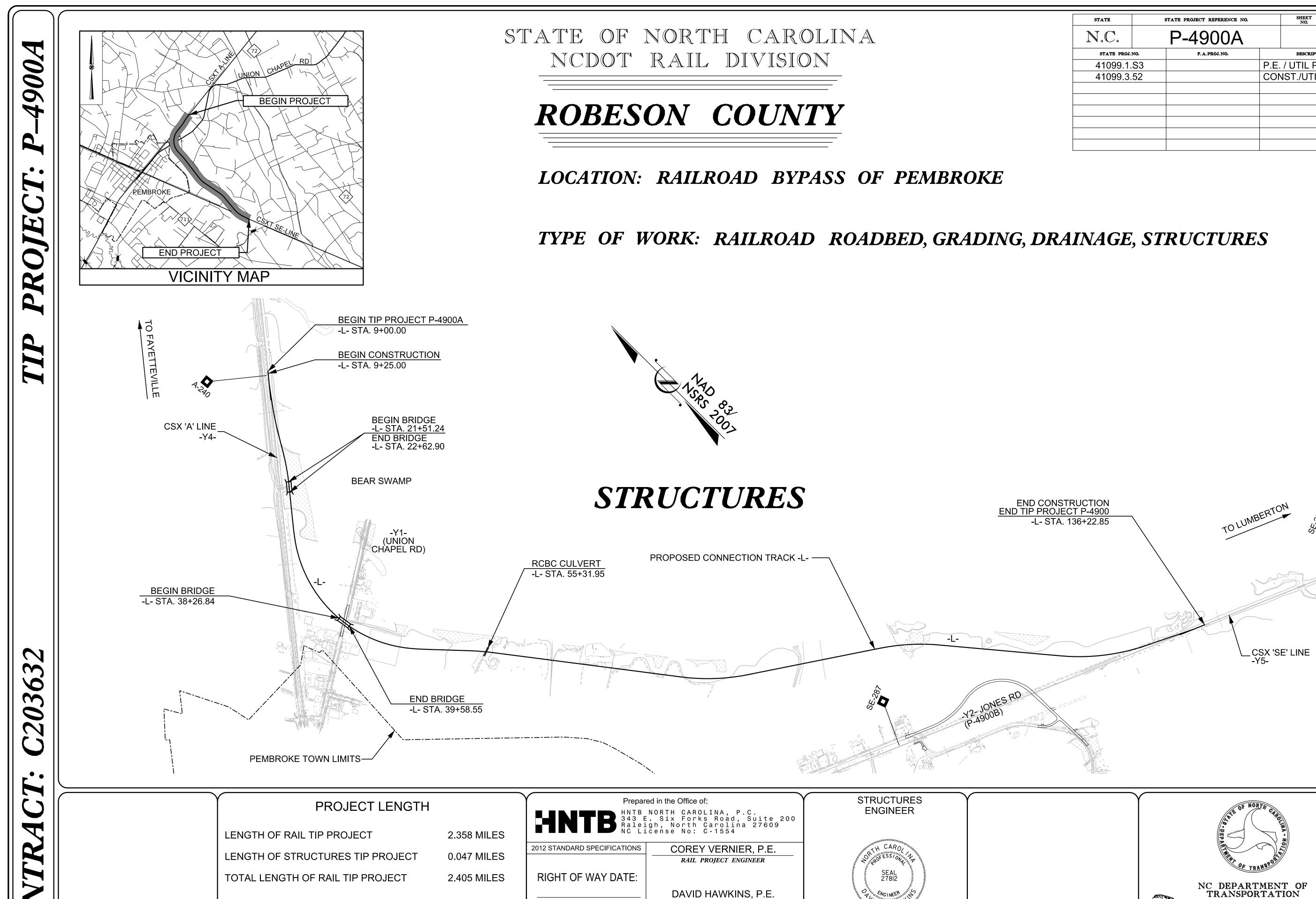
# This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page.

This file or an individual page shall not be considered a certified document.



LETTING DATE:

AUGUST 18, 2015

NCDOT CONTACT:

BRAD SMYTHE, P.E.

NCDOT PROJECT MANAGER

STRUCTURE PROJECT ENGINEER

BRAD SMYTHE, P.E.

NCDOT PROJECT MANAGER

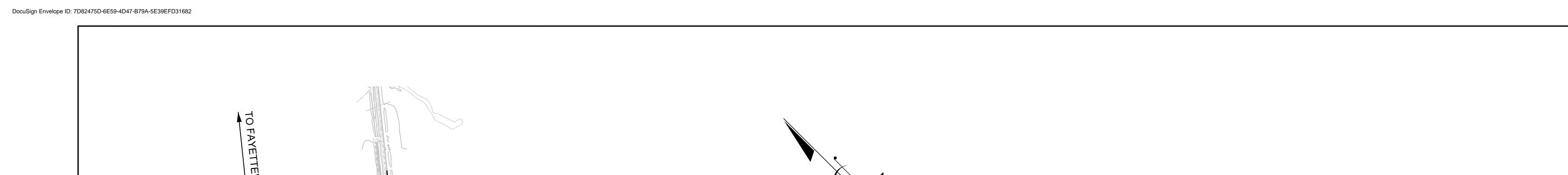
David W. Hawkins

6/22/2015

**SIGNATURE**:

P.E. / UTIL P.E. / ROW CONST./UTIL CONST

DESIGN AND CONSTRUCTION



Paul J. Barber SEAL 12916

SHEETS

S-1 THRU S-19

C-1 THRU C-6

S-20 THRU S-54

STATE OF NORTH CAROLINA David W. Hawkins DEPARTMENT OF TRANSPORTATION RALEIGH

PROJECT NO. P-4900A

ROBESON

COUNTY

INDEX

HNTB NORTH CAROLINA, P.C.
License No. C-1554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 REVISIONS SHEET NO. DATE: NO. BY: DATE: NO. BY: TOTAL SHEETS DRAWN BY J. BAYNE DATE 9/14
CHECKED BY D. HAWKINS DATE 10/14

SEAL 27812

CSX.A.TINE  1  BEAR SWAMP  CHAPEL RD)  PROPOSED CONNECTION TRACK-L-  2  3	TO LUMBERTON SUR
PEMBROKE TOWN LIMITS	CSX 'SE' LINE -Y5-

INDEX

DOUBLE 7 FT. × 6 FT. RCBC

STR. NO.

STATION

POS 22+07.07 -L-

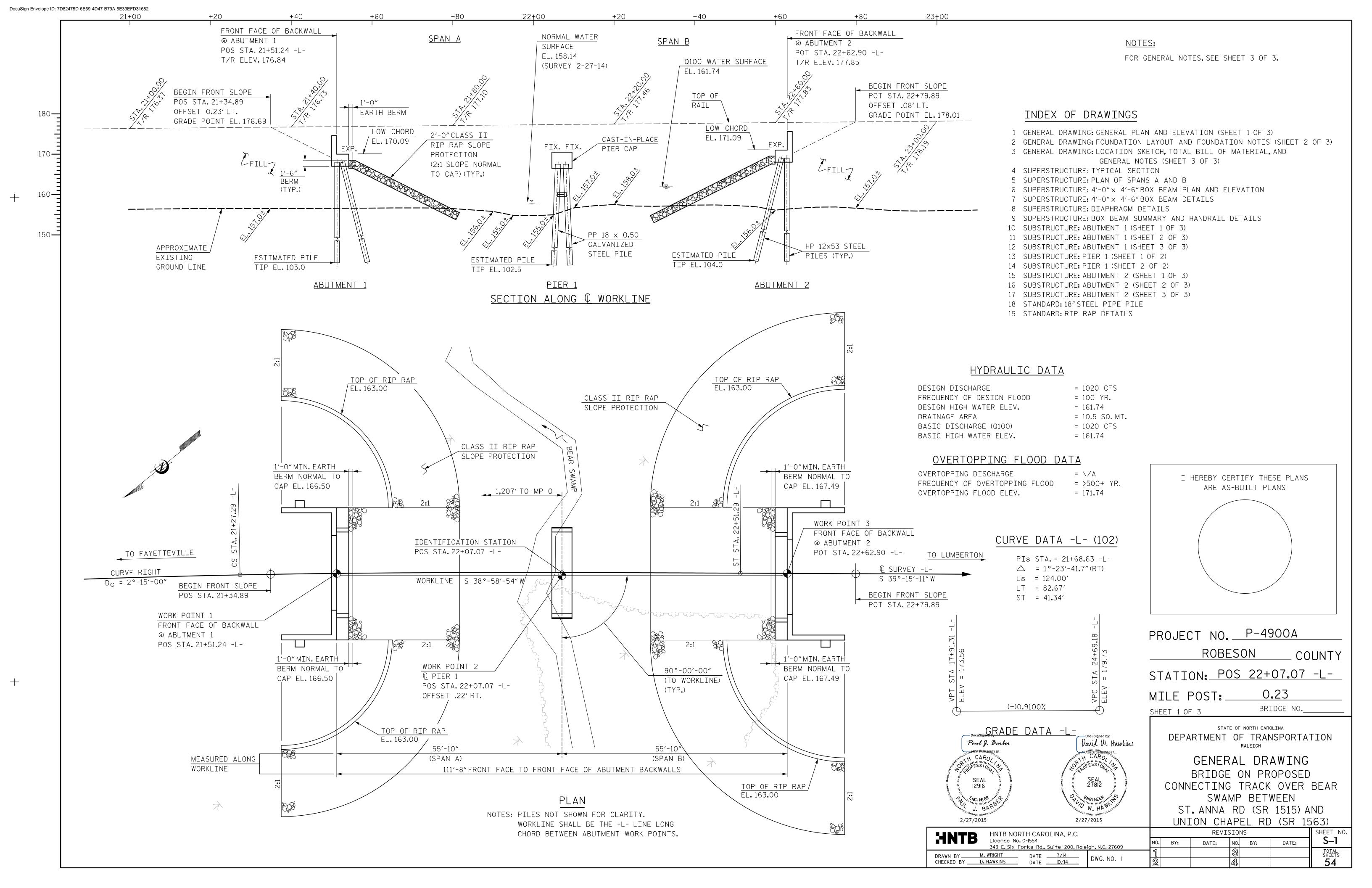
POC 38+93.36 -L-

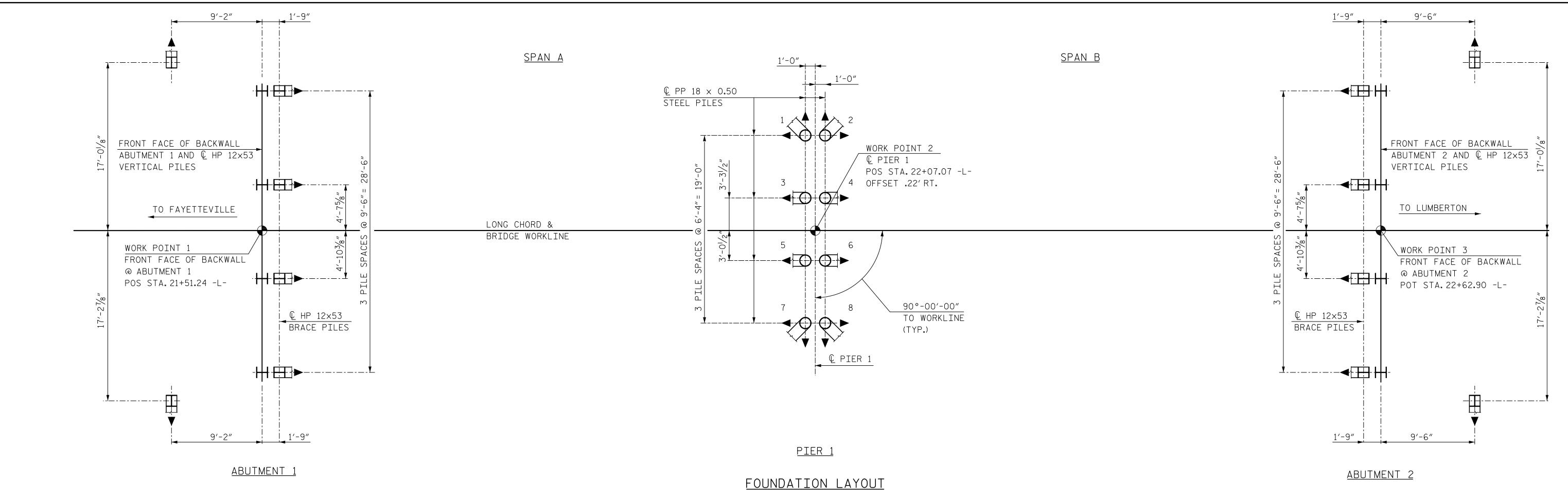
POC 55+31.95 -L-

DESCRIPTION

BRIDGE ON PROPOSED CONNECTING TRACK OVER BEAR SWAMP

BRIDGE ON PROPOSED CONNECTING TRACK OVER UNION CHAPEL ROAD





# FOUNDATION NOTES

FOR PILES, SEE SPECIAL PROVISIONS.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT ABUTMENT NO.1 AND ABUTMENT NO.2 IS 75 TONS PER PILE.

DRIVE PILES AT ABUTMENT NO.1 AND ABUTMENT NO.2 TO A REQUIRED BEARING CAPACITY OF 150 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT PIER NO.1 IS 115 TONS PER PILE.

DRIVE PILES AT PIER NO.1 TO A REQUIRED BEARING CAPACITY OF 240 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO PLUS ANY ADDITIONAL CAPACITY TO ACCOUNT FOR DOWNDRAG OR NEGATIVE SKIN FRICTION AND SCOUR.

INSTALL PILES AT PIER NO.1 TO A TIP ELEVATION NO HIGHER THAN 112.0 FT.

PIPE PILE PLATES ARE REQUIRED FOR PIPE PILES AT PIER NO.1. USE PIPE PILE PLATES WITH A DIAMETER EQUAL TO THE PIPE PILE DIAMETER.

STEEL PILE POINTS ARE REQUIRED FOR STEEL PILES AT ABUTMENT NO. 1 AND ABUTMENT NO. 2.

TESTING THE FIRST PRODUCTION PILE AT ABUTMENT NO.1 AND ABUTMENT NO.2 WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. FOR PDA TESTING, SEE SPECIAL PROVISIONS.

TESTING THE FIRST PRODUCTION PILE AT PIER NO.1 WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. FOR PDA TESTING. SEE SPECIAL PROVISIONS.

THE SCOUR CRITICAL ELEVATION FOR PIER NO.1 IS ELEVATION 141 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

DO NOT DRIVE PILES AT ABUTMENT NO.1 UNTIL AFTER OBSERVING A 3 MONTH WAITING PERIOD BEGINNING WHEN SURCHARGE HAS BEEN PLACED AS SHOWN IN RAIL PLANS FOR A MINIMUM DISTANCE OF 300 FT FROM ABUTMENT NO.1. SEE RAIL PLANS AND SURCHARGE SPECIAL PROVISIONS FOR SURCHARGE DETAILS.

DO NOT DRIVE PILES AT ABUTMENT NO.2 UNTIL AFTER OBSERVING A 3 MONTH WAITING PERIOD BEGINNING WHEN SURCHARGE HAS BEEN PLACED AS SHOWN IN RAIL PLANS FOR A MINIMUM DISTANCE OF 300 FT FROM ABUTMENT NO.2. SEE RAIL PLANS AND SURCHARGE SPECIAL PROVISIONS FOR SURCHARGE DETAILS.

NOTES:

ABUTMENTS AND PIERS ARE PARALLEL.

ALL DIMENSIONS ARE PARALLEL OR NORMAL TO © PIER 1 AND FRONT FACES OF ABUTMENT BACKWALLS.

INDICATES HP 12x53 PILE BATTER IN DIRECTION SHOWN.

INDICATES PP 18x0.50 PILE BATTER IN

BRACE PILES @ ABUTMENTS ARE TO BE BATTERED @ 3:12.

DIRECTION SHOWN.

BRACE PILES 1, 2, 7 & 8 AT PIER ARE TO BE

BATTERED @ 1/2:12 LONGITUDINALLY AND

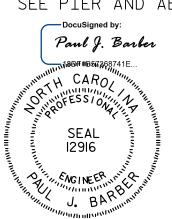
BATTERED @  $\frac{1}{2}$ :12 LONGITUDINALLY.

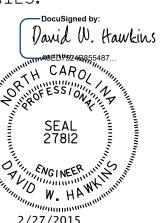
 $1\frac{1}{2}$ :12 TRANSVERSELY.

BRACE PILES 3, 4, 5 & 6 AT PIER ARE TO BE

ALL PILE DIMENSIONS ARE TO CENTERS OF PILES AT BOTTOM OF ABUTMENTS OR BOTTOM OF PIER 1.

FOR FOUNDATION ELEVATIONS AND DETAILS, SEE PIER AND ABUTMENT DETAILS.





PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L-

MILE POST: 0.23

SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOUNDATION LAYOUT

HNTB	HNTB NORTH CAROLINA, P.C.					SHEET NO.				
JUID	License No.C-I5	0, Raleigh, N.C. 27609	NO.	BY:	DATE:	NO.	BY:	DATE:	S-2	
DRAWN BY	J. BAYNE [	DATE <u>8/14</u>	DWG. NO. 2				<b>®</b> ₹			TOTAL SHEETS
CHECKED BY	D. RAGAN [	DATE <u>1/15</u>		2			4			54

### GENERAL NOTES

ASSUMED LIVE LOAD = AREMA E80 W/FULL DIESEL IMPACT OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING, "VOL. 2 STRUCTURES" AND CSX TRANSPORTATION'S "UNDERGRADE BRIDGE CRITERIA INCLUDING BALLAST DECK" AND THE STRUCTURE SPECIAL PROVISIONS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH SEISMIC PERFORMANCE CATEGORY A REQUIREMENTS OF THE AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", AND CHAPTER NINE "SEISMIC DESIGN FOR RAILWAY STRUCTURES" OF THE CURRENT EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING. "VOL. 2 STRUCTURES".

WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) BRIDGE WELDING CODE D1.5 AS AMENDED AND COMPLEMENTED BY THE AREMA MANUAL.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REINFORCING STEEL SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM DESIGNATION A615, GRADE 60. ALL DIMENSIONS RELATING TO BAR SPACING ARE TO BAR CENTERS UNLESS NOTED. FABRICATION TO BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE", A.C.I. 315-80.

WATERPROOFING: ALL CONSTRUCTION JOINTS AND ANY VISIBLE SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL SHALL BE WATERPROOFED WITH A COLD LIQUID APPLIED ELASTOMERIC MEMBRANE. FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

WATERPROOFING IS REQUIRED AT THE FOLLOWING LOCATIONS:

1. ALONG FULL LENGTH OF HORIZONTAL CONSTRUCTION JOINT AT TOP OF CAP
WITH BACKWALL AT EACH ABUTMENT (FILL FACE OF BACKWALL AND WINGWALLS
ONLY)

2. AT ANY CONCRETE SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL.

WATERPROOFING FOR LOCATIONS 1 AND 2 ABOVE SHALL BE 24"WIDE AND SHALL BE CENTERED OVER JOINTS OR CRACKS.

DAMPPROOFING SHALL BE APPLIED TO THE FOLLOWING SURFACES REGARDLESS OF WHETHER OR NOT THE PLAN DETAILS SO INDICATE:

1. ON FILL FACE OF FOOTING AND BACKWALL AT EACH ABUTMENT.

2. ON FILL FACE OF ABUTMENT WINGWALLS FROM BOTTOM OF WINGWALL TO TOP OF BACKWALL ELEVATION AT EACH ABUTMENT.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS.PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR REINFORCING, CONCRETE AND DAMPPROOFING, SEE "CAST-IN-PLACE CONCRETE" SPECIAL PROVISIONS.

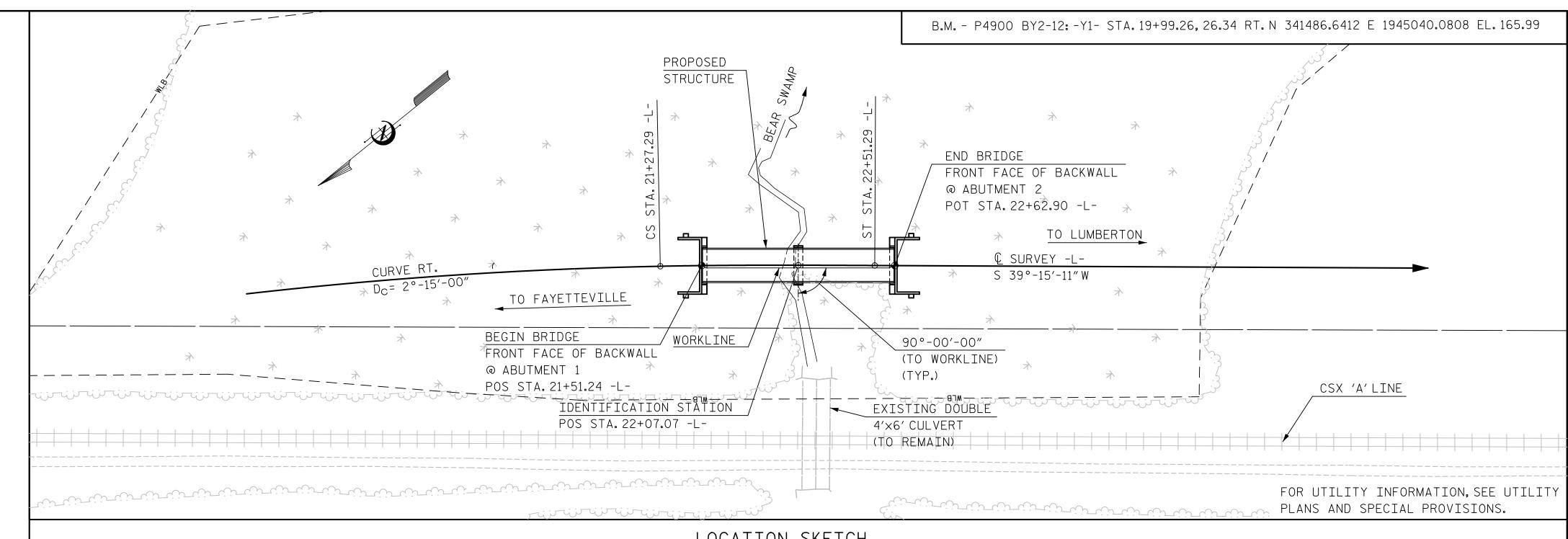
THE SCOUR CRITICAL ELEVATION FOR PIER NO.1 IS ELEVATION 145.5 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

FOR INTERIOR PIER NO.1, ONLY PARTIAL GALVANIZING OF THE PILES IS REQUIRED. SEE PIER 1 SHEET(S) FOR REQUIRED GALVANIZED LENGTHS. PAYMENT FOR PARTIALLY GALVANIZED PILES WILL BE MADE UNDER THE CONTRACT UNIT PRICE FOR GALVANIZED STEEL PILES.

ONLY ITEMS IN THE PROPOSAL AS PAY ITEMS WILL BE PAID FOR COMPENSATION FOR ALL LABOR, MATERIALS, TOOLS, EQUIPMENT, AND INCIDENTALS FOR THE ENTIRE CONTRACT SHALL BE INCLUDED IN THE PRICE BID FOR PAY ITEMS.

CONTROL OF WORK: ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILROAD STRUCTURE SHALL BE PERFORMED TO THE SATISFACTION OF THE ENGINEER AND/OR CSX TRANSPORTATION. ALL METHODS OF HANDLING WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY ENGINEER BEFORE PROCEEDING WITH THAT PORTION OF THE WORK. RAIL TRAFFIC SHALL, AT ALL TIMES, BE MAINTAINED AND PROTECTED. THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH RAIL OPERATIONS.

ALL CONSTRUCTION JOINTS SHOWN ON THESE PLANS SHALL BE REQUIRED UNLESS SHOWN AS OPTIONAL. CONSTRUCTION JOINTS SHALL NOT BE PERMITTED EXCEPT AS SHOWN ON THESE PLANS, OR WHERE WRITTEN APPROVAL HAS BEEN OBTAINED.



LU	CAI	TON	SKE	СП

	TOTAL BILL OF MATERIAL																
	PDA TESTING	CAST-IN-PLACE CONCRETE (4,000 PSI)	REINFORCING STEEL		2 x 53 . PILES	GALV	× 0.50 ANIZED PILES	STEEL PILE POINTS	PIPE PILE PLATES	PILE REDRIVES	CONCRETE BALLAST CURB	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PREST CON	× 4'-6" TRESSED ICRETE BEAMS	STEEL HANDRAIL
	EACH	CU. YDS.	LBS.	NO.	L.F.	NO.	L.F.	EACH	EACH	EACH	L.F.	TONS	SQ. YDS.	LUMP SUM	NO.	L.F.	L.F.
SUPERSTRUCTURE											222.6			LUMP SUM	10	556.5	222.8
ABUTMENT 1		36.8	5,713	10	650			10		10		281	312				38.2
PIER 1		16.5	4,784			8	520		8	8							
ABUTMENT 2		37.4	5,739	10	650			10		10		342	379				39.2
TOTAL	3	90.7	16,236	20	1,300	8	520	20	8	28	222.6	623	691	LUMP SUM	10	556.5	300.2

FOR STEEL HANDRAIL, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR BACKFILL BEHIND ABUTMENTS AND OTHER BACKFILL AROUND THE STRUCTURE, SEE SPECIAL PROVISION "BACKFILLING AROUND STRUCTURES".

FOR WATERSTOPS, SEE SPECIAL PROVISIONS.

FOR RAILROAD TRACKWORK. SEE RAILROAD TRACKWORK PLANS.

FOR CONCRETE BALLAST CURB, SEE SPECIAL PROVISIONS.

FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

METAL DRAINS BEHIND ABUTMENTS SHALL BE AS SHOWN ON THE PLANS OR IN THE SPECIAL PROVISIONS. DETAILS OF THE DRAINAGE SYSTEM SHALL BE SUBMITTED FOR REVIEW.

FOR FOUNDATION NOTES, SEE "FOUNDATION LAYOUT" SHEET.

FOR 4'-0" x 4'-6" PRESTRESSED CONCRETE BOX BEAM, SEE SPECIAL PROVISIONS.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY 2012, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS, EXCEPT AS NOTED HEREIN, ELSEWHERE ON THE PLANS, OR IN THE SPECIAL PROVISIONS.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. THE USE OF A TEMPORARY CAUSWAY OR WORK BRIDGE IS NOT PERMITTED.

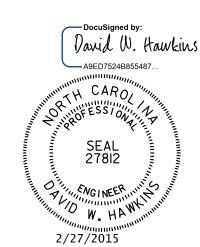
TOTAL BILL OF MATERIAL									
	STRUCTURE DRAINAGE SYSTEM AT STATION 22+07.07 -L-	DRAINAGE SYSTEM AT STATION							
	LUMP SUM	SQ. YDS.	SQ. YDS.						
SUPERSTRUCTURE	LUMP SUM								
ABUTMENT 1		46.6	12.3						
PIER 1									
ABUTMENT 2		47.6	12.5						
TOTAL	LUMP SUM	94.2	24.8						

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -LMILE POST: 0.23

SHEET 3 OF 3

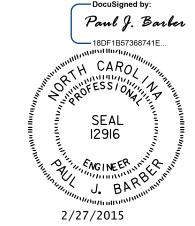


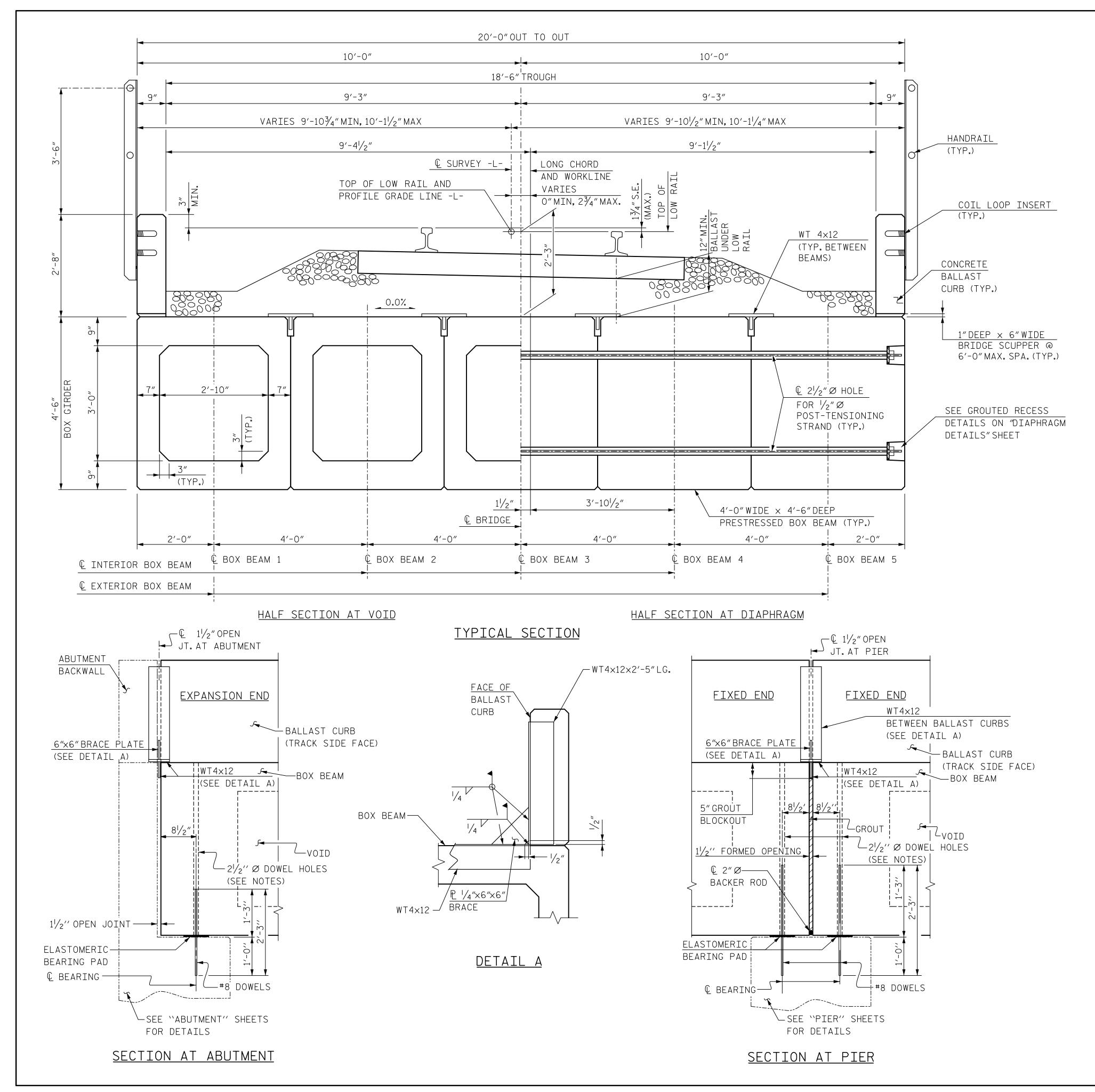
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

LOCATION SKETCH, TOTAL BILL OF MATERIAL AND GENERAL NOTES





### NOTES:

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO ASTM SPECIFICATION A416 (AASHTO M203) EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. INITIAL PRESTRESSING FORCE IS 31.0 KIPS PER  $\frac{1}{2}$   $\varnothing$  STRAND.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL CONFORM TO CURRENT ASTM SPECIFICATION A615, GRADE 60.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2^{1}/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT. THE  $2^{1}/_{2}$ " Ø DOWEL HOLES AT EXPANSION ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH JOINT SEALER MATERIAL TO  $1^{1}/_{2}$ " ABOVE THE TOP OF DOWELS AND THEN FILLED WITH GROUT.

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. THE 2" Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5.000 PSI.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS AND COATED WITH ASPHALT MASTIC.

THE LOCATION OF THE BOX VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

CURB CONCRETE SHALL BE CAST-IN-PLACE CONCRETE (5,000 PSI).

CURBS ARE NOT TO BE CAST ON THE GIRDERS UNTIL AFTER RELEASE OF PRESTRESSING FORCE.

FOR HANDRAIL DETAILS, SEE "BOX BEAM SUMMARY AND HANDRAIL DETAILS" SHEET.

WT MEMBERS AND BRACE PLATE SHALL BE ASTM A36 STEEL AND SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE WT MEMBER AND BRACE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

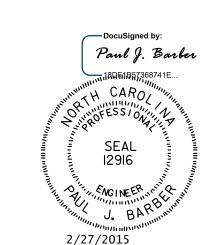
ALL CONCRETE IN PRECAST BOXES SHALL BE PER SECTION 1078 OF THE NCDOT STANDARD SPECIFICATIONS. CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 7,000 PSI.

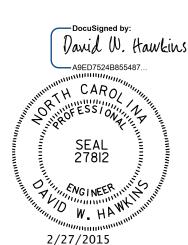
THE COST OF THE DIAPHRAGM POST-TENSIONING, JOINT SEALER, GROUT, BACKER ROD, ASPHALT MASTIC, AND WT  $4\times12$  WITH BRACE PLATES SHALL BE INCLUDED IN THE PRICE OF THE  $4'-0''\times4'-6''$  PRESTRESSED BOX GIRDER.

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L
MILE POST: 0.23





STATE OF NORTH CAROLINA

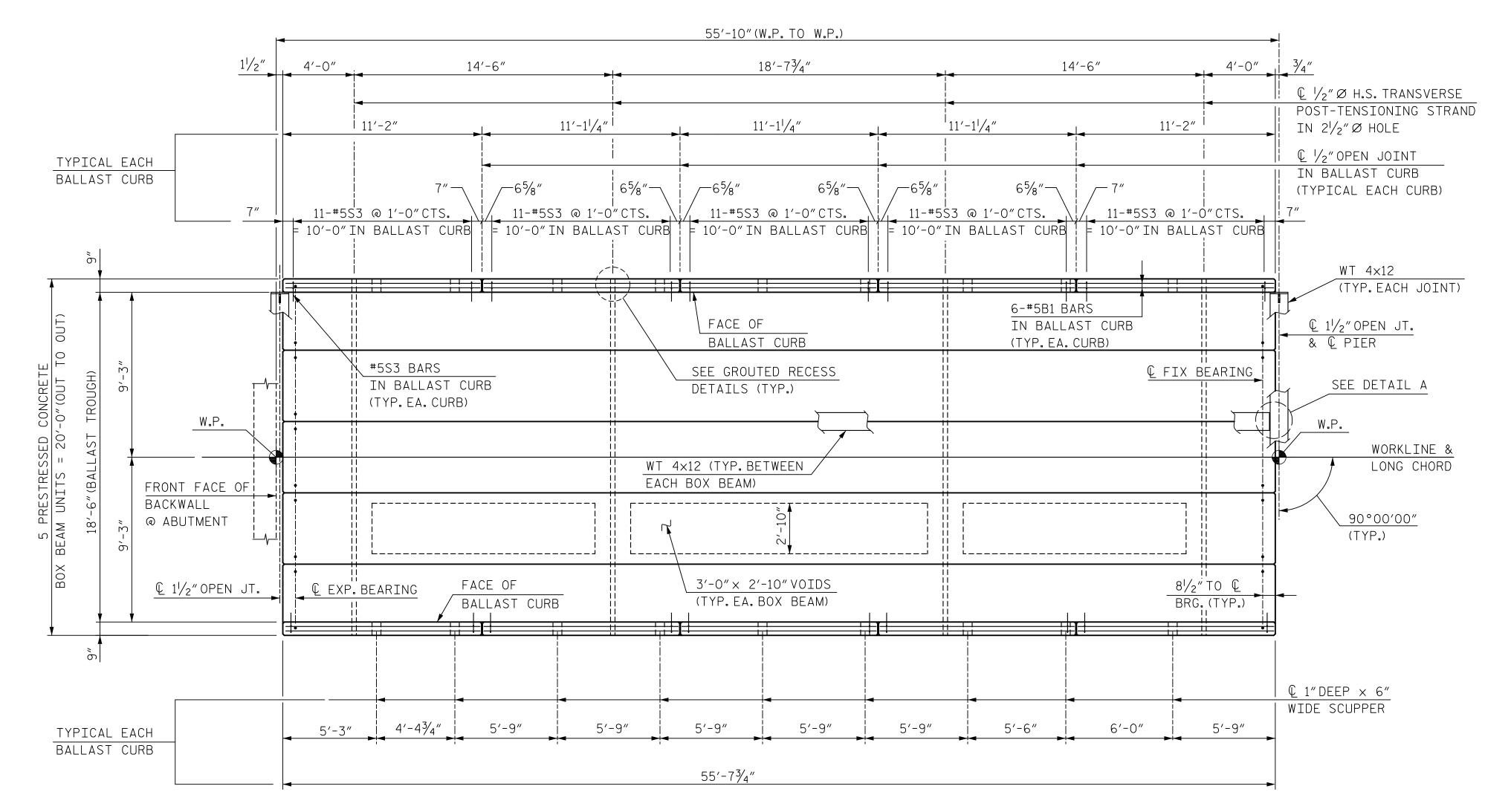
DEPARTMENT OF TRANSPORTATION

RALEIGH

SUPERSTRUCTURE

TYPICAL SECTION

, ,	. ,							
HNTB	HNTB NORTH CAROLINA, P.C.			REVIS	10I	٧S		SHEET NO.
HILD	License No. C-1554 343 E. Six Forks Rd Suite 200. Raleigh. N.C. 27609	NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
DDAWN DV	L BANGE	<b>一</b> 们			3			TOTAL SHEETS
DIVA 1111 D1	DATE 8/14 DWG. NO. 4  DEBARBER DATE 9/14	2			<u>ă</u>			54



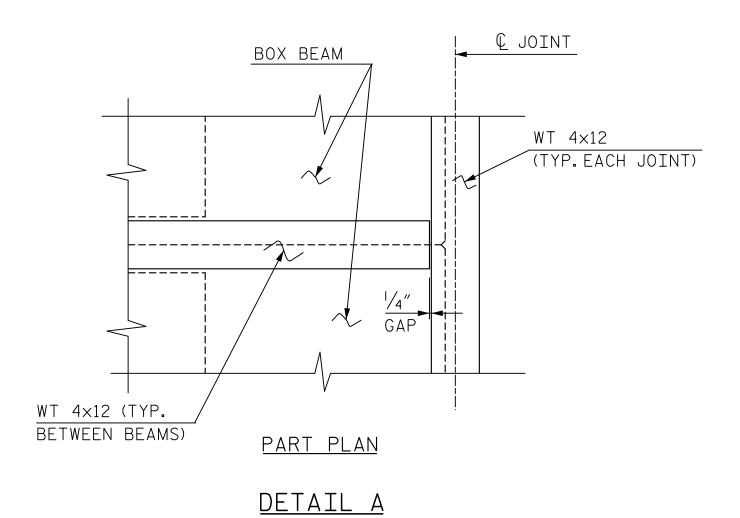
TYPICAL PLAN OF END SPAN (SPAN A SHOWN, SPAN B OPPOSITE HAND)

### NOTES:

FOR GROUTED RECESS DETAILS, SEE "DIAPHRAGM DETAILS" SHEET.

FOR BALLAST CURB AND HANDRAIL DETAILS, SEE "BOX BEAM SUMMARY AND HANDRAIL DETAILS" SHEET.

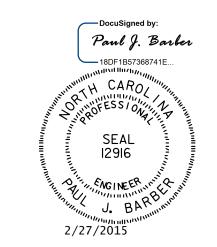
SCUPPER LOCATIONS MAY BE ADJUSTED SLIGHTLY IN FIELD TO AVOID "S3" BARS FROM BOX BEAM. CARE SHALL BE TAKEN TO RELOCATE ANY SCUPPERS AWAY FROM DIAPHRAGM GROUTED RECESSES AND OPEN JOINTS IN BALLAST CURB.

