NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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-L-	10+00.00-22+50.00	4	5	6-8
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-DR2-	10+10.00-11+46.79	4	5	
-DR3-	10+12.00-11+50.00	4	5	
SAMPLE RESUL	.TS	9		

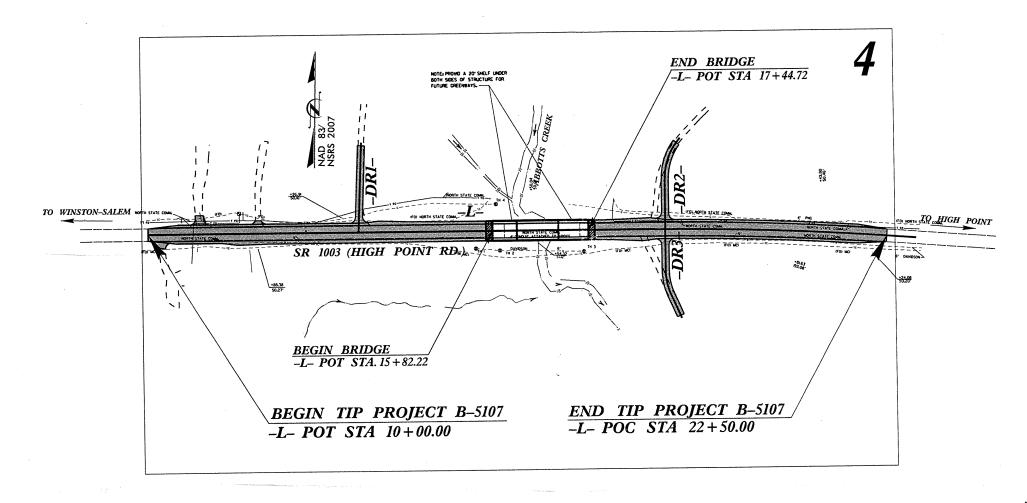
## STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# **ROADWAY** SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. <u>42244.1.1</u> (*B-5107*) F.A. PROJ. *NA* COUNTY FORSYTH PROJECT DESCRIPTION REPLACE BRIDGE NO. 34 OVER ABBOTT'S CREEK ON SR 1003 (HIGH POINT RD.)

## INVENTORY



STATE	STAT	SHEET NO.	TOTAL SHEETS			
N.C.	В	5107	1	9		
STATE	PROJ. NO.	P. A. PROJ. NO.	DESCRIPTION			
422	44.1.1	BRSTP-1003 (71)	P.E.			
422	44.2.1	BRSTP-1003 (71)	ROW &	UTIL		
422	44.3.1	BRSTP-1003 (71)	CON	ST		

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOLL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE NS. C DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA AND THE IN STILL WIN-PLACED TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INNERENT IN THE STANDARD TEST METHAD. THE OBSERVED WATER LEVELS OR SOIL MOSITURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MODISTURE CONDITIONS AND AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MODISTURE CONDITIONS OF CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSUPFACE PLANS ARE PFELRIMARY ONLY AND IM MANY CASES THE FINAL DESIGN DETAILS, ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CURRANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THE SETUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THE SUBSURFACE INFORMATION.

J. K. STICKNEY											
C. L. SMITH											

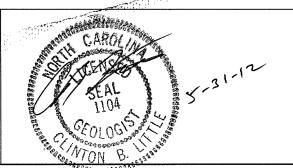
PERSONNEL

INVESTIGATED BY R. Q. CALLAWAY

C. B. LITTLE

SUBMITTED BY C. B. LITTLE

FEBRUARY 2012



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS. SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: C. E. BURRIS

