

**STATE OF NORTH CAROLINA**  
**DEPARTMENT OF TRANSPORTATION**  
**RAIL DIVISION**  
**GEOTECHNICAL ENGINEERING UNIT**



STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	P-5206A	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
52000.1.STR03T1B		P.E., UTIL P.E.	
52000.1.STR04T3		P.E., UTIL P.E.	
43219.2.STR08P5206		RW	
52000.3.STR01T4A		UTIL. CONST.	
52000.3.STR01T4A		CONST.	

# ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. P-5206A F.A. PROJ. \_\_\_\_\_  
 COUNTY ROWAN  
 PROJECT DESCRIPTION PEELER ROAD (SR 2538)/CEDAR SPRINGS ROAD (SR 1560)  
GRADE SEPARATION OVER NSNCRR AND US 29

**CONTENTS**

LINE	STATION	PLAN	PROFILE
-Y2-	11+50 TO 51+48	4-7	18, 19
-Y23-	10+00 TO 34+90	7, 9-11	20
-20-	10+00 TO 60+37	12-15	21-23
-Y21-	22+00 TO 30+50	6, 16	23, 24

**CROSS SECTIONS**

LINE	STATION	SHEET NO.
-Y2-	16+50 TO 33+00	25-33
-Y2-	34+50 TO 37+00	34, 35
-Y2-	38+50 TO 39+00	36
-Y2-	41+00 TO 41+50	37
-Y2-	46+50 TO 51+00	38-40
-Y20-	41+50 TO 56+91.07	41, 48
-Y20-	59+00 TO 60+00	49
-Y21-	20+50 TO 26+00	50-52
-Y23-	10+50 TO 12+00	53
-Y23-	14+00 TO 16+00	54
-Y23-	23+50 TO 27+00	55, 56

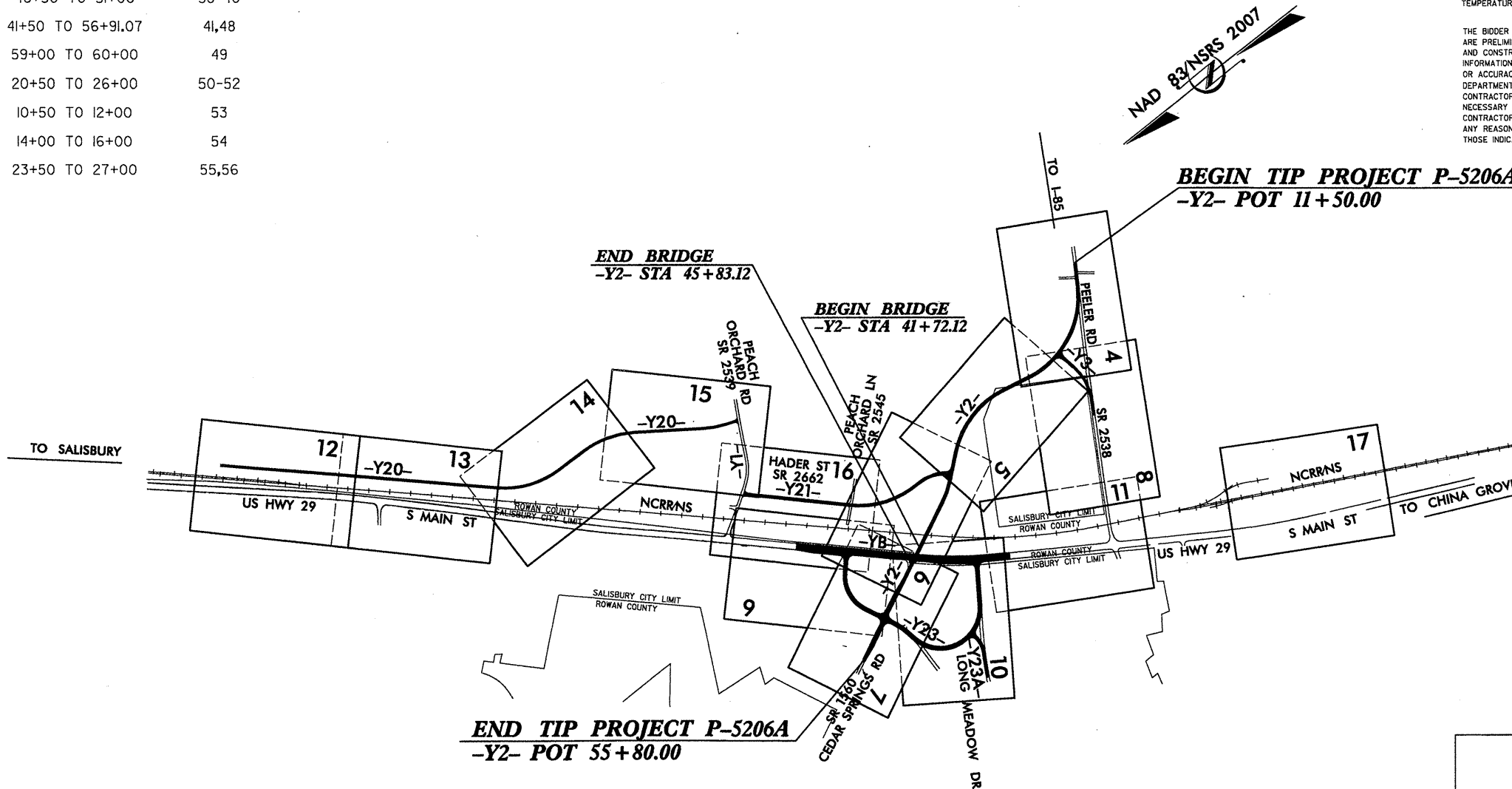
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THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