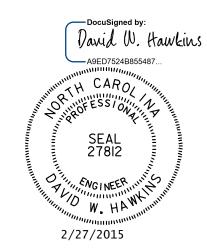


PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L
MILE POST: 0.23

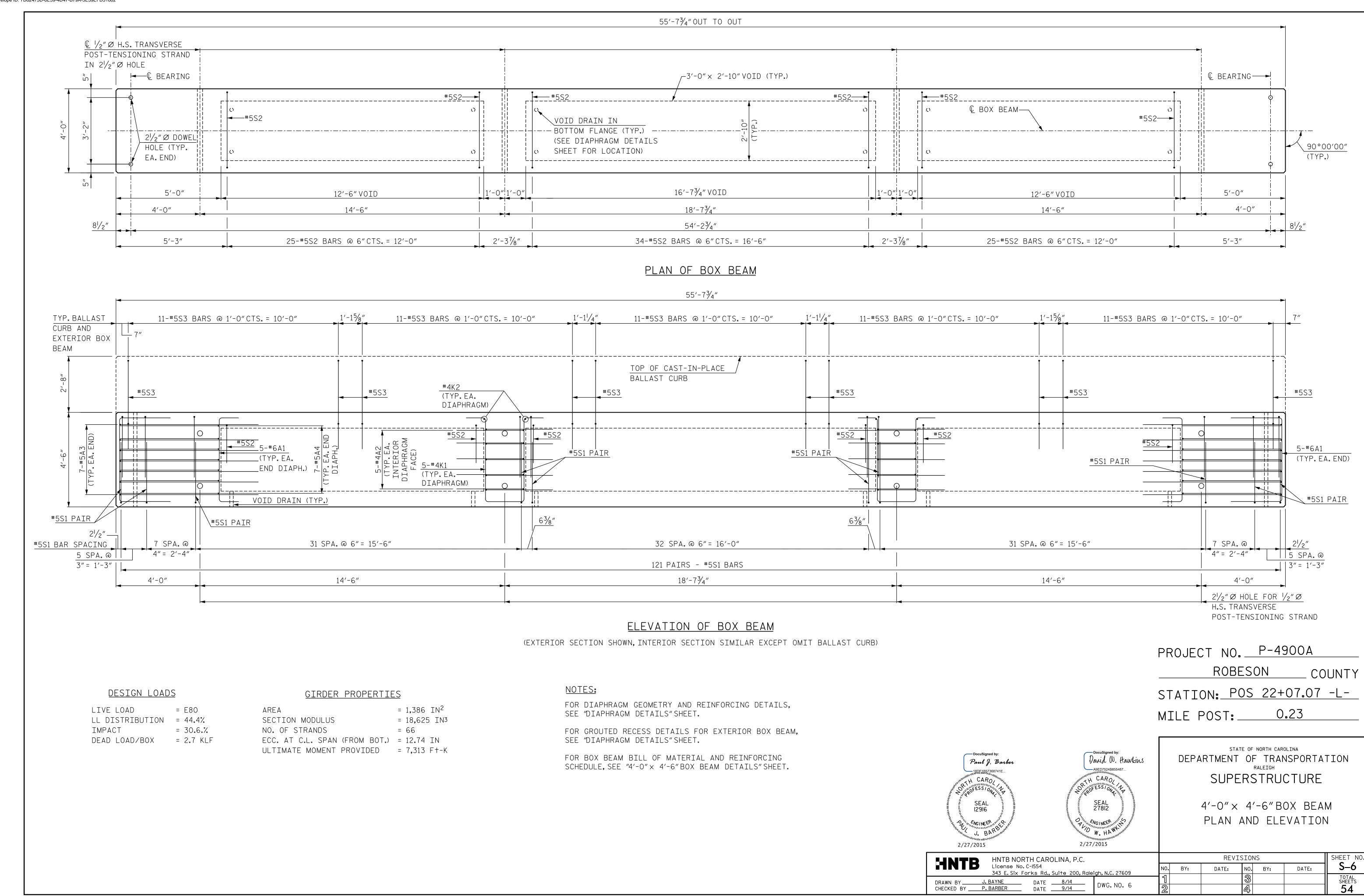


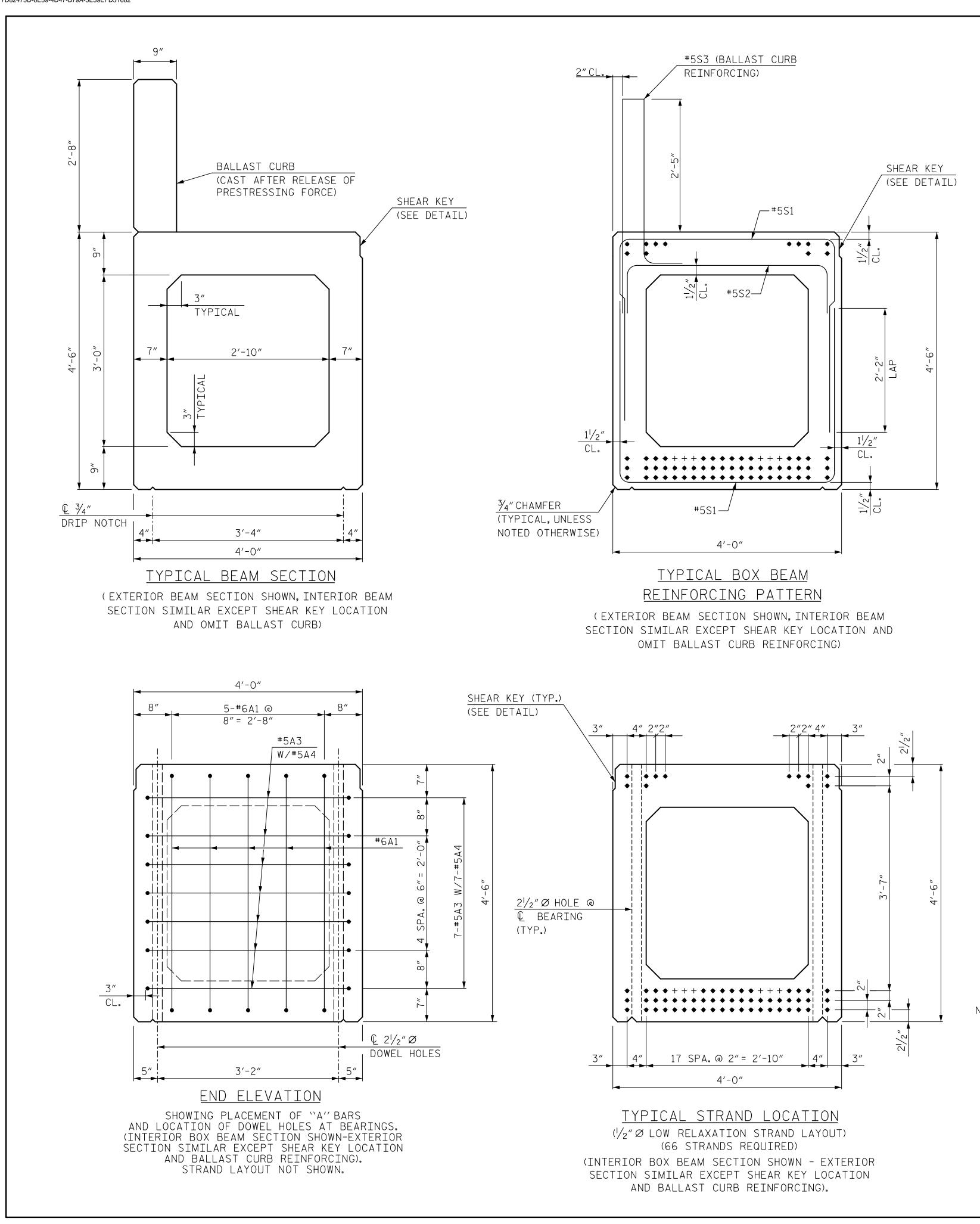


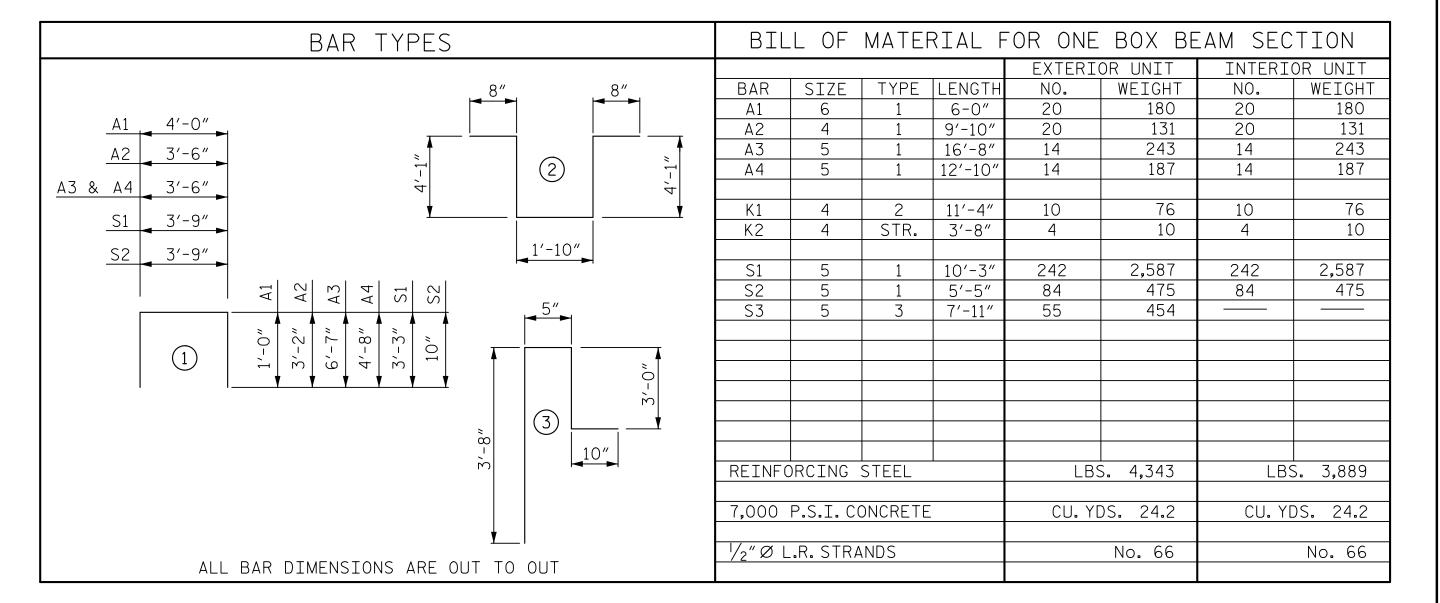
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUPERSTRUCTURE

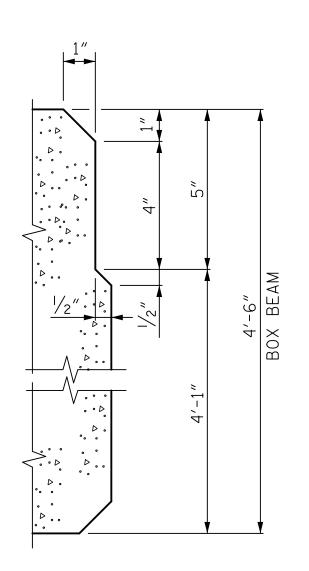
PLAN OF SPANS A AND B

HNTB NORTH CAROLINA, P.C. License No. C-1554					SHEET NO.				
HRID	License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609		NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
DRAWN BYJ.	BAYNE DATE 8/14		1			3			TOTAL SHEETS
	BARBER DATE 9/14	DWG. NO. 5	2			4			54









PART ELEVATION

# SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR BOX BEAMS.

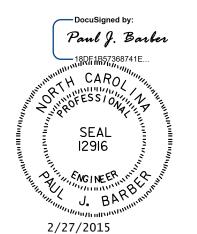
PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L
MILE POST: 0.23

NOTE: ALL STRANDS ARE BONDED FULL LENGTH.

GRADE 270	STRANDS
	½″∅ L.R.
AREA (SQUARE INCHES)	0.153
ULTIMATE STRENGTH (LBS.PER STRAND)	41,300
APPLIED PRESTRESS (LBS.PER STRAND)	30,980





STATE OF NORTH CAROLINA

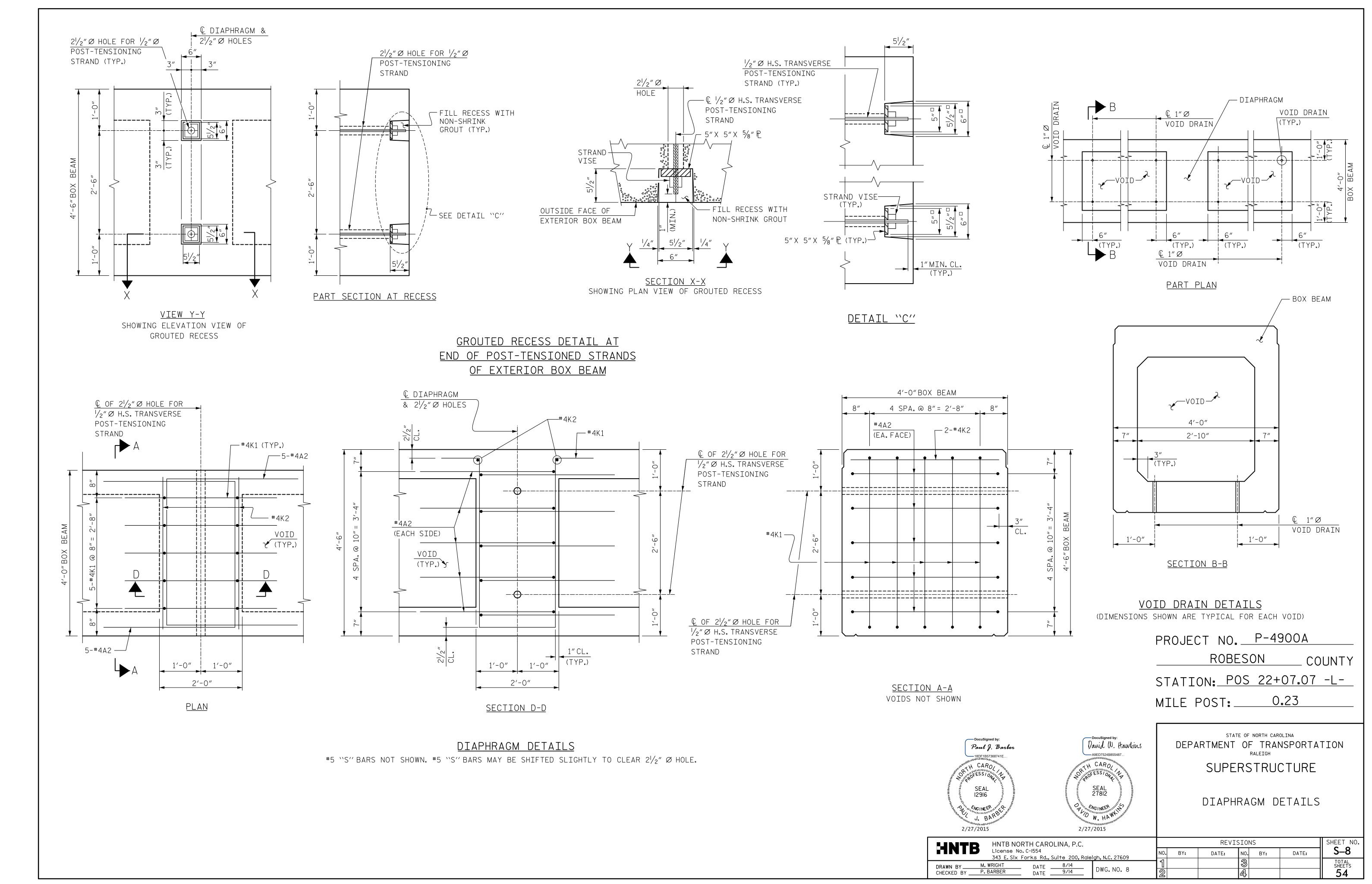
DEPARTMENT OF TRANSPORTATION

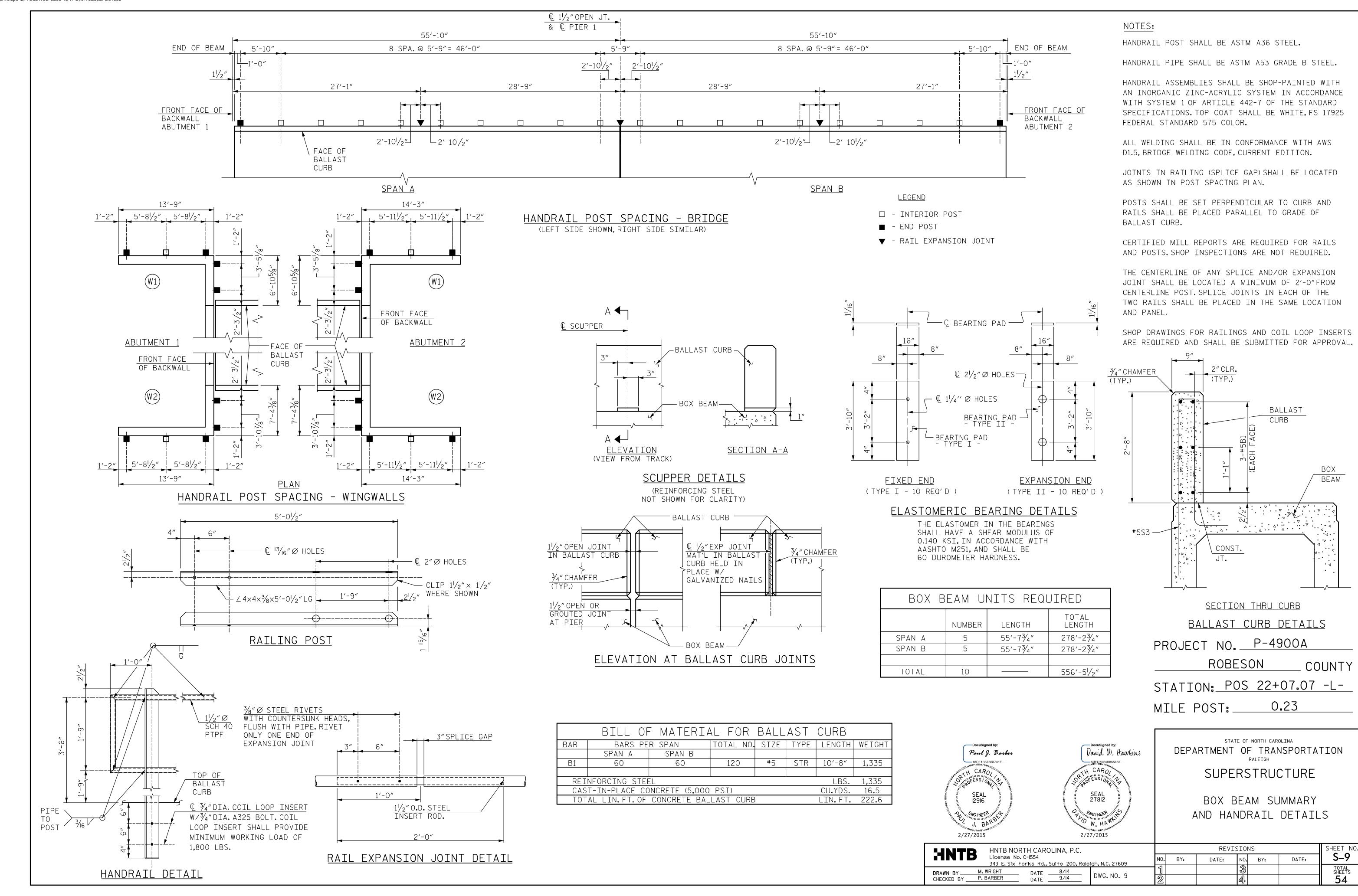
RALEIGH

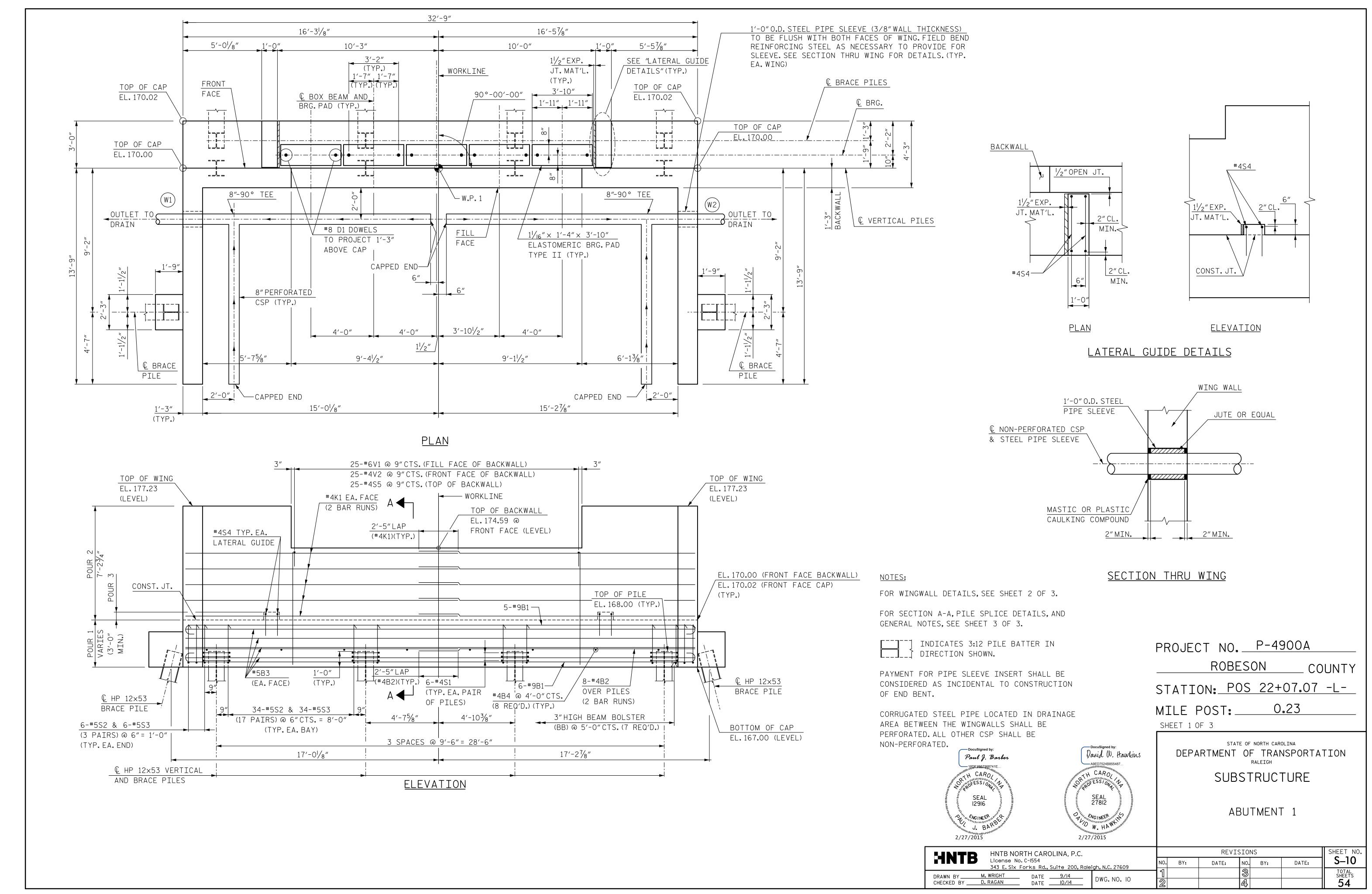
SUPERSTRUCTURE

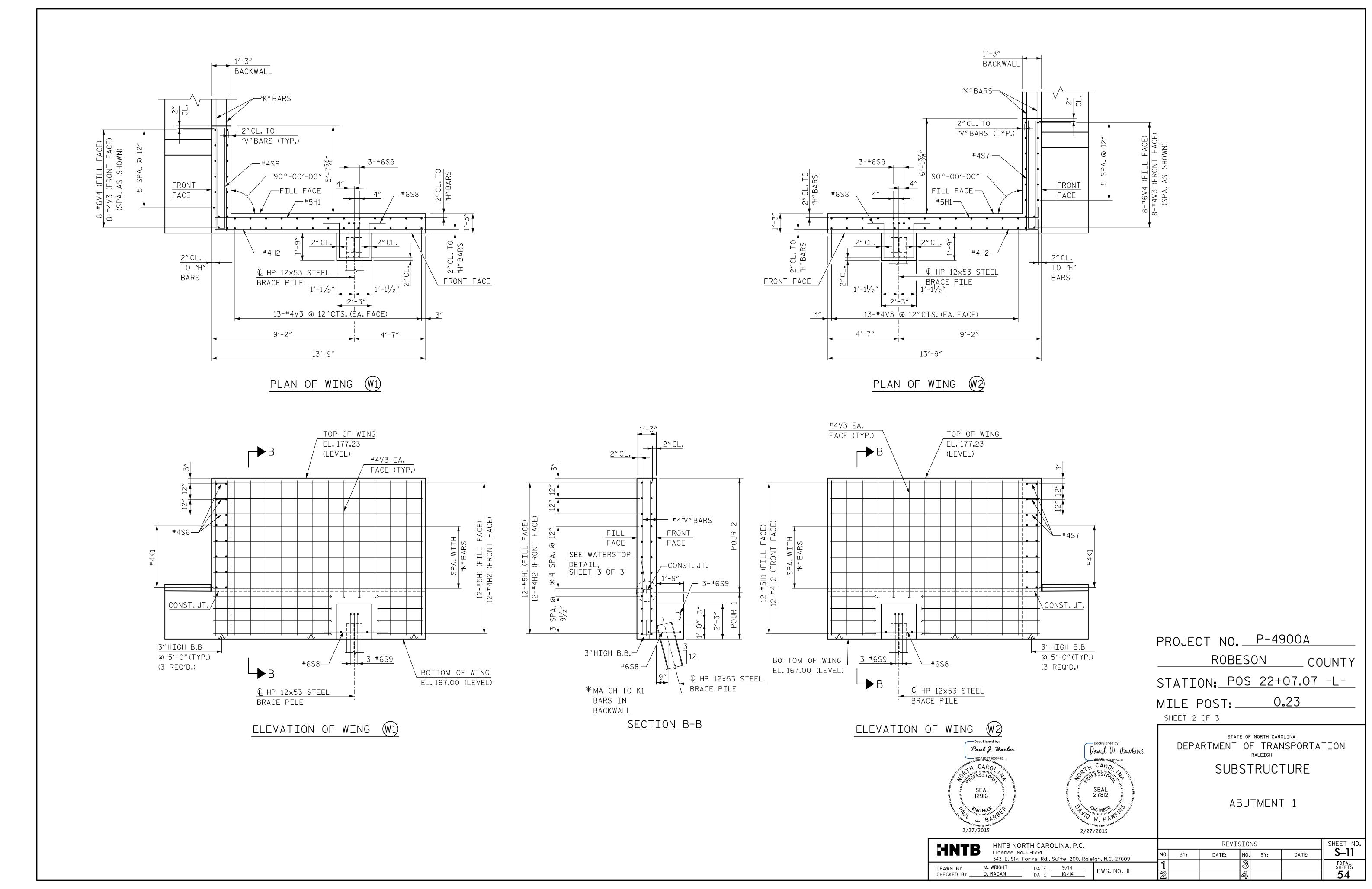
4'-0" × 4'-6" BOX BEAM DETAILS

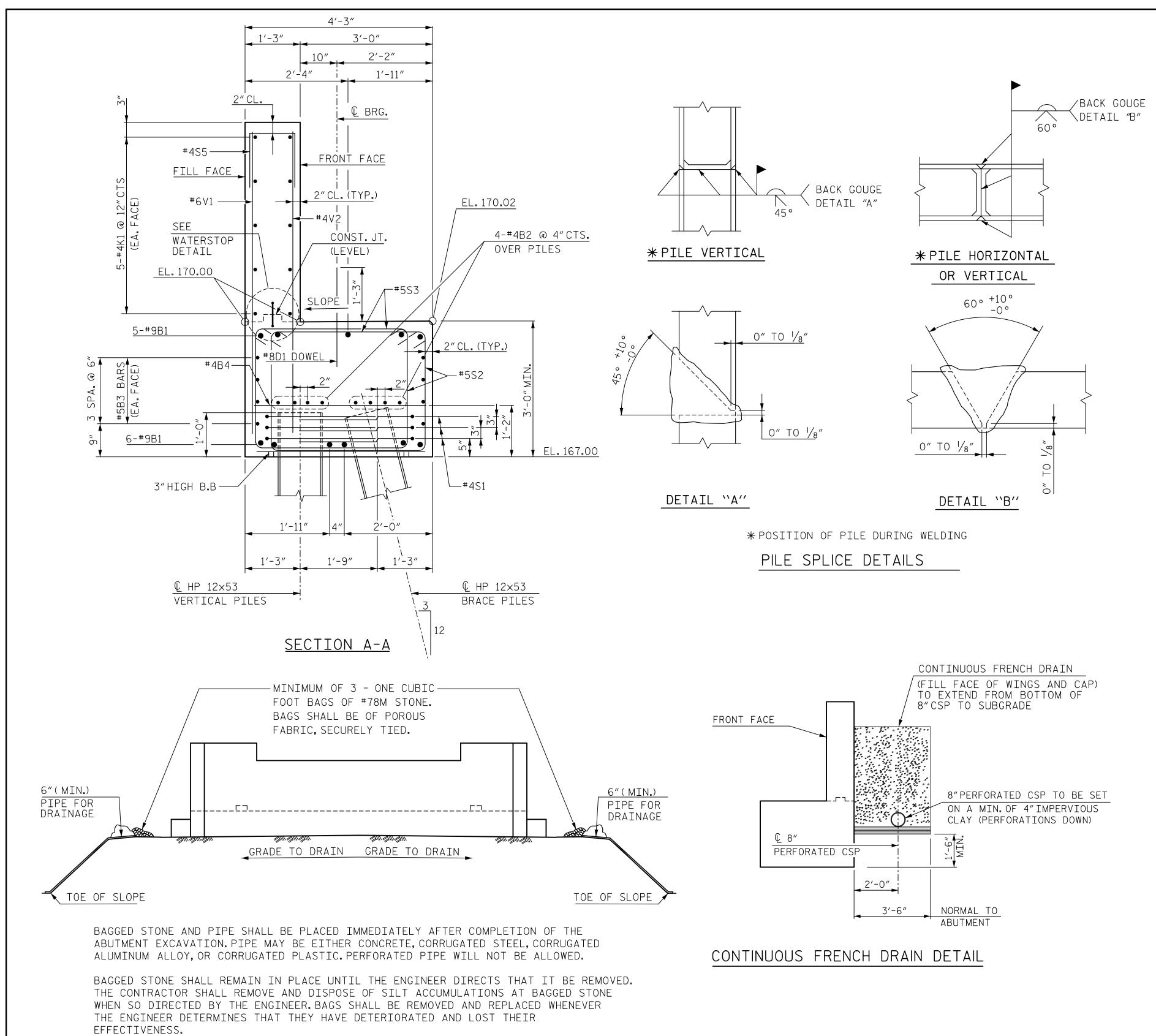
HNTB NORTH CAROLINA, P.C. License No. C-1554			REVIS]	EONS		SHEET NO.
License No. C-1554  343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY:	DATE: N	NO. BY:	DATE:	S-7
DDAWN BY M. WRIGHT DATE 8/14	1		4	3		TOTAL SHEETS
CHECKED BY P. BARBER DATE 9/14 DWG. NO. 7	9		/	<u>a</u>		51



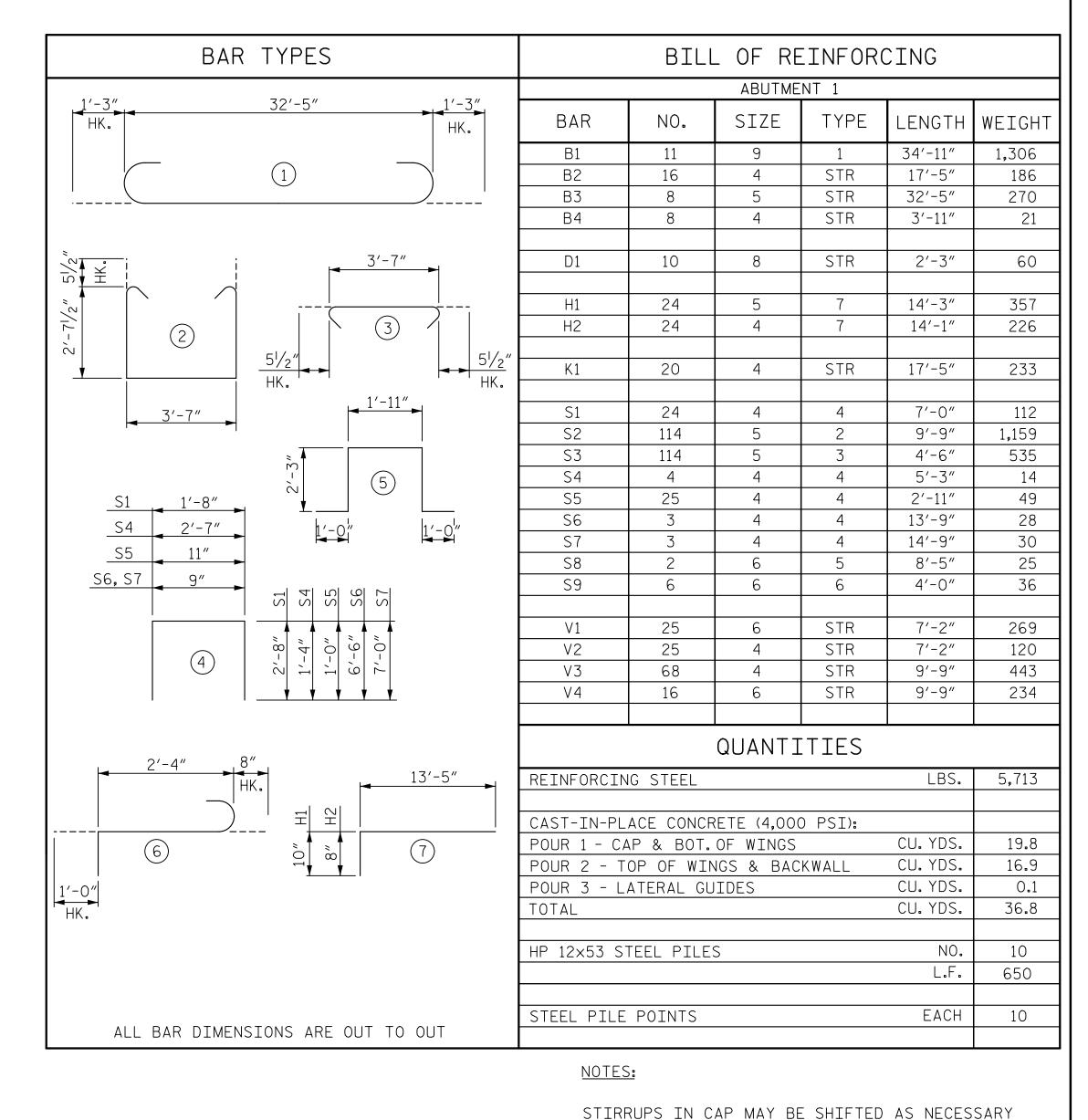








NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK



TO CLEAR #8D1 DOWELS. 71/2" 71/2" THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL BOX BEAMS ARE IN PLACE. CONST.JT. TOP OF CAP

ROBESON

SEAL 27812

STATION: POS 22+07.07 -L-

COUNTY

PROJECT NO. P-4900A

0.23 MILE POST: \_\_\_\_

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION David W. Hawkins SUBSTRUCTURE

ABUTMENT 1

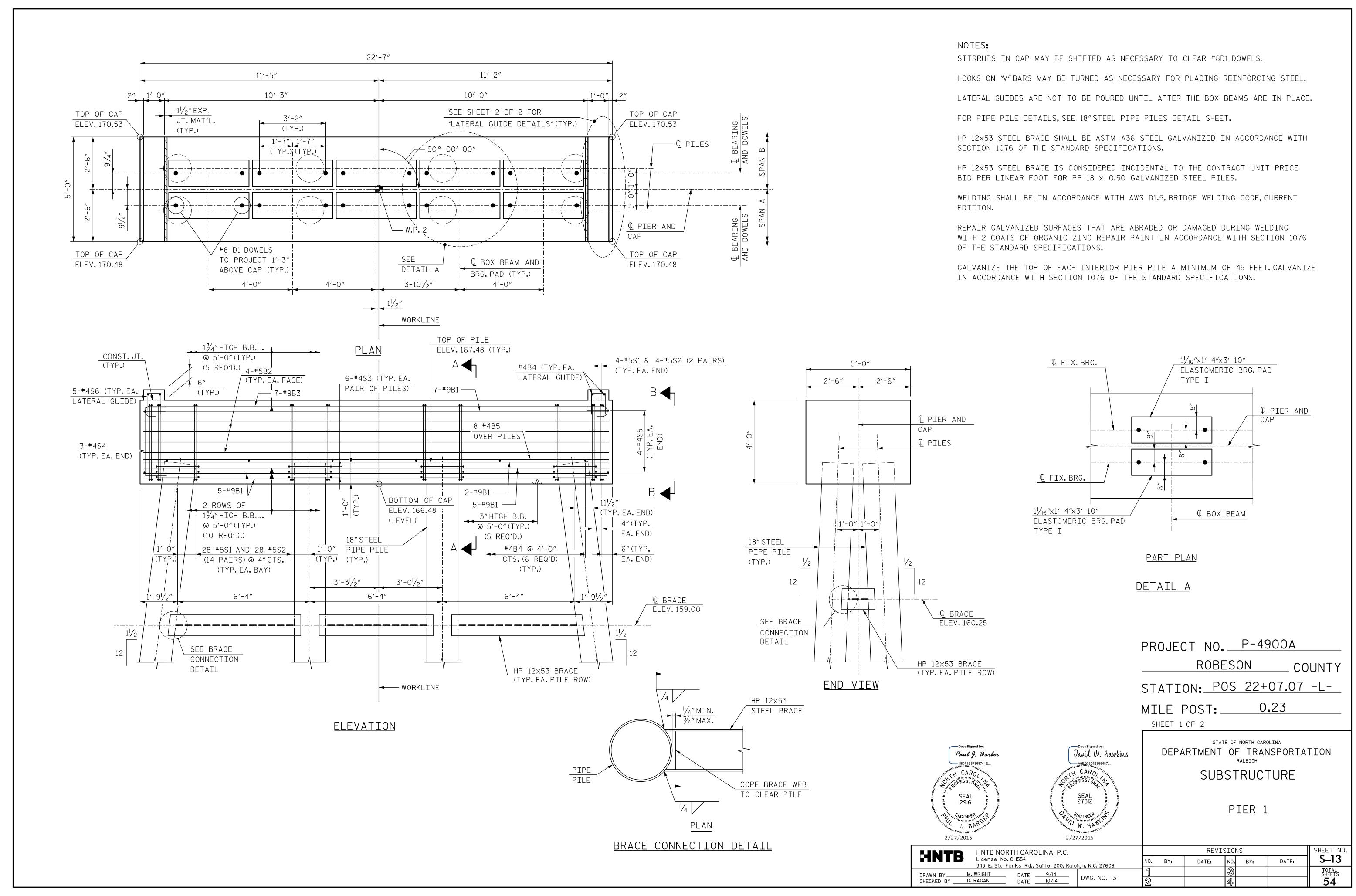
2/27/2015 REVISIONS SHEET NO. HNTB NORTH CAROLINA, P.C. License No. C-1554 S-12 NO. BY: DATE: DATE: BY: 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 TOTAL SHEETS **54** DRAWN BY M. WRIGHT DATE 9/14 DWG. NO. 12 CHECKED BY D. RAGAN DATE 10/14

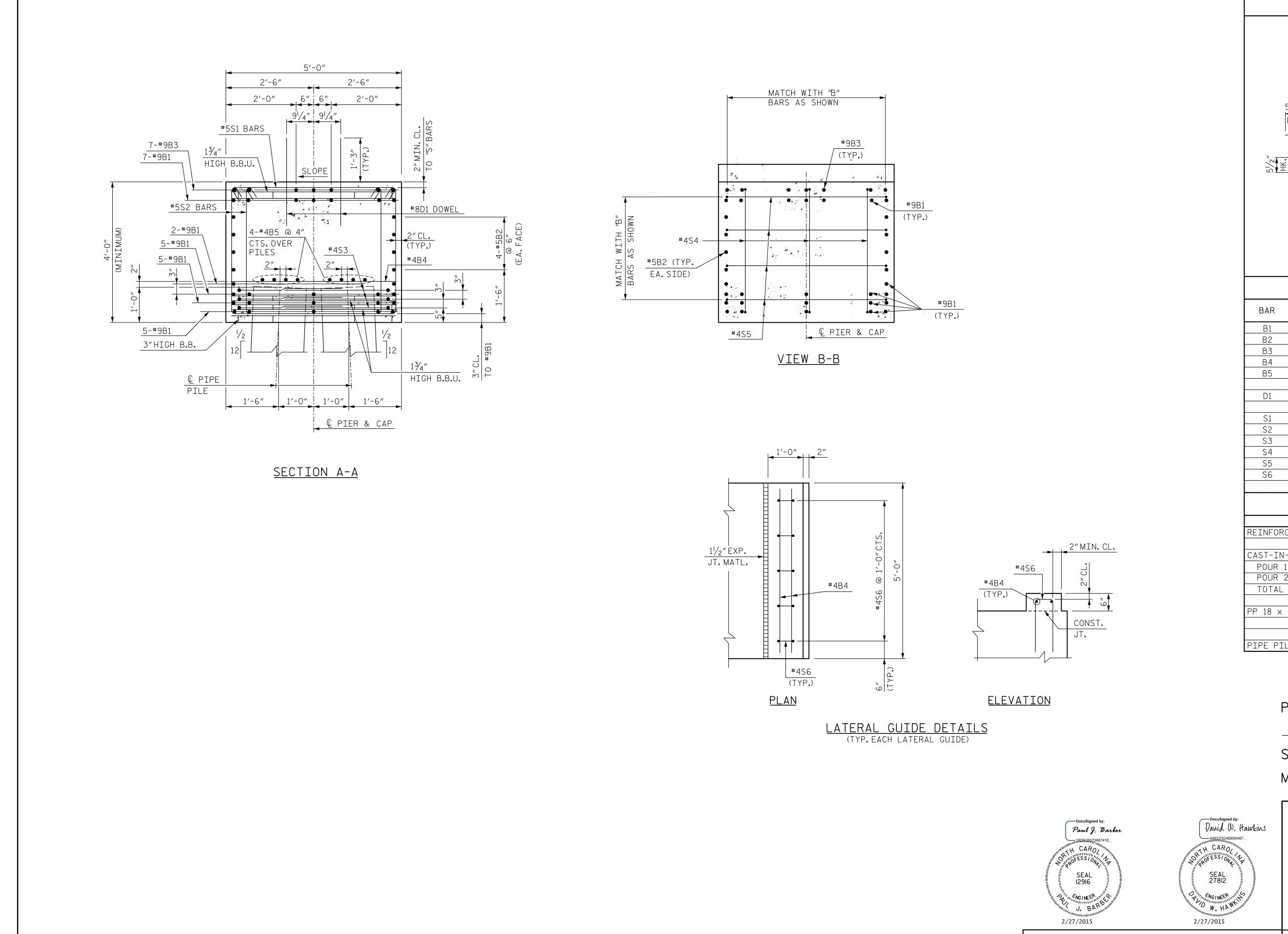
BACKWALL 1'-3" 6"FLEXIBLE P.V.C. WATERSTOP WATERSTOP DETAIL

CARO SEAL 12916

Paul J. Barber

SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE FOR THE SEVERAL PAY ITEMS. TEMPORARY DRAINAGE AT ABUTMENT





BAR TYPES

1'-3"

22'-3"

HK.

