLAB UNIT

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

B-5404

17+20.00-L-

COUNTY

		REVIS	SION	IS		SHEET NO.
•	BY:	DATE:	NO.	BY:	DATE:	S-7
			3			TOTAL SHEETS
			4			14

ASSEMBLED BY : H. T. BARBOUR DATE : 4-14-15 CHECKED BY: V. X. NGUYEN DATE : 6-15 DRAWN BY: MAA 6/10 MAA/TMG REV. 11/14 CHECKED BY : MKT 7/10

CONST. JT. —

SECTION THRU RAIL

HALT BLE)

3'-91/2" "GUTTERLINE ASPH/ RAIL HEIGHT" TAB

VARIES THICKNE

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VERTICAL CONCRETE BARRIER RAIL DETAILS

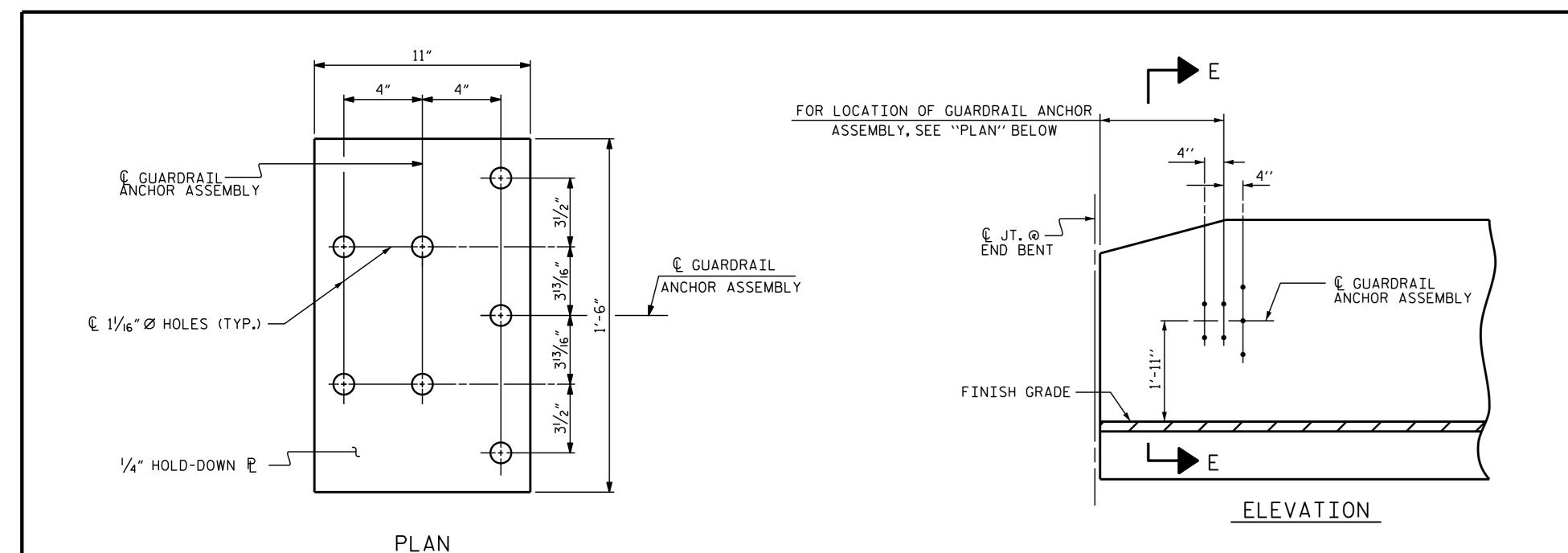
GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT

ASPHALT OVERLAY THICKNESS

@ MID-SPAN

2"

STD. NO. 24PCS3\_30\_60&120S



THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 1/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

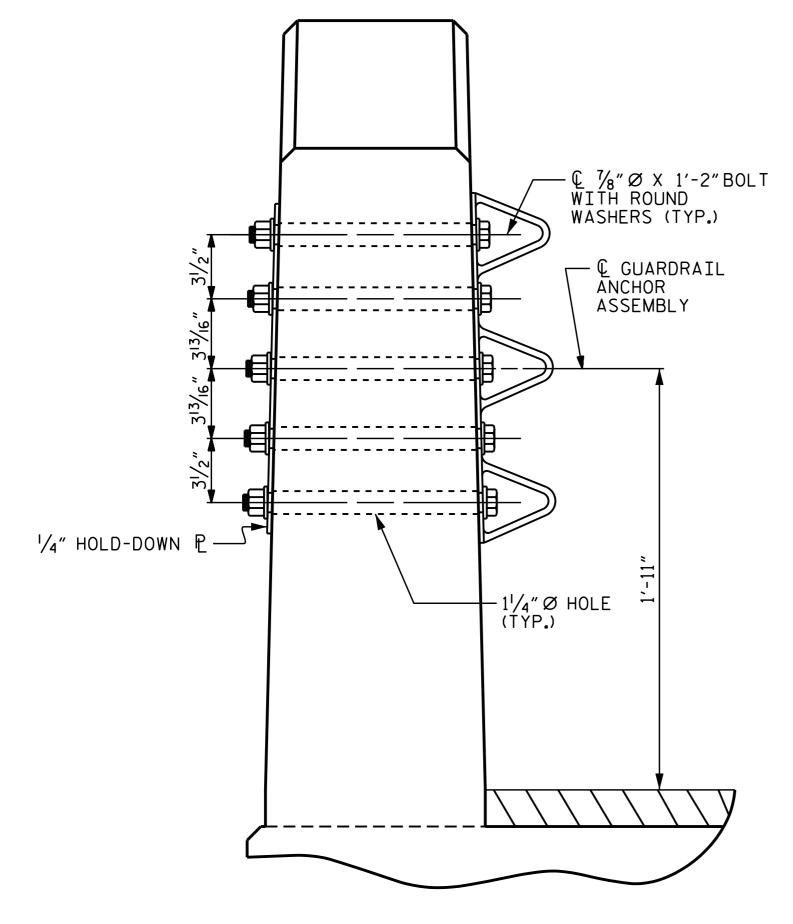
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

