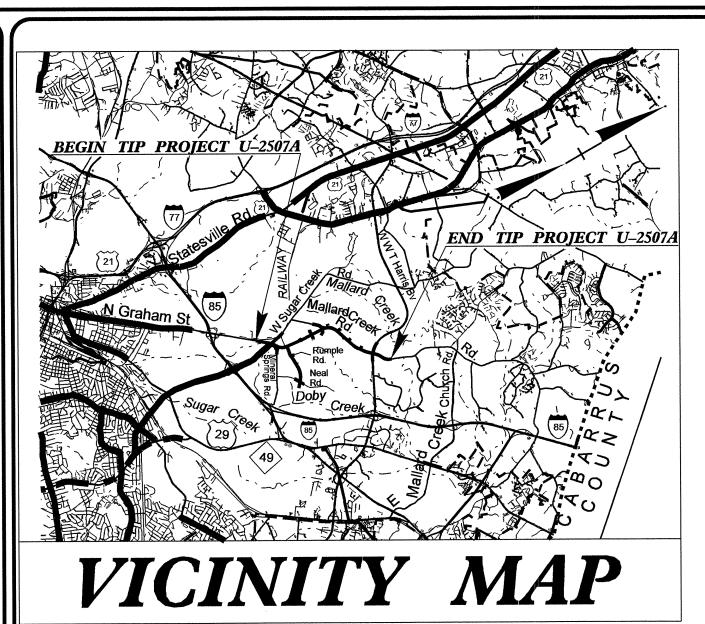
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STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

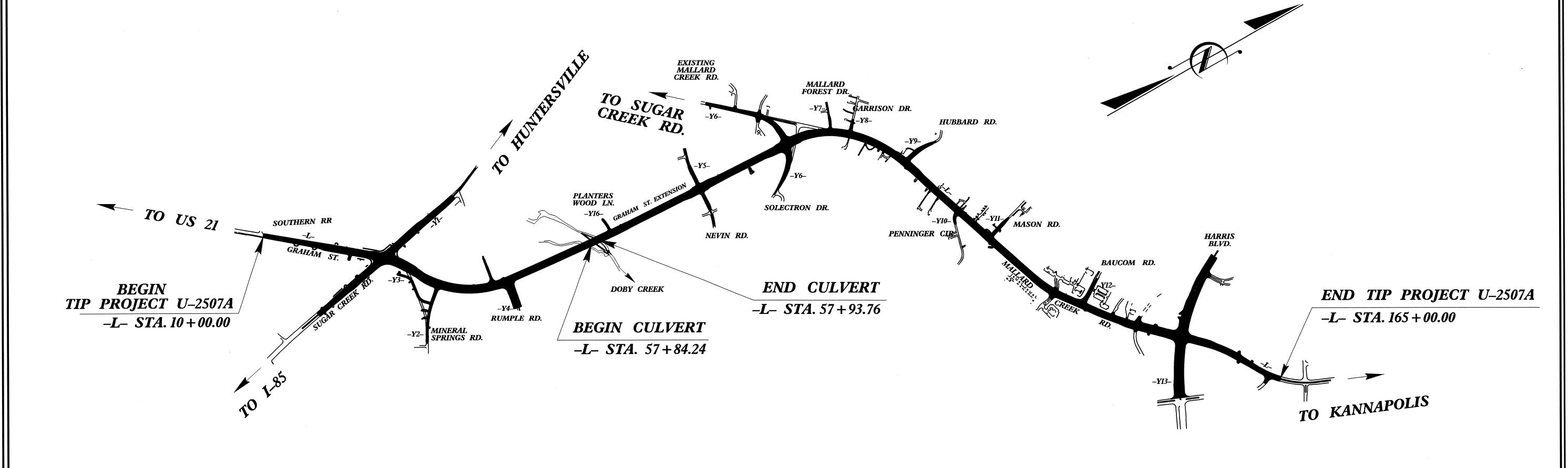
MECKLENBURG COUNTY

LOCATION: CHARLOTTE - SR 2467 (MALLARD CREEK ROAD)

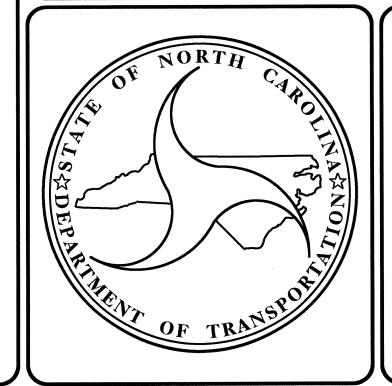
FROM SR 2480 (SUGAR CREEK ROAD) TO

SR 2665 (HARRIS BOULEVARD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, CULVERT AND SIGNALS



CULVERT



DESIGN DATA

ADT 2012 = 34,150 ADT 2030 = 45,600 DHV = 10 % D = 55 % T = 7 % * V = 50 MPH

* TTST 2 % DUAL 5 %

PROJECT LENGTH

LENGTH OF ROADWAY TIP PROJECT U-2507A = 2.94 MILES

Prepared in the Office of: DIVISION OF HIGHWAYS 2012 STANDARD SPECIFICATIONS B.C. HUNT, PE

LETTING DATE:

PROJECT ENGINEER

NOVEMBER 19, 2013

V.A. PATEL, PE

PROJECT DESIGN ENGINEER

STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR.

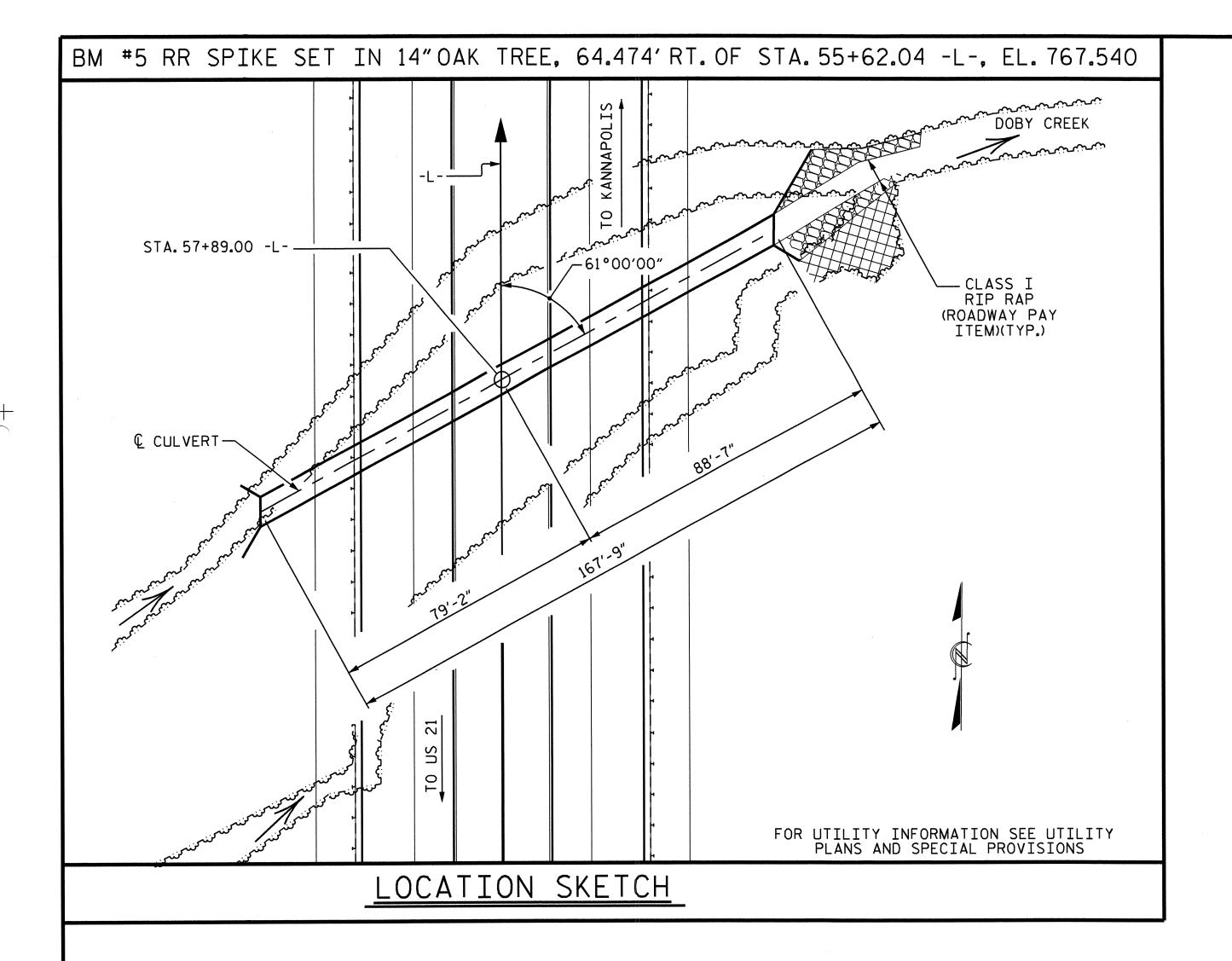
RALEIGH, N.C. 27610

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE DESIGN ENGINEER

DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED
DIVISION ADMINISTRATOR DA



