

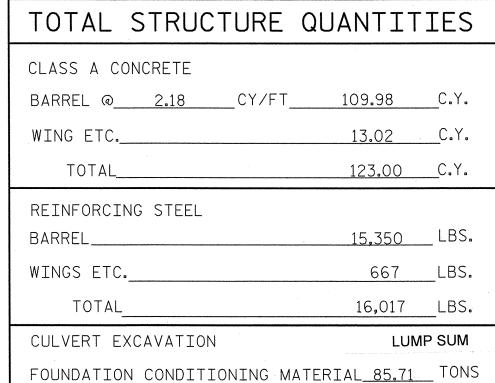
Ç -L-

PROFILE ALONG & CULVERT

15'-0"

15'-0"

15'-0"



HYDRAULIC DATA

DESIGN DISCHARGE	= 350 CFS
FREQUENCY OF DESIGN FLOOD	= 25 YR.
DESIGN HIGH WATER ELEVATION	= 202.9 FT.
DRAINAGE AREA	= 0.16 SQ.MI.
BASE DISCHARGE (Q100)	= 480 CFS
BASE HIGH WATER ELEVATION	= 203.25 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE:	=	600	CFS
FREQUENCY OF OVERTOPPING FLOOD	=	200	YR.
OVERTOPPING FLOOD ELEVATION	=	203.8	3 FT

NOTES:

ASSUMED LIVE LOAD -----HL-93 OR ALTERNATE LOADING.

DESIGN FILL-----3.0 FT.

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

- 1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITTED CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION, HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

SCOUR PROTECTION IS REQUIRED AT BOTH INLET AND OUTLET ENDS OF THE CULVERT. DO NOT PLACE RIP RAP ABOVE THE STREAM BED.

THE SCOUR CRITICAL ELEVATION IS THE AS-BUILT BOTTOM OF CULVERT FLOOR SLAB ELEVATION. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

> PROJECT NO. R5526 HARNETT 58+98 -L-STATION:___ SHEET 2 OF 8



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

_ COUNTY

SHEET NO

TOTAL SHEETS

DOUBLE BARREL 10 FT. X 5 FT. CONCRETE BOX CULVERT

67° SKEW REVISIONS

DATE:

NO. BY:

1616 E. MILLBROOK ROAD, SUITE #310

RALEIGH, NORTH CAROLINA 27609
(919) 876–6888 NCBEES #F-0326

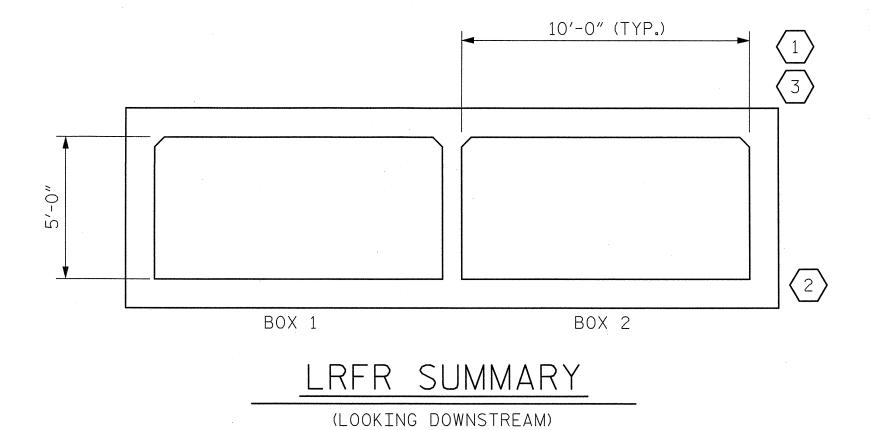
- NATURAL GROUND

DRAWN BY: ____C.R. DESROCHERS DATE: DEC. 2013 CHECKED BY : M. PAYNE

15'-0"

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

							STRENGTH I LIMIT STATE									
						MOMENT			SHEAR							
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y _{ll})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.63	****	1.75	1.63	2	TOP SLAB	21.67	1.89	2	TOP SLAB	21.67	•
DESIGN LOAD		HL-93 (OPERATING)	N/A		2.12		1.35	2.12	2	TOP SLAB	21.67	2.46	2	TOP SLAB	21.67	•
RATING		HS-20 (INVENTORY)	36.000	2	1.69	60.84	1.75	1.69	2	BOTTOM SLAB	21.67	2.09	2	TOP SLAB	21.67	•
		HS-20 (OPERATING)	36.000		2.19	78.84	1.35	2.19	2	BOTTOM SLAB	21.67	2.71	2	TOP SLAB	21.67	•
	SINGLE VEHICLE (SV)	SNSH	13.500		3.37	45.50	1.40	3.37	1	TOP SLAB	10.75	3.86	2	TOP SLAB	21.67	•
		SNGARBS2	20.000		3.12	62.40	1.40	3.12	2	TOP SLAB	21.67	3.58	2	TOP SLAB	21.67	•
		SNAGRIS2	22.000		3.07	67.54	1.40	3.07	2	BOTTOM SLAB	21.67	3.79	2	TOP SLAB	21.67	•
		SNCOTTS3	27.250		1.97	53.68	1.40	1.97	2	TOP SLAB	21.67	2.36	2	TOP SLAB	21.67	•
		SNAGGRS4	34.925		1.96	68.45	1.40	1.96	2	BOTTOM SLAB	21.67	2.89	2	TOP SLAB	21.67	•
		SNS5A	35.550		2.08	73.94	1.40	2.08	2	BOTTOM SLAB	21.67	2.69	. 2	TOP SLAB	21.67	•
		SNS6A	39.950		1.92	76.70	1.40	1.92	2	TOP SLAB	21.67	2.63	2	TOP SLAB	21.67	•
LEGAL		SNS7B	42.000		1.92	80.64	1.40	1.92	2	TOP SLAB	21.67	2.63	2	TOP SLAB	21.67	•
LOAD RATING	TOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000		2.23	73.59	1.40	2.23	2	BOTT. SLAB	21.67	3.41	2	TOP SLAB	21.67	•
		TNT4A	33.075		2.03	67.14	1.40	2.03	2	TOP SLAB	21.67	2.72	2	TOP SLAB	21.67	•
		TNT6A	41.600		1.98	82.37	1.40	1.98	. 2	TOP SLAB	21.67	2.63	2	TOP SLAB	21.67	•
		TNT7A	42.000		1.97	82.74	1.40	1.97	2	TOP SLAB	21.67	2.66	2	TOP SLAB	21.67	•
		TNT7B	42.000	·	1.97	82.74	1.40	1.97	2	TOP SLAB	21.67	2.67	2	TOP SLAB	21.67	•
	TRAC	TNAGRIT4	43.000		1.78	76.54	1.40	1.78	2	BOTTOM SLAB	21.67	2.61	2	TOP SLAB	21.67	•
	1	TNAGT5A	45.000		1.78	80.10	1.40	1.78	2	TOP SLAB	21.67	2.58	2	TOP SLAB	21.67	•
	TRUCK	TNAGT5B	45.000	3	1.67	75.15	1.40	1.67	2	TOP SLAB	21.67	2.52	2	TOP SLAB	21.67	•



LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

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RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

** SEE CHART FOR VEHICLE TYPE

3 LEGAL LOAD RATING **

PROJECT NO. R5526 HARNETT _ COUNTY STATION: 58+98 -L-

SHEET 3 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

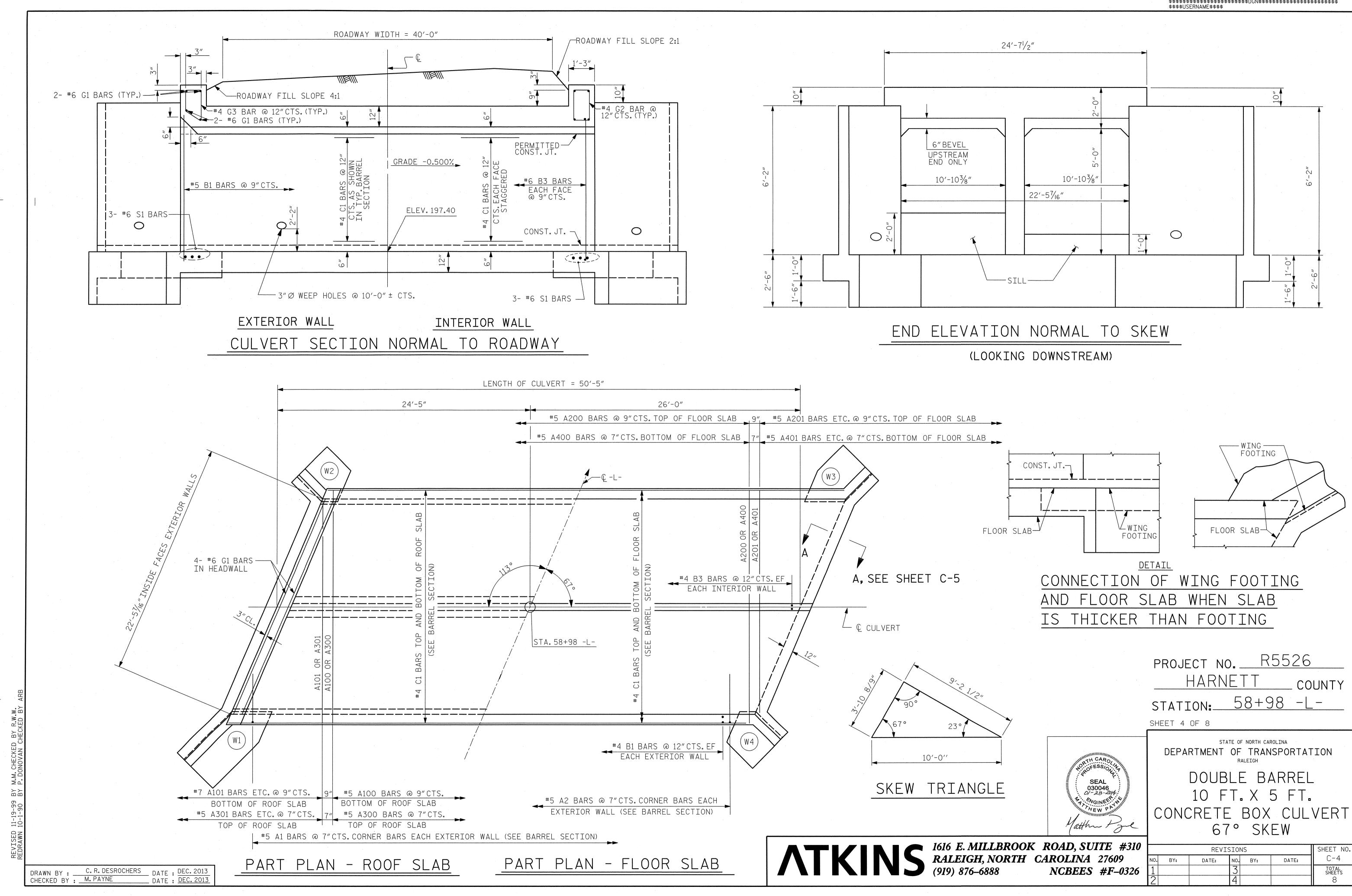
STANDARD

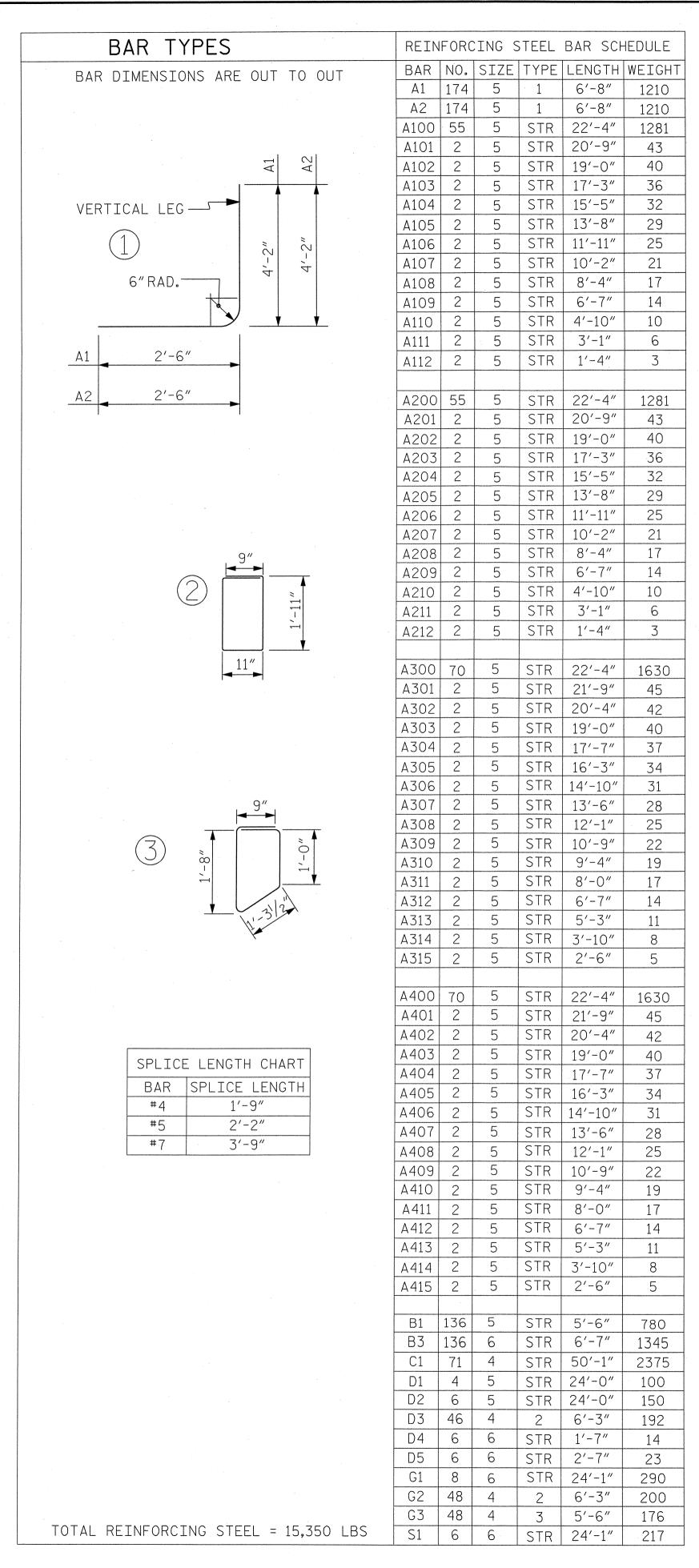
REVISIONS SHEET NO. C-3 NO. BY: DATE: TOTAL SHEETS

