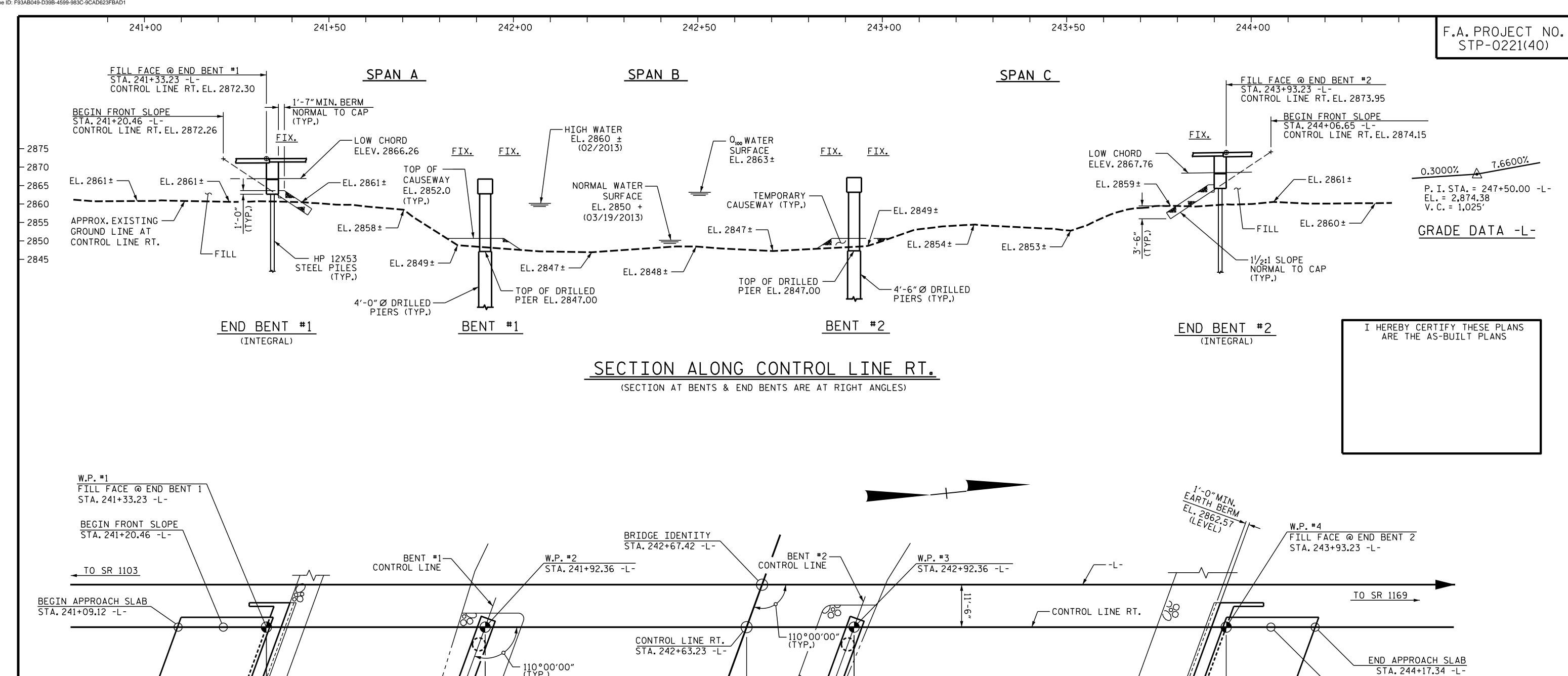
# This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page.

This file or an individual page shall not be considered a certified document.



- 110°00′00″ (TYP.) FILL FACE @-END BENT 1 BEGIN FRONT SLOPE STA. 244+06.65 -L--FILL FACE @ TEMPORARY -END BENT 2 CAUSEWAY (TYP.) PROJECT NO. R-2915B 70′-10½6″ 29'-1 9/16" 100'-107<sub>6</sub>"(SPAN C) 59'-1%6"(SPAN A) 100'-0"(SPAN B) STATION: 242+67.42 -L-

260'-0" TOTAL LENGTH OF BRIDGE, FILL FACE TO FILL FACE

> PLAN (PILES NOT SHOWN IN PLAN VIEW)

> > 1121 Situs Court **PLANS PREPARED BY:** Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

GENERAL DRAWING BRIDGE OVER SOUTH FORK NEW RIVER ON US 221 BETWEEN SR 1103 AND SR 1169 NBL SEAL \* 037180 REVISIONS

SHEET 1 OF 3

ASHE

COUNTY

BRIDGE NO. 541

SHEET NO. S05-1 NO. BY: DATE: DATE: BY: TOTAL SHEETS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

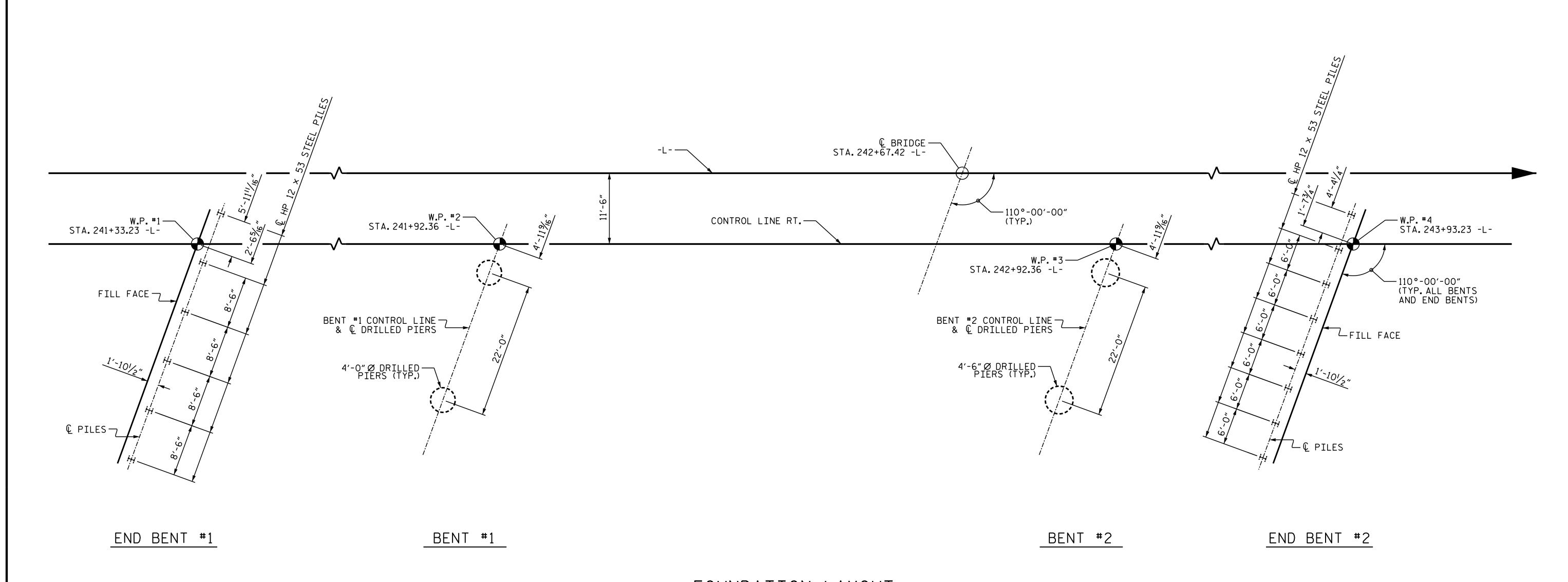
CHECKED BY : R.F. WERTMAN \_\_ DATE : 8/19/14 DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 10/3/14

DRAWN BY : T.J. KIRSCHBAUM

- CLASS II RIP RAP (TYP.)

DATE : 5/5/14

+



#### FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE.
DIMENSIONS LOCATING DRILLED PIERS ARE SHOWN TO DRILLED PIER CENTERLINE.

#### NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 2821.0 FT AND WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 590 TONS PER PIER.

CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30.0 TSF.

PERMANENT STEEL CASING WILL BE REQUIRED FOR DRILLED PIERS AT BENT NO.1.DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 2842.0 FT. WITHOUT PRIOR APPROVAL FROM THE ENGINEER. THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 2836.0 FT. THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE. INSTALL DRILLED PIERS AT BENT NO. 2 TO A TIP ELEVATION NO HIGHER THAN 2811.2 FT (LT) AND 2819.0 FT (RT) AND WITH THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 680 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30.0 TSF.

PERMANENT STEEL CASING WILL BE REQUIRED FOR DRILLED PIERS AT BENT NO.2. DO NOT EXTEND PERMANENT CASING BELOW ELEVATION 2840.0 FT. WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

THE SCOUR CRITICAL ELEVATION FOR BENT NO. 2 IS ELEVATION 2836.0 FT. THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS AT BENT NO.1 AND BENT NO. 2. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE. PROJECT NO. R-2915B

ASHE COUNTY

STATION: 242+67.42 -L-

SHEET 2 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE OVER SOUTH

FORK NEW RIVER ON US 221 BETWEEN SR 1103 AND SR 1169 NBL

SHEET NO. REVISIONS S05-2 NO. BY: BY: DATE: DATE: TOTAL SHEETS 35

1121 Situs Court Suite 170 Raleigh NC 27606-4279



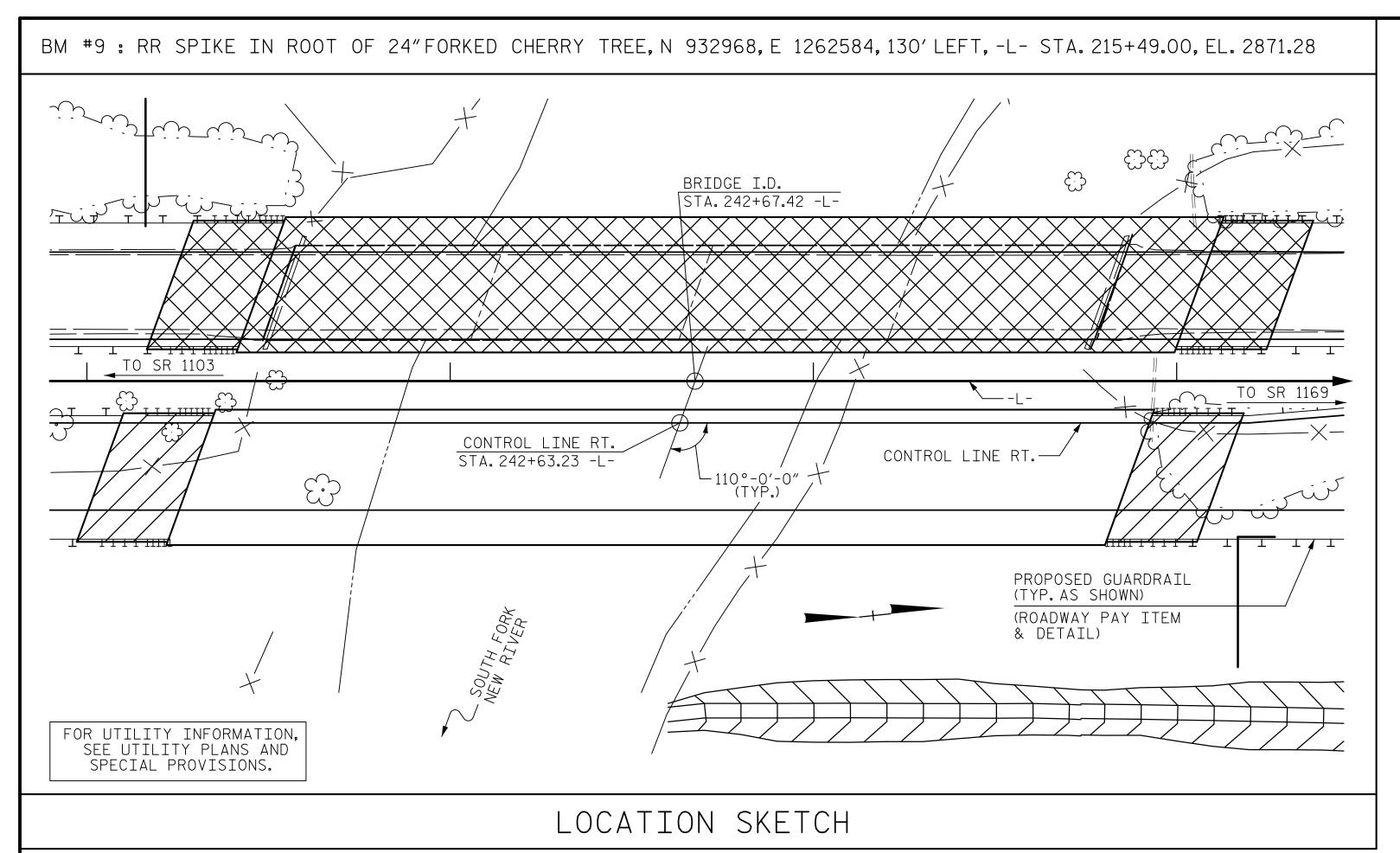
**Gannett Fleming** (919) 859-4880 Excellence Delivered As Promised NC Lic. No. F-0270

DRAWN BY : T.J. KIRSCHBAUM DATE : 10/29/14 \_ DATE : 11/12/14 CHECKED BY : R.F. WERTMAN DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/12/14

\* ROFESSION !

SEAL 037180

Cifaci & Walter



#### HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION

= 16,000 C.F.S. = 50 YRS. = 2862.2

DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

= 130 SQ.MI. = 19,000 C.F.S. = 2863.0

#### OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION

= 24,000+ C.F.S. = 500+ YRS. = 2869.2

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

NOTES:

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PLACE FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 242+67.42 -L-.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-5 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

_			OTAL B	ILL OF	MATER	XIAL —			
	CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPEMPORARY ACCESS	4'-0"Ø DRILLED PIERS IN SOIL	4'-0"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 4'-0"Ø DRILLED PIERS	4'-6"Ø DRILLED PIERS IN SOIL	4'-6"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 4'-6"Ø DRILLED PIERS	CSL TESTING	REINFORCED CONCRETE DECK SLAB
	LUMP SUM	LIN.FT.	LIN. FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	EACH	SQ.FT.
SUPERSTRUCTURE									9619
END BENT NO.1									
BENT NO.1		20.0	32.0	10.0					
BENT NO.2					24.8	39.0	14.0		
END BENT NO.2									
TOTAL	LUMP SUM	20.0	32.0	10.0	24.8	39.0	14.0	1	9619

				TOTAL	BILL	OF	<u> </u>	TE	RIA				
	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	С	54" ESTRESSED ONCRETE GIRDERS		12 X 53 EL PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS
	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	NO.	LIN.FT.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM
SUPERSTRUCTURE	9555		LUMP SUM			12	1023			516.5			LUMP SUM
END BENT NO.1		31.6		4346				6	165		77	85	
BENT NO.1		35.1		9530	2127								
BENT NO.2		42.6		10203	2811								
END BENT NO.2		32.5		4522				8	280		195	215	
TOTAL	9555	141.8	LUMP SUM	28601	4938	12	1023	14	445	516.5	272	300	LUMP SUM

1121 Situs Court **Gannett Fleming** Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION BRIDGE OVER SOUTH FORK NEW RIVER ON US 221 BETWEEN SR 1103 AND SR 1169

PROJECT NO. R-2915B

STATION: 242+67.42 -L-

COUNTY

ASHE

REVISIONS SHEET NO S05-3 NO. BY: DATE: BY: TOTAL SHEETS

STR.NO.5

SHEET 3 OF 3

DATE : <u>5/4/14</u>

DRAWN BY : T.J. KIRSCHBAUM

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																								
										STRE	NGTH	I LIM	IT ST	ATE				SE	RVICE	III	LIMI	TSTA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.08		1.75	0.899	1.08	A	Ŧ	28.0	1.014	1.37	A	I	4 <b>5.</b> 2	0.80	0.775	1.11	В	Ŧ	48.9	1 <b>;</b> 2
DESIGN LOAD		HL-93 (OPERATING)	N/A	-	1.40		1.35	0.899	1.40	A	Ŧ	28.0	1.022	2.24	E	Ŧ	78.6	N/A						1;2
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.37	49:32	1.75	0.899	1.37	A	Ŧ	28.0	10:14	2.09	A	Ŧ	33.8	0.80	0.775	1.56	В	Ŧ	48.9	1 <b>;</b> 2
		HS-20 (OPERATING)	36.000		1.77	63 <b>:</b> 72	1.35	0.899	1.77	A	Ŧ	28.0	1.022	2 <b>.</b> 99	E	Ŧ	78.6	N/A						1 <b>;</b> 2
		SNSH	13 <b>.</b> 500		3 <b>.</b> 67	49 <b>:</b> 55	1.40	0.899	3 <b>.</b> 67	A	Ŧ	28.0	1.022	7 <b>.</b> 65	E	Ŧ	78 <b>.</b> 6	0.80	0.775	3.70	В	Ŧ	48.9	1 <b>;</b> 2
	Ш	SNGARBS2	20.000	-•-	2 <b>.</b> 67	53 <b>:</b> 40	1.40	0.899	2 <b>.</b> 81	A	Ŧ	28.0	1.022	5 <b>.</b> 35	€	Ŧ	78.6	0.80	0.775	2.67	В	Ŧ	48.9	1;2
	ICL	SNAGRIS2	22.000	-•-	2 <b>.</b> 50	55 <b>:</b> 00	1.40	0.899	2.70	A	Ŧ	28.0	1.022	4 <b>.</b> 94	В	Ŧ	78.6	0.80	0.775	2.50	В	Ŧ	48.9	1 <b>;</b> 2
	VEH	SNCOTTS3	27.250	-•-	1.83	49:87	1.40	0.899	1.83	A	Ŧ	28.0	1.022	3 <b>.</b> 67	E	Ŧ	78.6	0.80	0.775	1.84	В	Ŧ	48.9	1 <b>;</b> 2
	SLE (S	SNAGGRS4	34.925	-•-	1.50	52:39	1.40	0.899	1.56	A	Ŧ	28.0	1.014	2 <b>.</b> 75	A	Ŧ	33.8	0.80	0.775	1.50	В	Ŧ	48.9	1 <b>;</b> 2
	SING	SNS5A	35 <b>.</b> 550	-•-	1.47	52 <b>:</b> 26	1.40	0.899	1.52	A	Ŧ	28.0	1.022	2 <b>.</b> 88	В	Ŧ	19.1	0.80	0.775	1.47	В	Ŧ	48.9	1 <b>;</b> 2
		SNS6A	39.950		1.34	53:53	1.40	0.899	1.41	A	Ŧ	28.0	1.022	2.60	В	Ŧ	19.1	0.80	0.775	1.34	В	Ŧ	48.9	1 <b>;</b> 2
LEGAL		SNS7B	42.000	-•-	1.27	53:34	1.40	0.899	1.34	A	Ξ	28.0	1.022	2.49	<b>©</b>	Ŧ	78.6	0.80	0.775	1.27	В	Ŧ	48.9	1 <b>;</b> 2
LOAD RATING	ER.	TNAGRIT3	33.000	-•-	1.63	53:79	1.40	0.899	1.72	A	Ŧ	28.0	1.022	3.09	В	Ŧ	19.1	0.80	0.775	1.63	В	I	48.9	1;2
	TRAII	TNT4A	33.075	-•-	1.63	53:91	1.40	0.899	1.73	A	Ŧ	28.0	1.022	3.26	В	Ŧ	78.6	0.80	0.775	1.63	В	Ŧ	48.9	1;2
	<u> </u>	TNT6A	41.600	-•-	1.32	54:91	1.40	0.899	1.43	A	Ŧ	28.0	1.022	2.68	В	Ŧ	19.1	0.80	0.775	1.32	В	Ŧ	48.9	1 <b>;</b> 2
	SEM ST)	TNT7A	42.000	-•-	1.32	55:44	1.40	0.899	1.44	A	Ŧ	28.0	1.022	2.63	В	I	19.1	0.80	0.775	1.32	В	Ŧ	48.9	1;2
	TOR (TT)	TNT7B	42.000		1.35	56:70	1.40	0.899	1.51	A	Ŧ	28.0	1.022	2 <b>.4</b> 8	В	Ŧ	19.1	0.80	0.775	1.35	В	Ŧ	48.9	1;2
	TRAC	TNAGRIT4	43.000		1.30	55 <b>:</b> 90	1.40	0.899	1.42	A	Ŧ	28.0	1.022	2.44	В	Ŧ	19.1	0.80	0.775	1.30	В	Ŧ	48.9	1;2
		TNAGT5A	45.000		1.23	55:35	1.40	0.899	1.34	A	Ŧ	28.0	1.022	2.33	В	Ŧ	19.1	0.80	0.775	1.23	В	Ŧ	48.9	1;2
	TRUCK	TNAGT5B	45.000	3	1.22	54 <b>:</b> 90	1.40	0.899	1.32	A	Ŧ	28.0	1.014	2.03	A	I	33.8	0.80	0.775	1.22	В	I	48.9	1;2

#### $56'-0\frac{1}{8}$ " (BRG. TO BRG.) 97'-9"(BRG. TO BRG.) 97'-9"(BRG. TO BRG.) SPAN A SPAN B SPAN C $\langle 3 \rangle$ END BENT 1 BENT 1 BENT 2 END BENT 2

<u>LRFR SUMMARY</u>

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 5/8/14 CHECKED BY: R.F. WERTMAN DATE: 7/31/14 DRAWN BY : MAA I/O8 REV. II/I2/O8RR MAA/GM REV. IO/I/II MAA/GM

Suite 170
Raleigh NC 27606-4279
(919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

1121 Situs Court

SEAL 037180 THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

#### LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

#### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

#### COMMENTS:

- 1. SPANS B AND C LENGTHS AND RATING FACTORS ARE EQUAL.
- 2. THE REDUCTION OF LOAD DISTRIBUTION FACTOR FOR MOMENT IN LONGITUDINAL BEAMS ON SKEWED SUPPORTS (AASHTO TABLE 4.6.2.2.2E-1) WAS NOT APPLIED.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\* \* SEE CHART FOR VEHICLE TYPE

#### GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

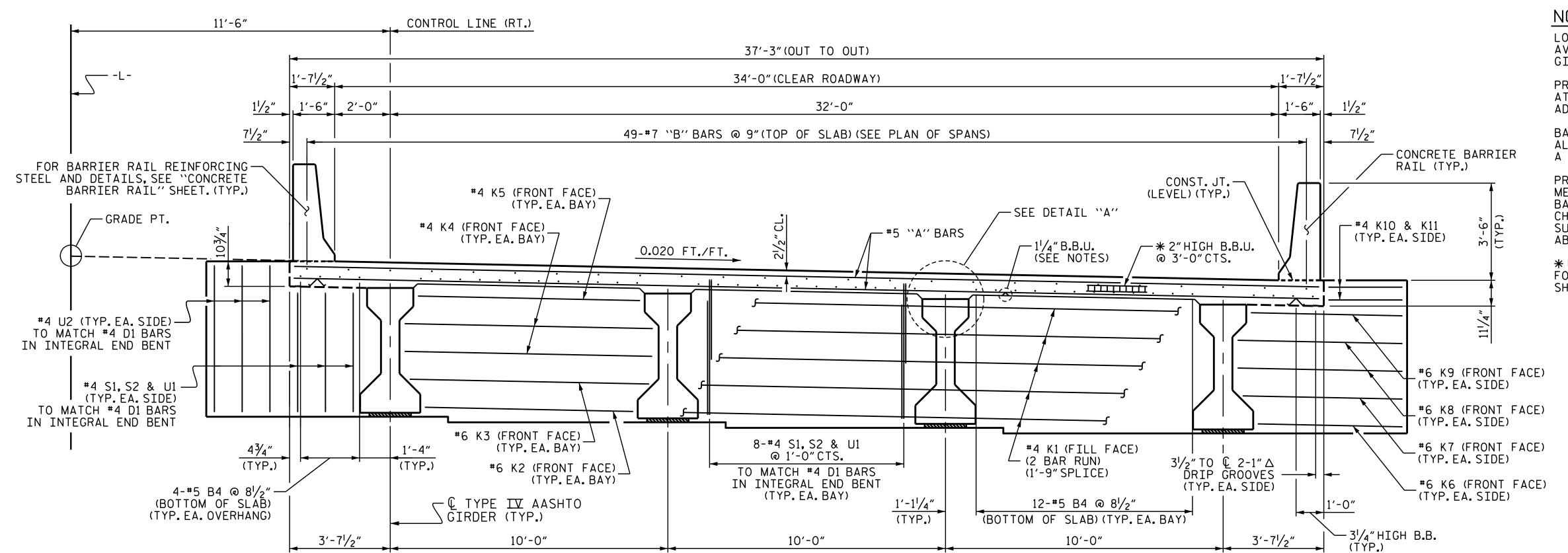
PROJECT NO. R-2915B ASHE \_ COUNTY STATION: 242+67.42 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD LRFR SUMMARY FOR
PRESTRESSED
CONCRETE GIRDERS
(NON-INTERSTATE TRAFFIC)

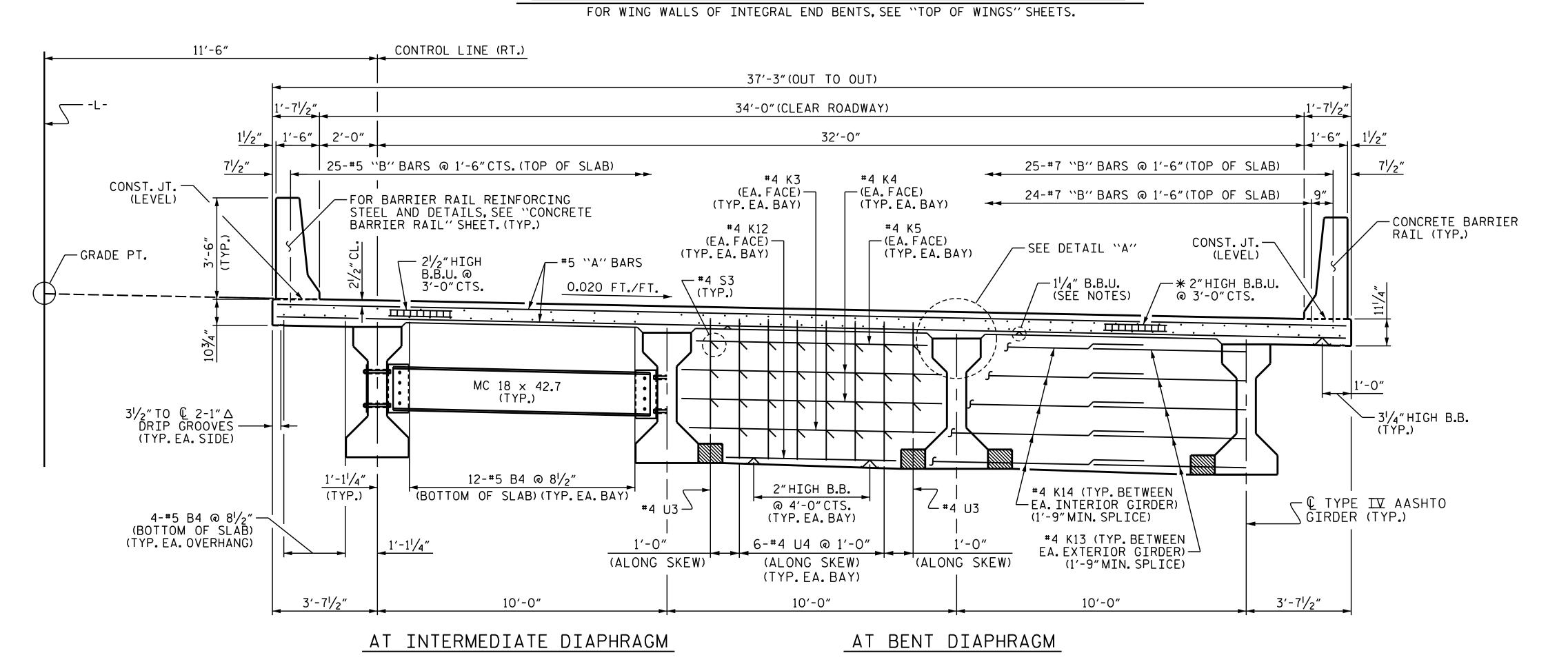
REVISIONS SHEET NO. S05-4 NO. BY: DATE: DATE: BY: 35

STD. NO. LRFR1

+



#### TYPICAL SECTION @ INTEGRAL END BENT



TYPICAL SECTION

**PLANS PREPARED BY:** Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880

1121 Situs Court Excellence Delivered As Promised NC Lic. No. F-0270

#### NOTES:

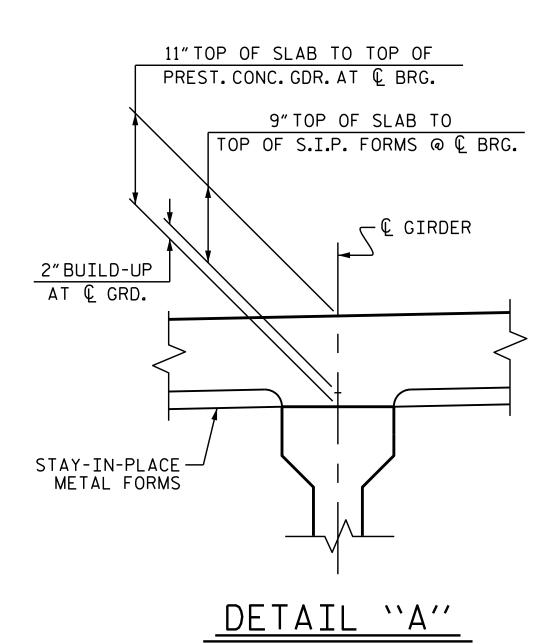
LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE

PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

PROVIDE 11/4" HIGH BEAM BOLSTERS UPPER AT 4'-0"CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-0"CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 21/2" ABOVE THE TOP OF THE REMOVABLE FORM.

\*USE THIS SIZE BAR SUPPORT IN THE AREAS WITH #7 "B" BARS. FOR OTHER AREAS WITH #4 "B" BARS, USE THE BAR SUPPORT AS SHOWN IN TYPICAL SECTION AT INTERMEDIATE DIAPHRAGM.



