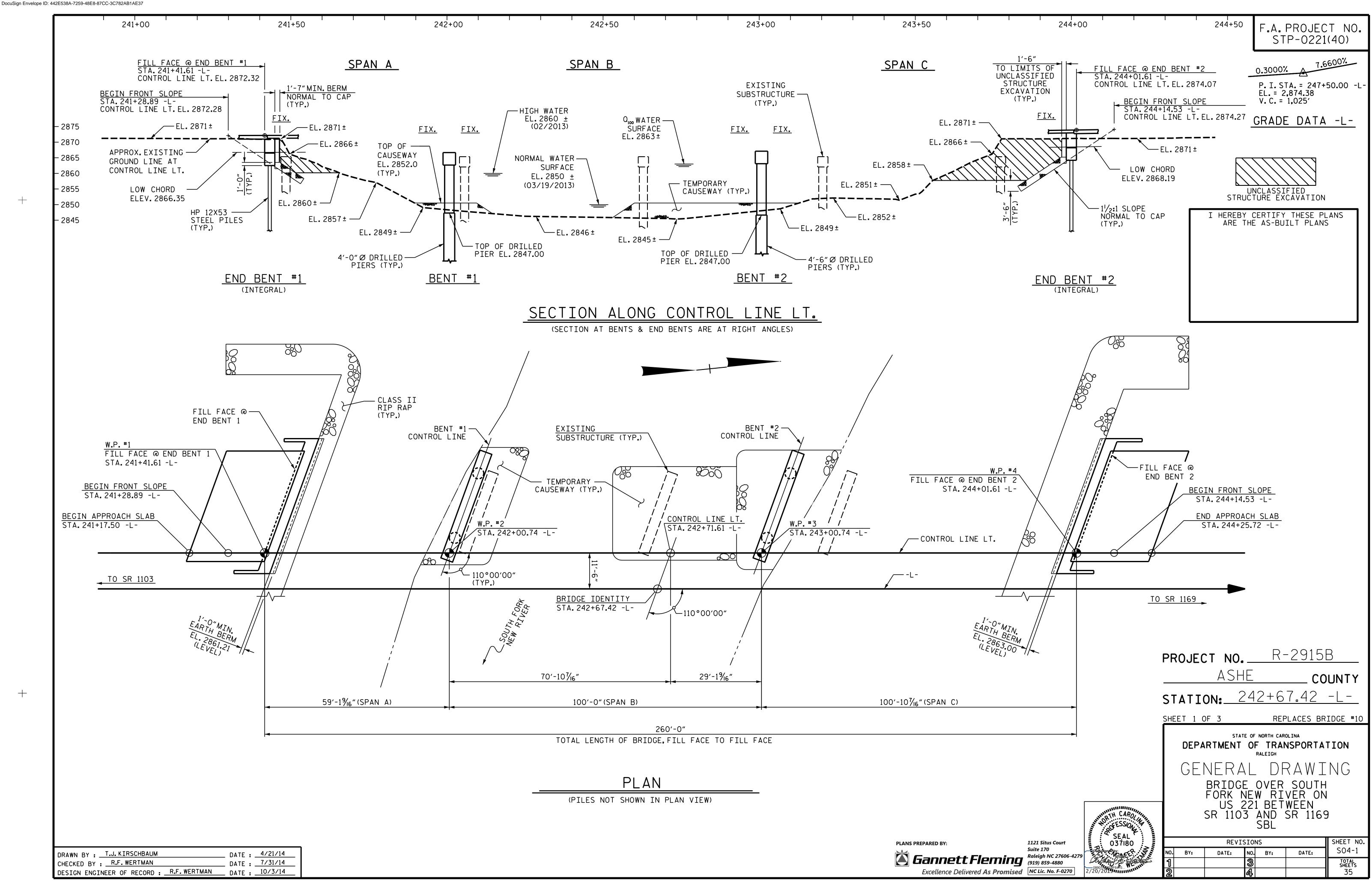
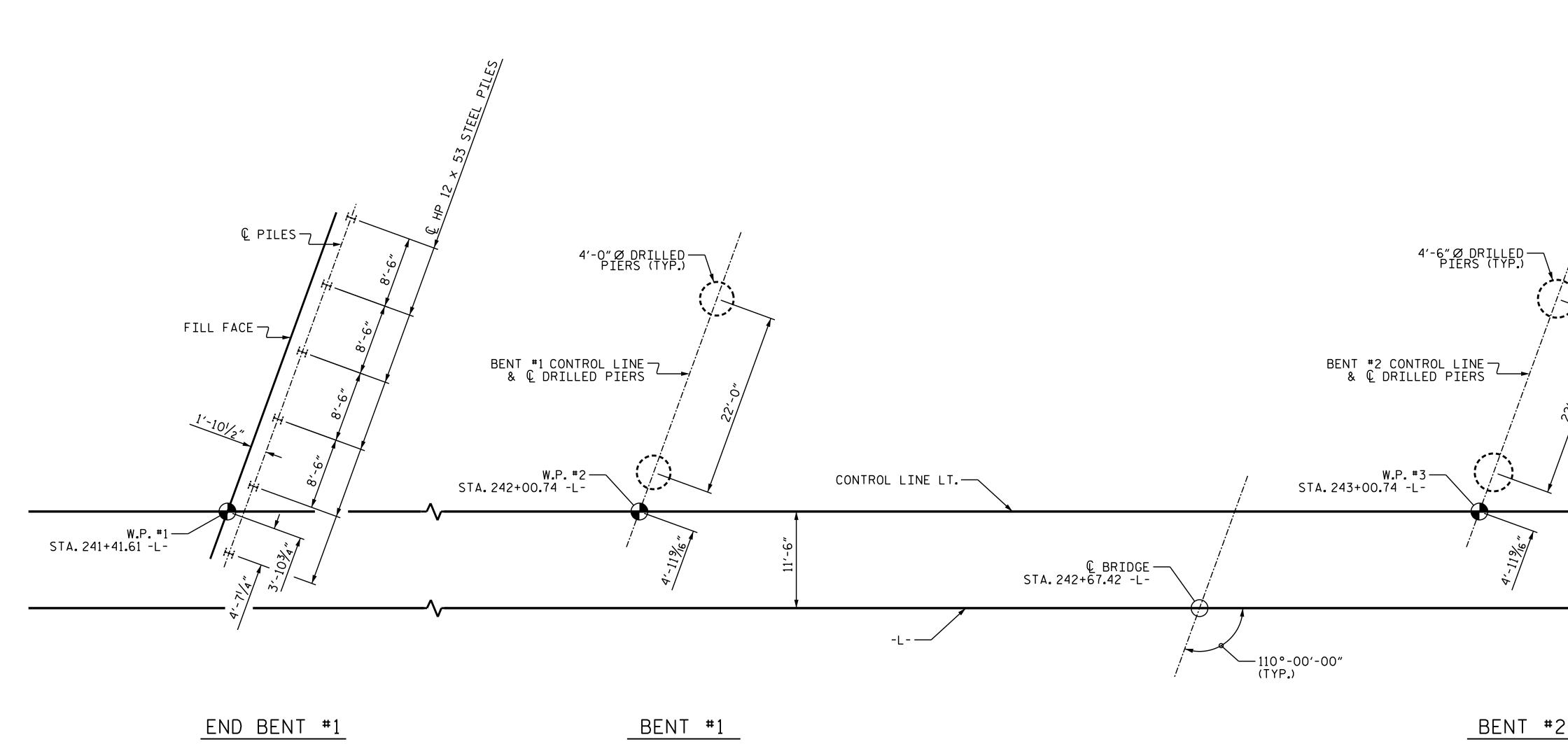
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STR.NO.4

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FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 2825.0 THE REQUIRED TIP RESISTANCE.

DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 590 TO CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30.0 TSF.

PERMANENT STEEL CASING WILL BE REQUIRED FOR DRILLED PIERS AT BENT NO.1.DO PERMANENT CASINGS BELOW ELEVATION 2839.0 FT.WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 2836.0 FT.THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

DRAWN BY :	DATE : 10/28/14
CHECKED BY :	_ DATE : <u>11/12/14</u>
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>11/12/14</u>

## FOUNDATION LAYOUT

DIMENSIONS LOCATING PILES ARE SHOWN TO PILE CENTERLINE. DIMENSIONS LOCATING DRILLED PIERS ARE SHOWN TO DRILLED PIER CENTERLINE.

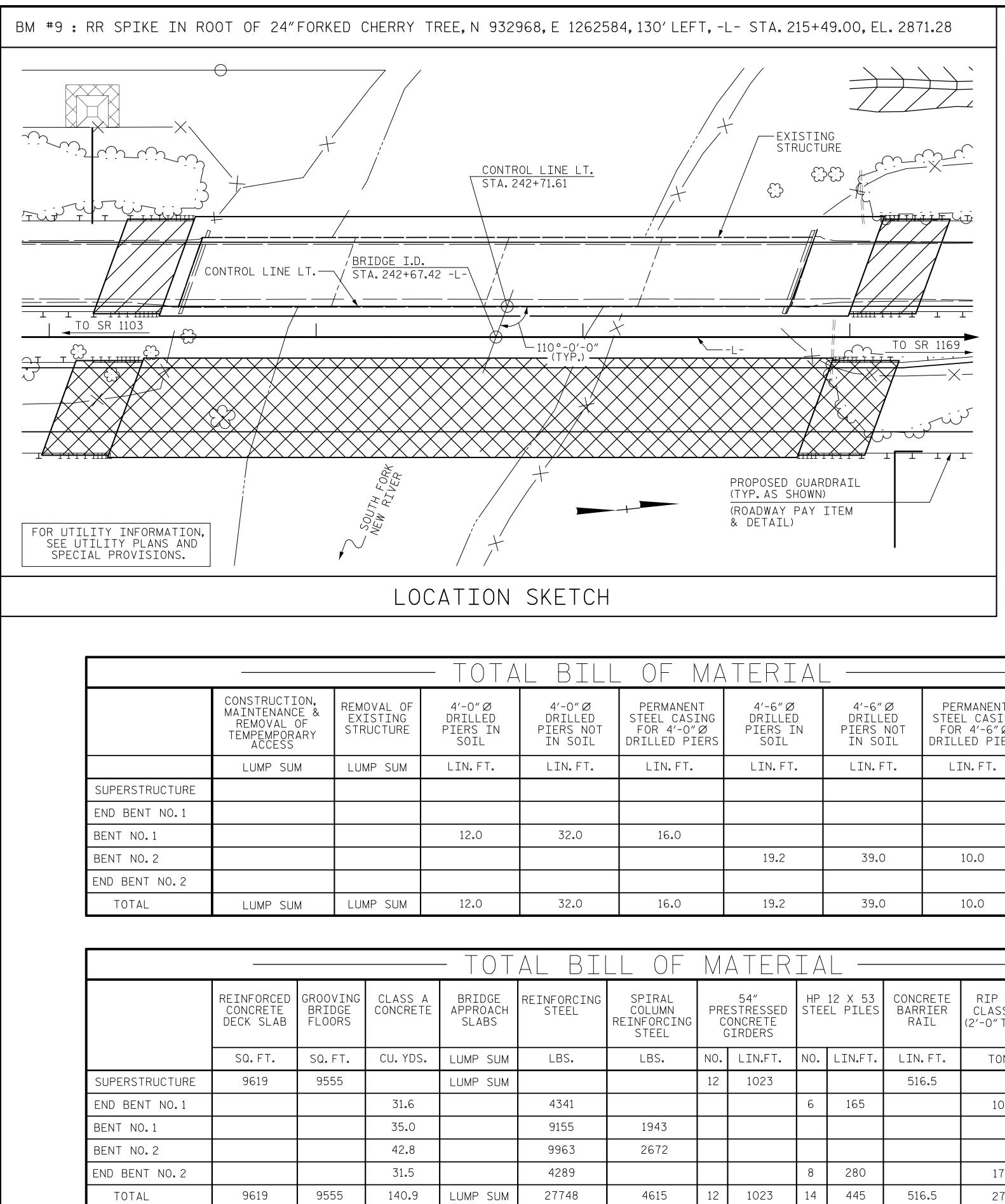
## NOTES:

PER PILE.	INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 2820.4 FT (LT) AN 2815.4 FT (RT) AND WITH THE REQUIRED TIP RESISTANCE.
S PER PILE.	DRILLED PIERS AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 680 TONS PER PI CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 30.0 TSF.
.O FT AND WITH	PERMANENT STEEL CASING WILL BE REQUIRED FOR DRILLED PIERS AT BENT NO.2.DO NOT EXTEND CASING BELOW ELEVATION 2842.0 FT.WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
TONS PER PIER.	THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS ELEVATION 2836.0 FT.THE SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCT
D NOT EXTEND	CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR THE DRILLED PIERS AT BENT AND BENT NO.2. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING.FOR CSL TESTING, SI SECTION 411 OF THE STANDARD SPECIFICATIONS.
	PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
JR CRITICAL F THE STRUCTURE.	DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.



	2.98° 133° 10° 6.0° 4.0°			NCE 14+01.61 -L		
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1121 Situs Court	DE DE CAROLANTINA SE AL 037180		REVIS	SBL		SHEET NO.
Suite 170 Raleigh NC 27606-4279 (919) 859-4880	Backbook & Warter	NO. ВҮ: 1		NO. BY:	DATE:	SO4-2 TOTAL SHEETS
NC Lic. No. F-0270	27289/2015-6470	2		4		35





DRAWN BY :	DATE :_	5/4/14
CHECKED BY : _ R.F. WERTMAN	DATE :_	7/31/14
DESIGN ENGINEER OF RECORD :	DATE :_	10/30/14

+

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AAS LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISION

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMP OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETER PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

REMOVABLE FORMS MAY BE USED IN LIEU OF METAL STAY-IN-PL FORMS IN ACCORDANCE WITH ARTICLE 420-3 OF THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 242+67.42 -L-.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

- MA	TERIAL				
IANENT CASING 4'-0″Ø D PIERS	4'-6″Ø DRILLED PIERS IN SOIL	4'-6"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 4'-6″Ø DRILLED PIERS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION
.FT.	LIN.FT.	LIN.FT.	LIN.FT.	EACH	LUMP SUM
					LUMP SUM
5.0					
	19.2	39.0	10.0		
					LUMP SUM
5.0	19.2	39.0	10.0	1	LUMP SUM

) [-	M	ATER	IΑ				_	
AL 1N CING L	С	54″ ESTRESSED ONCRETE GIRDERS		12 X 53 El PILES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS
	NO.	LIN.FT.	NO.	LIN.FT.	LIN.FT.	TONS	SQ.YDS.	LUMP SUM
	12	1023			516.5			LUMP SUM
			6	165		100	110	
5								
2								
			8	280		176	195	
- )	12	1023	14	445	516.5	276	305	LUMP SUM



## NOTES:

SHTO	THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-5 OF THE STANDARD SPECIFICATIONS.NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.
NS.	THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT.LEFT AND 12 FT.RIGHT OF BRIDGE WORK LINE AS DIRECTED BY THE ENGINEER.THIS WORK WILL BE PAID AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.
PLES UP EACH OF	THE EXISTING STRUCTURE CONSISTING OF FOUR SPANS, 57'-6"FT. EACH, WITH A CLEAR ROADWAY OF 26' ON REINFORCED CONCRETE DECK GIRDERS ON END BENTS WITH REINFORCED CONCRETE CAPS ON TIMBER PILES & INTERIOR BENTS WITH REINFORCED CONCRETE POSTS AND BEAMS LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED.
USED. THE RS.	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
ON	REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
D	THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ``HEC 18 - EVALUATING SCOUR AT BRIDGES''. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

## HYDRAULIC DATA

DESIGN DISCHARGE
FREQUENCY OF DESIGN FLOOD
DESIGN HIGH WATER ELEVATION
DRAINAGE AREA
BASE DISCHARGE (Q100)
BASE HIGH WATER ELEVATION

- = 16,000 C.F.S.
- = 50 YRS. = 2862.2
- = 130 SQ.MI.
- = 19,000 C.F.S. = 2863.0

## OVERTOPPING FLOOD DATA

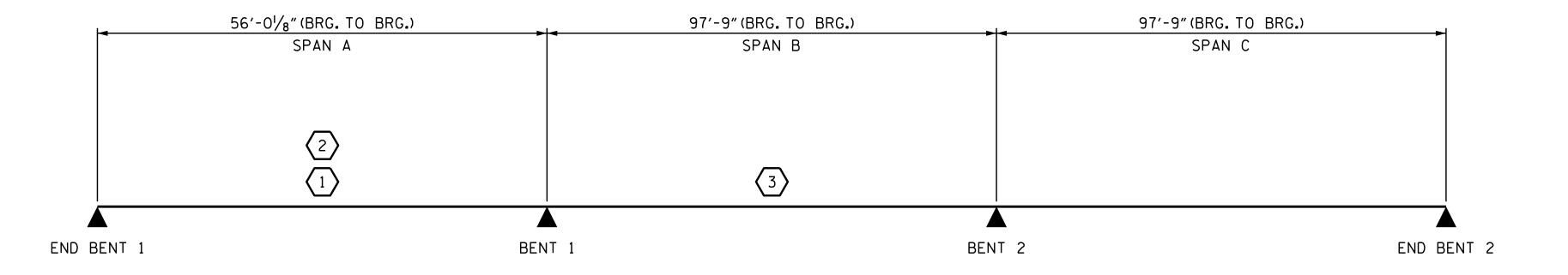
OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION

= 24,000+ C.F.S. = 500+ YRS. = 2869.2

	PROJE	CT NO. Ash		-2915	
	STATI			CO 7.42	UNTY -L-
	SHEET 3 (	)F 3	REI	places br	IDGE #10
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ourt	S	R 1103		SR 1169	
ourt SEAL		REVIS	SIONS		SHEET NO.
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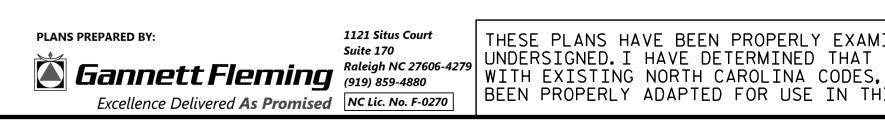
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		LOAD AN	D RES	SIST	ANCE	FAC	TOR	RAT	ING	(LRF	R) SL	JMMA	RY F	OR F	PRES	TRES	SED	CON	CRET	E GI	RDEF	۲S		
										STRE	NGTH	I LIM	IIT ST	TATE				SE	RVICE	III	LIMI	T STA	.ΤE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.08		1.75	0.899	1.08	A	Ŧ	28.0	1.014	1.37	A	Ŧ	45.2	0.80	0.775	1.11	В	Ŧ	48.9	
DESIGN		HL-93 (OPERATING)	N/A	-•-	1.40		1.35	0.899	1.40	A	Ŧ	28.0	1.022	2.24	C	Ŧ	78.6	N/A						
LOAD RATING		HS-20 (INVENTORY)	36.000	2	1.37	49 <b>:</b> 32	1.75	0.899	1.37	A	Ŧ	28.0	10:14	2.09	A	Ŧ	33.8	0.80	0.775	1.56	B	Ŧ	4 <b>8.</b> 9	
		HS-20 (OPERATING)	36.000	-	1.77	63 <b>:</b> 72	1.35	0.899	1.77	A	Ŧ	28.0	1.022	2.99	C	Ŧ	7 <b>8.</b> 6	NZA						
		SNSH	13.500		3.67	49 <b>:</b> 55	1.40	0.899	3 <b>.</b> 67	A	Ŧ	28.0	1.022	7.65	C	Ŧ	7 <b>8.</b> 6	0.80	0.775	3.70	B	Ŧ	4 <b>8.</b> 9	
	Ш	SNGARBS2	20.000	-	2.67	53 <b>:</b> 40	1.40	0.899	2 <b>.</b> <del>8</del> 1	A	Ŧ	28.0	1.022	5.35	C	Ŧ	7 <b>8.</b> 6	0.80	0.775	2.67	B	Ŧ	4 <b>8.</b> 9	
	ICL	SNAGRIS2	22.000	-•-	2.50	55 <b>:</b> 00	1.40	0.899	2.70	A	Ŧ	2 <b>8.</b> 0	1.022	4.94	B	Ŧ	78.6	0.80	0.775	2.50	В	Ŧ	4 <b>8.</b> 9	
	VEH	SNCOTTS3	27.250	-•-	1.83	49 <b>:</b> 87	1.40	0.899	1.83	A	Ŧ	2 <b>8.</b> 0	1.022	3.67	C	Ŧ	78.6	0.80	0.775	1.84	B	Ŧ	48.9	!
	CLE (S	SNAGGRS4	34.925		1.50	52 <b>:</b> 39	1.40	0.899	1.56	A	Ŧ	28.0	1.014	2.75	A	Ŧ	33.8	0.80	0.775	1.50	В	Ŧ	48.9	[!
	SINGL	SNS5A	35 <b>.</b> 550		1.47	52 <b>:</b> 26	1.40	0.899	1.52	A	Ŧ	2 <b>8.</b> 0	1.022	2.88	В	Ŧ	19.1	0.80	0.775	1.47	В	Ŧ	48 <b>.</b> 9	
		SNS6A	39.950		1.34	53 <b>:</b> 53	1.40	0.899	1.41	A	Ŧ	2 <b>8.</b> 0	1.022	2.60	В	Ŧ	19 <b>.</b> 1	0.80	0.775	1.34	В	Ŧ	48.9	
LEGAL LOAD		SNS7B	42.000	-•-	1.27	53 <b>:</b> 34	1.40	0.899	1.34	A	Ŧ	2 <b>8.</b> 0	1.022	2.49	C	Ŧ	7 <b>8.</b> 6	0.80	0.775	1.27	B	Ŧ	4 <b>8.</b> 9	
LOAD RATING	LER	TNAGRIT3	33.000	-•-	1.63	53 <b>:</b> 79	1.40	0.899	1.72	A	Ŧ	2 <b>8.</b> 0	1.022	3.09	B	Ŧ	19.1	0.80	0.775	1.63	B	Ŧ	4 <b>8.</b> 9	
	TRAI	TNT4A	33 <b>.</b> 075	-•-	1.63	53 <b>.</b> 91	1.40	0.899	1.73	A	Ŧ	28.0	1.022	3.26	B	Ŧ	78.6	0.80	0.775	1.63	B	Ŧ	4 <b>8.</b> 9	
	- I M	TNT6A	41.600	-•-	1.32	54 <b>.</b> 91	1.40	0.899	1.43	A	Ŧ	28.0	1.022	2.68	B	Ŧ	19.1	0.80	0.775	1.32	B	Ŧ	4 <b>8.</b> 9	<u>_</u> '
	R SE	TNT7A	42.000	-•-	1.32	55 <b>:</b> 44	1.40	0.899	1.44	A	Ŧ	2 <b>8.</b> 0	1.022	2.63	В	Ŧ	19.1	0.80	0.775	1.32	B	Ŧ	4 <b>8.</b> 9	
	CTOR (TT	TNT7B	42.000	-•-	1.35	56 <b>:</b> 70	1.40	0.899	1.51	A	Ŧ	28.0	1.022	2.48	B	Ŧ	19.1	0.80	0.775	1.35	B	Ŧ	4 <b>8.</b> 9	
	TRA	TNAGRIT4	43.000	-•-	1.30	55 <b>:</b> 90	1.40	0.899	1.42	A	Ŧ	28.0	1.022	2.44	B	Ŧ	19.1	0.80	0.775	1.30	B	Ŧ	4 <b>8.</b> 9	
	TRUCK	TNAGT5A	45.000	+	1.23	55 <b>:</b> 35	1.40	0.899	1.34	A	Ŧ	28.0	1.022	2.33	B	Ŧ	19.1	0.80	0.775	1.23	B	Ŧ	4 <b>8.</b> 9	
	TR	TNAGT5B	45.000	3	1.22	54 <b>:</b> 90	1.40	0.899	1.32	A	Ŧ	2 <b>8.</b> 0	1.014	2.03	A	Ŧ	33.8	0.80	0.775	1.22	В	Ŧ	4 <b>8.</b> 9	1



ASSEMBLED BY : T.J. KIRSCH CHECKED BY : R.F. WERTMAN	
DRAWN BY : MAA 1/08 CHECKED BY : GM/DI 2/08	REV.II/12/08RR MAA/GM REV.IO/1/II MAA/GM

<u>LRFR SUMMARY</u>



## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

## COMMENTS:

1. SPANS B AND C LENGTHS AND RATING FACTORS ARE EQUAL.

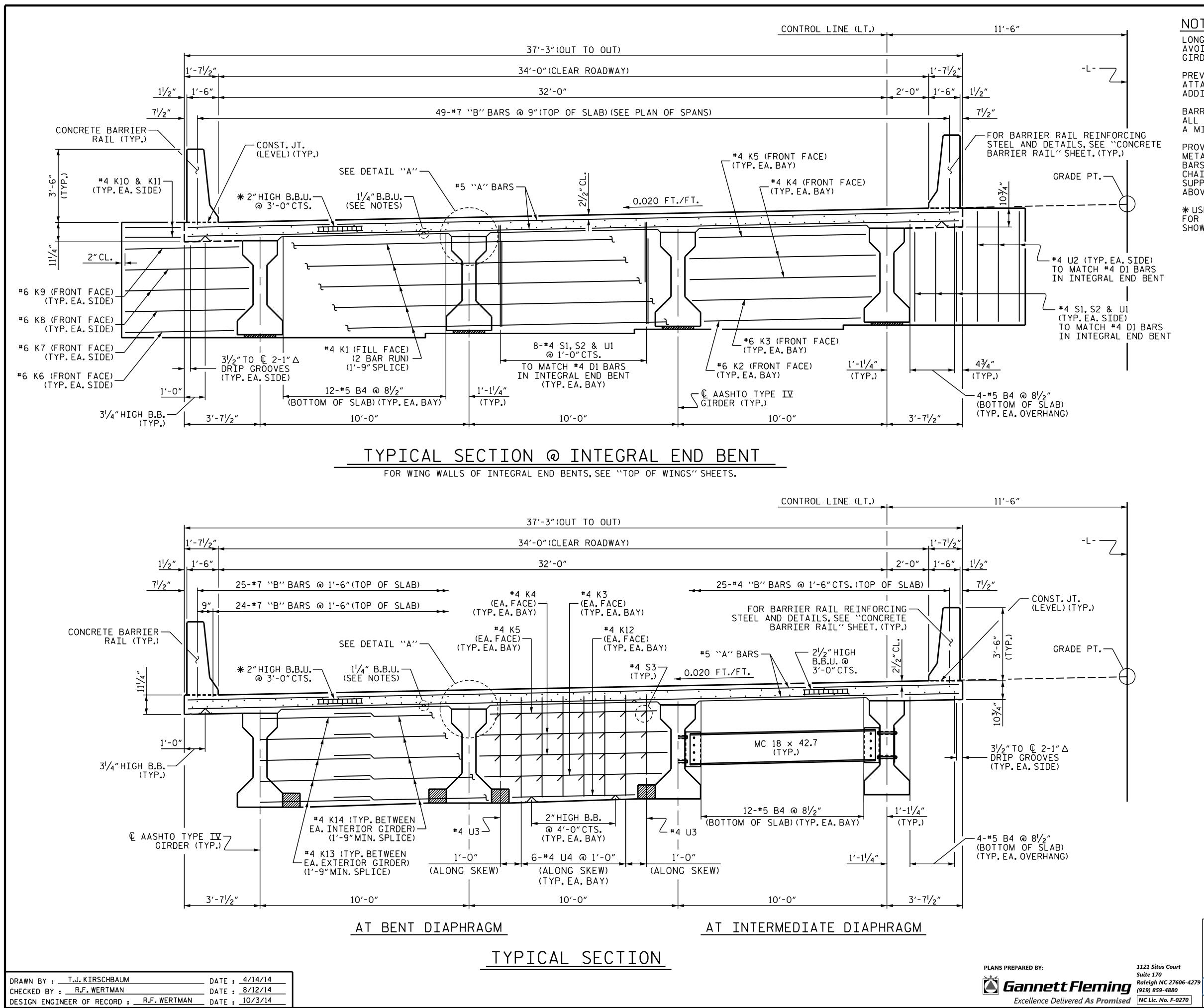
2. THE REDUCTION OF LOAD DISTRIBUTION FACTOR FOR MOMENT IN LONGITUDINAL BEAMS ON SKEWED SUPPORTS (AASHTO TABLE 4.6.2.2.2E-1) WAS NOT APPLIED.