203262

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS

## NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

## DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

				SOIL AND RO	CK LEGEND, TERM	is, symbols,	AND ABBREV	<b>TATIONS</b>				
	SOIL DESCRIPTION			GRADATION			ROCK	DESCRIPTION		TERMS AND DEFINITIONS		
SOIL IS CONSIDERED TO BE THE UNC	ICONSOLIDATED, SEMI-CONSOLIDATED, OR WEAT	THERED EARTH MATERIALS	UNIFORM - INDICATES THAT SO	SODD REPRESENTATION OF PARTICLE SIZES F DIL PARTICLES ARE ALL APPROXIMATELY THE	FROM FINE TO COARSE. E SAME SIZE.(ALSO	ROCK LINE INDICAT	TES THE LEVEL AT WHICH NON	N-COASTAL PLAIN MATERIAL WOULD Y	IELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.		
100 BLOWS PER FOOT ACCORDING TO	D STANDARD PENETRATION TEST (AASHTO T20	36, ASTM D-1586). SDIL	PODRLY GRADED) GAP-GRADED - INDICATES A MI	XTURE OF UNIFORM PARTICLES OF TWO OR I	MORE SIZES.	SPT REFUSAL IS P	ENETRATION BY A SPLIT SPOO	ON SAMPLER EQUAL TO OR LESS THA	ME			
				ANGULARITY OF GRAINS		OF WEATHERED ROCK.			THE PERSON OF THE POPULATION O	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.		
AS MINERALOGICAL COMPOSITION, AND	GULARITY, STRUCTURE, PLASTICITY, ETC. EXAMP	PLE:			TERMS: ANGULAR,				CPT N VALUES > 100	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.		
			ODDANOCEAN, ODDANOCHDED, OK		N .	ROCK (WR)	BLOWS PER FO	OOT IF TESTED.		ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL		
			MINERAL NAMES SUCH AS QUAR	TZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE I		CRYSTALLINE ROCK (CR)	L 13/2 13/3 WOOLD TIELD	OF I REPUSAL IF 165160. ROCK ITE	ROCK THAT E INCLUDES GRANITE.	GROUND SURFACE.		
			WHENEVER THEY ARE CONSIDER						ASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.		
	A-2 A-4 A-5 A-6 A-7		SI ICHTI Y COMPRESS		1 ECC THAN 31	ROCK (NCR)	SEDIMENTARY	ROCK THAT WOULD YEILD SPT REFUS	SAL IF TESTED. ROCK TYPE	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.		
0000000000	A-7-S		MODERATELY COMPRE	SSIBLE LIQUID LIMIT	EDUAL TO 31-50	COASTAL PLAIN	COASTAL PLAI	N SEDIMENTS CEMENTED INTO ROCK,		CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL		
000000000000000000000000000000000000000		1,,,,,,,	HIGHLY CONFRESSIBL			(CP)	SHELL BEDS, E	TC.	ANDSTUNE, CEMENTED	LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
* 10 50 MX		GRANULAR CLAY MULK.	ORGANIC MATERIAL	GRANULAR SILT - CLAY		<b></b>	WE	EATHERING		DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.		
	X 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN		TRACE OF ORGANIC MATTER					JOINTS MAY SHOW SLIGHT STAINING	ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE		
LIQUID LIMIT 48 MX	X 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN	enti e witu	LITTLE ORGANIC MATTER MODERATELY ORGANIC			1		NINED. SOME JOINTS MAY SHOW THIN	CLAY COATINGS IF OPEN.			
PLASTIC INDEX 6 MX NP 10 MX	K 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN	LITTLE OR HIGHLY	HIGHLY ORGANIC			(V SLI.) CRYSTA	als on a broken specimen f	FACE SHINE BRIGHTLY. ROCK RINGS U	NDER HAMMER BLOWS IF	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.		
	8 4 MX 8 MX 12 MX 16 MX No M	AMOUNTS OF SOILS		GROUND WATER		SLIGHT ROCK (	GENERALLY FRESH, JOINTS STA			FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE		
DE MAIDE GRAVEL AND FINE SIL		ORGANIC	l		DRILLING							
MATERIALS SAND	HAVEE AND SAIND SULES SULES			ATER LEVEL AFTER 24 HOURS		MODERATE SIGNIF	ICANT PORTIONS OF ROCK SHO	OW DISCOLORATION AND WEATHERING	EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM		
AS A EXCELLENT TO	GOOD FAIR TO POOR	FAIR TO POOR UNSUITABLE	PERCHED	WATER, SATURATED ZONE, OR WATER BEAR	NG STRATA					PARENT MATERIAL.		
	P IS < 11 - 30 - PI DE A-7-6 SUBGE	<u> </u>	SPRING OF	R SEEP		WITH F	RESH ROCK,			FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY   THE STREAM.		
		3		MISCELLANEOUS SYMBOLS	3	SEVERE AND DI	SCOLORED AND A MAJORITY SI	HOW KAOLINIZATION. ROCK SHOWS SE	VERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN		
