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tgarrison

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<u>VERTICAL CURVE</u> DATA -L-
+0.5500%
PI = 3155+80.00 -L- EL. = 7.86' VC = 180'
+2.1000%
PI = 3157+70.00 -L- EL. = 11.85' VC = 200'
HORIZ.CURVE DATA -W1-
PI = 3153+59.00 △ = 0°-15'-40.5"(LT) D = 1°-20'-18.4" L = 19.52' T = 9.76' R = 4,280.75'
PI = 3155+38.74 △ = 6°-48'-03.2"(LT) D = 2°-00'-10.2" L = 339.56' T = 169.98'



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COUNTY







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NOTES

FOR GENERAL NOTES AND NOTES REGARDING MATERIALS AND CORROSION PROTECTION, SEE GENERAL DRAWINGS. FOR CONCRETE SHEET PILE DESIGN PARAMETERS, SEE SHEET 6 OF 11. FOR CONCRETE SHEET PILE FOUNDATION NOTES, SEE SHEET 6 OF 11. TOP OF PILE ELEVATIONS SHOWN ARE THE MAXIMUM ALLOWABLE TOP OF PILE ELEVATION AND INCLUDE THE 3" ± TOLERANCE ALLOWED PER SECTION 452 OF THE STANDARD SPECIFICATIONS. FOR CONCRETE SHEET PILE WALL, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

	$\frac{VERTICAL CURVE}{DATA -L-}$ $\frac{-0.7000\%}{2.3500\%}$ $PI = 3183+80.00 -L-$ $EL = 10.99'$ $VC = 200'$ $\frac{-2.3500\%}{4.500\%}$
END CONCRETE SHEET PILE RETAINING WALL W.P. #W4-8 STA. 3186+03.58 -W4- STA. 3186+01.23 -L-	PI = 3186+40.00 -L- EL. = 4.88' VC = 320' <u>HORIZ. CURVE</u>
- W.P. #W4-8 TOP OF COPING EL. 7.15	$DATA = W4 =$ PI = 3184+05.17 $\Delta = 5^{\circ} - 57' - 59.2'' (LT)$ D = 2° - 00' - 10.2'' L = 297.90' T = 149.09' R = 2,860.75'
EL.15] EL.10	PI = $3185+78.78$ $\Delta = 0^{\circ}-31'-44.9''(LT)$ D = $1^{\circ}-04'-00.5''$ L = $49.60'$ T = $24.80'$ R = $5,370.75'$
EL. 5 EL. 0 EL5 EL10 EL15 EL20	
EL25 ∃EL30 ►	PROJECT NO. <u>B-2500AB</u> <u>DARE</u> COUNTY STATION: <u>3170+75.00</u> -L-
PRIMAR SEAL	SHEET 4 OF 11 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
Docusigned by:	CONCRETE SHEET PILE RETAINING WALL WALL 4
61EAF7523943466 8/3/2015	NO. BY: DATE: NO. BY: DATE: W-4 1 3 TOTAL SHEETS 3 13 13



P	ROJEC	CT NO.		B-2	<u>25007</u>	<u> 18</u>
		DAR	E		CO	UNTY
57	TATI	on <u>: 31</u>	7	0+7	5.00	<u>-L-</u>
H	EET 5 C	DF 11				
	DEPA	STAT	e of O	NORTH CARG	NSPORTA	TION
	CON R W	CRETI Retai All /	E N A M	SHE ING ND (ET P WAL Copin	'ILE L IG
C)ETĂ	ILS	A	ΤĒ	ND B	ÉNTS
		REVIS	SIO	NS		SHEET NO.
، 0	BY:	DATE:	N0.	BY:	DATE:	₩-5
ป			শু			SHEETS