1

2'-2"

3'-6"

4'-4"

3

4'-4"

3

4'-4"

4'-4"

3

4'-4"

ALL BAR DIMENSIONS ARE OUT TO OUT

	BILL OF REINFORCING												
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT								
B1	24	9	STR.	22′-3″	1,816								
B2	8	5	STR.	22′-3″	186								
В3	7	9	1	24'-9"	589								
В4	10	4	STR.	4′-8″	31								
B5	8	4	STR.	22′-3″	119								
D1	20	8	STR.	2'-3"	120								
S1	92	5	2	5′-3″	504								
S2	92	5	3	12'-6"	1,199								
S3	24	4	4	8'-2"	131								
S4	6	4	4	6′-6″	26								
S5	8	4	4	7′-6″	40								
S6	10	4	4	3′-6″	23								

QUANTITIES										
ITEM		TOTAL								
EINFORCING STEEL	LBS.	4,784								
AST-IN-PLACE CONCRETE (4,000 PSI):										
POUR 1 - CAP	CU. YDS.	16.3								
POUR 2 - LATERAL GUIDE	CU. YDS.	0.2								
TOTAL	CU. YDS.	16.5								
P 18 x 0.50 GALVANIZED STEEL PILES	NO.	8								
	L.F.	520								
IPE PILE PLATES	EACH	8								

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L-

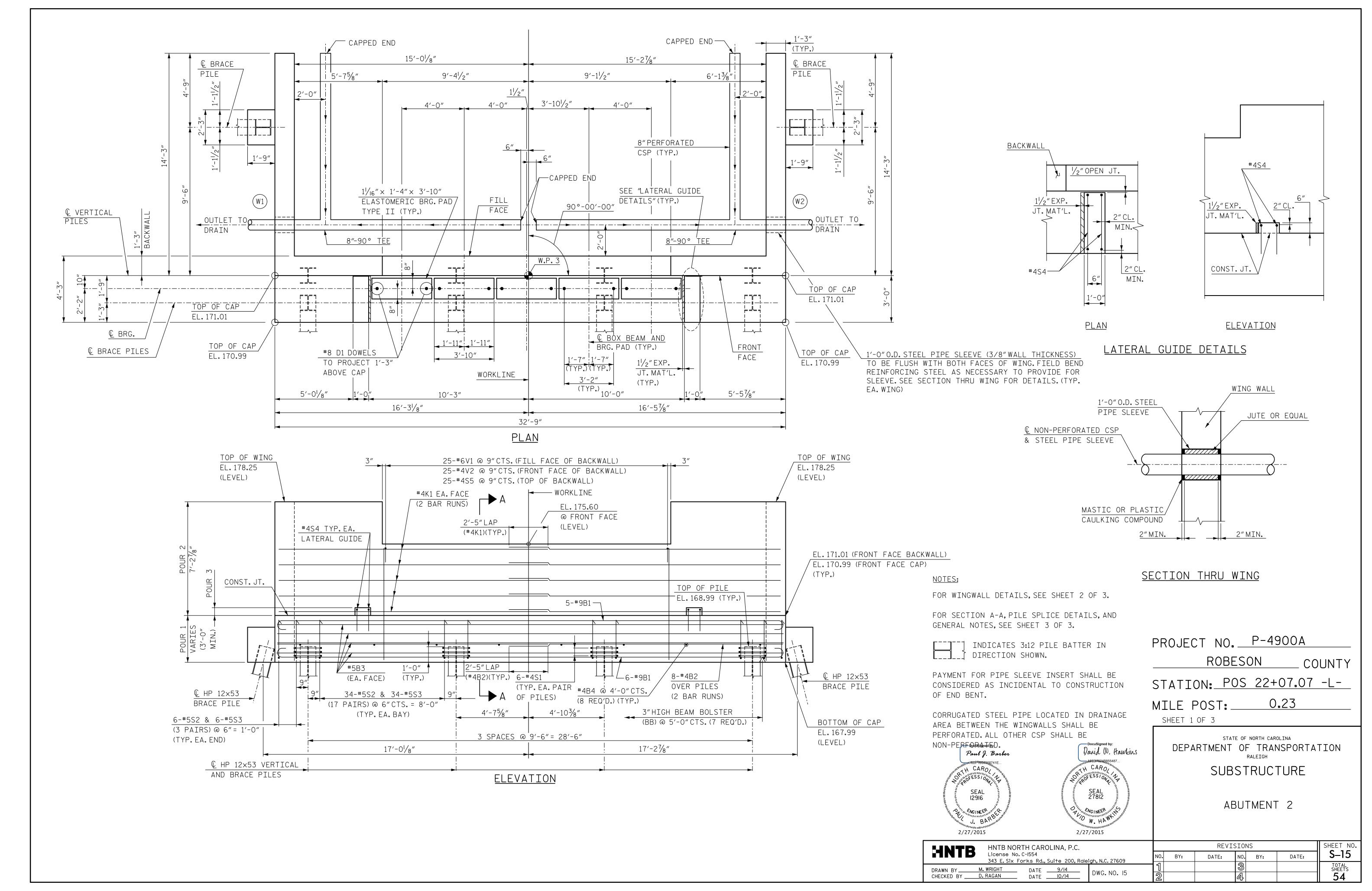
MILE POST: 0.23

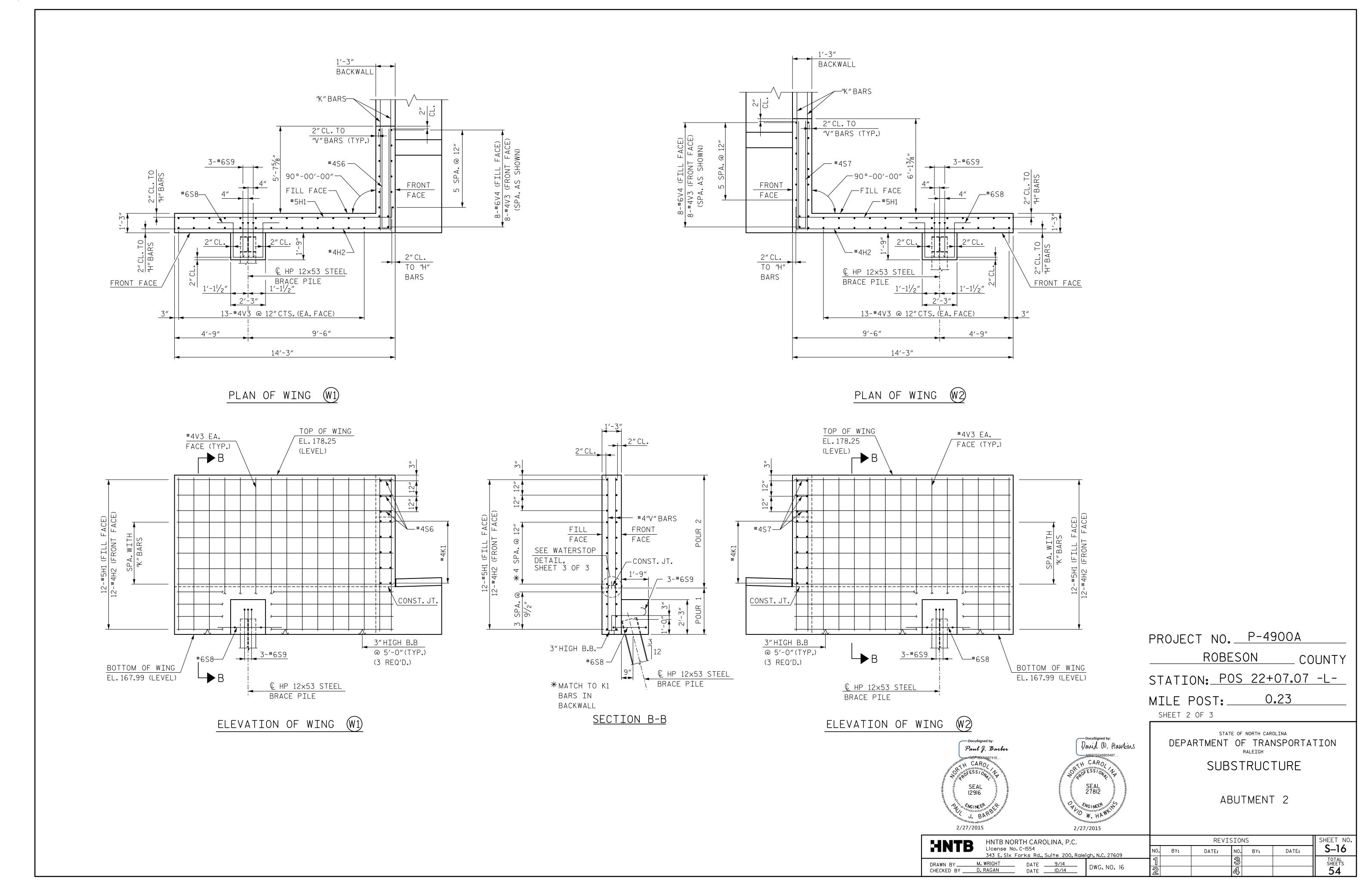
SHEET 2 OF 2

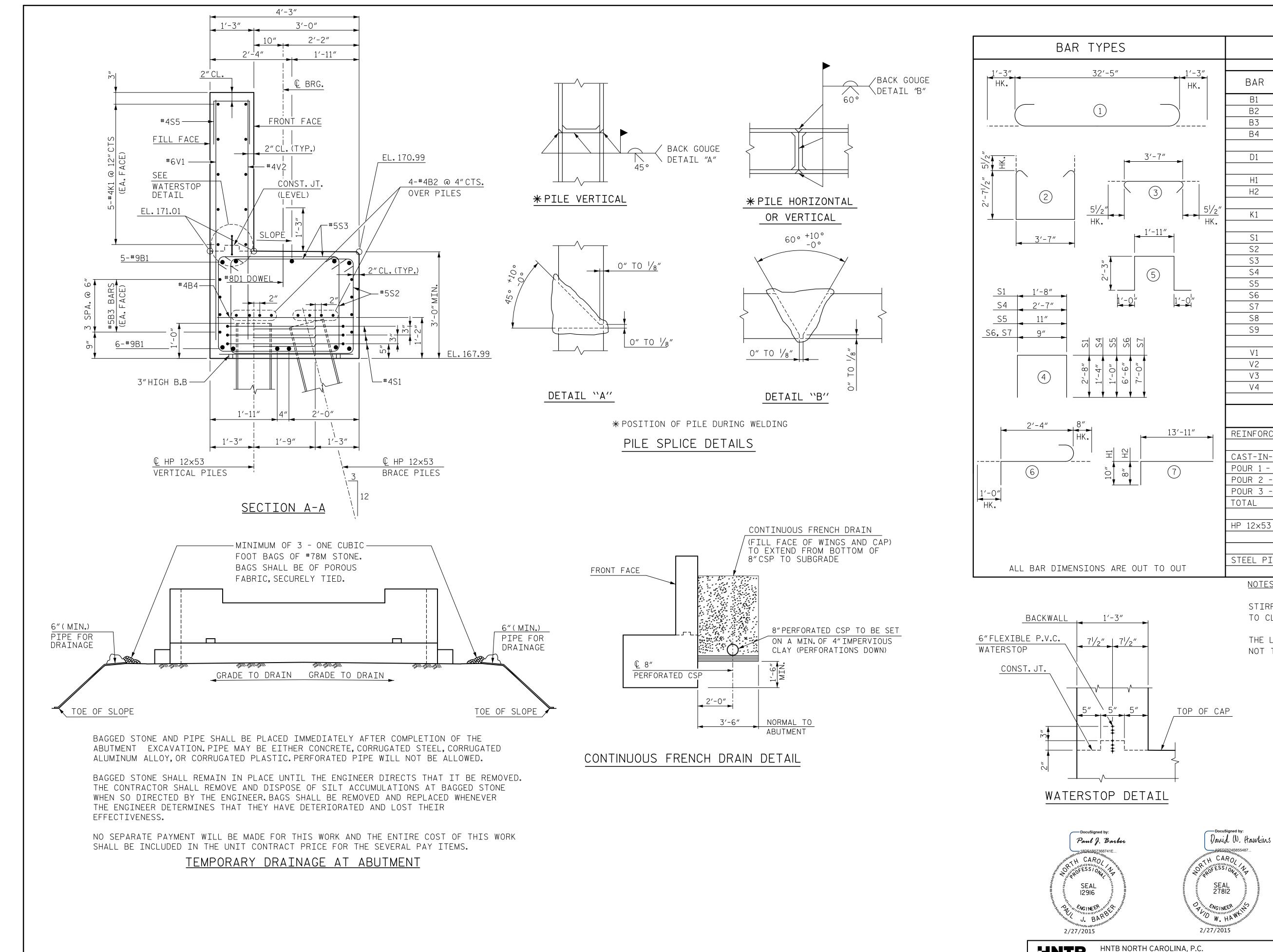
DEPARTMENT OF TRANSPORTATION RALEIGH
SUBSTRUCTURE

PIER 1

INTB HNTB NORTH CAROLINA, P.C.			REVIS	ION:	S		SHEET NO.			
License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N	C 27609	BY:	DATE:	NO.	BY:	DATE:	S-14			
DAWN DV M WRIGHT DATE 9/1/	1			3			TOTAL SHEETS			
HECKED BY D. RAGAN DATE 10/14	2 2			4			54			







SIZE TYPE WEIGH7 BAR LENGTH 1,306 В1 34'-11" 11 STR В2 16 17′-5″ 186 STR 270 32'-5" В3 В4 STR 21 8 3′-11″ D1 STR 2'-3" 60 10 H1 24 14'-9" 369 14'-7" 234 Н2 24 Κ1 STR 17′-5″ 233 20 7′-0″ 112 S1 24 1,159 S2 114 9′-9″ S3 4'-6" 535 114 S4 5′-3″ 14 4 S5 25 2'-11" 49 S6 3 13′-9″ 28 S7 3 14'-9" 30 S8 8′-5″ 25 S9 4'-0" 36 6 V1 25 STR 7′-2″ 269 25 STR 120 V2 7′-2″ 447 68 STR V3 9'-10" 16 STR 9'-10" 236 QUANTITIES REINFORCING STEEL LBS. 5,739 CAST-IN-PLACE CONCRETE (4,000 PSI): 20.0 POUR 1 - CAP & BOT. OF WINGS CU. YDS. POUR 2 - TOP OF WINGS & BACKWALL CU. YDS 17.3 POUR 3 - LATERAL GUIDES 0.1 CU. YDS 37.4 CU. YDS. HP 12×53 STEEL PILES NO. 10 L.F. 650 STEEL PILE POINTS EACH 10 NOTES: STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR #8D1 DOWELS. THE LATERAL GUIDE AT EACH END OF THE CAP IS NOT TO BE POURED UNTIL BOX BEAMS ARE IN PLACE. PROJECT NO. P-4900A ROBESON COUNTY STATION: POS 22+07.07 -L-MILE POST: 0.23 SHEET 3 OF 3

License No. C-1554

DRAWN BY M. WRIGHT DATE 9/14

CHECKED BY D. RAGAN DATE 10/14

343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DWG. NO. 17

BILL OF REINFORCING

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

ABUTMENT 2

NO. BY:

SHEET NO.

S-17

TOTAL SHEETS **54** 

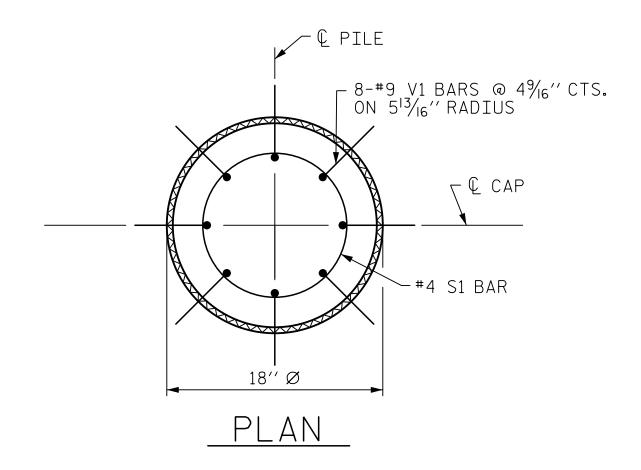
DATE:

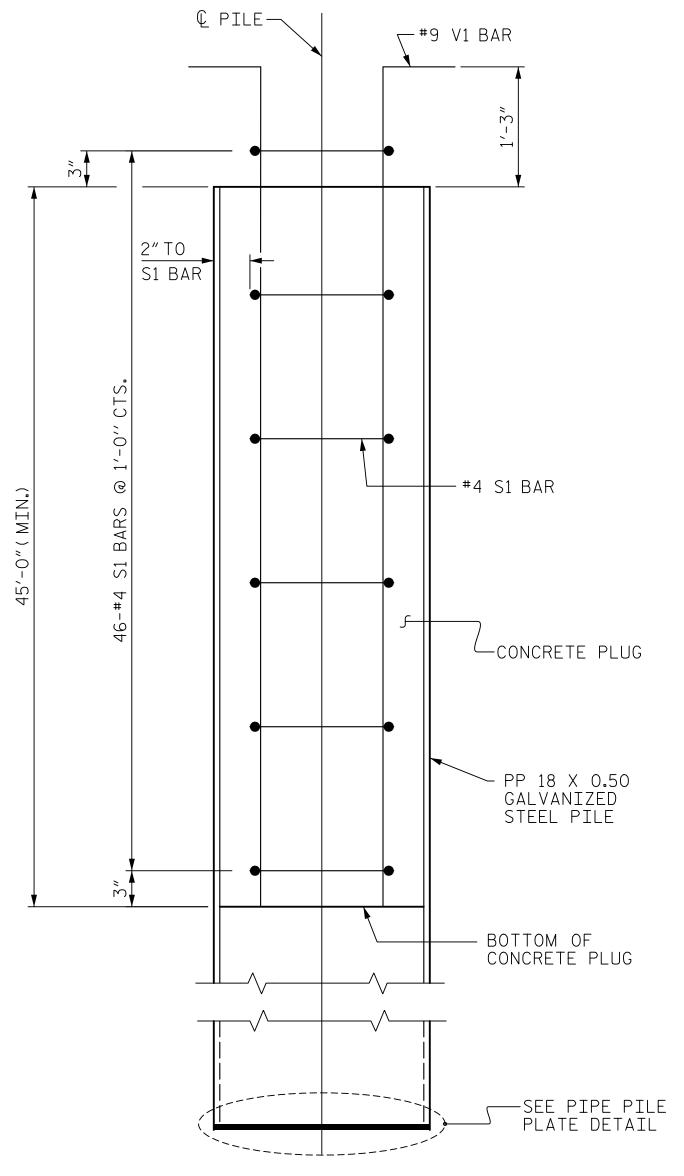
REVISIONS

DATE:

BY:

ABUTMENT 2





ELEVATION

PP 18 X 0.50 GALVANIZED STEEL PILE (CLOSED END.)

NOTES

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. GALVANIZING PIPE PILE PLATES IS NOT REQUIRED.

PIPE PILE PLATES, IF REQUIRED, SHALL BE IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

FOR CLOSED END PIPE PILES, REMOVE ALL SOIL AND WATER FROM INSIDE THE PILES JUST PRIOR TO PLACING REINFORCING STEEL AND CONCRETE FOR THE CONCRETE PLUG.

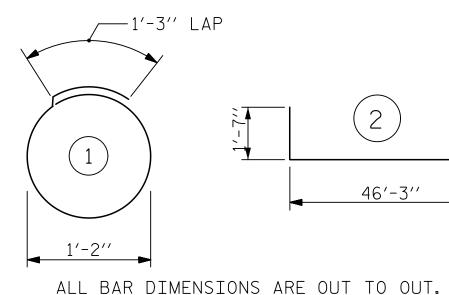
FORM THE CONCRETE PLUG SUCH THAT THE REINFORCING STEEL OR CONCRETE DOES NOT MOVE AND THE CLEARANCE FROM THE REINFORCING STEEL TO THE INSIDE OF THE PILE IS MAINTAINED AFTER CONCRETE PLACEMENT. DO NOT PLACE CONCRETE IN THE BENT CAP UNTIL THE CONCRETE PLUG HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

THE REINFORCING STEEL, CONCRETE, AND GALVANIZING ARE CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 18 X 0.50 GALVANIZED STEEL PILES.

GALVANIZE THE TOP OF EACH INTERIOR PIER PILE A MINIMUM OF 45 FEET. GALVANIZE IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

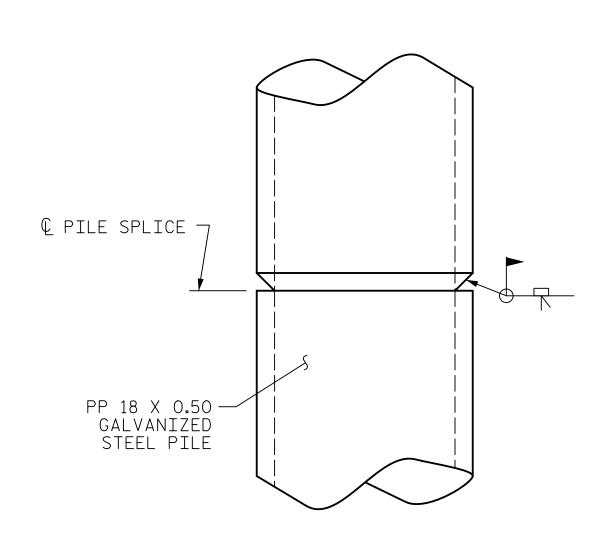
PLACE CAST-IN-PLACE CONCRETE WITH f'c = 2,500 PSI IN THE PIPE PILE PLATE UP TO THE BOTTOM OF THE REINFORCED CONCRETE PLUG.

BILL OF MATERIAL FOR ONE PP 18 X 0.50 GALVANIZED STEEL PILE BAR NO. | SIZE | TYPE LENGTH WEIGHT S1 46 4 1 4'-11'' 151 9 47′-10″ 1,301 V1 ITEM TOTAL REINFORCING STEEL LBS. 1**,**452 CAST-IN-PLACE CONCRETE (4,000 PSI): CU. YDS. 2.6 45'-0" MINIMUM PLUG CAST-IN-PLACE CONCRETE (2,500 PSI): CU. YDS. X.X BAR TYPES \_\_\_1'-3'' LAP



18" Ø X ¾" P

PIPE PILE PLATE DETAIL

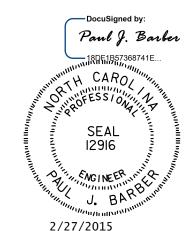


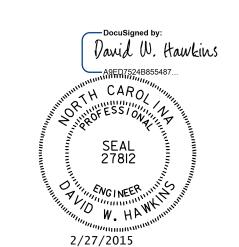
PIPE PILE SPLICE DETAIL

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POS 22+07.07 -L
MILE POST: 0.23





DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

STATE OF NORTH CAROLINA

18" STEEL PIPE PILE

HNTB NORTH CAROLINA, P.C.

License No. C-1554

343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY M. WRIGHT DATE 10/14

CHECKED BY D. RAGAN DATE 10/14

DWG. NO. 18

REVISIONS

NO. BY: DATE: NO. BY: DATE:

1 3 54

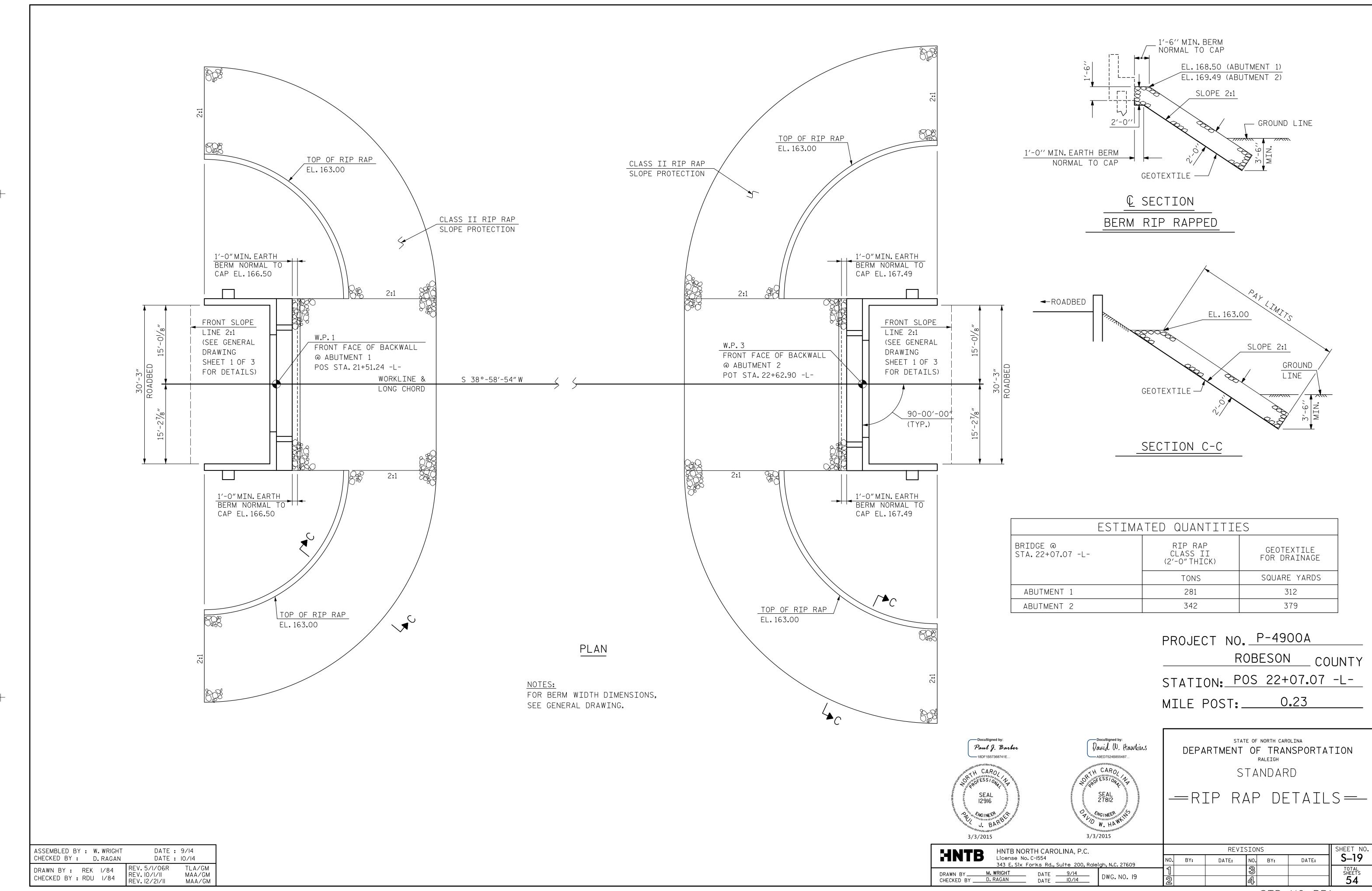
ASSEMBLED BY: M. WRIGHT
CHECKED BY: D. RAGAN

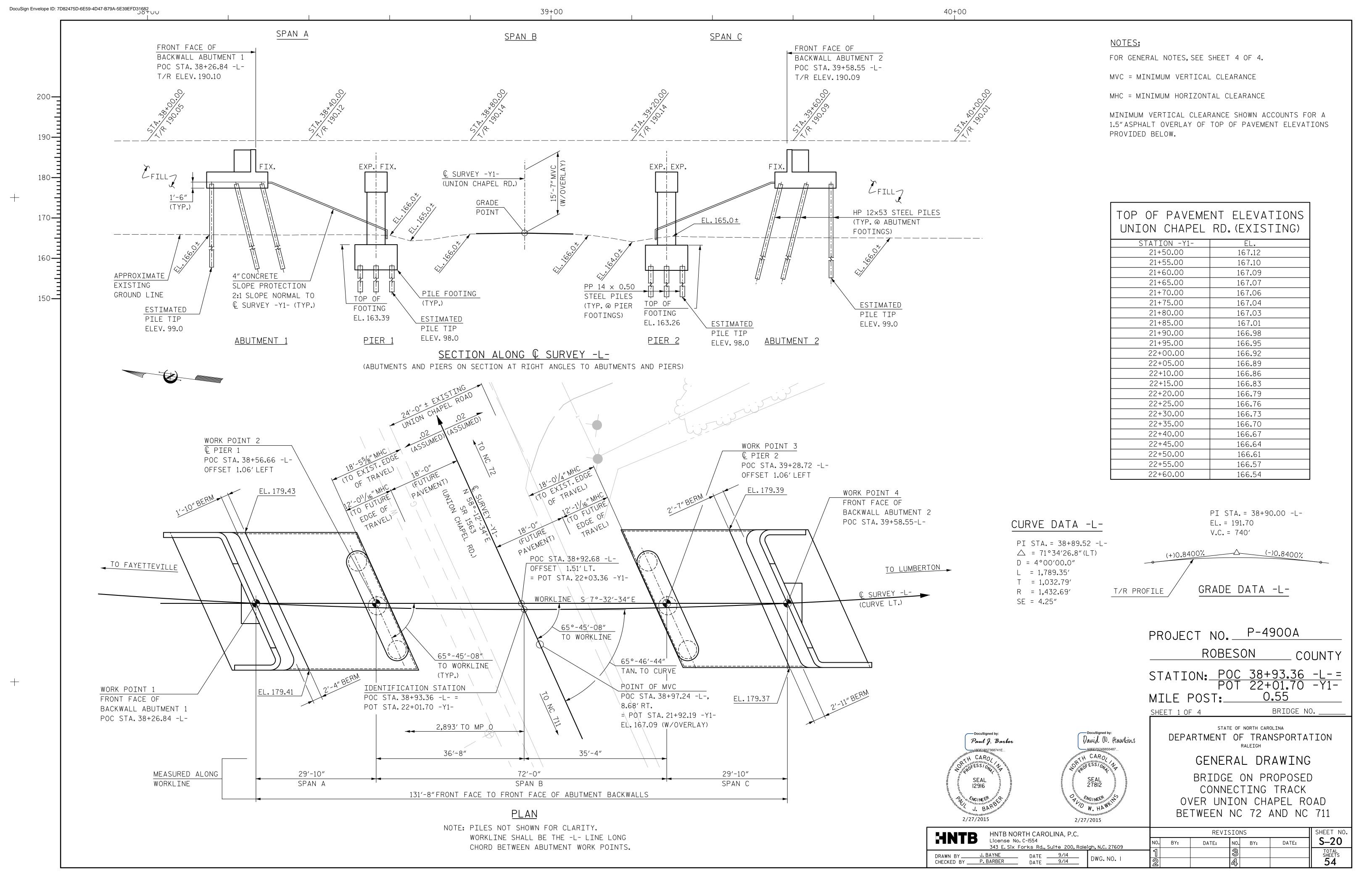
DATE: 10/14

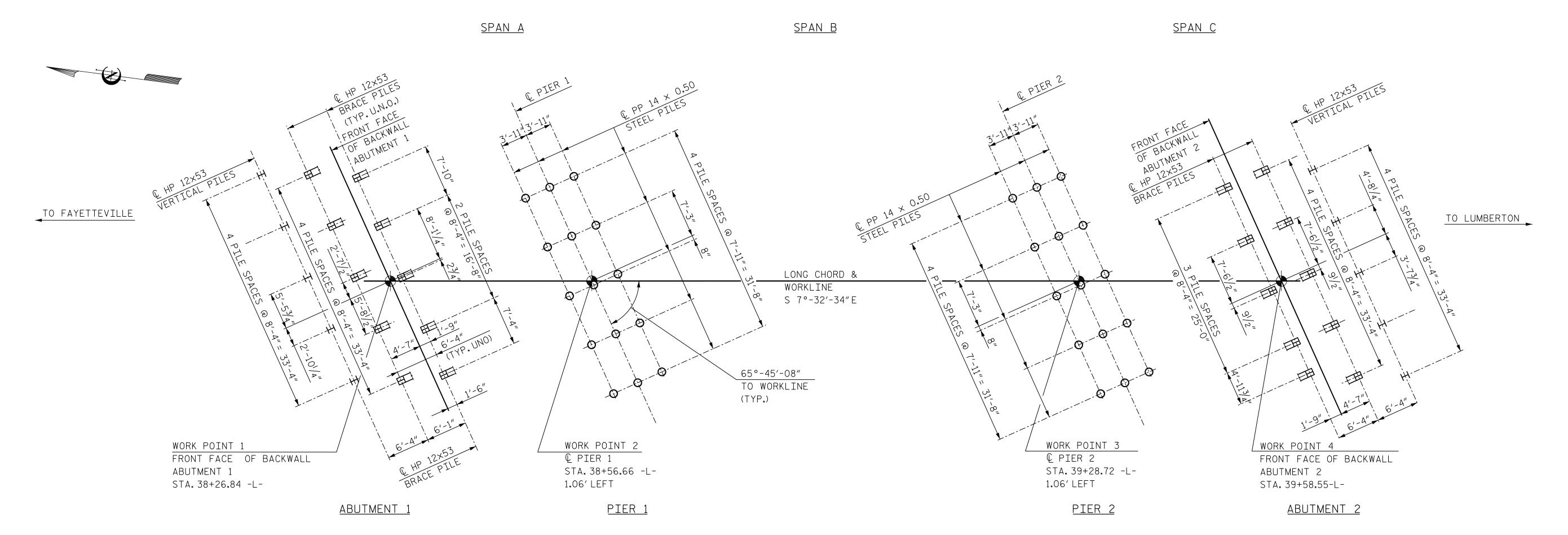
DRAWN BY: RWW 1/01
CHECKED BY: LES 1/01

REV. 10/1/05
REV. 5/1/06R
REV. 10/1/II

MAA/GM







### FOUNDATION NOTES

FOR PILES, SEE SPECIAL PROVISIONS.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT ABUTMENT NO.1 AND ABUTMENT NO.2 IS 75 TONS PER PILE.

DRIVE PILES AT ABUTMENT NO.1 AND ABUTMENT NO.2 TO A REQUIRED BEARING CAPACITY OF 150 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

THE ALLOWABLE BEARING CAPACITY FOR PILES AT PIER NO.1 AND PIER NO.2 IS 100 TONS PER PILE.

DRIVE PILES AT PIER NO.1 AND PIER NO.2 TO A REQUIRED BEARING CAPACITY OF 200 TONS PER PILE. THE REQUIRED BEARING CAPACITY IS EQUAL TO THE ALLOWABLE BEARING CAPACITY WITH A MINIMUM FACTOR OF SAFETY OF TWO.

PIPE PILE PLATES ARE REQUIRED FOR PIPE PILES AT PIER NO.1 AND PIER NO.2. USE PIPE PILE PLATES WITH A DIAMETER EQUAL TO THE PIPE PILE DIAMETER.

TESTING THE FIRST PRODUCTION PILE AT ABUTMENT NO.1 AND ABUTMENT NO.2 WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. FOR PDA TESTING, SEE SPECIAL PROVISIONS.

