

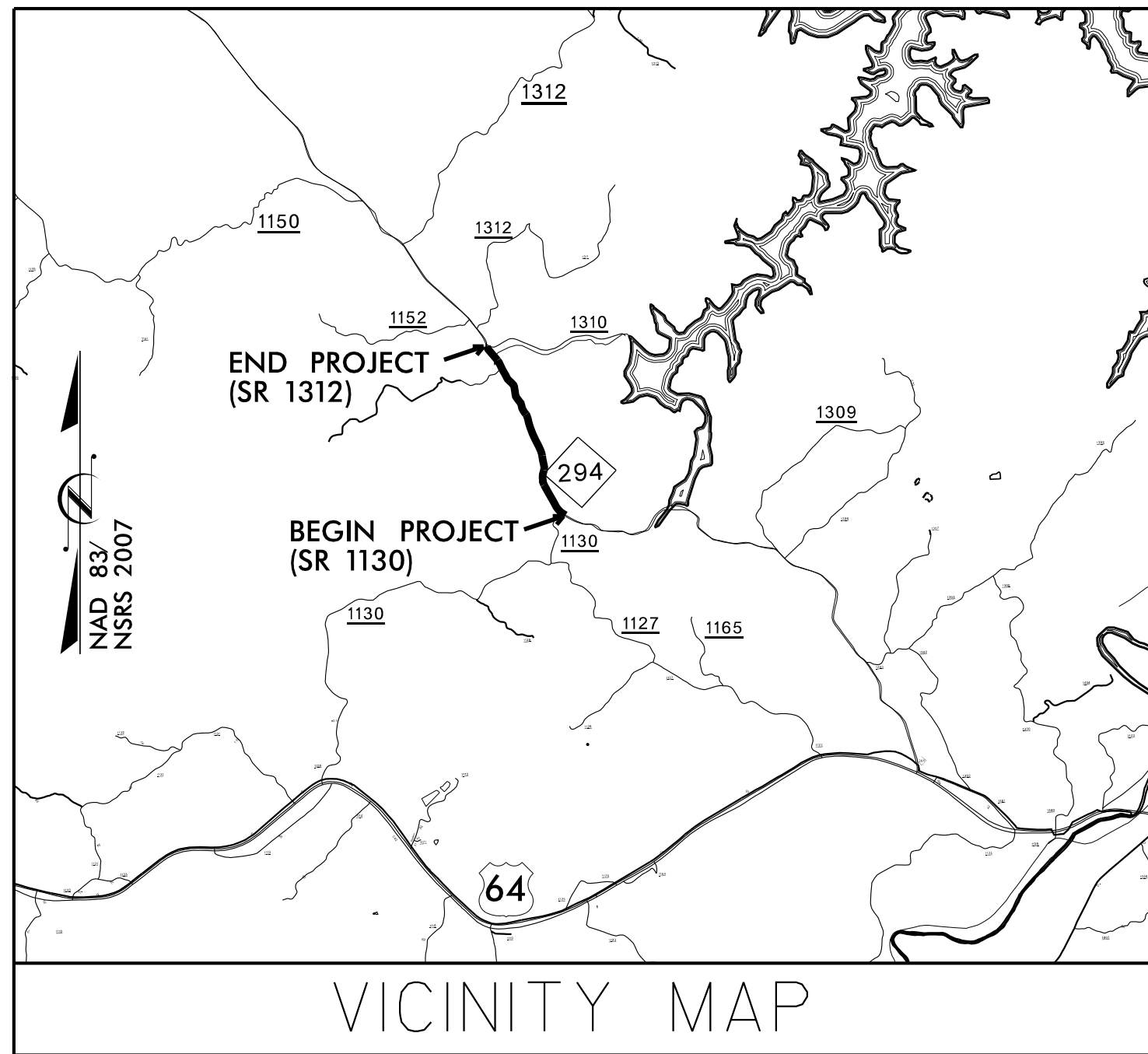
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09/28/2015

See Sheet 1-A For Index of Sheets



STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

**CHEROKEE COUNTY**

LOCATION: NC 294 FROM SR 1130 (SUNNY POINT ROAD)  
TO SR 1312 (UPPER BEAR PAW ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND CULVERT

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	R-3622B	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
38068.1.1		PE	
38068.2.R3		RW & UTILITIES	
38068.3.R3		CONST.	

**V&M**  
**Vaughn & Melton**  
Consulting Engineers

Asheville, North Carolina  
828-253-2796

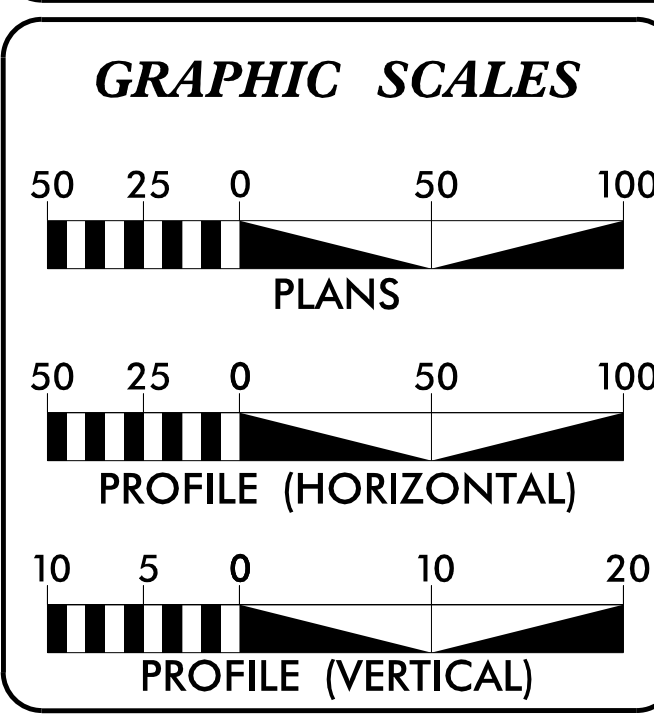
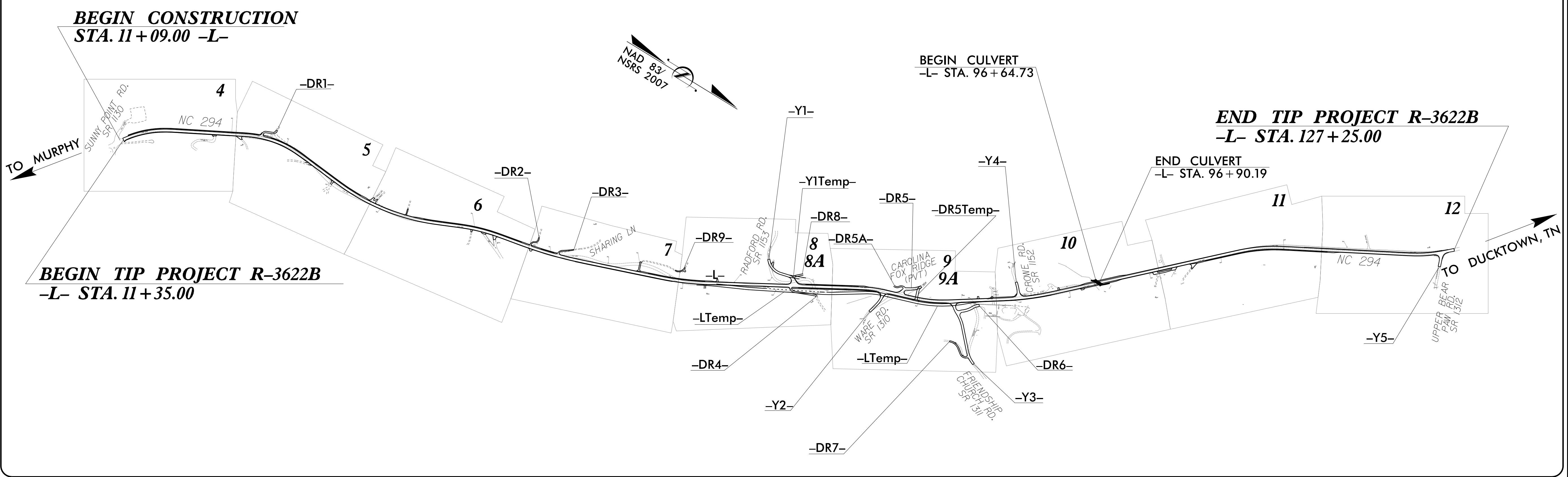
Boone, NC 828-355-9933  
Tri-Cities, TN 423-467-8401  
Knoxville, TN 865-546-5800  
Spartanburg, SC 864-574-4775  
Charleston, SC 843-974-5650  
Middlesboro, KY 606-248-6600  
Atlanta, GA 770-627-3509

Raleigh, NC 919-977-9455  
Charlotte, NC 704-357-0488

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TIP PROJECT: R-3622B

CONTRACT: C203648



**DESIGN DATA**

ADT	=	3,050
ADT	=	6,000
DHV	=	— %
D	=	— %
T	=	3 % *
V	=	50 MPH
* TTST 3% DUAL—%		
FUNC CLASS	=	MAJOR COLLECTOR REGIONAL TIER

**PROJECT LENGTH**

LENGTH ROADWAY TIP PROJECT R-3622B	=	2.190 MILES
LENGTH STRUCTURE TIP PROJECT R-3622B	=	0.005 MILES
TOTAL LENGTH OF TIP PROJECT R-3622B	=	2.195 MILES

Prepared In the Office of:  
**VAUGHN & MELTON, INC.**  
1318-F PATTON AVENUE ASHEVILLE, NC 28806 PHONE (828) 253-2796  
2012 STANDARD SPECIFICATIONS

**RIGHT OF WAY DATE:** MARCH 12, 2013  
**LETTING DATE:** JUNE 16, 2015

**REECE SCHULER, P.E.**  
PROJECT ENGINEER

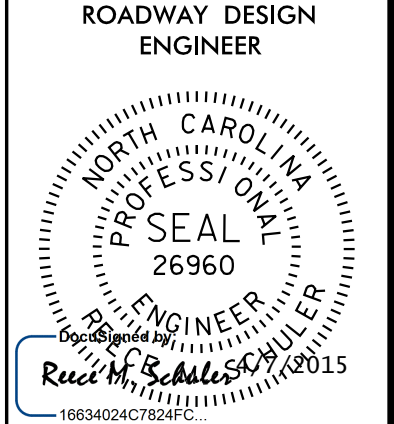
**NICK ASARO, PLS**  
PROJECT DESIGN ENGINEER

**HYDRAULICS ENGINEER**  
DocuSigned by: 4/13/2015  
**Aaron C. Carver**  
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SIGNATURE:

**ROADWAY DESIGN ENGINEER**  
DocuSigned by: 4/13/2015  
**Reece Schuler**  
18634024C7824FC...  
SIGNATURE:

**DIVISION OF HIGHWAYS  
STATE OF NORTH CAROLINA**

\$\$\$\$\$ SYSTEM \$\$\$\$\$\$  
\$\$\$\$\$ DGN \$\$\$\$\$\$  
\$\$\$\$\$ USERNAME \$\$\$\$\$\$



EFF. 01-17-2012  
REV. 10-30-2012

SHEET NUMBER	SHEET
1	TITLE SHEET
1-A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1-B	CONVENTIONAL SYMBOLS
1C-1 THRU 1C-4	SURVEY CONTROL SHEET
2A-1 THRU 2A-2	PAVEMENT SCHEDULE, TYPICAL SECTIONS, AND WEDGING DETAILS
2C-1	TRAFFIC BEARING DROP INLET DETAIL
2C-2	GUIDE FOR BERM DRAINAGE OUTLET DETAIL
2C-3	PIPE COLLAR DETAIL
2C-4	2-STRAND ELECTRIC WIRE FENCE DETAIL
2C-5	SLOTTED DRAIN DETAIL
2G-1 THRU 2G-3	STANDARD TEMPORARY WALL DETAIL SHEETS
2H-1	STOCKPILE CONTAINMENT DETAIL
3B-1	SUMMARY OF EARTHWORK & PAVEMENT REMOVAL SUMMARY
3B-2	GUARDRAIL SUMMARY, TEMPORARY GUARDRAIL SUMMARY
3D-1 THRU 3D-5	DRAINAGE SUMMARY
3G-1	GEOTECHNICAL SUMMARY SHEET
3P-1	PARCEL INDEX SHEET
4 THRU 24	PLAN AND PROFILE SHEETS
TMP-1 THRU TMP-17	TRAFFIC MANAGEMENT PLANS
PMP-1 THRU PMP-4	PAVEMENT MARKING PLANS
EC-1 THRU EC-23	EROSION CONTROL PLANS
RF-1	REFORESTATION DETAIL SHEET
SIGN-1 THRU SIGN-4	SIGNING PLANS
UO-1 THRU UO-10	UTILITIES BY OTHERS
X	CROSS-SECTION INDEX
X-A(1) THRU X-A(2)	CROSS-SECTION SUMMARY
X-1 THRU X-113	CROSS-SECTIONS
S-0 THRU S-17	STRUCTURE PLANS

**GENERAL NOTES:**

2012 SPECIFICATIONS  
EFFECTIVE: 01-17-2012  
REVISED: 10-31-2014

**GRADING AND SURFACING:**

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

**CLEARING:**

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

**SUPERELEVATION:**

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

**SHOULDER CONSTRUCTION:**

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

**SIDE ROADS:**

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

**BERM DITCHES:**

BERM DITCHES SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 240.01 AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

**GUARDRAIL:**

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

**TEMPORARY SHORING:**

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC NOT SHOWN ON THE PLANS WILL BE PAID FOR AT THE CONTRACT PRICE FOR "TEMPORARY SHORING".

**UTILITIES:**

UTILITY OWNERS ON THIS PROJECT ARE FRONTIER COMMUNICATIONS AND BLUE RIDGE MOUNTAIN EMC.

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

**RIGHT OF WAY MARKERS**

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS

**ROCK:**

ROCK IS ANTICIPATED BETWEEN -L- 58+50-60+00, 64+50-68+00, & 78+50-82+50. BLASTING MAY BE REQUIRED FOR EXCAVATION ON THE PROJECT. SEE SECTION 220 OF THE STANDARD SPECIFICATIONS AND IF APPLICABLE, ROCK BLASTING PROVISION.

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO.	TITLE
DIVISION 2 - EARTHWORK	
200.02	Method of Clearing - Method II
225.02	Guide for Grading Subgrade - Secondary and Local
225.04	Method of Obtaining Superelevation - Two Lane Pavement
225.06	Method of Grading Sight Distance at Intersections
240.01	Guide for Berm Ditch Construction
DIVISION 3 - PIPE CULVERTS	
300.01	Method of Pipe Installation
310.10	Driveway Pipe Construction
DIVISION 5 - SUBGRADE, BASES AND SHOULDERS	
560.01	Method of Shoulder Construction - High Side of Superelevated Curve - Method I
DIVISION 6 - ASPHALT BASES AND PAVEMENTS	
654.01	Pavement Repairs
DIVISION 8 - INCIDENTALS	
838.01	Concrete Endwall for Single and Double Pipe Culverts - 15" thru 48" Pipe 90 Skew
838.11	Brick Endwall for Single and Double Pipe Culverts - 15" thru 48" Pipe 90 Skew
838.80	Precast Endwalls - 12" thru 72" Pipe 90 Skew
840.00	Concrete Base Pad for Drainage Structures
840.17	Concrete Grated Drop Inlet Type 'A' - 12" thru 72" Pipe
840.18	Concrete Grated Drop Inlet Type 'B' - 12" thru 36" Pipe
840.19	Concrete Grated Drop Inlet Type 'D' - 12" thru 36" Pipe
840.24	Frames and Narrow Slot Sag Grates
840.25	Anchorage for Frames - Brick or Concrete or Precast
840.26	Brick Grated Drop Inlet Type 'A' - 12" thru 72" Pipe
840.27	Brick Grated Drop Inlet Type 'B' - 12" thru 36" Pipe
840.28	Brick Grated Drop Inlet Type 'D' - 12" thru 36" Pipe
840.29	Frames and Narrow Slot Flat Grates
840.31	Concrete Junction Box - 12" thru 66" Pipe
840.32	Brick Junction Box - 12" thru 66" Pipe
840.34	Traffic Bearing Junction Box - for Use with Pipes 42" and Under
840.35	Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates
840.41	Spring Box - Concrete or Brick
840.45	Precast Drainage Structure
840.46	Traffic Bearing Precast Drainage Structure
840.54	Manhole Frame and Cover
840.66	Drainage Structure Steps
846.01	Concrete Curb, Gutter and Curb & Gutter
846.02	Drop Inlet Installation in Expressway Gutter
846.04	Drop Inlet Installation in Shoulder Berm Gutter
850.01	Concrete Paved Ditches
850.10	Guide for Berm Drainage Outlet - 15" and 18" Pipe
850.11	Guide for Berm Drainage Outlet - 24" and 30" Pipe
862.01	Guardrail Placement
862.02	Guardrail Installation
862.03	Structure Anchor Units
866.02	Woven Wire Fence - with Wood Post
866.04	Barbed Wire Fence with Wood Posts (2 - 7 Strands)
876.01	Rip Rap in Channels
876.02	Guide for Rip Rap at Pipe Outlets
876.04	Drainage Ditches with Class 'B' Rip Rap

8/17/09















**Note: Not to Scale**

\*S.U.E. = *Subsurface Utility Engineering*












STATE OF NORTH CAROLINA  
DIVISION OF HIGHWAYS

# CONVENTIONAL PLAN SHEET SYMBOLS












## BOUNDARIES AND PROPERTY:

State Line	-----
County Line	-----
Township Line	-----
City Line	-----
Reservation Line	-----
Property Line	-----
Existing Iron Pin	----- 
Property Corner	----- 
Property Monument	----- 
Parcel/Sequence Number	----- 
Existing Fence Line	----- 
Proposed Woven Wire Fence	----- 
Proposed Chain Link Fence	----- 
Proposed Barbed Wire Fence	----- 
Existing Wetland Boundary	----- 
Proposed Wetland Boundary	----- 
Existing Endangered Animal Boundary	----- 
Existing Endangered Plant Boundary	----- 
Known Soil Contamination: Area or Site	----- 
Potential Soil Contamination: Area or Site	----- 

## BUILDINGS AND OTHER CULTURE:

Gas Pump Vent or U/G Tank Cap	----- 
Sign	----- 
Well	----- 
Small Mine	----- 
Foundation	----- 
Area Outline	----- 
Cemetery	----- 
Building	----- 
School	----- 
Church	----- 
Dam	----- 



















## HYDROLOGY:

Stream or Body of Water	----- 
Hydro, Pool or Reservoir	----- 
Jurisdictional Stream	----- 
Buffer Zone 1	----- 
Buffer Zone 2	----- 
Flow Arrow	----- 
Disappearing Stream	----- 
Spring	----- 
Wetland	----- 
Proposed Lateral, Tail, Head Ditch	----- 
False Sump	----- 
















## RAILROADS:

Standard Gauge	----- 
RR Signal Milepost	----- 
Switch	----- 
RR Abandoned	----- 
RR Dismantled	----- 


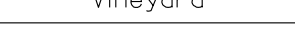
## RIGHT OF WAY:

Baseline Control Point	----- 
Existing Right of Way Marker	----- 
Existing Right of Way Line	----- 
Proposed Right of Way Line	----- 
Proposed Right of Way Line with Iron Pin and Cap Marker	----- 
Proposed Right of Way Line with Concrete or Granite RW Marker	----- 
Proposed Control of Access Line with Concrete CA Marker	----- 
Existing Control of Access	----- 
Proposed Control of Access	----- 
Existing Easement Line	----- 
Proposed Temporary Construction Easement	----- 
Proposed Temporary Drainage Easement	----- 
Proposed Permanent Drainage Easement	----- 
Proposed Permanent Drainage / Utility Easement	----- 
Proposed Permanent Utility Easement	----- 
Proposed Temporary Utility Easement	----- 
Proposed Aerial Utility Easement	----- 
Proposed Permanent Easement with Iron Pin and Cap Marker	----- 


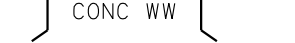
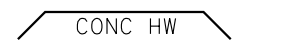

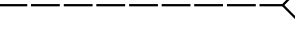
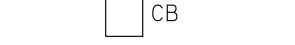



## ROADS AND RELATED FEATURES:

Existing Edge of Pavement	----- 
Existing Curb	----- 
Proposed Slope Stakes Cut	----- 
Proposed Slope Stakes Fill	----- 
Proposed Curb Ramp	----- 
Existing Metal Guardrail	----- 
Proposed Guardrail	----- 
Existing Cable Guiderail	----- 
Proposed Cable Guiderail	----- 
Equality Symbol	----- 
Pavement Removal	----- 
Single Tree	----- 
Single Shrub	----- 
Hedge	----- 
Woods Line	----- 












## VEGETATION:

Orchard	----- 
Vineyard	----- 














## EXISTING STRUCTURES:

MAJOR:	
Bridge, Tunnel or Box Culvert	----- 
Bridge Wing Wall, Head Wall and End Wall	----- 
MINOR:	
Head and End Wall	----- 
Pipe Culvert	----- 
Footbridge	----- 
Drainage Box: Catch Basin, DI or JB	----- 
Paved Ditch Gutter	----- 
Storm Sewer Manhole	----- 
Storm Sewer	----- 


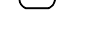





## UTILITIES:

POWER:	
Existing Power Pole	----- 
Proposed Power Pole	----- 
Existing Joint Use Pole	----- 
Proposed Joint Use Pole	----- 
Power Manhole	----- 
Power Line Tower	----- 
Power Transformer	----- 
U/G Power Cable Hand Hole	----- 
H-Frame Pole	----- 
Recorded U/G Power Line	----- 
Designated U/G Power Line (S.U.E.*)	----- 


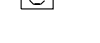






## TELEPHONE:

Existing Telephone Pole	----- 
Proposed Telephone Pole	----- 
Telephone Manhole	----- 
Telephone Booth	----- 
Telephone Pedestal	----- 
Telephone Cell Tower	----- 
U/G Telephone Cable Hand Hole	----- 
Recorded U/G Telephone Cable	----- 
Designated U/G Telephone Cable (S.U.E.*)	----- 
Recorded U/G Telephone Conduit	----- 
Designated U/G Telephone Conduit (S.U.E.*)	----- 
Recorded U/G Fiber Optics Cable	----- 
Designated U/G Fiber Optics Cable (S.U.E.*)	----- 






## WATER:

Water Manhole	----- 
Water Meter	----- 
Water Valve	----- 
Water Hydrant	----- 
Recorded U/G Water Line	----- 
Designated U/G Water Line (S.U.E.*)	----- 
Above Ground Water Line	----- 



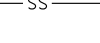
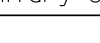


## TV:

TV Satellite Dish	----- 
TV Pedestal	----- 
TV Tower	----- 
U/G TV Cable Hand Hole	----- 
Recorded U/G TV Cable	----- 
Designated U/G TV Cable (S.U.E.*)	----- 
Recorded U/G Fiber Optic Cable	----- 
Designated U/G Fiber Optic Cable (S.U.E.*)	----- 


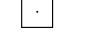

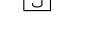

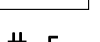






## GAS:

Gas Valve	----- 
Gas Meter	----- 
Recorded U/G Gas Line	----- 
Designated U/G Gas Line (S.U.E.*)	----- 
Above Ground Gas Line	----- 

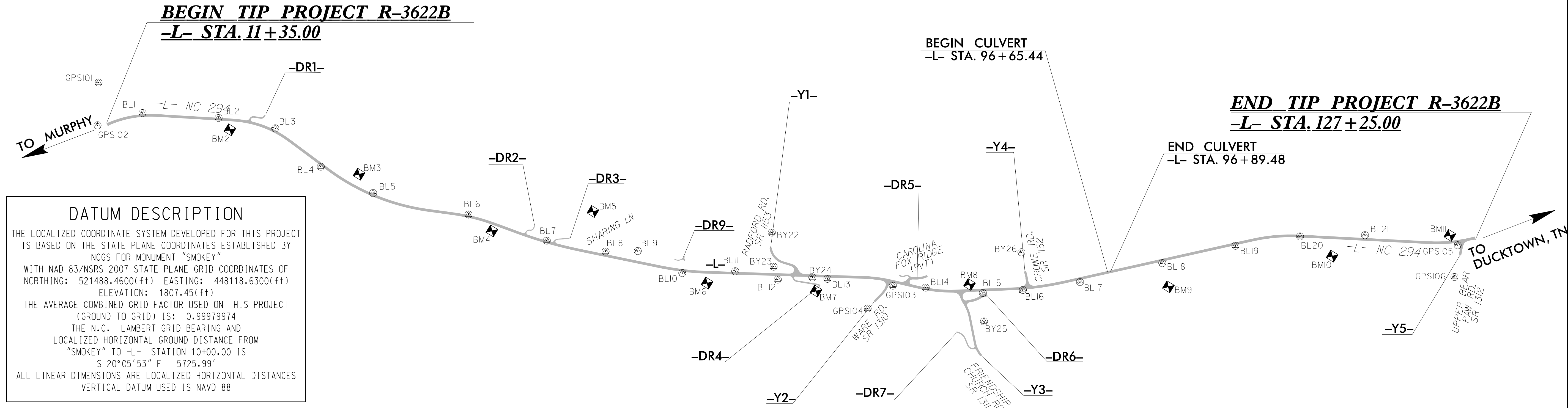
## SANITARY SEWER:

Sanitary Sewer Manhole	----- 
Sanitary Sewer Cleanout	----- 
U/G Sanitary Sewer Line	----- 
Above Ground Sanitary Sewer	----- 
Recorded SS Forced Main Line	----- 
Designated SS Forced Main Line (S.U.E.*)	----- 

## MISCELLANEOUS:

Utility Pole	----- 
Utility Pole with Base	----- 
Utility Located Object	----- 
Utility Traffic Signal Box	----- 
Utility Unknown U/G Line	----- 
U/G Tank; Water, Gas, Oil	----- 
Underground Storage Tank, Approx. Loc.	----- 
A/G Tank; Water, Gas, Oil	----- 
Geoenvironmental Boring	----- 
U/G Test Hole (S.U.E.*)	----- 
Abandoned According to Utility Records	----- 
End of Information	----- 

# SURVEY CONTROL SHEET R-3622B



### BASELINE DATA

BL							BY1						
POINT	DESC.	NORTH	EAST	ELEVATION	L STATION	OFFSET	POINT	DESC.	NORTH	EAST	ELEVATION	Y1 STATION	OFFSET
GPS102	GPS-102	516120.3300	450019.6587	1591.51	10+60.94	28.34 LT	BY22	BY-22	521310.9585	447862.9603	1825.74	OUTSIDE PROJECT LIMITS	
BL1	BL-1	516383.0242	449739.2084	1593.96	14+38.75	20.92 LT	BY23	BY-23	521470.7095	448094.3653	1809.90	11+22.73	65.46 RT
BL2	BL-2	516937.1029	449445.4691	1596.64	20+63.50	10.33 LT	BY24	BY-24	521788.7651	448002.9957	1774.15	OUTSIDE PROJECT LIMITS	
BL3	BL-3	517379.1091	449274.4887	1600.27	25+33.54	24.76 LT							
BL4	BL-4	517864.0485	449346.5083	1600.69	30+19.45	17.77 RT							
BL5	BL-5	518344.5709	449306.1925	1610.80	35+02.11	16.00 LT							
BL6	BL-6	519105.9806	449045.6393	1641.91	43+12.91	14.96 LT							
BL7	BL-7	519768.0565	448890.2807	1663.23	49+92.28	11.80 LT							
BL8	BL-8	520227.6549	448712.7440	1695.54	54+86.06	32.59 LT							
BL9	BL-9	520450.8535	448570.4003	1716.30	57+44.41	90.33 LT							
BL10	BL-10	520857.8514	448533.1796	1742.84	61+38.13	21.75 RT							
BL11	BL-11	521221.4639	448292.3159	1769.05	65+71.69	16.00 LT							
BL12	BL-12	521555.8558	448166.7511	1786.35	69+24.32	40.86 RT							
BL13	BL-13	521900.7454	447947.0865	1768.73	73+32.76	21.22 RT							
GPS103	GPS-103	522388.7173	447712.7217	1791.89	78+81.02	34.04 RT							
BL14	BL-14	522627.3231	447585.1054	1788.81	81+48.35	9.91 LT							
BL15	BL-15	523051.4142	447375.8733	1764.29	86+21.51	24.14 RT							
BL16	BL-16	523319.1122	447182.0673	1742.86	89+51.31	13.73 RT							
BL17	BL-17	523685.3381	446880.2061	1716.39	94+23.50	27.79 RT							
BL18	BL-18	524180.4464	446392.4640	1695.20	101+18.48	23.38 RT							
BL19	BL-19	524618.7361	445954.8748	1692.37	107+37.77	15.31 RT							
BL20	BL-20	525030.8359	445611.0944	1690.00	112+78.48	2.85 RT							
BL21	BL-21	525481.2416	445321.8763	1677.90	118+12.45	34.61 LT							
GPS105	GPS-105	526174.4484	444992.4680	1663.74	125+74.02	31.71 RT							
							BY2						
							POINT	DESC.	NORTH	EAST	ELEVATION	Y2 STATION	OFFSET
							GPS104	GPS-104	522310.7687	447982.42074	1779.69	OUTSIDE PROJECT LIMITS	
							BY3						
							POINT	DESC.	NORTH	EAST	ELEVATION	Y3 STATION	OFFSET
							BY25	BY-25	523182.3779	447571.4893	1793.82	12+90.34	107.32 LT
							BY4						
							POINT	DESC.	NORTH	EAST	ELEVATION	Y4 STATION	OFFSET
							BY26	BY-26	523146.1893	446923.7991	1730.91	OUTSIDE PROJECT LIMITS	
							BY5						
							POINT	DESC.	NORTH	EAST	ELEVATION	Y5 STATION	OFFSET
							GPS106	GPS-106	526289.1314	445228.9723	1663.87	OUTSIDE PROJECT LIMITS	

### BENCHMARK DATA

BM2 ELEVATION - 1606.54 N 517068 E 449480 L3 STATION 21+63.00 82 RIGHT RR SPIKE IN 30" POPLAR	BM5 ELEVATION - 1677.38 N 519966 E 448487 L3 STATION 53+17.00 334 LEFT RR SPIKE IN 14" PINE	BM8 ELEVATION - 1764.92 N 522921 E 447374 L3 STATION 85+14.00 50 LEFT RR SPIKE IN 18" POPLAR	BM11 ELEVATION - 1665.59 N 526077 E 444957 L3 STATION 125+19.00 56 LEFT RR SPIKE IN 24" CHERRY
BM3 ELEVATION - 1599.76 N 518165 E 449234 L3 STATION 33+24.00 105 LEFT RR SPIKE IN 36" POPLAR	BM6 ELEVATION - 1765.73 N 521075 E 448496 L3 STATION 63+44.00 89 RIGHT RR SPIKE IN 15" WHITE OAK	BM9 ELEVATION - 1702.98 N 524326 E 446532 L3 STATION 101+25.00 225 RIGHT RR SPIKE IN 36" TRIPLE POPLAR	
BM4 ELEVATION - 1659.11 N 519341 E 449059 L3 STATION 45+39.00 60 RIGHT RR SPIKE IN 12" HICKORY	BM7 ELEVATION - 1780.91 N 521871 E 448082 L3 STATION 72+40.00 124 RIGHT RR SPIKE IN 27" MAPLE	BM10 ELEVATION - 1673.73 N 525342 E 445611 L3 STATION 115+52.00 152 RIGHT RR SPIKE IN 24" CHERRY	

### NOTES:

- THE CONTROL DATA FOR THIS PROJECT CAN BE FOUND ELECTRONICALLY BY SELECTING PROJECT CONTROL DATA AT:  
[HTTPS://CONNECT.NCDOT.GOV/RESOURCES/LOCATION/](https://connect.ncdot.gov/resources/location/)
- THE FILES TO BE FOUND ARE AS FOLLOWS:  
R-3622B\_LS\_CONTROL\_DATA.HTM
- SITE CALIBRATION INFORMATION HAS NOT BEEN PROVIDED FOR THIS PROJECT. IF FURTHER INFORMATION IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.
- © INDICATES GEODETIC CONTROL MONUMENTS USED OR SET FOR HORIZONTAL PROJECT CONTROL BY THE NCDOT LOCATION AND SURVEYS UNIT.
- PROJECT CONTROL ESTABLISHED USING GLOBAL POSITIONING SYSTEM.
- NETWORK ESTABLISHED FROM NGS ONLINE POSITIONING SERVICE (OPUS)
- SEE GPS CALIBRATION SHEET FOR HORIZONTAL AND VERTICAL COORDINATE VALUES.

NOTE: DRAWING NOT TO SCALE

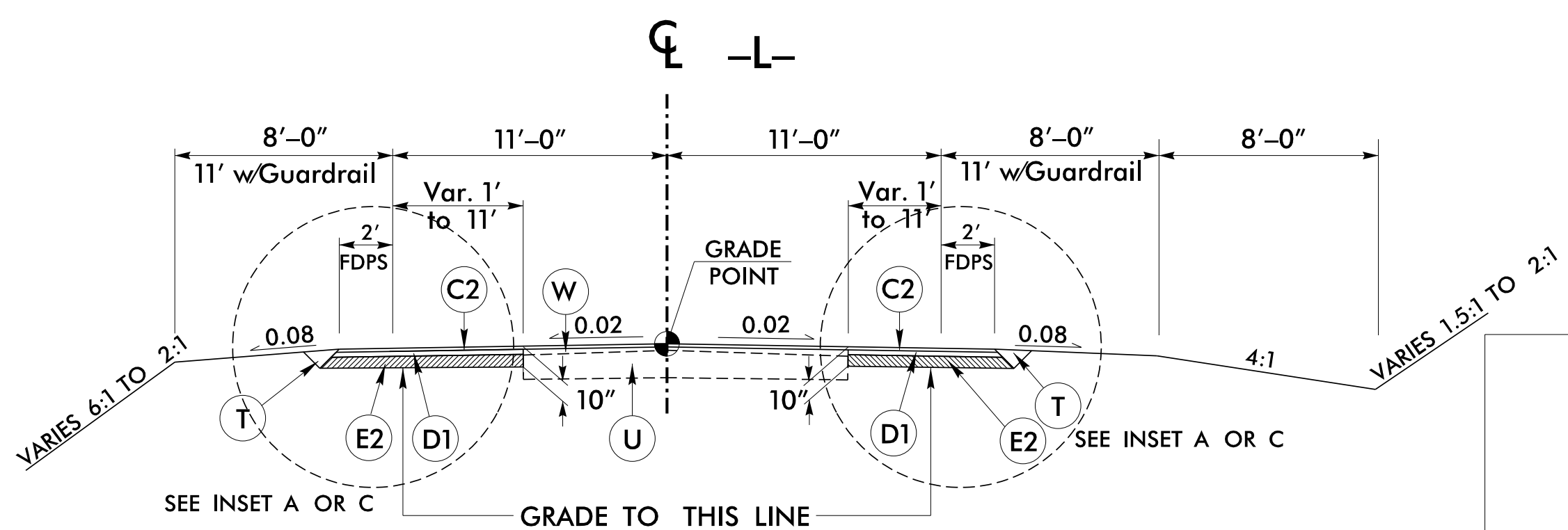






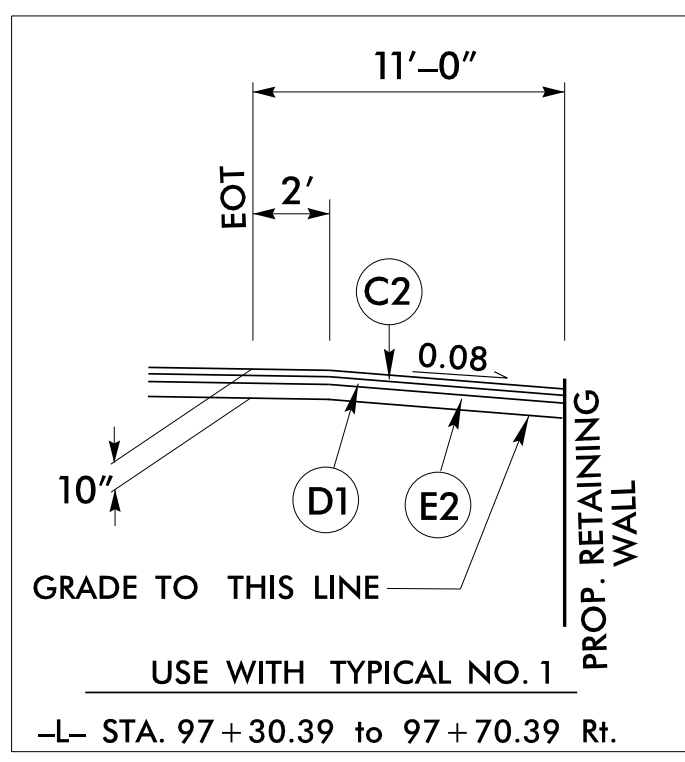


6/2/2015

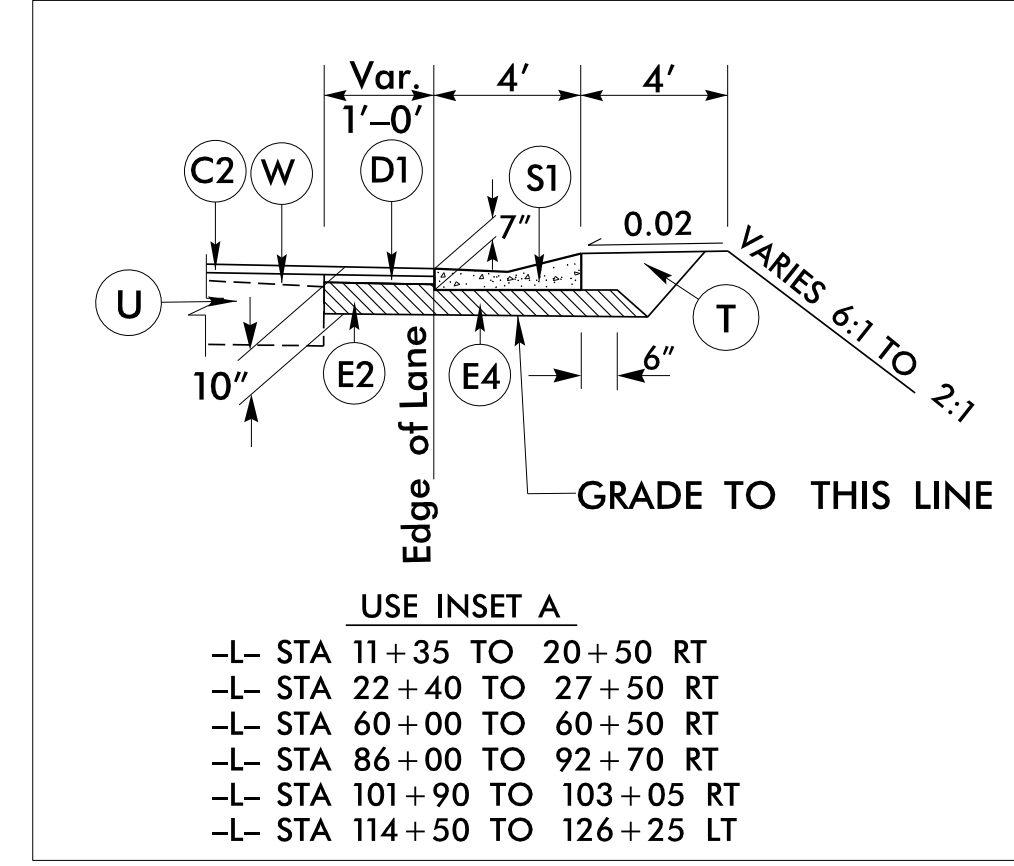


**TYPICAL SECTION NO. 1**

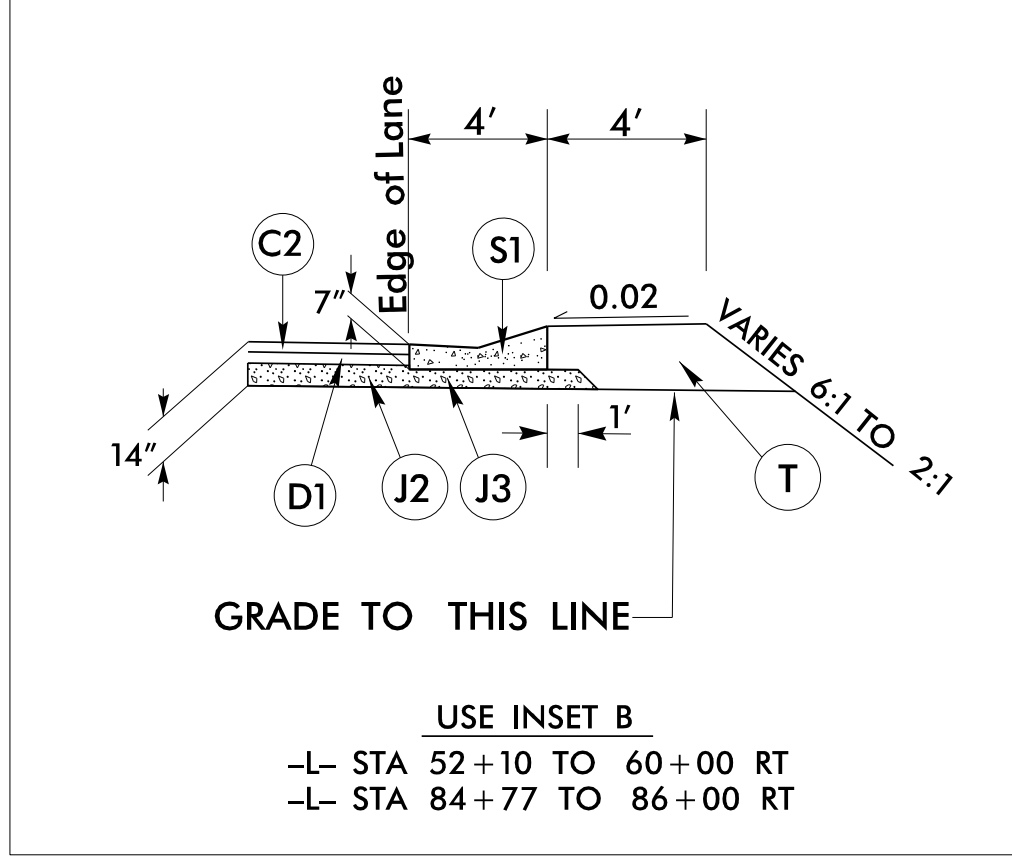
- USE TYPICAL SECTION NO. 1
- L- STA. 11+35 TO 27+50
  - L- STA. 32+50 TO 52+00
  - L- STA. 60+00 TO 61+50
  - L- STA. 62+50 TO 64+00
  - L- STA. 86+00 TO 127+25



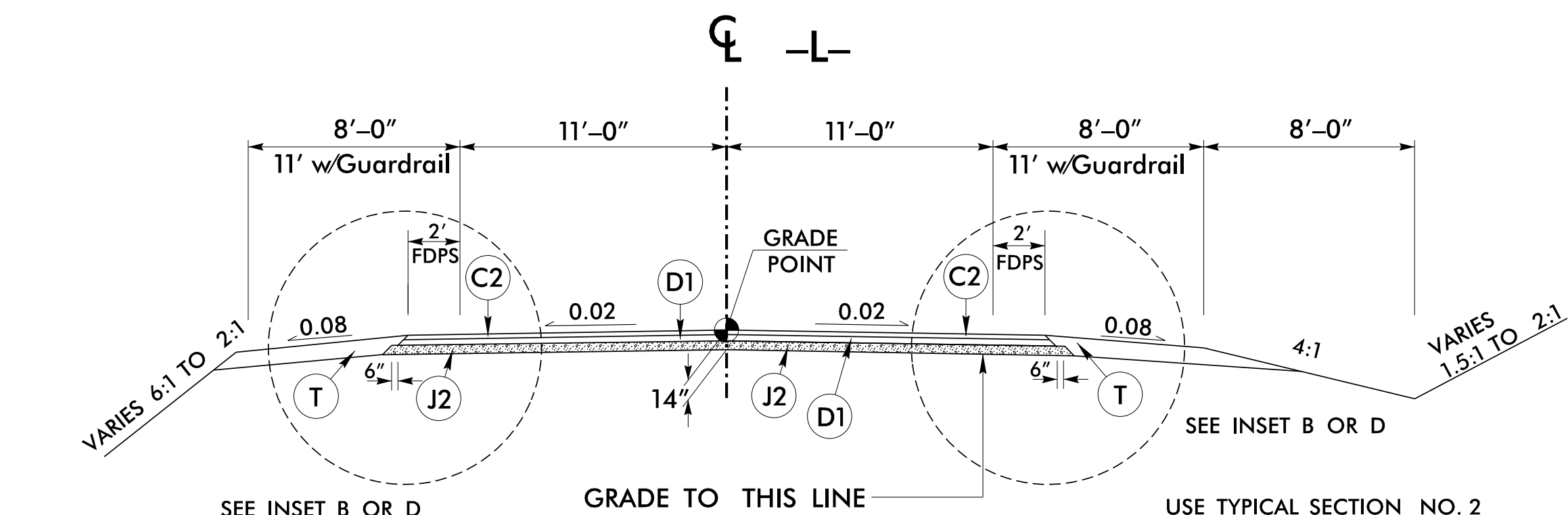
USE WITH TYPICAL NO. 1  
-L- STA. 97+30.39 TO 97+70.39 Rt.



- USE INSET A
- L- STA 11+35 TO 20+50 RT
  - L- STA 22+40 TO 27+50 RT
  - L- STA 60+00 TO 60+50 RT
  - L- STA 86+00 TO 92+70 RT
  - L- STA 101+90 TO 103+05 RT
  - L- STA 114+50 TO 126+25 LT

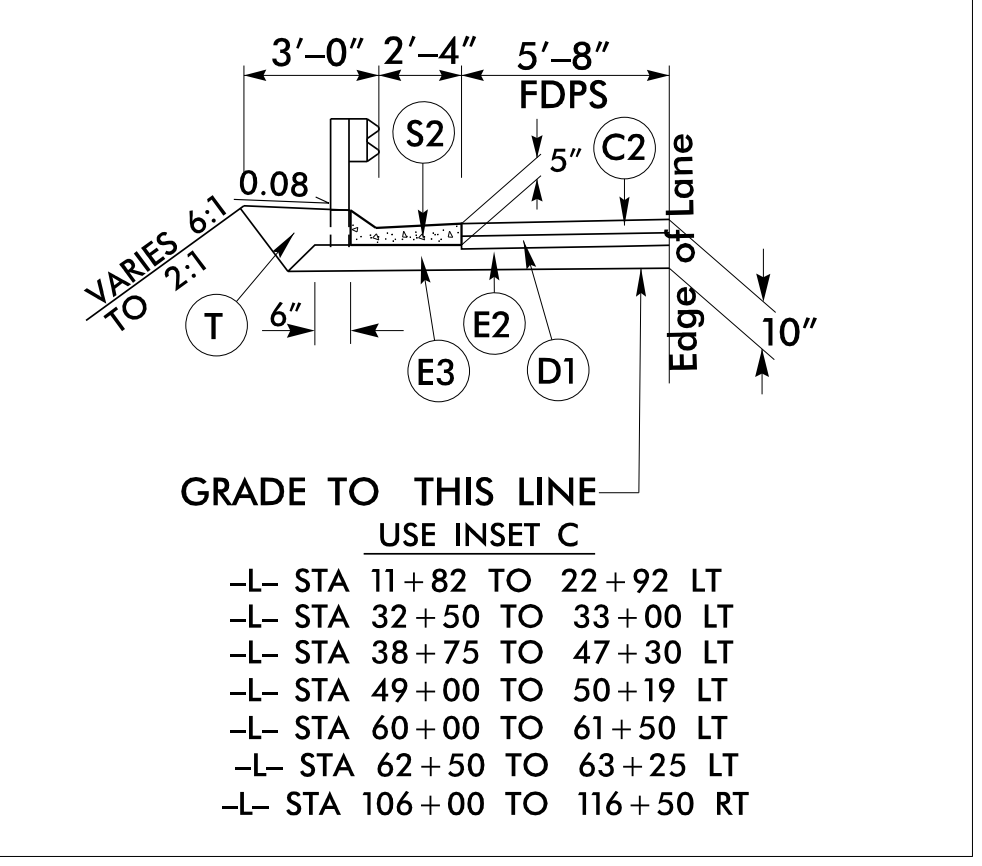


- USE INSET B
- L- STA 52+10 TO 60+00 RT
  - L- STA 84+77 TO 86+00 RT

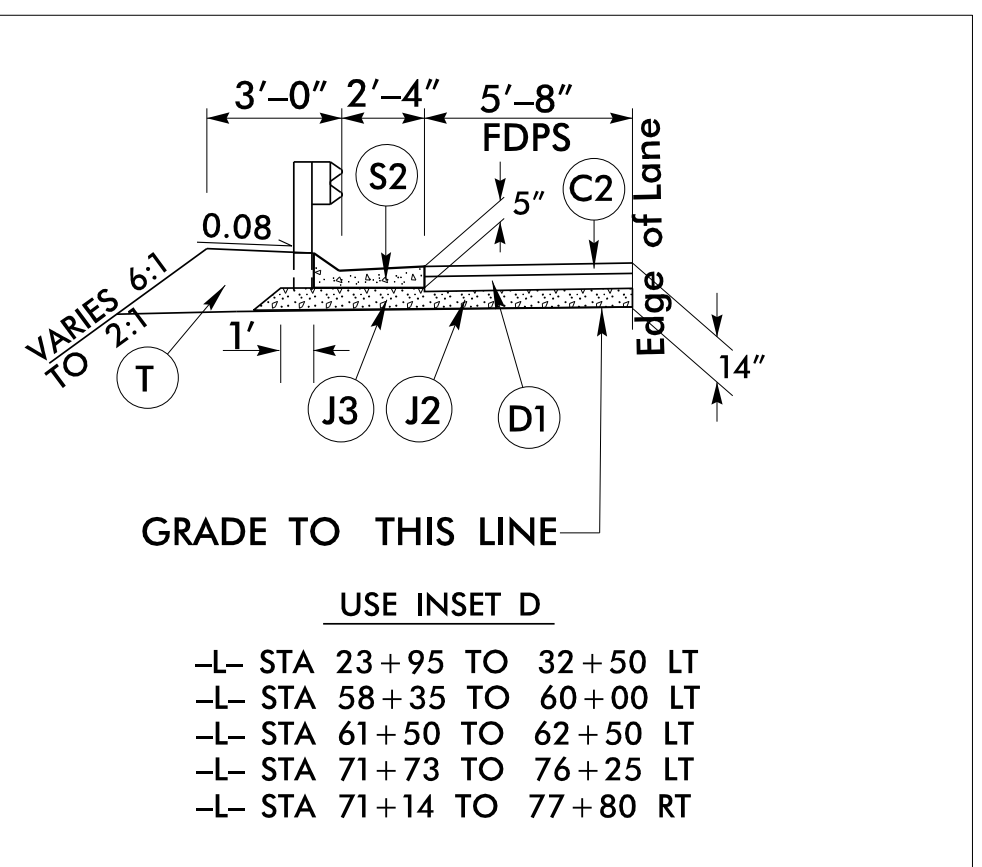


**TYPICAL SECTION NO. 2**

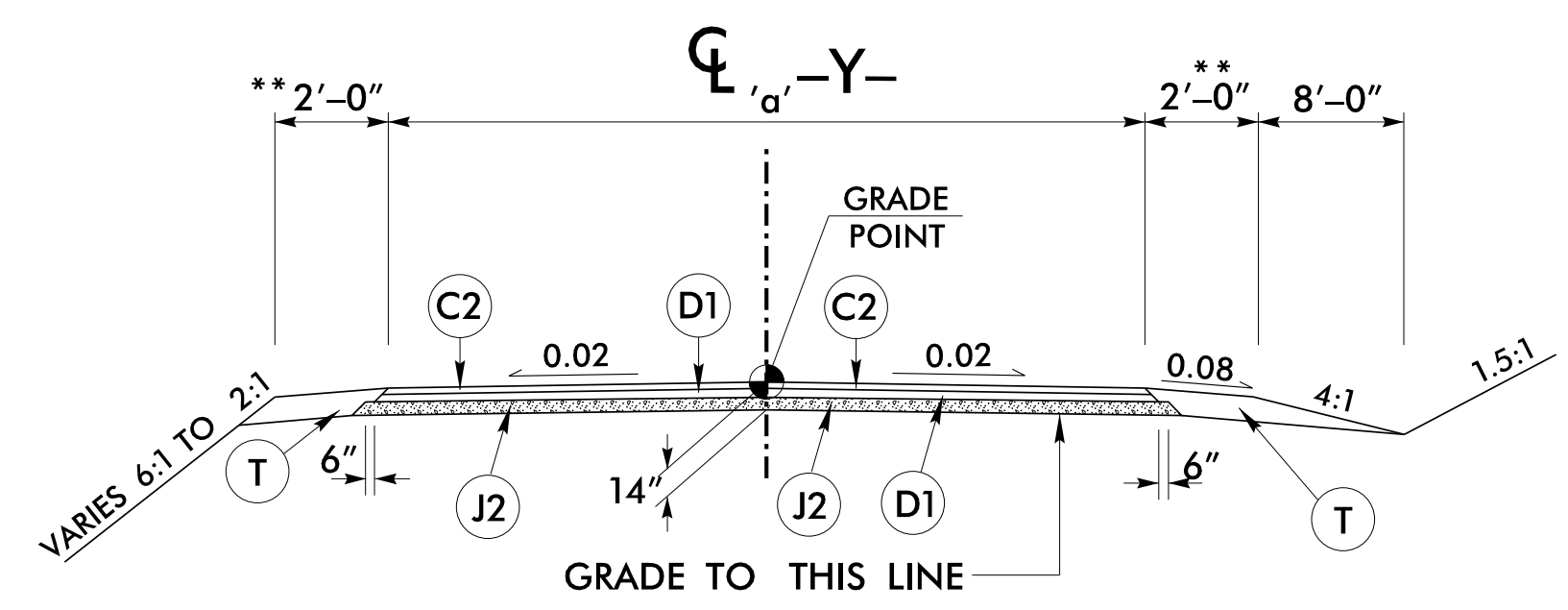
- USE TYPICAL SECTION NO. 2
- L- STA. 27+50 TO 32+50
  - L- STA. 52+00 TO 60+00
  - L- STA. 61+50 TO 62+50
  - L- STA. 64+00 TO 86+00



- USE INSET C
- L- STA 11+82 TO 22+92 LT
  - L- STA 32+50 TO 33+00 LT
  - L- STA 38+75 TO 47+30 LT
  - L- STA 49+00 TO 50+19 LT
  - L- STA 60+00 TO 61+50 LT
  - L- STA 62+50 TO 63+25 LT
  - L- STA 106+00 TO 116+50 RT



- USE INSET D
- L- STA 23+95 TO 32+50 LT
  - L- STA 58+35 TO 60+00 LT
  - L- STA 61+50 TO 62+50 LT
  - L- STA 71+73 TO 76+25 LT
  - L- STA 71+14 TO 77+80 RT



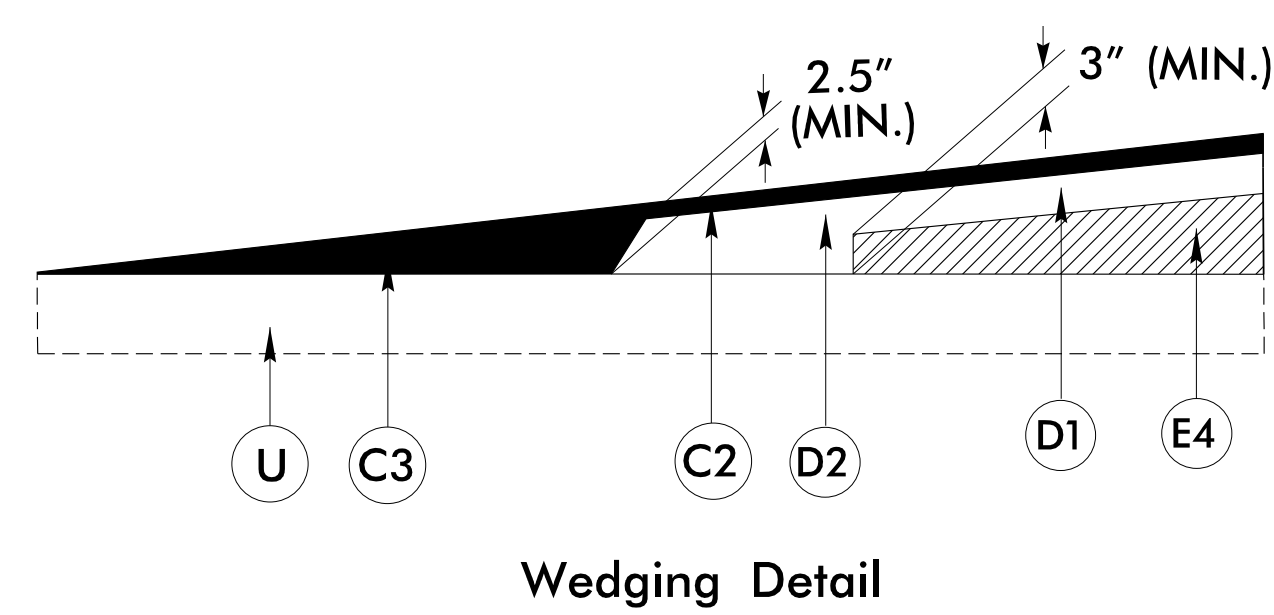
**TYPICAL SECTION NO. 3**

DIMENSION 'a'

LINE	WIDTH
-Y1-	18.0'
-Y2-	18.0'
-Y3-	18.0'
-Y4-	18.0'
-Y5-	20.0'
-DR1-	12.0'
-DR2-	8.0'
-DR3-	8.0'
-DR4-	12.0'
-DR5-	20.0'
-DR5A-	10.0'
-DR6-	20.0'
-DR7-	10.0'
-DR8-	12.0'

\*\* TOTAL SHOULDER WIDTH TO BE INCREASED TO 7' WHERE GUARDRAIL IS USED.

