STATE	STATE PR	DIECT REPERENCE NO	SHEET	TOTAL
N.C.	B-	-4651	1A	36
STATE	PROJ. NO.	P. A.PROJ. NO.	DESCRIP	TION
33	817.1.1	N/A	P.E.	
			CONS	Г.

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE PE	ROJECT33	3817.1.1	I.D. NO.	B-4651
F.A. PROJ	ECT	N/A		
COUNTY_	UNION			
	DESCRIPTION	N REPL	ACEMEN	T OF
	BRIDGE No.			
	OVER CROO	KED CREE	<	

CAUTION NOTICE

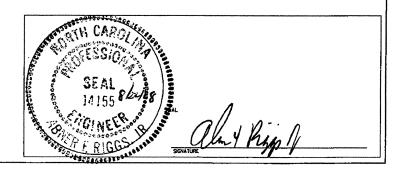
THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WAS MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PURPOSES. THE VARIOUS FELD BORNE (LOSS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL UNIT 0 (1919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA IS PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BORENOLE, THE LABORATORY SAMPLED DATA AND THE IN STU GIN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIBELITY WHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS AND VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WRID, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE OFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN BEFORMATION ON THES PROLECT, THE DEPARTMENT DOES NOT WARRANT OR GLARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BUDGER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS ENCESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED. THE MOLECT. THE CONTRACTOR SHALL HAVE NO CLAM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTE THE INFORMATION CONTAINED HEREM IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.
- NOTE BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED MEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

INVESTIGATED BY	S&ME	, INC.	PERSONNEL_	S. JOHNSON
CHECKED BY	A.F. RIGG	S, JR.		K. PLUMMER
SUBMITTED BY	S&ME	, INC.		J. LYNCH
DATE	AUGUST	22, 2008		J. MILLWOOD
			•	J. CANTRELL
				P. PHELPS
				T DEDE7



STATE STATE PR	OJECT REFERENCE NO.	SHEET NO.	SHEETS				
N.C.	3-4651	1B	36				
STATE PROJ.NO.	P.A.PROJ.NO.	DESCRIP	DESCRIPTION				
33817.1.1	N/A	P.E.					
		CONS	CONST.				

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

STATE P	ROJECT	33817.	l <u>.</u>	I.D. NO	B-4651
F.A. PRO	ECT	NZ.	A		
COUNTY_					
PROJECT			REPL	ACEMEN	Γ OF
	BRIDGE N				
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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

																	,		
				SOIL D	ESCR	IPTIO	ON							GRADA	TION				
WHICH CAN 188 BLDWS	ISIDERED TO BE PENETRA PER FOOT A	TED WI	TH A CONT	OLIDATED, S INUOUS FLIP NOARD PENE	EMI-CONS SHT POW TRATION	SOLIDAT ER AUG	ED OR ER, AND (AASHT)	128	H YIELDS LE B, ASTM D-15	iss than 186), soil		WELL GRADED- INDICATES A GO UNIFORM- INDICATES THAT SO! POORLY GRADED: GAP-GRADED- INDICATES A MIX	TURE OF UNIFO	DRM PARTICL	S OF TWO OR MC		IOARSE LSO		
CLASSIFICA CONSISTENC	TION IS BASI Y, COLOR, TE OGICAL COM	ED ON KTURE, POSITIO	THE AASH! MOISTURE, IN, ANGULA	O SYSTEM : AASHTO CLA NTY, STRUCT	AND BAS ISSIFICA 'URE, PLA	IC DESI TION, AP ASTICIT	CRIPTION NO OTHE Y, ETC. 1	R PER	NERALLY SH RYINENY FAC LE:	ALL INCLU	DE: f	THE ANGULARITY OR ROUNONES SUBANGULAR, SUBROUNDED, OR	S OF SOIL GR		OF GRAINS SIGNATED BY THE	TERMS: ANGU	LAR.		
			·	AND A								MINERALOGICAL COMPOSITION							
GENERAL CLASS.	GRA	NULAR	MATERIA SING *286	LS	SILT (>85)	CLAY I	HAYERIA ING "20	LS B)		NIC MATER		MINERAL NAMES SUCH AS QUARTZ, FELOSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.							
GROUP CLASS.	A-1 A-1-a A-1-b	A-3	A-Z-4 A-2	A-2 -5 A-2-6 A-2	A-4	A-5	_	A-7-8	A-1, A-2 A-3	A-4, A-5 A-6, A-7		SLIGHTLY COMPRESS MODERATELY COMPRE	IBLE	DMPRESS	LIQUID LIMIT LIQUID LIMIT	LESS THAN 3	9		
SYMBOL.	88888888888888888888888888888888888888			100				\leq				HIGHLY COMPRESSIBL	LE		LIQUID LIMIT	GREATER THA	N 50		
% PASSING = 10	58 HX								GRANULAR	SILT-	MUCK.	2004410 44470141		SILT- CLAY	F MATERIA				
- 48	38 HX 56 HX 15 HX 25 HX			JL	J				SOILS	CLAY SOILS	PEAT	TRACE OF ORGANIC MATTER	SOILS 2 - 3%	SOILS 3 - 5%	TRA	OTHER MATER	118L - 18X		
* 200 LIGUID LINGT	12 MYK2 HY	96 FIA			_	 	1				 	LITTLE ORGANIC MATTER MODERATELY ORGANIC	3 - 5% 5 - 10%	5 - 12% 12 - 28%	LIT SO:	TLE 10 ·	- 29% - 35%		
PLASTIC INDEX	6 MX	N.P.		N 48 MX4) I IX MN I					SOILS LITTLE	OR	HIGHLY	HIGHLY ORGANIC	>19%	>29%	H)G		AND ABOVE		
GROUP INDEX	0	8	0	4 HX	6 M3	12 HX	16 MX	lo MX	MODER AMOUN		ORGANIC SOILS	-		GROUND		hote - 7400			
USUAL TYPES OF HAJDA MATERIALS	STONE FRAGS. GRAYEL AND SAND	FINE SAND		OR CLAYEY AND SAND		ILTY DILS	SOIL		ORGAN MATTE			water level in bore hole immediately after drilling. Static water level after 24 Hours.							
GENL RATING			L		\top				FAIR TO	220				RATED ZONE	OR WATER BEAR	ING STRATA			
as a Subgrade			T TO GOO				TO POD		POOR	POOR	UNSUITABLE	HC HOLE CAN	VE IR SEEPAGE						
		P.1. OF		≤L.L3 ISTENC			-7-6 > ISENE	_	30			000		LLANEOL	S SYMBOLS				
DOIMADY	SOIL TYPE	T	OMPACTM	SS OR	RAN	GE OF	STANDA	100		OF UNCON		ROADWAY EMBANKI		(A) 201	CPT DAT TEST BORIN	IG 5A	MPLE		
PAIRM	SUIL TIPE	-	VERY LO		TENE IN	ON-YAL	UE)	7		ONS/FT2		ROADWAY EMBANKI WITH SOIL DESCRI	IPTION	_	AUGER BORING	DC 31	GNATIONS		
GENER GRANL		1	LOOSE			4 TO	18	- 1		N/A		SOIL SYMBOL		Ψ,	HOURT BURING		LK SAMPLE LIT SPOON		
MATER			MEDIUM I DENSE VERY DEI			10 TO 38 TO 35	59	1		17.4		ARTIFICIAL FILL ROADWAY EMBANKA	E NTS	ф-	CORE BORING	SA	ELBY TUBE		
		T	VERY SO	7		₹2 2 TO		T		⟨0.25	_	INFERRED SOIL B		* 0	MONITORING WE		MPLE ICK SAMPLE		
GENER SILT-	CLAY		MEDIUM	STIFF		4 TQ	8	- 1		.25 TO 0. 0.5 TO 1		INFERRED ROCK L		Δ	PIEZOMETER INSTALLATION		COMPACTED		
MATER (COHE			STIFF VERY ST	FF		8 TO	38	- 1		2 TO 4		TTTTT ALLUVIAL SOIL BO		\circ	SLOPE INDICATO	R TF	RIAXIAL SAMPLE		
ļ		Т.	HARD	XTURE	OP C	SK NIAD				>4		25/825 DIP/DIP DIRECTION ROCK STRUCTURES		\sim	INSTALLATION SPT N-VALUE	CBR -	CBR SAMPLE		
U.S. STO. S				4 18	4	9	68	200	278			• - SOUNDING ROD			SPI N-MALOE				
OPENING (A			T	.76 2.9		ASE	9.25	9.975 FINE		Т				ABBREVIA	TIONS		•		
BOULD		COB") DBBITE		AVEL SR.)	SA	ND . SD.)		SANC)] '	SILT (SL.)	(CL.)	AR - AUGER REFU BT - BORING TERI			PHT - PRESSU SD SAND, SAI		ா		
GRAIN	404 3Ø5		75	2.6			0.25	W . JI	9.05	0.005	5	CL CLAY CPT - COME PENE		7	SL SILT, SIL SLI SLIGHTL	TY			
SIZE	IN. 12"		3,					-	FERMA			CSE CDARSE DMT - DILATOMET			TCR - TRICON	REFUSAL			
SOIL	SI MOISTURE			JRE - C					FIELD MOI	PTUDE OF	CCDID710N	DPT - DYNAMIC PE		EST	7 - UNIT WE				
	RBERG LIM			DESCRI	PTION		GUIDE		FIELD MOI:	STURE DE	SCAIL LION	e - VOID RATIO F FINE	nout		W - MOISTURE V VERY	CONTENT			
, 477 P	LIOUI) LIMI	т_	- SATUR					IOUIDI VERY IN THE GRI			FOSS FOSSILIFE FRAC FRACTURE FRAGS FRAGMEN MED MEDIUM	D		VST - VANE SI	HEAR TEST			
PLASTIC RANGE				- WET	- (W)				REQUIRES		0		IPMENT U	ISED ON	SUBJECT P	ROJECT			
(PI) PL	- PLAST	IC LIM	417 _				HI IM	N OF	TIMON NO.	,		DRILL UNITS:	ADVANCING	TOOLS:		HAMMER T			
0₩ SL	-			- MOJS	T - (M)		SOLI	D; AT	OR NEAR	OPTIMUM	MOISTURE	MOBILE 8	2-17.	/18' DRAG BIT		AUTOR	HATIC MANUAL		
	T			- ORY	- (0)				ADDITIONAL TIMUM MOIS		ro	⊠ BK-51	=	NTINUOUS FLI LLOW AUGERS		CORE SIZE			
				Pi /	STIC	ITY						_ ~ ~ ·	=	FACED FING		⊠ ÷			
				PLASTICI					DRY STE	ENGTH		CME-45		-CARBIDE INS					
NONPLASTI					-5				VERY SLIG			☐ CME-750		NG 🔯 W/		HAND TOD			
MED. PLAS	ICITY			16-					MEDI HIG	UM		PORTABLE HOIST			STEEL TEETH	_	T. HOLE DIGGER		
HIGH PLAS	14C11A				OR MOI				- nio			OTHER DIEDRICH D-50 TRICONE TUMGCARB. HAND AUGER							
DESCR	IPTIONS MA	Y INC	LUDE CO				ONS (T/	N, RF	D, YEL-BAN	. BLUE-GR	AY)	OTHER DIEDRICH D-58	⊠ CORE	BIT			NOING ROO E SHEAR TEST		
	IERS SUCH											OTHER	Ø OTHE	R 2-1/4" H	.s.a.	OTH			
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ID	STATE	PROJECT	NO.	SHEET	NO.	TOTAL	SHEETS
B-4651		33817.1.1		28		3	6