ROADWAY DATA

= 770.63 GRADE POINT ELEV. @ STA. 57+89.00 -L-= 752.80 BED ELEV. @ STA. 57+89.00 -L-

= 2:1 ROADWAY SLOPES @ STA. 57+89.00 -L-

HYDRAULIC DATA

= 330 C.F.S. DESIGN DISCHARGE = 50 YEARS FREQUENCY OF DESIGN FLOOD = 761.50 DESIGN HIGH WATER ELEVATION = 0.16 SQ. MI. DRAINAGE AREA = 370 C.F.S. BASE DISCHARGE (Q100) = 762.10 BASE HIGH WATER ELEVATION

OVERTOPPING FLOOD DATA

= 900 C.F.S. OVERTOPPING DISCHARGE = 500+ YEARS FREQUENCY OF OVERTOPPING FLOOD = 768.20 OVERTOPPING FLOOD ELEVATION

TOTAL STRUCTURE QUANTITIES CLASS A CONCRETE BARREL @ 0.811 C.Y. / FT. = 136.0 C.Y. = 26.0 C.Y. WINGS ETC. TOTAL = 162.0 C.Y. REINFORCING STEEL BARREL = 15,363 LBS. WINGS ETC. = 1,661 LBS. TOTAL = 17,024 LBS. LUMP SUM CULVERT EXCAVATION, STA. 57+89.00 -L-= 146 TONS FOUNDATION COND. MATERIAL, BOX CULVERT

ASSUMED LIVE LOAD ------HL-93 OR ALTERNATE LOADING.

DESIGN FILL----- 9.98 FT

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEET.

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.

2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

TRANSVERSE CONSTRUCTION JOINTS SHALL BE USED IN THE BARREL, SPACED TO LIMIT THE POURS TO A MAXIMUM OF 70 FT. LOCATION OF JOINTS SHALL BE SUBJECT TO APPROVAL OF THE ENGINEER.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WALL ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

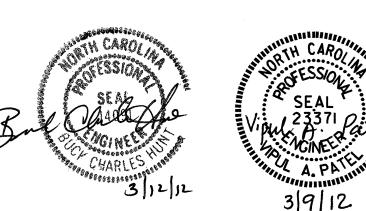
THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL

35'-0" 50'-0" 50'-0" 45'-0"

PROFILE ALONG & CULVERT



PROJECT NO. U-2507A MECKLENBURG COUNTY STATION: 57+89.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

CONCRETE BOX CULVERT 61° SKEW

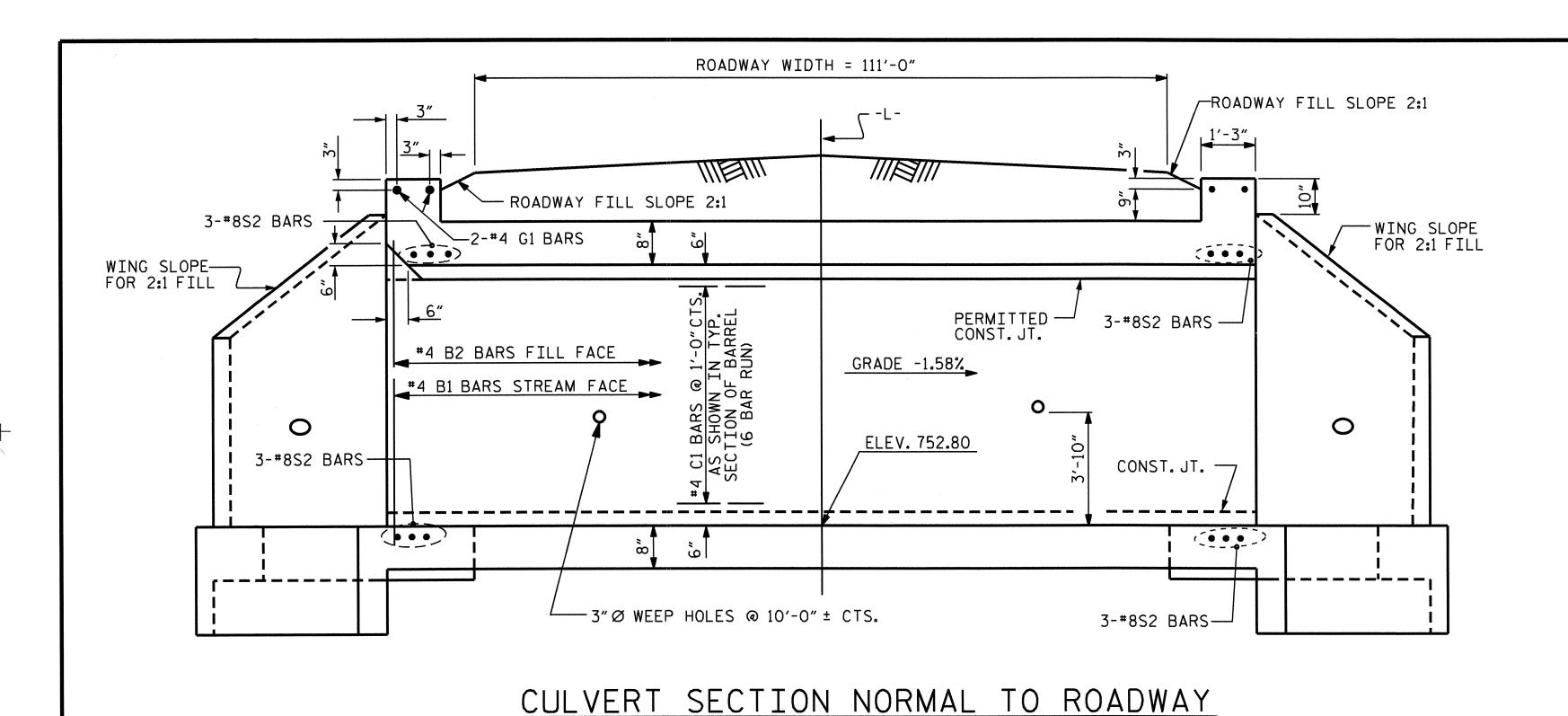
SHEET NO. REVISIONS C-1 DATE: BY: TOTAL SHEETS