## INVENTORY



FLORENCE & HUTCHESON  
PERSONNEL

M. GRAGG

R. DELOST

F. WOODARD

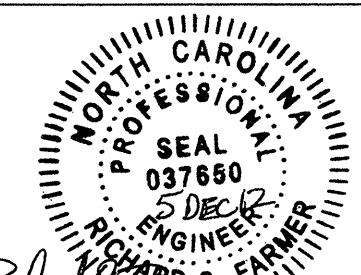
R. FARMER

INVESTIGATED BY R. DELOST

CHECKED BY R. FARMER

SUBMITTED BY R. FARMER

DATE OCTOBER 2012



*Richard S. Farmer*

**CONTRACT: C203143 ID: P-5206A**

DRAWN BY: W. SHUECRAFT/T. RIDEOUT

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION  
DIVISION OF HIGHWAYS  
GEOTECHNICAL ENGINEERING UNIT

## SUBSURFACE INVESTIGATION

### SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION										GRADATION										ROCK DESCRIPTION										TERMS AND DEFINITIONS									
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.										HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: WEATHERED ROCK (WR) - NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. CRYSTALLINE ROCK (CR) - FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. NON-CRYSTALLINE ROCK (NCR) - FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN SEDIMENTARY ROCK (CP) - COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.										ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SURFACES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSELE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOROUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.									
<b>SOIL LEGEND AND AASHTO CLASSIFICATION</b> GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS GROUP CLASS. A-1, A-2, A-3, A-4, A-5, A-6, A-7, A-1, A-2, A-3, A-4, A-5, A-6, A-7 SYMBOL [Diagrams showing soil patterns for A-1, A-2, A-3, A-4, A-5, A-6, A-7, and organic materials] % PASSING: 10, 40, 200 LIQUID LIMIT PLASTIC INDEX GROUP INDEX USUAL TYPES OF MAJOR MATERIALS: STONE FRAGS, GRAVEL, AND SAND; FINE SAND; SILTY OR CLAYEY GRAVEL AND SAND; SILTY SOILS; CLAYEY SOILS GEN. RATING AS A SUBGRADE: EXCELLENT TO GOOD; FAIR TO POOR; FAIR TO POOR; POOR; UNSUITABLE PI OF A-7-5 SUBGROUP IS <= LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30										<b>MINERALOGICAL COMPOSITION</b> MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE. <b>COMPRESSIBILITY</b> SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50 <b>PERCENTAGE OF MATERIAL</b> ORGANIC MATERIAL GRANULAR SOILS SILT-CLAY SOILS OTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE										<b>WEATHERING</b> FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE. VERY SLIGHT (V SL.) ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. SLIGHT (SL.) ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. MODERATE (MOD.) SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK. MODERATELY SEVERE (MOD. SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL. SEVERE (SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF. VERY SEVERE (V SEV.) ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF. COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.																			
<b>CONSISTENCY OR DENSENESS</b> PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT <sup>2</sup> ) GENERALLY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE 4 TO 10 MEDIUM DENSE 10 TO 30 DENSE 30 TO 50 VERY DENSE >50 GENERALLY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT 2 TO 4 SOFT 4 TO 8 MEDIUM STIFF 8 TO 15 STIFF 15 TO 30 VERY STIFF >30										<b>MISCELLANEOUS SYMBOLS</b> ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SOIL SYMBOL ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT INFERRED SOIL BOUNDARY INFERRED ROCK LINE ALLUVIAL SOIL BOUNDARY DIP & DIP DIRECTION OF ROCK STRUCTURES INFERRED BASE ROADWAY EMBANKMENT SPT DMT VST PNT TEST BORING AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD										<b>ROCK HARDNESS</b> VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROUDED OR COUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.																			
<b>TEXTURE OR GRAIN SIZE</b> U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 BOULDER (BLDR.) COBBLE (COB.) GRAVEL (GR.) COARSE SAND (CSE, SD.) FINE SAND (F SD.) SILT (SL.) CLAY (CL.) GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005										<b>ABBREVIATIONS</b> AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FIAD - FILLED IN AFTER DRILLING FOSS - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HI - HIGHLY MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PMT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLL - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY VST - VANE SHEAR TEST WE. - WEATHERED W - UNIT WEIGHT Wd - DRY UNIT WEIGHT SAMPLE ABBREVIATIONS S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO										<b>ROCK HARDNESS</b> VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY HARD CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. MEDIUM HARD CAN BE GROUDED OR COUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROUDED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. VERY SOFT CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.																			
<b>SOIL MOISTURE - CORRELATION OF TERMS</b> SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION LL - LIQUID LIMIT PL - PLASTIC LIMIT OM - OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - SATURATED - (SAT) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE										<b>EQUIPMENT USED ON SUBJECT PROJECT</b> DRILL UNITS: MOBILE B-8K-51 CME-45C CME-550 PORTABLE HOIST ADVANCING TOOLS: CLAY BITS 6" CONTINUOUS FLIGHT AUGER 8" HOLLOW AUGERS HARD FACED FINGER BITS TUNG-CARBIDE INSERTS CASING W/ ADVANCER TRICONE STEEL TEETH TRICONE TUNG-CARB. CORE BIT 3/4" HOLLOW AUGERS HAMMER TYPE: AUTOMATIC MANUAL CORE SIZE: B N Q2 H HAND TOOLS: POST HOLE DIGGER HAND AUGER SOUNDING ROD VANE SHEAR TEST										<b>FRACTURE SPACING</b> TERM SPACING VERY WIDE MORE THAN 10 FEET WIDE 3 TO 10 FEET MODERATELY CLOSE 1 TO 3 FEET CLOSE 0.16 TO 1 FEET VERY CLOSE LESS THAN 0.16 FEET <b>BEDDING</b> TERM THICKNESS VERY THICKLY BEDDED > 4 FEET THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET																			
<b>PLASTICITY</b> NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH										<b>INDURATION</b> FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.										<b>BENCH MARK:</b> ELEVATION: FT. <b>NOTES:</b> POTENTIAL UNCLASSIFIED EXC. UNSUITABLE MATERIALS POTENTIAL UNDERCUT EXC.																			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.																																							

See Sheet 1-A For Index of Sheets  
See Sheet 1-B For Conventional Symbols

STATE OF NORTH CAROLINA  
NCDOT RAIL DIVISION

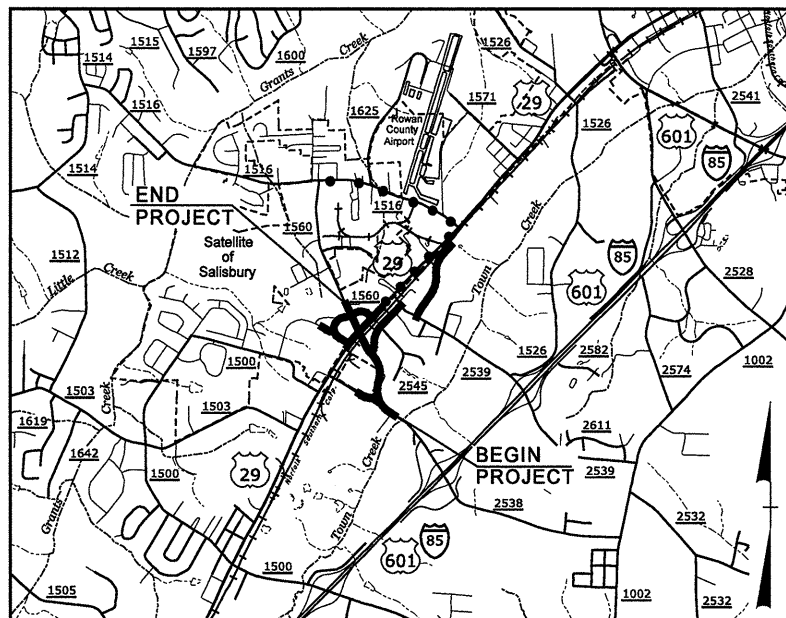


STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	P-5206A	2A	52
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
52000.1.STR03TIB		P.E.	

**ROWAN COUNTY**

**LOCATION: PEELER ROAD (SR 2538)/CEDAR SPRINGS ROAD (SR 1560)  
GRADE SEPARATION OVER NCRRS**

**TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND  
STRUCTURES**



**VICINITY MAP**

●●●●● OFF-SITE DETOUR ROUTE

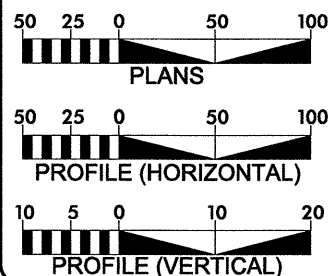
**TIP PROJECT: P-5206A**

**CONTRACT:**

**DESIGN DATA**

ADT 2009 = ?  
ADT 2035 = ?  
DHV = 8 %  
D = 55 %  
T = 4 % \*  
V = 50 MPH  
\* 2% TTST + 2% DUALS  
FUNC CLASS=LOCAL

**GRAPHIC SCALES**



A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF SALISBURY.  
CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD \_\_\_\_\_.