- USE TYPICAL SECTION NO. 3
- Y1- STA. 10+00 TO 13+38.56
  - Y2- STA. 10+11 TO 12+15.00
  - Y3- STA. 10+11 TO 15+65.00
  - DR1- STA. 10+40 TO 11+96.77
  - DR2- STA. 10+10 TO 11+25.49
  - DR3- STA. 10+25 TO 11+75.51
  - DR4- STA. 10+11 TO 12+60.00
  - DR5- STA. 10+00 TO 11+93.91
  - DR5A- STA. 10+00 TO 11+00.14
  - DR6- STA. 10+09 TO 12+00.00
  - DR7- STA. 10+50 TO 12+75.44
  - DR8- STA. 10+05 TO 10+92.72



Wedging Detail

PAVEMENT SCHEDULE	
A	PROP. 6" JOINTED CONCRETE PAVEMENT WITH 4X4 W3 X W3 WIRE REINFORCEMENT
C1	PROP. APPROX. 1.5" ASPHALT CONC. SURFACE COURSE, TYPE S9.5B AT AN AVGERAGE RATE OF 168 LBS. PER SQ. YD. IN ONE LAYER.
C2	PROP. APPROX. 3.0" ASPHALT CONC. SURFACE COURSE, TYPE S9.5B AT AN AVERAGE RATE OF 168 LBS PER SQ. YD. IN EACH OF TWO LAYERS.
C3	PROP.VAR. DEPTH ASPHALT CONC. SURFACE COURSE, TYPE S9.5B AT AN AVG. RATE OF 112 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 1.5" NOR GREATER THAN 2.0" IN DEPTH
D1	PROP. APPROX. 3.0" IN ASPHALT CONC. INTERMEDIATE COURSE, TYPE 119.0B AT AN AVG. RATE OF 342 LBS PER SQ. YD.
D2	PROP.VAR. DEPTH ASPHALT CONC. INTERMEDIATE COURSE, TYPE 119.0B AT AN AVG. RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 2.5" NOR GREATER THAN 4.0" IN DEPTH
E1	PROP. APPROX. 3" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 342 LBS. PER SQ. YD.
E2	PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E3	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 570 LBS. PER SQ. YD.
E4	PROP.VAR. DEPTH ASPHALT CONC. BASE COURSE, TYPE B25.0B AT AN AVG. RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3.0" NOR GREATER THAN 5.5" IN DEPTH
J1	4" AGGREGATE BASE COURSE
J2	8" AGGREGATE BASE COURSE
J3	VARIABLE DEPTH AGGREGATE BASE COURSE
P	PRIME COAT AT THE RATE OF 0.35 GALS. PER SQ. YD.
S1	EXPRESSWAY GUTTER (NCDOT STANDARD DWG 846.01)
S2	SHOULDER BERM GUTTER (NCDOT STANDARD DWG 846.01)
T	EARTH MATERIAL
U	EXISTING PAVEMENT
W	WEDGING (SEE DETAIL)

PAVEMENT EDGE SLOPES ARE 1:1 UNLESS OTHERWISE NOTED.

PROJECT REFERENCE NO. <b>R-3622B</b>	SHEET NO. <b>2A-1</b>
RW SHEET NO.	
ROADWAY DESIGN ENGINEER <b>Reece Schuler</b>	PAVEMENT DESIGN ENGINEER <b>Robert Cray</b>
<b>V&amp;M</b> Vaughn & Melton Consulting Engineers	
Asheville, North Carolina 828 253 2796	Charlotte, North Carolina 704 357 0488
	Tri-Cities, Tennessee 423 467 8401
	Knoxville, Tennessee 865 546 5800
	Middlesboro, Kentucky 606 248 6600
	Spartanburg, South Carolina 864 574 4775
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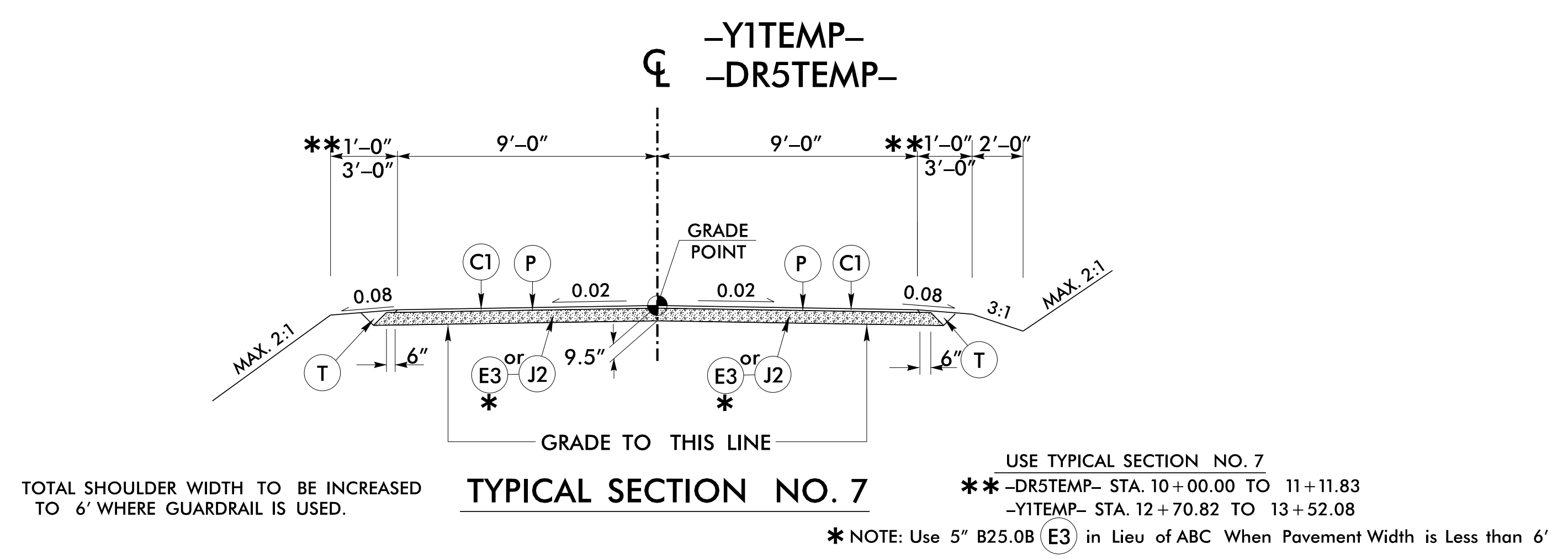
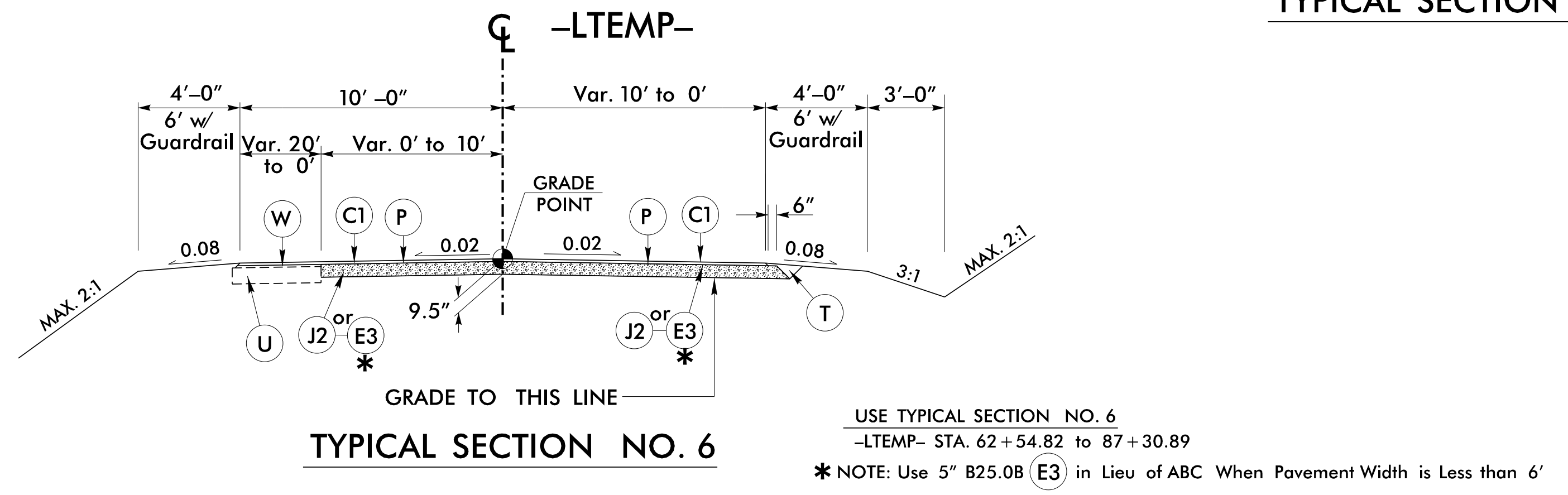
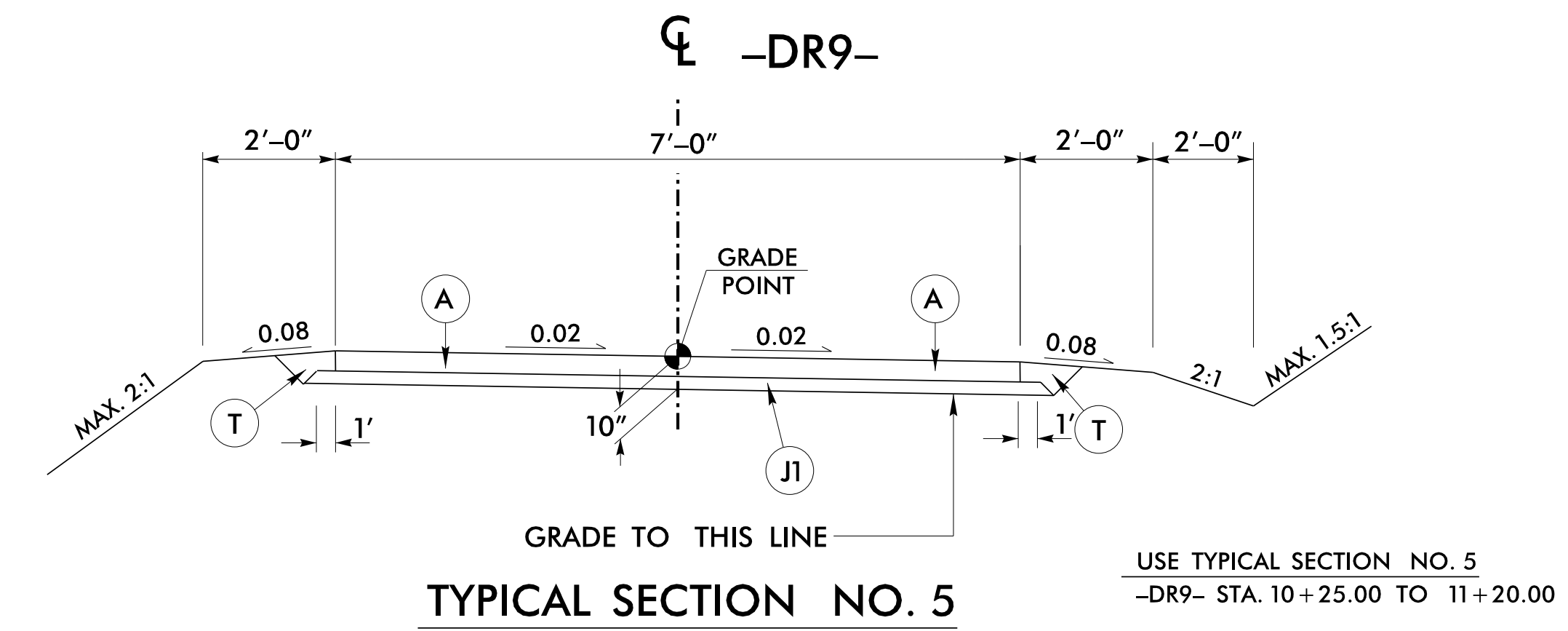
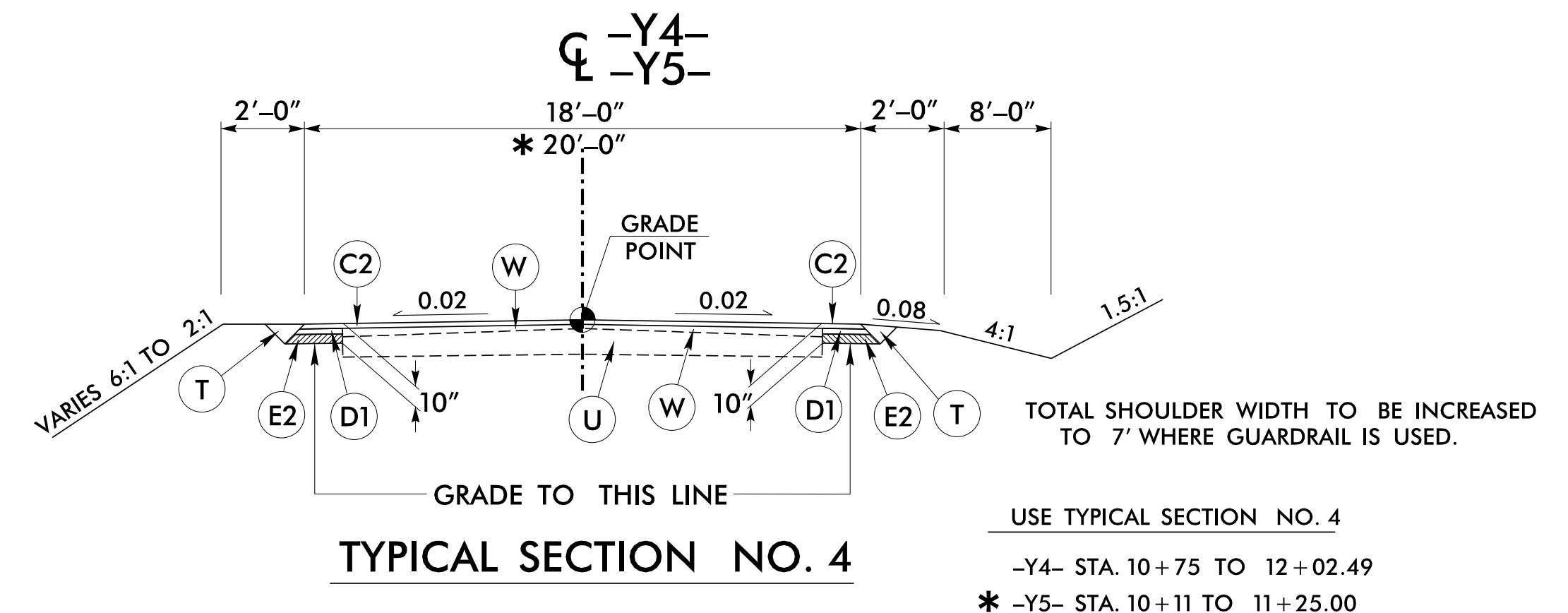
6/2/2015

PROJECT REFERENCE NO. <b>R-3622B</b>	SHEET NO. <b>2A-2</b>
ROADWAY DESIGN ENGINEER 	PAVEMENT DESIGN ENGINEER 

**V&M**  
Vaughn & Melton  
Consulting Engineers

Charlotte, North Carolina 704-351-0488  
Tri-Cities, Tennessee 423-467-8401  
Knoxville, Tennessee 865-546-5800  
Middlesboro, Kentucky 606-248-6600  
Asheville, North Carolina 828-253-2796  
Spartanburg, South Carolina 864-574-4775

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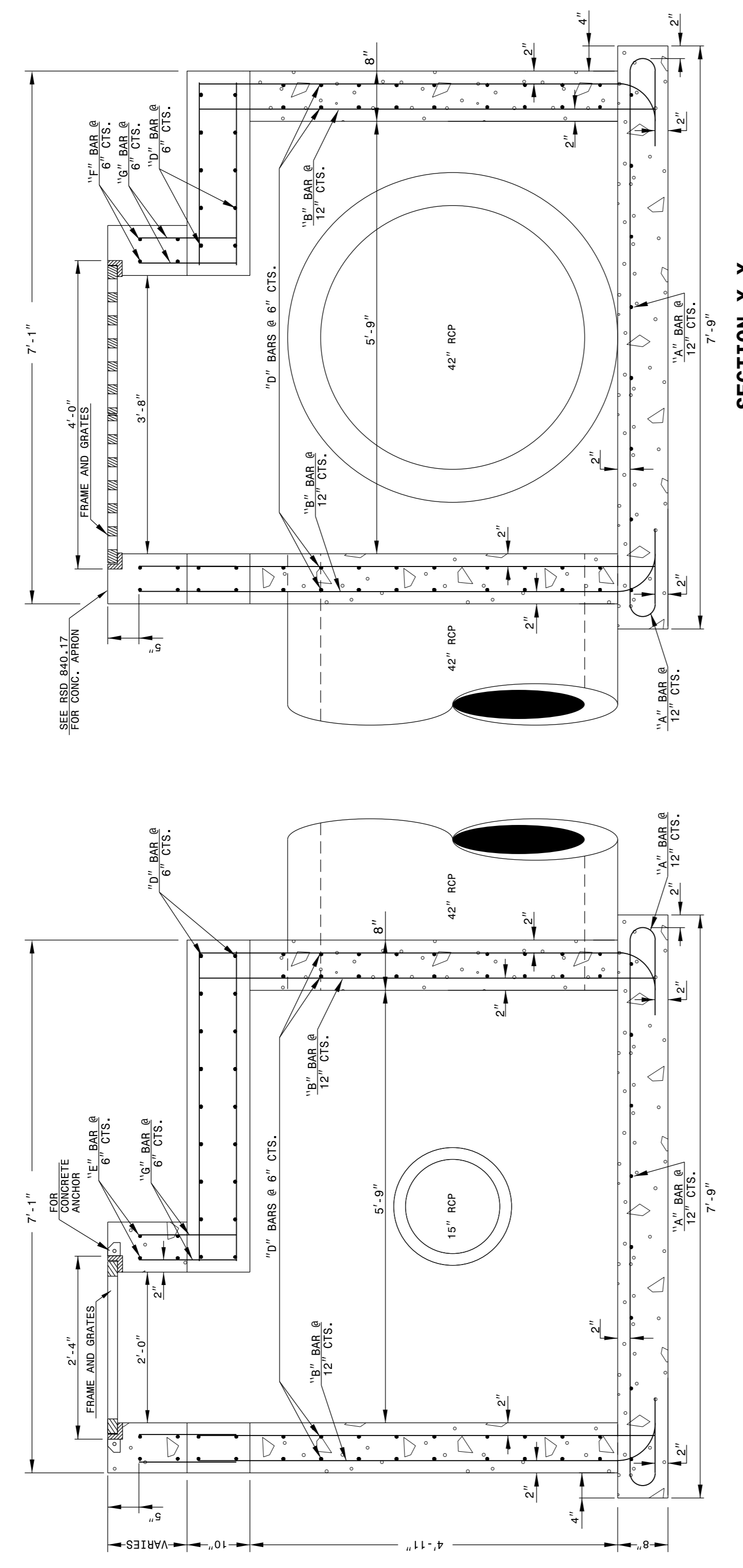


A	6" JOINTED CONCRETE w/WIRE MESH
C1	1.5" S9.5B
C2	3" S9.5B
C3	VARIABLE DEPTH S9.5B
D1	3" I19.0B
D2	VARIABLE DEPTH I19.0B
E1	3" B25.0B
E2	4" B25.0B
E3	5" B25.0B
E4	VARIABLE DEPTH B25.0B
J1	4" ABC
J2	8" ABC
J3	VARIABLE DEPTH ABC
S1	EXPRESSWAY GUTTER
S2	SHOULDER BERM GUTTER
P	PRIME COAT
T	EARTH MATERIAL
U	EXISTING PAVEMENT
W	ASPHALT WEDGING

STATE OF NORTH CAROLINA  
DEPT. OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR  
**TRAFFIC BEARING DROP INLET**  
FOR FRAME AND GRATES

SHEET 1 OF 2  
**840D35**



**GENERAL NOTES:**  
 -CHAMFER ALL EXPOSED CONCRETE CORNERS 3".  
 -USE FORMS TO CONSTRUCT THE BOTTOM SLAB.  
 -IF PIPES ARE SET IN THE BASE, FOLLOW CONSTRUCTION PROCEDURES FOR THE STANDARD. NO CONCRETE SHALL BE MADE OF CLASS "A". CONCRETE MAY BE USED IN LIEU CAST IN PLACE CONCRETE.  
 -REFERENCE STD. DWG. 840.25 FOR FRAME ANCHORAGE.  
 -PROVIDE DROP INLETS OVER 3'-6" DEEP WITH STEPS AS SHOWN.  
 -FRAME AND GRATES SHALL BE SEPARATE CONTRACT ITEM.

**NOTES:**  
 -ALL DIMENSIONS AND VERTICAL DIMENSIONS MAY BE ADJUSTED AS THE FIELD CONDITIONS AND/OR ALTERNATE DESIGN REQUIRE.  
 -ALL ADJUSTMENTS ARE TO BE MADE AS DIRECTED BY THE ENGINEER.

STATE OF NORTH CAROLINA  
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DIVISION OF HIGHWAYS  
RALEIGH, N.C.

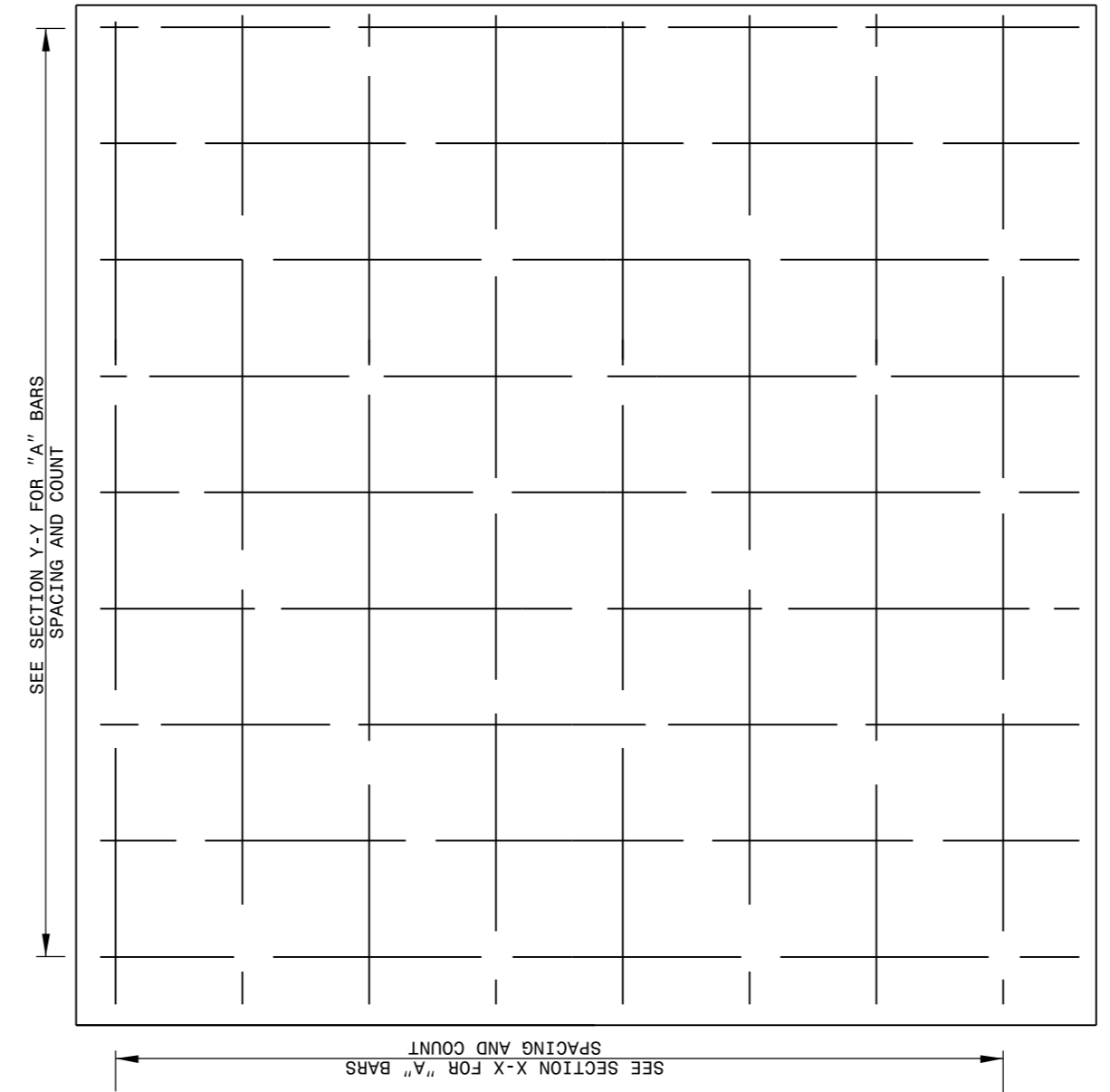
ENGLISH DETAIL DRAWING FOR  
**TRAFFIC BEARING DROP INLET**  
FOR FRAME AND GRATES

SHEET 1 OF 2  
**840D35**

STATE OF NORTH CAROLINA  
DEPT. OF TRANSPORTATION  
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RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR  
**TRAFFIC BEARING DROP INLET**  
FOR FRAME AND GRATES

SHEET 2 OF 2  
**840D35**

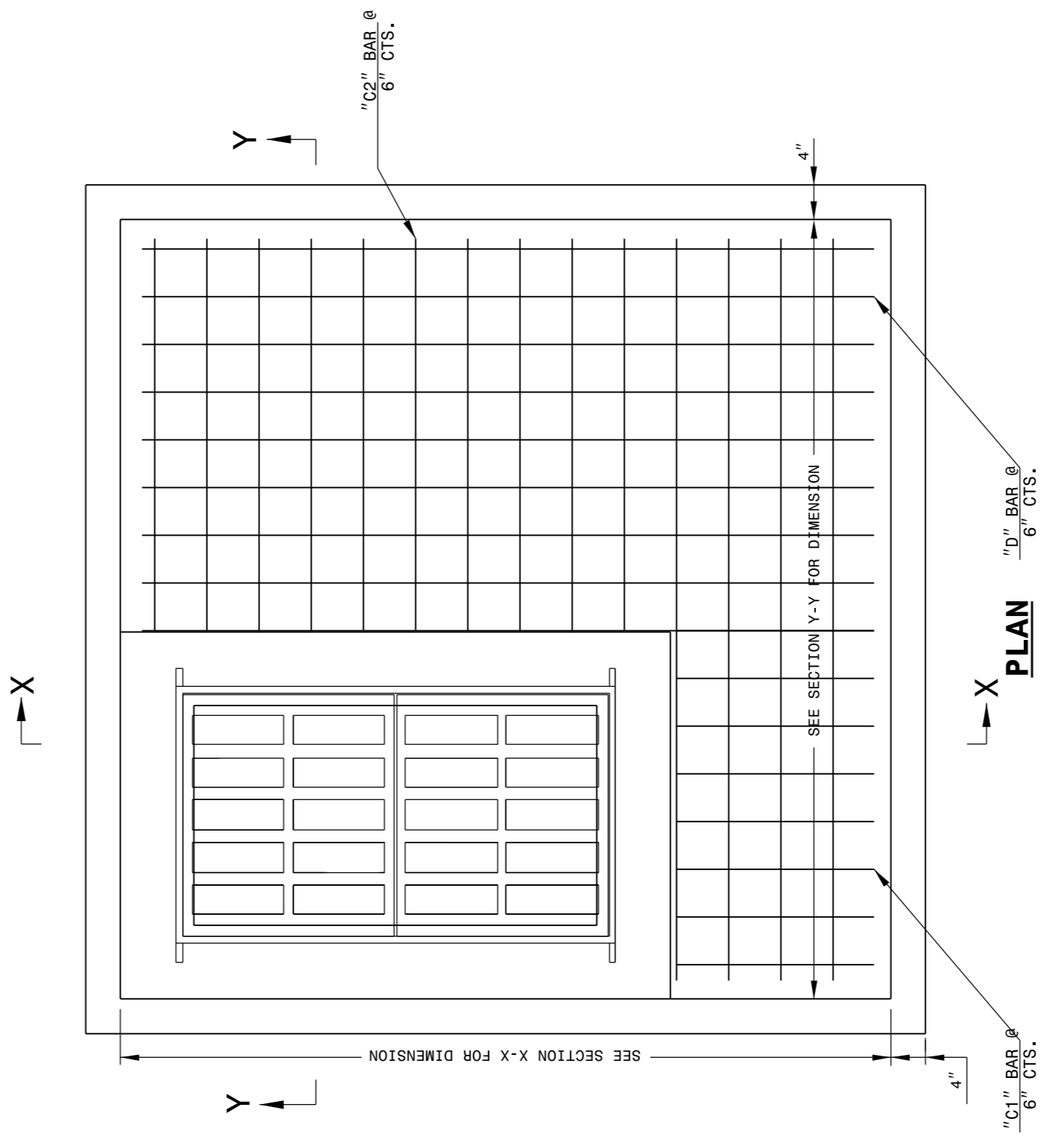


**PLAN OF BASE**

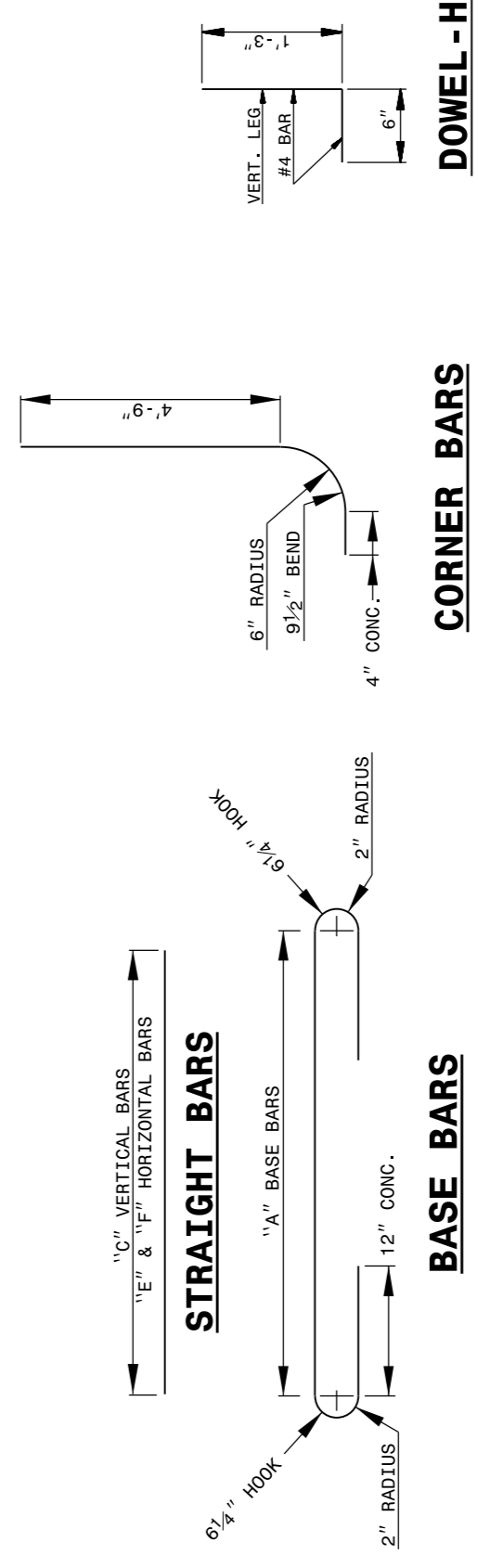
**BILL OF MATERIALS**

BAR	SIZE	LENGTH	QUANTITY	WEIGHT
A	#5	10'-0"	6	174
B	#5	5'-11"	64	395
C1	#5	3'-11"	12	39
C2	#5	4'-10"	18	676
D	#5	6'-9"	96	39
E	#5	4'-8"	8	25
F	#5	3'-0"	8	25
G	#5	1'-8"	42	1512
REINF. STEEL (TOTAL WEIGHT LBS.)				5.03
CONCRETE TOTAL (CU. YDS.)				5.03
NO DEDUCTIONS HAVE BEEN MADE TO ACCOMMODATE PIPES				

FOR EVERY 1 FOOT OF RISER USE 0.41 CU. YDS CONCRETE AND 390 LBS STEEL.



**PLAN**



**CORNER BARS**

**BASE BARS**

**STRAIGHT BARS**

**DOWEL-H**

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RALEIGH, N.C.

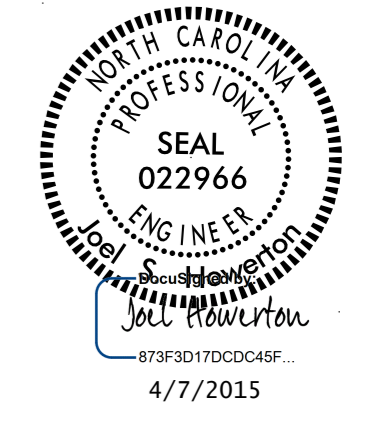
ENGLISH DETAIL DRAWING FOR  
**TRAFFIC BEARING DROP INLET**  
FOR FRAME AND GRATES

SHEET 2 OF 2  
**840D35**

**CONTRACT STANDARDS AND DEVELOPMENT UNIT**  
Office 919-707-6950 FAX 919-250-4119

**SEE PLATE FOR TITLE**

ORIGINAL BY: K. KEMPF DATE: 03-03-2015  
 MODIFIED BY: DATE:  
 CHECKED BY: DATE:  
 FILE SPEC.: s:details\kkempf\english\R3622B\_840d35\_42\_Tb2G1.dgn



STATE OF NORTH CAROLINA  
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DIVISION OF HIGHWAYS  
RALEIGH, N.C.

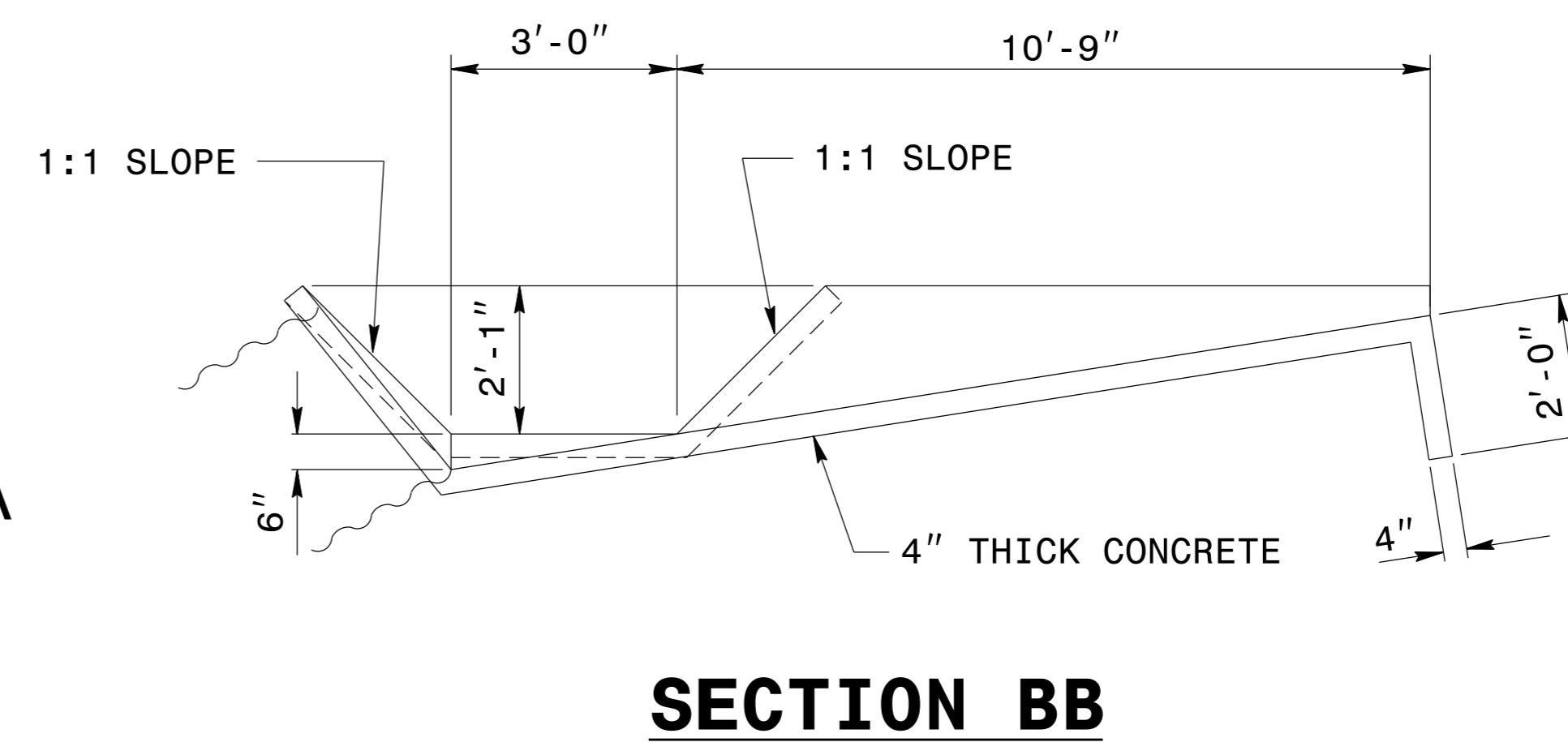
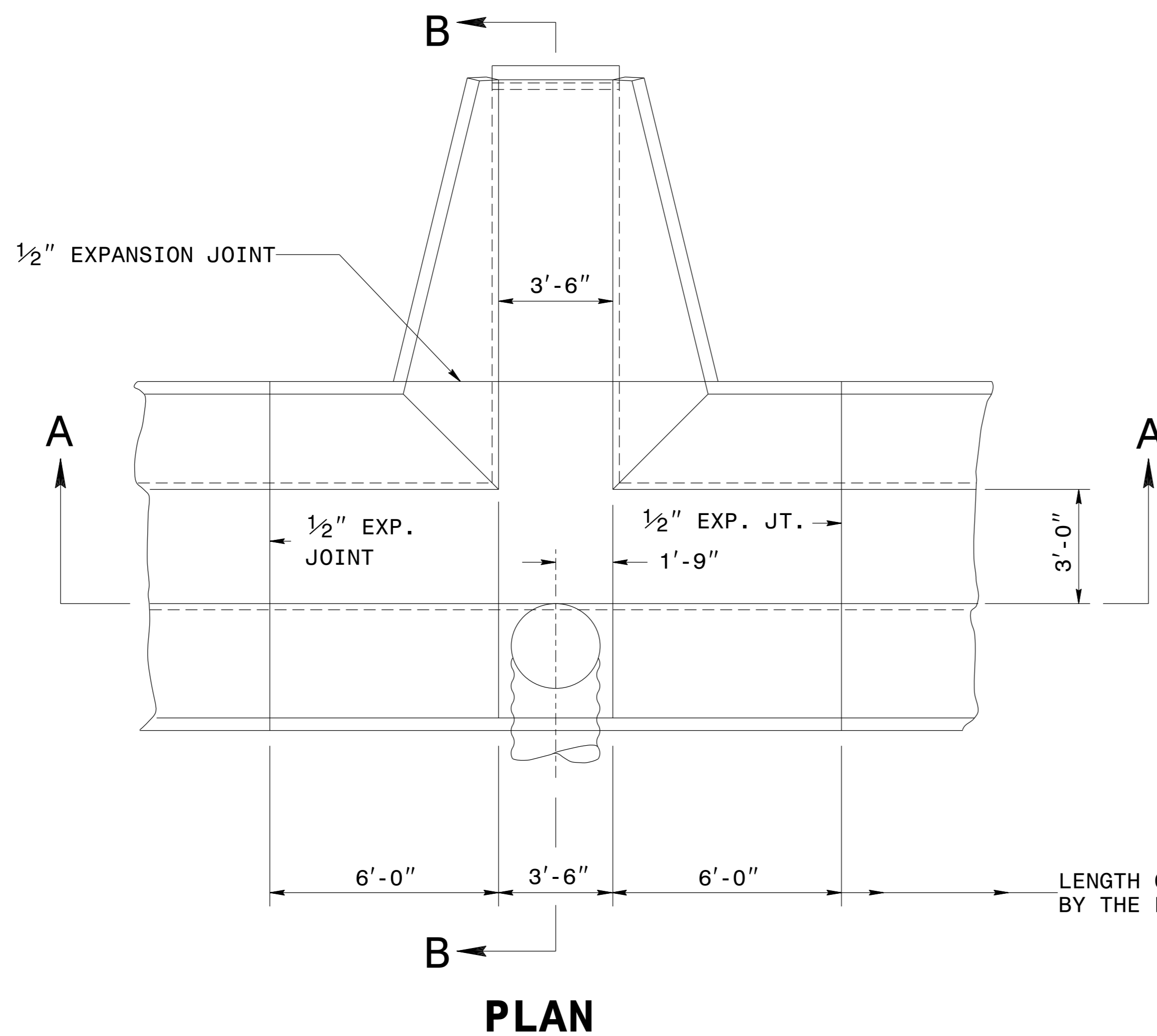
STATE OF NORTH CAROLINA  
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DIVISION OF HIGHWAYS  
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR  
**GUIDE FOR BERM DRAINAGE OUTLET**  
36" PIPE

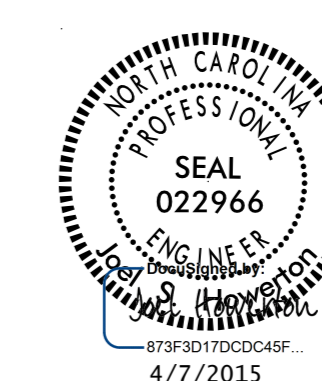
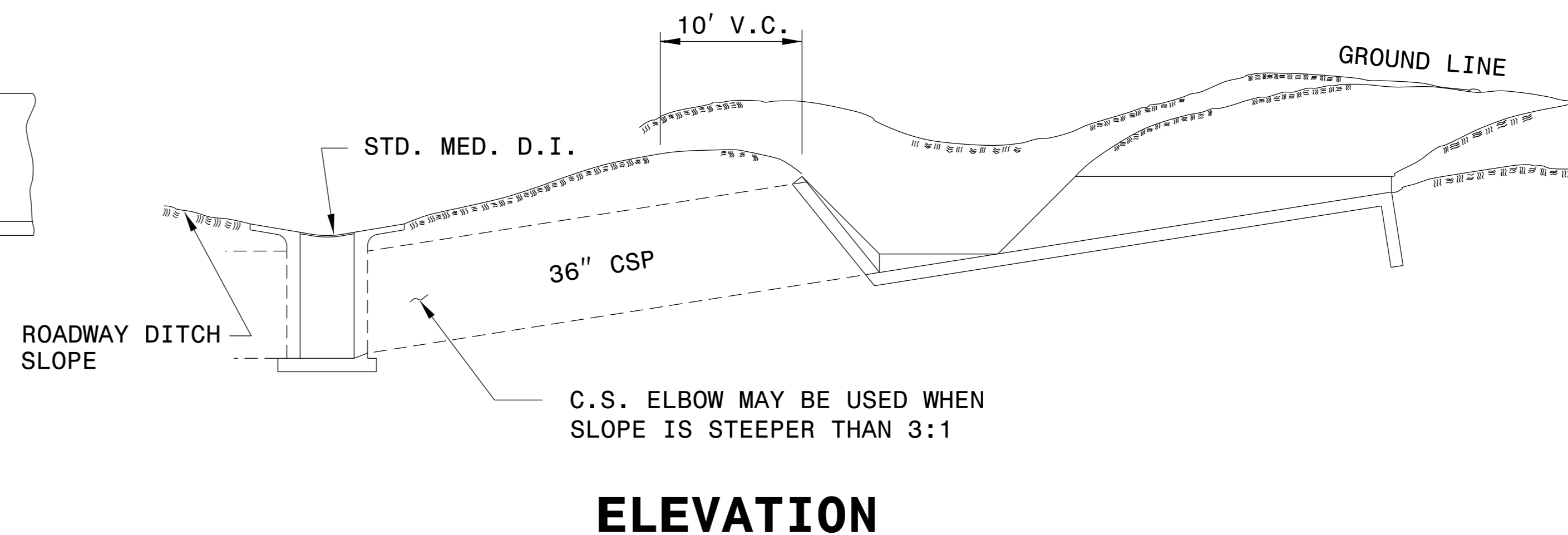
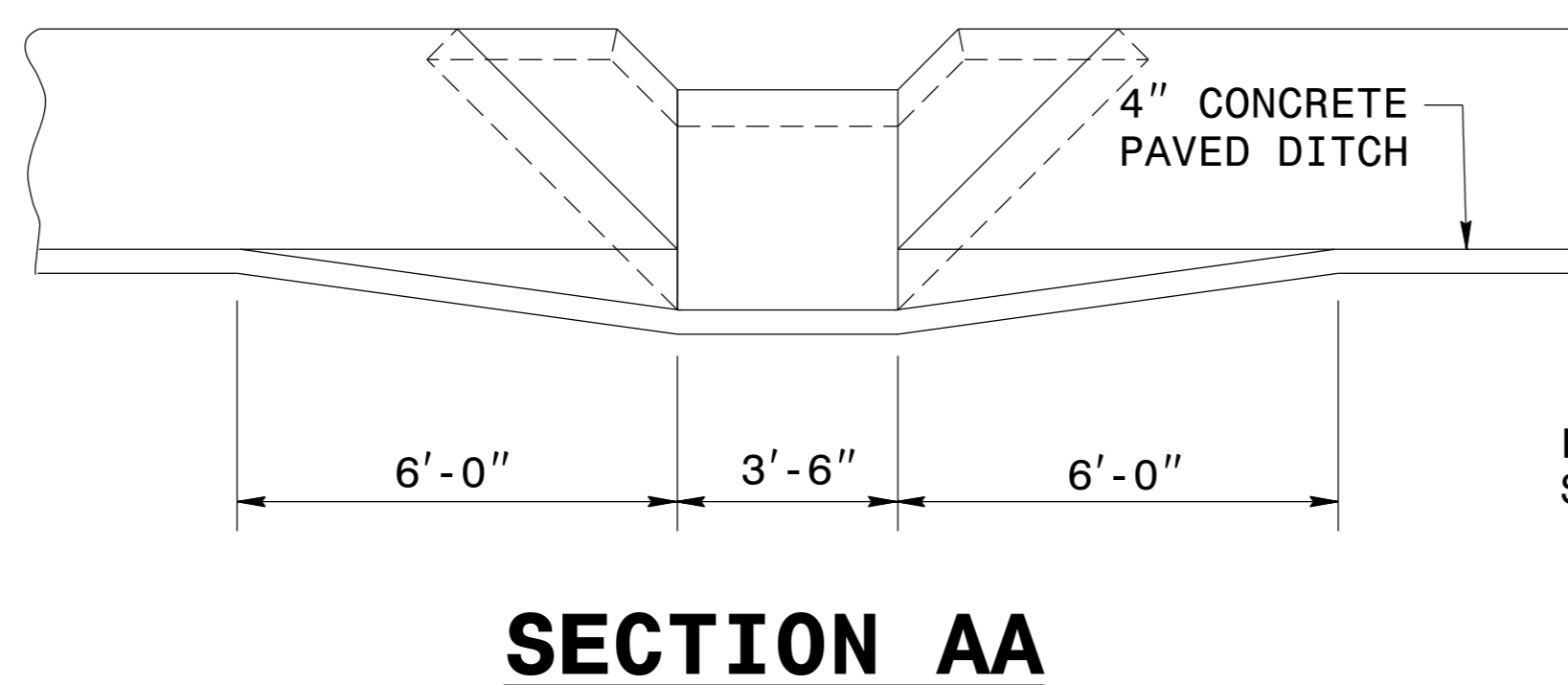
ENGLISH DETAIL DRAWING FOR  
**GUIDE FOR BERM DRAINAGE OUTLET**  
36" PIPE

SHEET 1 OF 1  
**850D11**

SHEET 1 OF 1  
**850D11**



GENERAL NOTES:  
WHERE NECESSARY, ELBOWS MAY BE USED TO SKEW PIPE TO FIT INLETS WHERE THERE IS OFFSET BETWEEN THE INLET END AT BERM AND THE D.I.