<pre>(#) CONTROLLING LOAD RATING</pre>
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
$\sqrt{3}$ LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

		PROJEC	T NO		-2915	
		STATI	<u>ASHE</u> 24: 24:			UNTY -L-
			RTMENT O STA	RALEIGH	<b>nsporta</b> RD	
	NOR OF ESSION AND THE	СС	FR SU PRES NCRET	TRES E G	SSED IRDEI	RS
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THEY COMPLY , AND HAVE HIS AREA.	2/20/2013 5470	NO. BY: 1 2	Date: No 33 الم	,	DATE:	TOTAL SHEETS 35
		STR.NO.4	ST	D.NO	.LRFR1	



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## NOTES:

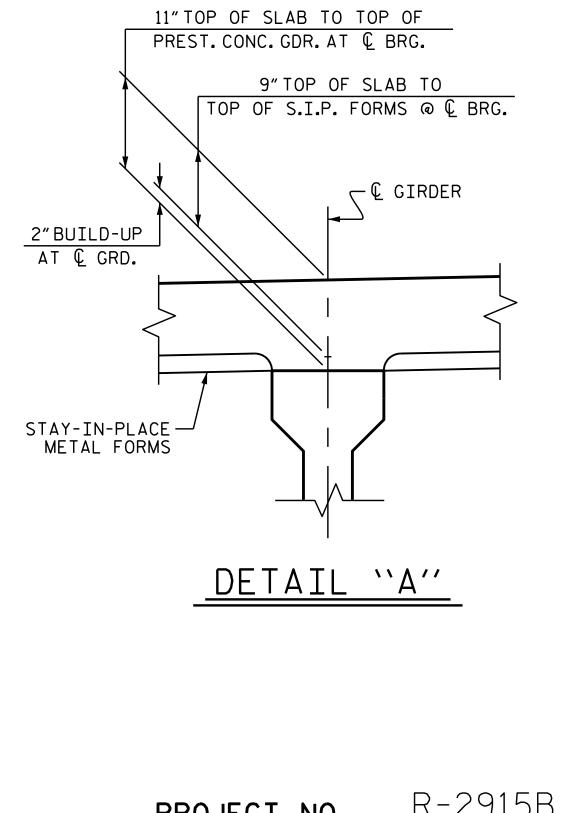
LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

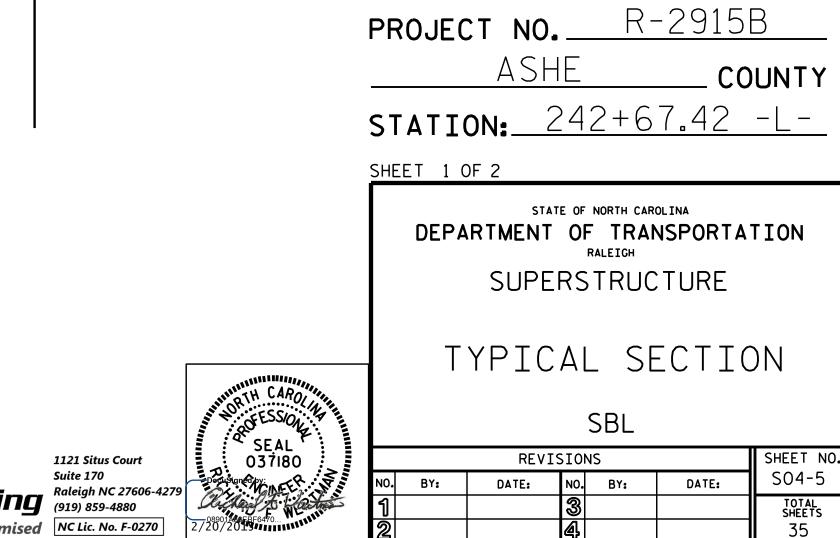
PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

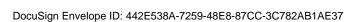
BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

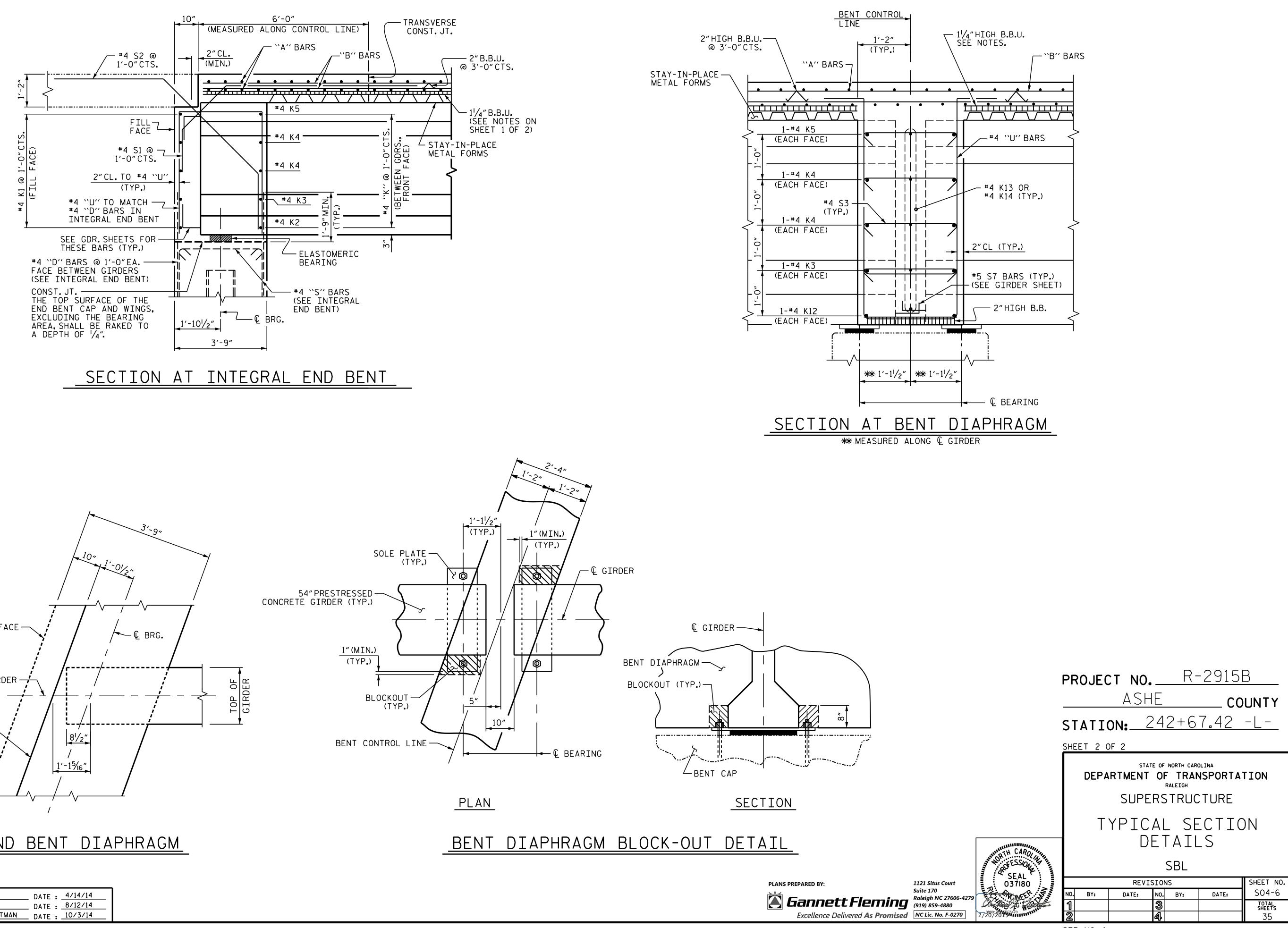
PROVIDE 11/4" HIGH BEAM BOLSTERS UPPER AT 4'-O"CTS. ATOP THE METAL STAY-IN-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS. WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DECK (C.H.C.M.) @ 4'-O"CTS. WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF  $2^{1}/_{2}$ " ABOVE THE TOP OF THE REMOVABLE FORM.

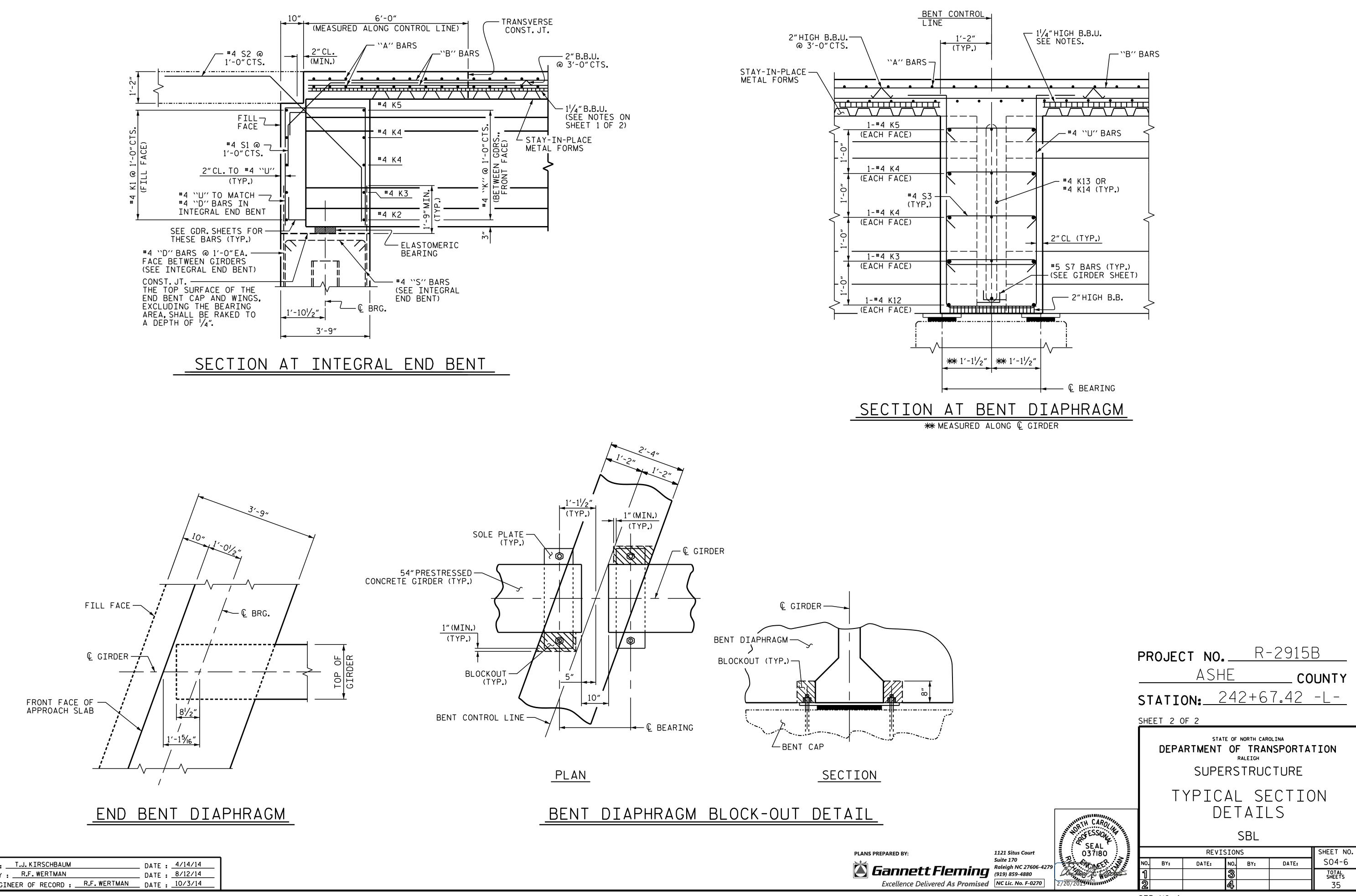
\* USE THIS SIZE BAR SUPPORT IN THE AREAS WITH #7 ``B'' BARS. FOR OTHER AREAS WITH #4 ``B'' BARS, USE THE BAR SUPPORT AS SHOWN IN TYPICAL SECTION AT INTERMEDIATE DIAPHRAGM.



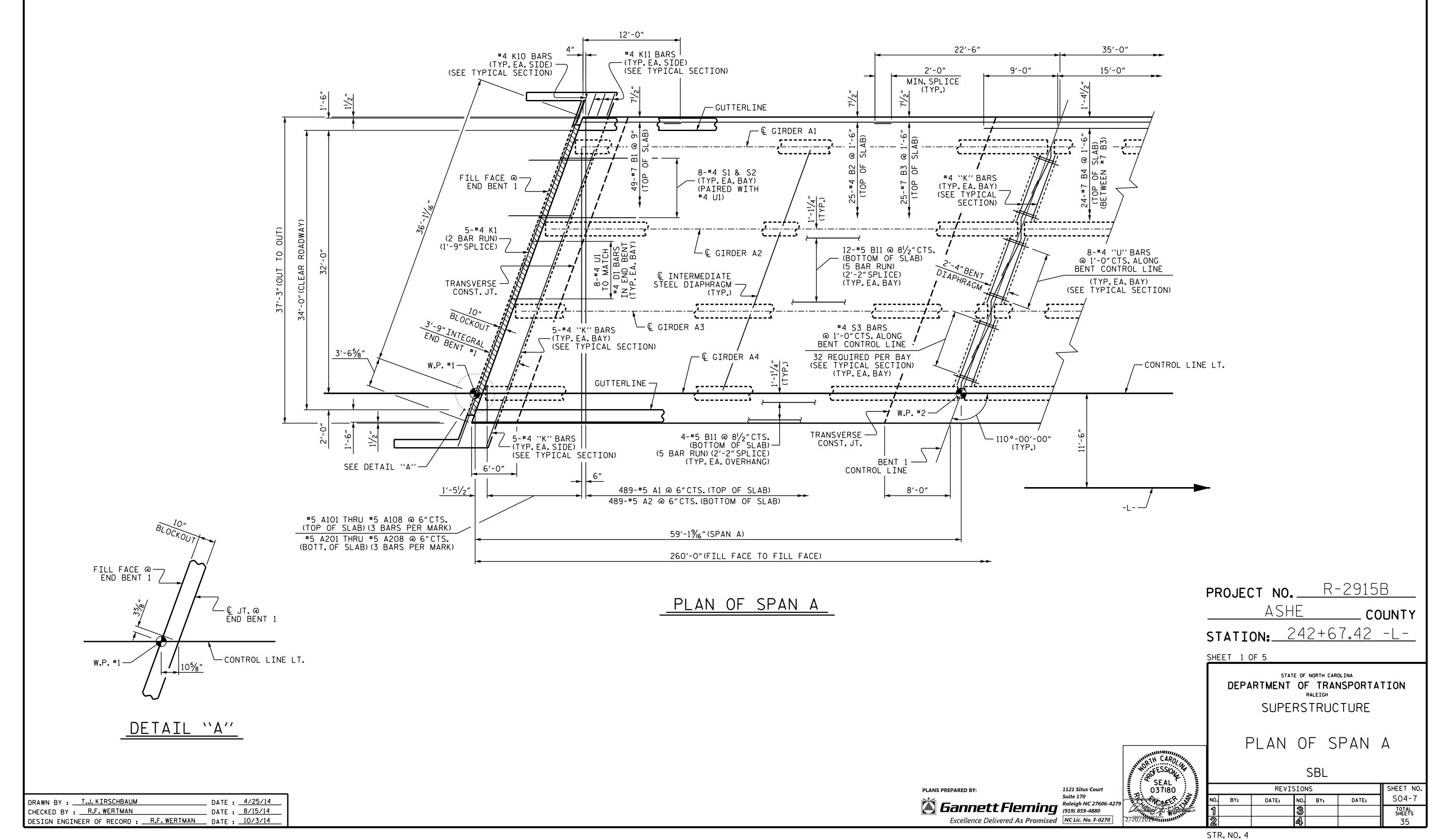


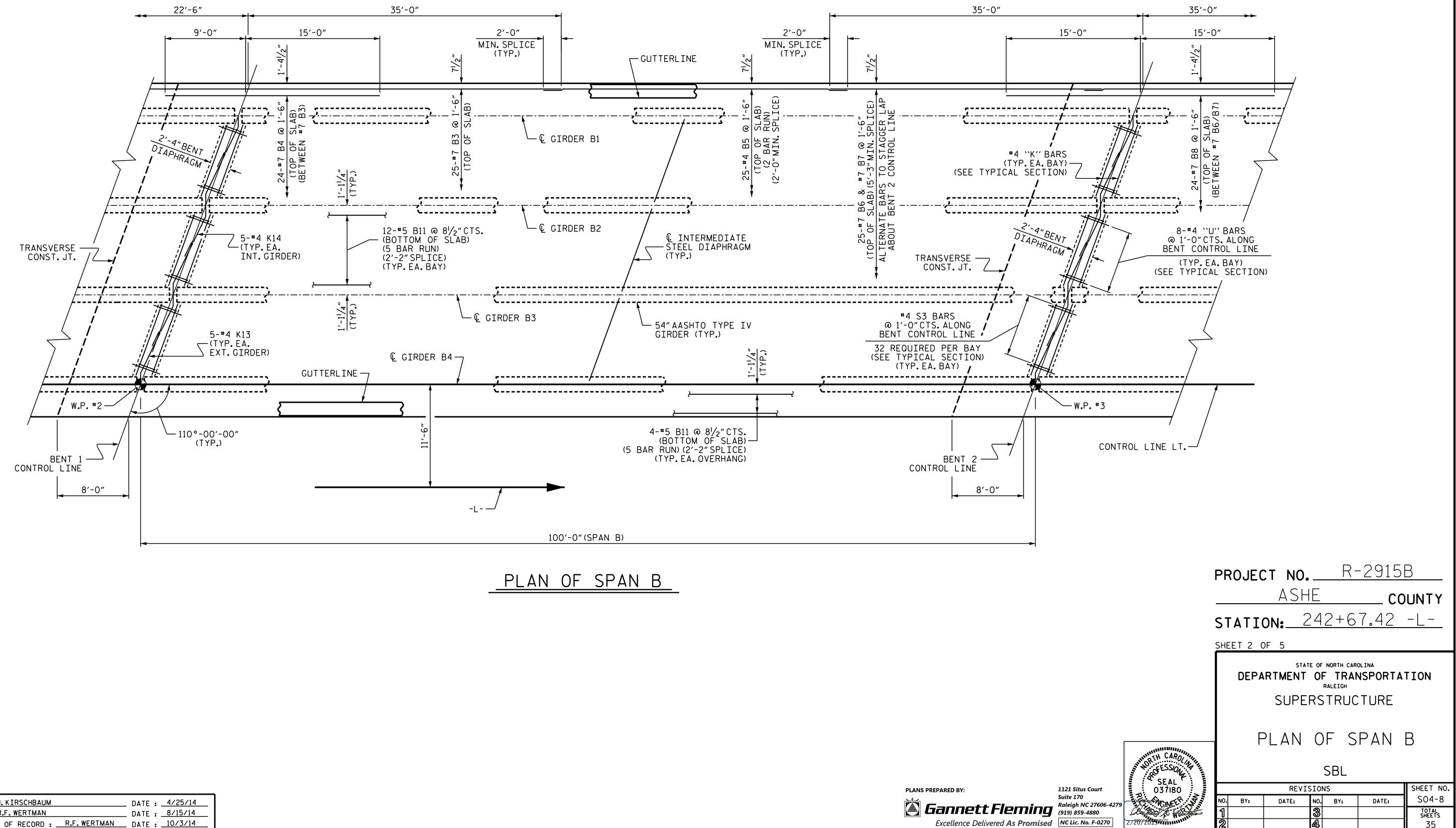




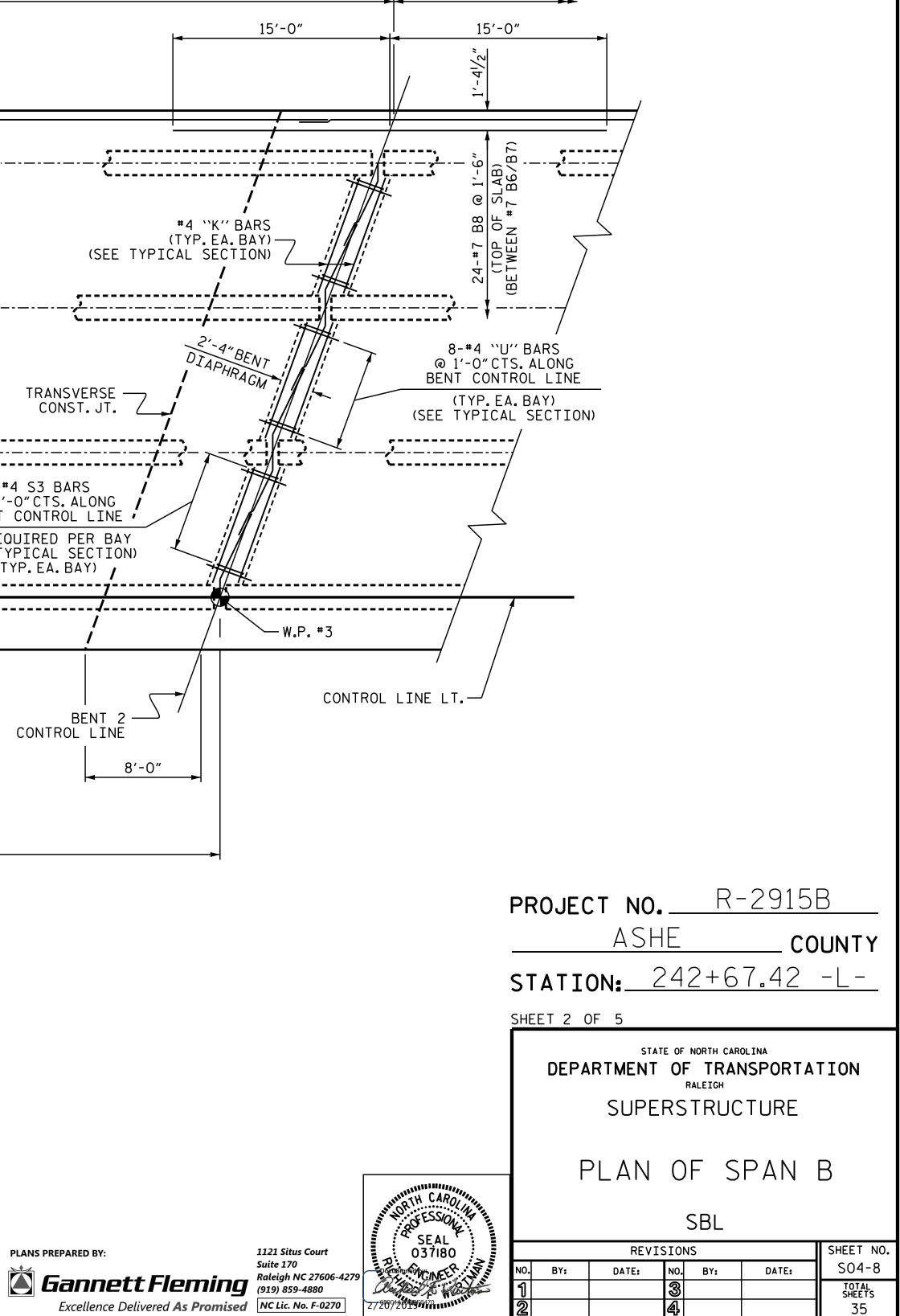


DRAWN BY :	T.J. KIRSCHBAUM	_ DATE : <u>4/14/14</u>
CHECKED BY : .	R.F. WERTMAN	_ DATE : <u>8/12/14</u>
DESIGN ENGINE	EER OF RECORD :R.F. WERTMAN	_ DATE : <u>10/3/14</u>

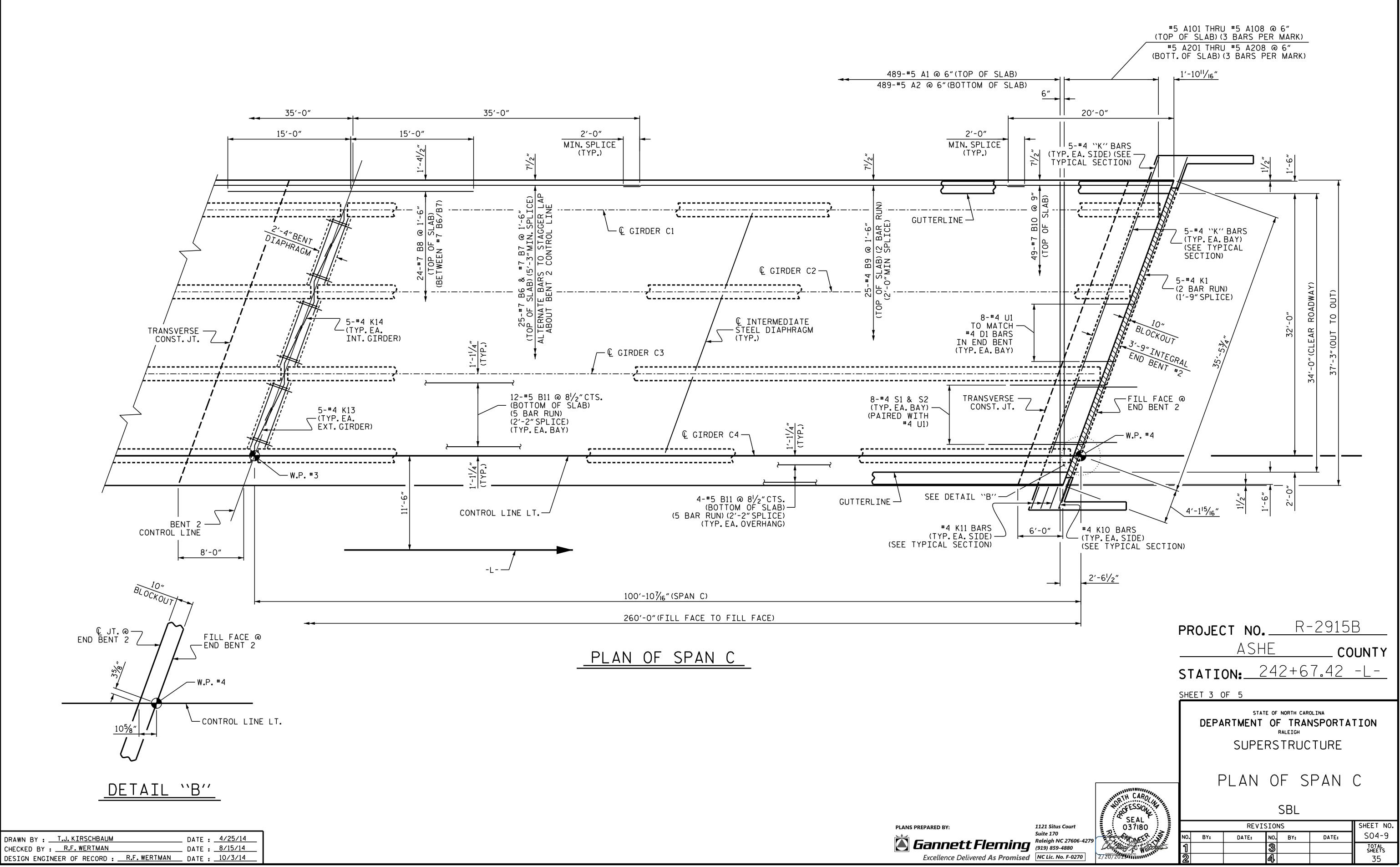


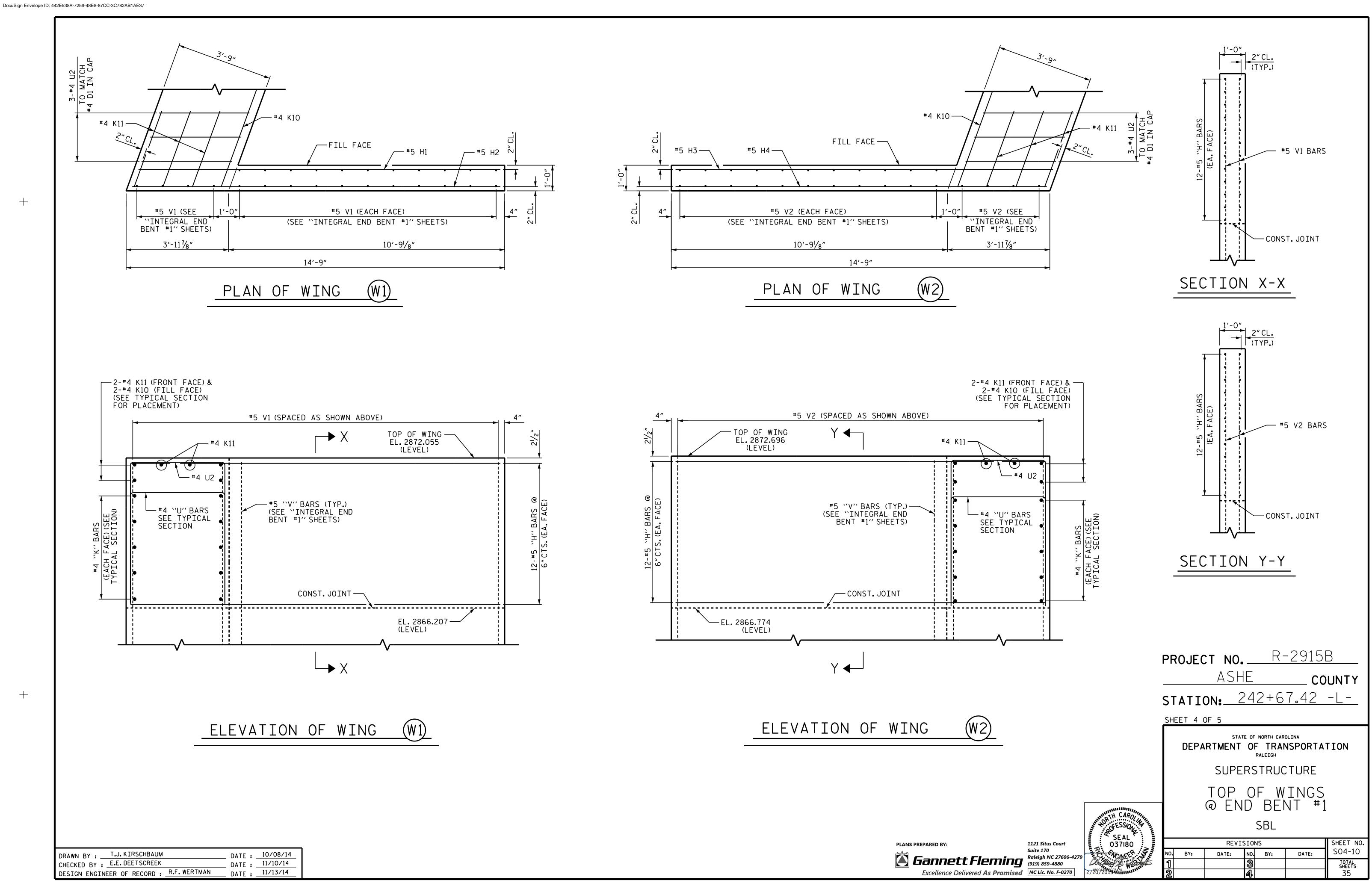


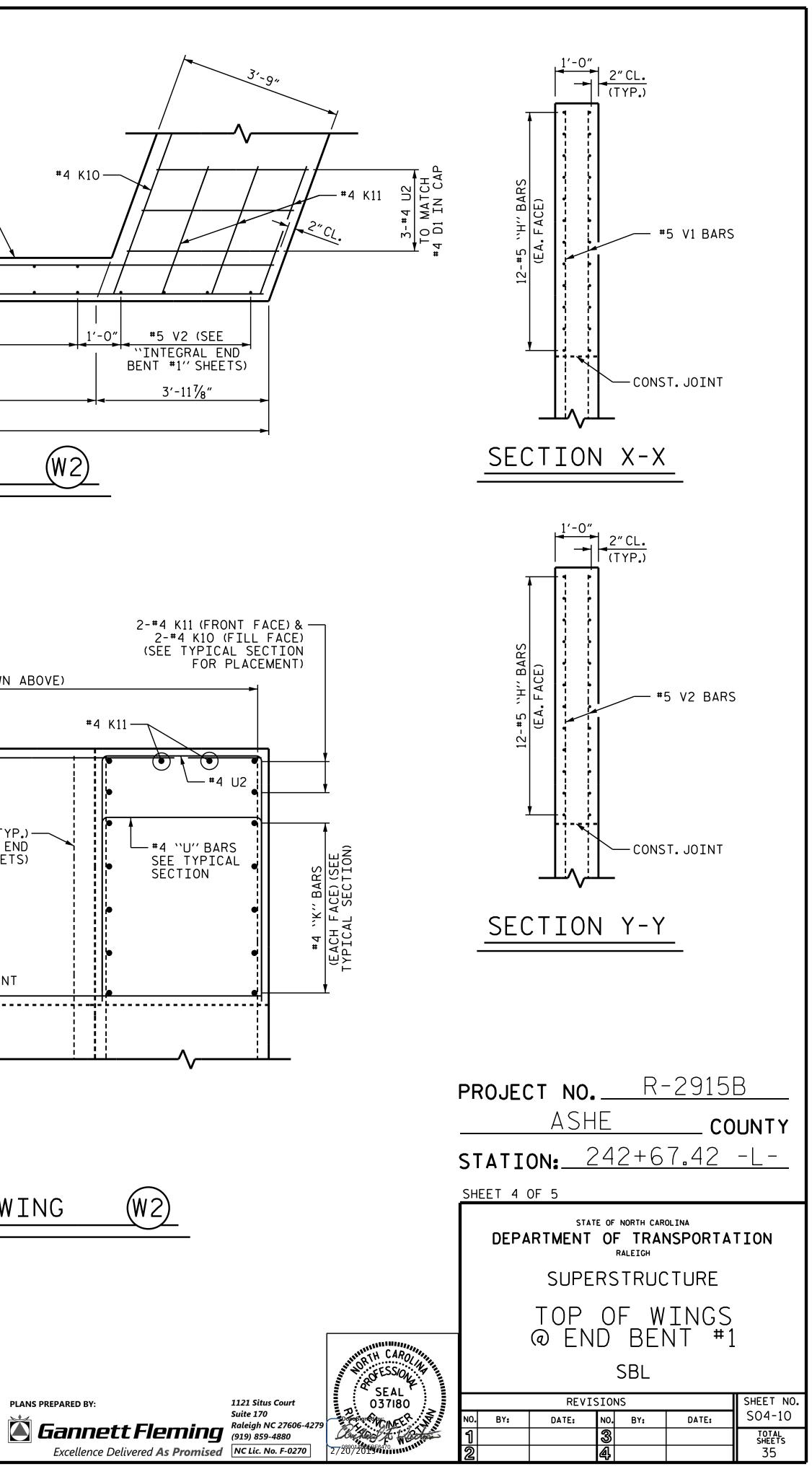
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CHECKED BY :	_ DATE : <u>8/15/14</u>
DESIGN ENGINEER OF RECORD :	_ DATE : <u>10/3/14</u>