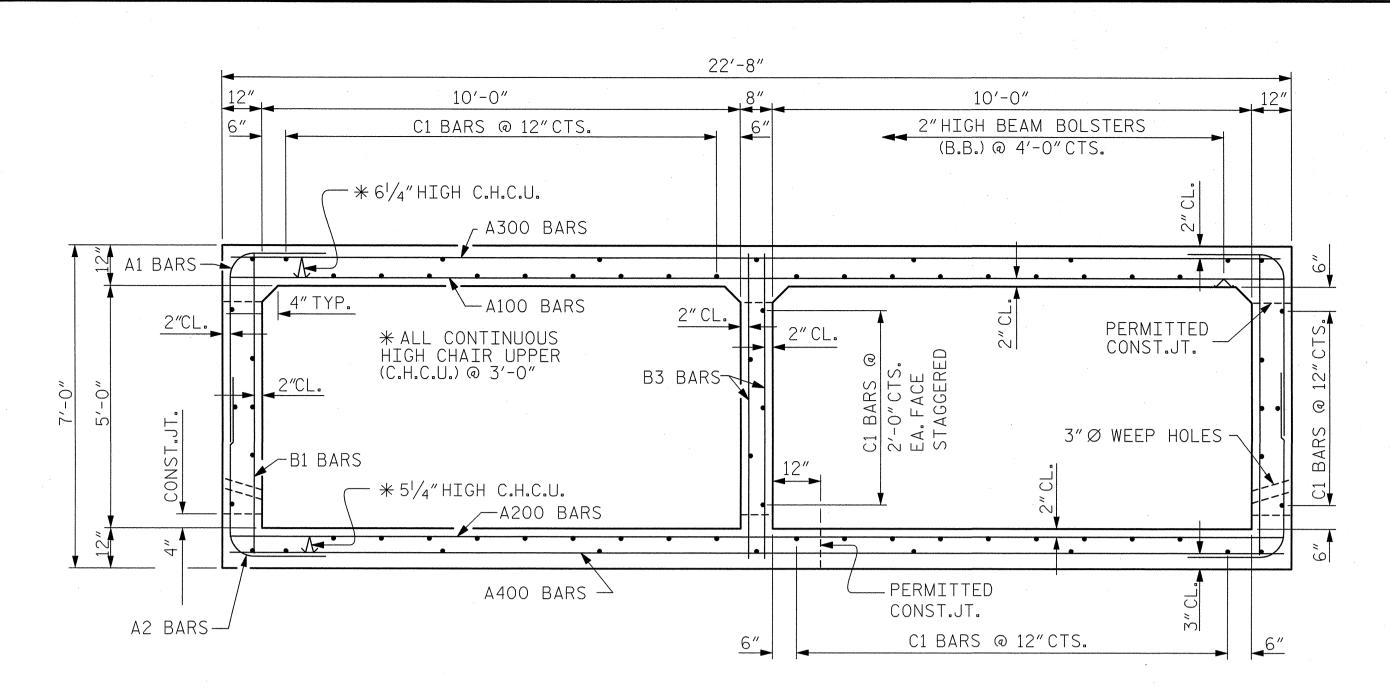
1616 E. MILLBROOK ROAD, SUITE #310

RALEIGH, NORTH CAROLINA 27609
(919) 876–6888 NCBEES #F-0326

DRAWN BY: C.R.DESROCHERS DATE: OCT. 2013
CHECKED BY: J. WERES DATE: OCT. 2013

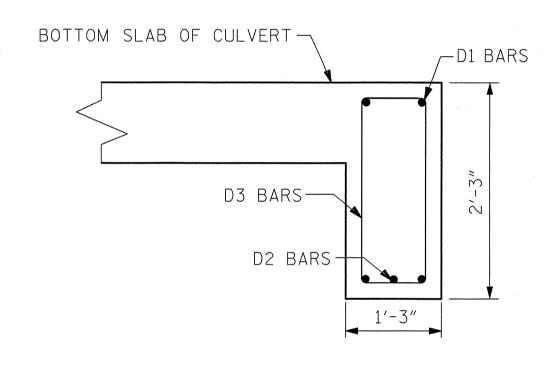






RIGHT ANGLE SECTION OF BARREL

THERE ARE 71 "C"BARS IN SECTION OF BARREL.



ELEVATION - LOOKING UPSTREAM

LOW SILL -

3- #6 D4 @ 4′CTS.

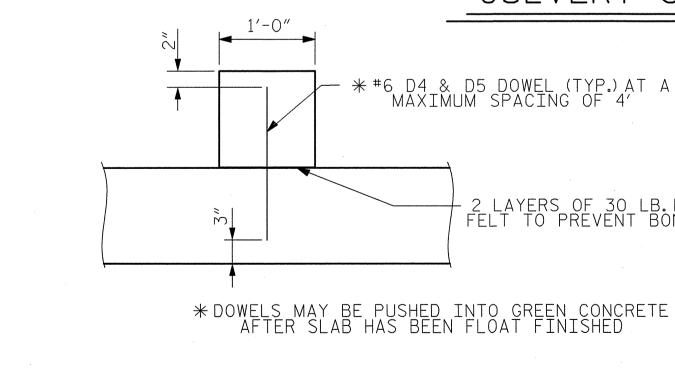
CURTAIN WALL SECTION A-A

(LEFT CURTAIN WALL SIMILAR, OPPOSITE HAND) (FOR LOCATION OF SECTION A-A, SEE SHEET C-4)

2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND (TYP.)

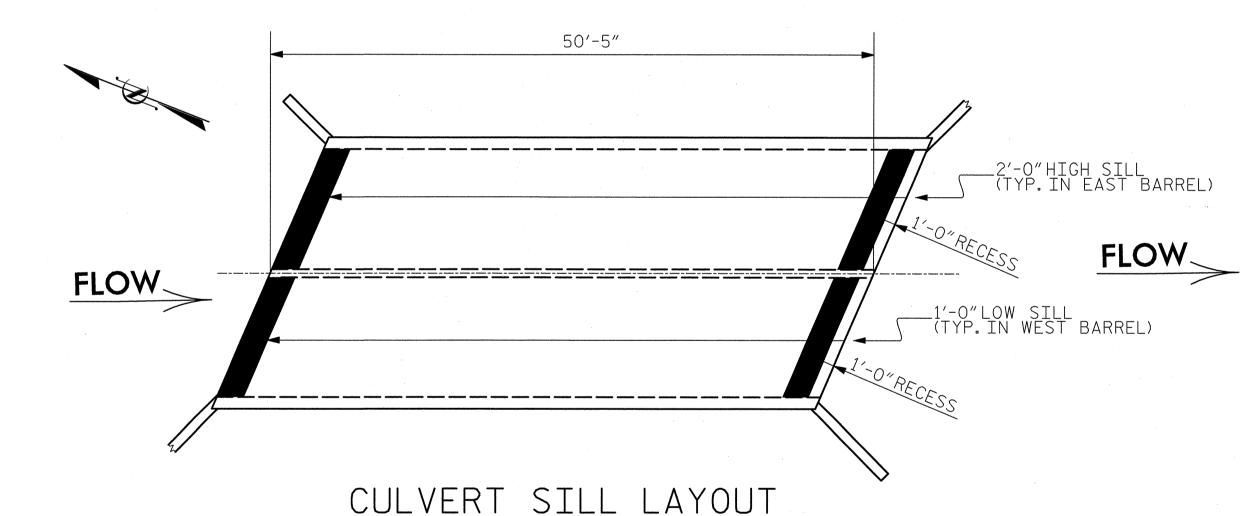
HIGH SILL

3- #6 D5 @ 4′CTS.