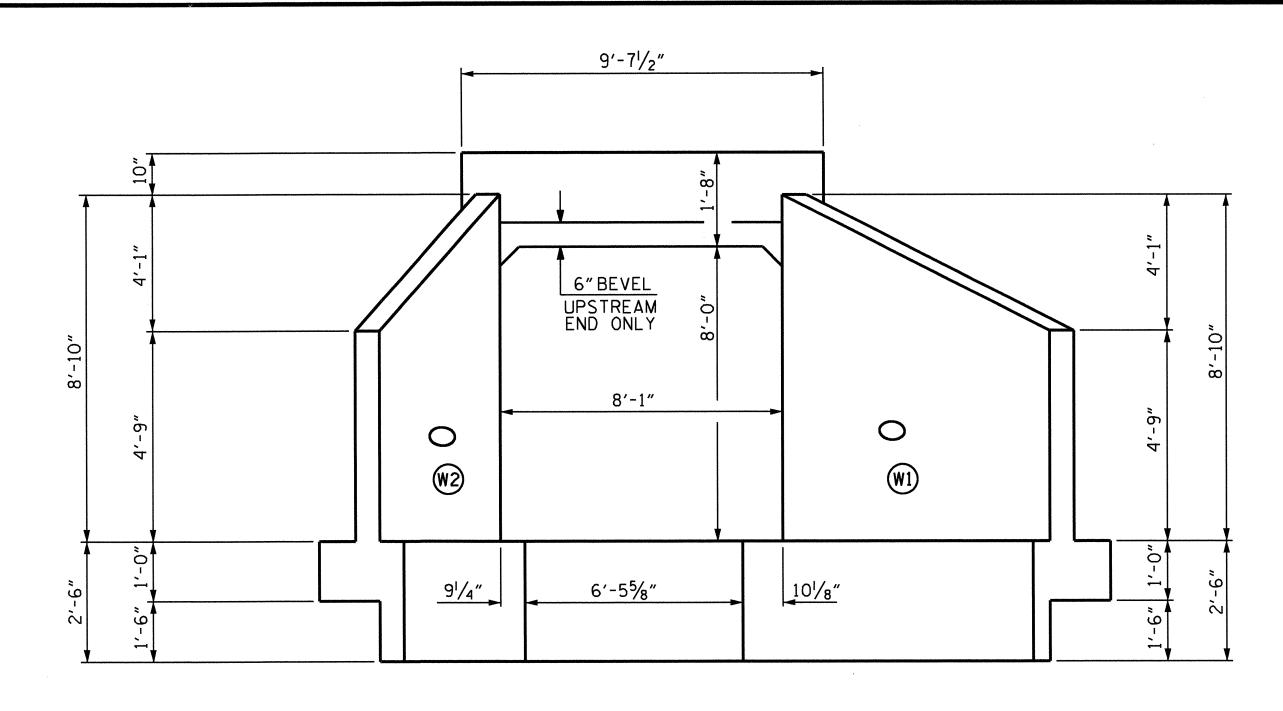
__ DATE : 2/21/12 __ DATE : 2/23/12

H.T. DIEU

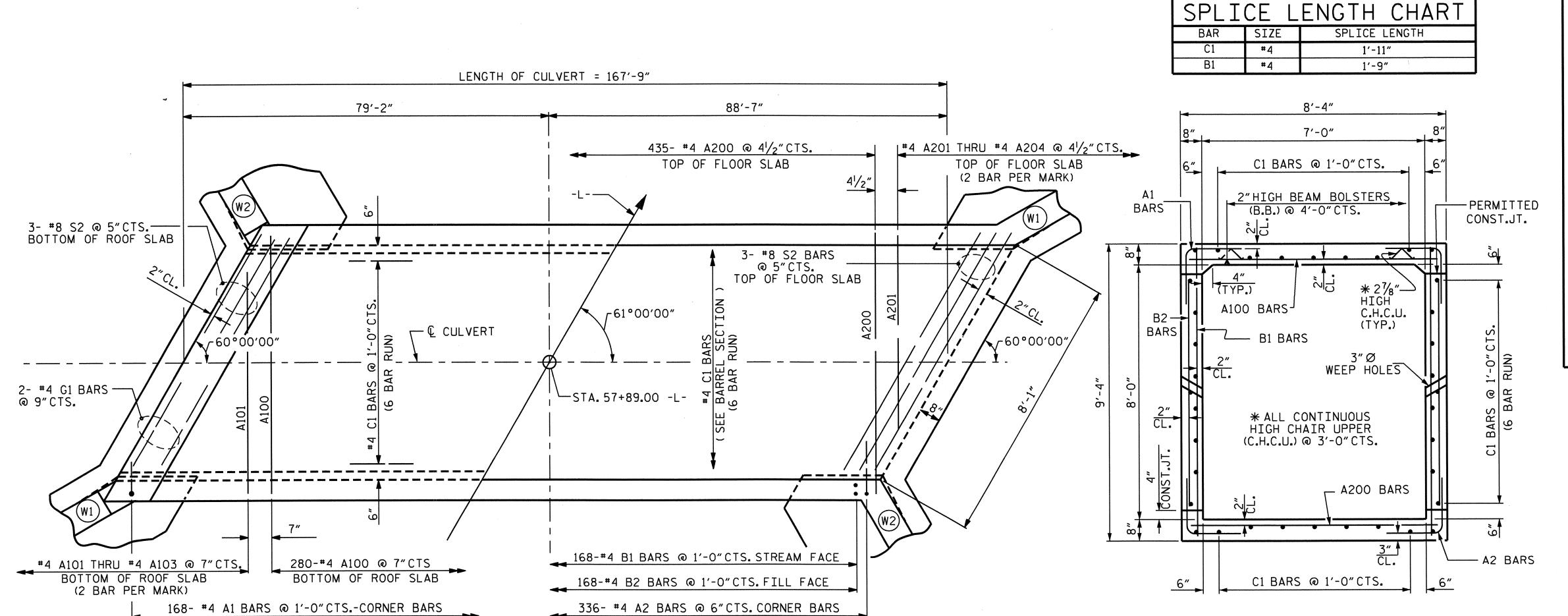
CHECKED BY : J.G. KHARVA

08-MAR-2012 08:32 R:\Structures\PLANS\U-2507A_SD_CU.dgn





END ELEVATION NORMAL TO SKEW



(SEE BARREL SECTION)

PART PLAN - FLOOR SLAB

TYPE BAR STR 5′-9″ A102 A103 STR 1'-10" 7'-11" | 2300 A201 6'-7" A202 5′-3" 6" R. ¬ A203 STR 4'-0" STR 2'-8" #4 A204 4 A1.A2 2'-51/2" 336 #4 4'-11" 672 #4 6 4'-10" 2170 336 #4 STR 7'-4" 216 #4 STR 29'-8" #4 STR 9'-2" #8 STR 9'-2" BAR DIMENSIONS ARE OUT TO OUT TOTAL REINFORCING STEEL 15363 LBS

PROJECT NO. U-2507A

MECKLENBURG COUNTY

STATION: 57+89.00 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SINGLE 7 FT. X 8 FT. CONCRETE BOX CULVERT 61° SKEW

REVISIONS

NO. BY: DATE: NO. BY: DATE: C-2

1 3 TOTAL SHEETS
4 4

RIGHT ANGLE SECTION OF BARREL

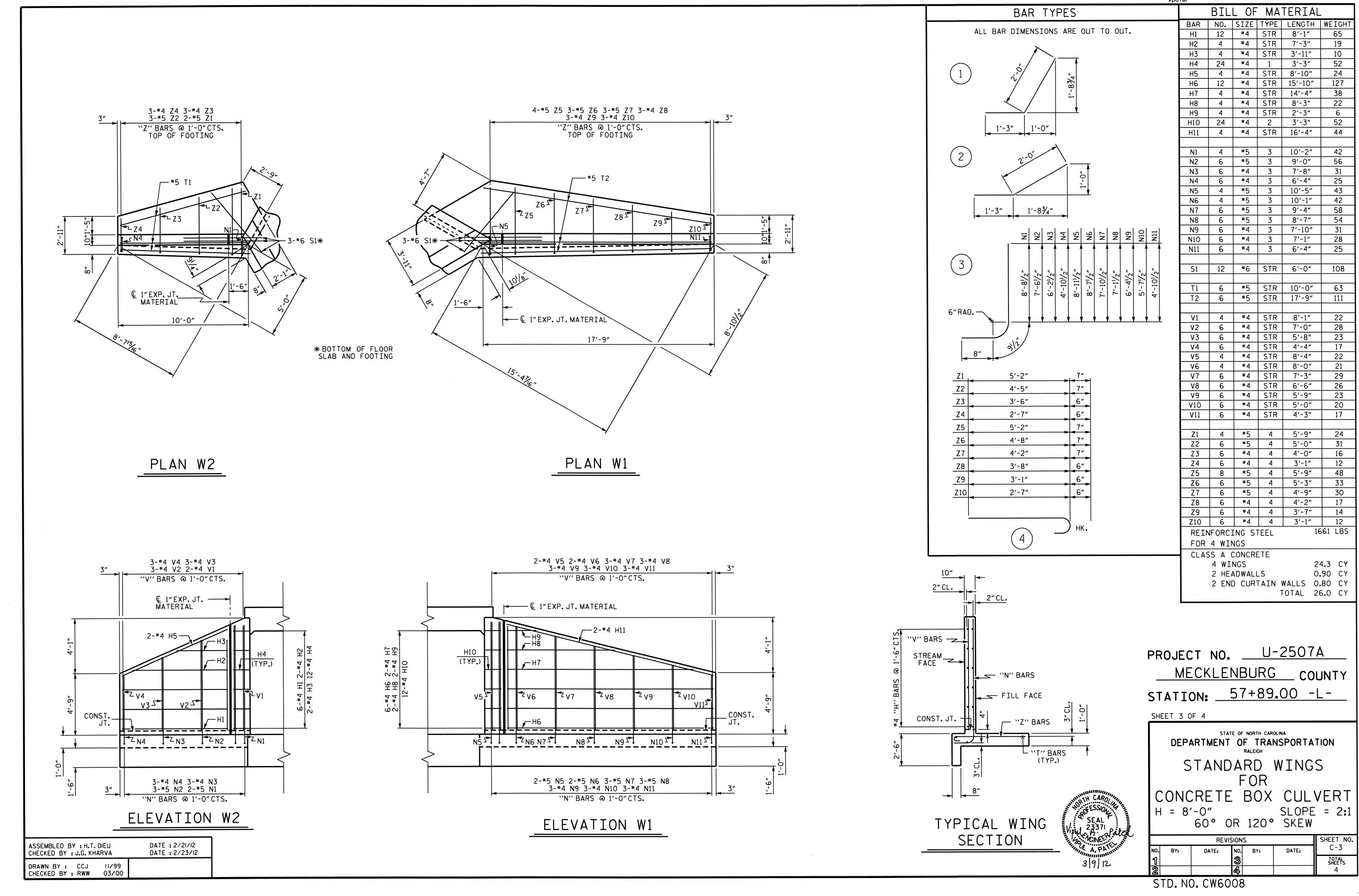
THERE ARE 36 C1 BARS IN SECTION OF BARREL

DRAWN BY: H.T. DIEU DATE: 2/20/12
CHECKED BY: K.G. KHARVA DATE: 2/23/12

08-MAR-2012 08:31 R:\Structures\PLANS\U-2507A_SD_CU.dgn

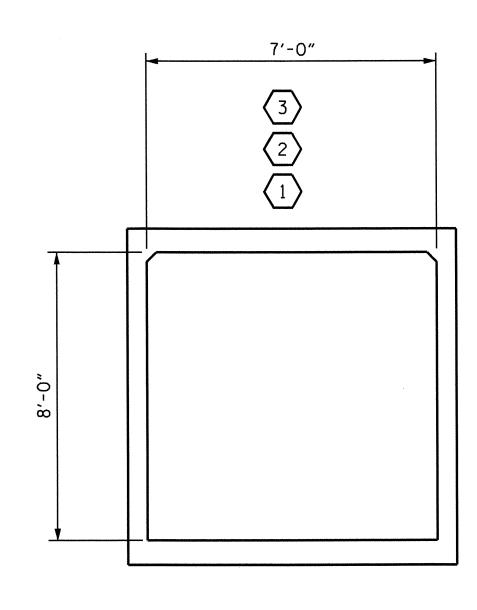
PART PLAN - ROOF SLAB

(SEE BARREL SECTION)



LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

										STRENGTH	I LIM	IT ST	ATE			
							MOMENT				SHEAR					
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (++)	RATING FACTOR	BOX NO.	EL EMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (++)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.04		1.75	1.04	1	TOP SLAB	3.83	1.52	1	BOTTOM SALB	0.81	
DESIGN LOAD		HL-93 (OPERATING)	N/A		1.34		1.35	1.34	1	TOP SLAB	3.83	1.97	1	BOTTOM SALB	0.81	
RATING		HS-20 (INVENTORY)	36.000	2	1.49	53.78	1.75	1.49	1	TOP SLAB	3.83	1.92	1	BOTTOM SALB	0.81	
		HS-20 (OPERATING)	36.000		1.94	69.72	1.35	1.94	1	TOP SLAB	3.83	2.49	1	BOTTOM SALB	0.81	
		SNSH	13.500		2.70	36.49	1.40	2.70	1	TOP SLAB	3.83	3.47	1	BOTTOM SALB	0.81	
	ICLE	SNGARBS2	20.000		2.53	50.69	1.40	2.53	1	TOP SLAB	3.83	3.27	1	BOTTOM SALB	0.81	
		SNAGRIS2	22,000		2.70	59.47	1.40	2.70	1	TOP SLAB	3.83	3.47	1	BOTTOM SALB	0.81	
	VEHICL!	SNCOTTS3	27.250	3	1.30	35.32	1.40	1.30	1	TOP SLAB	3.83	1.90	1	BOTTOM SALB	0.81	
		SNAGGRS4	34.925		1.70	59.36	1.40	1.70	1	TOP SLAB	3.83	2.52	1	BOTTOM SALB	0.81	
	SINGLI	SNS5A	35.550		1.54	54.84	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
		SNS6A	39.950		1.54	61.63	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
LEGAL		SNS7B	42.000		1.54	64.79	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
LOAD RATING	ER	TNAGRIT3	33.000		2.70	89.21	1.40	2.70	1.	TOP SLAB	3.83	3.47	1	BOTTOM SALB	0.81	
	TRAILER	TNT4A	33.075		1.54	51.02	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
	1 1	TNT6A	41.600		1.54	64.17	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
	SEMI-ST)	TNT7A	42.000		1.54	64.79	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
	TRACTOR (TTS	TNT7B	42.000		1.54	64.79	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	
	TRAC	TNAGRIT4	43.000		1.47	63.24	1.40	1.47	1	TOP SLAB	3.83	2.17	1	BOTTOM SALB	0.81	
		TNAGT5A	45.000		1.50	67.58	1.40	1.50	1	TOP SLAB	3.83	2.21	1	BOTTOM SALB	0.81	
	TRUCK	TNAGT5B	45.000		1.54	69.42	1.40	1.54	1	TOP SLAB	3.83	2.28	1	BOTTOM SALB	0.81	



ASSEMBLED BY: R.L. CHESSON DATE: 02/2012
CHECKED BY: H.T. DIEU DATE: 02/2012

DRAWN BY: WMC 7/II
CHECKED BY: GM 7/II

LRFR SUMMARY
(LOOKING DOWNSTREAM)

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR	
DC	1.25	0.90	
DW	1.50	0.65	
EV	1.30	0.90	
ЕН	1.35	0.90	
ES	1.35	0.90	
LS	1.75	0.00	
WA	1.00	0.00	

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

2

_

4.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

PROJECT NO. U-2507A

MECKLENBURG COUNTY

STATION: 57+89.00 -L-

SHEET 4 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)

REVISIONS

D. BY: DATE: NO. BY: DATE:

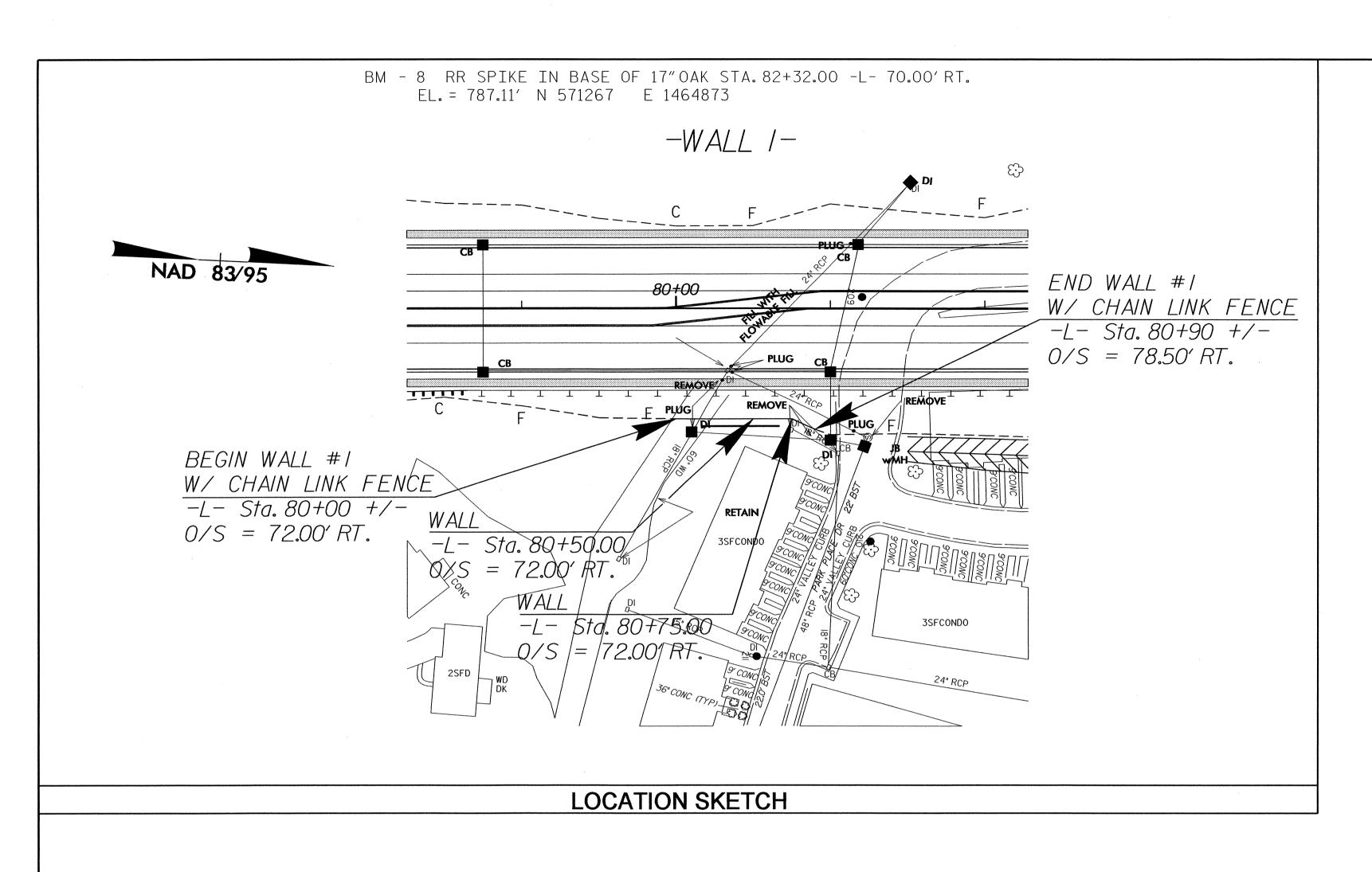
C-4

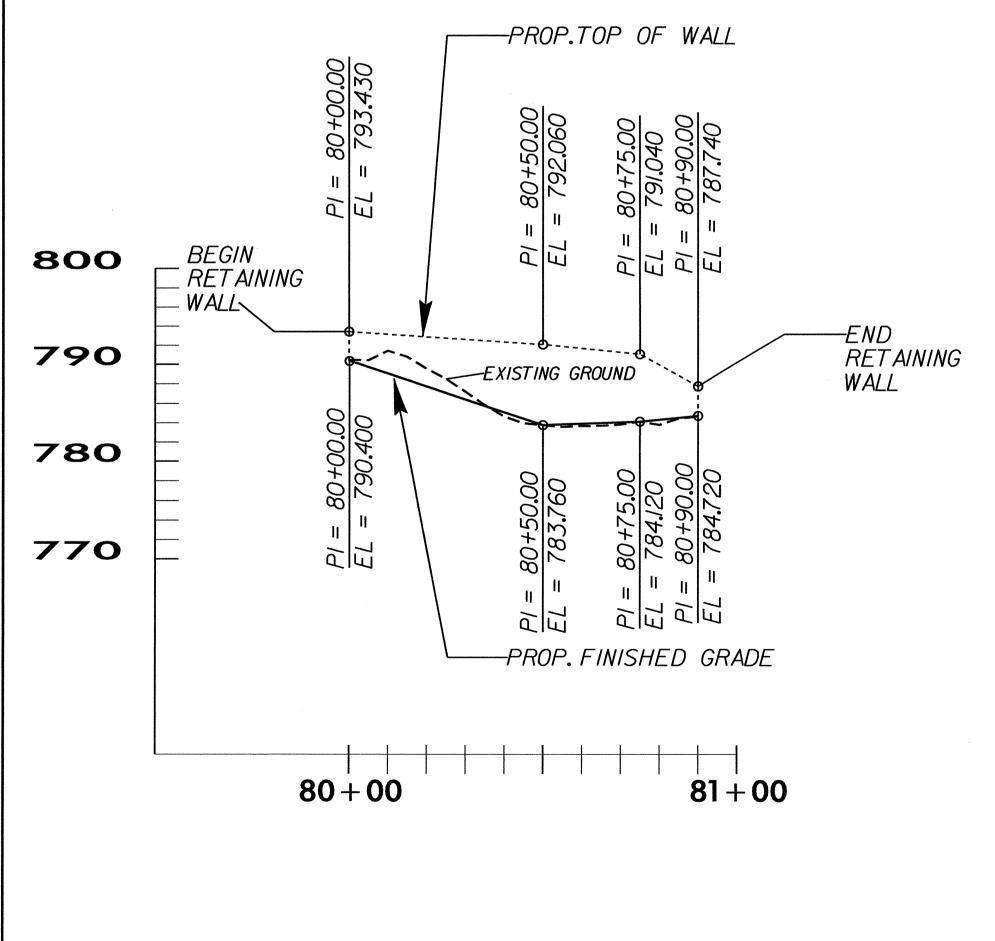
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STD. NO. LRFR5