PROJECT NO. R-2915B ASHE COUNTY

STATION: 242+67.42 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

TYPICAL SECTION

NBL

REVISIONS SHEET NO. NO. BY: S05-5 DATE: BY: DATE: TOTAL SHEETS

STR.NO.5

DRAWN BY : T.J. KIRSCHBAUM DATE : 4/14/14 CHECKED BY : R.F. WERTMAN \_\_ DATE : <u>8/12/14</u> DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 10/3/14

OF ESSION

SEAL 3 037180

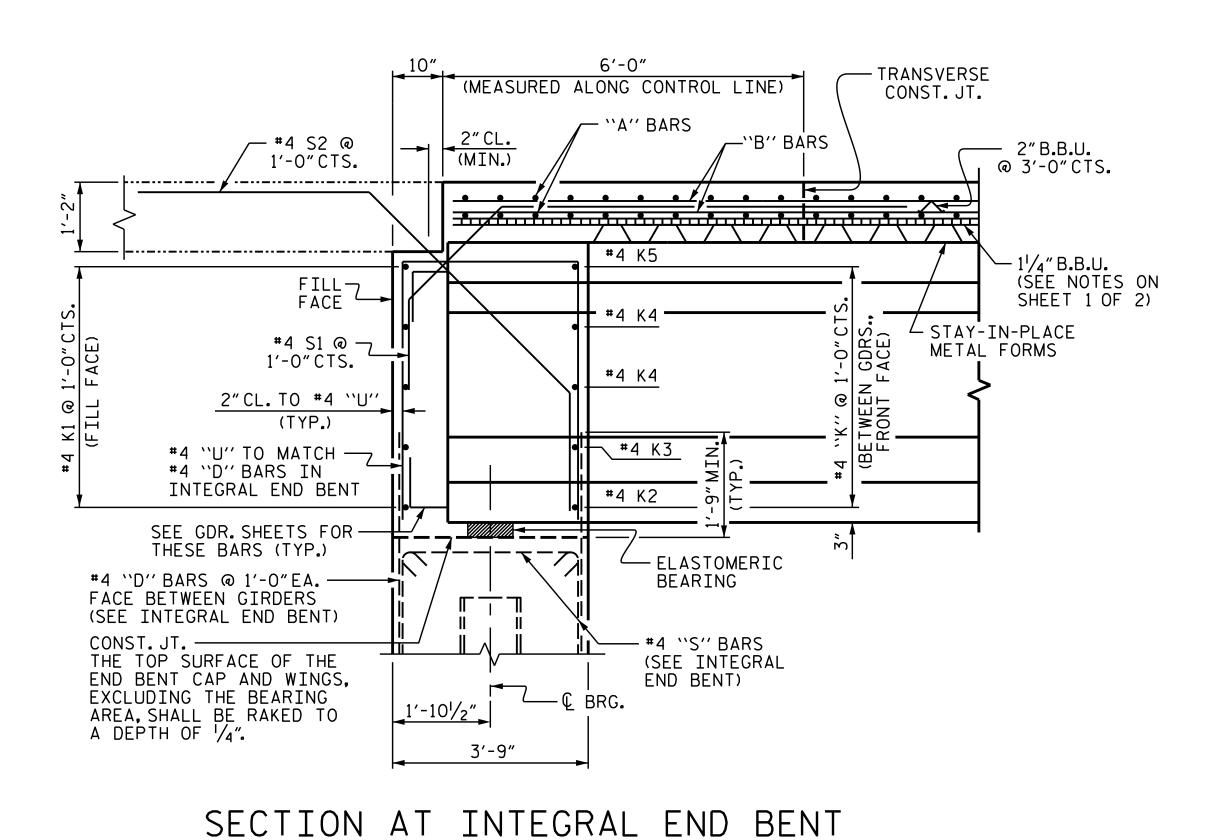
Dogo Signe Gly V

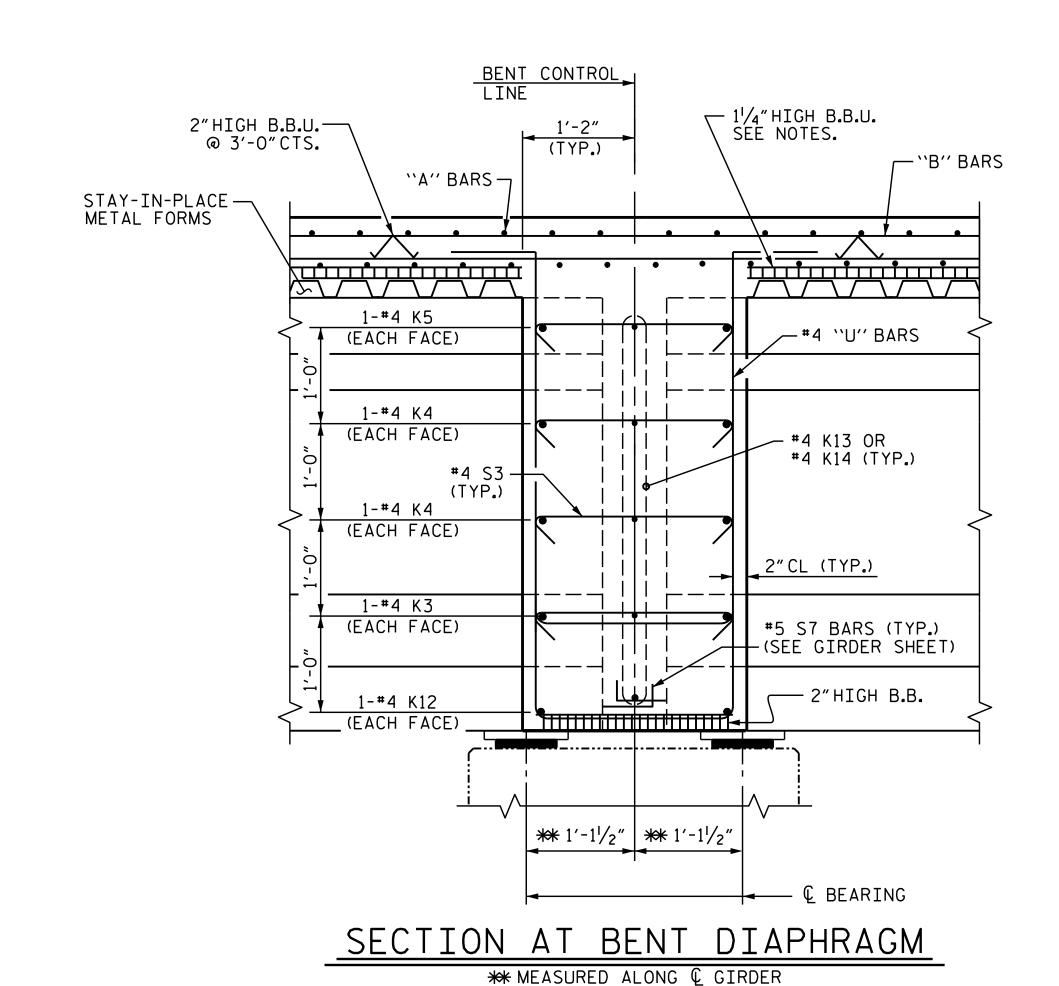
CHECKED BY : R.F. WERTMAN

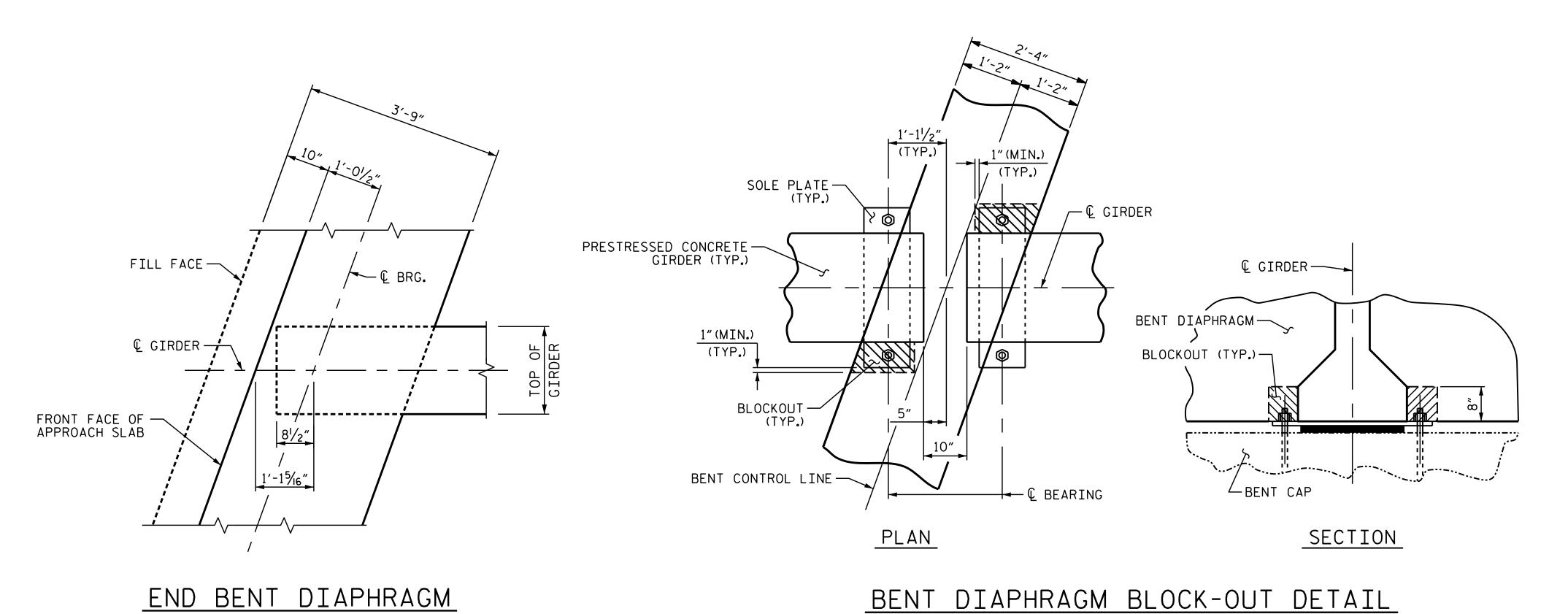
DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 10/3/14

DATE : 4/14/14

DATE: 8/12/14







PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH SUPERSTRUCTURE

TYPICAL SECTION DETAILS

NBL

REVISIONS SHEET NO. S05-6 NO. BY: DATE: DATE: BY: TOTAL SHEETS

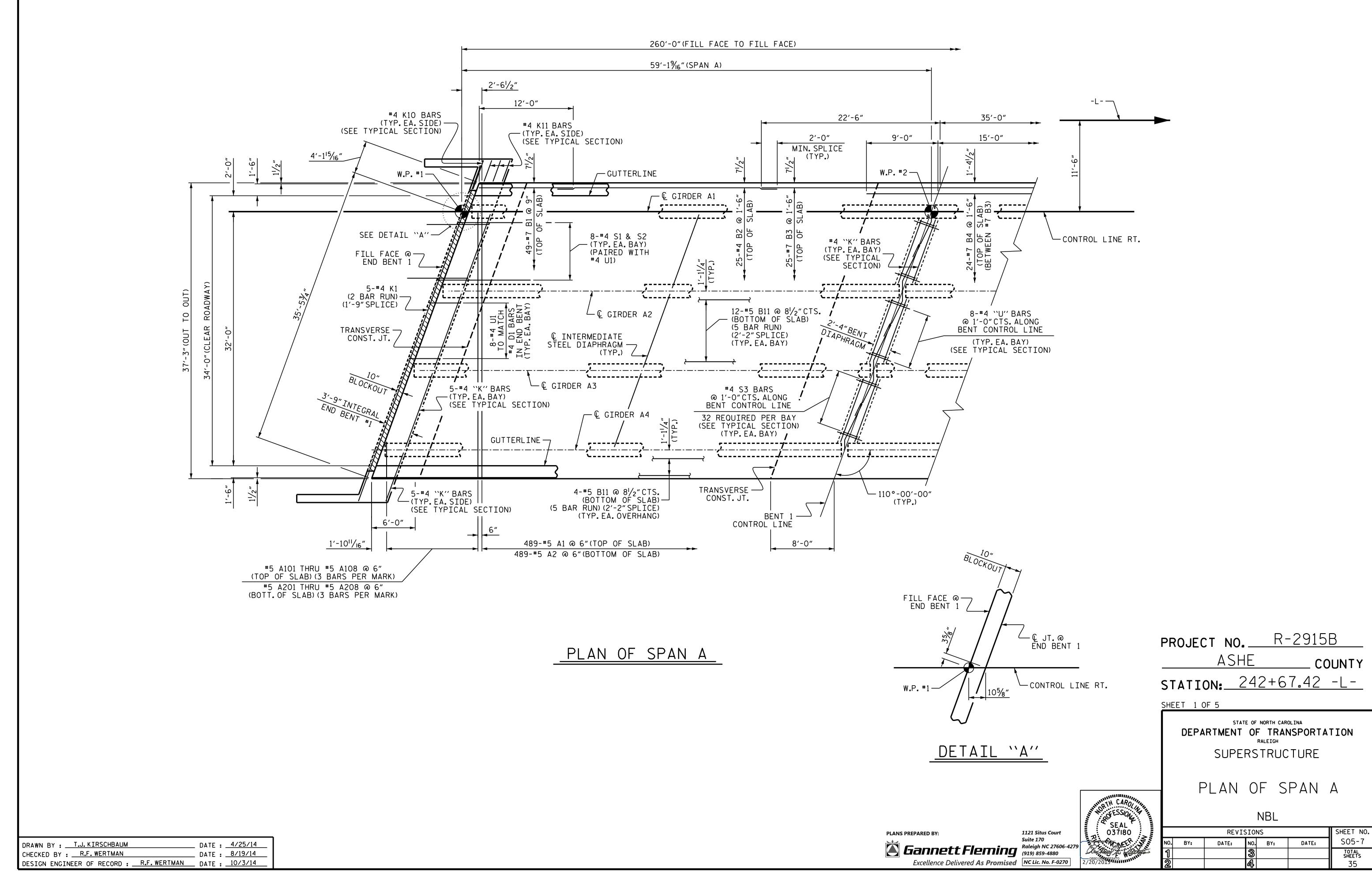
Suite 170
Raleigh NC 27606-4279
(919) 859-4880

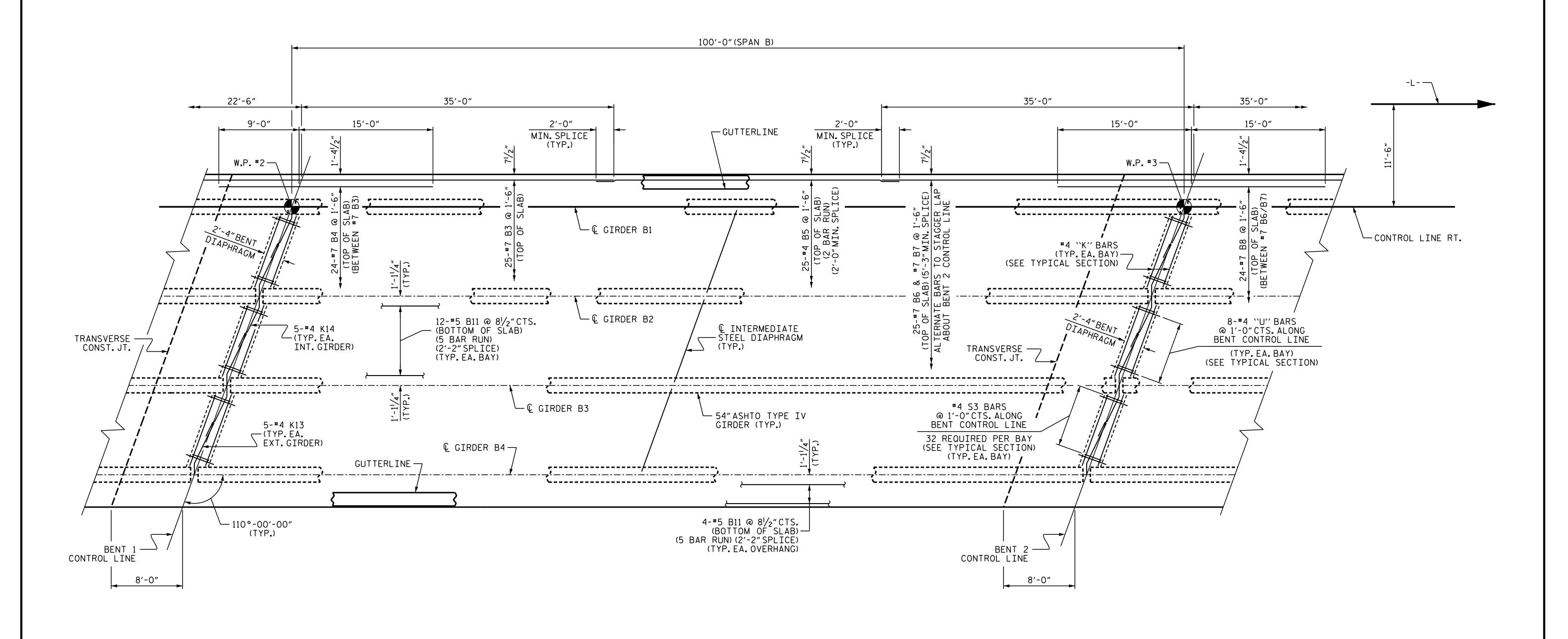
Excellence Delivered As Promised NC Lic. No. F-0270

STR.NO.5

SEAL \* 037180

1121 Situs Court





PLAN OF SPAN B

PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

SHEET 2 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

PLAN OF SPAN B

NBL

REVISIONS SHEET NO. S05-8 NO. BY: DATE: DATE: BY: TOTAL SHEETS

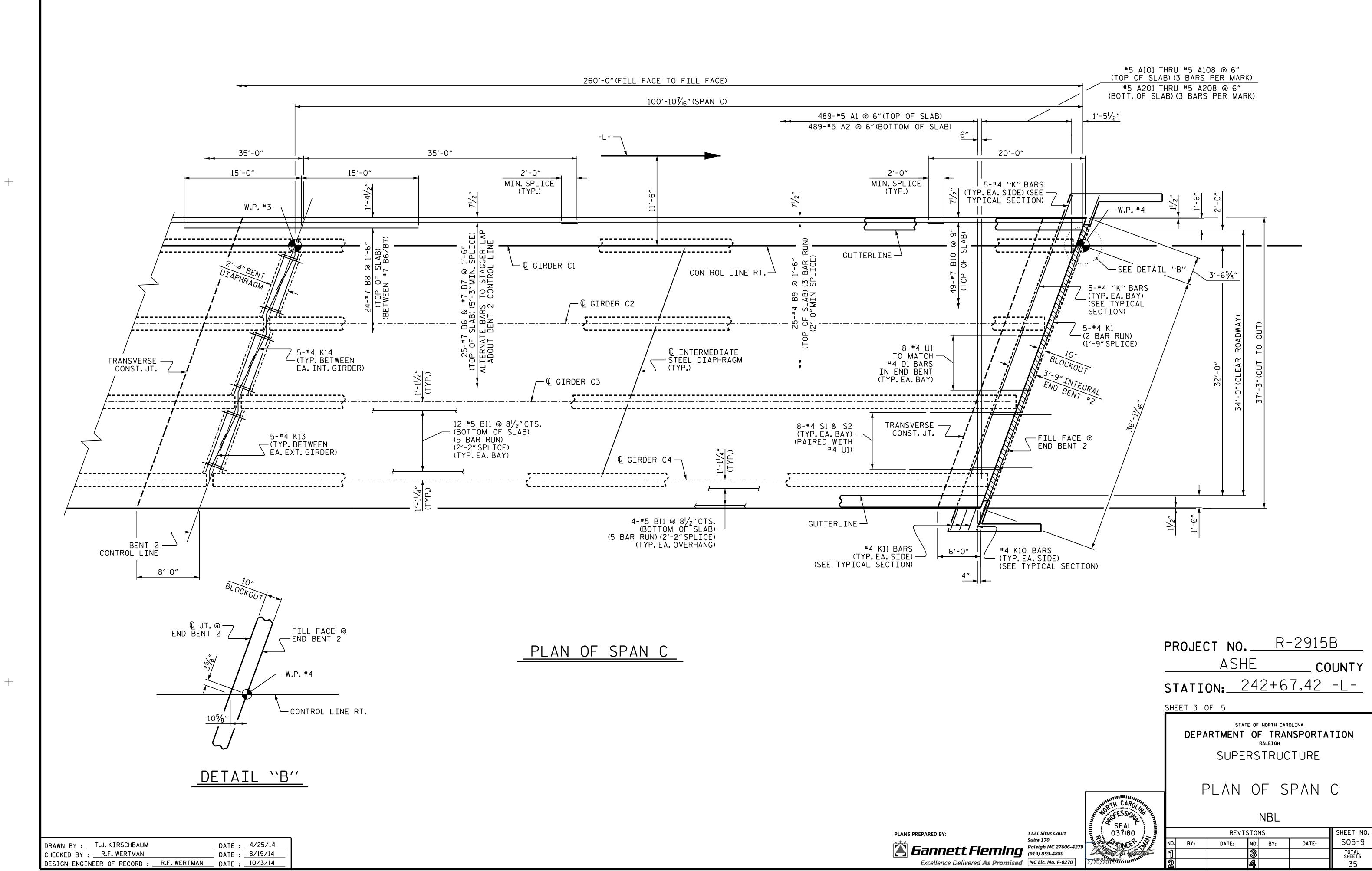
PLANS PREPARED BY:

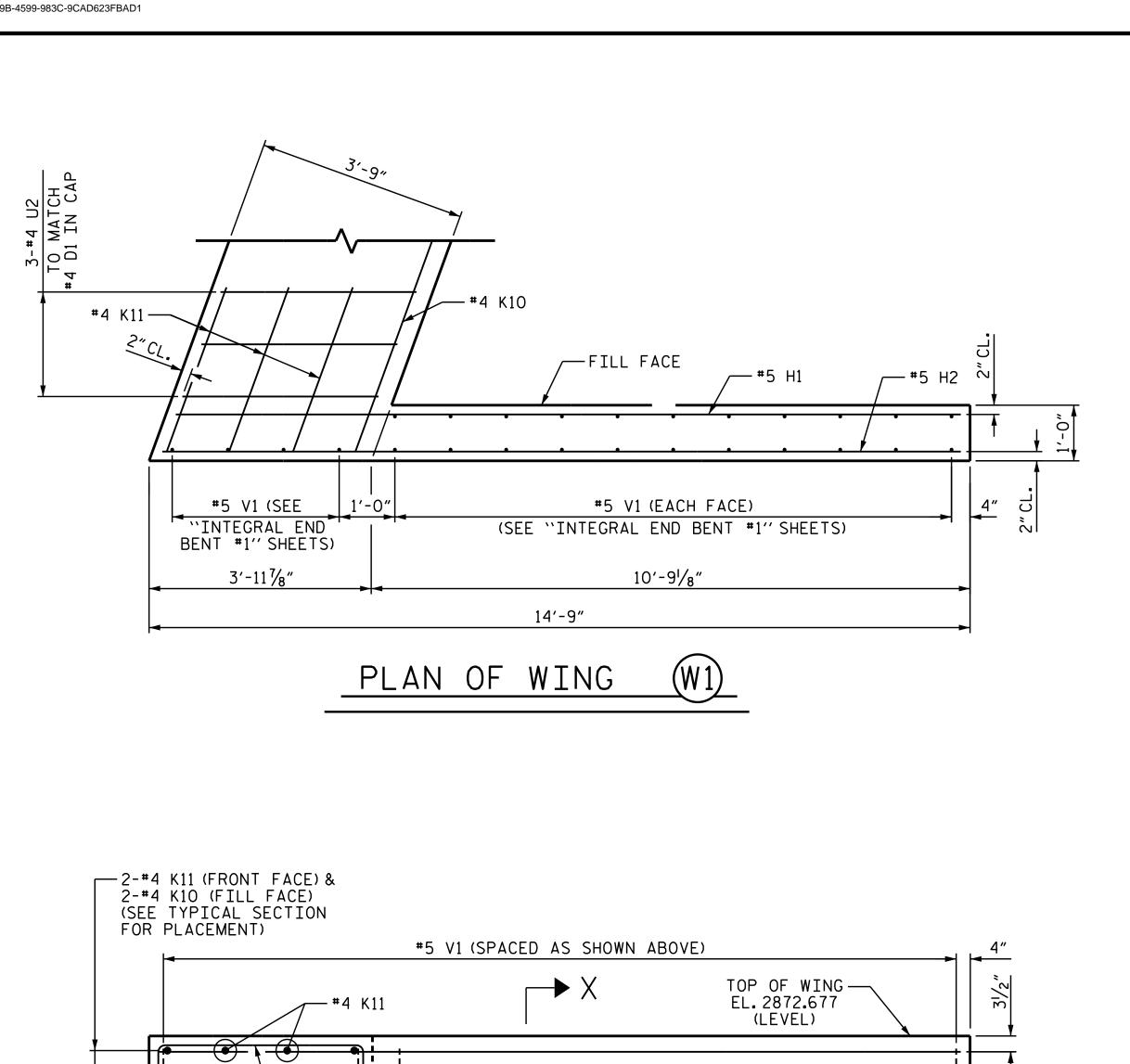
1121 Situs Court Suite 170
Raleigh NC 27606-4279
(919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

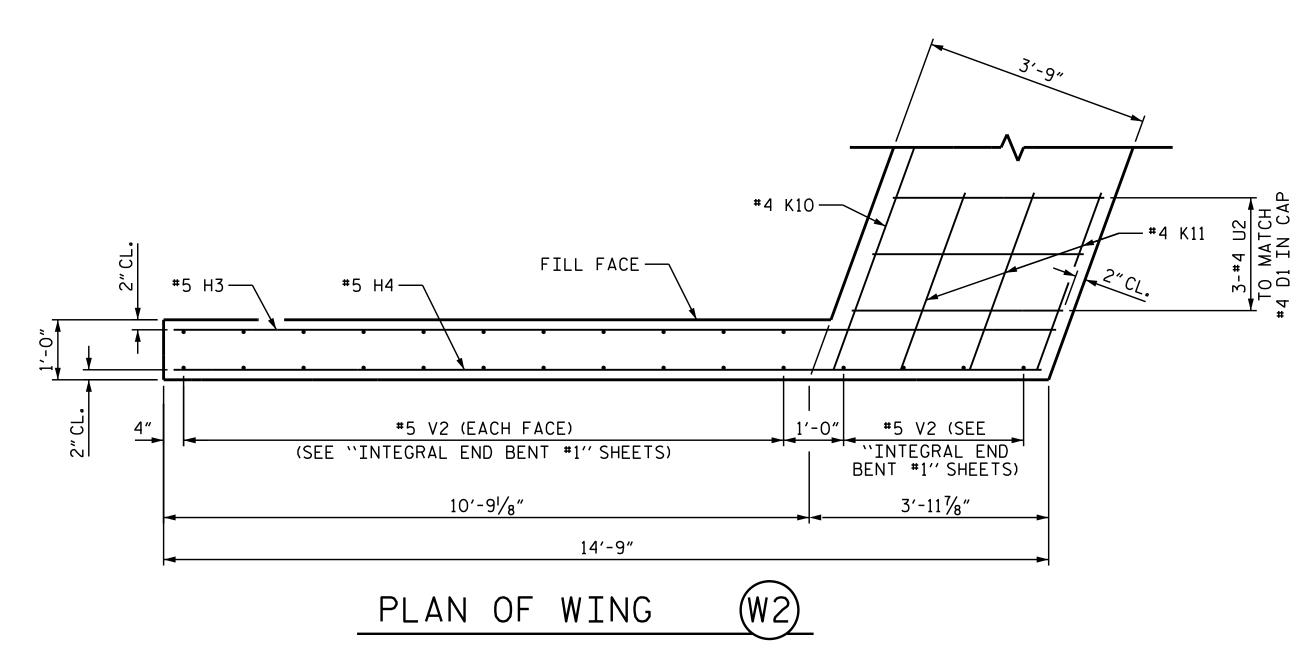
DATE : 4/25/14 DRAWN BY : T.J. KIRSCHBAUM CHECKED BY : R.F. WERTMAN DATE : 8/19/14 DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 10/3/14