SECTION THROUGH SILL

CULVERT SILL DETAILS



PROJECT NO. R5526 HARNETT 58+98 -L-STATION:__ SHEET 5 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

_ COUNTY

DOUBLE BARREL 10 FT. X 5 FT. CONCRETE BOX CULVERT 67° SKEW

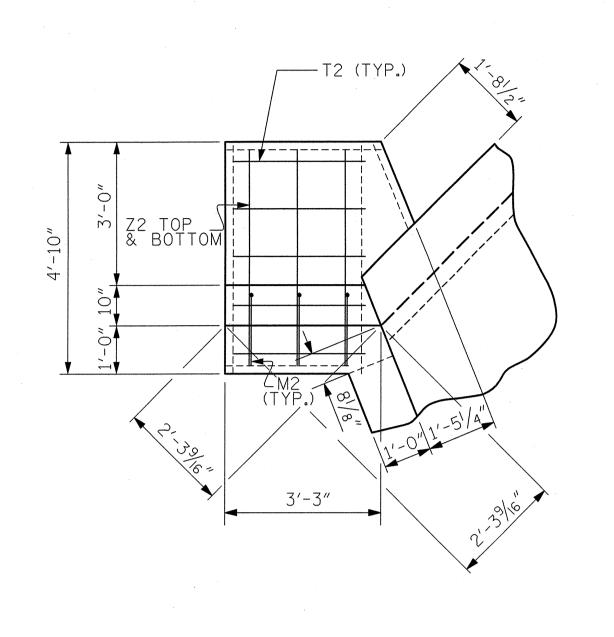
REVISIONS SHEET NO. C-5 NO. BY: DATE: DATE: TOTAL SHEETS

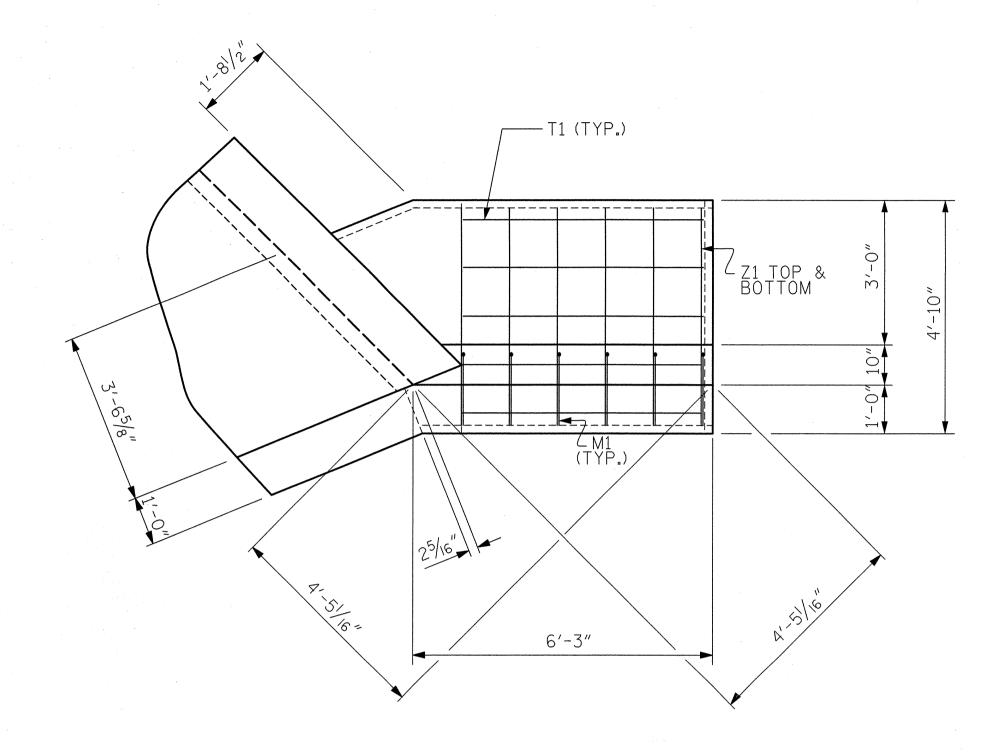
1616 E. MILLBROOK ROAD, SUITE #310 RALEIGH, NORTH CAROLINA 27609 (919) 876–6888 NCBEES #F-0326

SEAL 030046 01/28/2014 2 NGINEE A

- 2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND (TYP.)

DRAWN BY: C.R. DESROCHERS DATE: DEC. 2013
CHECKED BY: M. PAYNE DATE: DEC. 2013



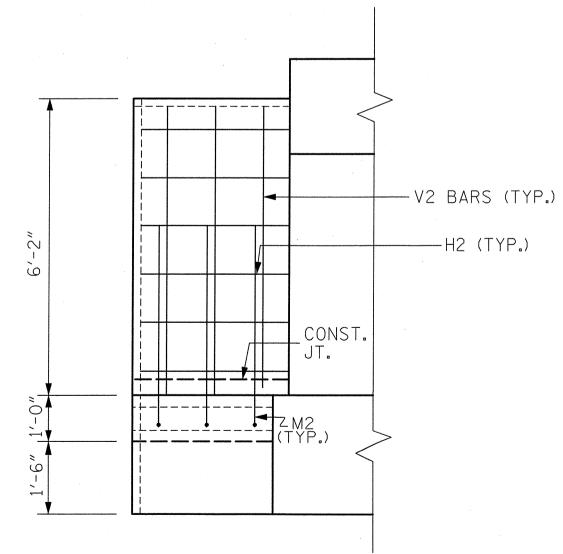


BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT ALL BAR DIMENSIONS ARE OUT TO OUT. 12 4 STR 5'-10" 47 12 4 STR 3'-1" 25 6 4 1 3'-7" 3′-7″ 6'-4" 57 6'-4" 3 | 6. | 29 10 | 4 | STR | 6'-0" VERTICAL LEG 10 4 STR 3'-0" T2 20 12 4 STR 6'-0" 48 V2 6'-0" 24 4 STR **Z**1 12 | 5 | STR | 4'-6" 56 "M" BAR 9" RAD. Z2 6 5 STR 4'-6" 28 1'-7" "M" BARS REINFORCING STEEL 395 LBS 1'-7" "N" BARS FOR 2 WINGS CLASS A CONCRETE 2 WINGS 4.04 CY 1 HEADWALLS 1.14 CY 1 END CURTAIN WALLS 1.83 CY TOTAL 7.0 CY