A. C.   A. C												
CUN	NSISTENCT (N-VALUE)		WITH SOIL DESCR			SEVERE ALL RI	OCK EXCEPT QUARTZ DISCOLOR	 RED OR STAINED.ROCK FABRIC CLEAR	AND EVIDENT BUT REDUCED			
GENERALLY VERY LOOSE			SOIL SYMBOL	AUGER BORING	SPT N-VALUE				E KADLINIZED TO SOME			
GENERALLY LOOSE CALL FELDSPARS GRANULAR LOOSE LOOSE AT 10 10 10 10 10 10 10 10 10 10 10 10 10			LENS - A BODY OF SDIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.									
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50			N N N	MV						MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.		
1 3		<0.25	INFERRED SOIL BO	O O O O O O O O O O O O O O O O O O O	LL	REMAIN	ING. SAPROLITE IS AN EXAMPL	LE OF ROCK WEATHERED TO A DEGRE	E SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN		
			INFERRED ROCK L									
MATERIAL ST		1 TO 2	***** ALLUVIAL SOIL BE		OR	SCATTE	RED CONCENTRATIONS. QUARTZ					
				TION OF		ALSO A		W HADDNEGO		ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN ANI		
	TEXTURE OR GRAIN SIZE		RDCK STRUCTURES	CONE PENETRON	METER TEST							
				SOUNDING ROD					PECIMENS REDUIRES	PARENT ROCK.		
OPENING (MM)		<del></del>		ABBREVIATIONS	· · · · · · · · · · · · · · · · · · ·			PICK ONLY WITH DIFFICULTY. HARD H	AMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL		
	GRAVEL SAND SAN	ID   SILI   CLAY						NOV	NOUTO DEED CAN DE	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.		
ļ	(GR.) (CSE. SD.) (F S	SD.) (SL.) (CL.)				HARD EXCA	ATED BY HARD BLOW OF A GI			SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR		
	2.0 0.25	0.05 0.005		TEST NP - NON PLASTIC				THICKER DEED BY ETDM PRECEIDE OF	VALLE UD DICK DUINT	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF		
	STURE - CORRELATION OF	TERMS	DMT - DILATOMETER TEST	PMT - PRESSUREMETER TEST		HARD CAN I	BE EXCAVATED IN SMALL CHIP	S TO PEICES 1 INCH MAXIMUM SIZE	BY HARD BLOWS OF THE	A 140 LB HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS		
SOIL MOISTURE SCALE	FIELD MOISTURE GUIDE FOR					1		Y BY KNIFE OR PICK, CAN BE EXCAN	ATED IN FRAGMENTS	THAN 0.1 FOOT PER 60 BLOWS.		
MATTERBERG LIMITS)			1			FROM	CHIPS TO SEVERAL INCHES I	N SIZE BY MODERATE BLOWS OF A P		STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH DF STRATUM AND EXPRESSED AS A PERCENTAGE.		
			FRAC FRACTURED, FRACTU						DE PICK, PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY		
					CBR - CALIFORNIA BEARING RATIO	SOFT OR MO	DRE IN THICKNESS CAN BE BR			TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.		
RANGE <					PROJECT			BEDDI	NG	<u>IOPSOIL (TS.)</u> - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.		
(PI) PL PLASTIC LIMIT					T			TERM	THICKNESS	BENCH MARK:		
OM OPTIMUM MOISTUR	RE - MOIST - (M) SOLID; A	T OR NEAR OPTIMUM MOISTURE	l	l	X AUTOMATIC MANUAL							
	-		MOBILE B			MODERATELY CLO	SE 1 TD 3 FEET			ELEVATION: FT.		
			Вк-51	1				THICKLY LAMINATED	0.008 - 0.03 FEET	NOTES:		
			1 =	1 =		-			( N'ANS LEF!			
	······································	DRY STRENGTH	CME-45C	1		FOR SEDIMENTARY RO			ING, HEAT, PRESSURE, ETC.			
NONPLASTIC	0-5	VERY LOW	X CME-550		□-н	FRIABLE		NG WITH FINGER FREES NUMEROUS G				
LOW PLASTICITY MED. PLASTICITY	6-15 16-25	SLIGHT MEDIUM	DODTAGE 5 ::0107	CASING W/ ADVANCER TRICONE STEEL TEETH	HAND TOOLS:	1		E BLOW BY HAMMER DISINTEGRATES				
HIGH PLASTICITY	26 DR MDRE	HIGH	PORTABLE HOIST		POST HOLE DIGGER	MODERATEL		S CAN BE SEPARATED FROM SAMPLE S EASILY WHEN HIT WITH HAMMER.	WITH STEEL PROBE;			
	COLOR		1	TRICONE TUNGCARB.	HAND AUGER SOUNDING ROD	INDURATED		S ARE DIFFICULT TO SEPARATE WITH	STEEL PROBE;			
	LOR OR COLOR COMBINATIONS (TAN, RED,			CORE BIT	VANE SHEAR TEST		DIFFIC	CULT TO BREAK WITH HAMMER.				
MUDIFIERS SUCH AS LIGHT, D	DARK, STREAKED, ETC. ARE USED TO DESC	HIBE APPEARANCE.				EXTREMELY		P HAMMER BLOWS REQUIRED TO BREA LE BREAKS ACROSS GRAINS.	( SAMPLE;			
· · · · · · · · · · · · · · · · · · ·	····	<del></del>	<del></del>	<del></del>	J					<u> </u>		

