

NOTE: SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 41141.1.1 F.A. PROJ. N/A

COUNTY MECKLENBURG

PROJECT DESCRIPTION SUGAR CREEK ROAD AND NCRR/NSRR

CROSSING NO. 715 352H GRADE SEPARATION

RECOMMENDATIONS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5008	1	36
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
WBS 41141.1.1		P.E.	
WBS 41141.2.2		R/W & UTIL.	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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LINE	STATION	PLAN	PROFILE	XSECT
-L-	10+00 - 40+50	4-6	9	14-18
-Y1-	10+00 - 12+71.42	4	-	19-20
-Y2-	10+00 - 14+55.24	4	-	-
-Y3-	10+00 - 19+67.74	7, 8	10	21-25
-Y3REV-	10+00 - 19+52.92	4, 7	-	-
-Y4-	10+00 - 17+73.87	5, 7	11	26-31
-Y5-	10+00 - 19+09.76	5-6	12	32-33
-Y6-	10+14.86 - 12+50	8	-	-
-Y7-	10+00 - 19+92.03	6	13	34-35
-Y8-	10+25 - 14+83.09	6	-	-
SOIL TEST RESULTS				36

ID: U-5008

CONTRACT:

DRAWN BY: R. Rahie

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

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S. Gower

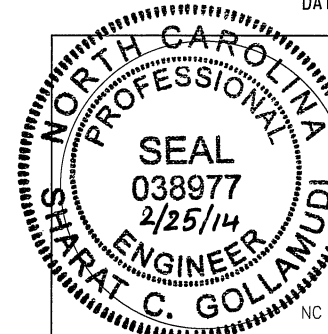
W. Trapp

INVESTIGATED BY AMEC E&I, Inc.

CHECKED BY S. Johnson

SUBMITTED BY S. Gollamudi

DATE February, 2014



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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

PROJECT REFERENCE NO. U-5008	SHEET NO. 2
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SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1, A-3, A-2, A-4, A-5, A-7, A-1, A-2, A-4, A-5, A-6, A-7 SYMBOL	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 PERCENTAGE OF MATERIAL ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	WEATHERING FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i> SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF</i> VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF</i> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	TERMS AND DEFINITIONS ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. 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TEXTURE OR GRAIN SIZE U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053 BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE. SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.) GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3	MISCELLANEOUS SYMBOLS 	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.	TERMS AND DEFINITIONS SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILT - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN REPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
CONSISTENCY OR DENSENESS PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²) GENERALLY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE <4 4 TO 10 10 TO 30 30 TO 50 >50 GENERALLY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD <2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 >30 <0.25 0.25 TO 1.0 1 TO 2 2 TO 4 >4	ABBREVIATIONS AR - AUGER REFUSAL MED. - MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA - MICACEOUS WEA. - WEATHERED CL. - CLAY MOD. - MODERATELY UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC DR - DRY UNIT WEIGHT CSE. - COARSE ORG. - ORGANIC SAMPLE ABBREVIATIONS DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST S - BULK DPT - DYNAMIC PENETRATION TEST SAP. - SAPROLITIC SS - SPLIT SPOON e - VOID RATIO SD. - SAND, SANDY ST - SHELBY TUBE F - FINE SL. - SILT, SILTY RS - ROCK FOSS. - FOSSILIFEROUS TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAC. - FRACTURED, FRACTURES TCR - TRICONE REFUSAL CBR - CALIFORNIA BEARING RATIO HI. - HIGHLY w - MOISTURE CONTENT	ROCK HARDNESS VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.	TERMS AND DEFINITIONS BENCH MARK: BORING ELEVATIONS OBTAINED FROM TIN FILE. ELEVATION: FT.
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT USUALLY LIQUID; VERY WET, USUALLY SATURATED - (SAT.) PL - PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM - OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: <input checked="" type="checkbox"/> MOBILE B-57 <input type="checkbox"/> CME-55 <input type="checkbox"/> CME-45C <input type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE HOIST ADVANCING TOOLS: <input type="checkbox"/> CLAY BITS <input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> TUNG-CARBIDE INSERTS <input checked="" type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER <input checked="" type="checkbox"/> TRICONE 2 7/8" STEEL TEETH <input type="checkbox"/> TRICONE * TUNG-CARB. <input checked="" type="checkbox"/> CORE BIT <input checked="" type="checkbox"/> 2 1/2" HSA HAMMER TYPE: <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL CORE SIZE: <input type="checkbox"/> B <input checked="" type="checkbox"/> N-Q <input type="checkbox"/> H HAND TOOLS: <input type="checkbox"/> POST HOLE DIGGER <input type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST	FRACTURE SPACING TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET BEDDING TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET INDURATION FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	TERMS AND DEFINITIONS BENCH MARK: BORING ELEVATIONS OBTAINED FROM TIN FILE. ELEVATION: FT. NOTES: FIAD - FILLED IMMEDIATELY AFTER DRILLING BT - BORING TERMINATED <input checked="" type="checkbox"/> UNDERCUT EXCAVATION <input checked="" type="checkbox"/> UNSUITABLE UNCLASSIFIED EXCAVATION
PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM HIGH PLASTICITY 26 OR MORE HIGH			
COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			

See Sheet 1-A For Index of Sheets

STATE OF NORTH CAROLINA
RAIL DIVISION



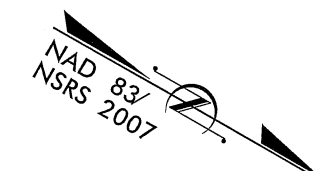
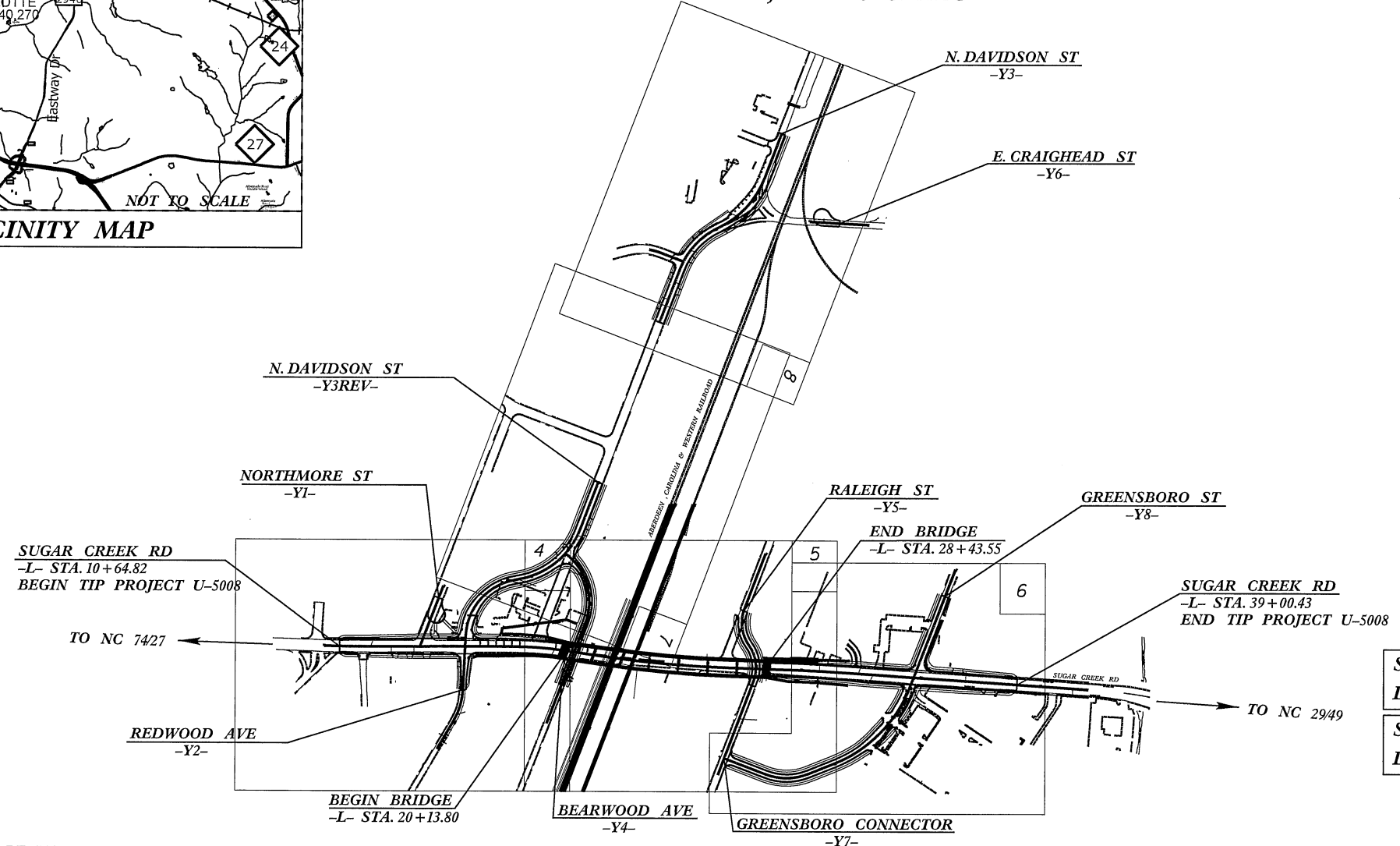
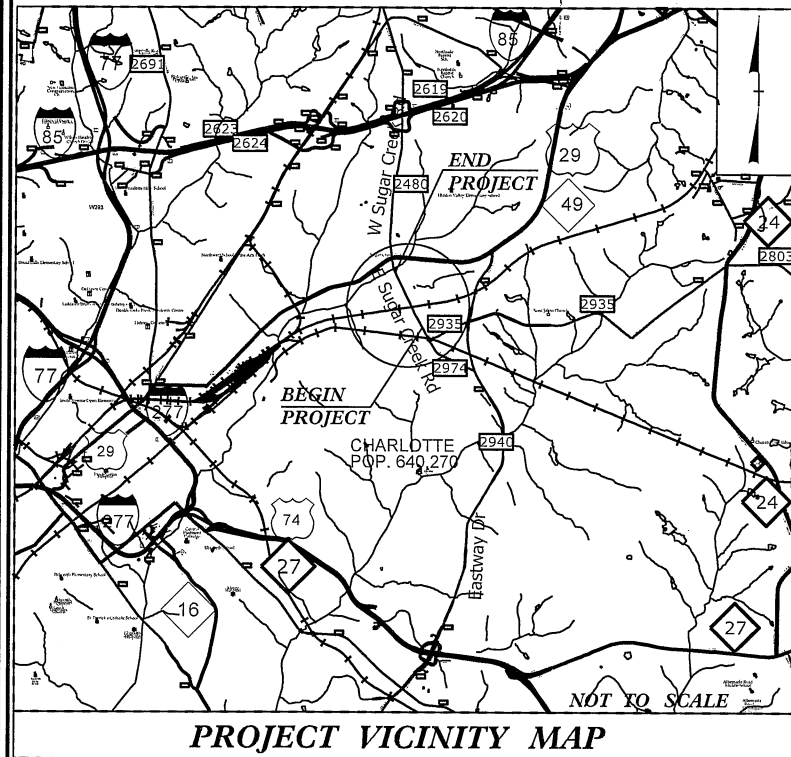
STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	U-5008	3	36
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
WBS 41141.1.1		PE	
WBS 41141.2.2		ROW/UTIL	



MECKLENBURG COUNTY

LOCATION: SUGAR CREEK ROAD AND NCRR/NSRR
CROSSING NO. 715 352H GRADE SEPARATION

TYPE OF WORK: GRADING, DRAINAGE, PAVING, STRUCTURE, CURB & GUTTER,
SIGNALS, AND SIGNING



SUBMITTAL: 25% PRELIMINARY PLANS
DATE: AUGUST 27, 2012

SUBMITTAL: 25% PRELIMINARY PLANS
DATE: JUNE 8, 2012

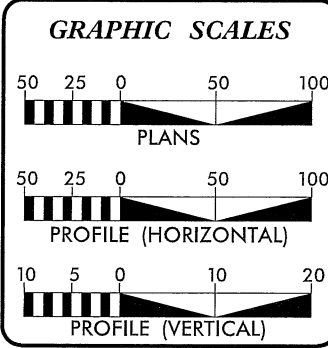
INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III
THIS PROJECT IS WITHIN MUNICIPAL BOUNDARIES OF CHARLOTTE

TIP PROJECT: U-5008

CONTRACT:



DESIGN DATA

ADT 2010 =	24,300
ADT 2035 =	30,300
DHV =	8 %
D =	55 %
T =	12 % *
V =	50 MPH
* TTST =	4% DUAL = 8%
FUNC CLASS =	URBAN COLLECTOR SUBREGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT U-5008 =	.381 MI
LENGTH OF STRUCTURE TIP PROJECT U-5008 =	.156 MI
TOTAL LENGTH OF TIP PROJECT U-5008 =	.537 MI
DESIGN EXCEPTIONS REQUIRED:	DESIGN SPEED

Prepared In the Office of:

SEPI
ENGINEERING & CONSTRUCTION
1025 Wade Avenue
Raleigh, NC 27609
Tel: 919-788-9977
Fax: 919-788-9991
License: C-2197

FOR THE NORTH CAROLINA DEPT. OF TRANSPORTATION

2012 STANDARD SPECIFICATIONS

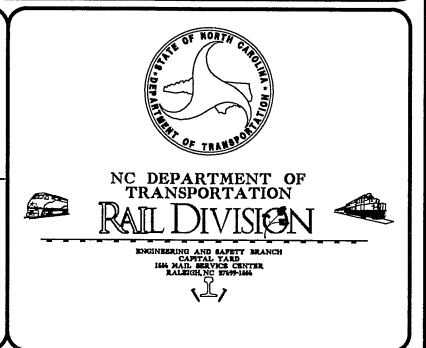
RIGHT OF WAY DATE: June 2013	STEVE THOMAS, PE PROJECT ENGINEER
LETTING DATE: November 2014	DAVID CLODGO, PE PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