TESTING THE FIRST PRODUCTION PILE AT PIER NO.1 AND PIER NO.2 WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED. FOR PDA TESTING, SEE SPECIAL PROVISIONS.

GROUNDWATER WAS FOUND TO BE ABOVE THE BOTTOM OF FOOTING ELEVATIONS AT PIER NO. 1 AND PIER NO. 2 AT THE TIME OF SUBSURFACE INVESTIGATION. THE CONTRACTOR SHALL USE A WELL POINT DEWATERING SYSTEM, APPROPRIATELY DESIGNED SHEET PILING COFFERDAM WITH GRAVEL BED AND SUMP PUMP, OR OTHER APPROPRIATE MEASURES AS NEEDED TO SAFELY CONSTRUCT THE FOOTING IN A DRY CONDITION. THE COSTS ASSOCIATED WITH THE ABOVE MENTIONED OPTIONS SHALL BE CONSIDERED INCIDENTAL TO THE COST OF THE FOUNDATION EXCAVATION.

DO NOT DRIVE PILES AT ABUTMENT NO.1 OR PIER NO.1 UNTIL AFTER OBSERVING A 3 MONTH WAITING PERIOD BEGINNING WHEN FILL AND SURCHARGE HAVE BEEN PLACED AS SHOWN IN RAIL PLANS FOR A MINIMUM DISTANCE OF 250 FT FROM ABUTMENT 1. SEE RAIL PLANS AND SURCHARGE SPECIAL PROVISION FOR SURCHARGE DETAILS.

DO NOT DRIVE PILES AT ABUTMENT NO.2 OR PIER NO.2 UNTIL AFTER OBSERVING A 3 MONTH WAITING PERIOD BEGINNING WHEN FILL AND SURCHARGE HAVE BEEN PLACED AS SHOWN IN RAIL PLANS FOR A MINIMUM DISTANCE OF 250 FT FROM ABUTMENT 2. SEE RAIL PLANS AND SURCHARGE SPECIAL PROVISION FOR SURCHARGE DETAILS.

# FOUNDATION LAYOUT

NOTES:

ALL DIMENSIONS ARE PARALLEL OR NORMAL TO © PIERS AND FRONT FACES OF BACKWALL.

INDICATES HP 12×53 PILE TO BE BATTERED 3:12 IN DIRECTION SHOWN.

ALL PILE DIMENSIONS ARE TO CENTERS OF PILES AT BOTTOM OF ABUTMENTS OR PIER FOOTINGS.

FOR FOUNDATION ELEVATIONS AND DETAILS, SEE PIER AND ABUTMENT DETAILS.

U.N.O. = UNLESS NOTED OTHERWISE.

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POC 38+93.36 -L-

MILE POST: 0.55

SHEET 2 OF 4

Docusigned by:

Paul J. Barber

Paul J. Barber

CARO

CARO

CHARLES I ON A CHARLE

SEAL

12916

SEAL

12916

BARBER

2/27/2015



DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

STATE OF NORTH CAROLINA

FOUNDATION LAYOUT

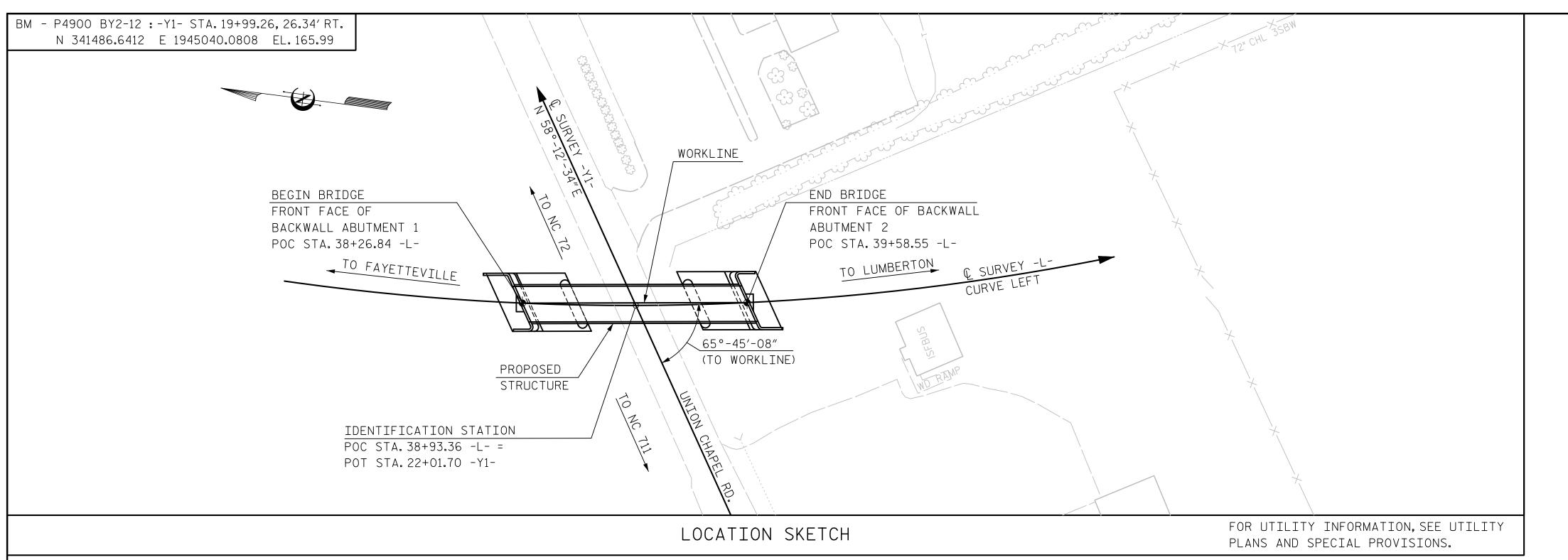
HNTB NORTH CAROLINA, P.C.

License No. C-1554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY J. BAYNE DATE 10/14
CHECKED BY D. RAGAN DATE 10/14

DWG. NO. 2

REVISIONS
NO. BY: DATE: N



# INDEX OF DRAWINGS

- GENERAL DRAWING: GENERAL PLAN AND ELEVATION (SHEET 1 OF 4)
- GENERAL DRAWING: FOUNDATION LAYOUT (SHEET 2 OF 4)
- 3 GENERAL DRAWING: LOCATION SKETCH AND TOTAL BILL OF MATERIAL (SHEET 3 OF 4) 4 GENERAL DRAWING: GENERAL NOTES (SHEET 4 OF 4)
- 5 SUPERSTRUCTURE: TYPICAL SECTION
  - SUPERSTRUCTURE: DECK DETAILS
- 7 SUPERSTRUCTURE: PLAN OF DECK SPAN A
- 8 SUPERSTRUCTURE: PLAN OF DECK SPAN B 9 SUPERSTRUCTURE: PLAN OF DECK - SPAN C
- 10 SUPERSTRUCTURE: FRAMING PLAN AND GIRDER DETAILS SPANS A & C
- 11 SUPERSTRUCTURE: FRAMING PLAN AND GIRDER DETAILS SPAN B
- 12 SUPERSTRUCTURE: STRUCTURAL STEEL DETAILS
- 13 SUPERSTRUCTURE: BEARING DETAILS SPANS A & C
- 14 SUPERSTRUCTURE: BEARING DETAILS SPAN B
- 15 SUPERSTRUCTURE: EXPANSION PLATE DETAILS
- 16 SUPERSTRUCTURE: PARAPET ELEVATION 17 SUPERSTRUCTURE: PARAPET PANEL DETAILS
- 18 SUPERSTRUCTURE: PARAPET PANEL FORM LINER FINISH DETAILS
- 19 SUPERSTRUCTURE: BILL OF MATERIAL
- 20 SUBSTRUCTURE: ABUTMENT 1 (SHEET 1 OF 3)
- 21 SUBSTRUCTURE: ABUTMENT 1 (SHEET 2 OF 3)
- 22 SUBSTRUCTURE: ABUTMENT 1 (SHEET 3 OF 3)
- 23 SUBSTRUCTURE: PIER 1 (SHEET 1 OF 2)
- 24 SUBSTRUCTURE: PIER 1 (SHEET 2 OF 2)
- 25 SUBSTRUCTURE: PIER 2 (SHEET 1 OF 2)
- 26 SUBSTRUCTURE: PIER 2 (SHEET 2 OF 2)
- 27 SUBSTRUCTURE: ABUTMENT 2 (SHEET 1 OF 3)
- 28 SUBSTRUCTURE: ABUTMENT 2 (SHEET 2 OF 3)
- 29 SUBSTRUCTURE: ABUTMENT 2 (SHEET 3 OF 3)
- 30 STRUCTURE DRAINAGE DETAILS (SHEET 1 OF 4) 31 STRUCTURE DRAINAGE DETAILS (SHEET 2 OF 4)
- 32 STRUCTURE DRAINAGE DETAILS (SHEET 3 OF 4)
- 33 STRUCTURE DRAINAGE DETAILS (SHEET 4 OF 4)
- 34 SLOPE PROTECTION DETAILS (SHEET 1 OF 2)
- 35 SLOPE PROTECTION DETAILS (SHEET 2 OF 2)

	TOTAL BILL OF MATERIAL																	
	FOUNDATION EXCAVATION FOR PIER NO.1 AT STA. 38+93.36 -L-	FOUNDATION EXCAVATION FOR PIER NO.2 AT STA. 38+93.36 -L-	PDA TESTING	CAST-IN-PLACE CONCRETE (5,000 PSI)	CAST-IN-PLACE CONCRETE (4,000 PSI)	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL	APPROX. 329,519 LBS. STRUCTURAL STEEL	PAINTING OF STRUCTURAL STEEL	HP 12 × 53 STEEL PILES		14 x 0.50 EL PILES	PIPE PILE PLATES	PILE REDRIVES	WATERPROOFING	DAMPPROOFING	CONCRETE PARAPET	4"SLOPE PROTECTION
	LUMP SUM	LUMP SUM	EACH	CU. YDS.	CU. YDS.	LBS.	LBS.	LUMP SUM	LUMP SUM	NO. L.F.	NO.	L.F.	EACH	EACH	SQ. YDS.	SQ. YDS.	L.F.	SQ. YDS.
SUPERSTRUCTURE				94.8			12,688	LUMP SUM	LUMP SUM						322.5		260.8	
ABUTMENT 1					123.7	9,884				15 1,200		-		8	13.2	80.1		134.3
PIER 1	LUMP SUM				136.8	21,536					15	900	15	8	9.4	58.2		
PIER 2		LUMP SUM			136.8	21,536					15	900	15	8	9.4	59.4		
ABUTMENT 2					123.0	9,823				15 1,200		_		8	13.2	79.2		134.7
TOTAL	LUMP SUM	LUMP SUM	4	94.8	520.3	62,779	12,688	LUMP SUM	LUMP SUM	30 2,400	30	1,800	30	32	367.7	276.9	260.8	269.0

TOTAL BILL OF MATERIAL										
	SELF- LUBRICATING EXPANSION BEARING ASSEMBLIES	STRUCTURE DRAINAGE SYSTEM AT STA. 38+93.36 -L-	APPLICATION OF BRIDGE COATING							
	LUMP SUM	LUMP SUM	LUMP SUM							
SUPERSTRUCTURE	LUMP SUM	LUMP SUM	LUMP SUM							
ABUTMENT 1			LUMP SUM							
PIER 1			LUMP SUM							
PIER 2			LUMP SUM							
ABUTMENT 2			LUMP SUM							
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM							

Docusigned by:

David W. Hawkins

PROJECT NO. P-4900A

ROBESON \_\_\_ COUNTY

STATION: POC 38+93.36 -L-

MILE POST: 0.55 SHEET 3 OF 4

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > GENERAL DRAWING

LOCATION SKETCH AND TOTAL BILL OF MATERIAL

									1	
LINTD	HNTB NORTH CAROLINA, P.C.			SHEET NO.	l					
HNTB NORTH CAROLINA, P.C. License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609			NO.	BY:	DATE:	NO.	BY:	DATE:	S-22	
DRAWN BYJ. BA	YNF DATE 9/14		<b>1</b> 1			3			TOTAL SHEETS	l
CHECKED BY D. RA	DATE IDWC NO 3		2			<u>a</u> ,			54	l

### GENERAL NOTES

ASSUMED LIVE LOAD = AREMA E80 WITH FULL DIESEL IMPACT OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING, "VOL. 2, STRUCTURES" AND CSX TRANSPORTATION'S "UNDERGRADE BRIDGE CRITERIA INCLUDING BALLAST DECK"AND THE STRUCTURE SPECIAL PROVISIONS.

WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) BRIDGE WELDING CODE D1.5 AS AMENDED AND COMPLEMENTED BY THE AREMA MANUAL.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

REINFORCING STEEL SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM DESIGNATION A615, GRADE 60. ALL DIMENSIONS RELATING TO BAR SPACING ARE TO BAR CENTERS UNLESS NOTED. ALL REINFORCING IN THE CONCRETE DECK SLAB AND PARAPETS SHALL BE EPOXY COATED. COMPATIBLE EPOXY COATED STEEL TIE WIRES SHALL BE USED WITH THE EPOXY COATED BARS.

EXPANSION JOINT MATERIAL SHALL BE EITHER RUBBER OR CORK CONFORMING WITH AASHTO SPECIFICATIONS M-153-84 EXCEPT AS SHOWN ON THE PLANS OR IN THE SPECIAL PROVISIONS. CELLULAR AND BULB TYPE WATERSTOPS AND RUBBER JOINT COMPOUNDS SHALL BE AS SHOWN ON THE PLANS AND IN THE SPECIAL PROVISIONS.

METAL DRAINS BEHIND ABUTMENTS AND IN BALLAST TROUGH OF BRIDGE, INCLUDING DUCTILE IRON PIPE COLLECTOR SYSTEM, SHALL BE AS SHOWN ON THE PLANS AND OUTLINED IN THE SPECIAL PROVISIONS. DETAILS OF THE DRAINAGE SYSTEM SHALL BE SUBMITTED FOR REVIEW.

WATERPROOFING: BRIDGE DECK, ALL CONSTRUCTION JOINTS AND ANY VISIBLE SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL SHALL BE WATERPROOFED WITH A COLD LIQUID - APPLIED ELASTOMERIC MEMBRANE. FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

WATERPROOFING IS REQUIRED AT THE FOLLOWING LOCATIONS:

- 1. OVER ENTIRE TOP SURFACE OF REINFORCED CONCRETE DECK SLAB AND INSIDE FACES OF CONCRETE PARAPETS TO PROPOSED BALLAST ELEVATION.
- 2. ALONG FULL LENGTH OF HORIZONTAL CONSTRUCTION JOINT AT TOP OF CAP/FOOTING WITH BACKWALL AT EACH ABUTMENT (FILL FACE OF BACKWALL AND WINGWALLS ONLY).
- 3. ALONG FULL CIRCUMFERENCE OF EACH BOTTOM OF COLUMN TO TOP OF FOOTING INTERFACE.
- 4. AT ANY CONCRETE SHRINKAGE CRACKS WHICH WILL BE COVERED BY FILL.

WATERPROOFING FOR LOCATIONS 2, 3 AND 4 ABOVE SHALL BE 24" WIDE AND SHALL BE CENTERED OVER JOINTS OR CRACKS.

DAMPPROOFING SHALL BE APPLIED TO THE FOLLOWING SURFACES REGARDLESS OF WHETHER OR NOT THE PLAN DETAILS SO INDICATE:

1. ON FILL SIDE TOP OF FOOTING AND FILL FACE OF BACKWALL AT EACH ABUTMENT. 2. ON FILL FACE OF ABUTMENT WINGWALLS FROM BOTTOM OF WINGWALL TO TOP OF BACKWALL ELEVATION AT EACH ABUTMENT.

3. TOP OF PIER FOOTINGS AND PIER COLUMNS UP TO PROPOSED GROUND LINE.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

FOR PLACING LOAD ON STRUCTURE MEMBERS. SEE SPECIAL PROVISIONS.

ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES", JANUARY 2012, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION (HEREIN CALLED STANDARD SPECIFICATIONS), EXCEPT AS NOTED HEREIN, ELSEWHERE ON PLANS, OR IN THE SPECIAL PROVISIONS.

FOR MAINTENANCE AND PROTECTION OF TRAFFIC BENEATH PROPOSED STRUCTURE. SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

THE ELEVATION(S) AND CLEARANCE(S) SHOWN ON THE PLANS AT THE POINT(S) OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION(S) ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

ALL CONSTRUCTION JOINTS SHOWN ON THESE PLANS SHALL BE REQUIRED UNLESS SHOWN AS OPTIONAL. CONSTRUCTION JOINTS SHALL NOT BE PERMITTED EXCEPT AS SHOWN ON THE PLANS, OR WHERE WRITTEN APPROVAL HAS BEEN OBTAINED.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH SEISMIC PERFORMANCE CATEGORY A REQUIREMENTS OF THE AASHTO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES", AND CHAPTER NINE "SEISMIC DESIGN FOR RAILWAY STRUCTURES" OF THE CURRENT EDITION OF AREMA'S MANUAL FOR RAILWAY ENGINEERING, "VOL. 2 STRUCTURES".

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES. SEE SPECIAL PROVISIONS.

SEE "STRUCTURAL STEEL DETAILS" SHEET FOR STRUCTURAL STEEL NOTES.

FOR BACKFILL BEHIND ABUTMENTS AND OTHER BACKFILL AROUND THE STRUCTURE, SEE SPECIAL PROVISION "BACKFILLING AROUND STRUCTURES".

FOR PAINTING STEEL STRUCTURES, SEE SPECIAL PROVISONS.

FOR WATERSTOPS, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC FLASHING, SEE SPECIAL PROVISIONS.

FOR RUBBER JOINT COMPOUNDS, SEE SPECIAL PROVISIONS.

FOR STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

FOR RAILROAD TRACKWORK, SEE RAILROAD TRACKWORK PLANS.

FOR ARCHITECTURAL TREATMENT, SEE SPECIAL PROVISIONS.

FOR WATERPROOFING, SEE SPECIAL PROVISIONS.

FOR REINFORCING, CONCRETE, AND DAMPPROOFING, SEE "CAST-IN-PLACE CONCRETE", SPECIAL PROVISION.

ONLY ITEMS IN THE PROPOSAL AS PAY ITEMS WILL BE PAID FOR. COMPENSATION FOR ALL LABORS, MATERIALS, TOOLS, EQUIPMENT AND INCIDENTALS FOR THE ENTIRE CONTRACT SHALL BE INCLUDED IN THE PRICE BID FOR PAY ITEMS.

FOR FOUNDATION NOTES, SEE "FOUNDATION LAYOUT" SHEET.

FOR APPLICATION OF BRIDGE COATING, SEE SPECIAL PROVISION.

FOR CONCRETE PARAPET, SEE SPECIAL PROVISION.

Paul J. Barber

CHECKED BY P. BARBER

FOR MASS CONCRETE AT PIER 1 AND PIER 2, SEE SPECIAL PROVISIONS.

PROJECT NO. P-4900A

ROBESON COUNTY

0.55

SHEET NO.

S-23

TOTAL SHEETS **54** 

DATE:

STATION: POC 38+93.36 -L-

MILE POST:\_\_\_\_

SHEET 4 OF 4

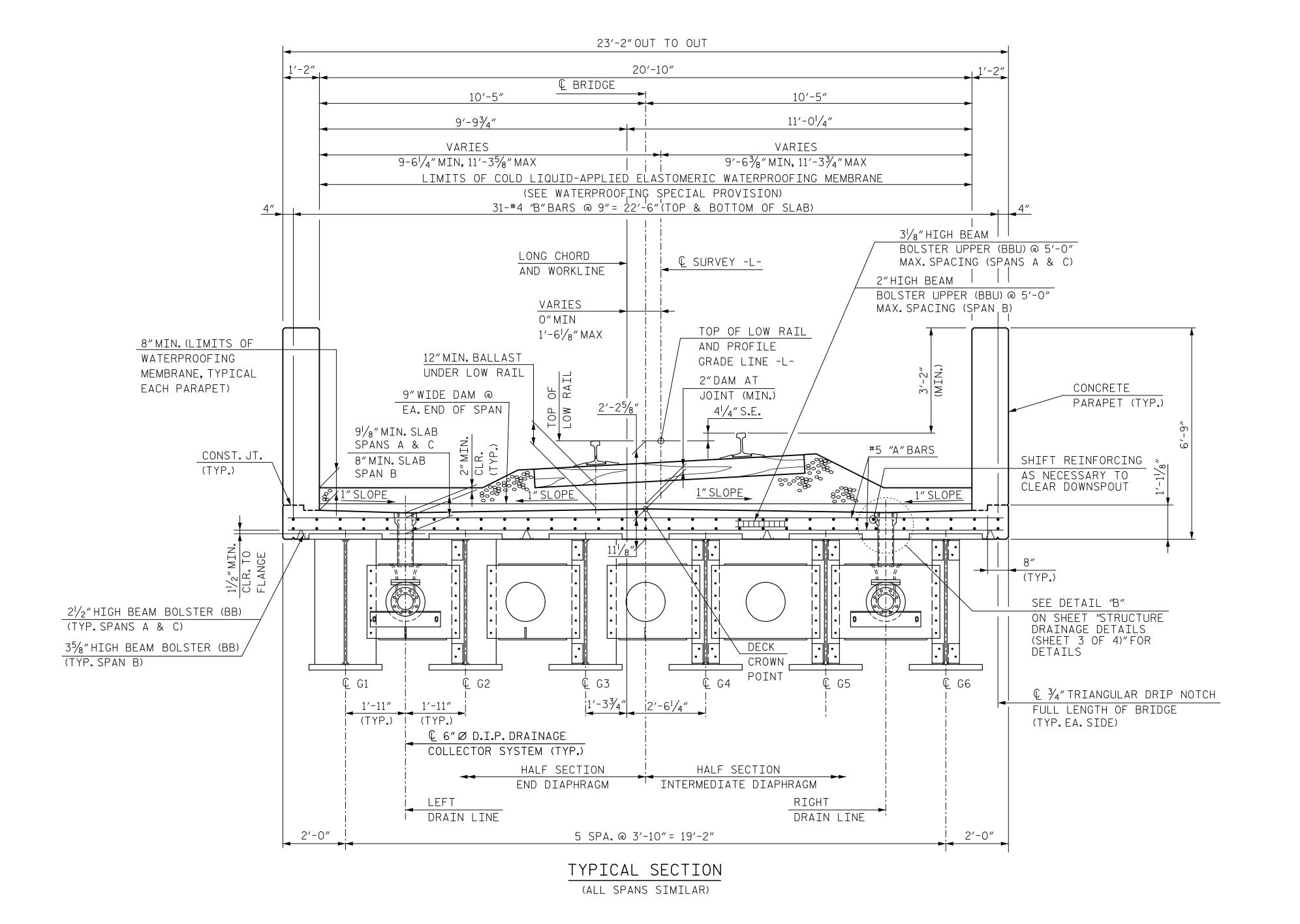
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

SEAL 27812 GENERAL NOTES REVISIONS HNTB NORTH CAROLINA, P.C. License No. C-1554 NO. BY: BY: DATE: 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 \_\_\_\_ DATE <u>8/14</u> DRAWN BY \_\_\_\_\_\_M. WRIGHT

DATE <u>9/14</u>

David W. Hawkins

DWG. NO. 4



NOTES:

ALL REINFORCING STEEL IN THE DECK AND PARAPETS SHALL BE EPOXY COATED. CLEAR COVER TO ALL REINFORCING IS 2"MINIMUM UNLESS NOTED OTHERWISE.

FOR CONCRETE PARAPET DETAILS, SEE "CONCRETE PARAPET DETAILS"

FOR ARCHITECTURAL TREATMENTS, SEE "PARAPET ARCHITECTURAL DETAILS" SHEET.

DESIGN INCLUDES WEIGHT OF 6"ADDITIONAL BALLAST TO ACCOUNT FOR FUTURE RESURFACING OF TRACK.

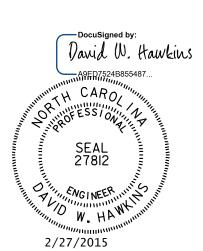
THE DECK SLAB SHALL BE CONSTRUCTED USING REMOVABLE FORMS.

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POC 38+93.36 -L-

MILE POST: 0.55



STATE OF NORTH CAROLINA

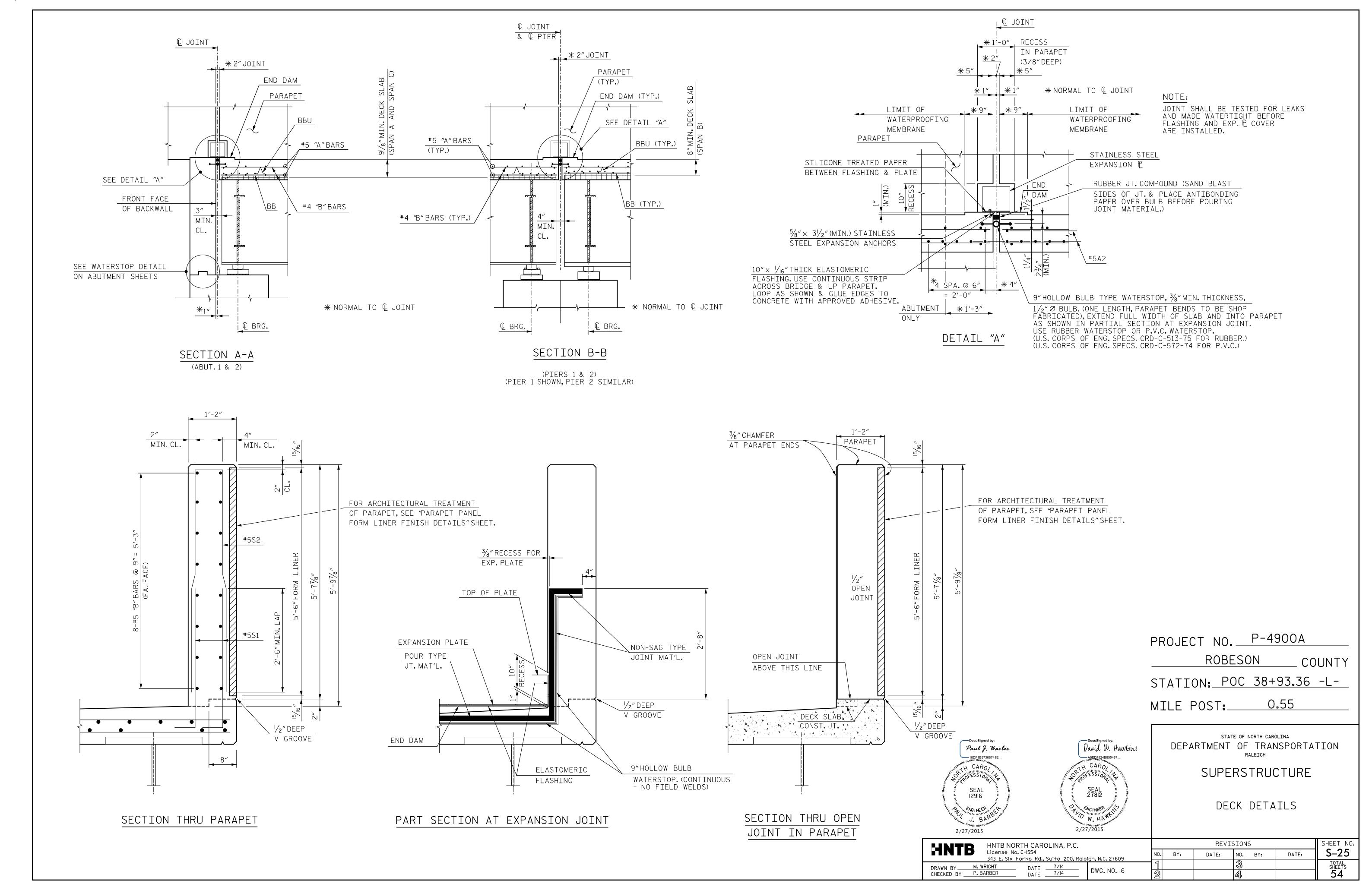
DEPARTMENT OF TRANSPORTATION

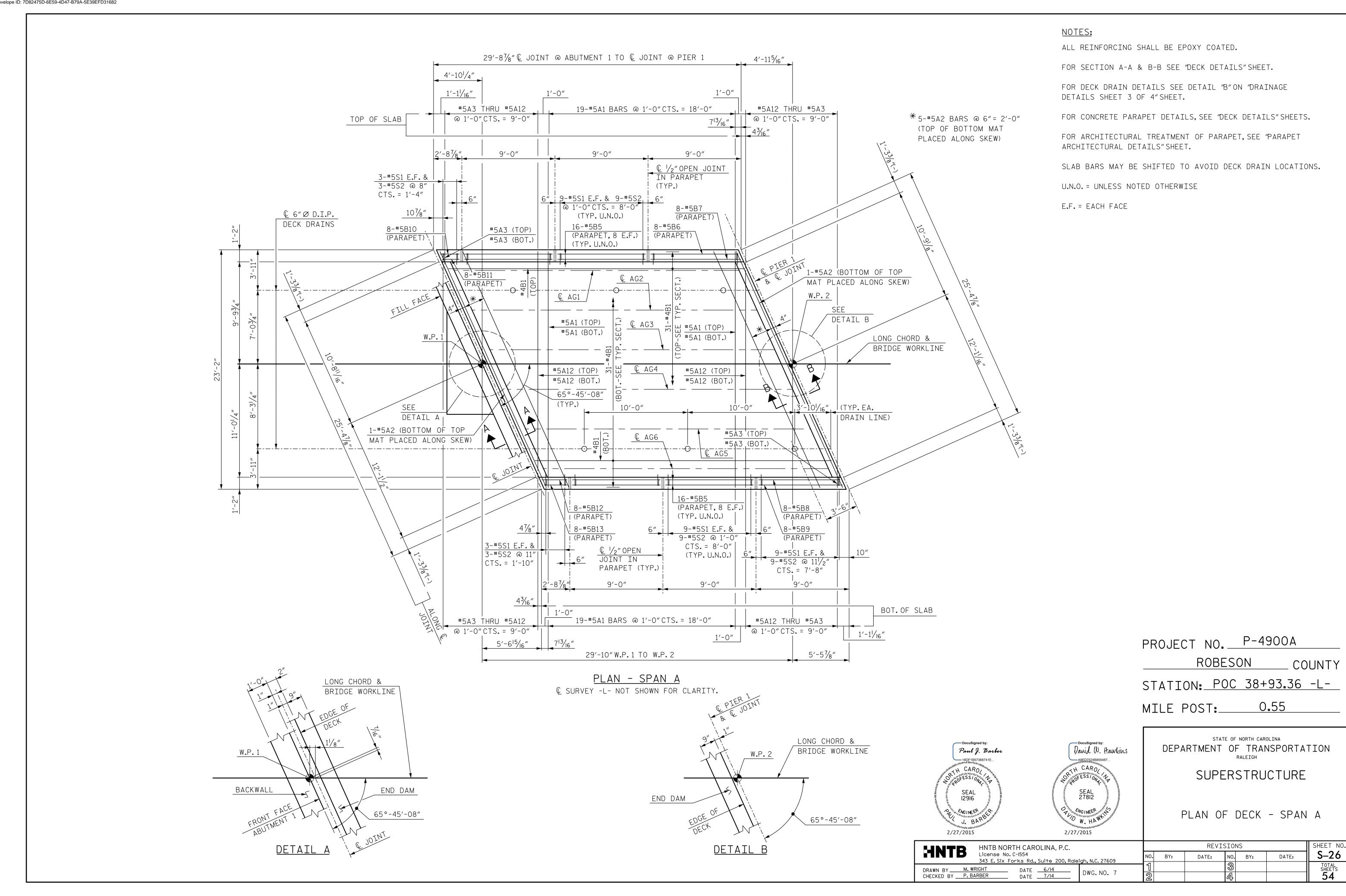
RALEIGH

SUPERSTRUCTURE

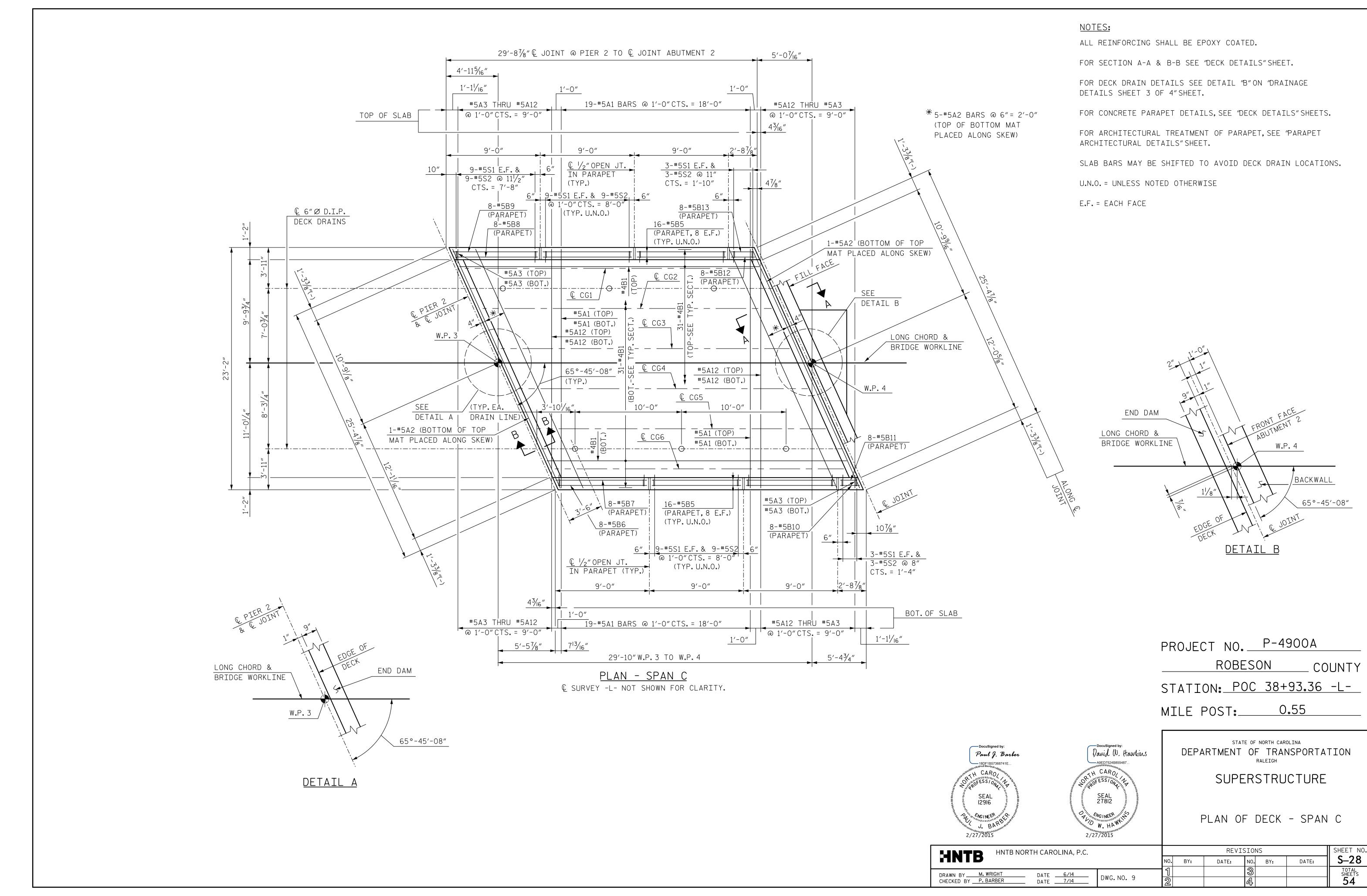
TYPICAL SECTION

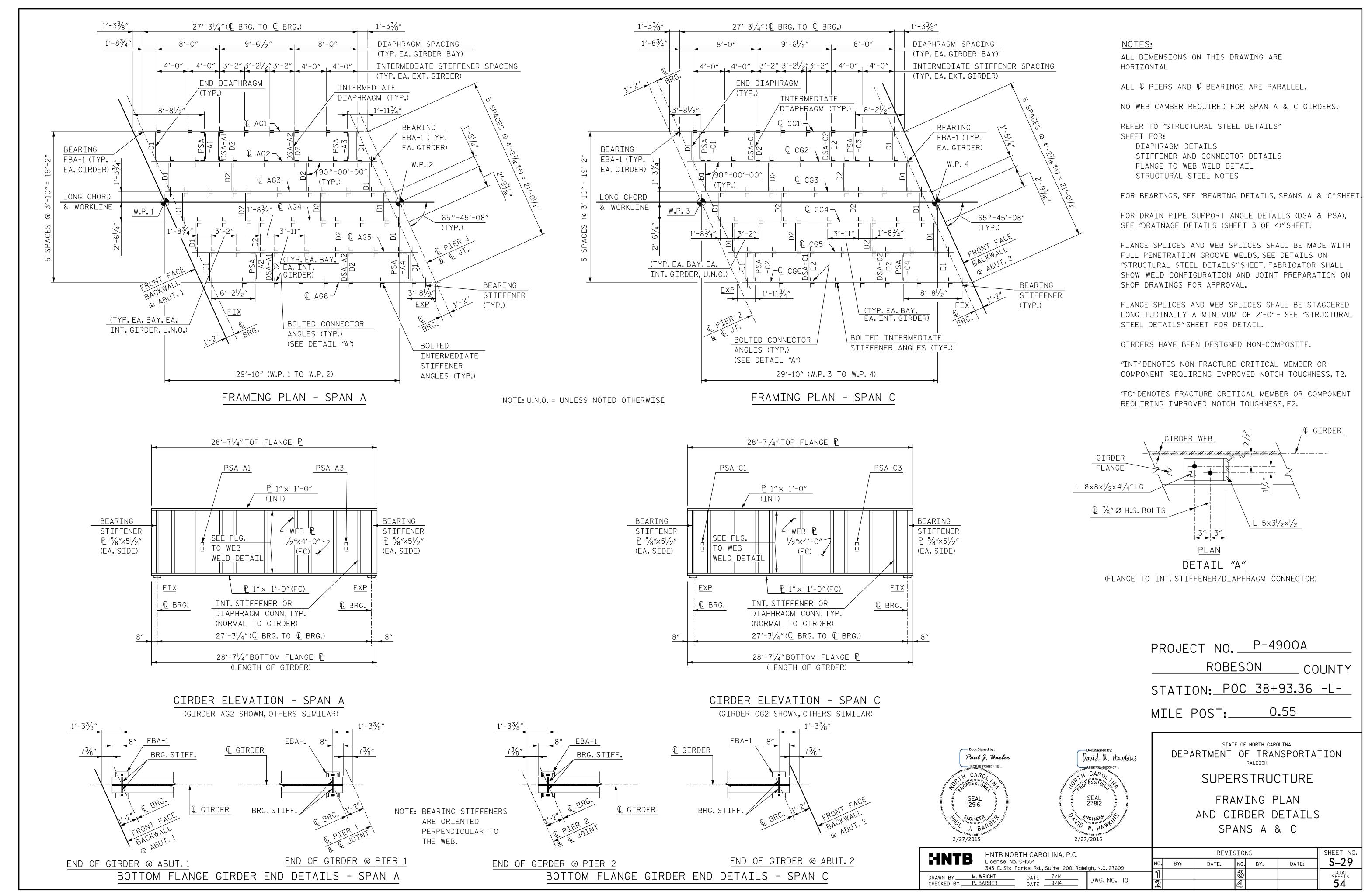
HNTB NORTH CAROLINA, P.C. License No. C-1554	HNTB NORTH CAROLINA, P.C.						REVISIONS						
License No. C-1554 343 E. Six Forks Rd., Suite 200, Re	ıleiah. N.C. 27609	NO.	BY:	DATE:	NO.	BY:	DATE:	S-24					
DRAWN BY M. WRIGHT DATE		11			3			TOTAL SHEETS					
CHECKED BY P. BARBER DATE 7/14	DWG. NO. 5	2			4			54					