NDOT RAIL DIVISION CONTACT: SANDRA STEPNEY, PE

**PROJECT LENGTH**  
LENGTH RDWY (-Y2-) TIP PROJECT P-5206A = 0.761 MILES  
LENGTH STR (-Y2-) TIP PROJECT P-5206A = 0.078 MILES  
TOTAL LENGTH (-Y2-) TIP PROJECT P-5206A = 0.839 MILES

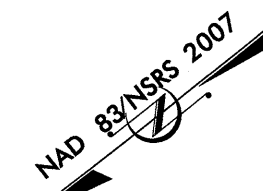
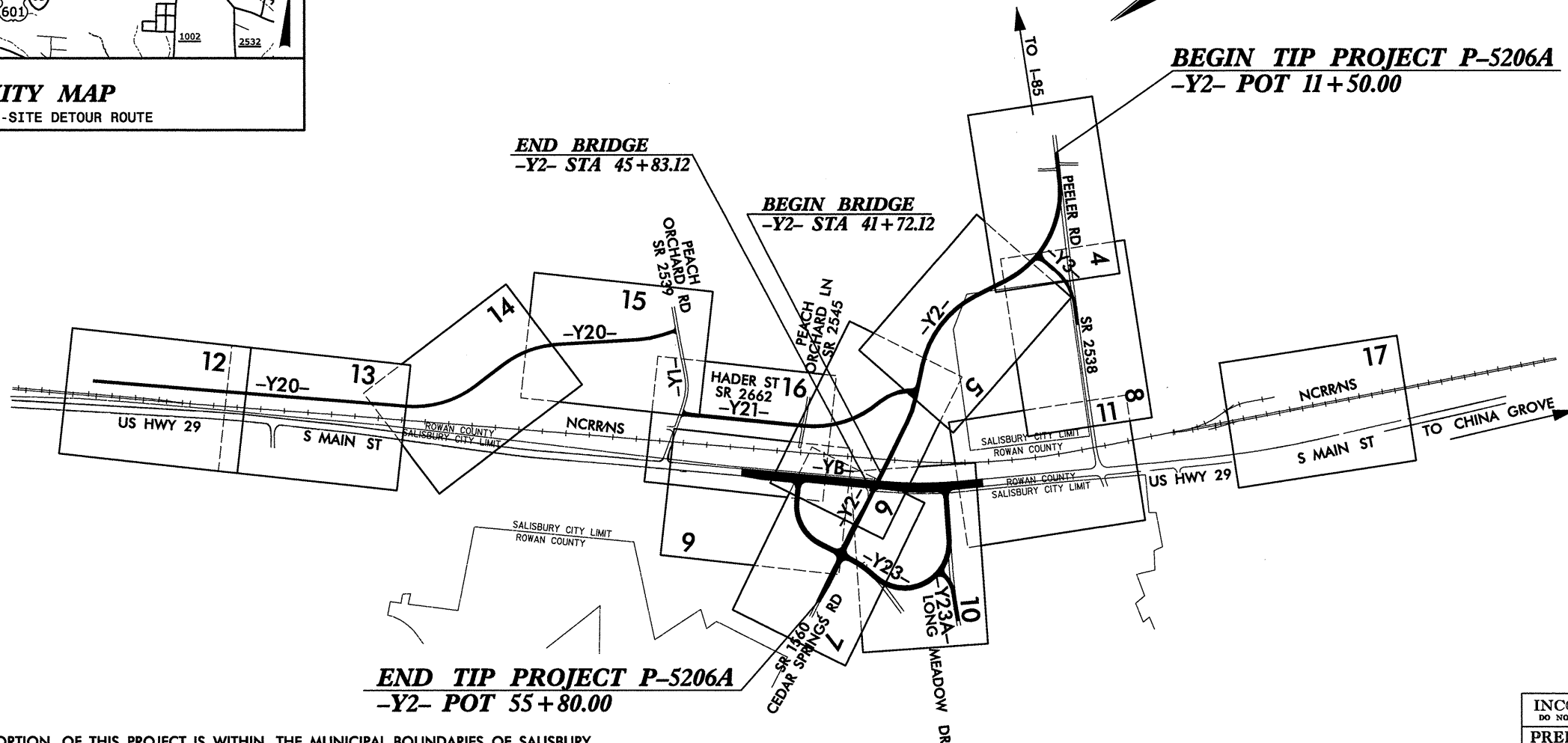
**SUBMITTAL: FINAL DESIGN FIELD  
INSPECTION PLANS**  
DATE: FEB. 17, 2012

**END BRIDGE**  
-Y2- STA 45+83.12

**BEGIN BRIDGE**  
-Y2- STA 41+72.12

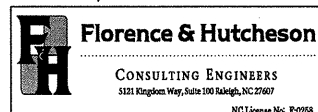
**END TIP PROJECT P-5206A**  
-Y2- POT 55+80.00

**BEGIN TIP PROJECT P-5206A**  
-Y2- POT 11+50.00



**INCOMPLETE PLANS**  
DO NOT USE FOR R/W ACQUISITION  
**PRELIMINARY PLANS**  
DO NOT USE FOR CONSTRUCTION

Prepared In the Office of:



**BRIAN A. WILES, PE**  
PROJECT ENGINEER

**LETTING DATE:**  
MARCH 19, 2013

**RIGHT OF WAY DATE:**  
MARCH 16, 2012

**ROADWAY ENGINEER**

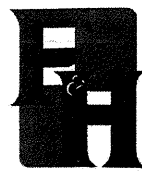
SIGNATURE: \_\_\_\_\_ P.E.

**HYDRAULICS  
ENGINEER**

SIGNATURE: \_\_\_\_\_ P.E.



12/5/2012 R:\geotech\investigation\Design\PlanPr of P5206A\_GEO\_inv\_001\_roadway\_title.dgn Florence & Hutcheson, Inc.



October 17, 2012 (REV December 4, 2012)

Sandra A. Stepney, PE, CPM  
Engineering and Safety Unit  
North Carolina Department of Transportation  
862 Capital Boulevard  
Raleigh, North Carolina 27603

Reference: **Roadway Inventory Report  
Peeler Road - Grade Separation Over  
North Carolina Railroad/  
Norfolk Southern Corporation  
From Reid to North Kannapolis  
Rowan County, North Carolina  
Project No. P-5206A  
F&H Project No. 11201**

Dear Ms. Stepney:

Florence & Hutcheson (F&H) has completed the authorized subsurface investigation and roadway inventory report for the above referenced project in Rowan County, North Carolina, and we submit the following information.