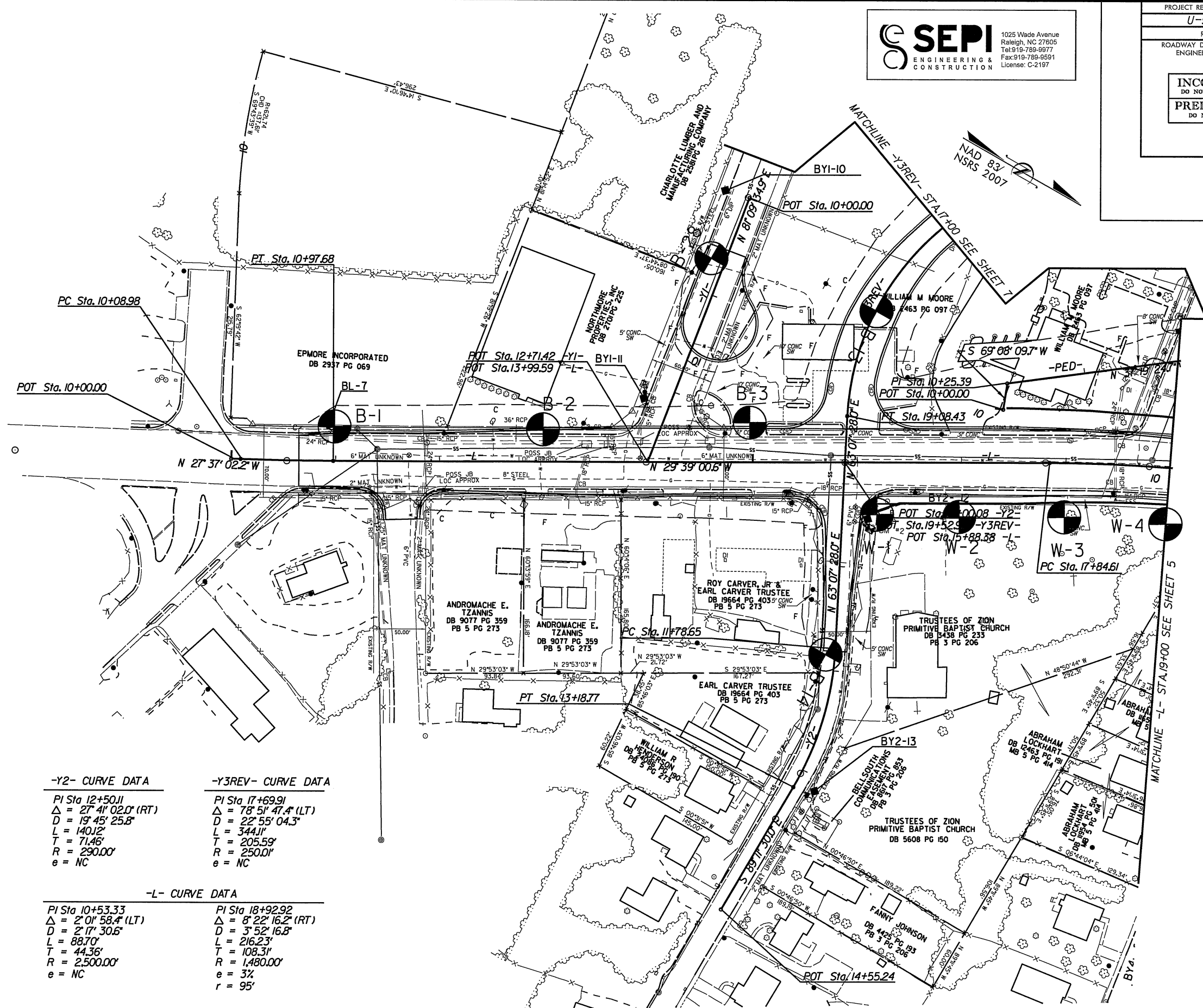
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ROADWAY DESIGN ENGINEER

SIGNATURE: _____ P.E.



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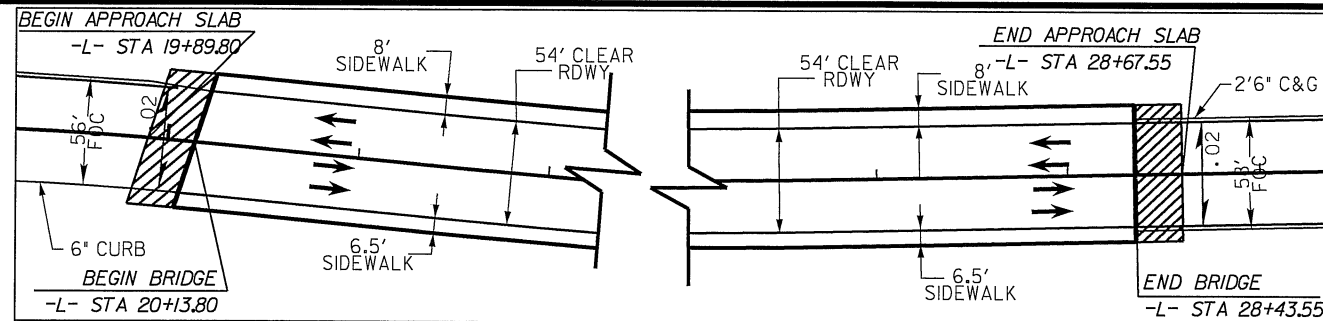
PI Sta 12+50.11
Δ = 27° 41' 02.0" (RT)
D = 19' 45' 25.8"
L = 140.12'
T = 71.46'
R = 250.00'
e = NC

-Y3REV- CURVE DATA

PI Sta 17+69.91
Δ = 78° 51' 47.4" (LT)
D = 22' 55' 04.3"
L = 344.11'
T = 205.59'
R = 250.00'
e = NC

-L- CURVE DATA

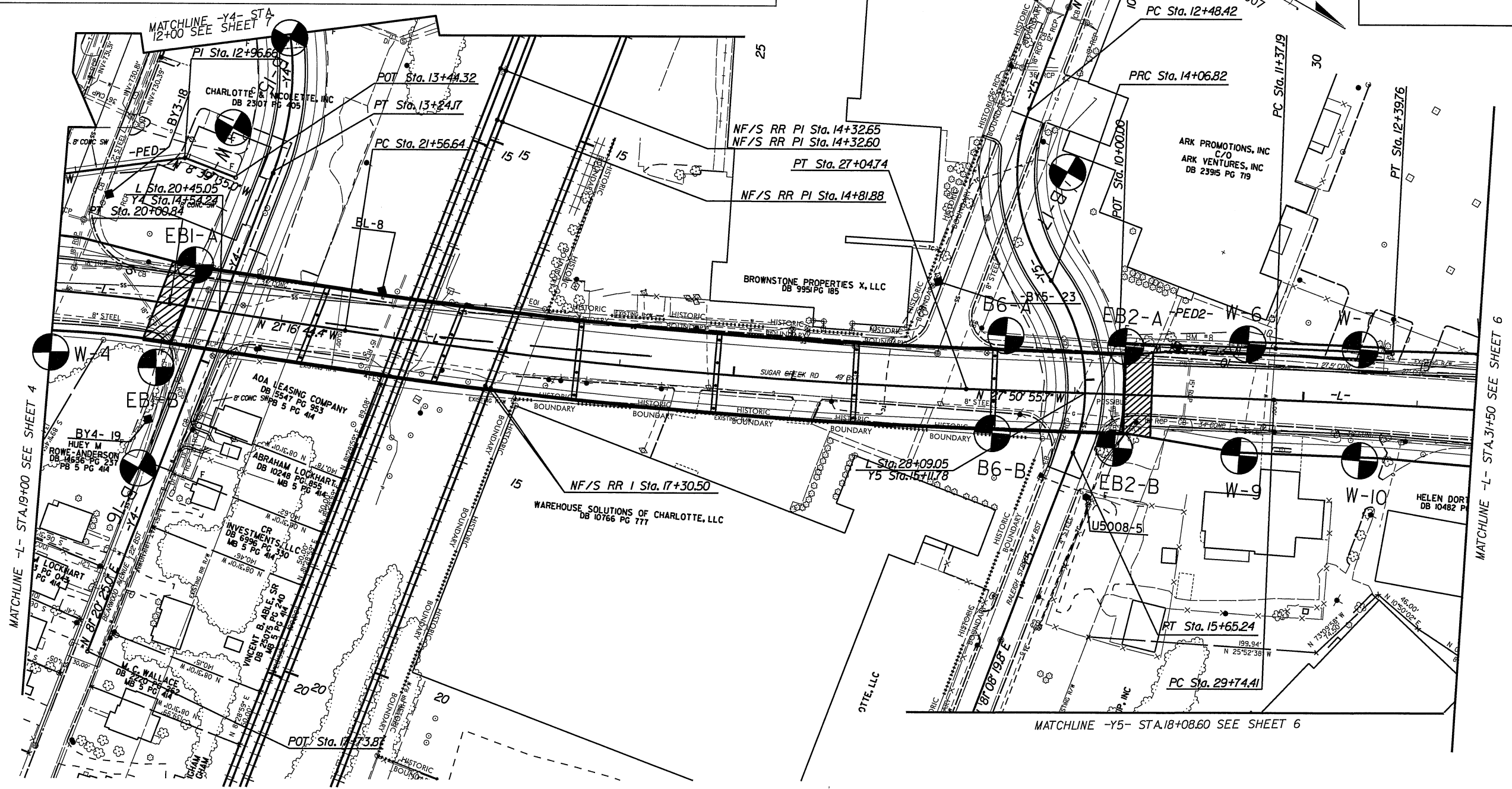
PI Sta 10+53.33	PI Sta 18+92.92
Δ = 2° 01' 58.4" (LT)	Δ = 8° 22' 16.2" (RT)
D = 2' 17' 30.6"	D = 3' 52' 16.8"
L = 88.70'	L = 216.23'
T = 44.36'	T = 108.31'
R = 2,500.00'	R = 1,480.00'
e = NC	e = 3%
	r = 95'



Sketch showing Dimensions of Pavement and Shoulder in Relation to Proposed Bridge Width



PROJECT REFERENCE NO.	SHEET NO.
U-5008	5
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



-Y4- CURVE DATA

PI Sta 11+94.56
$\Delta = 69^{\circ}18'55.3"$ (RT)
$D = 22^{\circ}55'05.9"$
$L = 302.45'$
$T = 172.84'$
$R = 250.00'$
$e = NC$

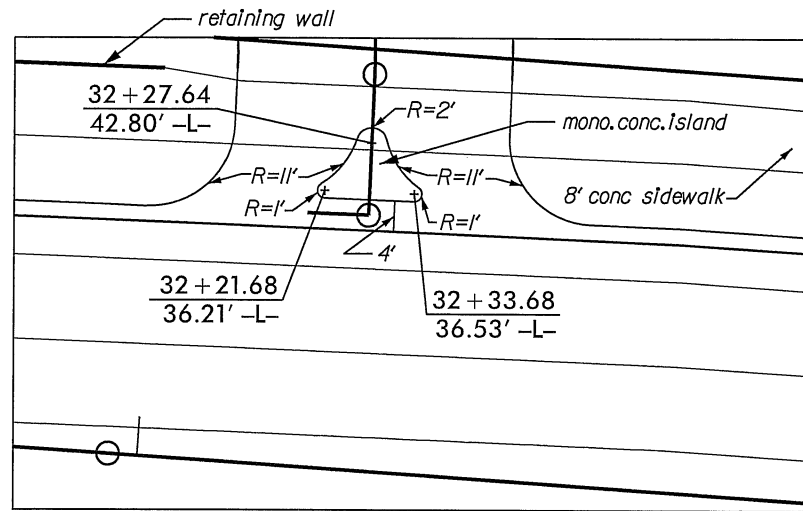
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PI Sta 18+92.92	PI Sta 24+30.99	PI Sta 30+85.30
$\Delta = 8^{\circ}22'16.2"$ (RT)	$\Delta = 6^{\circ}34'11.2"$ (LT)	$\Delta = 2^{\circ}07'03.0"$ (RT)
$D = 3^{\circ}52'16.8"$	$D = 1^{\circ}11'55.2"$	$D = 0^{\circ}57'17.7"$
$L = 216.23'$	$L = 548.10'$	$L = 221.74'$
$T = 108.31'$	$T = 274.35'$	$T = 110.88'$
$R = 1,480.00'$	$R = 4,780.00'$	$R = 6,000.00'$
$e = 3\%$	$e = NC$	$e = NC$
$r = 95'$		