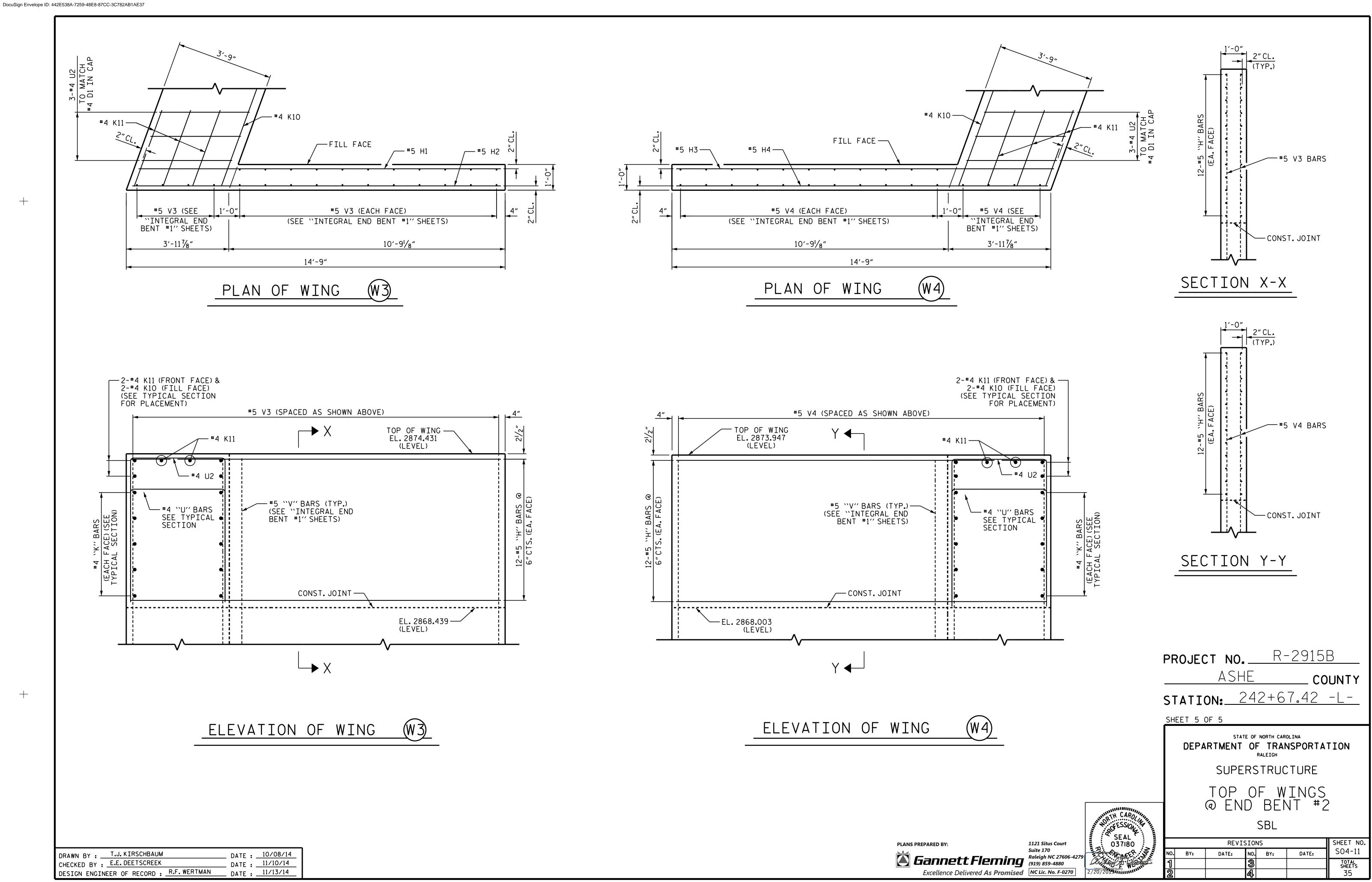
STR.NO.4





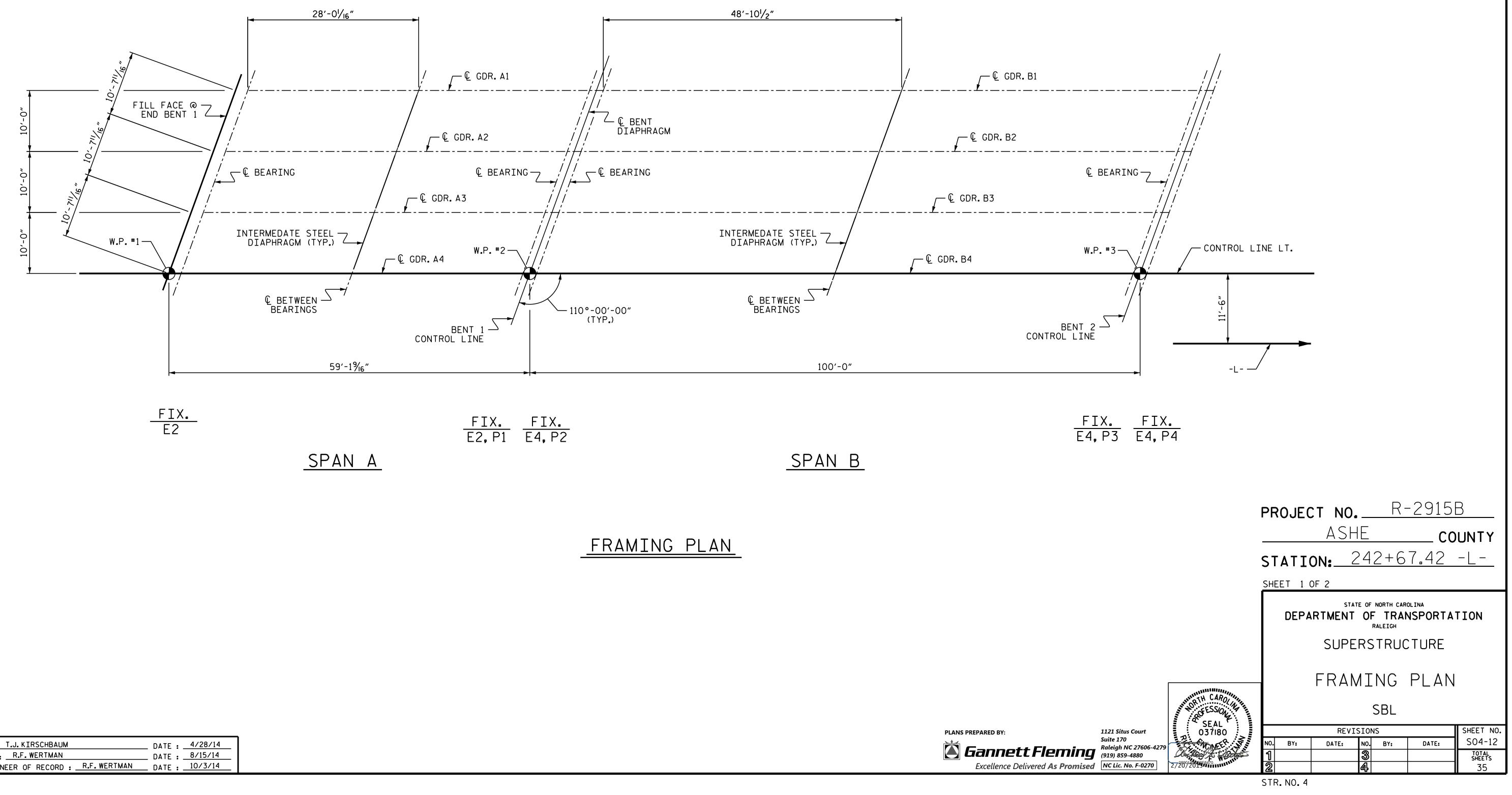


STR.NO.4

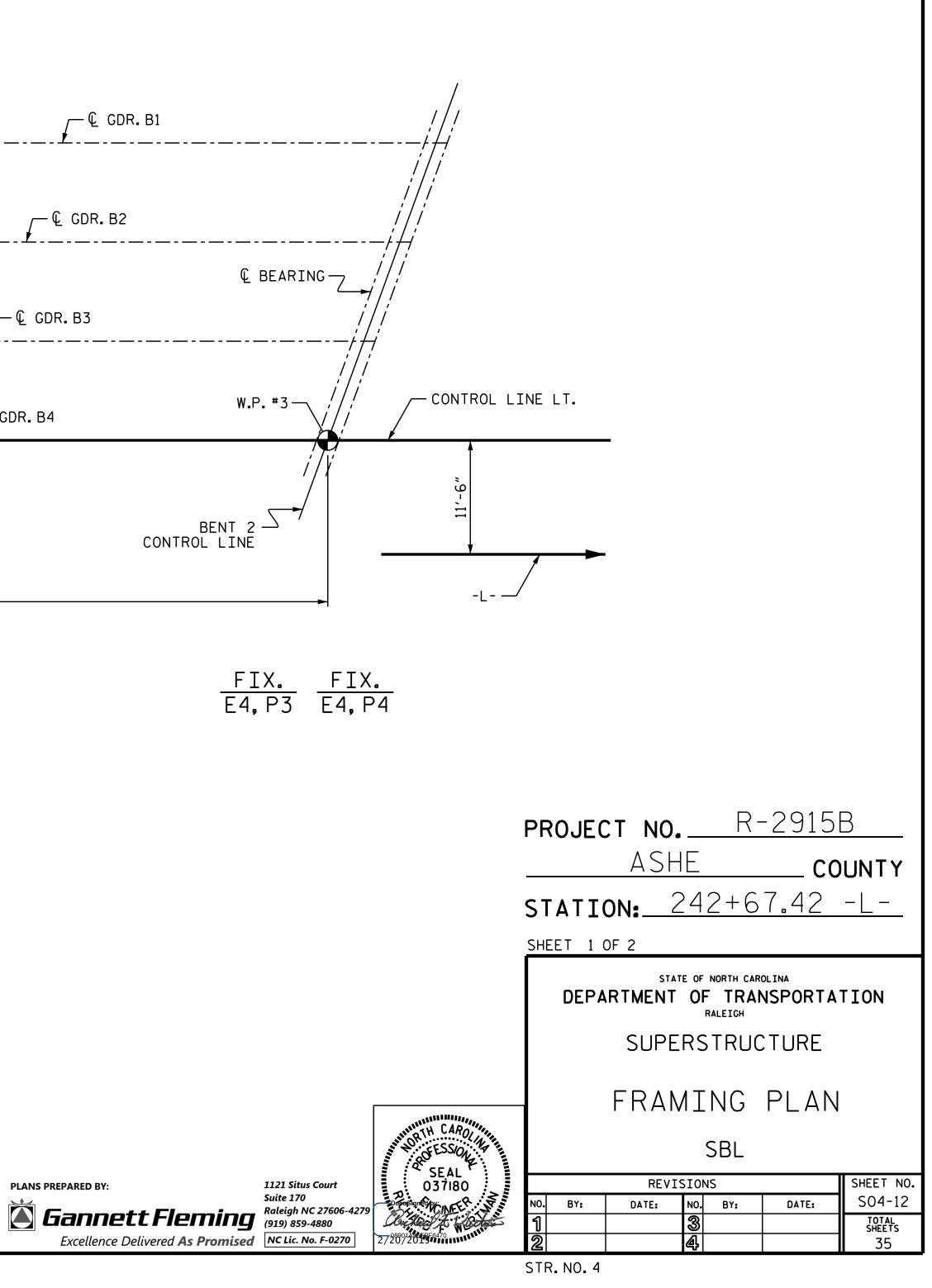


DRAWN BY :	DATE : _	10/08/14
CHECKED BY : E.E. DEETSCREEK	DATE :	11/10/14
	DATE : _	11/13/14

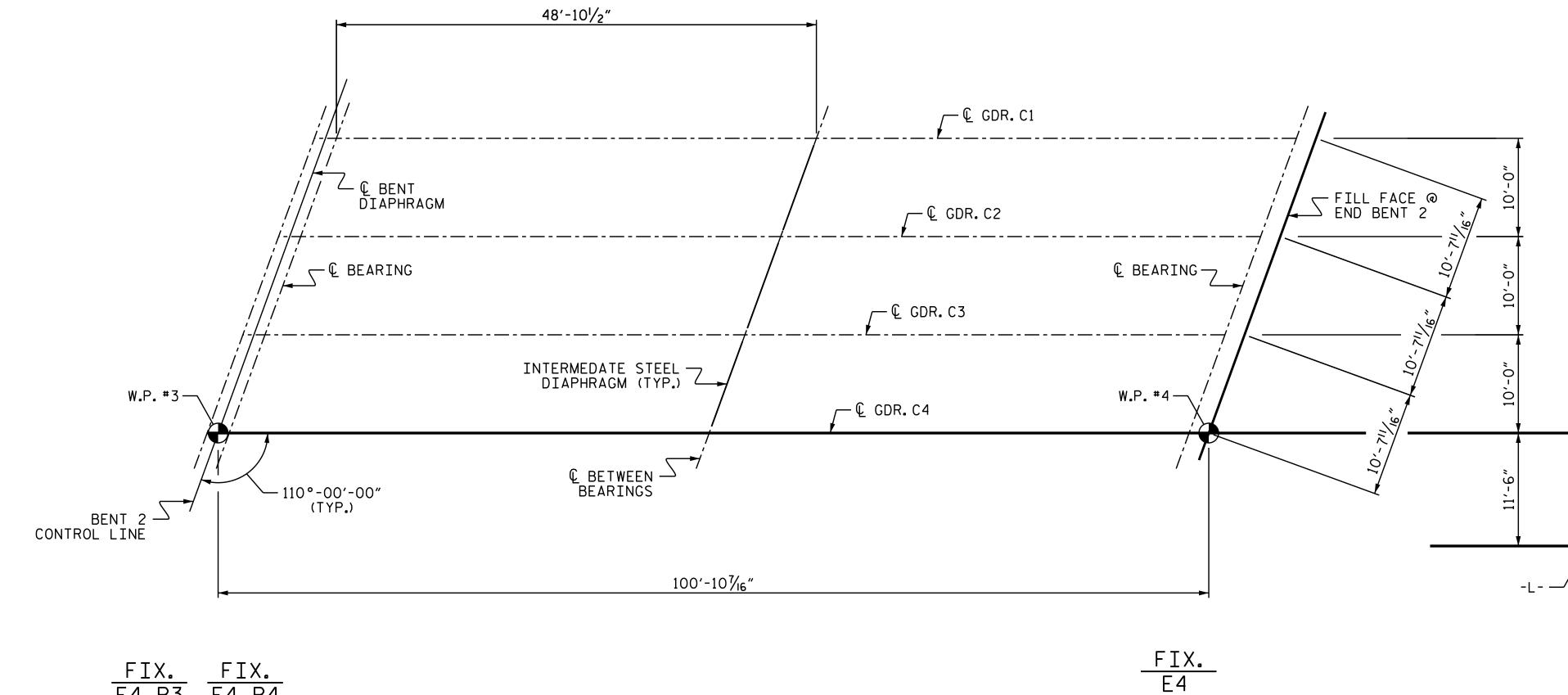
STR.NO.4



DRAWN BY :	DATE :	4/28/14
CHECKED BY : R.F. WERTMAN	DATE :	8/15/14
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE :	10/3/14



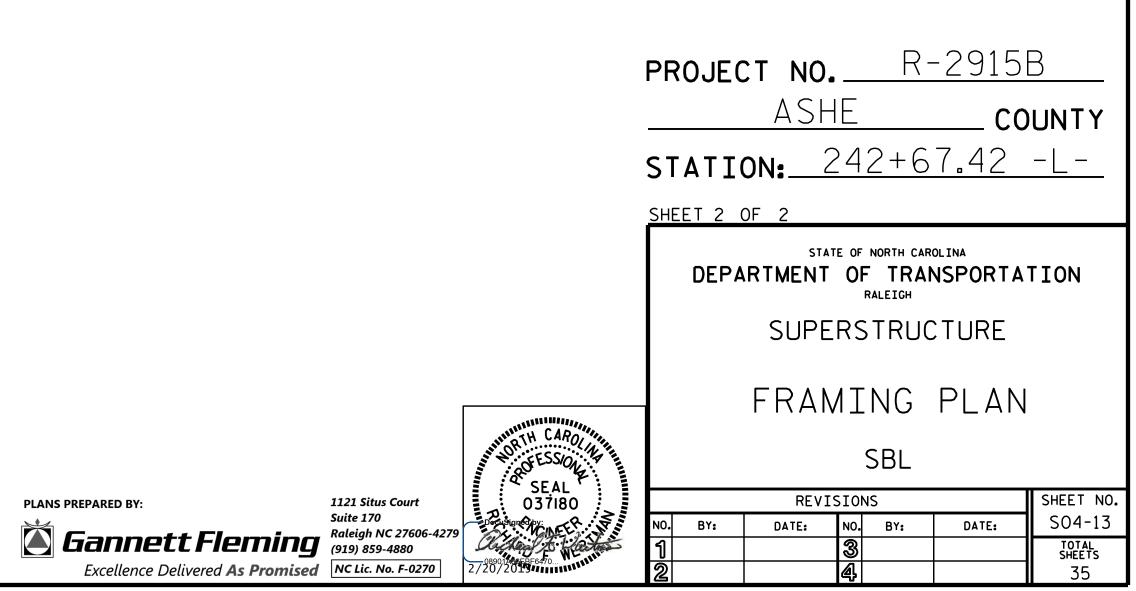
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DRAWN BY :	_ DATE : <u>4/28/14</u>
CHECKED BY : R.F. WERTMAN	DATE : 8/15/14
DESIGN ENGINEER OF RECORD :	

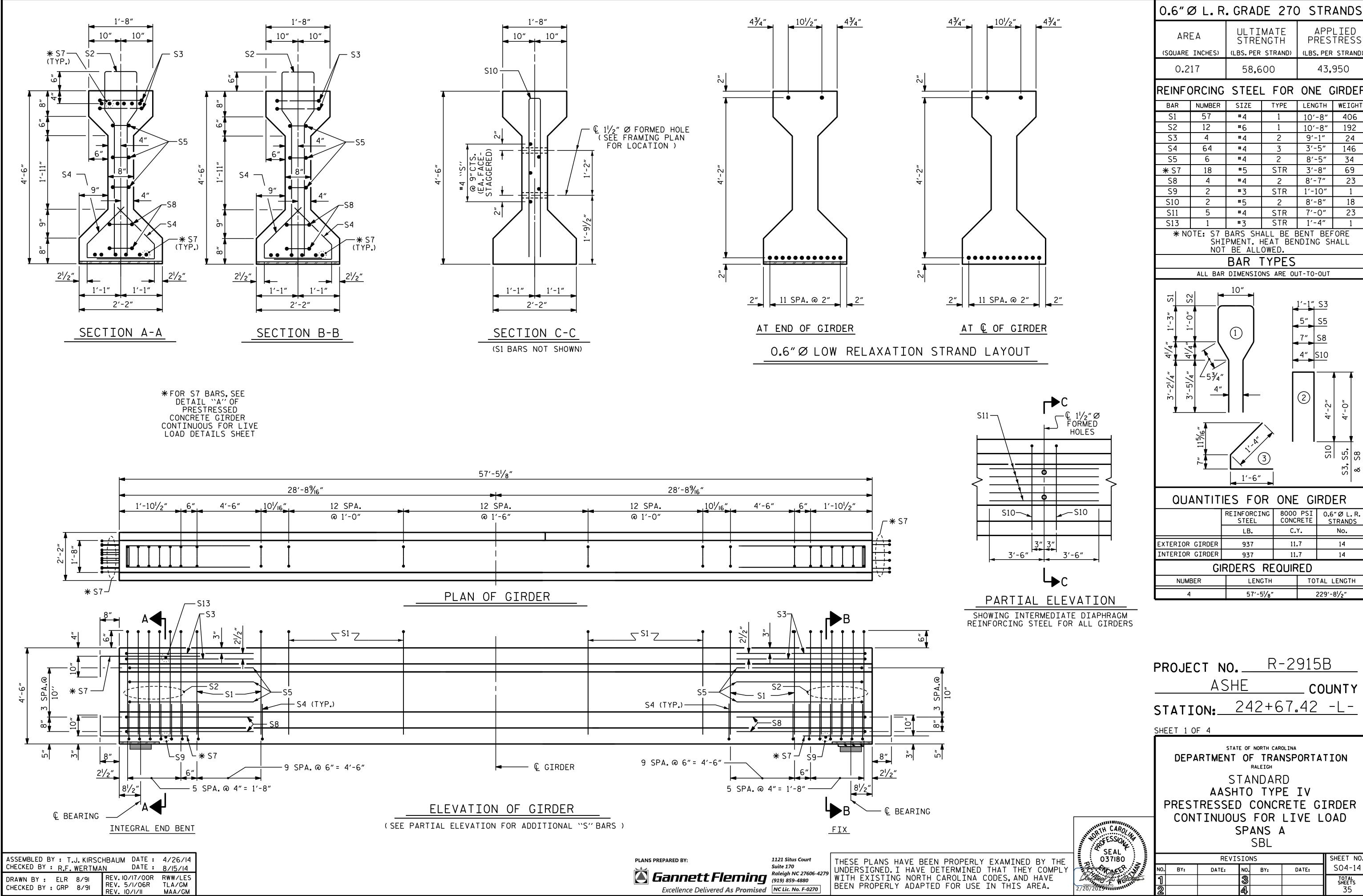
## SPAN C

FRAMING PLAN



- CONTROL LINE LT.

STR.NO.4



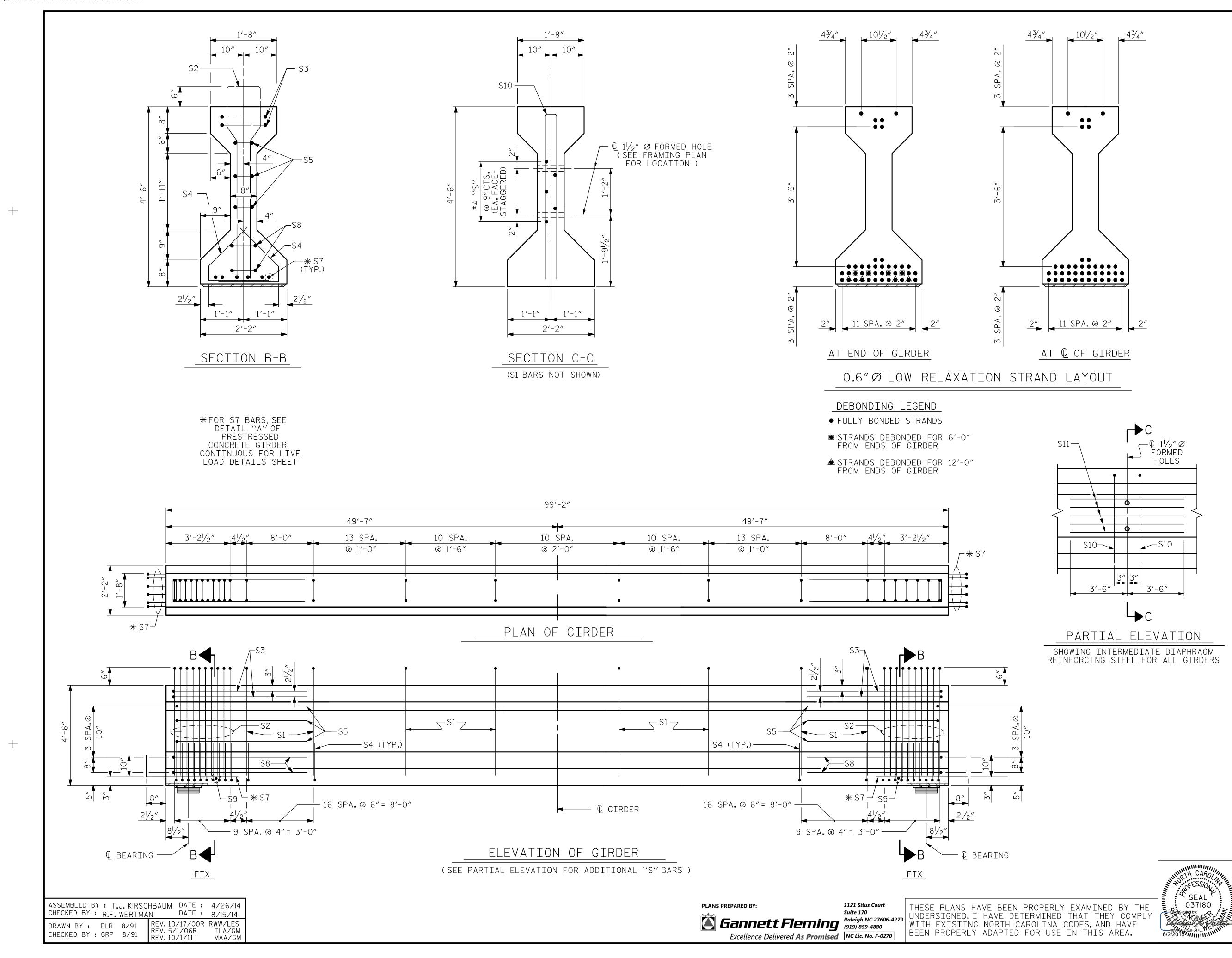
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a annell Fierning	(919) 859-4880	WITH EXISTING	HAVE DETERI NORTH CARO	MINED THAT THILINA CODES, A
Excellence Delivered As Promised		BEEN PROPERLY	ADAPTED FO	R USE IN THIS