**PROJECT DESCRIPTION**

This project consists of the construction of a roadway bridge to separate the at-grade intersection of SR 2538 (-Y2-, Peeler Road) with North Carolina Railroad/Norfolk Southern Corporation railroad line (-YB-) in the town of Salisbury. The proposed bridge will carry the two lanes of the relocated Peeler Road over the single track of the NCR/NS Corp. and the four lanes of U.S. Highway 29/South Main Street. The project begins southeast of the intersection of Peeler Road and Southwood Road/Pleasant Place Road and extends northwest 0.85 miles to intersect with Cedar Springs Road (SR 1560) north of U.S. Highway 29/South Main Street.

The geotechnical field investigation was conducted between February and June 2012. An F&H drilling crew performed the investigation of the subsurface using a tracked ATV-mounted CME-45 drilling rig. An F&H geologist worked with the drilling crew to sample and log the borings. Standard Penetration Tests (SPT) were performed at regular intervals in the borings. Representative soil samples were collected for visual classification in the field and submitted for laboratory analysis by F&H's soils laboratory.

The following alignments, totaling 2.0 miles, were investigated. Subsurface soil profiles and cross-sections of these alignments are included in this report:

<u>Line</u>	<u>Station</u>
-Y2-	10+00 to 55+80
-Y20-	35+00 to 60+37
-Y21-	20+31 to 30+50

-Y23- 10+00 to 34+90

**AREAS OF SPECIAL GEOTECHNICAL INTEREST**

- 1) Occurrences of highly plastic silts and clays (Plasticity Index greater than 25) are noted in the following sections:

<u>Line</u>	<u>Station</u>
-Y2-	26+50 to 27+50
-Y20-	45+50 to 46+50
-Y21-	23+50 to 25+00

There is a possibility that areas of high plasticity soils near subgrade may exist elsewhere along the project.

- 2) Occurrences of soft cohesive soils which have the potential to cause embankment stability and/or long-term settlement problems are noted in the following sections:

<u>Line</u>	<u>Station</u>
-Y2-	38+00 to 41+50
-Y2-	46+00 to 49+50
-Y21-	27+50 to 30+50

- 3) Artificial fill was encountered between Station 43+25 to Station 46+50 (-Y2-) with up to 13.5 feet of silty clay, rock fragments, and gravel.

**PHYSIOGRAPHY AND GEOLOGY**

The various proposed alignments lie within the Charlotte Belt as defined within North Carolina. The Charlotte Belt contains plutonic rock, large areas of metavolcanic rock, and very few metasedimentary rocks.

Published mapping indicates the following lithologic units may be anticipated underlying the proposed alignments: undivided Metavolcanic rocks composed of felsic, mafic, or intermediate types, probably interbedded and may be correlative in some part to the Battleground Formation. Immediately west lie soils developed upon the Churchland Pluton (Permian and Pennsylvanian) primarily composed of porphyritic, biotite monzogranite but may also be fine-grained. Artificial fill is anticipated within small localized accumulations.

Intercepted rock units are anticipated to exhibit steeply dipping bedding and foliation throughout the proposed alignment. A bedding dip value of 70° NW and foliation dip of 80°WNW are indicated on published mapping near the proposed alignments.

**SOIL PROPERTIES**

Borings advanced during the subsurface investigation generally exhibited thick residual soils with penetration to 71.0 feet without Standard Penetration Test (SPT) sampler or auger refusal encountered. Penetrated soils were predominantly residual in origin, soft to stiff, silty clay with varying fractions of fine- to coarse-grain sand and limited fractions of rock fragments, mineral or

quartz veining, and root fragments near the surface. Within deeper advanced borings, layers of very soft to hard, very sandy silt and fine- to coarse-grain sand and silt in relatively equal fractions were intercepted. Plasticity characteristics varied from low to high within silty clays while silts and sand and silt layers generally exhibited low to non-plastic characteristics.

Penetration of artificial fill was interpreted at Station 43+25 to Station 46+50 with up to 13.5 feet of silty clay, rock fragments, and gravel. Artificial fill overlies previously described residual silty clay.

Weathered rock was encountered within one (1) boring, R-12, at a depth of 54.8 feet. This one instance of weathered rock encountered was 0.5 feet in thickness, and the boring terminated within the unit. Though not apparently typical, thin, weathered rock intervals may occur between the advanced borings

One (1) boring, R-5, reported auger refusal at 11.2 feet on crystalline rock. Crystalline rock may be intercepted within three (3) feet of proposed grade between the indicated boring locations, or, as at Station 25+00, throughout the proposed alignment profile. These two instances were the only indications of rock indicated by the advanced borings.

**ROCK PROPERTIES**

While crystalline rock apparently caused auger refusal within boring R-5, advancement of the remaining borings suggests that rock or weathered rock lies sufficiently below proposed cut excavations. As with weathered rock, however, crystalline rock or large rock boulders may occur between the advanced boring locations throughout the proposed alignment profiles.

**GROUNDWATER**

Groundwater was not encountered in the borings completed for this project. The subsurface investigation was conducted during the typically wetter Winter/early Spring season of the year. We would expect groundwater to vary in depth across the entire project site. However, based on our investigation, we do not anticipate that groundwater will be a problem during construction on this project.

**UNDISTURBED SAMPLES**

Undisturbed thin-wall Shelby tube samples were collected at the following locations and submitted for testing:

<u>Line</u>	<u>Station</u>	<u>Depth</u>	<u>Test</u>
-Y2-	21+00, 20' LT	15.0-16.5	Classification
-Y2-	27+00, 30' LT	5.0-5.5	Classification
-Y2-	27+00, 30' LT	20.0-21.8	Classification
-Y2-	31+00, 20' RT	4.9-5.6	Classification
-Y2-	39+00, 23' LT	9.8-10.3	Classification/Consolidation
-Y2-	41+00, 23' RT	19.5-20.6	Class./Unconf. Compress.
-Y2-	41+00, 23' RT	39.5-41.5	Classification
-Y2-	41+00, 23' RT	59.5-60.8	Classification
-Y2-	49+00, 35' RT	29.9-31.0	Classification
-Y20-	44+00, 13' RT	10.0-11.5	Classification
-Y20-	46+00, 26' RT	5.0-6.2	Classification

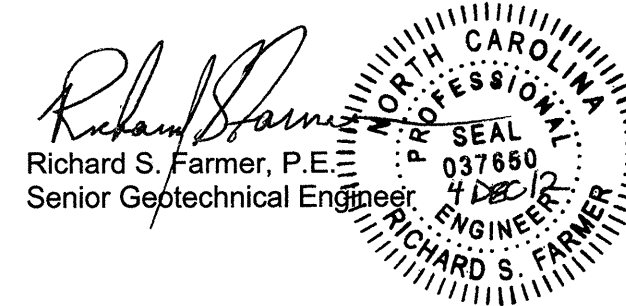
-Y20-	48+00, 18' LT	9.8-10.3	Classification
-Y20-	50+00, 31' LT	15.0-16.4	Classification
-Y20-	54+00, 18' RT	5.0-6.3	Classification
-Y21-	26+00, 23' RT	10.0-11.3	Classification
-Y23-	27+00, 22' RT	10.0-11.5	Classification

**CLOSURE**

Florence & Hutcheson appreciates the opportunity to provide geotechnical services to the Engineering and Safety Unit of the North Carolina Department of Transportation. If you have any questions, comments, or require additional information, please feel free to contact us.