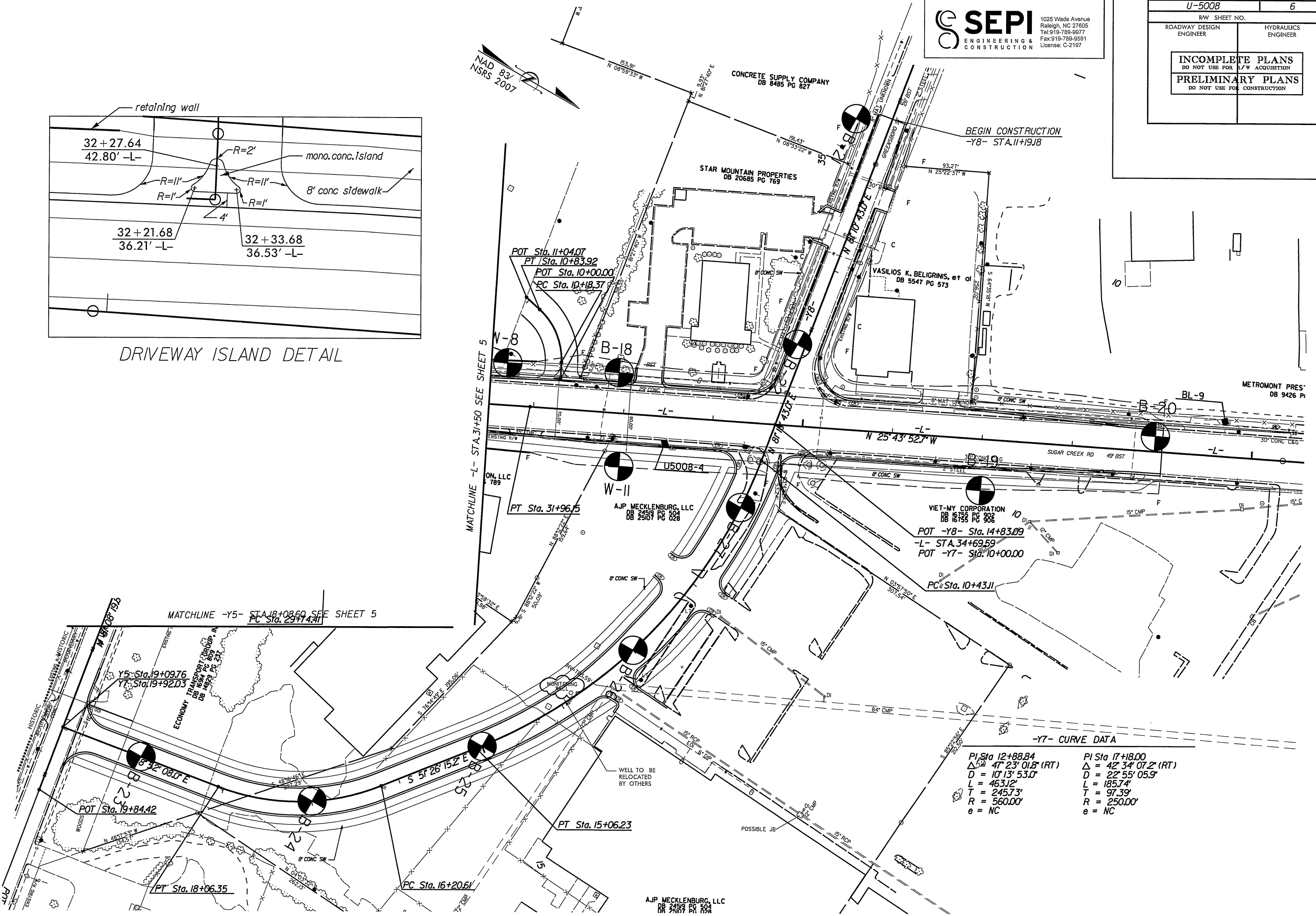
-Y5- CURVE DATA

PI Sta 13+34.72	PI Sta 14+93.12
$\Delta = 56^{\circ}28'22.2"$ (LT)	$\Delta = 56^{\circ}25'09.9"$ (RT)
$D = 35^{\circ}39'11.6"$	$D = 35^{\circ}36'48.3"$
$L = 158.39'$	$L = 158.42'$
$T = 86.30'$	$T = 86.30'$
$R = 160.70'$	$R = 160.88'$
$e = NC$	$e = NC$

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DRIVEWAY ISLAND DETAIL



-Y7- CURVE DATA

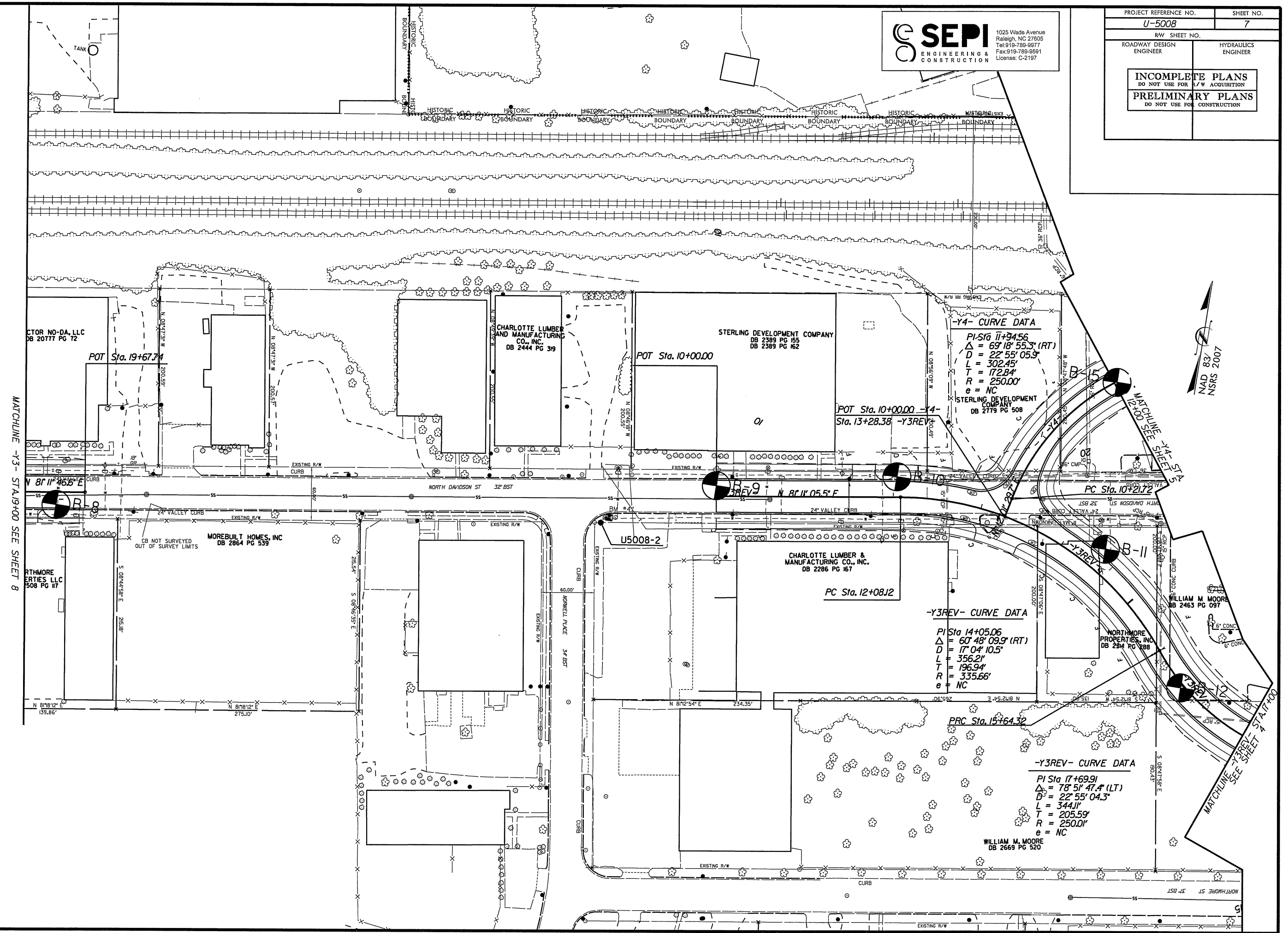
PI Sta 12+88.84	PI Sta 17+18.00
$\Delta = 47^{\circ} 23' 01.8''$ (RT)	$\Delta = 42^{\circ} 34' 07.2''$ (RT)
D = 10' 13" 53.0"	D = 22' 55" 05.9"
L = 463.12'	L = 185.74'
T = 245.73'	T = 97.39'
R = 560.00'	R = 250.00'
e = NC	e = NC

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Fax: 919-789-9591
License: C-2197

PROJECT REFERENCE NO.	SHEET NO.
U-5008	7
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
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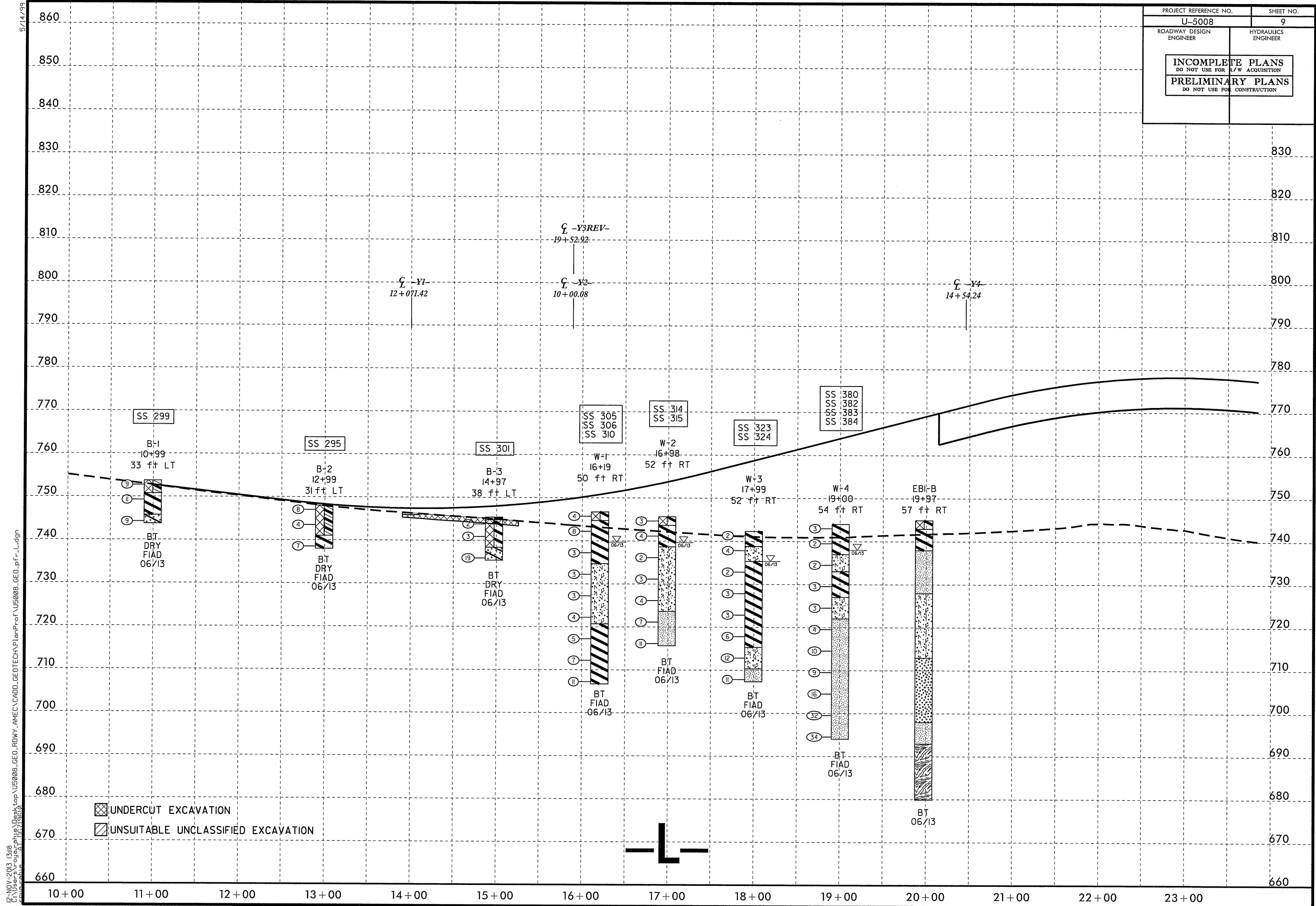


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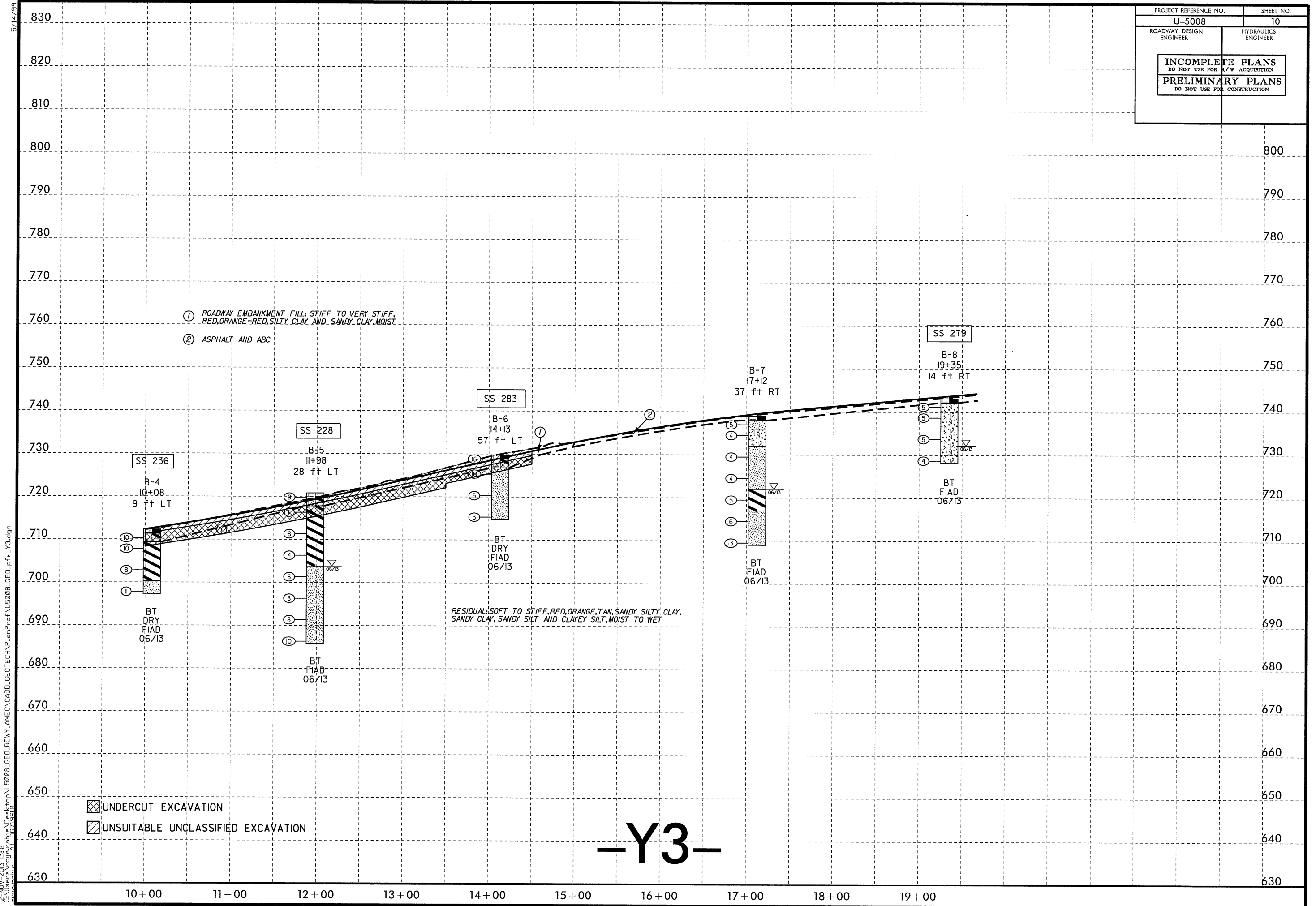
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MATCHLINE -Y4- STA. 12+00 SEE SHEET 10
 MATCHLINE -Y3- STA. 17+00 SEE SHEET 11
 MATCHLINE -Y3- STA. 17+00 SEE SHEET 12