	CONCRETE SHEET PILE DATA									
PILE TYPE	TOTAL NO. REQUIRED	NO.OF PILES IN STORED INVENTORY	REMAINING NUMBER REQUIRED	L (FT.)	X (FT.)	TOTAL LIN.FT.				
A1	2		2	32'-0"	7'-6"	64				
Α2	267		267	26'-0"	5'-0"	6,942				
Α3	146	134	12	29'-0"	6'-0"	348				
Δ4	198		198	32'-0"	7'-6"	6,336				
Α5	135	55	80	35'-0"	9'-0"	2,800				
A6	8		8	32'-0"	7'-6"	256				
Α7	1		1	35'-0"	9'-0"	35				
A8	1		1	35'-0"	9'-0"	35				
Α9	1		1	35'-0"	9'-0"	35				
A10	1		1	35'-0"	9'-0"	35				
C1	2		2	32'-0"	7'-6"	64				
C2	2		2	32'-0"	7'-6"	64				

SEE ``CONCRETE SHEET PILE RETAINING WALL, SHEET PILE DETAILS'', SHEETS 7 THRU 11 OF 11, FOR PILE TYPES.

A TOTAL OF 189 PRESTRESSED CONCRETE SHEET PILES WERE FABRICATED FOR ANOTHER PROJECT THAT HAS BEEN TERMINATED. THESE PILES WHICH ARE LISTED IN THE ABOVE TABLE HAVE BEEN STORED AND SHALL BE USED ON THIS PROJECT. SEE SPECIAL PROVISION FOR CONCRETE SHEET PILE WALL FOR THE STORAGE LOCATIONS AND REQUIRED COORDINATION TO OBTAIN THE STORED CONCRETE SHEET PILES.

DESIGN PARAMETERS

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CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS: 8000 PSI (MIN.)

CONCRETE COMPRESSIVE STRENGTH AT RELEASE OF PRESTRESSING: 5600 PSI (MIN.)

UNIFORM COMPRESSION AFTER PRESTRESSED LOSSES: 1000 PSI (MIN.)

PICK-UP, STORAGE, AND TRANSPORTATION: 0.0 PSI TENSION WITH 1.5 TIMES SELF WEIGHT

ALL PRESTRESSING STRANDS SHALL BE 0.6"DIA, 7-WIRE LOW RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203. STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED. OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

INSTALL PILES USING A METHOD APPROVED BY THE ENGINEER, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

ALL CORNERS TO BE CHAMFERED $\frac{3}{4}$ " UNLESS OTHERWISE NOTED.

PICK-UP OF PILE MAY BE EITHER A SINGLE POINT PICK-UP OR A TWO POINT PICK-UP AS SHOWN.

FOUNDATION NOTES

CONSTRUCT CONCRETE SHEET PILE RETAINING WALLS IN ACCORDANCE WITH SECTION 452 OF THE STANDARD SPECIFICATIONS EXCEPT USE CONCRETE SHEET PILES IN LIEU OF STEEL SHEET PILES.

INSTALL CONCRETE SHEET PILES TO TIP ELEVATIONS NO HIGHER THAN AS SHOWN ON THE PLANS.

USE JETTING, DRIVING, OR A COMBINATION OF JETTING AND DRIVING TO ATTAIN THE MINIMUM TIP ELEVATION NO HIGHER THAN REQUIREMENTS. SEE PILE JETTING SPECIAL PROVISION FOR JETTING REQUIREMENTS AND RESTRICTIONS.

IF USING IMPACT HAMMERS FOR CONCRETE SHEET PILE INSTALLATION, SUBMIT DRIVING EQUIPMENT AS OUTLINED IN SECTION 450-3 OF THE STANDARD SPCIFICATIONS.

FOR JETTING AND OFF-SITE JETTING SPOIL DISPOSAL, SEE PILE JETTING SPECIAL PROVISION.

FOR CONCRETE SHEET PILE RETAINING WALLS AND BRIDGE END BENT CONSTRUCTION SEQUENCE, SEE BRIDGE FOUNDATION NOTES ON PLANS.