PROJECT REFERENCE NO. 42244.I.I (B-5I07)

SHEET NO.

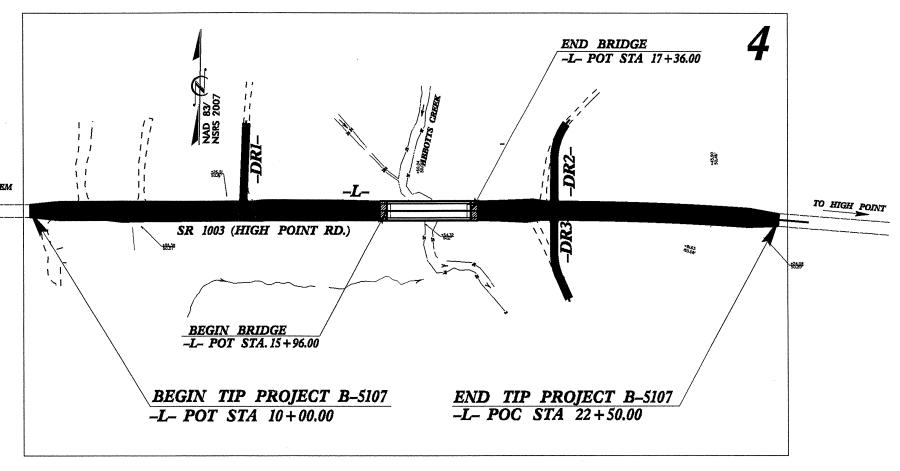
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# FORSYTH COUNTY

2AB-5107 STATE PROLNO P. A. PROL NO. 42244.1.1 P.E.

STATE

LOCATION: REPLACE BRIDGE NO. 34 OVER ABBOTT'S CREEK ON SR 1003 (HIGH POINT RD.) TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE



THIS PROJECT WAS DESIGNED USING THE SUB REGIONAL TIER DESIGN GUIDELINES FOR BRIDGE PROJECTS

THERE IS NO CONTROL OF ACCESS ON THIS PROJECT
THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

510

B

E

GRAPHIC	SCALI	zs Y
<b>2</b> 5	50	100

PLANS PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

## DESIGN DATA

ADT 2013 = 2146ADT 2033 = 3377 DHV = 12 %

D = 55 %T = 4 %V = 50 MPH

\* TTST = 1% DUAL 3% FUNC CLASS = MINOR ARTERIAL SUB-REGIONAL TIER

## PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-5107 = LENGTH STRUCTURE TIP PROJECT B-5107 = TOTAL LENGTH TIP PROJECT B-5107 =

0.210 MILES 0.027 MILES 0.237 MILES

### Prepared In the Office of: **DIVISION OF HIGHWAYS** 1000 Birch Ridge Dr., Raleigh NC, 27610 2006 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: AUGUST 17, 2012

LETTING DATE: AUGUST 20, 2013

TONY HOUSER P.E.

LEE ANN MOORE

# HYDRAULICS ENGINEER

ROADWAY DESIGN **ENGINEER** 

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

STATE HIGHWAY DESIGN ENGINEES



# STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE

EUGENE A. CONTI, JR.