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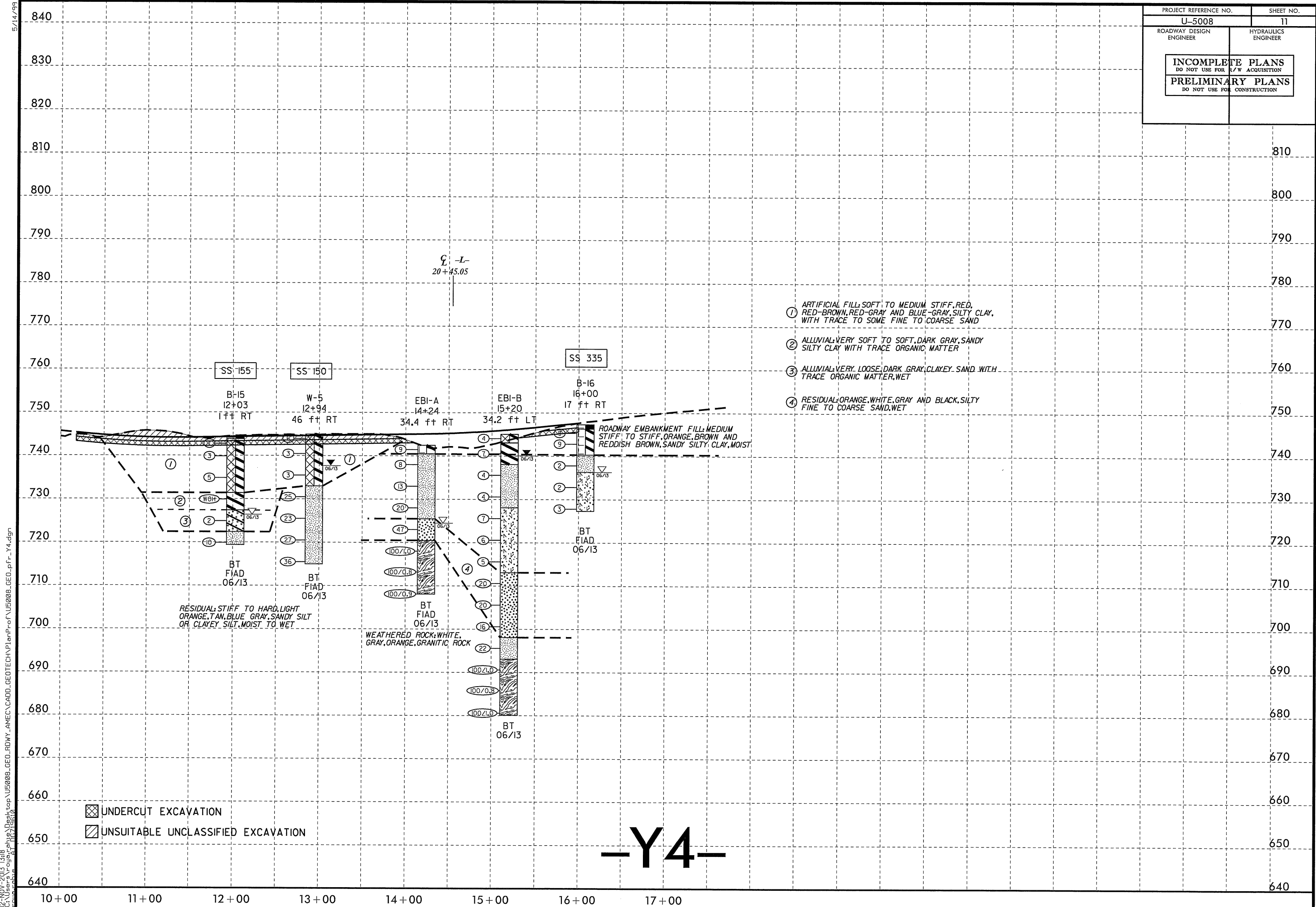
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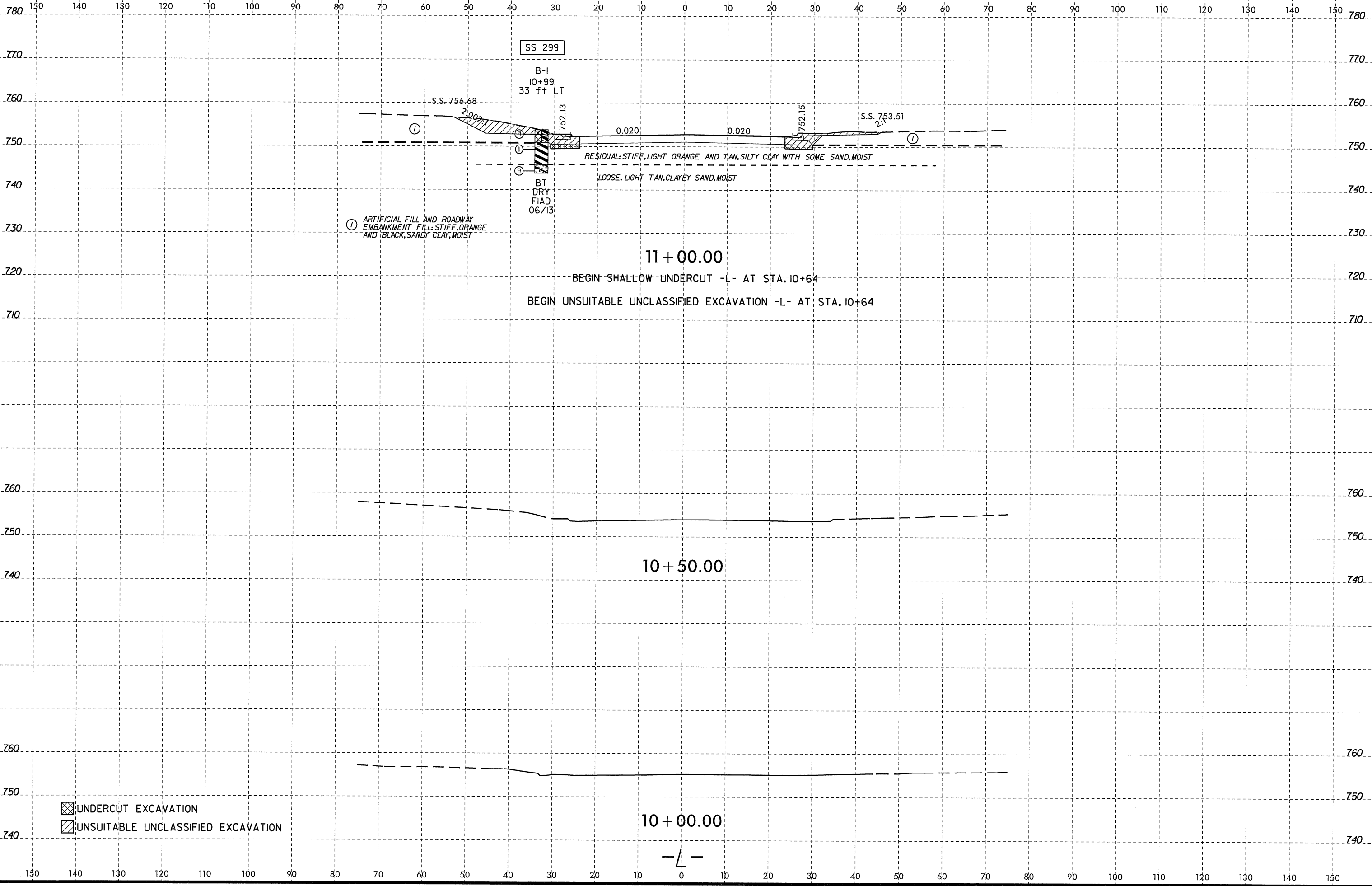
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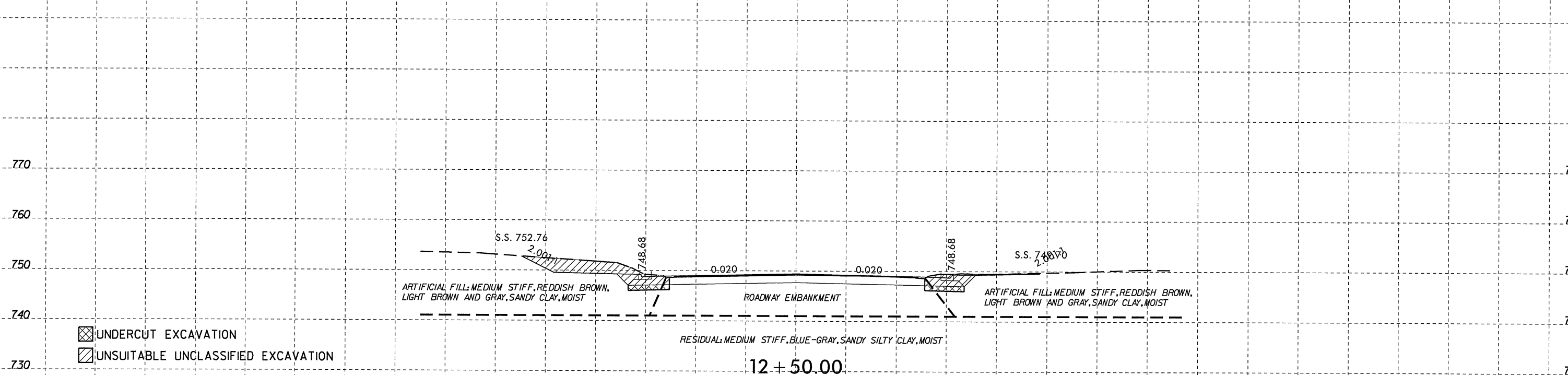
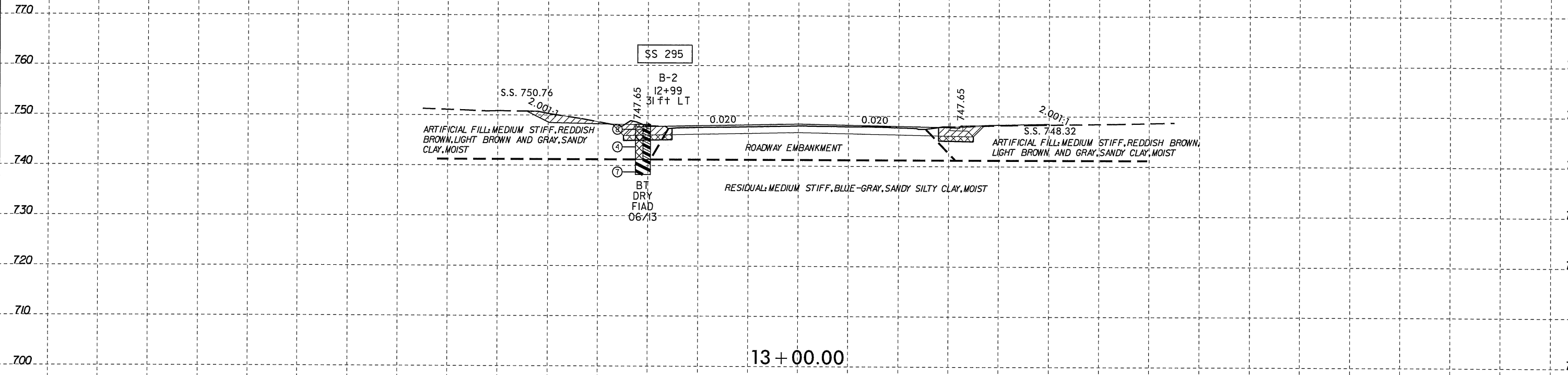
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8/23/99

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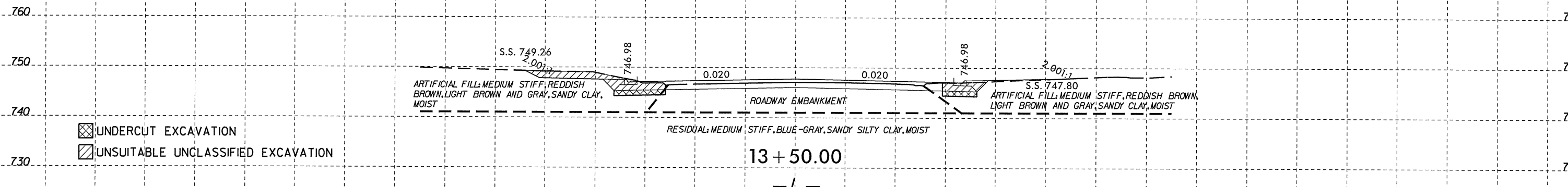
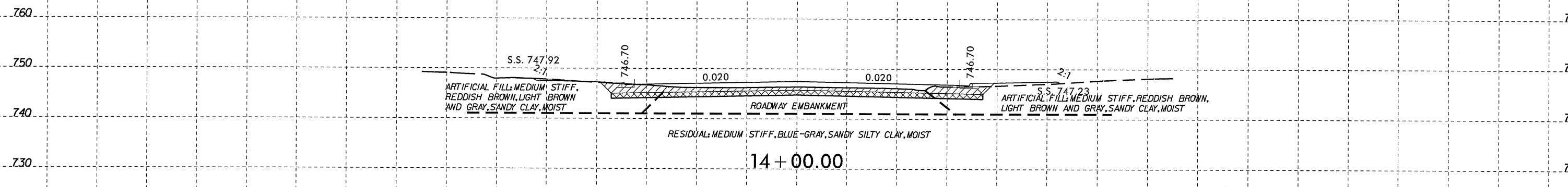
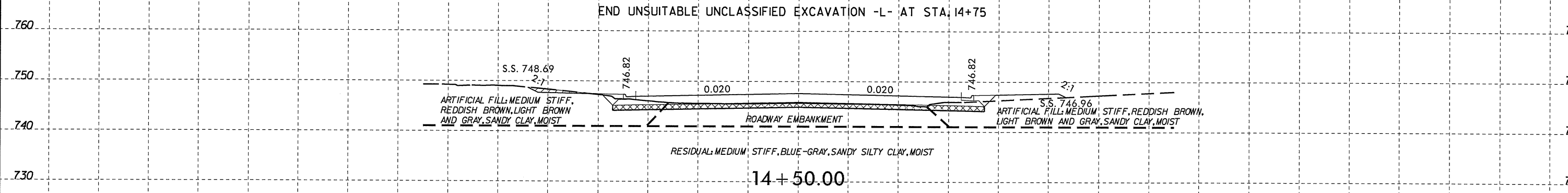
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- UNSUITABLE UNCLASSIFIED EXCAVATION

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 06/13/10

8/23/99

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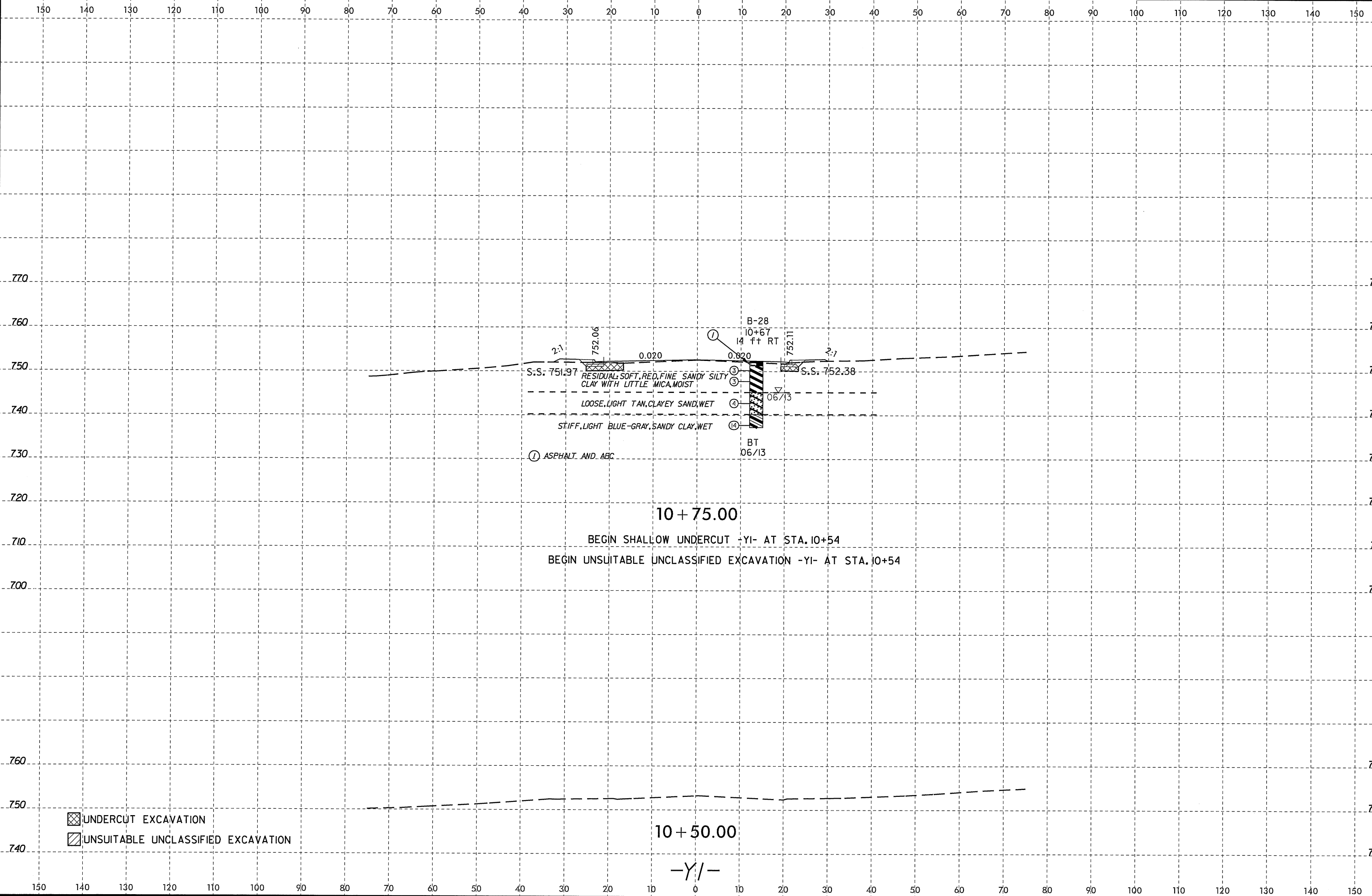


- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

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8/23/99



2:1
 752.06
 0.020
 0.020
 752.11
 2:1
 S.S. 751.97
 RESIDUAL, SOFT, RED, FINE SANDY SILTY CLAY WITH LITTLE MICA, MOIST
 LOOSE, LIGHT TAN, CLAYEY SAND, WET
 STIFF, LIGHT BLUE-GRAY, SANDY CLAY, WET
 ① ASPHALT AND ABC
 B-28
 10+67
 14 ft RT
 BT 06/13
 S.S. 752.38
 06/13

10 + 75.00

BEGIN SHALLOW UNDERCUT -YI- AT STA. 10+54

BEGIN UNSUITABLE UNCLASSIFIED EXCAVATION -YI- AT STA. 10+54

10 + 50.00

-Y/-

- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

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8/23/99

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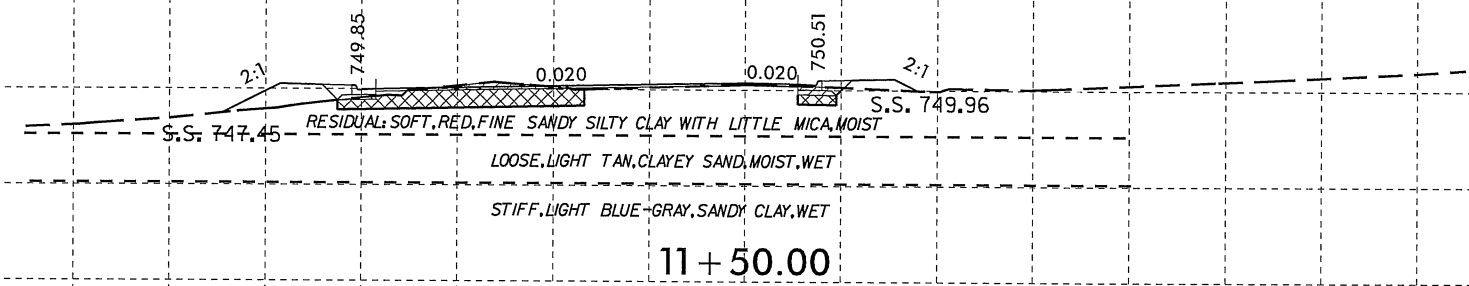
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750 750

740 740

730 730

END UNSUITABLE UNCLASSIFIED -YI- AT STA. 11+62
 END SHALLOW UNDERCUT -YI- AT STA. 11+62



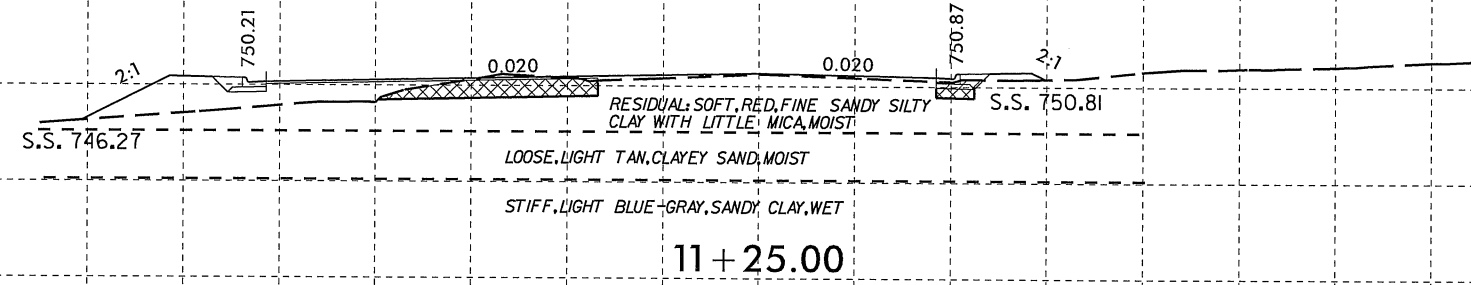
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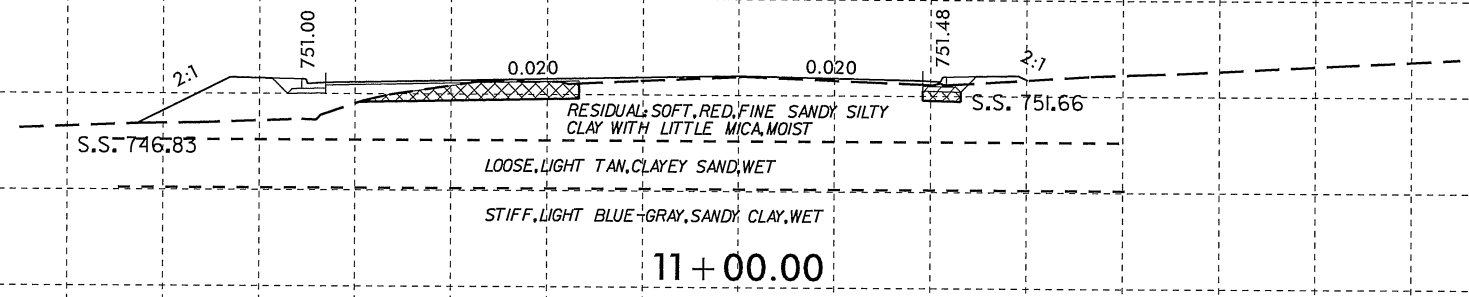
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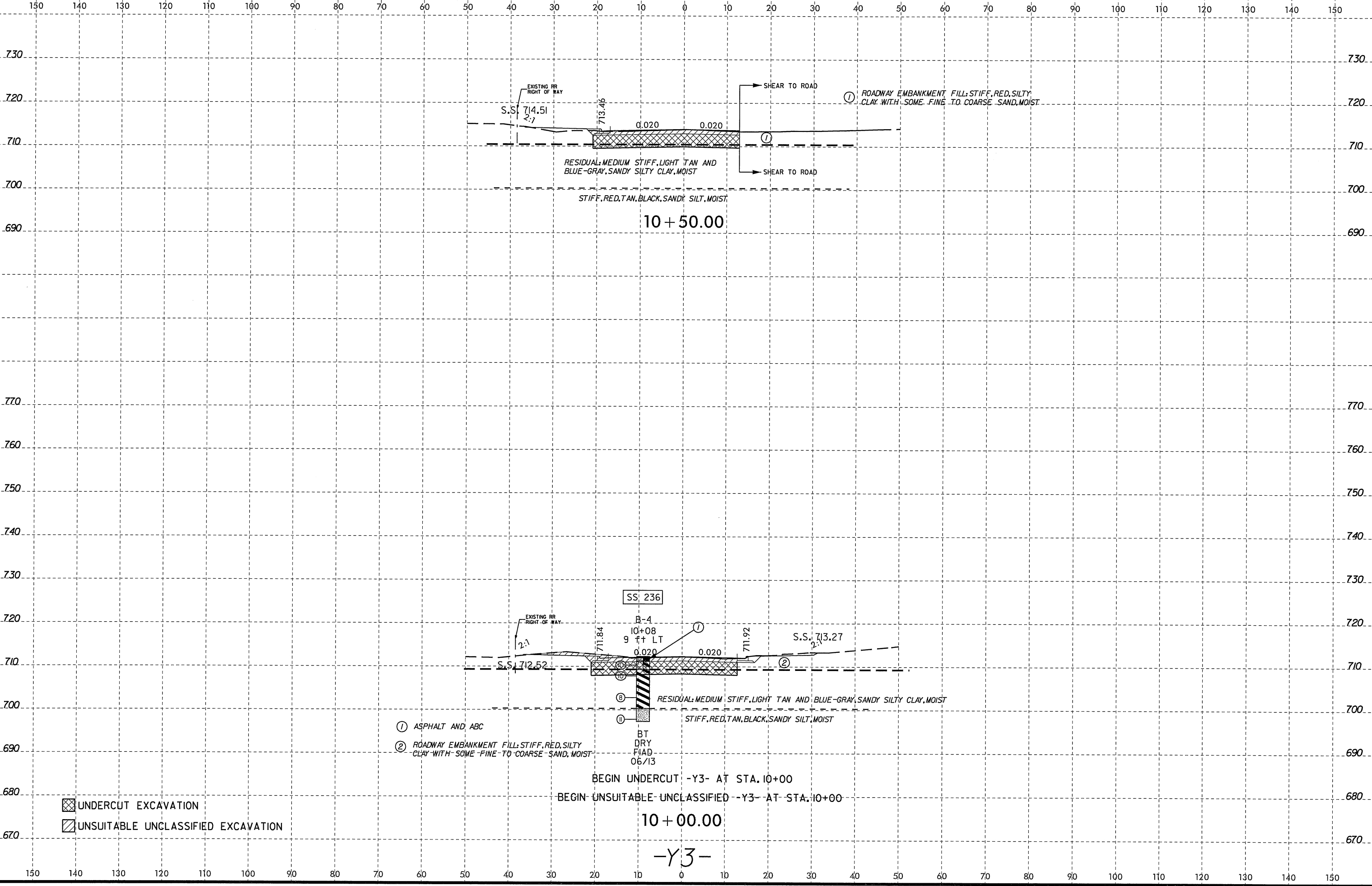
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-YI-

- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

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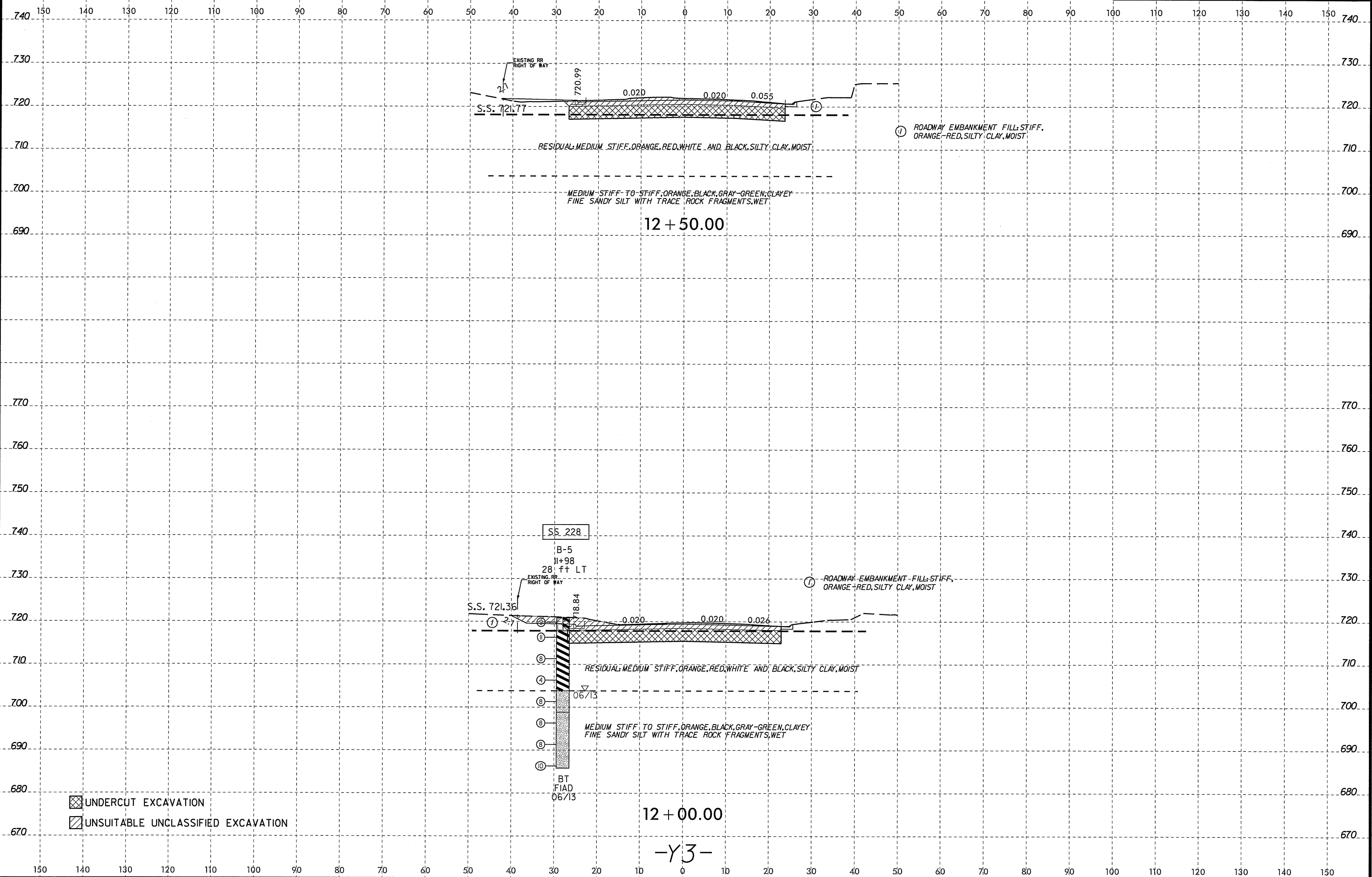
- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

- ① ASPHALT AND ABC
- ② ROADWAY EMBANKMENT FILL: STIFF, RED, SILTY CLAY WITH SOME FINE TO COARSE SAND, MOIST

BEGIN UNDERCUT -Y3- AT STA. 10+00
 BEGIN UNSUITABLE UNCLASSIFIED -Y3- AT STA. 10+00

10 + 00.00
 -Y3-

8/23/99





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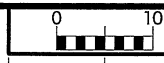
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12 + 00.00

-Y3-

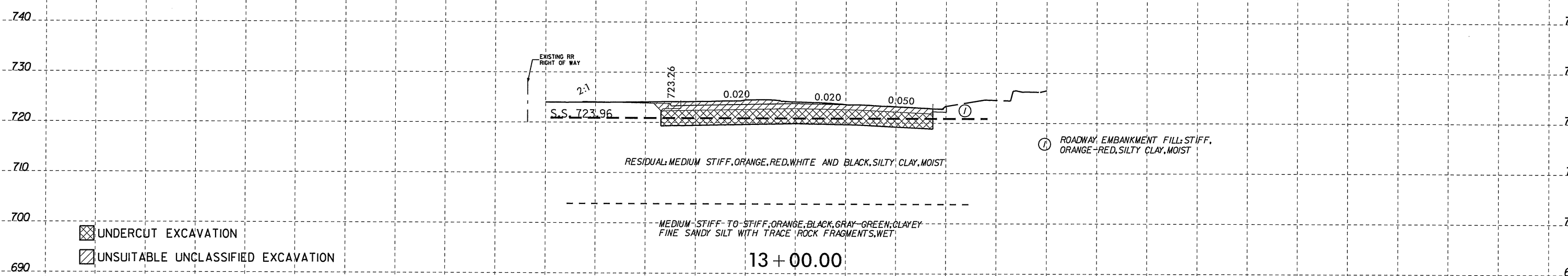
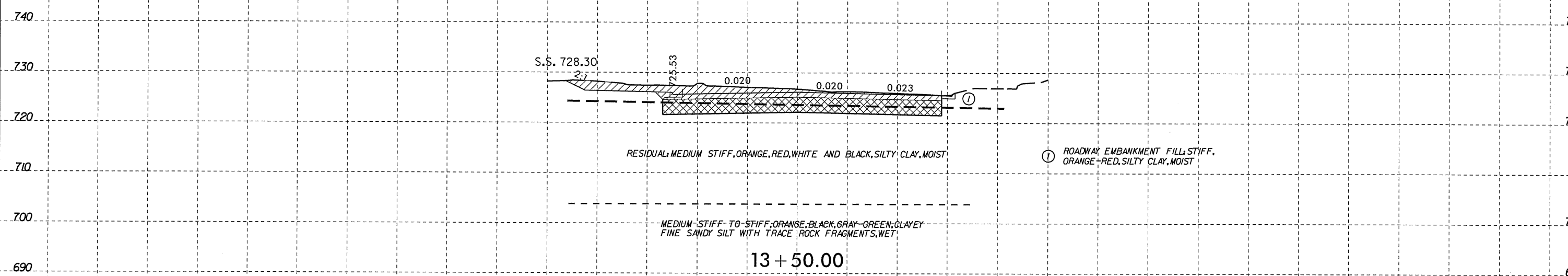
-  UNDERCUT EXCAVATION
-  UNSUITABLE UNCLASSIFIED EXCAVATION

8/23/99



PROJ. REFERENCE NO.	SHEET NO.
U-5008	24

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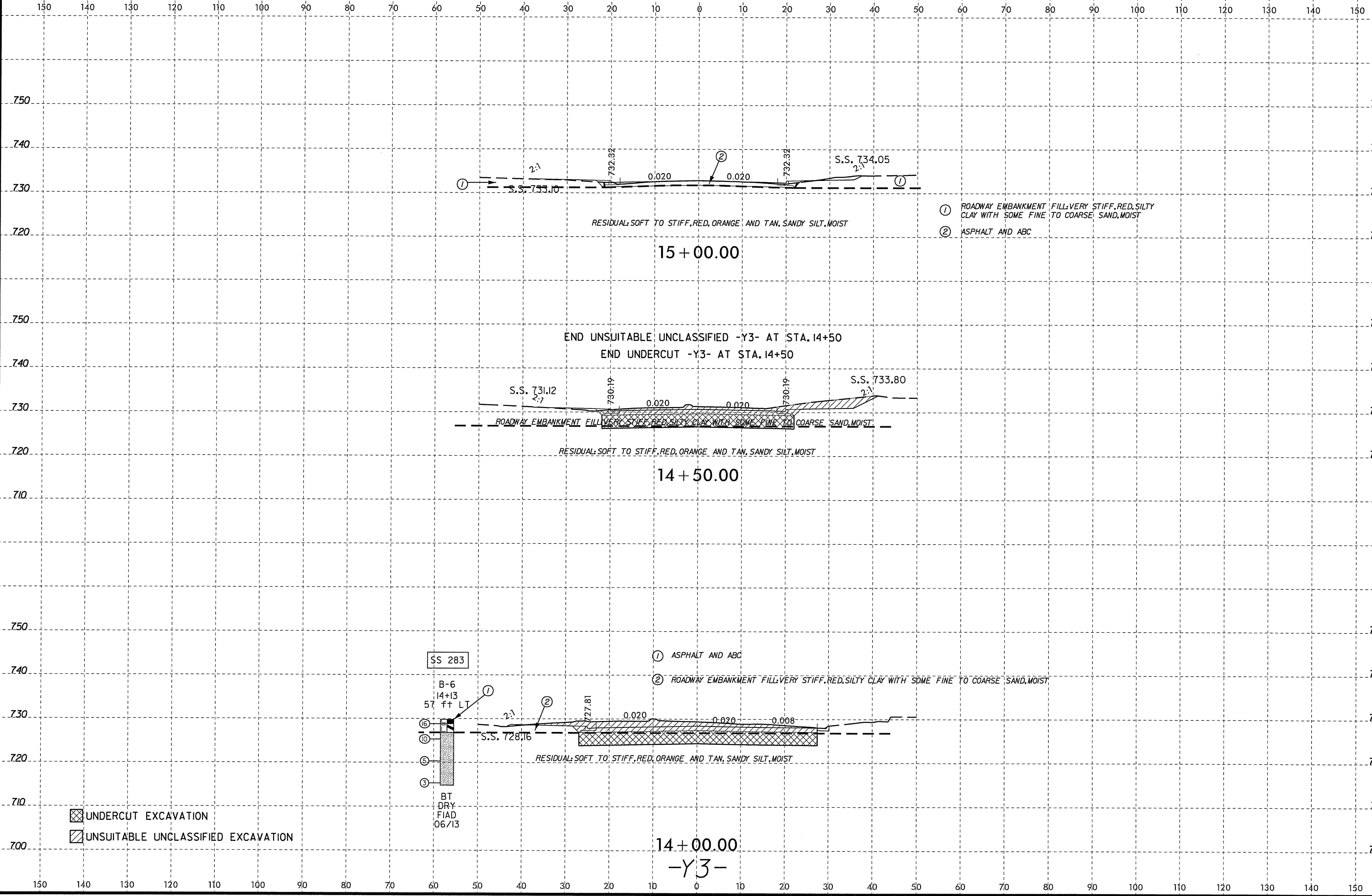
- UNDERCUT EXCAVATION
- UNSUITABLE UNCLASSIFIED EXCAVATION