THE SCOUR CRITICAL ELEVATIONS FOR ALL CONCRETE SHEET PILE WALLS ARE ELEVATION -3.0 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

_	MIN.	TIP	EL.	-19.00	(
	MIN.	TIP	EL.	-22.00	(

-L-



DRAWN BY :	T. M. GARRISON, P.E.	DATE :	6/15
CHECKED BY :	M. A. ALLEN	DATE :	6/15
DESIGN ENGINEER	OF RECORD: T.M. GARRISON, P.E.	DATE :	6/15





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TYPES			BTI	()F	MATFR	ΤΔΙ	
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1	<u>,</u> ♥			<u> </u>	<u>ZE</u> 1		LENGIH	WEIGHI
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		55 56	1	#/	1	1	0^{-1}	3
'-9 /2"	Υ Ρ."	50 57	1	# /	1	1	- 11 7"	2
'-11 ¹ /2"	4 ¹ /2	51	1		1	1		۷
1'-8"	► '	V1	4	#5	5	2	24'-4"	102
<u>1'-</u> <i>A</i> "			A2	(TY	PI	CAL PI	ILE)	
<u> </u>		BAR	NUMBER	SI	ZE	TYPE	LENGTH	WEIGHT
ζ'-ζ"		S1	9	# 2	1	1	5'-4"	32
1'-7"		S2	24	# 4	1	1	5′-8″	91
11″		S3	1	# 4	1	1	5'-1"	3
	►	S4	1	#2	1	1	4'-5"	3
'- Δ "								47
10"		V2	2	#5			20'-10"	43
-10			A3	(Y	$\mathbf{PT}($	CAL P.		
-10"		BAR	NUMBER	SI	ZE	TYPE	LENGTH	WEIGHT
-10″		S1	10	# 2	1	1	5'-4"	36
		S2	26	# 4	1	1	5′-8″	98
⁸ / ₂ ″	-	S3	1	# 2	1	1	5'-1"	3
		<u>S4</u>	1	#2	1	1	4'-5"	3
		1/7	2	# 0		2	22/ 10/	10
	, ,	<u>v</u> 5						
\checkmark			A4			CAL F.		
ONING ARE O	ING ARE OUT-TO-OUT S1 12				<u>2E</u>		LENGIH	WEIGHT
			12	#/	1	1	5'-8"	40
		52 53	1	#2	1	1	5'-1"	3
			1	# 2	1	1	4'-5"	3
2 ¹ /4″					<u>.</u>	-		
— j		V1	2	#5	5	2	24'-4"	51
4 [°] ζ			Α5	(TY	PI(CAL PI	ILE)	
		BAR	NUMBER	SI	ZE	TYPE	LENGTH	WEIGHT
			13	# 2	1	1	5'-4"	46
M 4		S2	29	# 2	1	1	5′-8″	110
		S3	1	# 2	1	1	5'-1"	3
4"		S4	1	#2	1	1	4'-5"	3
<u> </u>								
		V4	2	#5	5	2	25'-10"	54
<u>``C''</u>								
VE								
		<u></u>						
	QU/		IES F			INE P	ILE	
	PIL	Ξ	REINFORG	CING	8,0	00 PSI	0.6″ØL.	R.
			STEEL	-	00	NCREIE	STRAND	S
			LB.			C.Y.	No.	
	A1 (STARTE	R PILE)	260			3.0	22	
	A2 (TYPICA	L PILE)	172			2.4	22	
	A3 (TYPTCA	L PIIF)	188			2.7	22	
			202			3.0		
			202			2.0	22	
	AD UTPICA	L PILE)	216			J.2	22	
				L 1 4	`	R-2	5001	R
		PR	UJECI	N(J	υζ	JUUA	



PROJ 	ECT N DA	0. <u>B-</u> RE 3170+7	25004 co 5.00	AB UNTY -L-					
SHEET	7 OF 11								
DE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
CONCRETE SHEET PILE RETAINING WALL SHEET PILE DETAILS									
	REVISIONS SHEET NO.								
NO. BY	DATE:	NO. BY:	DATE:	W-7					
11		3		TOTAL SHEETS					

13



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0.6″ØL.R	.GRADE 270)
AREA	ULTIMATE STRENGTH	
(SOUARE INCHES)	(LBS.PER STRAND)	
0.217	58,600	