CONTRACT STANDARDS AND DEVELOPMENT UNIT  
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**SEE PLATE FOR TITLE**

ORIGINAL BY: 2002 STANDARDS DATE: 01-15-02  
MODIFIED BY: E.E. WARD DATE: 10-26-04  
CHECKED BY: DATE:  
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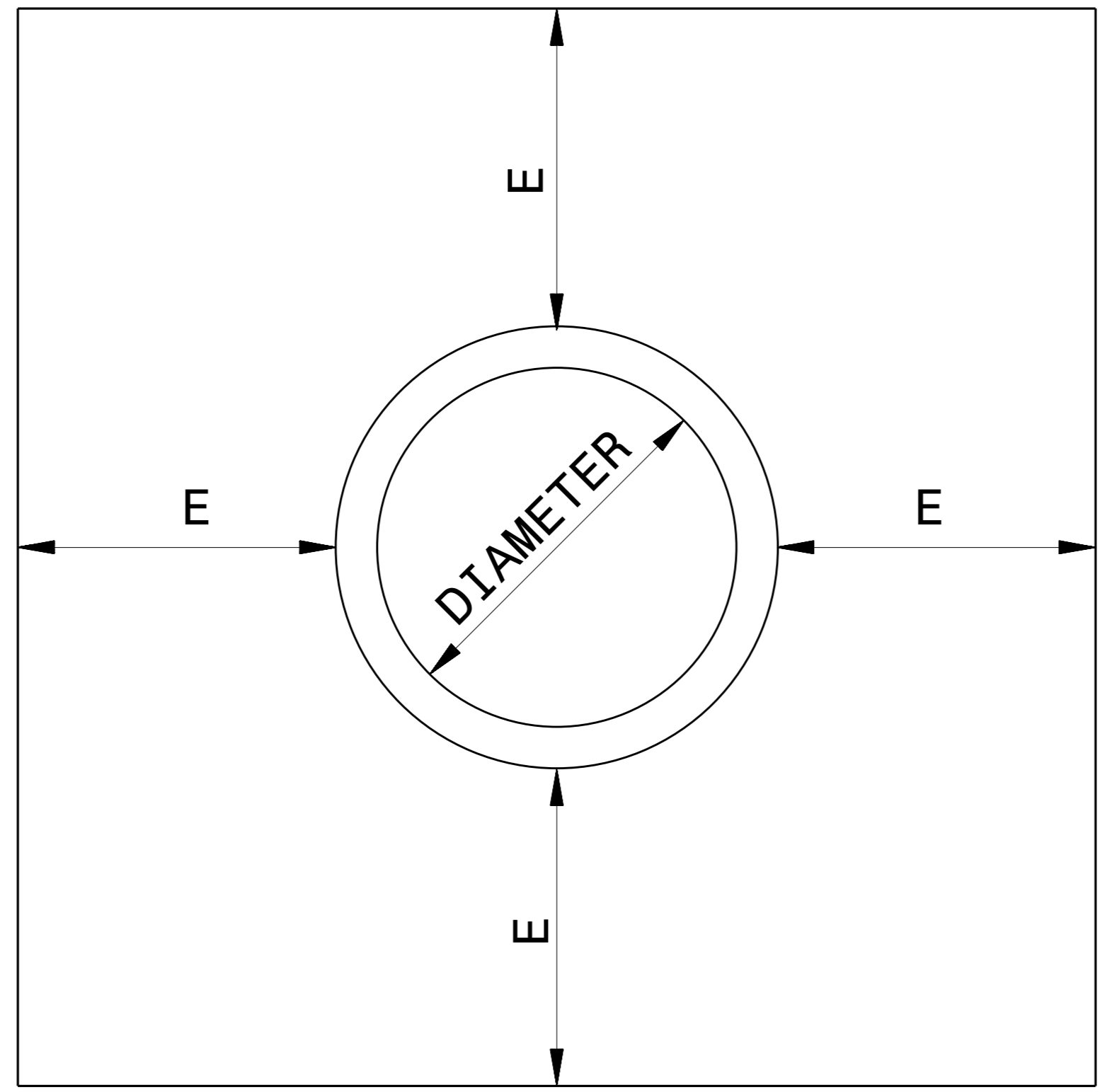
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STATE OF NORTH CAROLINA  
DEPT. OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
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ENGLISH DETAIL DRAWING FOR

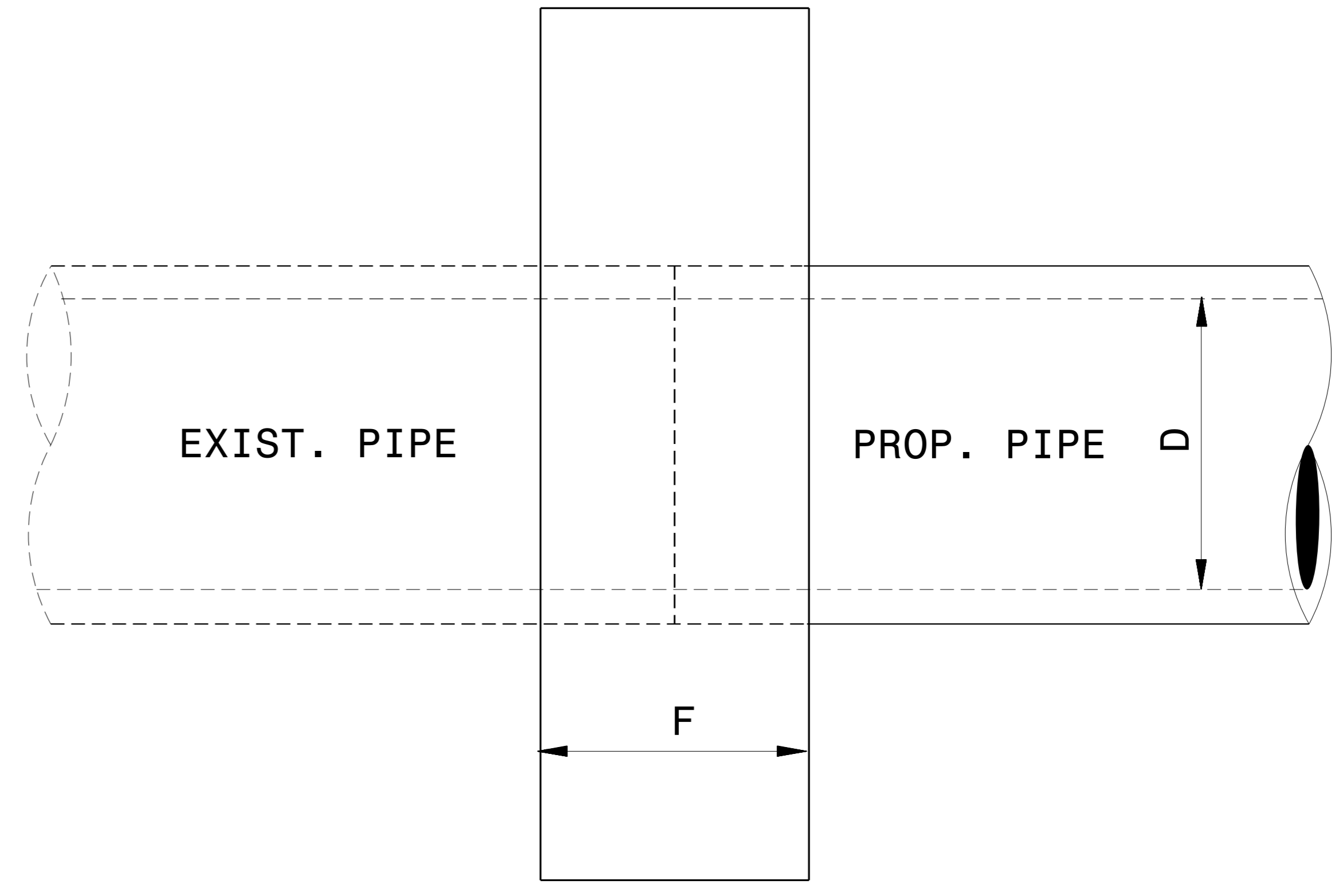
**PIPE COLLAR**

SHEET 1 OF 1  
**840D72**

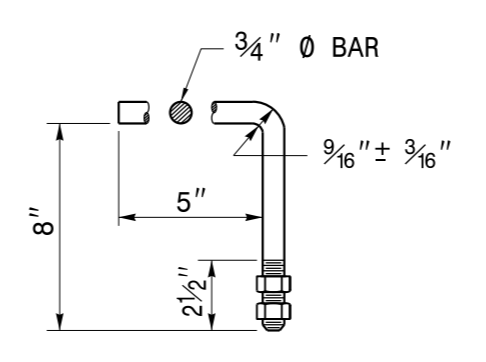


**ELEVATION**

**GENERAL NOTES:**  
USE PIPE COLLAR FOR EXTENDING EXISTING CONCRETE PIPE CULVERTS AT LOCATIONS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER. THIS INCLUDES EXTENDING EXISTING PIPES WITH PIPES OF DIFFERENT MATERIALS.  
CONSTRUCT THE PIPE COLLAR WITH CLASS "B" OR BETTER CONCRETE.  
OBSERVE ALL REQUIREMENTS OF SECTION 840 OF THE STANDARD SPECIFICATIONS.  
USE HOOK BOLTS TO ANCHOR CSP TO THE CONCRETE COLLAR.



**SIDE ELEVATION**



**HOOK BOLT**

HOOB BOLTS (INSTALL ANCHORS AT 2'-0" CTS. ALONG THE CIRCUMFERENCE OF THE CSP). EMBED THE HOOK BOLTS IN THE CONCRETE COLLAR 8" IN DEPTH. THE GALVANIZED 3/4" DIA. HOOK BOLTS MUST MEET ASTM A-307 OR ASTM A-836. BOTH BOLTS AND NUTS MUST BE IN ACCORDANCE WITH ASTM A-153 FOR GALVANIZING.

DIA.	E	F	CU. YD.
8"	12"	12"	0.2634
12"	12"	12"	0.3528
15"	12"	12"	0.3990
18"	12"	12"	0.4465
24"	12"	12"	0.5526
30"	12"	12"	0.6560
36"	12"	12"	0.7640
42"	12"	12"	0.8856
48"	12"	12"	1.0126
54"	18"	18"	2.5793
60"	18"	18"	2.8506
66"	18"	18"	3.1307
72"	18"	18"	3.4176
84"	18"	18"	3.9914
96"	18"	18"	4.5652

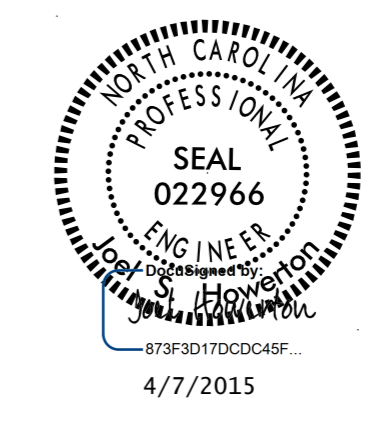
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RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR

**PIPE COLLAR**

SHEET 1 OF 1  
**840D72**

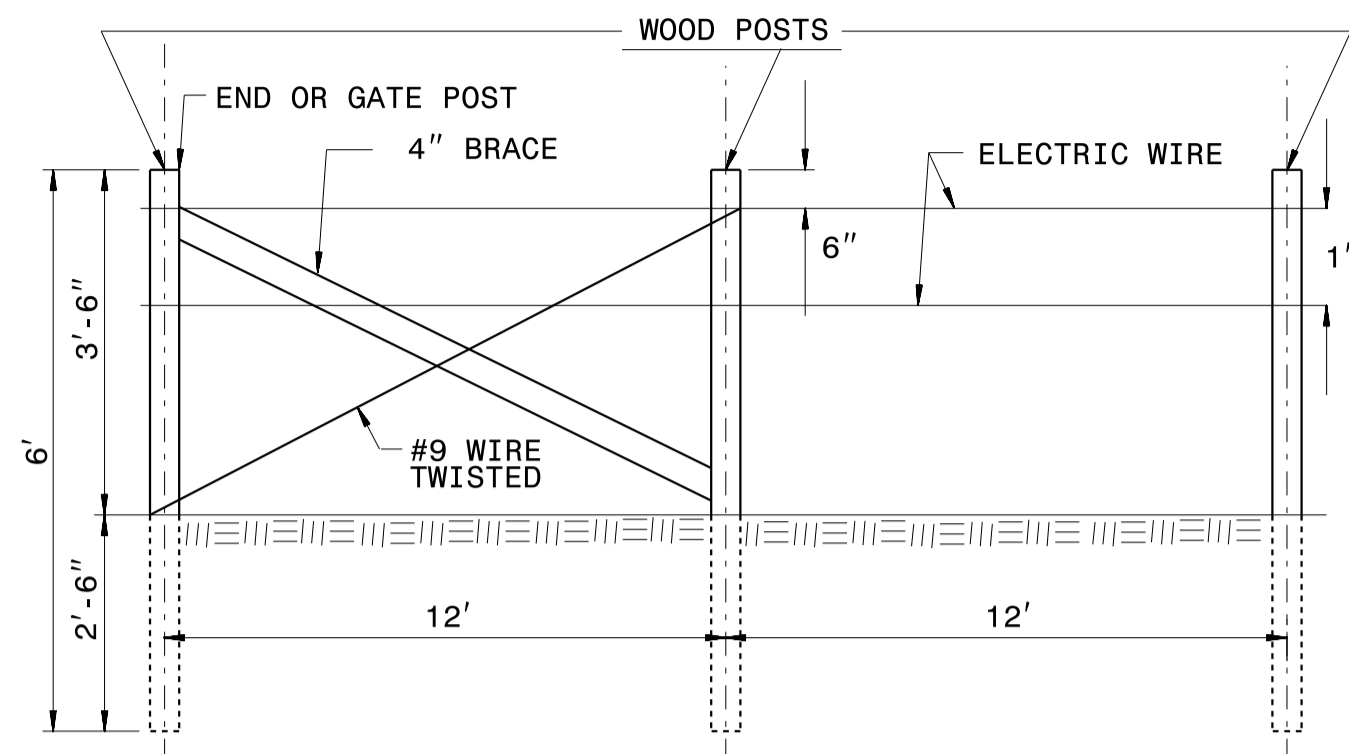
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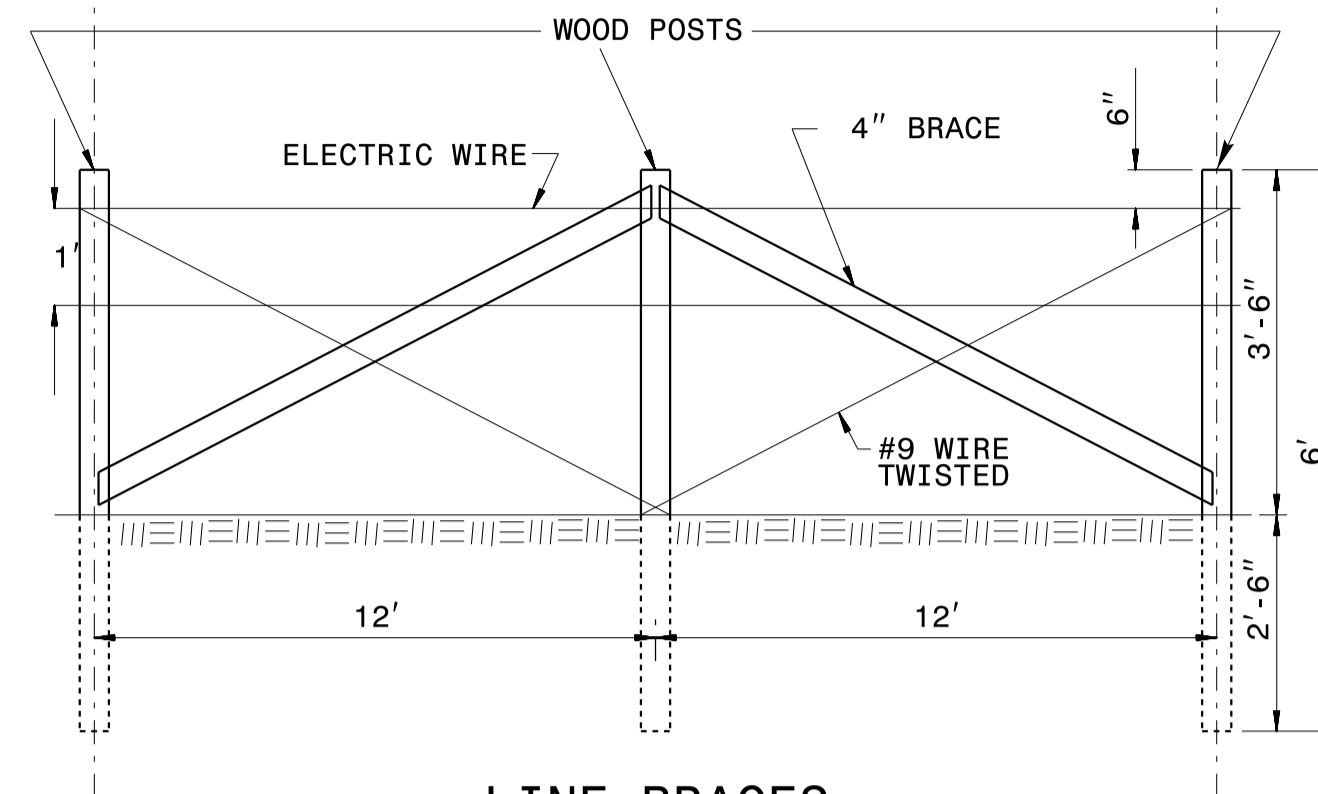
CONTRACT STANDARDS & DEVELOPMENT UNIT  
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**SEE PLATE FOR TITLE**

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MODIFIED BY: nbritt DATE: 03-29-04  
CHECKED BY: \_\_\_\_\_ DATE: \_\_\_\_\_  
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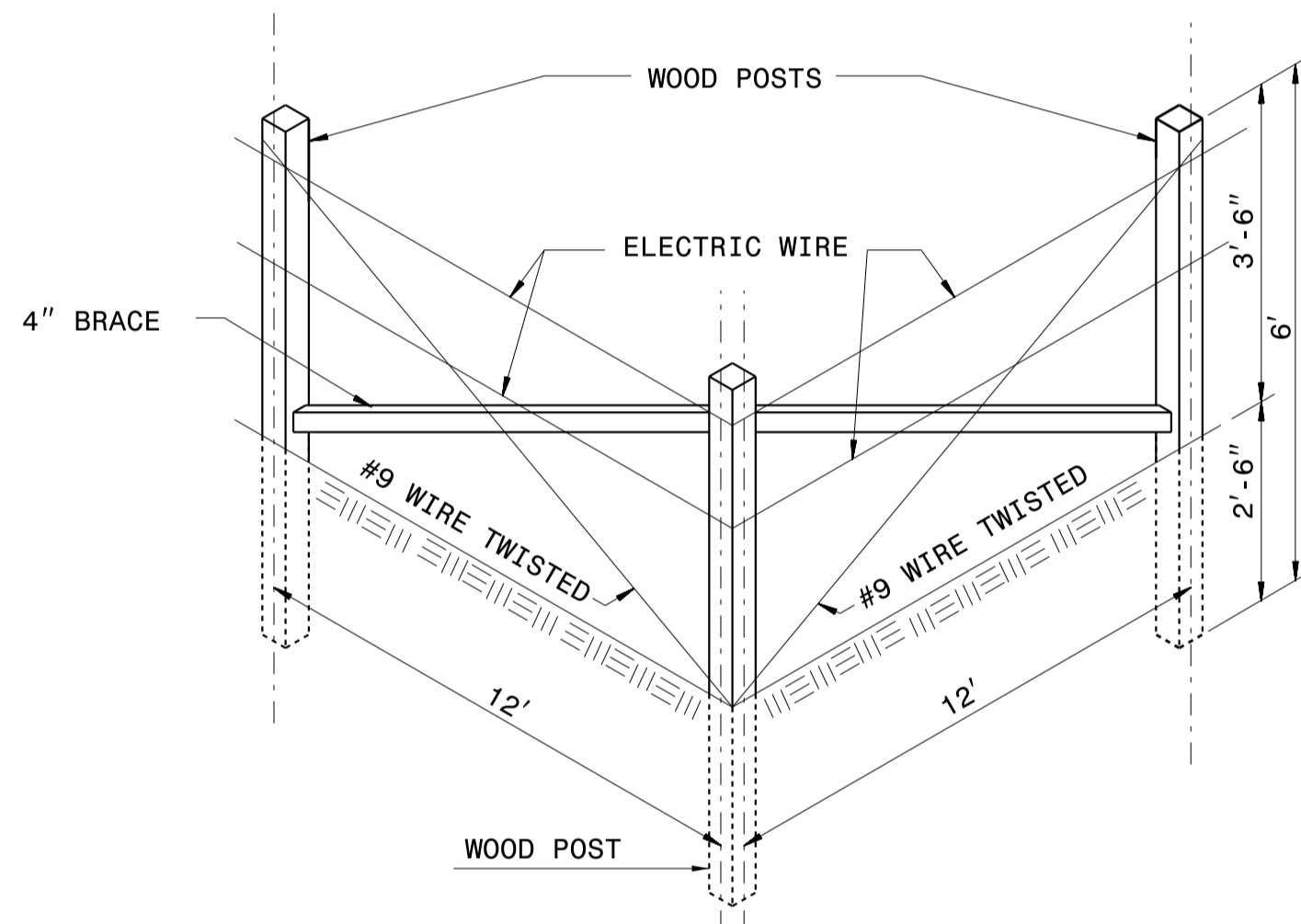


**ONE STRAND ELECTRIC BARBED WIRE FENCE END OR GATE LOCATION**

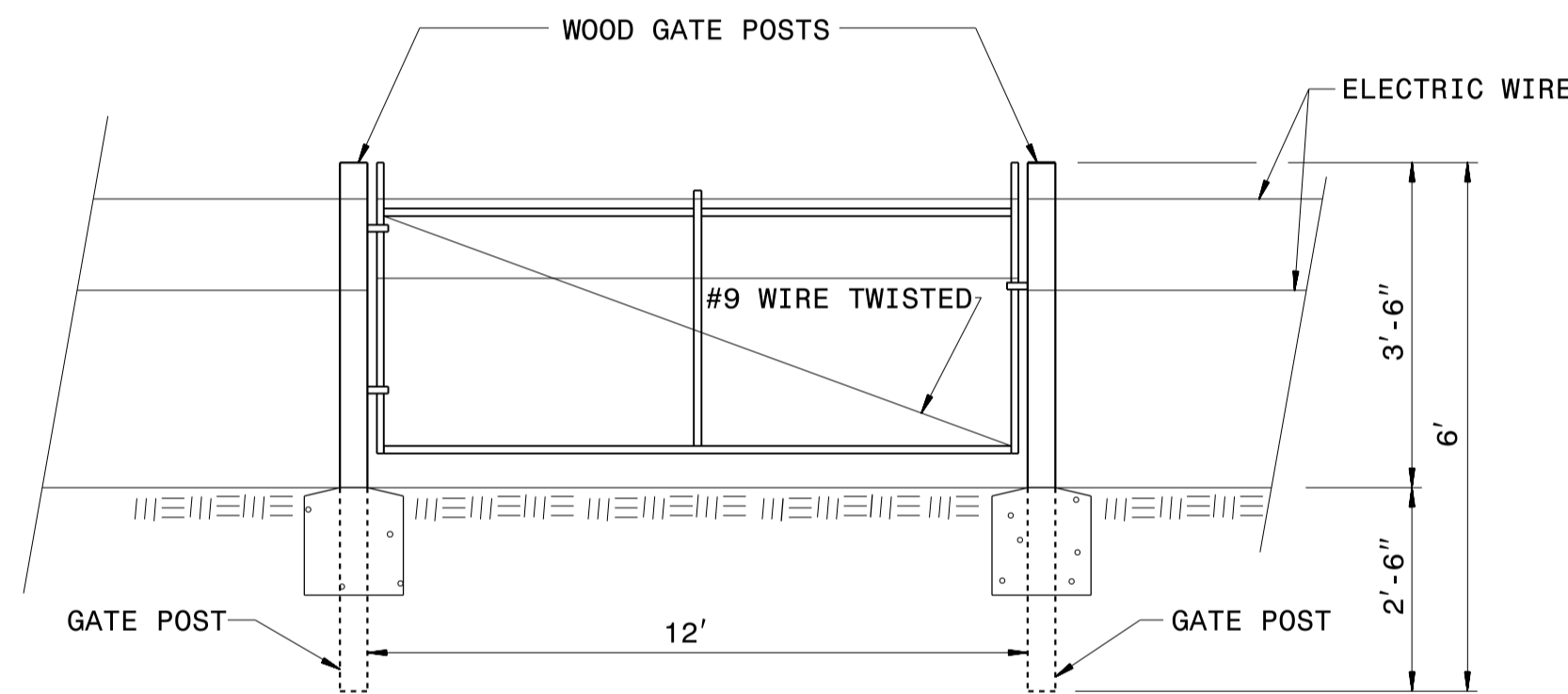


**LINE BRACES**

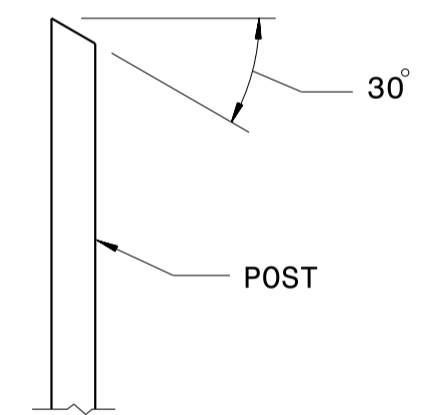
**NOTES :**  
 ELECTRIC WIRE, GATES, LATCH DEVICES, HINGES, AND INSULATORS SHALL BE OF A TYPE AND INSTALLED AS APPROVED BY THE ENGINEER.  
 CLASS B CONCRETE ANCHOR TO BE USED AT GATE POSTS OR WHERE REQUIRED BY SOIL CONDITIONS. CONCRETE ANCHOR MAY ALSO BE USED IN LIEU OF SETTING POSTS TO THEIR REQUIRED DEPTH.  
 THE FENCE SHALL BE INSTALLED FACING THE PROPERTY OWNER EXCEPT ON HORIZONTAL CURVES GREATER THAN 3 DEGREES WHERE THE FENCE SHALL BE INSTALLED AS TO PULL AGAINST ALL POSTS.  
 MAXIMUM SPACING OF LINE BRACES IS 6".  
 ALL POSTS AND BRACES MAY BE EITHER ROUND OR SQUARE AT THE OPTION OF THE CONTRACTOR, PROVIDED THAT THE SAME TYPE IS USED THROUGHOUT THE PROJECT FOR BOTH POSTS AND BRACES. DIMENSIONS SHOW THE DIAMETER OF ROUND POSTS OR EDGE DIMENSIONS OF SQUARE POSTS.  
 THE BRACE WIRE IS TO BE PLACED AROUND POSTS WITH ONE WIRE ON EACH SIDE OF THE BRACE. ALL BRACE WIRES TO BE TIGHTENED BY TWISTING BETWEEN BRACE AND EACH POST.  
 POSTS TO BE NOTCHED 1" FOR BRACES AND ATTACHED TO BRACES USING TWO GALVANIZED 12D NAILS AT EACH END OR AS DIRECTED BY THE ENGINEER.



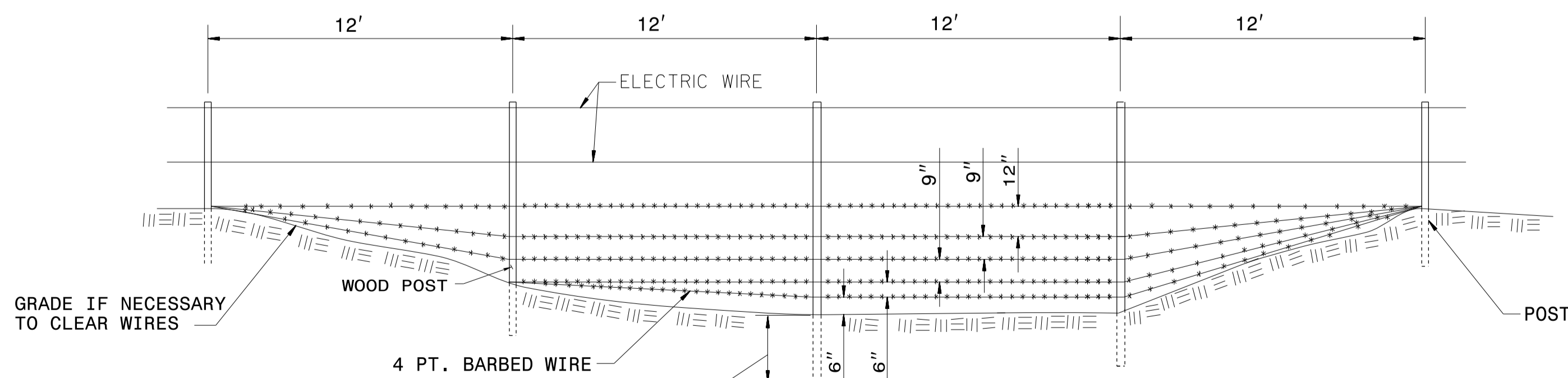
**FENCE CORNER**  
 TO BE USED WHEN CORNER ANGLE IS 15° OR GREATER



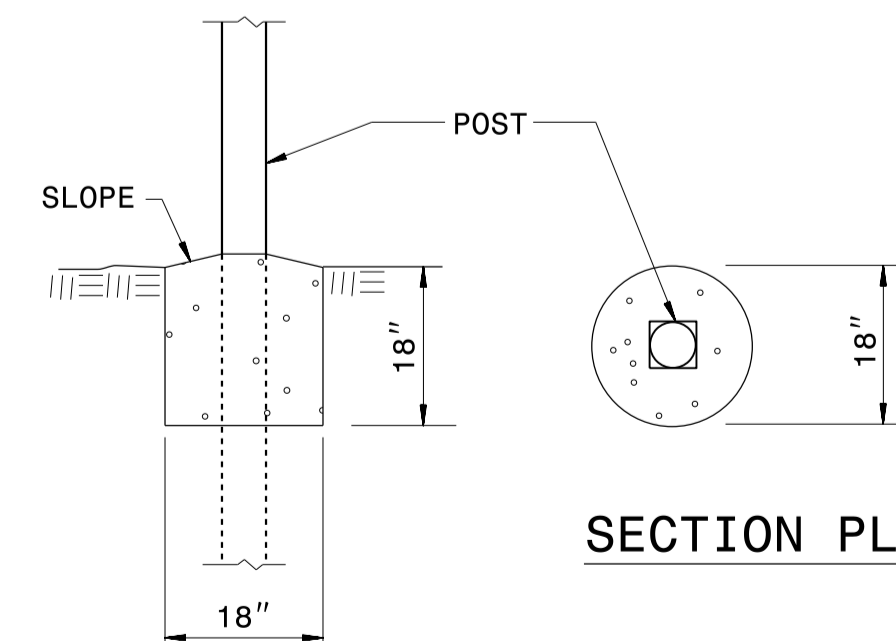
**GATE**



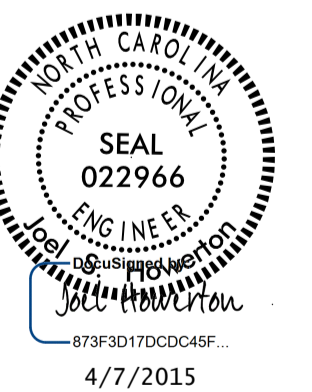
**DETAIL SHOWING METHOD OF CUTTING POST TOPS**



**DETAIL OF DITCH CROSSING**  
 750mm MINIMUM EMBEDMENT AS DIRECTED BY THE ENGINEER



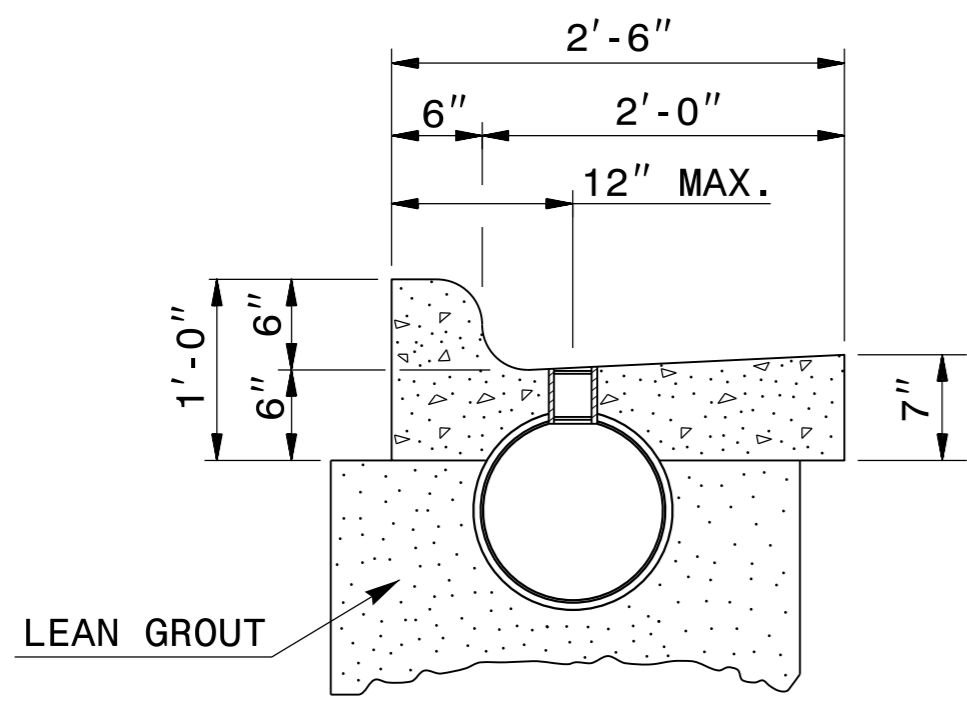
**DETAIL OF POST ANCHOR**



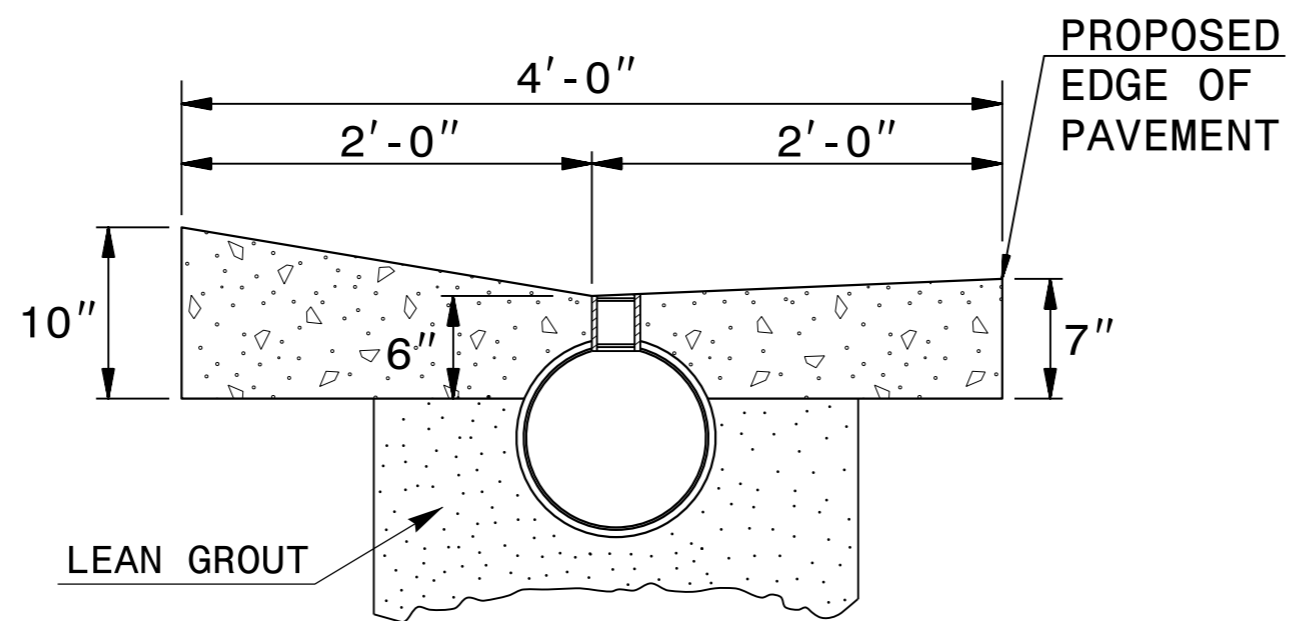
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**2 STRAND ELECTRIC WIRE FENCE**

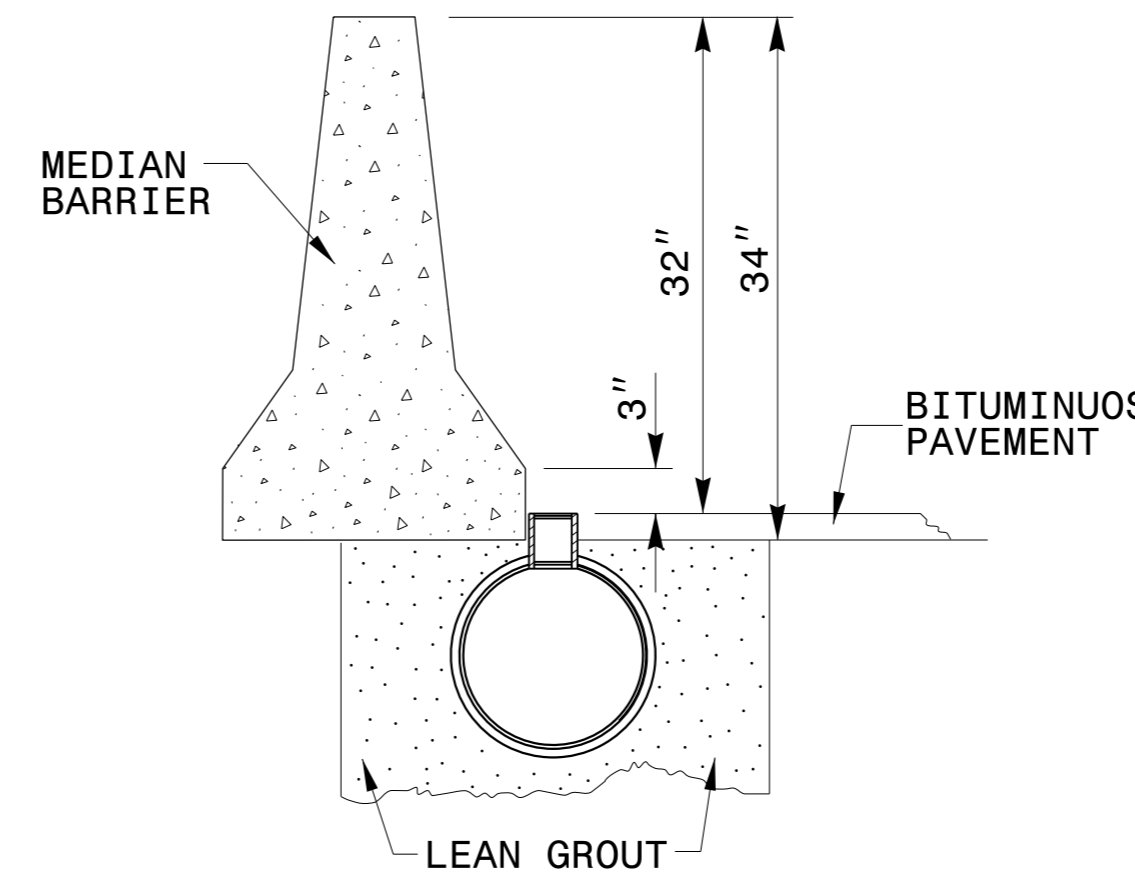
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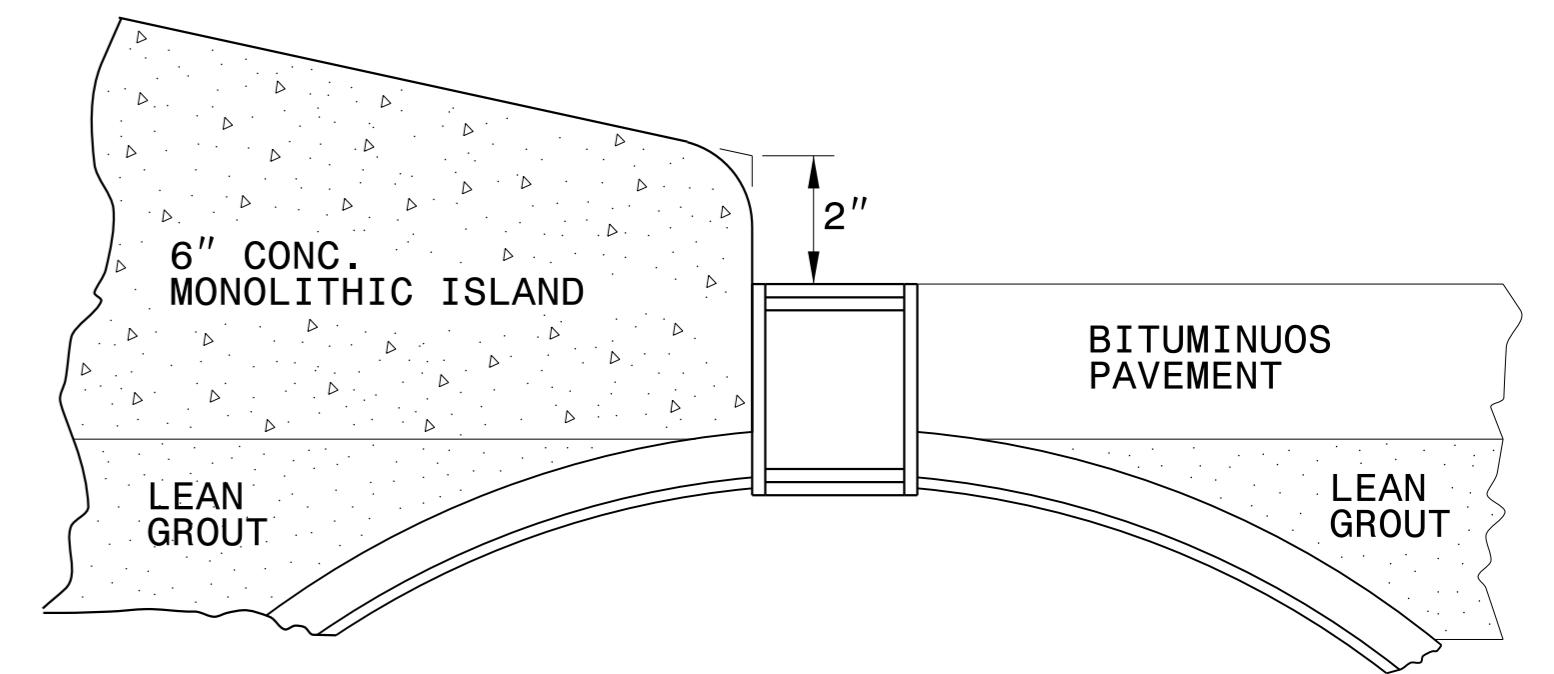
**ALTERNATE NO. 1**



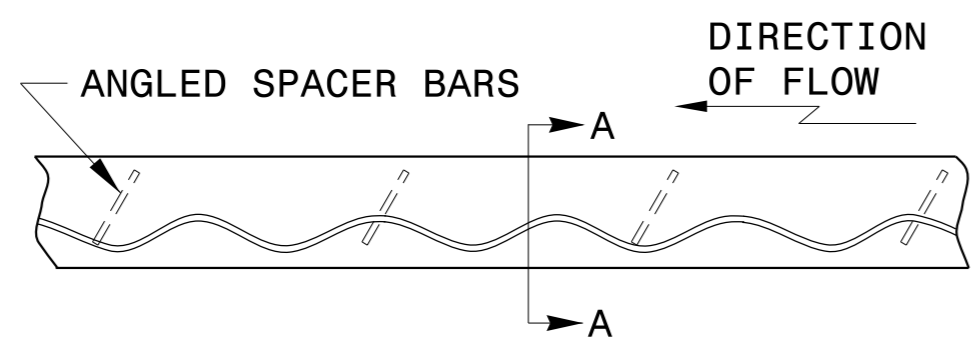
**ALTERNATE NO. 2**



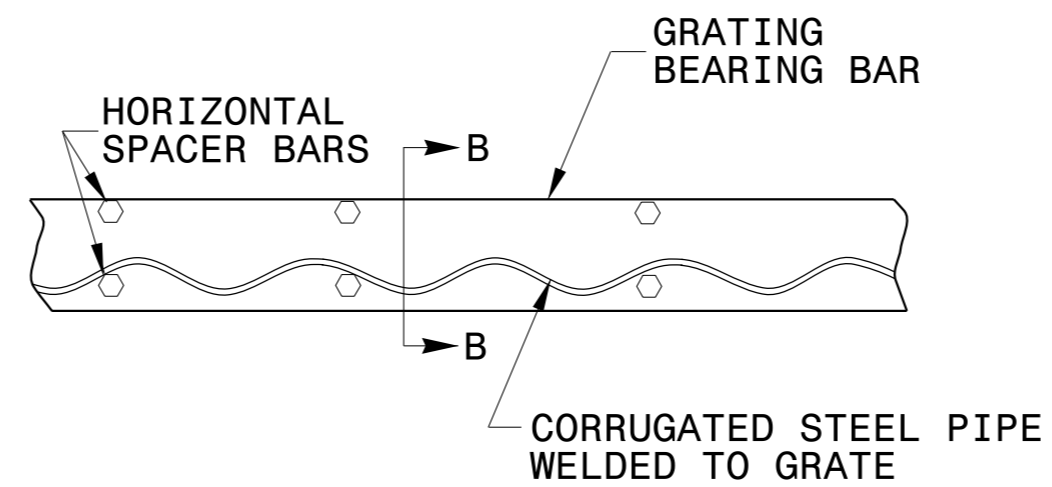
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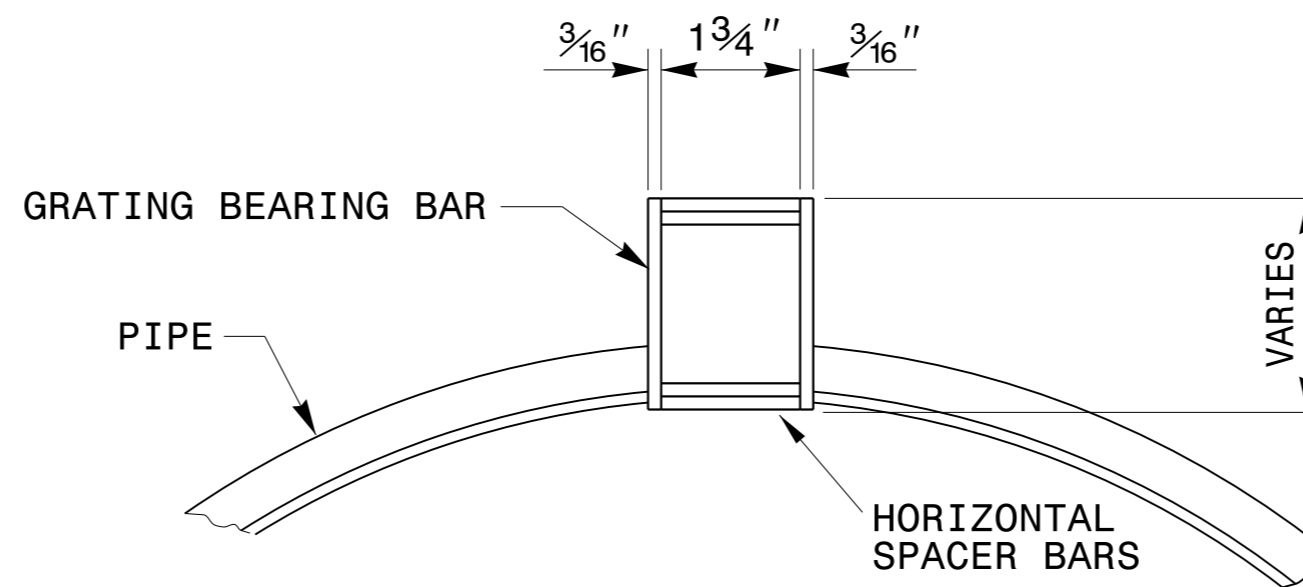
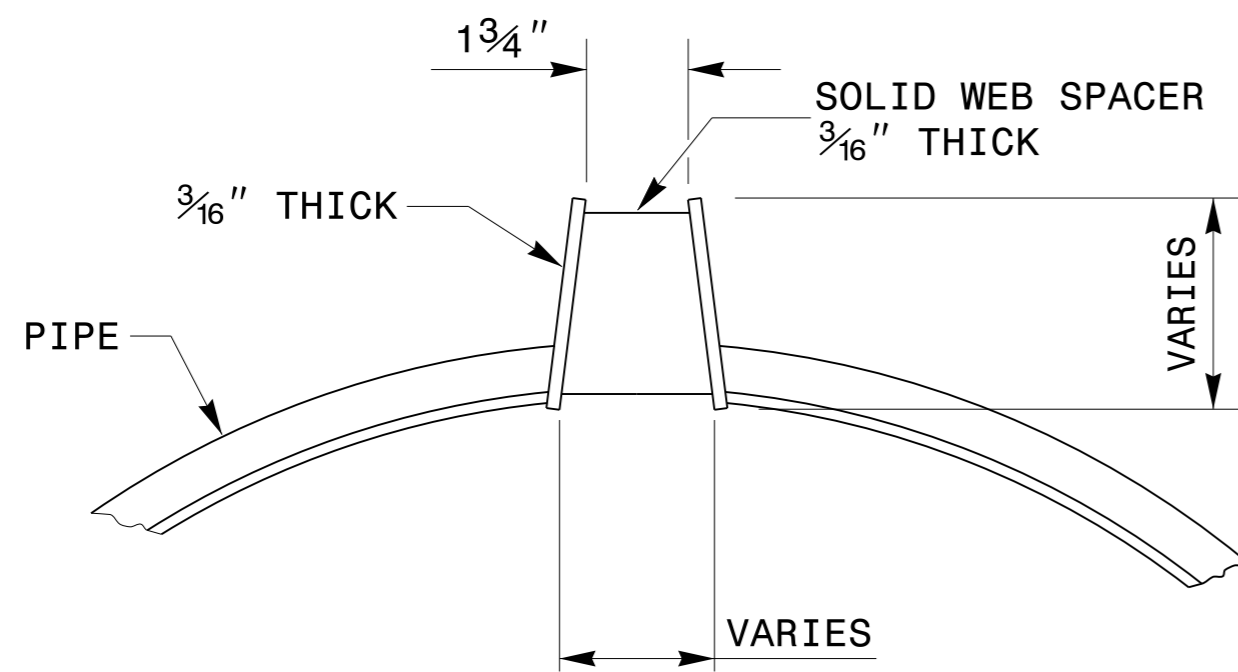
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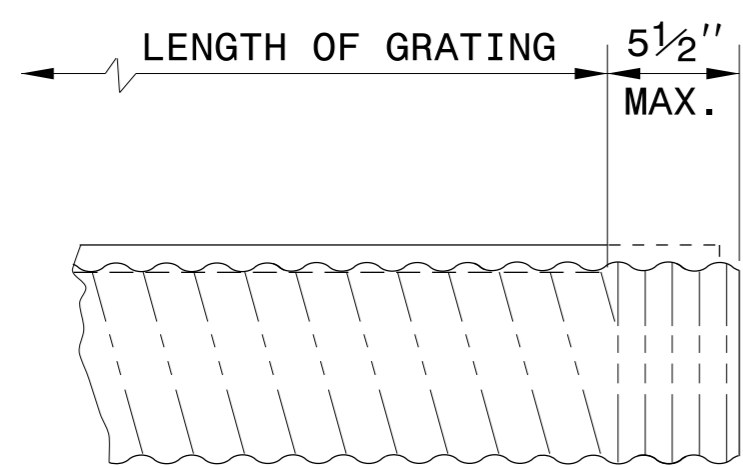
**SECTION A-A**



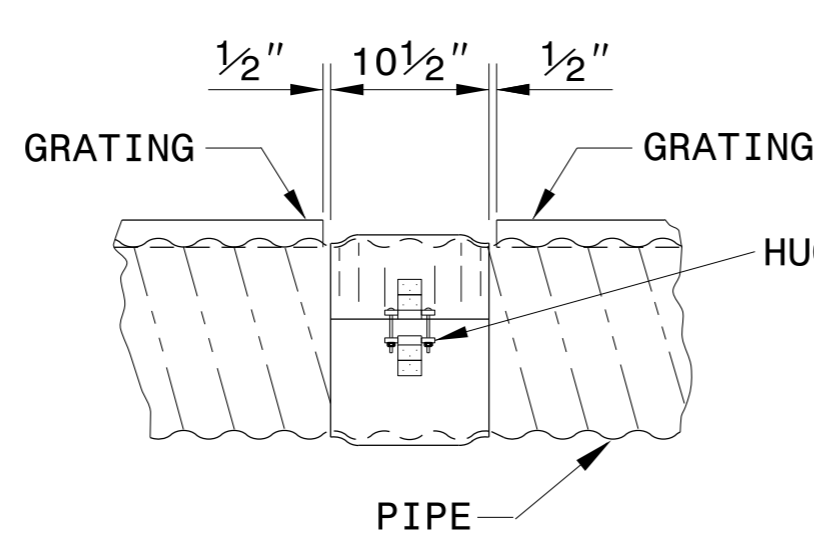
**SECTION B-B**



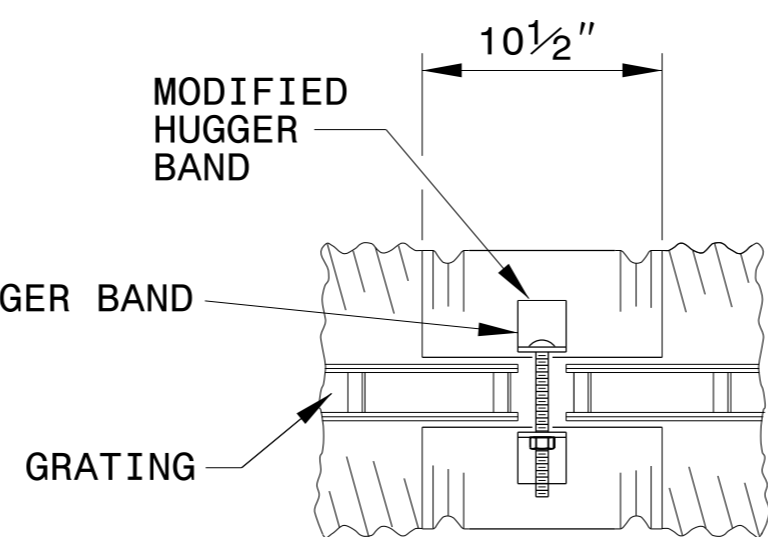
**TYPICAL GRATE DETAILS**



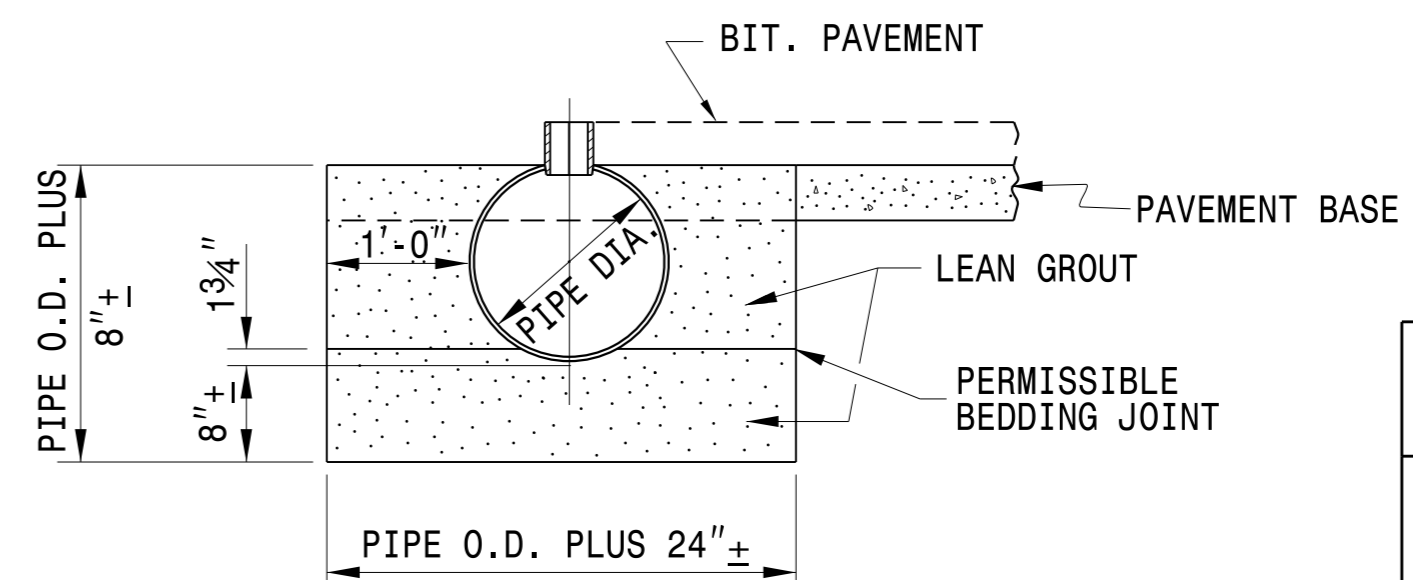
**DETAIL AT END OF PIPE**



**TYPICAL COUPLING BAND**



**MODIFIED COUPLING BAND**



**SLOTTED DRAIN PIPE INSTALLATION**

**NOTES:**

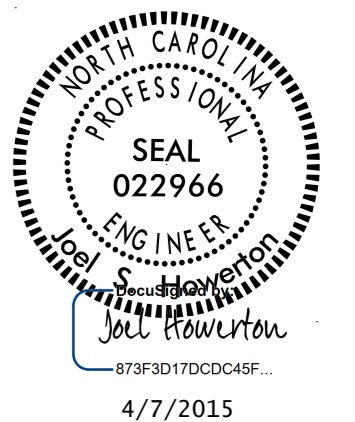
USE GRATE ASSEMBLIES FABRICATED FROM STRUCTURAL STEEL MEETING THE REQUIREMENTS OF ASTM A 570, GRADE 36 OR ASTM A 36.