STR.NO.4

STD. NO. PCG6

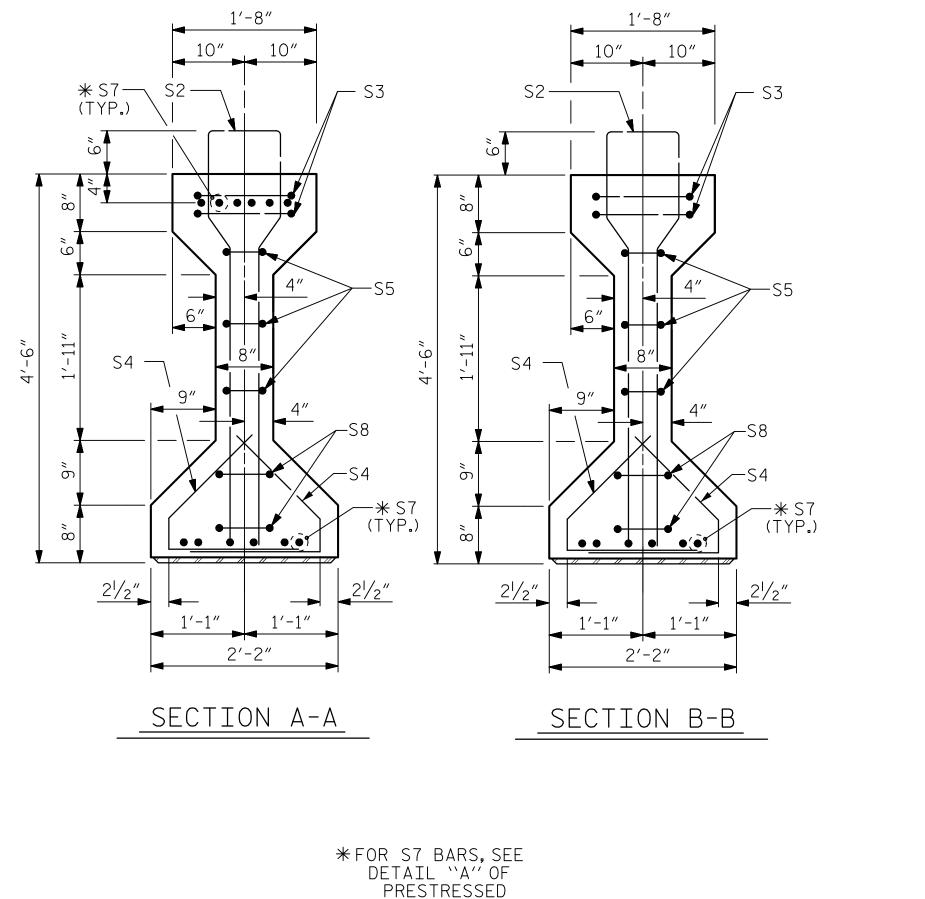
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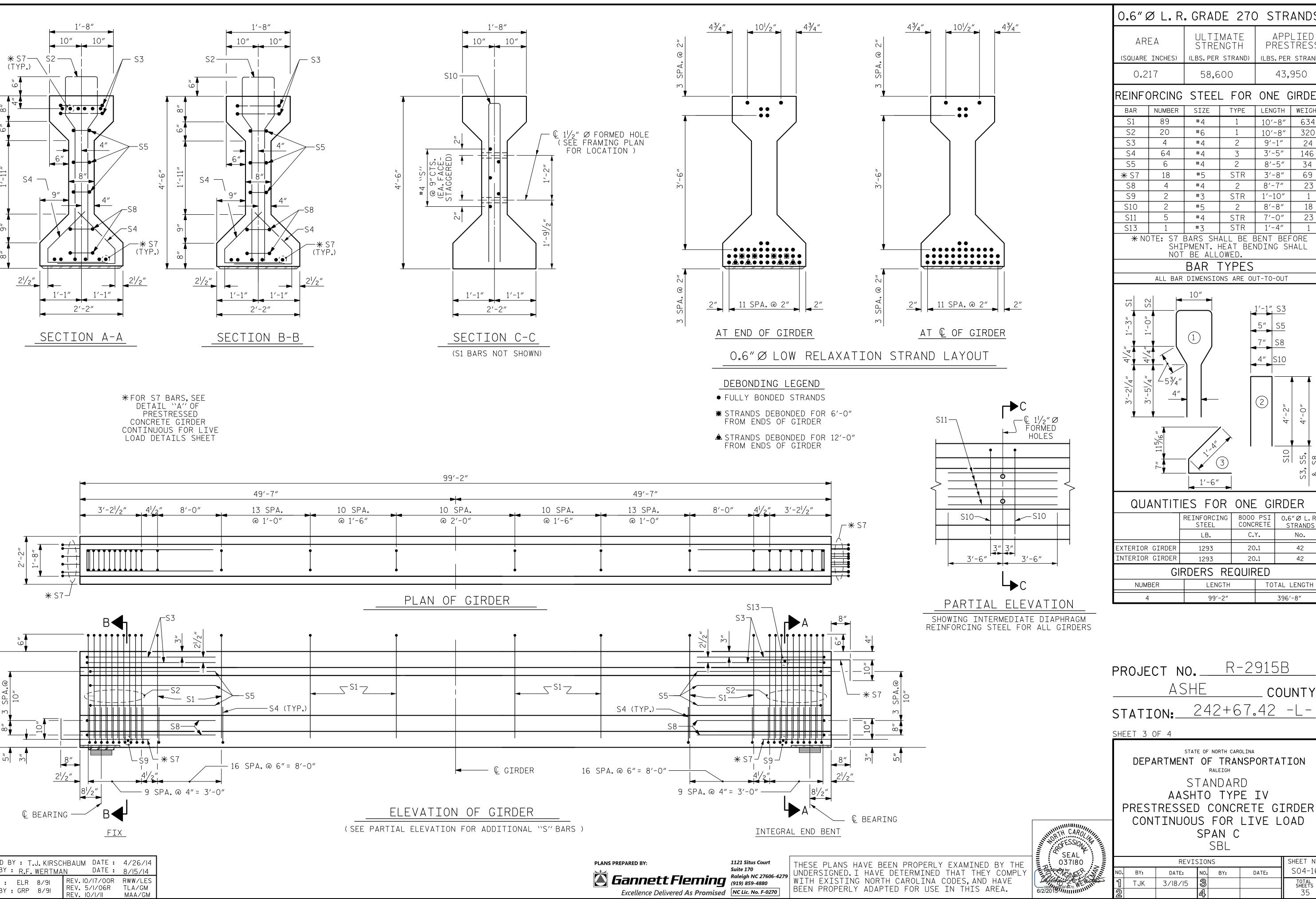
	0.6″	ØL.R	. GRA	DE 27	'0 ST	RANDS	
				ΜΑΤΕ	ΔF	PPLIED	
	ARI	<u>–</u> A	STRENGTH			PRESTRESS	
	(SQUARE	INCHES)	(LBS. PER	STRAND)	(LBS.F	PER STRAND)	
	0.2	17	58,6	00	4	3,950	
	REINFO	DRCING	STEEI	_ FOR	ONE ONE	GIRDER	
	BAR	NUMBER	SIZE	TYPE	LENGT	H WEIGHT	
	S1	89	#4	1	10'-8		
	S2 S3	20 4	#6 #4	1	<u> </u>		
		64	#4 #4	3	3'-5'		
	S5	6	#4	2	8'-5		
	<b>*</b> S7	12	#5	STR	3′-8	″ 46	
	S8	4	#4	2	8'-7'		
	S9 S10	2	#3 #5	STR 2	1'-10 8'-8		
	S10	5	#3 #4	Z STR	7'-0		
		TE: S7 I	BARS SH	ALL BE	BENT E	BEFORE	
		SHI NOT		HEAT BE OWED.	ENDING	SHALL	
		1101	BAR	TYPES	5		
		ALL BAR	DIMENSIO			UT	
			10″ _				
	S1				. 1′-1″. S	٦	
					1′-1″ S  ◀ ▶	<u> </u>	
	1'-3" 1'_0"				5″ S	5	
	↓		(1)		7″ S	8	
	1/4 "					_	
	4				4″ S1	0	
	, 4 , ,	× 453/4"			<b></b>	<b>A A</b>	
	3'-2 <sup>1</sup> /4" 3'-5 <sup>1</sup> /4"	0 4″					
	ά Ύ	ם 🗕	┥┝		(2)		
	<u> </u>	.♥				4'-2" 4'-0"	
		≈ ∔	/	<b>`</b> \			
		115/6"	A"	$\checkmark$		<u> </u>	
		11		$\sim$		S10 S5, S8	
		" <u></u> _		3)		0,0,	
		-	1'-6"			83 <b>.</b> 8	
	QU	ANTITI	ES FO	<u>R ON</u>	E GIR	DER	
			REINFORCI STEEL		O PSI ICRETE	0.6″ØL.R. STRANDS	
			LB.	(	C.Y.	No.	
	EXTERIOR		1269		20.1	42	
	INTERIOR		1269			42	
			RDERS				
	NUM		LEN			L LENGTH	
	4		99′-	-'Z''	] 3	96′-8″	
	PROJE	ICT N	0	$H^{-2}$	2915	<u> </u>	
		$\nabla <$	SHE		$\sim$		
		<u> </u>		~		UNTY	
	STAT	EON:	242	<u>+67</u>	.42	- <u> </u>	
	SHEET 2	UF 4					
			STATE OF NOF			<b>T T A A A</b>	
	DEF	PARTME	NT OF	TRANS	PORTA	TION	
			STAN				
		۸ ۸	STANI SHTO		ΤV		
	PPEC		SED CC				
			OUS F				
-					LVCL	UAU	
			SPA SF	n b 3L			
WILLING VA	NO. BY:	RE DATE:	EVISIONS	<sub>37</sub> .	DATE:	SHEET NO. SO4-15	
5	NO. BY: 1] TJK	3/18/1		BY:	DATE:	TOTAL SHEETS	
	2		。 《4			35	

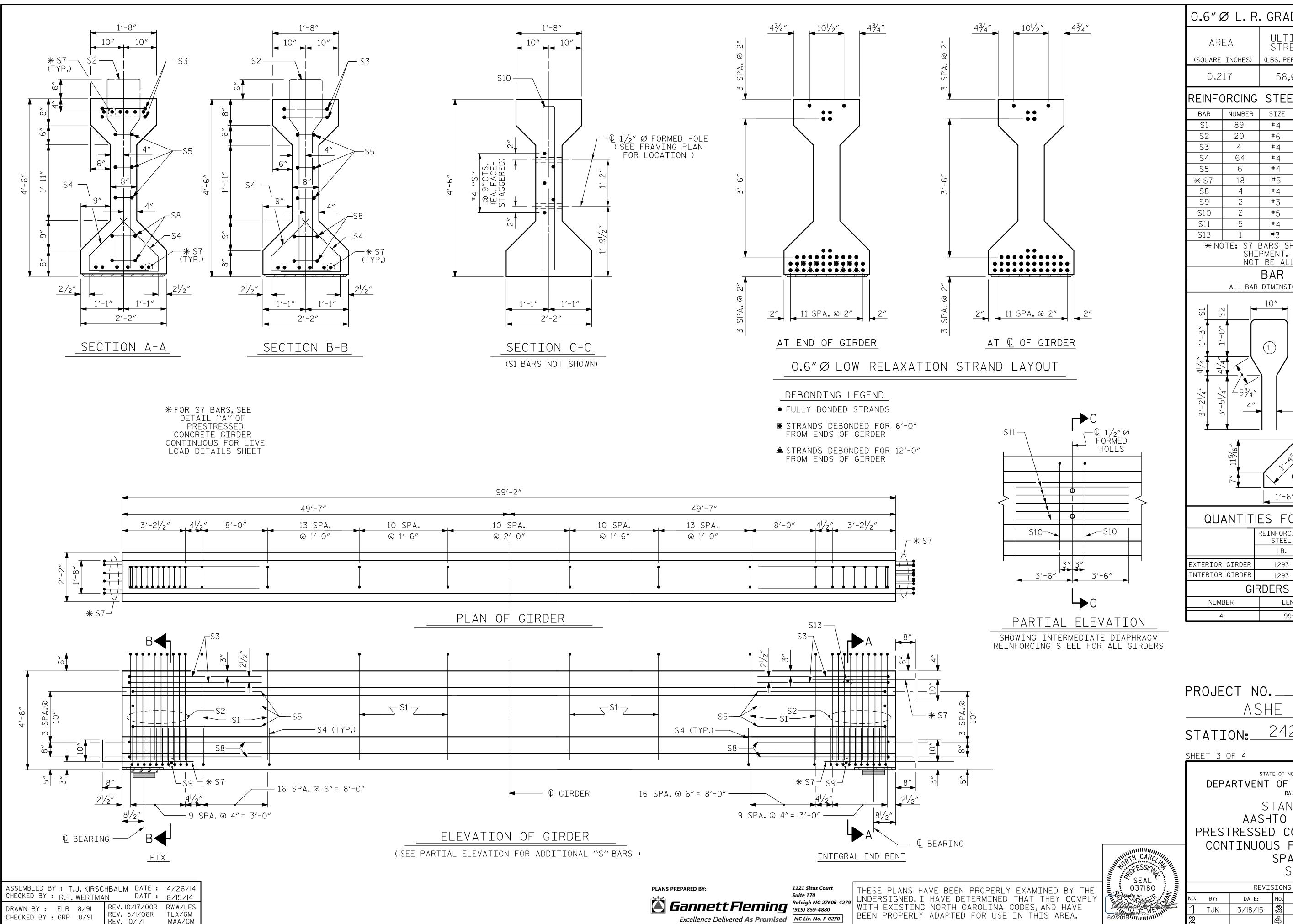
STR.NO.4

STD.NO.PCG6



CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS SHEET





+

+

0.6″¢	ØL.R	.GRAD	E 270	) ST	RANDS
AREA ULTIMATE APPLIED STRENGTH PRESTRESS (SQUARE INCHES) (LBS. PER STRAND) (LBS. PER STRAND)					ESTRESS
0.2		58,6			13,950
2 FINF(	)RCING	STEFI	STEEL FOR ONE GIRD		
BAR	NUMBER	SIZE	TYPE	LENG	
S1	89	#4	1		
S2	20	#6	1	10'-8 10'-8	
S3	4	#4	2	9'-1	
S4	64	#4	3	3'-5	
S5	6	#4	2	8′-5	
+ S7	18	#5	STR	3'-8	
<u>不 5 1</u> S8	4	#4	2	8'-7	
<u>50</u> S9	2	#3	STR	1'-10	
S10	2	#5	2	8'-8	
S10	5	#4	STR	7′-C	
S13	1	#3	STR	1'-4	
* NC	TE: S7 SHI	BARS SHA PMENT. H	LL BE E	BENT	BEFORE SHALL
	NOT		DWED.		
		BAR T	YPES		
	ALL BAR	DIMENSION	NS ARE OL	IT-TO-C	UT
3'-21/4" 41/4" 1'-3" S1		10" 1 1 1 1 1 1 1 1 1 1 1 1 1	R ONE	5″ S 7″ S 4″ S	S3, S5, 4'-2" 510 4'-2" 52 8, S8 8, S8 8, S8
		REINFORCIN	NG 8000	PSI	0.6″ØL.R.
	F	STEEL	CONC		STRANDS
		LB.	C.`		No.
XTERIOR	GIRDER	1293	20		42
NTERIOR		1293	20		42
	GIF	RDERS	REQUIR	RED	
NUME	BER	LENG	TH	ТОТ	AL LENGTH
4		99'-	2″		396′-8″
ROJECT NO. <u>R-2915B</u>					

COUNTY

SHEET NO.

S04-16

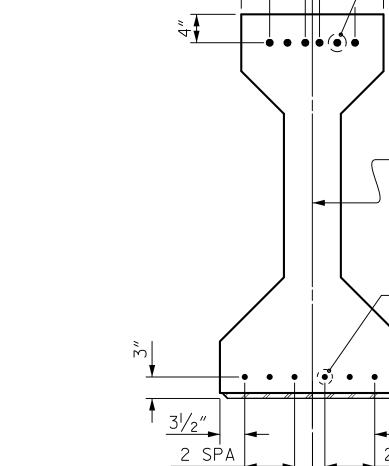
TOTAL SHEETS

35

DATE:

STD. NO. PCG6

STR.NO.4



2 SPA.-@ 2<sup>|</sup>/2"



	57 'P.)
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	-
DETAIL ``A''	- )

1'-8"

╼┝╢┝╾

- 米 S7

(TYP.)

— € GDR.

			DEA	DL	OAD	DE	FLE	CTI	ON	TAE	BLE	FOF	r G	IRDE	ERS								
			SPAN A																				
0.6″Ø LOW RELAXATION					(	GIRDE	ERS :	1& 4	1							(.	SIRDE	ERS 2	2 &	3			
TENTH POINTS		0	.1	<b>.</b> 2	.3	<b>.</b> 4	<b>.</b> 5	<b>.</b> 6	<b>.</b> 7	.8	.9	0	0	.1	.2	.3	<b>.</b> 4	.5	.6	.7	.8	.9	0
CAMBER (GIRDER ALONE IN PLACE)	<b>≜</b>	0	0.010	0.019	0.027	0.031	0.033	0.031	0.027	0.019	0.010	0	0	0.010	0.019	0.027	0.031	0.033	0.031	0.027	0.019	0.010	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	♦	0	0.005	0.011	0.015	0.017	0.018	0.017	0.015	0.011	0.005	0	0	0.006	0.012	0.016	0.019	0.020	0.019	0.016	0.012	0.006	0
FINAL CAMBER	ł	0	<sup> </sup> ⁄і6″	1/8″	<sup> </sup> /8″	<sup>3</sup> /16″	<sup>3</sup> /16″	3/16″	۱/ <sub>8</sub> ″	1/8″	1/16″	0	0	1/16″	1/16″	۱ <u>/</u> 8″	<sup> </sup> /8″	<sup> </sup> /8″	1/ <sub>8</sub> ″	1/8″	1/16″	1/16″	0

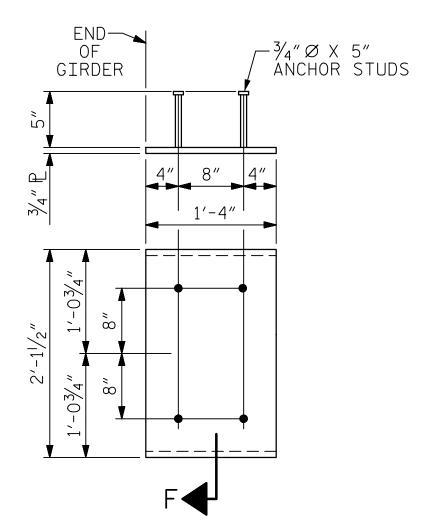
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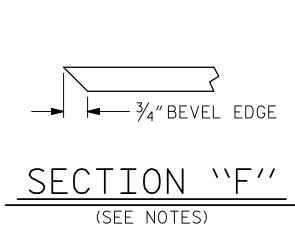
# INCLUDES FUTURE WEARING SURFACE.
ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT ``FINAL CAMBER'', WHICH IS GIVEN IN INCHES (FRACTION FORM).

DEAD LOAD DEFLECTION TABLE FOR GIRDERS																							
			SPANS B & C																				
0.6″Ø LOW RELAXATION					(	GIRD	ERS :	1 & 4	1							(	GIRDE	ERS 2	2&	3			
TENTH POINTS		0	.1	<b>.</b> 2	.3	<b>.</b> 4	.5	.6	.7	.8	.9	0	0	.1	.2	.3	<b>.</b> 4	<b>.</b> 5	.6	.7	.8	.9	0
CAMBER (GIRDER ALONE IN PLACE)	<b>≜</b>	0	0.062	0.118	0.162	0.189	0.199	0.189	0.162	0.118	0.062	0	0	0.062	0.118	0.162	0.189	0.199	0.189	0.162	0.118	0.062	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	↓	0	0.050	0.098	0.135	0.159	0.167	0.159	0.135	0.098	0.050	0	0	0.055	0.108	0.149	0.175	0.184	0.175	0.149	0.108	0.055	0
FINAL CAMBER	Å	0	1/8″	۱/ <sub>4</sub> ″	5/16″	3/ "	3/8″	3/8″	5/16″	1/4″	1/8″	0	0	1/16″	1/8″	3/16″	3/16″	3/16″	3/16″	3/16″	<sup> </sup> /8″	1/16″	0

\* INCLUDES FUTURE WEARING SURFACE. ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM),EXCEPT ``FINAL CAMBER'',WHICH IS GIVEN IN INCHES (FRACTION FORM).

ASSEMBLED BY : T.J. KIRSCI CHECKED BY : R.F. WERTMA		4/26/14 8/16/14
DRAWN BY : ELR 11/91 CHECKED BY : GRP 11/91	REV. 5/1/06 REV. 10/1/11 REV. 1/15	TLA/GM MAA/GM MAA/TMG





EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

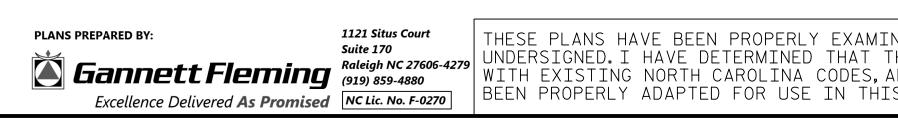
AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2" BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6400 PSI. DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET

DEPTH OF  $\frac{1}{4}$ ".

## EMBEDDED PLATE ``B-1" DETAILS FOR AASHTO TYPE IV GIRDER

(2 REQ'D PER GIRDER)



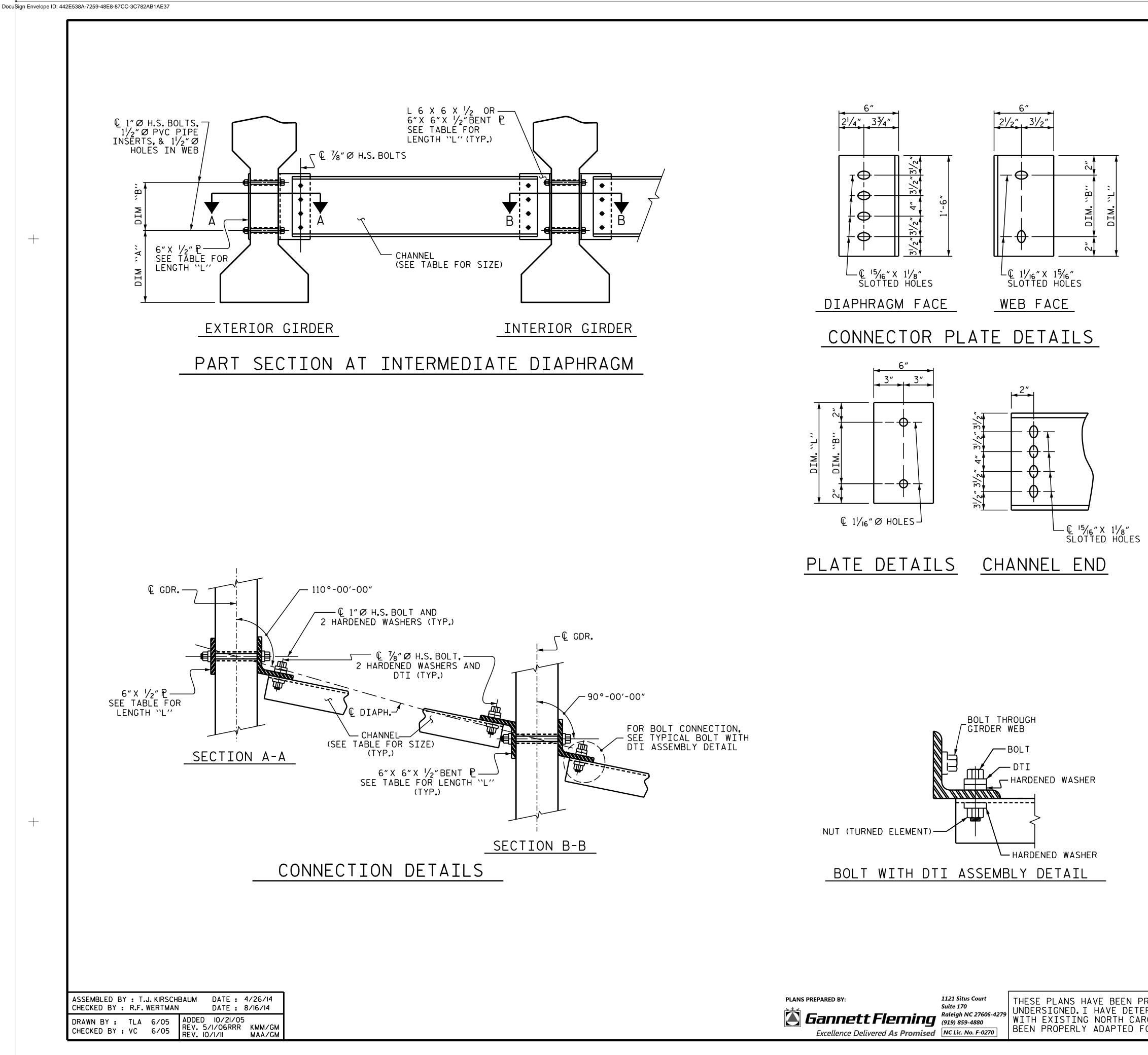
## NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER. THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A

		PROJECT NO. <u>R-2915B</u> <u>ASHE</u> COUNTY STATION: <u>242+67.42</u> -L- SHEET 4 OF 4
		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD
	PACIFIC DECLORED	PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS SBL
NED BY THE Hey comply	SEAL 037180	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: SO4-17
AND HAVE S AREA.	00000000000000000000000000000000000000	1         3         TOTAL SHEETS           2         4         35
		STR. NO. 4 STD. NO. PCG9



THESE PLANS HAVE BEEN PROPERLY EXAMINUTE UNDERSIGNED. I HAVE DETERMINED THAT T WITH EXISTING NORTH CAROLINA CODES, A BEEN PROPERLY ADAPTED FOR USE IN THE

## STRUCTURAL STEEL NOTES

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL  $\frac{1}{4}$  TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.99 PERCENT ZINC (W-Zn-1) THERMAL SPRAYED COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST  $\frac{1}{4}$  PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.