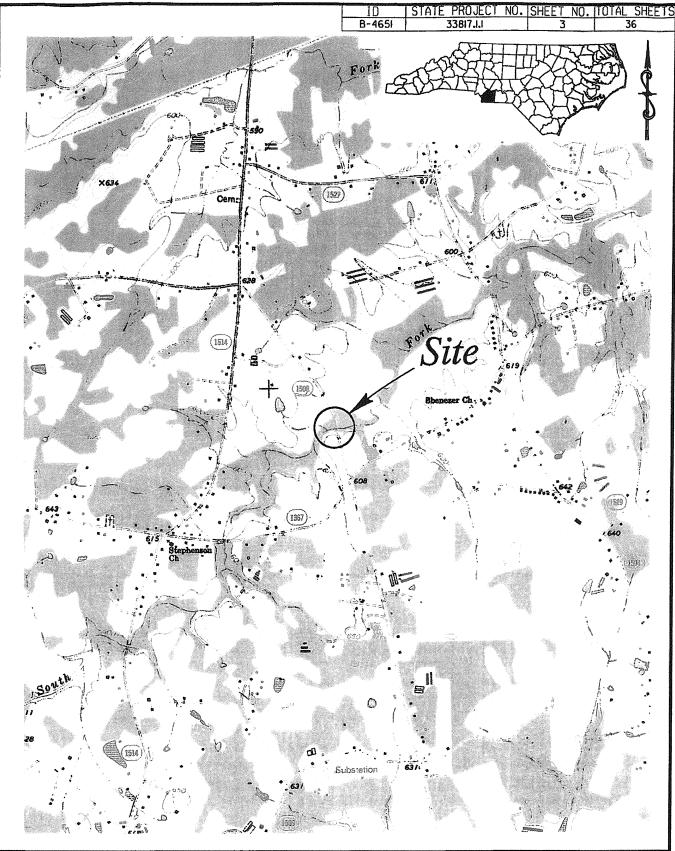
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

ROCK LINE INDICATES THE LEVEL SPT REFUSAL IS PERTARTION B IN NON-COASTAL PLAIN MATERIA OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY WEATHERED ROCK MATERIALS ROCK GETO NON-CRYSTALLINE ROCK OCR) NON-CRYSTALLINE ROCK OCR)	NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 180 BLOWS PER FOOT. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF YESTED. ROCK TYPE INCLUDES GRANITE, DAEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEQUIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IF TESTED. ROCK T INCLUDES PUTLLITE, SLANDSTOWN, ETC.	ADUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SMALE, SLATE, ETC. ARTESIAM - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.						
SPT REFUSAL IS PENETRATION BY IN NON-COSTAL PLAIN MATERIA OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY WEATHERED ROCK (MR) CRYSTALL INE ROCK (MR) COASTAL PLAIN SEDIMENTARY ROCK CP)	A SPLIT SPOON SAMPLER EDUAL TO OR LESS THAN BLI FOUT PER 68 BLU. DIVIDED AS FOLOWS: NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 180 BLOWS PER FOOT. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT MOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, DMEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN TETAMORPHIC AND NON-COASTAL PLAIN SEQUIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IF TESTED. ROCK T INCLUDES PUTLLITE, SLATE, SAMDSTOWS, ETC.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SMALE, SLATE, ETC. ARTESIAM - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.						
IN NON-COASTAL PLAIN MATERIAL OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY WEATHERED ROCK (MP) CRYSTALLINE ROCK (VCR) NON-CRYSTALLINE ROCK (VCR) COASTAL PLAIN ERDIMENTARY ROCK CP)	. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A DIVIDED AS FOLOWS: NON-COASTAL PLAIN MATERIAL THAT YIELOS SPT N VALUES > 180 BLOWS PER FOOT. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT MOULD VIELD SPT REFUSAL IF YESTED. ROCK TYPE INCLUDES GRAINITE, GMEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN HETAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IF TESTED. ROCK T INCLUDES PHYLLITE, SLATE, SAMOSTOWS, ETC.	ARGILLACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. OR HAVING A MOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAM - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.						
ROCK MATERIALS ARE TYPICALLY MEATHERED ROCK (MR) ROCK (MR) ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT YIELDS SPT N VALUES > 180 BLOWS PER FOOT. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF YESTED. ROCK TYPE INCLUDES GRANITE, DAEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEQUIMENTARY ROCK THAT WOULD YELD SPT REFUSAL IF TESTED. ROCK T INCLUDES PUTLLITE, SLANDSTOWN, ETC.	OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SMALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.						
CRYSTALLINE ROCK (CR) NON-CRYSTALLINE ROCK OCR) OMSTAL PLAIN EDIMENTARY ROCK CP)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD VIELD SY REFUSAL IF YESTED. ROCK TYPE INCLUDES GRANITE, DIKEISS, GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SCOINENTARY ROCK THAT WOULD YELD SYN REFUSAL IF TESTED. ROCK TIMCLUDES PHYLLITE, SLATE, SAMOSTOWE, ETC.	ARTESIAM - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IS IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.						
CRYSTAL INE ROCK (CR) ROCK (CR) ROCK OVER) COASTAL PLAIN SEDIMENTARY ROCK CP)	WOULD VIELD STI REFUSAL IF YESTED, ROCK TYPE INCLUDES GRANITE, DIKEISS, GABRIO, SCHIST, ETC. FINE TO COARSE GRAIN HETAMORPHIC AND NON-COASTAL PLAIN SCOINENTARY ROCK THAT WOULD YELLD STY REFUSAL IF TESTED, ROCK T' HICLUDES PHYLLITE, SLATE, SAMOSTOWE, ETC.	GROUND SURFACE.						
COASTAL PLAIN EDIMENTARY ROCK CP)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED, ROCK T INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	CALCAREOUS (CALC.) - SOILS WHICH CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.						
COASTAL PLAIN SEDIMENTARY ROCK CPI	INCLUDES PHYLLITE, SCRIE, SANUSTONE, ETC.	THE REPORT OF THE PROPERTY OF						
	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LEMGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TO LEMGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.						
	SHELL BEDS. ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT						
FRESH ROCK FRESH, CRYSTAL HAMMER IF CRYSTALI	S BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK. 102 - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.						
VERY SLIGHT ROCK GENERALLY FRO	ine. Suh, Joints Stained, some Joints May Show thin Clay Coatings if Opei En Specimen Face Shime Brightly. Rock Rings Under Hammer Blows if	N. DIP DIRECTION 1019 AZIMUTHO - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF						
DF A CRYSTALLINE A SLIGHT ROCK GENERALLY FRI	ATURE. ISH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	FAILT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.						
ISLIJ 1 INCH. OPEN JOINTS	MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME DCCASIONAL FELOSPAR AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOYS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.						
MODERATE SIGNIFICANT PORTION	s of rock show discoloration and weathering effects. In St feldspars are dull and discolored, some show clay. Rock has	FLOAT - ROCK FRAGMENTS ON SURFACE MEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL.						
DULL SOUND UNDER 1 WITH FRESH ROCK.	AMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (F.P.) - LAND BORDERING A STREAM BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.						
SEVERE AND DISCOLORED AND	ARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENG! ED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	THE PARTY OF THE P						
IF TESTED, WOLLD YI		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE HOVEMENT HAS DECURRED.						
ISEV.) IN STRENGTH TO STR	ONG SOIL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME ENTS OF STRONG ROCK USUALLY RENAIN.	ITS LATERAL EXTENT.						
VERY SEVERE ALL ROCK EXCEPT OF	<u>PT N VALUES > 180 BPF</u> HARTZ DISCOLORED OR STAINEO. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	LEMS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED MOT3 IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AGRATION AND LACK OF GOOD DRAINAGE.						
IV. SEV.) THE MASS IS EFFECT REMAINING, SAPROLIT	IVELY REDUCED TO SOIL STATUS, WITH DAILY FRAGMENTS OF STRONG ROCK E IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MID IGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT. N. YALUSS. C. 188 BP</i>	NOR PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF						
COMPLETE ROCK REDUCED TO SC SCATTERED CONCENTR ALSO AN EXAMPLE.	IL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND ATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	RESIDUAL SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (R.D.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY: TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN						
	ROCK HARDNESS	EXPRESSED AS A PERCENTAGE.						
	EO BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES IS OF THE GEOLOGISTS PICK.	SAPROLITE (SAP.) - RESIDUAL SOIL WHICH RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.						
	BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTERT, WHICH MAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS						
HARD EXCAVATED BY HAR	BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES CEEP CAN BE BLOW OF A GEOLOGISTS PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.						
MEDIUM CAN BE GROOVED OF HARD CAN BE EXCAVATED	5. 1 cougeo 8.95 inches deep by firm pressure of knife or pick point. In small chips to peices 1 inch maximum size by hard blows of the	STANDARD PENETRATION TEST (PENETRATION RESISTANCE 15FT) - NUMBER OF BLOWS IN OR B.F.J OF A 148 LB, HAMMER FALLING 38 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL. A 2 INCH QUISTIE DIAMETER SPLIT SPOON SAMPLER, SPI REFUSAL IS LESS THAN B.J FOOT PENETRAT						
POINT OF A GEDLOG SOFT CAN BE GROVED OR	ISTS PICK. GOUGEO READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	WITH 60 BLOWS.						
PIECES CAN BE BRO	IERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN KEN BY FINGER PRESSURE. H KNIFE, CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES I INCH	OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (S.R.O.D.) - A MEASURE OF ROCK QUALITY DESCRIBED BY:						
SOFT OR MORE IN THICKN FINGERNAIL.	ESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY							
FRACTURE SPAC								
	ACING TERM THICKNESS VERY THICKLY BEDDED > 4 FEET	BENCH MARK: RAILROAD SPIKE IN BASE OF TELEPHONE POLE						
VERY WIDE HORE T WIDE 3 TO H	HAN 18 FEET THICKLY BEODED 1.5 - 4 FEET	LOCATED AT STATION -BL- 19+90, 30' LEFT (STATION -L- 18+09.18, 29.52' RIGHT) ELEVATION: 591.34						
MODERATELY CLOSE 1 TO 3	FEET WERY THINK Y REPORTS 9.03 - 9.16 FEET	10111011 0 10 0 10 10 10 10 10 10 10 10						
	I FEET THICKLY LANIMATED 0.808 - 0.93 FEET THICKLY LANIMATED < 0.808 FEET THINLY LANIMATED < 0.808 FEET	NOTES:						
EUR CEUIMENTARY DOUVE IMPRATI	INDURATION N IS THE HARDENING OF THE MATERIAL BY CEMENTING, MEAT, PRESSURE, ETI	G.						
FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS: GENTLE BLOW BY MAMMER DISINTEGRATES SAMPLE.							
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.							
INDURATED	CRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE: DIFFICULT TO BREAK WITH HAMMER.							
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE: SAMPLE BREAKS ACROSS GRAINS.							