Sincerely,

**FLORENCE & HUTCHESON – AN ICA COMPANY**



PROJECT: P-5206A

COUNTY: Rowan

Volumes in Cubic Yards  
DATE: 1/21/2013

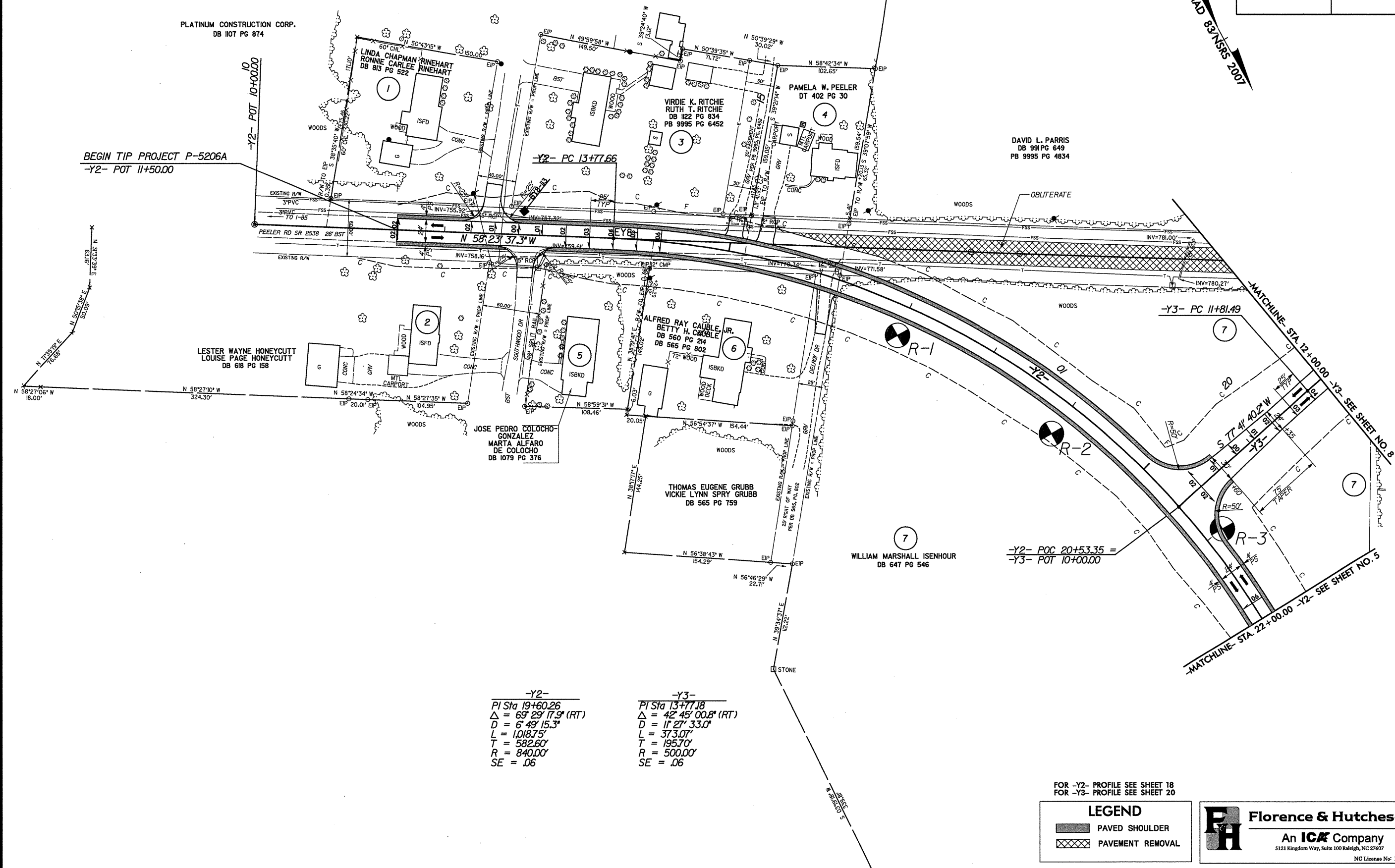
COMPILED BY: BAW

SHEET \_\_ OF \_\_ SHEETS

STATION	STATION	EXCAVATION					EMBANKMENT				BORROW	WASTE			
		TOTAL UNCLASS.	ROCK	UNDERCUT	UNSUIT. UNCLASS.	SUITABLE UNCLASS.	TOTAL	ROCK	EARTH	EMBANK. +20%		ROCK	SUITABLE	UNSUIT.	TOTAL
-Y2- 11+50	-Y2- 41+72.12	12,680		7,465	375	12,305	106,828		106,828	128,194	115,889			7,840	7,840
-Y3- 10+12	-Y3- 17+00	818				818	2,162		2,162	2,594	1,776			3,030	3,030
-Y21- 10+12.50	-Y21- 30+50.30	397		3,030		397	27,879		27,879	33,455	33,058				
	<b>SUBTOTAL</b>	13,895		10,495	375	13,520	136,869		136,869	164,243	150,723			10,870	10,870
-Y20- 33+50	-Y20- 60+26	4,154		210	90	4,064	9,953		9,953	11,944	7,880			300	300
	<b>SUBTOTAL</b>	4,154		210	90	4,064	9,953		9,953	11,944	7,880			300	300
-Y2- 45+83.12	-Y2- 55+80	558		3,335		558	28,651		28,651	34,381	33,823			3,335	3,335
-Y23- 10+42.00	-Y23- 18+01.07	55				55	6,398		6,398	7,678	7,623				
-Y23- 18+49.07	-Y23- 34+48.09	1,629				1,629	10,443		10,443	12,532	10,903				
-Y23A- 10+18.00	-Y23A- 13+55	294				294	167		167	200		94		94	
-YB- 11+00	-YB- 31+25	783				783	1,538		1,538	1,846	1,063				
	<b>SUBTOTAL</b>	3,319		3,335		3,319	47,197		47,197	56,636	53,411			94	3,335
	<b>TOTAL</b>	21,368		14,040	465	20,903	194,019		194,019	232,823	212,013			94	14,505
MATERIAL FOR SHOULDER CONSTRUCTION							3,384		3,384	4,061	4,061				
LOSS DUE TO CLEARING & GRUBBING															
ADDITIONAL UNDERCUT				2,200			2,200		2,200	2,640	2,640			2,200	2,200
ROCK WASTE TO REPLACE BORROW															
ADJUST FOR ROCK WASTE											-94			-94	-94
WASTE IN LIEU OF BORROW															
	<b>PROJECT TOTAL</b>	21,368		16,240	465	20,903	199,603		199,603	239,524	218,621			16,705	16,705
											10,931				
	EST. 5% TO REPLACE TOP SOIL ON BORROW PIT														
	<b>GRAND TOTAL</b>	21,368		16,240	465	20,903	199,603		199,603	239,524	229,552			16,705	16,705
	<b>SAY</b>	21,400									229,600				

BACKFILL UNDERCUT WITH SELECT GRANULAR MATERIAL.