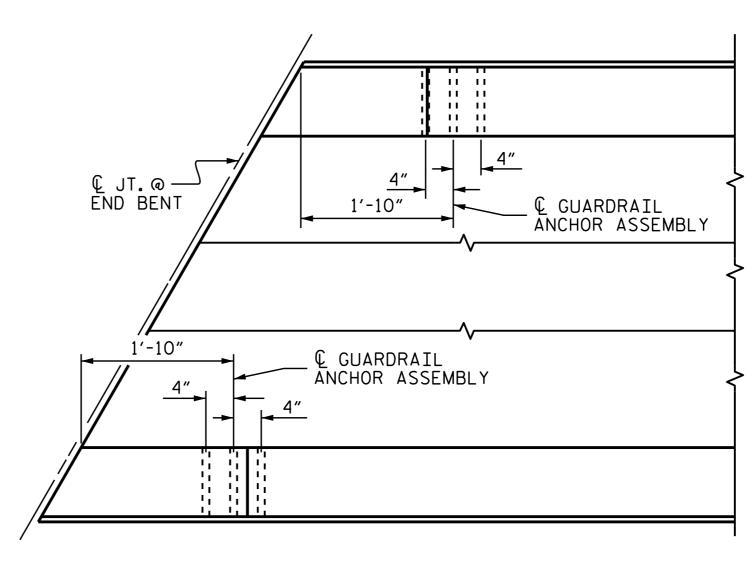
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE  $1 \frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



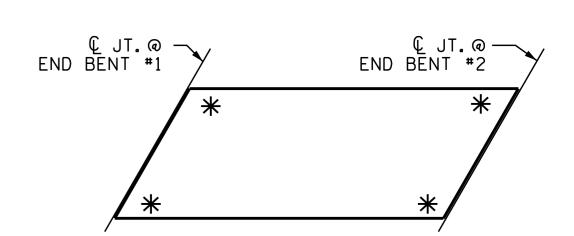
SECTION E-E
GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

# LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



# SKETCH SHOWING POINTS OF ATTACHMENT

★ DENOTES GUARDRAIL ANCHOR ASSEMBLY



11/18/2015

DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

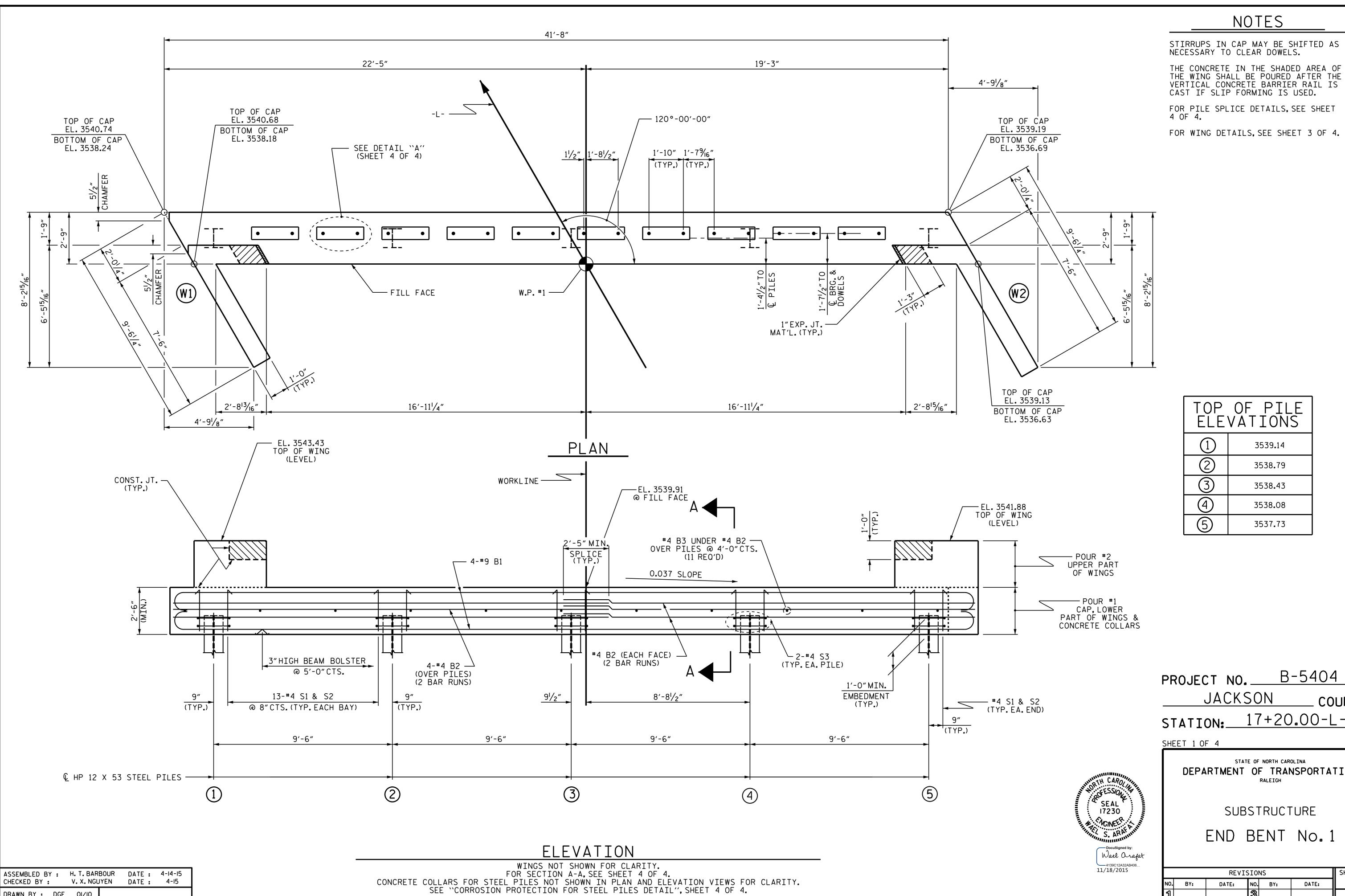
FOR VERTICAL CONCRETE

BARRIER RAIL

		REV	ISION	S		SHEE
0.	BY:	DATE:	NO.	BY:	DATE:	S
0			3			TO. She
2			4			1

ASSEMBLED BY: H. T. BARBOUR DATE: 4-14-15
CHECKED BY: V. X. NGUYEN DATE: 6-15

DRAWN BY: MAA 5/10
CHECKED BY: GM 5/10
REV. 12/5/11
REV. 6/13
REV. 1/15
MAA/GM
REV. 1/15



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET

FOR WING DETAILS, SEE SHEET 3 OF 4.