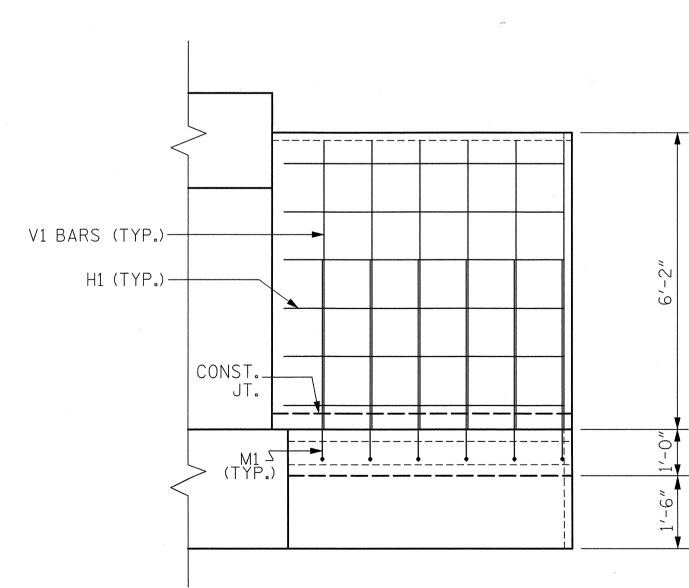
BILL OF MATERIAL

BAR TYPES

PLAN W2



ELEVATION W2



PROJECT NO. R5526 HARNETT COUNTY STATION: <u>58+98 -L-</u>

SHEET 6 OF 8

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

SLOPE = 2:1 " 67° SKEW

SHEET NO.

C-6

TOTAL SHEETS

1616 E. MILLBROOK ROAD, SUITE #310
RALEIGH, NORTH CAROLINA 27609
(919) 876–6888 NCBEES #F-0326

REVISIONS DATE: NO. BY:

ELEVATION W1

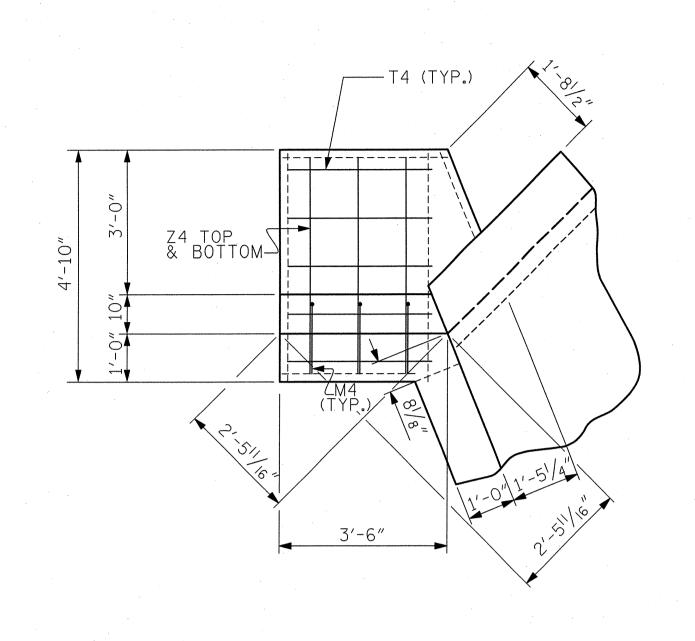
DRAWN BY: C.R.DESROCHERS DATE: OCT. 2013 CHECKED BY: J. WERES DATE: OCT. 2013

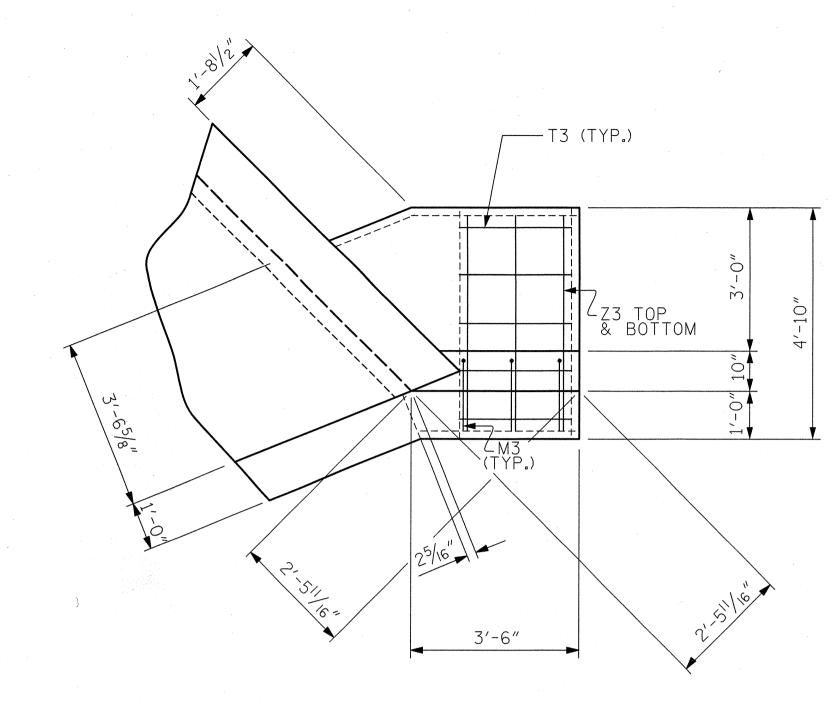
PLAN W1

V1/V2 BARS CONST -N1/N2 BARS

TYPICAL WING SECTION

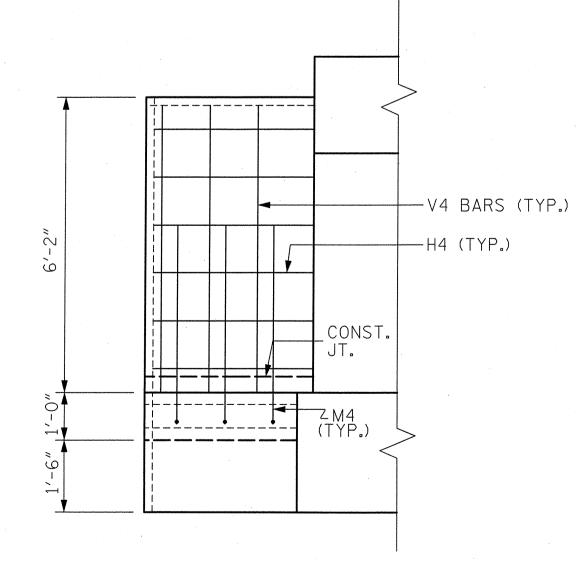
1'-0"