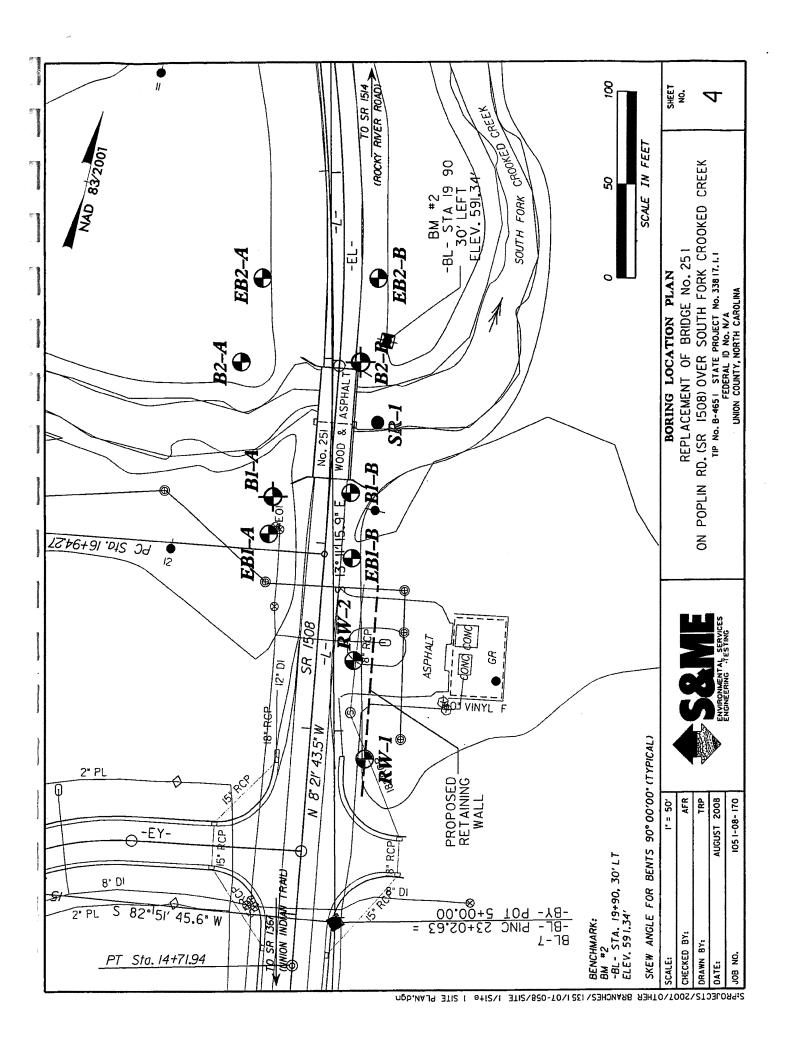
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DRAWN BY:	TRP
DATE:	AUGUST 2008
JOB NO.	105 1-08- 170

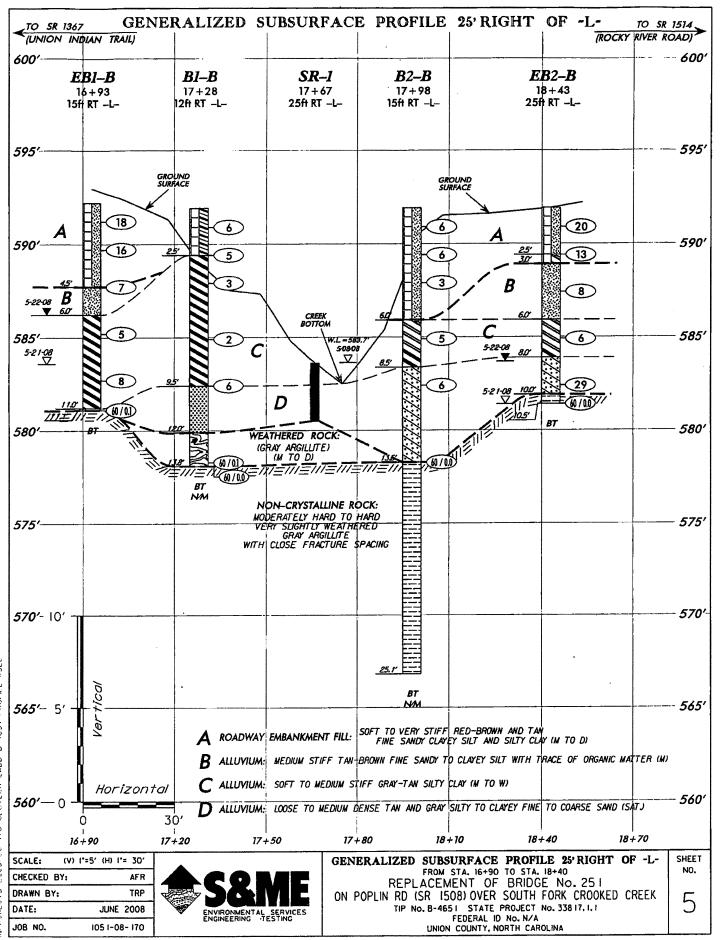
S.PROJECTS/2008/08-170/GEOTECH/CADD/8-4651 SITEVIC



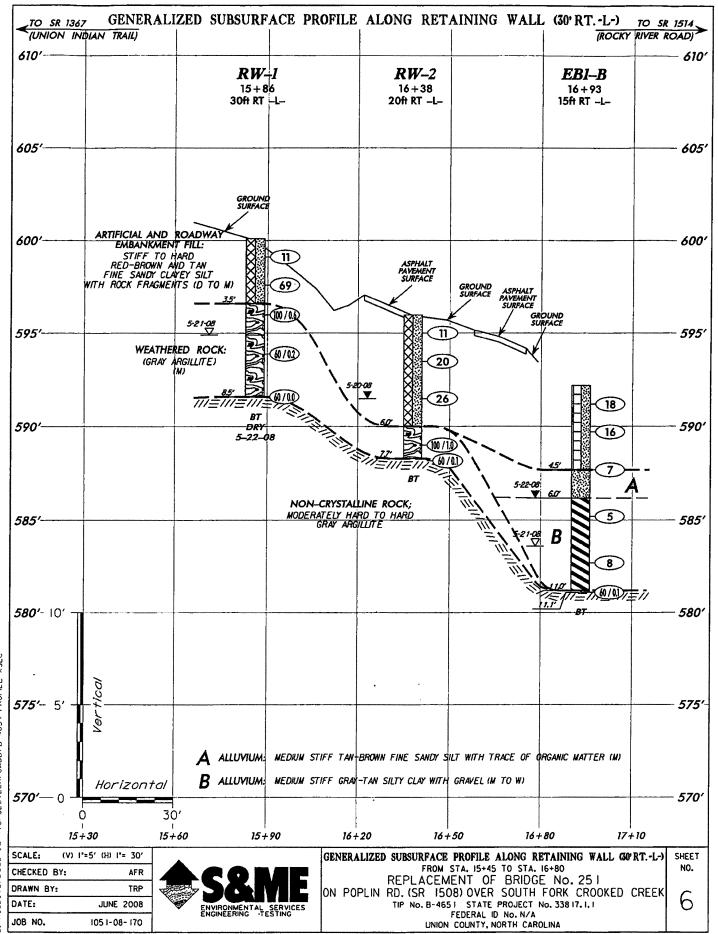
SITE VICINITY MAP

REPLACEMENT OF BRIDGE NO. 25 I
ON SR 1508 OVER CROOKED CREEK
STATE PROJECT NO. 338 I7. I. I TIP NO. 8-465 I
FEDERAL I.D. NO. N/A
UNION COUNTY, NORTH CAROLINA