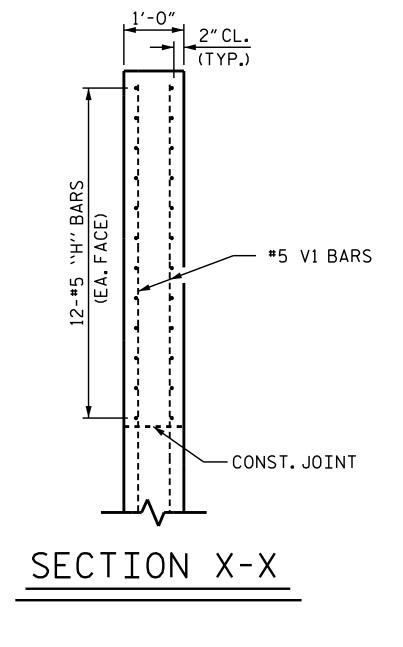
STR.NO.5

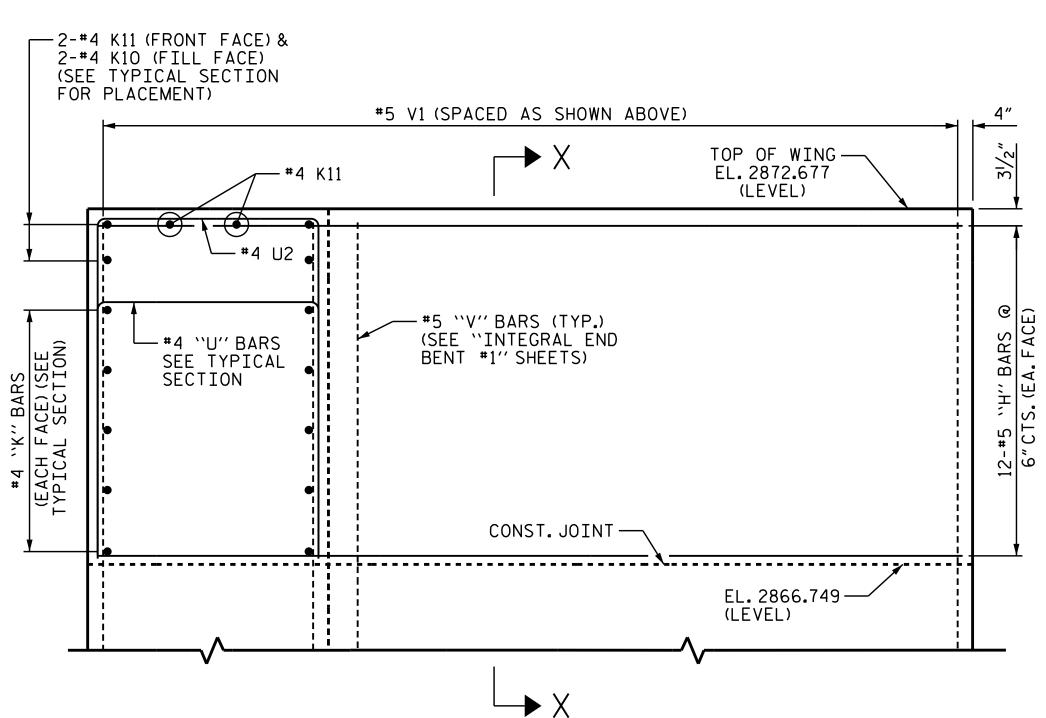
SEAL \* 037180



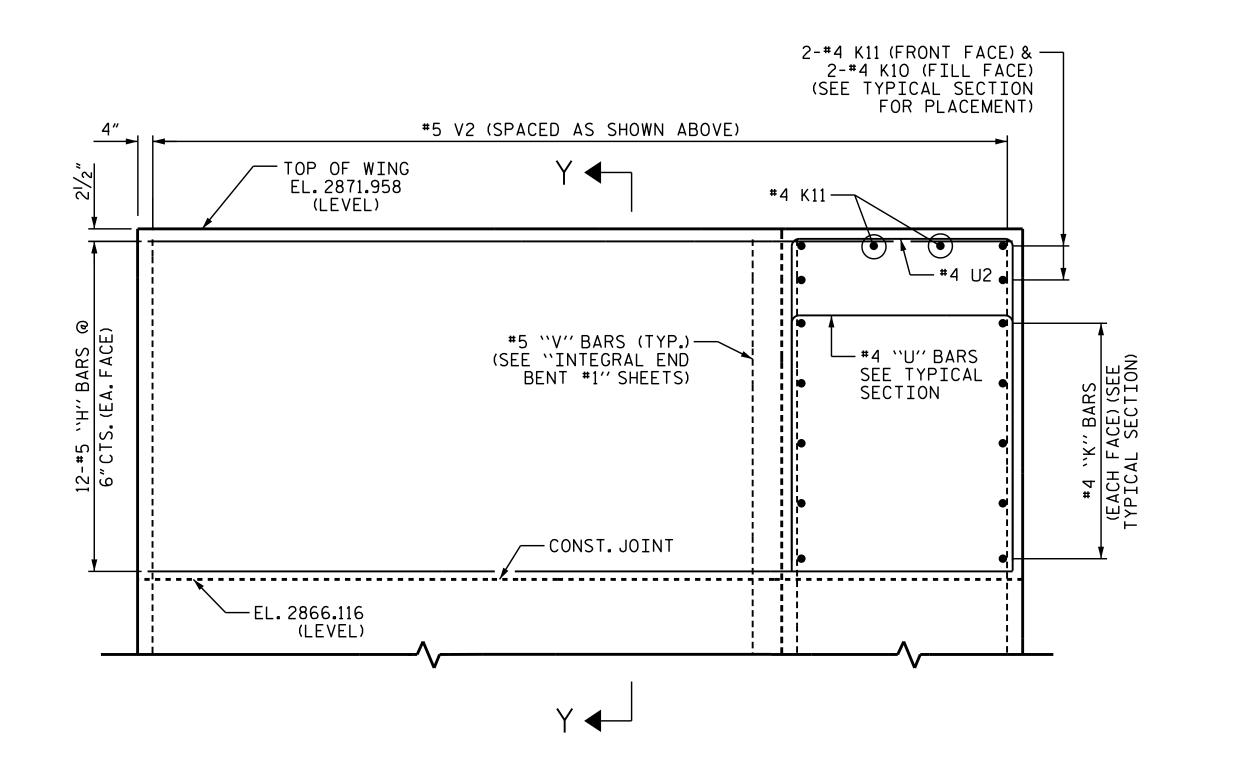


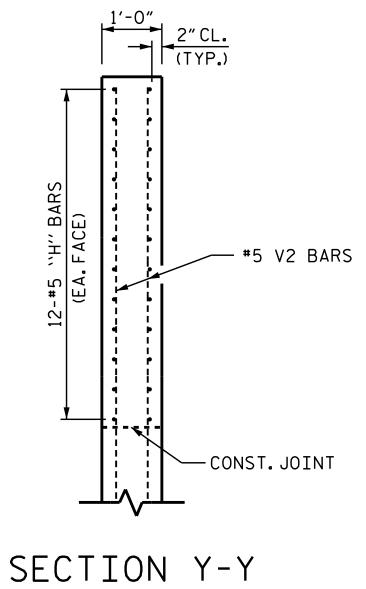






ELEVATION OF WING





PROJECT NO. R-2915B ASHE

COUNTY STATION: 242+67.42 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH SUPERSTRUCTURE

TOP OF WINGS @ END BENT #1

NBL

REVISIONS SHEET NO. S05-10 NO. BY: DATE: DATE: BY: TOTAL SHEETS 35

ELEVATION OF WING

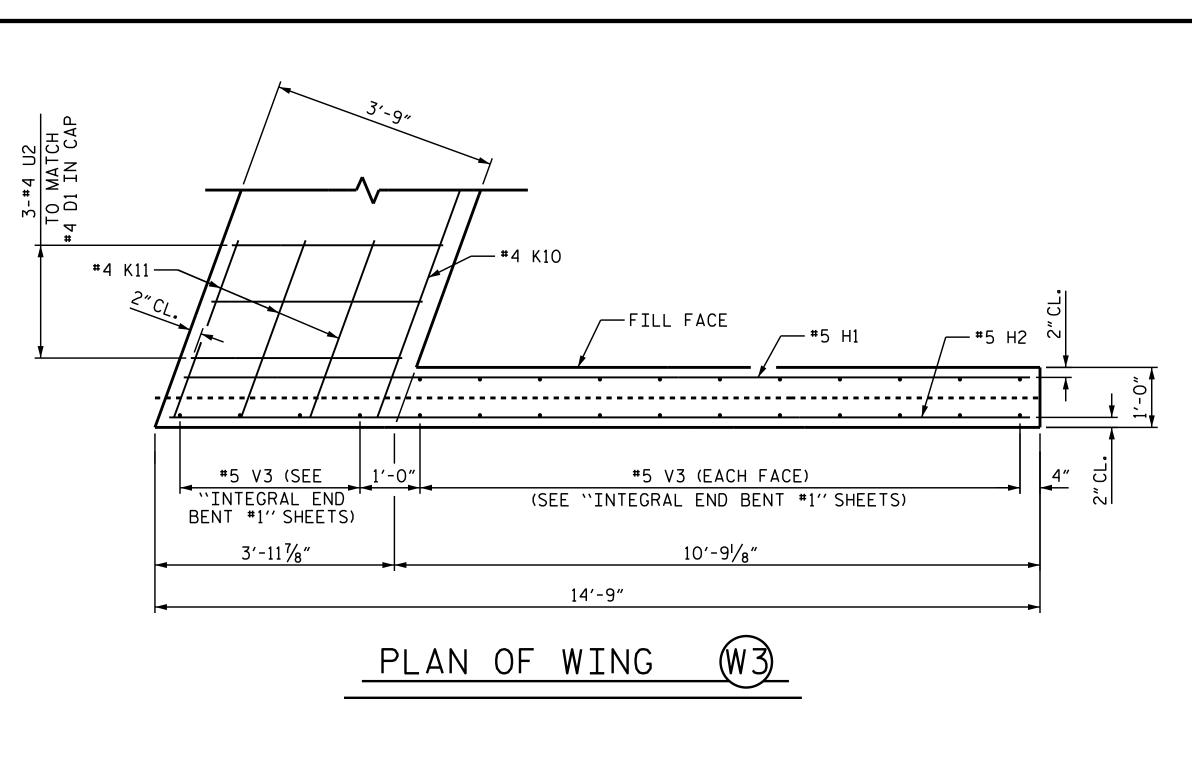
PLANS PREPARED BY:

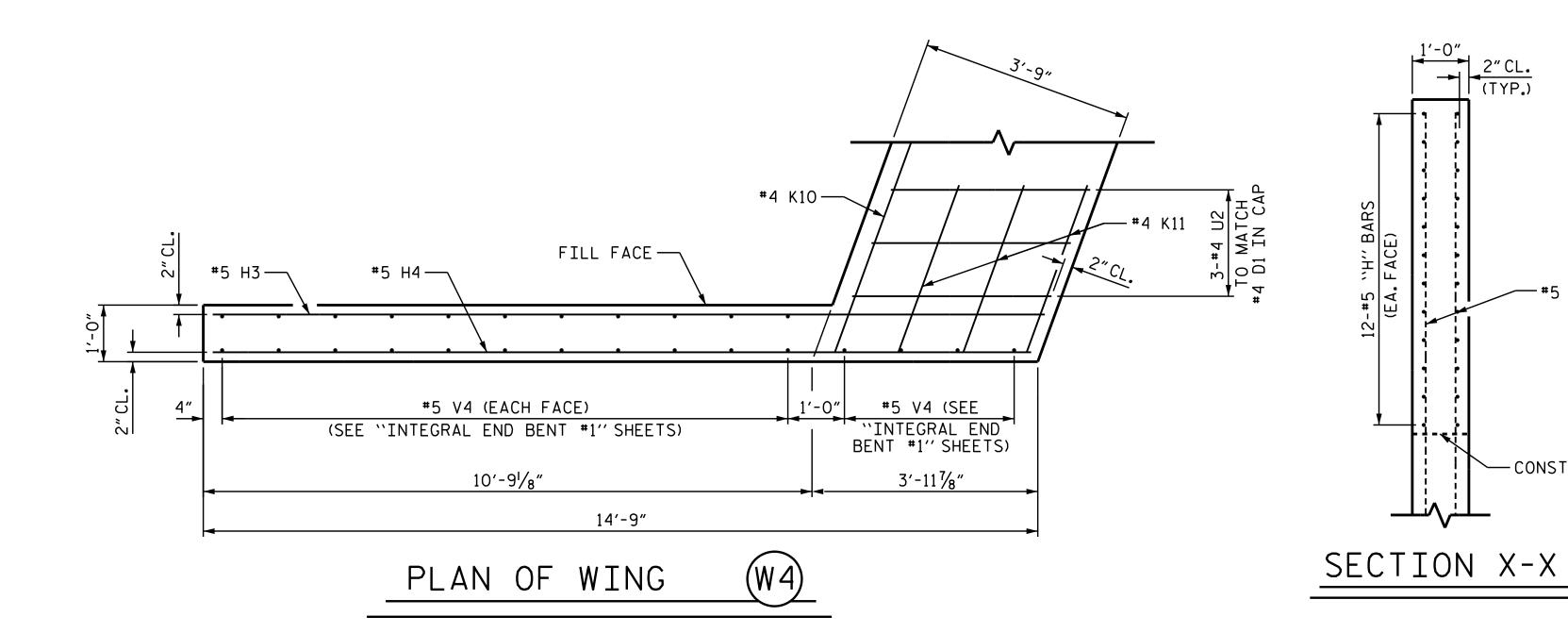
1121 Situs Court
Suite 170
Raleigh NC 27606-4279
(919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

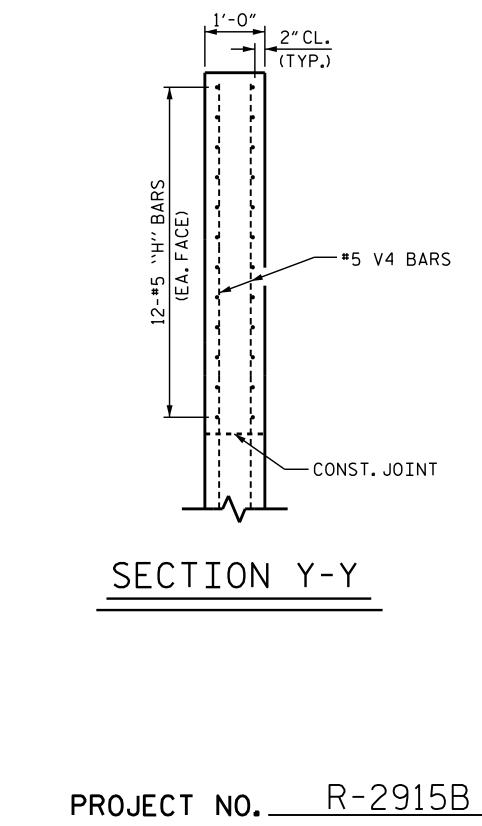
DRAWN BY : T.J. KIRSCHBAUM DATE : 10/08/14 CHECKED BY : E.E. DEETSCREEK DATE : 11/10/14 \_ DATE : 11/13/14 DESIGN ENGINEER OF RECORD : R.F. WERTMAN

STR.NO.5

SEAL 037180







ASHE

STATION: 242+67.42 -L-

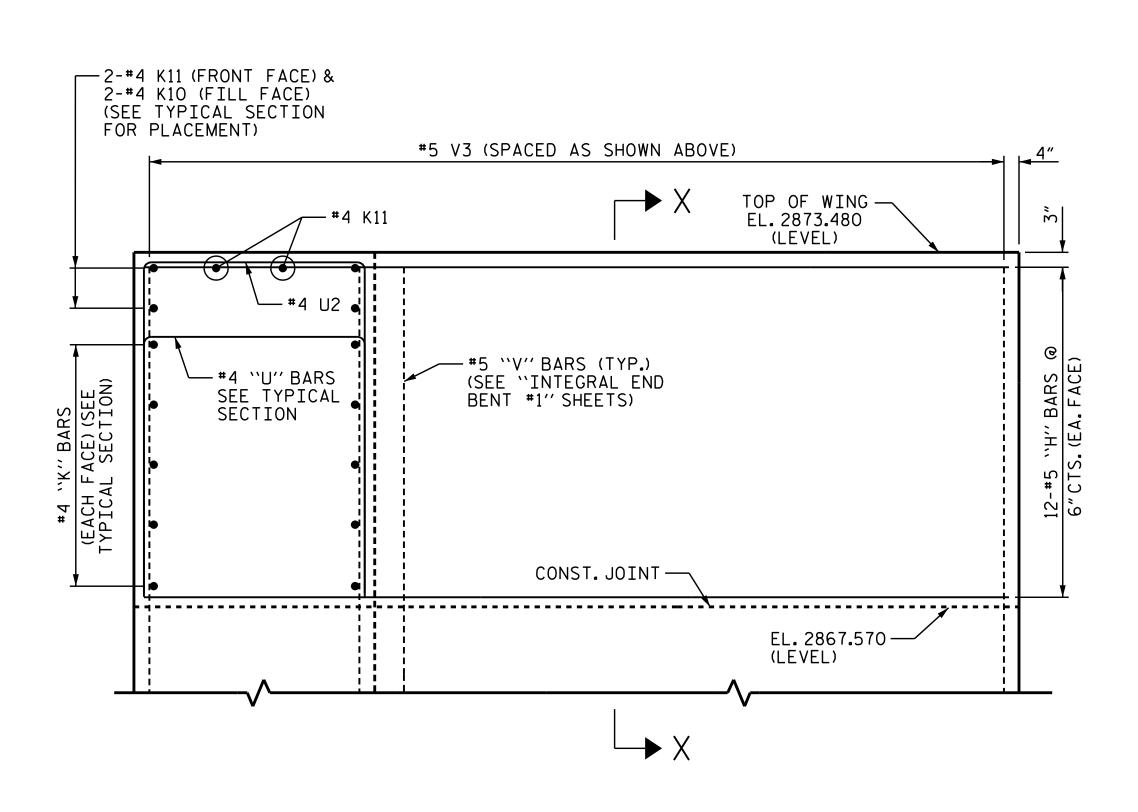
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

2" CL. (TYP.)

-#5 V3 BARS

CONST. JOINT



ELEVATION OF WING

ELEVATION OF WING

#5 V4 (SPACED AS SHOWN ABOVE)

#5 "V" BARS (TYP.)—— (SEE "INTEGRAL END BENT #1" SHEETS)

— CONST. JOINT

TOP OF WING EL. 2874.338

(LEVEL)

EL. 2868.319 (LEVEL)

12-#5 "H" BARS 6"CTS.(EA.FAC

2-#4 K11 (FRONT FACE) & — 2-#4 K10 (FILL FACE) (SEE TYPICAL SECTION FOR PLACEMENT)

#4 "U" BARS
SEE TYPICAL
SECTION

1121 Situs Court **PLANS PREPARED BY:** Suite 170
Raleigh NC 27606-4279
(919) 859-4880

THE CAROLANDIA			@ EN		NBL	VT #2	_
SEAL 037180			REVI	SION	S		S
	NO.	BY:	REVI	SION	S BY:	DATE:	S

SHEET 5 OF 5

DRAWN BY : T.J. KIRSCHBAUM DATE : 10/13/14 DATE : 11/10/14

DATE : 11/13/14 CHECKED BY : E.E. DEETSCREEK DESIGN ENGINEER OF RECORD : R.F. WERTMAN

+

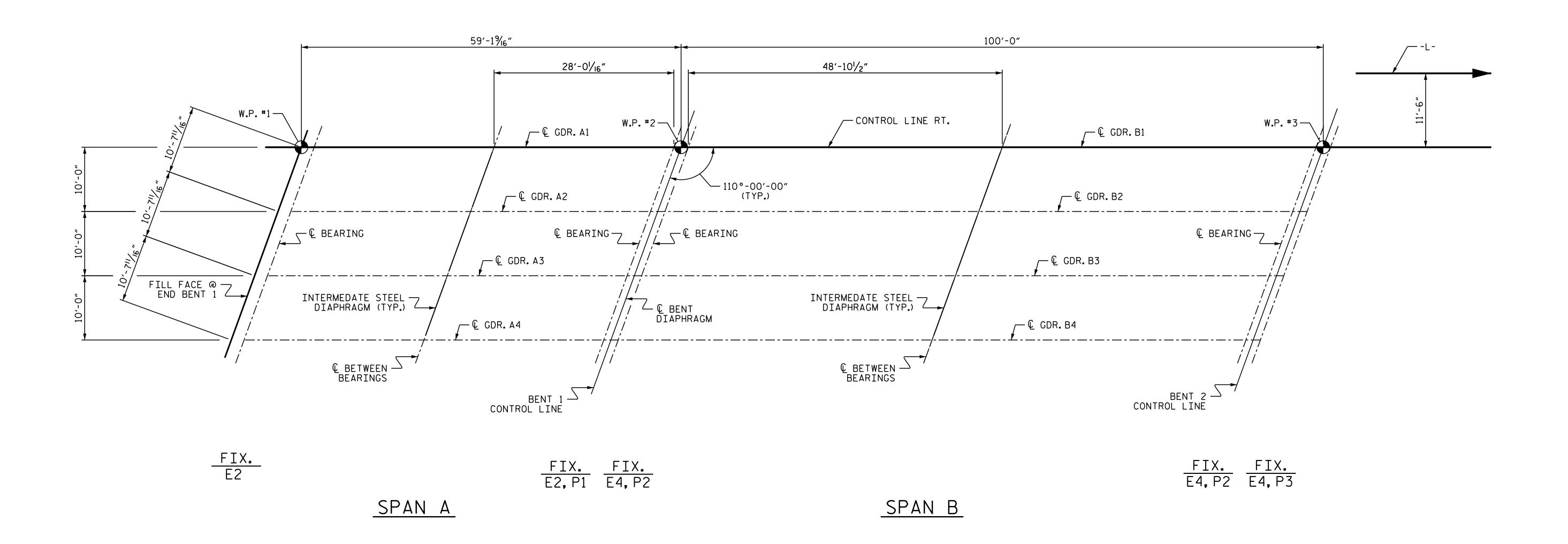
STR. NO. 5

Excellence Delivered **As Promised** NC Lic. No. F-0270

S05-11 DATE: TOTAL SHEETS

COUNTY

SHEET NO.



FRAMING PLAN

PROJECT NO. R-2915B ASHE \_\_\_\_\_ COUNTY STATION: 242+67.42 -L-SHEET 1 OF 2 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH SUPERSTRUCTURE

FRAMING PLAN

NBL

REVISIONS SHEET NO. S05-12 DATE: NO. BY: DATE: NO. BY: TOTAL SHEETS 35

PLANS PREPARED BY:

1121 Situs Court
Suite 170
Raleigh NC 27606-4279
(919) 859-4880

Excellence Delivered As Promised

NC Lic. No. F-0270

DRAWN BY : T.J. KIRSCHBAUM
CHECKED BY : R.F. WERTMAN DRAWN BY: T.J. KIRSCHBAUM

CHECKED BY: R.F. WERTMAN

DATE: 4/28/14

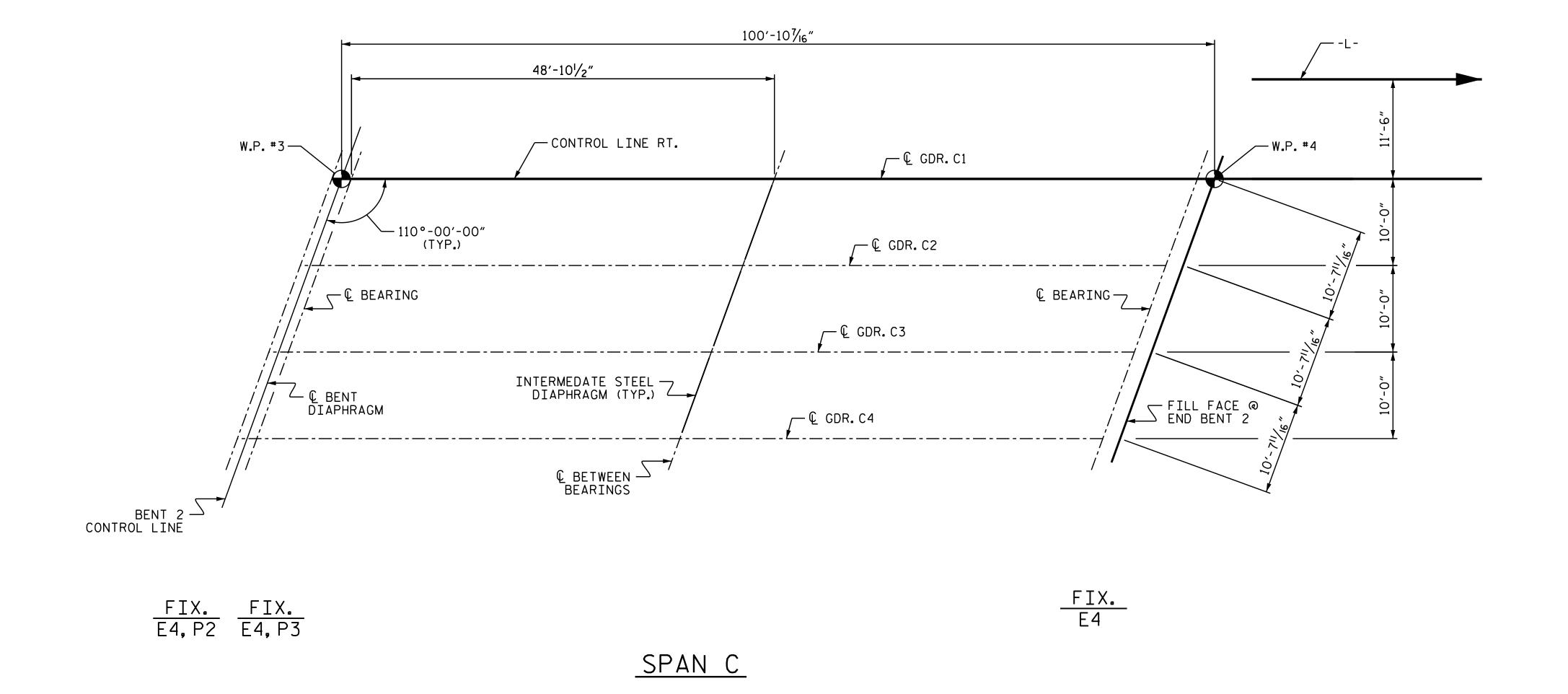
DESIGN ENGINEER OF RECORD: R.F. WERTMAN

DATE: 10/3/14

SEAL 037180

SEAL 037180

SEAL 037180



FRAMING PLAN

PROJECT NO. R-2915B ASHE \_\_\_\_\_ COUNTY STATION: 242+67.42 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH SUPERSTRUCTURE

FRAMING PLAN

NBL

REVISIONS SHEET NO. S05-13 DATE: NO. BY: DATE: NO. BY: TOTAL SHEETS 35

PLANS PREPARED BY:

1121 Situs Court
Suite 170
Raleigh NC 27606-4279
(919) 859-4880

Excellence Delivered As Promised

NC Lic. No. F-0270

2/



SEAL 037180

STR.NO.5

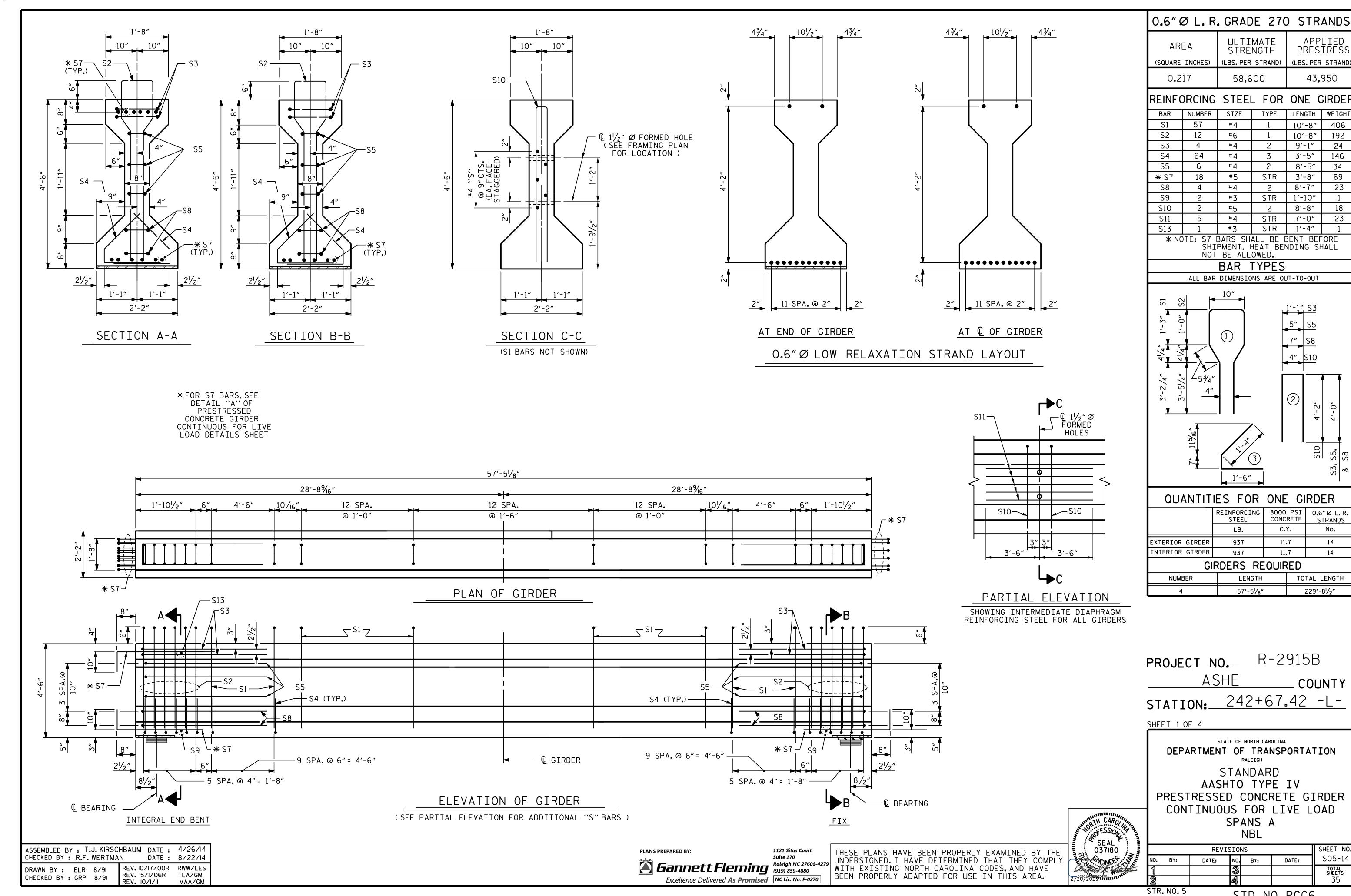
DRAWN BY: T.J. KIRSCHBAUM

CHECKED BY: R.F. WERTMAN

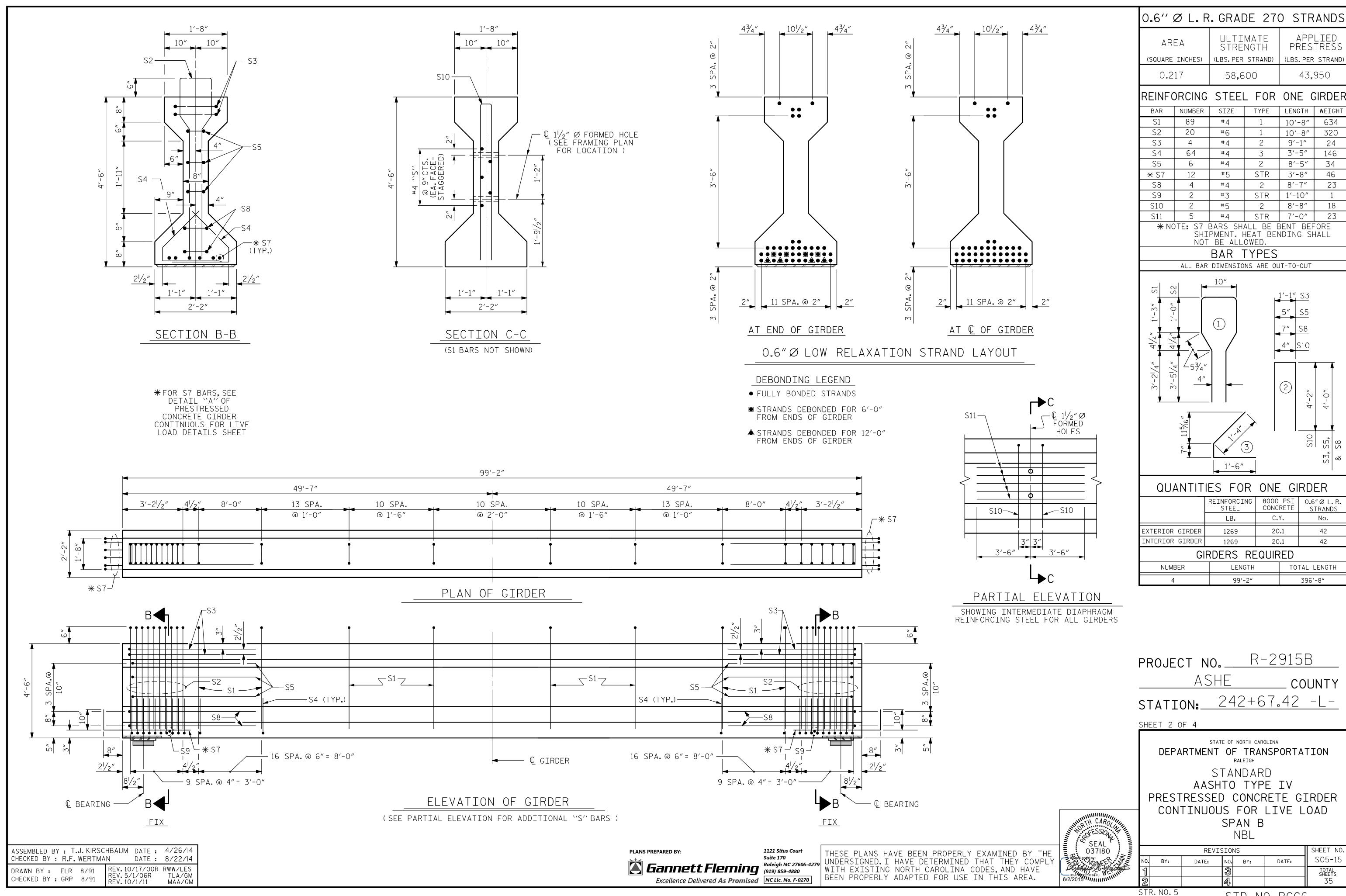
DATE: 4/28/14

DESIGN ENGINEER OF RECORD: R.F. WERTMAN

DATE: 10/3/14



STD. NO. PCG6



SHEET NO.