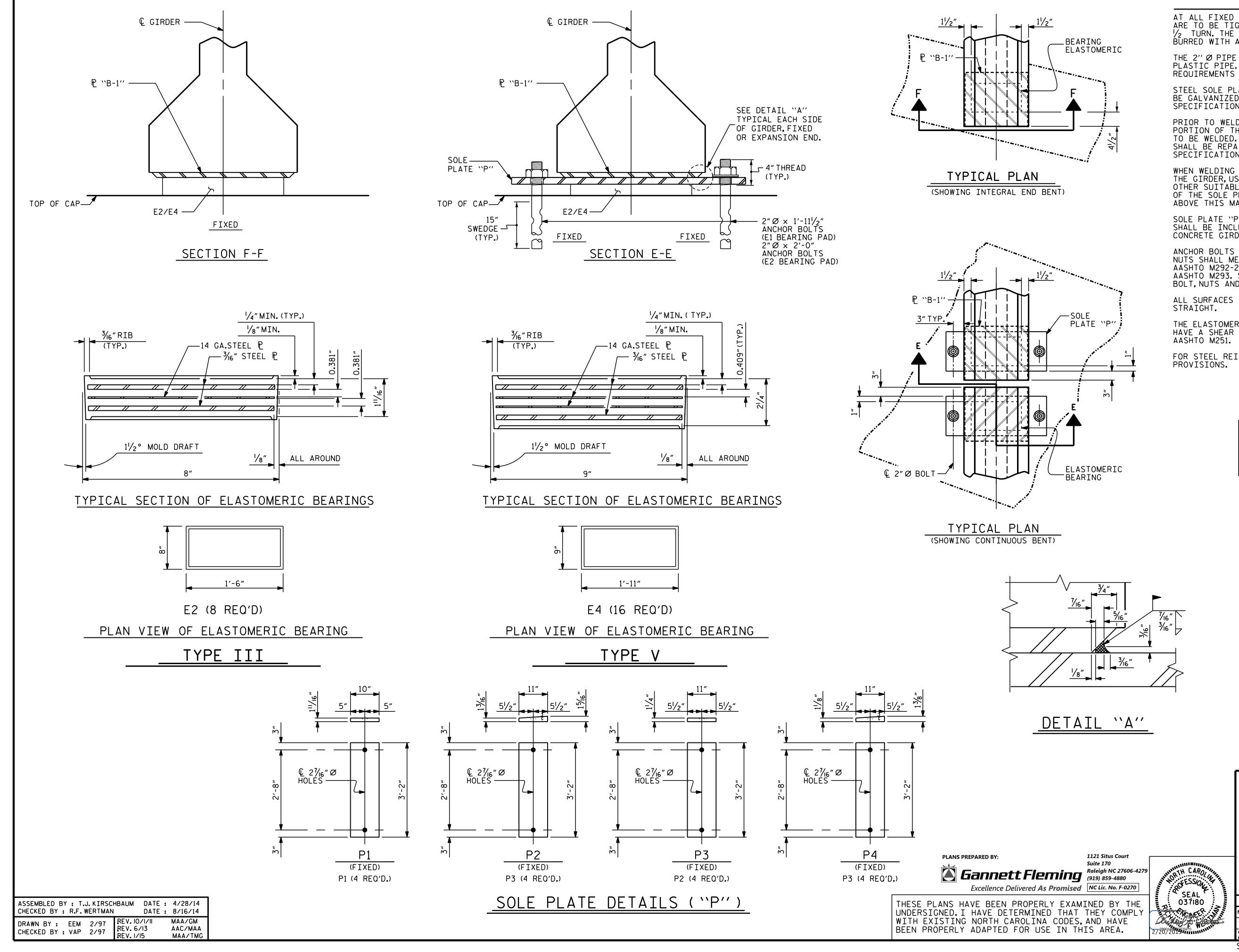
TABLE

GIRDER TYPE	CHANNEL SIZE	DIM ``A''	DIM ``B''	DIM ``L''
IV	MC 18 × 42.7	1′-9 <sup> </sup> /2″	1'-2"	1'-6"

		STATIO	ASH 0n:_2	HE 242+6		UNTY -L-
			RTMENT S INT STEEL TYPE	TANDAF DIAPH IV PR	NSPORTA RD ATE IRAGMS ESTRES	
	NOPESSION		CONCF	SBL	RDERS	
NED BY THE	SEAL 037180			SIONS	0.175	SHEET NO. SO4-18
THEY COMPLY AND HAVE IS AREA.	27209/2000 EFF6470	no. by: 1 2	DATE:	NO. BY: 3 4	DATE:	TOTAL SHEETS 35
		STR.NO.4		STD. NO.	PCG10	

PROJECT NO.\_

R-2915B



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NOTES AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF 1/2 TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

THE ELASTOMER IN THE STEEL REINFORCED BEARINGS SHALL HAVE A SHEAR MODULUS OF 0.160 KSI, IN ACCORDANCE WITH AASHTO M251.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

- LOAD RATINGS -							
	MAX.D.L. + L.L.						
TYPE III	170 K						
TYPE V	320 K						

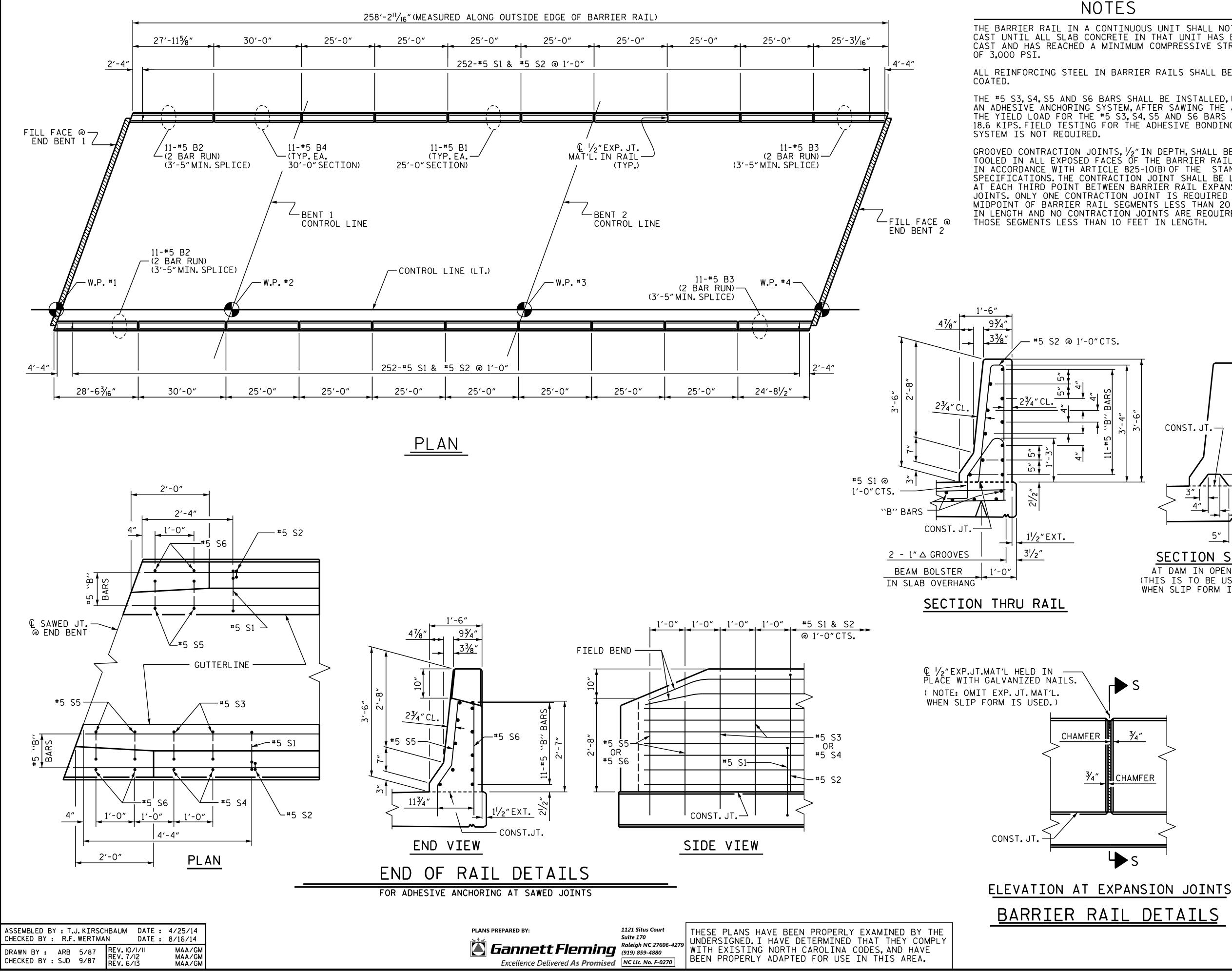
	ASHE	COUNTY
-	STATION: 242+6	7.42 -L-
	STATE OF NORTH CAR	
	DEPARTMENT OF TRANS	SPORTATION
	STANDAR	)
	ELASTOMERIC	BEARING
	_ DETAIL	S
NUMBER STORESSION	PRESTRESSED CONC SUPERSTRUC	

PROJECT NO.

				SHEET NO.			
	NO.	BY:	DATE:	NO.	BY:	DATE:	S04-19
5	1			ଞ			TOTAL SHEETS
	2			4			35
	STF	R.NO.4			STD.I STD.I	NO.EB3 NO.EB4	

R-2915B





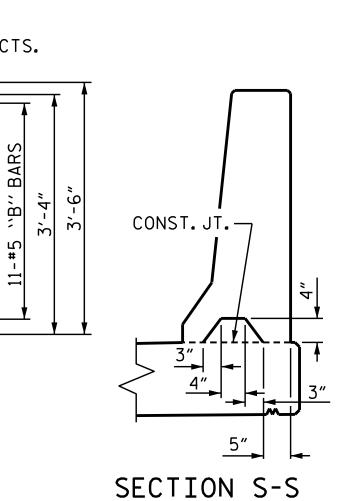
## NOTES

THE BARRIER RAIL IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THAT UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH

ALL REINFORCING STEEL IN BARRIER RAILS SHALL BE EPOXY

THE #5 S3, S4, S5 AND S6 BARS SHALL BE INSTALLED, USING AN ADHESIVE ANCHORING SYSTEM, AFTER SAWING THE JOINT. THE YIELD LOAD FOR THE #5 S3, S4, S5 AND S6 BARS IS 18.6 KIPS.FIELD TESTING FOR THE ADHESIVE BONDING

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. THE CONTRACTION JOINT SHALL BE LOCATE AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR

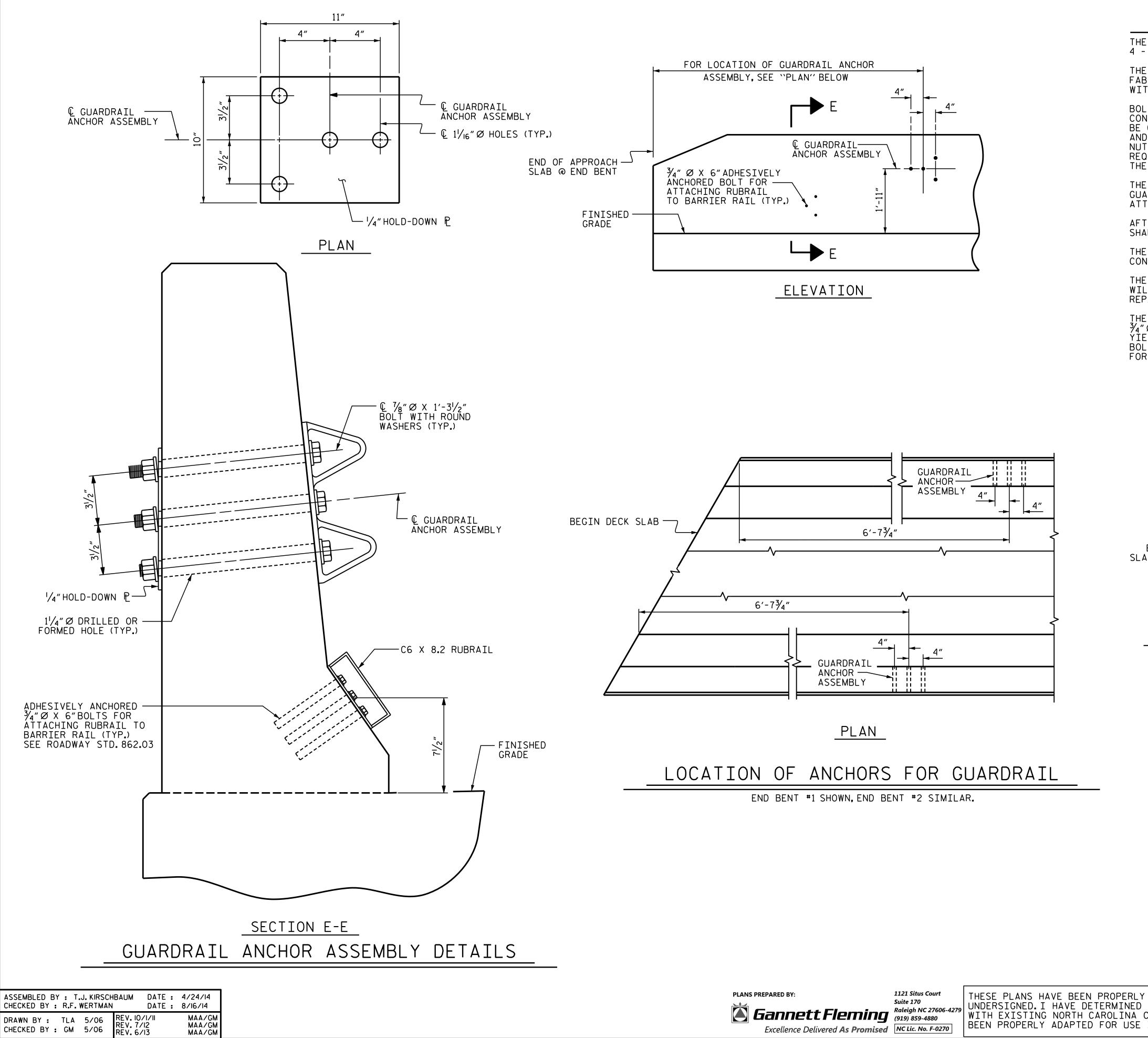


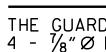
AT DAM IN OPEN JOINT (THIS IS TO BE USED ONL' WHEN SLIP FORM IS USED)

			BAF	<u>Υ</u> ΤΥ	PES	
			1′-0 <mark>/</mark> 2′			4″
		8	3 <sup>7</sup> ∕16″ ◀──►	 5¾″	- <b>•</b> •	
l						
Y		- , îv/- -	$\geq_{n}$	<b>ا</b> ٰ	3'-4"	3'-4"
	113/6"	$\mathbf{Z}$	21/4"	=	T / <b>I</b>	
I	+	$\geq$	2 <sup>1</sup> /4″ RAD.	1'-9 <sup>1</sup> /4"	↓	
	້ດ		(1)	Ļ,		3⁄4″
		8″	$\bigcirc$	I		
			5 <sup>1</sup> ∕	<sup>/</sup> 2″ 4 <sup> </sup> /	<u>′₄″ S3</u> (	2)
ED		<u>S3</u> S5	-	► 3 <sup>1</sup> /2		
					('-10 <sup>°</sup> 55	
२		2'-6¾" 1'-9¾"		2'-7"	,-10	
				/		
		61/2 "				
		101/2"			)	
		10		(3	)	
	ALL	BAR	DIMEN	SIONS	ARE OUT	TO OUT
					ATERIA	
		OR CON NO.	ICRETE SIZE	BARR: TYPE	IER RAIL LENGTH	
	BAR ¥B1	154	<b>#</b> 5	STR	24'-7"	WEIGHT 3949
	₭ B2 米 B3	44 44	#5 #5	STR STR	15'-10" 14'-2"	727 650
	* B3 * B4	22	#5	STR	29'-6"	677
	<b>*</b> S1	504	<b>#</b> 5	1	4'-10"	2541
	<b>米</b> S2	504	<b>#</b> 5	2	7'-0″	3680
	* S3 * S4	4	#5 #5	3 STR	4'-2" 4'-0"	17 17
	<b>*</b> S5	8	<b>#</b> 5	3	3′-5″	29
	* S6	8	<b>#</b> 5	STR	3'-3"	27
-	*EPOX REIN	Y COA FORCI		FI	12	314 LBS.
	CLASS	AA CO	NCRET	E	70 <b>.</b> 2 C	CU.YDS.
	CONCRE		NRIFK	RAIL	516.5 L	IN.FT.
T L Y						
D)				×	HINNING CAROL	
					₹ GESSION	
					037180	
					CINE HE	A star
				2720	2013/00000000000000000000000000000000000	
_			•			D
PR	OJEC			K	-2915	D
			SHE			DUNTY
ST	ATIC	DN:_	24	<u>2+6</u>	7.42	-L-
Γ			STATE OF	NORTH CAP	ROLINA	
	DEPA		NT OF		NSPORTA	TION
					2D	
		~	-		_	
		-				
		DAH	KHT	ΓK	RAIL	
				SBL		
NO.	BY:	RE DATE:	EVISION NO.	IS By:	DATE:	SHEET NO. SO4-20
1		UAIL:	3	יט:		TOTAL SHEETS
2 STR	. NO. 4		STD		CBR1	35
511						

► S <sup>3</sup>⁄4″ BCHAMFER **↓**S

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WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $\frac{7}{6}$ "  $\emptyset$  Galvanized Bolts, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION. THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE  $\frac{3}{4}$ " Ø X 6" BOLTS WITH WASHERS. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE  $\frac{3}{4}$ " Ø BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.

THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED. I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

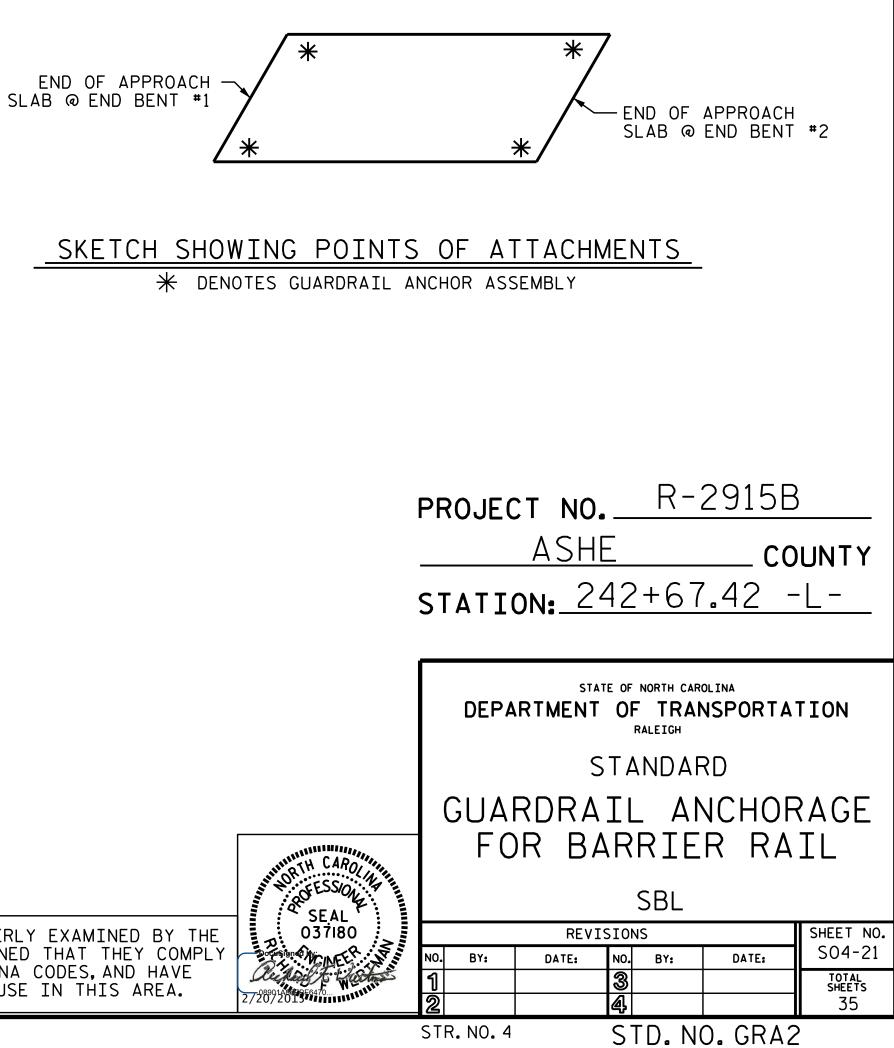
## NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 7/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED BOLTS.

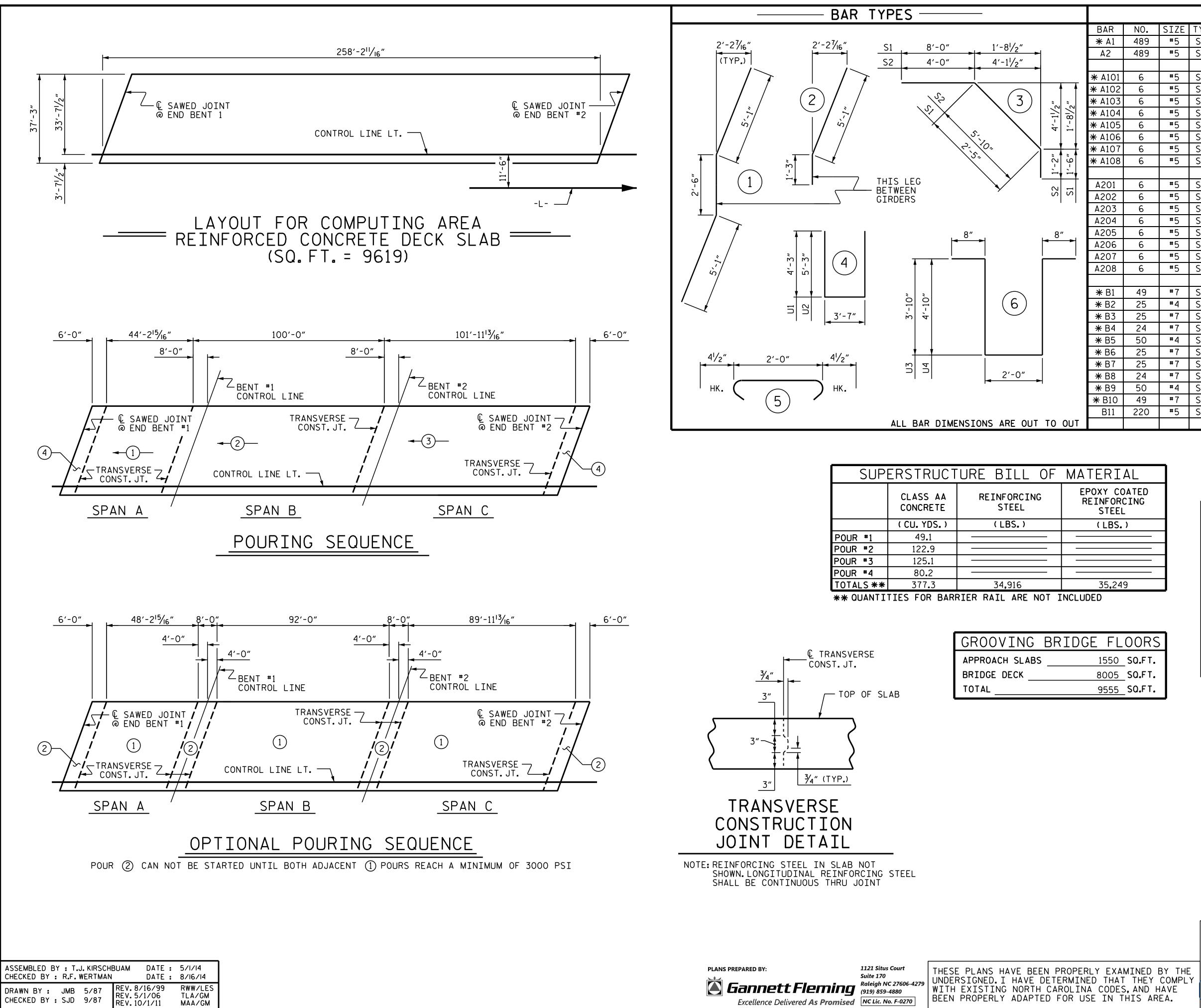
THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE  $1^{1}/_{4}^{\prime\prime}$  Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



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SUP	ERSTRUCT	URE BILL OF	MATERIAL
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
POUR #1	49.1		
POUR #2	122.9		
POUR #3	125.1		
POUR #4	80.2		
TOTALS **	377.3	34,916	35,249
** QUANTI	TIES FOR BAR	RIER RAIL ARE NOT I	NCLUDED

GROOVING	BRIDGE FL	OORS
APPROACH SLABS	1550	
BRIDGE DECK	8005	_ SQ.FT.
TOTAL	9555	_SQ.FT.

PLANS PREPARED BY:	1121 Situs Court
	Suite 170
🕅 Cappott Elomina	Raleigh NC 27606
🚺 Gannett Fleming	(919) 859-4880
Excellence Delivered As Promised	

THESE PLANS HAVE BEEN PROPERLY EXA
UNDERSIGNED.I HAVE DETERMINED THAT
WITH EXISTING NORTH CAROLINA CODES
BEEN PROPERLY ADAPTED FOR USE IN T

	BILL OF MATERIAL										
	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	489	<b>#</b> 5	STR	36′-11″	18829	H1	8	<b>#</b> 5	STR	9′-7″	80
	489	<b>#</b> 5	STR	36′-11″	18829	H2	8	<b>#</b> 5	STR	9'-4"	78
						Н3	8	<b>#</b> 5	STR	9'-11"	83
	6	<b>#</b> 5	STR	33'-4"	209	H4	8	<b>#</b> 5	STR	9'-8"	81
<b>)</b>	6	<b>#</b> 5	STR	29'-3"	183						
5	6	<b>#</b> 5	STR	25′-1″	157	K1	20	#4	STR	23'-9"	317
ļ	6	<b>#</b> 5	STR	21'-0"	131	K2	6	#4	STR	7'-11″	32
)	6	<b>#</b> 5	STR	16'-10"	105	К3	18	#4	STR	9'-2"	110
)	6	<b>#</b> 5	STR	12'-9"	80	K4	36	#4	STR	9′-7″	230
,	6	<b>#</b> 5	STR	8'-7"	54	K5	18	#4	STR	8'-6"	102
}	6	<b>#</b> 5	STR	4'-6"	28	K6	4	#4	STR	5′-5″	14
						K7	4	#4	STR	6'-1"	16
	6	<b>#</b> 5	STR	33'-4"	209	K8	8	#4	STR	6'-4"	34
	6	<b>#</b> 5	STR	29'-3"	183	К9	4	#4	STR	5′-9″	15
	6	<b>#</b> 5	STR	25'-1"	157	K10	8	#4	STR	3'-10"	20
	6	<b>#</b> 5	STR	21'-0"	131	K11	24	#4	STR	2'-10"	45
	6	<b>#</b> 5	STR	16'-10"	105	K12	12	#4	STR	7'-4"	59
	6	<b>#</b> 5	STR	12'-9"	80	K13	20	#4	2	6'-4"	85
	6	<b>#</b> 5	STR	8'-7"	54	K14	20	#4	1	12'-8"	169
	6	<b>#</b> 5	STR	4'-6"	28						
						<b>*</b> S1	60	#4	3	11'-11"	478
	49	<b>#</b> 7	STR	12'-0"	1202	<b>*</b> S2	60	#4	3	11'-0"	441
	25	#4	STR	27'-9″	463	S3	192	#4	5	2'-9"	353
	25	<b>#</b> 7	STR	57'-6"	2938						
	24	<b>#</b> 7	STR	24'-0"	1177	U1	60	#4	4	12'-1"	484
	50	#4	STR	18'-0"	601	U2	12	#4	4	14'-1"	113
	25	#7	STR	25'-0"	1278	U3	12	#4	6	11'-0"	88
	25	#7	STR	50′-3″	2568	U4	36	#4	6	13'-0"	313
	24	<b>#</b> 7	STR	30'-0"	1472						
	50	#4	STR	25'-6"	852						
1	49	<b>#</b> 7	STR	20'-0"	2003						
	220	<b>#</b> 5	STR	53'-3"	12219						
_											
						ORCING Y COAT				34,916 35,249	

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE						
FOLLOWING MINIMUM SPLICE LENGTHS						
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	APPROACH ARAPET,	APPROAC	PARAPET AND BARRIER		
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL	
#4	2'-0"	1'-9"	2'-0"	1′-9″	2'-9"	
<b>#</b> 5	2'-6"	2'-2"	2′-6″	2'-2″	3'-5″	
<b>#</b> 6	3'-0"	2'-7″	3′-10″	2'-7″	4'-4"	
<b>#</b> 7	5'-3″	3′-6″				
#8	6'-10"	4'-7"				

PROJECT NO.	R-2915B
ASHE	COUNTY
STATION: 242	+67.42 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