QUANTIT	IES FOR	ONE P	ILE
PILE	8,000 PSI CONCRETE	0.6″ØL.R. STRANDS	
	LB.	C.Y.	No.
A6 (MODIFIED A4)	175	2.4	16

	PROJEC	CT NO. DAR DAR ON: <u>31</u>	<u>B-2</u> E 70+7	25004 co 5.00	AB DUNTY -L-	
CAROLINA ESSIONE SEAL 33139	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
CINET: CONT	CON F SHE	RETA] EET F	E SHE NING PILE	DETA	ILE ILS	
Jour M. Junion		REVIS	SIONS		SHEET NO.	
-01EAF/023943400 /3/2015	NO. BY:	DATE:	NO. BY: 2	DATE:	W-8 TOTAL	
-,	2		<u>୬</u>		SHEETS	

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						BIL	L OF	MATER	RIAL	
						Δ7	(SPFC	TAL P	TIF)	
					BAR		ST7F	TYPF	LENGTH	WEIGHT
				ר I	S12	13	#4	1	5'-1"	44
0.6	″ØL.R	R. GRADE 270	STRANDS		S13	29	#4	1	5'-4"	103
		ULTIMATE	APPLIED		S14	1	#4	1	4'-9"	3
4	REA	STRENGTH	PRESTRESS		S15	1	#4	1	4'-1"	3
(SQUA	RE INCHES)	(LBS.PER STRAND)	(LBS.PER STRAND))		2	#5	2	25'-10"	54
0	.217	58,600	43.950		V 4					04
				┛		A8	ISPEU	IAL P		1
					BAR	NUMBER	SIZE			WEIGHT
					<u> </u>	29	#4 #4	1	4-5	90
						1	#4	1	4'-1"	3
					S19	1	#4	1	3'-5"	2
]			V4	2	#5	2	25'-10"	54
	10 8 6	64213579					BAR	TYPES		
	4 97 9	5 3 1 2 4 6 8 10 🗸								
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		UNITINO	-		<u></u> S12	2	<u> </u>	8″		H ∠ D.
					S13	3	1'-9	1/2"		41/2 (T
					<u> </u>	4	1'-	6″		
					 		<u>-</u> 1'_	2″		
							1 -	<u>د</u> ۸ <i>۳</i>		
					<u></u>	5	1'-	<u>4"</u>		
					<u>S1</u>	7	1'-5	1/2"		
					<u></u>	8	1'-	2″		
					S19	9	10)″		
						- -				
						◄	24'-10"			
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	-							 C.Y.	No.	Ť
	рлт ⁻		, ⊨	-					-	=
				(SPECIAL	_ PILE)	207		3.2	20	
	<u> </u>	JKNTNC	Α	8 (SPECIAL	_ PILE)	187		2.7	16	
					DD			B-2	2500A	B
					r- r					
							DAR	-	00	UNTY
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					ST	ATION	1: 71	10 + 15	0	<u>-L-</u>
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				M. GARRININ			┋╁╱ᡖ			
				~*******		SULE		TTC		TLS
				Jak M M			REVISI	ONS		SHEET NO.
				61EAF7523943466	NO.	BY:	DATE: N	IO. BY:	DATE:	W-9
				8/3/2015	1			3		TOTAL SHEETS
					2		4	왕		13

PILE TYPE A7 DETAILS

PILE TYPE A8 DETAILS

SEE CONCRETE SHEET PILE RETAINING WALL SHEET PILE DETAILS, SHEET 7 OF 11, FOR NOTES.