3539.14

3538.79

3538.43

3538.08

3537.73

JACKSON COUNTY 17+20.00-L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

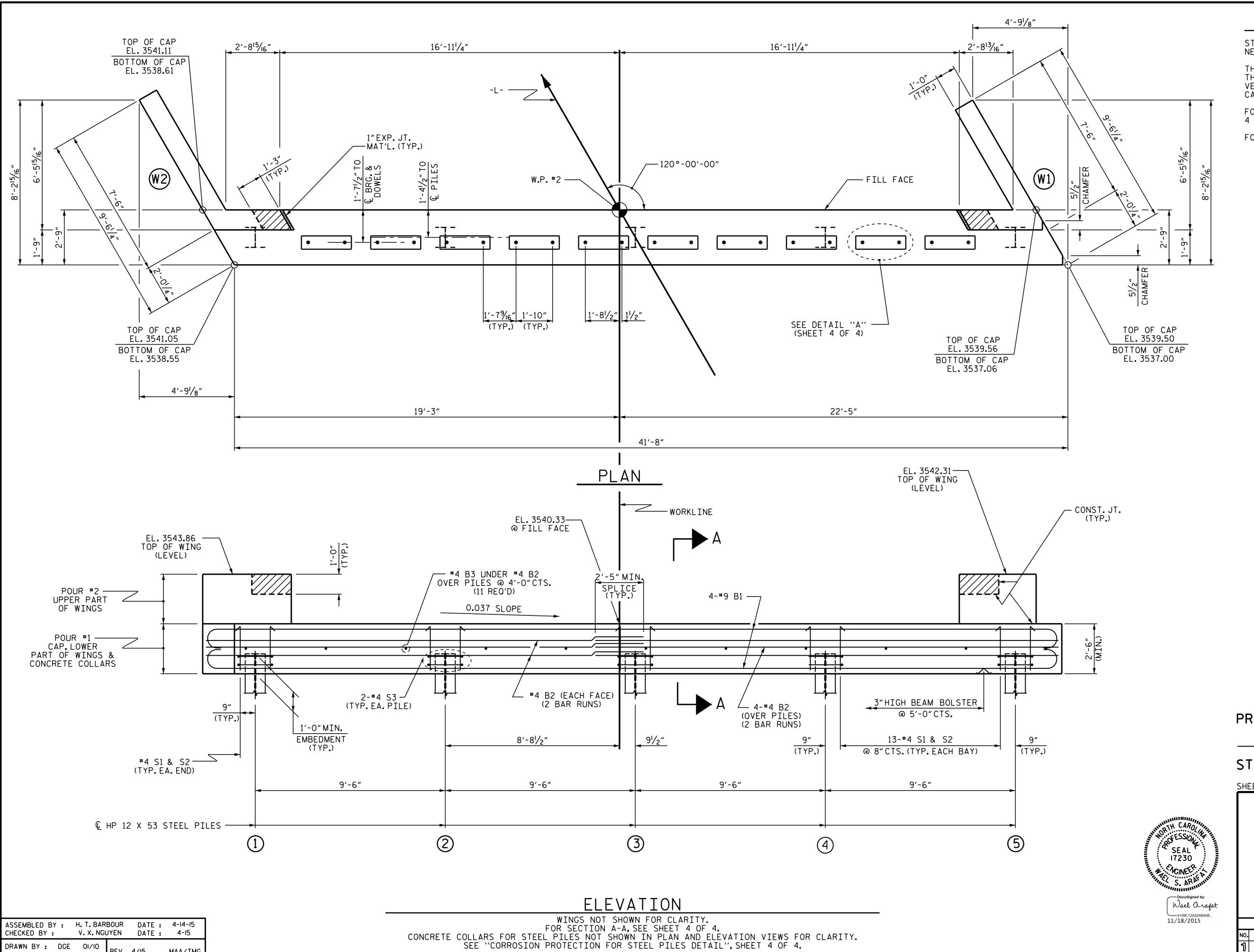
END BENT No. 1

REVISIONS						SHEET NO.
0.	BY:	DATE:	NO.	BY:	DATE:	S-9
]			3			TOTAL SHEETS
2	·		4			14

CHECKED BY: V. X. NGUYEN DATE: 4-15

MAA/TMG

DRAWN BY: DGE OI/IO
CHECKED BY: MKT OI/IO
REV. 4/15



STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET

FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP ELE	TOP OF PILE ELEVATIONS					
1	3539 <b>.</b> 51					
2	3539 <b>.</b> 16					
3	3538 <b>.</b> 80					
4	3538 <b>.</b> 45					
5	3538.10					

PROJECT NO. B-5404 JACKSON

COUNTY 17+20.00-L-STATION:\_

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No. 2

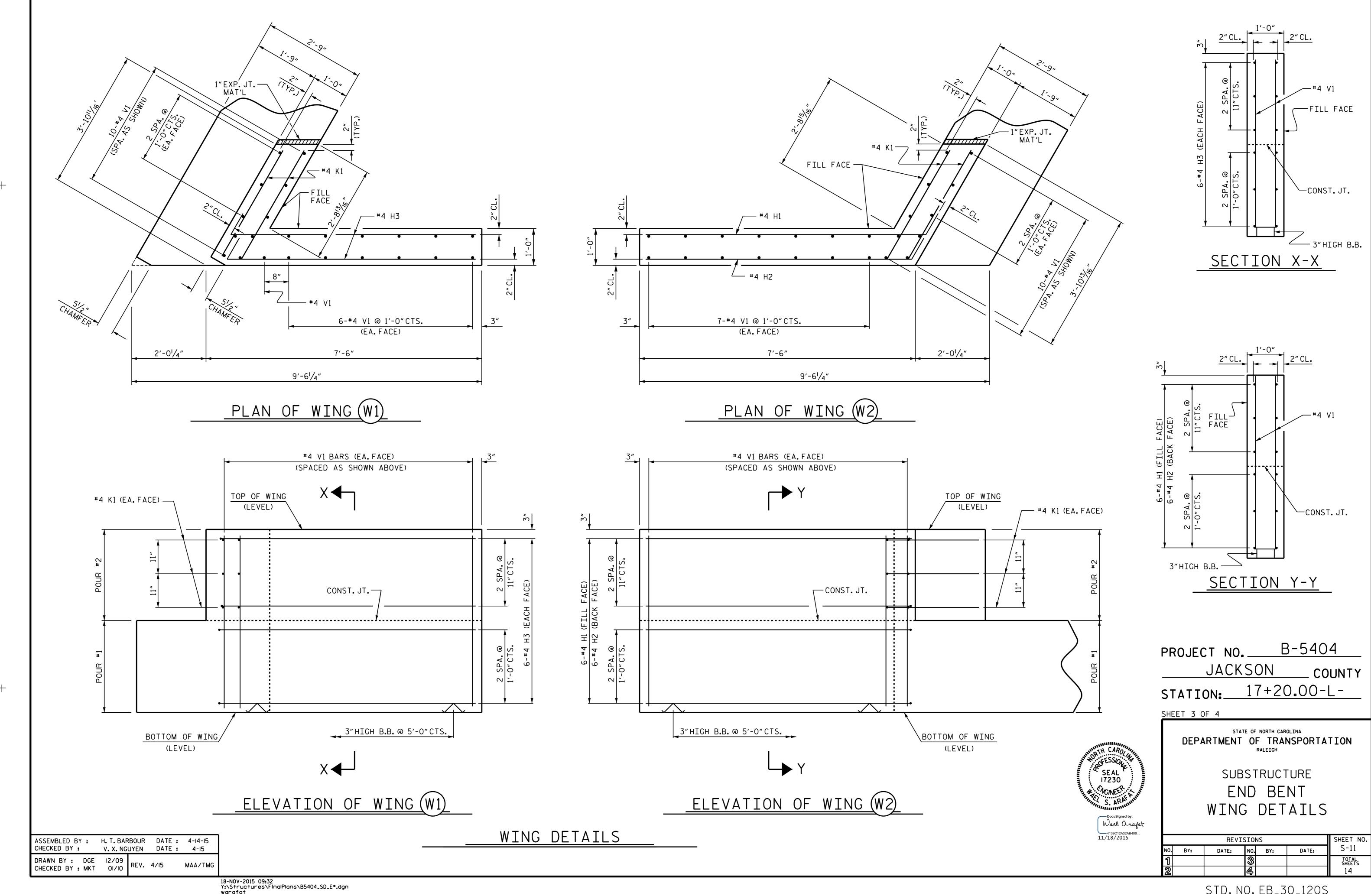
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BY:	DATE:	NO.	BY:	DATE:	S-10
		જ			TOTAL SHEETS
		4			14