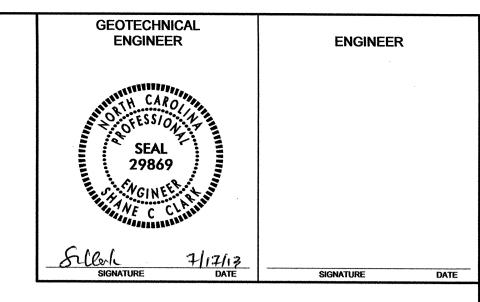


DATE: 5.13

DATE: 7.13

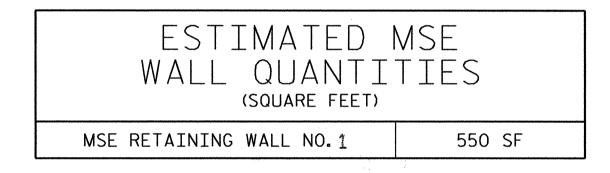
PREPARED BY: J.T.W.

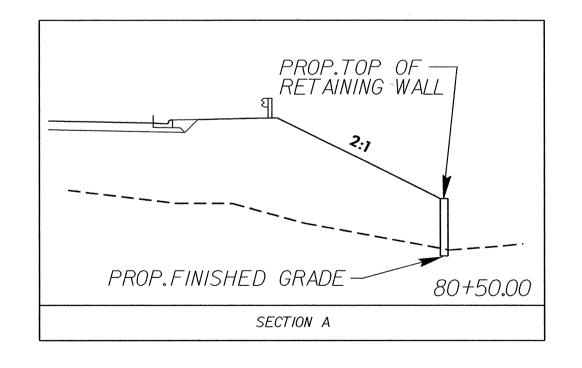
REVIEWED BY: S.C.C.



RETAINING WALL ELEVATIONS								
-L- STA	OFFSET FROM (Q (RIGHT)	ELEV @ TOP OF WALL	* PROPOSED FINISHED GRADE	*EXPOSED WALL HEIGHT	** DESIGN WALL HEIGHT "H"			
80+00.00	72.00	793.430	790.400	3.03	2.53			
80+50.00	72.00	792.060	783.760	8.30	7.80			
80+75.00	72.00	791.040	784.120	6.92	6.42			
80+90.00	78.50	787.740	784.720	3.02	2.52			

- * ELEVATION @ PROPOSED FINISHED GRADE AND EXPOSED WALL HEIGHT DO NOT INCLUDE EMBEDMENT DEPTH
- ** FOR DESIGN WALL HEIGHT "H" AND ADDITIONAL CONSTRUCTION DETAILS, SEE SHEET 2 OF 2





U-2507A PROJECT NO.:

> **MECKLENBURG COUNTY**

STATION: 80+00.00 to 80+90.00 -L-

SHEET 1 OF 3

GEOTECHNICAL ENGINEERING UNIT **EASTERN REGIONAL OFFICE**

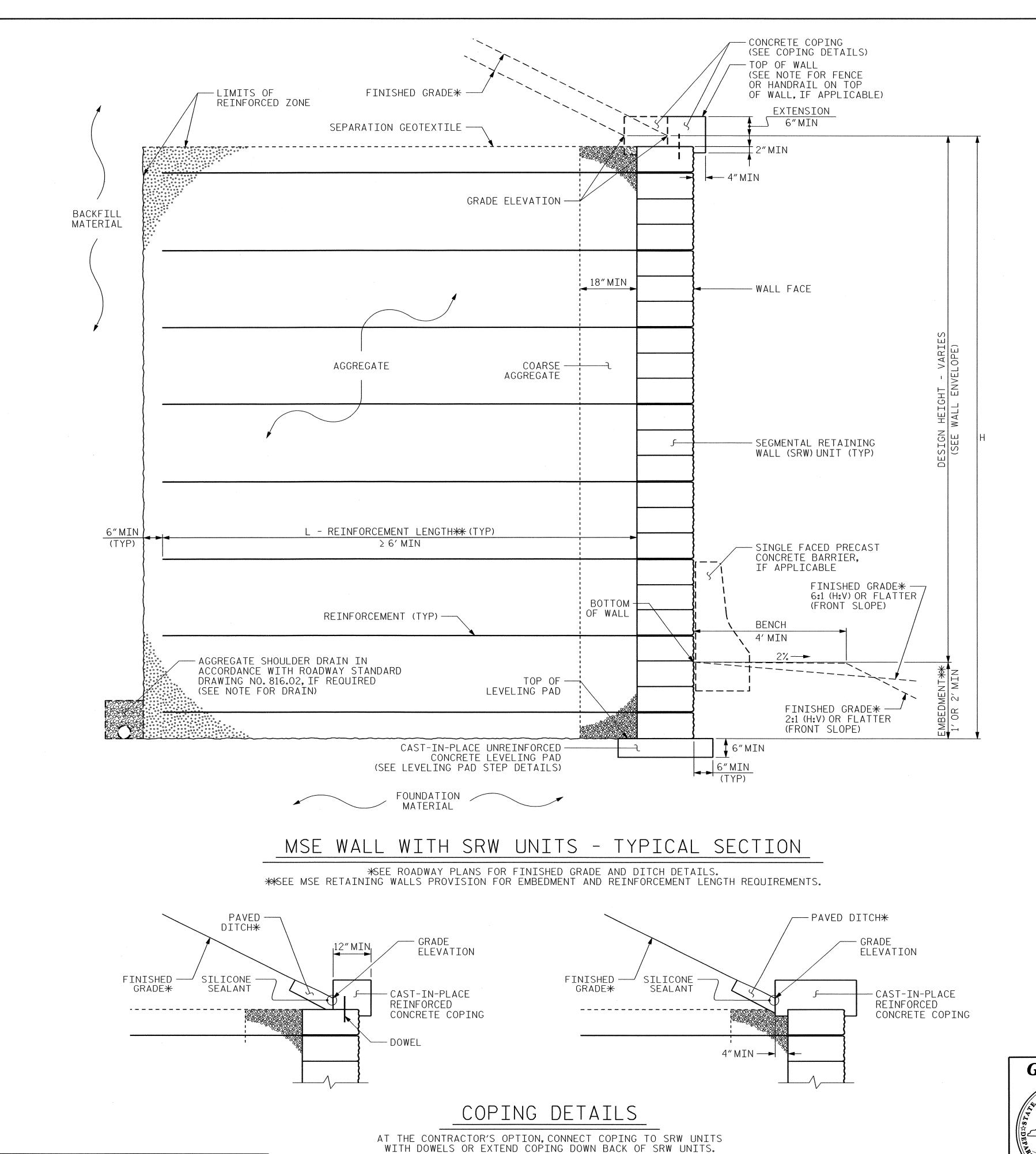
X WESTERN REGIONAL OFFICE

☐ CONTRACT OFFICE STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RALEIGH

MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL NO. 1

	SHEET NO.				
BY	DATE	NO.	BY	DATE	W-1
		3			TOTAL SHEETS
		4			5



*SEE ROADWAY PLANS FOR FINISHED GRADE AND DITCH DETAILS.

PREPARED BY: JTW

REVIEWED BY: SCC

DATE: 5.13

DATE: 7.13

GEOTECHNICAL **ENGINEER ENGINEER**

U-2507A PROJECT NO.:

MECKLENBURG

COUNTY

80+00.00 to 80+90.00 -L-**STATION:**

SHEET 2 OF 3

GEOTECHNICAL ENGINEERING UNIT

STATE OF NORTH CAROLINA

DETAILS REVISIONS SHEET NO. W-2DATE DATE NO. BY TOTAL SHEETS

MECHANICALLY STABILIZED EARTH

(MSE) RETAINING WALL NO. 1

EASTERN REGIONAL OFFICE

X WESTERN REGIONAL OFFICE CONTRACT OFFICE

DEPARTMENT OF TRANSPORTATION

RALEIGH

NOTES:

FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS PROVISION.