S05-15

TOTAL SHEETS

PRESTRESS

43,950

23

-5″

4′-

S10

No.

42

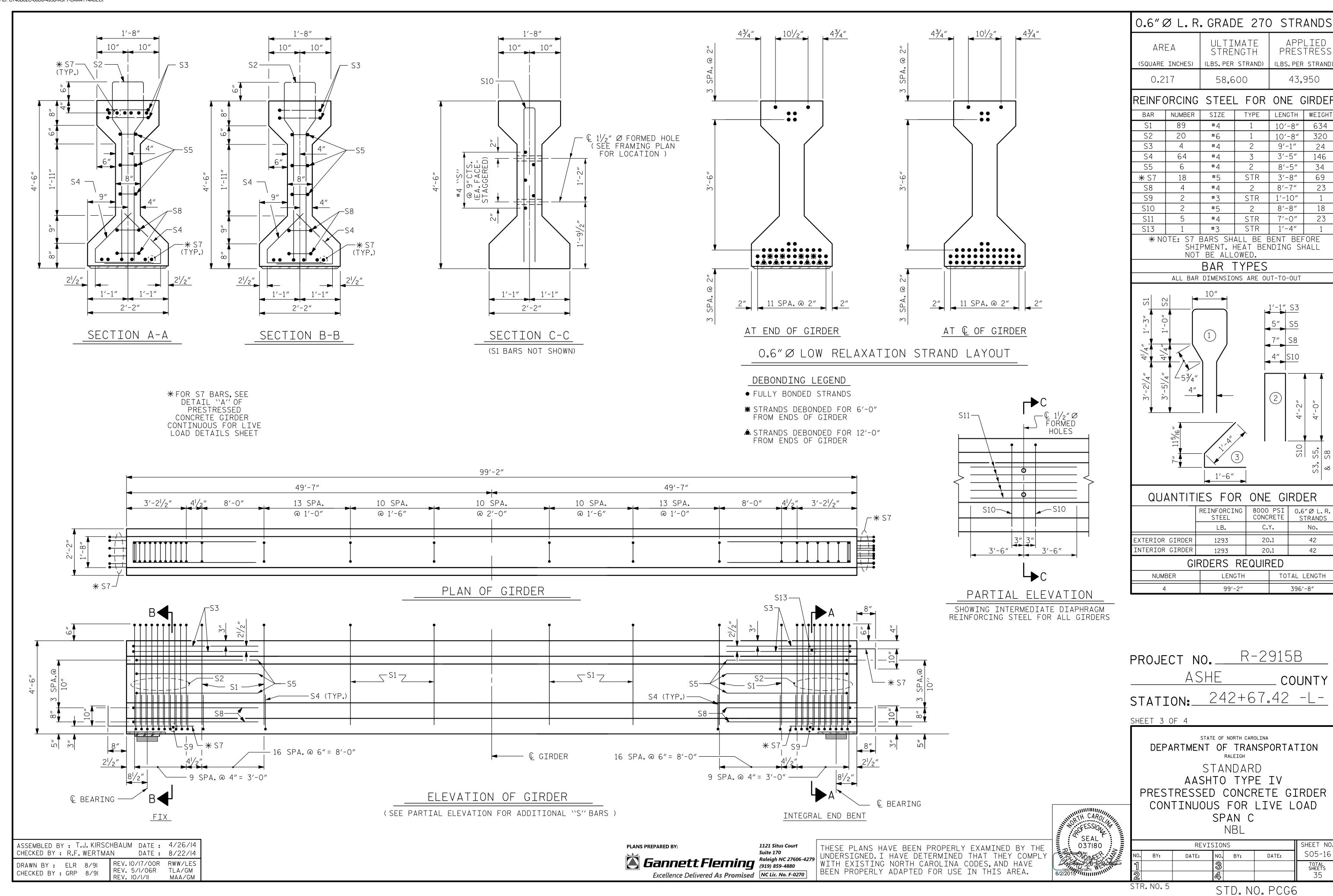
42

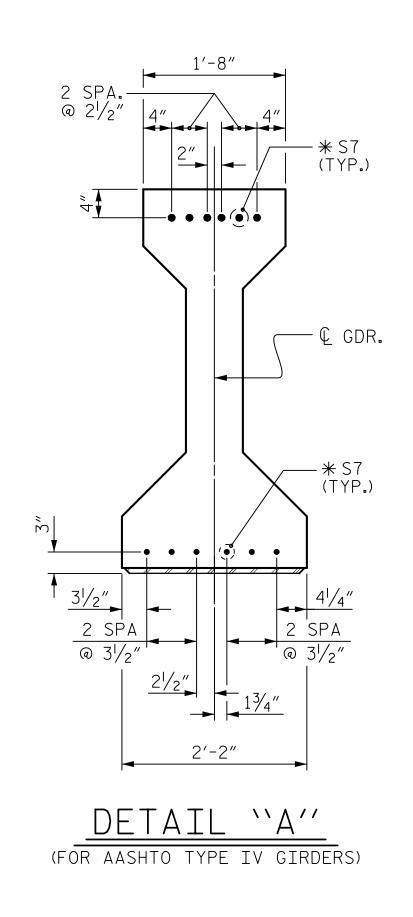
TOTAL LENGTH

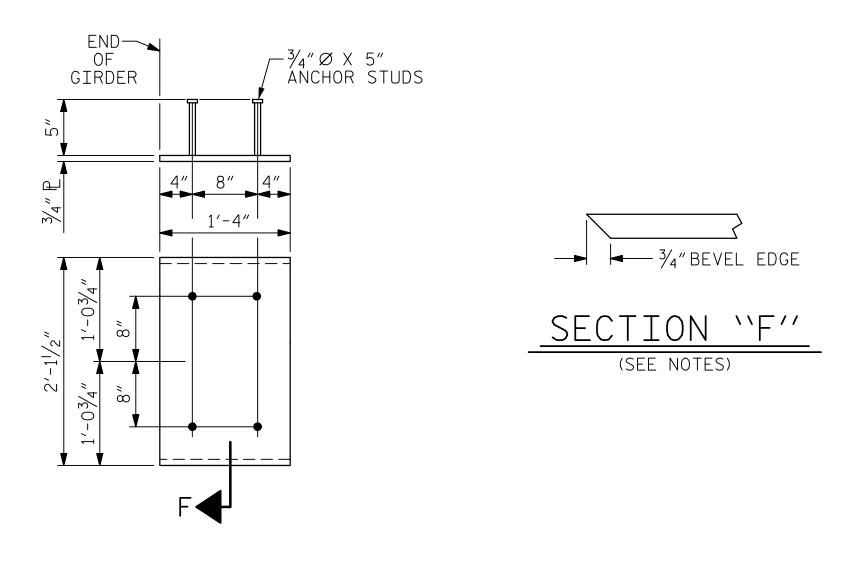
396′-8″

COUNTY

STD. NO. PCG6







#### DEAD LOAD DEFLECTION TABLE FOR GIRDERS SPAN A GIRDERS 1 & 4 GIRDERS 2 & 3 0.010 | 0.019 | 0.027 | 0.031 | 0.033 | 0.031 | 0.027 | 0.019 | 0.010 0.010 | 0.019 | 0.027 | 0.031 | 0.033 | 0.031 | 0.027 | 0.019 | 0.010 0.005 | 0.011 | 0.015 | 0.017 | 0.018 | 0.017 | 0.015 | 0.011 | 0.005

1/16"

1/16"

EMBEDDED PLATE "B-1" DETAILS

FOR AASHTO TYPE IV GIRDER

(2 REQ'D PER GIRDER)

\* INCLUDES FUTURE WEARING SURFACE.

O.6"∅ LOW RELAXATION

TENTH POINTS

\* DEFLECTION DUE TO SUPERIMPOSED D.L.

CAMBER (GIRDER ALONE IN PLACE)

FINAL CAMBER

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

1/8"

1/16"

3/<sub>16</sub>"

1/8"

3/<sub>16</sub>"

3/16"

1/8"

1/8"

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																						
										SP	ANS	В &	С									
O.6″∅ LOW RELAXATION				(	GIRD	ERS	1 & 4	4							(	GIRDE	ERS 2	2 &	3			
TENTH POINTS	0	.1	.2	.3	.4	<b>.</b> 5	.6	.7	.8	.9	0	0	.1	.2	.3	.4	<b>.</b> 5	.6	.7	.8	.9	0
CAMBER (GIRDER ALONE IN PLACE)	0	0.062	0.118	0.162	0.189	0.199	0.189	0.162	0.118	0.062	0	0	0.062	0.118	0.162	0.189	0.199	0.189	0.162	0.118	0.062	0
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0	0.050	0.098	0.135	0.159	0.167	0.159	0.135	0.098	0.050	0	0	0.055	0.108	0.149	0.175	0.184	0.175	0.149	0.108	0.055	0
FINAL CAMBER	0	1/8"	1/4"	5/16"	3/8"	3/8″	3/8"	5/16"	1/4"	1/8"	0	0	1/16"	1/8"	3/16"	3/16"	3/16"	3/16"	3/16"	1/8"	1/16"	0

\* INCLUDES FUTURE WEARING SURFACE.

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT "FINAL CAMBER", WHICH IS GIVEN IN INCHES (FRACTION FORM).

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 4/26/14 CHECKED BY : R.F. WERTMAN DATE: 8/22/14 REV. 5/1/06 TLA/GM REV. 10/1/11 MAA/GM REV. 1/15 MAA/TMG DRAWN BY: ELR 11/91 CHECKED BY : GRP 11/91

Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered As Promised NC Lic. No. F-0270

1121 Situs Court

0.006 | 0.012 | 0.016 | 0.019 | 0.020 | 0.019 | 0.016 | 0.012 | 0.006

1/8"

1/8"

1/8"

1/16"

|/<sub>16"</sub>

1/8"

1/8"

SEAL 037180
Docuseigned by: THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6400 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

> PROJECT NO. R-2915B COUNTY STATION: 242+67.42 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

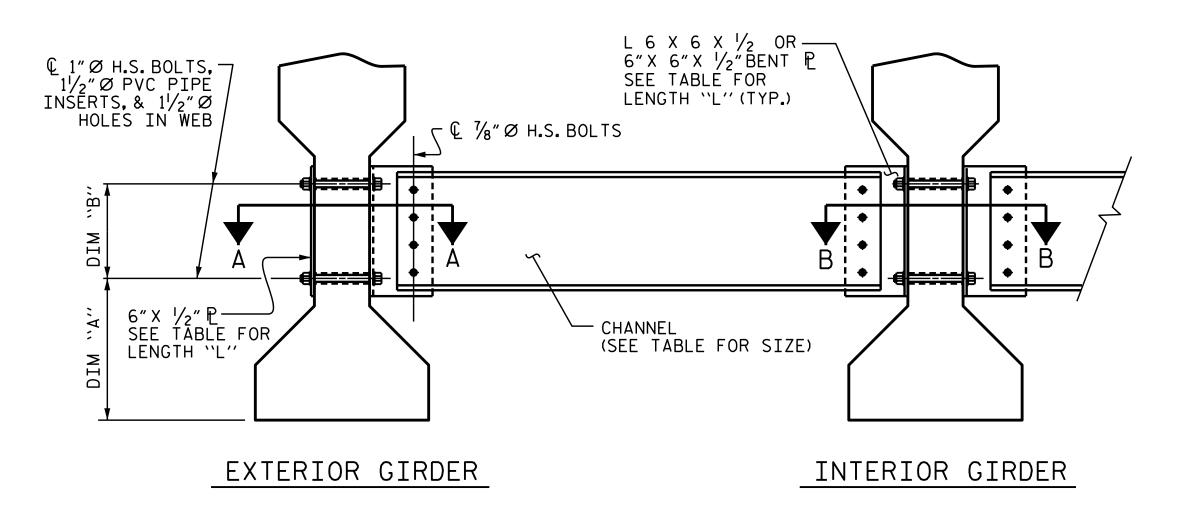
STANDARD

PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS

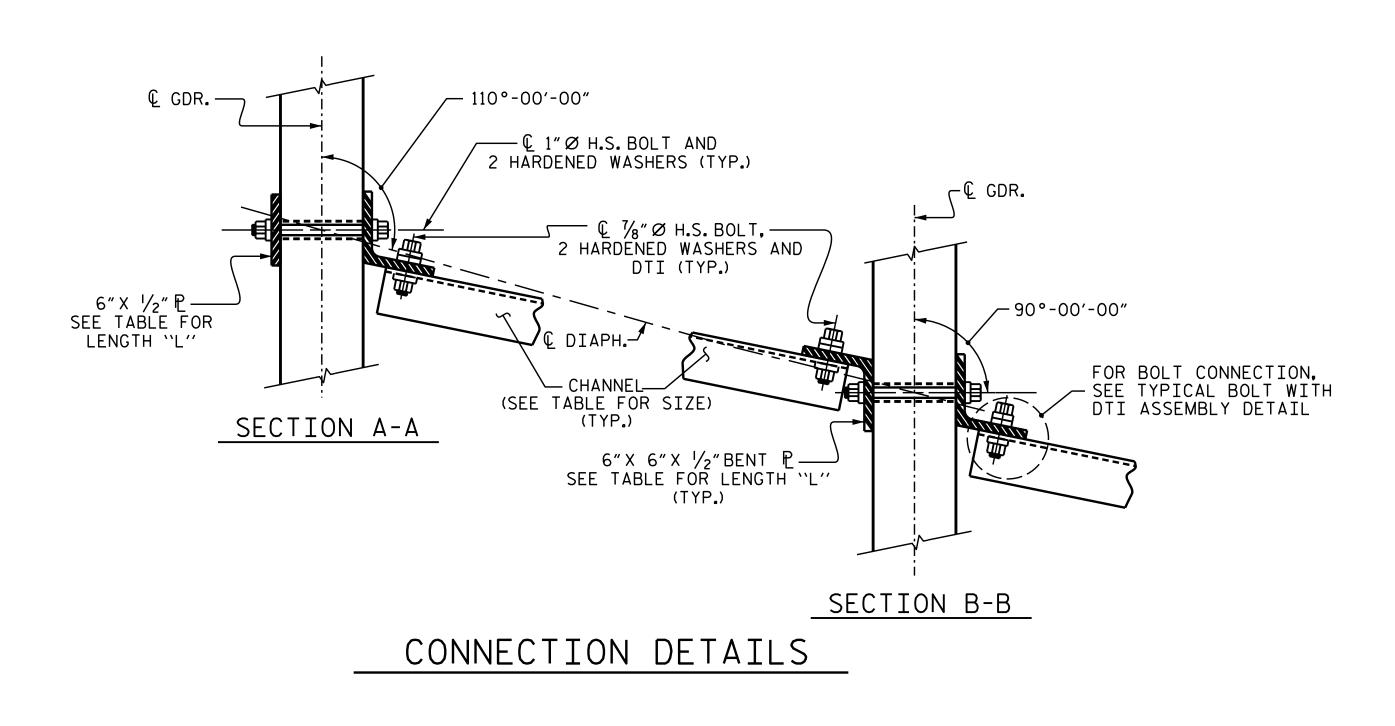
REVISIONS SHEET NO. S05-17 NO. BY: DATE: BY: DATE: SHEETS

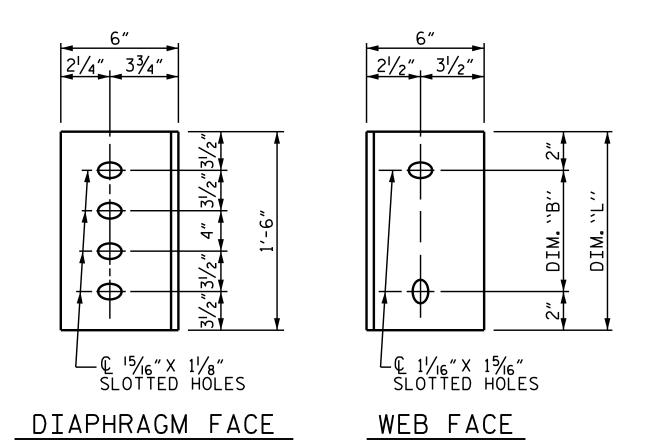
STR. NO. 5

STD. NO. PCG9



PART SECTION AT INTERMEDIATE DIAPHRAGM





#### CONNECTOR PLATE DETAILS

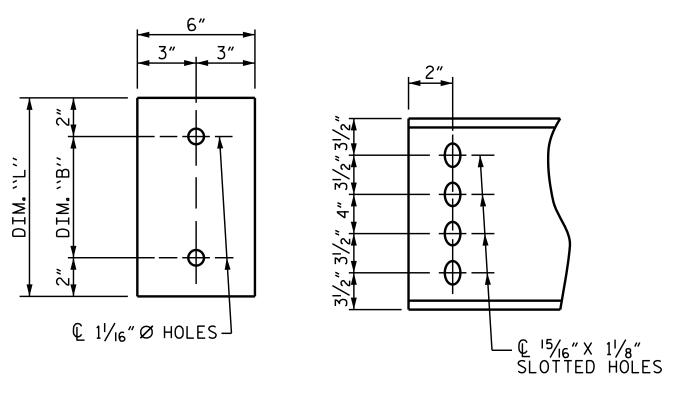
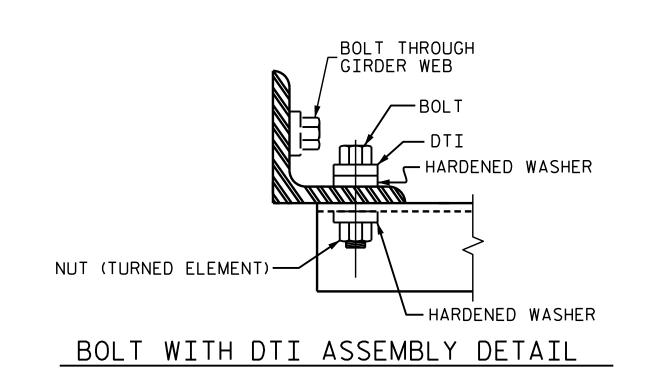


PLATE DETAILS CHANNEL END



STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL  $\frac{1}{4}$  TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST  $\frac{1}{4}$ " PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

#### TABLE

GIRDER TYPE	CHANNEL SIZE	DIM "A"	DIM "B"	DIM "L"
IV	MC 18 × 42.7	1'-91/2"	1'-2"	1'-6"

R-2915B PROJECT NO. \_\_ ASHE COUNTY STATION: 242+67.42 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS

NBL

SHEET NO **REVISIONS** S05-18 DATE: DATE: NO. BY:

STD. NO. PCG10

**PLANS PREPARED BY:** 



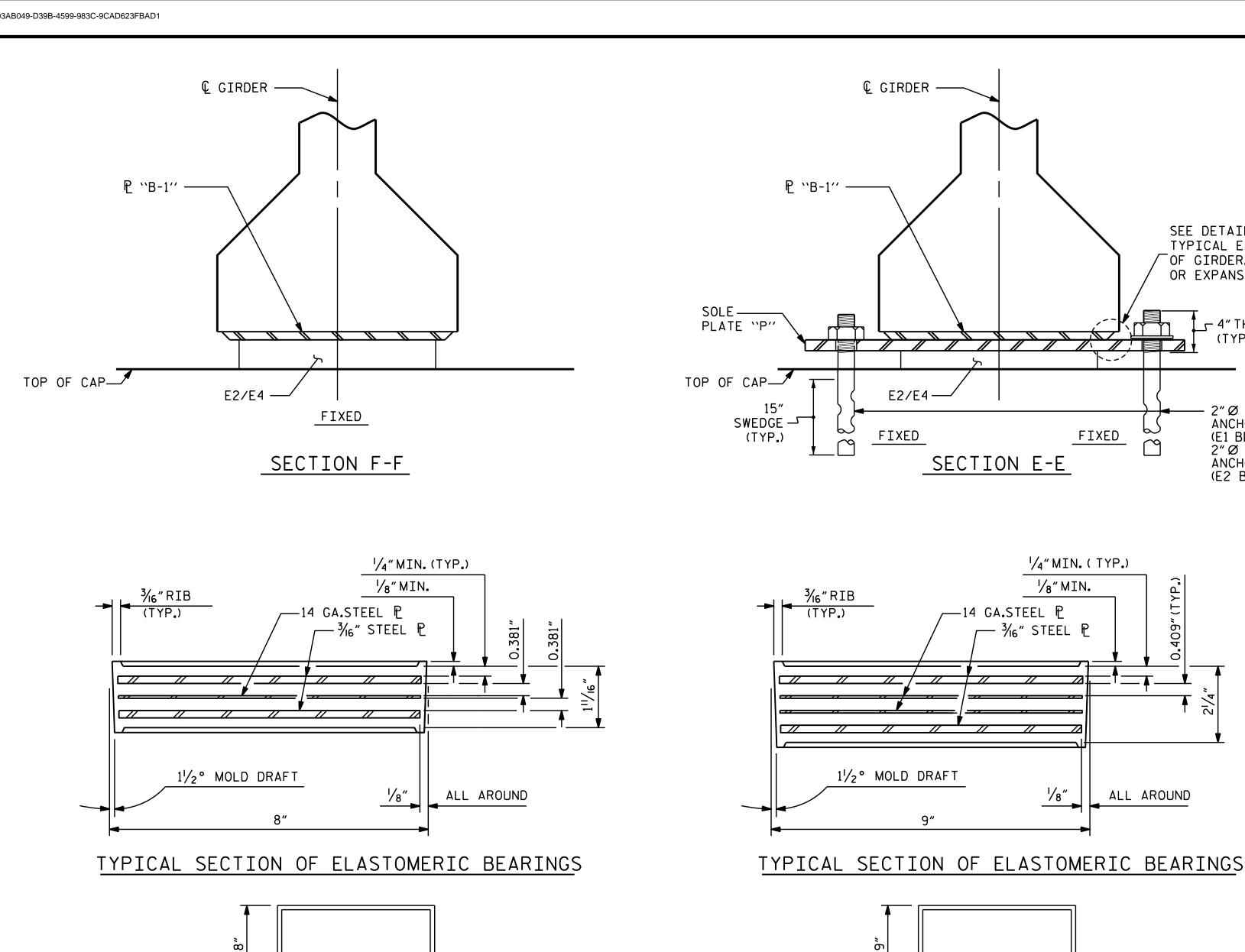
1121 Situs Court

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

SEAL \* 037180

STR. NO. 5

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 4/26/14 CHECKED BY : R.F. WERTMAN DATE: 8/22/14 ADDED 10/21/05 REV. 5/1/06RRR KMM/GM REV. 10/1/11 MAA/GM DRAWN BY: TLA 6/05 CHECKED BY : VC 6/05



1'-6"

E2 (8 REQ'D)

PLAN VIEW OF ELASTOMERIC BEARING

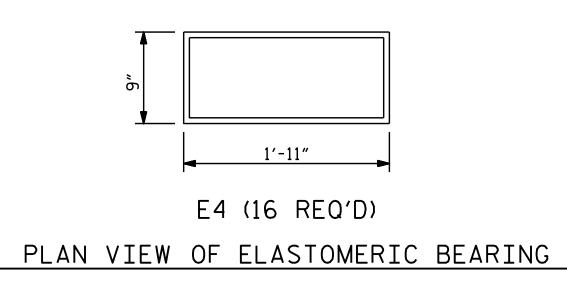
ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 4/28/14

DATE: 8/22/14

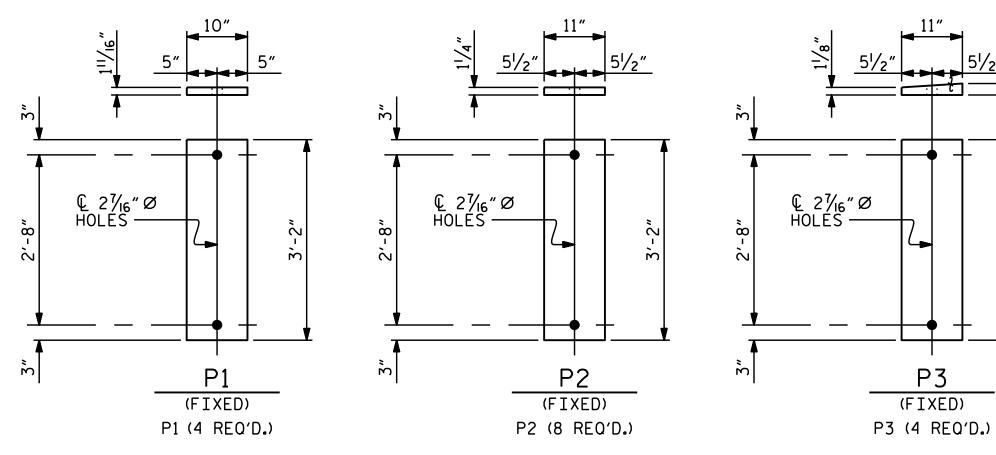
MAA/GM AAC/MAA MAA/TMG

CHECKED BY : R.F. WERTMAN

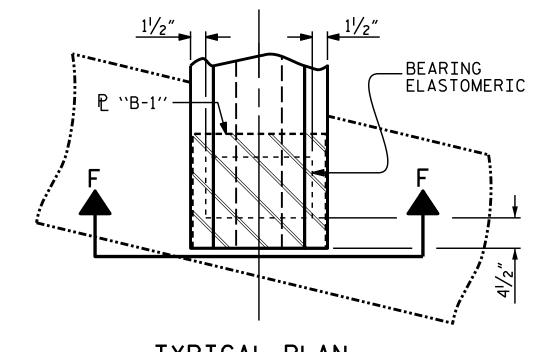
DRAWN BY: EEM 2/97 REV. 10/1/11 REV. 6/13 REV. 1/15



#### TYPE III TYPE V



SOLE PLATE DETAILS ( "P")



SEE DETAIL "A" TYPICAL EACH SIDE

OF GIRDER, FIXED OR EXPANSION END.

← 4" THREAD

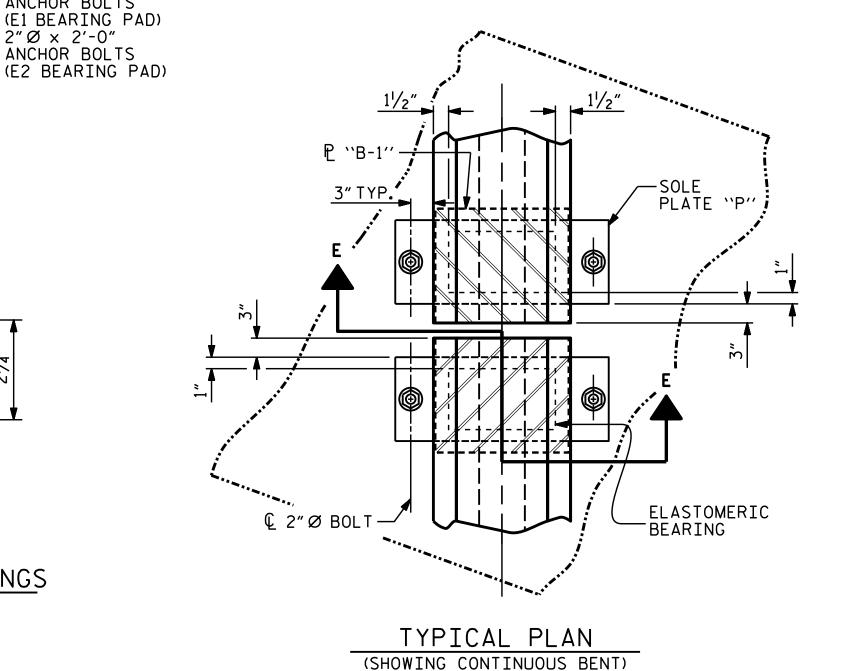
 $2'' \varnothing \times 1'-11 \frac{1}{2}''$ ANCHOR BOLTS

2"Ø × 2'-0"

ANCHOR BOLTS

(E1 BEARING PAD)

TYPICAL PLAN (SHOWING INTEGRAL END BENT)



NOTES

AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

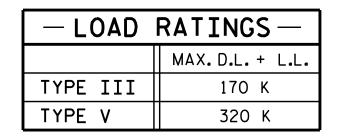
SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

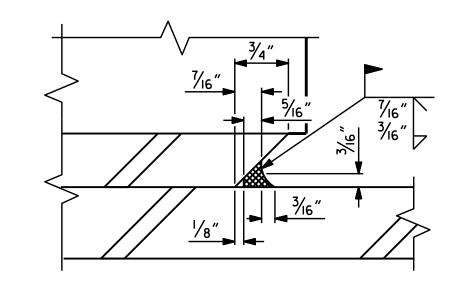
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.