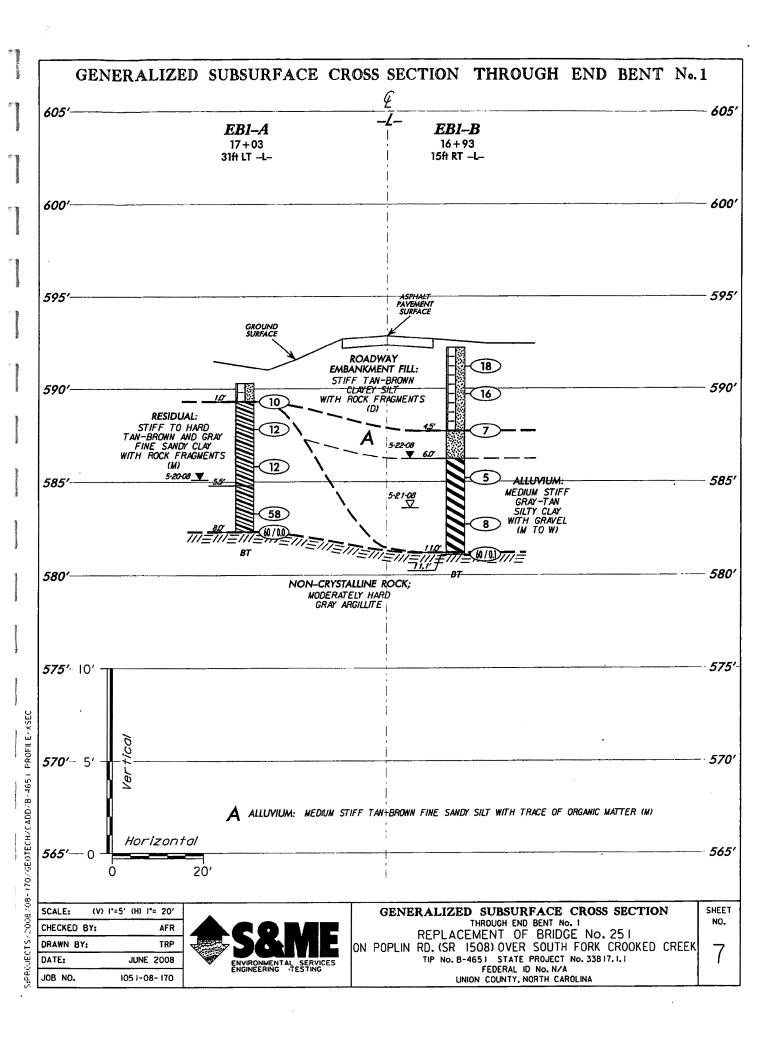


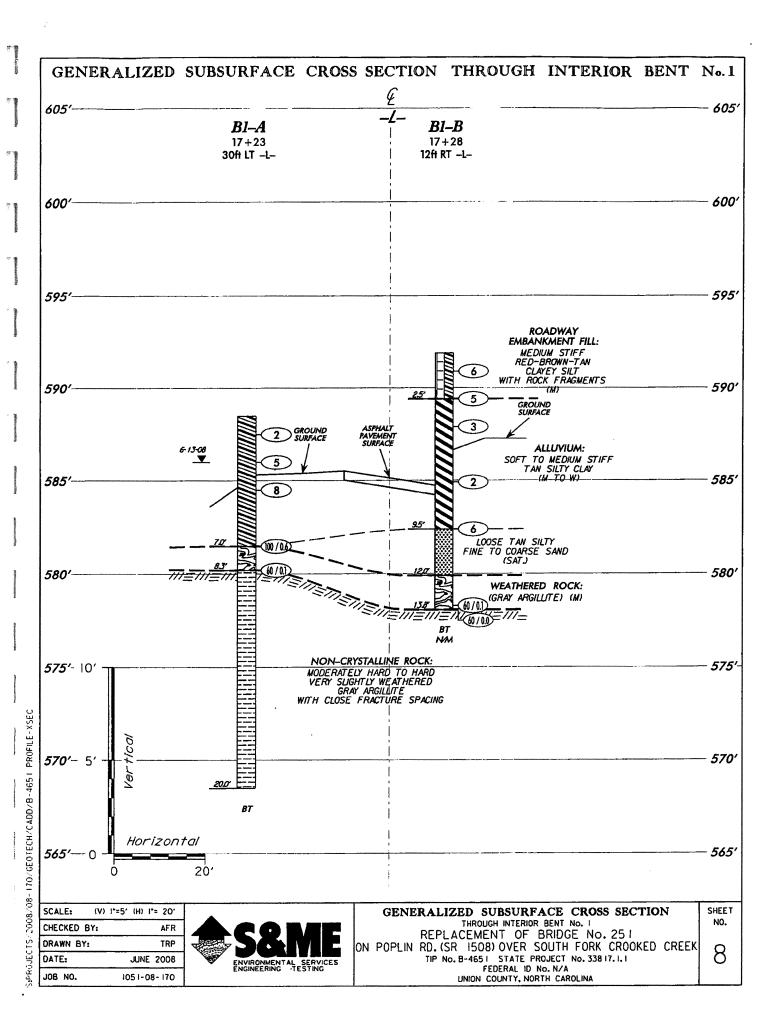


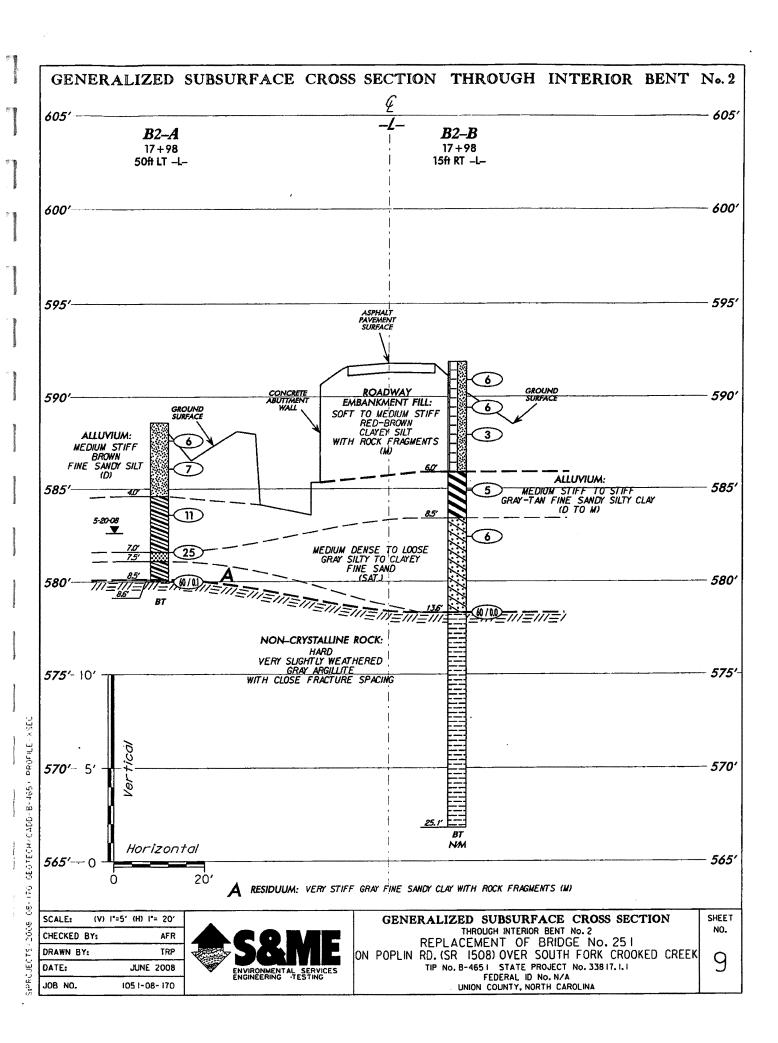
Ó8. Ó8-170 GEOTECH (CADD∕B-465). PROFILE-XSEC

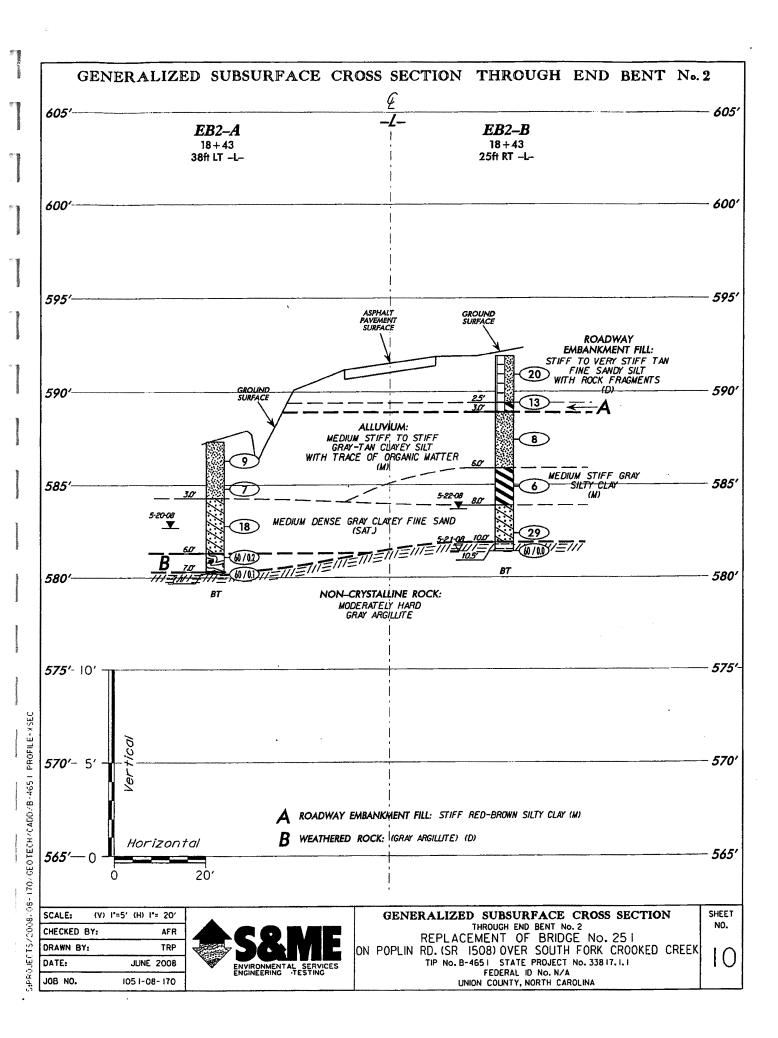


38:08-170/GEOTECH/CADD/B-4651 PROFILE-XSEC









PRO.	ECT NO). 338	17.1.1		ID.	B-4	651				COL	JNTY	U	nion				G	EOLOGIST K.P		
SITE	DESCRI	PTION	Brid	ge No.	. 251	on S	.R. 150	08 ove	Sou	ıth Fork	Crook	ed Cr	eel	κ						GROUND \	•
BORI	NG NO.	EB1-	A		S	TAT	ION 1	7+03			OFF	SET	31	RLT			ALIGNMEN	T	-L-	0 HR.	N/A
COLL	AR ELE	V. 59	0.3 ft		T	ATC	L DEP	rh 8.	0 ft		NOF	THIN	3	488,14	10		EASTING	1,	527,893	24 HR.	5.2
DRIL	L MACH	INE E	K-51		D	RILL	METH	OD 2	-1/4	" HSA									HAMMER TYPE	Manual	
STAF	T DATE	05/1	9/08		C	OMF	, DATE	05/1	9/08	3	SUR	FACE	W	ATER	DEPT	H	N/A		DEPTH TO ROC	K N/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLC 0.5ft	0.5ft		0		BLO ¹ 25		ER FOO	75 	100	11	SAMP. NO.	MOI	00	ELEV. (ft)	sc	OIL AND ROCK DES	CRIPTION	DEPTH (
590 585	595	0.0 1.5 3.5	4 4 6	4 5 6	6 7 6		10 _			· · · · · · · · · · · · · · · · · · ·					т. М		Cla Resi	aye idu	GROUND SURF, y Embankment Fill: y SILT (A-4) With Ro al: Stiff Tan-Brown G Clay (A-6) al: Hard Gray Fine S With Rock Fragm	Stiff Tan-Brow ck Fragments ray Fine Sand andy Clay (A-6	
	582.3	8.0	60/0.0			H.		ــــــــــــــــــــــــــــــــــــــ	• •	.●58.	<u>: :</u>	-60/0.0 ⁶	┥				582.3	В	oring Terminated with	Standard	
580	4	-															Peni ft o	etra In N	ition Test Refusal at lon-Crystalline Rock:	Elevation 582.3 Gray Argillite.	3
	1																-	1) A	dvanced 2-1/4" HSA	to 8.0 feet.	
575	1	-											1				-				
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NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