## SUPERSTRUCTURE

BILL OF MATERIAL

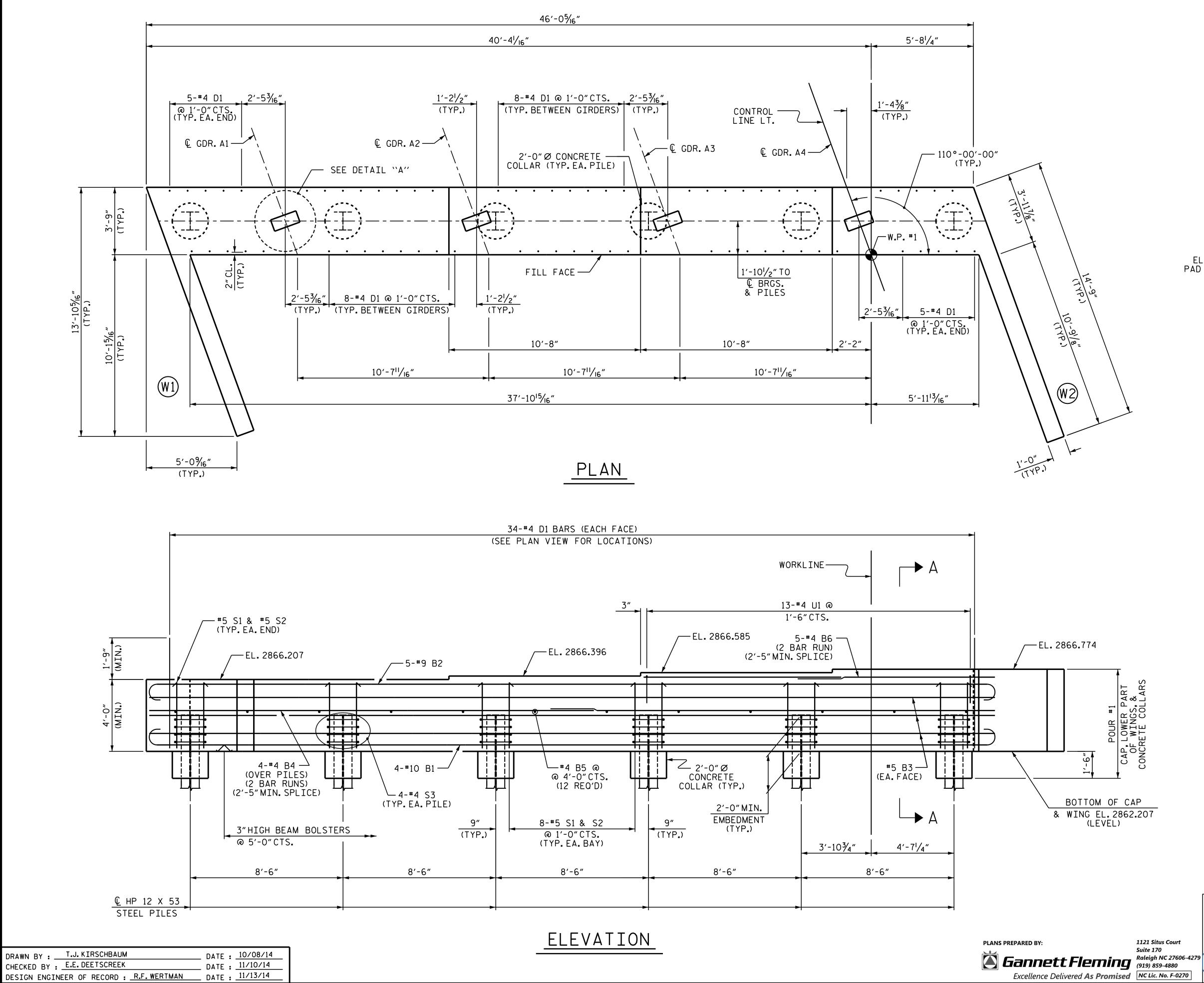
SBL

		SHEET NO.				
N0.	BY:	DATE:	NO.	BY:	DATE:	S04-22
12			3			TOTAL SHEETS
2			4			35
ST	R.NO.4		S	TD. NO	D. BOM1	

TH CARO FESSION

SEAL \* 037180

Cuflet & 1248



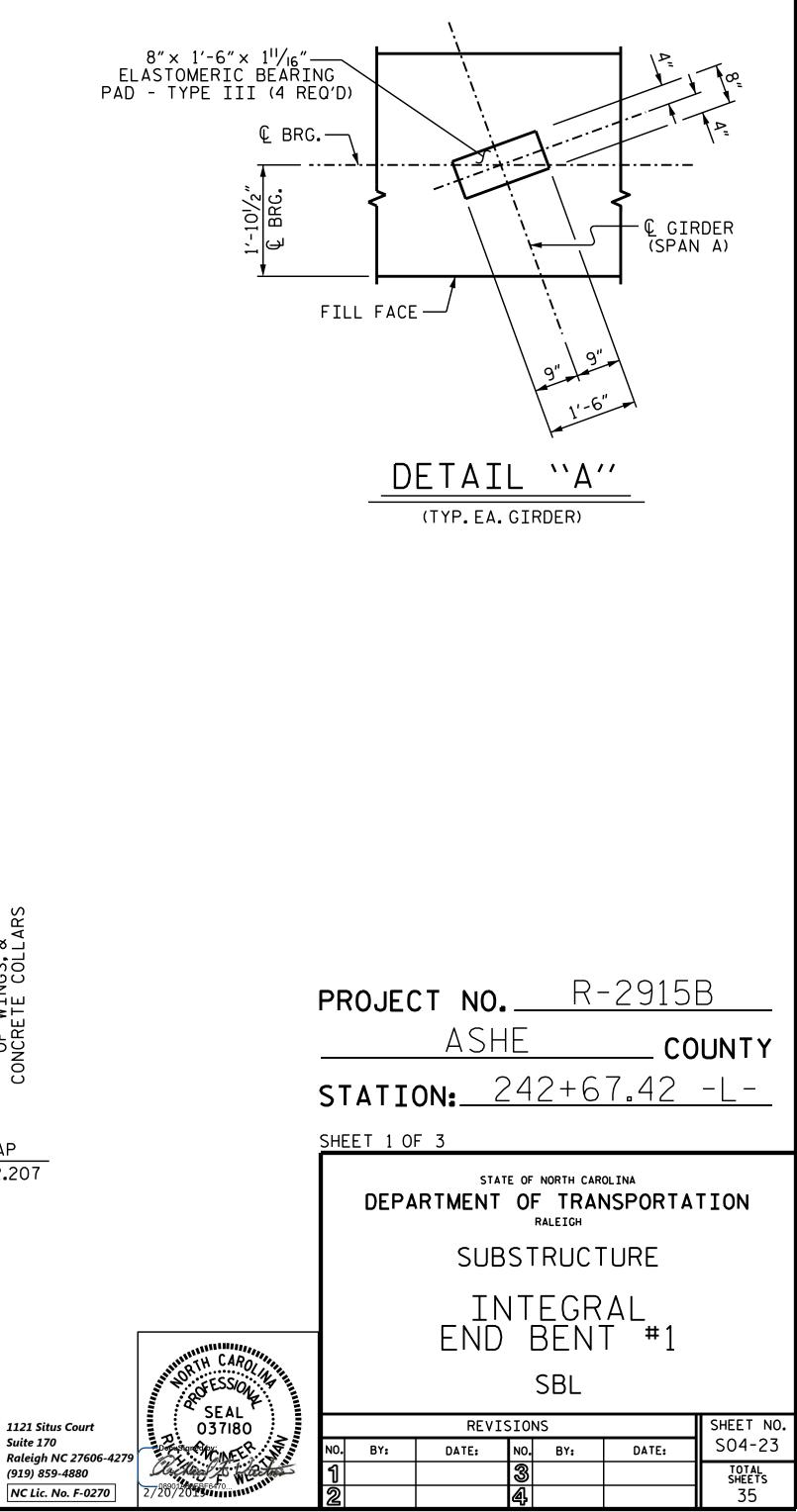
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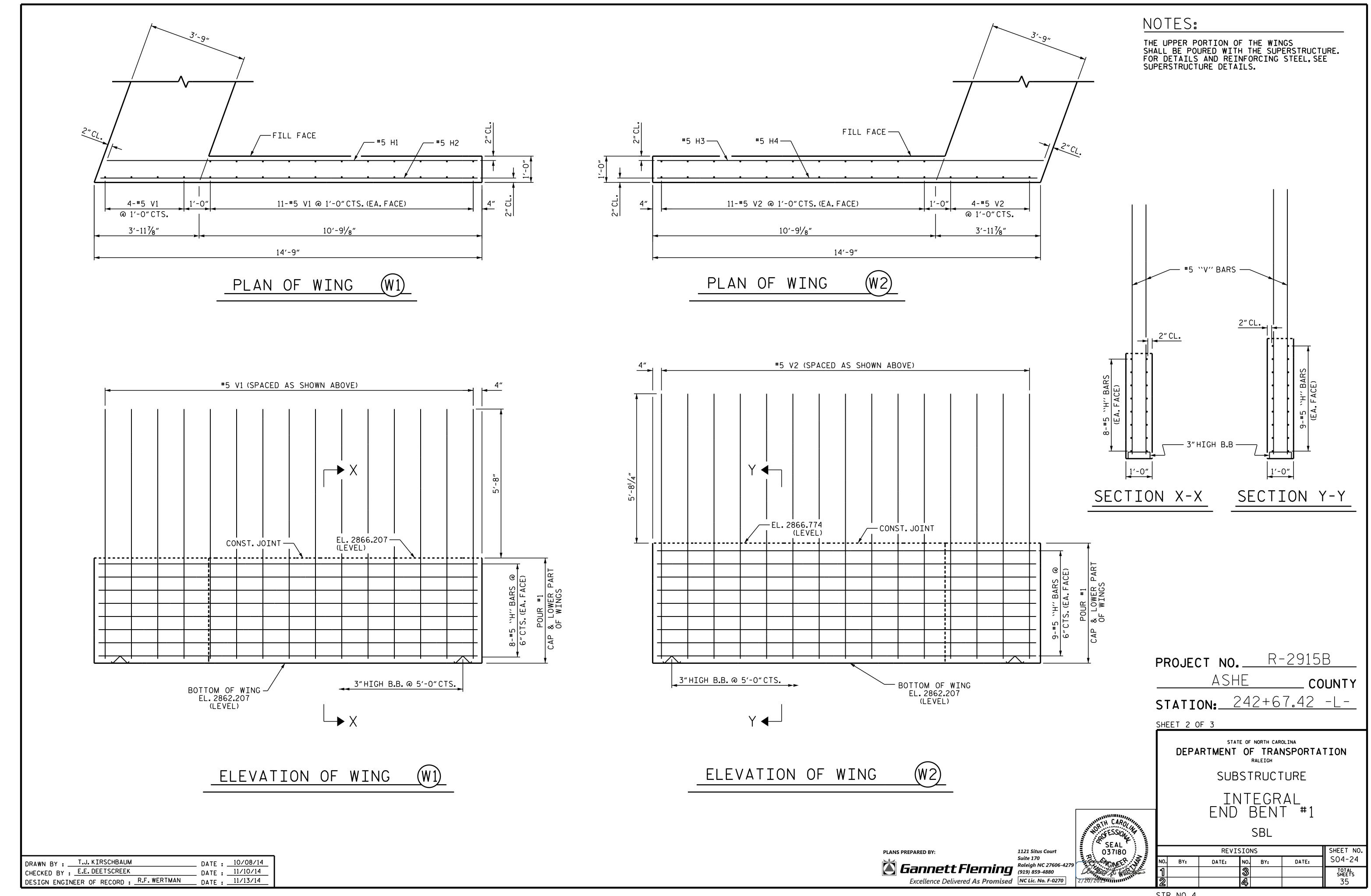
Excellence Delivered As Promised NC Lic. No. F-0270

NOTES:

INSTALL THE 4"DIA.DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS.SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

#4 D1 BARS MAY BE SHIFTED SLIGHTLY TO AVOID STIRRUPS IN CAP. SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.





STR.NO.4

+

MINIMUM OF 3- ONE CUBIC -FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6" (MIN.) PIPE FOR DRAINAGE GRADE TO DRAIN -TOE OF SLOPE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

## TEMPORARY DRAINAGE AT END BENT

5-#4 B6

5-**#**9 B2

1-#5 B3 (EA.FACE)

1-#5 B3 (EA.FACE)

1-#5 B3 (EA.FACE)

4-#10 B1

€ HP 12 X 53-STEEL PILES

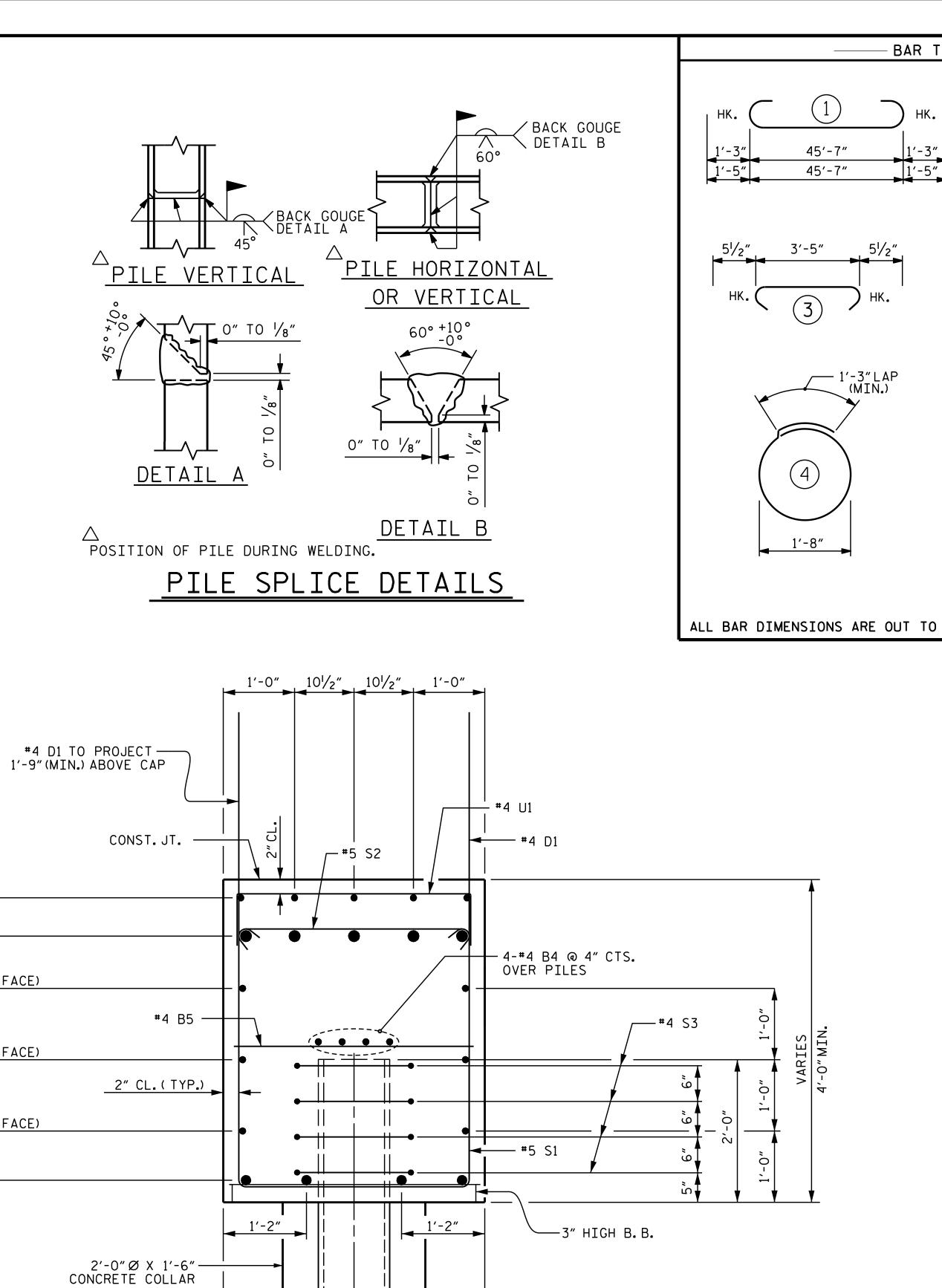
1'-10<mark>'/</mark>2"

3'-9"

SECTION A-A

1′-10<mark>′/</mark>2″

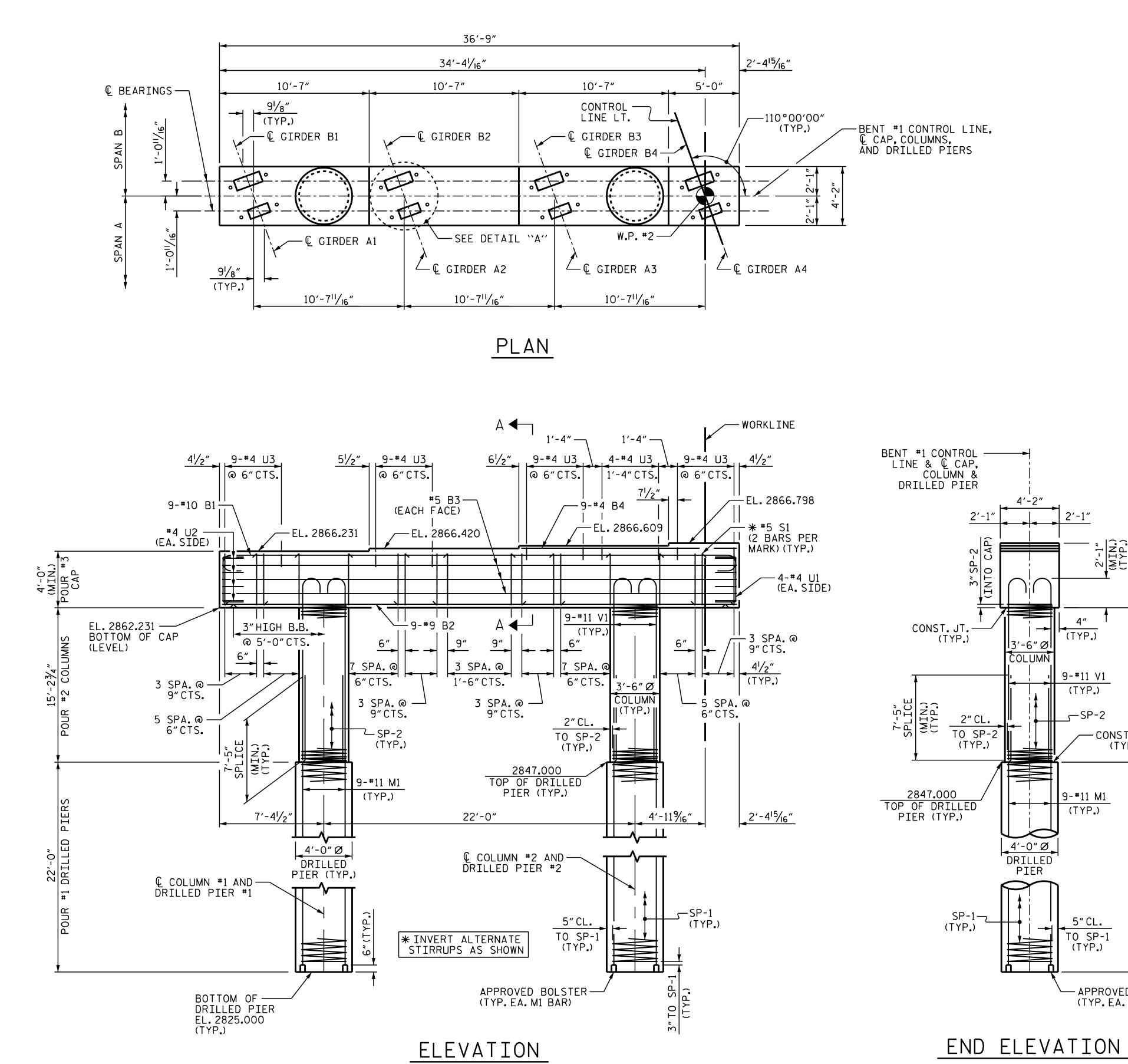
DRAWN BY : _	T.J. KIRSCHBAUM	DATE :_	10/08/14
	E.E. DEETSCREEK	DATE :	11/10/14
	EER OF RECORD : R.F. WERTMAN	DATE : _	11/13/14



PLANS PREPARED BY: **Gannett Fleming** <sup>Rale</sup> (919 Excellence Delivered As Promised NC

IYPES —			BI	LL OF	ΜΑΤΕ	RIAL	
				END B	SENT	#1	
		BAR	No.	SIZE	TYPE	LENGTH	WEIGHT
1		B1	4	<b>#</b> 10	1	48′-5″	833
		B2	5	#9	1	48'-1"	817
B2		B3	6	<b>#</b> 5	STR	45′-8″	286
B1	HK.	B4	8	#4	STR	24'-1"	129
		B5	12	#4	STR	3′-5″	27
	$\land \land \neg \neg \uparrow$	B6	10	#4	STR	11'-0"	73
	<sup>2</sup> "	D1	68	#4	STR	6'-2″	280
	3'-71/2"						
	μ M	H1	8	<b>#</b> 5	STR	14'-1"	118
		H2	8	<b>#</b> 5	STR	14'-4"	120
		Н3	9	<b>#</b> 5	STR	14'-8"	138
	71 41	H4	9	<b>#</b> 5	STR	14'-5″	135
	3'-4"						
		S1	42	<b>#</b> 5	2	11'-6″	504
		S2	42	<b>#</b> 5	3	4'-4"	190
		S3	24	#4	4	6'-6"	104
	3'-5"	U1	13	#4	5	6′-5″	56
<b>—</b>		V1	26	#5	STR	9′-6″	258
		V2	26	#5	STR	10'-1"	273
1'-6"	(5)						
÷		REIN	NFORCIN	NG STEEL			4341 LBS.
•							
				CLASS A			
		POUR #	1 (CAP,	LOWER WI			
					ТОТ		31.6 C.Y.
				- 12 X 53	STEEL		
OUT.		No.	= 6			165	LIN.FT.

	PROJEC	T NO. ASH		-2915 co	
	STATIO	<b>DN:</b> _2	42+6	7.42	<u>-L-</u>
	SHEET 3 O	F 3			
	DEPA		E OF NORTH CARG		TION
		SUB	STRUCT	URE	
			ITEGR BEN		
1121 Situs Court Suite 170			SBL		
1121 Situs Court 037180		REVIS	SIONS		SHEET NO.
Suite 170 Raleigh NC 27606-4279	NO. BY:	DATE:	NO. BY:	DATE:	S04-25
(919) 859-4880 NC Lic. No. F-0270 2/20/2015	1		ଙ୍କ		total sheets 35



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DRAWN BY :	T.J. KIRSCHBAUM	DATE : 10/21/14
	R.F. WERTMAN	DATE : 11/7/14
	INEER OF RECORD :	DATE : <u>11/7/14</u>

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. HOOKS ON VI BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

FOR DRILLED PIERS, SEE SECTION 411 OF STANDARD SPECIFICATIONS. ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL". THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA

LENGTH.

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE GROUND LINE. SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIER WILL NOT BE

PERMITTED.

2'-1' MIN TYP

-CONST.JT.

(TYP.)

- APPROVED BOLSTER (TYP. EA. M1 BAR)

4″

AP

COLUMNS POUR #2

PIERS #1

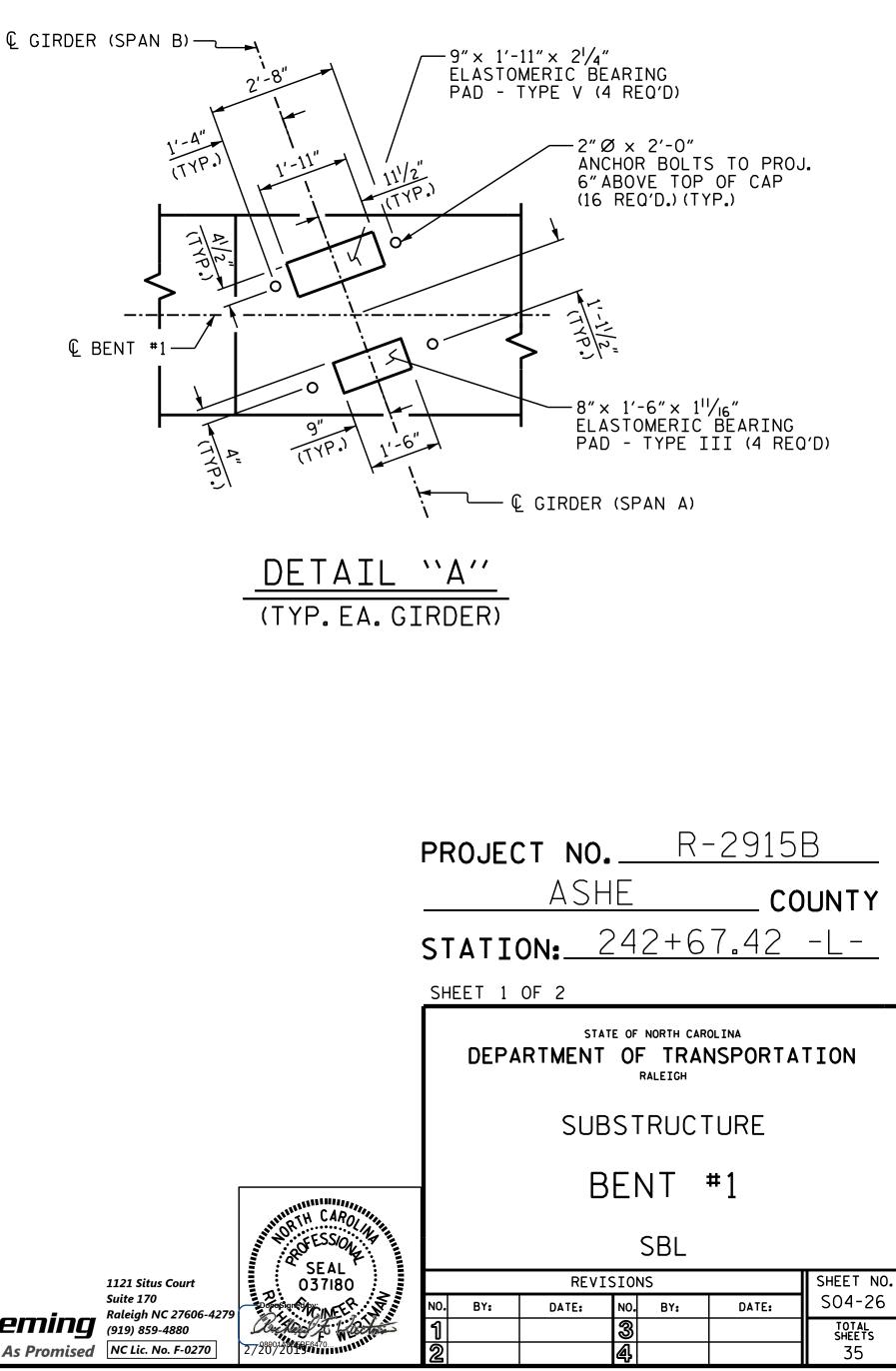
DRILLED | POUR

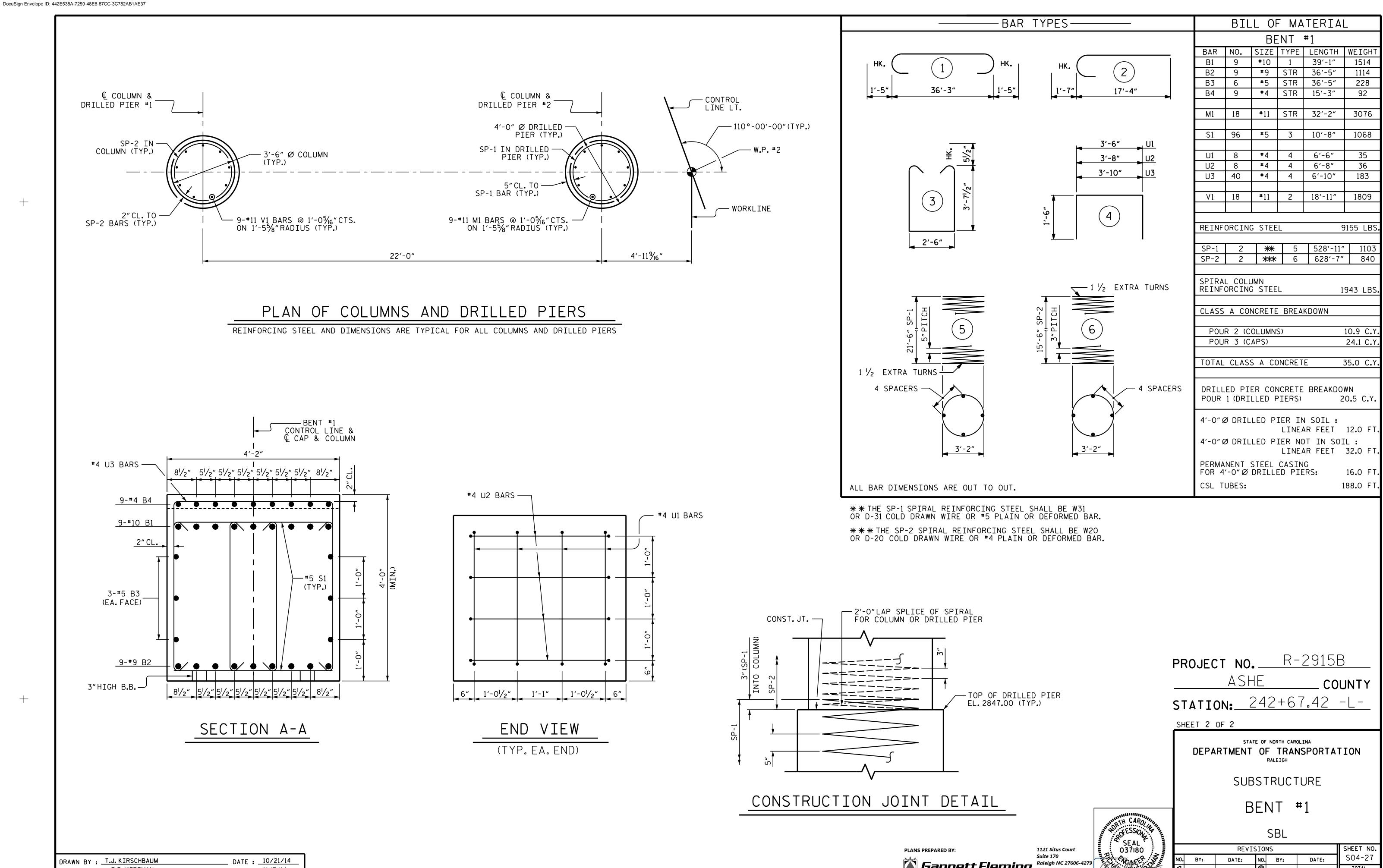
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FOR PERMANENT STEEL CASING, SEE SECTION 411 OF STANDARD SPECIFICATIONS.