DETAIL "A"

PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

ELASTOMERIC BEARING 

PRESTRESSED CONCRETE GIRDER SUPERSTRUCTURE NBL

SHEET NO. REVISIONS S05-19 NO. BY: DATE: DATE:

Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered As Promised NC Lic. No. F-0270 THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

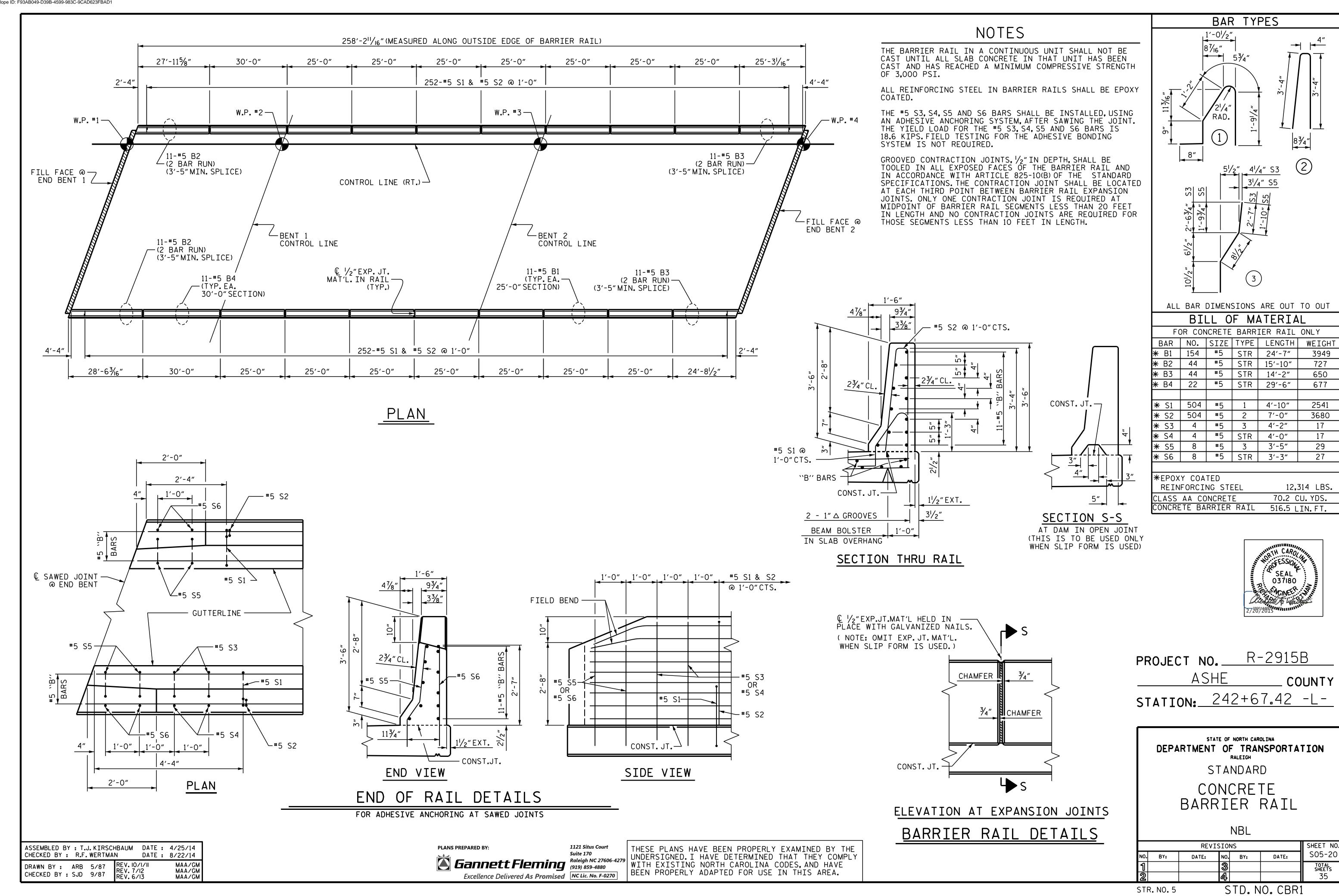
**PLANS PREPARED BY:** 

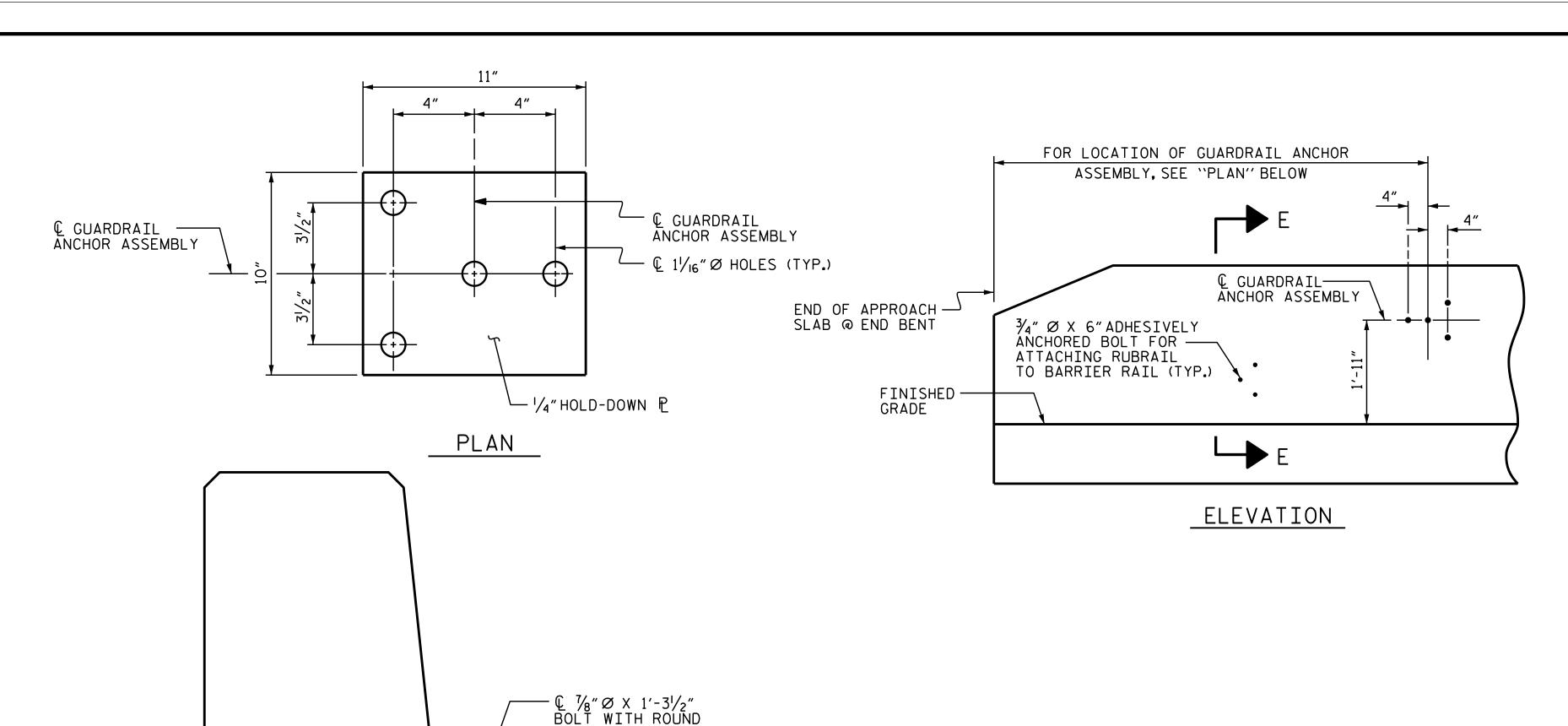
1121 Situs Court

SEAL 037180

STR. NO. 5

STD. NO. EB3 STD. NO. EB4





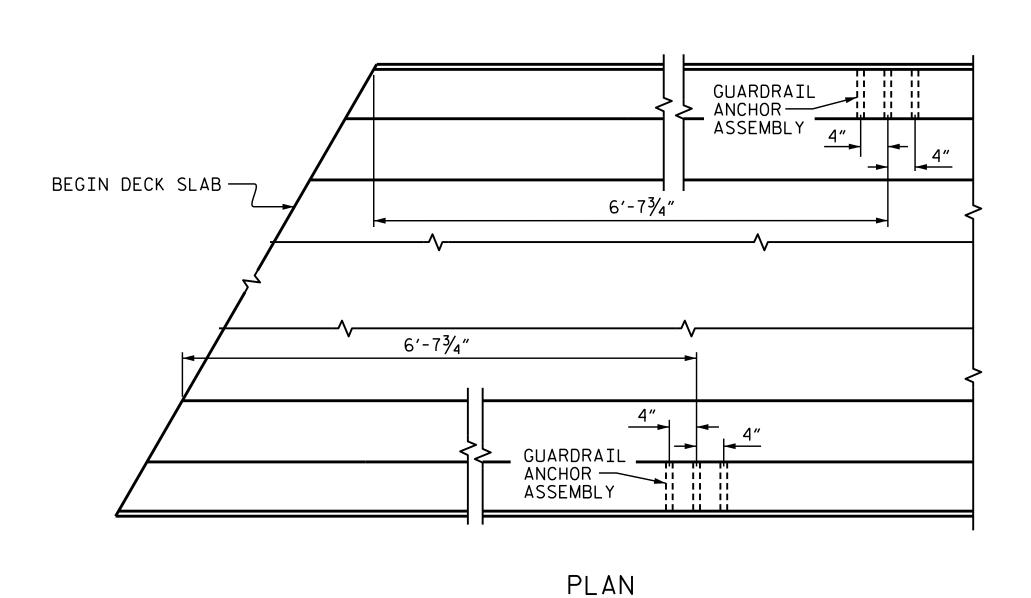
WASHERS (TYP.)

CGUARDRAIL

-C6 X 8.2 RUBRAIL

— FINISHED GRADE

ANCHOR ASSEMBLY



#### LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

#### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ÁDHESIVELY ANCHORED BOLTS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $\frac{7}{8}$ "  $\varnothing$  GALVANIZED BOLTS. NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

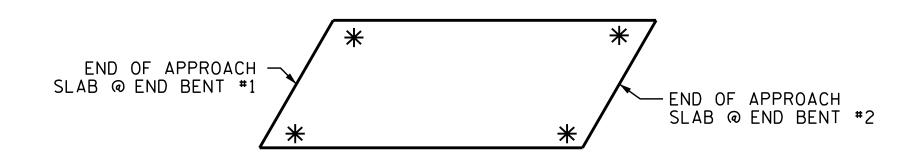
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE  $1^{1}/4^{\prime\prime}$  Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE 3/4" Ø X 6"BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



#### SKETCH SHOWING POINTS OF ATTACHMENTS \* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > STANDARD

GUARDRAIL ANCHORAGE FOR BARRIER RAIL

NBL

SHEET NO. REVISIONS S05-21 BY: DATE: NO. BY:

**PLANS PREPARED BY:** 

Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered As Promised NC Lic. No. F-0270

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

ASSEMBLED BY : T.J. KIRSCHBAUM DATE : 4/24/14 DATE: 8/16/14 CHECKED BY : R.F. WERTMAN MAA/GM MAA/GM MAA/GM DRAWN BY: TLA 5/06 CHECKED BY : GM 5/06

SECTION E-E

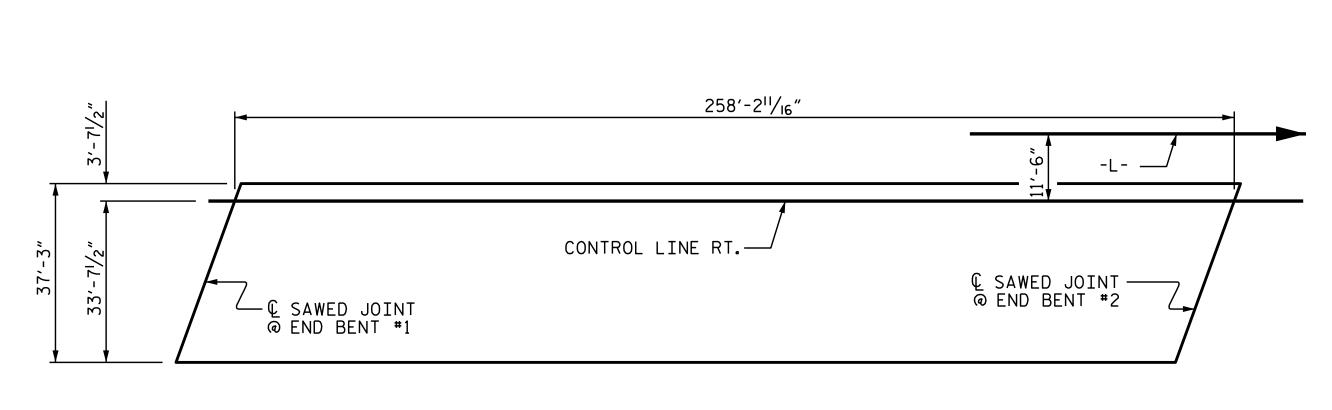
GUARDRAIL ANCHOR ASSEMBLY DETAILS

1/4" HOLD-DOWN ₽—

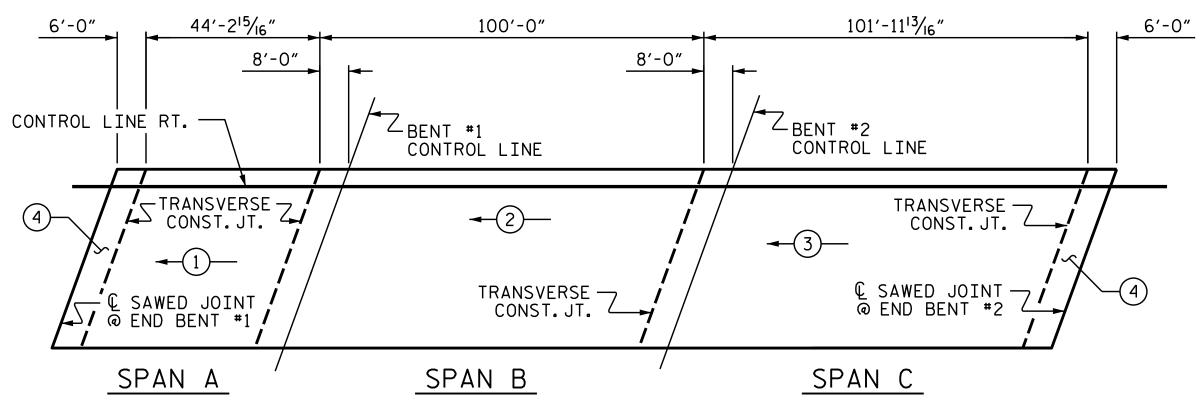
11/4" Ø DRILLED OR FORMED HOLE (TYP.)

ADHESIVELY ANCHORED — 3/4" Ø X 6"BOLTS FOR ATTACHING RUBRAIL TO BARRIER RAIL (TYP.)

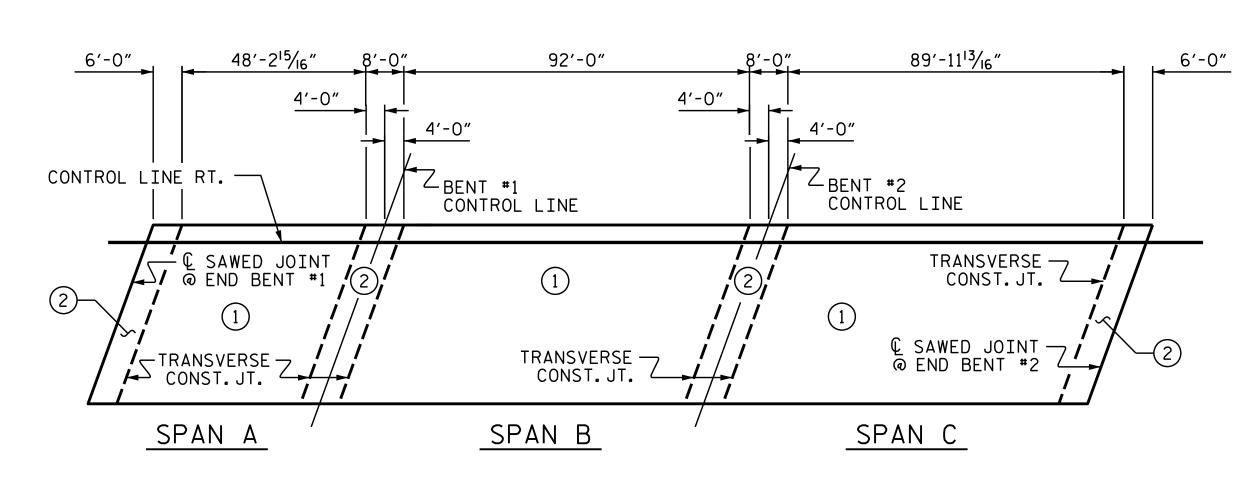
SEE ROADWAY STD. 862.03



## LAYOUT FOR COMPUTING AREA REINFORCED CONCRETE DECK SLAB (SQ.FT. = 9619)

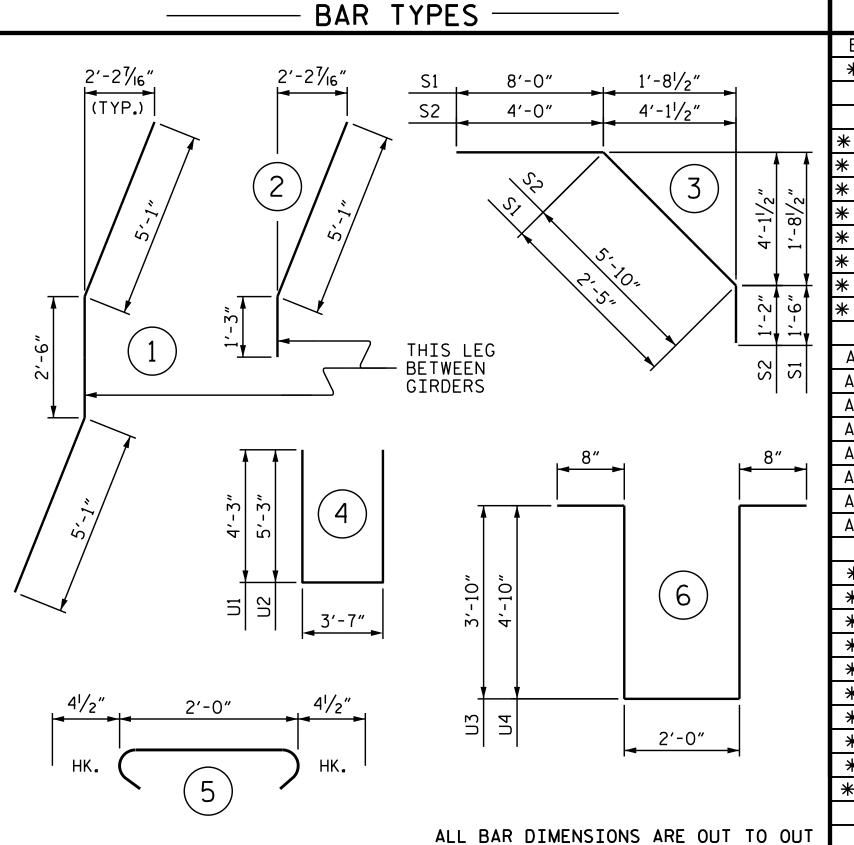


#### POURING SEQUENCE



#### OPTIONAL POURING SEQUENCE

POUR ② CAN NOT BE STARTED UNTIL BOTH ADJACENT ① POURS REACH A MINIMUM OF 3000 PSI



	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGH
	* A1	489	<b>#</b> 5	STR	36′-11″	18829	H1	8	#5	STR	9'-7"	80
	Α2	489	#5	STR	36′-11″	18829	H2	8	#5	STR	9'-4"	78
							Н3	8	#5	STR	9'-11"	83
	<b>*</b> A101	6	<b>#</b> 5	STR	33'-4"	209	Н4	8	<b>#</b> 5	STR	9'-8"	81
1	<b>*</b> A102	6	<b>#</b> 5	STR	29'-3"	183						
<u> </u>	<b>*</b> A103	6	<b>#</b> 5	STR	25′-1″	157	K1	20	#4	STR	23'-9"	317
8/2	<b>*</b> A104	6	<b>#</b> 5	STR	21'-0"	131	K2	6	#4	STR	7'-11"	32
-	<b>*</b> A105	6	<b>#</b> 5	STR	16′-10″	105	К3	18	#4	STR	9'-2"	110
	<b>*</b> A106	6	<b>#</b> 5	STR	12'-9"	80	K4	36	#4	STR	9'-7"	230
$\frac{1}{2}$	<b>*</b> A107	6	<b>#</b> 5	STR	8'-7"	54	K5	18	#4	STR	8'-6"	102
ြတို	<b>*</b> A108	6	<b>#</b> 5	STR	4'-6"	28	К6	4	#4	STR	5′-5″	14
<b>-</b> -↓							K7	4	#4	STR	6'-1"	16
	A201	6	<b>#</b> 5	STR	33'-4"	209	K8	8	#4	STR	6'-4"	34
S1	A202	6	<b>#</b> 5	STR	29'-3"	183	К9	4	#4	STR	5′-9″	15
·	A203	6	<b>#</b> 5	STR	25′-1″	157	K10	8	#4	STR	3′-10″	20
	A204	6	<b>#</b> 5	STR	21'-0"	131	K11	24	#4	STR	2'-10"	45
	A205	6	<b>#</b> 5	STR	16′-10″	105	K12	12	#4	STR	7′-4″	59
<b>-</b>	A206	6	<b>#</b> 5	STR	12'-9"	80	K13	20	#4	2	6'-4"	85
_	A207	6	<b>#</b> 5	STR	8'-7"	54	K14	20	#4	1	12'-8"	169
	A208	6	<b>#</b> 5	STR	4'-6"	28						
							<b>*</b> S1	60	#4	3	11'-11"	478
	<b>∗</b> B1	49	#7	STR	12'-0"	1202	<b>*</b> S2	60	#4	3	11'-0"	441
	<b>∗</b> B2	25	#4	STR	27'-9"	463	S3	192	#4	5	2'-9"	353
	<b>∗</b> B3	25	#7	STR	57'-6"	2938						
	<b>∗</b> B4	24	#7	STR	24'-0"	1177	U1	60	#4	4	12'-1"	484
	<b>∗</b> B5	50	#4	STR	18'-0"	601	U2	12	#4	4	14'-1"	113
	<b>∗</b> B6	25	#7	STR	25'-0"	1278	U3	12	#4	6	11'-0"	88
	<b>∗</b> B7	25	#7	STR	50'-3"	2568	U4	36	#4	6	13'-0"	313
	<b>∗</b> B8	24	<b>#</b> 7	STR	30'-0"	1472						
	<b>∗</b> B9	50	#4	STR	25′-6″	852						
	<b>∗</b> B10	49	#7	STR	20'-0"	2003						
	B11	220	<b>#</b> 5	STR	53'-3"	12219						
UT												

BILL OF MATERIAL

REINFORCING STEEL = 34,916 \* EPOXY COATED REINF. STEEL = 35,249

SUPI	ERSTRUCT	URE BILL OF	MATERIAL
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU. YDS.)	(LBS.)	(LBS.)
POUR #1	49.1		
POUR #2	122.9		
POUR #3	125.1		
POUR #4	80.2		
TOTALS **	377.3	34,916	35,249
** QUANTI	TIES FOR BAR	RIER RAIL ARE NOT I	NCLUDED

APPROACH SLABS

BRIDGE DECK

TOTAL

© TRANSVERSE CONST. JT.
3" TOP OF SLAB
3"-11
3" 3/4" (TYP.)

TRANSVERSE CONSTRUCTION JOINT DETAIL

NOTE: REINFORCING STEEL IN SLAB NOT SHOWN. LONGITUDINAL REINFORCING STEEL SHALL BE CONTINUOUS THRU JOINT

SUP	ERSTRU	CTURE	REI	NFOF	RCIN	IG S	TEEL
	LENGTI	HS ARE	E BA	SED	ON	THE	
FOLL	OWING	MININ	MUN	SPL	ICE	LEN	GTHS
	CLIDEDCEDI	ICTUDE					

BAR SIZE	SUPERSTF EXCEPT A SLABS, P AND BARR	APPROACH ARAPET,	APPROAC	H SLABS	PARAPET AND BARRIER
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL
#4	2'-0"	1'-9"	2′-0″	1'-9"	2'-9"
#5	2′-6″	2'-2"	2′-6″	2'-2"	3′-5″
#6	3′-0″	2'-7"	3′-10″	2'-7"	4'-4"
#7	5′-3″	3′-6″			
#8	6'-10" 4'-7"				

PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE

BILL OF MATERIAL

NBL

SEAL \* 037180 Dografiand W. WELL.

Gannett Fleming
Raleigh NC 27606-4279
(919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

1121 Situs Court Suite 170

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

GROOVING BRIDGE FLOORS

1550 **SQ.FT.** 

8005 **SQ.FT.** 

9555 **SO.FT.** 

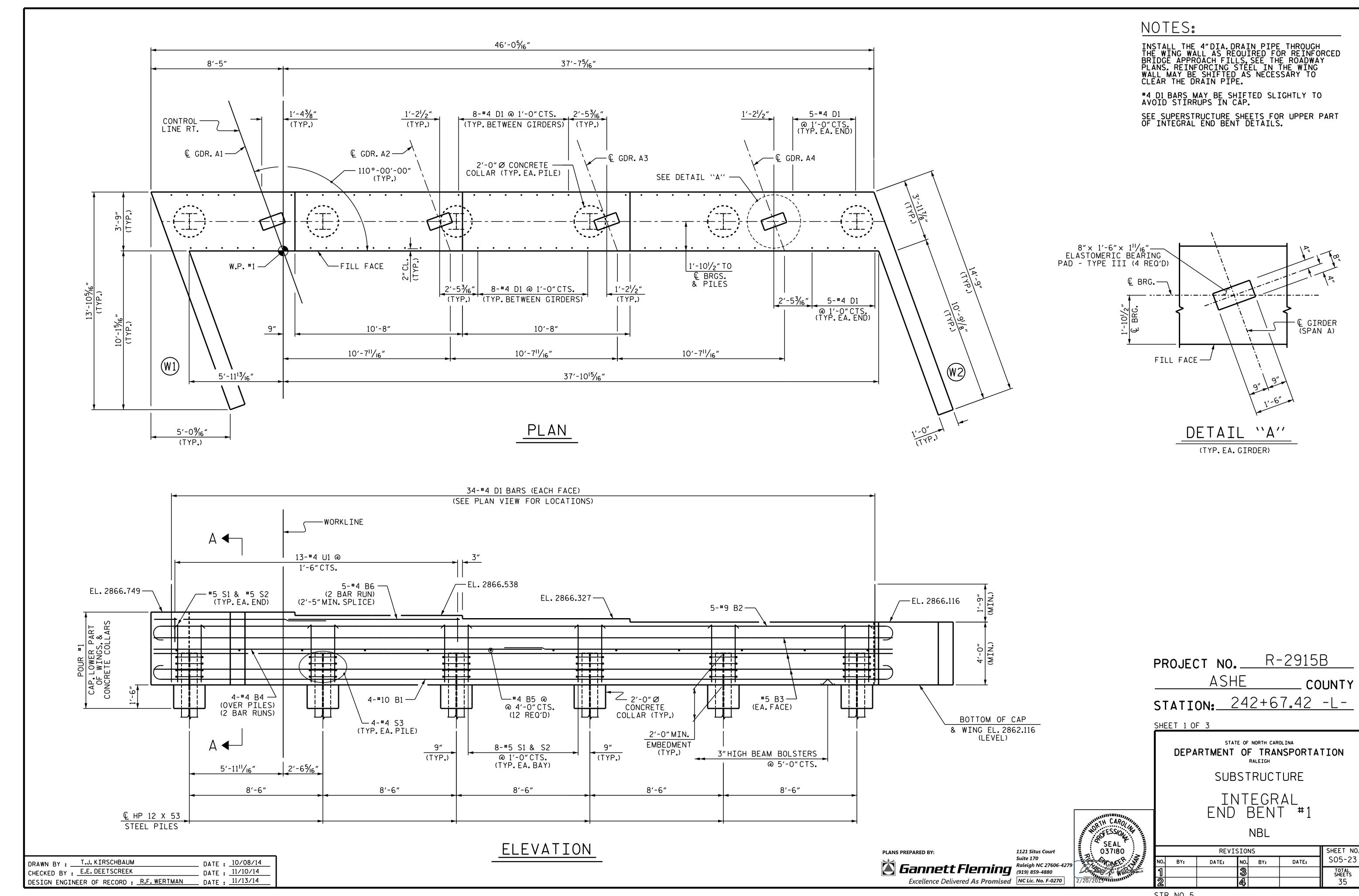
SHEET NO. REVISIONS S05-22 NO. BY: DATE: DATE: BY: TOTAL SHEETS 35

STD. NO. BOM1 STR. NO. 5

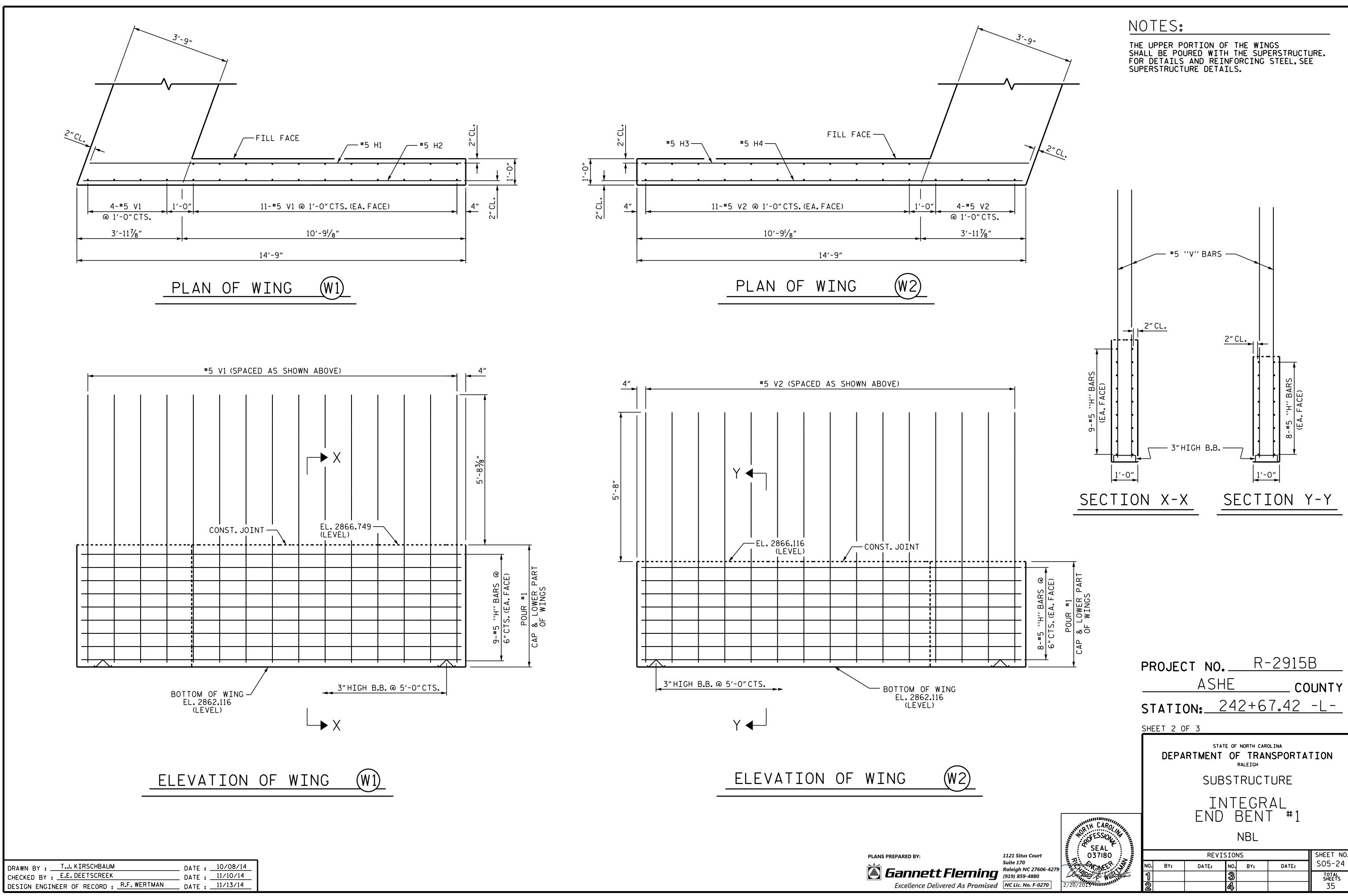
ASSEMBLED BY : T.J. KIRSCHBUAM DATE : 5/1/14 CHECKED BY : R.F. WERTMAN DATE: 8/22/14 REV. 8/16/99 REV. 5/1/06 REV. 10/1/11 RWW/LES TLA/GM MAA/GM DRAWN BY: JMB 5/87 CHECKED BY : SJD 9/87