-Y3-

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 11/16/10
 jph

8/22/99



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8/23/99



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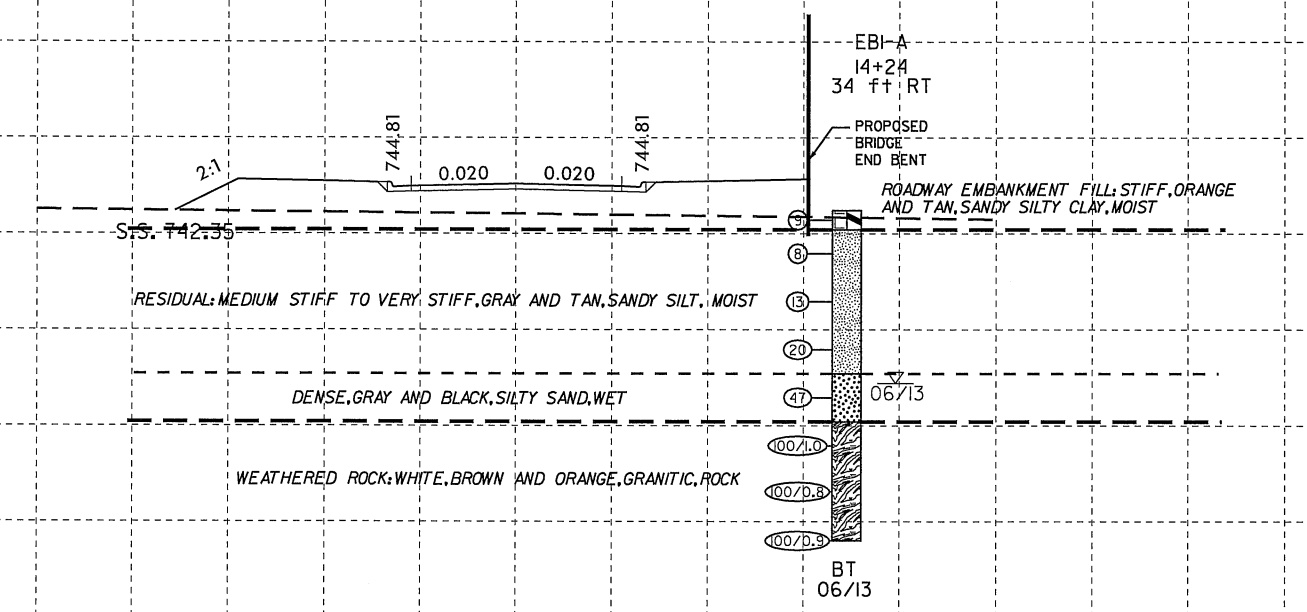
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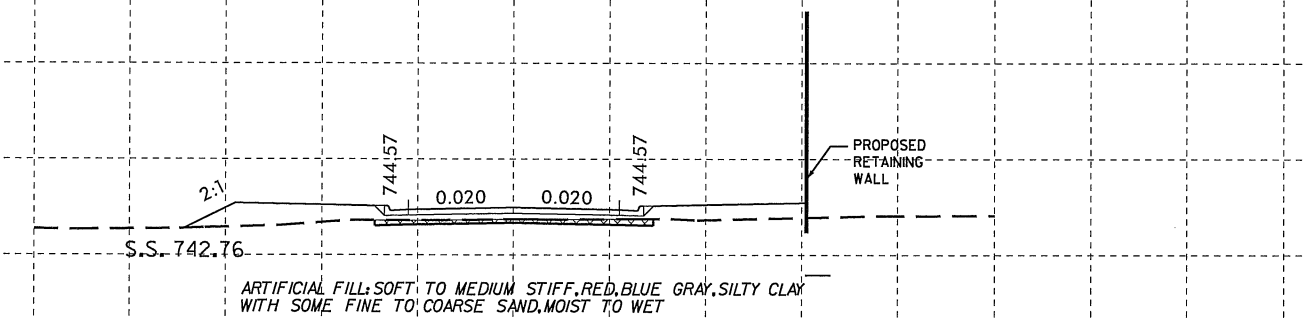
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14 + 50.00



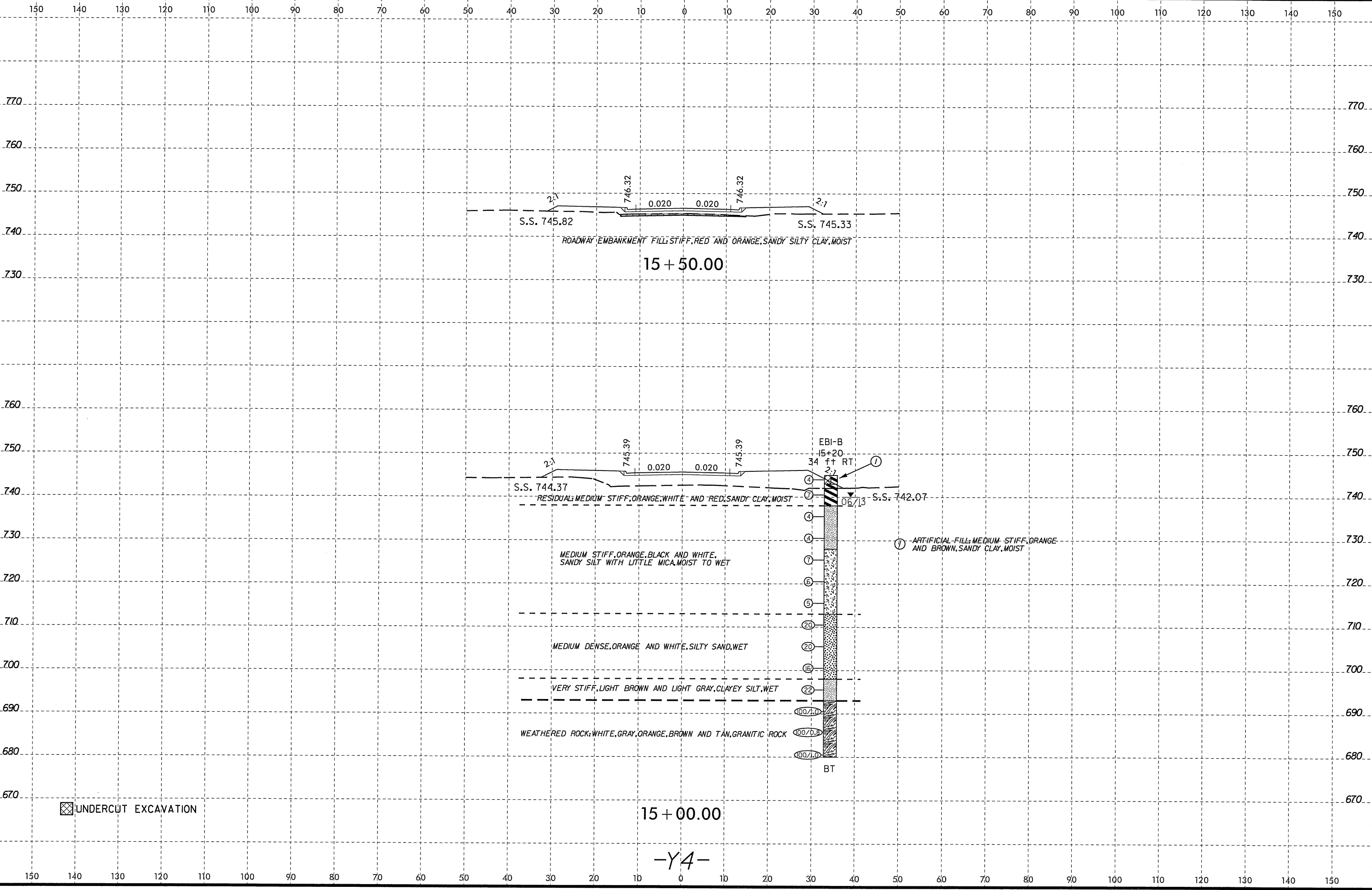
14 + 00.00

UNDERCUT EXCAVATION

-Y4-

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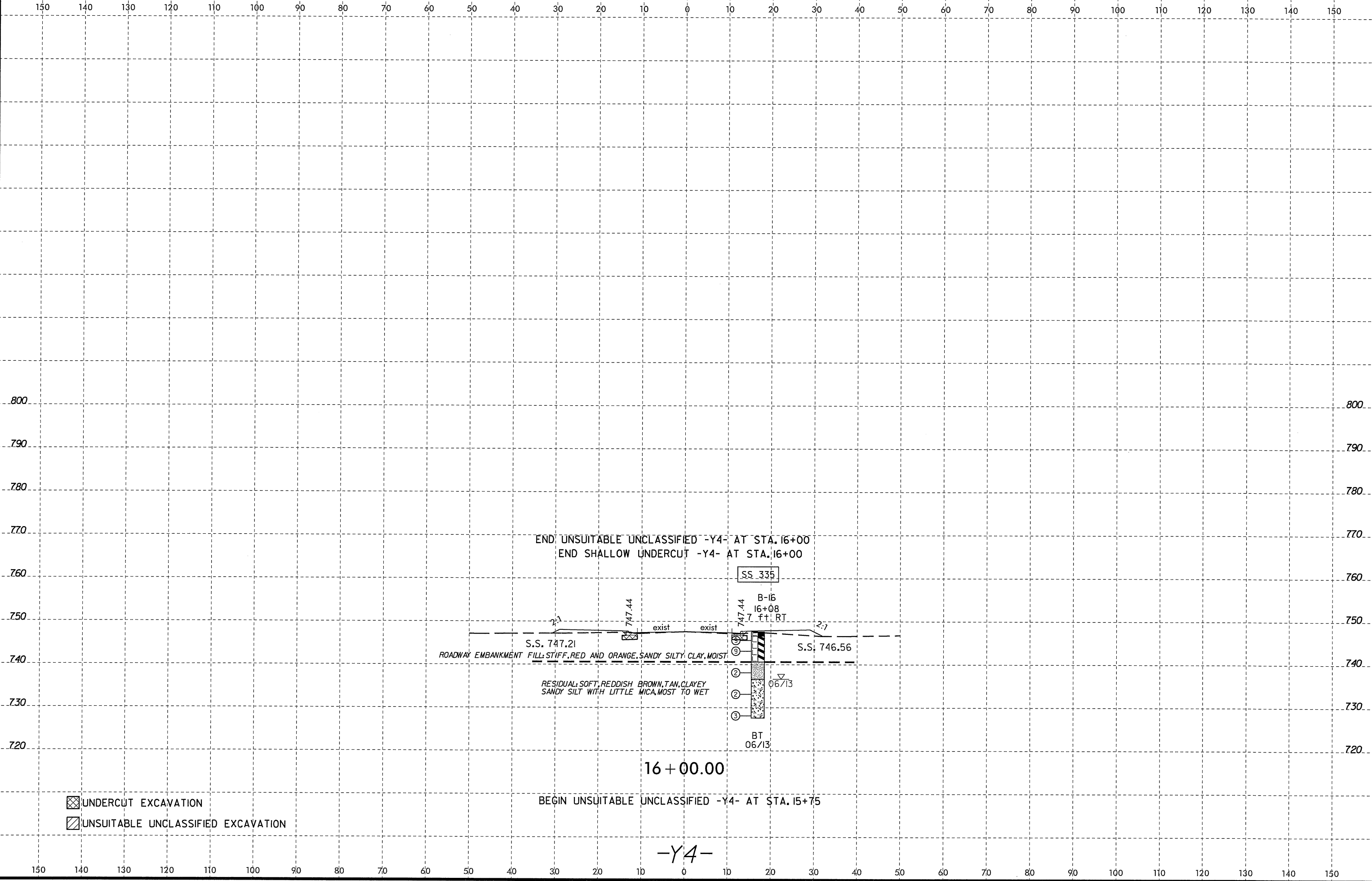
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


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-Y4-

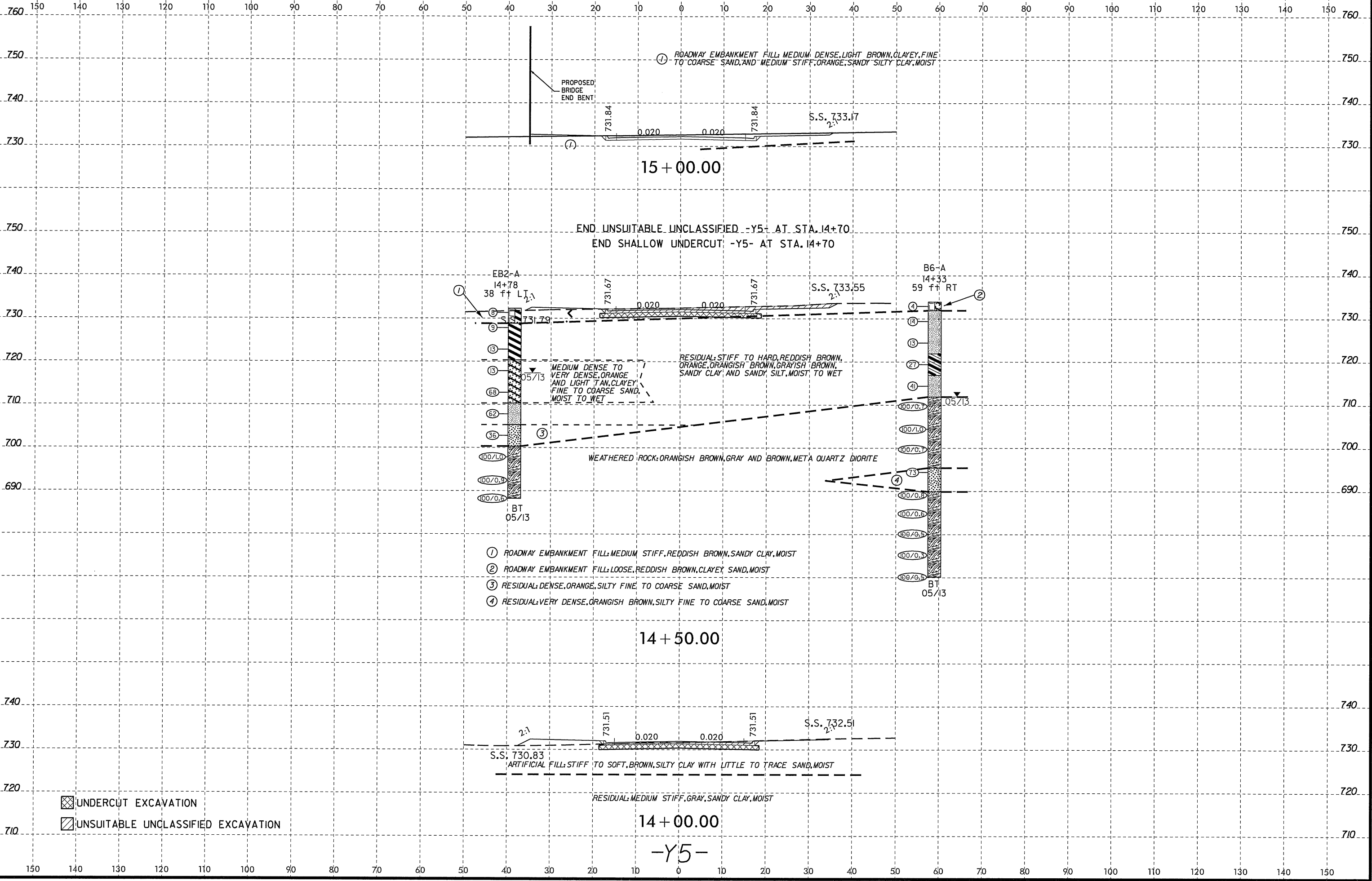
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-  UNDERCUT EXCAVATION
-  UNSUITABLE UNCLASSIFIED EXCAVATION

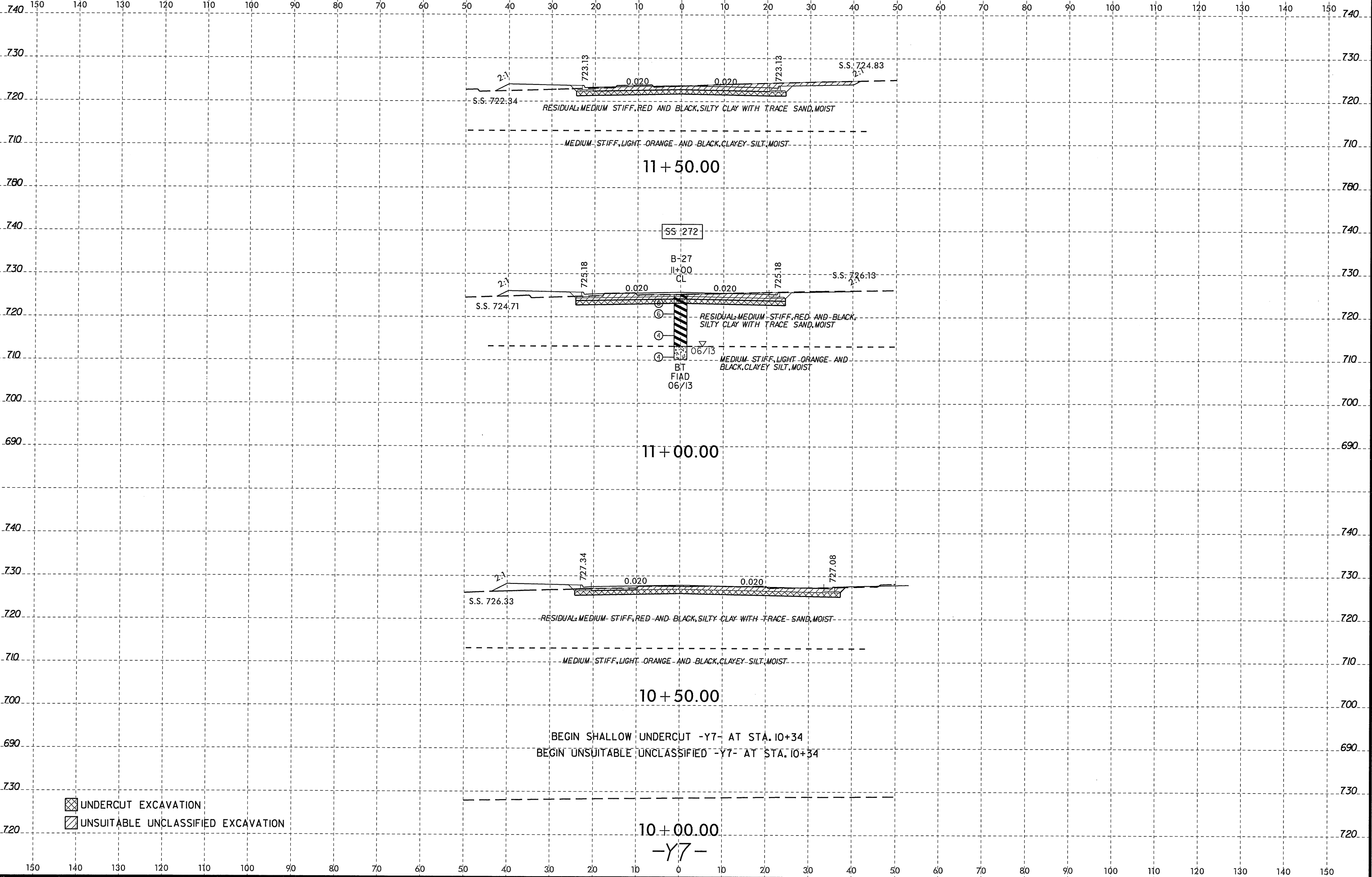
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8/23/99



11 + 50.00



11 + 00.00

10 + 50.00

10 + 00.00

-Y7-

BEGIN SHALLOW UNDERCUT -Y7- AT STA. 10+34
 BEGIN UNSUITABLE UNCLASSIFIED -Y7- AT STA. 10+34

-  UNDERCUT EXCAVATION
-  UNSUITABLE UNCLASSIFIED EXCAVATION

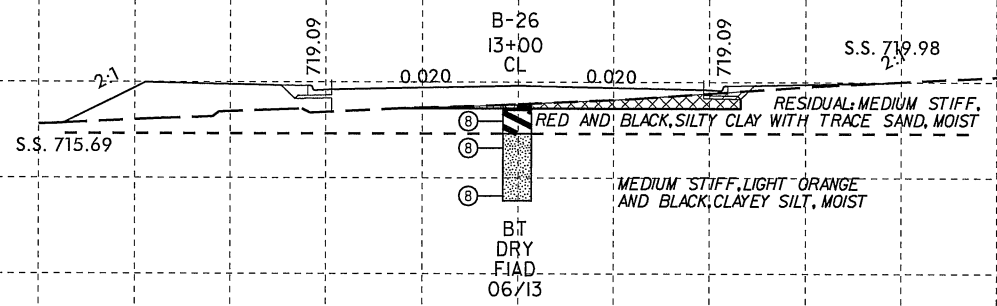
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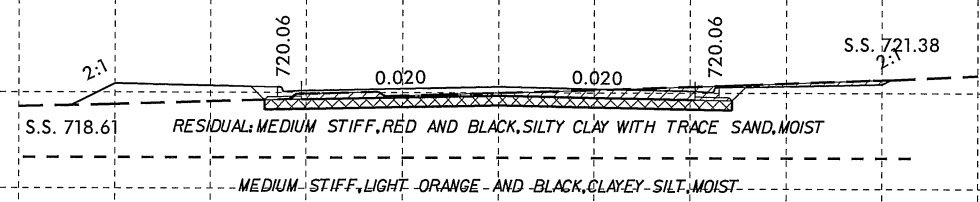


END UNSUITABLE UNCLASSIFIED -Y7- AT STA. 13+25
END SHALLOW UNDERCUT -Y7- AT STA. 13+25

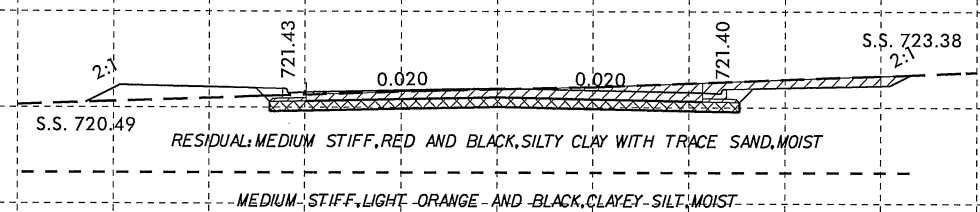
SS 269



13 + 00.00


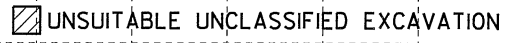


12 + 50.00



12 + 00.00

-Y7-

-  UNDERCUT EXCAVATION
-  UNSUITABLE UNCLASSIFIED EXCAVATION

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SOIL TEST RESULTS

Boring	SAMPLE	OFFSET	STATION	DEPTH	AASHTO	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			MOISTURE	ORGANIC	Alignment
								C.SAND	F.SAND	SILT	CLAY	10	40	200			
B-1	SS 299	33 LT	10+99	3.5-5.0	A-7-5(44)	83	50	15.6	7.2	32.4	44.8	100	88.5	78.6	28.6		L
B-2	SS 295	31 LT	12+99	0.0-1.5	A-7-6(10)	45	22	33.1	21.1	21.5	24.4	98.8	77.2	55.8	20.8		L
B-3	SS 301	38 LT	14+97	0.5-2.0	A-7-6(21)	53	26	9.3	12.3	45.5	32.9	95.5	91.9	77.0	30.9		L
B-4	SS 236	9 LT	10+08	1.0-2.5	A-7-6(27)	58	33	9.1	13.7	25.1	52.1	99.8	95.1	78.8	23.2		Y3
B-5	SS 228	28 LT	11+98	0.0-1.5	A-7-5(26)	62	30	11.7	11.6	32.2	44.5	100	93.8	78.6	27.2		Y3
B-6	SS 283	57 LT	14+13	1.0-2.5	A-7-5(24)	62	26	9.7	14.3	57.5	18.5	100	96.6	79.9	26.2		Y3
B-8	SS 279	14 RT	19+35	1.0-2.5	A-5(14)	49	10	3.8	11.3	66.5	18.3	99.8	96.5	91.8	28.8		Y3
B-10	SS 208	23 LT	12+04	0.8-2.3	A-5(2)	42	9	38.0	14.7	37.3	10.1	93.4	63.6	47.5	26.5		Y3REV
B-11	SS 162	26 LT	14+41	3.5-5.0	A-7-6(10)	47	23	35.2	12.6	32.7	19.5	98.9	73.2	53.6	24.7		Y3REV
B-13	SS 292	1 LT	18+03	3.5-5.0	A-7-5(10)	55	25	37.7	13.1	20.4	28.8	99.0	69.7	51.3	29.0		Y3REV
B-14	SS 332	9 RT	11+85	1.0-2.5	A-7-6(14)	47	24	22.3	13.8	33.8	30.1	99.6	85.9	65.9	25.4		Y2
B-15	SS 155	1 RT	12+03	0.6-2.1	A-7-5(28)	57	25	1.6	0.6	46.2	51.6	95.0	94.1	93.2	32.5		Y4
B-16	SS 335	17 RT	16+08	1.0-2.5	A-7-5 (17)	62	27	27.8	15.5	32.4	24.3	99.3	78.4	62.6	26.6		Y4
B-20	SS 249	17 LT	38+92	1.0-2.5	A-7-5 (24)	52	22	0.1	0.5	60.6	38.8	91.8	91.7	91.7	29.1		L
B-21	SS 240	38 RT	11+33	0.8-2.3	A-7-6 (12)	41	20	20.3	14.1	38.8	26.7	98.4	84.7	67.5	20.9		Y8
B-26	SS 269	CL	13+00	0.7-2.2	A-7-6 (21)	52	23	6.8	15.8	46.2	31.2	98.9	95.7	81.2	35.7		Y7
B-27	SS 272	CL	11+00	1.0-2.5	A-7-5 (50)	82	44	5	3.1	37.2	54.7	98.1	94.7	93.1	65.3		Y7
W-1	SS 305	50 RT	16+19	3.5-5.0	A-7-5(19)	65	27	26.2	8.4	38.5	26.9	98.0	78.0	65.8	29.2		L
W-1	SS 306	50 RT	16+19	8.5-10.0	A-7-5(4)	50	14	36.8	16.6	36.4	10.2	92.9	66.3	46.6	36.5		L
W-1	SS 310	50 RT	16+19	28.5-30.0	A-7-5 (10)	46	14	17.5	16.2	55.1	11.2	99.3	85.6	70.1	32.1		L
W-2	SS 314	52 RT	16+98	3.5-5.0	A-7-5(17)	60	23	23.4	9.5	41.0	26.1	97.8	79.9	67.8	35.6		L
W-2	SS 315	52 RT	16+98	8.5-10.0	A-5(6)	50	10	30.1	15.4	42.5	11.9	97.3	75.3	57.0	40.9		L
W-3	SS 323	52 RT	17+99	13.5-15.0	A-7-6(6)	41	14	26.9	20.6	43.6	8.9	96.9	77.2	58.0	38.1		L
W-3	SS 324	52 RT	17+99	18.5-20.0	A-7-5(6)	44	13	29.5	17.0	42.9	10.7	96.7	75.1	56.6	39.7		L
W-4	SS 380	54 RT	19+00	3.5-5.0	A-7-5(11)	51	18	27.7	12.6	40.2	19.5	98.5	77.6	61.9	34.1		L
W-4	SS 382	54 RT	19+00	13.5-15.0	A-7-5(10)	50	19	27.3	18.1	44.5	10.1	97.3	76.9	58.5	43.4		L
W-4	SS 383	54 RT	19+00	18.5-20.0	A-5(7)	42	10	15.9	22.6	51.5	10.0	100	92.6	67.6	41.6		L
W-4	SS 384	54 RT	19+00	23.5-25.0	A-4(3)	38	8	28.6	23.6	40.8	6.9	98.2	78.0	53.7	33.0		L
W-5	SS 150	46 RT	12+94	8.5-10.0	A-7-6 (19)	56	29	24.9	10.5	35	29.6	100	80.1	67.6	35.0		Y4
W-6	SS 26	48 LT	29+50	3.5-5.0	A-7-6 (15)	52	27	25.7	16.7	33.6	24	98.3	80.1	60.6	23.1		L
W-7	SS 35	47 LT	30+48	8.5 -10.0	A-7-6 (14)	44	15	5.3	20.3	54.2	20.2	100	97.7	82.2	31.8		L
B6-A	SS 2	48 LT	27+38	3.5-5.0	A-4(6)	38	10	16.8	26.7	45.9	10.6	100	90.9	64.9	22.3		L
B6-A	SS 3	48 LT	27+38	13.5-15.0	A-6(6)	39	11	15.4	29.3	44.0	11.3	99.6	91.7	63.5	16.7		L