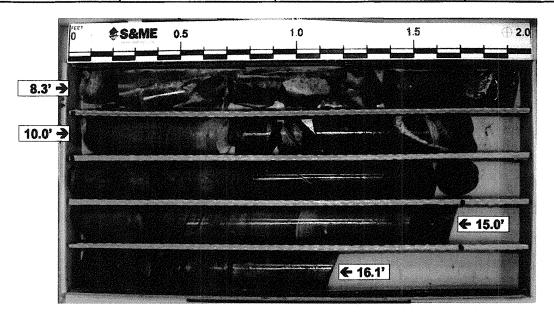
\leq	<u>/ \ </u>		BU	KE	LU	ואכ		UKI										
	JECT NO					B-465					DUNTY					GEOLOGIST		
SITE	DESCR	IPTION	Brid	lge No					outh For									OWTR (ft)
BOR	NG NO.	EB1-	В			TATION						15ft RT			ALIGNMEN		0 HR.	
COLI	AR ELE	EV. 59	2.2 ft		TO	DTAL E	EPT	1 11.1	ft	NC	PRTHING	3 488,	37		EASTING		24 HR.	6.0
DRIL	L MACH	INE E	3K-51		D	RILL M	ETHC	D 2-1/	4" HSA								YPE Manual	
STAF	RT DATE	05/2	1/08		C	OMP. D	ATE	05/21/	08	SI	JRFACE	WATER	DEP	TH I	N/A	DEPTH TO	ROCK 11.0 ft	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	O.5ft	0.5ft	0.5ft	0	25 1		PER FOO	75	100	NO.	MOI	O	ELEV. (ft)	SOIL AND ROCK	DESCRIPTION	DEPTH (ft)
	595	0.0	8	9	9		.•18			· 1			D				nt Fill: Very Stiff	
590	590.7 <u> </u>	- 1.5 - 3.5	10	7	9		716 16			+	· · · · ·		D	Ħ	587.7	• • •	Mith Rock Fragme	4.5
	586.2	6.0				•7-	•						Y		- coo o Alla	vium: Medium S	Stiff Tan-Brown Fir h Trace of Organi	ie co
585	583.7	- 8.5 -	5	3	6	58			 : : :	:	: : : :		W		Alluv	Mal	ter ff Gray-Tan Silty 0	Clay
580	581.2	11.0	60/0.1			I · I			<u> </u>	لمنا	60/0.1	-	w		581.2 581.1 \ Non	-Crystalline Ro	ck Moderately Ha	11.0 rd / 11.1
	_	-	33.0.	1											_ Penel	on-Crystalline R	d with Standard sal at Elevation 58 ock: Moderately H	1.1 ard
575	_		,												 - - 1).	Gray A "Advanced 2-1/4	rgillite. HSA to 11.0 Feet	•
570	-														- - - -			•
565	-	-													- - - -			
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PRO.	JECT NO					G REP B-4651		COUNTY	Union				GEOLOGIST K.	Plummer	
SITE	DESCR	IPTION	Brid	ge No	. 251	on S.R. 150	B over South For	k Crooked C	reek					GROUND WTI	R (ft)
	NG NO.					TATION 17		OFFSET				ALIGNMEN	T -L-	0 HR.	N/A
COLI	AR ELE	V. 58	8.5 ft	*********	TO	OTAL DEPT	H 20.0 ft	NORTHI	IG 488,	160		EASTING	1,527,890	24 HR.	2.5
DRIL	L MACH	INE C	iedric	h D-50) DI	RILL METHO	OD 3-1/4" HSA,I	NQ2 Core B	arrel				HAMMER TYPE	Automatic	
STAF	RT DATE	06/1	2/08		C	OMP. DATE	06/12/08	SURFAC	E WATER	R DEP	TH 1	N/A	DEPTH TO RO	CK 8.3 ft	
ELEV (ft)	DDIVE.	DEPTH (ft)		0.5ft	UNT 0.5ft	0 2	BLOWS PER FOC 5 50	OT 75 10	SAMP NO.	MO	L O G	S ELEV. (ft)	SOIL AND ROCK DE		n) HTe
585	590 _ 588.5 - 587.0 - 585.5 -	- 0.0 1.5 - 3.0	WOH 2 2	1 2 3	1 3 5	\$2				M W W		- 588.5 Allu	GROUND SUR Ivium: Soft to Mediur CLAY (A-6	n Stiff Tan Silty	0.
	582.5 -	- 6.0										-			_
580	580.3	8.2	3 60/0.1	30	70/0.1	: !::: -	-::::	100/0	41	D		Non-	Weathered Rock: (G Crystalline Rock: Mo Very Slightly Weathe	oderately Hard to red Gray Argillite	7. 8.
575	-											- - - -	With Close Fracture	e Spacing	
570	-											- - 568.5 - Borii	ng Terminated at Elev	vation 568.5 ft in	20.
565	-	<u>-</u>					•					<u> -</u> - 1	Crystalline Rock: Ha Weathered Gray Advanced 3-1/4" HS	Argillite. A to 8.2 feet.	
560												3) A	pproximate Drilling Fli pcf. dvanced NQ2 Core B 19.9 feet. Creek Water Used as	arrel from 8.3 to	
555	-	† - -										[5) 	No Loss of Drilling Fi	uid Observed.	
550	-											-			
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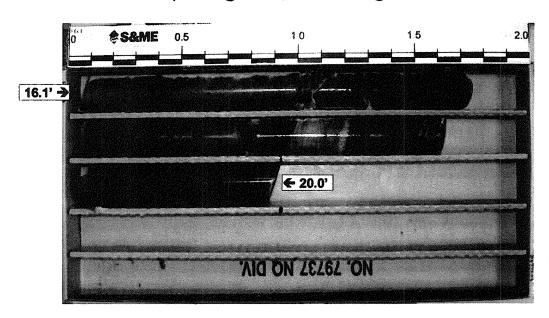
2	Y Y		CO	RE B	OR	ING	RE	PO	RT			_		
PRO.	JECT NO). 338	17.1.1	ı	D . B-	4651				СО	JNTY Union	GEOLOGIST K.P	lummer	
SITE	DESCR	PTION	Brid	ge No. 2	51 on	S.R. 1	508 over	South	Fork	Croc	ked Creek		GROUND W	TR (ft)
BOR	NG NO.	B1-A			STAT	TION	17+23			OF	SET 30ft LT ALIGNME	NT -L-	0 HR.	N/A
COLI	AR ELE	V. 58	8.5 ft		TOT	AL DE	PTH 20.	0 ft		NO	RTHING 488,160 EASTING	1,527,890	24 HR.	2.5
DRIL	L MACH	INE C	iedric	h D-50	DRIL	L MET	HOD 3-	1/4" H	ISA,N	Q2 C	ore Barrel	HAMMER TYPE	Automatic	
STAF	RT DATE	06/1	2/08		COM	P. DA	TE 06/1:	2/08		su	RFACE WATER DEPTH N/A	DEPTH TO ROC	K 8.3 ft	
COR	E SIZE	NQ2			TOTA	L RUI	N 11.7 f			DR	LLER J.Millwood			
ELEV	RUN ELEV	DEPTH	RUN	DRILL RATE	REC.	JN RQD	SAMP.	STR REC.	ATA RQD	O L	DESCRIPTION	AND REMARKS		
(ft)	(ft)	(ft)	(ft)	(Min/ft)	(ft) %	(ft) %	NO.	(ft) %	(fi) %	Ğ	ELEV. (fl)		DE	EPTH (fl
	580.2	 				(0.0)		(44.6)	/7 0			ing @ 8.3 ft	athy Moathered	8.3
	580.2 578.5	- 8.3 - 10.0	1.7 5.0	3:30 1:30/0.7	(1.7) 100%	(0.0) 0%		(11.6) 99%	(7.2) 62%		580.2 Non-Crystalline Rock: Moderately Gray Argillite With Close Fracture 9.2 Feet, 6 Fractures at 15°, 14	Spacing-1 Vertical Fractu	re From 8.4 to	0.4
	1		5.0	3:30 1:30 2:00	(4.9) 98%	(3.3) 66%					9.2 Feet, 6 Fractures at 15 , 14	riactules at 35 , 5 riaci	luies at 40	
575	573.5 -	- 15.0		2:00 2:00							_			
	3	-	5.0	2:15 1:30	(5.0) 100%	(3.9) 78%								
570		_		2:00 2:00	100%	,0,0					-			
	568.5	20.0		2:15				<u> </u>			568.5 Boring Terminated at Elevation 568	5 ft in Non-Crystalline R	ock: Hard Verv	20.0
]										Slightly Weath	ered Gray Argillite.		
565	-										1) Advanced 3-	/4" HSA to 8.2 feet. ng Fluid Density 62.4 pcf		
		-									3) Advanced NQ2 Core	Barrel from 8.3 to 19.9 for Ised as Drilling Fluid.	eet.	
560	_	-										lling Fluid Observed.		
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CORE PHOTOS

Project No.: 33817.1.1	ID No.: B-4651	Location: Union Co., NC	Boring No.: B1-A
Site Description: Bridge No	o. 251 on SR1508 over South	h Fork Crooked Creek	Driller: J. Millwood
Collar Elev.: 588.5 ft.	Core Size: NQ2	Equipment: Diedrich D-50	Geologist: K. Plummer
Elev. at T.D.: 568.5 ft.	Total Depth: 20.0 ft.	Total Run: 11.7 ft.	Date: 6/12/2008



Box 1 of 2
Top of Box @ 8.3 feet; Bottom of Box @ 16.1 feet



Box 2 of 2
Top of Box @ 16.1 feet; Bottom of Box @ 20.0 feet

C	y \mathbf{U}		30	KE	_0(s KE	:P	ORT											
PRO	JECT NO). 338			1	B-4651					COUNTY					GEOL	OGIST	K.Plummer	
SITE	DESCR	PTION	Brid	lge No					uth Fork	_	ooked Cre							GROUND V	
BOR	NG NO.	B1-B			S.	TATION	17-	+28			OFFSET	,			ALIGNMEN			0 HR.	N/M
COL	AR ELE	V. 59	1.9 ft					1 13.8 f		1	IORTHING	488,1	71		EASTING			24 HR.	N/M
DRIL	L MACH	INE C	iedric	h D-50	D	RILL ME	THO	D 3-1/4	" HSA									YPE Automatic	
STAF	RT DATE	06/1	2/08		C	OMP. DA	TE	06/12/0	8	<u></u>	SURFACE		DEP		N/A	DEP	тн то і	ROCK N/A	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLC 0.5ft	0.5ft		0	25	BLOWS	PER FOC	7! 	5 100	NO.	MO	0 1 G	ELEV, (ft)	SOIL AN	D ROCK	DESCRIPTION	DEPTH (ft)
590	595 _ 591.9 _	0.0	4	3	3	• 6 ·	[· · · ·	• 7			м		D-4	dway Er	nbankme	SURFACE ant Fill: Medium Stiff LAY (A-6) With Rock	0.0
290	588.9	3.0	2	2	3	9 5.			1	•			w		\		Fragm	ents Silty CLAY (A-7-6)	
	505.0	-	2	1	2	∳ 3 · ·	::	: : : :	: : :				VV		- AI	invinu:	Soft (an	SIRY CLAY (A-7-0)	
585	585.9	6.0	1	1	1	2			ļ <u>.</u>	-			w		F				
	583.4	8.5	WOH	1	5	/ :			: : :	-			Sat.		582.4			6W 5' 1 0	9.5
500	-	-				🕶	::		:::	:		1			579.9 Allu	ıvium: Lo	oose Tan SAND (/	Silty Fine to Coarse A-2-4)	12.0
580	578.4 -	- 13.5							 	·	50/0.1		м		578.1			: (Gray Argillite)	13.8
575	578.1 Z	13.8	60/0.1 60/0.0								60/0.0 60/0.0		M		Pene ft	etration T	est Refus	d with Standard sal at Elevation 578.1 e Rock: Hard Gray ite.	
570		-													t 1 - 1) Advanc	ed 3-1/4"	HSA to 13.8 feet.	٠
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	JECT NO					B-4651			COUNT					GEOLOGIST	K.Plummer	
SITE	DESCR	PTION	Brid	ge No	y	on S.R. 1508		th Fork	Crooked (Creek			——————————————————————————————————————		GROUND	WTR (ft)
BOR	ING NO.	B2-A	\		s ⁻	TATION 17	+98		OFFSET	50ft L1			ALIGNMEN	IT -L-	0 HR.	N/A
COL	LAR ELE	V. 58	8.6 ft		TO	OTAL DEPTI	1 8.6 ft		NORTHI	NG 488	,227		EASTING	1,527,857	24 HR.	6.0
DRIL	L MACH	INE E	3K-51		D	RILL METHO	D 2-1/4"	HSA						HAMMER	TYPE Manual	
STAF	RT DATE	05/1	9/08	·····	C	OMP. DATE	05/19/08		SURFAC	E WATE	R DEP		N/A	DEPTH TO	ROCK 8.5 ft	
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)		0.5ft		0 25	BLOWS P			SAMI NO.	17	0 G	ELEV. (ft)	SOIL AND ROC	K DESCRIPTION	DEPTH (fi
	590 _ 588.6 -	- 0.0							_				_ 588.6		SURFACE	0.1
	587.1	1.5	3	3	3	6			: : :		D		[(A-4)	у
585	584.6	- - 4.0				 			+				- 584.6	uliana Chiff Cana	Fine Sandy Clay (A-6	4.0
	582.6	- 6.0	4	5	6	11 .	::::		: : :	:	4		581.6	vium. Suii Gray	rifle Salidy Clay (A-C	7.t
580	580.1	- 8.5	5	5	20	{ : :-:•	25		: : :		М		581.1 A Allu		Dense Gray Silty Fine) /\
	-	-	60/0.1			<u> </u>			60/0	.1			1	duum: Very Stiff	(A-2-4) Gray Fine Sandy Cla	. // 1
		-									1			(A-6) With Ro	ock Fragments ock Moderately Hard	11
575	-	- - -											- Pene	Gray / Boring Terminat tration Test Refu	Argillite ed with Standard usal at Elevation 580.	
570	1	-											F "		e Rock: Hard Gray illite.	
	7	•											F 1) Advanced 2-1/4	4" HSA to 8.5 feet.	
	1	•											[
565	1	-											F			
		-											F			
560	1	-											-			
300	1	-											F			
	1	•											F			
555		-											F			
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DOT.GDT