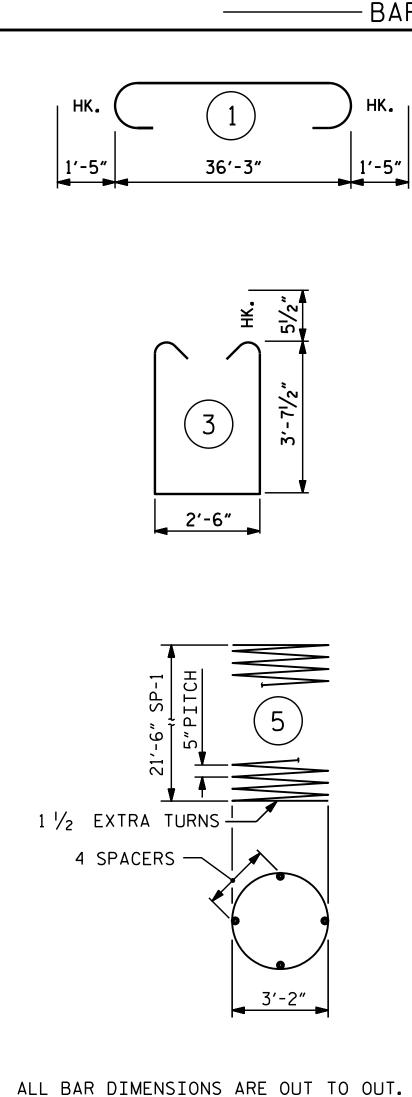
**PLANS PREPARED BY:** Suite 170 Raleigh NC 27606-4279 (919) 859-4880 Excellence Delivered As Promised NC Lic. No. F-0270

## NOTES



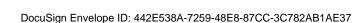


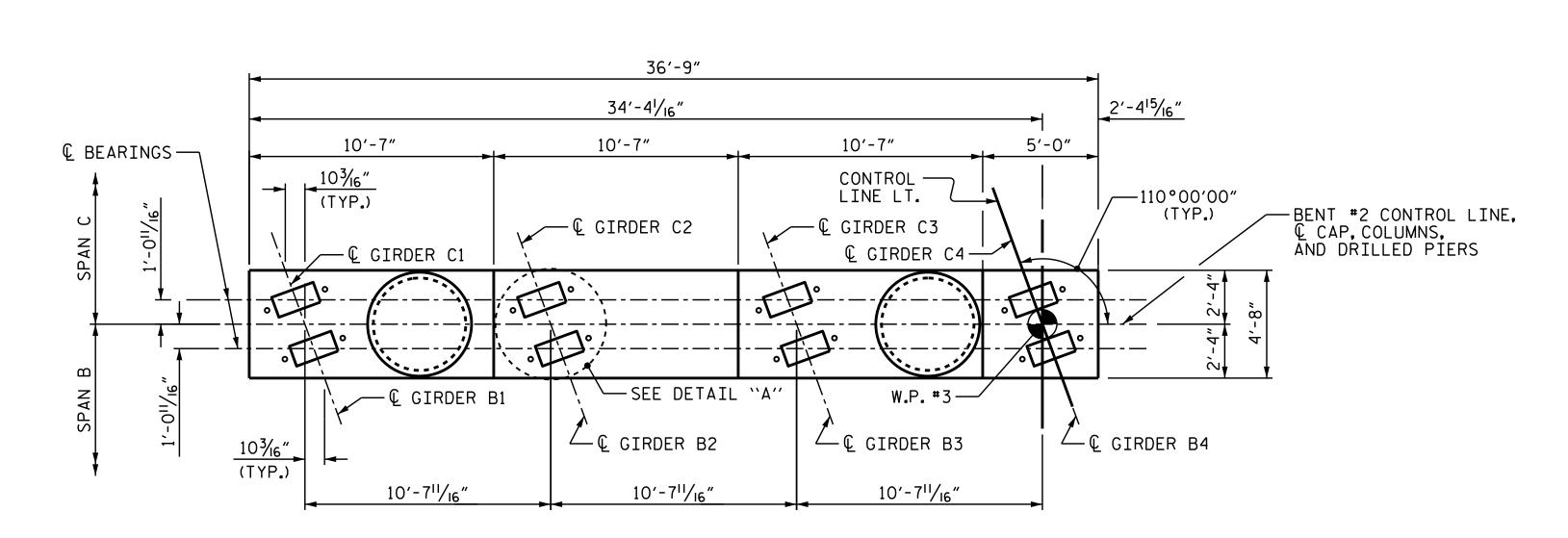
CHECKED BY : R.F. WERTMAN DATE : <u>11/7/14</u> DESIGN ENGINEER OF RECORD : R.F. WERTMAN DATE : 11/7/14



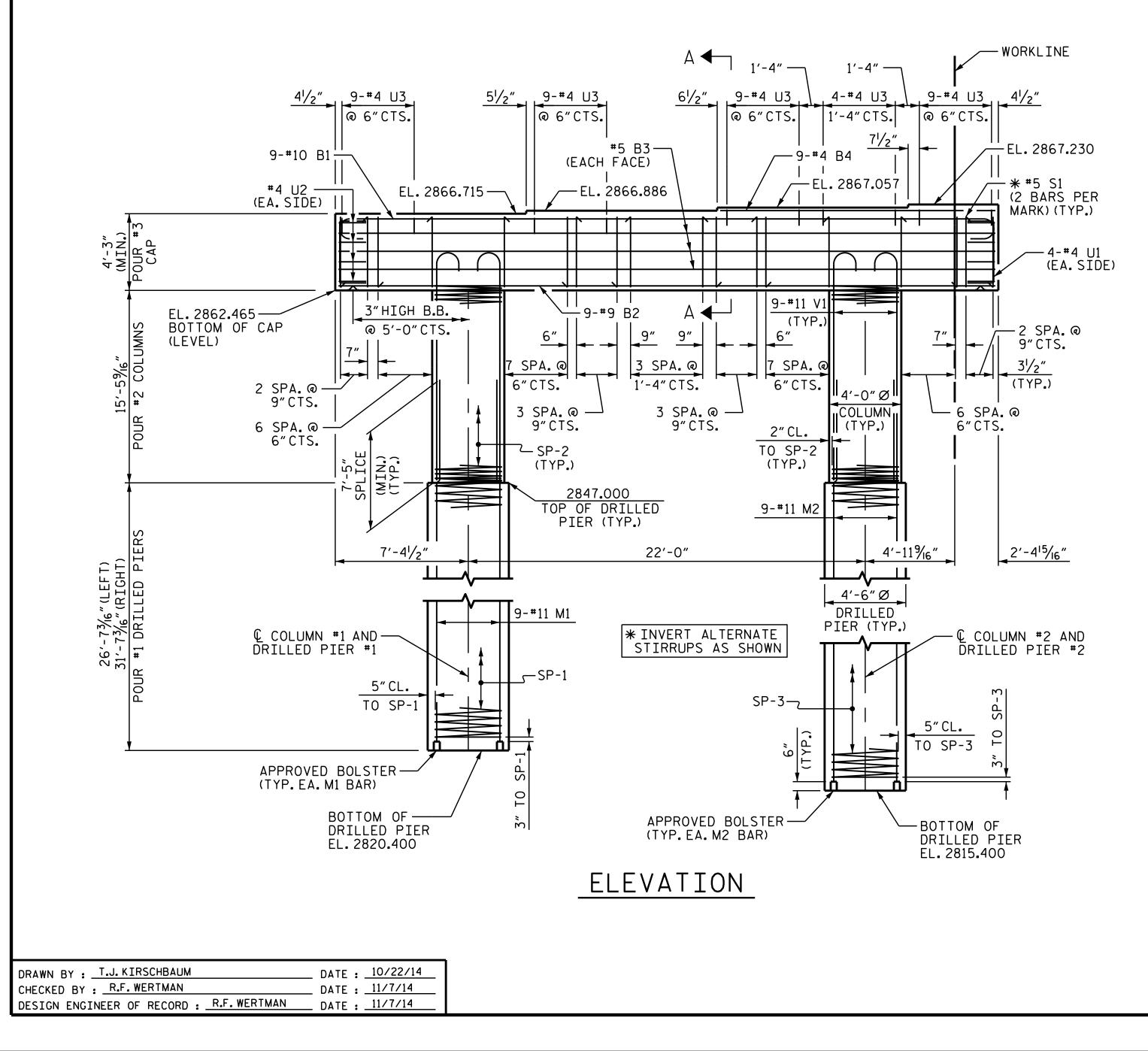
🙆 Gannett Fleming Excellence Delivered As Promise

	PROJECT NO. <u>R-2915E</u> <u>ASHE</u> COL STATION: <u>242+67.42</u> sheet 2 of 2	JNTY
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTAT RALEIGH	ION
	SUBSTRUCTURE	
NUMBTH CAROL	BENT #1	
SEAL	SBL	
1121 Situs Court Suite 170	REVISIONS NO. BY: DATE: NO. BY: DATE:	SHEET NO. SO4-27
Raleigh NC 27606-4279         (919) 859-4880         NC Lic. No. F-0270	1         3         4	total sheets 35





PLAN



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STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR ANCHOR BOLTS. HOOKS ON VI BARS MAY BE TURNED AS NECESSARY FOR PLACING REINFORCING STEEL.

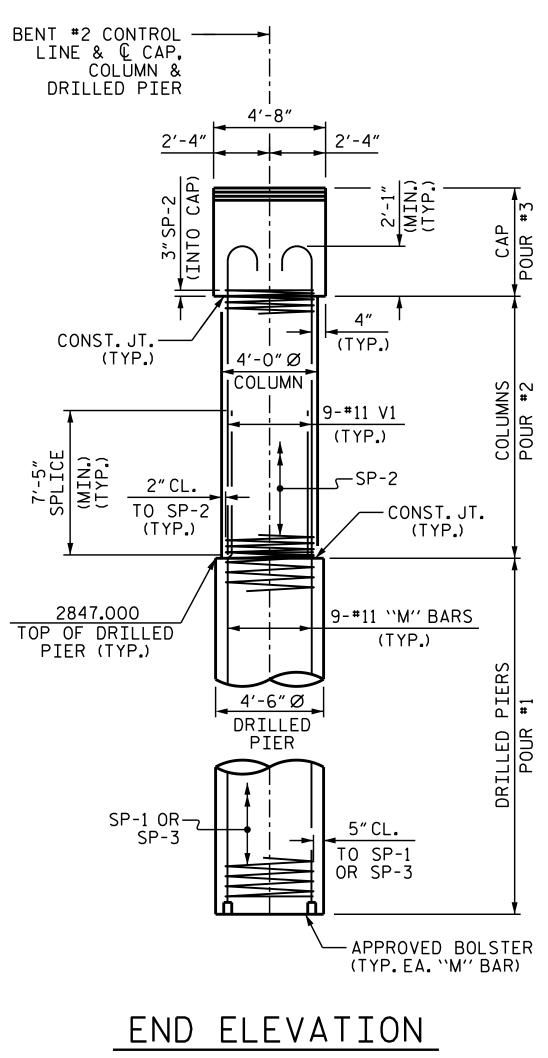
FOR DRILLED PIERS. SEE SECTION 411 OF STANDARD SPECIFICATIONS. ALL STEEL IN THE DRILLED PIERS IS INCLUDED IN THE PAY ITEMS FOR "REINFORCING STEEL" AND "SPIRAL COLUMN REINFORCING STEEL". THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT THE LONGITUDINAL REINFORCEMENT FOR THE DRILLED PIERS IS DETAILED WITH 3 FEET OF EXTRA

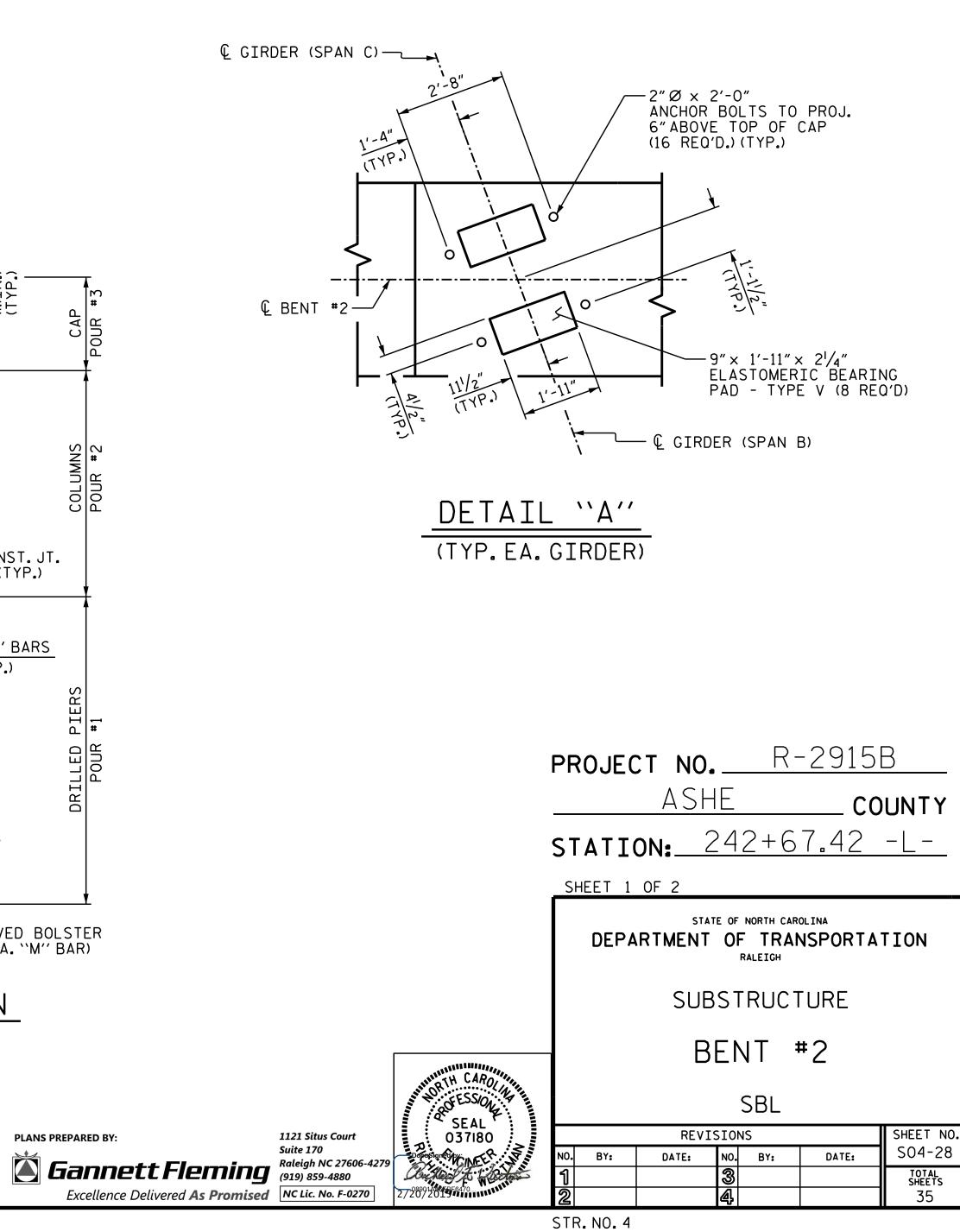
LENGTH.

THE LOCATION OF THE CONSTRUCTION JOINT IN THE DRILLED PIERS IS BASED ON AN APPROXIMATE GROUND LINE ELEVATION. IF THE CONSTRUCTION JOINT IS ABOVE THE ACTUAL GROUND LINE ELEVATION, THE CONTRACTOR SHALL PLACE THE CONSTRUCTION JOINT 1 FT. BELOW THE GROUND LINE. SPLICING OF THE LONGITUDINAL BARS IN THE DRILLED PIER WILL NOT BE

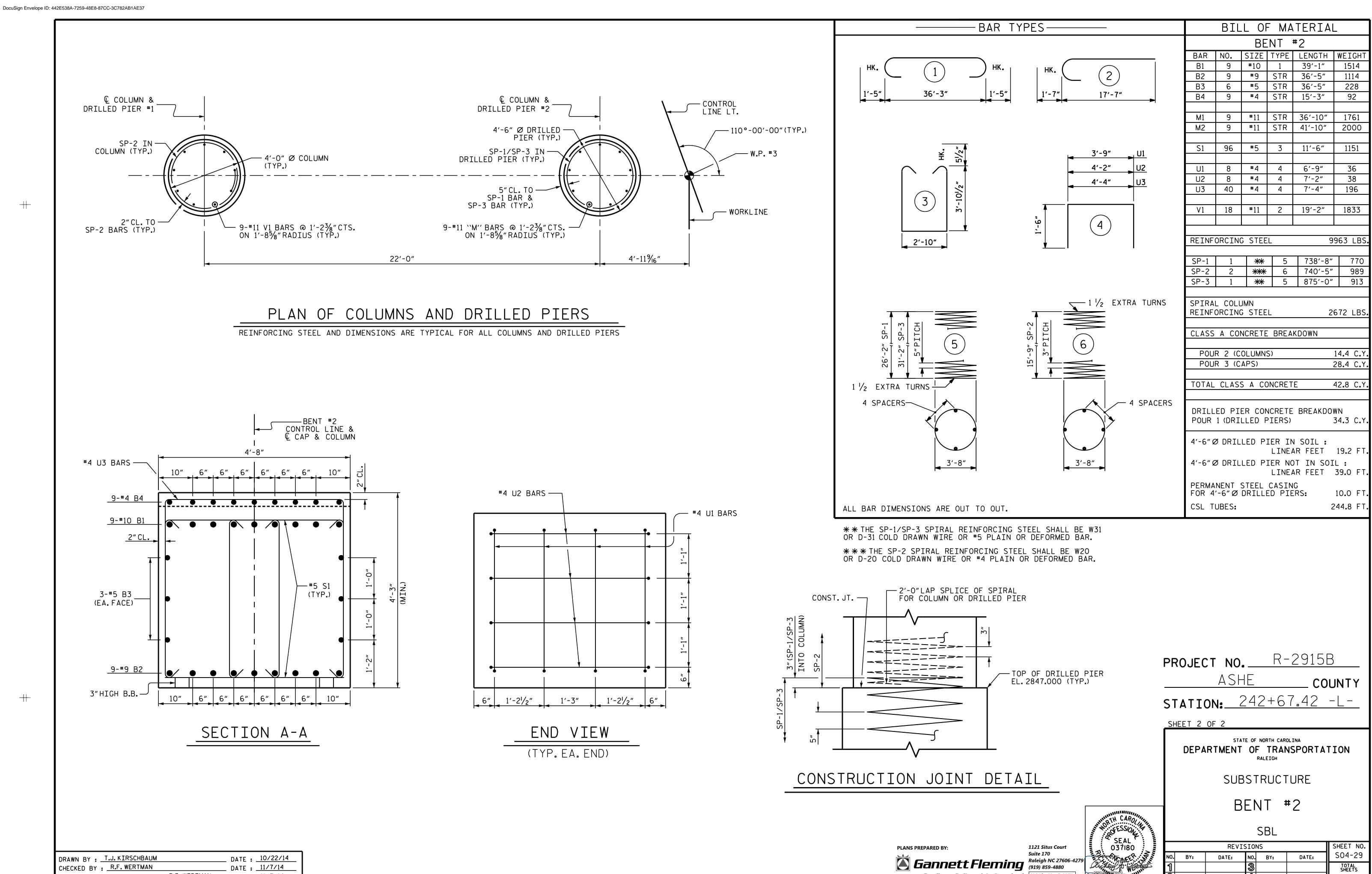
PERMITTED.

FOR PERMANENT STEEL CASING, SEE SECTION 411 OF STANDARD SPECIFICATIONS.

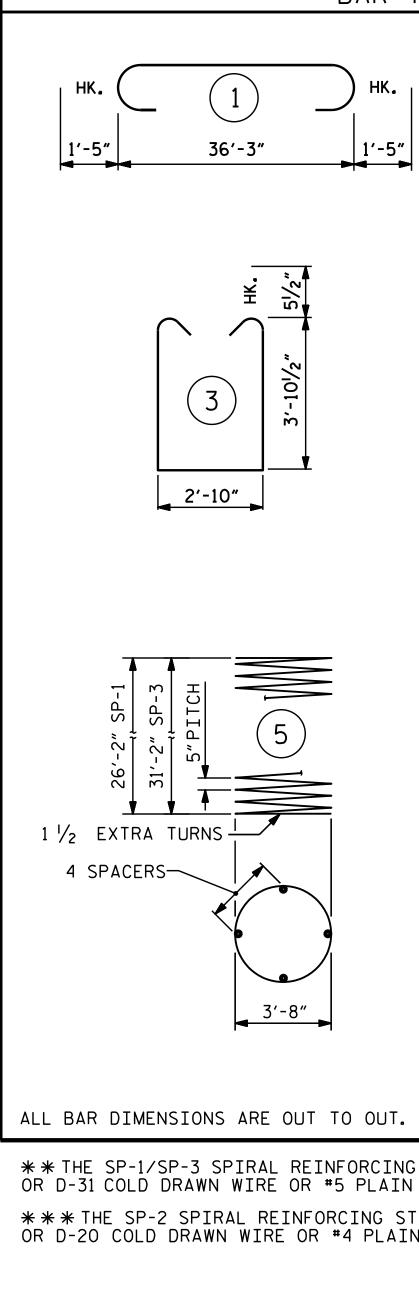




## NOTES



DESIGN ENGINEER OF RECORD : \_\_\_\_\_\_\_\_ R.F. WERTMAN \_\_\_\_\_ DATE : \_\_\_\_\_\_\_



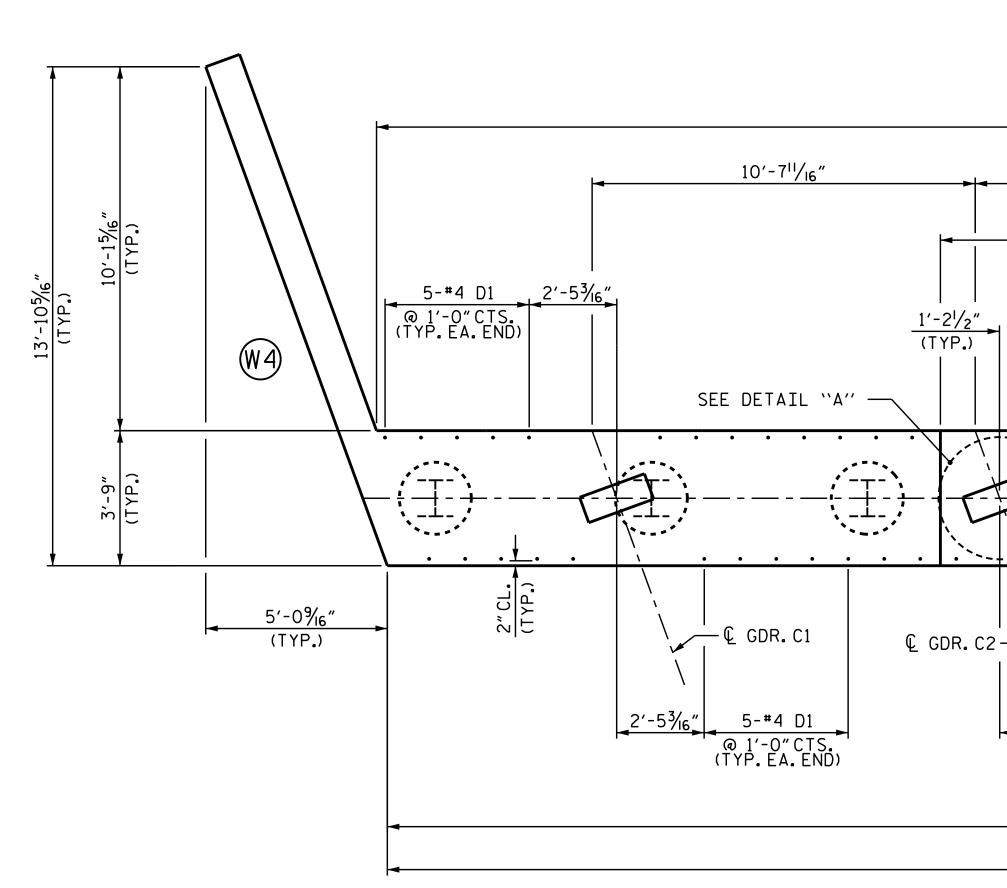


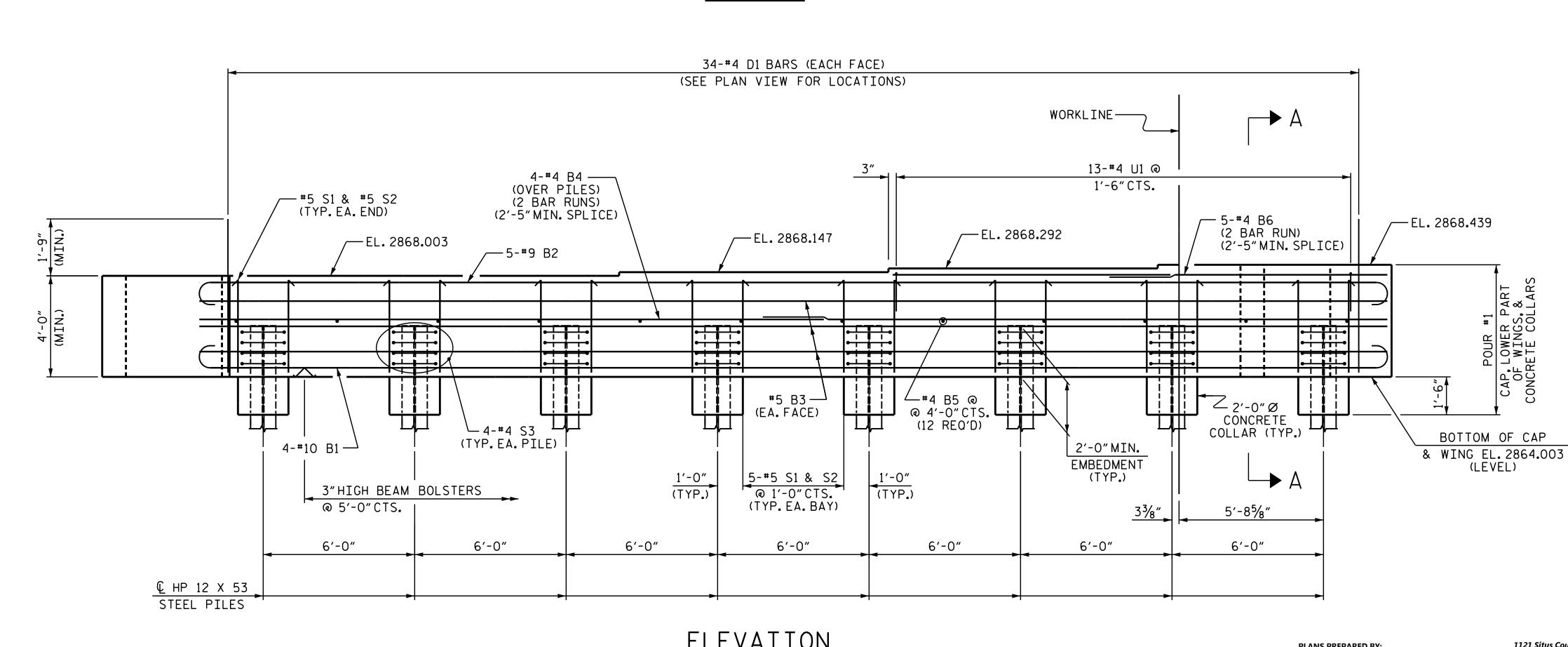
STATION:	242+67.42	- [_

35

STR.NO.4

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DRAWN B	Y :	_ DATE : 10/08/14
	BY : E.E. DEETSCREEK	DATE : 11/10/14
		_ DATE : <u>11/13/14</u>





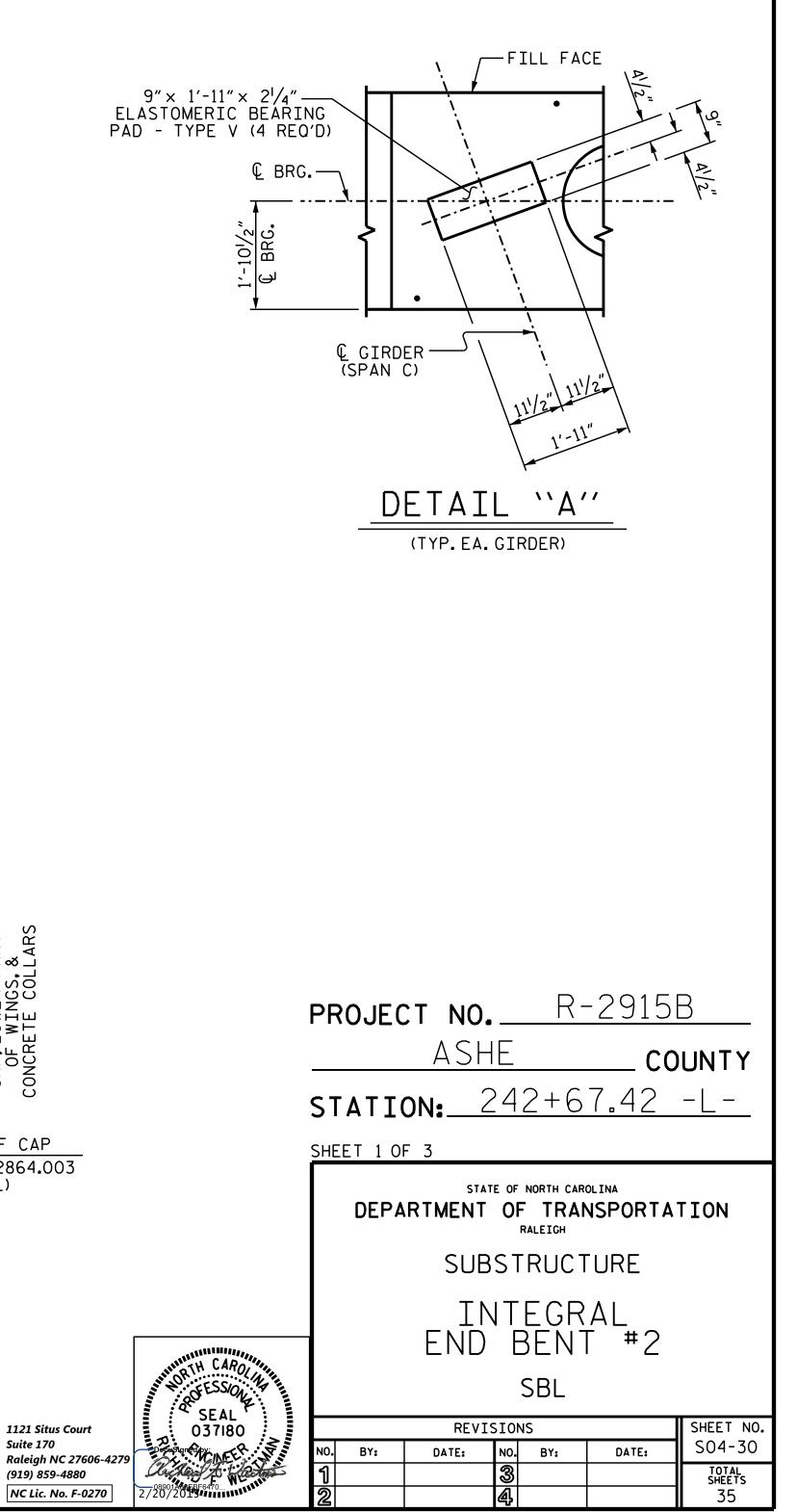
PLAN

1'-0" (TYP.) 37′-10<sup>15</sup>⁄16″ 5′-11<sup>13</sup>/16 10′-7<sup>11</sup>/<sub>16</sub>″ 10′-7<sup>11</sup>/<sub>16</sub>″ 10'-91/8" 10'-8" 10'-8" 11″ (TYP) シ 8-#4 D1 @ 1'-0"CTS. 2'-5<sup>3</sup>/<sub>16</sub>" (W3) (TYP.BETWEEN GIRDERS) (TYP.) 1'-10<sup>1</sup>/2" TO € BRGS. & PILES — FILL FACE 1-117/8" W.P. #4 — \*\*=\* \* - -• • • • ∕— € GDR.C4 - 2'-0" Ø CONCRETE COLLAR (TYP.EA.PILE) ∕— € GDR.C3 <u>1'-4¾″</u> CONTROL 1'-2<sup>1</sup>/2" 2'-5<sup>3</sup>/16" 8-#4 D1 @ 1'-0"CTS. (TYP.) LINE LT. (TYP.) (TYP. BETWEEN GIRDERS) (TYP.) 37′-75⁄<sub>16</sub>″ 8′-5″ 46′-05⁄<sub>16</sub>″

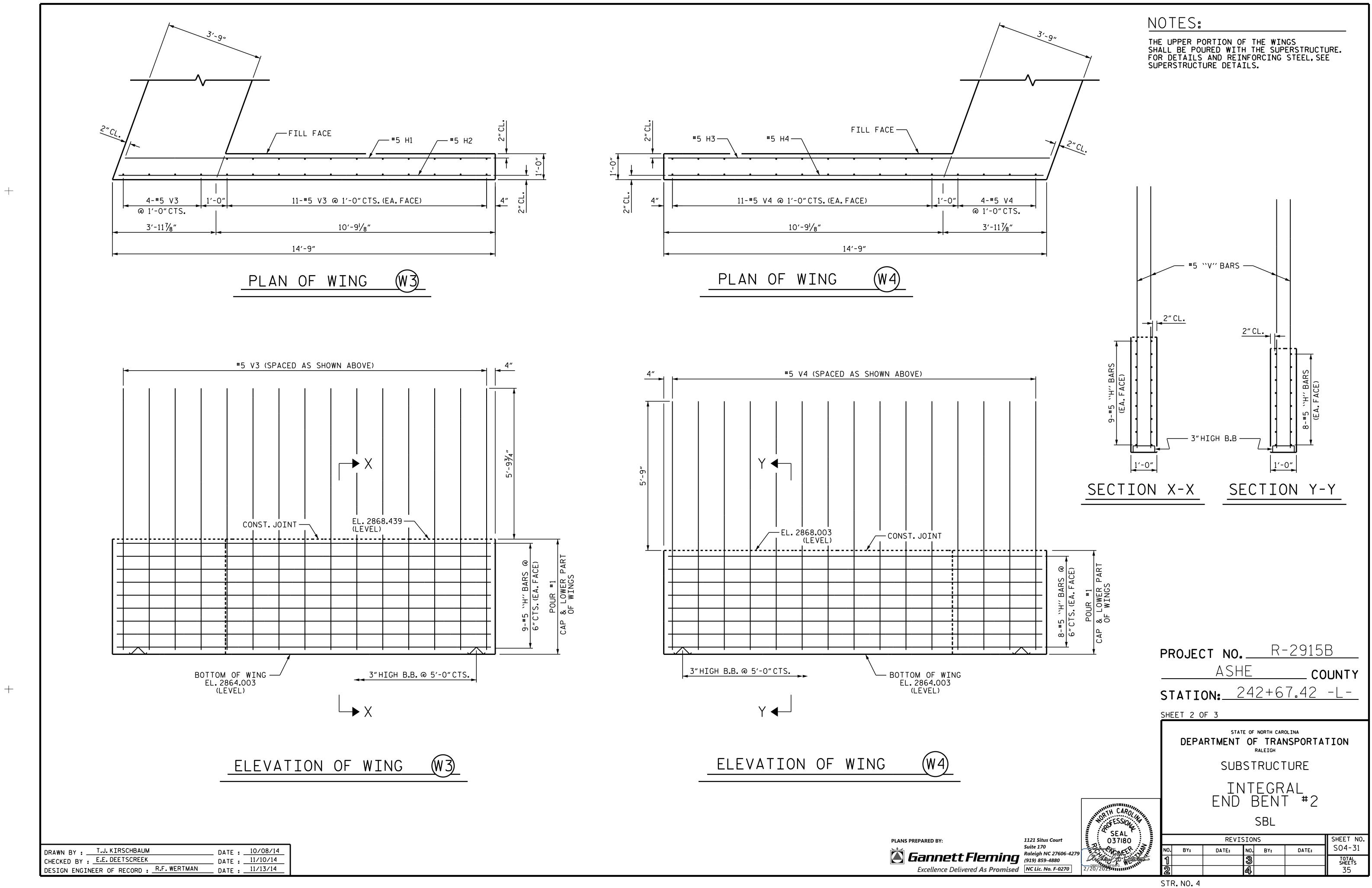
NOTES:

INSTALL THE 4"DIA.DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS.SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

#4 D1 BARS MAY BE SHIFTED SLIGHTLY TO AVOID STIRRUPS IN CAP. SEE SUPERSTRUCTURE SHEETS FOR UPPER PART OF INTEGRAL END BENT DETAILS.