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				BILL OF MATERIAL				
r	A9 (SPECIAL PILE)							
0.6″ØL.R	.GRADE 270	STRANDS	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
AREA		APPLIED	S20 S21	29	#4 #4	1	4'-6" 4'-10"	59 94
(SOLIARE INCHES)	SIKENGIA (IBS. PFR STRAND)	(IRS PER STRAND)	S22	1	#4	1	4'-1"	3
∩ 217		17 950	S23	1	#4	1	3'-5"	2
0.211	000,000	40,000	V4	2	#5	2	25'-10"	54
				A10	(SPEC	IAL P	ILE)	
			BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
			S24 S25	13	#4 #⊿	1	4'-3" Δ'-7"	37
			S26		#4	1	3'-9"	3
			S27	1	#4	1	3'-1"	2
	• • • • • • (V4	2	#5	2	25'-10"	54
8642	2 1 3 5 7 J				BAR	TYPES		
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				21	1′-6 ¹ /	/2″		4 ^{1/2} , (T)
			<u></u>	22	1'-2	, 11		
			<u></u>	23	10′	,		
			<u></u>	24	1'-3	"	>	
			<u></u>	25	1'-5	"		
			<u></u>	26	1'-C	"		
			<u>_S</u>	27	8″			
				·			-	
				◀	24'-10"	×1× ^{81/} 2″		
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				ALL BAR DI	[MENSION	ING ARE	OUT-TO-O	UT
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$\int_{-6}^{753} \frac{5}{642}$	$\begin{array}{c}1 & 2 & 4 & 6\\1 & 3 & 5 & 7\end{array}$		ΟΠΑΝΤΙ	TIFS F	OR ()	NF P	II F	
<u> </u>	• • • • •					NO PST		<u> </u>
			FILC	STEEL		NCRETE	STRAND	ĸ. S
PATTI	ERN FOR			LB.		C.Y.	No.	
BUF	RNING	A9 (S	PECIAL PILE)	192		2.8	16	
		A10 (SPECIAL PILE) 185		2.6	14	
	PROJECT NOB-2500AB						B	
STATION: <u>31(0+(5.00</u> -				-L-				
			SF	FFT 10 OF	11			
			ſ		STATE O			
			ARO	DEPARI	MENT O	F TRAN	SPORTAT	ION



PILE TYPE A10 DETAILS

SEE CONCRETE SHEET PILE RETAINING WALL SHEET PILE DETAILS, SHEET 7 OF 11, FOR NOTES.

NALEION		
CONCRETE SHEET A RETAINING WAL SHEET PILE DETA	>IL L ILS	E
REVISIONS	SHEET	NC

BY:

DATE:

NO.

DATE:

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BY:

8/3/2015

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BILL OF MATERIAL							
270 STRAND			ONE	PILE			
	,5	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT
AIL APPLIEL GTH PRESTRES		S28	12	#4	1	5'-1"	41
TRAND) (LBS. PER STRA	ND)	529	26	#4 #⊿	<u> </u>	5'-3" 2'-9"	91 3
0 43.950		S31	1	#4	1	4'-5"	3
45,950		S32	1	#4	1	4'-1"	3
				# 5	2		50
Г		64		TVC		25'-1"	52
			<i>D</i> ,		LJ		
				R2 . R6) _		
			-		-		
				P3	R6		
			R1				
				$\begin{pmatrix} 1 \end{pmatrix}$	R4		
				_			
				4 ∕₂″ нк			
				(TYP.)	-		
				R5			
			1'-2 ¹ /2"	22′-10″	. 8 ¹ / ₂ ″		
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BURNING -							
ŀ	S28	1'-1 ³ /4"	83⁄4″	7"	8 ³ ⁄⊿"	1'-1 ³ ⁄⊿"	5″
	S29	1'-1 ³ /4"	9″	8 ¹ /2″	73⁄4″	1'-3"	6″
	S30	1′-0 ^l /2″	71/2″	7 ¹ /2"	7 ¹ /4″	1'-0¾"	5 ¹ /4″
	S31	1'-0 ¹ /4"	7 ¹ /4″	5 ¹ /4″	8 ¹ /2″	11″	3¾″
l	S32	10 ¹ /4″	73⁄4″	31/2"	7 ³ ⁄4″	10 ¹ /4″	2 ¹ /2"
		ΝΙΤΙΤ					
	QUA		IES FU		NE PI		
	PILE		REINFORC]		O PSI (CRETE	0.6"ØL.F	۲.
				C	Y.	No.)
			107		, ,	10	_
		PILE)	193	3).3	16	_
	CURNER	PILE)	192		0.0	16	
		D			B-2	500A	В
		1 1		- • • • • •			_
				UAKE		COI	JNTY
CONCRETE SHEET P	ILE	C.	ΓΔΤΤΛΝ	. 317	0 + 75	.00	-L-
AINING WALL SHEET E DETAILS.SHEET 7		J					
1, FOR NOTES.		SH	EET 11 OF	11			
				STATE OF	NORTH CAROL		TAN
ALL	SSION						
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	INFER S		CONC	<u>ĸ</u> ĔŤĔ			LLE
4,00	CARRIN	,	C I I F	ΞŢΑĬĹ	йТЙĊ	<u>₩</u> ₽Ŀ	
*******	111111111.		SHEE	. I P.	LLE l	JETA.	TL2
,n	ocuSigned bv:						
	Josef M. Dentin		· ·	REVISIO	NS		SHEET NO.
در م در م	1EAF7523943466	N0.	BY:	DATE: NO.	BY:	DATE:	W-11
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	, 2025	2		୍କ ଜ			SHEETS 13