HOT-DIP GALVANIZE GRATES AFTER FABRICATION TO MEET ASTM A123.

USE SLOTTED DRAIN PIPE THAT IS ADEQUATE FOR AASHTO H20 LOADING WHEN INSTALLED AS SHOWN.

USE SLOTTED DRAIN PIPE FABRICATED FROM ALUMINIZED CORRUGATED STEEL PIPE MEETING THE REQUIREMENTS OF AASHTO M274 TYPE 2.

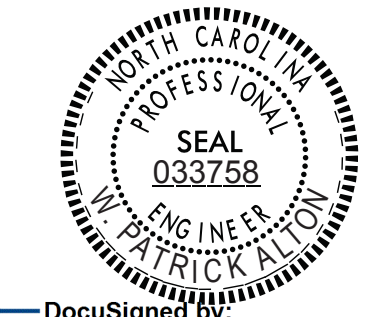
NCDOT ALLOWS THE USE OF SIMILAR GRATE CONFIGURATIONS MEETING THE REQUIREMENTS OF THIS DETAIL, THE REQUIREMENTS OF THE SPECIAL PROVISIONS, AND THE APPROVAL OF THE ENGINEER.

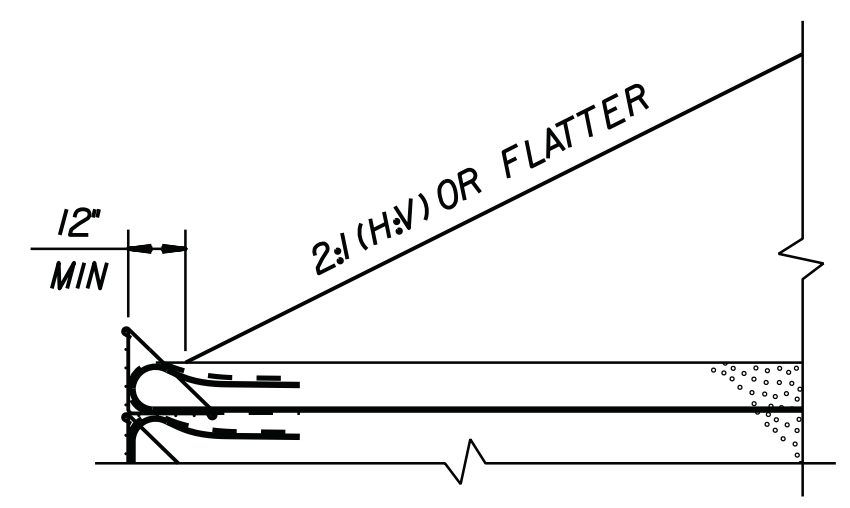


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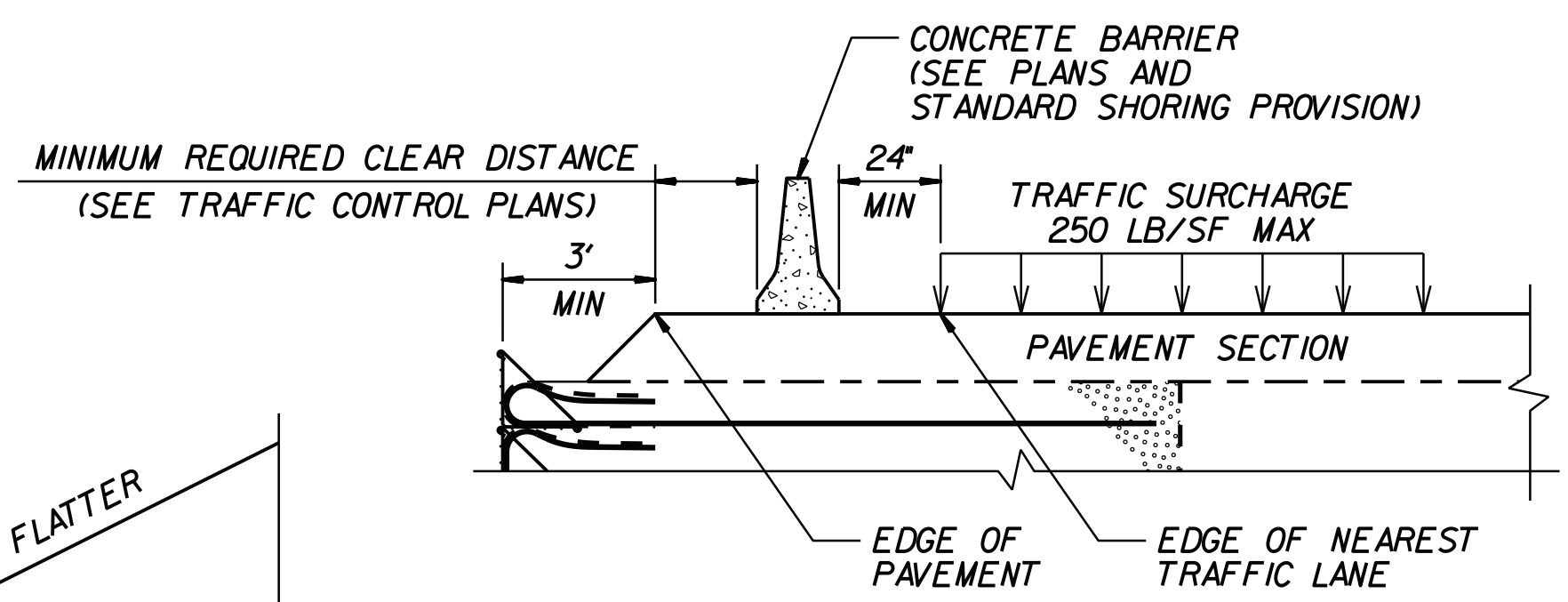
**DETAILS OF SLOTTED DRAIN 12" THRU 36" DIAMETER PIPE**

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MODIFIED BY: DATE:  
CHECKED BY: DATE:  
FILE SPEC.: s:\usr\details\stand\slottdrain.dgn

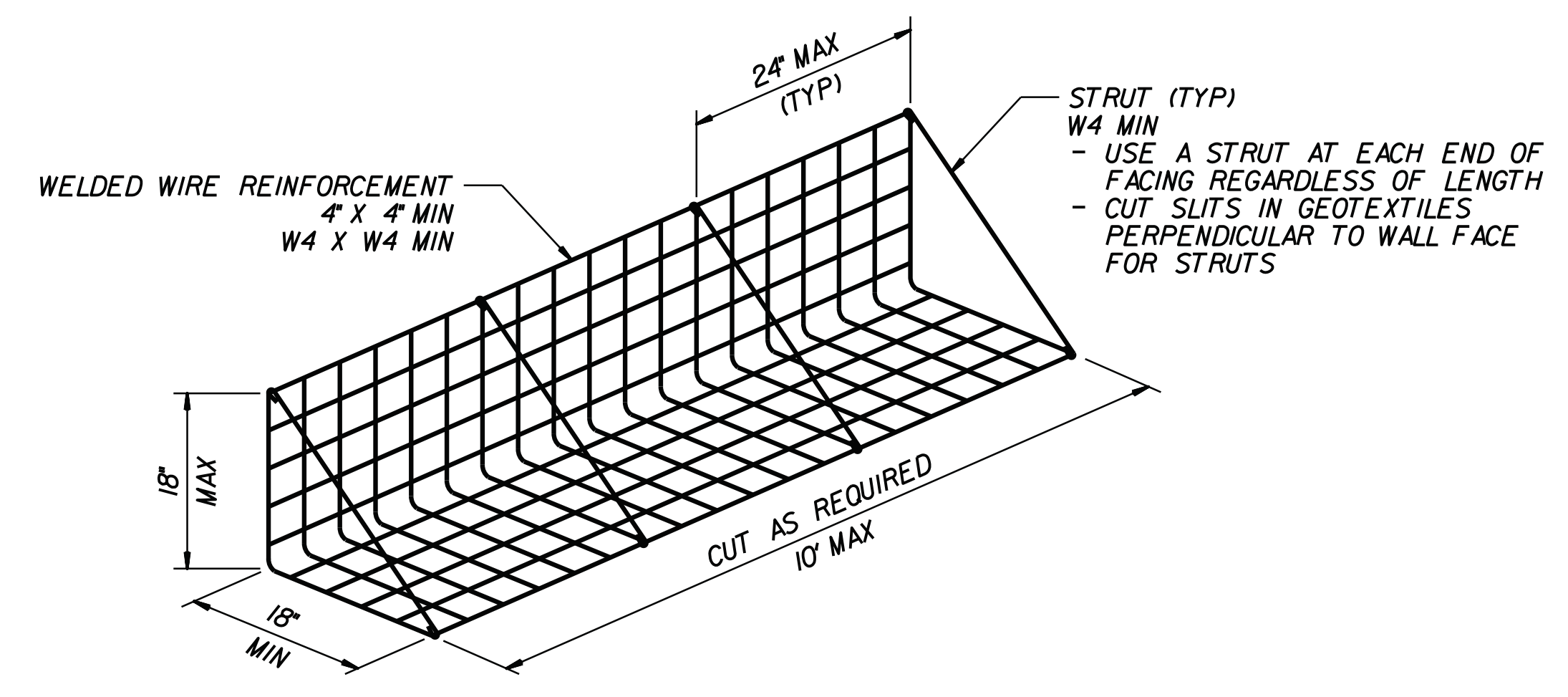
<b>PROJECT REFERENCE NO.</b> R-3622B	<b>SHEET NO.</b> 2G-1
GEOTECHNICAL ENGINEER  DocuSigned By: W. Patrick Altton	ENGINEER



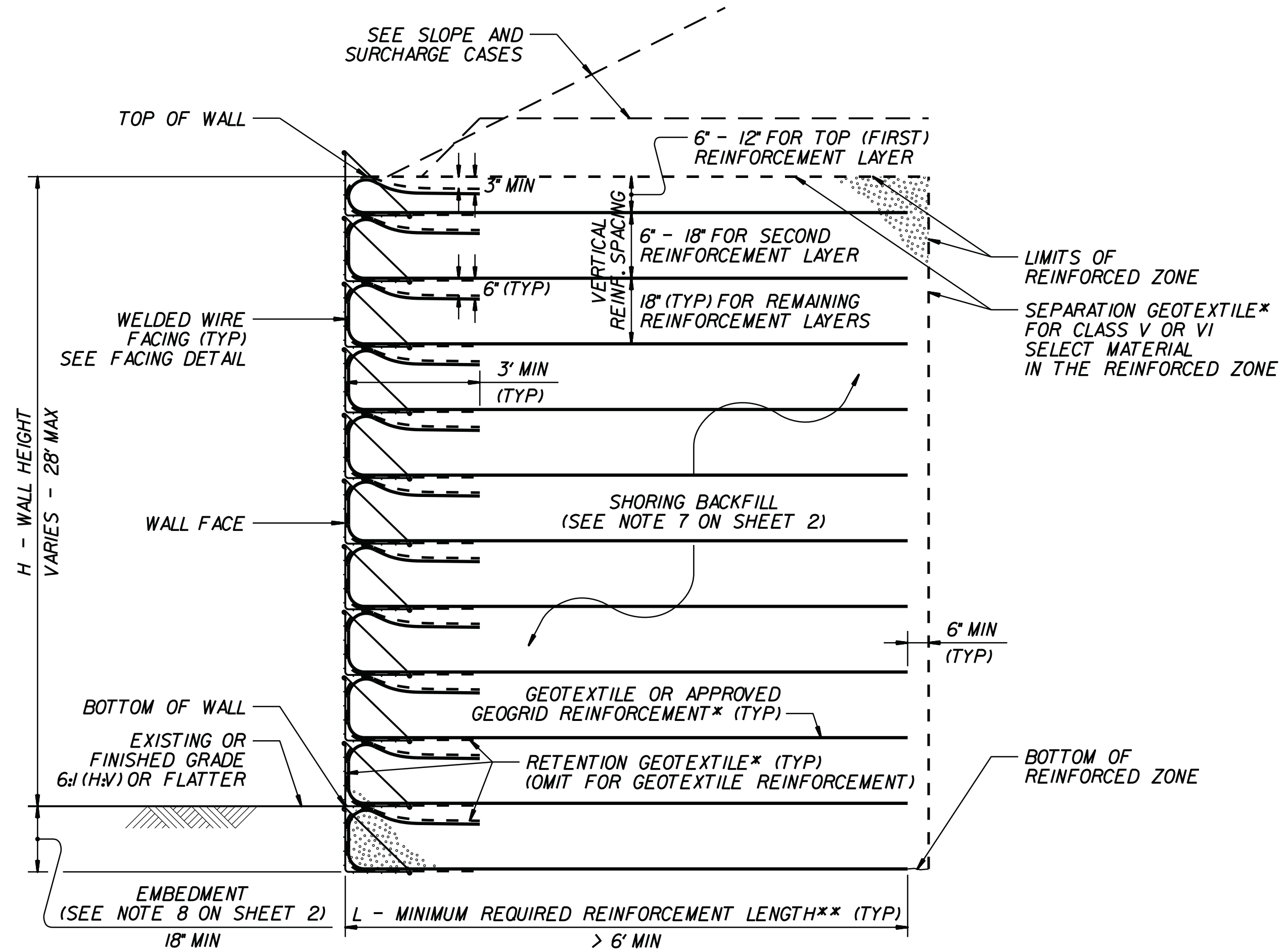
**SLOPE CASE**



**SURCHARGE CASE**

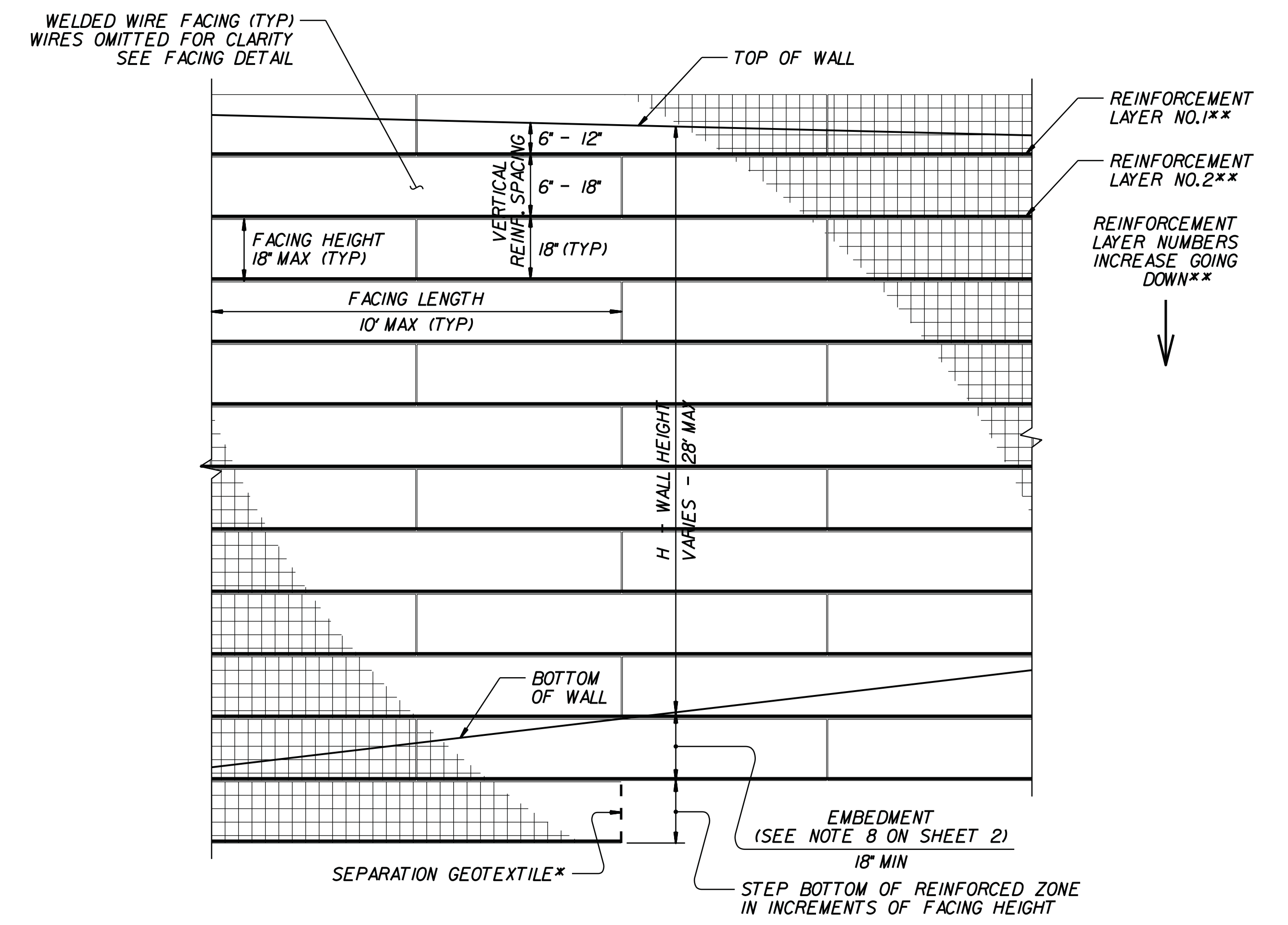


**FACING DETAIL**




**STANDARD TEMPORARY WALL**

(FOR STANDARD TEMPORARY WALLS ON STRUCTURES, SEE TEMPORARY WALL ON STRUCTURE DETAIL ON SHEET 2.)  
 \*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.  
 \*\*SEE REINFORCEMENT TABLES ON SHEET 3.




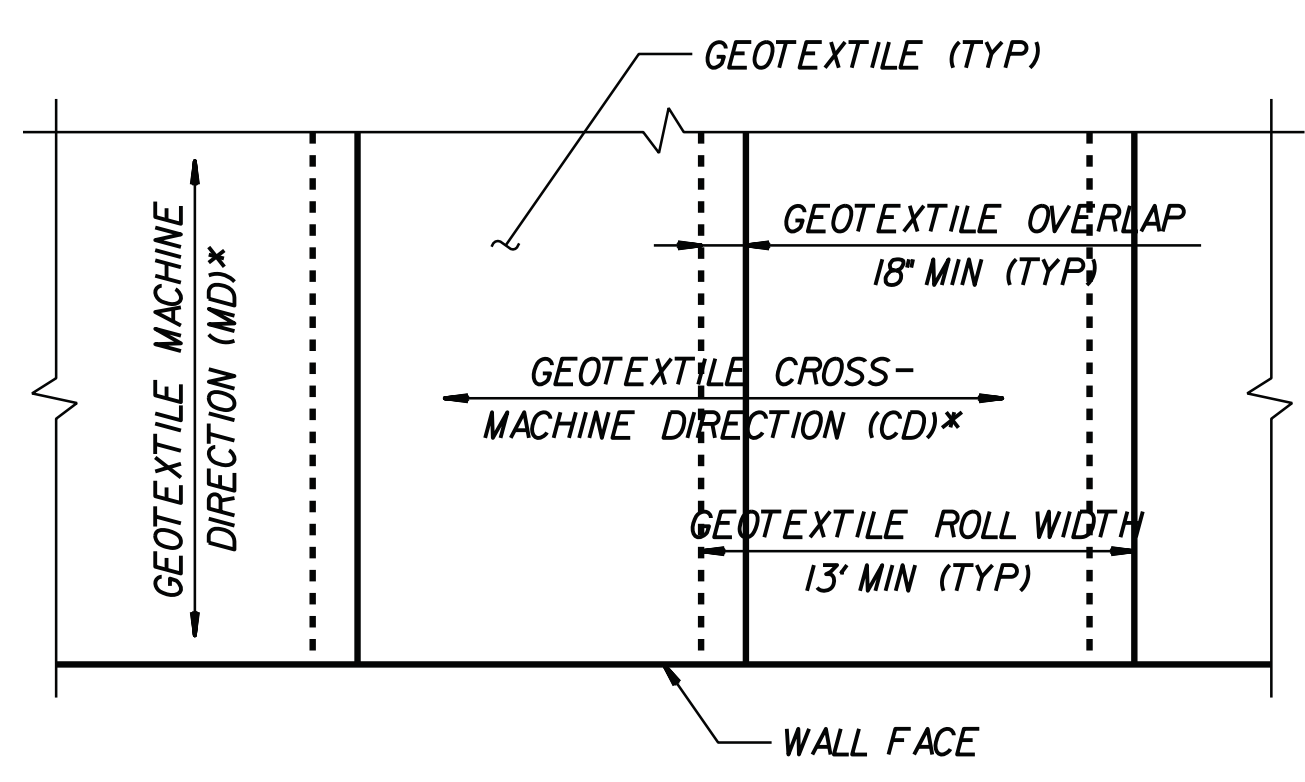
**STANDARD TEMPORARY WALL - PARTIAL ELEVATION**

\*SEE GEOSYNTHETIC PLACEMENT DETAILS ON SHEET 2.  
 \*\*SEE REINFORCEMENT TABLES ON SHEET 3.

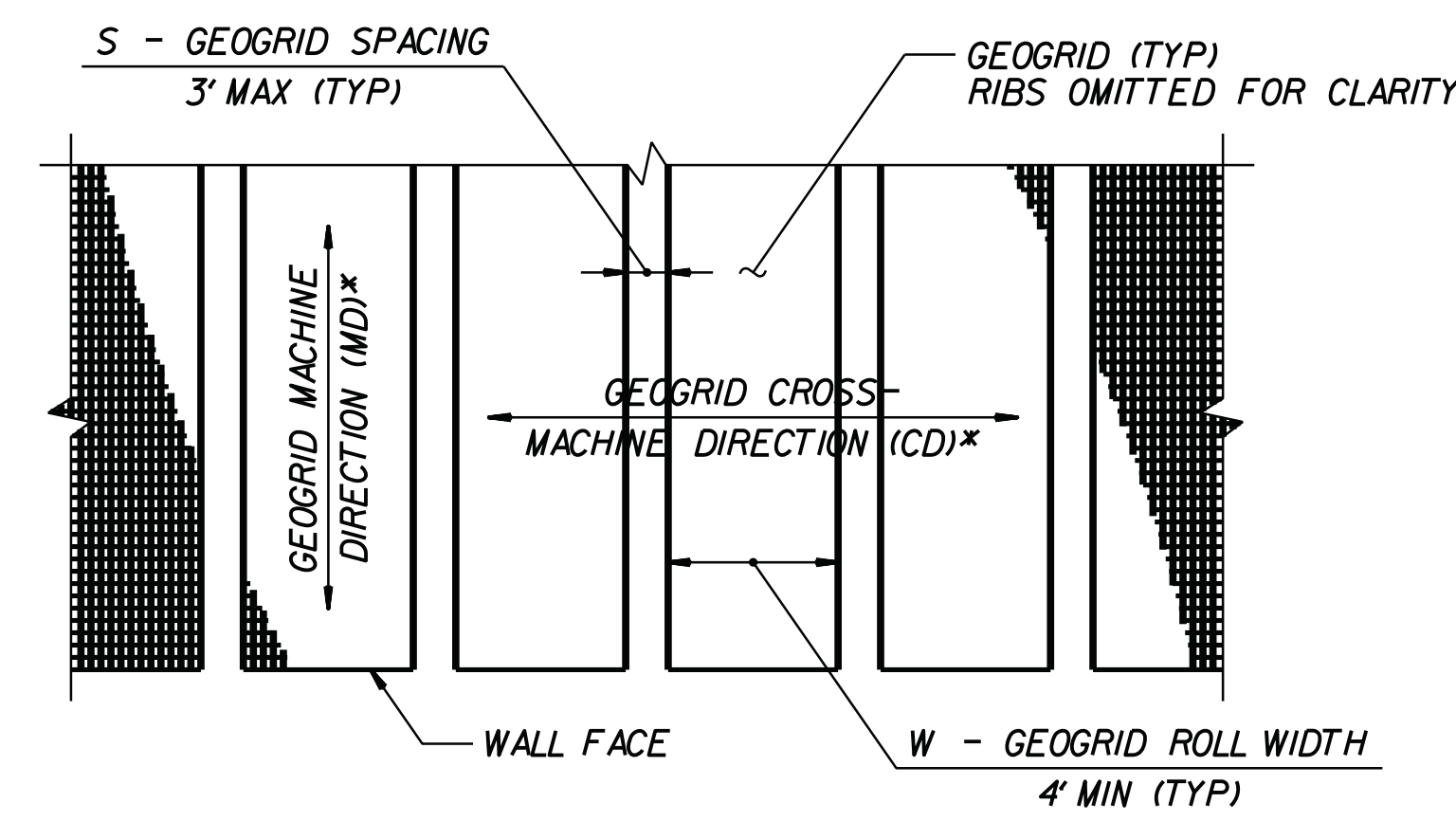
 NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS <b>GEOTECHNICAL ENGINEERING UNIT</b>	STANDARD DETAIL NO. 1801.02
	STANDARD TEMPORARY WALL SHEET 1 OF 3 DATE: 11-19-13



<b>PROJECT REFERENCE NO.</b> R-3622B		<b>SHEET NO.</b> 2G-2	
GEO TECHNICAL ENGINEER		ENGINEER	
			
DocuSigned by: <i>W. Patrick Alton</i>			
SIGNATURE		SIGNATURE	
DATE		DATE	

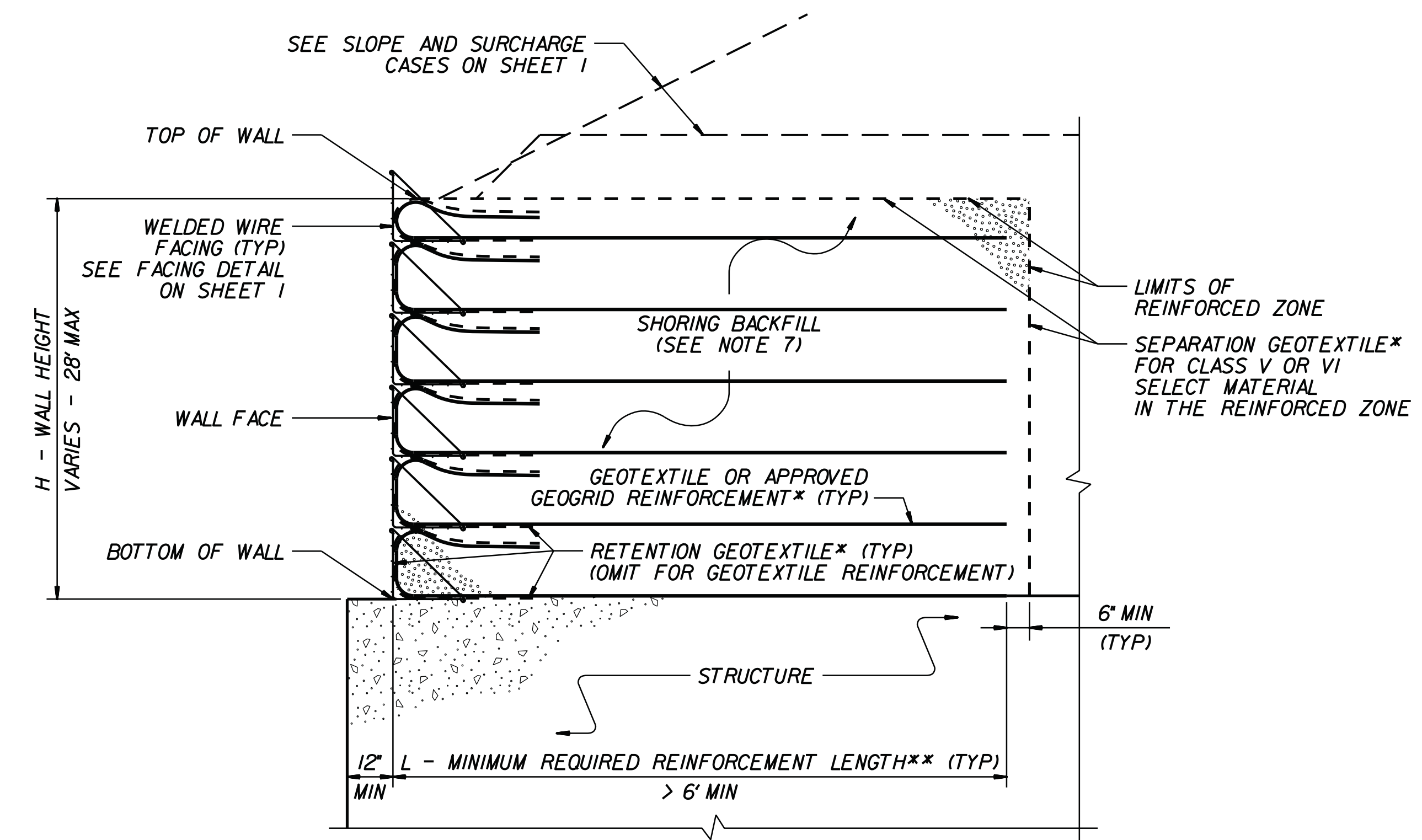


**GEOTEXTILE PLACEMENT**  
(100% COVERAGE MIN FOR GEOTEXTILE REINFORCEMENT)



**GEOGRID PLACEMENT**  
(80% COVERAGE MIN FOR GEOGRID REINFORCEMENT -  $\frac{W}{W+S} \times 100 \geq 80\%$ , SEE NOTE 11)

**GEOSYNTHETIC PLACEMENT DETAILS**  
(PLAN VIEW)  
\*SEE NOTE 12.




**TEMPORARY WALL ON STRUCTURE DETAIL**  
\*SEE GEOSYNTHETIC PLACEMENT DETAILS.  
\*\*SEE REINFORCEMENT TABLES ON SHEET 3.

**NOTES:**


1. AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY WALLS AS NOTED IN THE PLANS.
2. FOR STANDARD TEMPORARY WALLS, SEE STANDARD SHORING PROVISION.
3. STANDARD TEMPORARY WALLS ARE BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS:  
UNIT WEIGHT,  $\gamma = 120$  LB/CF  
FRICTION ANGLE,  $\phi = 30$  DEGREES  
COHESION,  $c = 0$  LB/SF
4. DO NOT USE STANDARD TEMPORARY WALLS IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
5. DO NOT USE STANDARD TEMPORARY WALLS WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS BELOW TEMPORARY WALLS.
6. USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, ASSUME GROUNDWATER DEPTH IS LESS THAN 7' BELOW BOTTOM OF REINFORCED ZONE. DO NOT USE STANDARD TEMPORARY WALLS IF GROUNDWATER IS ABOVE BOTTOM OF REINFORCED ZONE.
7. DO NOT USE A-2-4 SOIL FOR STANDARD TEMPORARY WALLS AROUND CULVERTS OR IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS FOR SLOPE CASES. DO NOT USE CLASS VI SELECT MATERIAL IN THE REINFORCED ZONE OF STANDARD TEMPORARY WALLS WITH GEOTEXTILE REINFORCEMENT.
8. EMBEDMENT IS NOT REQUIRED FOR STANDARD TEMPORARY WALLS ON STRUCTURES OR ROCK AS DETERMINED BY THE ENGINEER.
9. DO NOT USE MORE THAN 4 DIFFERENT REINFORCEMENT STRENGTHS FOR EACH STANDARD TEMPORARY WALL.
10. GEOGRIDS ARE TYPICALLY APPROVED FOR ULTIMATE TENSILE STRENGTHS IN THE MACHINE DIRECTION (MD) AND CROSS-MACHINE DIRECTION (CD) OR SHORT-TERM DESIGN STRENGTHS FOR A 3-YEAR DESIGN LIFE IN THE MD BASED ON MATERIAL TYPE. THE LIST OF APPROVED GEOGRIDS WITH DESIGN STRENGTHS IS AVAILABLE FROM: [connect.ncdot.gov/resources/Materials/Pages/SoilsLaboratory.aspx](http://connect.ncdot.gov/resources/Materials/Pages/SoilsLaboratory.aspx)  
DEFINE MATERIAL TYPE FROM THE WEBSITE ABOVE FOR SHORING BACKFILL AS FOLLOWS:

MATERIAL TYPE	SHORING BACKFILL
BORROW	A-2-4 SOIL
FINE AGGREGATE	CLASS II, TYPE I OR CLASS III SELECT MATERIAL
COARSE AGGREGATE	CLASS V OR VI SELECT MATERIAL

- IF THE WEBSITE DOES NOT LIST A SHORT-TERM DESIGN STRENGTH FOR AN APPROVED GEOGRID, USE A SHORT-TERM DESIGN STRENGTH EQUAL TO THE ULTIMATE TENSILE STRENGTH DIVIDED BY 3.5 FOR THE GEOGRID REINFORCEMENT.
11. FOR GEOGRID REINFORCEMENT WITH LESS THAN 100% COVERAGE, STAGGER REINFORCEMENT SO GEOGRIDS ARE CENTERED OVER GAPS IN THE REINFORCEMENT LAYER BELOW.
  12. AT THE CONTRACTOR'S OPTION, REINFORCEMENT MAY BE INSTALLED WITH THE MD PARALLEL TO THE WALL FACE IF BOTH OF THE FOLLOWING CONDITIONS OCCUR:  
- W (REINFORCEMENT ROLL WIDTH)  $\geq$  (MINIMUM REQUIRED REINFORCEMENT LENGTH) + 4.5' AND  
- REINFORCEMENT STRENGTH IN CD  $\geq$  MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD.
  13. SUBMIT A "STANDARD TEMPORARY WALL SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING TEMPORARY WALL CONSTRUCTION. STANDARD SHORING SELECTION FORMS ARE AVAILABLE FROM: [connect.ncdot.gov/resources/Geological/Pages/Geotech\\_Forms\\_Details.aspx](http://connect.ncdot.gov/resources/Geological/Pages/Geotech_Forms_Details.aspx)
  14. DO NOT PLACE SHORING BACKFILL OR REINFORCEMENT UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.
  15. FOR STANDARD TEMPORARY WALLS WITH PILE FOUNDATIONS IN THE REINFORCED ZONE, DRIVE PILES THROUGH REINFORCEMENT AFTER CONSTRUCTING TEMPORARY WALLS.
  16. DO NOT SPLICE OR OVERLAP REINFORCEMENT SO SEAMS ARE PARALLEL TO THE WALL FACE.
  17. CONTACT THE ENGINEER WHEN EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT.
  18. FOR STANDARD TEMPORARY WALLS WITH INTERIOR ANGLES LESS THAN 90 DEGREES, WRAP GEOSYNTHETICS AT ACUTE CORNERS AS DIRECTED BY THE ENGINEER.
  19. FOR STANDARD TEMPORARY WALLS WITH TOP OF WALL WITHIN 5' OF FINISHED GRADE, REMOVE TOP FACING AND INCORPORATE TOP REINFORCEMENT LAYER INTO FILL WHEN PLACING FILL IN FRONT OF WALL.


  
**NORTH CAROLINA**  
**DEPARTMENT OF TRANSPORTATION**  
**DIVISION OF HIGHWAYS**  
  
**GEO TECHNICAL**  
**ENGINEERING UNIT**

**STANDARD DETAIL NO. 1801.02**  
  
**STANDARD**  
**TEMPORARY WALL**  
**SHEET 2 OF 3**  
  
 DATE: 11-19-13

<b>PROJECT REFERENCE NO.</b> R-3622B	<b>SHEET NO.</b> 2G-3
 <p>DocuSigned by: <i>W. Patrick Altan</i></p>	ENGINEER

SLOPE OR SURCHARGE CASE	GROUNDWATER DEPTH BELOW BOTTOM OF REINFORCED ZONE (SEE NOTE 6 ON SHEET 2) (FT)	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)	H - WALL HEIGHT (FT)																									
			< 4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	
SLOPE CASE	> 0	CLASS II, TYPE I, CLASS III, CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	8	9	11	12	13	13	14	15	16	17	18	19	20	21	22	23	24	24	25	26	27	27	
SURCHARGE CASE	> 0 TO 7 FOR H < 20' > 0 TO 10 FOR H ≥ 20'	ALL SHORING BACKFILL TYPES	6	7	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	17	17	18	19	19	20	21	22	
	> 7 FOR H < 20' > 10 FOR H ≥ 20'	A-2-4 SOIL	6	6	7	8	8	9	9	10	11	11	12	12	13	14	14	15	16	16	17	18	18	19	20	20	21	
		CLASS II, TYPE I OR CLASS III SELECT MATERIAL	6	6	7	7	8	8	9	10	10	11	11	12	12	13	14	15	15	16	16	17	17	18	18	19	20	
		CLASS V OR CLASS VI SELECT MATERIAL	6	6	7	7	7	8	8	9	9	10	10	11	12	13	13	14	14	15	15	16	17	17	18	19	19	

**L - MINIMUM REQUIRED REINFORCEMENT LENGTH (FT)**  
(FOR ALL REINFORCEMENT TYPES)

WALL HEIGHT (H) + EMBEDMENT (FT)	NUMBER OF REINFORCEMENT LAYERS*
2.5 - 4	3
4 - 5.5	4
5.5 - 7	5
7 - 8.5	6
8.5 - 10	7
10 - 11.5	8
11.5 - 13	9
13 - 14.5	10
14.5 - 16	11
16 - 17.5	12
17.5 - 19	13
19 - 20.5	14
20.5 - 22	15
22 - 23.5	16
23.5 - 25	17
25 - 26.5	18
26.5 - 28	19
28 - 29.5	20

\*BASED ON VERTICAL REINFORCEMENT SPACING SHOWN ON SHEET 1.

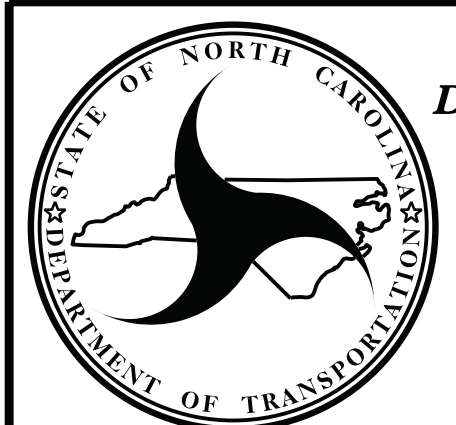
REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V SELECT MATERIAL
1	2400	2400	2400	2400	2400
2	2400	2400	2400	2400	2400
3	2400	2400	2400	2400	2400
4	2400	2400	2500	2400	2400
5	2500	2400	3000	2400	2400
6	3000	2400	3500	2800	2400
7	3500	2700	4000	3200	2600
8	4000	3100	4500	3600	2900
9	4500	3500	5000	4000	3200
10	5000	3900	5500	4400	3500
11	5500	4300	6000	4800	3800
12	6000	4700	6500	5200	4100
13	6500	5100	7000	5600	4400
14	7000	5400	7500	6000	4700
15	7500	5800	8000	6400	5000
16	8000	6200	8500	6800	5300
17	8500	6600	9000	7200	5600
18	9000	7000	9500	7600	5900
19	9500	7400	10000	8000	6200
20	10000	7800	10500	8400	6500

**GEOTEXTILE REINFORCEMENT**  
ULTIMATE TENSILE STRENGTH (LB/FT)

REINFORCEMENT LAYER NUMBER*	SHORING BACKFILL TYPE IN THE REINFORCED ZONE (SEE NOTE 7 ON SHEET 2)				
	SLOPE CASE		SURCHARGE CASE		
	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL	A-2-4 SOIL	CLASS II, TYPE I OR CLASS III SELECT MATERIAL	CLASS V OR CLASS VI SELECT MATERIAL
1	240	200	340	290	240
2	380	310	520	430	350
3	530	420	700	570	460
4	690	550	870	720	570
5	860	690	1050	860	680
6	1030	830	1220	1000	790
7	1200	970	1400	1150	900
8	1370	1110	1580	1290	1010
9	1550	1240	1750	1430	1120
10	1720	1380	1930	1580	1230
11	1890	1520	2100	1720	1340
12	2060	1660	2280	1860	1450
13	2240	1800	2450	2010	1560
14	2410	1940	2630	2150	1670
15	2580	2080	2800	2290	1780
16	2750	2220	2980	2440	1890
17	2930	2360	3160	2580	2000
18	3100	2500	3330	2720	2110
19	3270	2640	3510	2860	2220
20	3440	2780	3690	3000	2330

**GEOGRID REINFORCEMENT**  
SHORT-TERM DESIGN STRENGTH (LB/FT)  
(SEE NOTE 10 ON SHEET 2.)

**MINIMUM REQUIRED REINFORCEMENT STRENGTH IN MD**  
(SEE NOTE 9 ON SHEET 2.)  
\*SEE PARTIAL ELEVATION ON SHEET 1 FOR REINFORCEMENT LAYER NUMBERING.



NORTH CAROLINA  
DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS

**GEOTECHNICAL  
ENGINEERING UNIT**

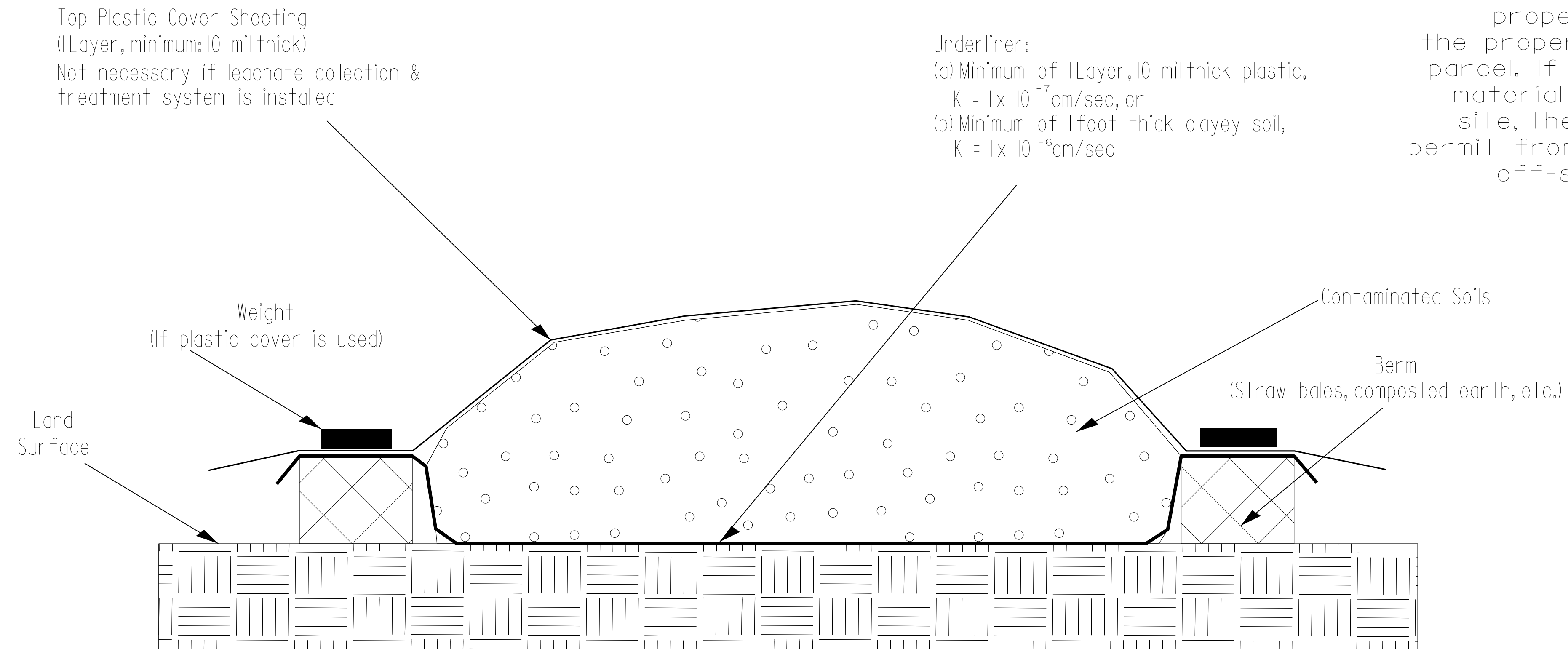
STANDARD DETAIL NO. 1801.02

STANDARD  
TEMPORARY WALL  
SHEET 3 OF 3

DATE: 11-19-13

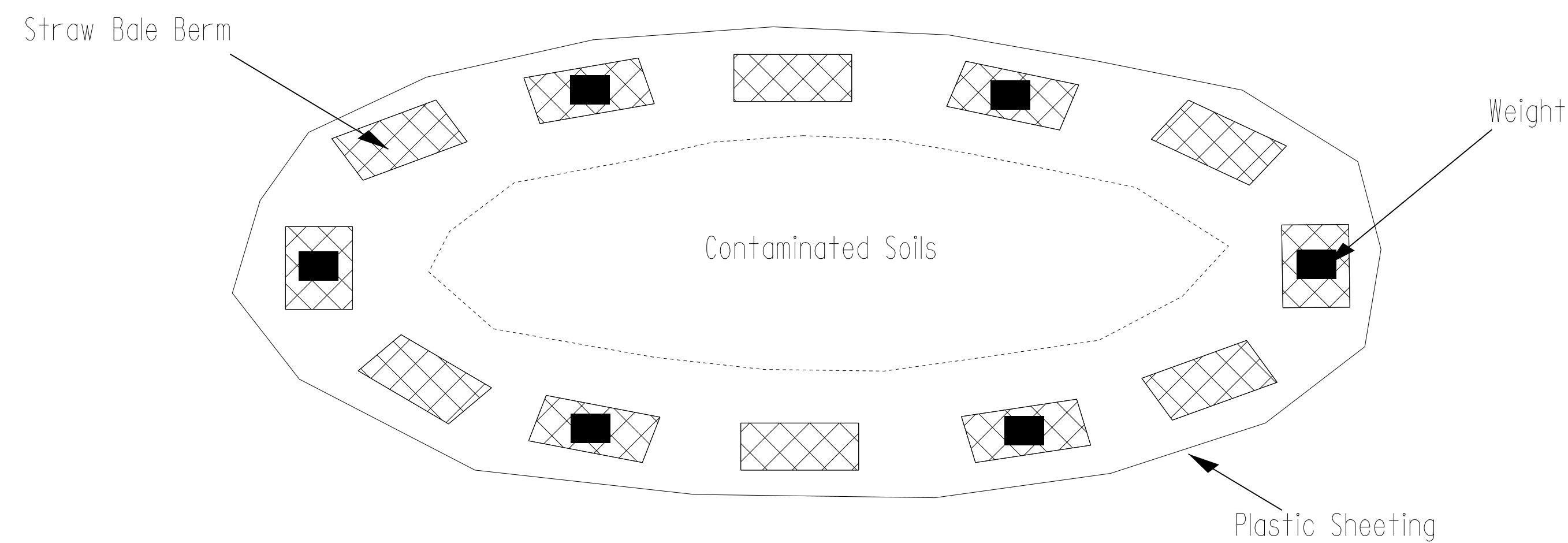
## Detail for Temporary Containment of Contaminated Soil

### Cross-Section View



**NOTE:**  
The Contractor shall stockpile all contaminated soil excavated from a property in a location within the property boundaries of the source parcel. If the volume of contaminated material exceeds available space on site, the Contractor shall obtain a permit from the NCDENR UST Section for off-site temporary storage.