+

+



+

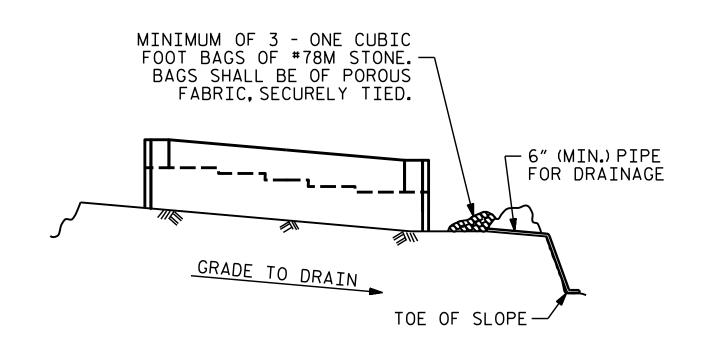


+

DRAWN BY : T.J. KIRSCHBAUM

CHECKED BY : E.E. DEETSCREEK

DESIGN ENGINEER OF RECORD : R.F. WERTMAN



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

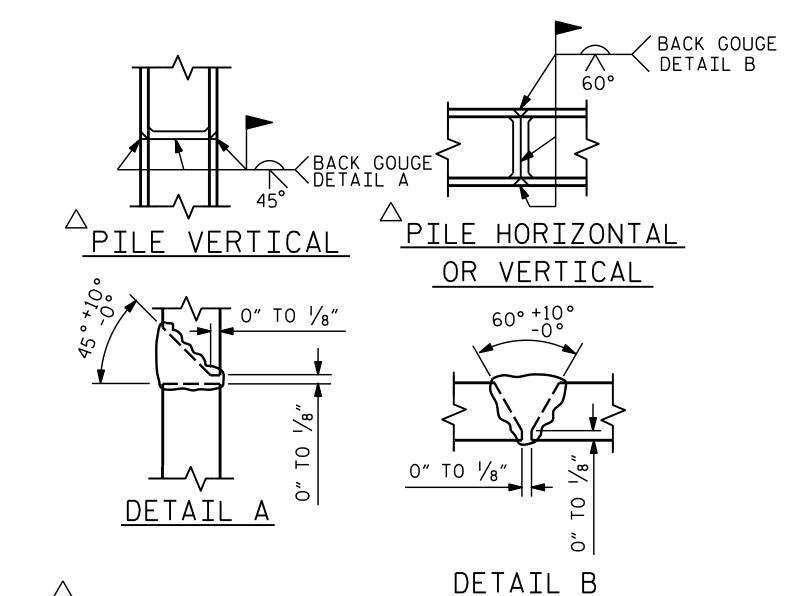
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

#### TEMPORARY DRAINAGE AT END BENT

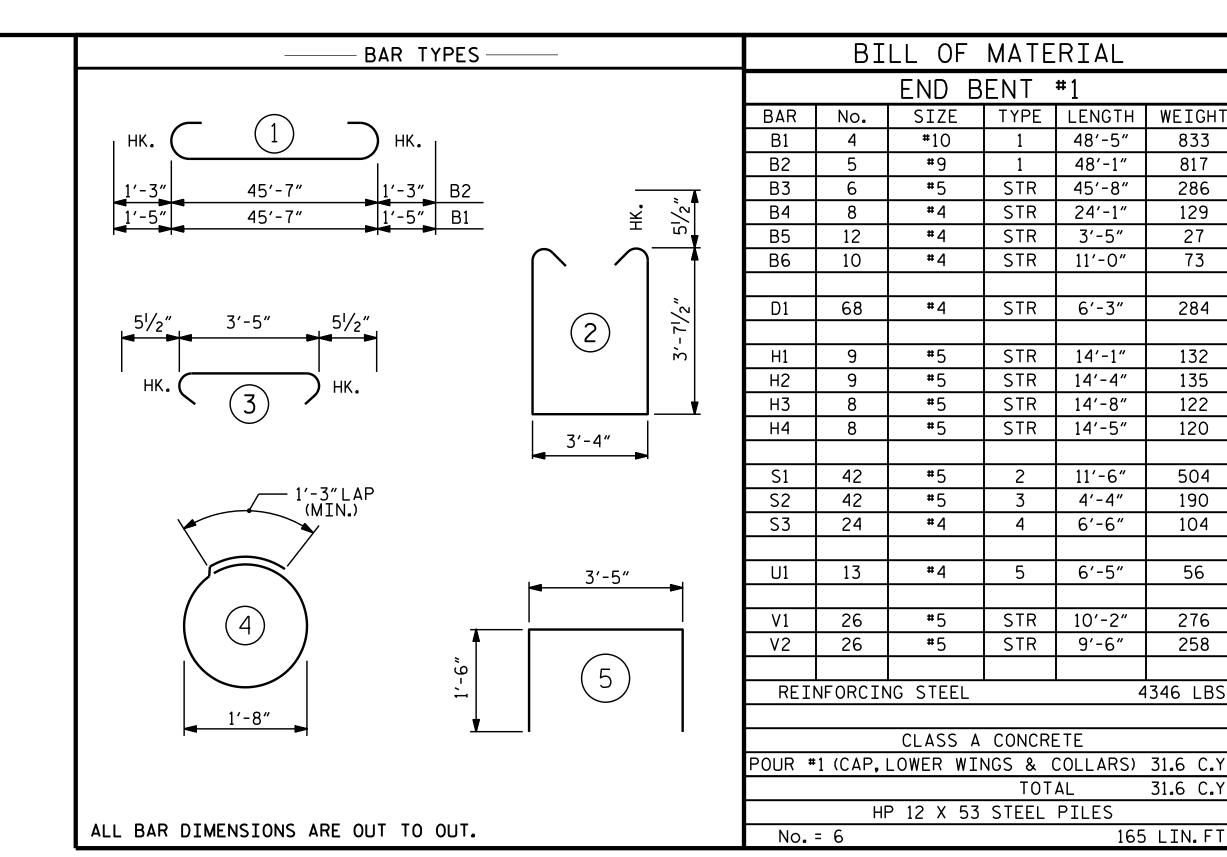
DATE : 10/08/14

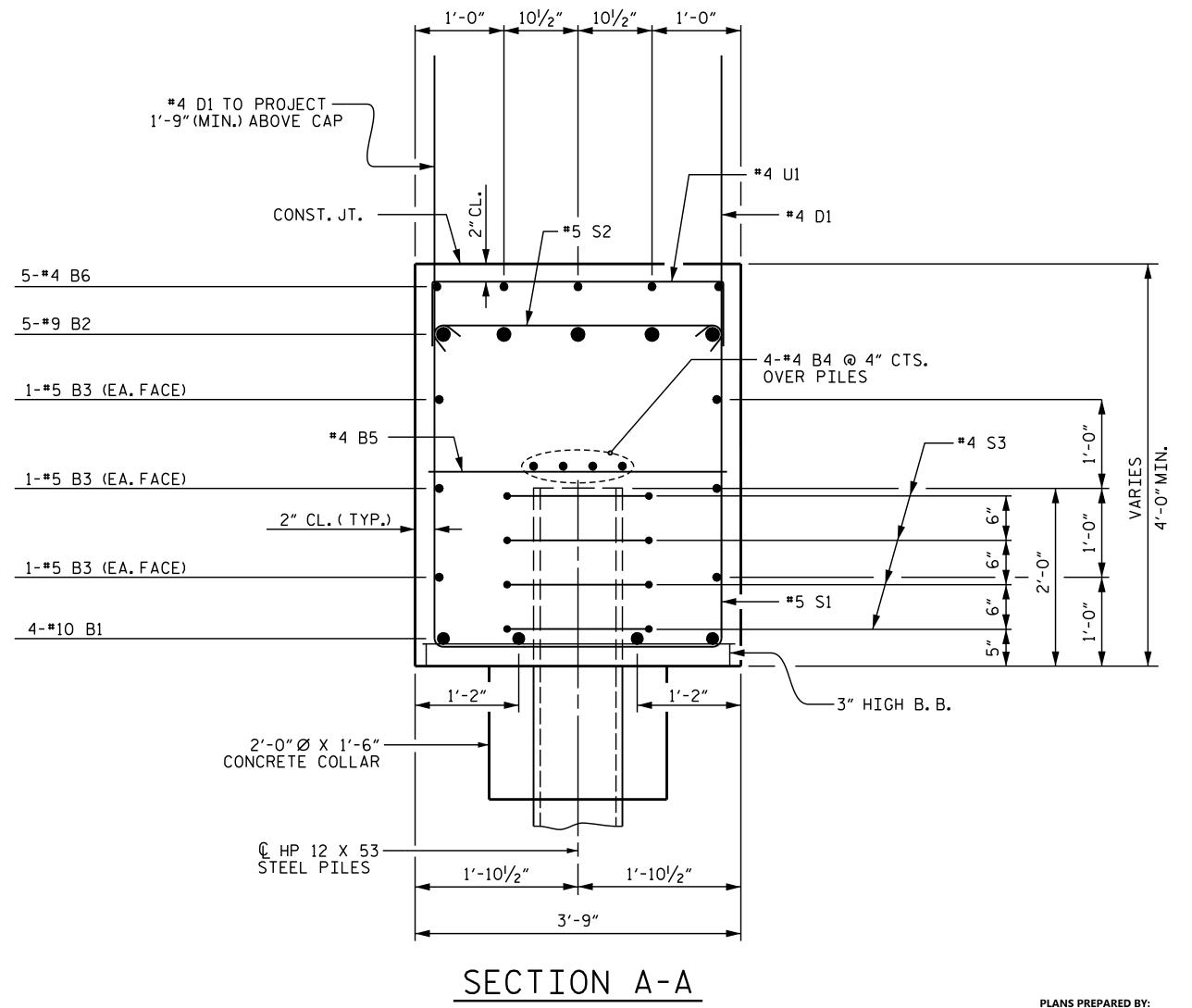
\_ DATE : 11/10/14

\_ DATE : 11/13/14



POSITION OF PILE DURING WELDING. PILE SPLICE DETAILS





1121 Situs Court Suite 170
Raleigh NC 27606-4279
(919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

SEAL \* 037180

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

PROJECT NO. R-2915B

STATION: 242+67.42 -L-

ASHE

833

817

286

129

27

73

284

132

135

122

120

504

190

104

56

276

258

4346 LBS

31.6 C.Y.

165 LIN.FT

COUNTY

INTEGRAL END BENT #1

NBL

SHEET NO. REVISIONS S05-25 NO. BY: DATE: DATE: BY: TOTAL SHEETS

SHEET 3 OF 3

+

DRAWN BY : T.J. KIRSCHBAUM

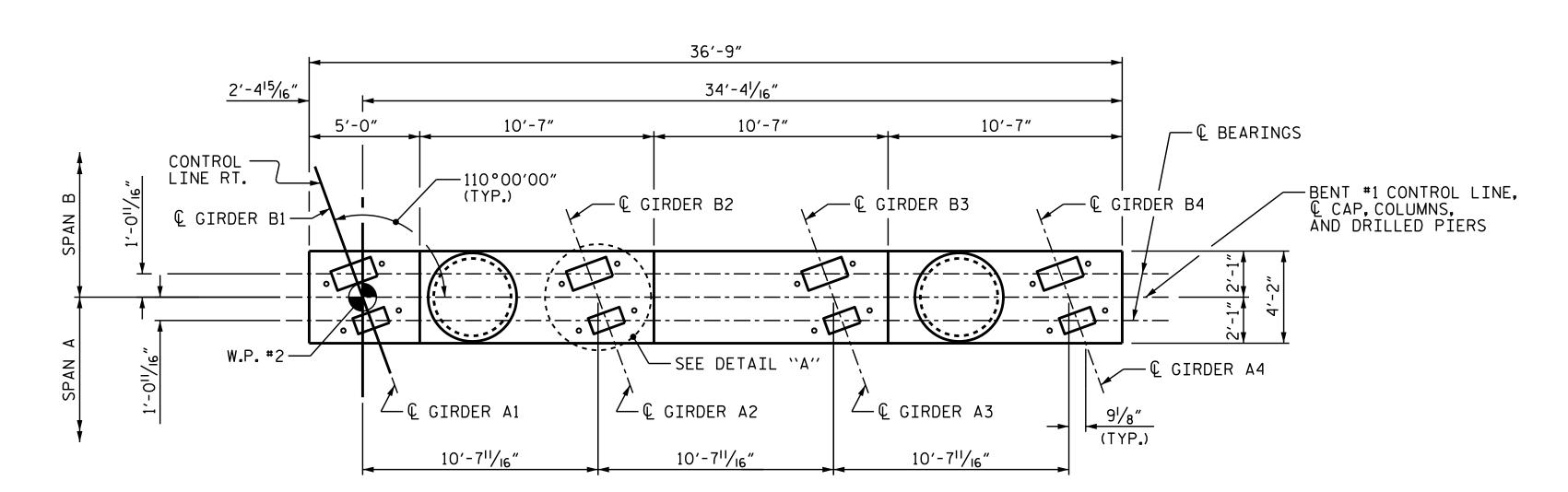
CHECKED BY : R.F. WERTMAN

DESIGN ENGINEER OF RECORD : R.F. WERTMAN

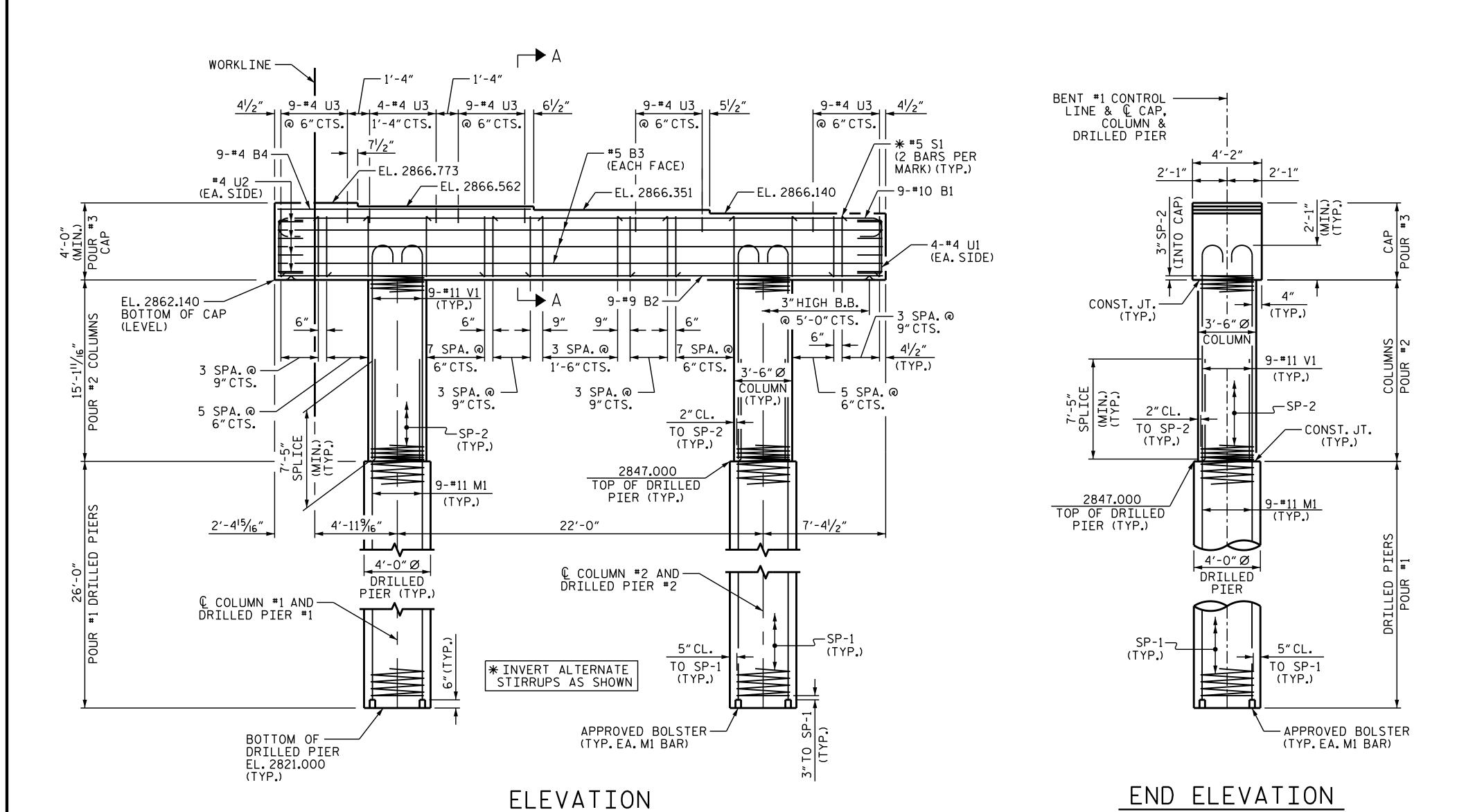
DATE : 10/21/14

DATE : 11/7/14

\_\_ DATE : <u>11/7/14</u>



PLAN



#### NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. HOOKS ON VI BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

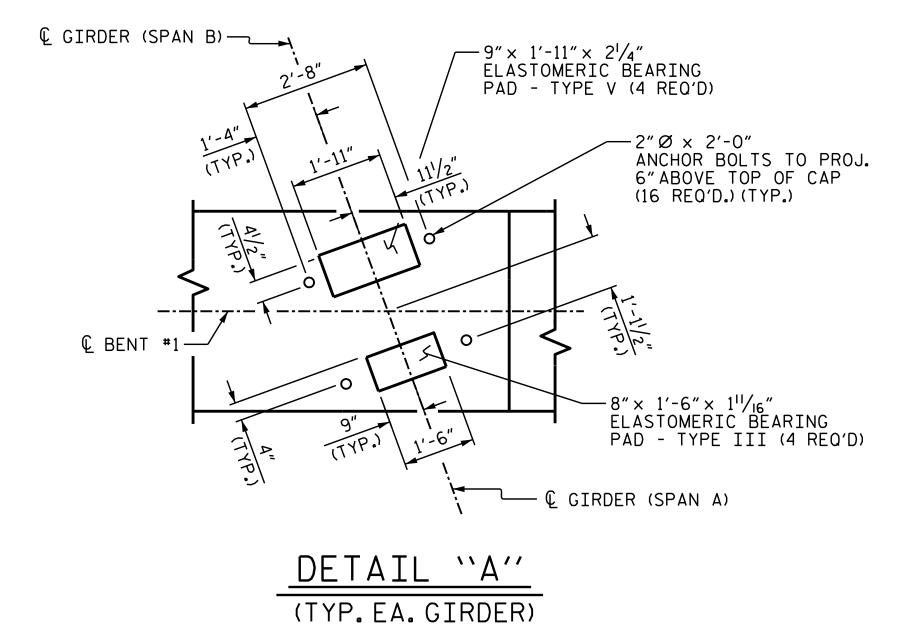
FOR DRILLED PIERS. SEE SECTION 411 OF STANDARD SPECIFICATIONS. ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE GROUND LINE.

SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIER WILL NOT BE PERMITTED.

FOR PERMANENT STEEL CASING, SEE SECTION 411 OF STANDARD SPECIFICATIONS.



PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

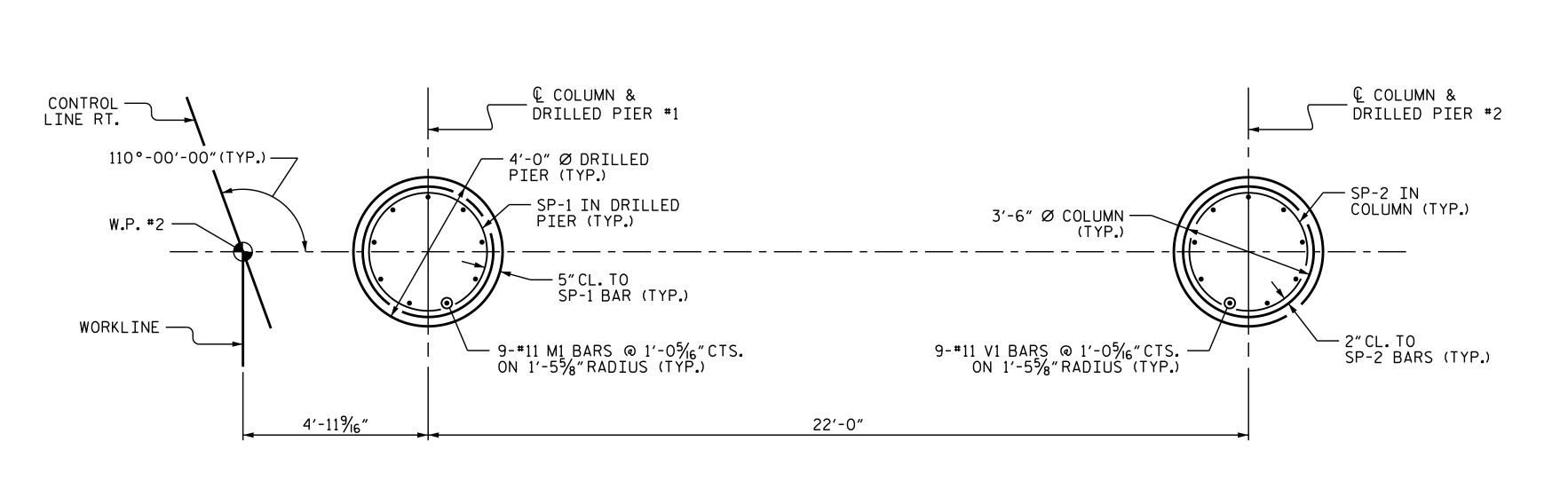
BENT #1

NBL

REVISIONS SHEET NO. S05-26 NO. BY: DATE: DATE: BY: TOTAL SHEETS

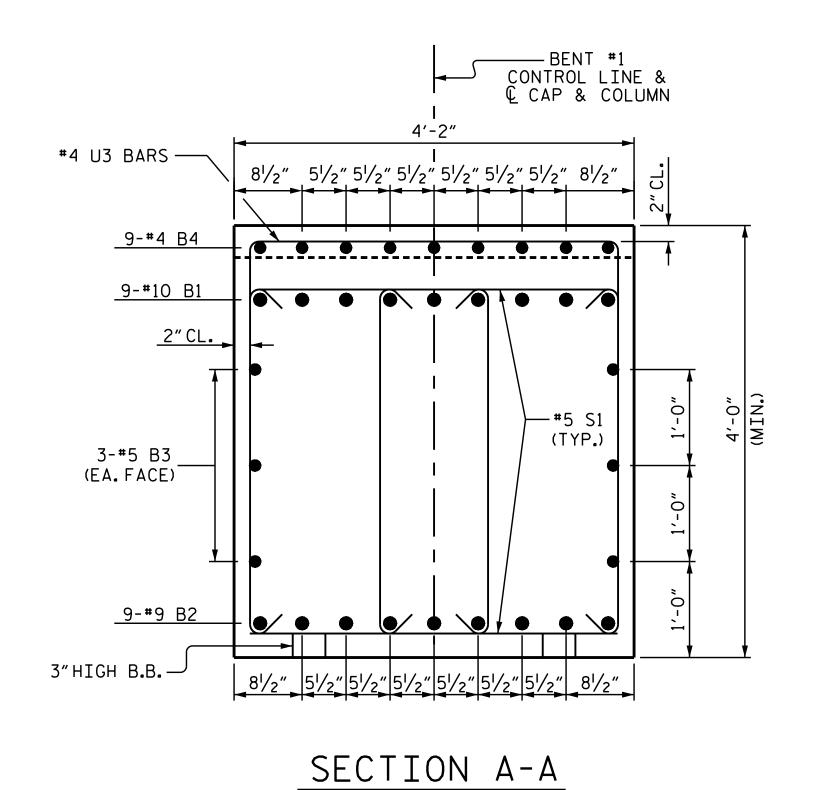
**PLANS PREPARED BY:** 

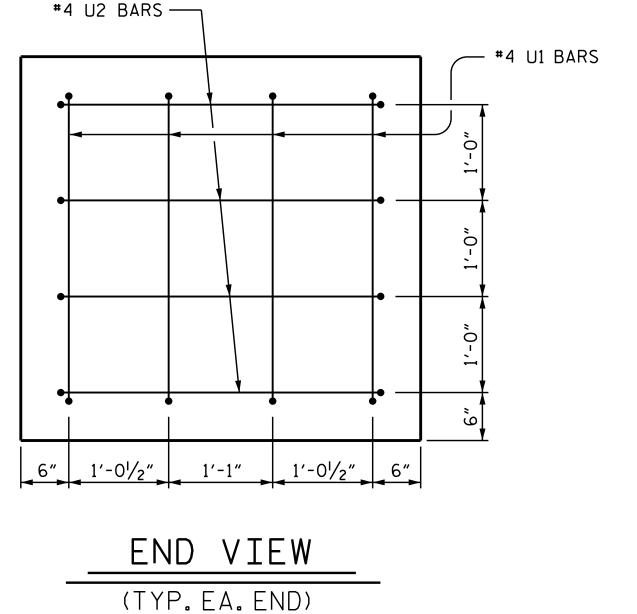
1121 Situs Court Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 SEAL \* 037180



#### PLAN OF COLUMNS AND DRILLED PIERS

REINFORCING STEEL AND DIMENSIONS ARE TYPICAL FOR ALL COLUMNS AND DRILLED PIERS

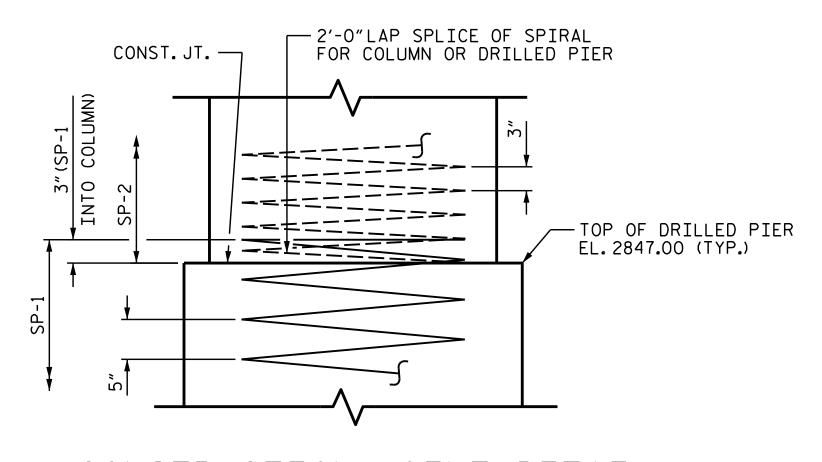




3'-6" U1 3′-8" U2 3'-10" U3 40 #4 4 V1 | 18 4 REINFORCING STEEL 2′-6″ SP-2 SPIRAL COLUMN REINFORCING STEEL 1 1/2 EXTRA TURNS CLASS A CONCRETE BREAKDOWN TOTAL CLASS A CONCRETE  $1 \frac{1}{2}$  EXTRA TURNS 4 SPACERS — — 4 SPACERS DRILLED PIER CONCRETE BREAKDOWN POUR 1 (DRILLED PIERS) 4'-0" Ø DRILLED PIER IN SOIL: 4'-0" Ø DRILLED PIER NOT IN SOIL: 3′-2″ 3′-2″ PERMANENT STEEL CASING FOR 4'-0" Ø DRILLED PIERS: CSL TUBES: ALL BAR DIMENSIONS ARE OUT TO OUT. \*\* THE SP-1 SPIRAL REINFORCING STEEL SHALL BE W31 OR D-31 COLD DRAWN WIRE OR \*5 PLAIN OR DEFORMED BAR. \*\*\* THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

BAR TYPES

1'-5"



CONSTRUCTION JOINT DETAIL

**PLANS PREPARED BY:** 

Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

1121 Situs Court Suite 170

	ST	ΑΤΙ	ON:	24	2+6	7.42		
		ET 2						
		DEPA	•	OI	NORTH CAR TRAN	OLINA NSPORTA	TION	
	SUBSTRUCTURE							
			Е	BEI	NT :	<b>#</b> 1		
Thuman The		NBL						
'iiiiii	NO.	- DV	REVI	1		24.75	SHEET N	
2	NO.	BY:	DATE:	NO.	BY:	DATE:		

PROJECT NO. R-2915B

ASHE

TOTAL SHEETS 35

DATE : 10/21/14 DRAWN BY : T.J. KIRSCHBAUM CHECKED BY : R.F. WERTMAN \_ DATE : <u>11/7/14</u> DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/7/14

BILL OF MATERIAL

BENT #1

BAR NO. SIZE TYPE LENGTH WEIGHT

#9 STR 36'-5"

#5 STR 36′-5″

**#**10 1

#4 STR

M1 18 #11 STR 36'-2"

**#**5 3

**#**4 4

**#**4 4

**#**11 2

\*\*\*

POUR 2 (COLUMNS)

POUR 3 (CAPS)

B1

B2

В4

S1 96

8

8

2

17'-3"

39'-1"

15′-3"

10'-8"

6'-6"

6′-8"

6'-10"

18'-10"

1114

228

92

3459

1068

35

36

183

1801

9530 LBS

2127 LB

10.8 C.Y

24.3 C.Y

35.1 C.Y

24.2 C.Y

10.0 FT.

220.0 FT.

COUNTY

LINEAR FEET 20.0 FT.