GOVERNOR

SECRETARY

March 29, 2012

STATE PROJECT:

42244.1.1 (B-5107)

FEDERAL PROJECT:

N/A

COUNTY:

Forsyth

**DESCRIPTION:** 

Bridge 34 on SR 1003 over Abbotts Creek, (High Point Road)

SUBJECT:

Geotechnical Report – Inventory

## **Project Description**

The project location is in the southeast corner of Forsyth County, 4000 feet south of US 311, and about a mile west of NC-66. SR 1003 (High Point Road) is a rural, two-lane (24' BST) roadway. The existing bridge over Abbots Creek is 127' long, 32' wide, and three spans, built in 1936. Proposed improvements will replace the existing bridge, raise the grade about 6' and improve the roadway approaches. Total length of the project is 1250'.

This report addresses the roadway portion of the project. The approach roadway will be widened to provide two 12' travel lanes plus 8' shoulders (4' paved shoulder). Most of the widening is in embankment section, but there is a cut section from Station 20+00 to the end at 22+50 –L- left side, that lays back the existing slope. The cuts are for ditches and back slopes – typically 3' deep ditches and 10' high cuts. Maximum embankment height is on the order of 15', a 5' increase above the existing embankment.

The Geotechnical Engineering Unit conducted a total of three Standard Penetration Test borings and two standard auger borings. One SPT boring and the two standard auger borings were in the cut section to determine presence of rock. The other two borings were intended to characterize the alluvial soils under the proposed widened embankments. Two undisturbed samples were collected from the clayey soil section at –L-14+47 43 RT.

## Areas of Special Geotechnical Interest

Rock was encountered above grade in the left ditch, and in the slope layback from -L-21+00 to -L-22+50.

Soft clayey soil was encountered at 0 to 3', 5' to 20' in the boring at 14+47, 47RT at the EB1 approach. Very soft silt was encountered above rock at 11.0 to 17.1 at 17+36, 17 RT, at the EB2 approach.

The area is subject to flooding with groundwater levels at or near ground surface.

MAILING ADDRESS:
NC DEPARTMENT OF TRANSPORTATION
GEOTECHNICAL ENGINEERING UNIT
1589 MAIL SERVICE CENTER
RALEIGH NC 27699-1589

TELEPHONE: 919-707-6850 Fax: 919-250-4237

www.ncdot.gov/doh/preconstruct/highway/geotech

LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC 27610

Page 3a

## Physiography and Geology

The existing roadway begins at elevation 835' at -L- 10+00, and descends on embankment fill to the beginning of the existing bridge at -L- 15+96 and 814' elevation. After crossing the bridge the road climbs on an embankment on a topographic bench from 814' at -L- 17+36-L- 20+00, elevation 820 where the road goes into a cut section on the left. Rock boulder outcrop is apparent from 21+25 to the end at 22+50 elevation 832.

## Hydrology

The streambed elevation is about 800'. The floodplain surface is near 805'. There is a defined topographic valley from 10+00 to 20+00 that contains the active stream channel.

Abbots Creek flows from north to south within the project area. The stream channel is about 35' wide; depth of water is two foot or less at normal flow.

## Geology

The Geology at the site is Charlotte Belt Ppg, Pennsylvanian to Permian age granite. Rock core samples were not obtained. An aggregate quarry about a mile to the east, exploits this resource. Residual soil, weathered rock, and rock outcrops, are consistent with this identification.

### Rock

Drilling at station 17+36 found rock at elevation 794, 11feet below land surface. At station 20+40, no rock was found; the boring was terminated at elevation 812.

## Soils

Alluvial

A boring at –L-14+47, 43RT, EB1 side, found soft to very soft mostly clayey alluvial soil from the floodplain surface, elevation 805, down to elevation 785, with a sandy layer at elevation 801 to 803. At –L-17+36, 18.5RT, EB2 side, a boring found very soft A-4 alluvial soil, from elevation 801down to elevation 795'.

### Residual

Residual soil was highly weathered granite, sand or silt. The thickness of the residual soil above rock, as seen in the borings, varied from less than a foot, to more than 14.8'. Fill

Fill soil was drilled only on the EB2 approach where soft, moist to wet, clayey sandy silt was found from the top of the embankment down to 801.8' elevation. The original floodplain surface was expected at 805, so there is a possibility some settling occurred during construction.

## Groundwater

The boring at Station 14+47 found water at elevation 805, ground surface. The boring at station 17+36 found water at elevation 798. On 02/26/2012 the hydrology report found the stream surface at elevation 797.