PROJECT REFERENCE NO. P-5206A	SHEET NO. 4
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



BEGIN TIP PROJECT P-5206A  
-Y2- POT 11+50.00

-Y2- POT 10+00.00

-Y2- PC 13+77.66

-Y3- PC 11+81.49

-Y2- POC 20+53.35 =  
-Y3- POT 10+00.00

-MATCHLINE- STA. 12+00.00 -Y3- SEE SHEET NO. 8  
-MATCHLINE- STA. 22+00.00 -Y2- SEE SHEET NO. 5

-Y2-  
PI Sta 19+60.26  
Δ = 69° 29' 17.9" (RT)  
D = 6' 49' 15.3"  
L = 1018.75'  
T = 582.60'  
R = 840.00'  
SE = .06

-Y3-  
PI Sta 13+77.18  
Δ = 42° 45' 00.8" (RT)  
D = 11' 27' 33.0"  
L = 373.07'  
T = 195.70'  
R = 500.00'  
SE = .06

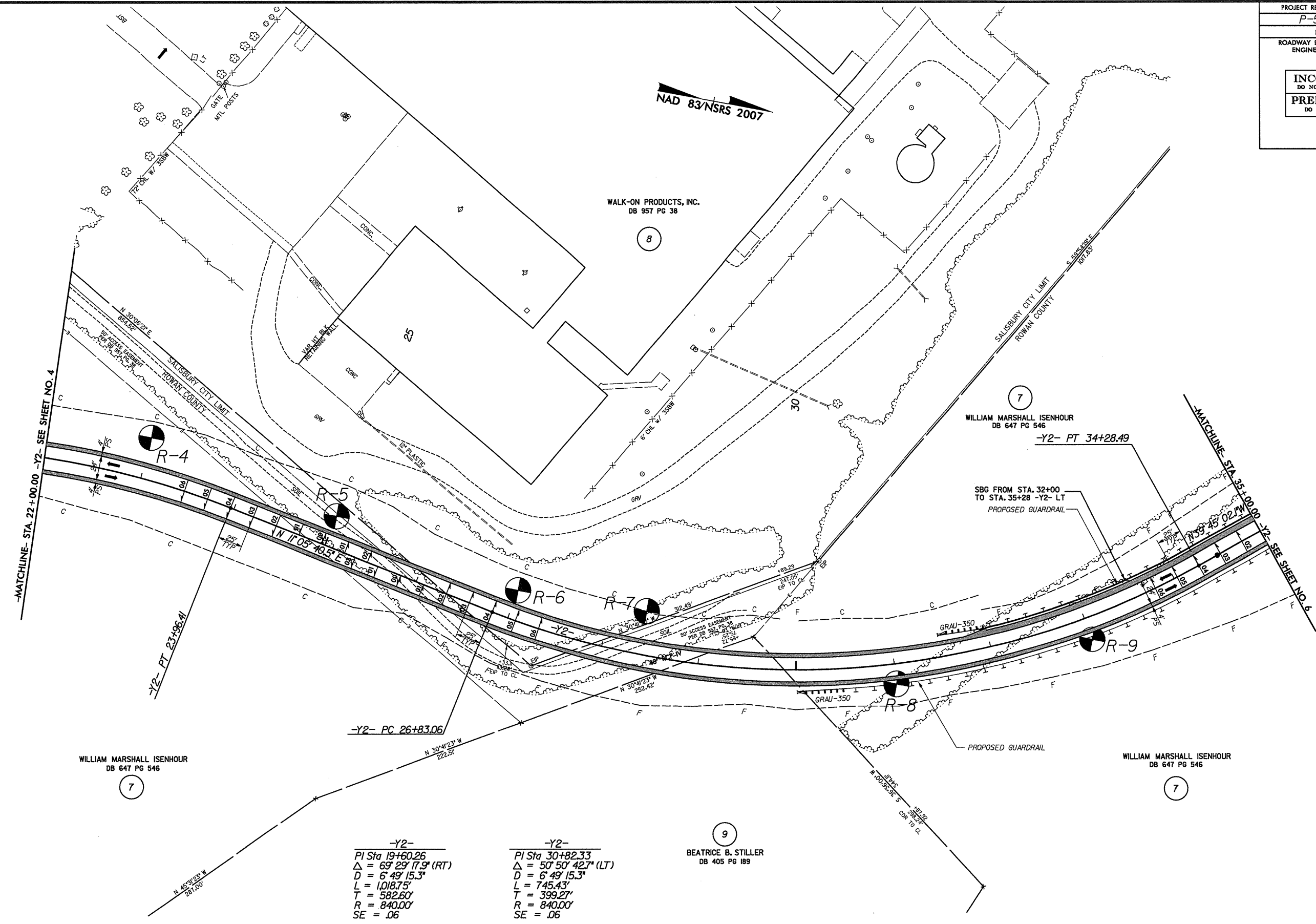
FOR -Y2- PROFILE SEE SHEET 18  
FOR -Y3- PROFILE SEE SHEET 20

LEGEND	
	PAVED SHOULDER
	PAVEMENT REMOVAL

**Florence & Hutcheson**  
An ICA Company  
5121 Kingdom Way, Suite 100 Raleigh, NC 27607  
NC License No: P-0268

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 Florence & Hutcheson, Inc.

PROJECT REFERENCE NO. P-5206A	SHEET NO. 5
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



WILLIAM MARSHALL ISENHOUR  
DB 647 PG 546

WILLIAM MARSHALL ISENHOUR  
DB 647 PG 546

BEATRICE B. STILLER  
DB 405 PG 189

-Y2-  
PI Sta 19+60.26  
 $\Delta = 69^{\circ} 29' 17.9''$  (RT)  
D = 6' 49' 15.3"  
L = 1018.75'  
T = 582.60'  
R = 840.00'  
SE = .06

-Y2-  
PI Sta 30+82.33  
 $\Delta = 50^{\circ} 50' 42.7''$  (LT)  
D = 6' 49' 15.3"  
L = 745.43'  
T = 399.27'  
R = 840.00'  
SE = .06