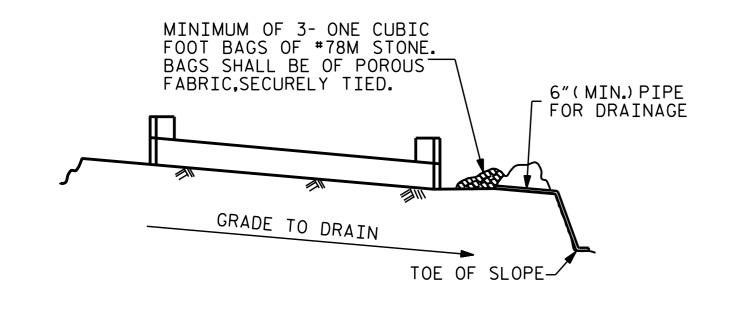
CHECKED BY :

DRAWN BY: DGE 01/10
CHECKED BY: MKT 01/10
REV. 4/15

V. X. NGUYEN DATE: 4-15

MAA/TMG



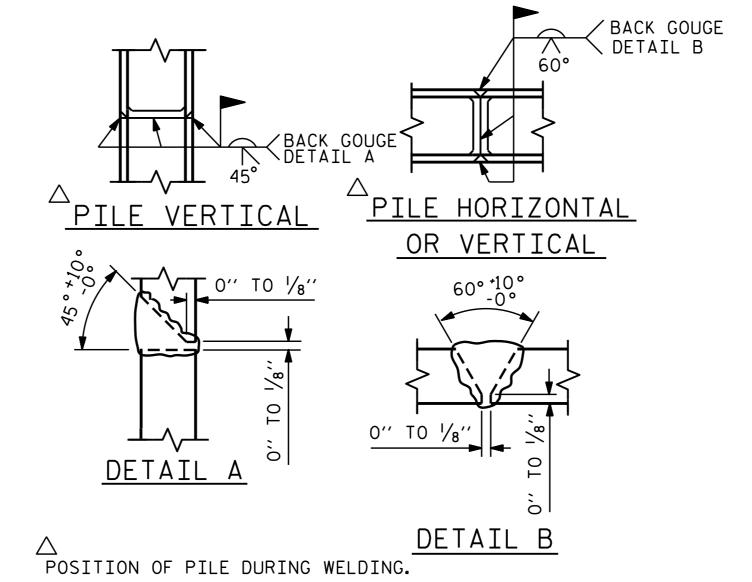


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

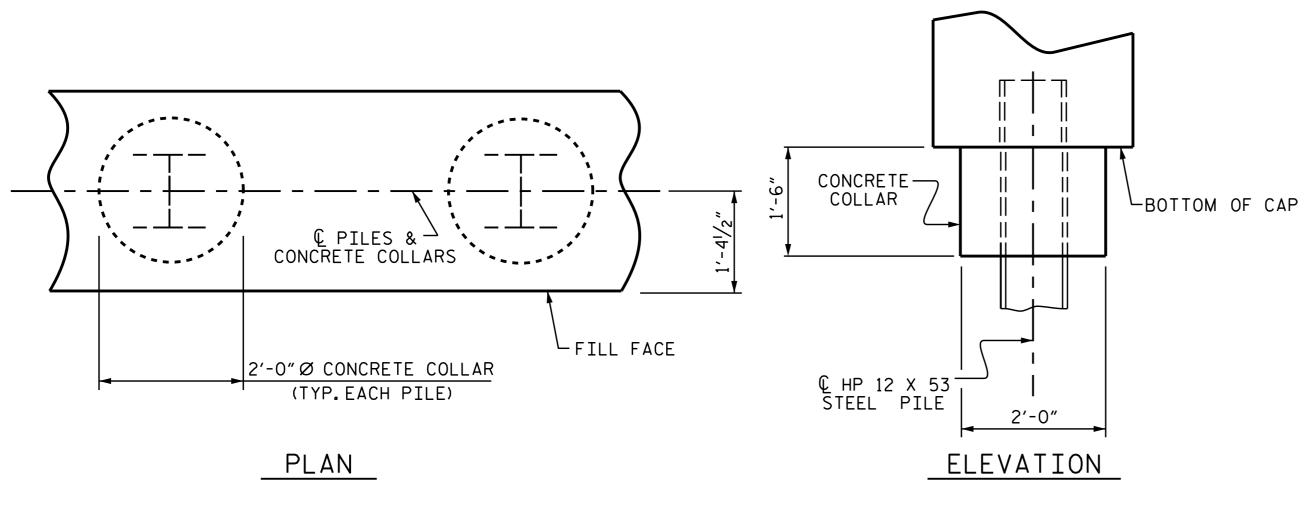
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