Respectfully Submitted

Roger Q Callaway

Project Engineering Geologist

Volumes in Cubic Yards

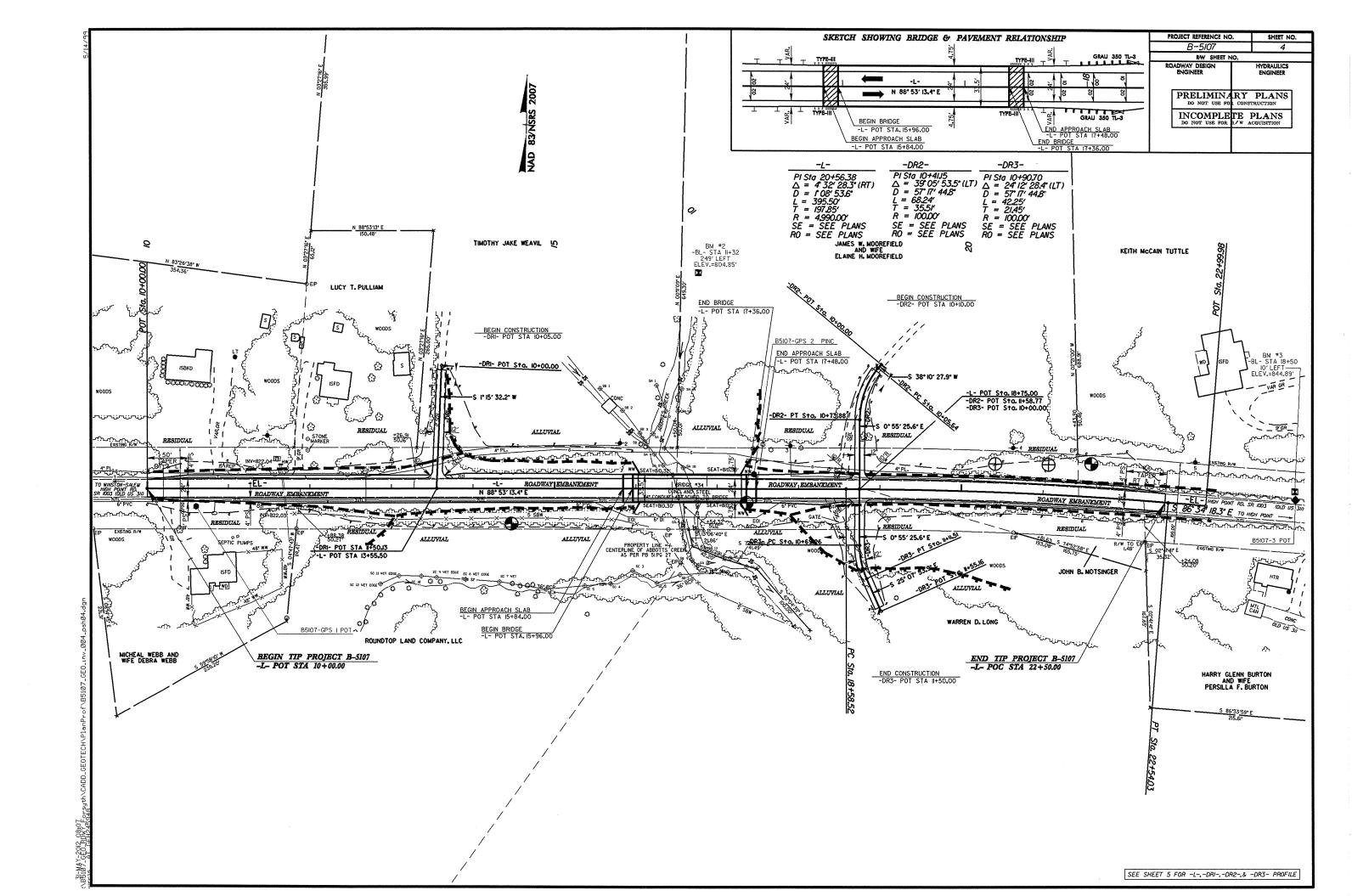