FOR -Y2- PROFILE SEE SHEET 18

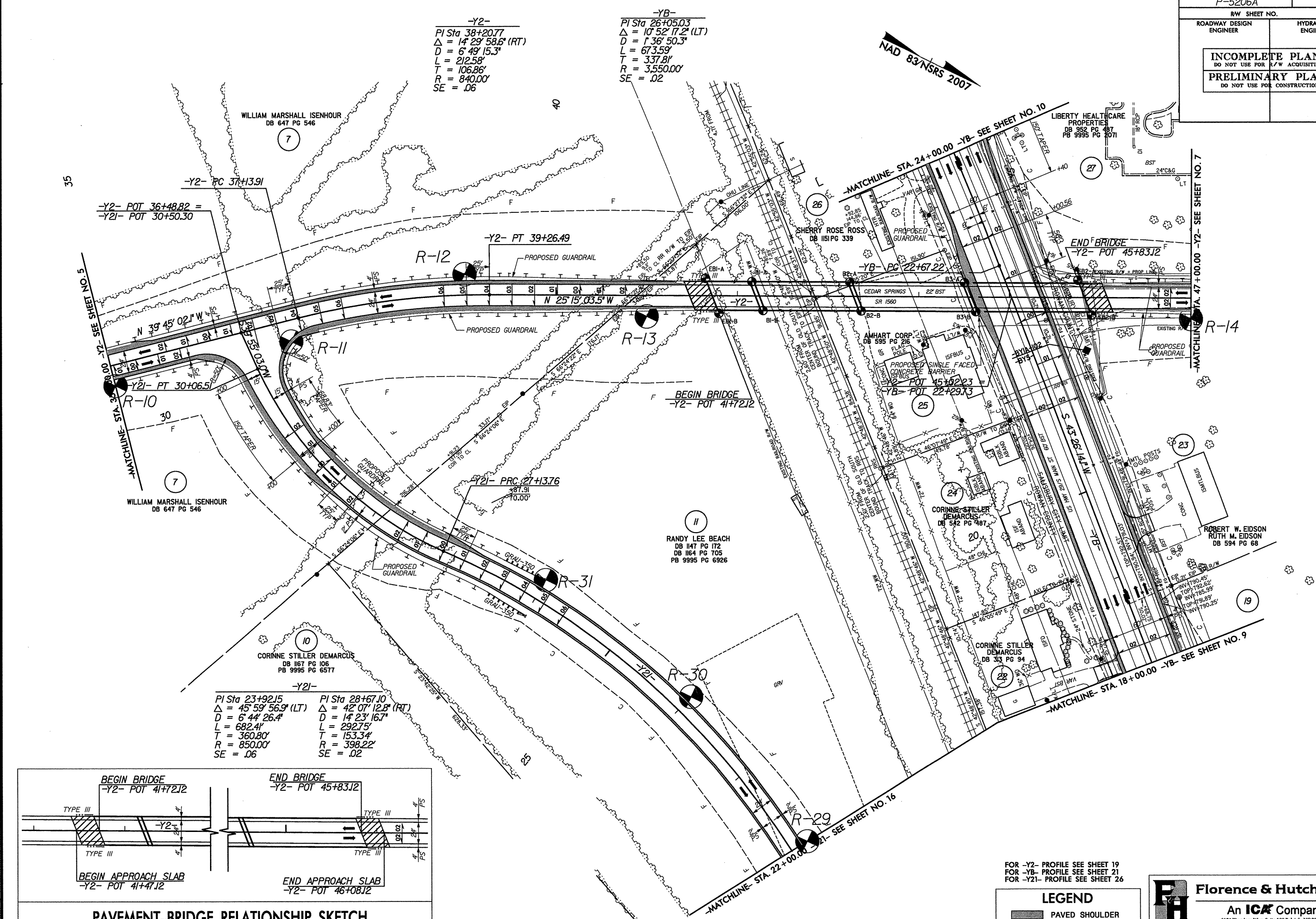
LEGEND	
	PAVED SHOULDER

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NAD 83/NSRS 2007

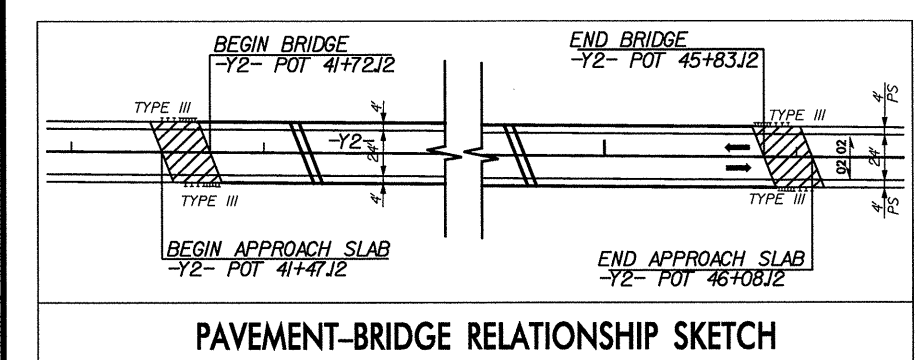


**-Y2-**  
 PI Sta 38+20.77  
 $\Delta = 14^\circ 29' 58.6''$  (RT)  
 $D = 6' 49' 15.3''$   
 $L = 212.58'$   
 $T = 106.86'$   
 $R = 840.00'$   
 $SE = .06$

**-YB-**  
 PI Sta 26+05.03  
 $\Delta = 10^\circ 52' 17.2''$  (LT)  
 $D = 1' 36' 50.3''$   
 $L = 673.59'$   
 $T = 337.81'$   
 $R = 3,550.00'$   
 $SE = .02$

**-Y21-**  
 PI Sta 23+92.15  
 $\Delta = 45^\circ 59' 56.9''$  (LT)  
 $D = 6' 44' 26.4''$   
 $L = 682.41'$   
 $T = 360.80'$   
 $R = 850.00'$   
 $SE = .06$

**-Y21-**  
 PI Sta 28+67.10  
 $\Delta = 42^\circ 07' 12.8''$  (RT)  
 $D = 14' 23' 16.7''$   
 $L = 292.75'$   
 $T = 153.34'$   
 $R = 398.22'$   
 $SE = .02$