LINEAR FEET 32.0 FT.

840

617′-0″ 1287

628′-7″

+

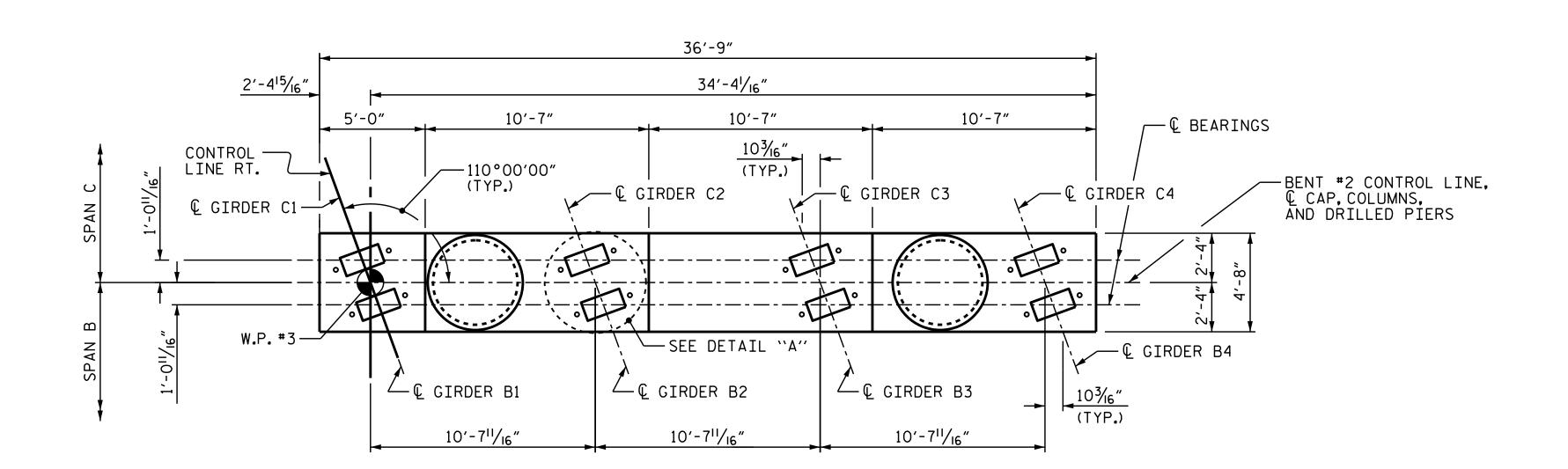
DRAWN BY : T.J. KIRSCHBAUM

CHECKED BY : R.F. WERTMAN

DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/7/14

DATE : 10/22/14

DATE : 11/7/14



#### NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. HOOKS ON VI BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

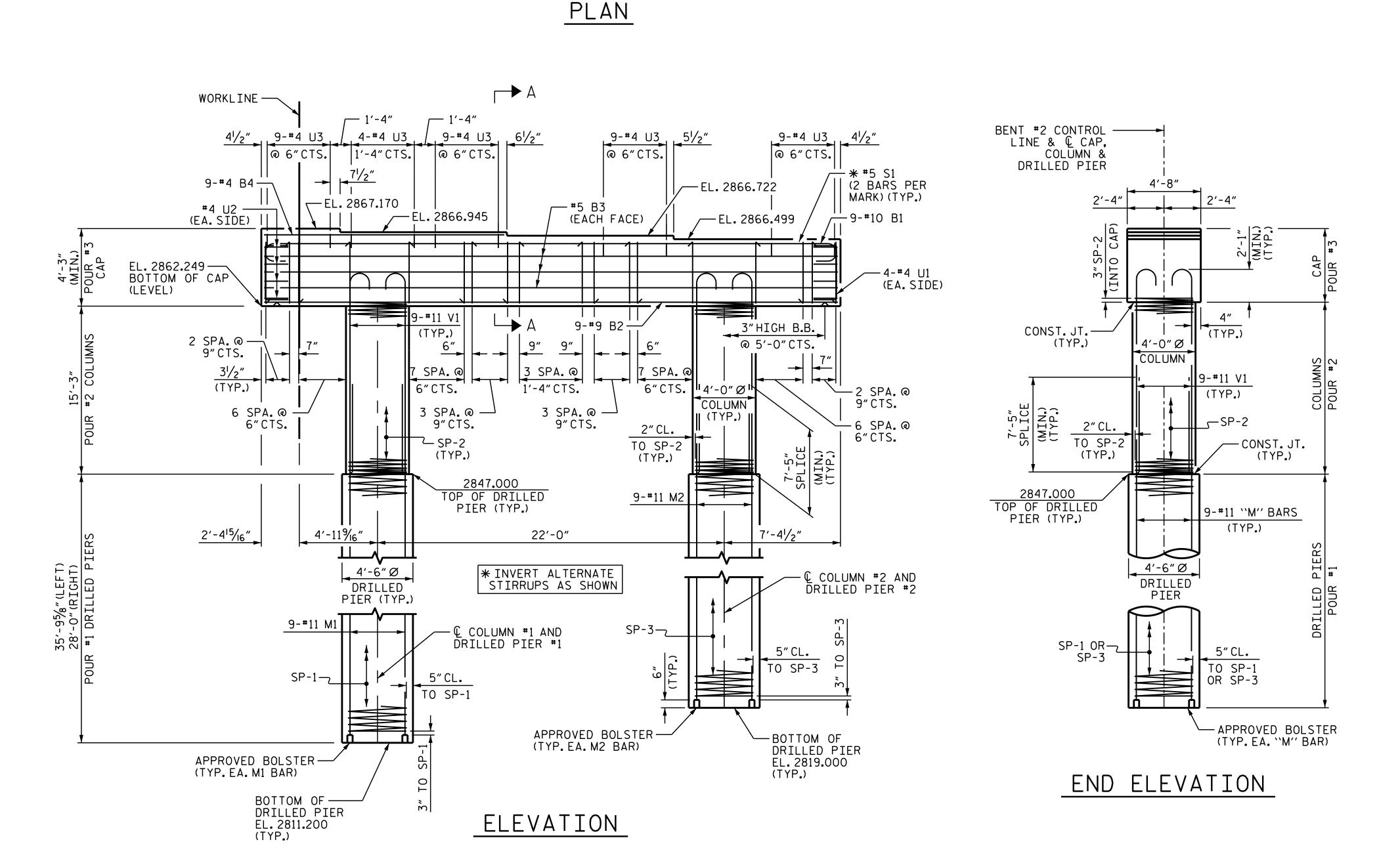
FOR DRILLED PIERS, SEE SECTION 411 OF STANDARD SPECIFICATIONS. ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL".

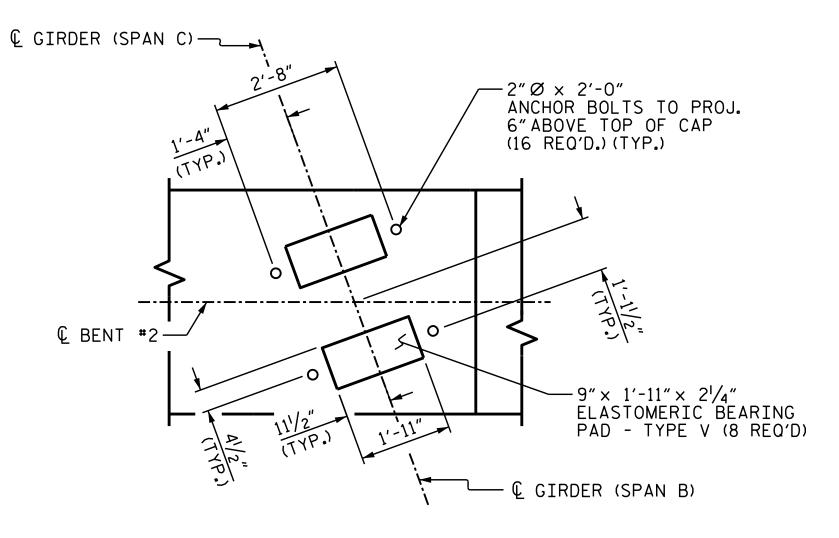
THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA LENGTH.

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE GROUND LINE.

SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIER WILL NOT BE PERMITTED.

FOR PERMANENT STEEL CASING, SEE SECTION 411 OF STANDARD SPECIFICATIONS.





DETAIL "A" (TYP.EA.GIRDER)

> PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

BENT #2

NBL

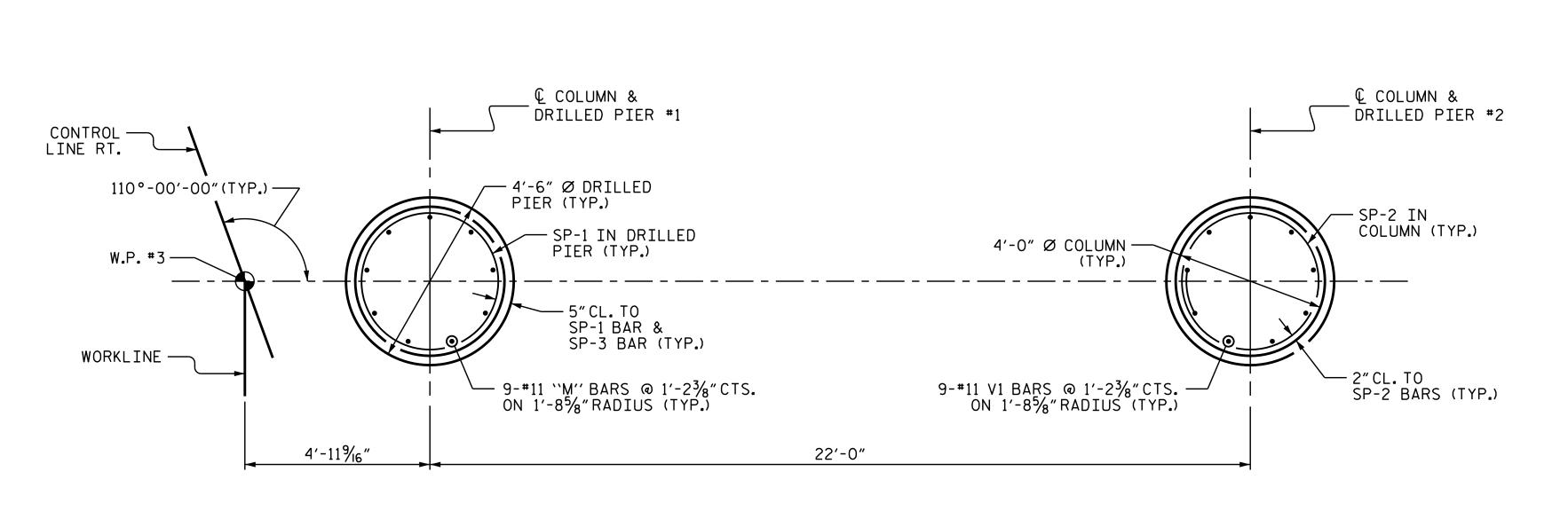
TOTAL SHEETS

SHEET NO. REVISIONS S05-28 NO. BY: DATE: DATE: BY:

Gannett Fleming (919) 859-4880

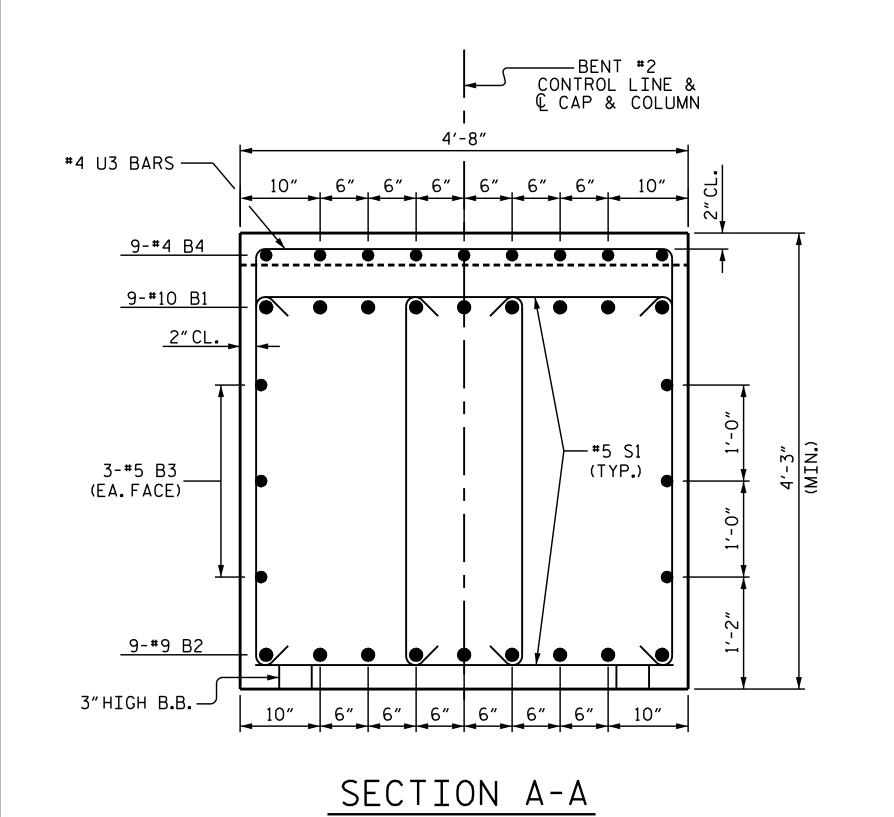
Excellence Delivered As Promised NC Lic. No. F-0270

SEAL \* 037180 1121 Situs Court

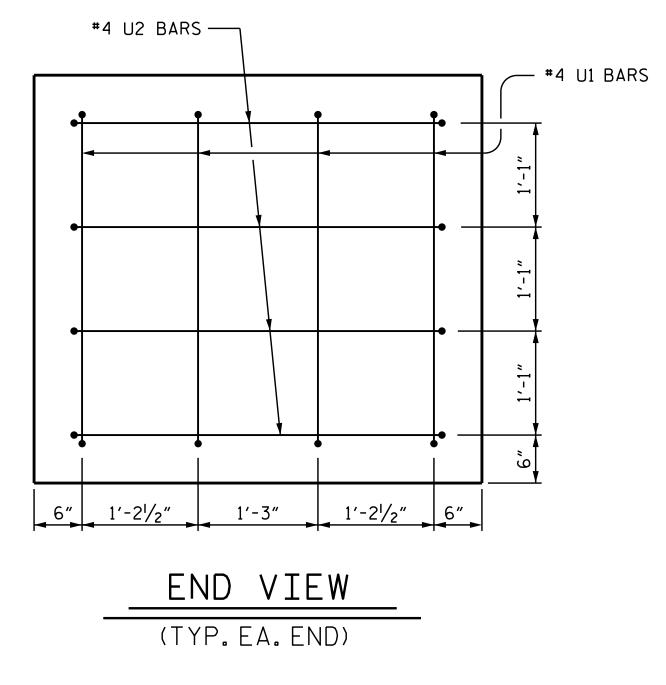


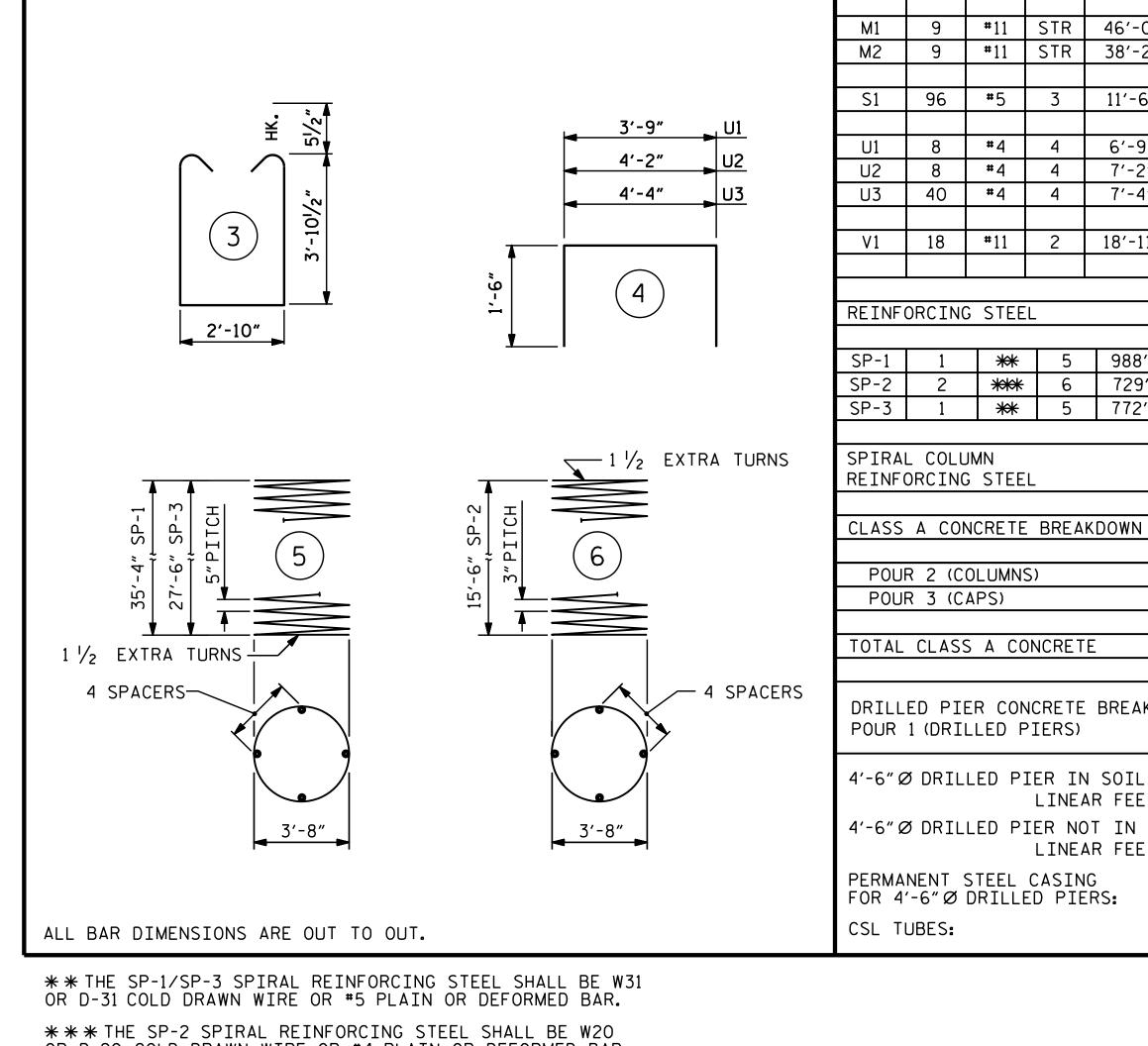
#### PLAN OF COLUMNS AND DRILLED PIERS

REINFORCING STEEL AND DIMENSIONS ARE TYPICAL FOR ALL COLUMNS AND DRILLED PIERS



DATE : 10/22/14



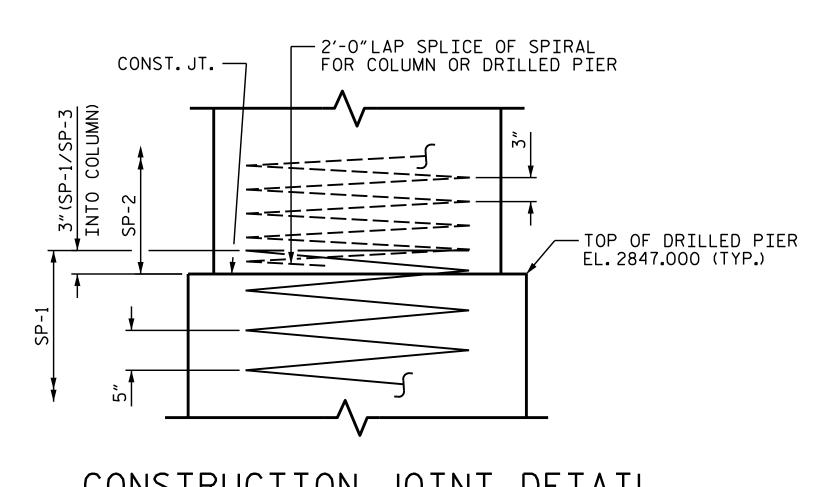


-BAR TYPES

1'-5"

1'-7"

\*\*\* THE SP-2 SPIRAL REINFORCING STEEL SHALL BE W20 OR D-20 COLD DRAWN WIRE OR #4 PLAIN OR DEFORMED BAR.

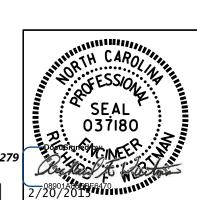


CONSTRUCTION JOINT DETAIL

**PLANS PREPARED BY:** 

Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

1121 Situs Court Suite 170



ASHECOUNT
ATION: 242+67.42 -L-
EET 2 OF 2
STATE OF NORTH CAROLINA  DEPARTMENT OF TRANSPORTATION  RALEIGH
SUBSTRUCTURE
BENT #2

NBL SHEET NO. REVISIONS S05-29 NO. BY: DATE: DATE: TOTAL SHEETS

BY:

CHECKED BY : R.F. WERTMAN DATE : 11/7/14 DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/7/14

DRAWN BY : T.J. KIRSCHBAUM

STR.NO.5

POUR 2 (COLUMNS) 14.2 C. POUR 3 (CAPS) 28.4 C.

BILL OF MATERIAL

BENT #2

#9 STR 36′-5″

#5 | STR | 36'-5"

39'-1"

15′-3"

11'-6"

6′-9"

7'-2"

7'-4"

18'-11"

1514

1114

228

92

2200

1825

1151

36

38

196

1809

974

806

10203 LBS

2811 LBS

988'-8" 1031

729'-1"

5 772'-9"

BAR NO. SIZE TYPE LENGTH WEIGHT

**#**10 1

#4 STR

9 #11 STR 46'-0"

96 #5 3

8 #4 4

8 #4 4

40 #4 4

\*\*\*

\*\*

6

18 #11

9 #11 STR 38'-2"

B1

B2

В4

2

17'-4"

9

TOTAL CLASS A CONCRETE 42.6 C.Y

DRILLED PIER CONCRETE BREAKDOWN POUR 1 (DRILLED PIERS) 37.6 C.Y.

4'-6" Ø DRILLED PIER IN SOIL : LINEAR FEET 24.8 FT. 4'-6" Ø DRILLED PIER NOT IN SOIL:

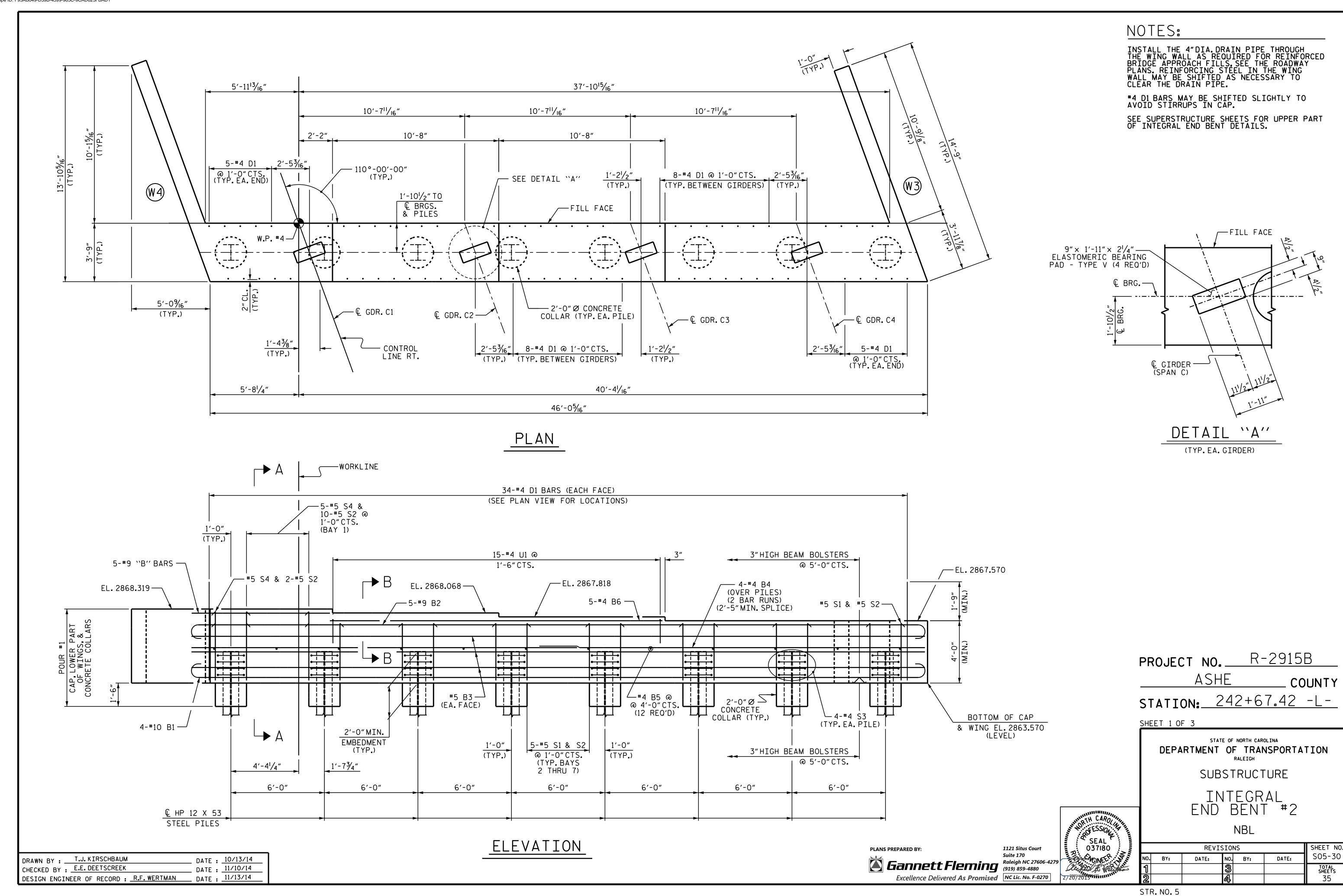
LINEAR FEET 39.0 FT. PERMANENT STEEL CASING 14.0 FT.

FOR 4'-6" Ø DRILLED PIERS: CSL TUBES: 267.2 FT.