### Map View



PREPARED BY:	DATE:
REVIEWED BY:	DATE:

**GEOTECHNICAL ENGINEERING UNIT**

EASTERN REGIONAL OFFICE  
 WESTERN REGIONAL OFFICE  
 CONTRACT OFFICE

STATE OF NORTH CAROLINA  
 DEPARTMENT OF TRANSPORTATION  
 RALEIGH

### STOCKPILE CONTAINMENT DETAIL

REVISIONS					
NO.	BY	DATE	NO.	BY	DATE
1			3		
2			4		

# SUMMARY OF EARTHWORK IN CUBIC YARDS

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L- Sta. 11+35 TO -L- 41+35	6,169		17,521	11,352	
-DR1- Sta. 10+40 TO 11+96.77	12		2,233	2,221	
SUBTOTAL	6,181		19,754	13,573	
-L- Sta. 41+35 TO -L- 71+35	50,034		11,729		38,305
-DR2- Sta. 10+10 TO 11+25.49			531	531	
-DR3- Sta. 10+25 TO 11+75.51	78		499	421	
-DR4- Sta. 10+11 TO 11+19.65	130		5		125
-Y1- Sta. 10+00 TO 13+38.85	7,585				7,585
-Y1Temp- Sta. 12+70.82 TO 13+85	830				830
-Y1Temp- Removal Sta. 12+70.82 TO 13+85			955	955	
-DR8- Sta. 10+05 TO 10+72.13	434				434
-DR9- Sta. 10+20 TO 11+20	41		29		12
-LTemp- Sta. 62+39.88 TO 71+36.51	10,643		12		10,631
-LTemp- Removal Sta. 62+39.88 TO 71+36.51	15		3,160	3,145	
SUBTOTAL	69,790		16,920	5,052	57,922
-L- Sta. 71+35 TO -L- 101+35	11,797	1,014	38,905	27,108	1,014
-Y2- Sta. 10+11 TO 12+15	903		672		231
-DR4- Sta. 11+19.65 TO 12+68	40		16		24
-DR5- Sta. 10+00 TO 11+93.98	2,428				2,428
-DR5Temp- Sta. 10+00 TO 11+22.39	47		17		30
-DR5Temp- Removal Sta. 10+00 TO 11+22.39	15		54	39	
-DR5A- Sta. 10+00 TO 11+00	3,277				3,277
-Y3- Sta. 10+15.45 TO 15+65	19,198				19,198
-DR6- Sta. 10+09 TO 12+00	3,548		67		3,481
-DR7- Sta. 10+00 TO 12+75.44	978				978
-Y4- Sta. 10+75 TO 12+02.49	100		78		22
-LTemp- Sta. 71+36.51 TO 88+26.70	9,056		7,559		1,497
-LTemp- Removal Sta. 71+36.51 TO 88+26.70	800		4,901	4,101	
SUBTOTAL	52,187	1,014	52,268	31,248	32,180

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L- Sta. 101+35 TO 127+25	1,600		14,860	13,260	
-Y5- Sta. 10+11.73 TO 11+25	164		39		125
SUBTOTAL	1,764		14,899	13,260	125
TOTAL	129,922	1,014	103,842	63,134	90,228
LOSS DUE TO CLEARING AND GRUBBING	-5,500				-5,500
ADDITIONAL UNDERCUT		3,850	4,428	4,428	3,850
ROCK WASTE TO OFFSET BORROW				-610	-610
ADJUST FOR ROCK WASTE			-92	-92	
WASTE IN LIEU OF BORROW				-66,860	-66,860
GRAND TOTAL	124,422	4,864	108,178	0	21,108
SAY	125,000	4,900			

EST. DDE = 710 CY  
 EST. SHALLOW UNDERCUT = 100 CY  
 EST. CLASS IV SUBGRADE STABILIZATION = 200 TONS  
 PAVEMENT STRUCTURE VOLUME = 3640 CY

## PAVEMENT REMOVAL SUMMARY IN SQUARE YARDS

SURVEY LINE	STATION	STATION	LOCATION LTRT/CL	ASPHALT REMOVAL	ASPHALT BREAKUP
-L-	11+35	14+87	LT	265.0	
-L-	14+95	24+38	RT	447.4	
-L-	24+40	27+50	LT	311.0	
-L-	27+40	30+30	RT	727.4	
-L-	30+42	39+40	RT	1055.4	
-L-	48+06	50+76	RT	110.7	
-L-	50+73	61+10	LT	2374.1	
-L-	63+45	72+15	RT	1821.0	
-Y1--L-	10+00	72+20	LT	1025.6	
-L-	61+55	63+35	RT		286.6
-L-	70+70	78+25	CL		1684.4
-L-	72+30	72+60	CL		236.6
-L-	78+25	86+00	CL	1915.7	
-Y2-	10+10	12+10	LT	584.8	
-DR5-	10+00	11+96	CL	440.2	
-Y3-	10+25	15+05	LT	960.8	
-L-	86+95	107+85	RT	907.0	
-L-	108+43	127+25	LT	2454.0	
-LTEMP-	62+54	73+50	CL	1810.2	
-LTEMP-	73+50	78+00	CL		1000.0
-LTEMP-	78+00	87+31	CL	1779.7	
-Y1TEMP-	12+70.82	13+52.08	CL	162.5	
-DR5TEMP-	10+00	11+22.39	CL	244.8	
			TOTAL	19,397.4	3,207.6
			SAY	19,400	3,210















COMPUTED BY: PHS DATE: 3-19-15  
 CHECKED BY: JCL DATE: 3-19-15

**(3-19-15)**

PROJECT NO. R-3622B	SHEET NO. 3G-1
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**STATE OF NORTH CAROLINA  
 DIVISION OF HIGHWAYS**

**SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION**

LINE	Station	Station	Aggregate Type ASU/AST	Aggregate Thickness INCHES	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Soil Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
L	28+75	33+24					1500		
L	96+30	96+56					100		
L	29+83						170		
L	73+89						360		
L	113+96						70		
	CONTINGENCY				100	200	11000		
			<b>TOTAL CY/TONS/SY:</b>		100	200	13200*	0	0

ASU = Aggregate Subgrade, AST = Aggregate Stabilization  
 \*Total square yards of Geotextile for Soil Stabilization is only the estimated quantity for ASU/AST and may only represent a portion of the geotextile quantity shown in the Item Sheets of the Proposal.

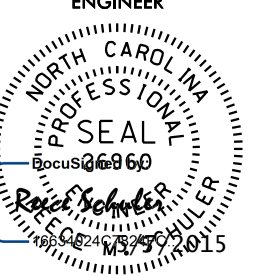
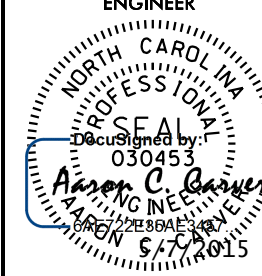


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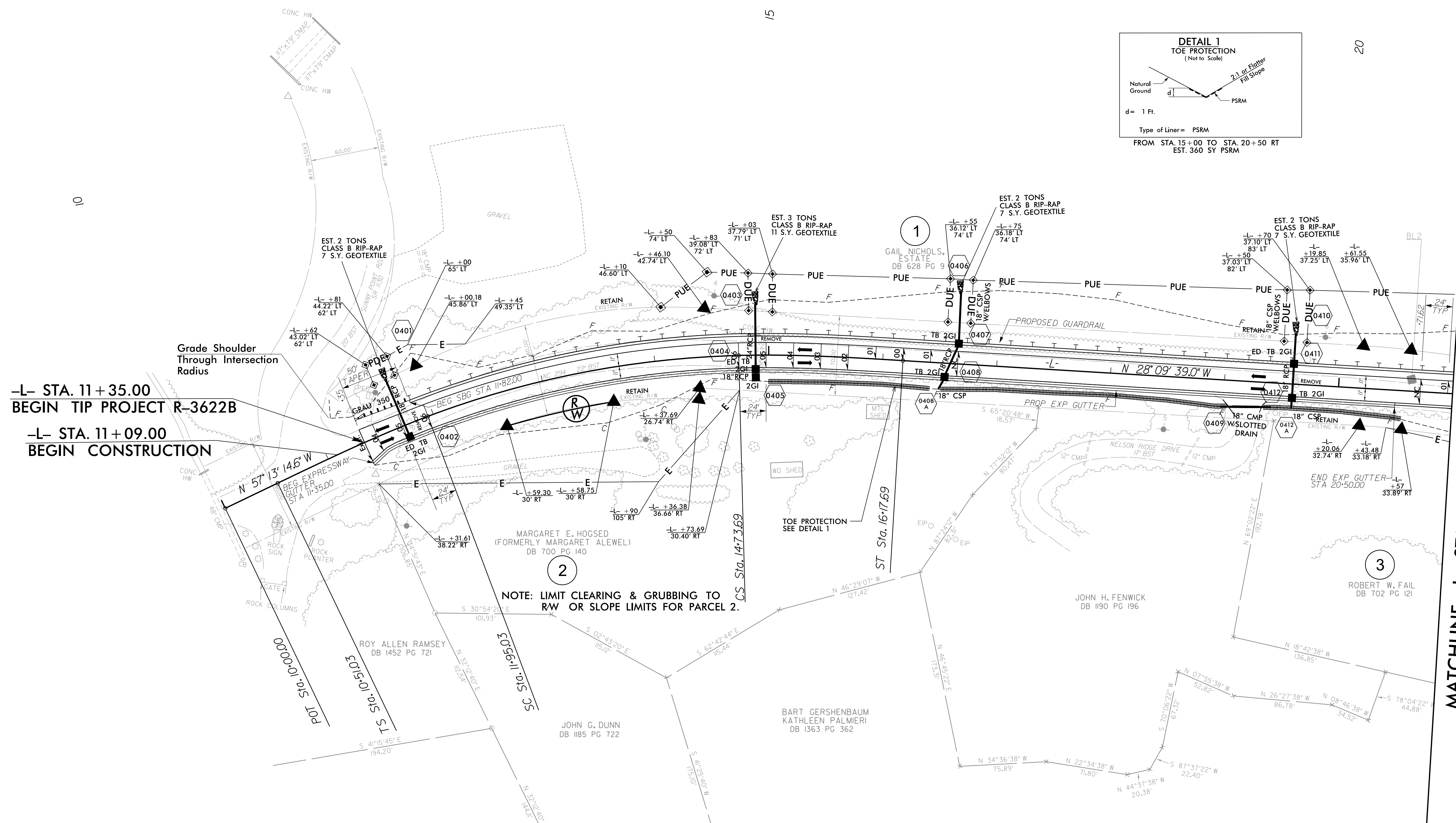
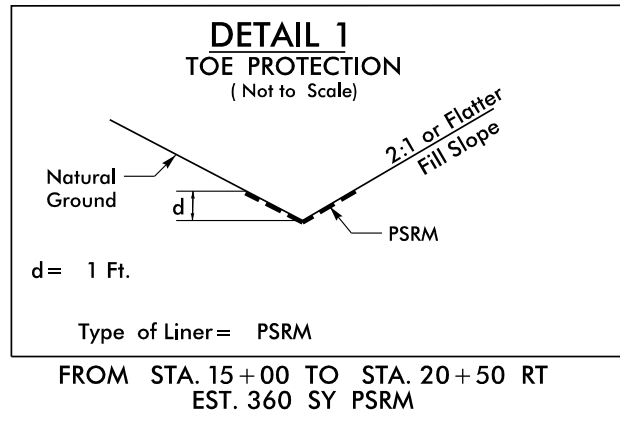
Asheville, North Carolina  
 828-253-2796

Charlotte, North Carolina  
 104-357-0488  
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PROJECT REFERENCE NO. R-3622B	SHEET NO. 4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
 SEAL 030453 R. S. Schuler 08/15	 SEAL 030453 A. C. B... 08/15

NAD 83/NSRS 2007



-L- STA. 11+35.00  
 BEGIN TIP PROJECT R-3622B

-L- STA. 11+09.00  
 BEGIN CONSTRUCTION

MATCHLINE -L- STA. 21+00 SEE SHEET 5

**DATUM DESCRIPTION**

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCGS FOR MONUMENT "SMOKEY" WITH NAD 83/NSRS 2007 STATE PLANE GRID COORDINATES OF NORTHING: 521488.4600(ft) EASTING: 448118.6300(ft) ELEVATION: 1807.451(ft)

THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.99979974

THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "SMOKEY" TO -L- STATION 10+00.00 IS S 20°05'53" E 5725.99'

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 88

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

-L-

Pls Sta 11+47.07 @s = 4° 57' 01.3" Ls = 144.00' LT = 96.04' ST = 48.03'	Pls Sta 13+35.67 Δ = 19° 09' 33.1" (RT) D = 6° 52' 31.8" L = 278.66' T = 140.64' R = 833.33'	Pls Sta 15+21.73 @s = 4° 57' 01.3" Ls = 144.00' LT = 96.04' ST = 48.03'
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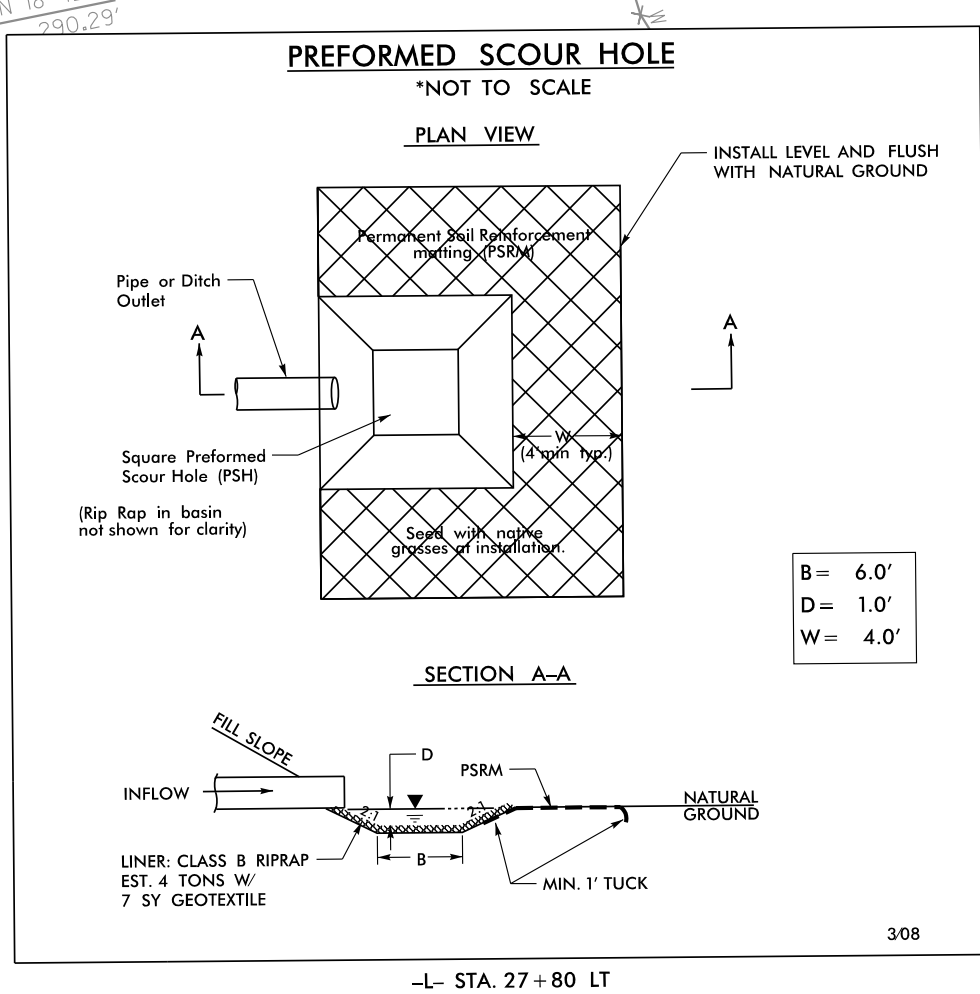
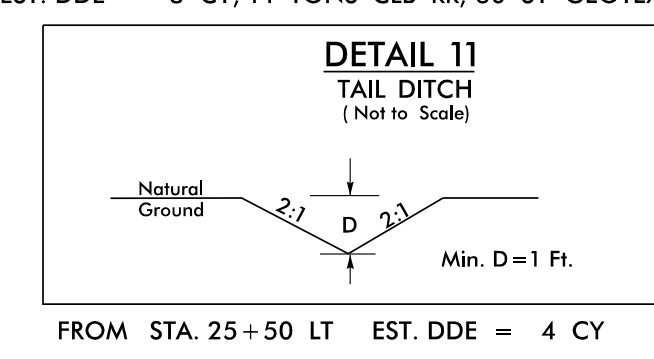
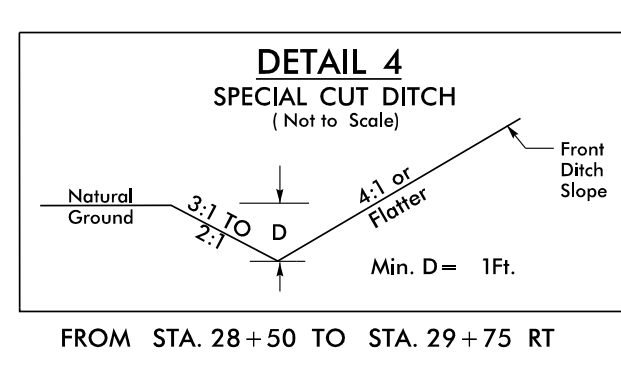
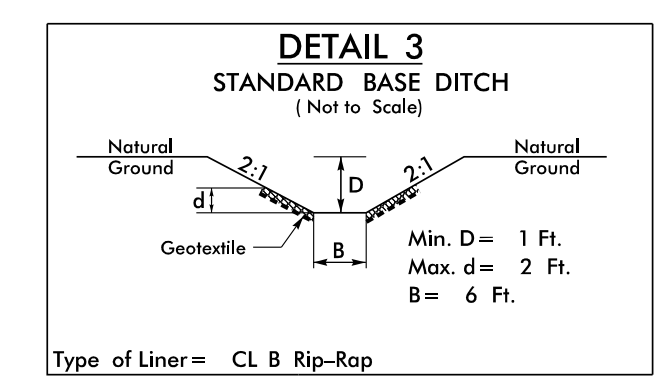
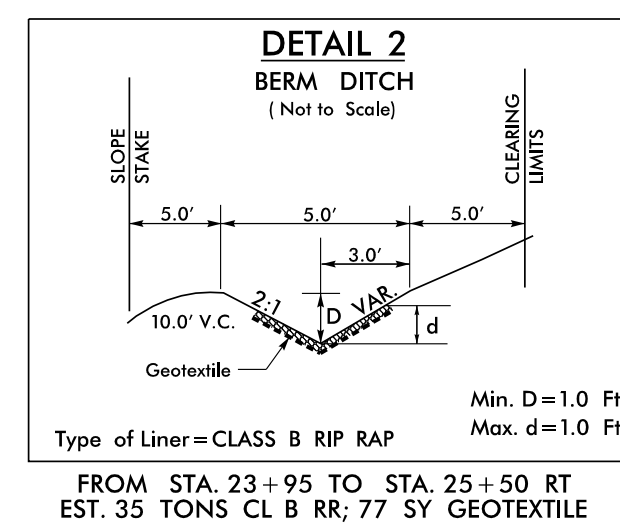
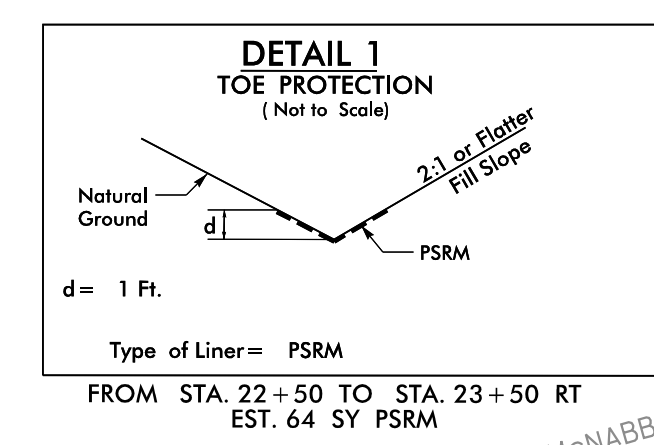
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Knoxville, Tennessee 37919  
Middlesboro, Kentucky 40349  
Spartanburg, South Carolina 29583

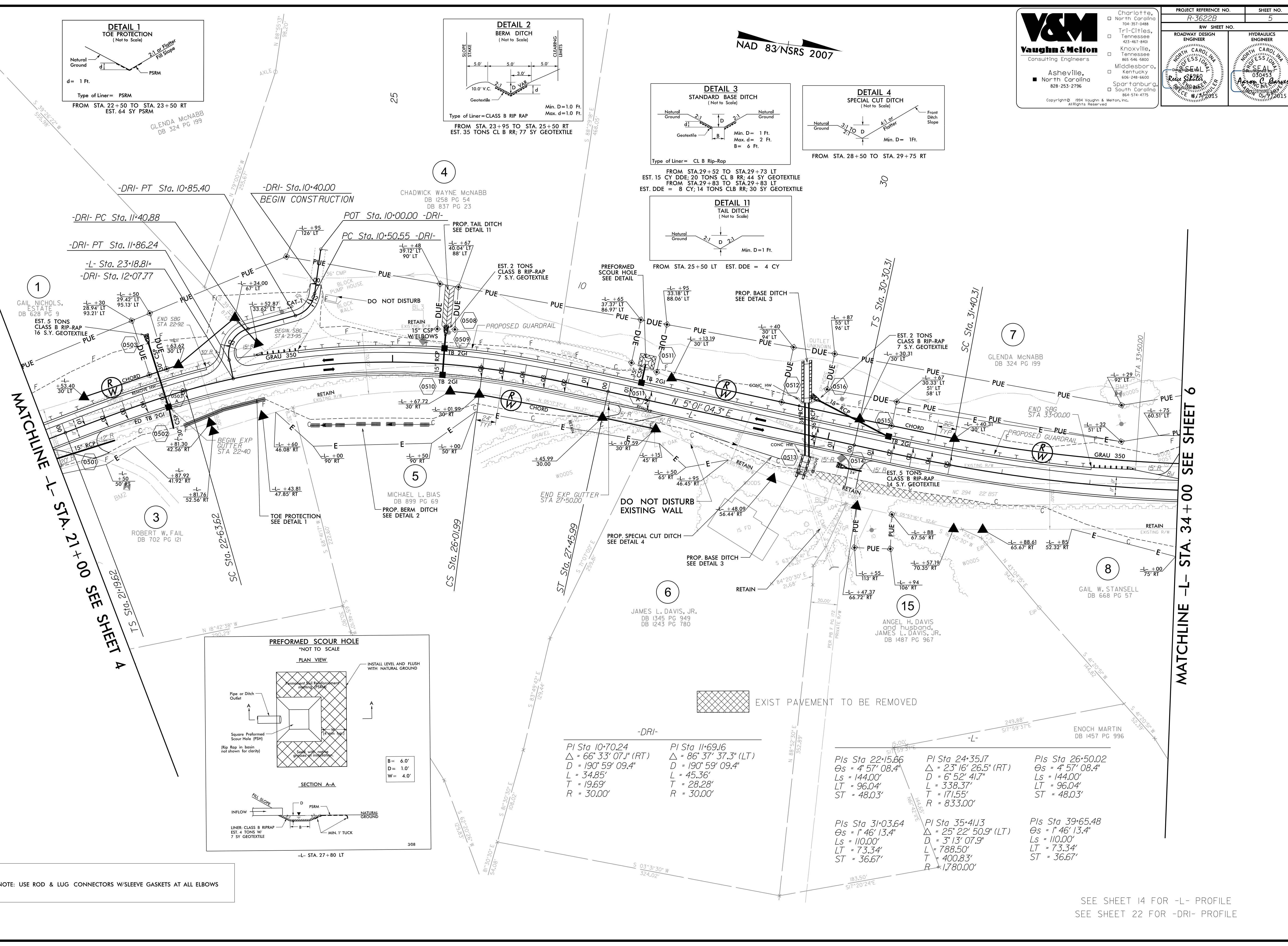
Asheville, North Carolina 28801  
828.253.2796

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PROJECT REFERENCE NO. R-3622B	SHEET NO. 5
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS



<p>PI Sta 10+70.24 Δ = 66° 33' 07.1" (RT) D = 190' 59' 09.4" L = 34.85' T = 19.69' R = 30.00'</p>	<p>PI Sta 11+69.16 Δ = 86° 37' 37.3" (LT) D = 190' 59' 09.4" L = 45.36' T = 28.28' R = 30.00'</p>	<p>PIs Sta 22+15.66 Δs = 4° 57' 08.4" Ls = 144.00' LT = 96.04' ST = 48.03'</p>	<p>PI Sta 24+35.17 Δ = 23° 16' 26.5" (RT) D = 6' 52' 41.7" L = 338.37' T = 171.55' R = 833.00'</p>	<p>PIs Sta 26+50.02 Δs = 4° 57' 08.4" Ls = 144.00' LT = 96.04' ST = 48.03'</p>
<p>PIs Sta 31+03.64 Δs = 1° 46' 13.4" Ls = 110.00' LT = 73.34' ST = 36.67'</p>	<p>PI Sta 35+41.13 Δ = 25° 22' 50.9" (LT) D = 3' 13' 07.9" L = 788.50' T = 400.83' R = 1,780.00'</p>	<p>PIs Sta 39+65.48 Δs = 1° 46' 13.4" Ls = 110.00' LT = 73.34' ST = 36.67'</p>		

SEE SHEET 14 FOR -L- PROFILE  
SEE SHEET 22 FOR -DRI- PROFILE

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Knoxville, Tennessee  
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Middlesboro, Kentucky  
 606-248-6600

Spartanburg, South Carolina  
 864-574-4775

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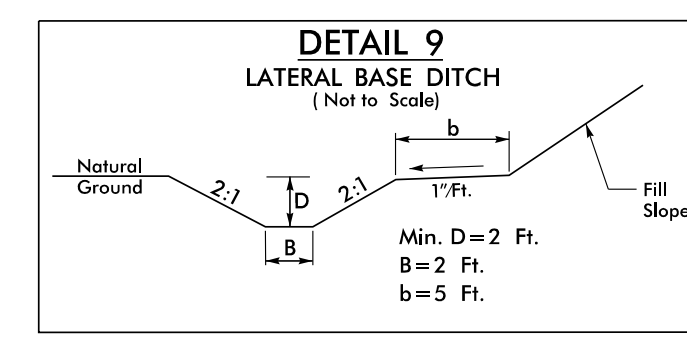
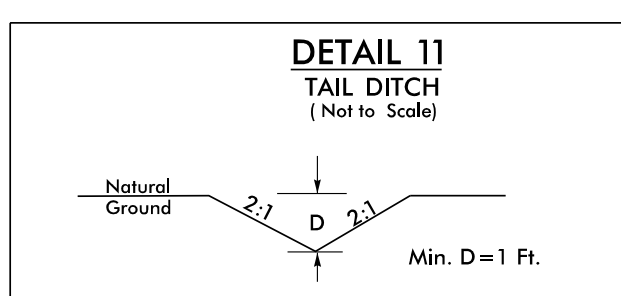
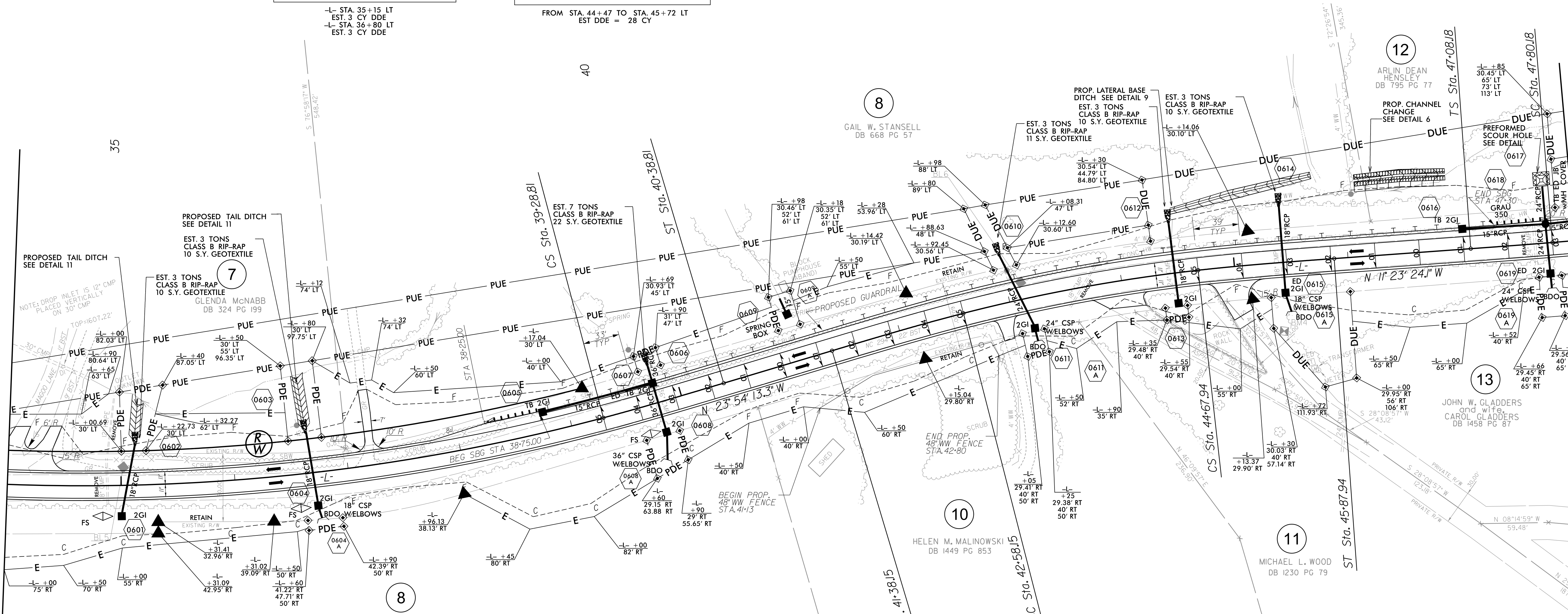
PROJECT REFERENCE NO. R-3622B  
 SHEET NO. 6

ROADWAY DESIGN ENGINEER  
 NORTH CAROLINA PROFESSIONAL SEAL  
 030453

HYDRAULICS ENGINEER  
 NORTH CAROLINA PROFESSIONAL SEAL  
 030453

MATCHLINE -L- STA. 34+00 SEE SHEET 5

MATCHLINE -L- STA. 48+00 SEE SHEET 7

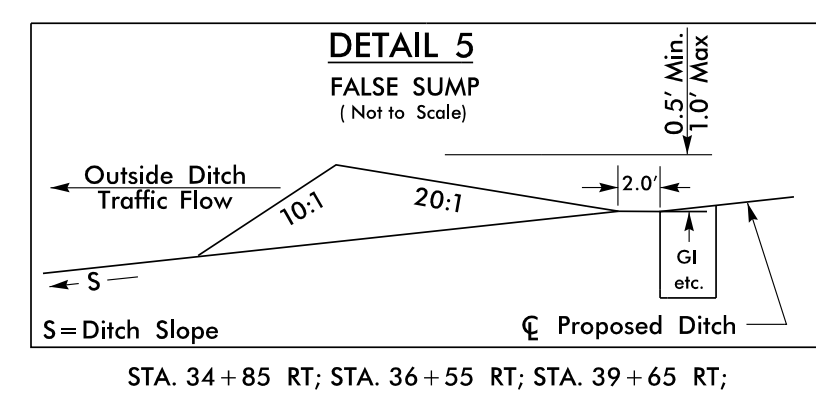


FROM STA. 44+47 TO STA. 45+72 LT  
 EST DDE = 28 CY

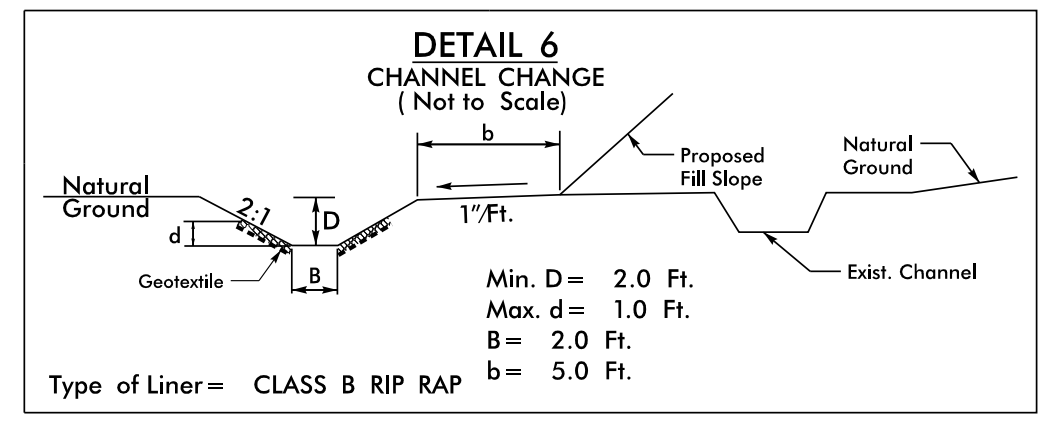
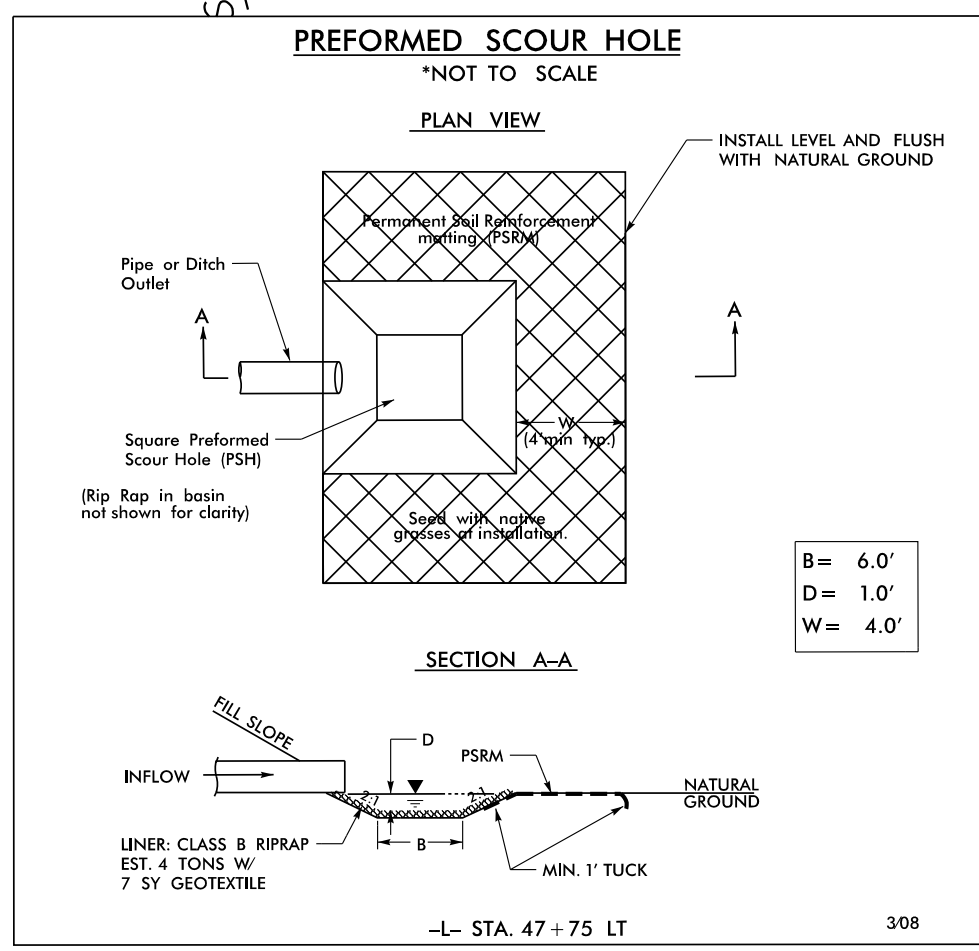
Pis Sta 31+03.64 $\Theta_s = 1^\circ 46' 13.4''$ $L_s = 110.00'$ $LT = 73.34'$ $ST = 36.67'$	Pi Sta 35+41.13 $\Delta = 25^\circ 22' 50.9''$ (LT) $D = 3' 13' 07.9''$ $L = 788.50'$ $T = 400.83'$ $R = 1,780.00'$	Pis Sta 39+65.48 $\Theta_s = 1^\circ 46' 13.4''$ $L_s = 110.00'$ $LT = 73.34'$ $ST = 36.67'$
--	--	--

Pis Sta 42+18.15 $\Theta_s = 2^\circ 16' 36.0''$ $L_s = 120.00'$ $LT = 80.01'$ $ST = 40.01'$	Pi Sta 43+63.21 $\Delta = 7^\circ 57' 37.3''$ (RT) $D = 3' 47' 39.9''$ $L = 209.79'$ $T = 105.06'$ $R = 1,510.00'$	Pis Sta 45+07.94 $\Theta_s = 2^\circ 16' 36.0''$ $L_s = 120.00'$ $LT = 80.01'$ $ST = 40.01'$
--	---	--

Pis Sta 47+56.18 $\Theta_s = 0^\circ 35' 33.8''$ $L_s = 72.00'$ $LT = 48.00'$ $ST = 24.00'$	Pi Sta 50+03.78 $\Delta = 7^\circ 21' 10.1''$ (LT) $D = 1' 38' 47.1''$ $L = 446.59'$ $T = 223.60'$ $R = 3,480.00'$	Pis Sta 52+50.77 $\Theta_s = 0^\circ 35' 33.8''$ $L_s = 72.00'$ $LT = 48.00'$ $ST = 24.00'$
---	---	---



NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS



SEE SHEET 15 FOR -L- PROFILE

REVISIONS

8/17/09

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Asheville, North Carolina  
 828-253-2196

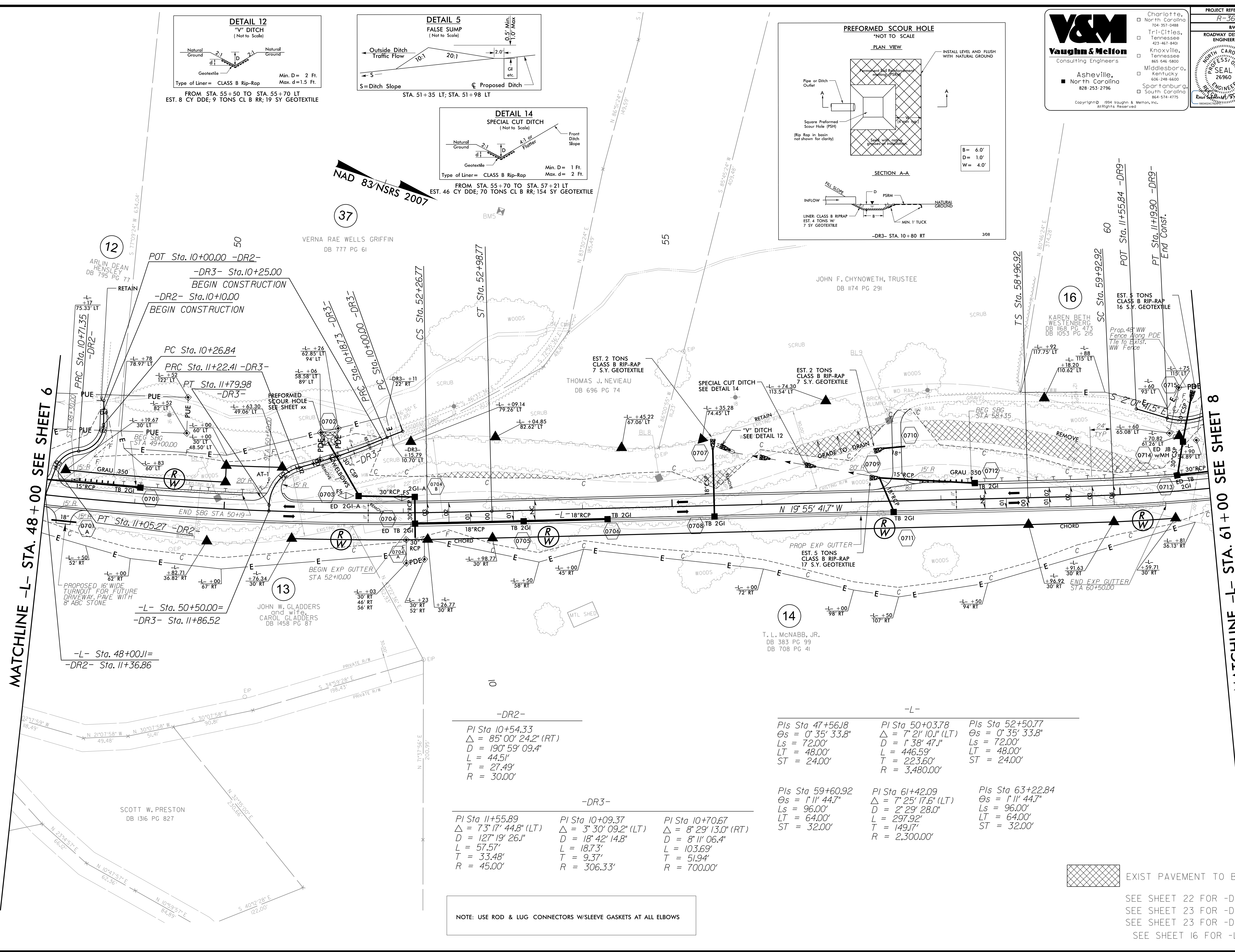
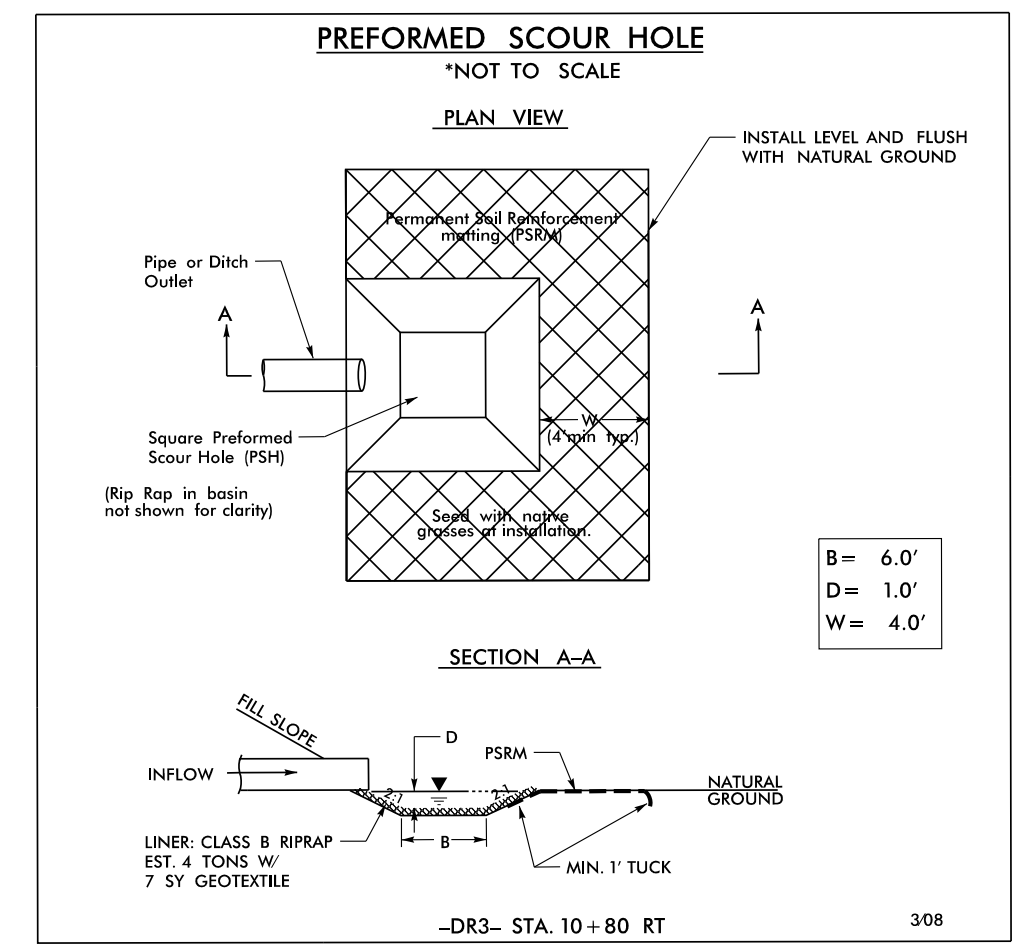
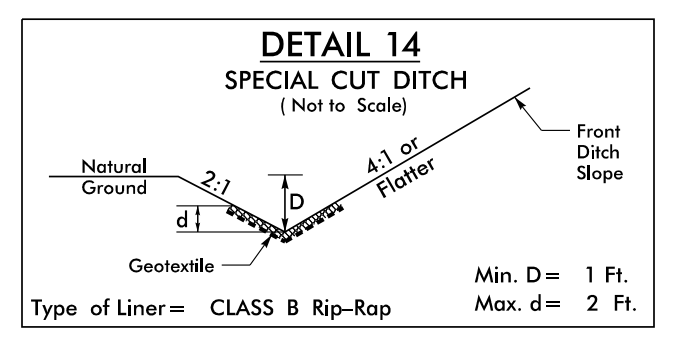
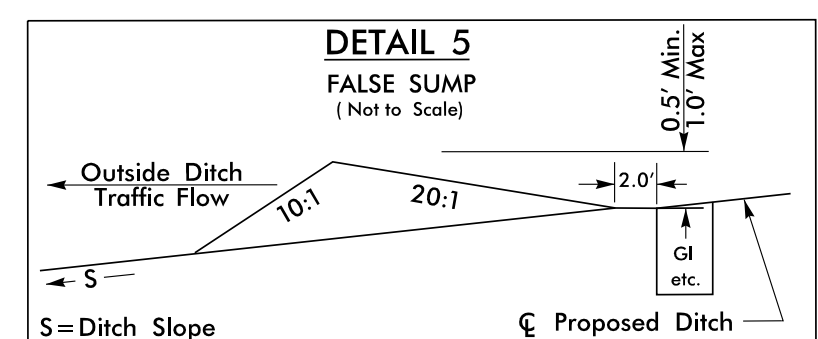
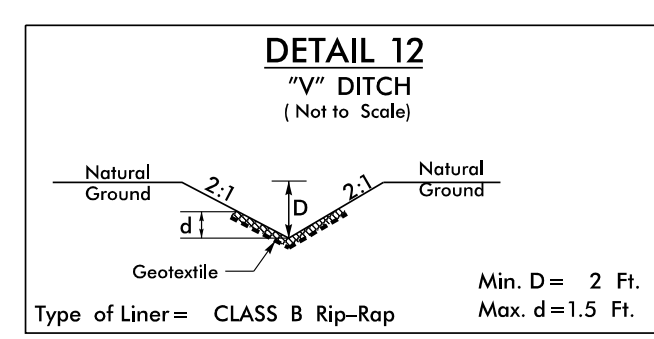
Charlotte, North Carolina 704-351-0488  
 Tri-Cities, Tennessee 423-467-8401  
 Knoxville, Tennessee 865-546-5800  
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PROJECT REFERENCE NO. R-3622B  
 SHEET NO. 7

ROADWAY DESIGN ENGINEER  
 PROFESSIONAL SEAL  
 26960

HYDRAULICS ENGINEER  
 PROFESSIONAL SEAL  
 030453



**-DR2-**

PI Sta 10+54.33  
 $\Delta = 85^{\circ}00' 24.2''$  (RT)  
 $D = 190^{\circ}59' 09.4''$   
 $L = 44.51'$   
 $T = 27.49'$   
 $R = 30.00'$

**-L-**

Pls Sta 47+56.8	PI Sta 50+03.78	Pls Sta 52+50.77
$\Delta = 0^{\circ}35' 33.8''$	$\Delta = 7^{\circ}21' 10.1''$ (LT)	$\Delta = 0^{\circ}35' 33.8''$
$Ls = 72.00'$	$D = 1^{\circ}38' 47.1''$	$Ls = 72.00'$
$LT = 48.00'$	$L = 446.59'$	$LT = 48.00'$
$ST = 24.00'$	$T = 223.60'$	$ST = 24.00'$
	$R = 3,480.00'$	

**-DR3-**

PI Sta 11+55.89	PI Sta 10+09.37	PI Sta 10+70.67
$\Delta = 73^{\circ}17' 44.8''$ (LT)	$\Delta = 3^{\circ}30' 09.2''$ (LT)	$\Delta = 8^{\circ}29' 13.0''$ (RT)
$D = 127^{\circ}19' 26.1''$	$D = 18^{\circ}42' 14.8''$	$D = 8^{\circ}11' 06.4''$
$L = 57.57'$	$L = 18.73'$	$L = 103.69'$
$T = 33.48'$	$T = 9.37'$	$T = 51.94'$
$R = 45.00'$	$R = 306.33'$	$R = 700.00'$

EXIST PAVEMENT TO BE REMOVED

SEE SHEET 22 FOR -DR2- PROFILE  
 SEE SHEET 23 FOR -DR3- PROFILE  
 SEE SHEET 23 FOR -DR9- PROFILE  
 SEE SHEET 16 FOR -L- PROFILE