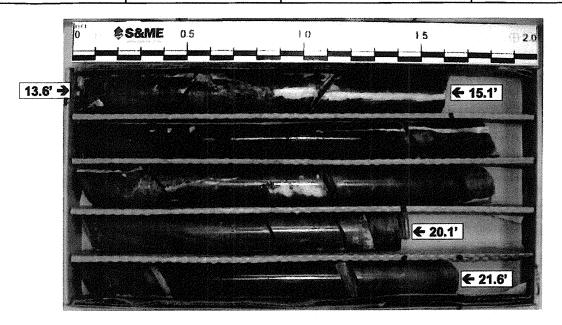
170.GPJ NC

COUNTY Union **GEOLOGIST** K.Plummer ID. B-4651 PROJECT NO. 33817.1.1 GROUND WTR (ft) SITE DESCRIPTION Bridge No. 251 on S.R. 1508 over South Fork Crooked Creek ALIGNMENT -L-0 HR. N/M STATION 17+98 OFFSET 15ft RT BORING NO. B2-B 24 HR. N/M TOTAL DEPTH 25.1 ft **NORTHING 488,241 EASTING** 1,527,921 COLLAR ELEV. 591.9 ft DRILL METHOD 3-1/4" HSA w/Rotary Wash,NW2 Core Barrel **HAMMER TYPE** Automatic DRILL MACHINE Diedrich D-50 SURFACE WATER DEPTH N/A DEPTH TO ROCK 13.6 ft **START DATE 06/12/08 COMP. DATE 06/12/08** SAMP DRIVE ELEV **BLOWS PER FOOT BLOW COUNT** DEPTH ELEV SOIL AND ROCK DESCRIPTION 0 100 (ft) (ft) 0.5ft | 0.5ft | 0.5ft MOI DEPTH (ft) (ft) G ELEV. (ft) **GROUND SURFACE** 0.0 591.9 591.9 0.0 Roadway Embankment Fill: Soft to Medium 2 М 590.4 1.5 Stiff Red-Brown Clayey SILT (A-4) With Rock Fragments 590 3 4 2 М 588.9 3.0 2 2 585.9 585.9 6.0 Alluvium: Medium Stiff Gray-Tan Silty CLAY (A-7-6) With Trace of Organic Matter 585 2 3 583.4 Alluvium: Loose Gray Clayey Fine SAND Sat. 3 3 (A-2-6)580 13.6 578.3 + 13.6 578.3 ₹60/0.0° Non-Crystalline Rock Hard Very Slightly 60/0.0 Weathered Gray Argillite With Close Fracture Spacing 575 570 566.8 Boring Terminated at Elevation 566.8 ft in Non-Crystalline Rock: Hard Very Slightly Weathered Gray Argillite. 565 Advanced 3-1/4" HSA to 13.6 feet.
 Advanced Casing to 13.6 feet.
 Approximate Drilling Fluid Density 62.4 560 pcf.
4) Advanced NQ2 Core Barrel from 13.6 to 25.1 feet. 5) Creek Water Used As Drilling Fluid. 555 6) No Loss of Drilling Fluid Observed. 550 545 540 535 530 525 520

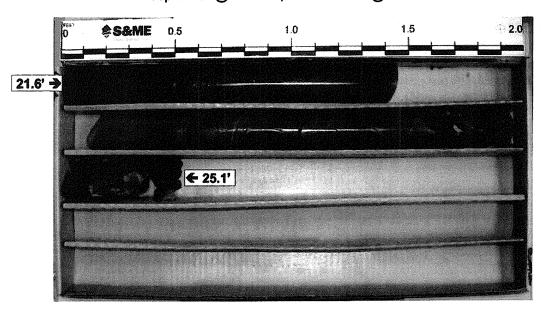
PROJECT NO. 33817.1.1 ID. B-4651 **COUNTY** Union GEOLOGIST K.Plummer GROUND WTR (ft) SITE DESCRIPTION Bridge No. 251 on S.R. 1508 over South Fork Crooked Creek ALIGNMENT BORING NO. B2-B STATION 17+98 OFFSET 15ft RT 0 HR. N/M COLLAR ELEV. 591.9 ft TOTAL DEPTH 25.1 ft **NORTHING 488,241 EASTING** 1,527,921 24 HR. N/M DRILL METHOD 3-1/4" HSA w/Rotary Wash,NW2 Core Barrel **HAMMER TYPE** Automatic DRILL MACHINE Diedrich D-50 COMP. DATE 06/12/08 SURFACE WATER DEPTH N/A DEPTH TO ROCK 13.6 ft **START DATE 06/12/08** CORE SIZE NQ2 TOTAL RUN 11.5 ft DRILLER J.Millwood DRILL RUN RQD (ft) REC. **ELEV** DEPTH RUN SAMP RATE DESCRIPTION AND REMARKS **ELEV** (ft) (ft) NO. (Min/ft) Ġ (ft) ELEV. (ft) Begin Coring @ 13.6 ft
Non-Crystalline Rock: Hard Very Slightly Weathered Gray Argillite With 578.3 1.5 1:30 N=60/0 1:30/0 (1.5) 100% (8.1)Close Fracture Spacing and Vertical Quartz Vein-1 Vertical Fracture from 13.6 to 14.6 feet, 12 Fractures at 35°, 1 Vertical Fracture from 16.9 to 17.2 feet, 2 Fractures at 15°, 1 Fracture at 45° 5.0 575 (4.0) 80% (4.8) 571.8 20.1 5.0 (3.6) 72% 2:00 2:00 2:00 570 100% 2:00 566.8 25.1 25.1 566.8 Boring Terminated at Elevation 566.8 ft in Non-Crystalline Rock: Hard Very 565 Slightly Weathered Gray Argillite. Advanced 3-1/4" HSA to 13.6 feet.
 Advanced Casing to 13.6 feet.
 Approximate Drilling Fluid Density 62.4 pcf. 560 4) Advanced NQ2 Core Barrel from 13.6 to 25.1 feet. Creek Water Used As Drilling Fluid.
 No Loss of Drilling Fluid Observed. 555 550 545 540 535 530 525 520 6 515 알 510 505 500

CORE PHOTOS

Project No.: 33817.1.1	ID No.: B-4651	Location: Union Co., NC	Boring No.: B2-B
Site Description: Bridge No	o. 251 on SR1508 over Sout	h Fork Crooked Creek	Driller: J. Millwood
Collar Elev.: 591.9 ft.	Core Size: NQ2	Equipment: Diedrich D-50	Geologist: K. Plummer
Elev. at T.D.: 566.8 ft.	Total Depth: 25.1 ft.	Total Run: 11.5 ft.	Date: 6/12/2008



Box 1 of 2
Top of Box @ 13.6 feet; Bottom of Box @ 21.6 feet



Box 2 of 2
Top of Box @ 21.6 feet; Bottom of Box @ 25.1 feet

NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

PRO.	JECT NO	D. 338	17.1.1		ID.	B-4651	COUNTY Union		GEOLOGIST K.F	Plummer
SITE	DESCR	IPTION	Brid	ge No.	251	on S.R. 1508 over South For	k Crooked Creek			GROUND WTR
BOR	NG NO.	EB2-	A		s	STATION 18+43	OFFSET 38ft LT	ALIGNN	NENT -L-	OHR. N
COLI	AR ELE	EV. 58	7.3 ft		T	TOTAL DEPTH 7.1 ft	NORTHING 488,274	EASTIN	G 1,527,859	24 HR. 4
DRIL	L MACH	INE B	K-51		D	ORILL METHOD 2-1/4" HSA			HAMMER TYPE	Manual
STAF	RT DATE	05/1	9/08		С	COMP. DATE 05/19/08	SURFACE WATER DEPTI	H N/A	DEPTH TO ROC	K 7.0 ft
(ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLC 0.5ft	0.5ft	0.5ft	BLOWS PER FO	75 100 NO. MOI	C ELEV. (ft)	SOIL AND ROCK DES	CRIPTION DEPTI
585	587.3 - 585.8 - 583.8 - 581.3 - 580.3 -		3 3 8 60/0.2	4 3 7	5 4 11		M M M Sat	584.3	GROUND SURF Alluvium: Medium Stiff to Clayey Sitt (A-4) With Tra Matter Miluvium: Medium Dense G Sand (A-2-6) Weathered Rock: (Gra Non-Crystalline Rock:	Stiff Gray-Tan ce of Organic Gray Clayey Fine ay Argillitte) Gray Argillite
575								5802 F	Boring Terminated with Penetration Test Refusal at t in Non-Crystalline Rock: N Gray Argillite	Elevation 580.2 Moderately Hard
	-	E						E	1) Advanced 2-1/4" HSA	to 7.0 feet.
570		E						<u> </u>		
	-								•	
65	-	<u> </u>						 -		
	-	<u> </u>								
560	-	‡						F		
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GEOLOGIST K.Plummer **COUNTY** Union PROJECT NO. 33817.1.1 ID. B-4651 **GROUND WTR (ft)** SITE DESCRIPTION Bridge No. 251 on S.R. 1508 over South Fork Crooked Creek 0 HR. 10.5' CI OFFSET 25ft RT ALIGNMENT -L-BORING NO. EB2-B STATION 18+43 8.2 **EASTING** 1,527,920 24 HR. **NORTHING 488,287** COLLAR ELEV. 591.9 ft TOTAL DEPTH 10.5 ft HAMMER TYPE Manual DRILL METHOD 2-1/4" HSA **DRILL MACHINE BK-51** SURFACE WATER DEPTH N/A DEPTH TO ROCK N/A COMP. DATE 05/21/08 **START DATE 05/21/08** SAMP. **BLOWS PER FOOT** DRIVE **BLOW COUNT** DEPTH SOIL AND ROCK DESCRIPTION o ELEV 100 50 NO. DEPTH (ft) (ft) MOI 0.5ft 0.5ft 0.5ft ELEV. (ft) G 595 **GROUND SURFACE** 591.9 591.9 Roadway Embankment Fill: Stiff to Very D 14 9 11 590.4 Stiff Tan Fine Sandy Silt (A-4) With Rock 590 589.4 M 6 8 Fragments 588.4 3.5 Fill: Stiff Red-Brown Silty CLAY (A-7-5) 588.9 М 5 3 Alluvium: Medium Stiff Tan Clayey Silt (A-4) 585.9 585.9 6.0 Alluvium: Medium Stiff Gray Silty Clay 3 2 (A-7-6)583.4 8.5 Alluvium: Medium Dense Gray Clayey Fine 10.0 Sat. 15 14 581.9 **Q29** Sand (A-2-7) 10.5 581.4 D 60/0.0¹ Weathered Rock: (Gray Argillite) 60/0.0 581.4 580 Boring Terminated with Standard Penetration Test Refusal at Elevation 581.4 ft on Non-Crystalline Rock: Moderately Hard Gray Argillite. 575 1) Advanced 2-1/4" HSA to 10.5 feet. 570 565 560 555 550 545 540 535 DOT.GDT 530 170.GPJ NC 525 520

