U: 15-4841	•
10 EC 1: 58611.1.1	

DRAWN BY: _ C.R. SUMNER

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-		COUNTY WAYNE

RESTIGATION

_ F.A. PROJ. <u>BRZ-1227(4)</u> PROJECT DESCRIPTION BRIDGE NO. 215 ON SR 1227 (OAKLAND CHURCH RD.) OVER BEAVERDAM CREEK AT STA. 12+46.5

SINIE	SIALE P	CORCT REFERENCE NO.	N	<u> </u>	SHEE	
N.C.	38611	l.1.1 (B-4841)	1		8	
STATE	PROJ. NO.	F.A. PROJ. NO.	DES	RIP	TION	
386	11,1,1	BRZ-1227(4)		P.E.		
			RW	&	UTIL.	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 250-0438. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOLI TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A CEMERAL SOL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A CEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSIFIERCE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACETEST DATA CAN BE RELIED ON ONLY TO THE DECREE OF RELIED IN ANDLY TO STREED THE SOURCE OF RELIED IN THE SUBSURFACE INTESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLIDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN OETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CURRANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE. NOR THE INTERPRETATIONS MADE. OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH NOBERIDERY SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THE OCCUPANT CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THE ACTUAL CONDITIONS.

PERSUNNEL
JRS
CMW
RES
JME

INVESTIGATED BY J.R. SWARTLEY

D.N. ARGENBRIGHT

SUBMITTED BY D.N. ARGENBRIGHT

AUGUST 2010



NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSCIEND TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

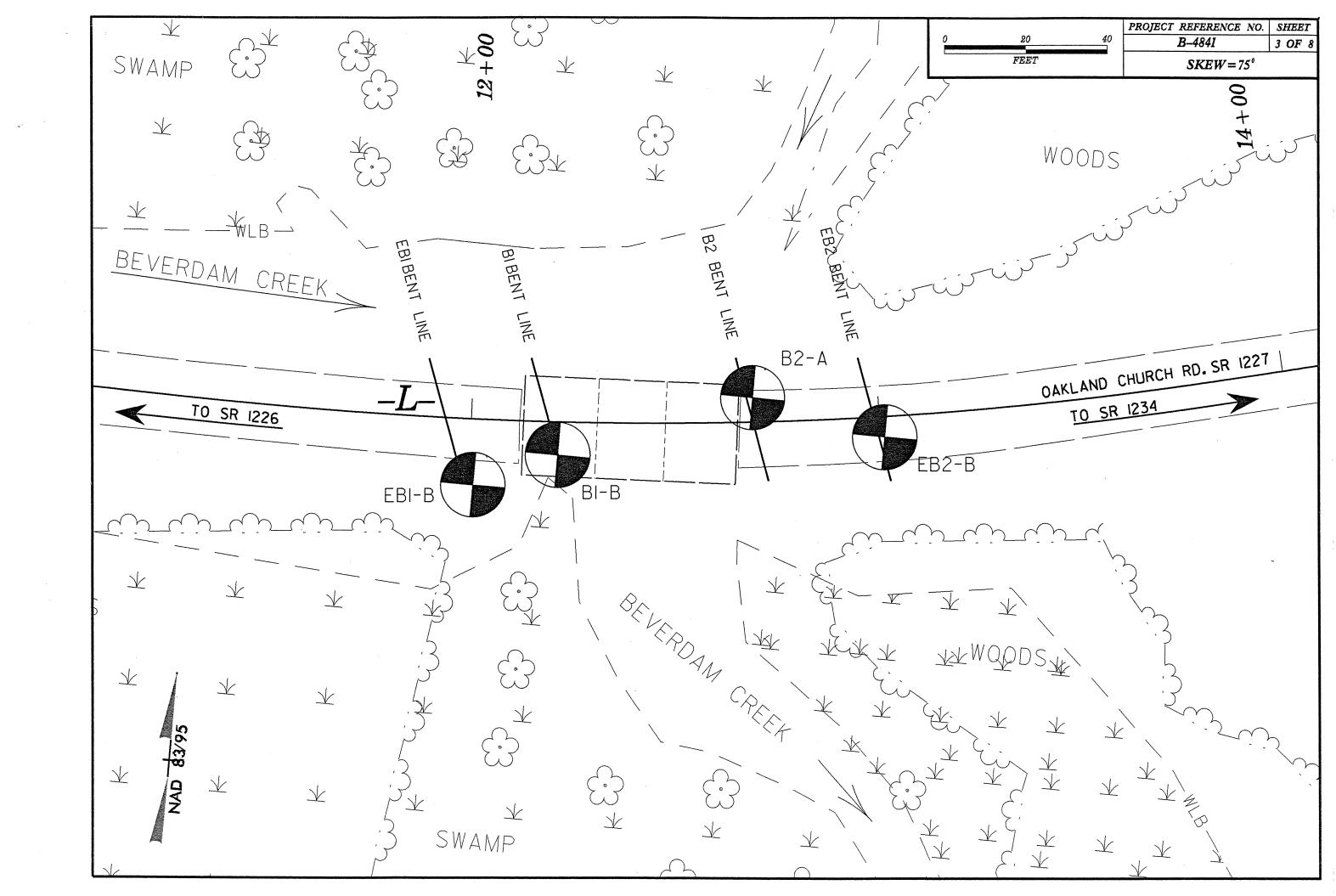
GEOTECHNICAL ENGINEERING UNIT

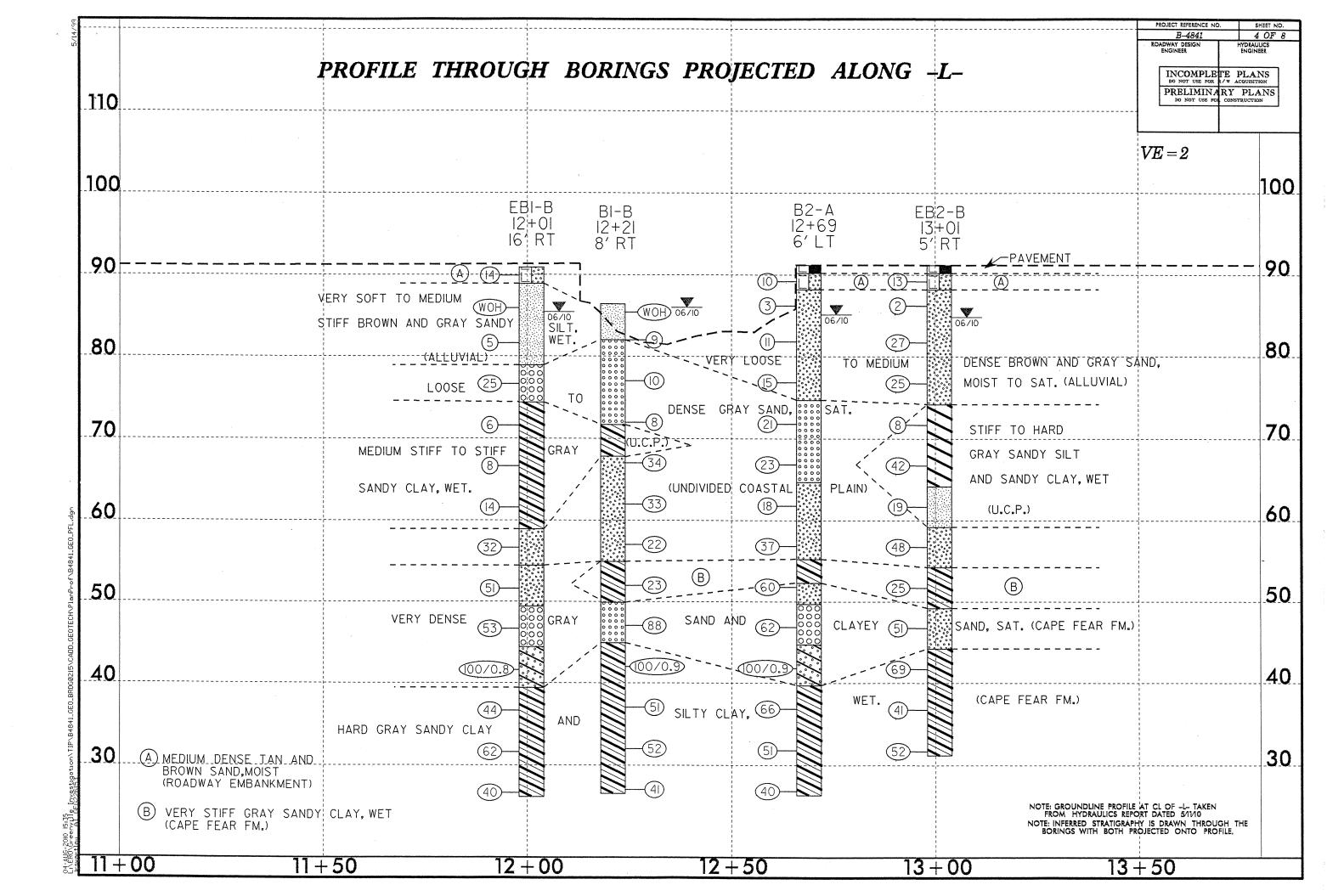
SUBSURFACE INVESTIGATION

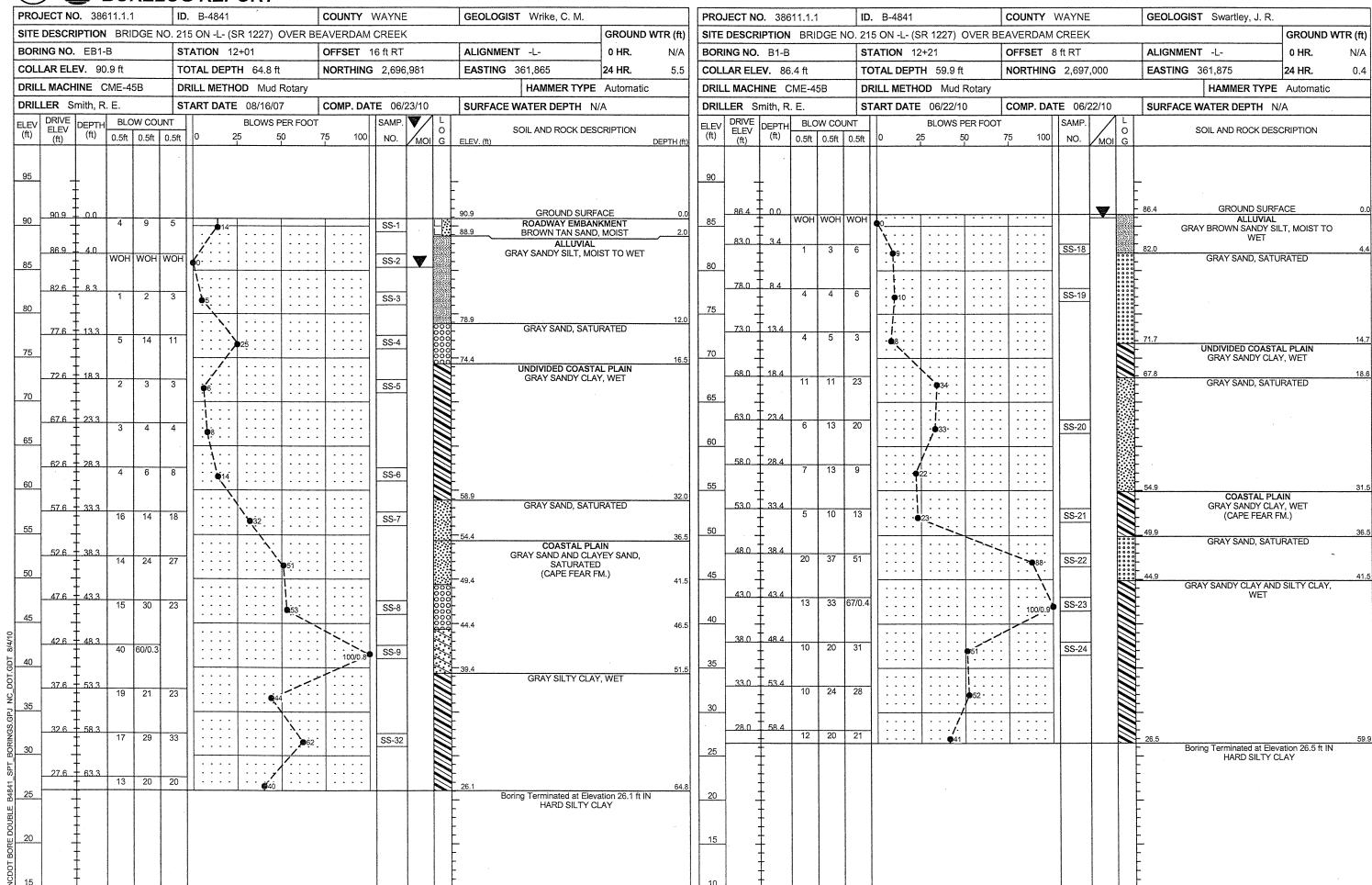
	SOIL AND ROCK	LEGEND, TERMS	, SYMBOLS, AND	ABBREVIATIONS	
SOIL DESCRIPTION	GRADATION			ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POMER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ARSHTD T206, ASTM D-1506). SOIL CLASSIFICATION IS BASED ON THE ARSHTD SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, ARSHTD CLASSIFICATION, AND OTHER PERTIMENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: VEW STRF, SAW, SUIT OUR, MOST WITH WIERBEDDED FAE SAMO LACERSHOW PLASTIC, A-7-6	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAM POORLY GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO DR MORE ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDRESS OF SOIL GRAINS IS DESIGNATED BY THE TERM: SUBANGULAR, SUBROUNDED, OR ROUNDED.	SIZES. (ALSO SIZES. SIS: ANGULAR,	ROCK LINE INDICATES THE LEVEL SPT REFUSAL IS PENETRATION BY	N MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL, A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 E . THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY DIVIDED AS FOLLOWS: NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 10	A ZONE ARENACEDUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARENACEDUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS. ARENACEDUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS.
SOIL LEGEND AND AASHTO CLASSIFICATION GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS OPERAND MATERIALS	MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED	IN DECEMBRIDA	ROCK (WR)	BLOWS PER FOOT IF TESTED. FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) CRONDE PRICEINGLES GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-2-6 A-3 A-6, A-7	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS		NON-CRYSTALLINE ROCK (NCR)	ENEISS, GABBRO, SCHIST, ETC. FINE TO CORREE GRAIN METAMORPHIC AND NON-CDASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. TYPE COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL COORDOODS STATE OF THE S	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUA HIGHLY COMPRESSIBLE LIQUID LIMIT GREA	AL TO 31-50 ATER THAN 50	CDASTAL PLAIN SEDIMENTARY ROCK	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS. RTC.	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
2 PASSING		ER MATERIAL		WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
40 38 MX 58 MX 51 MN 25 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE	1 - 10%	FRESH ROCK FRESH, CRYSTAL: HAMMER IF CRYSTALL:	S BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER INE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
LIGUID LIMIT PLASTIC INDEX 6 MX NP 18 MX 18 MX 11 MN 12 MX 12 MX 13 MN 46 MX 41 MN SOILS WITH FROMP INDEX 8 MX NP 18 MX 18 MX 11 MN 11 MN 12 MX 18 MX 13 MN 13 MX				SH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF O EN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS STUBE.	EN. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF
USUAL TYPES STONE FRAGS. FINE SILTY OR CLAYEY SILTY CLAYEY ORGANIC DF MAJOR GRAVEL AND CAND SPANE SOLLS SOLLS ORDER MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILL		SLIGHT ROCK GENERALLY FRE (SLI.) 1 INCH. OPEN JOINTS	SH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
GENERALIS SAND GENERATING AS A FYCELLENT TO GOOD FAIR TO POOD FAIR TO POOD INCULTA	STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING S'		MODERATE SIGNIFICANT PORTIONS (MOD.) GRANITOID ROCKS, MOS	S OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN ST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
PI DF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP		WITH FRESH ROCK.	AMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARE	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS		SEVERE AND DISCOLORED AND	ARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DL A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRE	L I
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH	RDADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SPT DMT TEST BORING SPT DMT TEST BORING	TEST BORING W/ CORE	IF TESTED, WOULD YIE		JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS DCCURRED.
GENERALLY	SOIL SYMBOL AUGER BORING		(SEV.) IN STRENGTH TO STRE	ARTZ DISCOLORED OR STAINED ROCK FABRIC CLEAR AND EVIDENT BUT F DNG SOIL, IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SO ENTS OF STRONG ROCK USUALLY REMAIN.	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
MATERIAL MILDION DENNE 18 TO 30 N/A (NON-COHESIVE) DENNE 38 TO 58 VERY DENSE >50 VERY SOFT <2 <8.25	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY MMONITORING WELL PIEZOMETER PIEZOMETER		VERY SEVERE ALL ROCK EXCEPT QUI (V SEV.) THE MASS IS EFFECT! REMAINING. SAPROLITE	<u>PT N VALUES > 100 BPF</u> ARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBL VELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROC I SIS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY GINAL ROCK FABRIC REMAIN. <u>IF TESTED, YIELDS SPT N VALUES < 100</u>	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. INDR PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN
SER-HALLY SUFF 2 10 4 0.25 TO 0.50	INSTALLATION TTTTC ALLUVIAL SOIL BOUNDARY SLOPE INDICATOR INSTALLATION 25/825 DIP & DIP DIRECTION OF		COMPLETE ROCK REDUCED TO SOI	IL ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE DNLY IN SMALL ANI ATIONS. DUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES (A) CONE PENETROMETER	RIEST	VERY HARD CANNOT BE SCRATCH	ROCK HARDNESS ED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 DPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	SOUNDING ROD ABBREVIATIONS		SEVERAL HARD BLOW HARD CAN BE SCRATCHED	S OF THE GEOLOGIST'S PICK. BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REDUIF	PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL
BOULDER COBBLE (CDB.) GRAVEL (GR.) COARSE (FINE SAND SAND SAND (CDB.) CLAY (CDL.) (BLDR.) (CDB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	AR - AUGER REFUSAL HI HIGHLY BT - BORING TERMINATED MED MEDIUM CL CLAY MICA MICACEDUS	W MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST VST - VANE SHEAR TEST M M M M M M M M M M M M M M M M M M M		ECIMEN. BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE BLOW OF A GEDLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR
GRAIN MM 305 75 2.0 0.25 0.005 0.005	CPT - COME PENETRATION TEST MOD MODERATELY CSE COARSE NP - NON PLASTIC DMT - DILATOMETER TEST ORG ORGANIC	WEA WEATHERED 7 - UNIT WEIGHT 7 - DRY UNIT WEIGHT	BY MODERATE BLOWS MEDIUM CAN BE GROOVED OR		
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST - VOID RATIO SAP SAPROLITIC F - FINE SD SAND, SANDY	1		IST'S PICK. GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS ERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL,TI	THAN 6.1 FOOT PER 60 BLOWS. N STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABL LL LIQUID LIMIT	FOSS FOSSILIFEROUS SL SILT, SILTY FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL		PIECES CAN BE BROWN VERY CAN BE CARVED WITH	KEN BY FINGER PRESSURE. H KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 IN SS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY	H STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY
PLASTIC SEMISOLID: REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PRO	JECT	FINGERNAIL. FRACTURE SPACI	NG BEDDING	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER
(PI) PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE		HAMMER TYPE:	TERM SP	ACING TERM THICKNESS	BENCH MARK: -BL- 101
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTUR SL SHRINKAGE LIMIT	MOBILE B- CLAY BITS	X AUTOMATIC MANUAL	VERY WIDE MORE THE WIDE 3 TO 10 MODERATELY CLOSE 1 TO 3 F	##N 10 FEE1	N 604571.8140 E 2263597.3930 ELEVATION: 103.87 FT.
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE		CORE SIZE:	CLOSE Ø.16 TO		NOTES:
PLASTICITY PLASTICITY INDEX (PI)	X CME-45B HARD FACED FINGER BITS	¬-N	FOR SEDIMENTARY ROCKS, INDURATION	INDURATION 1 S THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE,	TC.
PLASTICITY INDEX (PI) DRY STRENGTH NONPLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS]-+ [*]	FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM	I XI CASING I W ADVANCER	HAND TOOLS: POST HOLE DIGGER		GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
HIGH PLASTICITY 26 OR MORE HIGH COLOR	TRICONE TUNGCARB.	HAND AUGER	MODERATELY INDURATED	BREAKS EASILY WHEN HIT WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT	SOUNDING ROD	INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		VANE SHEAR TEST	EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	

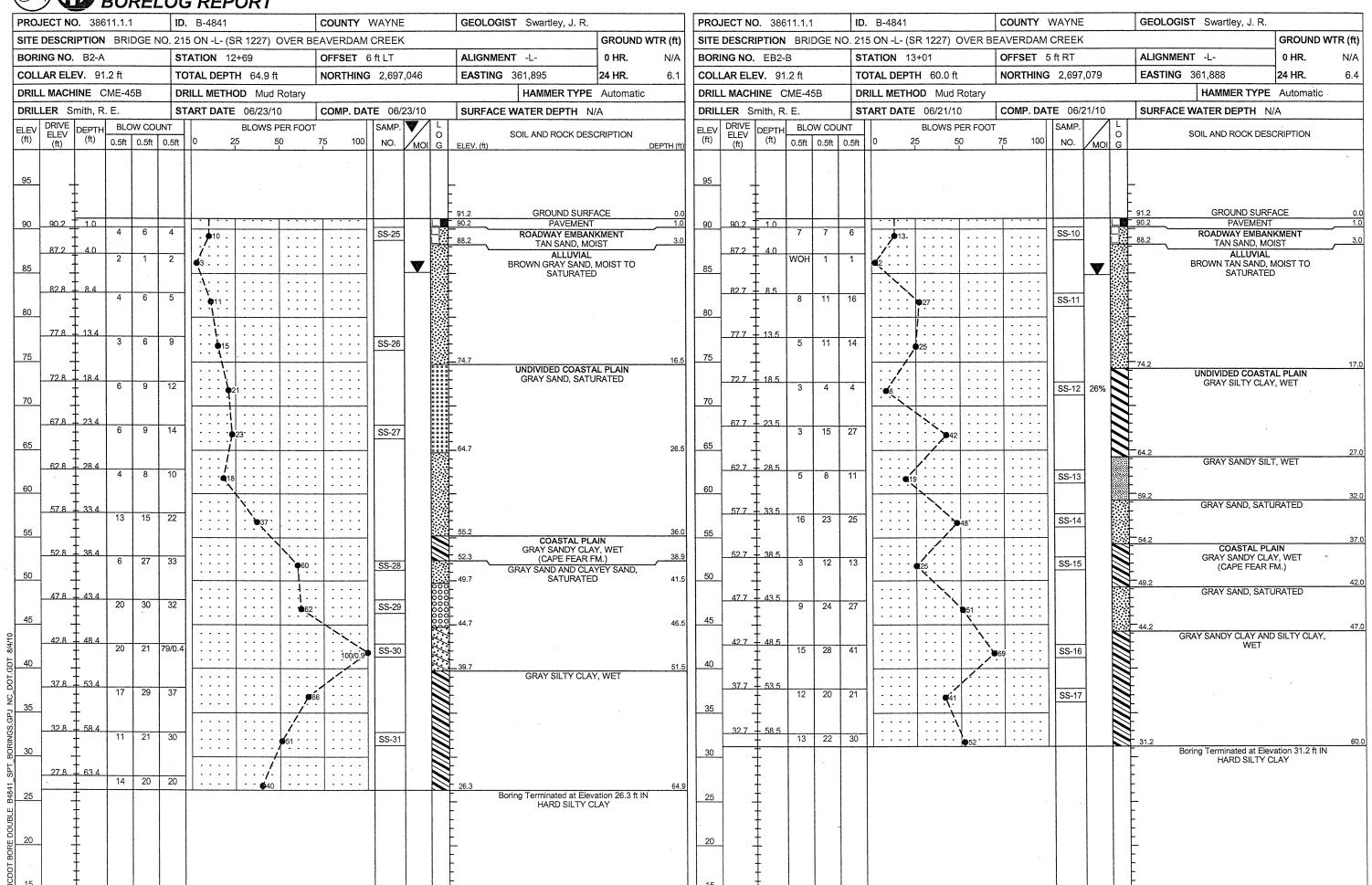
PROJECT REFERENCE NO. B-484I

SHEET NO. 2 OF 8









B-4841 38611.1.1

BRIDGE NO. 215 ON SR 1227 OVER BEAVERDAM CREEK

				SOIL T	ES	T	RESU	JLTS	EB1	-B					
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	C.SAND		WEIGHT	CLAY	% PA	88ING (8	IEVES)	MOISTURE	ORGANIC
SS- 1	16 RT	12+01	1. 0- 1. 5	A- 2- 4(0)	16	2	56. 6	22. 9	8. 4	12. 1	94	60	22	-	-
SS-2	16 RT	12+01	4.0-5.5	A- 4(0)	27	8	27.9	33. 9	24.0	14. 1	100	86	41	-	-
SS-3	16 RT	12+01	8. 3- 9. 8	A- 4(0)	20	6	21.0	43.0	19.8	16. 2	90	79	39	-	-
SS- 4	16 RT	12+01	13. 3- 14. 8	A- 1- b(0)	22	NP	74.8	19.6	3. 5	2.0	97	47	7	-	-
SS- 5	16 RT	12+01	18. 3- 19. 8	A-6(12)	33	16	0.8	34.9	36.0	28.3	100	99	81	-	
SS- 6	16 RT	12+01	28. 3- 29. 8	A-6(2)	28	12	0.4	62. 2	9. 1	28. 3	100	100	48	-	-
\$\$-7	16 RT	12+01	33. 3- 34. 8	A- 2- 4(0)	23	NP	6.4	80. 3	6.3	7.1	100	100	17	-	-
SS-8	16 RT	12+01	43. 3- 44. 8	A- 1- b(0)	16	NP	70.0	18. 3	5.7	6. 1	54	25	7	-	-
55-9	16 RT	12+01	48. 3- 49. 1	A- 2-6(0)	31	17	51.5	16.8	11.5	20.2	59	35	20	-	-
SS- 32	16 RT	12+01	58. 3- 59. 8	A-6(10)	28	12	1. 2	11.5	44.8	42. 4	100	99	95	-	-

				SOIL TEST RESULTS B1-B						\overline{B}	,				
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P,I,	C.SAND	% BY	WEIGHT	CLAY	% PAS	SING (8	IEVES)	MOISTURE	% ORGANIC
SS- 18	8 RT	12+21	3, 4-4, 4	A-4(1)	24	10	18.8	39.4	17.6	24.2	100	92	47	-	-
SS- 19	8 RT	12+21	8. 4- 9. 9	A- 3(0)	18	NP	66. 3	25.9	5. 9	2.0	99	65	9	-	-
SS- 20	8 RT	12+21	23. 4- 24. 9	A- 2- 4(0)	23	NP	12. 3	78.0	7.7	2.0	100	99	11	-	-
55-21	8 RT	12+21	<i>33. 4- 34. 9</i>	A-6(8)	28	16	5. 3	31.9	22. 4	40.4	100	100	70	•	-
55-22	8 RT	12+21	38. 4- 39. 9	A- 3(0)	19	NP	74.7	16.8	6.5	2.0	88	54	9	-	-
SS- 23	8 RT	12+21	43. 4- 44. 8	A-6(3)	28	15	18.6	15. 4	27.7	38. 4	65	56	46	-	-
SS- 24	8 RT	12+21	48. 4- 49. 9	A-6(16)	33	18	1. 2	9.3	41.0	48.5	99	98	95	-	-

				SOIL	TES	\overline{ST}	RES	\overline{ULTS}	B2-	B					
SAMPLE	OFFSET	STATION	DEPTH	AASHTO	L.L.	P.I.			WEIGHT		% PAS	SSING (S	IEVES)	%	%
NO.			INTERVAL	CLASS.		1	C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
SS- 25	6 LT	12+69	1.0-2.5	A- 2- 4(0)	20	5	53.7	25. 9	6.3	14. 1	98	67	22	-	-
SS- 26	6 LT	12+69	13. 4- 14. 9	A- 1- b(0)	19	NP	75.6	19. 4	5. 1	0.0	97	49	6	-	-
SS- 27	6 LT	12+69	23. 4- 24. 9	A- 3(0)	23	NP	50. 1	43.8	6. 1	0.0	100	77	7	-	-
SS- 28	6 LT	12+69	38. 9- 39. 9	A-2-4(0)	19	NP	29.3	52. 5	8. 1	10. 1	100	98	20	-	-
SS- 29	6 LT	12+69	43. 4- 44. 9	A- 1- b(0)	18	NP	66. 1	25. 3	6.7	2.0	67	35	7	-	-
SS- 30	6 LT	12+69	48. 4- 49. 8	A-2-6(1)	26	14	34. 5	16.6	20.6	28. 3	61	44	33	-	-
SS- 31	6 IT	12+69	58 4-59 9	A-6(12)	32	16	8 3	13. 1	32.1	46. 5	99	94	8.3	-	-

	SOIL TEST RESULTS EB2-B														
SAMPLE	OFFSET	STATION	DEPTH	AASHTO	L.L.	P.I.			WEIGHT		% PAS	· · · · · · · · · · · · · · · · · · ·	IEVES)	%	%
NO.			INTERVAL	CLASS.	1		C.SAND	F.SAND	SILT	CLAY	10	40	200	MOISTURE	ORGANIC
S\$- 10	5 RT	13+01	1.0-2.5	A-2-4(0)	18	2	40.8	38.8	10.3	10. 1	91	68	21	-	-
S\$-11	5 RT	13+01	8. 5- 10. 0	A- 2- 4(0)	19	NP	50. 9	37.6	5. 5	6. 1	76	53	11	-	-
S\$- 12	5 RT	13+01	18. 5- 20. 0	A-7-6(34)	52	35	2.6	6.3	36.6	54.5	96	94	91	26. 2	
55- 13	5 RT	13+01	28. 5- 30. 0	A- 4(0)	23	7	0.8	66.7	8.3	24.2	100	100	38		-
SS- 14	5 RT	13+01	33. 5- 35. 0	A- 2- 4(0)	22	NP	10.7	78.4	6.9	4.0	100	99	13	-	-
SS- 15	5 RT	13+01	38. 5- 40. 0	A-6(5)	27	16	9.7	38. 2	15.8	36. 4	100	99	56	-	-
SS- 16	5 RT	13+01	48. 5- 50. 0	A-6(2)	29	17	37.6	12.9	19.2	30. 3	67	45	36	-	-
SS- 17	5 RT	13+01	53. 5- 55. O	A-6(17)	34	19	1.6	8.9	41.0	48.5	98	97	93	-	-

FIELD SCOUR REPORT

WBS:	38611.1.1	TIP:	B-4841	COU	INTY: WAYNE		
DESCRIPTION(1):	BRIDGE NO. 2	15 ON SR ⁻	1227 OVER BEA	VERDAM	CREEK		
			EXISTING	BRIDGE			
Information from:		nspection _ (explain) _	X Micr	ofilm	_ (reel po	os:)
Bridge No.: _ 2 Foundation Type: ַ	215 Length TIMBER PILES	:53'	Total Bents:4	Bents	in Channel: 2	Bents in	Floodplain: 2
EVIDENCE OF S Abutments or E		: NONE NO	OTED				
Interior Bents:	NONE NOTED						
Channel Bed:	NONE NOTED	· · · · · · · · · · · · · · · · · · ·					
Channel Bank:	NONE NOTED						
EXISTING SCOL Type(3):	IR PROTECTIC WOODEN WIN						
Extent(4):	6' OUTSIDE ED	GE OF BF	RIDGE		**************************************		
Effectiveness(5):	EFFECTIVE		•				
Obstructions(6):	NONE			·			

INSTRUCTIONS

- 1 Describe the specific site's location, including route number and body of water crossed.
- 2 Note scour evidence at existing end bents or abutments (e.g. undermining, sloughing, degradations).
- 3 Note existing scour protection (e.g. rip rap).
- 4 Describe extent of existing scour protection.
- 5 Describe whether or not the scour protection appears to be working.
- 6 Note obstructions such as dams, fallen trees, debris at bents, etc.
- Describe the channel bed material based on observation and/or samples. Include any lab results with report.
- Describe the channel bank material based on observation and/or samples. Include any lab results with report.
- 9 Describe the material covering the banks (e.g. grass, trees, rip rap, none).
- 10 Determine the approximate floodplain width from field observation or a topographic map.
- 11 Describe the material covering the floodplain (e.g. grass, trees, crops).
- 12 Use professional judgement to specify if the stream is degrading, aggrading, or static.
- 13 Describe potential and direction of the stream to migrate laterally during the bridge's life (approx. 100 years).
- 14 Give the design scour elevation (DSE) expected over the life of the bridge (approx. 100 years). This elevation can be given as a range across the site, or for each bent. Discuss the relationship between the Hydraulics Unit theoritical scour and the DSE. If the DSE is dependent on scour counter measures, explain (e.g. rip rap armoring on slopes). The DSE is based on the erodability of materials, giving consideration to the influence of joints, foliation, bedding characteristics, % core recovery, % RQD, differential weathering, shear strength, observations at existing structures, other tests deemed appropriate, and overall geologic conditions at the site.

			DES	SIGN IN	FORM	ATIO	<u>1</u>				
Channel Bed Ma	aterial(7):	GRAY A	ND BRO	NN SAI	NDY SIL	r and	SAND			· · · · · · · · · · · · · · · · · · ·	
Channel Bank Ma	aterial(8): ַ	GRAY A	ND BRO	DWN SAI	NDY SIL	ΓAND	SAND				
Channel Bank	Cover(9):	TREES	AND SH	RUBS							
Floodplain V	Vidth(10):	750'						100-100-100	· ·		and the same of th
Floodplain C	over(11):	TREES	AND SH	IRUBS	andre green and a second a second and a second a second and a second a			.,			
Strea	am is(12):	Ag	grading	Management of the Park	Degra	ading _		Sta	itic X		
nannel Migration Tend	ency(13):	SLIGHTI	LYTOT	HE EAS	Т						
Observations and Oth	har Camm	ente:									
Observations and Otl	nei Comm										
											*
DESIGN SCOUR EL	EVATION	S(14)				Fe	et X	Mete	ers	-	
	BENTS										
	B1	B2	В3	B4							
	70	70	<u></u>	T T							
Comparison of DSE t	to Hydrauli	ics Unit tl	heoretic	al scour:				•			
THE GEOTECHNICA	AL UNIT A	GREES	WITH T	HE HYDI	RAULIC	UNIT'S	THEOR	ETICAL S	COUR		
										·····	
COU ANALYSIS DE	CIU TO FI	OM CU	ANNE	DED AN	DEANIZ	NAATE	DIAI				
Bed or Bank	SUL IS FI	KOW CH	ANNEL	DED AN	DEANN	IVIAIL	NIAL			Τ	
Sample No.			_								
Retained #4					<u> </u>						
Passed #10							-				
Passed #40							—				
Passed #200		── Se	e Sheet	7.							
Coarse Sand		1		Results",			-			1	
Fine Sand		1	sample							 	
Silt				-26 (CHA	NNEL B	ED)	<u> </u>				
Clay				-26 (CHA			<u> </u>			1	
LL			,			,	<u> </u>			1	
PI										†	
AASHTO		— <u> </u>									
Station					 		<u></u>				
Offset			_		 					†	
Depth					 					 	
20001					1		·····		<u> </u>	<u> </u>	
										Template F	Revised 02/07/0

Date: 8/3/2010