	GEOTECH ENGINE	NICAL ER	ENGINEER
ND REINFORCED APPROACH FI	L STUTH CAR		
L- STA 3159+06 +/- LEFT		N. N.	
	SEAL	2	
	Fr. WGINE		
	DocuSigned by	ALL IN ALL OF AL	
	Michael Valiquette 20FDC306F2F640B	27/15/2015	
	SIGNATURE	DATE	SIGNATURE DATE
FND RFINFORGED APPROACH	- // /		
-L- STA 3/58+93 +/- RIGHT	* ====		
7			
= REINFORCED APPROACH FILLS			
REINFORCED APPROA	CH FILLS		
ESTIMATED QUAN	ITIES		
ORCED APPROACH FILLS" AT EB1		SQ.FT.	
ORCED APPROACH FILLS"AT EB2	5,590	SQ.FT.	
"REINFORCED APPROACH FILLS"		SQ.FT.	
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ILL			
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	PROJECT NO.: R	-2500AR	
			ΓΟΙΝΤ
	STATION: 3170+	/5.00 -L-	-
	SHEET 1 OF 2		
NGINEERING UNIT			
EASTERN REGIONAL OFFICE	KE		
WESTERN REGIONAL OFFICE	AH		ЪН
CONTRACT OFFICE		FILLS	
OF NODTH CADOLINIA	DET	ISIONS	OUEET NO
ENT OF TRANSPORTATION	NO. BY DATE	NO.	BY DATE W-12
RALEIGH	1	3	TOTAL SHEET
		4	13



- 1. FOR REINFORCED APPROACH FILLS, SEE REINFORCED APPROACH FILLS SPECIAL PROVISION.

SIGNATURE

DATE



NOTES

4. INSTALL CONCRETE SHEET PILES AND END BENT PILES PRIOR TO CONSTRUCTING REINFORCED APPROACH FILLS.

5. EXCAVATE AS NECESSARY TO CONSTRUCT BOTTOM OF REINFORCED APPROACH FILLS TO THE ELEVATION SHOWN ON THE PLANS.

6. INSTALL GEOTEXTILE RETENTION FABRIC UP BACK FACE OF CONCRETE SHEET PILES AS SHOWN ON TYPICAL SECTION. SUBMIT CONSTRUCTION ADHESIVE TO THE ENGINEER FOR REVIEW AND APPROVAL TO HOLD GEOTEXTILE RETENTION FABRIC IN PLACE UNTIL SPACE BETWEEN APPROACH FILL FACING AND CONCRETE SHEET PILE RETAINING WALLS HAS BEEN BACKFILLED.