GEOLOGIST K.Plummer ID. B-4651 **COUNTY** Union PROJECT NO. 33817.1.1 **GROUND WTR (ft)** SITE DESCRIPTION Bridge No. 251 on S.R. 1508 over South Fork Crooked Creek OFFSET 30ft RT ALIGNMENT -L-OHR. 5.2' CI STATION 15+86 BORING NO. RW-1 **EASTING** 1,527,970 24 HR. Dry **NORTHING** 488,033 COLLAR ELEV. 600.1 ft TOTAL DEPTH 8.5 ft HAMMER TYPE Manual DRILL METHOD 2-1/4" HSA DRILL MACHINE BK-51 DEPTH TO ROCK N/A COMP. DATE 05/21/08 SURFACE WATER DEPTH N/A **START DATE 05/21/08** SAMP. DRIVE **BLOW COUNT BLOWS PER FOOT** DEPTH SOIL AND ROCK DESCRIPTION **ELEV** 100 50 NO. (ft) (ft) 0.5ft 0.5ft 0.5ft MOI DEPTH (ft) G ELEV. (ft) 605 **GROUND SURFACE** 0.0 600.1 600 600.1 Artificial Fill: Stiff to Hard Tan Fine Sandy M 6 598.6 Silt (A-4) With Rock Fragments М 50 15 19 596.6 596.6 D Weathered Rock: 80 20/0.1 100/0.6 595 (Gray Argillite) 594.1 6.0 D 60/0.2 60/0.2 591 6 591.6 -8.5 D Boring Terminated with Standard Penetration Test Refusal at Elevation 591.6 ft on Non-Crystalline Rock: Hard Gray 60/0.0 590 Araillite. 1) Advanced 2-1/4" HSA to 8.5 feet. 585 580 575 570 565 560 555 550 545 DOT.GDT 170.GPJ NC 535 530

NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

PROJ	ECT NO). 338				B-4	RE /						CO	UNT	Υl	Jnion					G	EOLOGIST K.	Plummer	
	DESCRI							508	over	Sou	ıth F	ork	Croc	ked	Cre	ek							GROUND W	/TR (ft
	NG NO.						ON			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						Oft RT				ALIGNMEN	Т	-L-	0 HR.	N/A
	AR ELE				TO	OTAI	L DE	PTH	7.7	7 ft			NO	RTH	IING	488,0	083			EASTING	1,	527,953	24 HR.	4.5
	MACH				D	RILL	MET	THOI	D 2	-1/4'	" HS	Α	·		.,						T	HAMMER TYPE	Manual	
	T DATE				C	OMP	. DA	TE	05/1	9/08	3		SU	RFA	CE	NATER	DEF	TH	N	/A		DEPTH TO RO	CK N/A	
LEV (ft)	DD11 (F.	DEPTH (ft)		0.5ft	JNT 0.5ft	0		25 	BLO		ER F	тоо	75		100	SAMP NO.	17) L O)	ELEV. (ft)	so	OIL AND ROCK DE		DEPTH (
595	600 _ 596.0 _	0.0	2	5	6			-							•		M	- IXI		596.0 Red-B	A	GROUND SUR Artificial Fill: Stiff to wn-Tan Clayey SIL	Very Stiff T (A-4) With Rock	<u> </u>
	592.5 -	- 3.5	5	13	7]]:		20									M	, X	¥			Fragments	}	
590	590.0	6.0	1	1	25	:	: :		26						-		M	$\overline{\aleph}$	▓₺	590.0				6
"	588.4 -	- 7. 6	40	60/0.4	1	11.		_					$oxed{\int}$	_100	11 0 3		D		3	588.3		leathered Rock: (T		7
85	-		60/0.1											- 60	0/0.1 -] _			Pene	etra	oring Terminated wi ation Test Refusal a lon-Crystalline Roc	t Elevation 588.3	
	<u>-</u>																		ŧ	1)) 🗚	Advanced 2-1/4" HS	A to 7.6 feet.	
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75	-	•						•											-	_				
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SUMMARY OF LABORATORY TEST DATA



. Soil Classification and Gradation

S&ME Project #:	1051-08-170			Test Date:	6/25/2008
State Project No.:	33817.1.1	County:	Union	Report Date:	6/17 - 6/25/08
Federal ID No.:	N/A	TIP No.:	B-4651		
Project Name:	Bridge No. 251 on SR 1508	508 Over South Fork	rk ·		
Client Name:	Ko & Associates				
Client Address:	Raleigh, North Carolina				

		Sample	Sample AASHTO	OTI		Fotal %	Total % Passing		T	Total Mortar Fraction	ar Fractio	ā				Moisture
Boring	Sample	Depth	Boring Sample Depth Classification	cation		Sieve #	e #		Coarse	Fine			TT	PL	PI	Content
No.	No. No. (Feet)	(Feet)			10	40	40 60	200	Sand Sand	Sand	Silt	Clay				%
Bank	S-1	0 - 1	Bank S-1 0-1 A-7-6 (4)	4)	71	59	55	48	23	14	31	32	41	27	14	14 38.1
Bed	S-2	0 - 1	Bed S-2 0-1 A-7-5 (1)	1)	83	56	47	37	44	13	28	15	45	32	13	35.7

Signature

Geotechnical Engineer

Technical Responsibility:

Notes:

B. Riggs



FIELD SCOUR REPORT

WBS:	33817.1.1	TIP:	B-4651	COUNTY: Union	
			EXISTING	BRIDGE .	
Information from:	Field In Other	nspection (explain)	x Mice Provided Hydrau	rofilm (reel lic Report	_ pos:)
Bridge No.: Foundation Type:	251 Length Shallow founda	: 60.5' tions	Total Bents:	Bents in Channel:	1 Bents in Floodplain: 2
EVIDENCE OF S Abutments or E	SCOUR(2) End Bent Slopes	: Evidenc	e of some erosion	n at End Bents No. 1 and	No.2.
Interior Bents:	None observed				
Channel Bed:	Some scour bei	neath brid	ge (north side of	channel)	
Channel Bank:	North bank ups	tream of b	ridge and south b	oank downstream of bridg	ge
EXISTING SCO			ving walls at both	and honts	
Type(3):	I imper abuntme	ents and v	ving walls at both	end bents.	
Extent(4):	Wing walls exte	nd past th	ne abutments.	and the state of t	
Effectiveness(5):	Scour protection	n appears	to be working		A
Obstructions(6):	Tree debris alor	ng both si	des of bridge		

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- 7 Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- B Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

			DES	SIGN IN	NFORM	ATION					
Channel Bed Material(7): Tan Brown Fine to Coarse Sandy Silty CLAY (A-7-5)											
<u> </u>	S I. II # . #	(0): T D		4- 0		CIL. CI	AV /A 7	e)			
Channel Bank Material(8): Tan Brown Fine to Coarse Sandy Silty CLAY (A-7-6)											
Channe	l Bank Cover	(9): Low grow	ving veg	getation,	grassy a	reas and	i large to	small di	ameter h	ardwood	trees.
Flood	dplain Width(1	0): <u>50 feet so</u>	outh of	bridge to	approxim	mately 4	00 feet to	the nort	h of the l	oridge.	
Flood	lplain Cover(1	1): Residenti	al area	s to the	south and	d woode	d and ag	ricultural	areas to	the nort	h
	Stream is(1	2): Agg	grading		Degr	ading_	<u>x</u>	Sta	atic	-	
Channel Migratio	n Tendency(1	3): Channel	Migratio	on appea	ars to be	to the No	orth.				
Observations	and Other Co	mments: Hea	avy tree	debris ı	upstream	&downs	tream of	bridge, s	and bar i	n creek	east of
		bridge (de	ownstre	eam), ne	wiy cons	tructed p	ump sta	lion souti			
		Reported by	/:		Abner F.	Riggs J	r		Date:	5/19/2	2008
				,	ADITO 1.	rtiggs, o					
DESIGN SCOUR ELEVATIONS(14) Feet X Meters											
	BEN	TS									
	B1	B2	B3	B4		· · · · · · · · · · · · · · · · · · ·				,	,
	Left 581					ļ	ļ				· ·
	Right 579	.9 578.3				ļ		<u> </u>		ļ	<u> </u>
							<u> </u>	 			
					 	ļ	ļ			<u> </u>	
					<u> </u>	<u> </u>	ļ	<u> </u>			
Comparison o	f DSE to Hydr	aulice Unit th	eoretic:	al scour	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	1
Companson o	i DSE io nyui	auncs Orni ur	eoi euc	ai scoui.							