A FENCE OR HANDRAIL IS REQUIRED ON TOP OF RETAINING WALL NO.1. SEE ROADWAY PLANS FOR FENCE OR HANDRAIL ATTACHMENT DETAILS.

USE AN MSE WALL SYSTEM WITH SRW UNITS THAT MEET ARTICLE 1040-4 OF THE STANDARD SPECIFICATIONS FOR RETAINING WALL NO.1.

WHEN USING AN MSE WALL SYSTEM WITH SEGMENTAL RETAINING WALL (SRW) UNITS FOR RETAINING WALL NO. 1, FREEZE-THAW DURABLE SRW UNITS THAT MEET ARTICLE 1040-4 OF THE STANDARD SPECIFICATIONS ARE

CAST-IN-PLACE REINFORCED CONCRETE COPING IS REQUIRED FOR RETAINING WALL NO.1.

A DRAIN IS NOT REQUIRED FOR RETAINING WALL NO.1.

BEFORE BEGINNING MSE WALL DESIGN FOR RETAINING WALL NO.1, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL NO.1 FOR THE FOLLOWING:

1) H = DESIGN HEIGHT + EMBEDMENT

2) DESIGN LIFE = 100 YEARS

3) MAXIMUM FACTORED VERTICAL STRESS ON FOUNDATION MATERIAL = 3000 LB/SF

4) MINIMUM EMBEDMENT ELEVATION = 2 FT

5) AGGREGATE PARAMETERS:

J/ AUUNEUATE T ANAMETEN	.						
AGGREGATE TYPE∗	UNIT WEIGHT (γ) LB/CF	FRICTION ANGLE (þ) DEGREES	COHESION (c) LB/SF				
COARSE	110	38	0				
FINE	125	34	0				
*SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE MATERIAL REQUIREMENTS.							

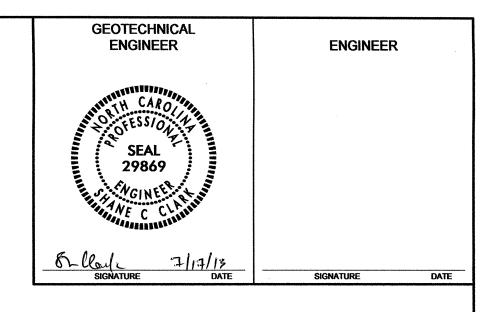
6) IN-SITU ASSUMED MATERIAL PARAMETERS:

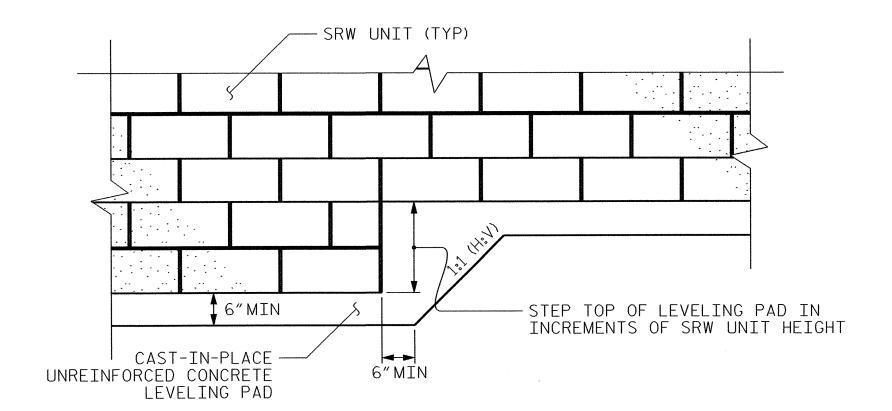
MATERIAL TYPE	UNIT WEIGHT (y) LB/CF	FRICTION ANGLE (þ) Degrees	COHESION (c) LB/SF
BACKFILL	120	30	0
FOUNDATION	120	30	0

DESIGN RETAINING WALL NO.1 FOR A LIVE LOAD (TRAFFIC) SURCHARGE.

DO NOT PLACE LEVELING PAD CONCRETE, AGGREGATE OR REINFORCEMENT FOR RETAINING WALL NO.1 UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.

AT THE CONTRACTOR'S OPTION, "TEMPORARY SHORING FOR WALL CONSTRUCTION" MAY BE USED TO CONSTRUCT RETAINING WALL NO. 1. SEE MSE RETAINING WALLS PROVISION FOR TEMPORARY SHORING FOR WALL CONSTRUCTION.





SEGMENTAL RETAINING WALL (SRW) UNITS

LEVELING PAD STEP DETAILS

PROJECT NO.: U-2507A

MECKLENBURG STATION:

COUNTY 80+00.00 to 80+90.00 -L-

SHEET 3 OF 3

GEOTECHNICAL ENGINEERING UNIT **EASTERN REGIONAL OFFICE** WESTERN REGIONAL OFFICE

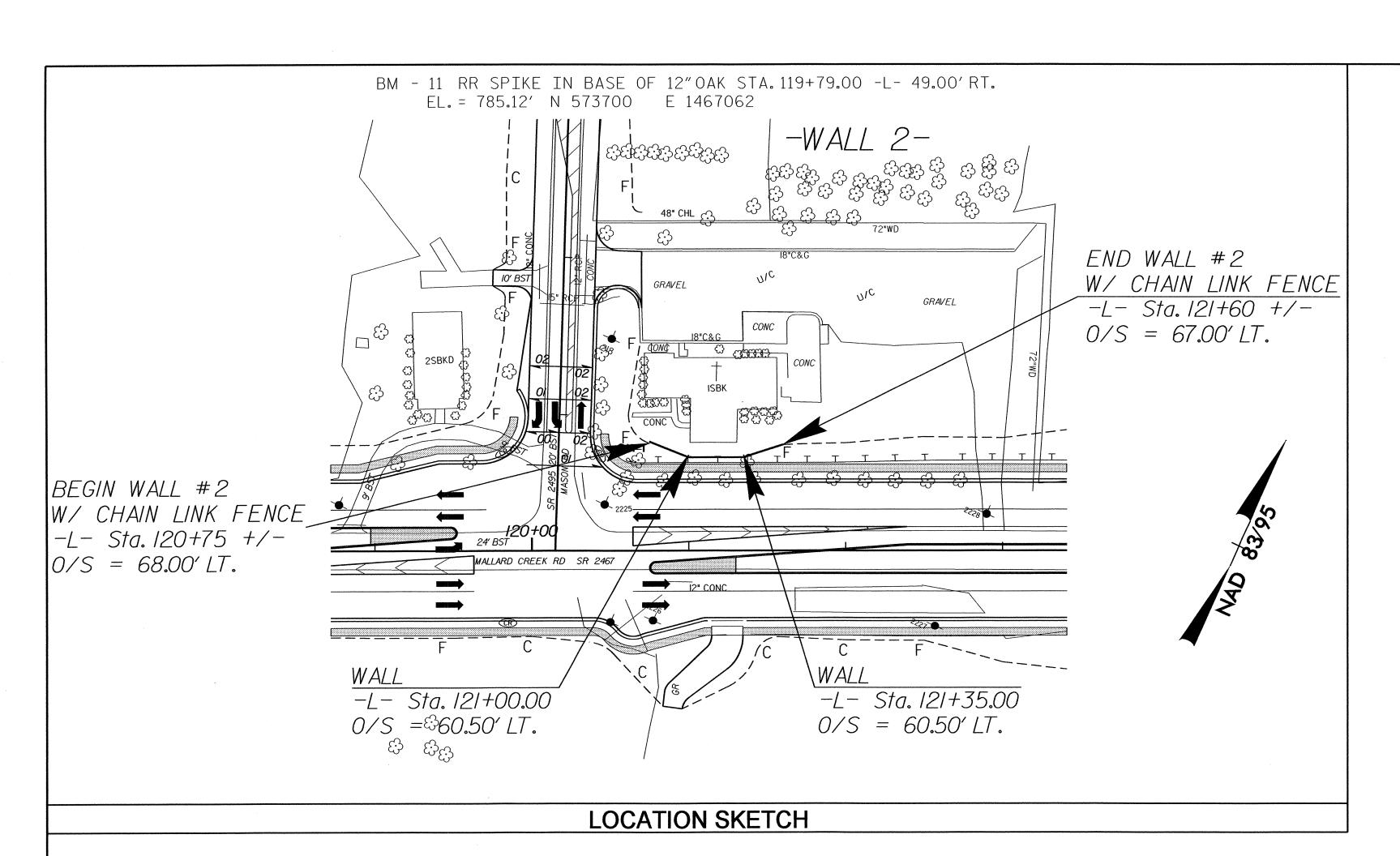
☐ CONTRACT OFFICE

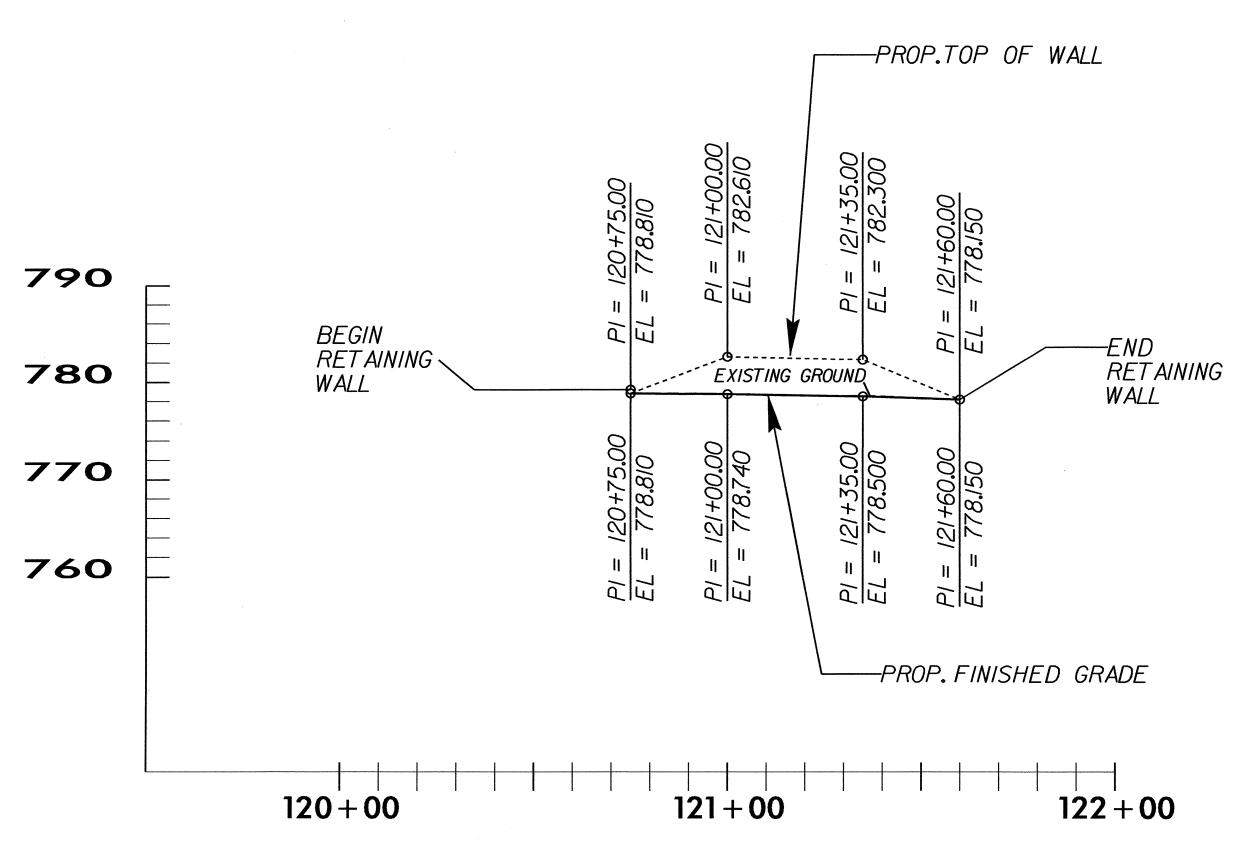
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

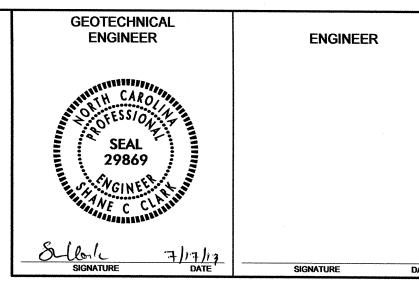
MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALL NO. 1 **DETAILS**

> REVISIONS SHEET NO. ω -3 DATE NO. DATE BY TOTAL SHEETS

DATE: 5.13 PREPARED BY: JTW DATE: 7.13 REVIEWED BY: SCC



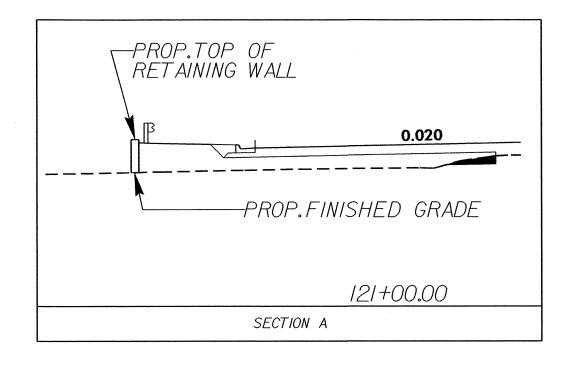




RETAINING WALL ELEVATIONS								
-L- STA	OFFSET FROM (L (LEFT)	ELEV @ TOP OF WALL	* PROPOSED FINISHED GRADE	₩ EXPOSED WALL HEIGHT	** DESIGN WALL HEIGHT "H"			
120+75.00	68.00	778.810	778.810	0.00	0.00			
121+00.00	60.50	782.610	778.740	3.87	3.37			
121+35.00	60.50	782.300	778.500	3.80	3.30			
121+60.00	67.00	778.150	778.150	0.00	0.00			

- * ELEVATION @ PROPOSED FINISHED GRADE AND EXPOSED WALL HEIGHT DO NOT INCLUDE EMBEDMENT DEPTH
- ** FOR DESIGN WALL HEIGHT "H" AND ADDITIONAL CONSTRUCTION DETAILS, SEE SHEET 2 OF 2

ESTIMATED WALL QUANTITIES							
RETAINING WALL NO.	SEGMENTAL GRAVITY RETAINING WALLS (SQUARE FEET)						
1	230 SF						
TOTAL QUANTITY = 230 SF							



PROJECT NO.: U-2507A

MECKLENBURG

COUNTY STATION: 120+75.00 to 121+60.00 -L-

SHEET 1 OF 2

GEOTECHNICAL ENGINEERING UNIT

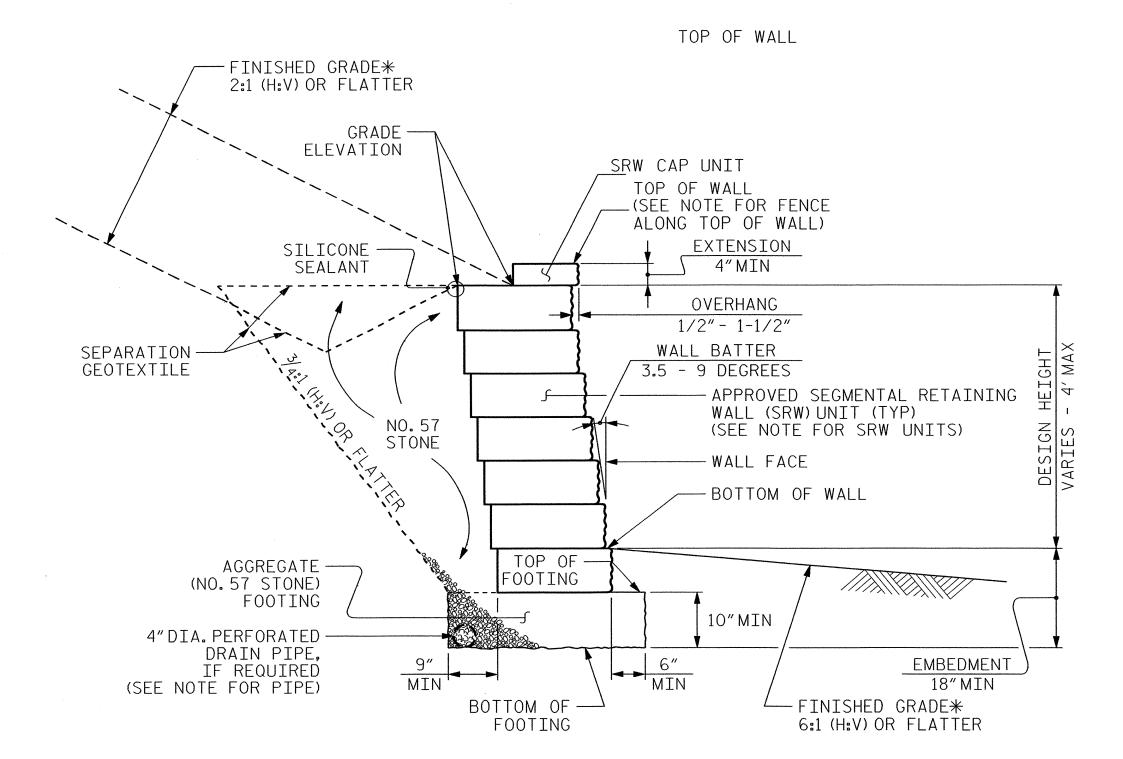
- **EASTERN REGIONAL OFFICE**
- X WESTERN REGIONAL OFFICE **CONTRACT OFFICE**
- STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