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS



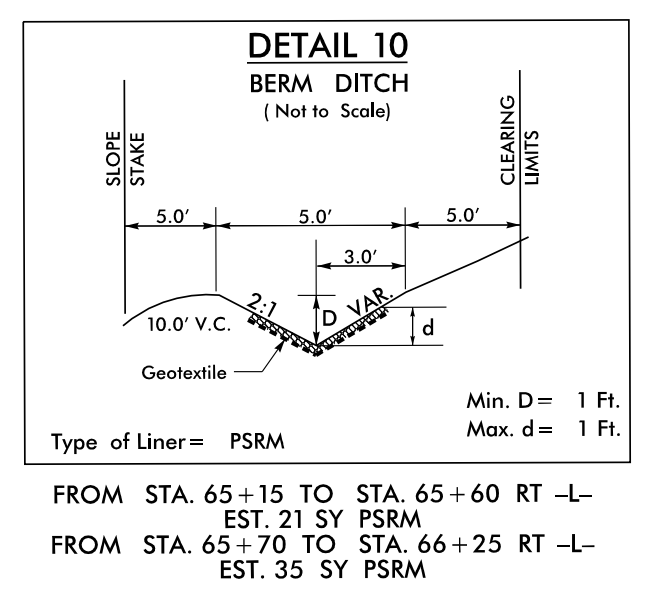
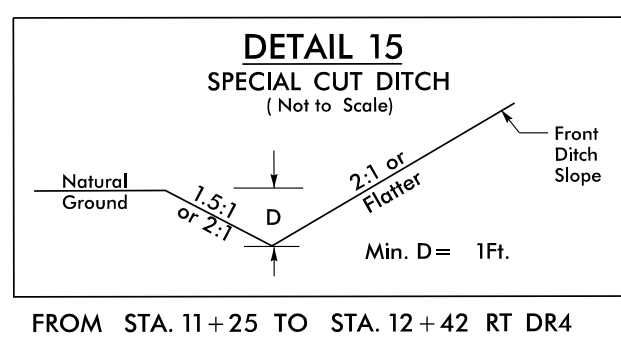
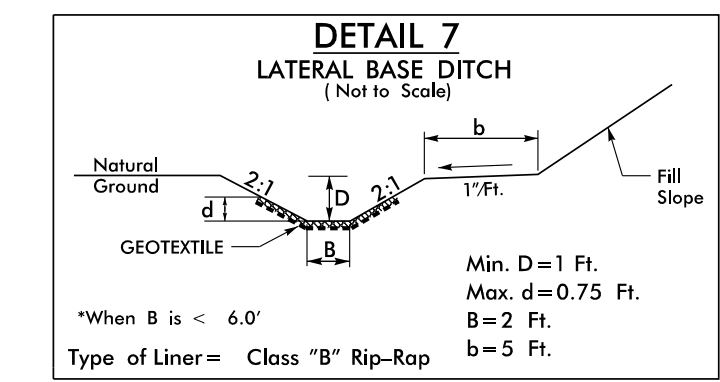
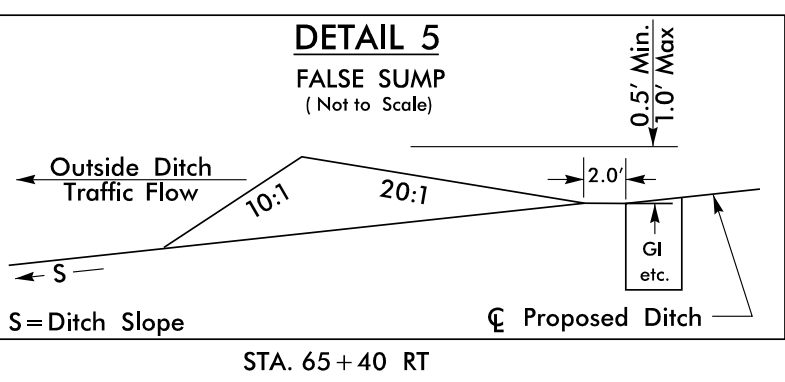
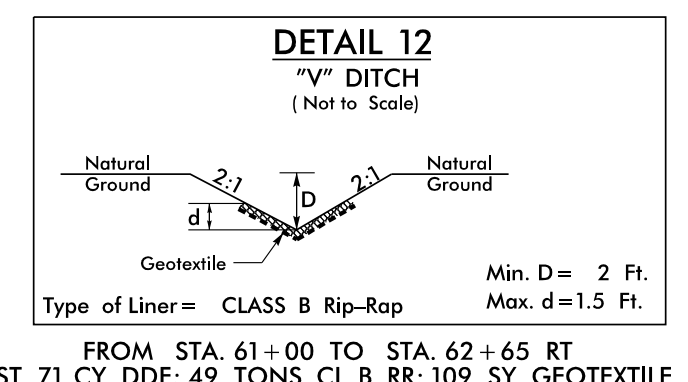
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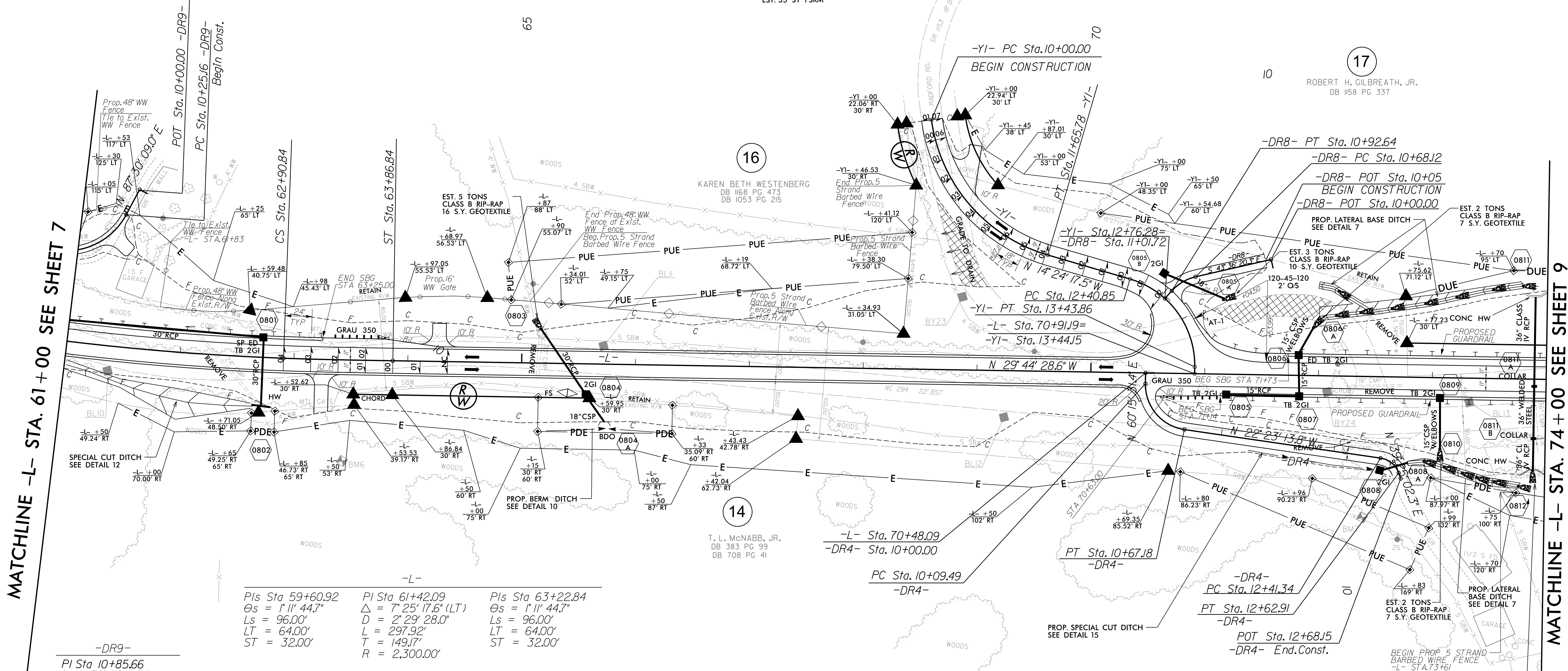
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PROJECT REFERENCE NO.	R-3622B
SHEET NO.	8
ROADWAY DESIGN ENGINEER	SEAL 26960
HYDRAULICS ENGINEER	SEAL 030453



-Y1-

PI Sta 10+93.44	PI Sta 12+93.44
Δ = 65° 57' 36.5" (LT)	Δ = 66° 39' 05.0" (RT)
D = 39° 47' 19.4"	D = 70° 44' 07.9"
L = 165.78'	L = 94.23'
T = 93.44'	T = 53.26'
R = 144.00'	R = 81.00'



MATCHLINE -L- STA. 61+00 SEE SHEET 7

MATCHLINE -L- STA. 74+00 SEE SHEET 9

-DR9-  
 PI Sta 10+85.66  
 Δ = 90° 28' 09.5" (RT)  
 D = 95° 29' 34.7"  
 L = 94.74'  
 T = 60.49'  
 R = 60.00'

-L-	-DR4-	-DR8-
PIs Sta 59+60.92	PI Sta 61+42.09	PIs Sta 63+22.84
Δs = 1° 11' 44.7"	Δ = 7° 25' 17.6" (LT)	Δs = 1° 11' 44.7"
Ls = 96.00'	D = 2° 29' 28.0"	Ls = 96.00'
LT = 64.00'	L = 297.92'	LT = 64.00'
ST = 32.00'	T = 149.17'	ST = 32.00'
	R = 2,300.00'	

PI Sta 10+44.66	PI Sta 12+53.31
Δ = 82° 38' 45.3" (LT)	Δ = 61° 48' 16.1" (RT)
D = 143° 14' 22.0"	D = 286° 28' 44.0"
L = 57.70'	L = 21.57'
T = 35.17'	T = 11.97'
R = 40.00'	R = 20.00'

PI Sta 10+80.78
Δ = 35° 07' 06.9" (LT)
D = 143° 14' 22.0"
L = 24.52'
T = 12.66'
R = 40.00'

EXIST PAVEMENT TO BE REMOVED

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

SEE SHEET 8A FOR TEMPORARY CONSTRUCTION  
 SEE SHEET 22 FOR -Y1- PROFILE  
 SEE SHEET 23 FOR -DR4- & -DR8- PROFILE  
 SEE SHEET 17 FOR -L- PROFILE

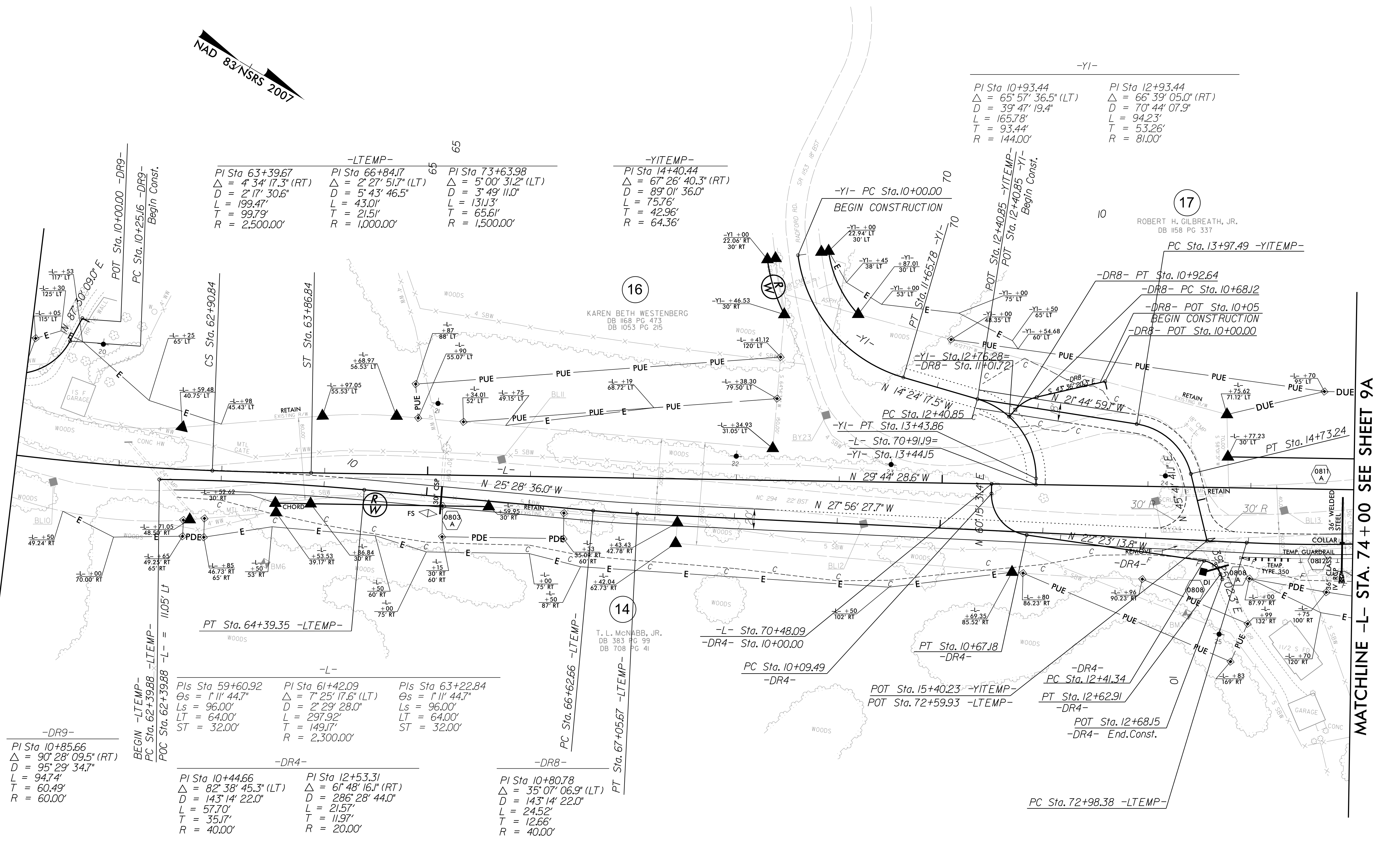
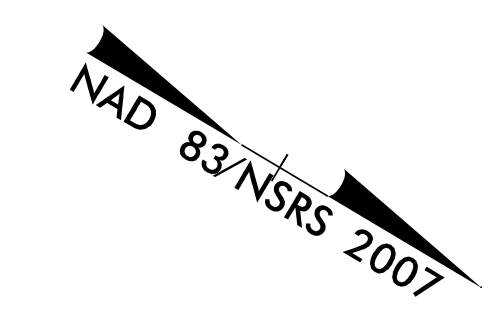
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PROJECT REFERENCE NO.	R-3622B
SHEET NO.	8A
ROADWAY DESIGN ENGINEER	SEAL 26960
HYDRAULICS ENGINEER	SEAL 030453



**-YITEMP-**

PI Sta 14+40.44
Δ = 67° 26' 40.3" (RT)
D = 89° 01' 36.0"
L = 75.76'
T = 42.96'
R = 64.36'

**-YI-**

PI Sta 10+93.44	PI Sta 12+93.44
Δ = 65° 57' 36.5" (LT)	Δ = 66° 39' 05.0" (RT)
D = 39° 47' 19.4"	D = 70° 44' 07.9"
L = 165.78'	L = 94.23'
T = 93.44'	T = 53.26'
R = 144.00'	R = 81.00'

**-DR9-**

PI Sta 10+85.66
Δ = 90° 28' 09.5" (RT)
D = 95° 29' 34.7"
L = 94.74'
T = 60.49'
R = 60.00'

**-L-**

PIs Sta 59+60.92	PIs Sta 61+42.09	PIs Sta 63+22.84
Δs = 1° 11' 44.7"	Δ = 7° 25' 17.6" (LT)	Δs = 1° 11' 44.7"
Ls = 96.00'	D = 2° 29' 28.0"	Ls = 96.00'
LT = 64.00'	L = 297.92'	LT = 64.00'
ST = 32.00'	T = 149.17'	ST = 32.00'
	R = 2,300.00'	

**-DR4-**

PI Sta 10+44.66	PI Sta 12+53.31	PI Sta 10+80.78
Δ = 82° 38' 45.3" (LT)	Δ = 61° 48' 16.1" (RT)	Δ = 35° 07' 06.9" (LT)
D = 143° 14' 22.0"	D = 286° 28' 44.0"	D = 143° 14' 22.0"
L = 57.70'	L = 21.57'	L = 24.52'
T = 35.17'	T = 11.97'	T = 12.66'
R = 40.00'	R = 20.00'	R = 40.00'

SEE SHEET 24 FOR -LTEMP- PROFILE  
SEE SHEET 24 FOR -YITEMP- PROFILE

SHEET 8A TO BE USED ONLY FOR TEMPORARY CONSTRUCTION

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MATCHLINE -L- STA. 74 + 00 SEE SHEET 9A

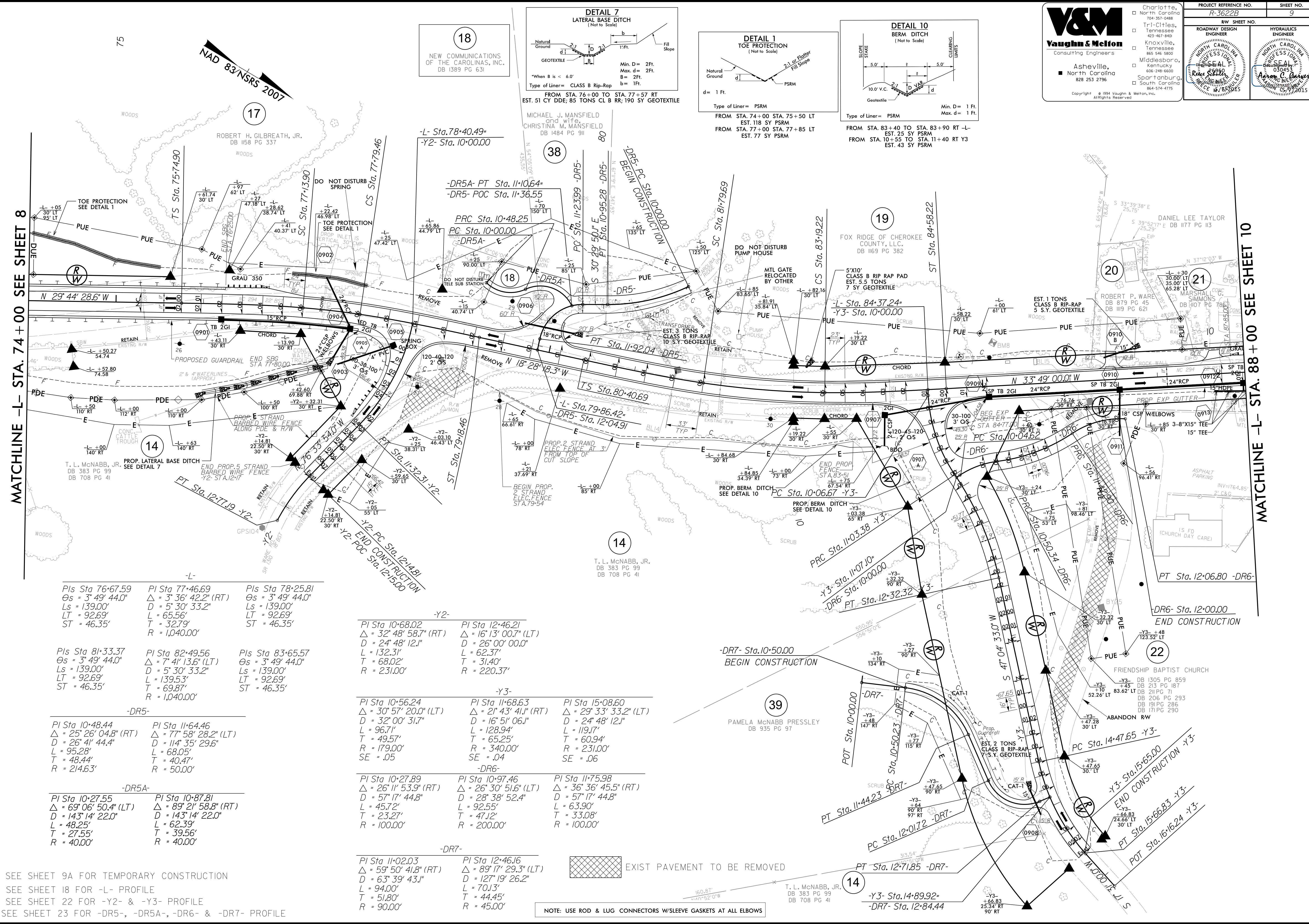
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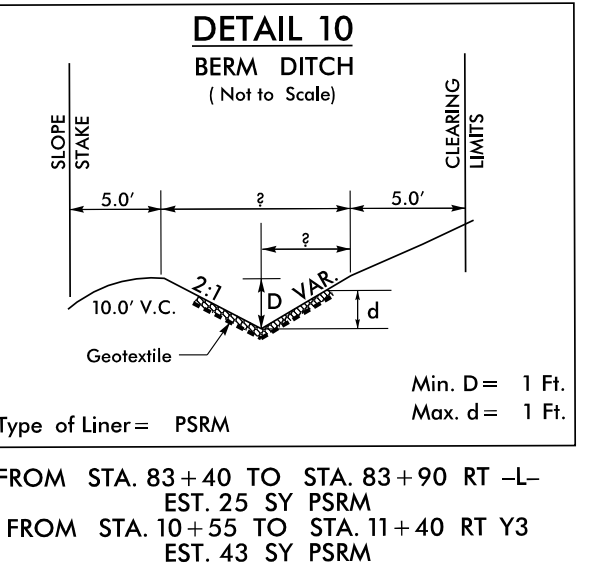
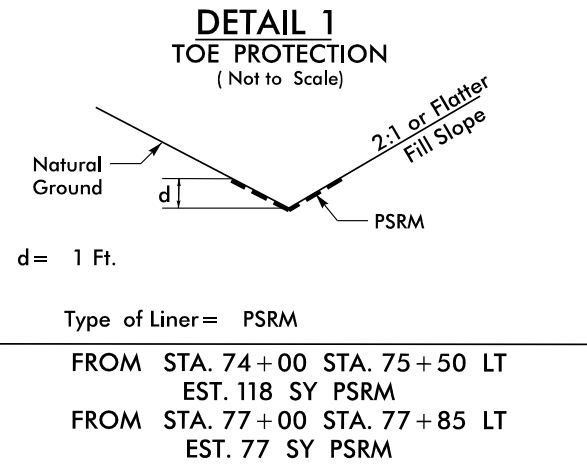
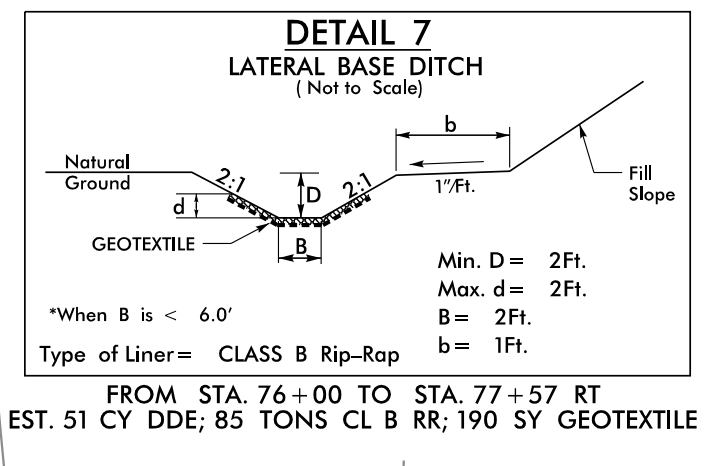
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PROJECT REFERENCE NO. R-3622B	SHEET NO. 9
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



**18**  
NEW COMMUNICATIONS OF THE CAROLINAS, INC.  
DB 1389 PG 631



MATCHLINE -L- STA. 74+00 SEE SHEET 8

MATCHLINE -L- STA. 88+00 SEE SHEET 10

<b>-L-</b> Pls Sta 76+67.59 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'  Pls Sta 81+33.37 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'  -DR5- Pls Sta 10+48.44 Δ = 25° 26' 04.8" (RT) D = 26° 41' 44.4" L = 95.28' T = 48.44' R = 214.63'  Pls Sta 10+87.81 Δ = 89° 21' 58.8" (RT) D = 143° 14' 22.0" L = 62.39' T = 39.56' R = 40.00'	<b>-Y2-</b> Pls Sta 77+46.69 Δ = 3° 36' 42.2" (RT) Ls = 139.00' LT = 92.69' ST = 46.35'  Pls Sta 82+49.56 Δ = 7° 41' 13.6" (LT) D = 5° 30' 33.2" Ls = 139.00' LT = 92.69' ST = 46.35'  -DR6- Pls Sta 10+27.55 Δ = 69° 06' 50.4" (LT) D = 143° 14' 22.0" L = 48.25' T = 27.55' R = 40.00'  Pls Sta 10+87.81 Δ = 89° 21' 58.8" (RT) D = 143° 14' 22.0" L = 62.39' T = 39.56' R = 40.00'	<b>-Y3-</b> Pls Sta 78+25.81 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'  Pls Sta 83+65.57 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'  -DR6- Pls Sta 11+68.63 Δ = 21° 43' 41.1" (RT) D = 16° 51' 06.1" L = 128.94' T = 65.25' R = 340.00'  Pls Sta 11+07.10 Δ = 29° 33' 33.2" (LT) D = 24° 48' 12.1" L = 119.17' T = 60.94' R = 231.00'  -DR7- Pls Sta 10+27.89 Δ = 26° 11' 53.9" (RT) D = 57° 17' 44.8" L = 45.72' T = 23.27' R = 100.00'  Pls Sta 10+97.46 Δ = 28° 38' 52.4" L = 92.55' T = 47.12' R = 200.00'  Pls Sta 11+75.98 Δ = 36° 36' 45.5" (RT) D = 57° 17' 44.8" L = 63.90' T = 33.08' R = 100.00'
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EXIST PAVEMENT TO BE REMOVED

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

SEE SHEET 9A FOR TEMPORARY CONSTRUCTION  
SEE SHEET 18 FOR -L- PROFILE  
SEE SHEET 22 FOR -Y2- & -Y3- PROFILE  
SEE SHEET 23 FOR -DR5-, -DR5A-, -DR6- & -DR7- PROFILE

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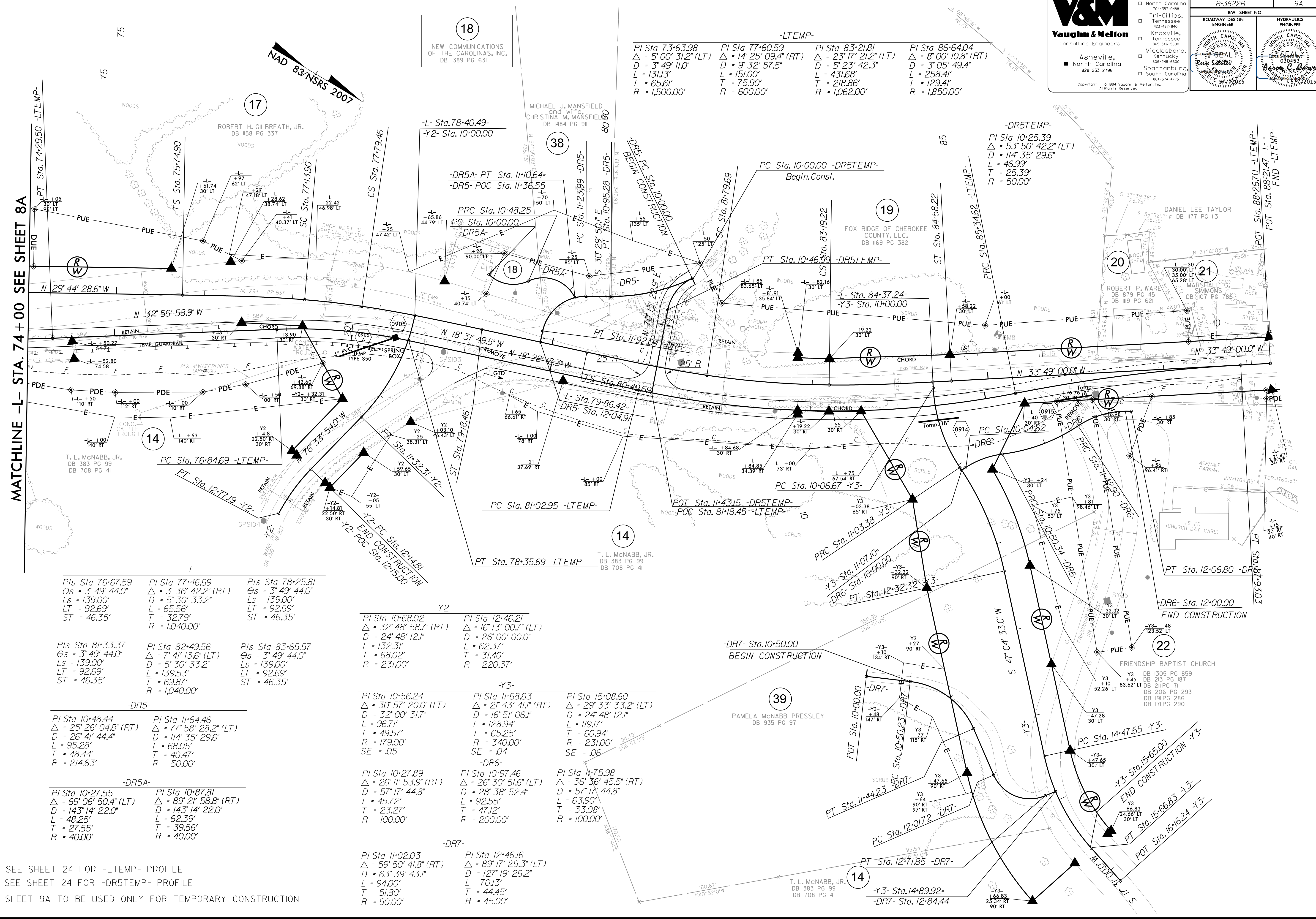
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PROJECT REFERENCE NO.	SHEET NO.
R-3622B	9A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
North Carolina Professional Engineer License No. 52635	North Carolina Professional Engineer License No. 030453

MATCHLINE -L- STA. 74+00 SEE SHEET 8A



18  
NEW COMMUNICATIONS OF THE CAROLINAS, INC.  
DB 1389 PG 631

-LTEMP-			
PI Sta 73+63.98 Δ = 5° 00' 31.2" (LT) D = 3° 49' 11.0" L = 131.13' T = 65.61' R = 1,500.00'	PI Sta 77+60.59 Δ = 14° 25' 09.4" (RT) D = 9° 32' 57.5" L = 151.00' T = 75.90' R = 600.00'	PI Sta 83+21.81 Δ = 23° 17' 21.2" (LT) D = 5° 23' 42.3" L = 431.68' T = 218.86' R = 1,062.00'	PI Sta 86+64.04 Δ = 8° 00' 10.8" (RT) D = 3° 05' 49.4" L = 258.41' T = 129.41' R = 1,850.00'

-DR5TEMP-	
PI Sta 10+25.39 Δ = 53° 50' 42.2" (LT) D = 114° 35' 29.6" L = 46.99' T = 25.39' R = 50.00'	

-L-		
PIs Sta 76+67.59 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'	PIs Sta 77+46.69 Δ = 3° 36' 42.2" (RT) D = 5° 30' 33.2" L = 65.56' T = 32.79' R = 1,040.00'	PIs Sta 78+25.81 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'

-DR5-		
PIs Sta 81+33.37 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'	PIs Sta 82+49.56 Δ = 7° 41' 13.6" (LT) D = 5° 30' 33.2" L = 139.53' T = 69.87' R = 1,040.00'	PIs Sta 83+65.57 Os = 3° 49' 44.0" Ls = 139.00' LT = 92.69' ST = 46.35'

-DR5A-	
PI Sta 10+48.44 Δ = 25° 26' 04.8" (RT) D = 26° 41' 44.4" L = 95.28' T = 48.44' R = 214.63'	PI Sta 11+64.46 Δ = 77° 58' 28.2" (LT) D = 143° 14' 22.0" L = 62.39' T = 40.47' R = 50.00'

-Y2-	
PI Sta 10+68.02 Δ = 32° 48' 58.7" (RT) D = 24° 48' 12.1" L = 132.31' T = 68.02' R = 231.00'	PI Sta 12+46.21 Δ = 16° 13' 00.7" (LT) D = 26° 00' 00.0" L = 62.37' T = 31.40' R = 220.37'

-Y3-		
PI Sta 10+56.24 Δ = 30° 57' 20.0" (LT) D = 32° 00' 31.7" L = 96.71' T = 49.57' R = 179.00' SE = .05	PI Sta 11+68.63 Δ = 21° 43' 41.1" (RT) D = 16° 51' 06.1" L = 128.94' T = 65.25' R = 340.00' SE = .04	PI Sta 15+08.60 Δ = 29° 33' 33.2" (LT) D = 24° 48' 12.1" L = 119.17' T = 60.94' R = 231.00' SE = .06

-DR6-		
PI Sta 10+27.89 Δ = 26° 11' 53.9" (RT) D = 57° 17' 44.8" L = 45.72' T = 23.27' R = 100.00'	PI Sta 10+97.46 Δ = 26° 30' 51.6" (LT) D = 28° 38' 52.4" L = 92.55' T = 47.12' R = 200.00'	PI Sta 11+75.98 Δ = 36° 36' 45.5" (RT) D = 57° 17' 44.8" L = 63.90' T = 33.08' R = 100.00'

-DR7-	
PI Sta 11+02.03 Δ = 59° 50' 41.8" (RT) D = 63° 39' 43.1" L = 94.00' T = 51.80' R = 90.00'	PI Sta 12+46.16 Δ = 89° 17' 29.3" (LT) D = 127° 19' 26.2" L = 70.13' T = 44.45' R = 45.00'

SEE SHEET 24 FOR -LTEMP- PROFILE  
SEE SHEET 24 FOR -DR5TEMP- PROFILE  
SHEET 9A TO BE USED ONLY FOR TEMPORARY CONSTRUCTION

\$\$\$\$\$SYTIME\$\$\$\$\$  
\$\$\$\$\$SERNAME\$\$\$\$\$

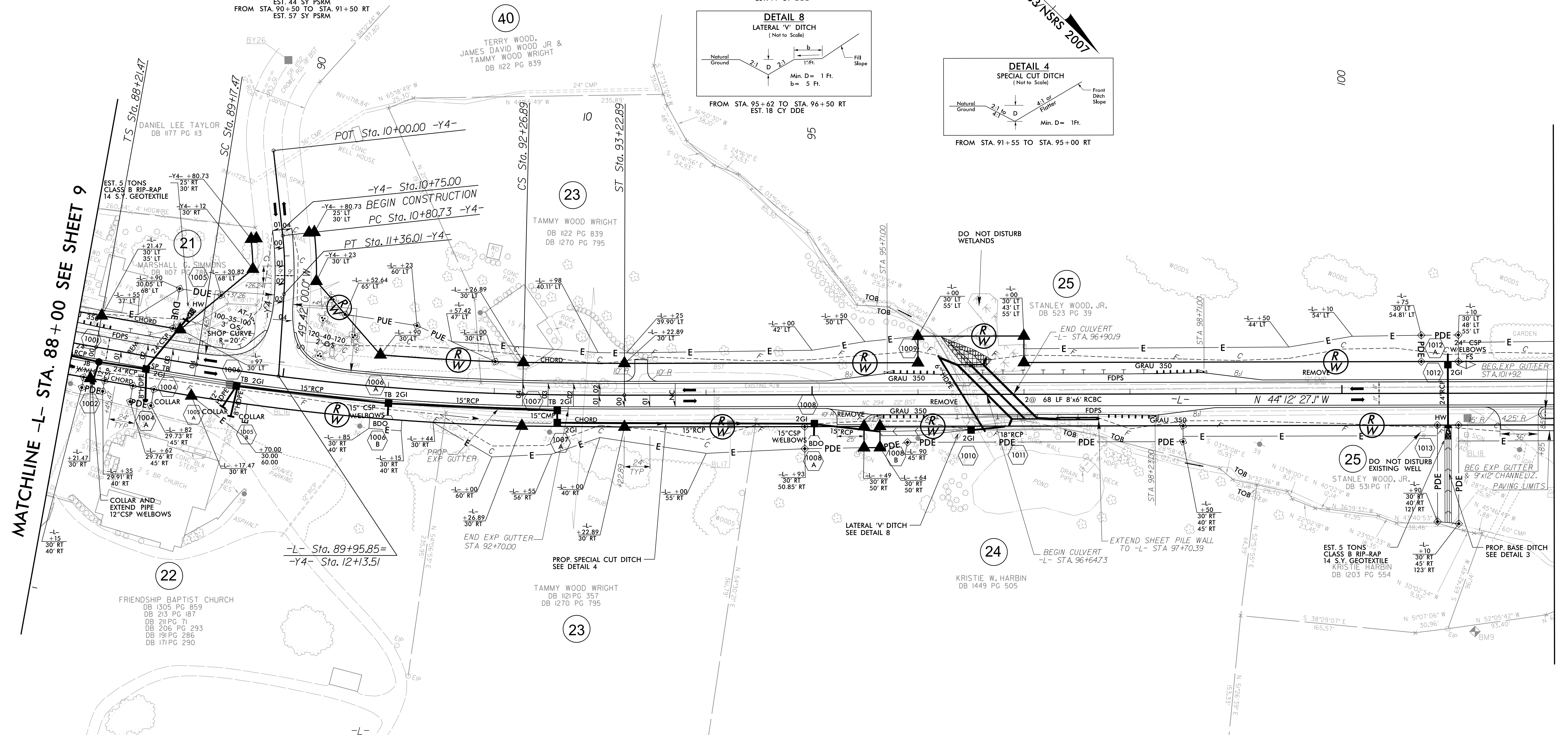
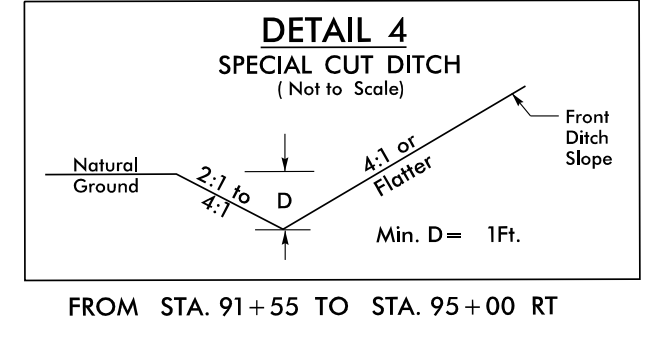
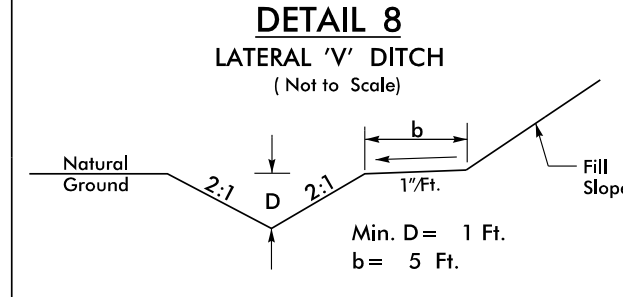
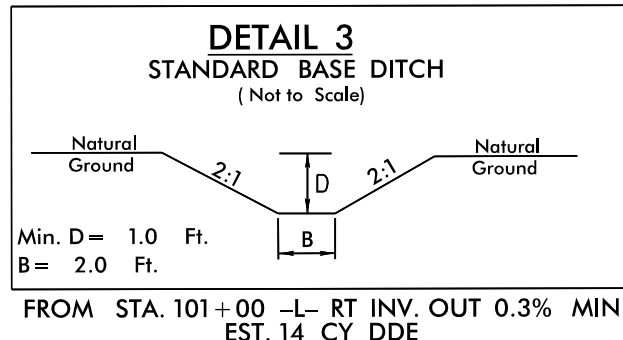
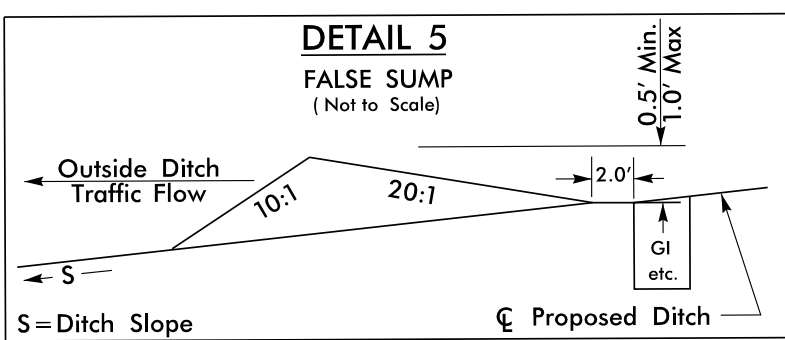
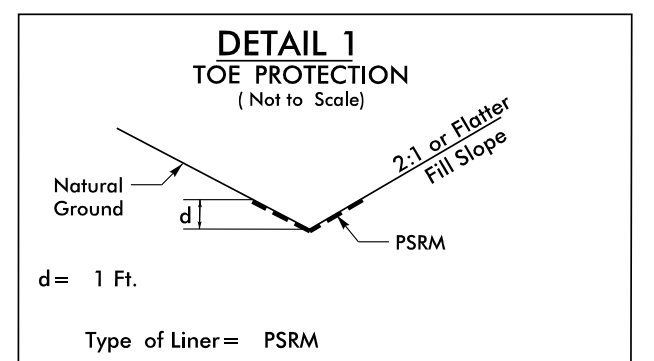
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PROJECT REFERENCE NO. R-3622B	SHEET NO. 10
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCHLINE -L- STA. 88+00 SEE SHEET 9

MATCHLINE -L- STA. 102+00 SEE SHEET 11

PIs Sta 88+85.47 Δs = 1' 11" 44.7" Ls = 96.00' LT = 64.00' ST = 32.00'	PI Sta 90+72.41 Δs = 7' 42" 28.8" (LT) D = 2' 29" 28.0" L = 309.42' T = 154.94' R = 2,300.00'	PIs Sta 92+58.89 Δs = 1' 11" 44.7" Ls = 96.00' LT = 64.00' ST = 32.00'
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-Y4-

PI Sta 11+08.44  
Δs = 9' 57" 00.0" (RT)  
D = 18' 00" 00.0"  
L = 55.28'  
T = 27.71'  
R = 318.31'

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

SEE SHEETS S-1 THRU S-17 FOR CULVERT PLANS  
SEE SHEET 19 FOR -L- PROFILE  
SEE SHEET 22 FOR -Y4- PROFILE

REVISIONS

8/17/99

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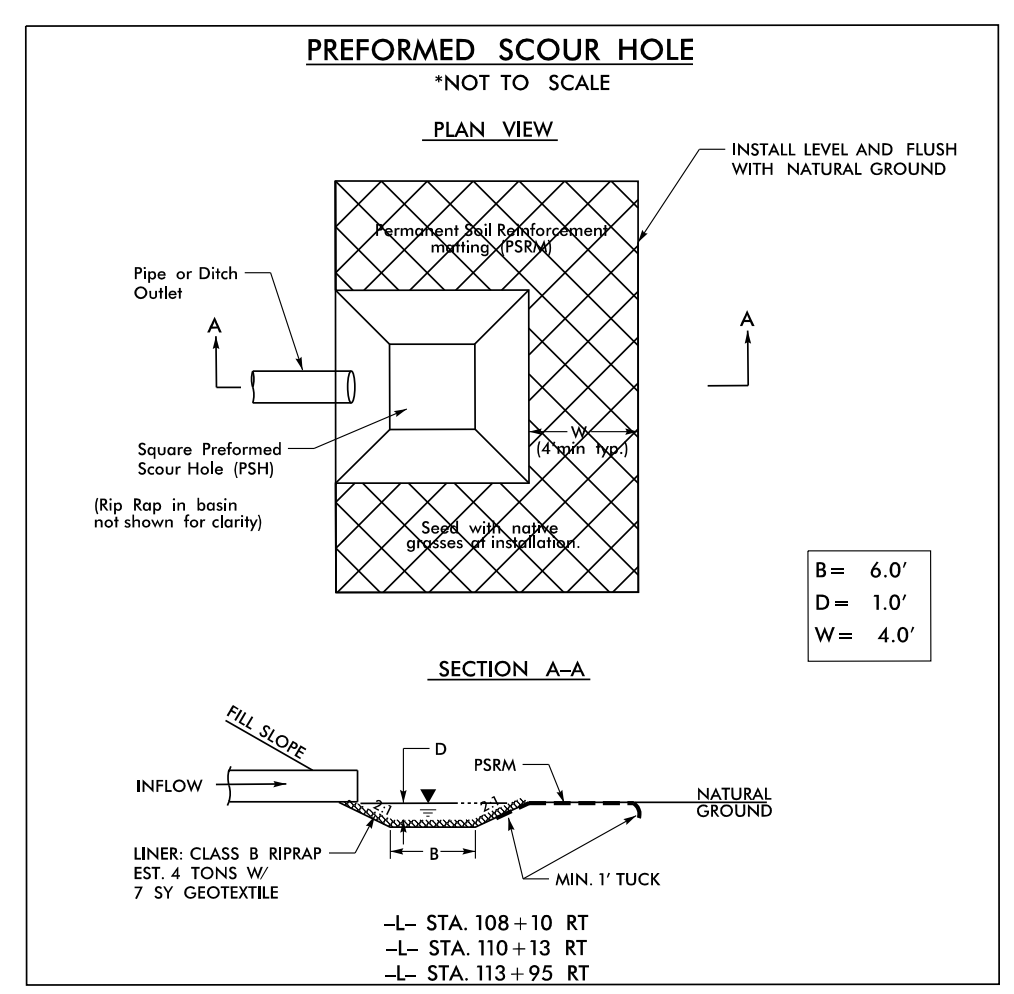
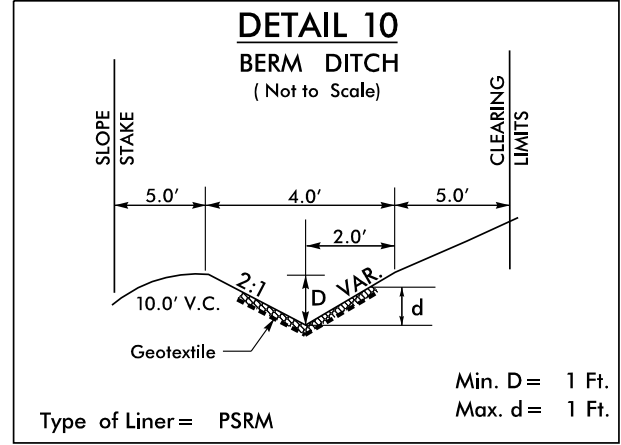
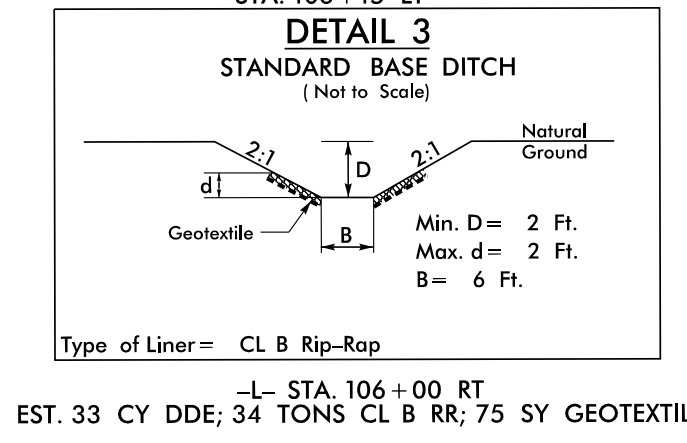
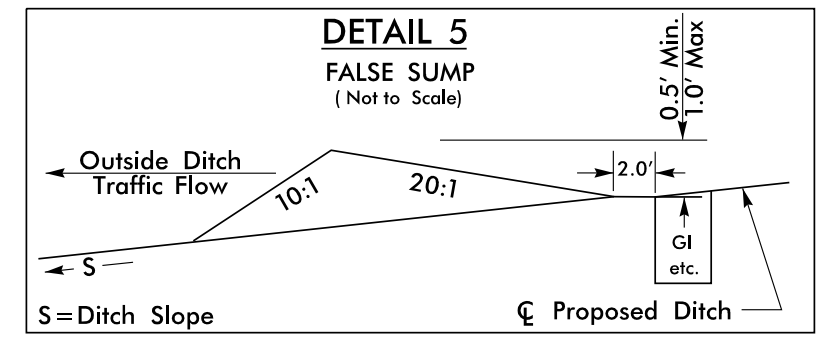
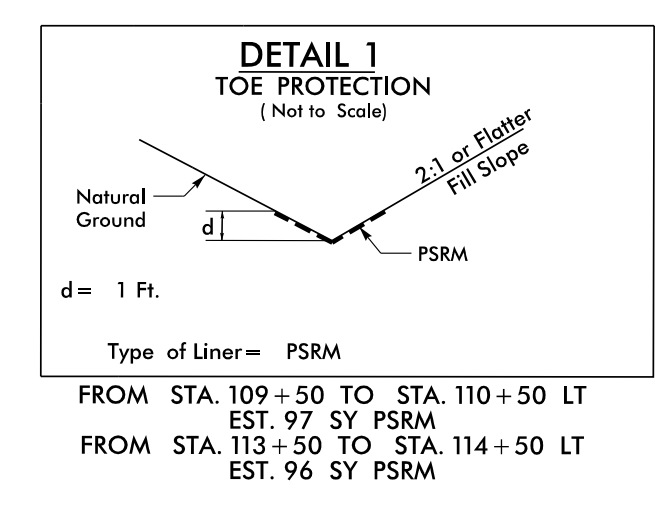
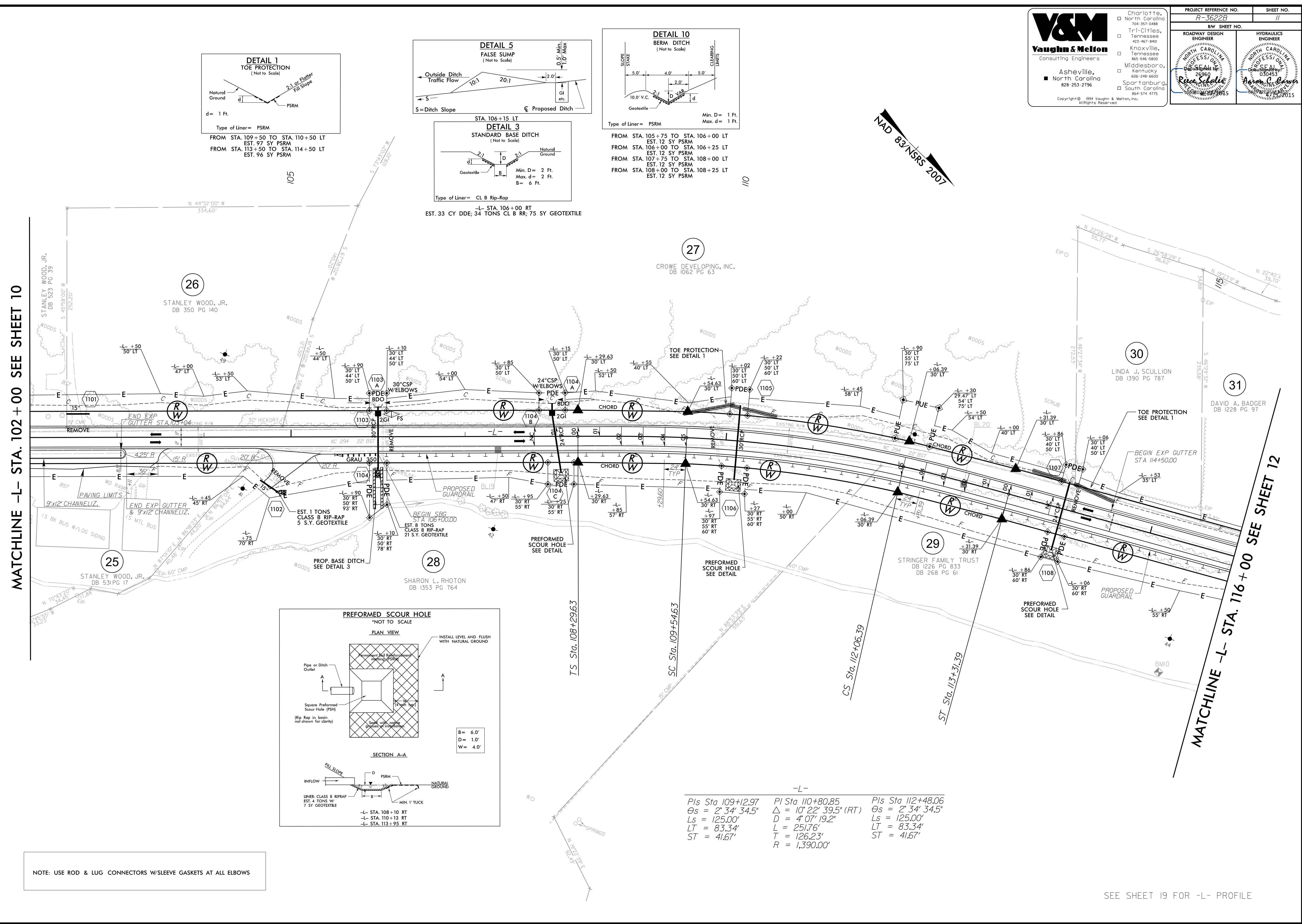
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PROJECT REFERENCE NO. R-3622B	SHEET NO. 11
ROADWAY DESIGN ENGINEER R. Schaefer	HYDRAULICS ENGINEER B. O. Bowen

MATCHLINE -L- STA. 102 + 00 SEE SHEET 10

MATCHLINE -L- STA. 116 + 00 SEE SHEET 12

REVISIONS



-L-

Pls Sta 109+12.97	Pl Sta 110+80.85	Pls Sta 112+48.06
Os = 2' 34' 34.5"	Δ = 10' 22' 39.5" (RT)	Os = 2' 34' 34.5"
Ls = 125.00'	D = 4' 07' 19.2"	Ls = 125.00'
LT = 83.34'	L = 251.76'	LT = 83.34'
ST = 41.67'	T = 126.23'	ST = 41.67'
	R = 1,390.00'	

NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

**V&M**  
Vaughn & Melton  
Consulting Engineers

Asheville, North Carolina  
828-253-2796

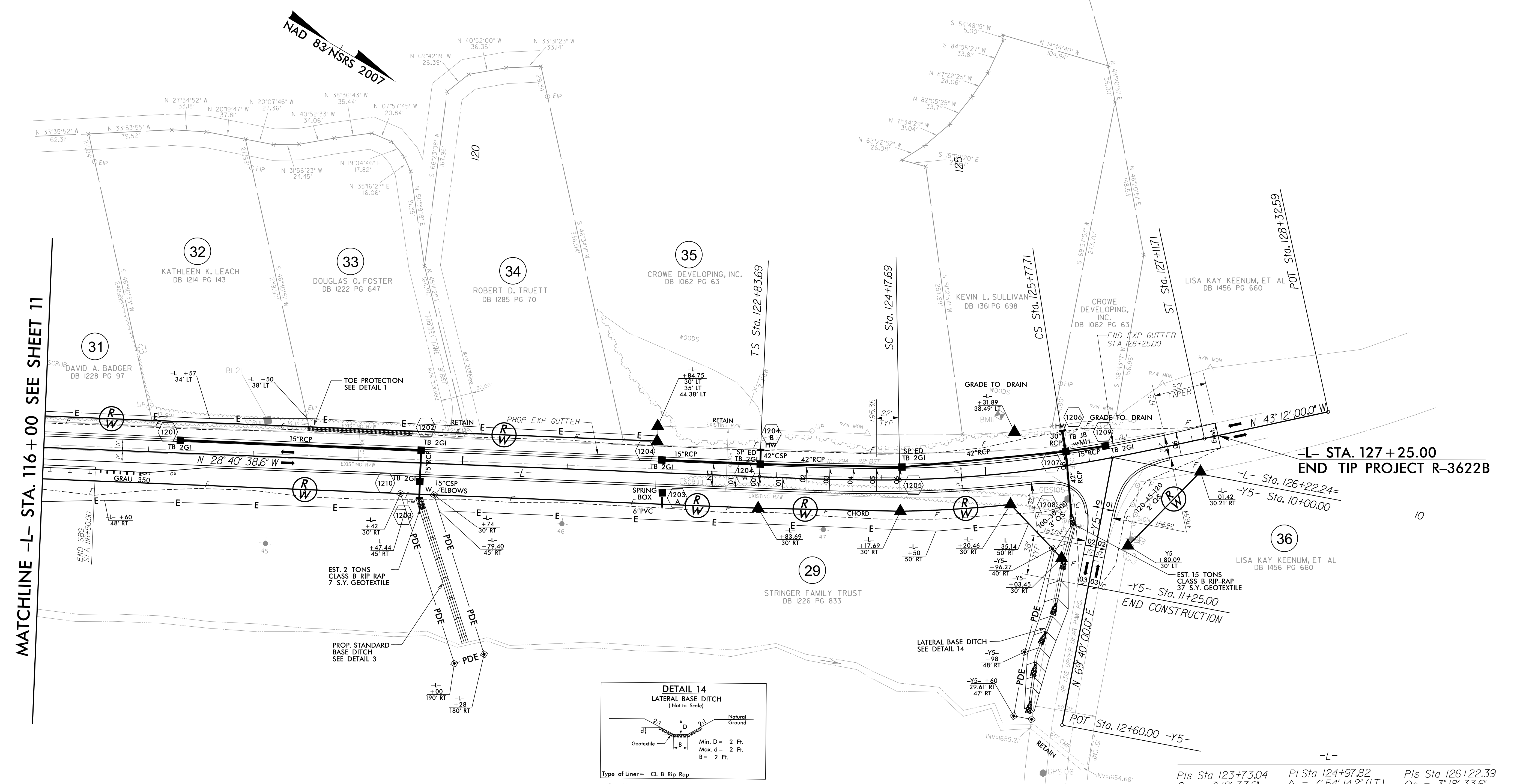
Charlotte, North Carolina 704-357-0488  
Tri-Cities, Tennessee 423-487-9480  
Knoxville, Tennessee 865-546-5800  
Middlesboro, Kentucky 606-248-6600  
Spartanburg, South Carolina 864-574-4775

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PROJECT REFERENCE NO. R-3622B	SHEET NO. 12
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

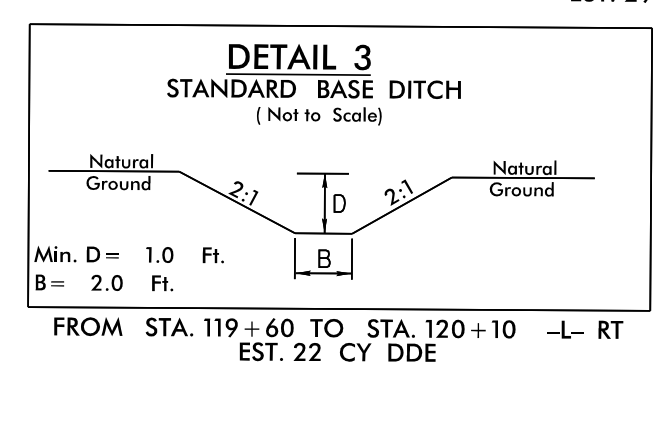
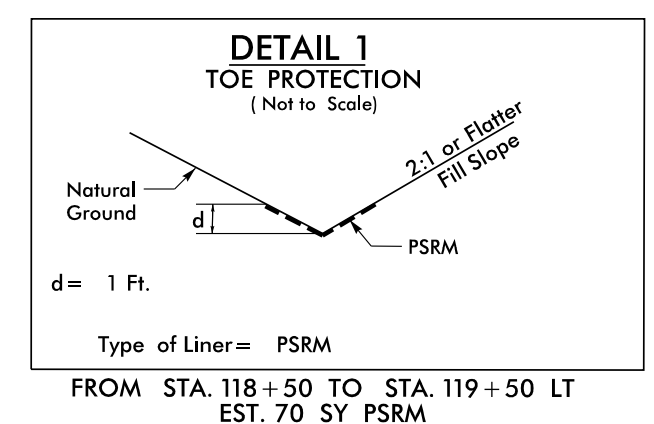
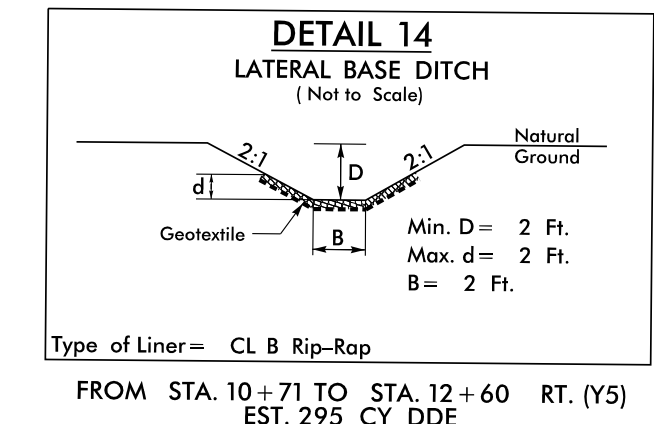
Professional Engineer Seal: North Carolina, License No. 26960, Name: SEAL, Date: 03/04/03

Professional Engineer Seal: North Carolina, License No. 26960, Name: SEAL, Date: 03/04/03



MATCHLINE -L- STA. 116 + 00 SEE SHEET 11

-L- STA. 127 + 25.00  
END TIP PROJECT R-3622B

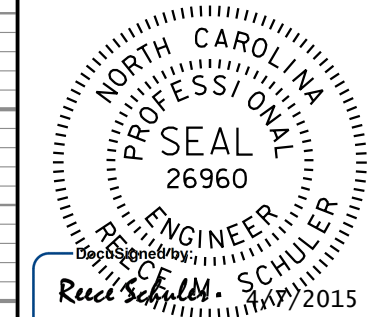



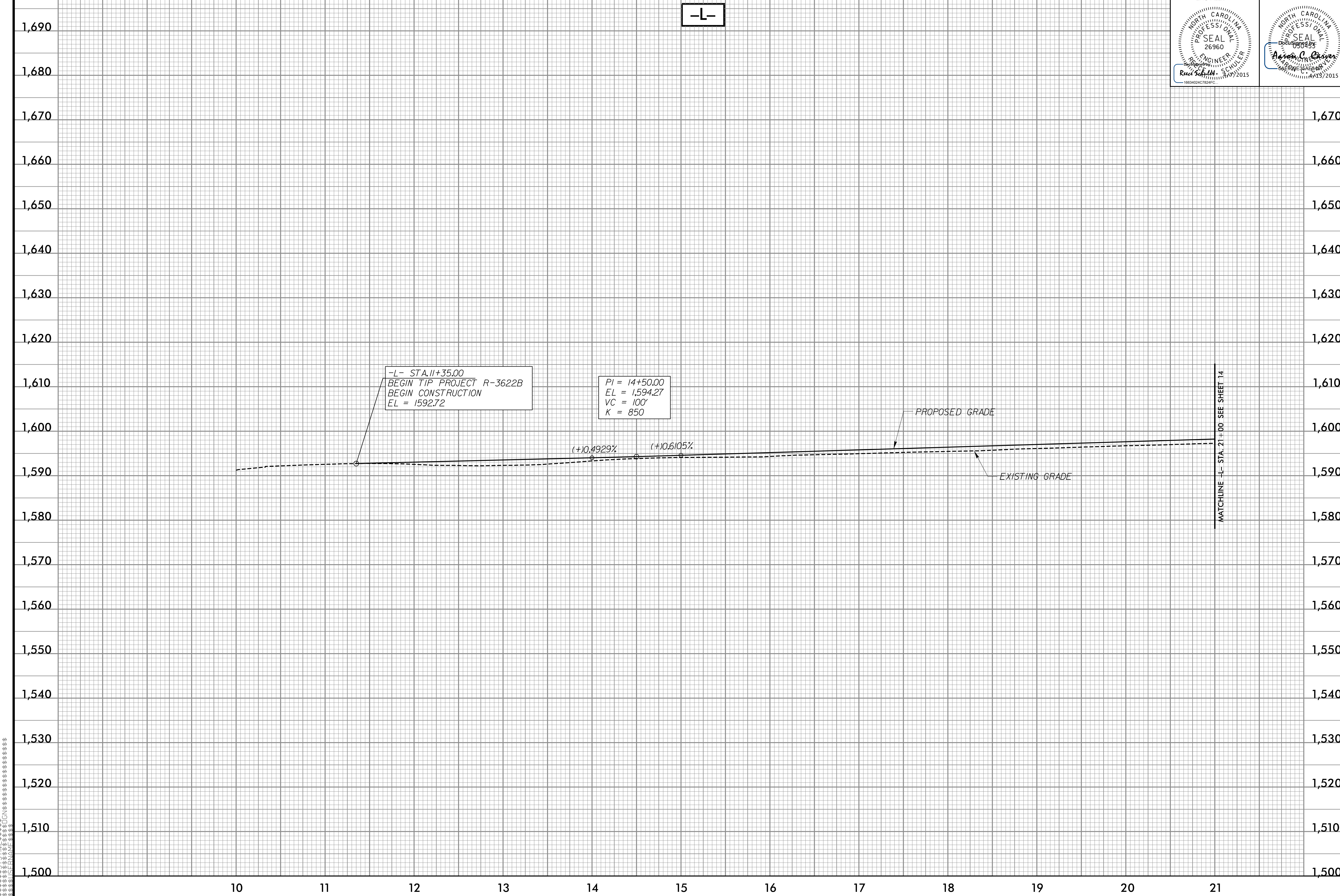
NOTE: USE ROD & LUG CONNECTORS W/SLEEVE GASKETS AT ALL ELBOWS

Pls Sta 123+73.04 $\Theta_s = 3^\circ 18' 33.6''$ $L_s = 134.00'$ $LT = 89.35'$ $ST = 44.68'$	Pls Sta 124+97.82 $\Delta = 7^\circ 54' 14.2''$ (LT) $D = 4^\circ 56' 21.4''$ $L = 160.02'$ $T = 80.14'$ $R = 1,160.00'$	Pls Sta 126+22.39 $\Theta_s = 3^\circ 18' 33.6''$ $L_s = 134.00'$ $LT = 89.35'$ $ST = 44.68'$
---	---	---

SEE SHEET 21 FOR -L- PROFILE  
SEE SHEET 22 FOR -Y5- PROFILE

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>13</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	



-L- STA. 11+35.00  
BEGIN TIP PROJECT R-3622B  
BEGIN CONSTRUCTION  
EL = 1592.72

PI = 14+50.00  
EL = 1594.27  
VC = 100'  
K = 850

(+0.4929%      (+0.6105%)

PROPOSED GRADE

EXISTING GRADE

MATCHLINE -L- STA. 21+00 SEE SHEET 14

5/14/99



5/14/99

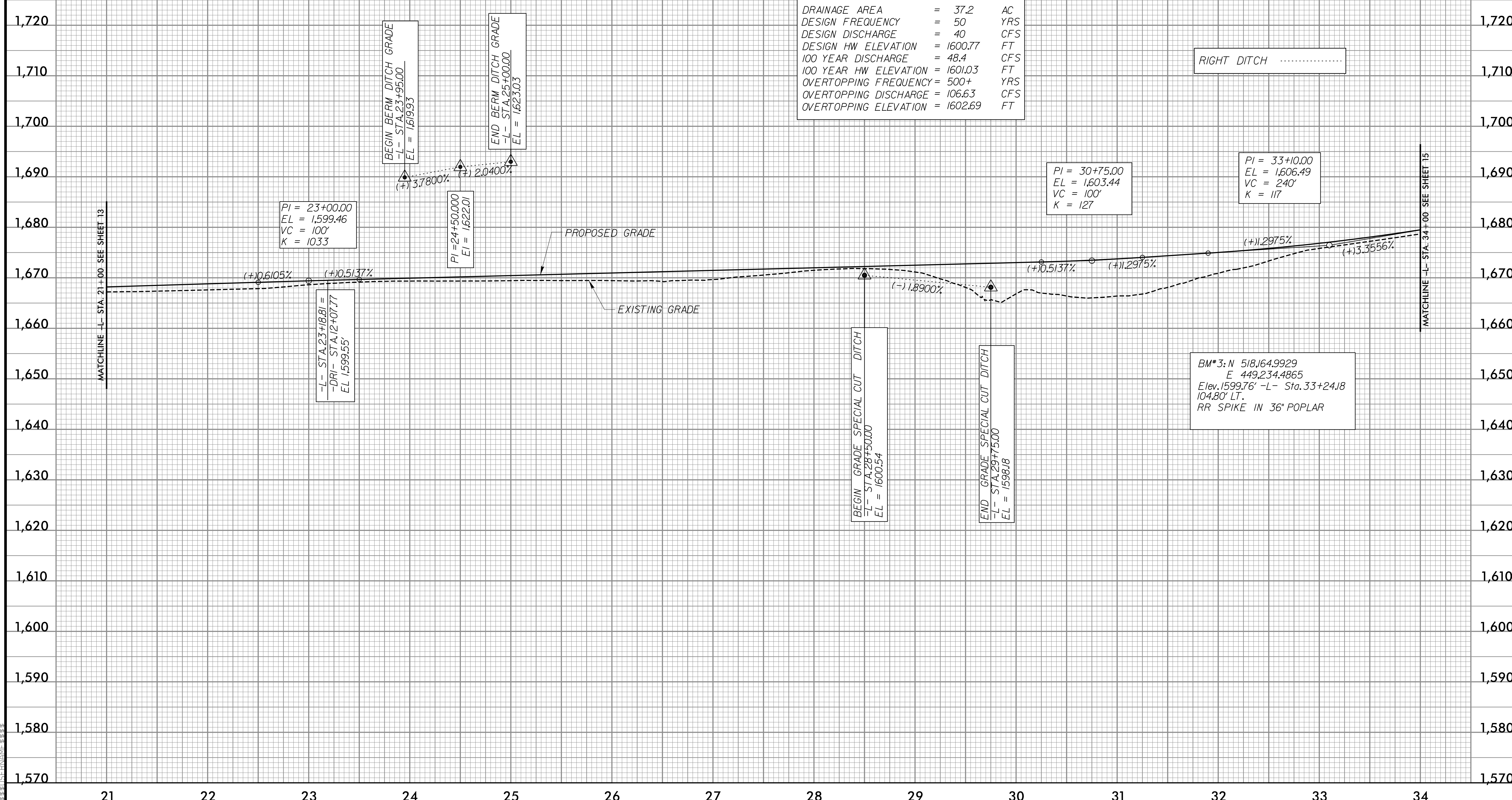
PROJECT REFERENCE NO. R-3622B	SHEET NO. 14
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

-L-

BM\*2: N 517,068,1438  
E 449,479,8485  
Elev. 1606.54' -L- Sta. 21+63.46  
81.7' RI.  
RR SPIKE IN 30" POPLAR

PIPE HYDRAULIC DATA  
36" RCP Sta. 29+75.75

DRAINAGE AREA	=	37.2	AC
DESIGN FREQUENCY	=	50	YRS
DESIGN DISCHARGE	=	40	CFS
DESIGN HW ELEVATION	=	1600.77	FT
100 YEAR DISCHARGE	=	48.4	CFS
100 YEAR HW ELEVATION	=	1601.03	FT
OVERTOPPING FREQUENCY	=	500+	YRS
OVERTOPPING DISCHARGE	=	106.63	CFS
OVERTOPPING ELEVATION	=	1602.69	FT



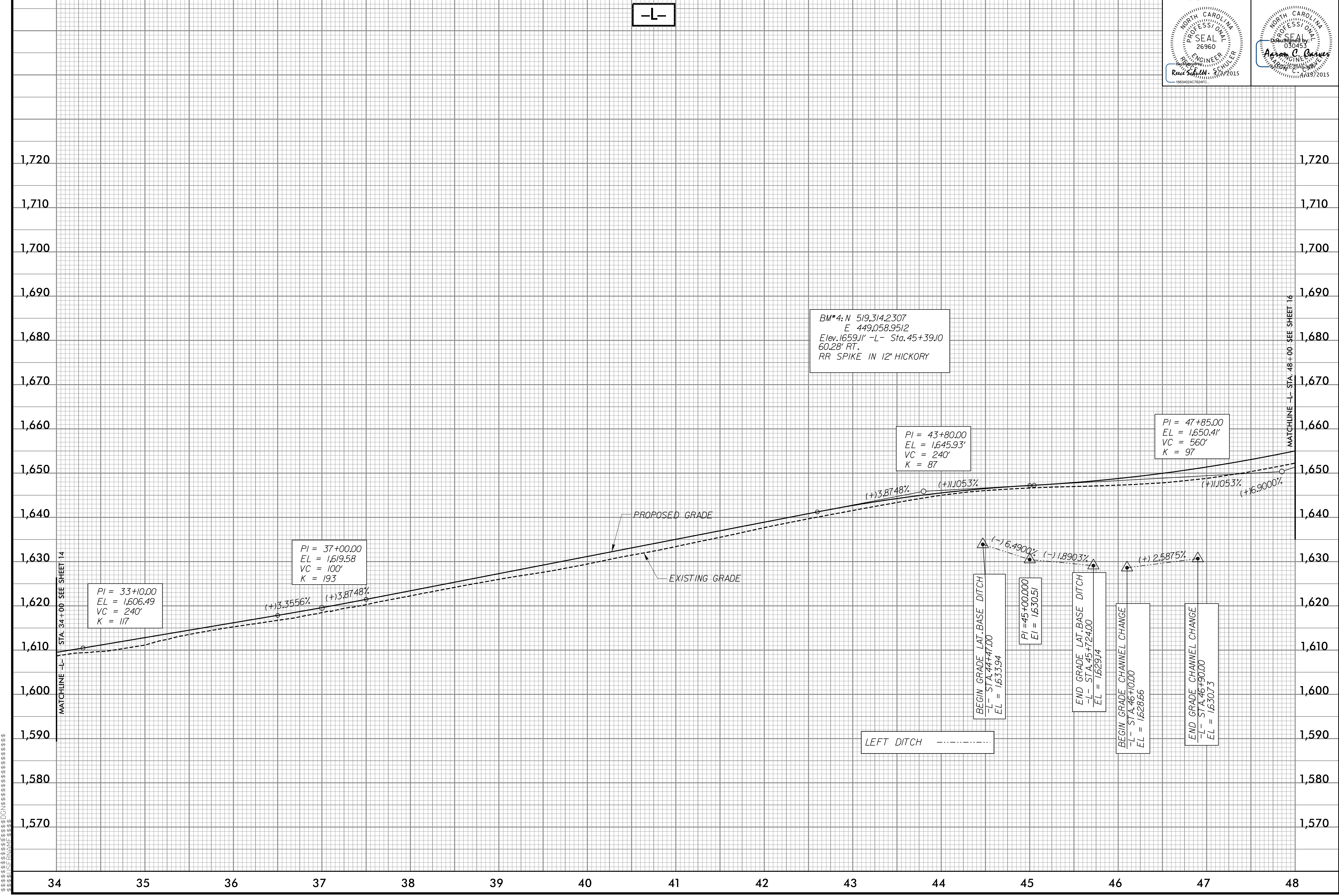
MATCHLINE -L- STA. 21+00 SEE SHEET 13

MATCHLINE -L- STA. 34+00 SEE SHEET 15

5/14/99

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>15</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



BM\*4: N 519,314.2307  
 E 449,058.9512  
 Elev. 1,659.11' -L- Sta. 45+39.10  
 60.28' RT.  
 RR SPIKE IN 12" HICKORY

PI = 43+80.00  
 EL = 1,645.93'  
 VC = 240'  
 K = 87

PI = 47+85.00  
 EL = 1,650.41'  
 VC = 560'  
 K = 97

PI = 33+10.00  
 EL = 1,606.49  
 VC = 240'  
 K = 117

PI = 37+00.00  
 EL = 1,619.58  
 VC = 100'  
 K = 193

BEGIN GRADE LAT. BASE DITCH  
 -L- STA. 44+77.00  
 EL = 1,633.94

PI = 45+00.00  
 EL = 1,630.51

END GRADE LAT. BASE DITCH  
 -L- STA. 45+24.00  
 EL = 1,629.14

BEGIN GRADE CHANNEL CHANGE  
 -L- STA. 46+10.00  
 EL = 1,628.66

END GRADE CHANNEL CHANGE  
 -L- STA. 46+90.00  
 EL = 1,630.73

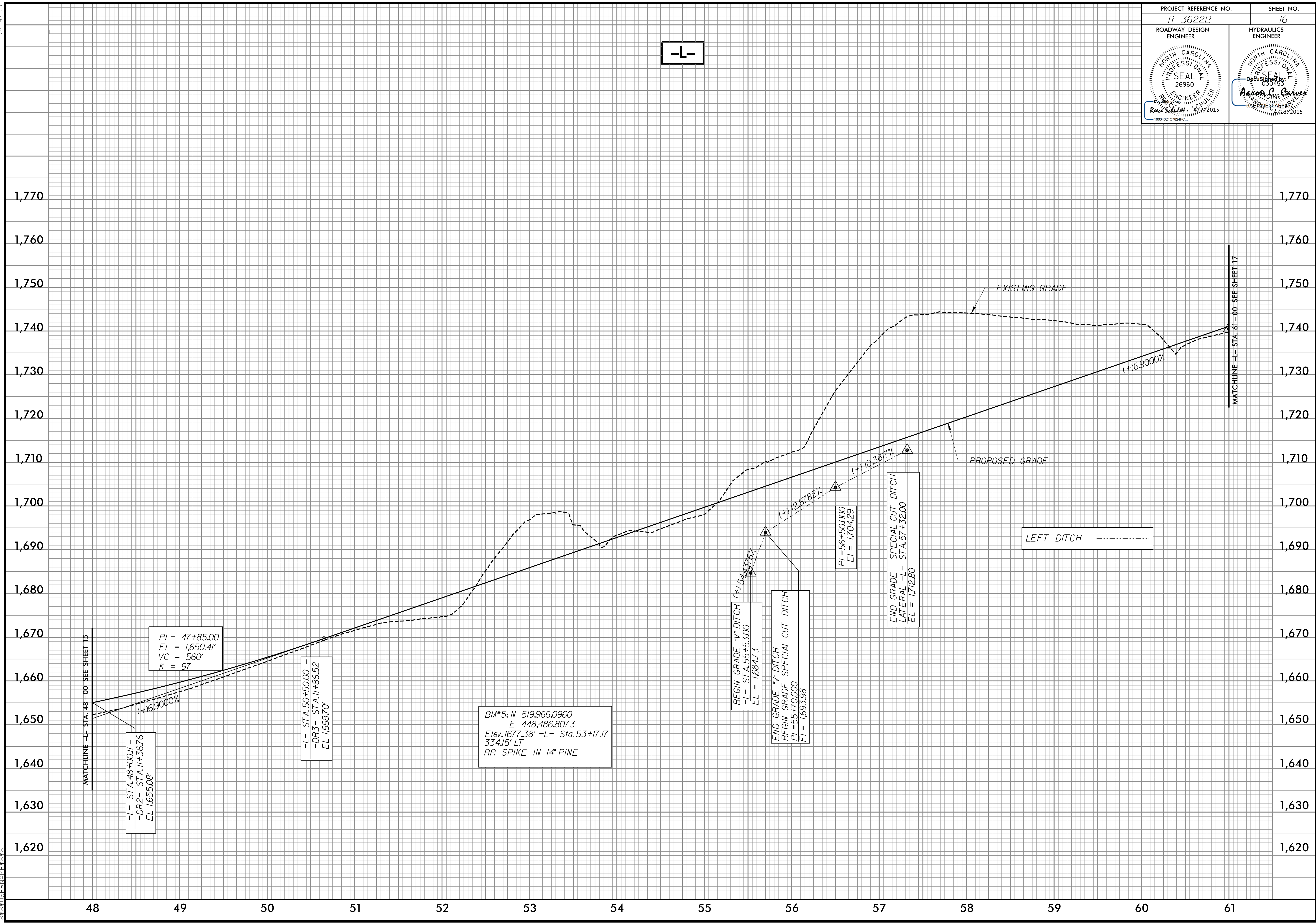
LEFT DITCH

MATCHLINE -L- STA. 48+00 SEE SHEET 16

MATCHLINE -L- STA. 34+00 SEE SHEET 14

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>16</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCHLINE -L- STA. 48+00 SEE SHEET 15

-L- STA. 48+00.11 =  
-DR2 - STA. 11+36.76  
EL 1655.08'

PI = 47+85.00  
EL = 1650.41'  
VC = 560'  
K = 97

-L- STA. 50+50.00 =  
-DR3 - STA. 11+86.52  
EL 1668.70'

BM\*5: N 519,966.0960  
E 448,486.8073  
Elev. 1677.38' -L- Sta. 53+17.17  
334.15' LT  
RR SPIKE IN 14" PINE

BEGIN GRADE "V" DITCH (+) 5.44376%  
-L- STA. 55+53.00  
EL = 1684.73

END GRADE "V" DITCH  
BEGIN GRADE SPECIAL CUT DITCH  
-L- STA. 55+70.00  
PI = 55+70.00  
EI = 1693.98

PI = 56+50.000  
EI = 1704.29

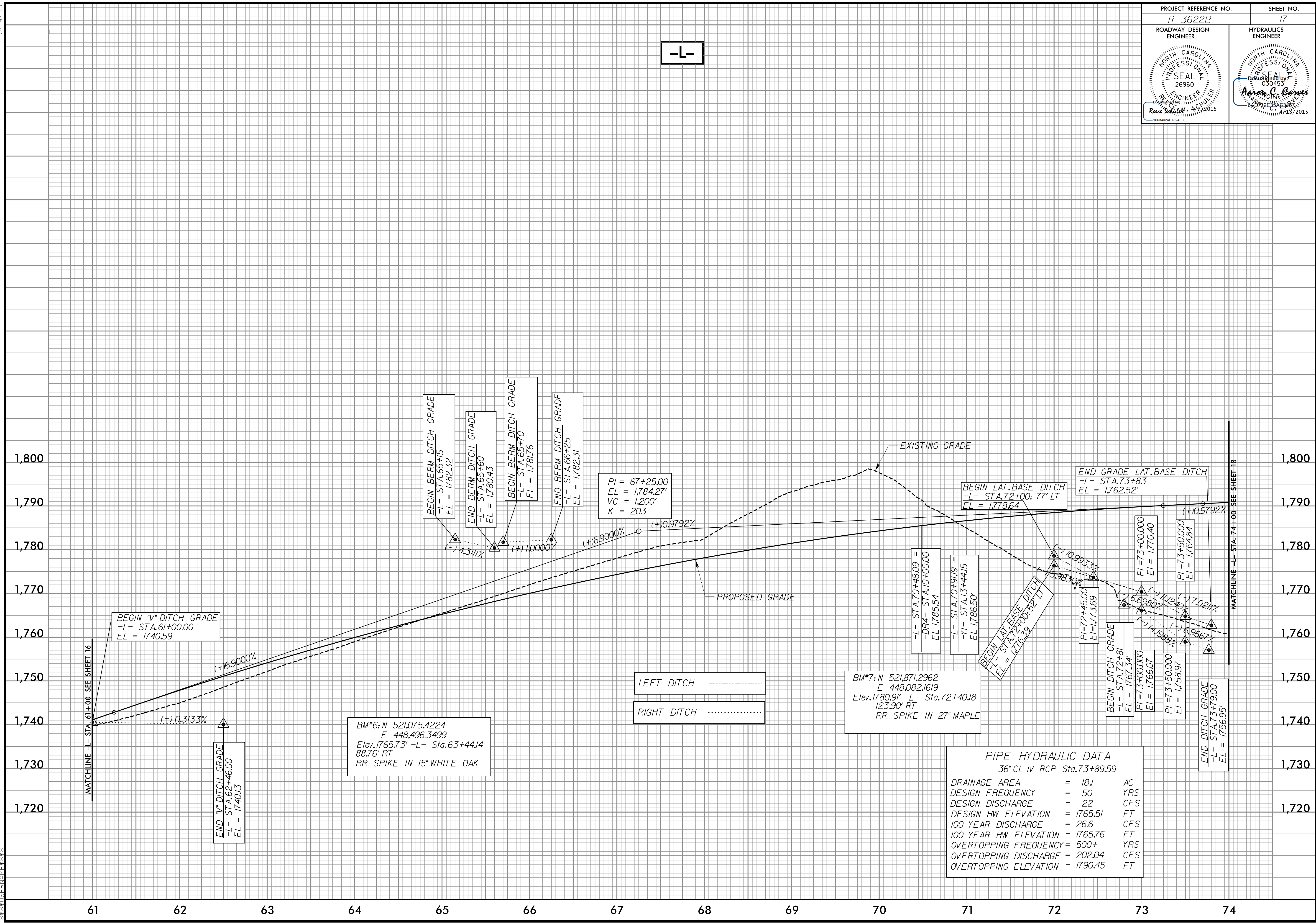
END GRADE SPECIAL CUT DITCH  
LATERAL -L- STA. 57+32.00  
EL = 1712.80

LEFT DITCH

MATCHLINE -L- STA. 61+00 SEE SHEET 17

5/14/99

PROJECT REFERENCE NO. R-3622B	SHEET NO. 17
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



MATCHLINE -L- STA. 61+00 SEE SHEET 16

BEGIN "V" DITCH GRADE  
-L- STA. 61+00.00  
EL = 1740.59

END "V" DITCH GRADE  
-L- STA. 62+46.00  
EL = 1740.13

BM\*6: N 521,075.4224  
E 448,496.3499  
Elev. 1765.73' -L- Sta. 63+44.14  
88.76' RT  
RR SPIKE IN 15" WHITE OAK

BEGIN BERM DITCH GRADE  
-L- STA. 65+15  
EL = 1782.32

END BERM DITCH GRADE  
-L- STA. 65+60  
EL = 1780.43

BEGIN BERM DITCH GRADE  
-L- STA. 65+70  
EL = 1781.76

END BERM DITCH GRADE  
-L- STA. 66+25  
EL = 1782.31

PI = 67+25.00  
EL = 1784.27'  
VC = 1,200'  
K = 203

LEFT DITCH

RIGHT DITCH

BM\*7: N 521,871.2962  
E 448,082.1619  
Elev. 1780.91' -L- Sta. 72+40.18  
123.90' RT  
RR SPIKE IN 27" MAPLE

PIPE HYDRAULIC DATA  
36" CL IV RCP Sta. 73+89.59

DRAINAGE AREA	= 181	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 22	CFS
DESIGN HW ELEVATION	= 1765.51	FT
100 YEAR DISCHARGE	= 26.6	CFS
100 YEAR HW ELEVATION	= 1765.76	FT
OVERTOPPING FREQUENCY	= 500+	YRS
OVERTOPPING DISCHARGE	= 202.04	CFS
OVERTOPPING ELEVATION	= 1790.45	FT

BEGIN LAT. BASE DITCH  
-L- STA. 72+00: 77' LT  
EL = 1778.64

-L- STA. 70+48.09 =  
-DR4- STA. 70+00.00  
EL 1785.54

-L- STA. 70+91.19 =  
-Y1- STA. 73+44.15  
EL 1786.50'

BEGIN LAT. BASE DITCH  
-L- STA. 72+00: 32' LT  
EL = 1776.59

PI = 72+45.00  
EL = 1773.69

BEGIN DITCH GRADE  
-L- STA. 72+81  
EL = 1767.34'

PI = 73+00.000  
EI = 1766.07

PI = 73+50.000  
EI = 1758.97

END DITCH GRADE  
-L- STA. 73+79.00  
EL = 1756.95'

END GRADE LAT. BASE DITCH  
-L- STA. 73+83  
EL = 1762.52'

PI = 73+00.000  
EI = 1770.40

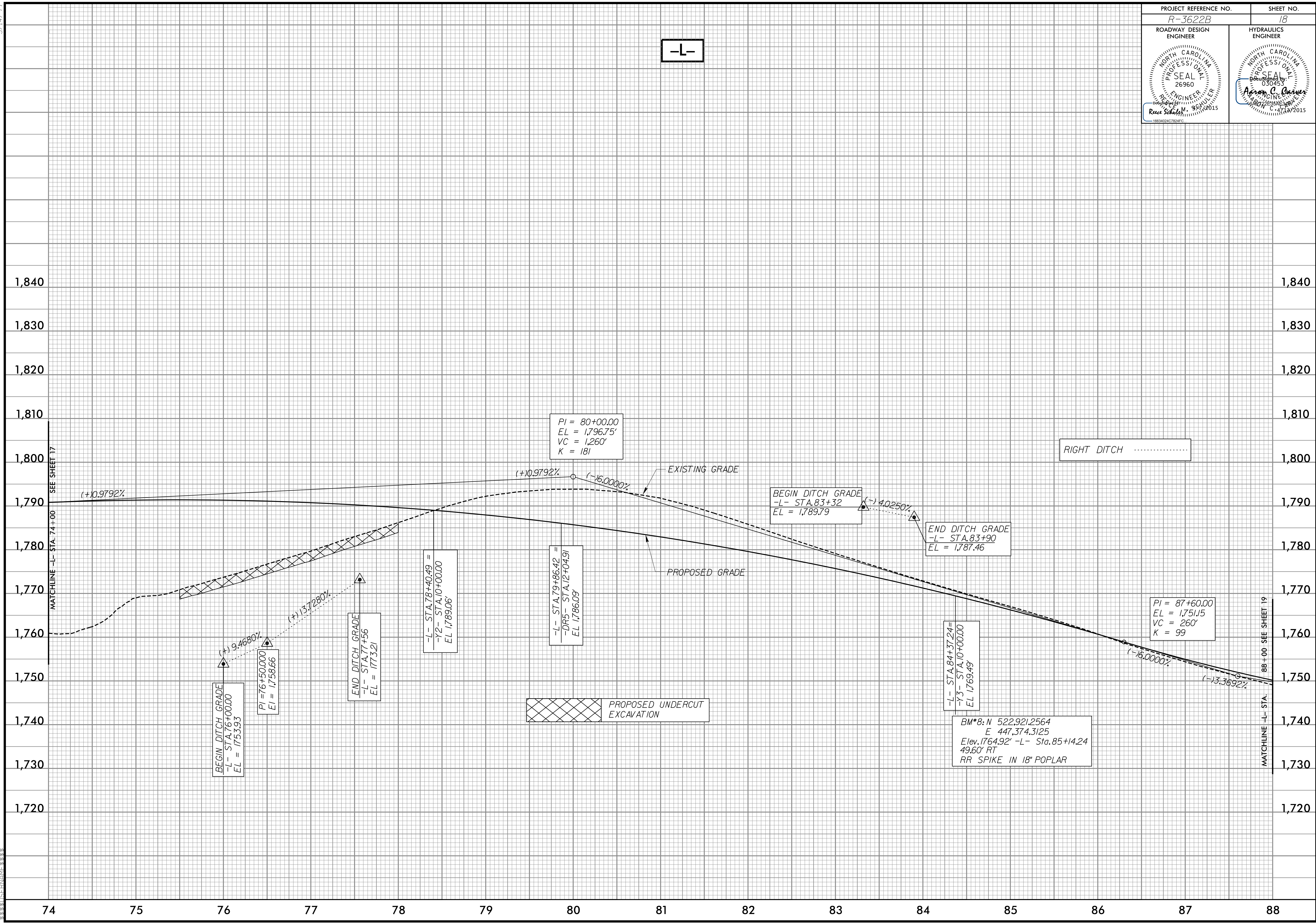
PI = 73+50.000  
EI = 1764.84

MATCHLINE -L- STA. 74+00 SEE SHEET 18

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>18</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

**-L-**



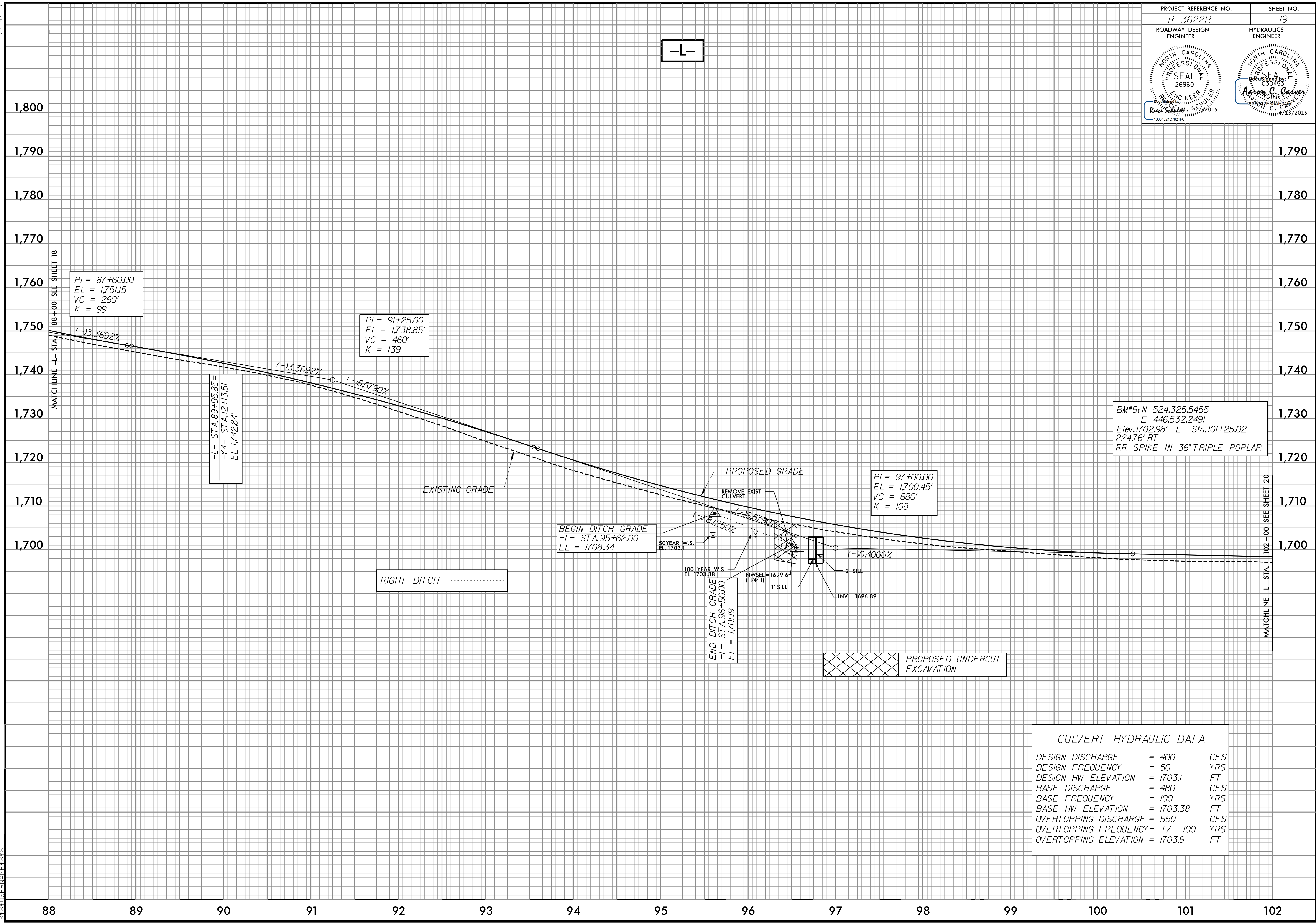
5/14/99

MATCHLINE -L- STA. 88+00 SEE SHEET 19

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>19</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

**-L-**



$PI = 87+60.00$   
 $EL = 1,751.15$   
 $VC = 260'$   
 $K = 99$

$PI = 91+25.00$   
 $EL = 1,738.85'$   
 $VC = 460'$   
 $K = 139$

$PI = 97+00.00$   
 $EL = 1,700.45'$   
 $VC = 680'$   
 $K = 108$

$BM^*9: N 524,325.5455$   
 $E 446,532.2491$   
 $Elev. 1702.98' -L- Sta. 101+25.02$   
 $224.76' RT$   
 $RR SPIKE IN 36" TRIPLE POPLAR$

**BEGIN DITCH GRADE**  
 $-L- STA. 95+62.00$   
 $EL = 1708.34$

**END DITCH GRADE**  
 $-L- STA. 96+50.00$   
 $EL = 1701.9$

PROPOSED UNDERCUT EXCAVATION

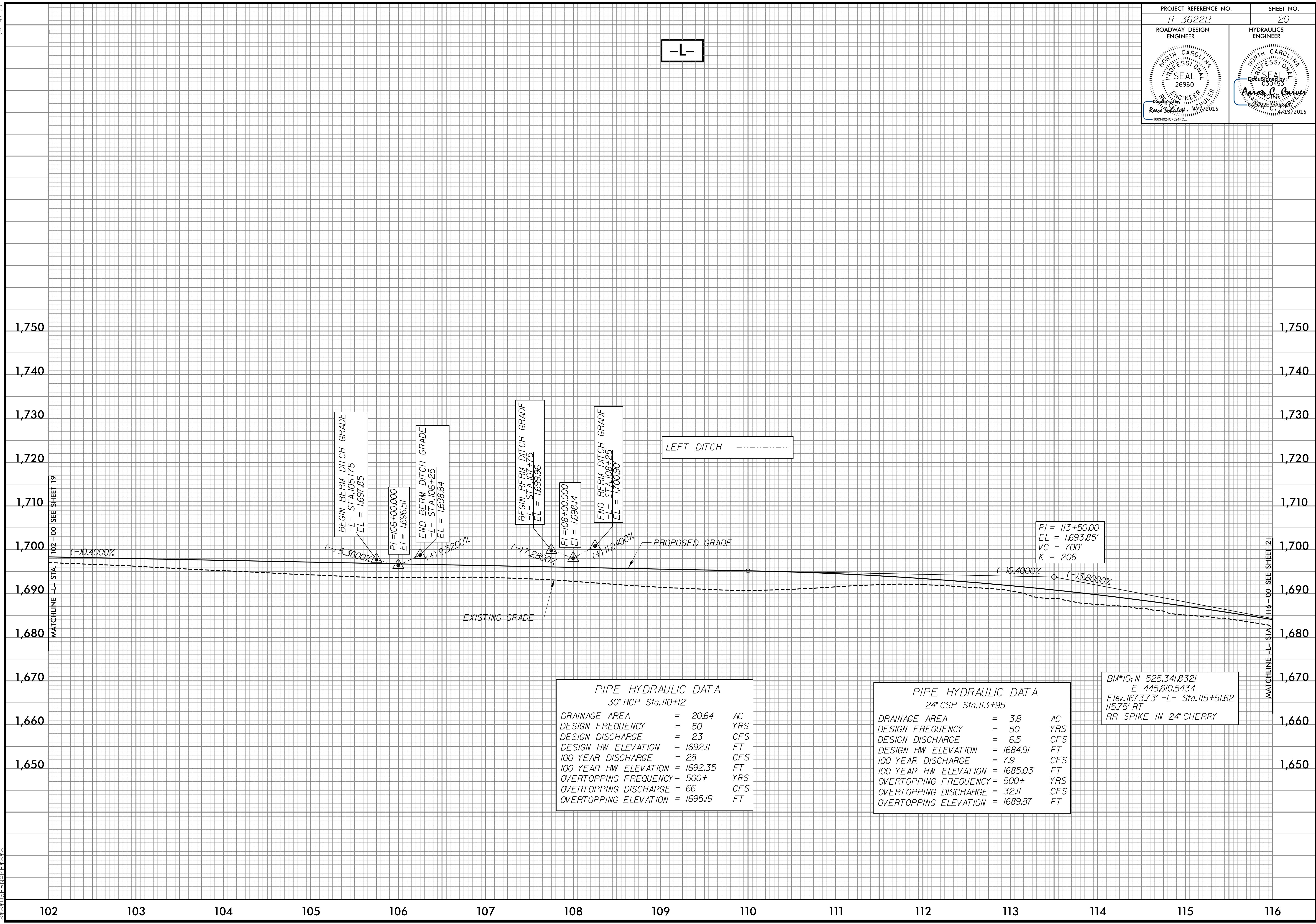
CULVERT HYDRAULIC DATA		
DESIGN DISCHARGE	= 400	CFS
DESIGN FREQUENCY	= 50	YRS
DESIGN HW ELEVATION	= 1703J	FT
BASE DISCHARGE	= 480	CFS
BASE FREQUENCY	= 100	YRS
BASE HW ELEVATION	= 1703.38	FT
OVERTOPPING DISCHARGE	= 550	CFS
OVERTOPPING FREQUENCY	= +/- 100	YRS
OVERTOPPING ELEVATION	= 1703.9	FT

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5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. 20
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

-L-



PIPE HYDRAULIC DATA  
30" RCP Sta. 110+12

DRAINAGE AREA	= 20.64	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 23	CFS
DESIGN HW ELEVATION	= 1692.11	FT
100 YEAR DISCHARGE	= 28	CFS
100 YEAR HW ELEVATION	= 1692.35	FT
OVERTOPPING FREQUENCY	= 500+	YRS
OVERTOPPING DISCHARGE	= 66	CFS
OVERTOPPING ELEVATION	= 1695.19	FT

PIPE HYDRAULIC DATA  
24" CSP Sta. 113+95

DRAINAGE AREA	= 3.8	AC
DESIGN FREQUENCY	= 50	YRS
DESIGN DISCHARGE	= 6.5	CFS
DESIGN HW ELEVATION	= 1684.91	FT
100 YEAR DISCHARGE	= 7.9	CFS
100 YEAR HW ELEVATION	= 1685.03	FT
OVERTOPPING FREQUENCY	= 500+	YRS
OVERTOPPING DISCHARGE	= 32.11	CFS
OVERTOPPING ELEVATION	= 1689.87	FT

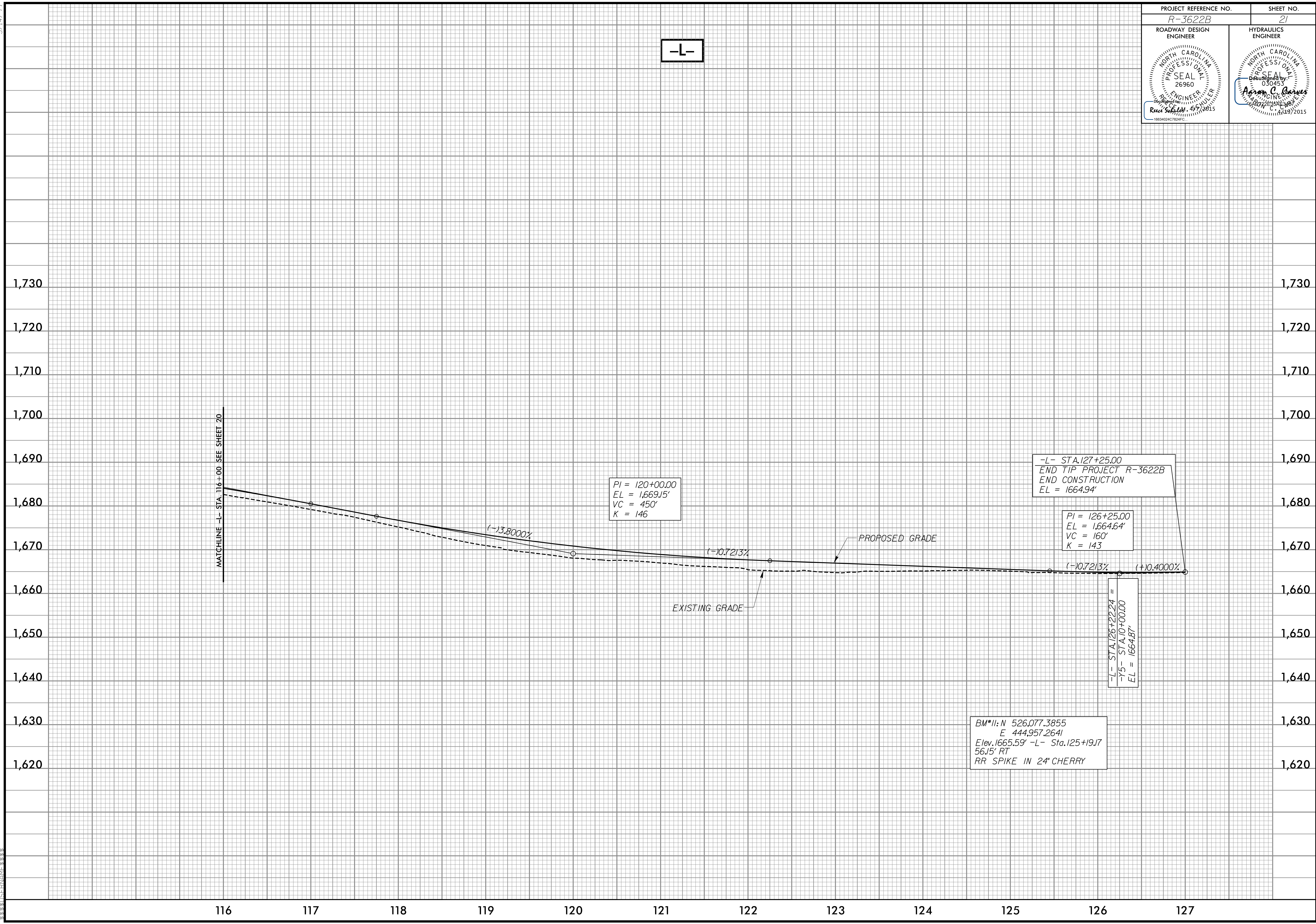
BM\*10: N 525.341.8321  
E 445.610.5434  
Elev. 1673.73' -L- Sta. 115+51.62  
115.75' RT  
RR SPIKE IN 24" CHERRY

102 103 104 105 106 107 108 109 110 111 112 113 114 115 116

5/14/99

PROJECT REFERENCE NO. <i>R-3622B</i>	SHEET NO. <i>21</i>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

**-L-**



PI = 120+00.00  
 EL = 1,669.15'  
 VC = 450'  
 K = 146

-L- STA.127+25.00  
 END TIP PROJECT R-3622B  
 END CONSTRUCTION  
 EL = 1664.94'

PI = 126+25.00  
 EL = 1,664.64'  
 VC = 160'  
 K = 143

-L- STA.126+22.24 =  
 -75' - STA.10+00.00  
 EL = 1664.87'