	DSE 4	etermined by	<i>,</i> .			•			Date:		
	DSE Q	eternimed by	•						Duto.	-	
SOIL ANALY	SIS RESULTS	FROM CHA	NNEL	BED AN	ID BANK	MATE	RIAL				
Bed or Bank		Bed									
Sample No.	S-1	S-2			<u> </u>					 	
Retained #4	22.3	3.3			 					<u> </u>	—
Passed #10	71 59	83 56									
Passed #40 Passed #200	48	37									
Coarse Sand	23	44			 	<u> </u>					
Fine Sand	14	13			 						
Silt	31	28									
Clay	32	15			1						
LL	41	45	_		 						
PI	14	13	 -		1				•		
AASHTO	A-7-6(4)	A-7-5(1)	1								
Station	17+30	17+67									
Offset		25 FT RT									
Depth	0-1	0-1			1			1			

Particle Size Analysis of Soils



AASHTO T 88 as Modified by NCDOT

S&ME Project #: Report Date: 6/25/2008 1051-08-170 Test Date(s): 06/18 - 06/23/2008 Project Name: Crooked Creek Bridge Ko & Associates, Inc. Client Name: Client Address: TIP NO: B-4651 F.A. Project No: NA State Project #: 33817.1.1 Sample Date: 5/19/08 Sample #: S-1 Boring #: Channel Bank Depth: 0 - 1.0' STA 17+30 Offset: 25 FT RT. Location: Tan Brown Fine to Coarse Sandy Silty CLAY A-7-6 (4) Sample Description: #60 #100 #200‡270 #10 #20 #40 "1.5 "1"3/4 "1/2"3/8 #4 100% 90% 80% 70% Percent Passin 60% 50% 40% 30% 20% 10% 0% 0.1 0.01 0.001 Particle Size (mm) < 0.25 mm and > 0.05 mmFine Sand As Defined by NCDOT < 0.05 and > 0.005 mm $< 75 \, \text{mm} \, \text{and} > 2.00 \, \text{mm}$ Silt Gravel < 0.005 mm Clay < 2.00 mm and > 0.25 mmCoarse Sand 22.0% Coarse Sand 15.9% Silt 1.0" Maximum Particle Size Fine Sand Clay · 23.0% 10.1% 29.2% Gravel % Passing #200 48.0% Moisture Content 38.1% Apparent Relative Density 14 Plastic Index 27 Plastic Limit Liquid Limit 41 Soil Mortar (-#10 Sieve) 32.0% Silt 31.3% Clay Fine Sand 14.2% Coarse Sand 22.5% Weathered & Friable Hard & Durable Soft Description of Sand & Gravel Particles: Rounded [Angular 🗆 Sodium Hexametaphosphate: 40 g./ Liter Dispersing Agent: Mechanical Stirring Apparatus (A) Length of Dispersion Period:

AASHTO M 145: The Classification of Soils and Soil Aggregate Mixtures for Highway Construction Purposes

AASHTO T89: Determining the Liquid Limit of Soils

AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test

AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT

Mal Krajan

ASTM D 854: Specific Gravity of Soils Laboratory Supervisor

AASHTO T265: Laboratory Determination of Moisture Content of Soils

AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils

Signature

References:

Particle Size Analysis of Soils



AASHTO T 88 as Modified by NCDOT

S&ME Project #:

1051-08-170

Crooked Creek Bridge

Report Date:

6/25/2008

Project Name: Client Name:

Ko & Associates, Inc.

Test Date(s):

06/18 - 06/23/2008

Client Address:

State Project #:

33817.1.1

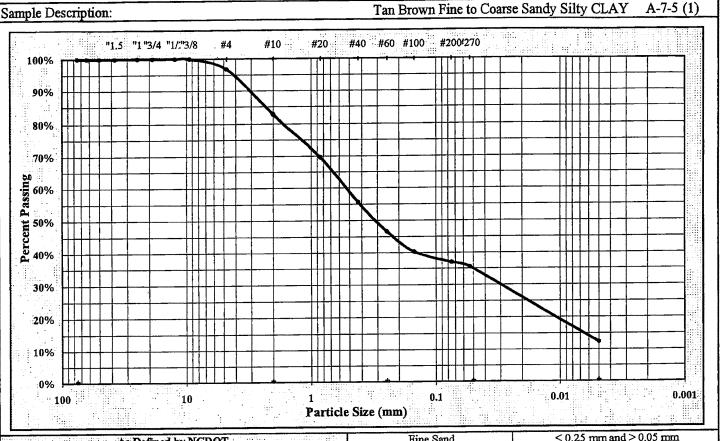
F.A. Project No: NA

TIP NO:

B-4651

Sample Date: 5/19/08 Sample #: S-2 Channel Bed Boring #: Depth: 0 - 1.0' Offset: 25 FT RT.

Location: STA 17+67



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Gravel		nm and > 2.00 mm		Silt	< 0.05 and > 0.005 mm		
Coarse Sand	< 2.00	mm and > 0.25 mm		Clay	< 0.005 mm		
Maximum I	Particle Size	3/8"	Coarse Sand	36.2%	Silt	23.0%	
	Gravel	17.3%	Fine Sand	11.0%	Clay	12.0%	

36.9% % Passing #200 Moisture Content 35.7% Apparent Relative Density 32 Plastic Index 13 **Plastic Limit** Liquid Limit 45

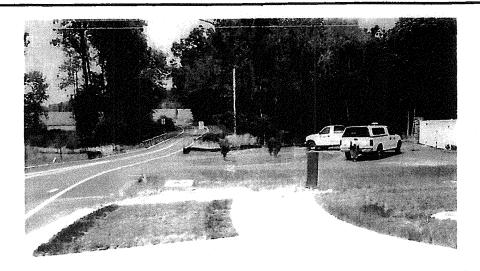
Soil Morton (#10 Sieve)

Son Mortar (-#10 Sieve)									
Coarse Sand	43.8%	Fine Sand 13.3	%	Silt	28.0%	Clay	14.9%		
Description of Sand	& Gravel Particle	es: Rounded 🗆	Angular 🗆	Hard & Durable	□ Soft □	Weathered	& Friable 🛚		
Mechanical Stirring App		Length of Dispersion Period:	1 min.	Dispersing Agent:	Sodium Hexameta	ohosphate:	40 g./ Liter		
References: AASHTO T88: Particle Size Analysis of Soils as Modified by the NCDOT									
AASHTO T87: Dry Preparation of Disturbed Soil and Soil Aggregate Samples for Test AASHTO T265: Laboratory Determination of Moisture Content of									
AASHTO T89: Determining the Liquid Limit of Soils AASHTO T90: Determining the Plastic Limit & Plasticity Index of Soils									
							avity of Soils		

Technical Responsibility:

Mal Krajan

Laboratory Supervisor Signature Signature



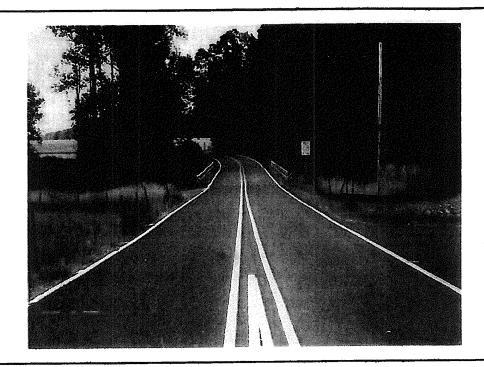
Photograph No. 1:

This photograph was taken from the right side of the -L- alignment, looking north, along the proposed Retaining Wall.

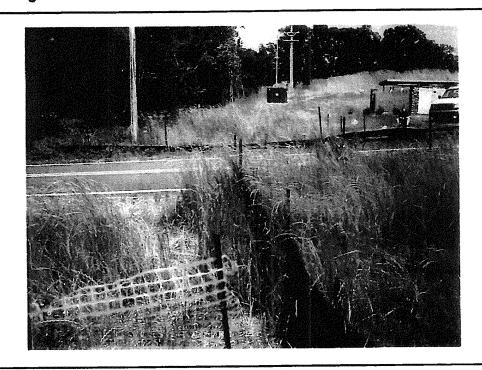


Photograph No. 2:

This photograph was taken from the right side of the -L- alignment, looking south, along the proposed Retaining Wall.



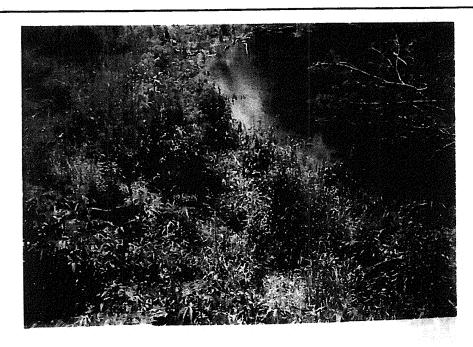
Photograph No. 3: This photograph was taken from the south approach along the centerline of the -L-alignment, looking north.



Photograph No. 4: This photograph was taken from left side of the -L- alignment, looking east, across proposed End Bent No. 1.



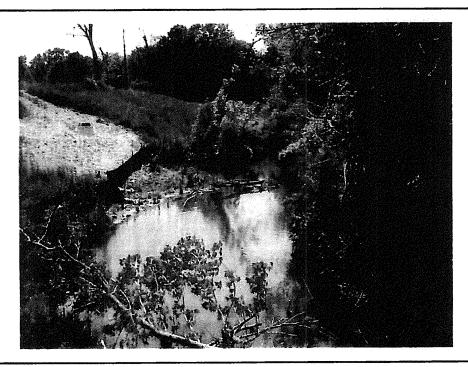
Photograph No. 5: This photograph was taken from right side of the -L- alignment, looking west, across proposed End Bent No. 1.



Photograph No. 6: This photograph was taken from the centerline of the -L- alignment, looking west, across proposed Interior Bent No. 1.



Photograph No. 7: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed Interior Bent No. 1.



Photograph No. 8: This photograph was taken from the existing bridge, looking west (upstream).



Photograph No. 9: This photograph was taken from the existing bridge, looking east (downstream).



Photograph No. 10:

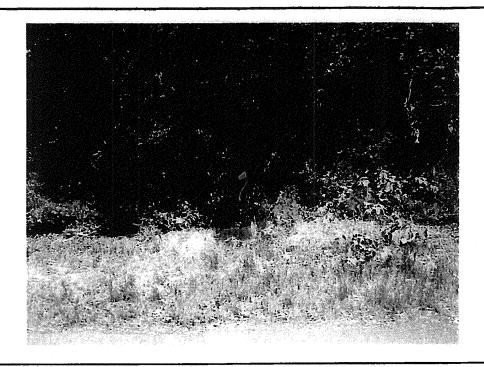
This photograph was taken from the centerline of the -L- alignment, looking west, across proposed Interior Bent No. 2.



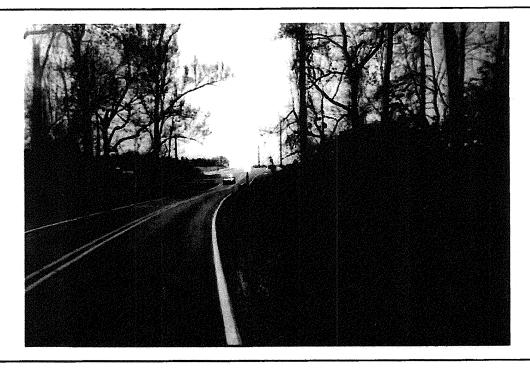
Photograph No. 11: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed Interior Bent No. 2.



Photograph No. 12: This photograph was taken from the centerline of the -L- alignment, looking west, across proposed End Bent No. 2.



Photograph No. 13: This photograph was taken from the centerline of the -L- alignment, looking east, across proposed End Bent No. 2.



Photograph No. 14: This photograph was taken from the north approach along the centerline of the -L-alignment, looking south.