8. GEOTEXTILE RETENTION FABRIC SHALL MEET THE REQUIREMENTS OF TYPE 1 GEOTEXTILE AS SHOWN IN TABLE 1056-1 OF THE

9. PLACE GEOTEXTILE REINFORCEMENT FABRIC AT LOCATIONS AND ELEVATIONS SHOWN ON THIS PLAN SHEET AND IN SLIGHT

12. PLACE WELDED WIRE FORMS AS NEAR TO VERTICAL AS POSSIBLE WITH NO NEGATIVE BATTER. CONSTRUCT REINFORCED APPROACH FILLS WITH A MAXIMUM VERTICAL AND HORIZONTAL TOLERANCE OF 3" WHEN MEASURED WITH A 10'-0" STRAIGHT EDGE AND AN OVERALL VERTICAL PLUMBNESS (BATTER) AND HORIZONTAL ALIGNMENT OF LESS THAN 6".

13. DO NOT SPLICE OR OVERLAP GEOTEXTILE REINFORCEMENT FABRIC IN THE MACHINE DIRECTION (MD), i.e., PARALLEL TO THE REINFORCED APPROACH FILL FACE. OVERLAPS ONLY ARE ALLOWED IN THE CROSS-MACHINE DIRECTION (CMD).

14. PLACE BACKFILL WITHIN REINFORCED APPROACH FILLS IN 8" TO 10" THICK LIFTS AND COMPACT IN ACCORDANCE WITH SUBARTICLE 235-3(C) OF THE STANDARD SPECIFICATIONS. USE ONLY HAND OPERATED COMPACTION EQUIPMENT WITHIN 3'-0"

16. DO NOT DAMAGE GEOTEXTILE REINFORCEMENT FABRIC OR WELDED WIRE FORMS WHEN PLACING AND COMPACTING BACKFILL. DO NOT OPERATE HEAVY EQUIPMENT ON GEOTEXTILE REINFORCEMENT FABRIC UNTIL IT IS COVERED WITH AT LEAST 8" OF BACKFILL. DO NOT USE SHEEPSFOOT, GRID ROLLERS OR OTHER TYPES OF COMPACTION EQUIPMENT WITH FEET.

17. CONSTRUCT REINFORCED APPROACH FILLS TO BOTTOM OF COPING AND BOTTOM OF END BENT CAP ELEVATIONS PRIOR TO FILLING SPACE BETWEEN FACE OF REINFORCED APPROACH FILL AND BACK OF CONCRETE SHEET PILING WITH COMMON BORROW.

18. BACKFILL SPACE WITH COMMON BORROW BETWEEN FACE OF REINFORCED APPROACH FILL AND BACK OF CONCRETE SHEET PILING PRIOR TO PLACING FINAL FABRIC LIFT AS SHOWN IN DETAILS. ADD LIMITED AMOUNTS OF WATER AS DIRECTED BY THE

19. COMPACT TOP OF COMMON BORROW PLACED BETWEEN FACE OF REINFORCED APPROACH FILL AND BACK OF CONCRETE SHEET PILING PRIOR TO PLACING FINAL FABRIC LIFT USING HAND OPERATED COMPACTION EQUIPMENT TO THE SATISFACTION OF

20. CONSTRUCT RETAINING WALL COPINGS AND END BENT CAPS PRIOR TO PLACING FINAL GEOTEXTILE REINFORCING FABRIC LIFT. PLACE FINAL GEOTEXTILE REINFORCING FABRIC LIFT DIRECTLY AGAINST IN PLACE RETAINING WALL COPINGS AND END BENT

	PROJECT NO.: B-2500AB					
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	STATION: 3170+75.00 -L-					
	SHEET 2 OF 2					
NGINEERING UNIT						
EASTERN REGIONAL OFFICE						
WESTERN REGIONAL OFFICE						
CONTRACT OFFICE	TILLS					
OF NORTH CAROLINA	REVISIONS SHEET NO.					
NT OF TRANSPORTATION	NO. BY	DATE	NO.	BY	DATE	W-13
RALEIGH	1		3			TOTAL SHEETS
	2		4			13

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS. ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION. SETTLEMENT. ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990