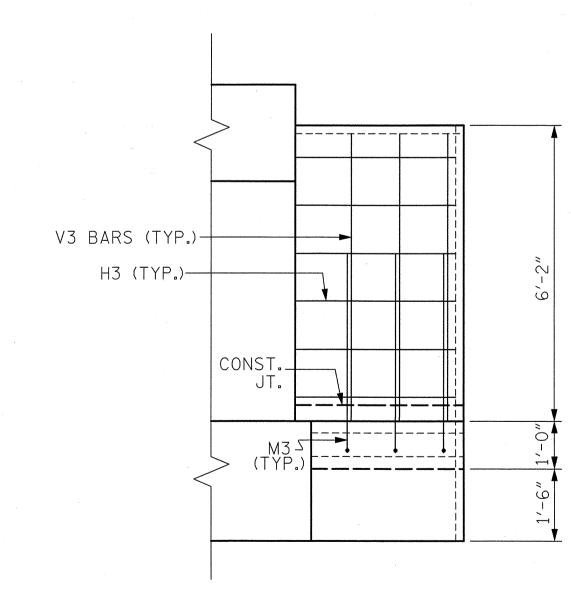




PLAN W4

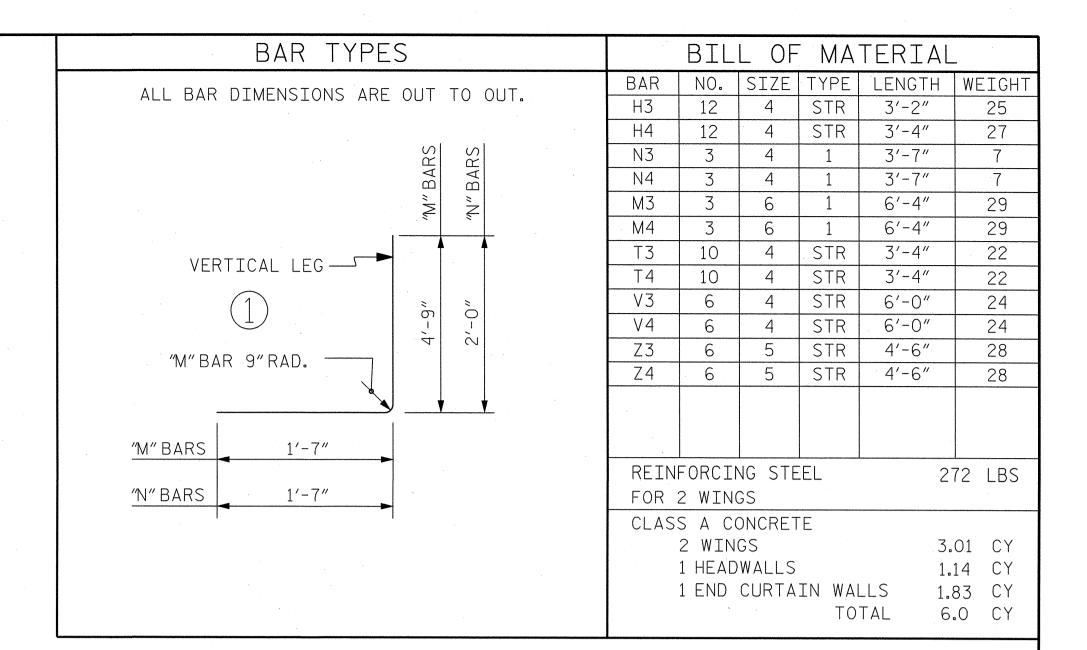
PLAN W3

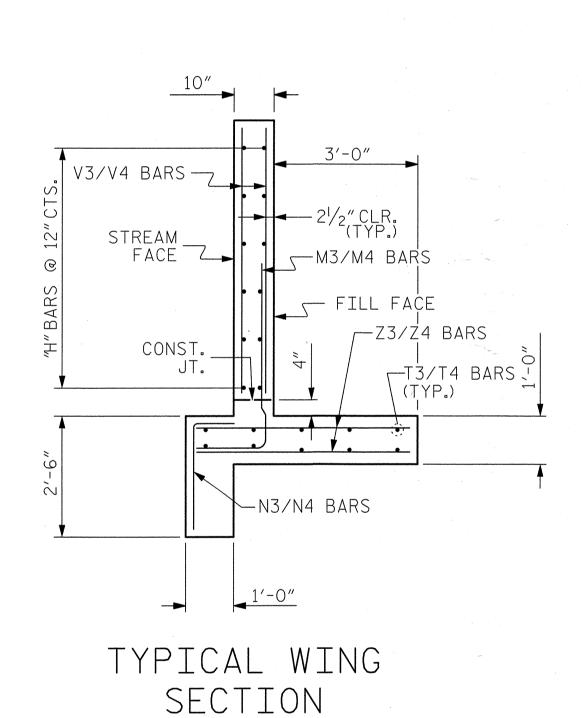




ELEVATION W4

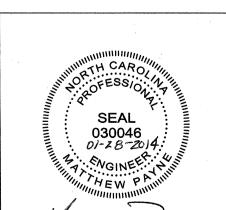
ELEVATION W3





PROJECT NO. R5526 HARNETT COUNTY STATION: <u>58+98 -L-</u>

SHEET 7 OF 8



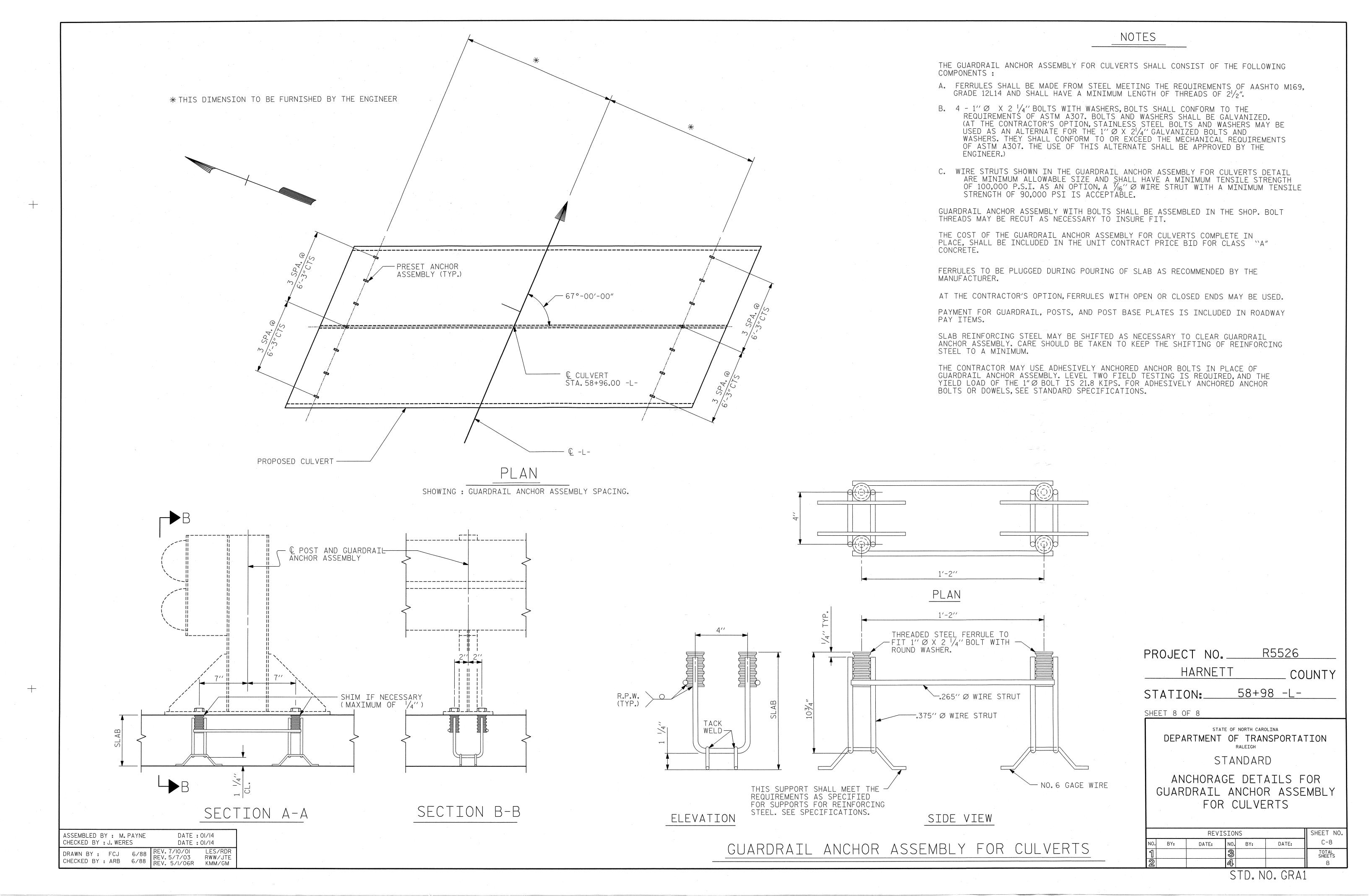
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

H = 5'-0" 67° SKEW

REVISIONS C-7 DATE: NO. BY:

1616 E. MILLBROOK ROAD, SUITE #310
RALEIGH, NORTH CAROLINA 27609
(919) 876–6888 NCBEES #F-0326

DRAWN BY: <u>C.R. DESROCHERS</u> DATE: DEC. 2013 CHECKED BY: <u>M. PAYNE</u> DATE: DEC. 2013



STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS ---- SEE PLANS LIVE LOAD IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF - 20,000 LBS. PER SQ. IN. STRUCTURAL STEEL - AASHTO M270 GRADE 36 - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR 1,800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ----375 LBS. PER SQ. IN.

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST

BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH