

**STONE VENEERED CONCRETE MASONRY UNIT (CMU) RETAINING WALL****(SPECIAL)****1.0 GENERAL**

The natural stone (masonry) walls are to be in accordance with the plans and the Standard Specifications with the following additions and exceptions.

**A. Quality Assurance**

An experienced stone mason with a minimum of 7 years of practice shall be required to construct the stone walls and columns, if applicable. Mason must submit references and samples of built work prior to beginning work.

The Stone Veneer will be constructed of hard, durable stone matching that which is currently on site or in the immediate vicinity as defined by the Engineer. If applicable, existing rocks in the stone walls are to be reused where possible, at the direction of the Engineer. The Stone capping is to be constructed using a one stone thickness of the wall stone with a minimum 2" thickness. All exposed edges are to be veneered with natural stone to 12" below finished grade as defined by the Engineer. Stone height shall vary but be similar in size and shape to that which is currently present in the area. Prior to construction of masonry work, actual stone samples or sections of stone shall be submitted for approval of color and texture. After approval, a sample stone wall is to be erected using proposed materials, and bond and joint tooling required for final work. Provide special features as directed for caulking and contiguous work. The sample walls shall be built at the site, where directed, of full thickness and contain approximately a minimum of 20 square feet of veneer surface area and a minimum of 5 linear feet of capping, indicating the proposed range of color, texture and workmanship to be expected in the completed work. The sample wall shall be a separate wall or may be incorporated into proposed walls as shown on plans, or as directed by the Engineer. Written acceptance of visual qualities of the panel(s) shall be obtained prior to constructing any other masonry work.

An accepted panel shall be retained during construction as a standard for judging quality of completed masonry work. Approved panel shall not be altered, moved, or destroyed until all masonry work is completed. Sample panel may be used to test proposed cleaning procedures.

**B. Protection of Work**

Cold weather protection shall be in accordance with the Standard Specifications.

During erection, cover top of walls with heavy waterproof sheeting at end of each days' work. Cover partially completed structures when work is not in progress. Extend cover a minimum of 1.0 feet down both sides and hold cover securely in place.

Do not apply concentrated loads for at least 3 days after building masonry walls.

## 2.0 MATERIALS

### A. Stone Masonry Units, General

Obtain natural stone from one or more quarries as needed to produce a veneering that meets the criteria noted above, or reuse cleaned stones from the existing walls.

### B. Masonry Wall and Footing

Concrete shall be Class "A" and meet the requirements of Section 1000 of the Standard Specifications. Reinforcing steel shall meet the requirements of Section 1070. Concrete block shall be "Ivany Block" specifically manufactured for reinforced masonry wall construction.

Excavate and pour reinforced concrete footing, build reinforced block wall, and lay stone veneer and coping as shown on the drawings. Place reinforcing steel as described in Section 425. Allow the masonry to cure a minimum of 7 days prior to placing backfill.

### C. Masonry Accessories

Ties shall be 3/16" diameter wire  
7 1/2" anchor with 3 1/4" tie min.

Use Heckman Double Eye-Rod Anchor/Tie No. 263, Hohmann and Banard Adjustable Wall Tie No. 600, or National No. 915.

Backfill shall be Class I Select as described in Section 1016.

### D. Mortar

Use Type S mortar that is in accordance with Section 830-3 of the Standard Specifications. The color of the mortar sand shall match as close as possible the mortar of other adjacent walls on the project.

## 3.0 INSTALLATION

### A. General

Excavate as necessary for Stone Veneered CMU Retaining Walls in accordance with the accepted submittals. If applicable and at the Contractor's option, use temporary shoring for wall construction instead of temporary slopes to construct Stone Veneered CMU Retaining Walls. Define "temporary shoring for wall construction" as temporary shoring not shown in the plans or required by the Engineer including shoring for OSHA reasons or the Contractor's convenience.

Build single-wythe veneer to maximum thickness of the stone masonry units, using units of nominal thickness shown or specified.

Wet stones, prior to setting in mortar, that have ASTM C-67 absorption rates greater than 25 gal./(min. x 19.4 mm<sup>2</sup>).

Frozen Materials and Work:

Do not use frozen materials or materials mixed or coated with ice or frost. For masonry which is specified to be wetted, comply with the BIA recommendations. Do not build on frozen work. Remove and replace masonry work damaged by frost or freezing.

#### Stone Veneer Pattern:

Lay exposed stone masonry except where otherwise shown or specified, in a random pattern that approximates what is presently on site, or as directed by the Engineer.

Layout veneer in advance for accurate spacing of surface bond patterns, with varying joint widths to properly locate openings, movement-type joints, returns and offsets. Avoid the use of less-than-half size units at corners and where possible at other locations.

Lay-up stone veneer plumb and true and with courses level, accurately spaced and coordinated with other work.

#### Stopping and Resuming Work:

Rack back  $\frac{1}{2}$  masonry unit length in each course; do not tooth. Clean exposed surfaces of set masonry, wet units lightly (if specified to be wetted), and remove loose masonry units and mortar prior to laying fresh masonry.

#### B. Batch Control

Measure/batch materials by volume or weight, such that required proportions for mortar can be accurately controlled and maintained. Measurement of sand by shovel will not be permitted.

Mix mortars with maximum amount of water consistent with workability to provide maximum tensile bond strength within the capacity of the mortar.

Mix mortar ingredients for a minimum of 3 minutes and a maximum of 5 minutes in a mechanical batch mixer. Use water clean/free of deleterious materials which would impair the work. Discard mortar which has begun to set, or if more than 2 hours has elapsed since initial mixing or  $1\frac{1}{2}$  hr. in hot weather. Retemper mortar during first  $1\frac{1}{2}$  hr. period as required to restore workability.

Lay stone units with completely filled bed, head and collar joints; butter ends with sufficient mortar to fill head joints and shove into place. Do not slush head joints.

#### C. Joints

Maintain consistent joint widths, except for minor variations required to maintain stone alignment. Deeply rake exposed joints between stones. Rake out mortar in preparation for application of caulking or sealants where shown.

Remove stone masonry units disturbed after laying; clean and relay in fresh mortar. Do not pound corners at jambs to fit stretcher units which have been set in position. If adjustments are required, remove units, clean off mortar, and reset in fresh mortar.

D. Anchoring Brick Veneer Work

Anchor single wythe stone masonry veneer to backing with metal ties as follows:

Anchor veneer to CMU core wall as directed by manufacture's recommendations or as directed by the Engineer. Provide anchors with flexible tie section, unless otherwise indicated.

Space veneer anchors not more than 1.25 ft o.c. vertically and 2.0 ft o.c. horizontally. Provide additional anchors with 1.0 ft of openings and space not more than 3.0 ft around perimeter.

E. Control and Expansion Joints

Provide vertical control joints in brick masonry veneer where shown. Build-in related masonry accessory items as masonry work progresses.

F. Repair, Pointing and Cleaning

Replace stone masonry units which are loose, chipped broken, stained or otherwise damaged, or if units do not match adjoining units as intended.

Provide new units to match adjoining units and install in fresh mortar pointed to eliminate evidence of replacement.

During tooling of joints, enlarge any voids or holes, except weepholes, and completely fill with mortar. Point-up all joints at corners, openings and adjacent work to provide neat, uniform appearance, properly prepared for application of caulking or sealant compounds.

**4.0 BASIS OF PAYMENT**

*Stone Veneered CMU Retaining Walls* will be measured and paid in square feet. *Stone Veneered CMU Retaining Walls* will be measured as the square feet of exposed wall face area with the height equal to the difference between top and bottom of wall elevations. Define "top of wall" as top of stone veneering. Define "bottom of wall" as shown in the plans and no measurement will be made for portions of *Stone Veneered CMU Retaining Walls* embedded below bottom of wall elevations. In areas where two surfaces are veneered only one side will be measured for payment.

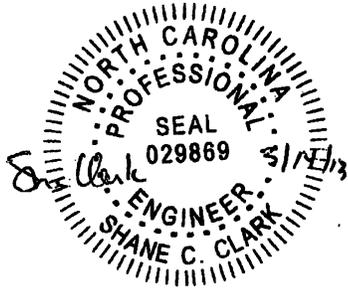
Payment for the *Stone Veneered CMU Retaining Walls* will be made at the contract bid price for which price and payment shall be full compensation for all work covered by this special provision including but not limited to furnishing all materials (including native stone, mortar, masonry accessories, anchors, samples, etc.), labor tools, and equipment necessary for installing these units in place and accepted. Reinforcing steel, backfill, and coarse gravel filter will be incidental to the work and no separate payment will be made for these items.

No separate payment will be made for temporary shoring for wall construction. Temporary shoring for wall construction will be incidental to the contract unit price for *Stone Veneered CMU Retaining Walls*.

Payment as described above will be full compensation for all work covered by this section including but not limited to footing excavation, furnishing and installing reinforcing steel, concrete, block, stone veneer, foundation drain, backfill, and other incidental material; and all labor and equipment necessary to complete the work.

Payment will be made under:

*Stone Veneered CMU Retaining Walls* .....Sq Ft.



**TEMPORARY SOIL NAIL WALLS:**

(10-16-12)

**Description**

Construct temporary soil nail walls consisting of soil nails spaced at a regular pattern and connected to a reinforced shotcrete face. A soil nail consists of a steel bar grouted in a drilled hole inclined at an angle below horizontal. At the Contractor's option, use temporary soil nail walls instead of temporary shoring for full cut sections. Design and construct temporary soil nail walls based on actual elevations and wall dimensions in accordance with the contract and accepted submittals. Use a prequalified Anchored Wall Contractor to construct temporary soil nail walls. Define "soil nail wall" as a temporary soil nail wall and "Soil Nail Wall Contractor" as the Anchored Wall Contractor installing soil nails and applying shotcrete. Define "nail" as a soil nail.

**Materials**

Refer to Division 10 of the *Standard Specifications*.

| <b>Item</b>                  | <b>Section</b> |
|------------------------------|----------------|
| Anchor Pins                  | 1056-2         |
| Geocomposites                | 1056           |
| Neat Cement Grout, Nonshrink | 1003           |
| Reinforcing Steel            | 1070           |
| Shotcrete                    | 1002           |
| Steel Plates                 | 1072-2         |

Provide soil nails consisting of grouted steel bars and nail head assemblies. Use deformed steel bars that meet AASHTO M 275 or M 31, Grade 60 or 75. Splice bars in accordance with Article 1070-9 of the *Standard Specifications*.

Fabricate centralizers from schedule 40 PVC plastic pipe or tube, steel or other material not detrimental to steel bars (no wood). Size centralizers to position bars within 1" of drill hole centers and allow tremies to be inserted to ends of holes. Use centralizers that do not interfere with grout placement or flow around bars.

Provide nail head assemblies consisting of nuts, washers and bearing plates. Use steel plates for bearing plates and steel washers and hex nuts recommended by the Soil Nail Manufacturer.

Provide Type 6 material certifications for soil nail materials in accordance with Article 106-3 of the *Standard Specifications*. Store steel materials on blocking at least 12" above the ground and protect it at all times from damage; and when placing in the work make sure it is free from dirt, dust, loose mill scale, loose rust, paint, oil or other foreign materials. Load, transport, unload and store soil nail wall materials so materials are kept clean and free of damage. Damaged or deformed materials will be rejected.

**Preconstruction Requirements****(A) Concrete Barrier**

Define "clear distance" behind concrete barrier as the horizontal distance between the barrier and edge of pavement. The minimum required clear distance for concrete barrier is shown in the plans. At the Contractor's option or if the minimum required clear distance is not available, set concrete barrier next to and up against traffic side of soil nail

walls except for barrier above walls. Concrete barrier with the minimum required clear distance is required above soil nail walls.

**(B) Soil Nail Wall Designs**

Before beginning soil nail wall design, survey existing ground elevations in the vicinity of wall locations to determine actual design heights (H). Use a prequalified Anchored Wall Design Consultant to design soil nail walls. Provide designs sealed by a Design Engineer approved as a Geotechnical Engineer (key person) for the Anchored Wall Design Consultant.

Submit 8 copies of working drawings and 3 copies of design calculations and a PDF copy of each for soil nail wall designs in accordance with Article 105-2 of the *Standard Specifications*. Submit working drawings showing plan views, wall profiles, typical sections and details of soil nail wall design and construction sequence. Include details in working drawings of soil nail locations, unit grout/ground bond strengths, shotcrete reinforcement and if necessary, obstructions extending through walls or interfering with nails. Include details in construction sequence of excavation, grouting, installing reinforcement, nail testing and shotcreting with mix designs and shotcrete nozzleman certifications. Do not begin soil nail wall construction until a design submittal is accepted.

Design soil nail walls in accordance with the plans and allowable stress design method in the *FHWA Geotechnical Engineering Circular No. 7 "Soil Nail Walls"* (Publication No. FHWA-IF-03-017) unless otherwise required.

Design soil nails that meet the following unless otherwise approved:

- (1) Horizontal and vertical spacing of at least 3 ft,
- (2) Inclination of at least 12° below horizontal and
- (3) Diameter of 4" to 10".

Do not extend nails beyond right-of-way or easement limits. If existing or future obstructions such as foundations, pavements, pipes, inlets or utilities will interfere with nails, maintain a clearance of at least 6" between obstructions and nails.

Design soil nail walls for a traffic surcharge of 250 lb/sf if traffic will be above and within H of walls. This traffic surcharge does not apply to construction traffic. Design soil nail walls for any construction surcharge if construction traffic will be above and within H of walls.

Place geocomposite drain strips with a horizontal spacing of no more than 10 ft and center strips between adjacent nails. Attach drain strips to excavation faces. Use shotcrete at least 4" thick and reinforce shotcrete with #4 waler bars around nail heads. Two waler bars (one on each side of nail head) in the horizontal and vertical directions are required for a total of 4 bars per nail.

**(C) Preconstruction Meeting**

Before starting soil nail wall construction, hold a preconstruction meeting to discuss the construction, inspection and testing of the soil nail walls. Schedule this meeting after all soil nail wall submittals have been accepted. The Resident, District or Bridge

Maintenance Engineer, Bridge or Roadway Construction Engineer, Geotechnical Operations Engineer, Contractor and Soil Nail Wall Contractor Superintendent will attend this preconstruction meeting.

### **Construction Methods**

Control drainage during construction in the vicinity of soil nail walls. Direct run off away from soil nail walls and areas above and behind walls.

Install foundations located behind soil nail walls before beginning wall construction. Do not excavate behind soil nail walls. If overexcavation occurs, repair walls with an approved method and a revised soil nail wall design may be required.

Install concrete barrier in accordance with the contract and accepted submittals. Use PCB in accordance with Section 1170 of the *Standard Specifications* and Standard Drawing No. 1170.01 of the *Roadway Standard Drawings*.

#### **(A) Excavation**

Excavate for soil nail walls from the top down in accordance with the accepted submittals. Excavate in staged horizontal lifts with no negative batter (excavation face leaning forward). Excavate lifts in accordance with the following:

- (1) Heights not to exceed vertical nail spacing,
- (2) Bottom of lifts no more than 3 ft below nail locations for current lift and
- (3) Horizontal and vertical alignment within 6" of location shown in the accepted submittals.

Remove any cobbles, boulders, rubble or debris that will protrude more than 2" into the required shotcrete thickness. Rocky ground such as colluvium, boulder fills and weathered rock may be difficult to excavate without leaving voids.

Apply shotcrete to excavation faces within 24 hours of excavating each lift unless otherwise approved. Shotcreting may be delayed if it can be demonstrated that delays will not adversely affect excavation stability. If excavation faces will be exposed for more than 24 hours, use polyethylene sheets anchored at top and bottom of lifts to protect excavation faces from changes in moisture content.

If an excavation becomes unstable at any time, suspend soil nail wall construction and temporarily stabilize the excavation by immediately placing an earth berm up against the unstable excavation face. When this occurs, repair walls with an approved method and a revised soil nail wall design may be required.

Do not excavate the next lift until nail installations and testing and shotcrete application for the current lift are accepted and grout and shotcrete for the current lift have cured at least 3 days and 1 day, respectively.

#### **(B) Soil Nails**

Drill and grout nails the same day and do not leave drill holes open overnight. Control drilling and grouting to prevent excessive ground movements, damaging structures and pavements or fracturing rock and soil formations. If ground heave or subsidence occurs, suspend soil nail wall construction and take corrective action to minimize movement. If

property damage occurs, make repairs with an approved method and a revised soil nail wall design may be required.

(1) **Drilling**

Use drill rigs of the sizes necessary to install soil nails and with sufficient capacity to drill through whatever materials are encountered. Drill straight and clean holes with the dimensions and inclination shown in the accepted submittals. Drill holes within 6" of locations and 2° of inclination shown in the accepted submittals unless otherwise approved.

Stabilize drill holes with temporary casings if unstable, caving or sloughing material is anticipated or encountered. Do not use drilling fluids to stabilize drill holes or remove cuttings.

(2) **Steel Bars**

Center steel bars in drill holes with centralizers. Securely attach centralizers along bars at no more than 8 ft centers. Attach uppermost and lowermost centralizers 18" from excavation faces and ends of holes.

Do not insert steel bars into drill holes until hole locations, dimensions, inclination and cleanliness are approved. Do not vibrate, drive or otherwise force bars into holes. If a steel bar cannot be completely and easily inserted into a drill hole, remove the bar and clean or redrill the hole.

(3) **Grouting**

Remove oil, rust inhibitors, residual drilling fluids and similar foreign materials from holding tanks/hoppers, stirring devices, pumps, lines, tremie pipes and any other equipment in contact with grout before use.

Inject grout at the lowest point of drill holes through tremies, e.g., grout tubes, casings, hollow-stem augers or drill rods, in one continuous operation. Fill drill holes progressively from ends of holes to excavation faces and withdraw tremies at a slow even rate as holes are filled to prevent voids in grout. Extend tremies into grout at least 5 ft at all times except when grout is initially placed in holes.

Provide grout free of segregation, intrusions, contamination, structural damage or inadequate consolidation (honeycombing). Cold joints in grout are not allowed except for test nails. Remove any temporary casings as grout is placed and record grout volume for each drill hole.

(4) **Nail Heads**

Install nail head assemblies after shotcreting. Before shotcrete reaches initial set, seat bearing plates and tighten nuts so plates contact shotcrete uniformly. If uniform contact is not possible, install nail head assemblies on mortar pads so nail heads are evenly loaded.

**(C) Drain Strips**

Install geocomposite drain strips as shown in the accepted submittals. Before installing shotcrete reinforcement, place drain strips with the geotextile side against excavation

faces. For highly irregular faces and at the discretion of the Engineer, drain strips may be placed after shotcreting over weep holes through the shotcrete. Hold drain strips in place with anchor pins so strips are in continuous contact with surfaces to which they are attached and allow for full flow the entire height of soil nail walls. Discontinuous drain strips are not allowed. If splices are needed, overlap drain strips at least 12" so flow is not impeded. Cut off excess drain strip length and expose strip ends below shotcrete when soil nail wall construction is complete.

**(D) Shotcrete**

Clean ungrouted zones of drill holes and excavation faces of loose materials, mud, rebound and other foreign material. Moisten surfaces to receive shotcrete. Install shotcrete reinforcement in accordance with the contract and accepted submittals. Secure reinforcing steel so shooting does not displace or vibrate reinforcement. Install approved thickness gauges on 5 ft centers in the horizontal and vertical directions to measure shotcrete thickness.

Apply shotcrete in accordance with the contract, accepted submittals and Subarticle 1002-3(F) of the *Standard Specifications*. Use approved shotcrete nozzlemen who made satisfactory preconstruction test panels to apply shotcrete. Direct shotcrete at right angles to excavation faces except when shooting around reinforcing steel. Rotate nozzle steadily in small circular patterns and apply shotcrete from bottom of lifts up.

Make shotcrete surfaces uniform and free of sloughing or sagging. Completely fill ungrouted zones of drill holes and any other voids with shotcrete. Taper construction joints to a thin edge over a horizontal distance of at least the shotcrete thickness. Wet joint surfaces before shooting adjacent sections.

Repair surface defects as soon as possible after shooting. Remove any shotcrete which lacks uniformity, exhibits segregation, honeycombing or lamination or contains any voids or sand pockets and replace with fresh shotcrete to the satisfaction of the Engineer. Protect shotcrete from freezing and rain until shotcrete reaches initial set.

**(E) Construction Records**

Provide 2 copies of soil nail wall construction records within 24 hours of completing each lift. Include the following in construction records:

- (1) Names of Soil Nail Wall Contractor, Superintendent, Nozzleman, Drill Rig Operator, Project Manager and Design Engineer;
- (2) Wall description, county, Department's contract, TIP and WBS element number;
- (3) Wall station and number and lift location, dimensions, elevations and description;
- (4) Nail locations, dimensions and inclinations, bar types, sizes and grades and temporary casing information;
- (5) Date and time drilling begins and ends, steel bars are inserted into drill holes, grout and shotcrete are mixed and arrives on-site and grout placement and shotcrete application begins and ends;
- (6) Grout volume, temperature, flow and density records;

- (7) Ground and surface water conditions and elevations if applicable;
- (8) Weather conditions including air temperature at time of grout placement and shotcrete application; and
- (9) All other pertinent details related to soil nail wall construction.

After completing each soil nail wall or stage of a wall, provide a PDF copy of all corresponding construction records.

### **Nail Testing**

“Proof tests” are performed on nails incorporated into walls, i.e., production nails. Define “test nail” as a nail tested with a proof test. Proof tests are typically required for at least one nail per nail row per soil nail wall or at least 5% of production nails, whichever is greater. More or less test nails may be required depending on subsurface conditions encountered. The Engineer will determine the number and locations of proof tests required. Do not test nails until grout and shotcrete attain the required 3 day compressive strength.

#### **(A) Test Equipment**

Use the following equipment to test nails:

- (1) Two dial gauges with rigid supports,
- (2) Hydraulic jack and pressure gauge and
- (3) Jacking block or reaction frame.

Provide dial gauges with enough range and precision to measure the maximum test nail movement to 0.001". Use pressure gauges graduated in 100 psi increments or less. Submit identification numbers and calibration records for load cells, jacks and pressure gauges with the soil nail wall design. Calibrate each jack and pressure gauge as a unit.

Align test equipment to uniformly and evenly load test nails. Use a jacking block or reaction frame that does not damage or contact shotcrete within 3 ft of nail heads. Place dial gauges opposite each other on either side of test nails and align gauges within 5° of bar inclinations. Set up test equipment so resetting or repositioning equipment during nail testing is not needed.

#### **(B) Test Nails**

Test nails include both unbonded and bond lengths. Grout only bond lengths before nail testing. Provide unbonded and bond lengths of at least 3 ft and 10 ft, respectively.

Steel bars for production nails may be overstressed under higher test nail loads. If necessary, use larger size or higher grade bars with more capacity for test nails instead of shortening bond lengths to less than the minimum required.

#### **(C) Proof Tests**

Determine maximum bond length ( $L_B$ ) using the following:

$$L_B \leq (C_{RT} \times A_t \times f_y) / (Q_{ALL} \times 1.5)$$

Where,

$L_B$  = bond length (ft),

- $C_{RT}$  = reduction coefficient, 0.9 for Grade 60 and 75 bars or 0.8 for Grade 150 bars,  
 $A_t$  = bar area (in<sup>2</sup>),  
 $f_y$  = bar yield stress (ksi) and  
 $Q_{ALL}$  = allowable unit grout/ground bond strength (kips/ft).

Determine design test load (DTL) based on as-built bond length and allowable unit grout/ground bond strength using the following:

$$DTL = L_B \times Q_{ALL}$$

Where,

DTL = design test load (kips).

Perform proof tests by incrementally loading nails to failure or a load of 150% of DTL based on the following schedule:

| Load     | Hold Time                     |
|----------|-------------------------------|
| AL*      | Until movement stabilizes     |
| 0.25 DTL | Until movement stabilizes     |
| 0.50 DTL | Until movement stabilizes     |
| 0.75 DTL | Until movement stabilizes     |
| 1.00 DTL | Until movement stabilizes     |
| 1.25 DTL | Until movement stabilizes     |
| 1.50 DTL | 10 or 60 minutes (creep test) |
| AL*      | 1 minute                      |

\* Alignment load (AL) is the minimum load needed to align test equipment and should not exceed 0.05 DTL.

Reset dial gauges to zero after applying alignment load. Record test nail movement at each load increment and monitor test nails for creep at the 1.5 DTL load increment. Measure and record movement during creep test at 1, 2, 3, 5, 6 and 10 minutes. If test nail movement between 1 and 10 minutes is greater than 0.04", maintain the 1.5 DTL load increment for an additional 50 minutes and record movement at 20, 30, 50 and 60 minutes. Repump jack as needed to maintain load during hold times.

#### (D) Test Nail Acceptance

Submit 2 copies of test nail records including load versus movement and time versus creep movement plots within 24 hours of completing each proof test. The Engineer will review the test nail records to determine if test nails are acceptable. Test nail acceptance is based in part on the following criteria.

- (1) Total movement during creep test is less than 0.04" between the 1 and 10 minute readings or less than 0.08" between the 6 and 60 minute readings and creep rate is linear or decreasing throughout hold time.
- (2) Total movement at maximum load exceeds 80% of the theoretical elastic elongation of the unbonded length.
- (3) Pullout failure does not occur at or before the 1.5 DTL load increment. Define "pullout failure" as the inability to increase load while movement continues.

Record pullout failure load as part of test nail data.

Maintain stability of unbonded lengths for subsequent grouting. If a test nail is accepted but the unbonded length cannot be satisfactorily grouted, do not incorporate the test nail into the soil nail wall and add another production nail to replace the test nail.

If the Engineer determines a test nail is unacceptable, either perform additional proof tests on adjacent production nails or revise the soil nail design or installation methods for the production nails represented by the unacceptable test nail as determined by the Engineer. Submit a revised soil nail wall design for acceptance, provide an acceptable test nail with the revised design or installation methods and install additional production nails for the nails represented by the unacceptable test nail.

After completing nail testing for each soil nail wall or stage of a wall, provide a PDF copy of all corresponding test nail records.

### **Measurement and Payment**

Temporary soil nail walls will be measured and paid in square feet. Temporary soil nail walls will be paid for at the contract unit price for *Temporary Shoring*. Temporary soil nail walls will be measured as the square feet of exposed wall face area. No measurement will be made for any embedment or pavement thickness above soil nail walls.

The contract unit price for *Temporary Shoring* will be full compensation for providing soil nail wall designs, submittals, labor, tools, equipment and soil nail wall materials, excavating, hauling and removing excavated materials, installing and testing soil nails, grouting, shotcreting and supplying drain strips and any incidentals necessary to construct soil nail walls. No additional payment will be made and no extension of completion date or time will be allowed for repairing property damage, overexcavations or unstable excavations, unacceptable test nails or thicker shotcrete.