STR.NO.4



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MINIMUM OF 3- ONE CUBIC -FOOT BAGS OF #78M STONE. BAGS SHALL BE OF POROUS FABRIC, SECURELY TIED. 6" (MIN.) PIPE FOR DRAINAGE GRADE TO DRAIN -TOE OF SLOPE

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

## TEMPORARY DRAINAGE AT END BENT

5-**#**4 B6

5-**#**9 B2

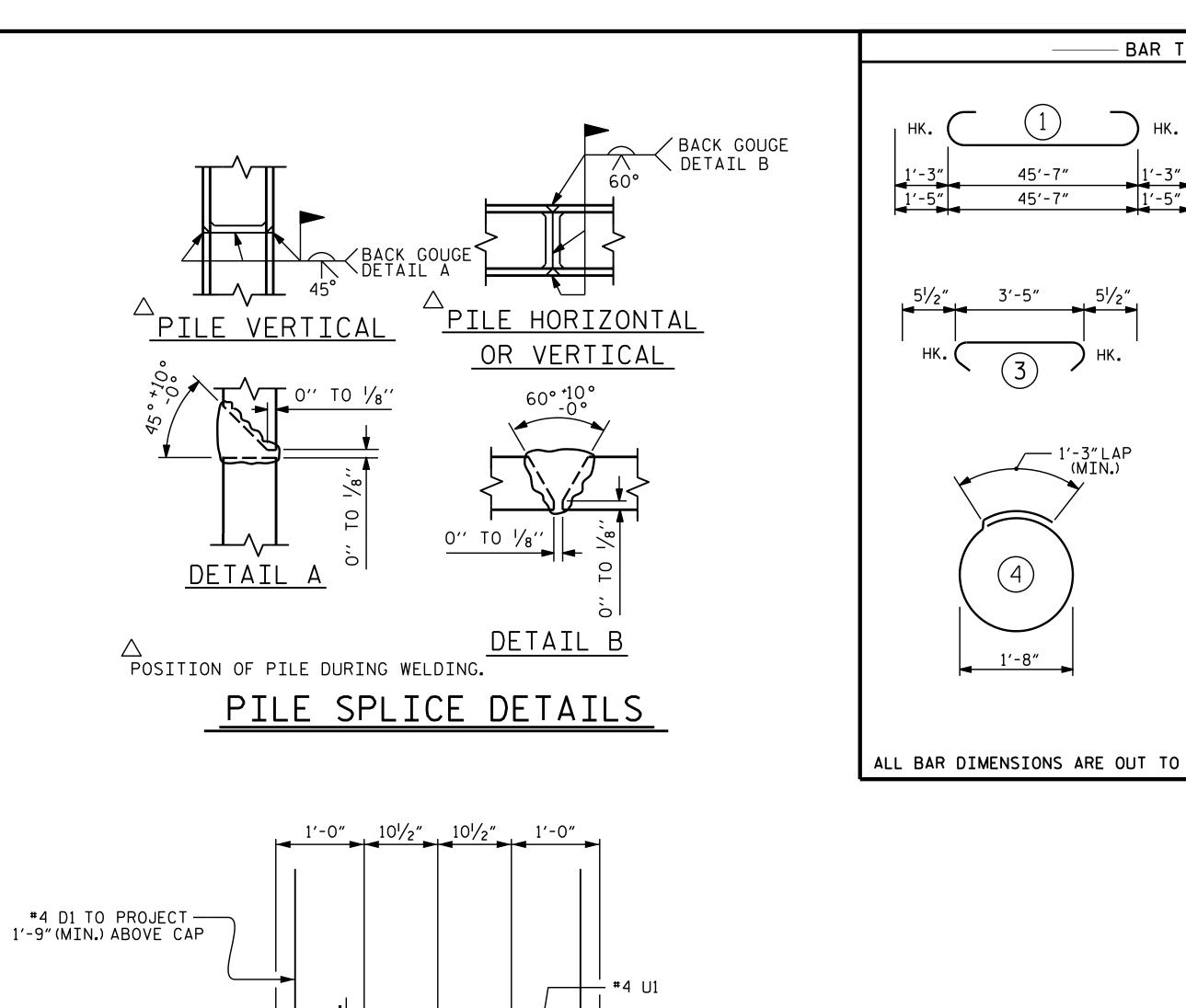
1-#5 B3 (EA.FACE)

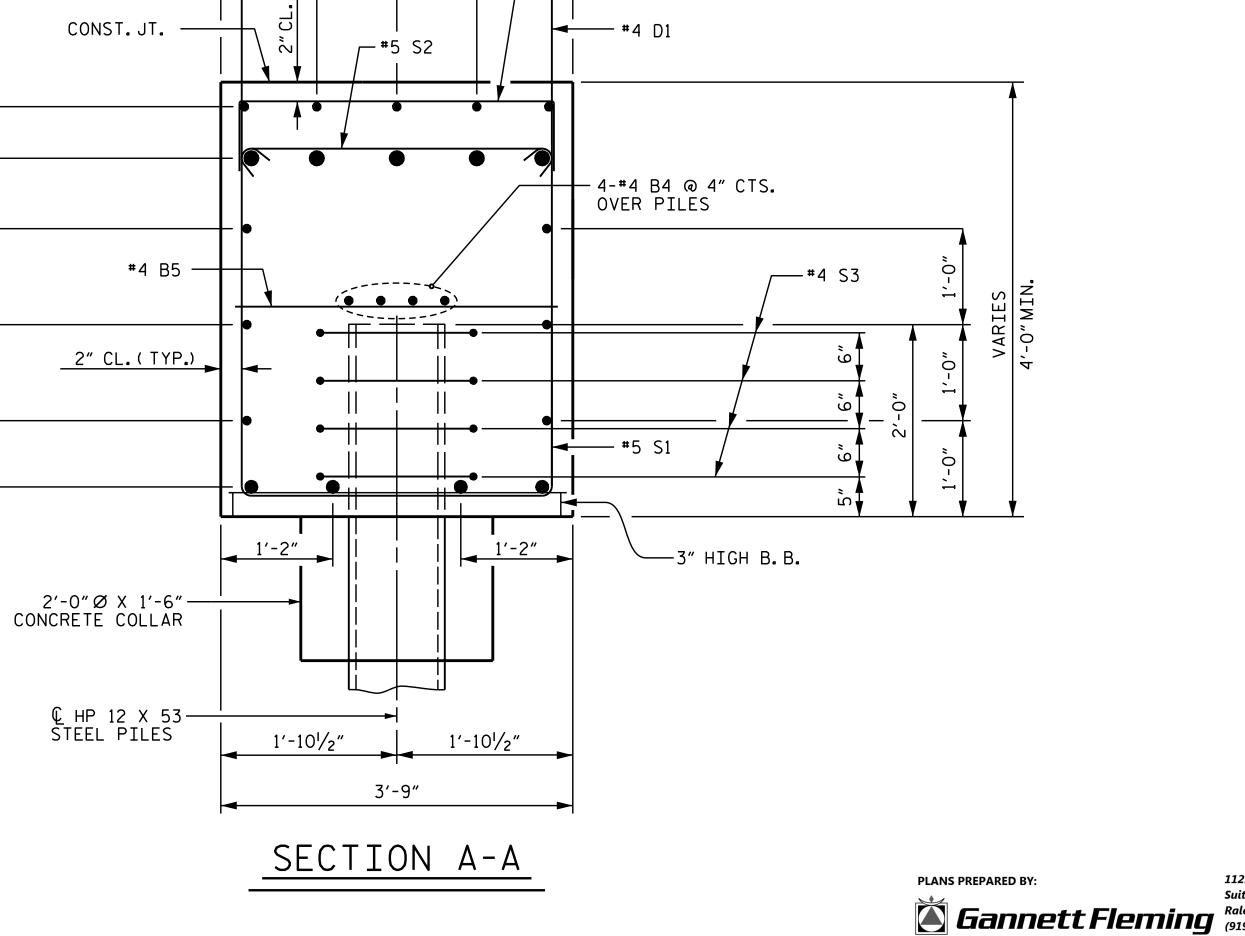
1-#5 B3 (EA.FACE)

1-#5 B3 (EA.FACE)

4-**#**10 B1

DRAWN BY :	T.J. KIRSCHBAUM	DATE :	10/08/14
	E.E. DEETSCREEK	DATE :	11/10/14
	EER OF RECORD : R.F. WERTMAN	DATE : .	11/13/14

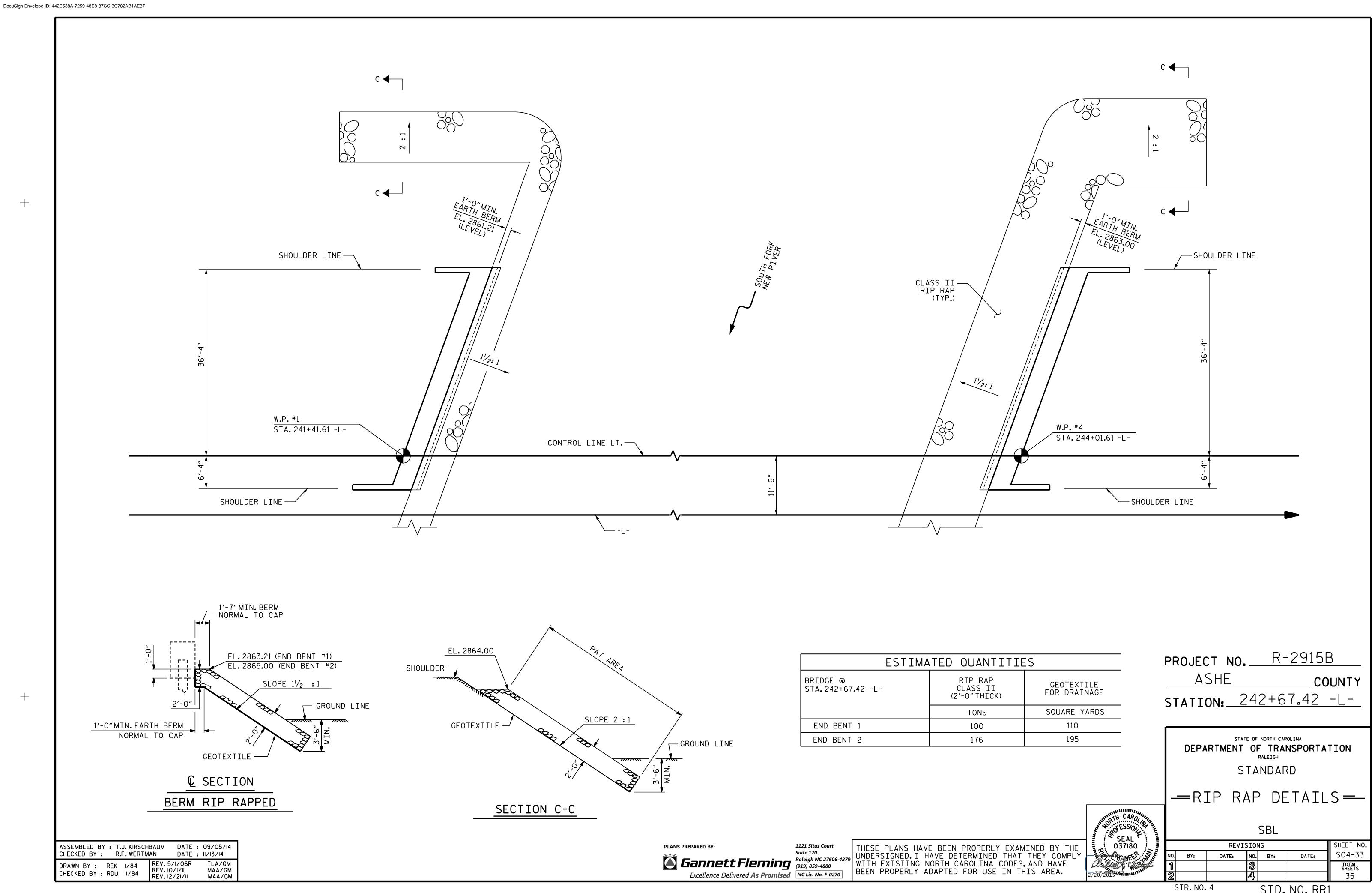




YPES —			BI	LL OF	ΜΑΤΕ	RIAL	
				END B	ENT	#2	
		BAR	No.	SIZE	TYPE	LENGTH	WEIGHT
1		B1	4	<b>#</b> 10	1	48′-5″	833
		B2	5	#9	1	48'-1"	817
B2	<b>_</b>	B3	6	<b>#</b> 5	STR	45′-8″	286
B1	HK.	B4	8	#4	STR	24'-1"	129
+		B5	12	#4	STR	3′-5″	27
	$\land \land \neg \neg \uparrow$	B6	10	#4	STR	11'-0"	73
	<sup>5</sup> ″	D1	68	#4	STR	6'-1"	276
	3'-71/2"						
	м. М	H1	9	<b>#</b> 5	STR	14'-1"	132
		H2	9	<b>#</b> 5	STR	14'-4"	135
		Н3	8	<b>#</b> 5	STR	14'-8"	122
	7/ 4//	H4	8	<b>#</b> 5	STR	14'-5″	120
	3'-4"						
		S1	37	<b>#</b> 5	2	11'-6″	444
		S2	37	<b>#</b> 5	3	4'-4"	167
		S3	32	#4	4	6'-6"	139
	3'-5"	U1	13	#4	5	6'-5"	56
<b>.</b>		٧3	26	#5	STR	10'-1"	273
	$\frown$	V4	26	#5	STR	9'-7"	260
1'-6"	(5)						
		REIN	NFORCIN	NG STEEL		4	289 LBS.
•							
				CLASS A			
		POUR #	1 (CAP,	LOWER WI		COLLARS)	
					ТОТ		31.5 C.Y.
				P 12 X 53	STEEL		
OUT.		No.	= 8			280	LIN.FT.

	PROJECT NO.       R-2915B         ASHE       COUNTY         STATION:       242+67.42       -L-         SHEET 3 OF 3       SHEET 3 OF 3
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE INTEGRAL END BENT #2
1121 Situs Court Suite 170 Raleigh NC 27606-4279 (919) 859-4880 NC Lic. No. F-0270	SBL         REVISIONS       SHEET NO.         NO.       BY:       DATE:       NO.       BY:       DATE:       SO4-32         1       3       5       35       35

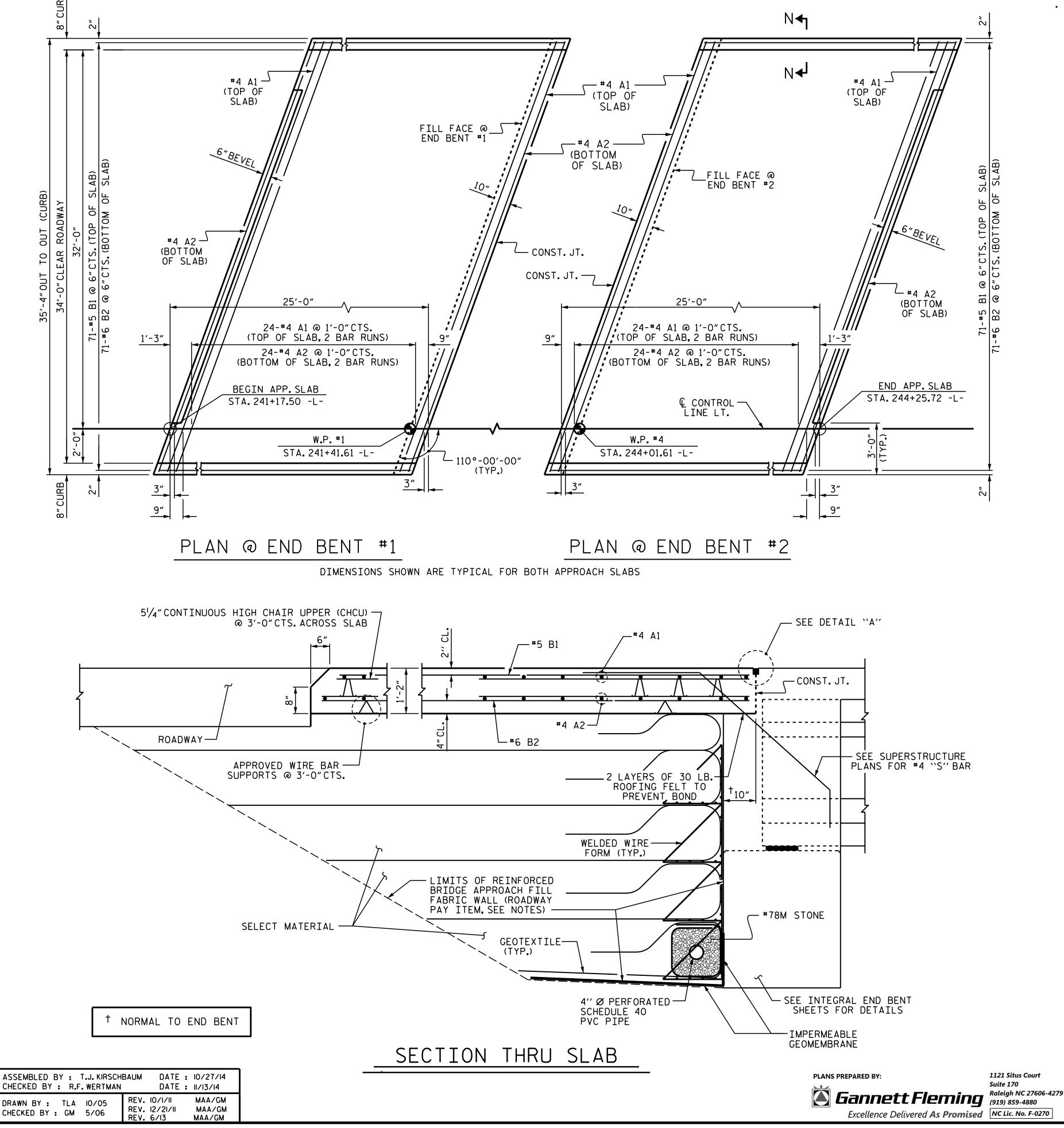
Excellence Delivered As Promised



MATED QUANTITIE
RIP RAP CLASS II (2'-0" THICK)
TONS
100
176

		<b>r</b>
PLANS PREPARED BY: <b>Gannett Fleming</b> Excellence Delivered As Promised	1121 Situs Court Suite 170 Raleigh NC 27606-4279 (919) 859-4880 NC Lic. No. F-0270	THESE PLANS HAVE UNDERSIGNED.I HAV WITH EXISTING NOF BEEN PROPERLY ADA

STD. NO. RR1



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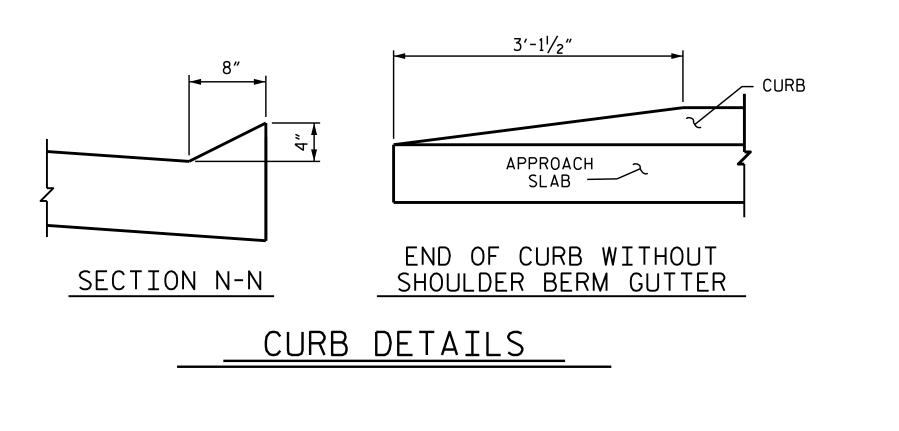
## **NOTES:**

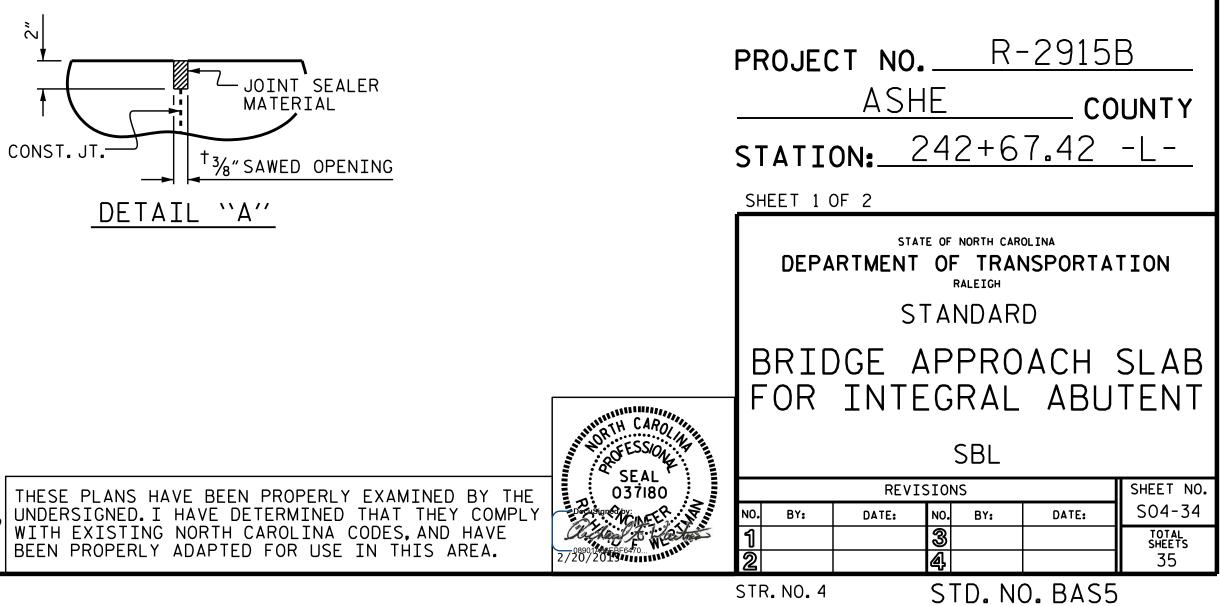
APPROACH SLAB SHALL NOT BE CONSTRUCTED PRIOR TO COMPLETION OF THE BRIDGE DECK.

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

THE JOINT OPENING AT THE APPROACH SLAB/DECK INTERFACE SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028-3 OF THE STANDARD SPECIFICATIONS.





1121 Situs Court Suite 170

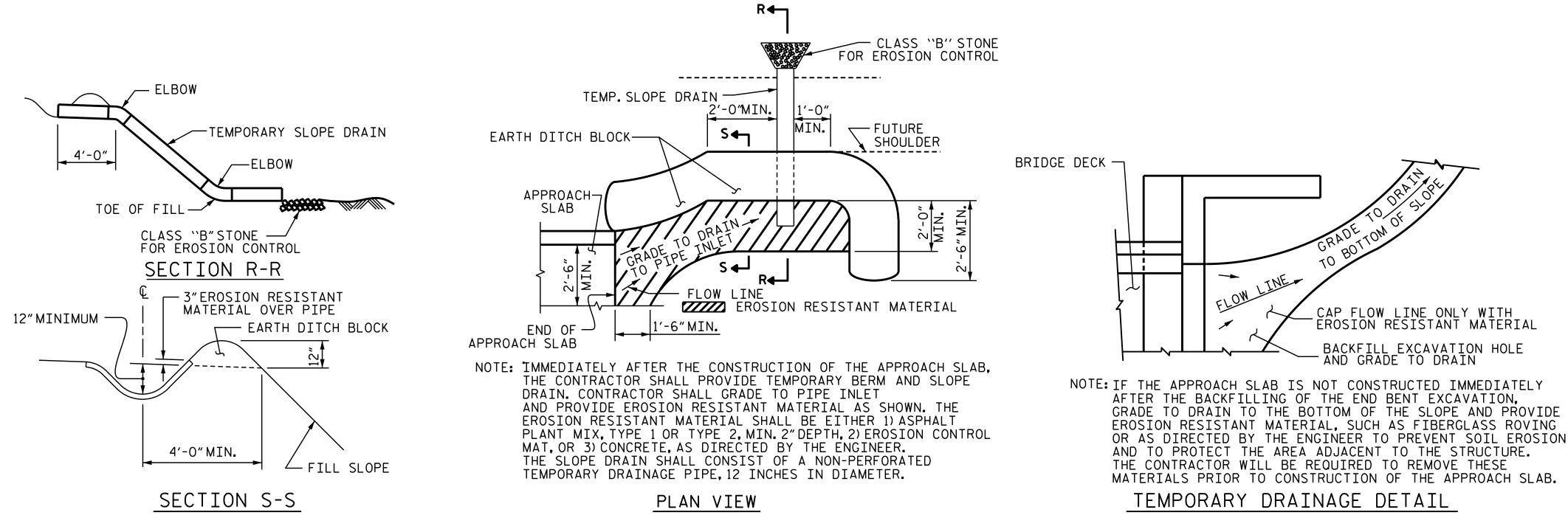
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BEEN I	PROPERL	Y AD	APTED	FOR	USE	ΙN	٦

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BILL OF MATERIAL							
FOR ONE APPROACH SLAB (2 REQUIRED)							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
<b>*</b> A1	5,2	<b>*</b> 4	SIR	19'-8"	683		
A2	5,2	<b>*</b> 4	STR	19'-6″	677.		
•	•	•	•	•	•		
<b>米</b> B1	7,1	<b>#.</b> 5	STR	24′-2″	179Q		
B2	7,1	<b>#</b> 6	STR	24′-8″	2631		
REINF	FORCI	NG STE	EL	LBS.	3308		
* EPOXY COATED . REINFORCING STEEL LBS. 2473							
<b>CLASS</b>	5 A A (	CONCRE	TE	C.Y.	40.7		

SPLICE LENGTHS CHART					
	EPOXY COATED	UNCOATED			
#4	2'-0"	1'-9"			
<b>#</b> 5	2'-6"	2'-2"			
<b>#</b> 6	3'-10"	2'-7"			

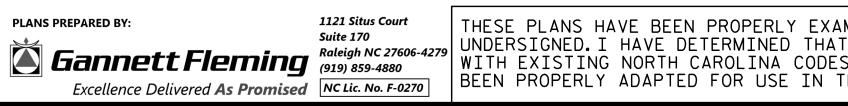
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ASSEMBLED BY : T.J.KIRSC CHECKED BY : R.F.WERTMA		10/27/14 11/13/14
DRAWN BY : FCJ II/88 CHECKED BY : ARB II/88	REV. 10/1/11 REV. 7/12 REV. 6/13	MAA/GM MAA/GM MAA/GM

## TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



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STR. NO. 4

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