RALEIGH

SEGMENTAL GRAVITY RETAINING WALL NO. 2

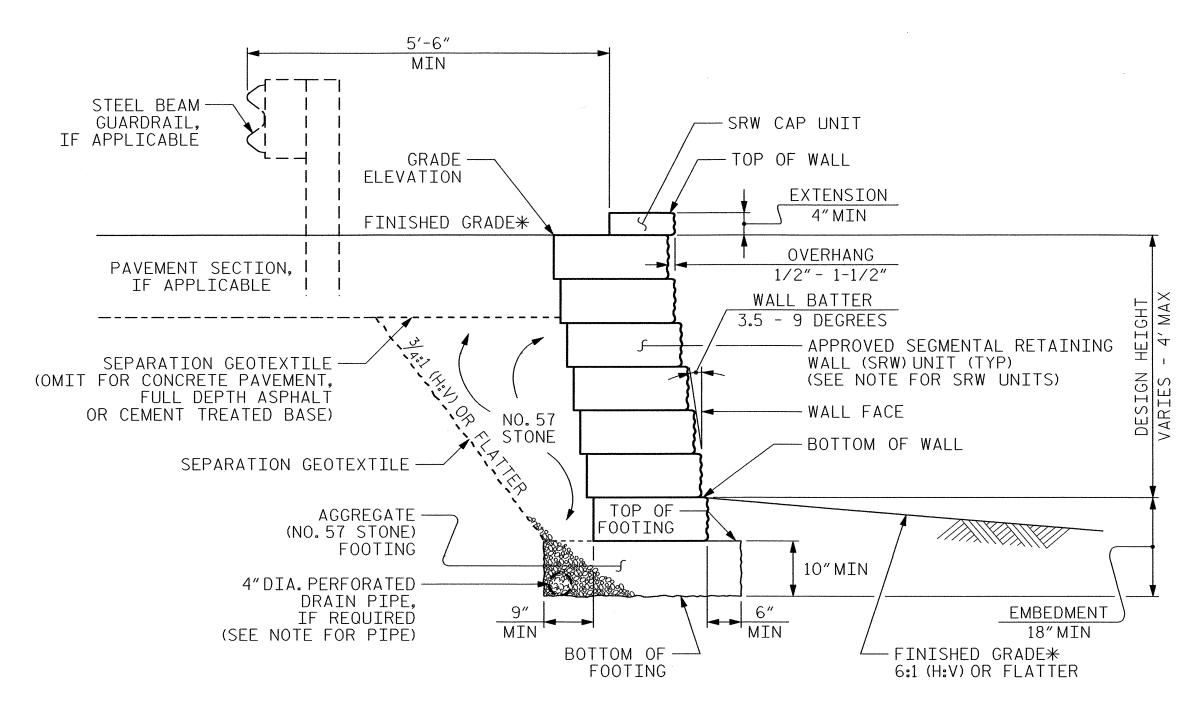
	SHEET NO.					
10.	BY	DATE	NO.	BY	DATE	W-4
1			3			TOTAL SHEETS
2			4			5

PREPARED BY: J.T.W. DATE: 5.13 REVIEWED BY: S.C.C. DATE: 7.13



STANDARD SEGMENTAL GRAVITY WALL WITH SLOPE

*SEE ROADWAY PLANS FOR FINISHED GRADE AND DITCH DETAILS.



STANDARD SEGMENTAL GRAVITY WALL WITHOUT SLOPE

*SEE ROADWAY PLANS FOR FINISHED GRADE DETAILS.

NOTES:

FOR STANDARD SEGMENTAL GRAVITY RETAINING WALLS, SEE SEGMENTAL GRAVITY RETAINING WALLS PROVISION.

FOR STEEL BEAM GUARDRAIL, SEE ROADWAY PLANS AND SECTION 862 OF THE STANDARD SPECIFICATIONS.

A FENCE IS REQUIRED ALONG THE BACK EDGE OF THE RETAINING WALL. DO NOT ATTACH FENCES OR HANDRAILS TO STANDARD SEGMENTAL GRAVITY WALLS. SEE ROADWAY PLANS FOR DETAILS.

DO NOT USE STANDARD SEGMENTAL GRAVITY WALLS FOR INTERSTATE HIGHWAY OR RAILROAD PROJECTS.

DO NOT USE STANDARD SEGMENTAL GRAVITY WALLS WHEN SURCHARGE LOADS WILL BE WITHIN 5'-6"OF THE BACK OF SRW CAP UNITS.

DO NOT USE STANDARD SEGMENTAL GRAVITY WALLS WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS BELOW WALLS.

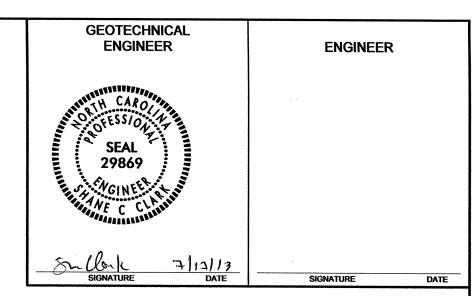
SEGMENTAL RETAINING WALL (SRW) UNITS ARE APPROVED FOR EITHER 2'OR 4' MAXIMUM DESIGN HEIGHTS. FOR DETAILS AND DIMENSIONS OF APPROVED SRW UNITS AND MAXIMUM DESIGN HEIGHTS, SEE www.ncdot.org/doh/preconstruct/highway/geotech/seggravwalls

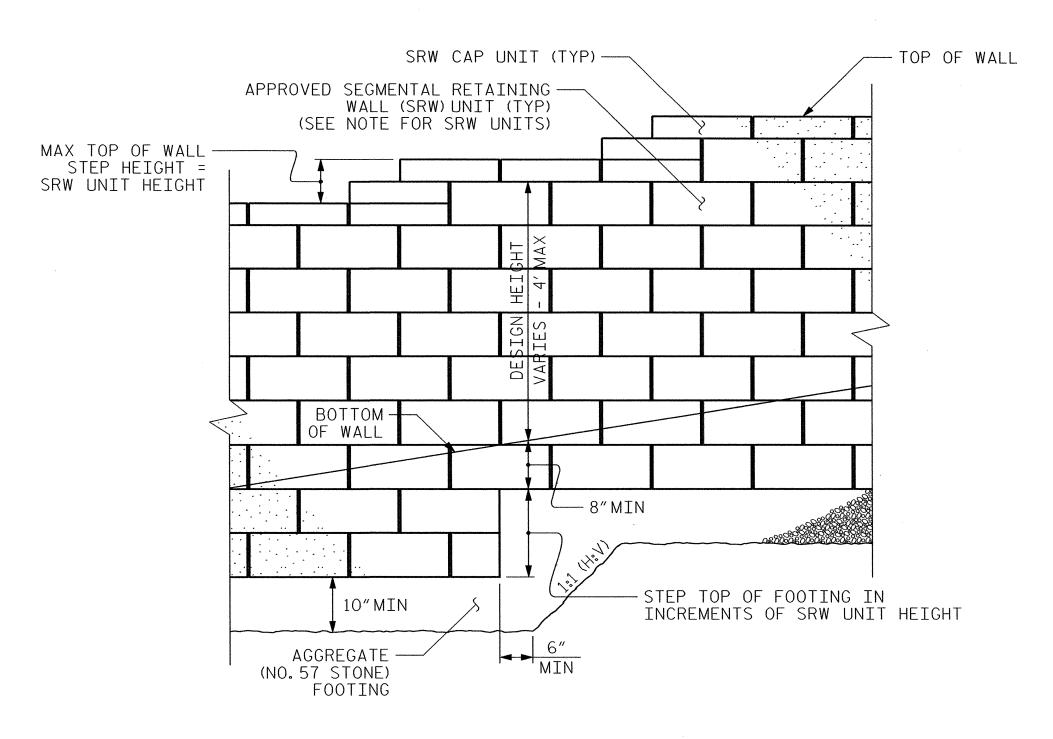
DO NOT MIX APPROVED SRW UNITS FROM DIFFERENT VENDORS ON THE SAME STANDARD SEGMENTAL GRAVITY WALL. USE THE SAME SIZE APPROVED SRW UNITS FOR EACH WALL SECTION.

BEFORE BEGINNING STANDARD SEGMENTAL GRAVITY WALL CONSTRUCTION. SURVEY WALL LOCATIONS AND SUBMIT WALL PROFILE VIEWS (WALL ENVELOPES) FOR REVIEW. FOR WALL ENVELOPES, INCLUDE BOTTOM OF WALL, EXISTING GROUND AND GRADE ELEVATIONS AND OTHER ELEVATIONS AS NEEDED AT INTERVALS OF 25' OR LESS ALONG WALLS. DO NOT START WALL CONSTRUCTION UNTIL WALL ENVELOPES ARE ACCEPTED.

A DRAIN PIPE IS REQUIRED IF GROUNDWATER IS ABOVE BOTTOM OF FOOTINGS.

DO NOT PLACE NO. 57 STONE FOR FOOTINGS UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.





STANDARD SEGMENTAL GRAVITY WALL - PARTIAL ELEVATION

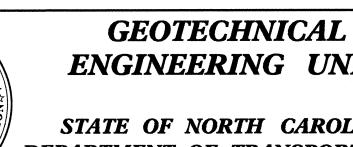
PROJECT NO.: U-2507A

MECKLENBURG

STATION: 120+75.00 to 121+60.00 -L-

STANDARD DRAWING NO. 453.02

SHEET 2 OF 2



ENGINEERING UNIT

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

SEGMENTAL GRAVITY RETAINING WALL

DATE: 1-17-12

 ω -5 TOTAL SHEETS 5

SHEET NO

COUNTY

STANDARD NOTES

DESIGN DATA:

CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN.

CONCRETE IN SHEAR ----- SEE A.A.S.H.T.O.

UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS.PER SQ.IN.

COMPRESSION PERPENDICULAR TO GRAIN
OF TIMBER - - - - 375 LBS.PER SQ.IN.

EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS. PER CU. FT.

(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE

FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990