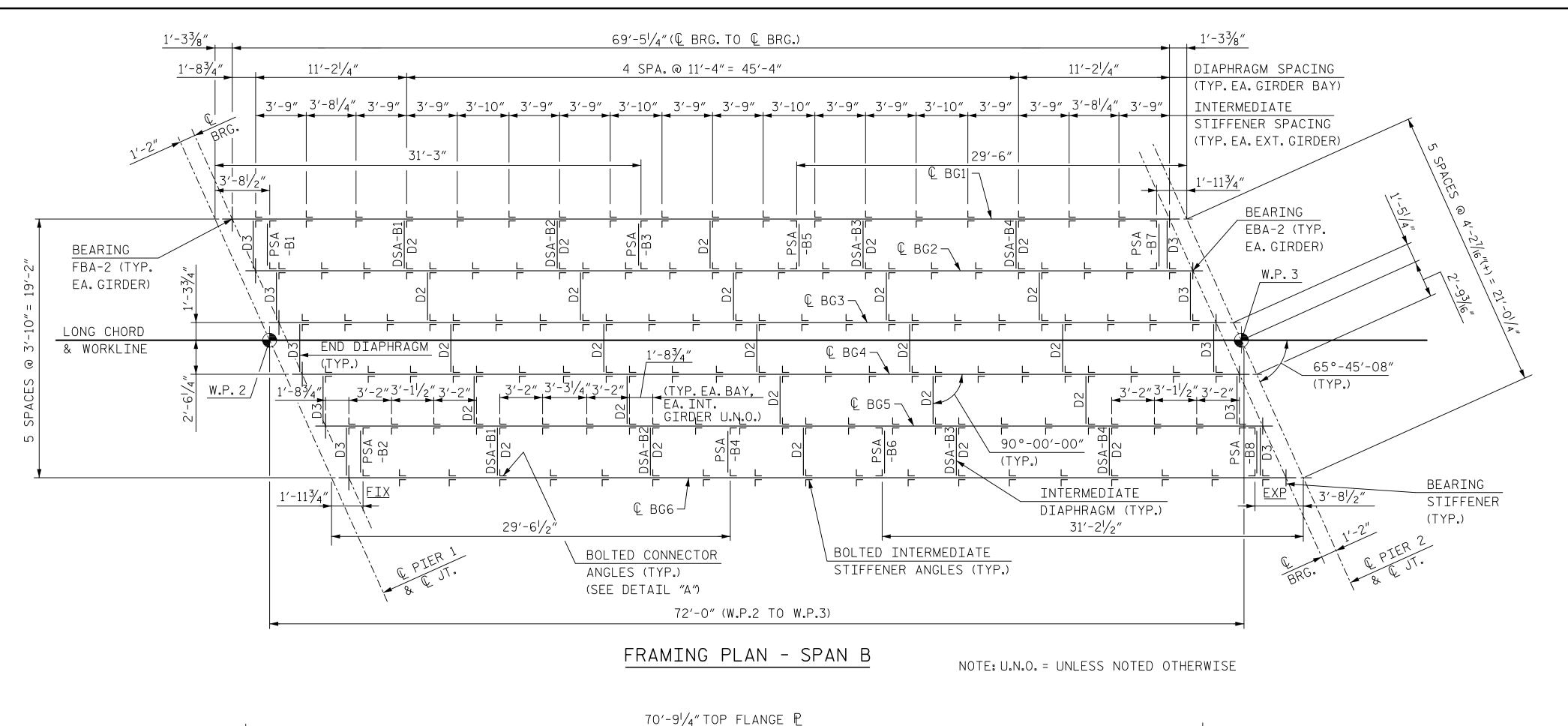


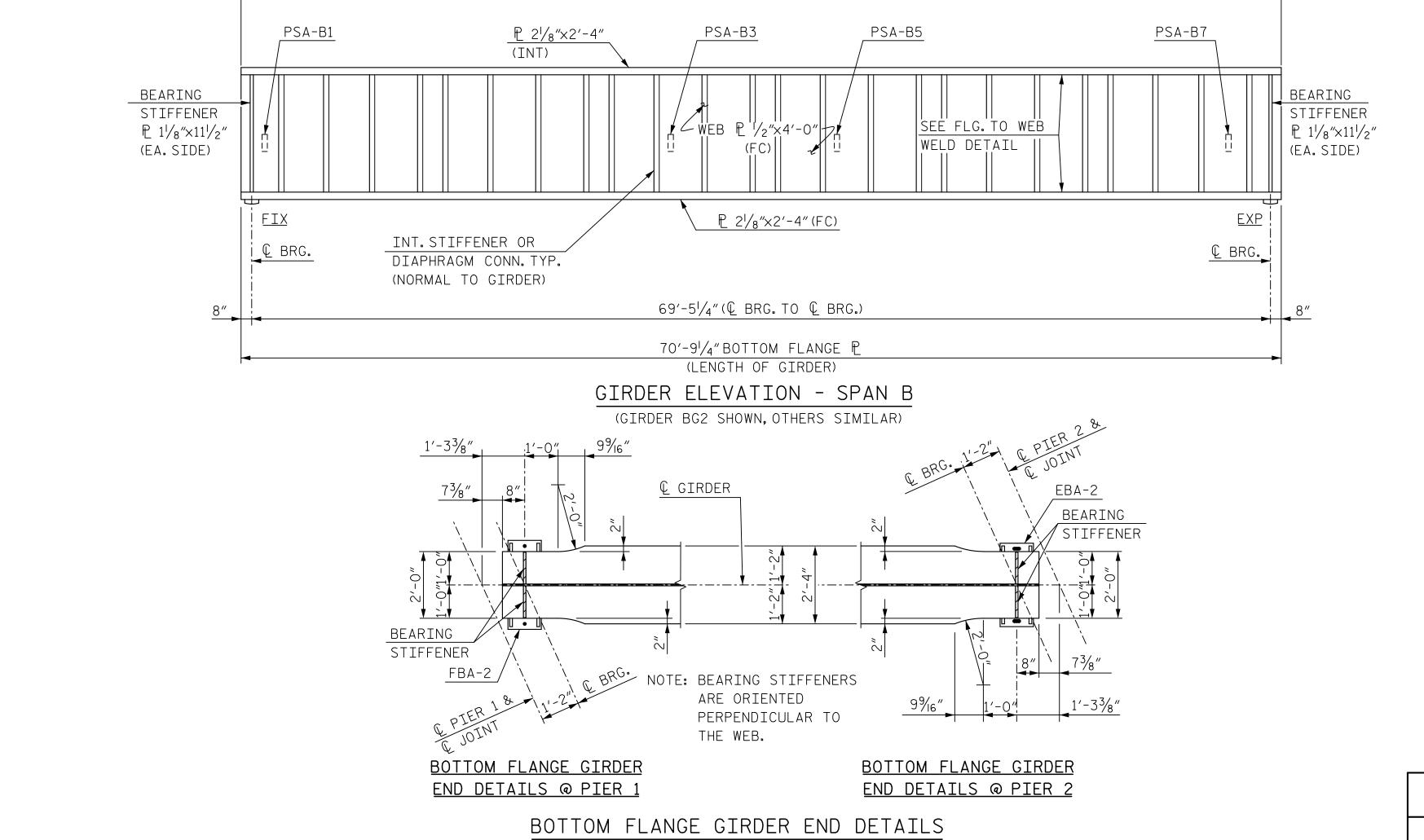


NOTES: ALL REINFORCING SHALL BE EPOXY COATED. FOR ARCHITECTURAL TREATMENT OF PARAPETS, SEE "PARAPET ARCHITECTURAL DETAILS" SHEET. FOR SECTION B-B SEE "DECK DETAILS" SHEET. SLAB BARS MAY BE SHIFTED TO AVOID DECK DRAIN LOCATIONS. FOR DECK DRAIN DETAILS SEE DETAIL "B" ON "DRAINAGE DETAILS SHEET 3 OF 4"SHEET. U.N.O. = UNLESS NOTED OTHERWISE FOR CONCRETE PARAPET DETAILS, SEE "DECK DETAILS" SHEETS. E.F. = EACH FACE 4'-11<sup>5</sup>/<sub>16</sub>" 72'-0"€ JOINT @ PIER 1 TO € JOINT @ PIER 2 4'-11<sup>5</sup>/<sub>16</sub>" \_ 1'-1<sup>|</sup>/<sub>16</sub>" #5A3 THRU #5A12 62-#5A1 BARS @ 1'-0"CTS. = 61'-0" #5A12 THRU #5A3 TOP OF SLAB @ 1'-0" CTS. = 9'-0" @ 1'-0"CTŞ.= 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" ₡ 6″Ø D.I.P. 6" 9-#5S1 E.F. & 9-#5S2 6" € ½"OPEN JOINT IN PARAPET (TYP.) 9-#5S1 E.F. & 16-#5B5 DECK DRAINS @ 1'-0"CTS. = 8'-0" 9-#5S2 @  $11\frac{1}{2}$ " CTS. = 7'-8" 8-#5B6 (PARAPET) (PARAPET, 8 E.F.) #5A3 (TOP) (TYP. U.N.O.) (TYP. U.N.O.) #5A3 (BOT.) 1-#5A2 (BOTTOM OF TOP MAT PLACED ALONG SKEW) \\_8-#5B7 √8-#5B8 2'-0"SPLICE ℚ BG2 (PARAPET) (PARAPET) (TYP, #4 "B" BARS) #5A1 (TOP) #5A12 (TOP) ℚ BG1 (PARAPET #5A12 (BOT.) #5A1 (BOT.) 1-#5A2 (BOTTOM OF TOP ₽ BG3 5-#5A2 BARS @ 6" = 2'-0" 5-#5A2 BARS @ 6" = 2'-0" DETAIL B MAT PLACED ALONG SKEW) (TOP OF BOTTOM MAT (TOP OF BOTTOM MAT W.P.2 PLACED ALONG SKEW) PLACED ALONG SKEW) © BG4 #5A12 (TOP) #5A1 (TOP) #5A12 (BOT.) LONG CHORD & #5A1 (BOT.) BRIDGE WORKLINE \_\_65°-<u>45′-08</u>″ ℚ BG5 9'-8" 9′-8″ 9′-8″ 9′-8″ (TYP.EA. (TYP.EA. DRAIN LINE) DETAIL A | DRAIN LINE) 2'-0"SPLICE 8-#5B8 ₽ BG6 (TYP. #4 "B" BARS) (PARAPET) 8-#5B7 (PARAPET) #5A3 (TOP) 8-#5B9 (PARAPET) 9-#5S1 E.F. & 9-#5S2 6" #5A3 (BOT.) 16-#5B5 @ 1'-0"CTS. = 8'-0" (PARAPET, 8 E.F.) 8-#5B6 (PARAPET) 9-#5\$1 E.F. & (TYP. U.N.O.) (TYP. U.N.O.) © 1/2"OPEN JOINT IN PARAPET (TYP.) 9-#5S2 @ 11½" CTS. = 7′-8" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" 9'-0" BOT.OF SLAB 62-#5A1 BARS @ 1'-0"CTS. = 61'-0" #5A12 THRU #5A3 #5A3 THRU #5A12 @ 1'-0" CTS. = 9'-0" @ 1'-0"CTŞ. = 9'-0" 5′-57/8″ 72'-0"W.P.2 TO W.P.3 PROJECT NO. P-4900A 5′-5<sup>7</sup>/<sub>8</sub>″ <u>PLAN - SPAN B</u> ROBESON COUNTY STATION: POC 38+93.36 -L-0.55 MILE POST:\_\_\_ STATE OF NORTH CAROLINA David W. Hawkins LONG CHORD & LONG CHORD & DEPARTMENT OF TRANSPORTATION END DAM Paul J. Barber END DAM BRIDGE WORKLINE BRIDGE WORKLINE CARO SUPERSTRUCTURE SEAL 27812 SEAL 12916 W.P.2 PLAN OF DECK - SPAN B 65°-45′-08″ 65°-45′-08″ HNTB NORTH CAROLINA, P.C. License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 SHEET NO. REVISIONS <u>DETAIL A</u> DETAIL B S-27 NO. BY: DATE: DATE: BY: TOTAL SHEETS **54** DRAWN BY M. WRIGHT
CHECKED BY P. BARBER DATE <u>6/14</u>
DATE <u>7/14</u> DWG. NO. 8









NOTES:

ALL DIMENSIONS ON THIS DRAWING ARE HORIZONTAL

ALL & PIERS AND & BEARINGS ARE PARALLEL.

NO WEB CAMBER REQUIRED FOR SPAN B GIRDERS.

REFER TO "STRUCTURAL STEEL DETAILS"
SHEET FOR:
DIAPHRAGM DETAILS
STIFFENER AND CONNECTOR DETAILS

DIAPHRAGM DETAILS
STIFFENER AND CONNECTOR DETAILS
FLANGE TO WEB WELD DETAIL
STRUCTURAL STEEL NOTES

FOR BEARINGS, SEE "BEARING DETAILS, SPAN B" SHEET.

FOR DRAIN PIPE SUPPORT ANGLE DETAILS (DSA & PSA), SEE "DRAINAGE DETAILS (SHEET 3 OF 4)" SHEET.

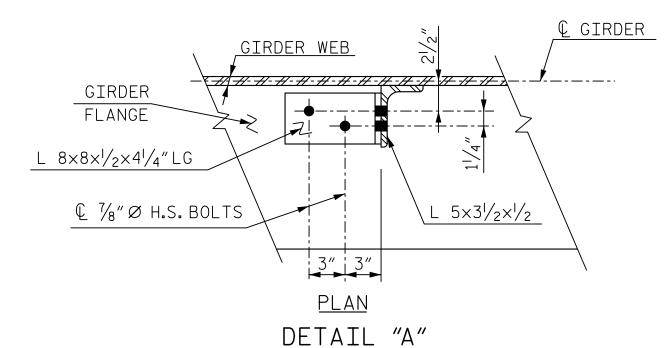
FLANGE SPLICES AND WEB SPLICES SHALL BE MADE WITH FULL PENETRATION GROOVE WELDS, SEE DETAILS ON "STRUCTURAL STEEL DETAILS" SHEET. FABRICATOR SHALL SHOW WELD CONFIGURATION AND JOINT PREPARATION ON SHOP DRAWINGS FOR APPROVAL.

FLANGE SPLICES AND WEB SPLICES SHALL BE STAGGERED LONGITUDINALLY A MINIMUM OF 2'-O" - SEE "STRUCTURAL STEEL DETAILS" SHEET FOR DETAIL.

GIRDERS HAVE BEEN DESIGNED NON-COMPOSITE.

"INT" DENOTES NON-FRACTURE CRITICAL MEMBER OR COMPONENT REQUIRING IMPROVED NOTCH TOUGHNESS, T2.

"FC" DENOTES FRACTURE CRITICAL MEMBER OR COMPONENT REQUIRING IMPROVED NOTCH TOUGHNESS, F2.



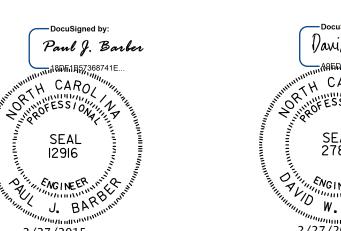
(FLANGE TO INT. STIFFENER/DIAPHRAGM CONNECTOR)

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POC 38+93.36 -L-

MILE POST: 0.55



Docusigned by:

David W. Hawkins

HARDITATABBS5487

CAROL

STATE OF NORTH CAROLINA

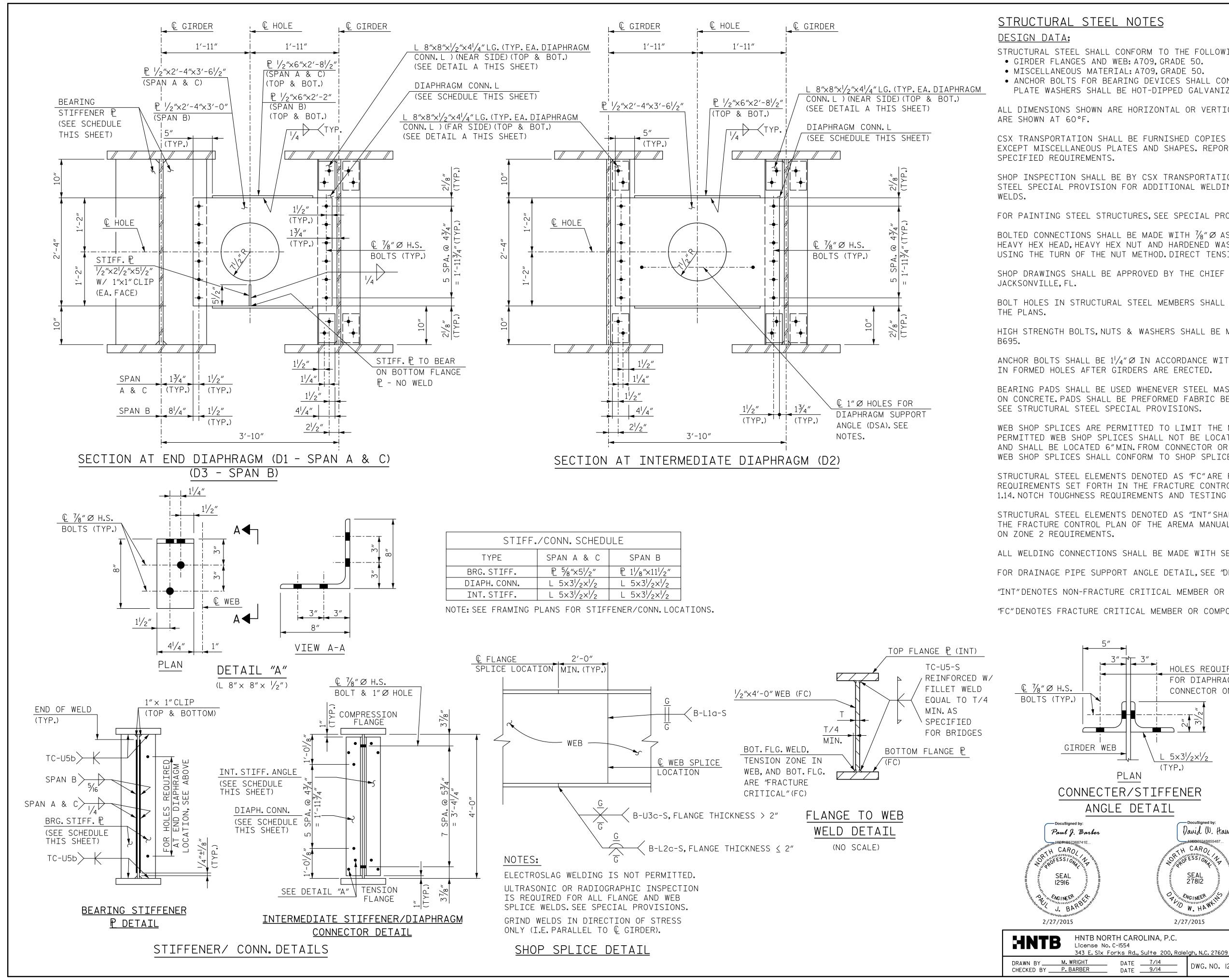
DEPARTMENT OF TRANSPORTATION

RALEIGH

SUPERSTRUCTURE

FRAMING PLAN AND GIRDER DETAILS SPAN B

HNTB	HNTB NORT	H CAROLINA, P.C				REVI	SIO	NS		SHEET NO.
MID		license No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609			BY:	DATE:	NO.	BY:	DATE:	S-30
DRAWN BYM. \	WRIGHT	DATE 7/14		1			3			TOTAL SHEETS
CHECKED BY P. E	BARBER	DATE 9/14	- DWG. NO. II	2			4			54



STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:

- ANCHOR BOLTS FOR BEARING DEVICES SHALL CONFORM TO ASTM A449. ANCHOR BOLTS, NUTS, AND PLATE WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH AASHTO M232.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL UNLESS OTHERWISE NOTED. ALL DIMENSIONS

CSX TRANSPORTATION SHALL BE FURNISHED COPIES OF MILL TEST REPORTS FOR ALL MATERIALS EXCEPT MISCELLANEOUS PLATES AND SHAPES. REPORTS SHALL INDICATE COMPLIANCE WITH ALL

SHOP INSPECTION SHALL BE BY CSX TRANSPORTATION OR ITS AUTHORIZED AGENT. SEE STRUCTURAL STEEL SPECIAL PROVISION FOR ADDITIONAL WELDING INSPECTION OF FLANGE PLATE TO WEB PLATE

FOR PAINTING STEEL STRUCTURES, SEE SPECIAL PROVISIONS.

BOLTED CONNECTIONS SHALL BE MADE WITH  $\frac{7}{8}$ "  $\varnothing$  ASTM A325, TYPE 1 HIGH STRENGTH BOLTS WITH HEAVY HEX HEAD, HEAVY HEX NUT AND HARDENED WASHERS IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS USING THE TURN OF THE NUT METHOD. DIRECT TENSION INDICATORS SHALL NOT BE USED.

SHOP DRAWINGS SHALL BE APPROVED BY THE CHIEF ENGINEER-BRIDGES, CSX TRANSPORTATION,

BOLT HOLES IN STRUCTURAL STEEL MEMBERS SHALL BE STANDARD SIZE UNLESS OTHERWISE INDICATED ON

HIGH STRENGTH BOLTS, NUTS & WASHERS SHALL BE MECHANICALLY GALVANIZED IN ACCORDANCE WITH ASTM

ANCHOR BOLTS SHALL BE  $1^{1}/4^{\prime\prime} \varnothing$  IN ACCORDANCE WITH A.R.E.M.A. SPECIFICATIONS AND SHALL BE GROUTED IN FORMED HOLES AFTER GIRDERS ARE ERECTED.

BEARING PADS SHALL BE USED WHENEVER STEEL MASONRY PLATE, OR OTHER STEEL BEARING PLATE, BEARS ON CONCRETE. PADS SHALL BE PREFORMED FABRIC BEARING PADS,  $\frac{1}{2}$ " THICK. FOR PAD REQUIREMENTS, SEE STRUCTURAL STEEL SPECIAL PROVISIONS.

WEB SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED WEB PIECE LENGTHS TO 45'-O". PERMITTED WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 15'-0" OF MAXIMUM DEAD LOAD DEFLECTION AND SHALL BE LOCATED 6"MIN.FROM CONNECTOR OR INTERMEDIATE STIFFENER CONNECTIONS.FLANGE AND WEB SHOP SPLICES SHALL CONFORM TO SHOP SPLICE DETAILS SHOWN ON THE PLANS.

STRUCTURAL STEEL ELEMENTS DENOTED AS "FC" ARE FRACTURE CRITICAL AND SHALL MEET IMPACT TEST REQUIREMENTS SET FORTH IN THE FRACTURE CONTROL PLAN OF THE AREMA MANUAL, CHAPTER 15, SECTION 1.14. NOTCH TOUGHNESS REQUIREMENTS AND TESTING SHALL BE BASED ON ZONE 2 REQUIREMENTS.

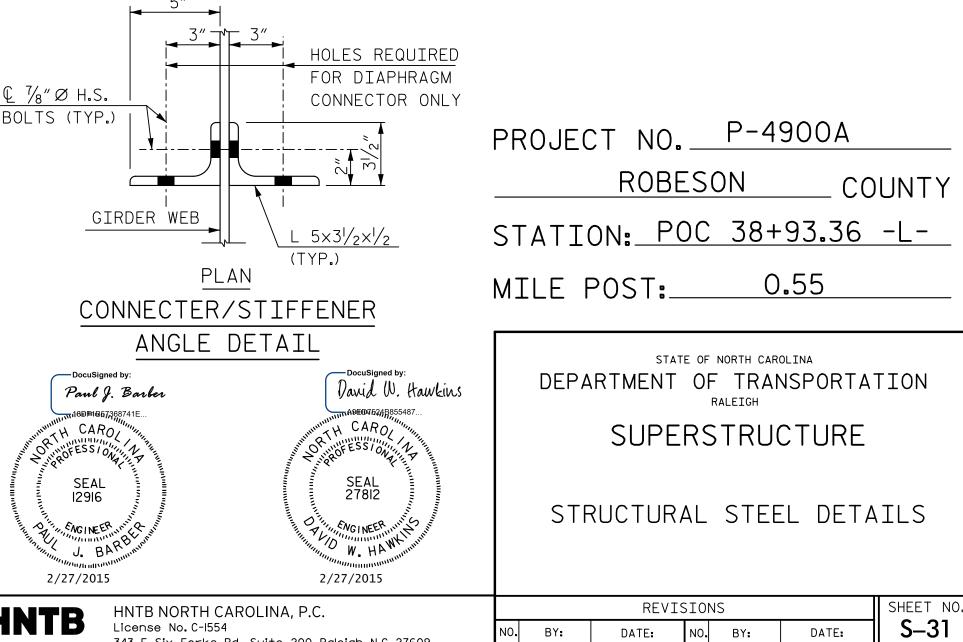
STRUCTURAL STEEL ELEMENTS DENOTED AS "INT" SHALL MEET IMPACT TEST REQUIREMENTS SET FORTH IN THE FRACTURE CONTROL PLAN OF THE AREMA MANUAL CHAPTER 15 SECTION 1.2 TESTING SHALL BE BASED

ALL WELDING CONNECTIONS SHALL BE MADE WITH SERIES E70 WELDING ELECTRODES.

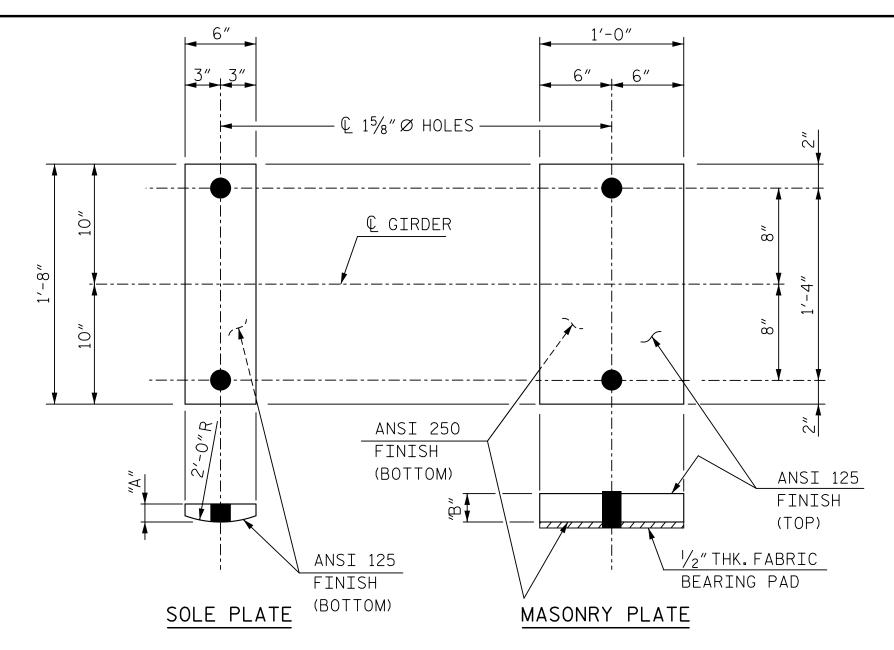
FOR DRAINAGE PIPE SUPPORT ANGLE DETAIL, SEE "DRAINAGE DETAILS (SHEET 3 OF 4)".

"INT" DENOTES NON-FRACTURE CRITICAL MEMBER OR COMPONENT REQUIRING IMPROVED NOTCH TOUGHNESS. T2.

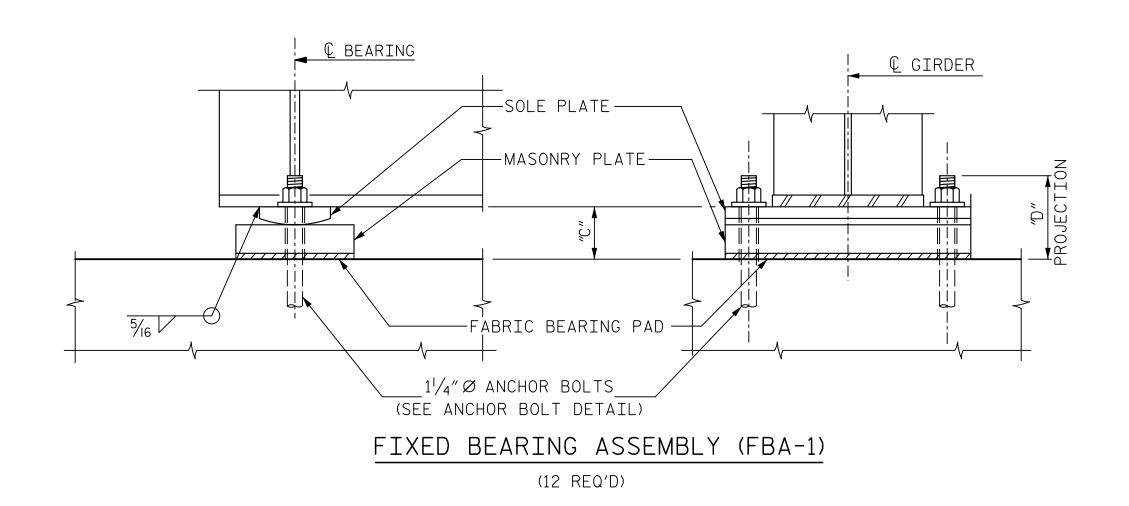
"FC" DENOTES FRACTURE CRITICAL MEMBER OR COMPONENT REQUIRING IMPROVED NOTCH TOUGHNESS, F2.



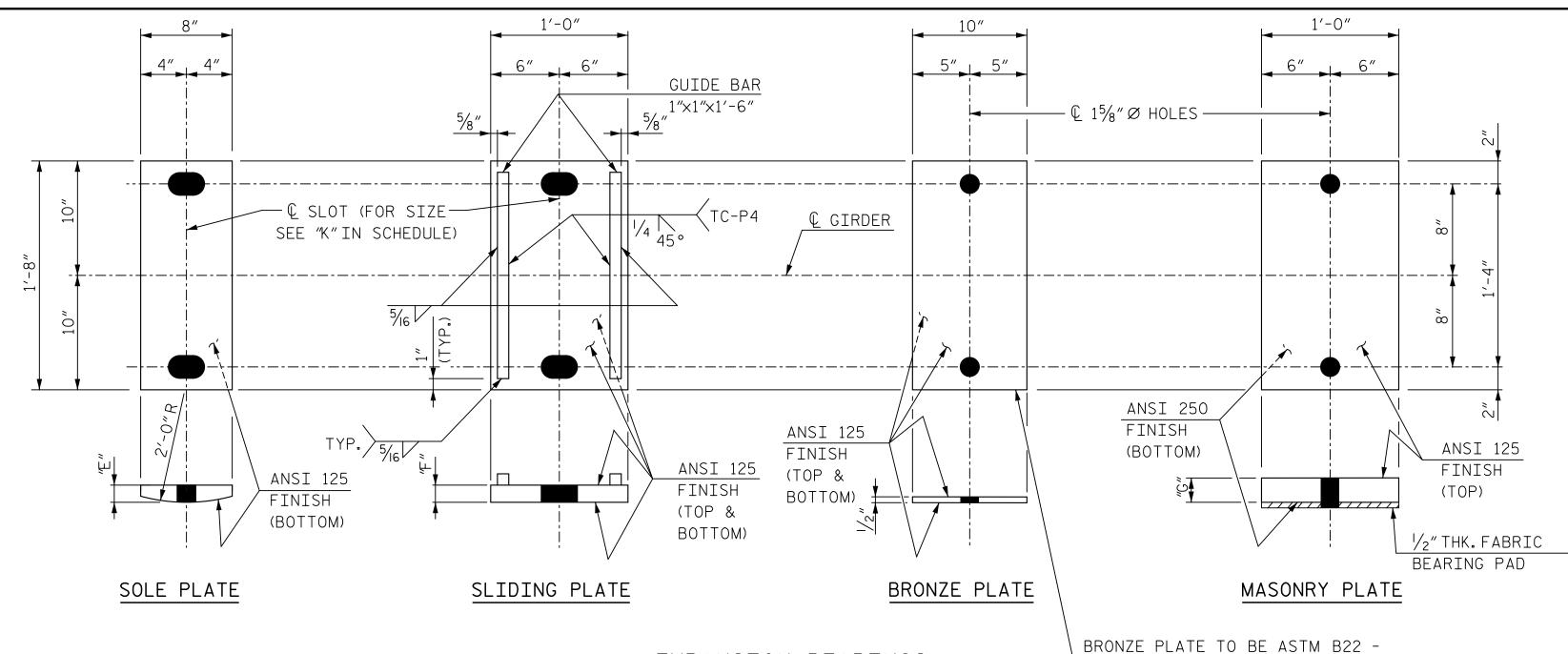
TOTAL SHEETS **54** 



FIXED BEARINGS



BEARING PLATE SCHEDULE													
	DIMENSIONS												
GIRDER	FIXED BEARING (FBA-1) EXPANSION BEARING (EBA-1)												
	"A"	<b>"</b> B"	"C"	<b>″</b> D″	<b>"</b> E"	<b>"</b> F"	"G"	<b>"</b> H"	"J"	<b>"</b> K"			
AG1	11/2"	23/8"	43/8"	7"	11/2"	11/2"	21/8"	61/8"	9"	2″x3″			
AG2	11/2"	23/8"	43/8"	7"	11/2"	11/2"	21/8"	61/8"	9″	2″x3″			
AG3	11/2"	23/8"	43/8"	7"	11/2"	11/2"	21/8"	61/8"	9″	2″x3″			
AG4	11/2"	23/8"	43/8"	7"	11/2"	11/2"	21/8"	61/8"	9"	2″x3″			
AG5	11/2"	21/2"	41/2"	7"	11/2"	11/2"	21/8"	61/8"	9"	2″x3″			
AG6	11/2"	21/2"	41/2"	7"	11/2"	11/2"	21/8"	61/8"	9"	2″×3″			
CG1	11/2"	21/2"	41/2"	7"	11/2"	11/2"	35/8"	75/8″	101/2"	2″x3″			
CG2	11/2"	23/8"	43/8"	7"	11/2"	11/2"	35/8"	75/8″	101/2"	2″×3″			
CG3	11/2"	23/8"	43/8"	7"	11/2"	11/2"	35/8"	75/8″	101/2"	2″×3″			
CG4	11/2"	23/8"	43/8"	7"	11/2"	11/2"	35/8"	75/8″	101/2"	2″x3″			
CG5	11/2"	23/8"	43/8"	7"	11/2"	11/2"	31/2"	71/2"	101/2"	2″x3″			
CG6	11/2"	23/8"	43/8"	7"	11/2"	11/2"	31/2"	71/2"	101/2"	2″x3″			



EXPANSION BEARINGS

TO BE SELF LUBRICATING. ℚ BEARING ₡ GIRDER GUIDE BAR — SOLE PLATE — -SLIDING PLATE-— BRONZE PLATE — - MASONRY PLATE ---— FABRIC BEARING PAD — 11/4" Ø ANCHOR BOLTS 11/2"Ø STANDARD PIPE

EXPANSION BEARING ASSEMBLY (EBA-1) (12 REQ'D)

(SEE ANCHOR BOLT DETAIL)

SLEEVE THRU SOLE PLATE & SLIDING PLATE &  $\frac{1}{8}$ "ABOVE SOLE PLATE (GALVANIZED STEEL PIPE)(TYP.)

ABUTMENT 1 & 2 (24 REQ'D) 1'-9" PIER 1 (12 REQ'D) 1'-101/2" PIER 2 (12 REQ'D) STD.HEX NUT 3″∅ X¾″THICK WASHER WITH 15/16" Ø HOLE 1'-0" EMBEDMENT 10"SWEDGE ANCHOR BOLT DETAIL

1'-7"

NOTES:

ANCHOR BOLTS, SLIDING PLATE (EXPANSION BEARING) AND MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL BE HOT DIPPED GALVANIZED.

ALL PLATE SURFACES SHALL BE PAINTED WITH A 3 COAT PAINT SYSTEM EXCEPT AS SPECIFIED BELOW.

(A) THE SLIDING PLATE (EXPANSION BEARING) SHALL NOT BE PAINTED BUT SHALL RECEIVE A COAT OF LUBRICATION.

(B) THE MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL NOT BE PAINTED.

(C) THE BOTTOM SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED BUT SHALL RECEIVE A SINGLE COAT OF PRIMER APPLIED IN THE SHOP.

(D) THE TOP SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED IN THE VICINITY OF THE WELD BETWEEN THE SOLE PLATE AND THE BOTTOM FLANGE.

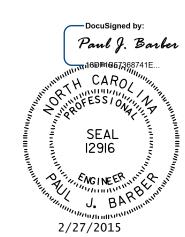
FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

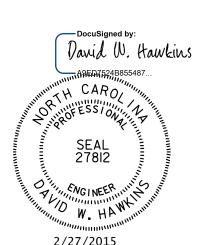
FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES, SEE SPECIAL PROVISIONS.

PROJECT NO. P-4900A ROBESON COUNTY

STATION: POC 38+93.36 -L-

0.55 MILE POST:\_\_\_\_





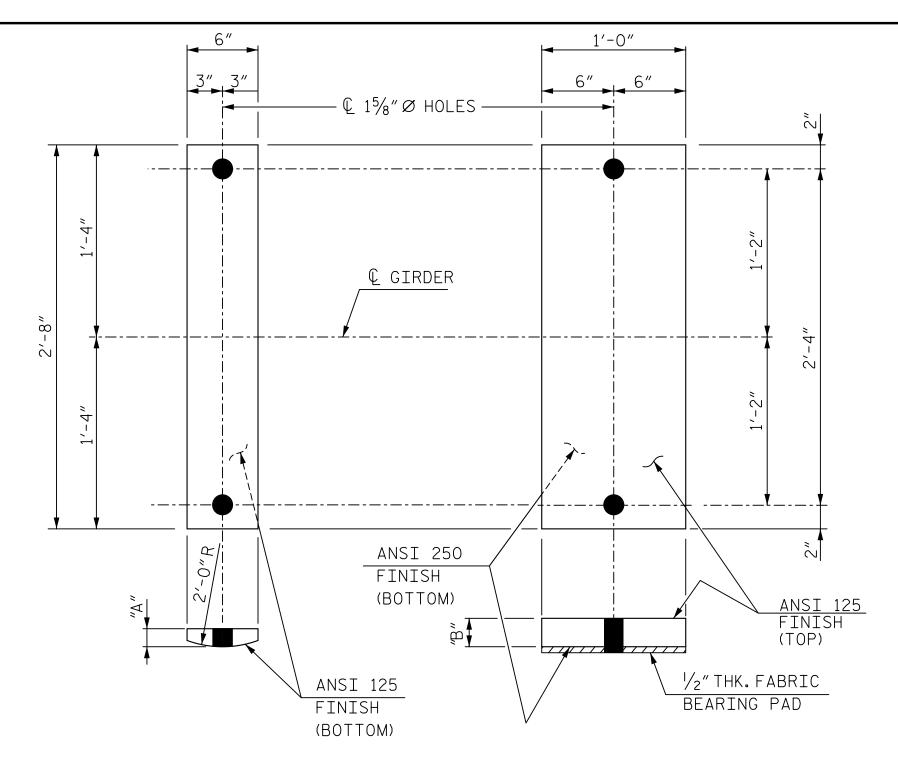
COPPER ALLOY 911 OR ASTM B100 -

COPPER ALLOY 510. TOP SURFACE

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUPERSTRUCTURE

> BEARING DETAILS SPANS A & C

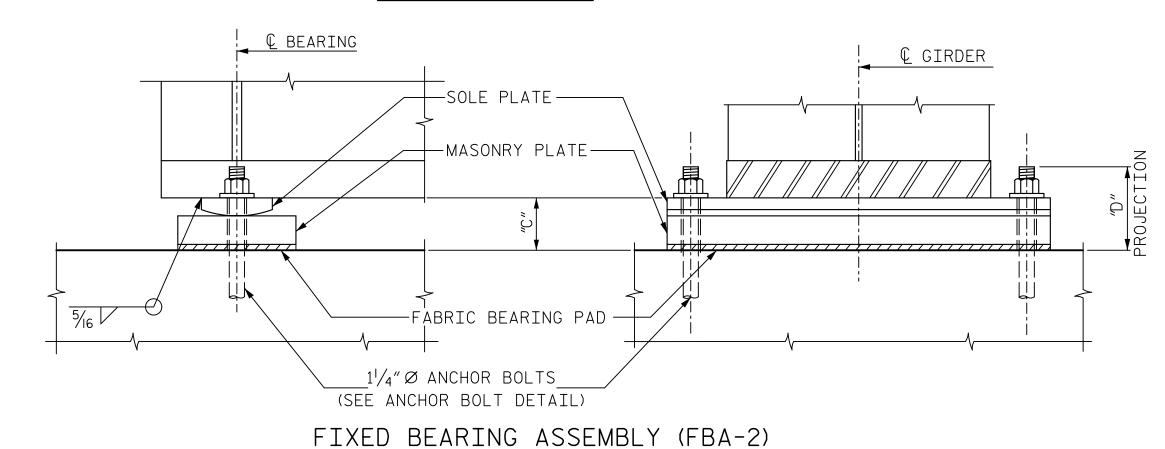
. ,										
HNTB NORTH CAROLINA, P.C. License No. C-1554						REVI:	OIS	IS		SHEET NO.
License No. C-1554  343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609				NO.	BY:	DATE:	NO.	BY:	DATE:	S-32
			111, 14:0: 21003	1			3			TOTAL SHEETS
DIVA 1111 D1		7/I4 8/I4	DWG. NO. 13	2			4			54



SOLE PLATE

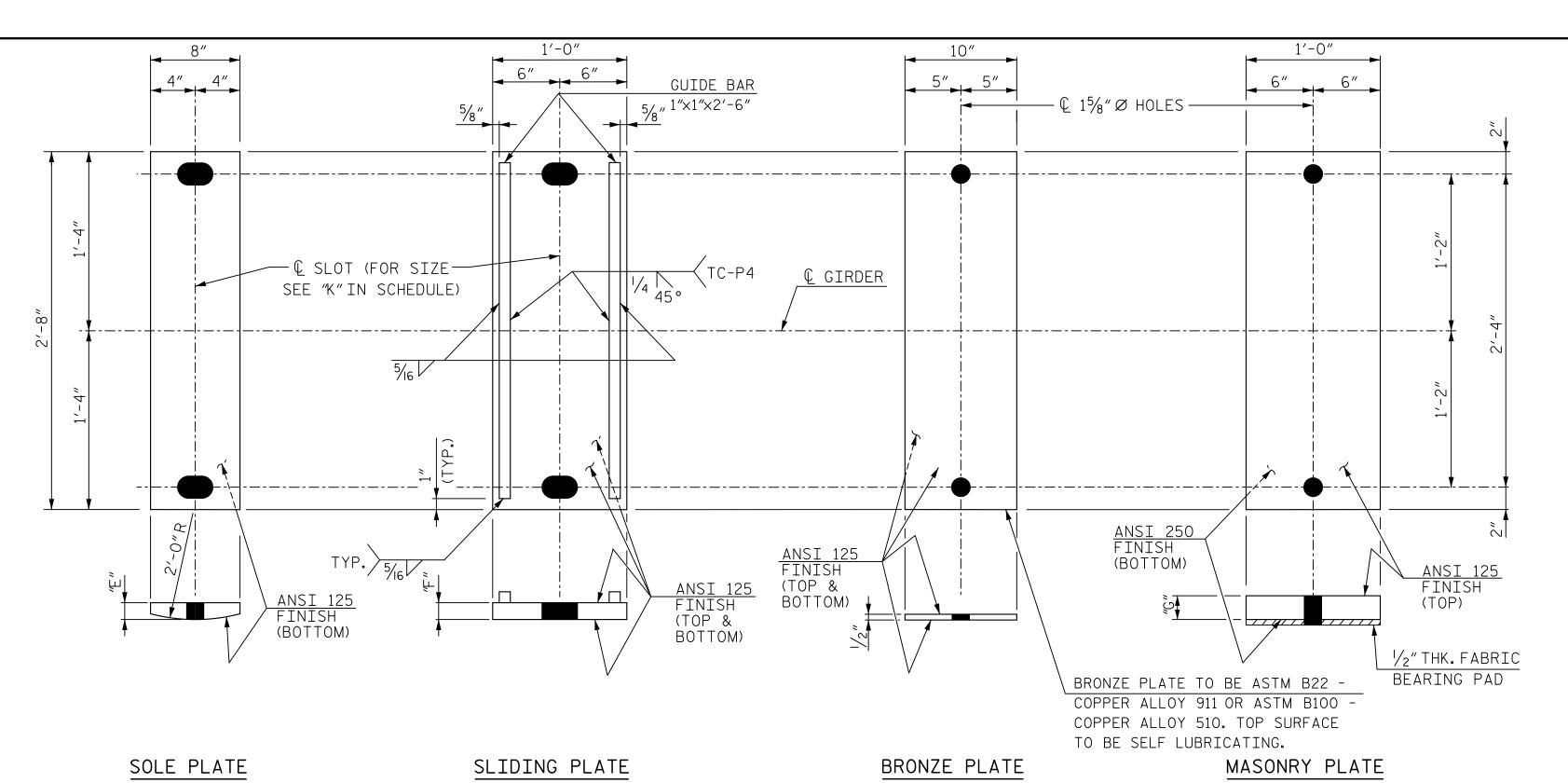
MASONRY PLATE

# FIXED BEARINGS



(6 REQ'D)

BEARING PLATE SCHEDULE DIMENSIONS EXPANSION BEARING (EBA-2) GIRDER FIXED BEARING (FBA-2) **″**D″ **″**E″  $7\frac{1}{2}''$   $1\frac{1}{2}''$  $1\frac{1}{2}''$ 2<sup>1</sup>/2" 6<sup>1</sup>/2" BG1 2"×4" BG2  $2\frac{1}{2}''$ 6<sup>1</sup>/2" 2"×4" 7<sup>1</sup>/2"  $1\frac{1}{2}''$  $1\frac{1}{2}''$ 11/2" 21/2" 2"×4" BG3 11/2" 3"  $7\frac{1}{2}''$  $1\frac{1}{2}''$  $1\frac{1}{2}''$ 6<sup>|</sup>/2" 9" 21/2" 6<sup>|</sup>/2" BG4  $7\frac{1}{2}''$ 11/2" 11/2" 2"×4" 11/2" 2<sup>1</sup>/2" BG5 7½" 11/2" 61/2" 2"×4" BG6  $1^{1/2}''$ 3" 5″  $7\frac{1}{2}''$   $1\frac{1}{2}''$  $1^{1/2}''$  $2^{1/2''}$ 6<sup>1</sup>/2" 9" 2"×4"



EXPANSION BEARINGS ₡ GIRDER —SOLE PLATE— -SLIDING PLATE-

GUIDE BAR - BRONZE PLATE-- MASONRY PLATE ackslash Fabric bearing pad-11/2" Ø STANDARD PIPE 1 1/4" Ø ANCHOR BOLTS SLEEVE THRU SOLE PLATE & (SEE ANCHOR BOLT DETAIL) SLIDING PLATE & 1/8"ABOVE EXPANSION BEARING ASSEMBLY (EBA-2) SOLE PLATE (GALVANIZED STEEL (6 REQ'D) PIPE)(TYP.)

1'-71/2" PIER 1 (12 REQ'D) PIER 2 1'-9" (12 REQ'D) STD.HEX NUT 3″Ø X¾″THICK WASHER WITH 15/16" Ø HOLE 1'-0" EMBEDMENT 10" SWEDGE THREAD ANCHOR BOLT DETAIL

NOTES:

ANCHOR BOLTS, SLIDING PLATE (EXPANSION BEARING) AND MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL BE HOT DIPPED GALVANIZED.

ALL PLATE SURFACES SHALL BE PAINTED WITH A 3 COAT PAINT SYSTEM EXCEPT AS SPECIFIED BELOW.

(A) THE SLIDING PLATE (EXPANSION BEARING) SHALL NOT BE PAINTED BUT SHALL RECEIVE A COAT OF LUBRICATION.

(B) THE MASONRY PLATE (FIXED AND EXPANSION BEARINGS) SHALL NOT BE PAINTED.

(C) THE BOTTOM SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED BUT SHALL RECEIVE A SINGLE COAT OF PRIMER APPLIED IN THE SHOP.

(D) THE TOP SURFACE OF THE SOLE PLATE SHALL NOT BE PAINTED IN THE VICINITY OF THE WELD BETWEEN THE SOLE PLATE AND THE BOTTOM FLANGE.

FOR PAINTING STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

FOR SELF-LUBRICATING EXPANSION BEARING ASSEMBLIES, SEE SPECIAL PROVISIONS.

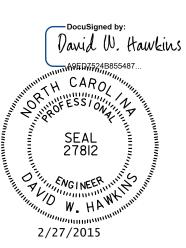
PROJECT NO. P-4900A ROBESON

STATION: POC 38+93.36 -L-

COUNTY

0.55 MILE POST:\_\_\_

Paul J. Barber CARO SEAL 12916 W. CNGINEER

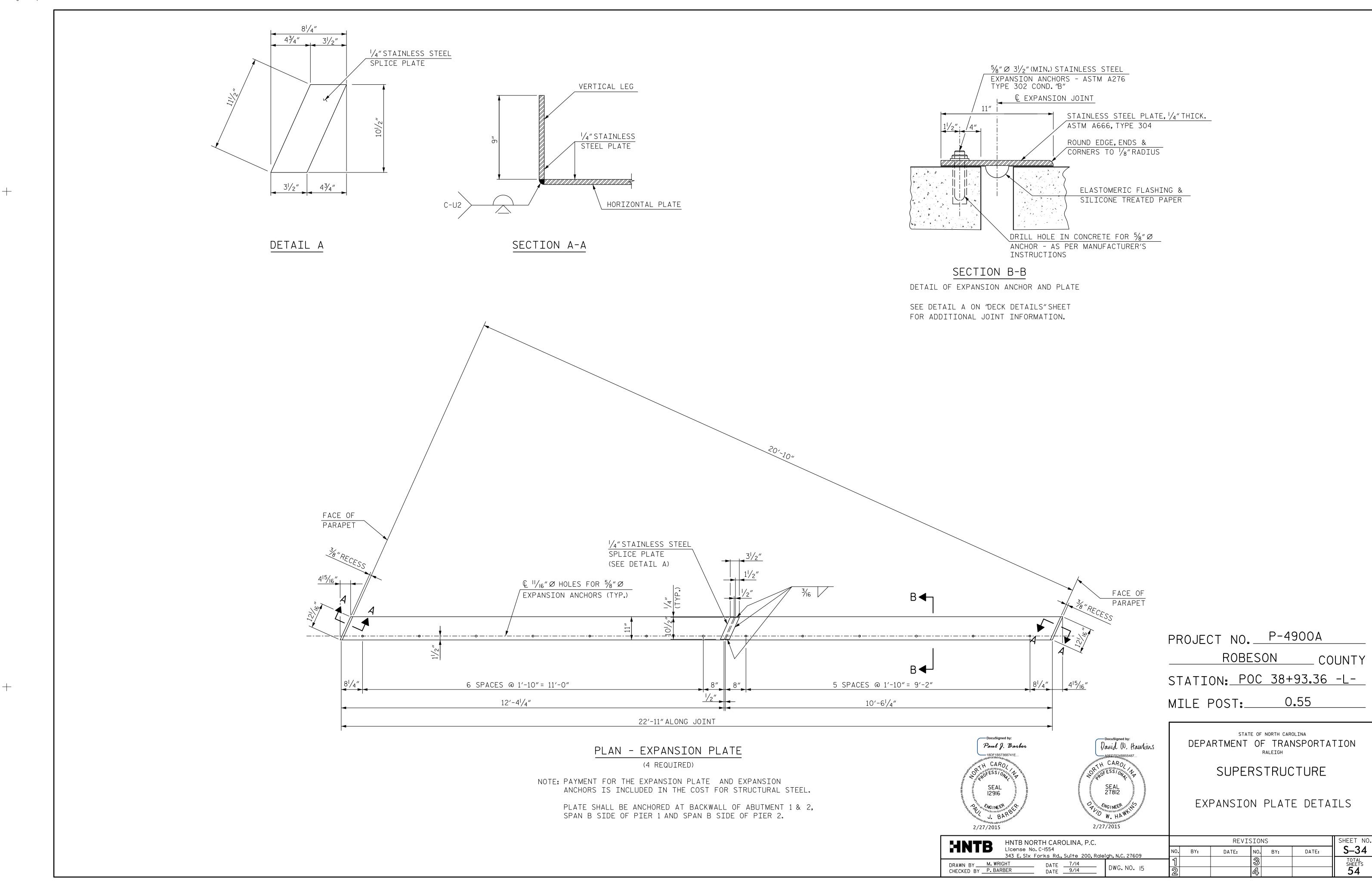


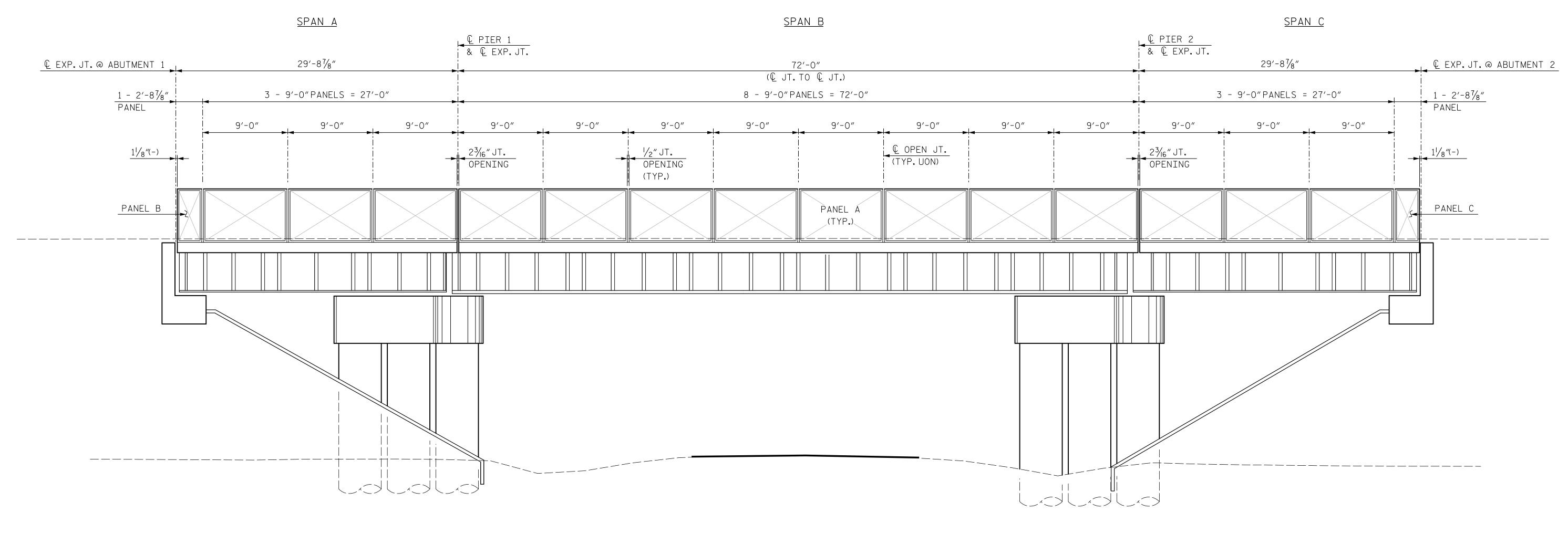
SUPERSTRUCTURE BEARING DETAILS

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SPAN B

REVISIONS SHEET NO. HNTB NORTH CAROLINA, P.C. License No. C-1554 **S**–33 NO. BY: DATE: DATE: BY: 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 TOTAL SHEETS **54** DRAWN BY J. BAYNE DATE 7/14 DWG. NO. 14 CHECKED BY D. RAGAN DATE 8/14





ELEVATION

LOOKING EAST
(ELEVATION LOOKING WEST SIMILAR)

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: POC 38+93.36 -L
MILE POST: 0.55

SHEET 1 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUPERSTRUCTURE

PARAPET ELEVATION

HNTB NORTH CAROLINA, P.C.

License No. C-1554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY M. WRIGHT DATE 7/14
CHECKED BY P. BARBER DATE 9/14

DWG. NO. 16

REVISIONS

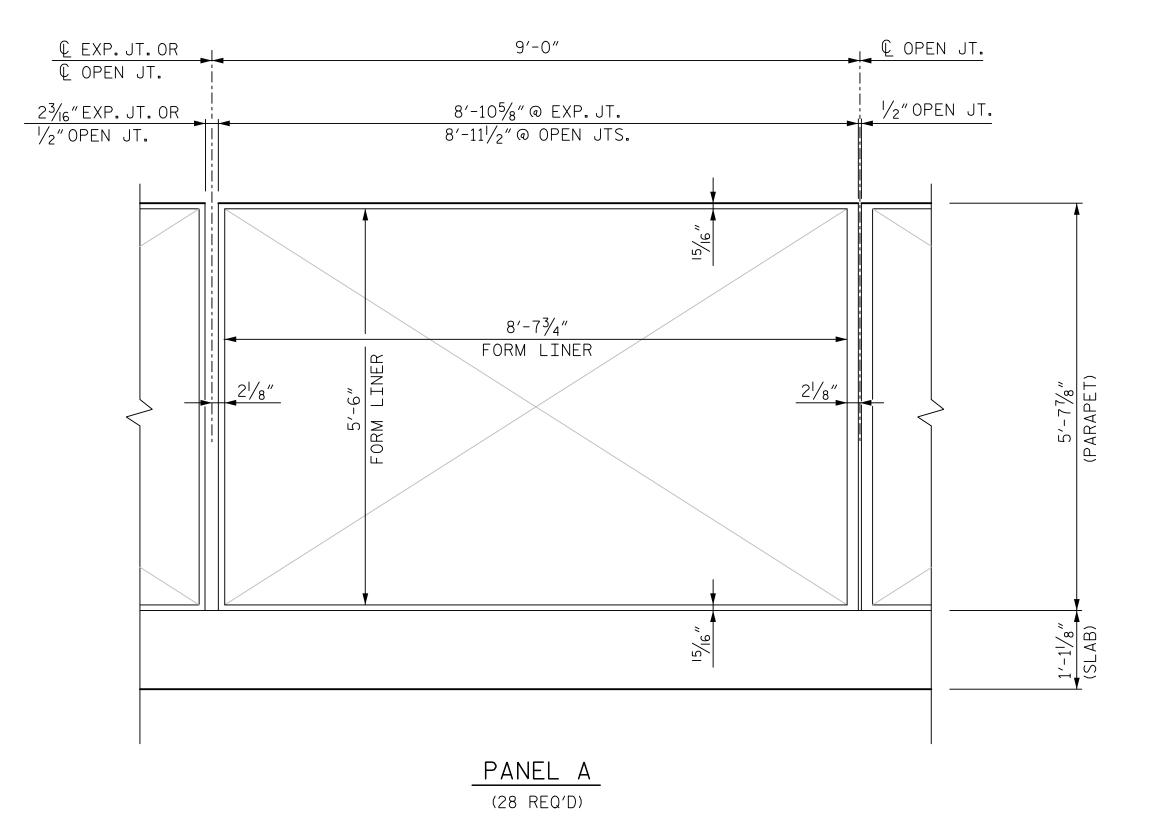
NO. BY: DATE: NO. BY: DATE:

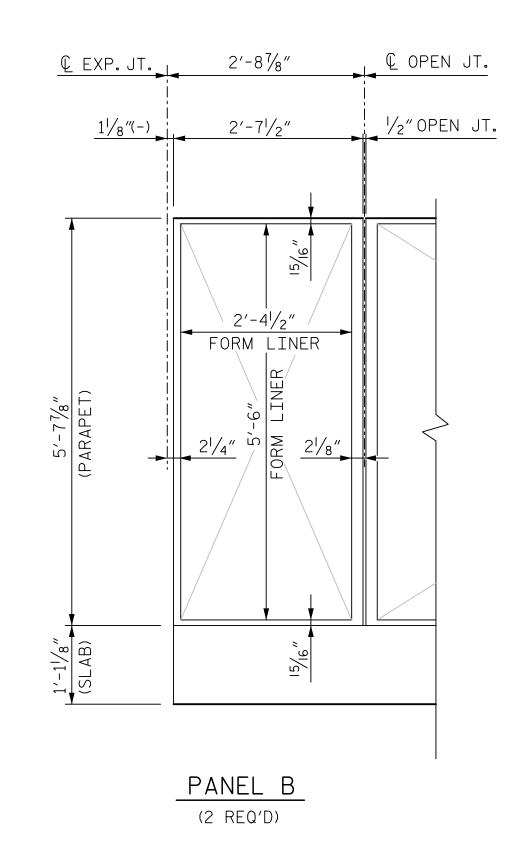
DWG. NO. 16

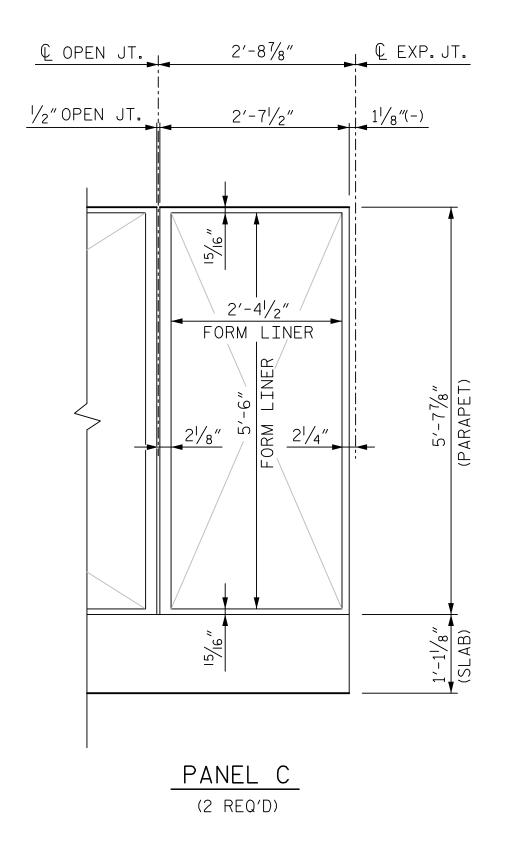
DWG. NO. 16

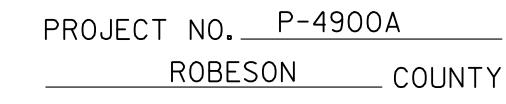
David W. Hawkins

—DocuSigned by: Paul J. Barber









STATION: <u>POC 38+93.36 -L-</u>

MILE POST: 0.55

SHEET 2 OF 4

Docusigned by:

Paul J. Barber

Dawid

Docusigned by:

Dawid W. Hawkins

AREN7524B855487...

CARO

MARINTALIA

SEAL

27812

SEAL

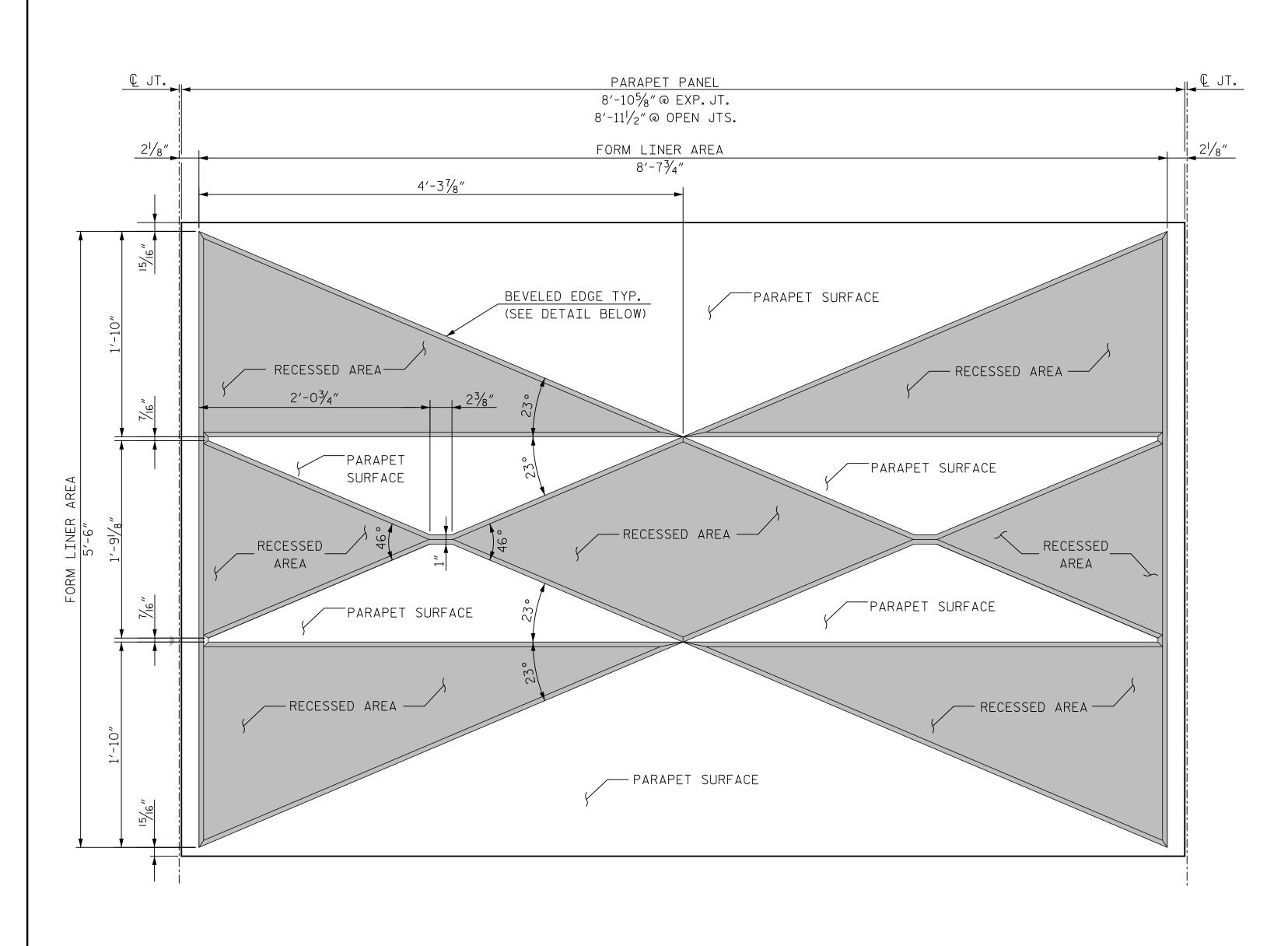
27812

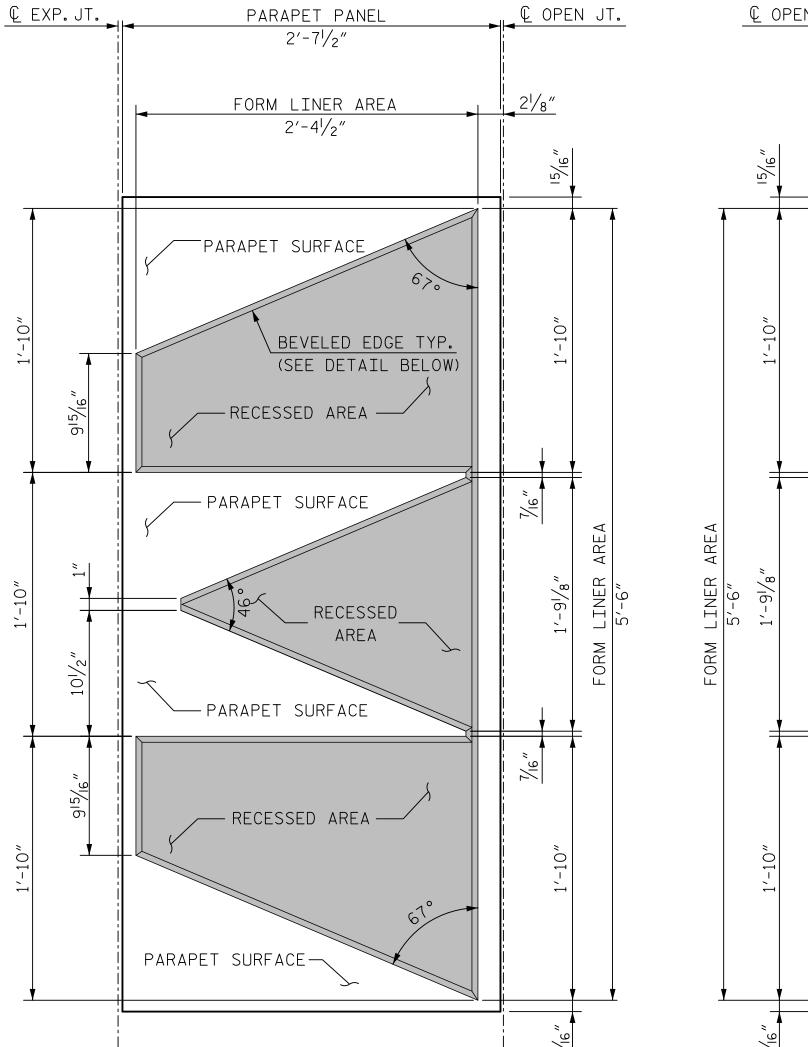
DEPARTMENT OF TRANSPORTATION
RALEIGH

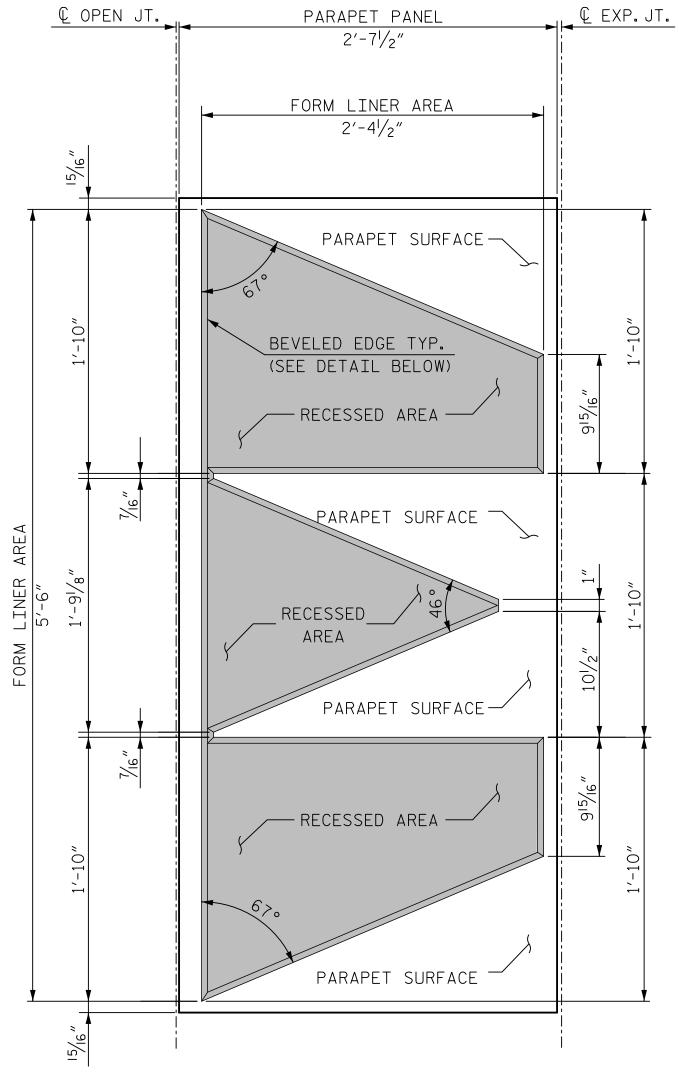
SUPERSTRUCTURE

PARAPET PANEL DETAILS

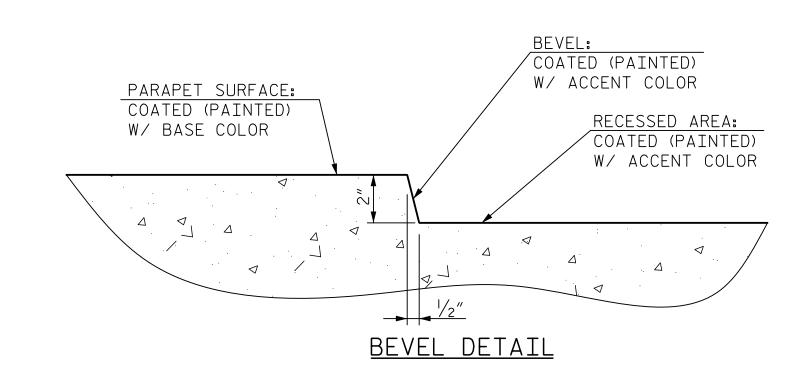
HATE HNTB NORTH CAROLINA, P.C.				REVISIONS						
	HNTB NORTH CAROLINA, P.C. License No. C-1554 343 E. Six Forks Rd Suite 200. Raleigh.	NC 27609	10.	BY:	DATE:	NO.	BY:	DATE:	S-36	
t	DRAWN BY M. WRIGHT DATE 8/14	11	1			3			TOTAL SHEETS	
	CHECKED BY P. BARBER DATE 9/14	WG. NO. 17	2			4			54	







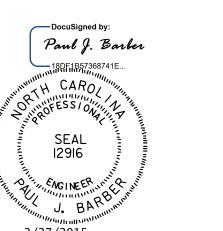
### PANEL A FORM LINER FINISH

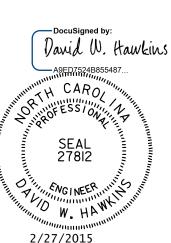


## PANEL B FORM LINER FINISH

#### **GENERAL NOTES:**

- 1. FOR CONCRETE FINISH REQUIREMENTS ON ALL PARAPET SURFACES, BEVEL SURFACES AND RECESSED SURFACES, SEE SPECIAL PROVISION.
- 2. BEVEL ALL EDGES OF RECESSED AREAS ACCORDING TO THE BEVEL DETAIL.
- 3. ALL VISIBLY EXPOSED CONCRETE SURFACES INCLUDING THE PARAPET, OUTER SLAB EDGE AND OVERHANG, ABUTMENT, AND PIERS SHALL BE COATED (PAINTED) THE BASE COLOR.
- 4. THE RECESSED AREAS AND BEVELED EDGE SHALL BE PAINTED RED, AN ACCENT COLOR FROM THE PARAPET.
- 5. THE COATING COLOR PALETTE FOR THE BASE AND ACCENT COLOR SHALL BE A MATTE COLOR FINISH AND SHALL BE SUBMITTED TO THE





PROJECT NO. P-4900A

PANEL C FORM LINER FINISH

ROBESON COUNTY

STATION: POC 38+93.36 -L-

0.55 MILE POST:\_\_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE

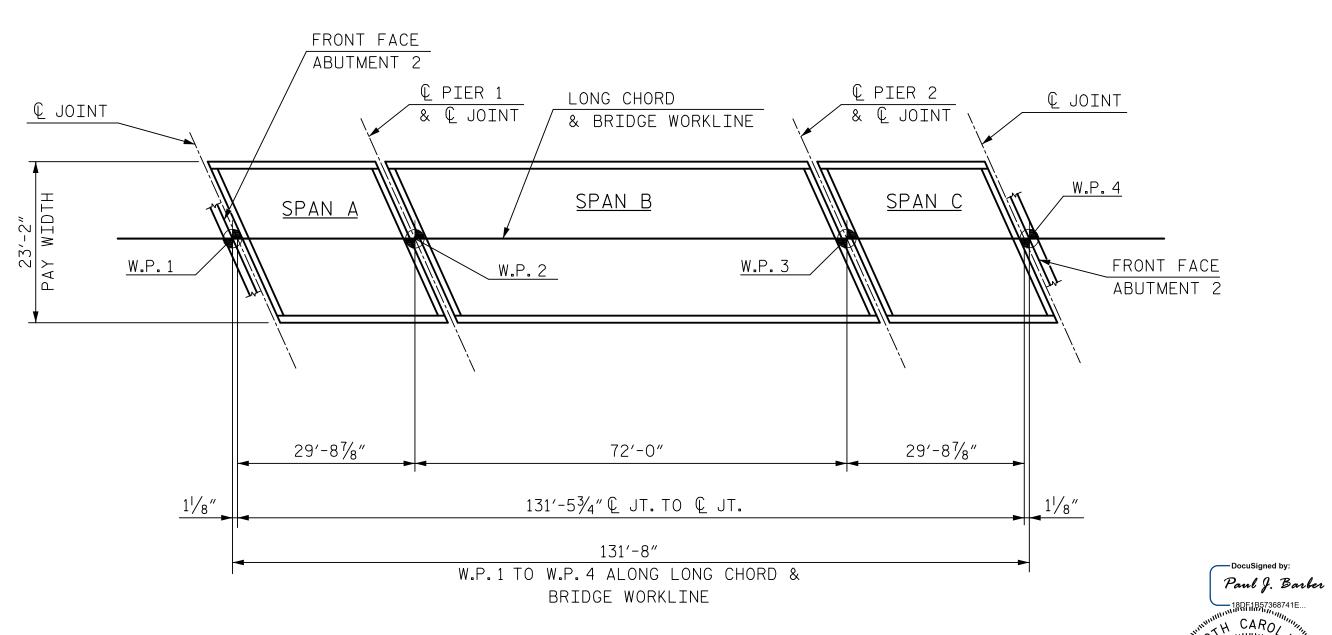
PARAPET PANEL FORM LINER FINISH DETAILS

SHEET NO. REVISIONS HNTB NORTH CAROLINA, P.C. License No. C-1554 S-37 NO. BY: DATE: DATE: BY: 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 TOTAL SHEETS **54** DRAWN BY R. BRICKER DATE 9/14
CHECKED BY P. BARBER DATE 9/14 DWG. NO. 18

ENGINEER FOR FINAL COLOR SELECTION.

	EPOXY-COATED REINFORCING STEEL																
SPAN A							S	PAN B					S	PAN C			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	38	5	STR.	22'-10"	905	A1	124	5	STR.	22′-10″	2,953	H A1	38	5	STR.	22′-10″	905
A2	12	5	STR.	25′-0″	313	A2	12	5	STR.	25′-0″	313	A2	12	5	STR.	25′-0″	313
A3	4	5	STR.	1'-6"	6	A3	4	5	STR.	1'-6"	6	A3	4	5	STR.	1'-6"	6
Α4	4	5	STR.	3′-9″	16	Α4	4	5	STR.	3′-9″	16	Δ4	4	5	STR.	3′-9″	16
A5	4	5	STR.	6′-0″	25	A5	4	5	STR.	6'-0"	25	A5	4	5	STR.	6'-0"	25
А6	4	5	STR.	8'-2"	34	А6	4	5	STR.	8'-2"	34	A6	4	5	STR.	8'-2"	34
Α7	4	5	STR.	10′-5″	43	Α7	4	5	STR.	10′-5″	43	A7	4	5	STR.	10′-5″	43
A8	4	5	STR.	12'-8"	53	А8	4	5	STR.	12'-8"	53	A8	4	5	STR.	12'-8"	53
А9	4	5	STR.	14'-10"	62	А9	4	5	STR.	14'-10"	62	A9	4	5	STR.	14'-10"	62
A10	4	5	STR.	17′-1″	71	A10	4	5	STR.	17'-1"	71	A10	4	5	STR.	17'-1"	71
A11	4	5	STR.	19'-4"	81	A11	4	5	STR.	19'-4"	81	A11	4	5	STR.	19'-4"	81
A12	4	5	STR.	21'-6"	90	A12	4	5	STR.	21′-6″	90	A12	4	5	STR.	21′-6″	90
B1	62	4	STR.	29'-2"	1,208	B2	124	4	STR.	30′-0″	2,485	B1	62	4	STR.	29′-2″	1,208
<b>∗</b> B5	64	5	STR.	8′-7″	573	B3	62	4	STR.	15′-6″	642	₩ B5	64	5	STR.	8′-7″	573
<b></b> ₩ B6	8	5	STR.	8'-8"	72	<b>★</b> B5	192	5	STR.	8′-7″	1,719	₩ B6	8	5	STR.	8'-8"	72
<b>∗</b> B7	8	5	STR.	8'-11"	74	<b>★</b> B6	16	5	STR.	8′-8″	145	₩ B7	8	5	STR.	8′-11″	74
<b></b> ★ B8	8	5	STR.	8'-1"	67	<b>★</b> B7	16	5	STR.	8′-11″	149	₩ B8	8	5	STR.	8'-1"	67
<b></b> ₩ B9	8	5	STR.	8'-4"	70	<b>★</b> B8	16	5	STR.	8′-1″	135	₩ B9	8	5	STR.	8'-4"	70
<b>★</b> B10	8	5	STR.	2'-0"	17	<b>★</b> B9	16	5	STR.	8'-4"	139	₩B10	8	5	STR.	2'-0"	17
<b>★</b> B11	8	5	STR.	1'-10"	15							₩ B11	8	5	STR.	1′-10″	15
<b>★</b> B12	8	5	STR.	2′-8″	22	* S1	288	5	1	4′-5″	1,327	₩B12	8	5	STR.	2′-8″	22
<b>★</b> B13	8	5	STR.	2′-5″	20	* S2	144	5	2	11'-4"	1,702	₩ B13	8	5	STR.	2′-5″	20
* S1	120	5	1	4′-5″	553							₩ S1	120	5	1	4′-5″	553
* S2	60	5	2	11'-4"	709							₩ S2	60	5	2	11'-4"	709
				TOTAL	5,099					TOTAL	12,190					TOTAL	5,099

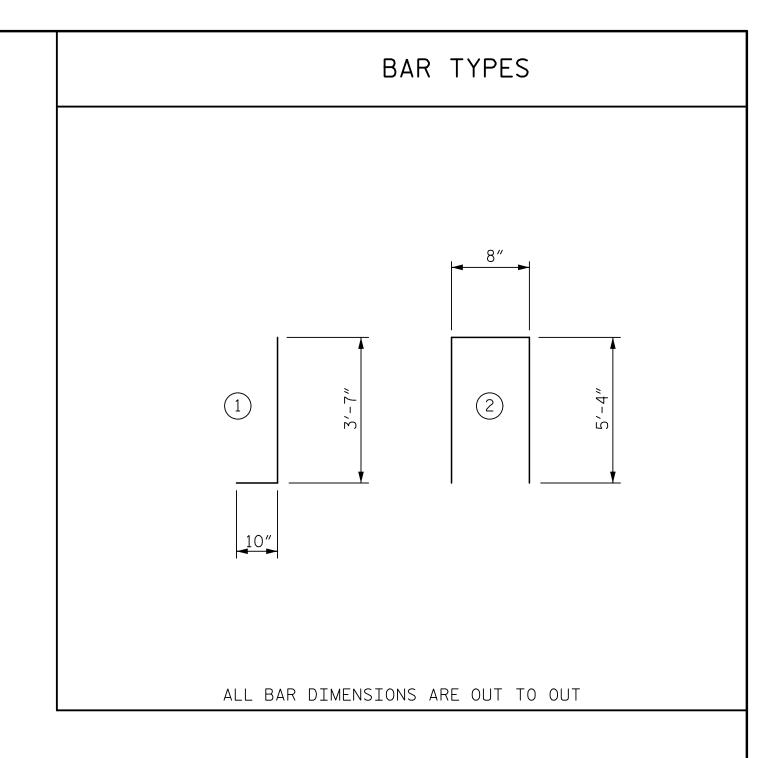
\* REINFORCING INCLUDED IN PARAPET TOTAL



LAYOUT FOR COMPUTING AREA

OF REINFORCED CONCRETE DECK SLAB

(SQ. FEET = 3,045.9)



QUANTITY BREAKDOWN BY SPAN								
	EPOXY COATED  REINFORCING STEEL  (LBS.)	CAST-IN CONCRETE ( (CU.)	(5,000 PSI) YDS.)					
SPAN "A"	5,099	DECK SLAB 22.7	PARAPETS 13.6					
SPAN "B"	12,190	49.4	33.1					
SPAN "C"	5,099	22.7	13.6					
TOTALS	22,388	94.8	60.3					

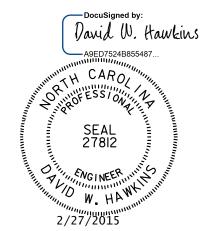
	TOTAL SUPERSTRUCTURE QUANTITIES							
	EPOXY COATED REINFORCING STEEL	CAST-IN-PLACE CONCRETE (5,000 PSI)						
	LBS.	CU. YDS.						
DECK SLAB	12,688	94.8						
PARAPET	9,700	60.3						
TOTALS	22,388	155.1						

PROJECT NO. P-4900A

ROBESON COUNTY

STATION: <u>POC 38+93.36 -L-</u>

MILE POST: 0.55



SEAL 12916 STATE OF NORTH CAROLINA

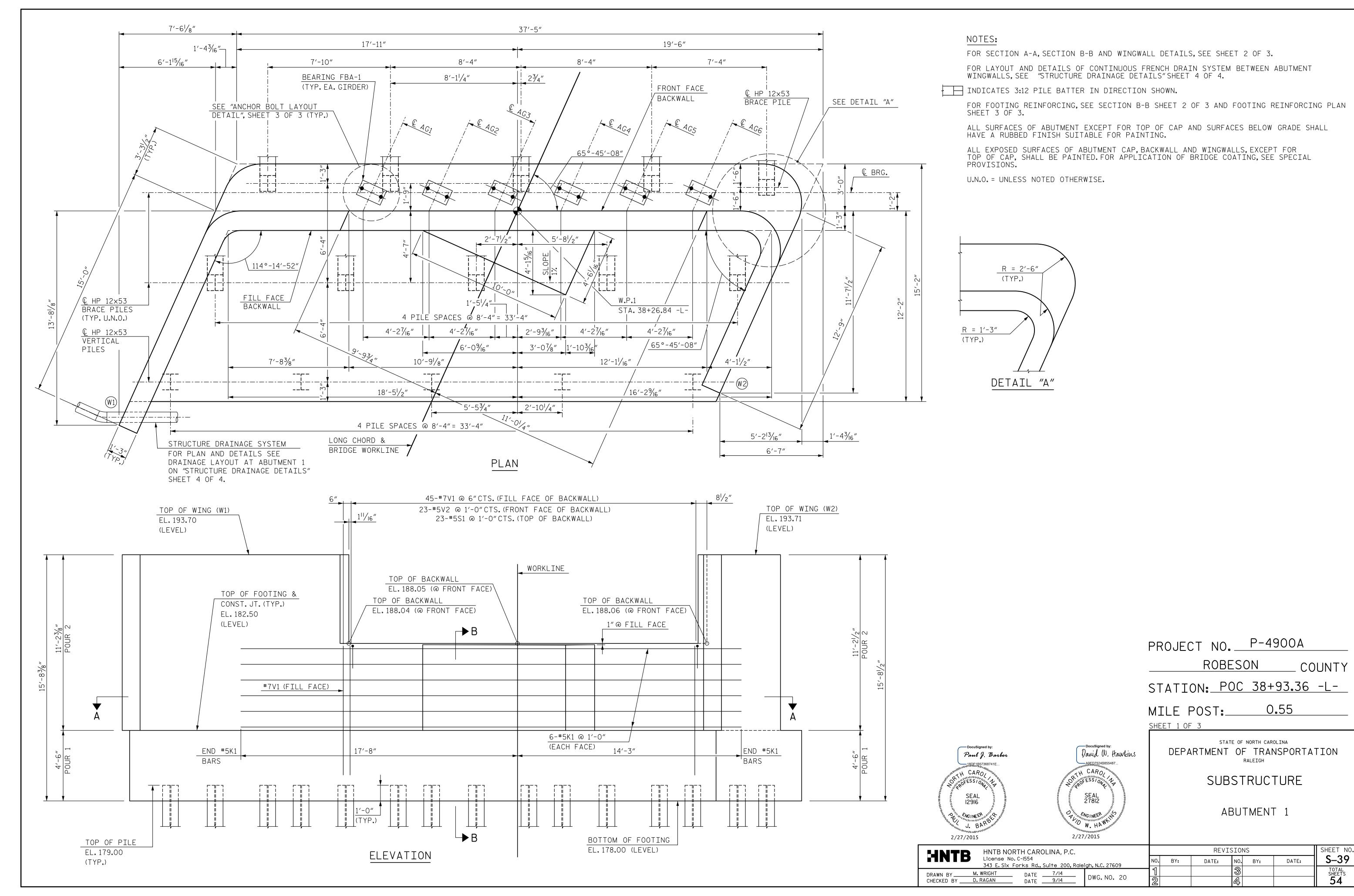
DEPARTMENT OF TRANSPORTATION

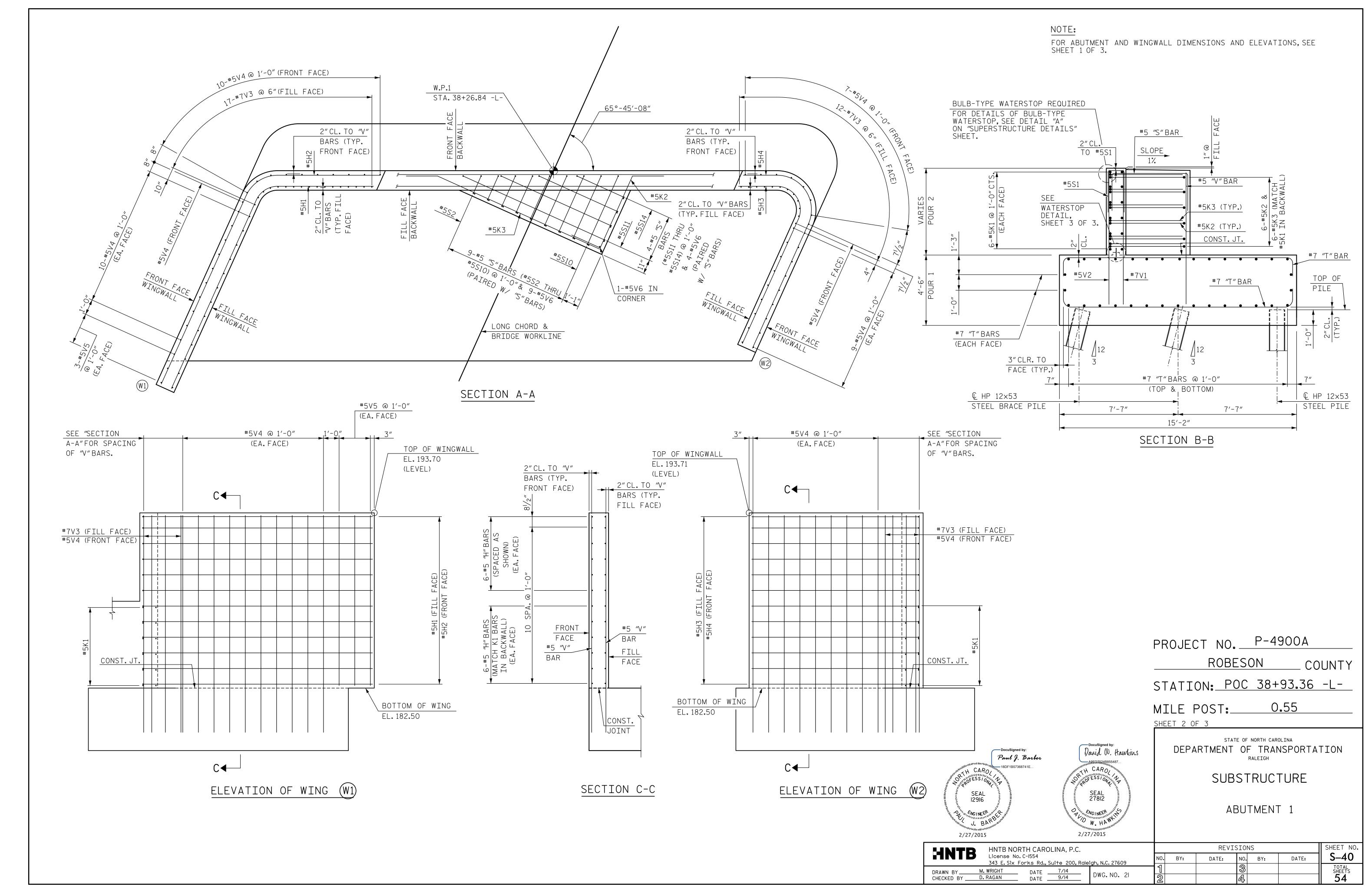
RALEIGH

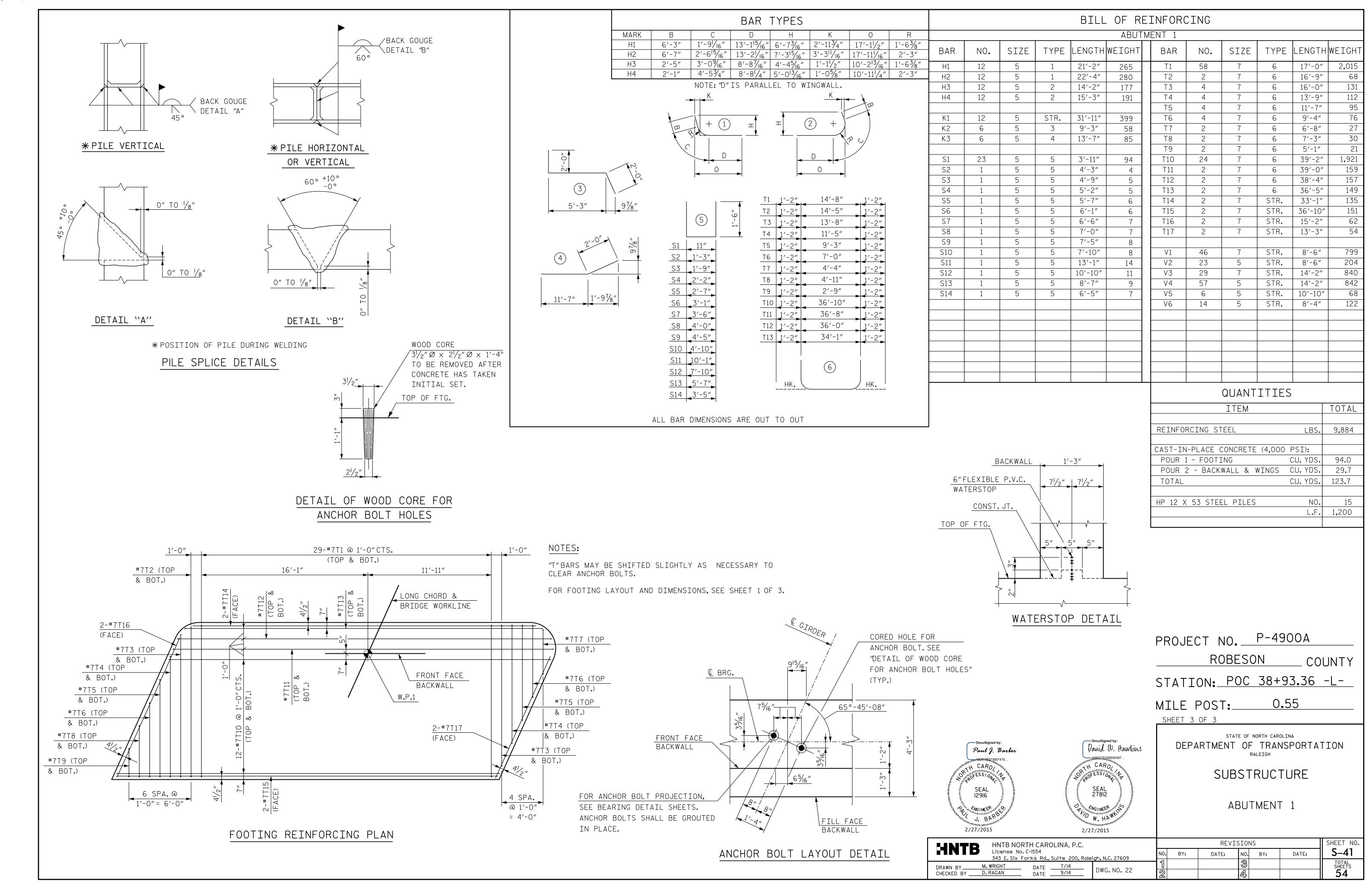
SUPERSTRUCTURE

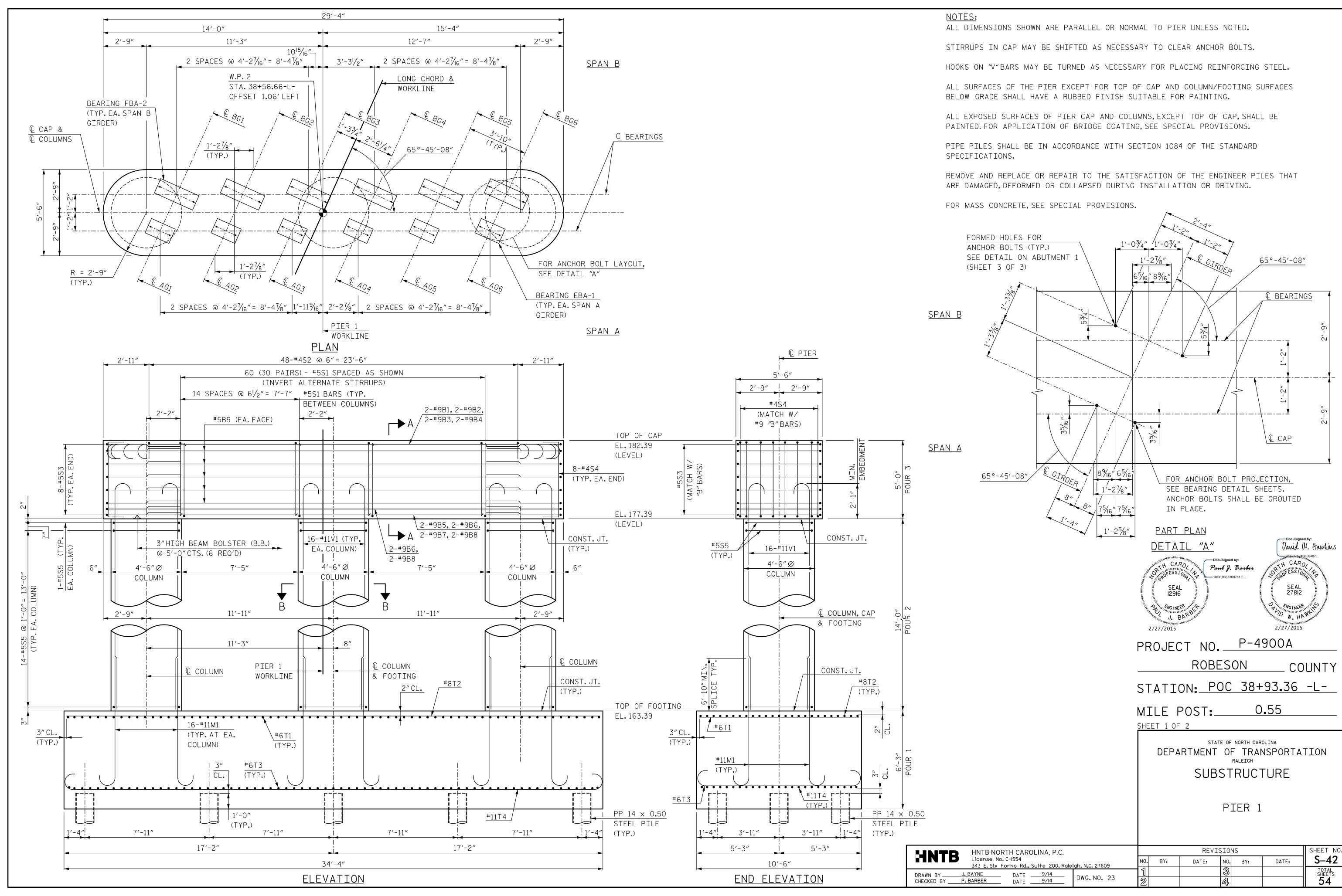
BILL OF MATERIAL

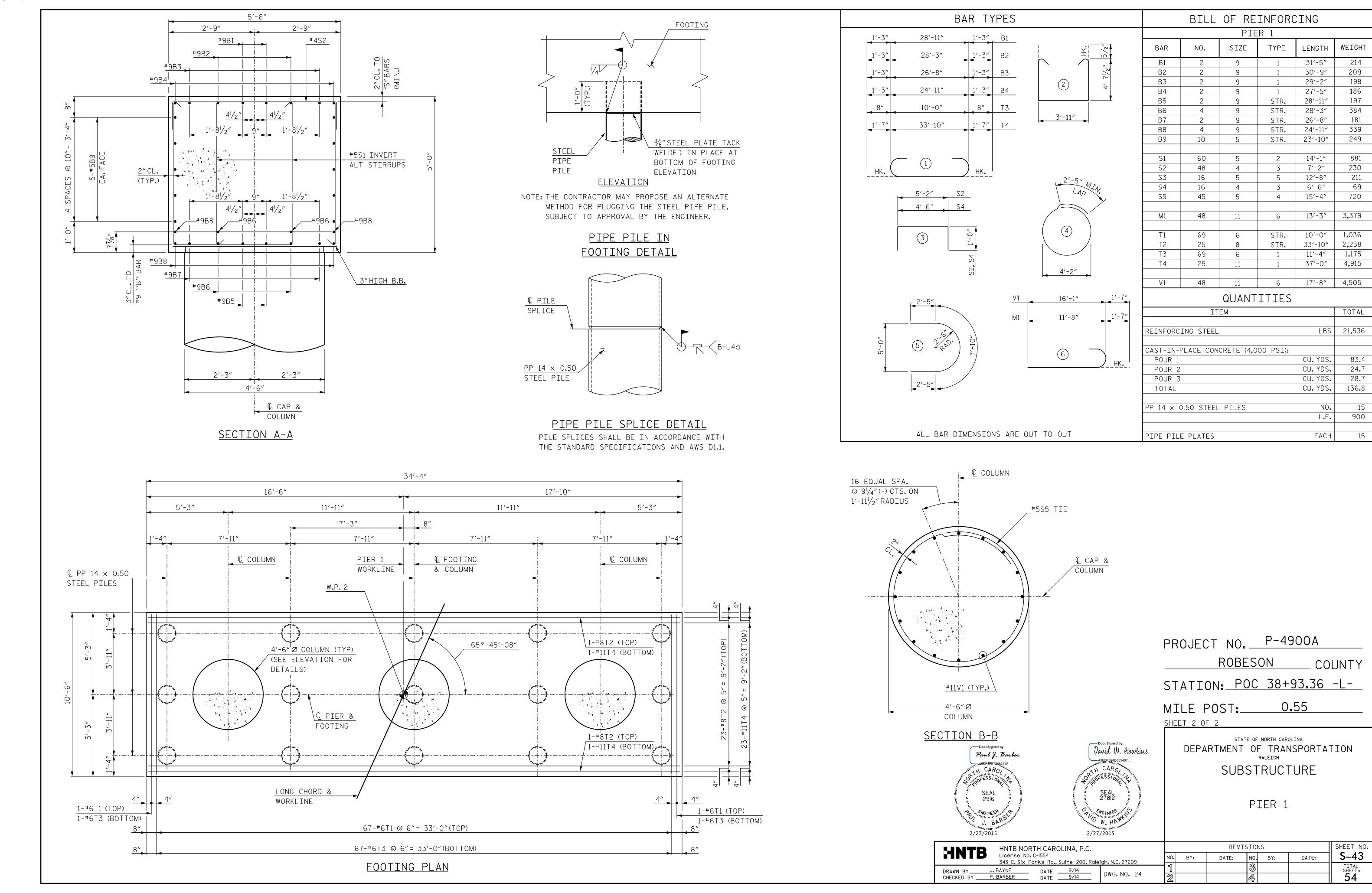
HNTB NORTH CAROLINA, P.C.		SHEET NO.		
License No. C-1554  343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO. BY:	DATE: NO. BY:	DATE:	S-38
N. WEIGHT	11	3		TOTAL SHEETS
DRAWN BY M. WRIGHT DATE 6/14 DWG. NO. 19	2			54

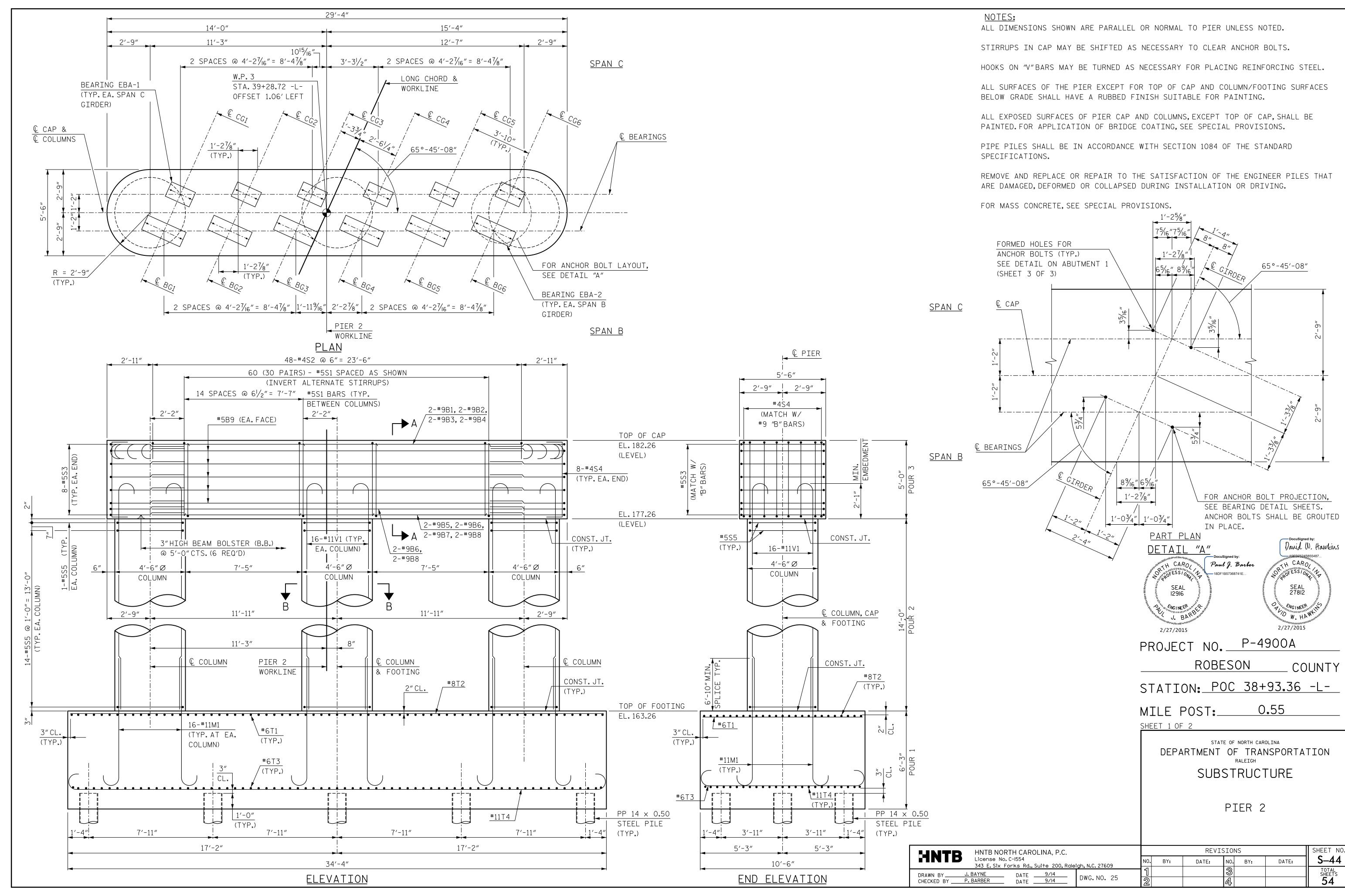


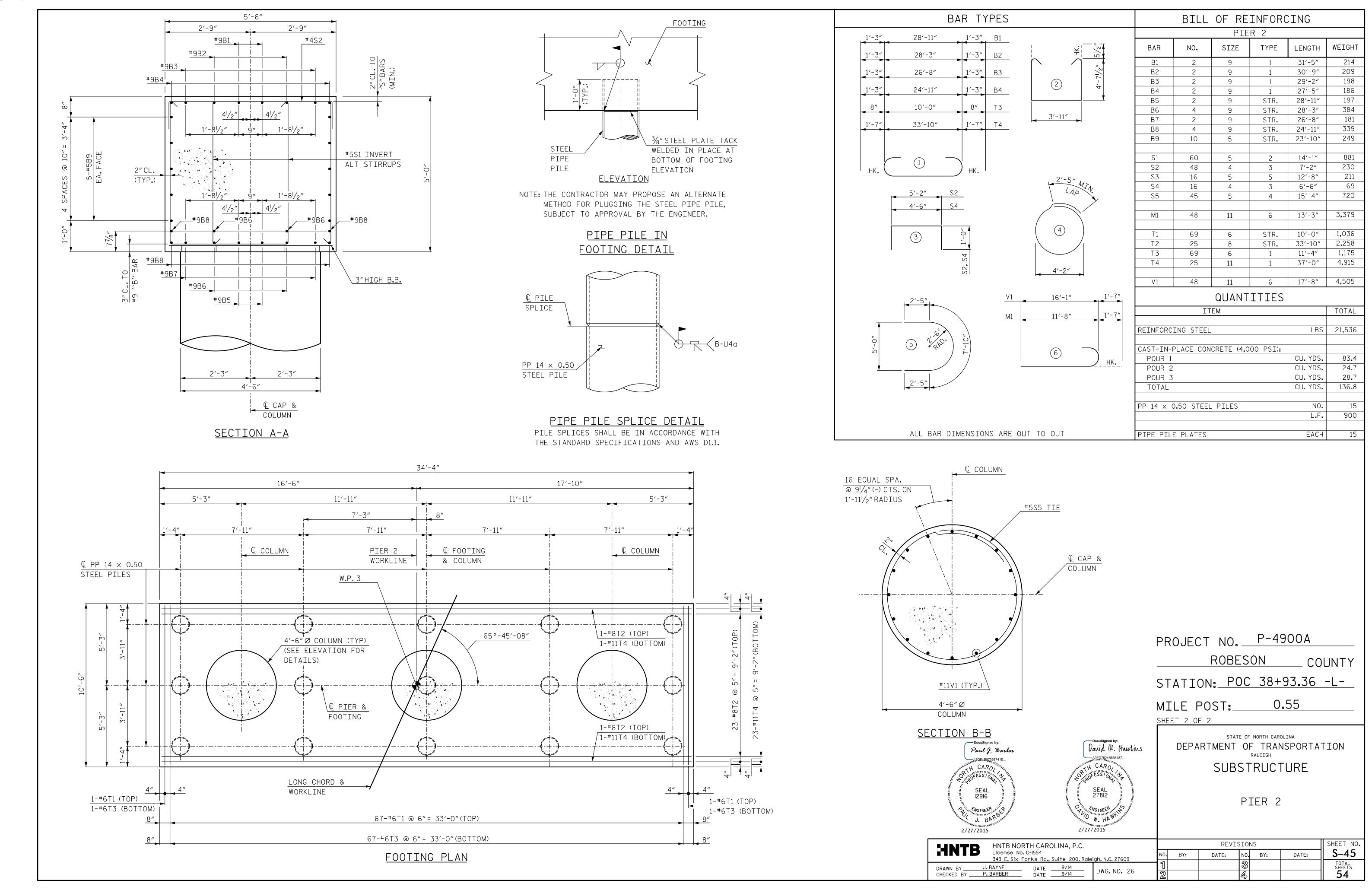


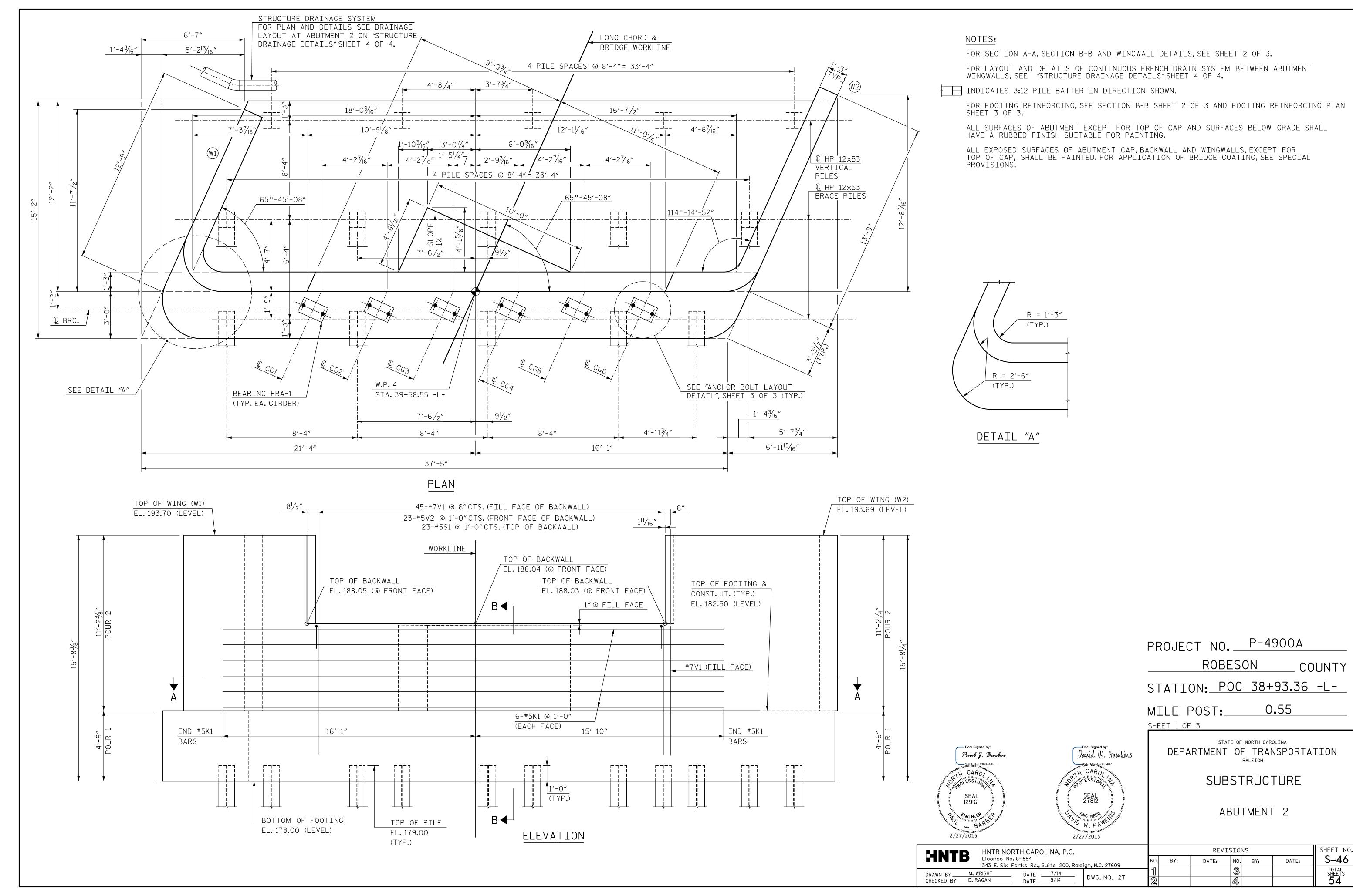


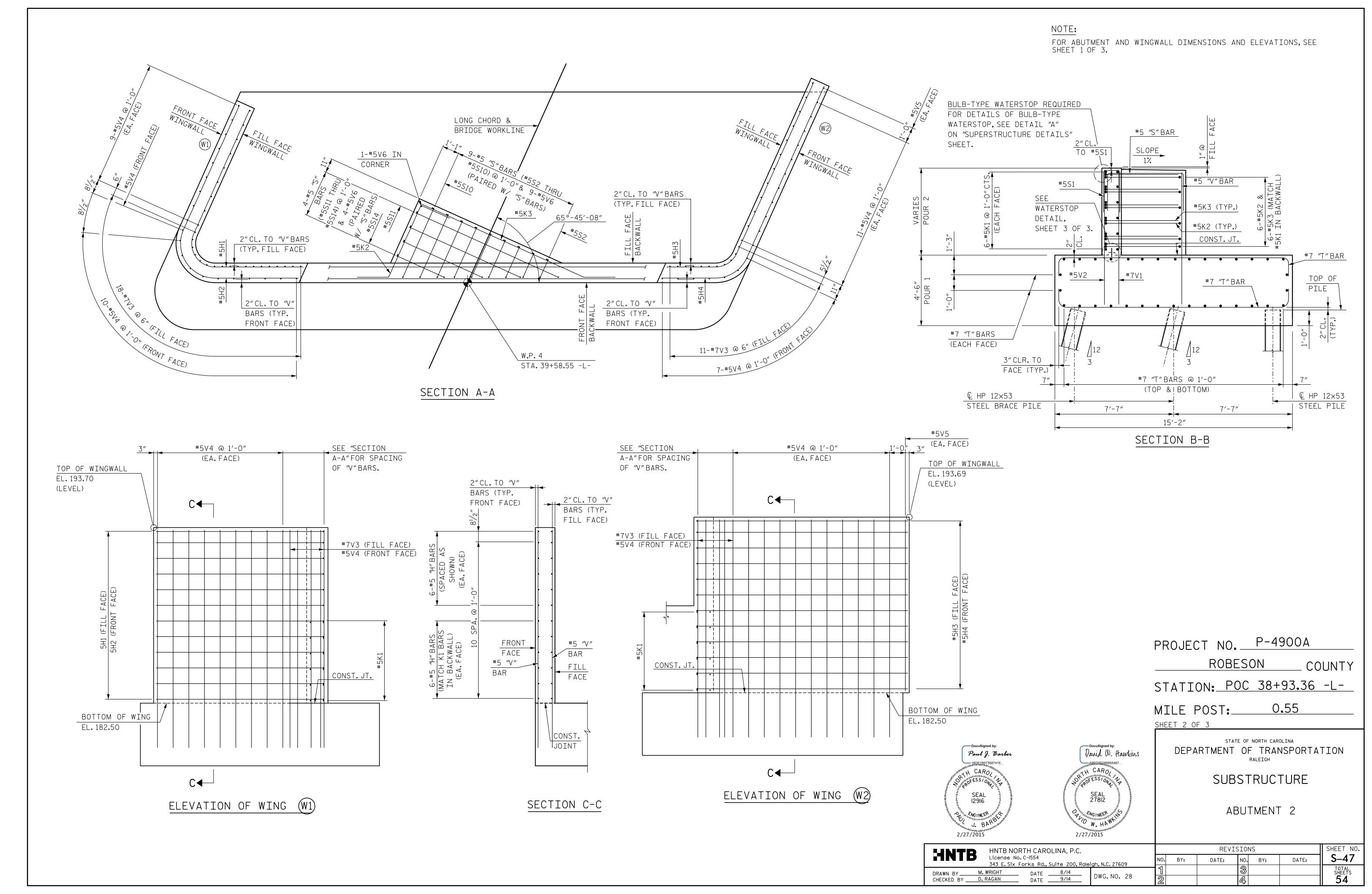


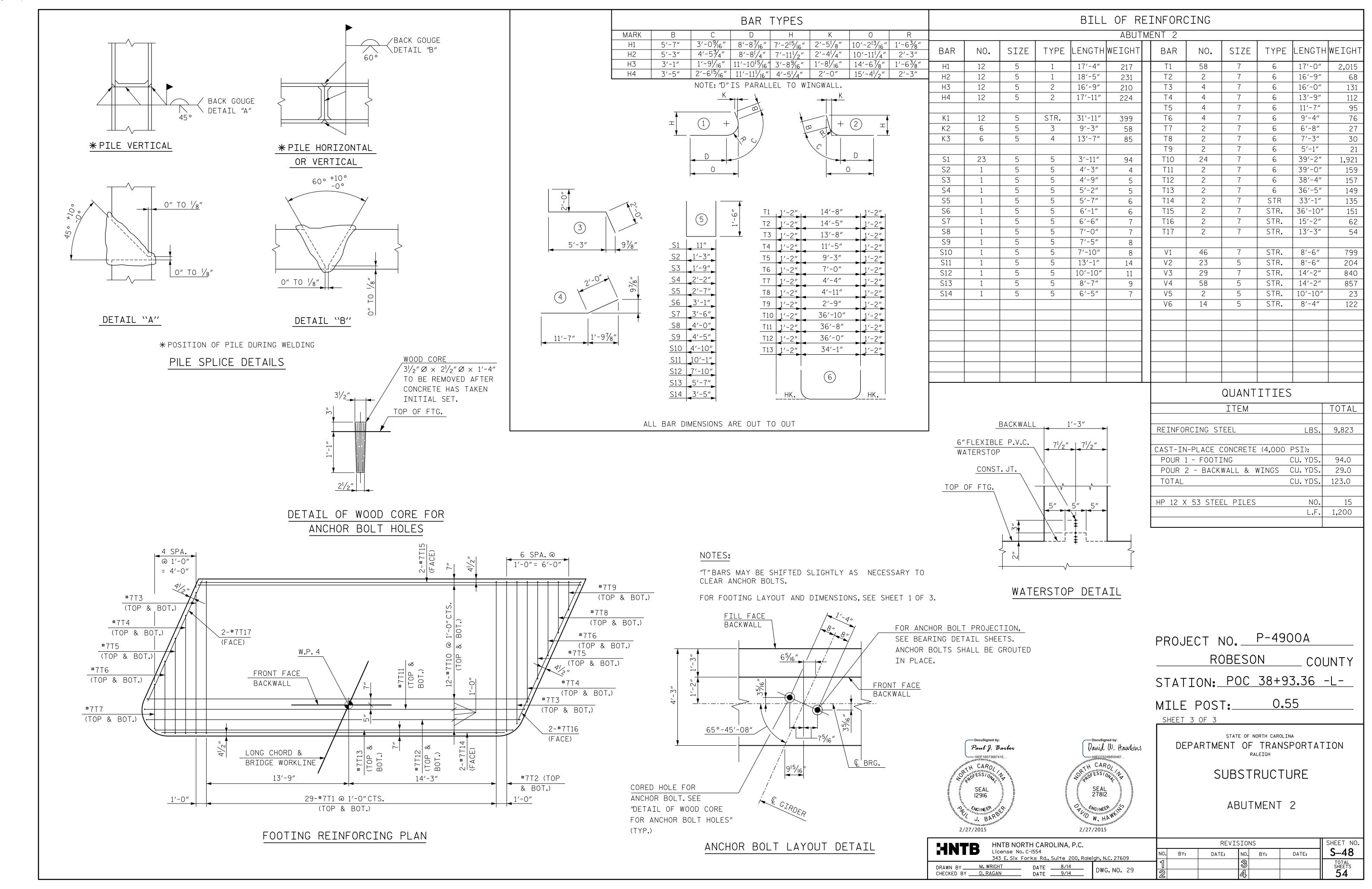


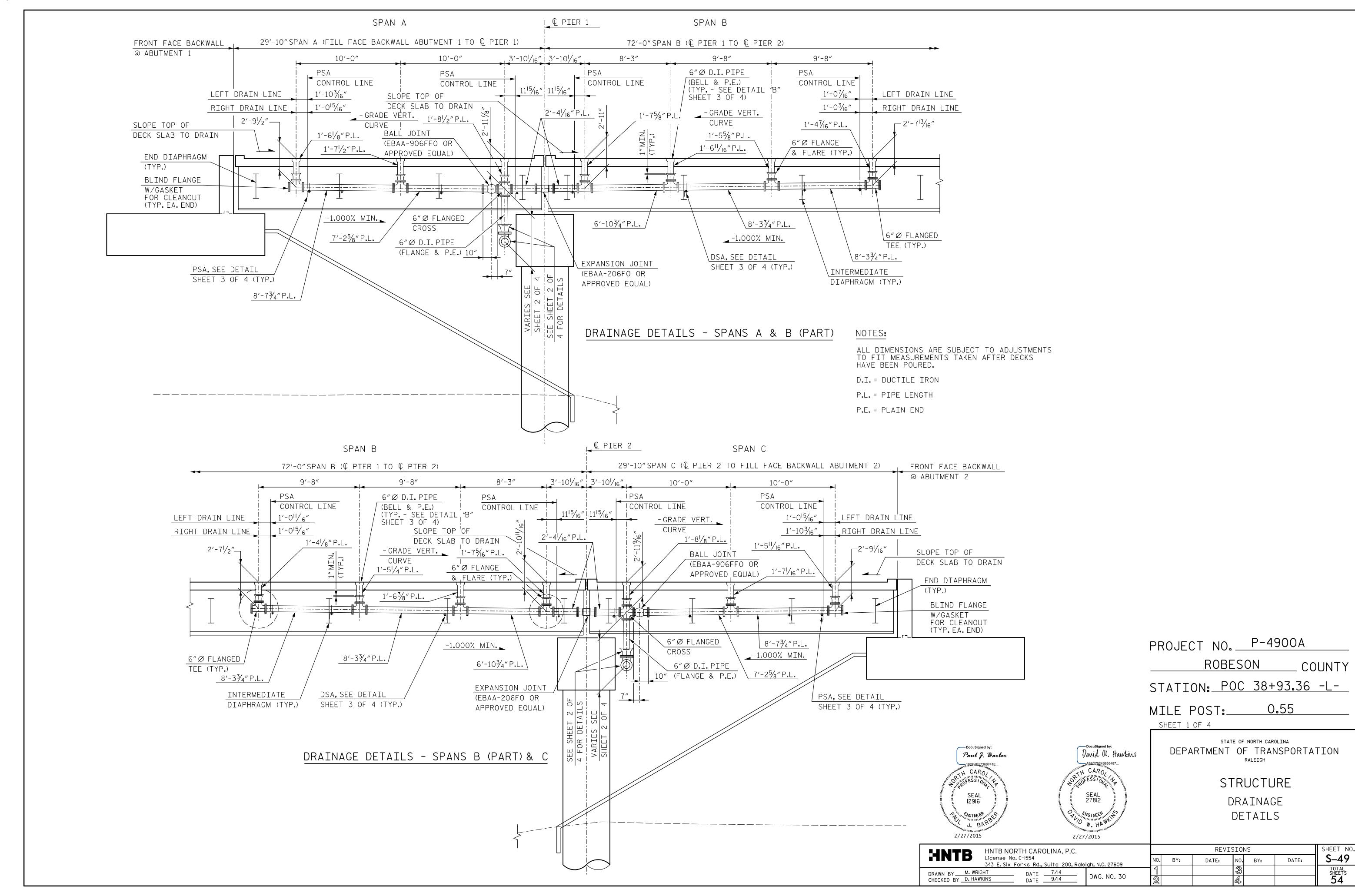


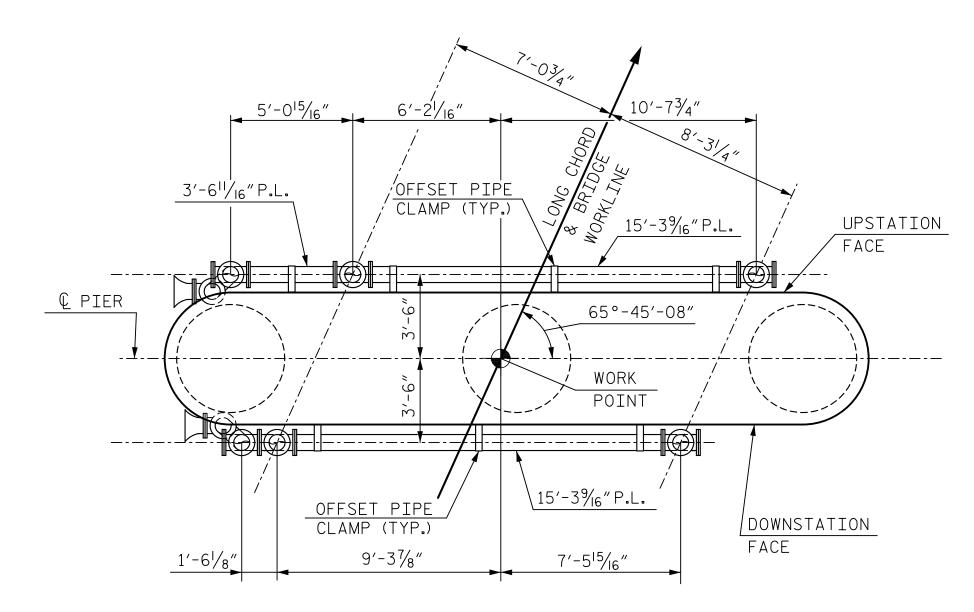






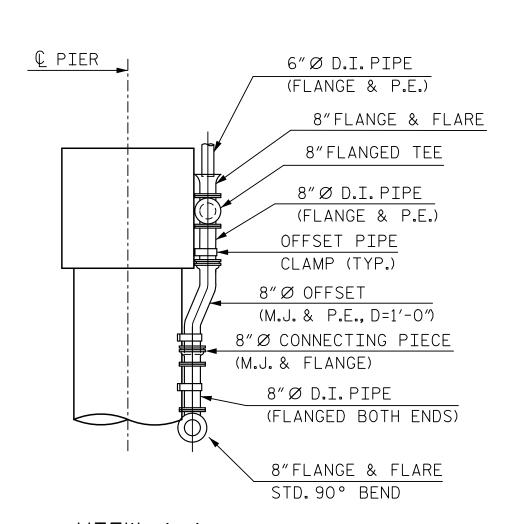




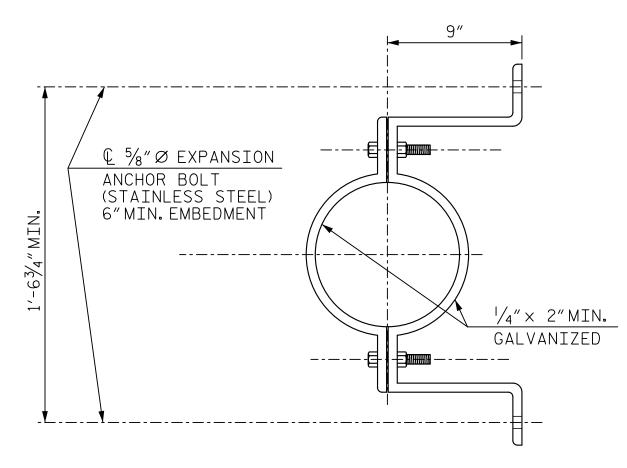


TYPICAL DRAINAGE LAYOUT DETAILS AT PIERS

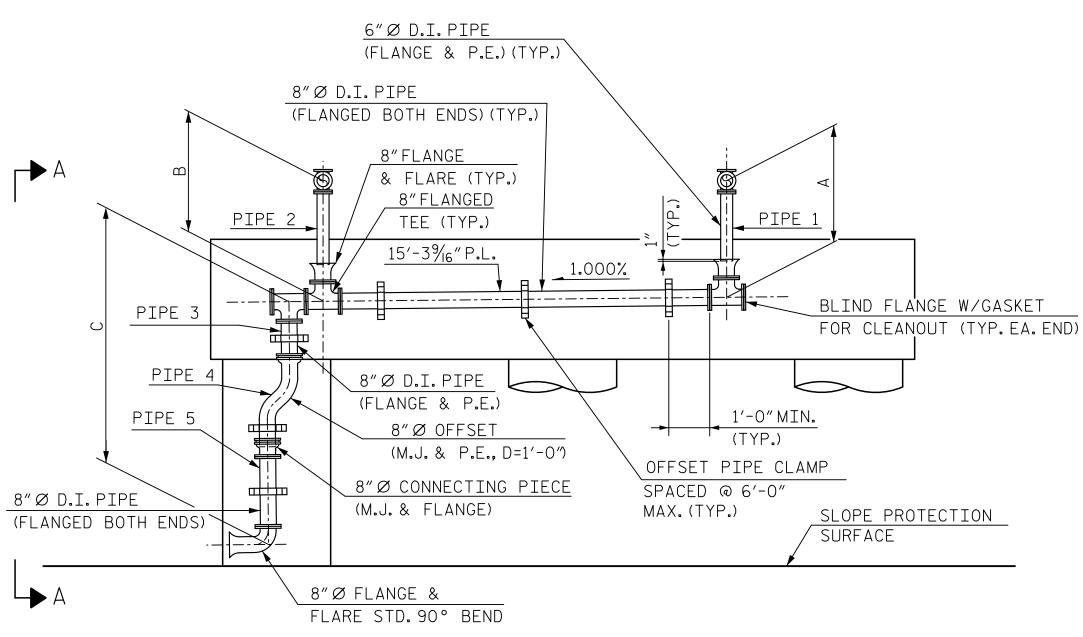
(DOWNSTATION FACE PIER 1 ONLY; UPSTATION FACE PIER 2 ONLY)



VIEW A-A (PIER 1 SHOWN, PIER 2 OPPOSITE HAND)



OFFSET PIPE CLAMP DETAIL



ELEVATION - PIER 1

BLIND FLANGE W/GASKET FOR CLEANOUT (TYP.EA.I	PIPE 1  1.000%  1.000%  1.000%  OFFSET PIPE CLAMP	15'-3%'6" P.L. 3'-6"/16" P.L. 8"Ø D.I. PIPE (FLANGE & P.E. 8"Ø OFFSET (M.J. & P.E., D=1'-0")	PIPE 2  PIPE 3  PIPE 4  PIPE 5	A <b>4</b>
SLOPE PROTECTION SURFACE		8"Ø CONNECTING PIE( (M.J. & FLANGE)  8"Ø FLANGE &	SE 8" Ø D.I. P. (FLANGED E	IPE BOTH ENDS)
	<u>EL</u> EVAT:	FLARE STD. 90  ION - PIER 2	, RFIND	A <b>↓</b>

DIMENSIONS									
LOCATION	А	В	С						
PIER 1, SPAN A	4'-113/8"	5′-11/2″	8'-11 1/2"						
PIER 2, SPAN C	5′-1 <sup>1</sup> /8″	5′-3 <sup>l</sup> / <sub>4</sub> ″	8′-7 <sup> </sup> / <sub>2</sub> ″						

PIPE LENGTHS								
LOCATION	1	2	3	4	5			
PIER 1, SPAN A	2'-91/8"	2'-111/4"	1'-6"	3′-0″	2'-31/8			
PIER 2, SPAN C	2'-10 1/8"	3′-1″	1′-6″	3′-0″	1'-11 1/8			

NOTES: ALL DIMENSIONS ARE SUBJECT TO ADJUSTMENTS TO FIT MEASUREMENTS TAKEN AFTER DECKS

D.I. = DUCTILE IRON

HAVE BEEN POURED.

P.L. = PIPE LENGTH

P.E. = PLAIN END

M.J. = MECHANICAL JOINT



Paul J. Barber

SEAL 12916

David W. Hawkins

PROJECT NO. P-4900A ROBESON \_ COUNTY STATION: POC 38+93.36 -L-

0.55 MILE POST:\_\_\_\_

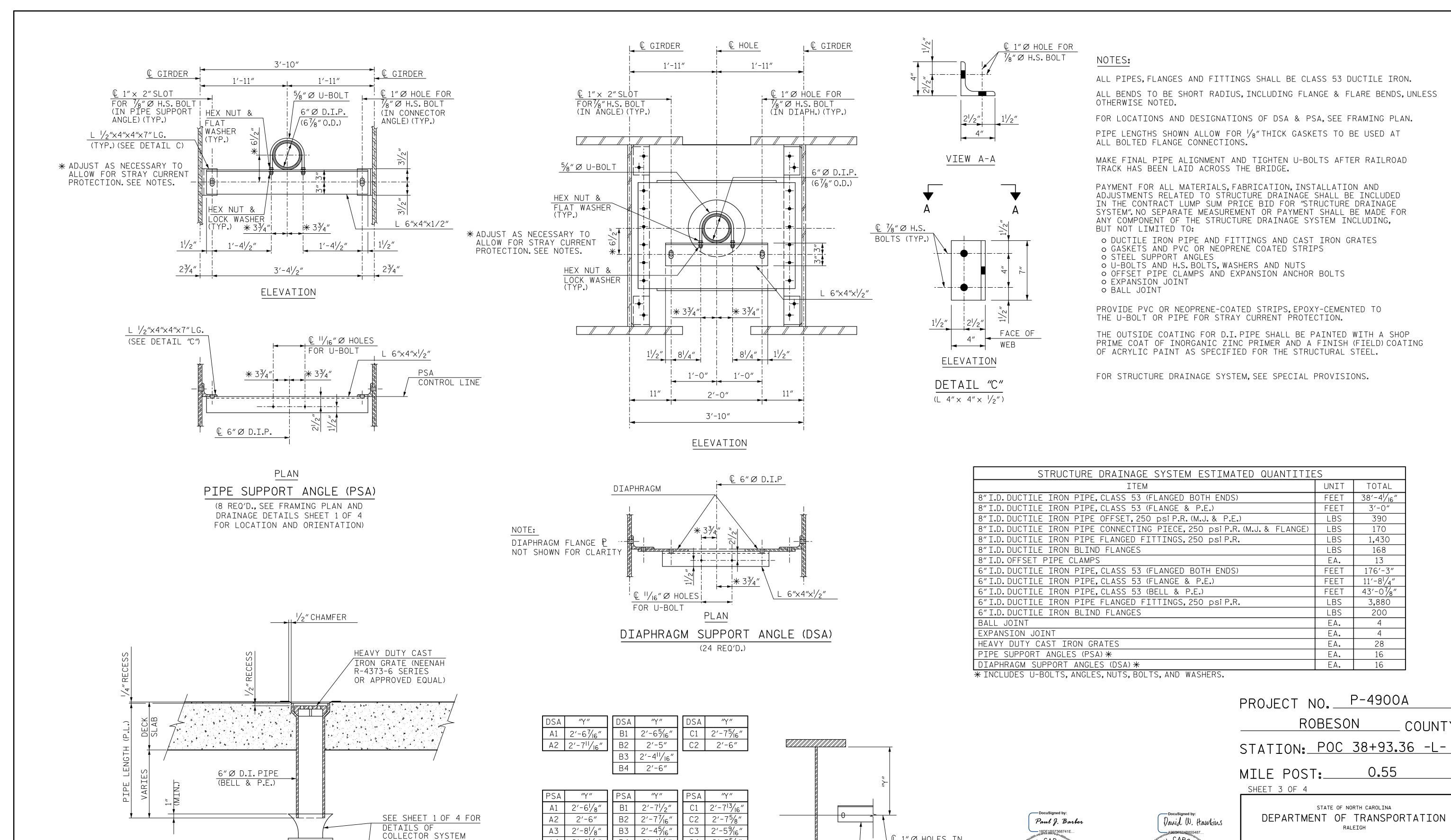
SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STRUCTURE

DRAINAGE DETAILS

2/27/2015	2/27/2015							
HNTB	HNTB NORTH CAROLINA, P.C.			REV	OIZI	15		SHEET NO.
	License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO.	BY:	DATE:	NO.	BY:	DATE:	S-50
DRAWN BYM. WRIG	CHT DATE 7/14	1			3			TOTAL SHEETS
CHECKED BY D. HAWK	KINS DATE 9/14 DWG. NO. 31	匆						I 54



A4 2'-8 /8"

DETAIL "B"

B4 2'-4<sup>1</sup>/<sub>4</sub>"

2'-4"

2'-4"

2'-71/8" 2'-7<sup>3</sup>/<sub>16</sub>" C4 2'-5<sup>5</sup>/<sub>8</sub>"

DETAIL "A"

HNTB NORTH CAROLINA, P.C. License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 REVISIONS SHEET NO. S-51 NO. BY: DATE: BY: DATE: TOTAL SHEETS **54** DRAWN BY M. WRIGHT
CHECKED BY P. BARBER DATE 7/14 DATE 9/14 DWG. NO. 32

SEAL 27812

1"Ø HOLES IN

DIAPH.WEB POR

CONNECTOR ANGLE

ANGLE

CARO

12916

TOTAL

 $38'-4\frac{1}{16}''$ 

3'-0"

390

170

1,430

168

176′-3″

11'-8|/4"

43'-01/8'

3,880

200

28

16

0.55

STRUCTURE

DRAINAGE

DETAILS

COUNTY

UNIT

FEET

FEET

LBS

LBS

LBS

LBS

EA.

FEET

FEET

FEET

LBS

LBS

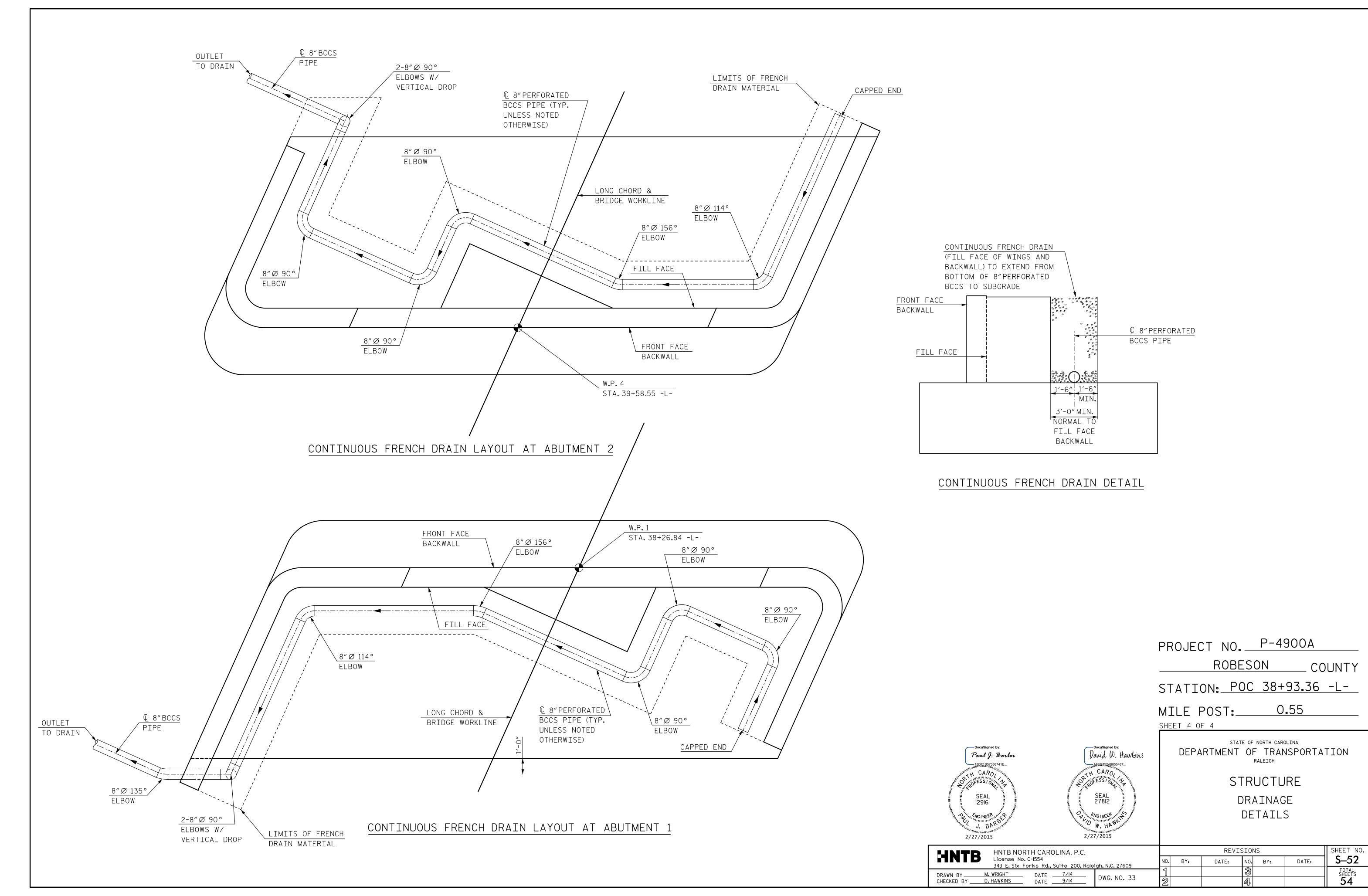
EA.

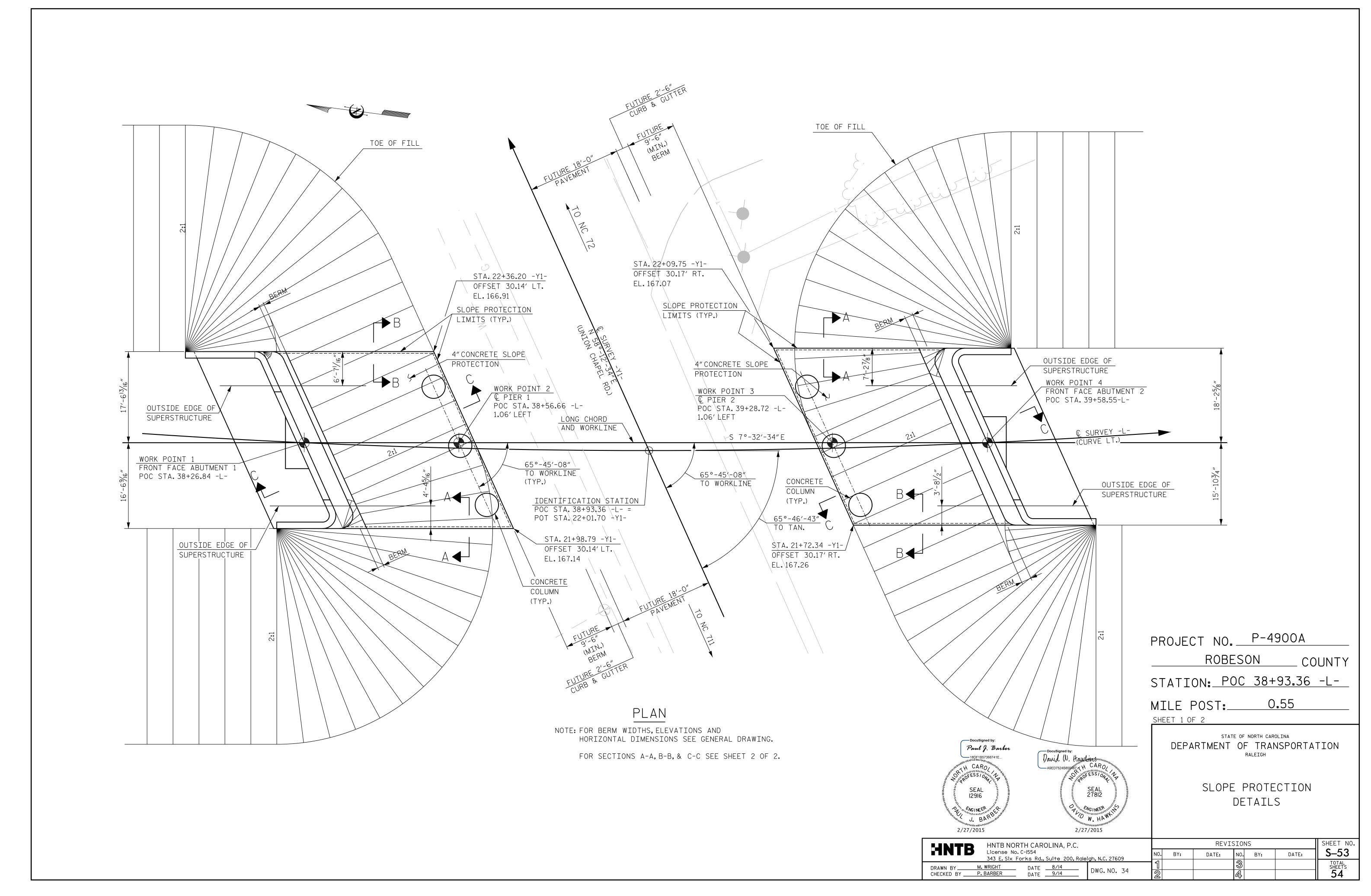
EA.

EA.

EA.

EA.





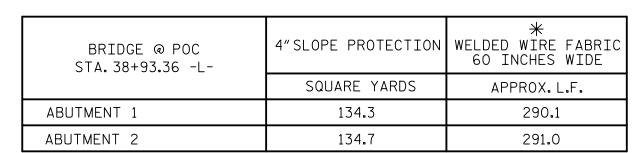
ASSEMBLED BY : MEW CHECKED BY : PJB

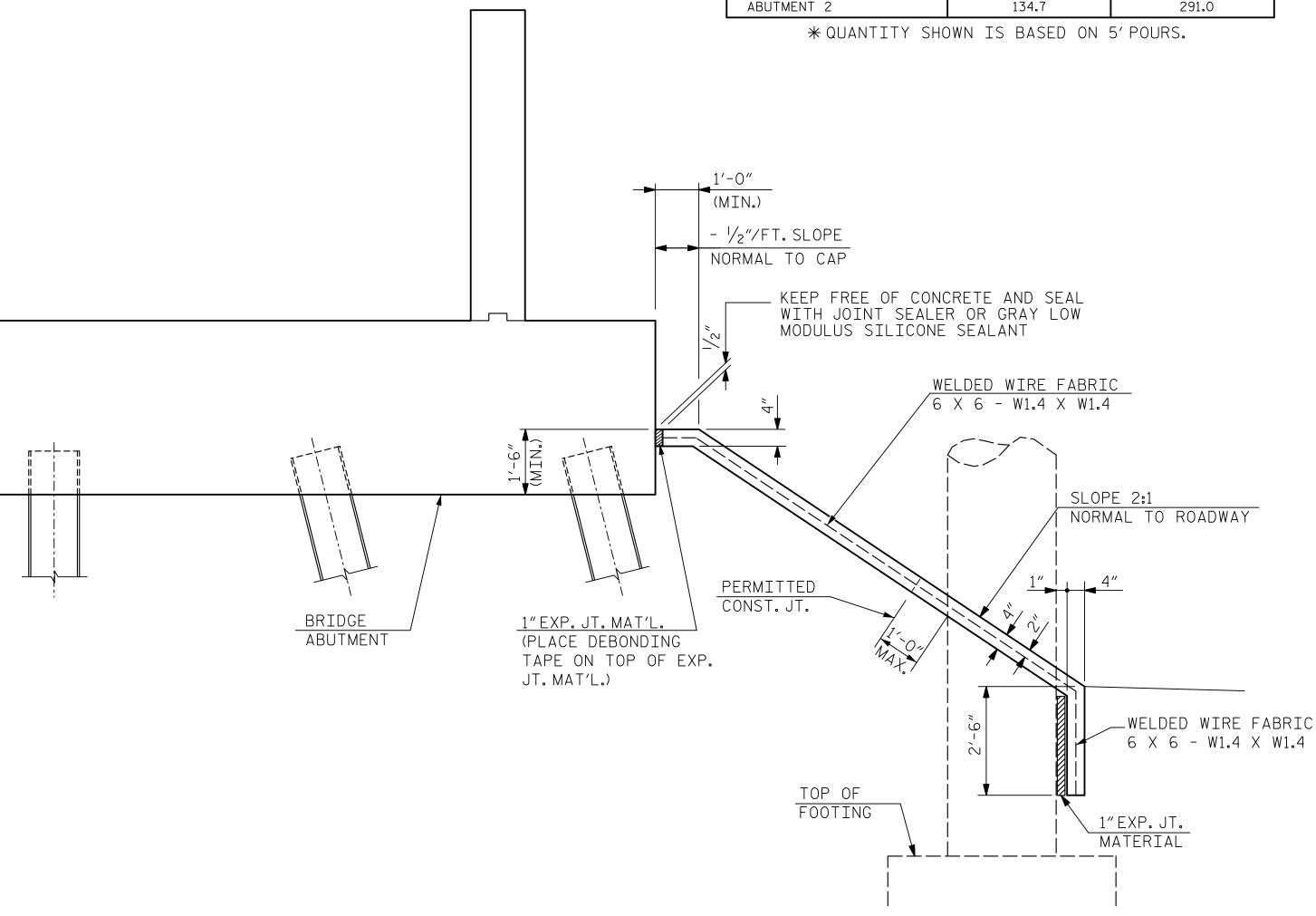
DRAWN BY: ELR 5/92 CHECKED BY: GRP 6/92

DATE : 7/14 DATE : 9/14

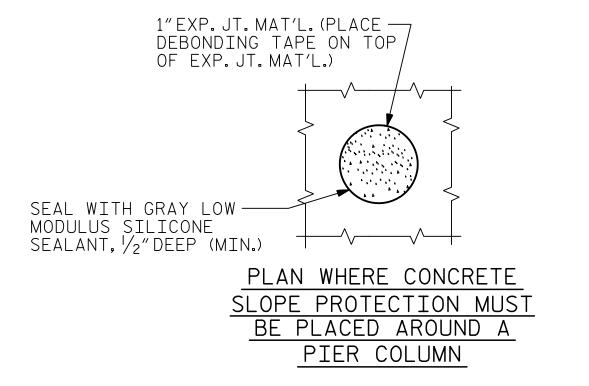
TLA/GM MAA/GM MAA/GM

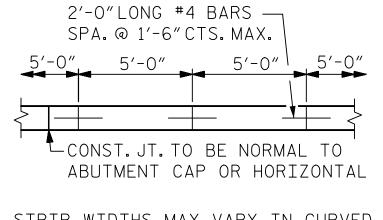
REV. 5/I/06 REV. I0/I/II REV. I2/2I/II



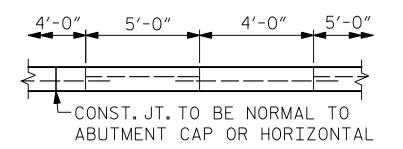


# SECTION C-C





STRIP WIDTHS MAY VARY IN CURVED PORTION. POURING DETAIL

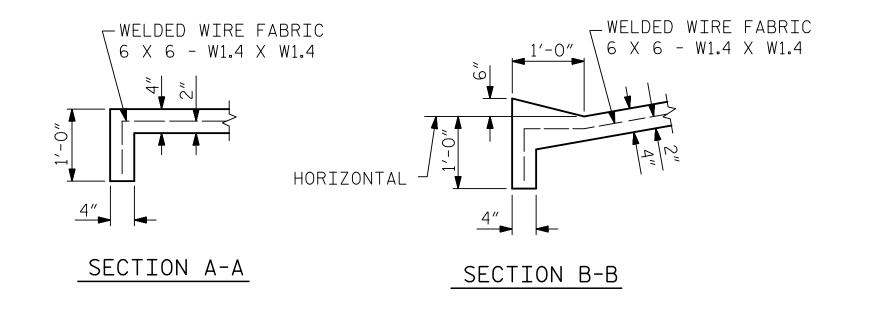


POUR A 4'-0" STRIP FIRST. STRIP WIDTHS MAY VARY IN CURVED PORTION. OPTIONAL POURING DETAIL

#### GENERAL NOTES

SLOPE PROTECTION SHALL BE PLACED UNDER THE ENDS OF THE BRIDGE AS SHOWN IN THE DETAILS. MEASUREMENT AND PAYMENT SHALL BE AS PRESCRIBED IN SECTION 462 OF THE STANDARD SPECIFICATIONS. FOR BERM WIDTH, SEE GENERAL DRAWING.

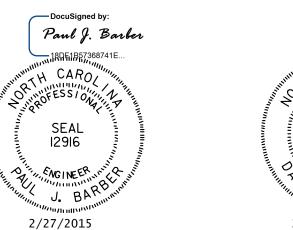
SLOPE PROTECTION SHALL CONSIST OF 4" POURED-IN-PLACE CONCRETE PAVING AS SHOWN IN THE DETAILS ON THIS SHEET. CONCRETE SHALL BE CLASS "B". THE CONCRETE SURFACE SHALL BE FLOATED WITH A WOODEN FLOAT AND FINISHED. WELDED WIRE FABRIC REINFORCING SHALL BE 6 X 6 - W1.4 X W1.4, 60" WIDE. SLOPE PROTECTION SHALL BE POURED IN 5'STRIPS AS SHOWN IN THE "POURING DETAIL" WITH 2'-0"LONG #4 BARS PLACED ALONG THE SLOPE BETWEEN STRIPS AT 1'-6" MAXIMUM SPACING. SLOPE PROTECTION MAY BE POURED IN ALTERNATE 4'AND 5'STRIPS AS SHOWN IN THE "OPTIONAL POURING DETAIL" WITH ADJACENT RUNS OF WELDED WIRE FABRIC LAPPING AT LEAST 6". THE COST OF THE WELDED WIRE FABRIC AND #4 BARS, IF USED, SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID PER SQUARE YARD FOR SLOPE PROTECTION.

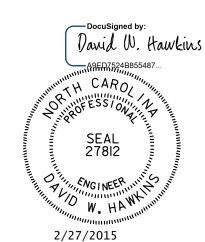


PROJECT NO. P-4900A ROBESON COUNTY STATION: POC 38+93.36 -L-

0.55 MILE POST: \_\_\_\_

SHEET 2 OF 2





DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

STATE OF NORTH CAROLINA

SLOPE PROTECTION DETAILS

HNTB NORTH CAROLINA, P.C.	REVISIONS	SHEET
HNTB NORTH CAROLINA, P.C. License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609	NO. BY: DATE: NO. BY:	DATE: <b>S-5</b> 4
DRAWN BY M. WRIGHT DATE 7/14 DWG. NO. 35	1 2 4	TOTAL SHEETS