# TEMPORARY DRAINAGE AT END BENT

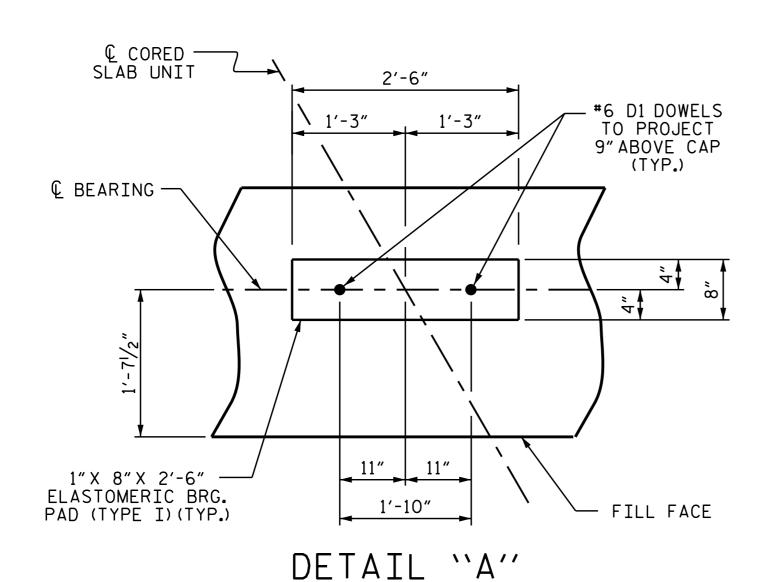


# PILE SPLICE DETAILS



# CORROSION PROTECTION FOR STEEL PILES DETAIL

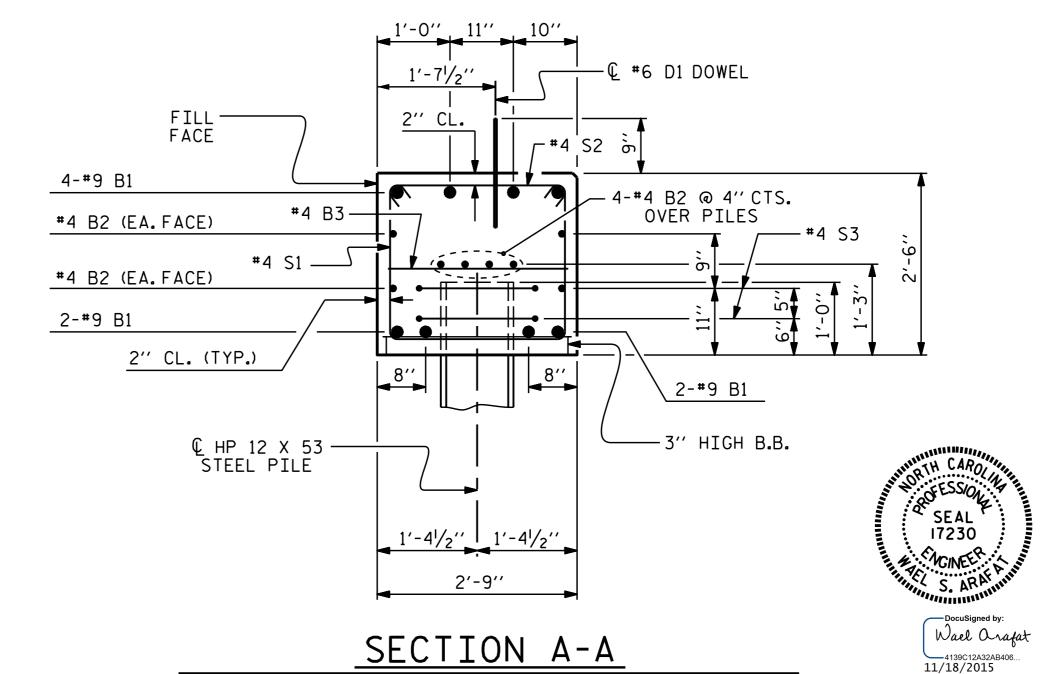
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



ASSEMBLED BY : H. T. BARBOUR DATE: 4-14-15 CHECKED BY: V. X. NGUYEN DATE : 4-15 DRAWN BY: DGE 12/09
CHECKED BY: MKT 01/10
REV. 11/14

MAA/TMG

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



(CONCRETE COLLAR NOT SHOWN FOR CLARITY.

SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

BAR TYPES

(3)

6'-8"

(4)

2'-5"

END BENT No.1

HP 12 X 53 STEEL PILES

STEEL PILE POINTS

LIN.FT.= 115

NO: 5

NO: 5

B-5404 PROJECT NO. \_\_\_\_ JACKSON COUNTY 17+20.00-L-STATION:

BILL OF MATERIAL

BAR | NO. |

16

12

54

10

11 #4 STR

D1 | 20 | #6 | STR | 1'-6"

12 | #4 | 3

#4

#4

CLASS A CONCRETE BREAKDOWN

(FOR ONE END BENT)

POUR #1 CAP, LOWER PART

POUR #2 UPPER PART OF

WINGS

TOTAL CLASS A CONCRETE

V1 47 #4 STR

REINFORCING STEEL

(FOR ONE END BENT)

#4 | 2

#4 2

#4 | STR |

**#**4 5

4

OF WINGS & COLLARS

B1

B2

В3

H2

Н3

K1

S2

S3

S1 | 54 |

H2

7′-7″

7'-2"

2'-5"

(6)

1'-8"Ø

END BENT No. 2

HP 12 X 53 STEEL PILES

STEEL PILE POINTS

LIN. FT.= 115

ALL BAR DIMENSIONS ARE OUT TO OUT.

NO: 5

NO: 5

FOR ONE END BENT

#4 | STR | 21'-11"

| SIZE | TYPE | LENGTH | WEIGHT

43'-8"

2′-5″

8′-3"

7′-10″

7′-4″

3′-3"

7′-5"

3'-2"

6′-6″

4'-8"

1188

234

18

45

33

31

59

26

268

114

43

147

2206 LBS.

12.7 C.Y.

2.1 C.Y.

14.8 C.Y.

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

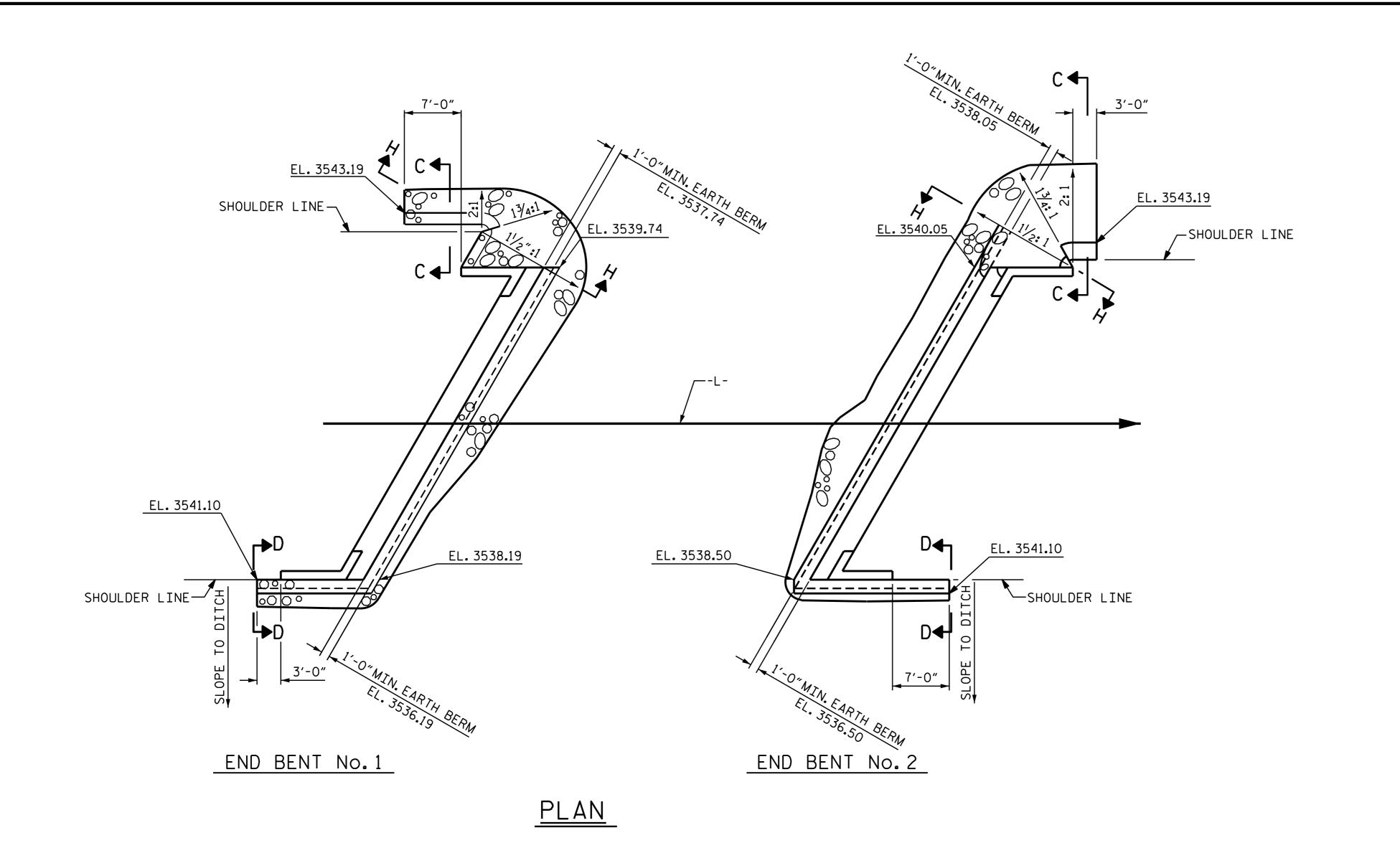
SUBSTRUCTURE

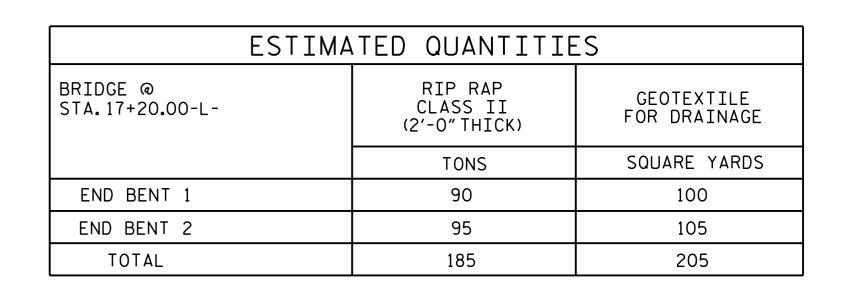
END BENT No.1 & 2 DETAILS

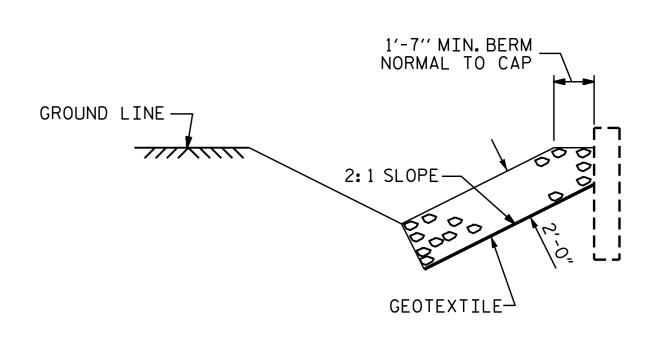
REVISIONS						SHEET NO
NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS
2			4			14

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STD. NO. EB\_30\_120S





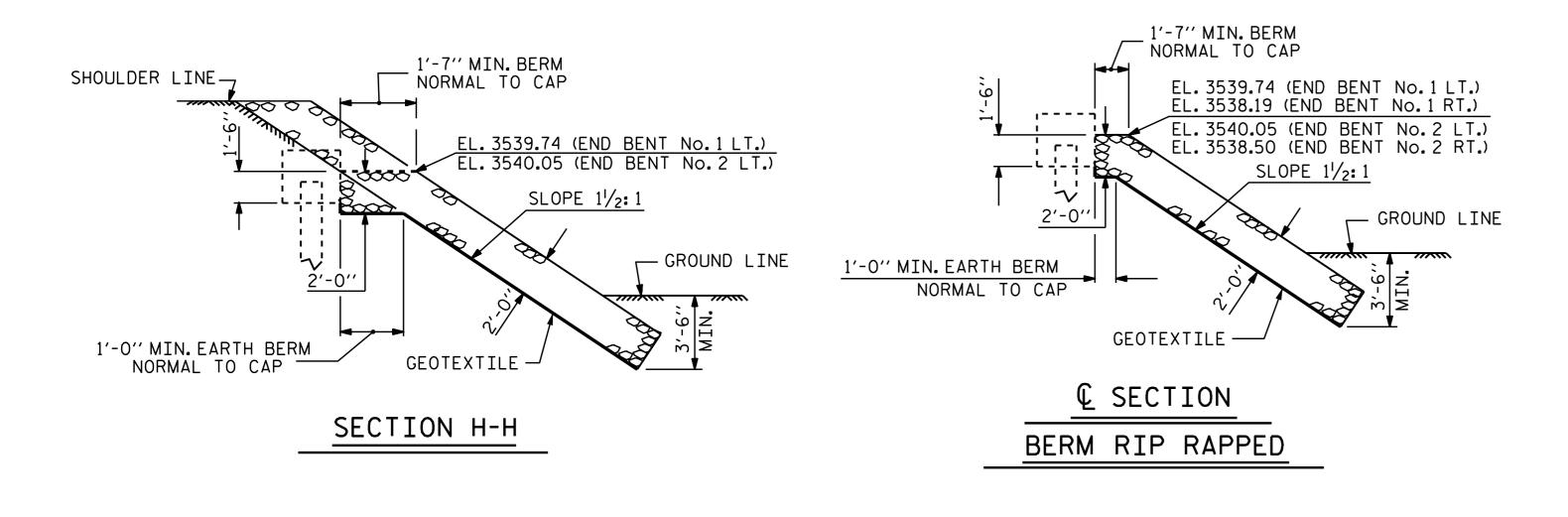


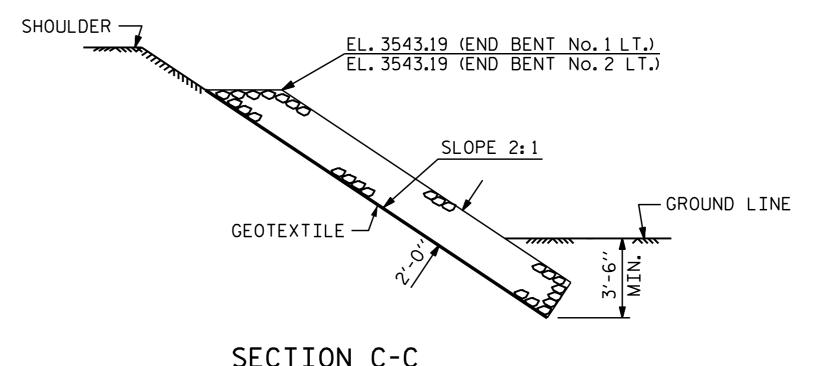
VIEW D-D

SEAL 17230

DocuSigned by:
Wael Orafat

4139C12A32AB406. 11/18/2015





SECTION C-C

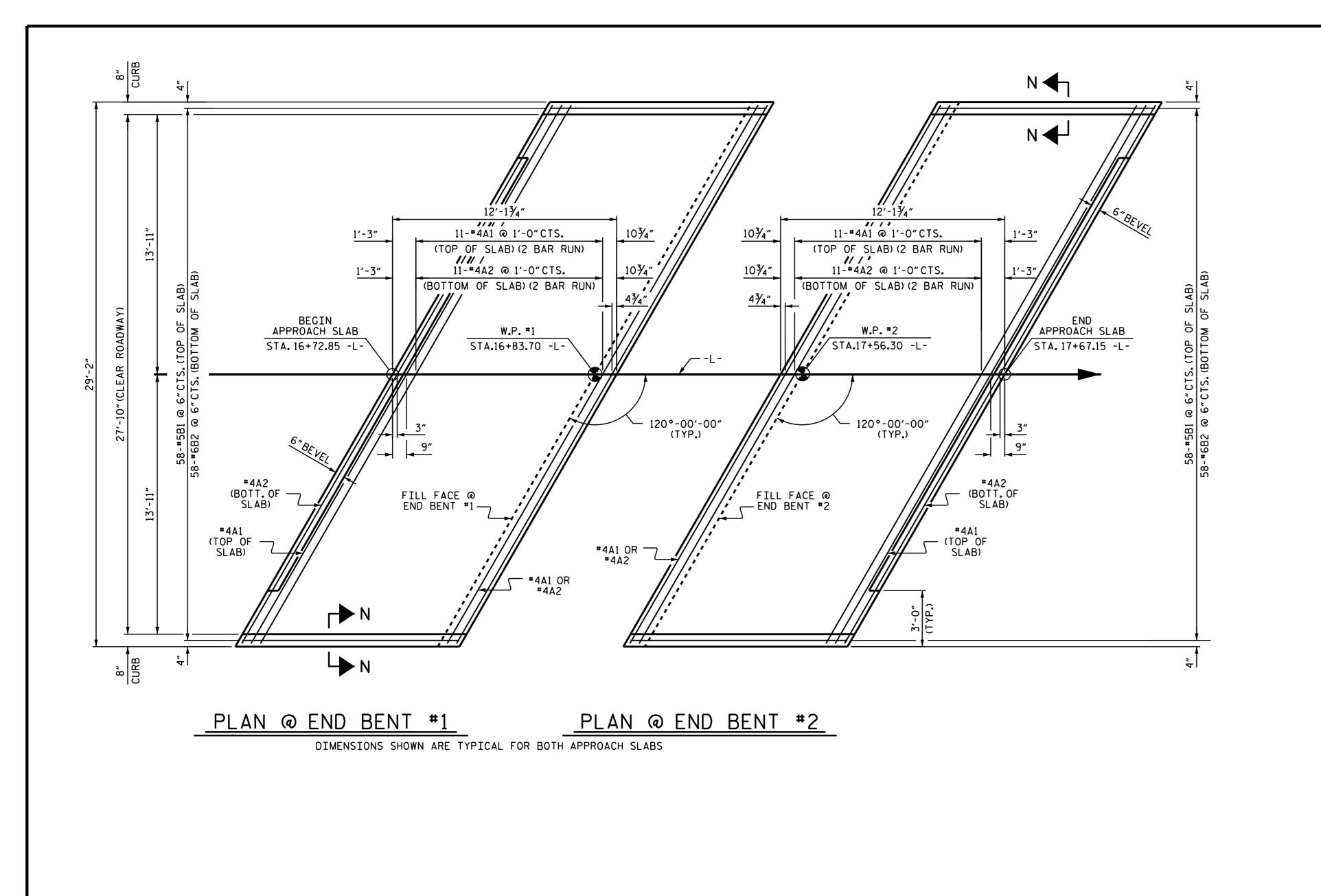
PROJECT NO. B-5404 JACKSON COUNTY 17+20.00-L-STATION:\_

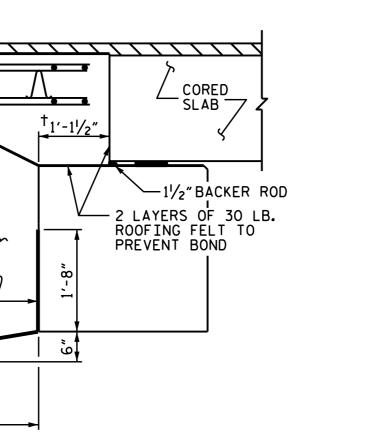
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

-RIP RAP DETAILS-

	SHEET NO				
BY:	DATE:	NO.	BY:	DATE:	S-13
		<u></u>			TOTAL SHEETS
		٧			14

ASSEMBLED BY: H.T.BARBOUR CHECKED BY: V.X.NGUYEN DATE : 4-17-15 DATE : 5-4-15 REV. 5/I/06R REV. I0/I/II REV. I2/2I/II TLA/GM MAA/GM MAA/GM DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84





<sup>†</sup>2 :1 SLOPE ·

#78M — STONE — BACKFILL

 $^{ackslush}$ GEOTEXTILE $\overline{ackslush}$ 

3'-0"

SECTION THRU SLAB

# NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

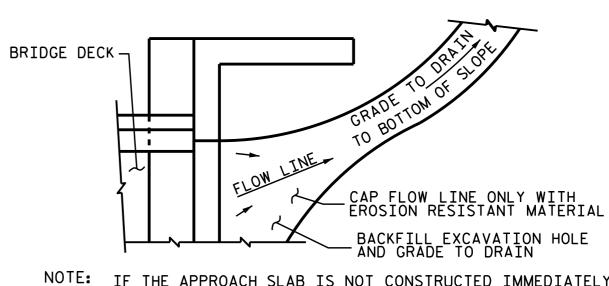
\*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

\*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

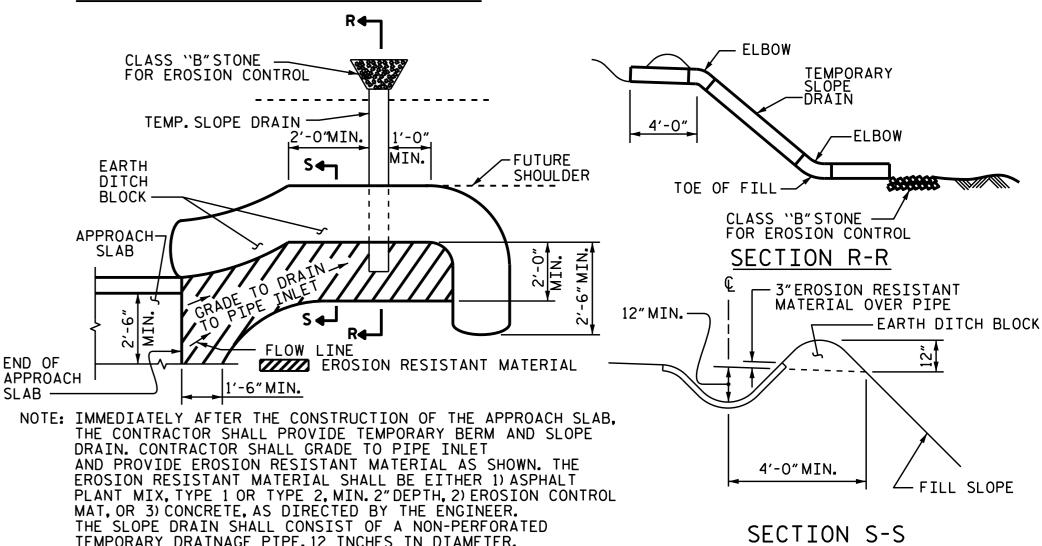
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



PLAN VIEW

TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

# TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

B-5404 PROJECT NO. JACKSON COUNTY 17+20.00 -L-STATION:

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

APPROACH SLAB AT EB #2

BAR NO. SIZE TYPE LENGTH WEIGHT

26 | #4 | STR | 17'-8"

58 #6 STR 11'-7"

26 #4 STR

58 | #5 | STR |

17'-7"

11'-1"

11'-7"

LBS.

LBS.

C.Y.

17'-7"

11'-1"

LBS.

LBS.

C.Y.

307

305

670

1009

1314

977

18.3

307

305

670

1009

1314

977

18.3

26 #4 | STR | 17'-8"

26 #4 STR

58 | #5 | STR |

58 | #6 | STR |

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

REINFORCING STEEL

\* EPOXY COATED

SEAL 17230 : CACINEES

11/18/2015

Wael Orafat

DEPARTMENT OF TRANSPORTATION STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE

CORED SLAB UNIT (SUB-REGIONAL TIER) 120° SKEW

STATE OF NORTH CAROLINA

SHEET NO. **REVISIONS** S-14 NO. BY: DATE: BY: DATE: TOTAL SHEETS

END OF CURB WITHOUT SHOULDER BERM GUTTER CURB DETAILS SPLICE LENGTHS EPOXY COATED UNCOATED

3'-11/2"

APPROACH

SLAB -

SECTION N-N

2'-0" | 1'-9"

2'-6"

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-51/4" CONTINUOUS HIGH CHAIR UPPER (CHCU) @ 3'-0" CTS. ACROSS SLAB

#4A2 -

11/2:1 SLOPE
OR STEEPER
(TO BE DETERMINED
BY THE CONTRACTOR)

4"Ø PERFORATED — SCHEDULE 40 PVC PIPE

PROPOSED – ASPHALT PAVEMENT

NORMAL TO END BENT

DATE : 7-15

MAA/AAC MAA/TMG

ROADWAY-

APPROVED WIRE BAR - SUPPORTS @ 3'-0"CTS.

ASSEMBLED BY: H. T. BARBOUR DATE: 4-14-15

CHECKED BY: V. X. NGUYEN

CHECKED BY : BCH 5-09

DRAWN BY : SHS/MAA 5-09 REV. 12-11 REV. 8-14

# STANDARD NOTES

#### DESIGN DATA:

SPECIFICATIONS A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR ---- 1.800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ----375 LBS. PER SQ. IN. EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS. PER CU. FT.

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

## CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

# DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.

SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

## REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990

(MINIMUM)