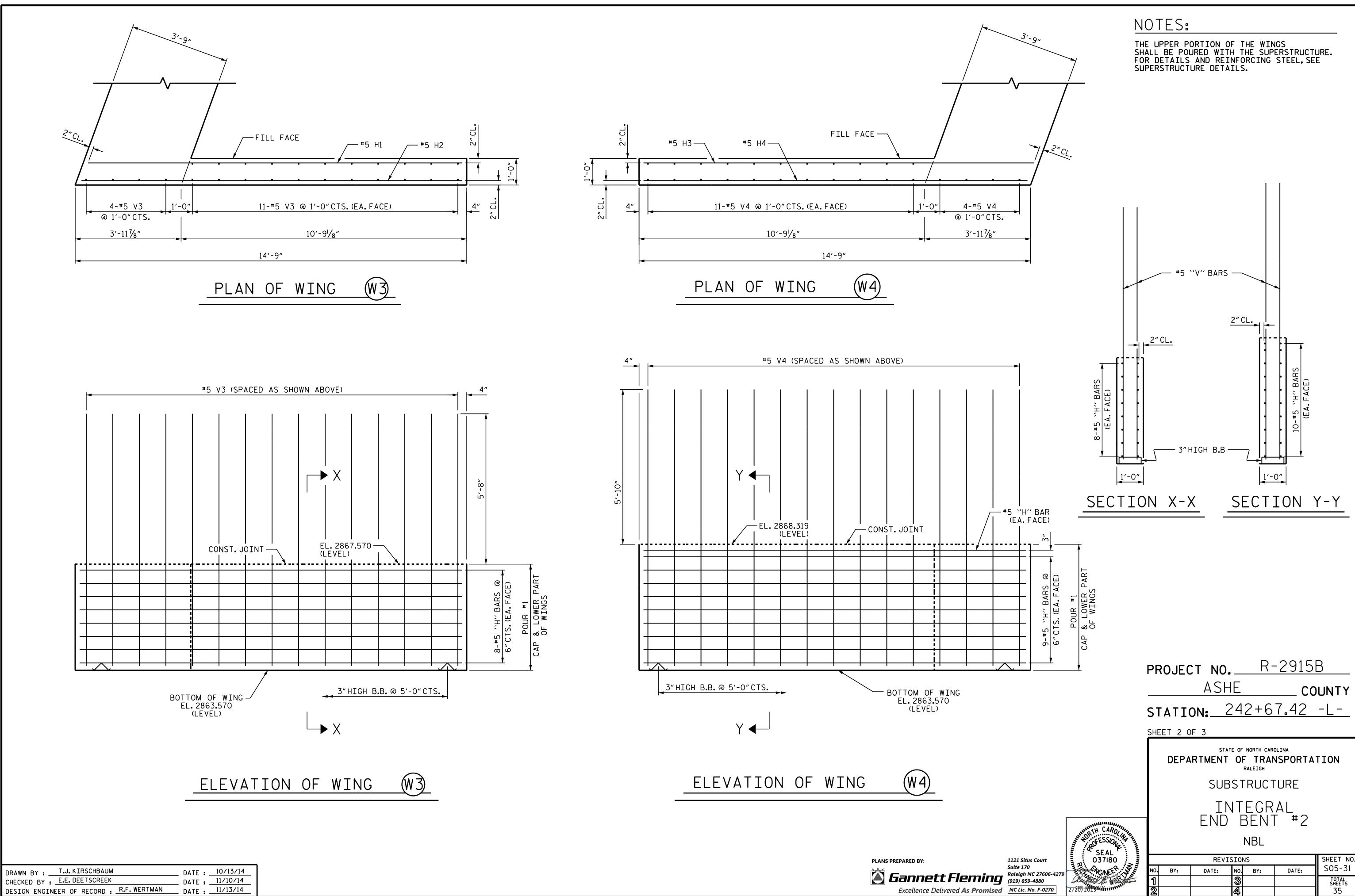
R-2915B PROJECT NO.\_  $\Lambda \subset \sqcup \Gamma$ 

STA

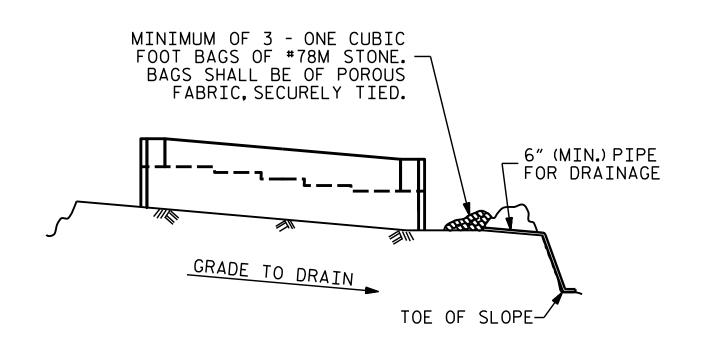
+



+



+

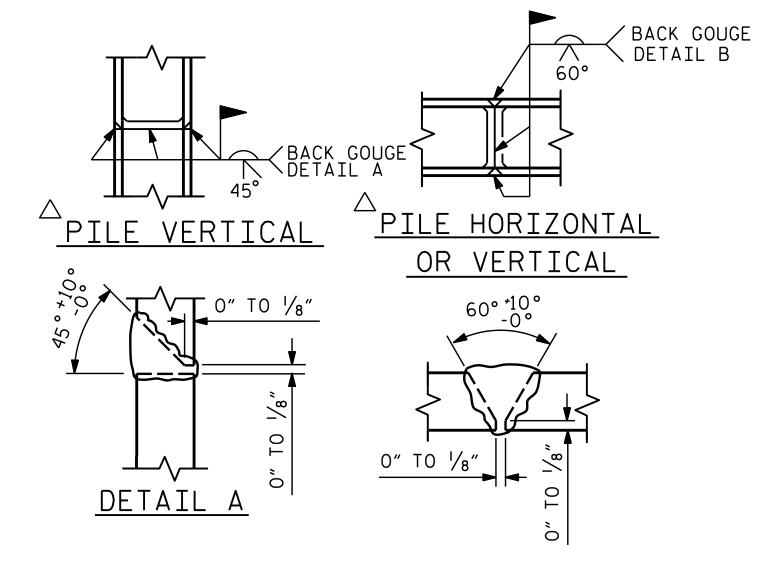


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

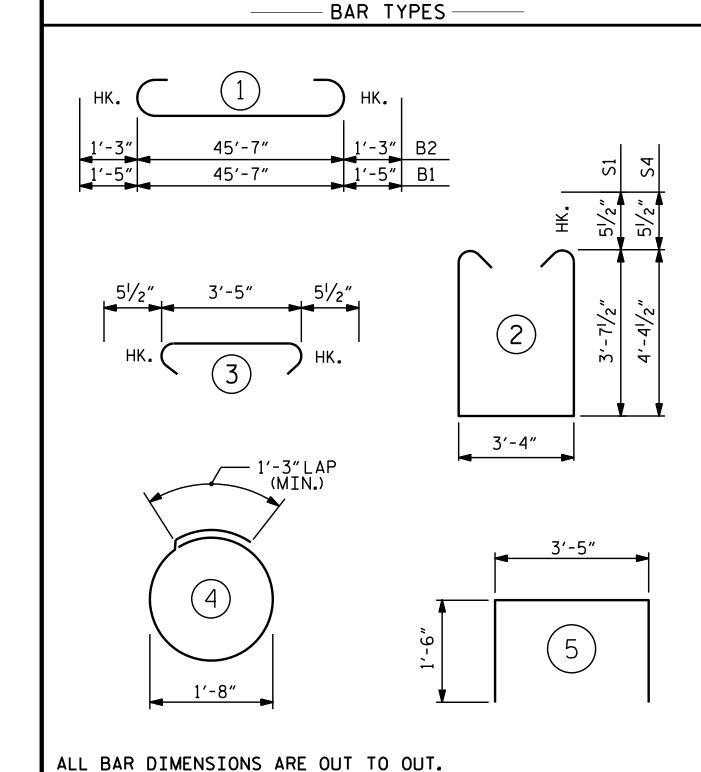
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

#### TEMPORARY DRAINAGE AT END BENT



DETAIL B POSITION OF PILE DURING WELDING.

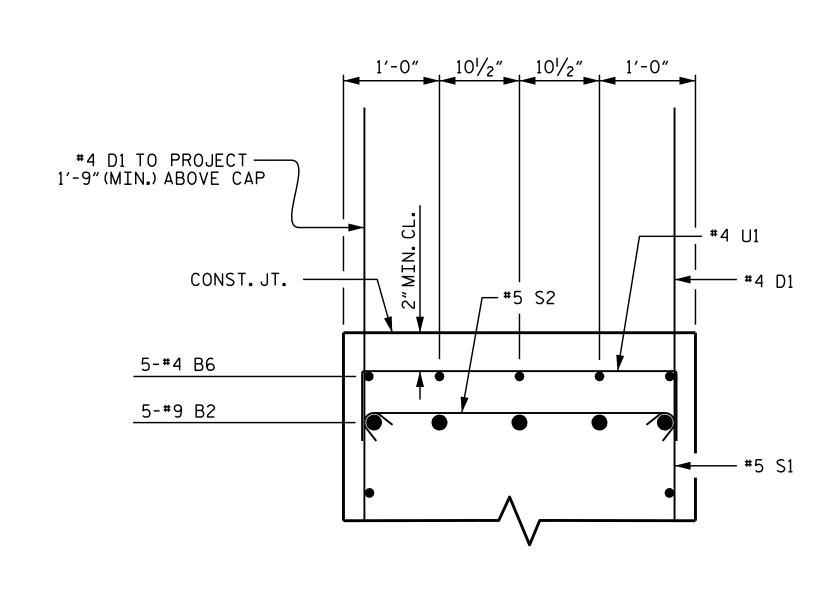
#### PILE SPLICE DETAILS



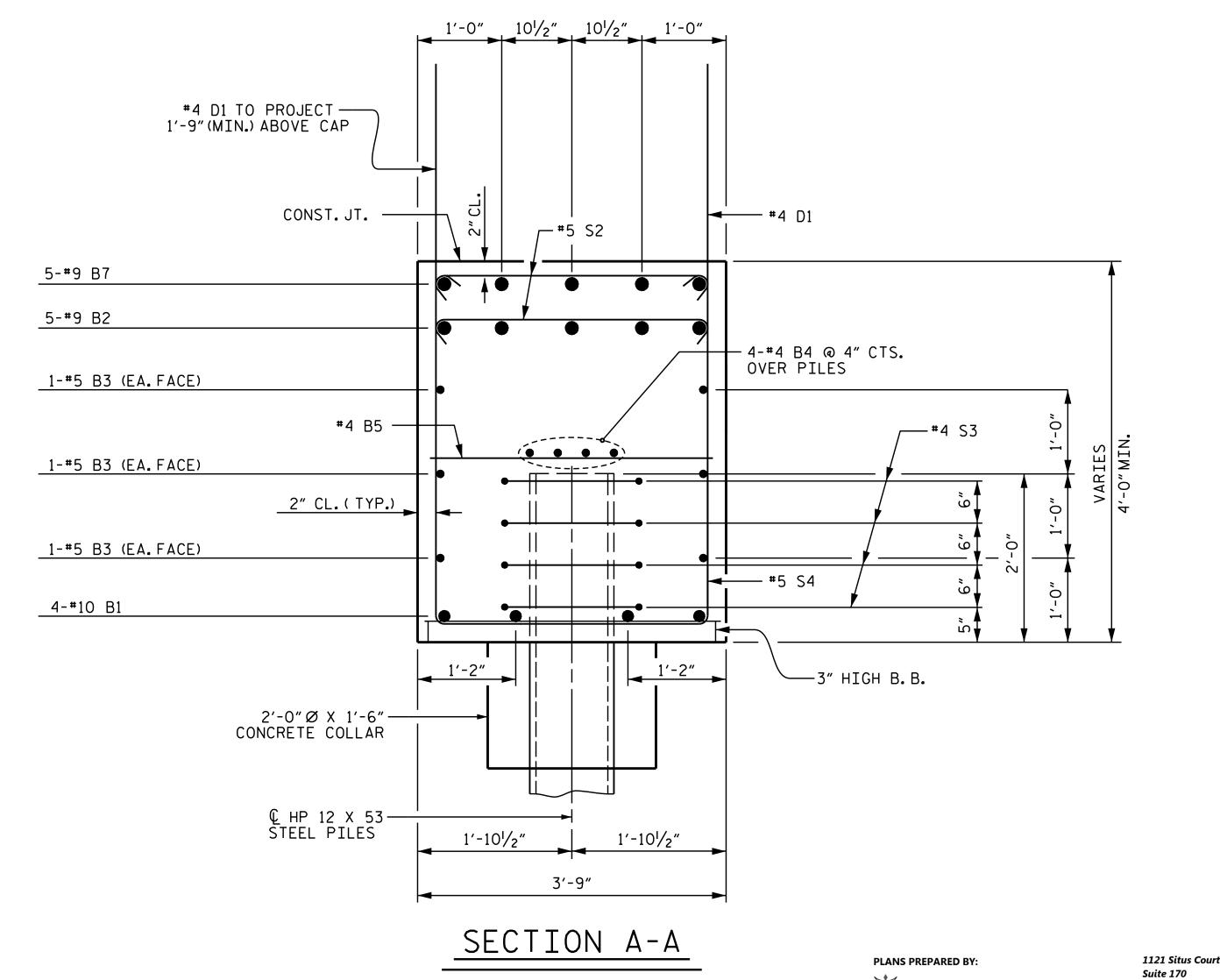
	BILL OF MATERIAL							
	END BENT #2							
	BAR	No.	SIZE	TYPE	LENGTH	WEIGHT		
	B1	4	#10	1	48'-5"	833		
	B2	5	#9	1	48'-1"	817		
	В3	6	<b>#</b> 5	STR	45'-8"	286		
	B4	8	#4	STR	24'-1"	129		
	B5	12	#4	STR	3′-5″	27		
	В6	5	#4	STR	21'-4"	71		
	В7	1	#9	STR	8'-9"	30		
	В8	1	#9	STR	8′-6″	29		
	В9	1	#9	STR	8'-2"	28		
	B10	1	#9	STR	7′-10″	27		
	B11	1	#9	STR	7'-7"	26		
	D1	68	#4	STR	6′-4″	288		
	H1	8	<b>#</b> 5	STR	14'-1"	118		
	H2	8	<b>#</b> 5	STR	14'-4"	120		
	Н3	10	<b>#</b> 5	STR	14'-8"	153		
	Н4	10	<b>#</b> 5	STR	14'-5"	150		
	S1	31	<b>#</b> 5	2	11'-6"	372		
	S2	43	<b>#</b> 5	3	4'-4"	194		
	S3	32	#4	4	6′-6″	139		
	S4	6	<b>#</b> 5	2	13'-0"	81		
	U1	15	#4	5	6′-5″	64		
	٧3	26	#5	STR	9′-6″	258		
	V4	26	<b>#</b> 5	STR	10'-5"	282		
	REI	NFORCIN	NG STEEL			1522 LBS.		
CLASS A CONCRETE								

CLASS A CONCRETE POUR #1 (CAP, LOWER WINGS & COLLARS) 32.5 C.Y TOTAL 32.5 C.Y. HP 12 X 53 STEEL PILES

280 LIN.FT. No. = 8



SECTION B-B



PROJECT NO. R-2915B ASHE COUNTY

STATION: 242+67.42 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

INTEGRAL END BENT #2

NBL

SHEET NO. REVISIONS S05-32 NO. BY: DATE: DATE: BY: TOTAL SHEETS

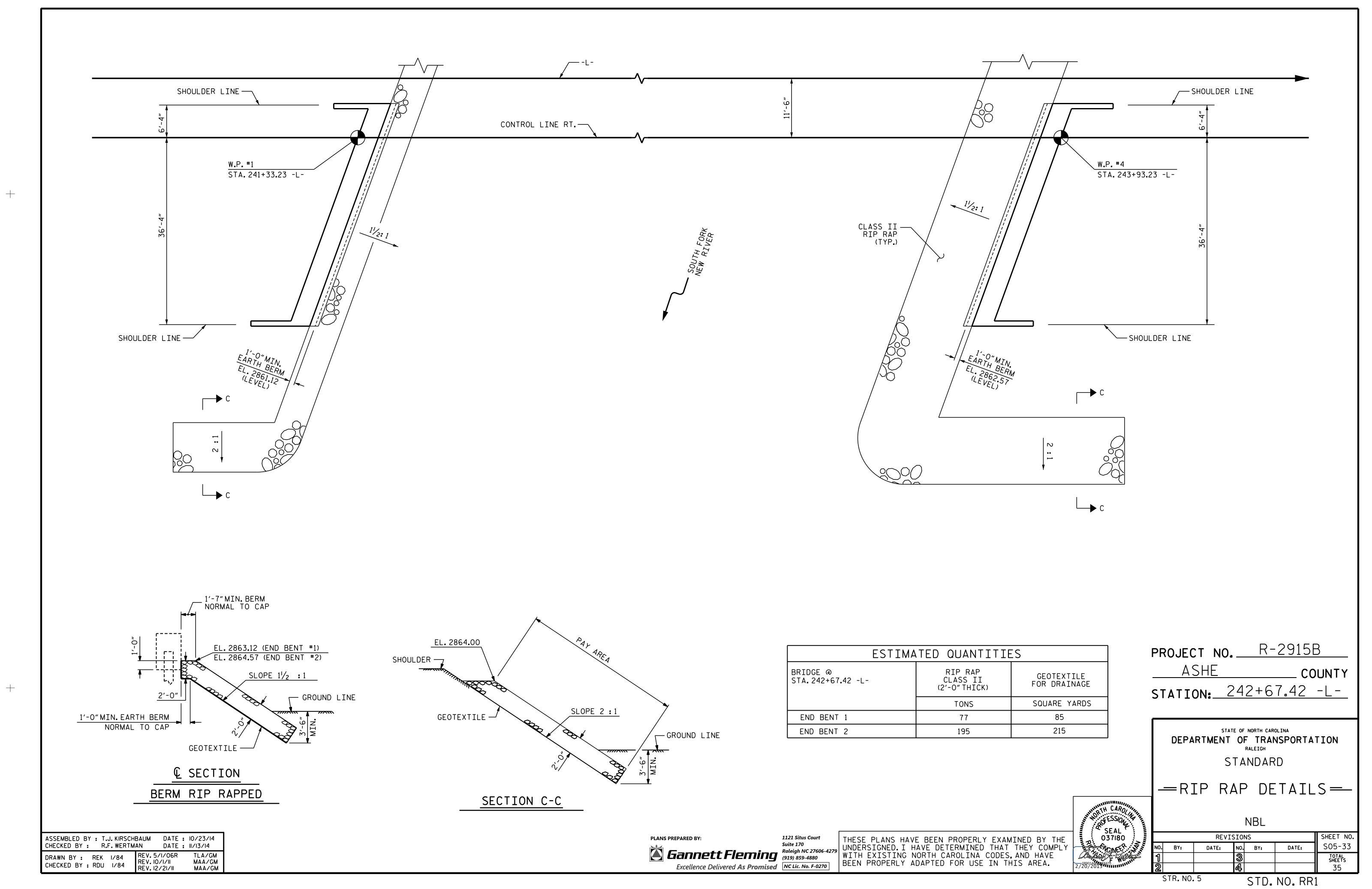
DRAWN BY : T.J. KIRSCHBAUM DATE : 10/13/14 CHECKED BY : E.E. DEETSCREEK \_ DATE : 11/10/14 DESIGN ENGINEER OF RECORD : R.F. WERTMAN \_ DATE : 11/13/14

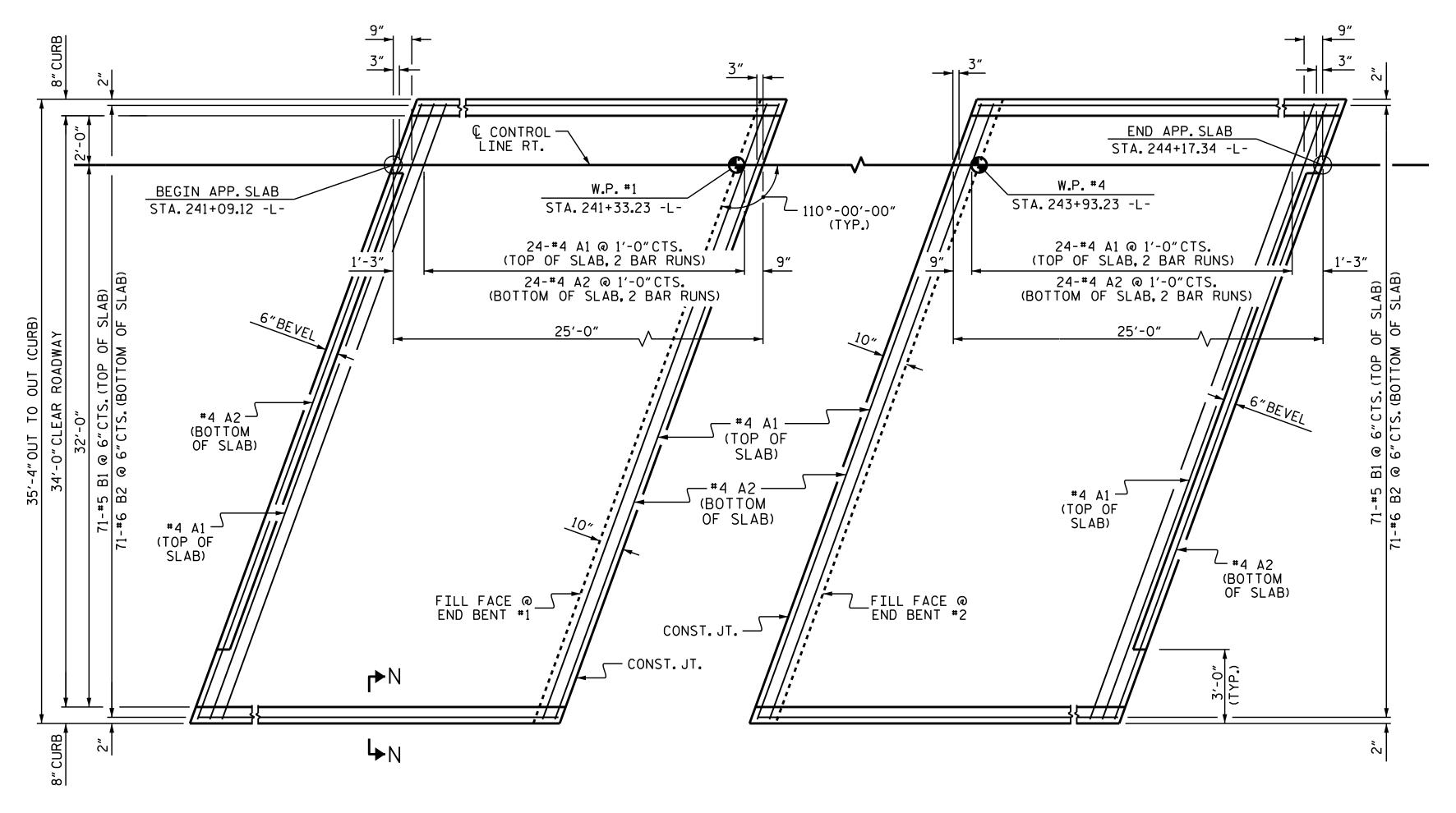
Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered **As Promised** NC Lic. No. F-0270

Outain & Walter

STR.NO.5

SEAL \* 037180





#### NOTES:

APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.

[	BILL	OF	MAT	ERIA	L	
FOR	ONE (2		PRO QUIF	•	SL	AB

AR	NO.	SIZE	TYPE	LENGTH	WEIGHT
<b>A</b> 1	5,2	<b>#.</b> 4	SIR	19'8"	683
<b>A</b> 2	5,2	<b>#.</b> 4	SIR	19'6"	677.
•	•	•	•	•	•
B1	7.1	<b>#.</b> 5	SIR	24′2″	179Q
B2	7.1	#,6	SĮR	24′8″	2631

REINFORCING STEEL	LBS.	330&
* EPOXY COATED . REINFORCING STEEL	LBS.	2473

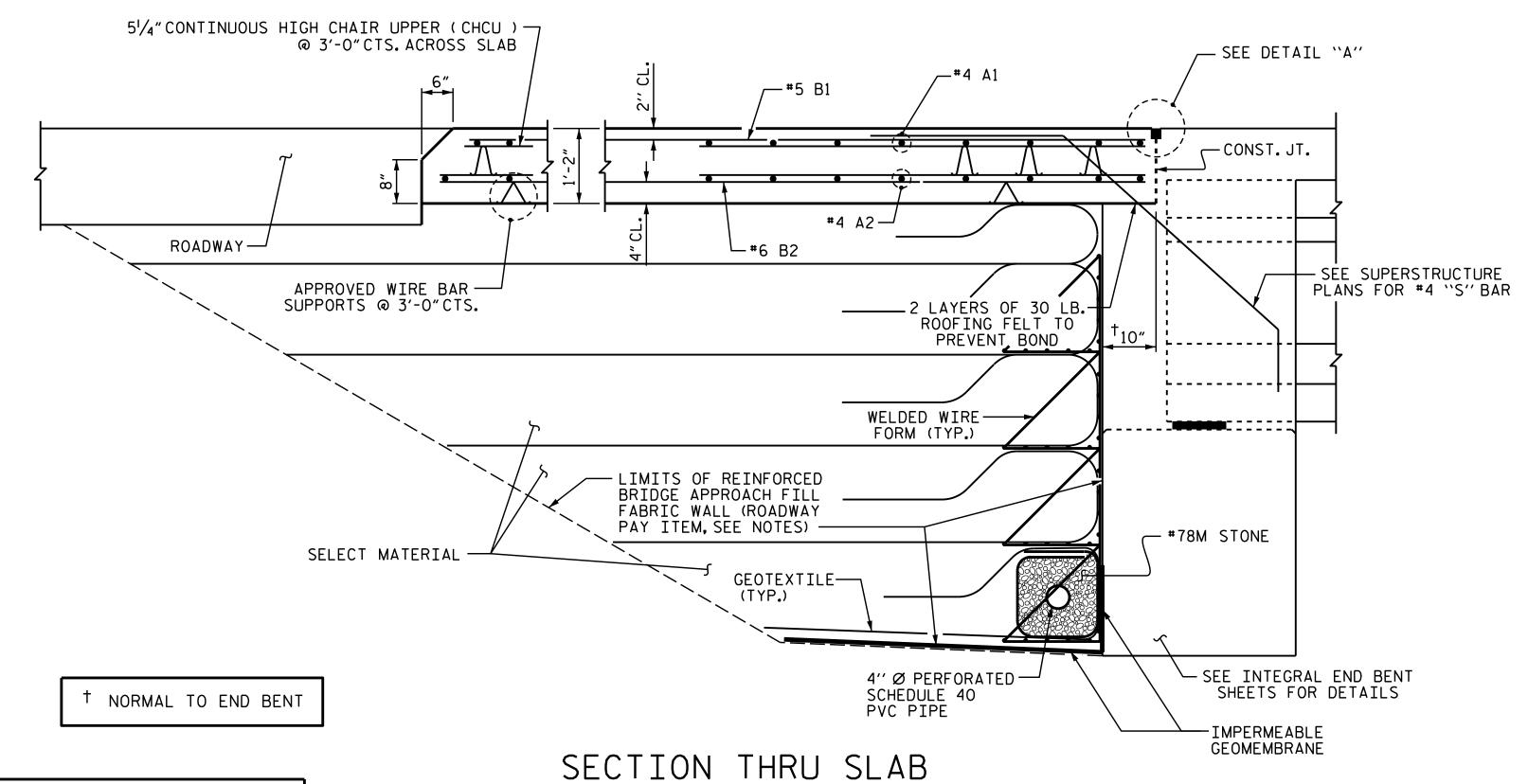
40.7

C.Y.

CLASS AA CONCRETE

SPI	ICE	LENC	STHS	CHART
	R EF	OXY ATED	UNCOATED	
#4	2	2'-0"	1	′-9″
<b>#</b> 5	2	2′-6″	2	2'-2"

### PLAN @ END BENT #2 DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS



PLAN @ END BENT #1

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 10/27/14

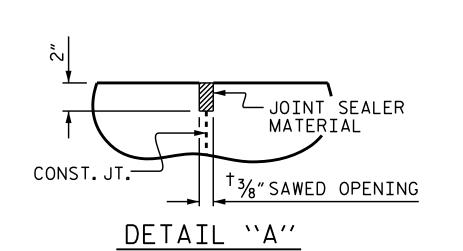
DATE : 11/13/14

MAA/GM MAA/GM

CHECKED BY: R.F. WERTMAN

DRAWN BY: TLA 10/05 REV. 10/1/11 REV. 12/21/11 REV. 6/13

3'-1<sup>1</sup>/<sub>2</sub>" CURB APPROACH SLAB — END OF CURB WITHOUT SECTION N-N SHOULDER BERM GUTTER CURB DETAILS



1121 Situs Court

Excellence Delivered **As Promised** NC Lic. No. F-0270

PLANS PREPARED BY:

PROJECT NO. R-2915B ASHE COUNTY

STATION: 242+67.42 -L-

SHEET 1 OF 2

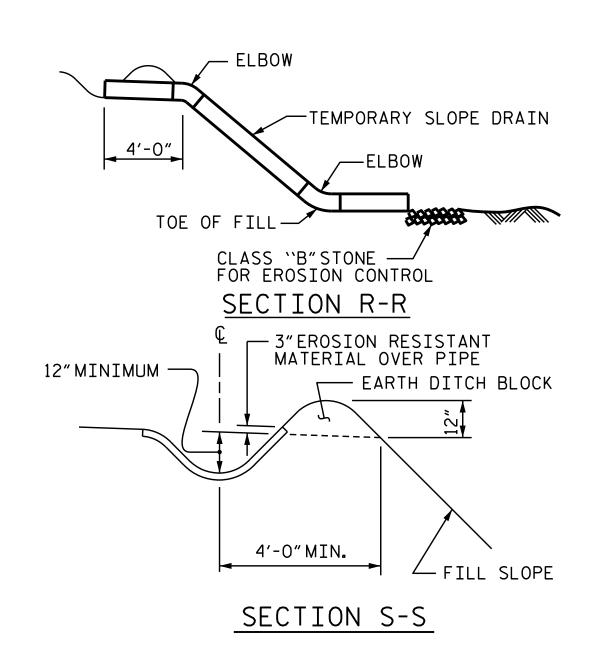
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

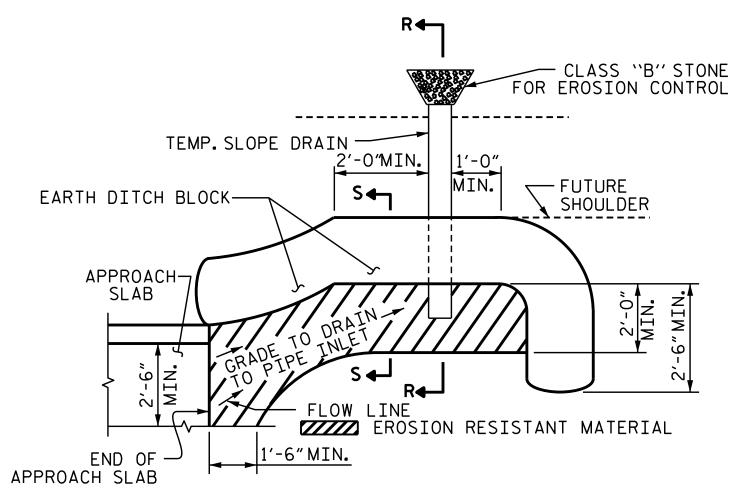
BRIDGE APPROACH SLAB FOR INTEGRAL ABUTENT

NBL

		SHEET NO.				
,	BY:	DATE:	NO.	BY:	DATE:	S05-34
			3			TOTAL SHEETS
			A			35

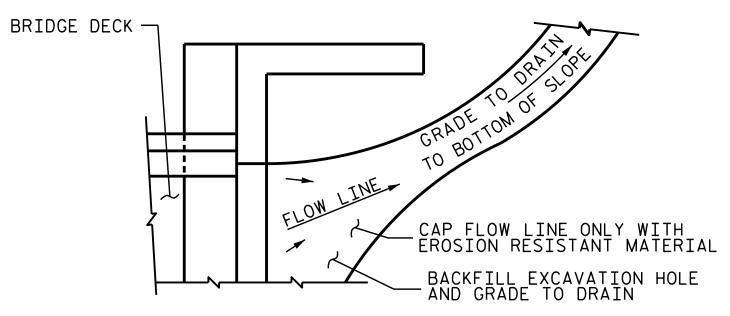
Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880





NOTE: IMMEDIATELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB, THE CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE DRAIN. CONTRACTOR SHALL GRADE TO PIPE INLET AND PROVIDE EROSION RESISTANT MATERIAL AS SHOWN. THE EROSION RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT PLANT MIX, TYPE 1 OR TYPE 2, MIN. 2" DEPTH, 2) EROSION CONTROL MAT, OR 3) CONCRETE, AS DIRECTED BY THE ENGINEER. THE SLOPE DRAIN SHALL CONSIST OF A NON-PERFORATED TEMPORARY DRAINAGE PIPE, 12 INCHES IN DIAMETER.

PLAN VIEW



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. TEMPORARY DRAINAGE DETAIL

#### TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

PROJECT NO. R-2915B ASHE COUNTY STATION: 242+67.42 -L-

SHEET 2 OF 2

SEAL 037180

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

BRIDGE APPROACH SLAB DETIALS

SHEET NO. REVISIONS NO. BY: S05-35 DATE: DATE: BY: TOTAL SHEETS

PLANS PREPARED BY: Gannett Fleming Raleigh NC 27606-4279 (919) 859-4880

Excellence Delivered **As Promised** NC Lic. No. F-0270

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

ASSEMBLED BY: T.J. KIRSCHBAUM DATE: 10/27/14 CHECKED BY: R.F. WERTMAN DATE: 11/13/14 DRAWN BY : FCJ II/88 REV. IO/I/II CHECKED BY : ARB 11/88 REV. 7/12 REV. 6/13 MAA/GM MAA/GM

STD. NO. BAS4 STR.NO.5