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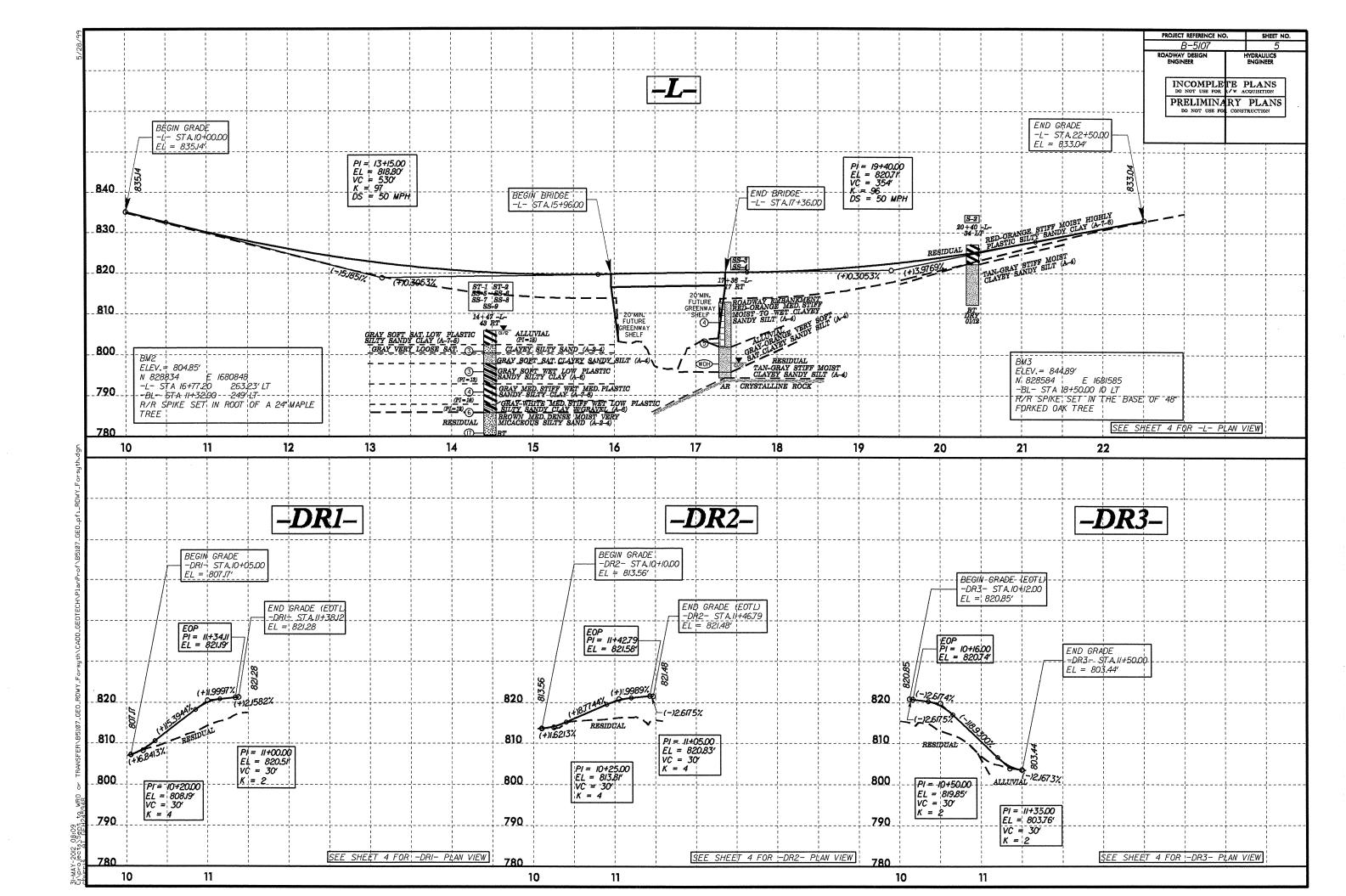
John Braxton

SHEET 1 OF 1 SHEETS

		VOIC	unics in Cubic
PROJECT: B-5107	COUNTY: Forsyth	DATE:	5/18/2012
	<u> </u>		

	EXCAVATION						EMBAN	KMENT			WASTE				
STATION	STATION	TOTAL UNCLASS.	ROCK	UNDERCUT		SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +20%	BORROW	ROCK	SUITABLE	UNSUIT.	TOTAL
L 10+00.00	L 15+82.22 (Beg. BR.)	126			<u> </u>	126	5,252		5,252	6,302	6,176				
L 17+44.72 (End BR.)	L 22+50.00	707	120			587	3,116	120	2,996	3,715	3,008				
					·										
	SUBTOTAL	833	120			713	8,368	120	8,248	10,018	9,185				
DD1 10:05:00	DDI 11.00.10						1,050		1.050	1.260	1.260	Hamananian di Karamanian			
DR1 10+05.00	DRI 11+38.12						349		1,050 349	1,260	1,260 419				
DR2 10+10.00 DR3 10+12.00	DR2 11+46.79 DR3 11+50.00	. 4				4	563	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	563	676	672				
DR3 10+12.00	DK3 11+30.00							1.	303	070	072	<b> </b>			
	SUBTOTAL	4				4	1,962		1,962	2,354	2,350				
<u>Mananananananananananananananananananan</u>															
				ļ			i			ļ				,	
	CLIDEOULA	<b> </b>													
	SUBTOTAL														
		***													
gar par i rasportunt desput i parcioles de l'adol colonido d'antici del Malei dans dans d'antici del material de l'acceptant d															
# ## / MAN HAIR BOOK AND								A PARTICIPATION OF THE PROPERTY OF THE PARTICIPATION OF THE PARTICIPATIO							***************************************
							·								
	SUBTOTAL														
BODA X		025	100			717	10.220	120	10.210	12 272	11.525				
TOTAL		837	120			717	10,330	120	10,210	12,372	11,535				
LOSS DUE TO CLEARING &	& CDIIDDING	-200				-200		15.			200	·			
LOSS DOE TO CLEARING &	& GKODDING	-200			<del> </del>	-200				<del> </del>	200				
				<del>                                     </del>											
		·													
PROJECT TOTAL		637	120			517	10,330	120	10,210	12,372	11,735	,			
															.,
EST. 5% TO REPLACE TOP	SOIL ON BORROW PIT										587				
					-		10.710	100	10.010	12	10.000	<b></b>			
GRAND TOTAL	Fig. 42 (Special Control of Control	637	120		AMERICAN CONTRACTOR OF THE PROPERTY OF THE PRO	517	10,330	120	10,210	12,372	12,322				
SAY Pavement Structure Volumn		640 28 CY									12,400				
Class IV Subgrade Stabiliza	ation	100 tons													
Contingency Undercut	auvii	250 CY		9			THE RESERVE AND ADDRESS OF THE PARTY OF THE								
Contingency Undercut Shallow Undercut		50 CY								<del></del>					
NOTE: EARTHWORK OUA	NITITIES ADE CALCIU ATE		ADWAYDE	CICNIUNIT '	THESE EADT	HWODK OIL	NITITIES ADI	E BASED IN	DART ON SI	IBSTIDEACET	NATA PROVIDEI	BY THE GEOTI	CHNICAL ENGIN	JEERING UNIT	····





	SOIL TEST RESULTS															
SAMPLE	T· T		DEPTH	AASHTO				% BY V	VEIGHT		% PAS	SING (S	SIEVES)	%	%	Line or
NO.	OFFSET	STATION	INTERVAL	CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC	Boring ID
SS-1	40 LT	21+57	3.6-4.6	A-7-6(11)	55	28	30.6	18.6	18.3	32.4	100	78	51		-	L
S-2	34 LT	20+40	4.8-14.8	A-4(0)	26	5	39.1	20.0	24.7	16.2	91	64	38			·L
SS-3	17 RT	17+36	4.6-5.6	A-4(1)	30	10	32.6	23.3	23.8	20.3	98	77	44	-		L
SS-4	17 RT	17+36	14.6-15.6	A-4(1)	33	7	29.6	25.7	28.5	16.2	100	82	45		-	L
SS-5	43 RT	14+47	4.5-6.5	A-2-4(0)	28	NP	36.7	37.7	15.5	10.1	100	79	26	-	•	L
SS-6	43 RT	14+47	9.5-10.5	A-6(10)	40	13	13.2	12.4	42.0	32.4	100	92	75	-	-	L
SS-7	43 RT	14+47	14.5-15.5	A-7-6(12)	42	18	15.6	14.0	38.0	32.4	100	91	71	-	•	L
SS-8	43 RT	14+47	19.5-20.5	A-6(1)	34	12	41.1	14.2	22.4	22.3	81	54	37	-	-	L
SS-9	43 RT	14+47	24.5-25.5	A-2-4(0)	38	NP	14.8	55.7	25.4	4.1	100	95	32	-	•	L
ST-2	43 RT	14+47	1.1-3.1	A-7-5(9)	45	15	13.8	23.9	32.2	30.1	100	93	64			
ST-1#1	43 RT	14+47	5.0-7.0	A-2-4(0)	27	NP	42.5	30.8	14.5	12.1	95	66	27			
ST-1-2&3	43 RT	14+47	5.0-7.0	A-4(1)	30	5	18.1	34.3	25.4	22.2	100	90	51			