BM#11: N 526,077.3855  
 E 444,957.2641  
 Elev. 1665.59' -L- Sta. 125+19.17  
 56.15' RT  
 RR SPIKE IN 24" CHERRY

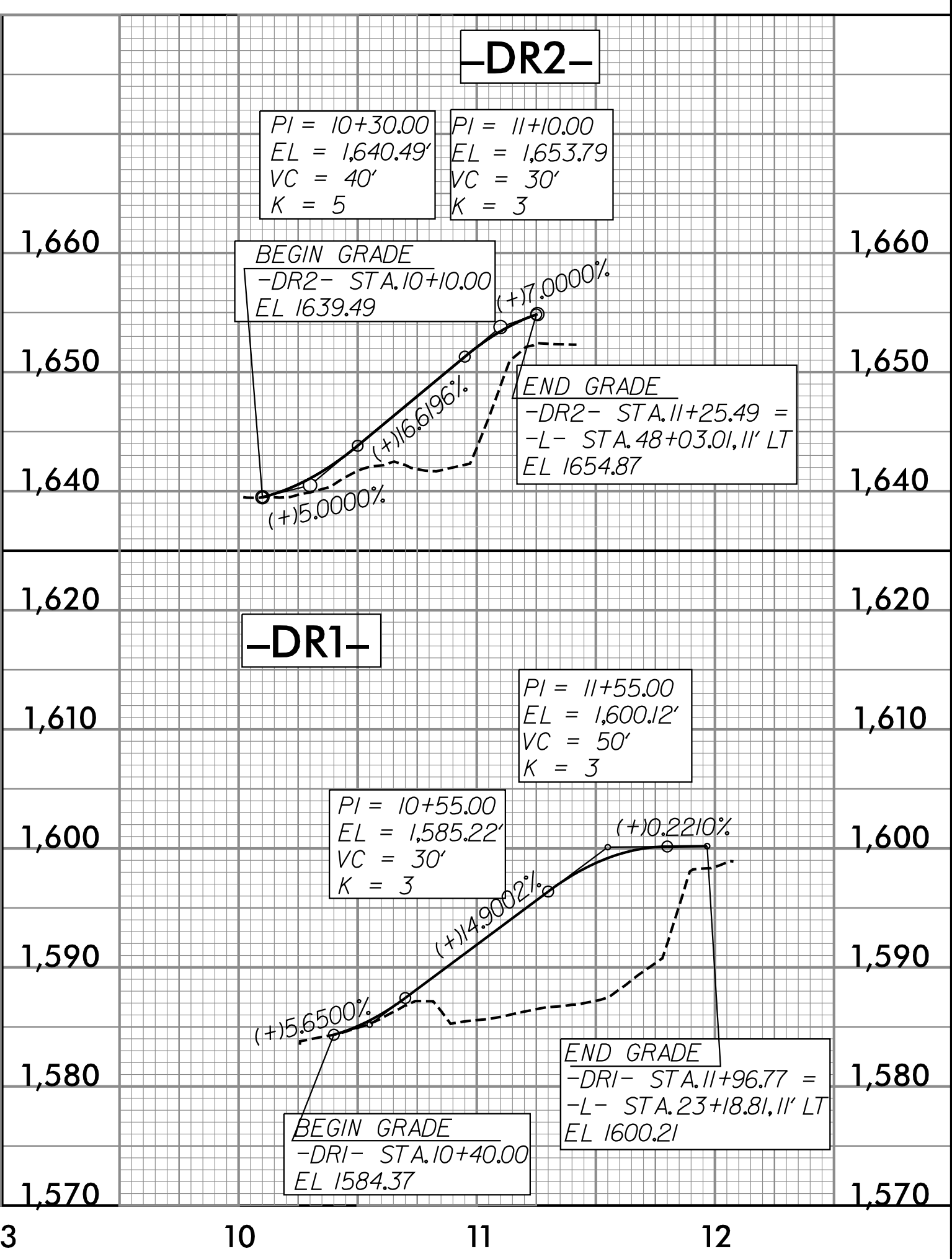
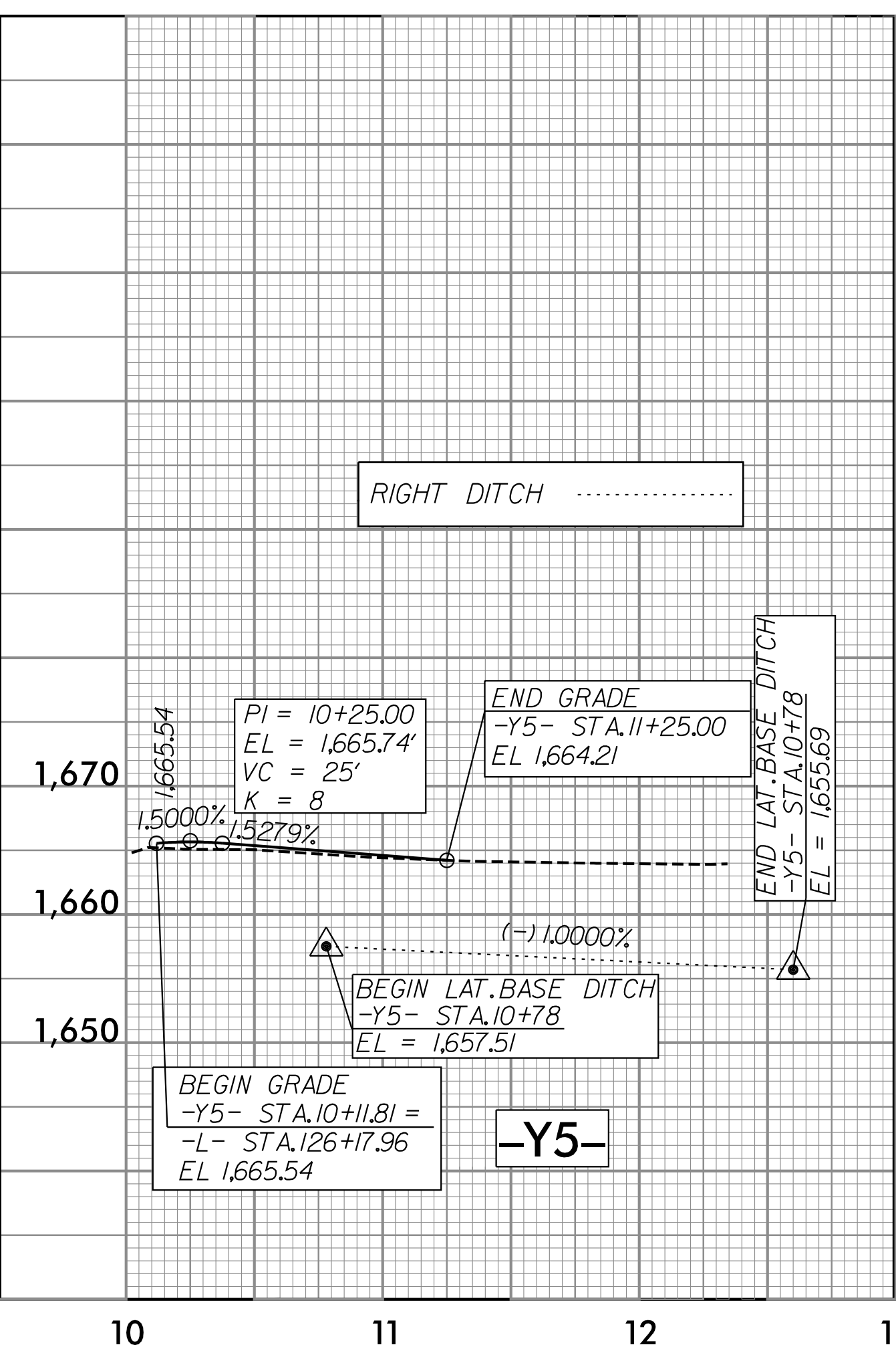
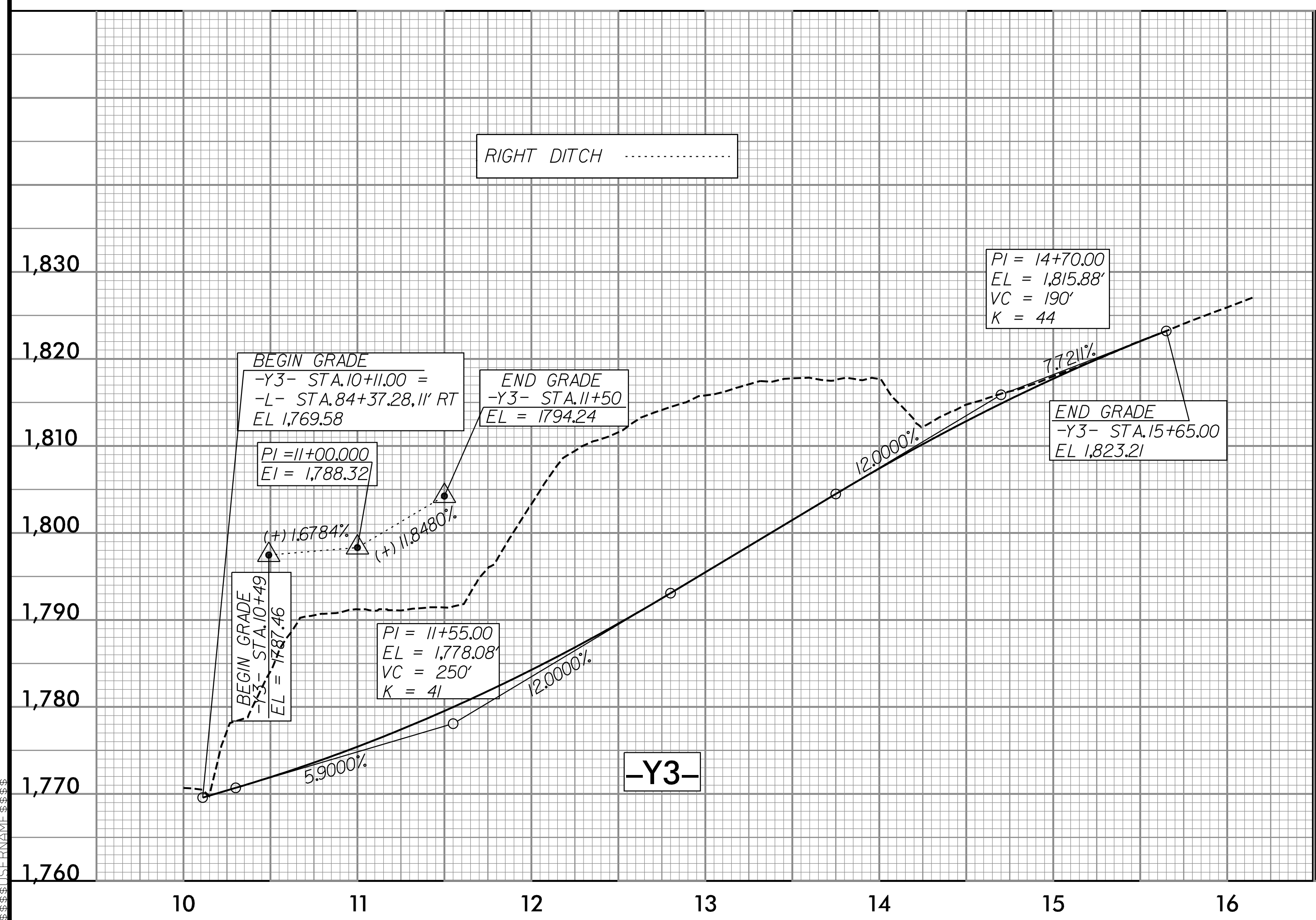
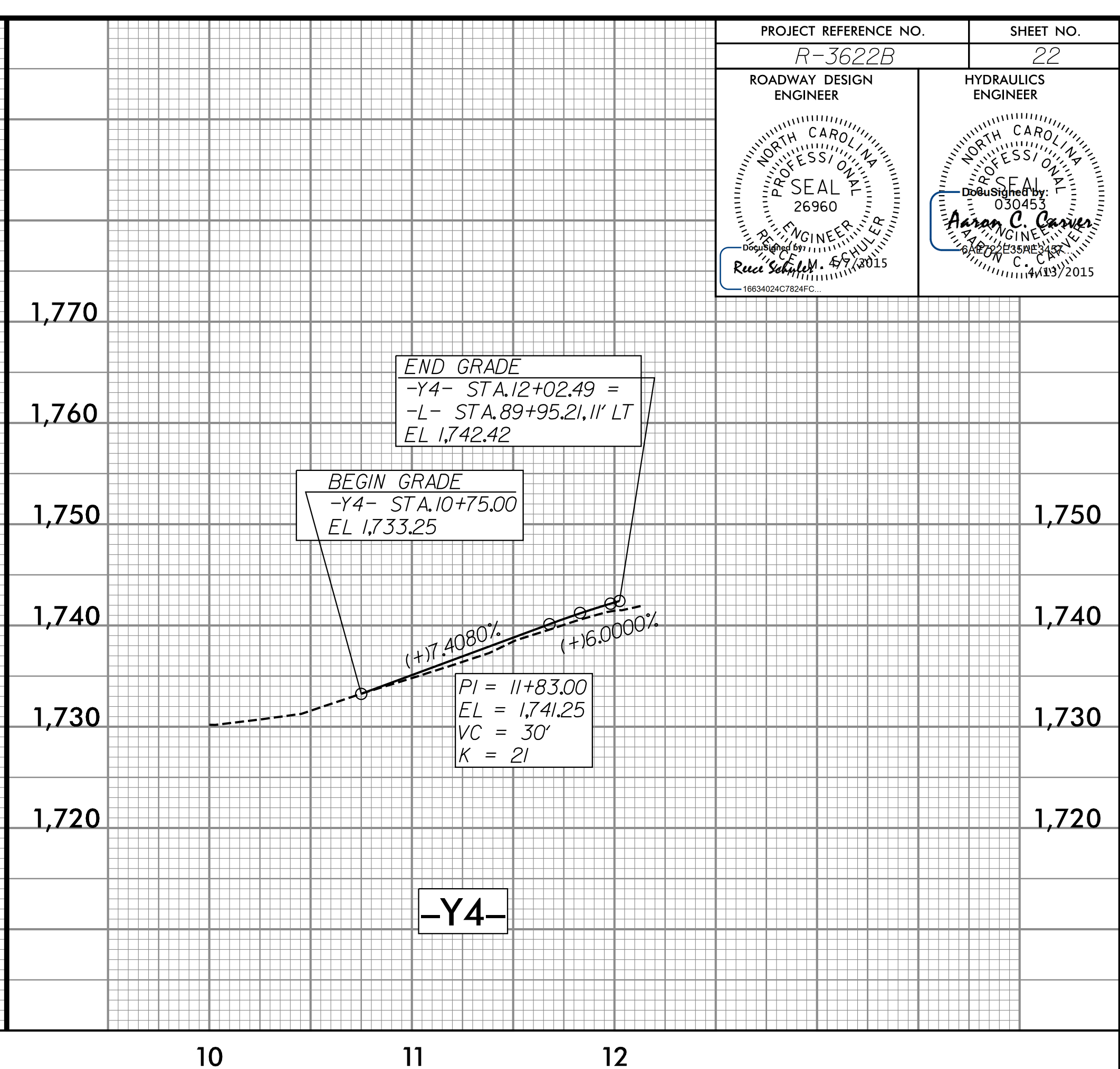
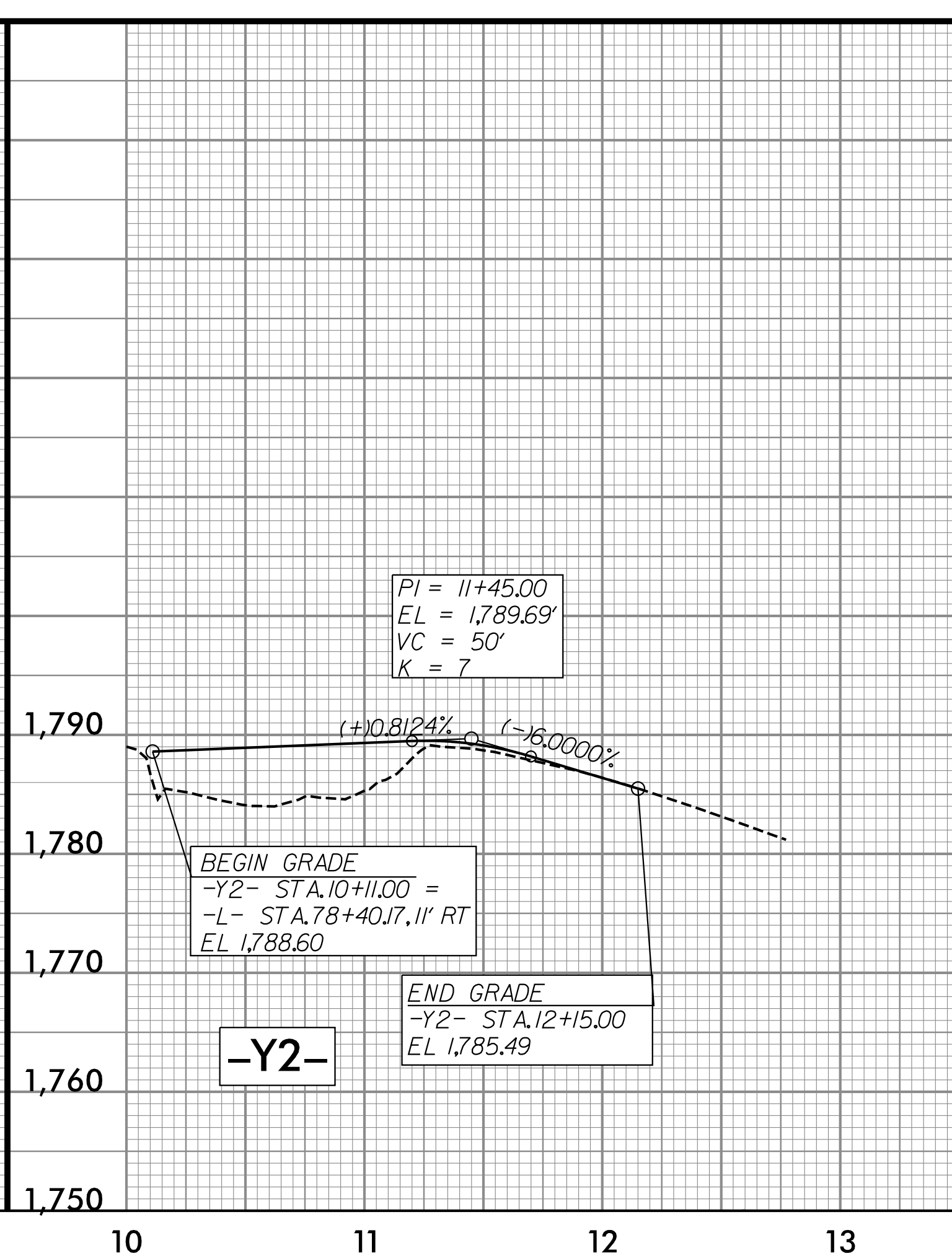
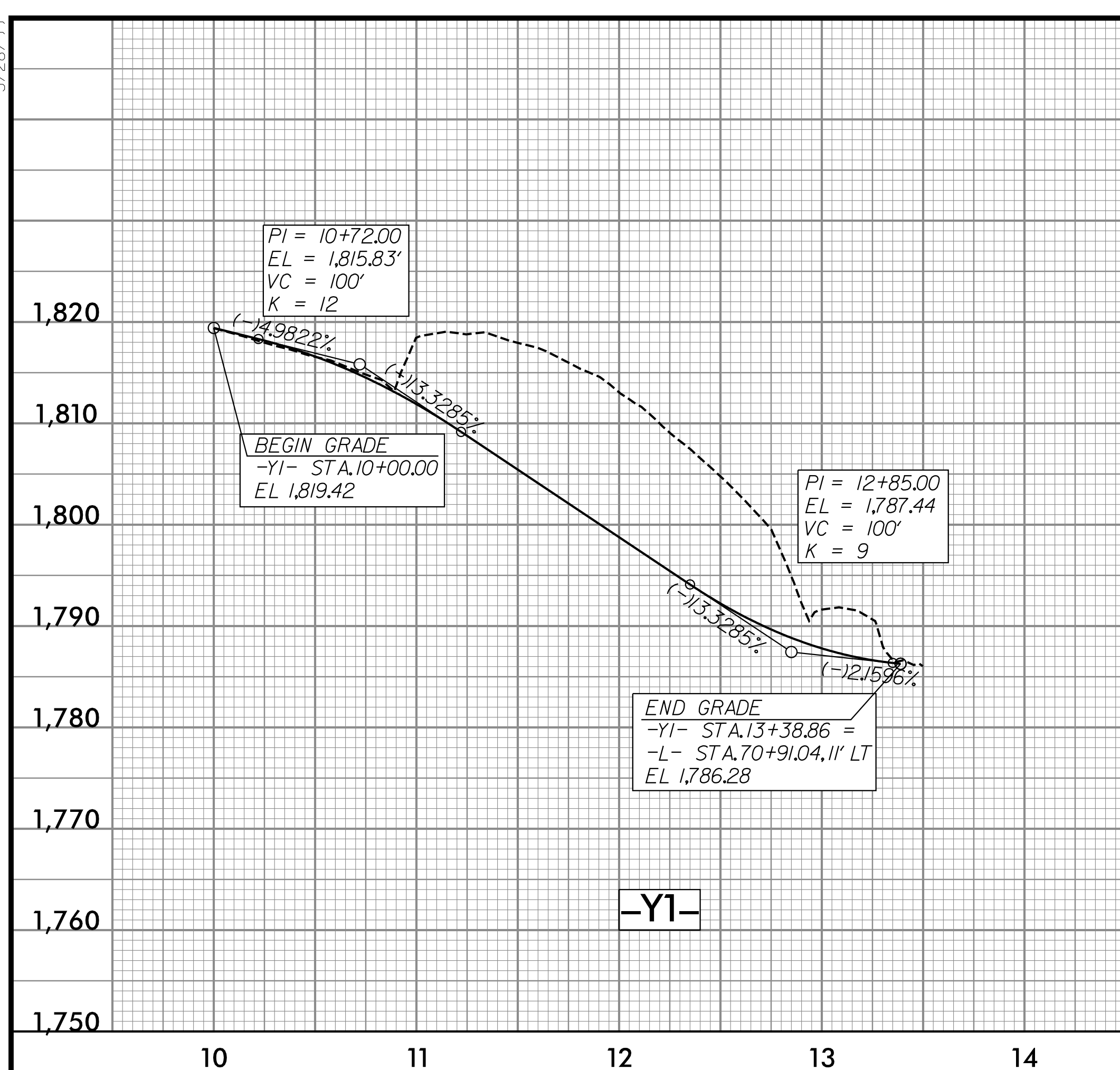
MATCHLINE -L- STA. 116+00 SEE SHEET 20

5/14/99



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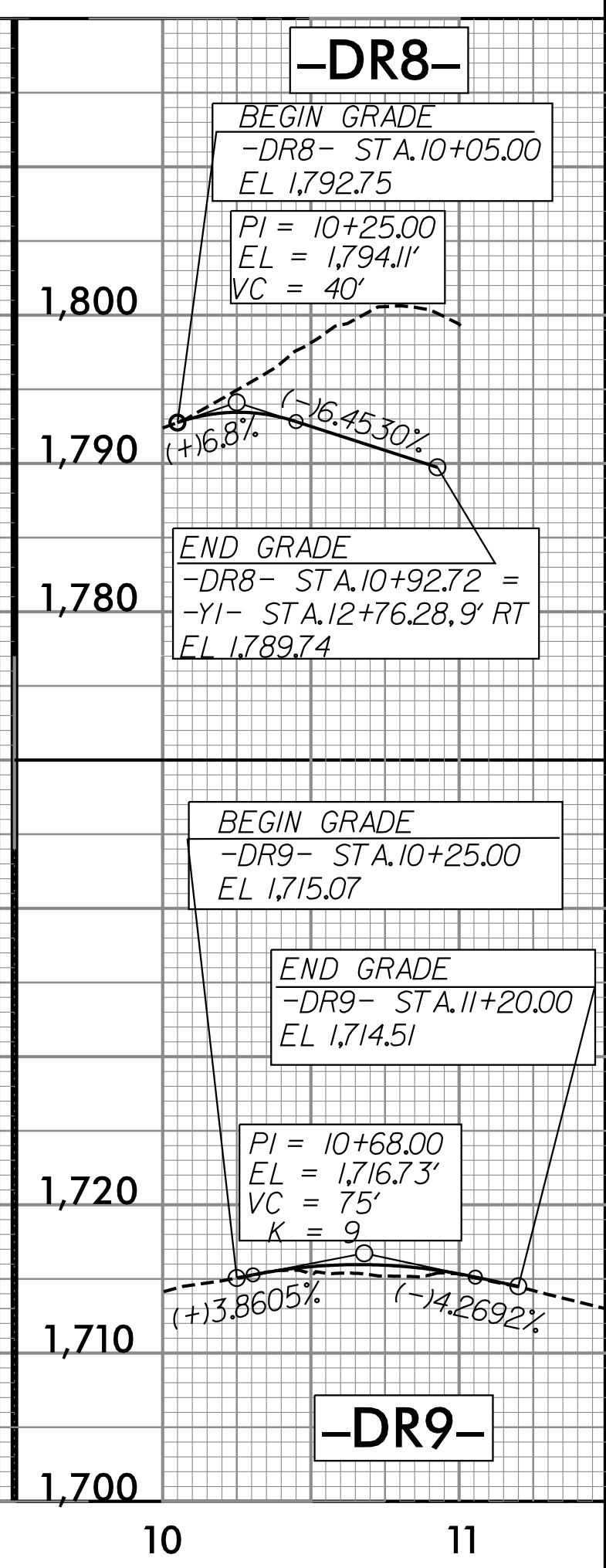
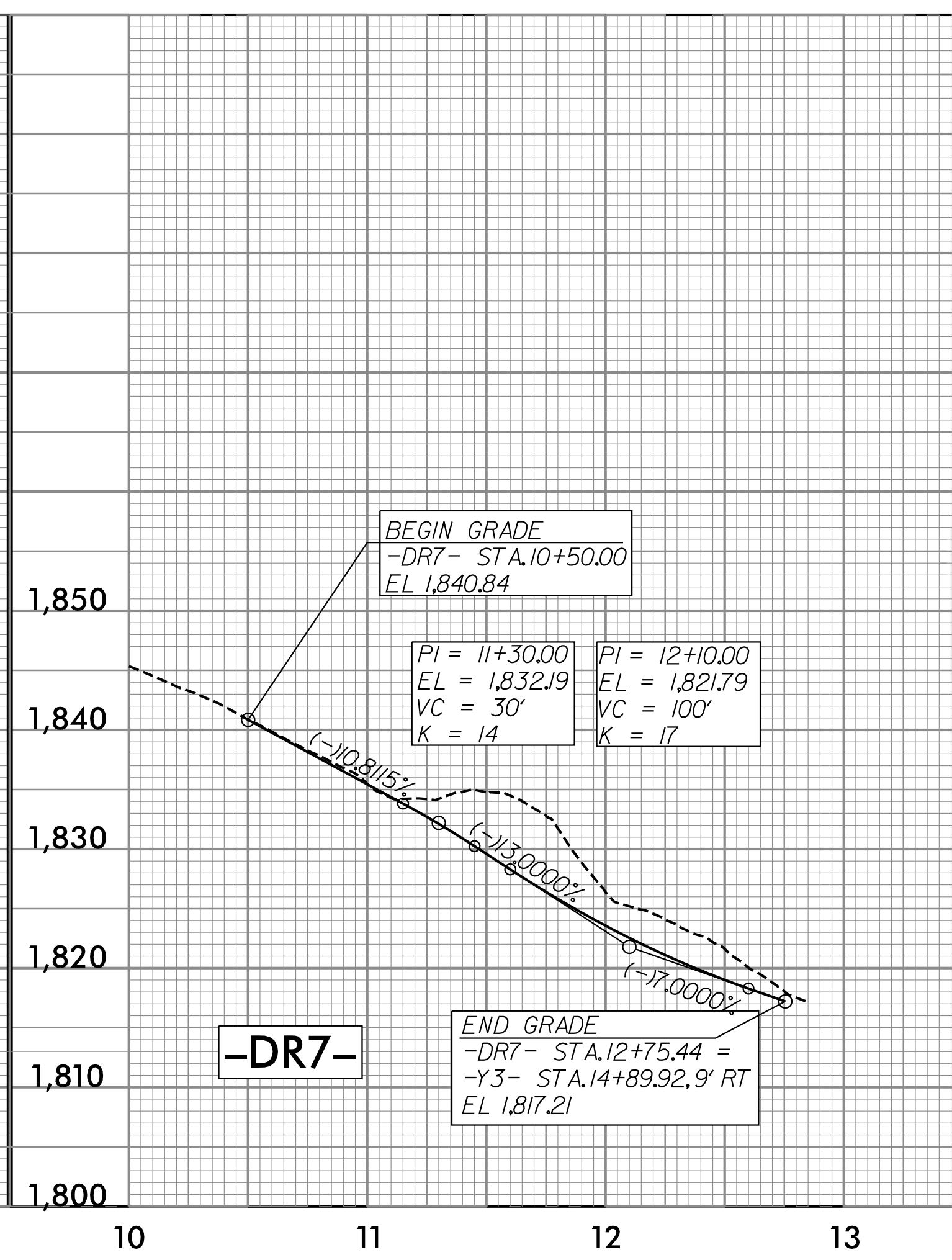
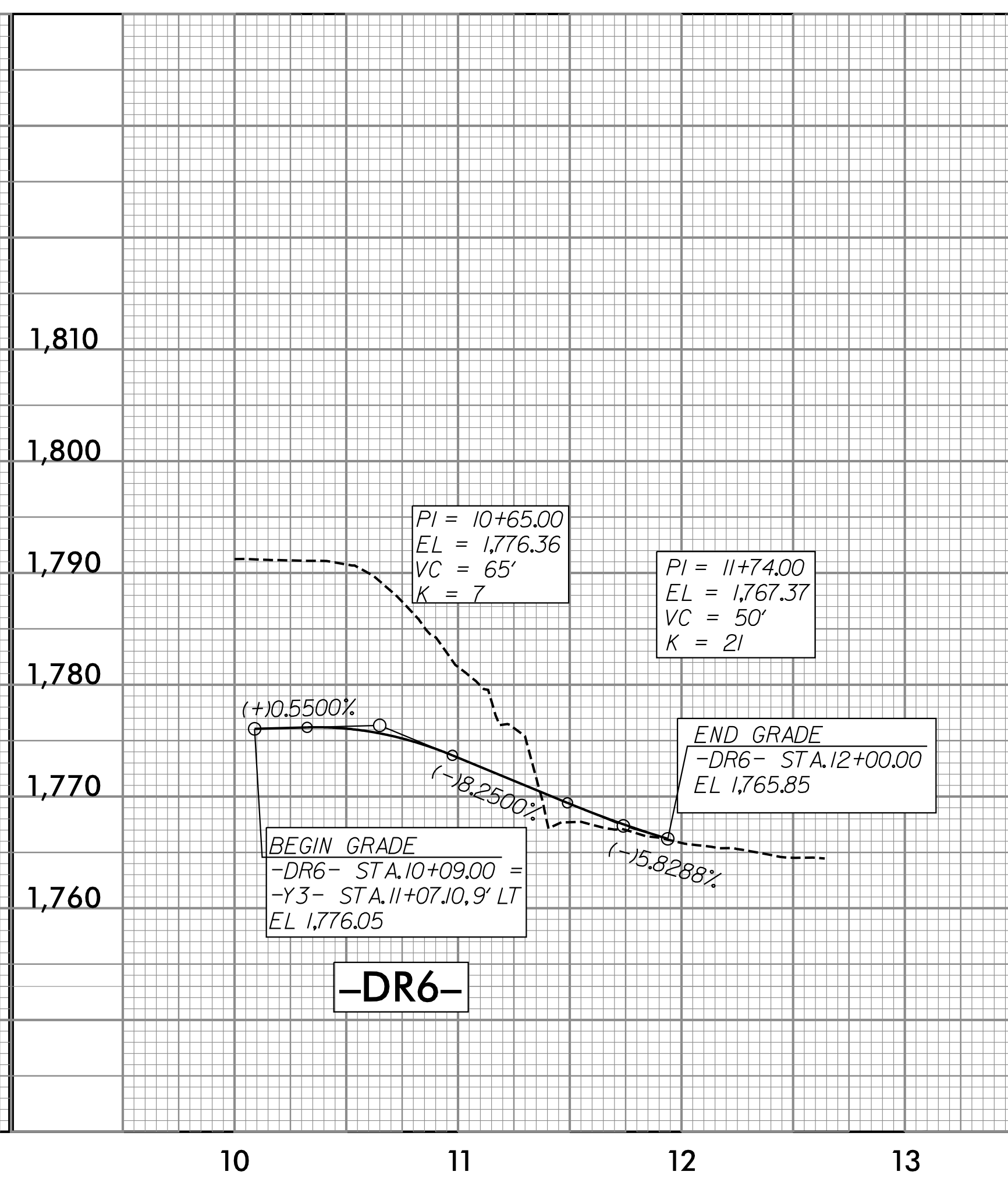
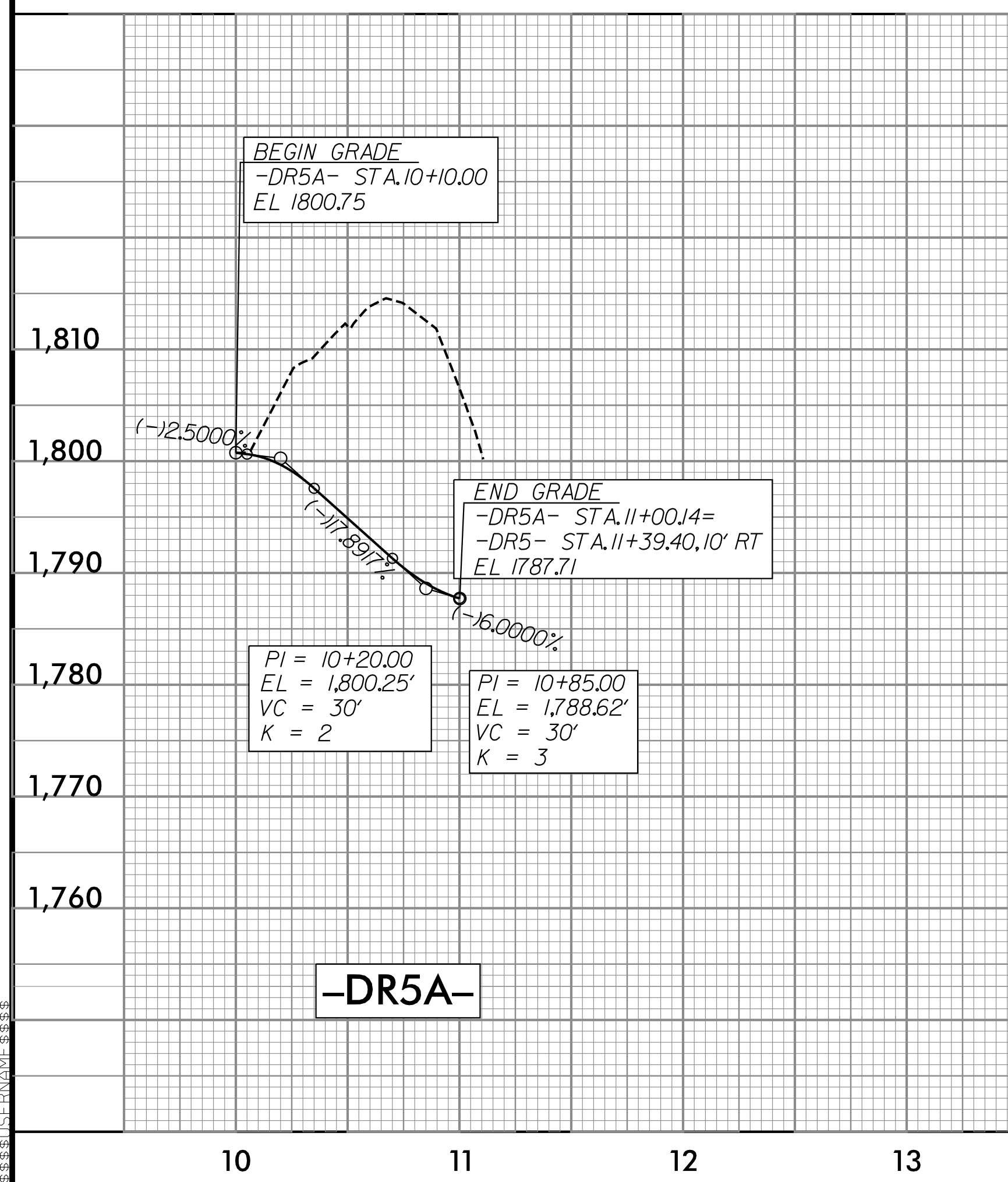
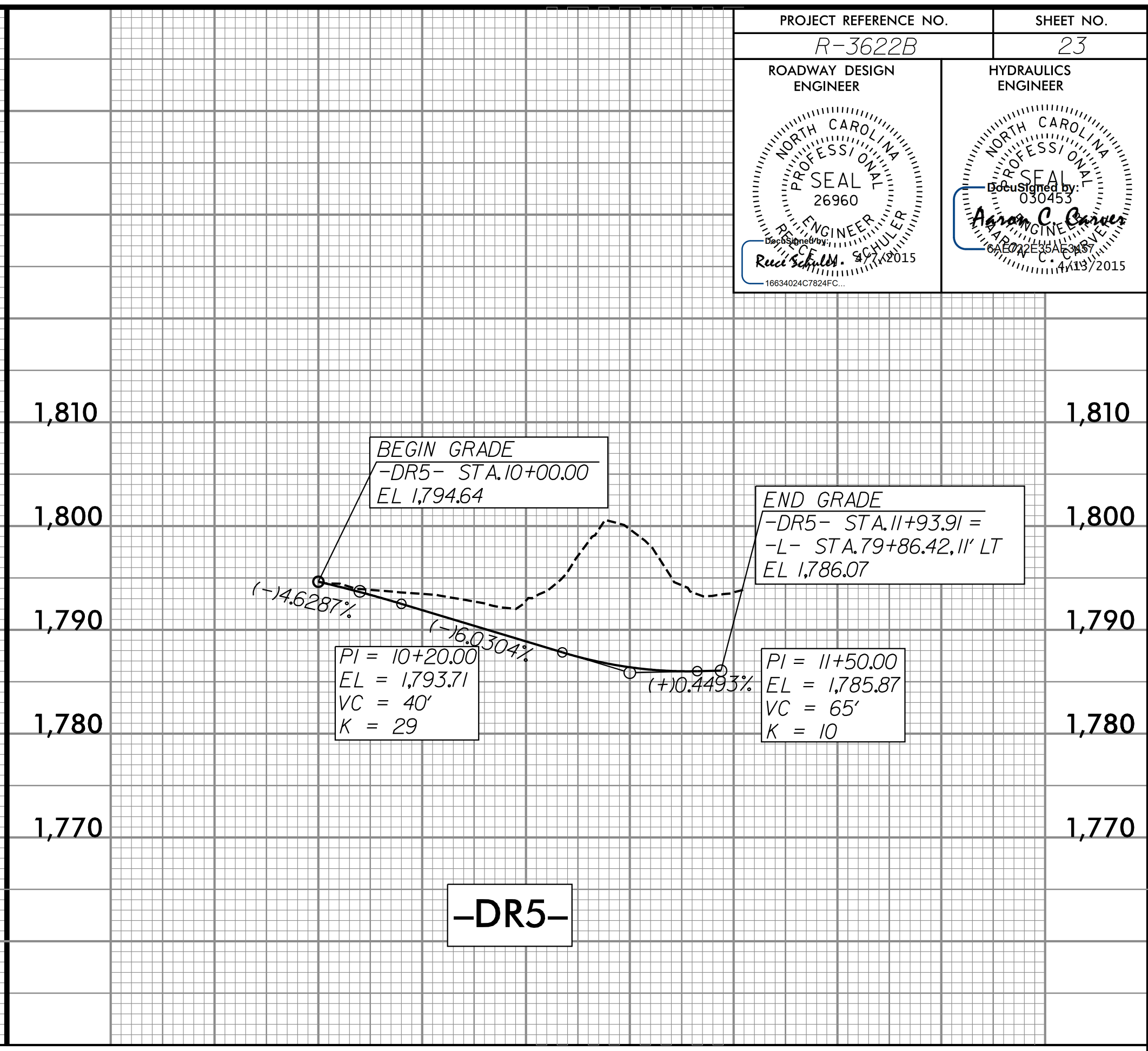
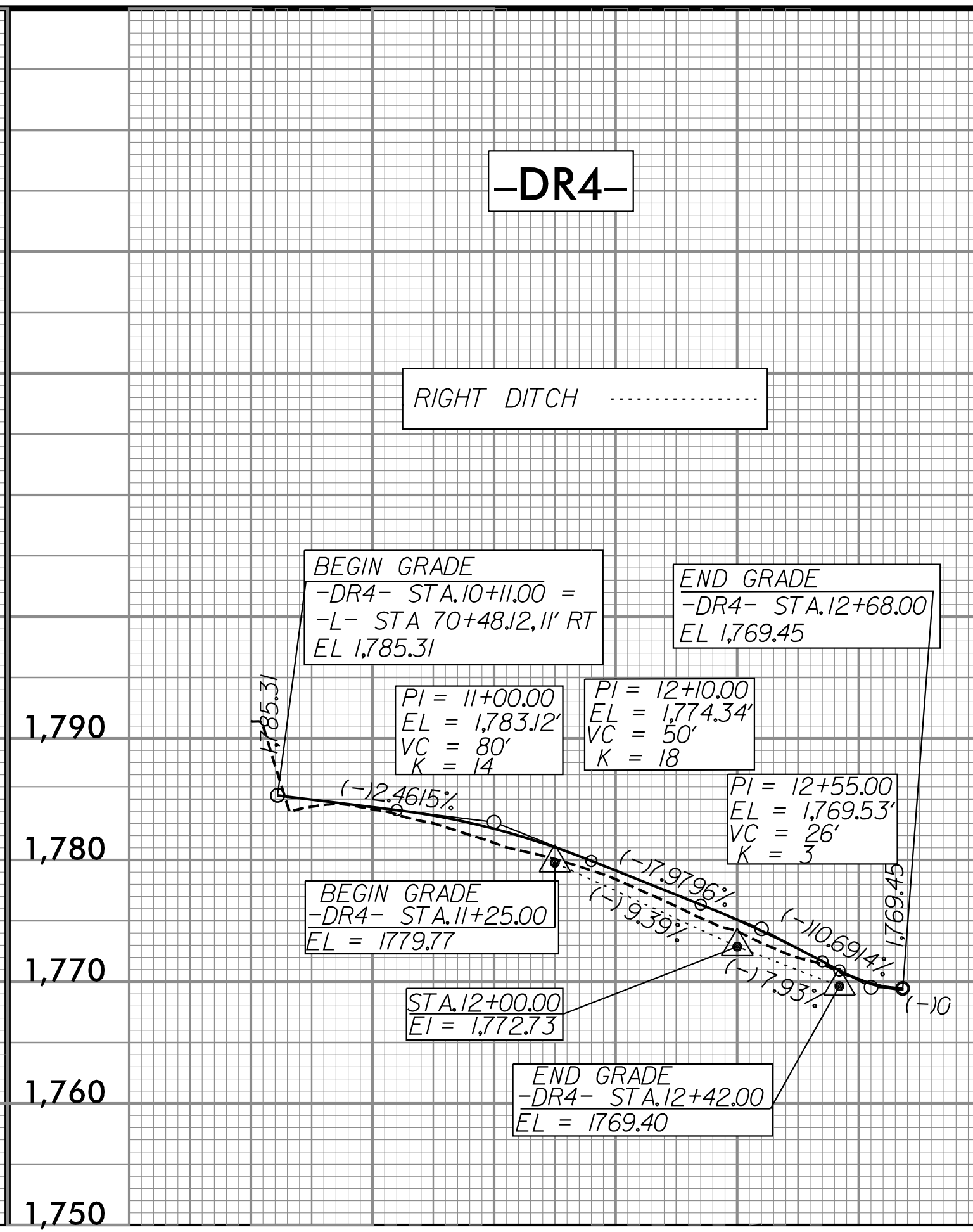
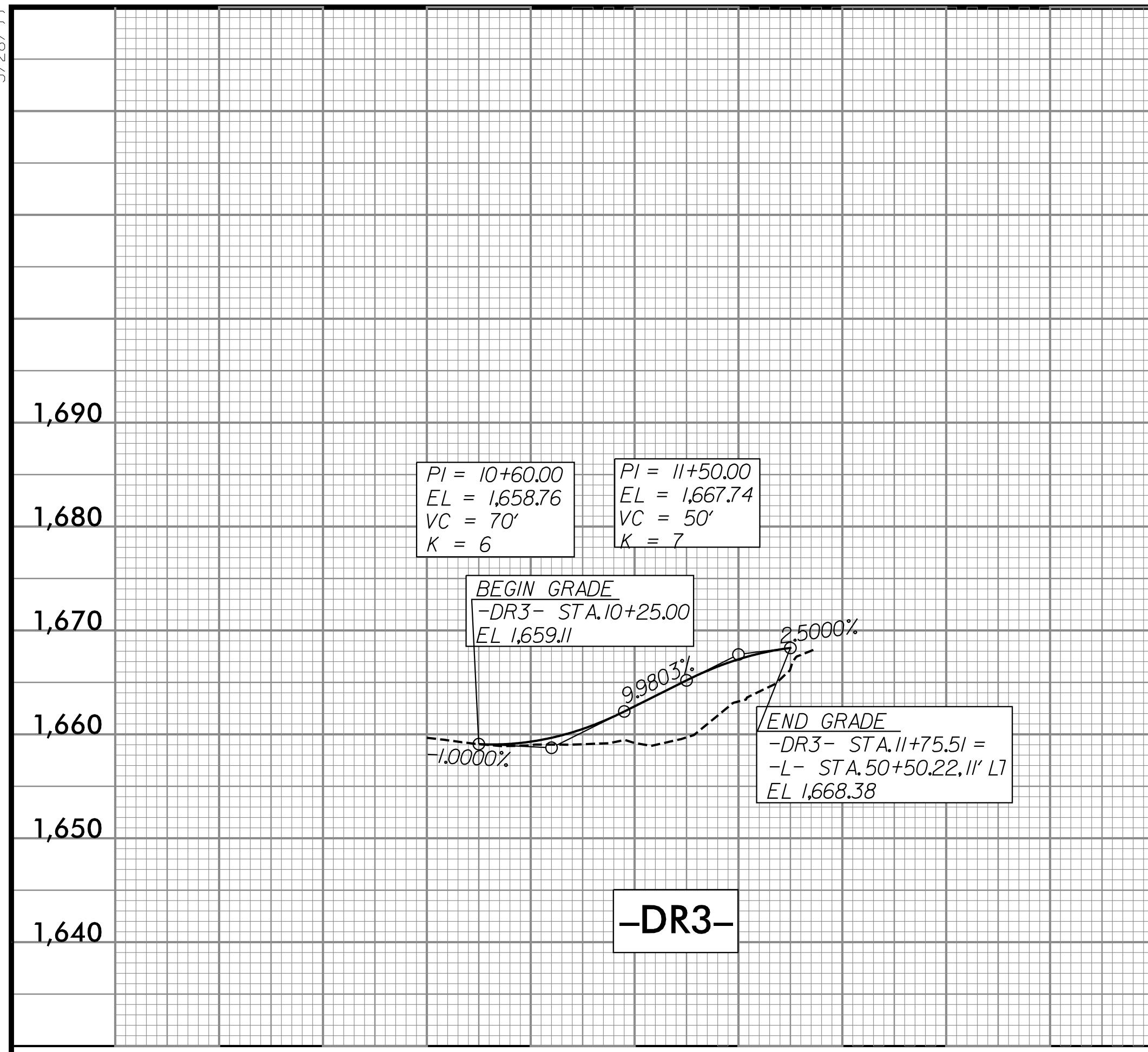
PROJECT REFERENCE NO. R-3622B	SHEET NO. 22
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



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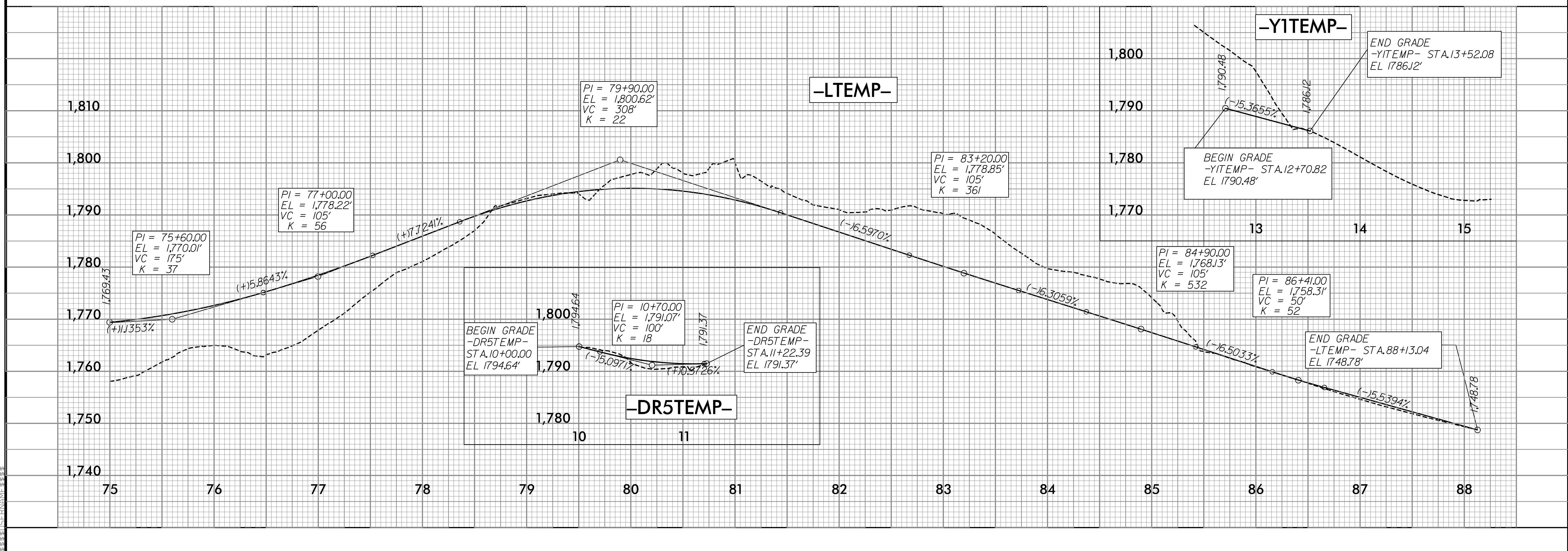
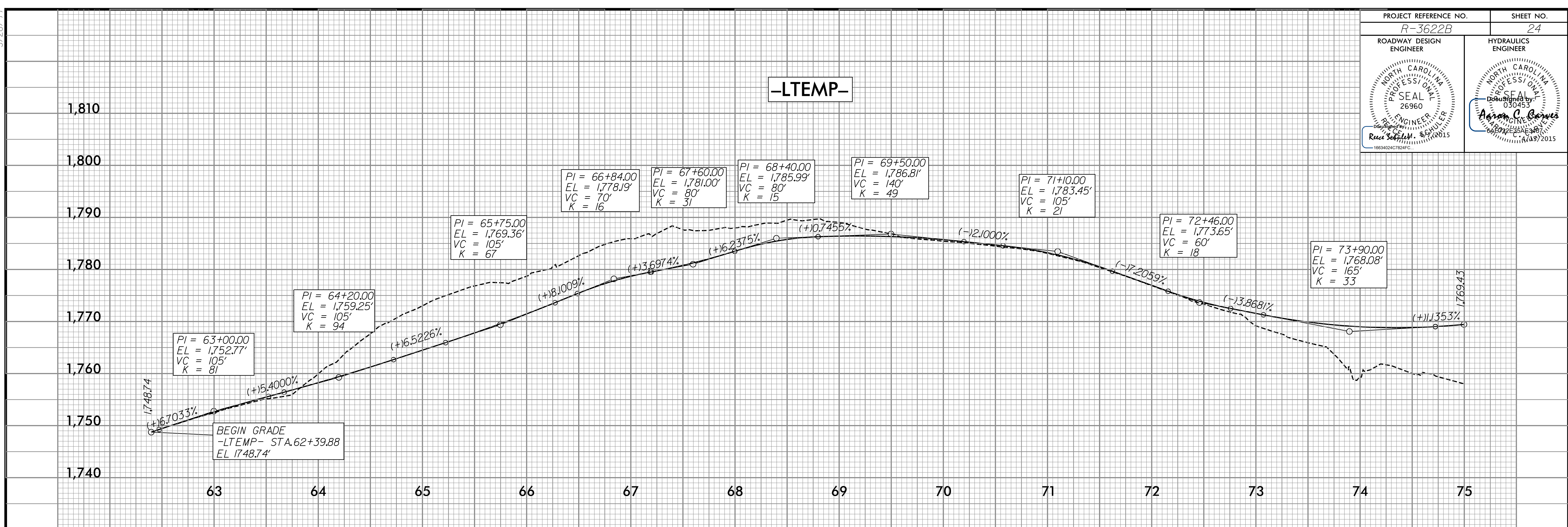
5/28/19

PROJECT REFERENCE NO. R-3622B	SHEET NO. 23
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



5/28/19

PROJECT REFERENCE NO. R-3622B	SHEET NO. 24
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
RECEIVED MAY 29 2019	DESIGNED BY A. C. CONNER



5/28/19