FOR -Y2- PROFILE SEE SHEET 19  
 FOR -YB- PROFILE SEE SHEET 21  
 FOR -Y21- PROFILE SEE SHEET 26

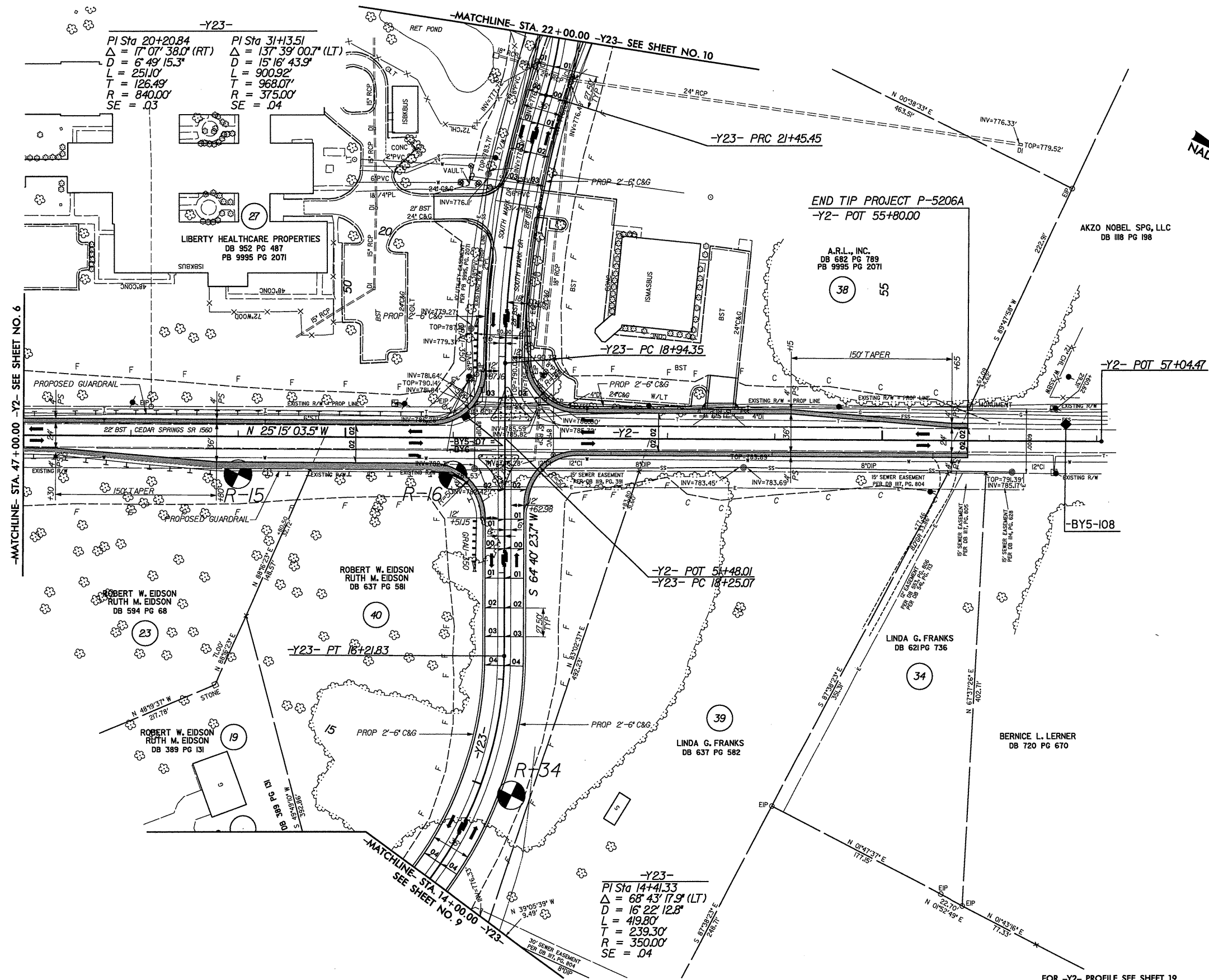
**LEGEND**

PAVED SHOULDER

**FH Florence & Hutcheson**  
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 NC License No: P-0258

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 Florence & Hutcheson, Inc.

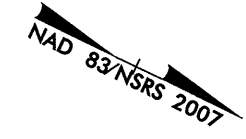
PROJECT REFERENCE NO. P-5206A	SHEET NO. 7
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



**-Y23-**  
 PI Sta 20+20.84  
 $\Delta = 17^{\circ} 07' 38.0''$  (RT)  
 $D = 6' 49' 15.3''$   
 $L = 251.0'$   
 $T = 126.49'$   
 $R = 840.00'$   
 $SE = .03$

PI Sta 31+13.51  
 $\Delta = 137^{\circ} 39' 00.7''$  (LT)  
 $D = 15' 16' 43.9''$   
 $L = 900.92'$   
 $T = 968.07'$   
 $R = 375.00'$   
 $SE = .04$

**-Y23-**  
 PI Sta 14+41.33  
 $\Delta = 68^{\circ} 43' 17.9''$  (LT)  
 $D = 16' 22' 12.8''$   
 $L = 419.80'$   
 $T = 239.30'$   
 $R = 350.00'$   
 $SE = .04$



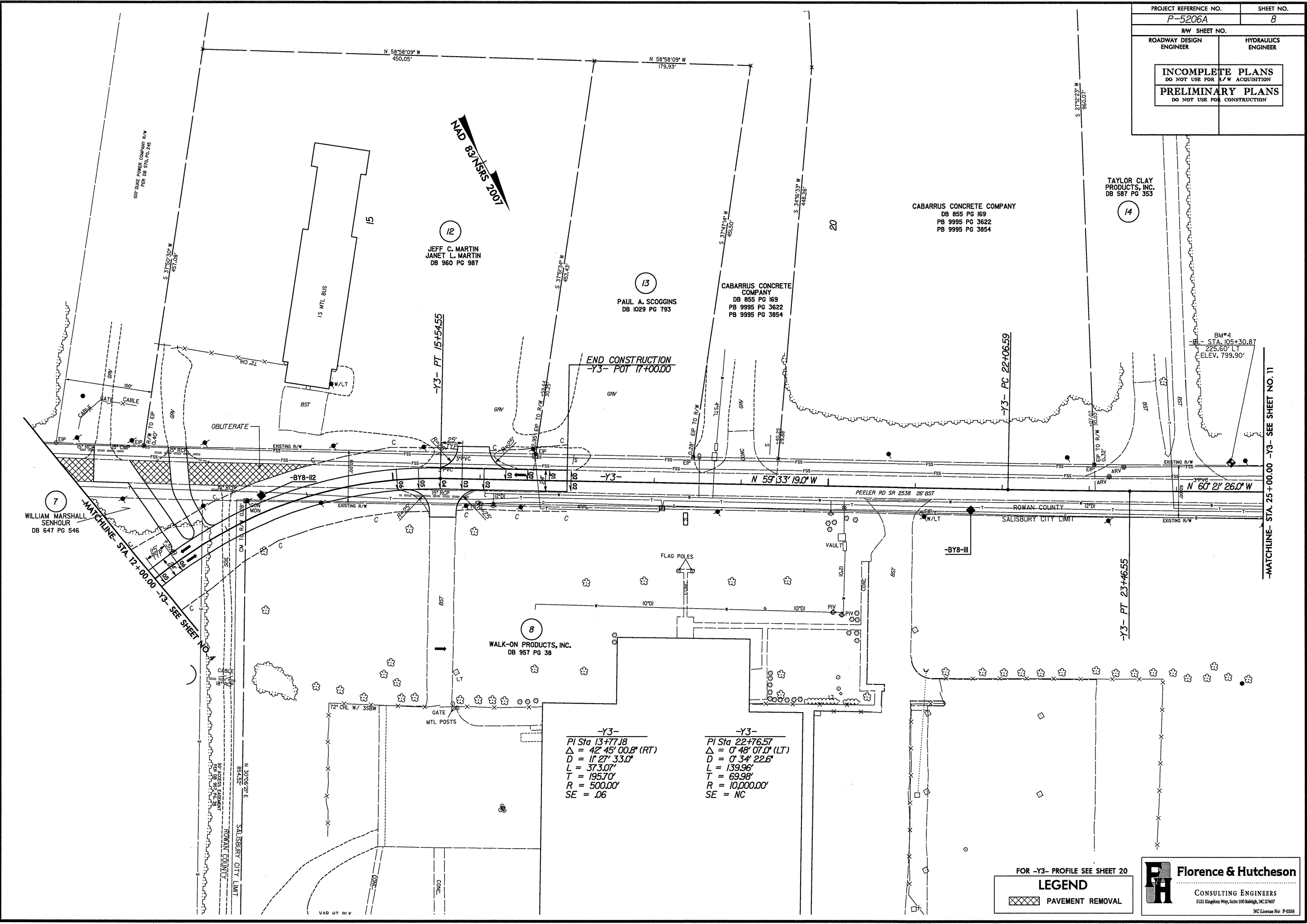
FOR -Y2- PROFILE SEE SHEET 19  
 FOR -Y23- PROFILE SEE SHEET 22

LEGEND	
	PAVED SHOULDER

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 Florence & Hutcheson, Inc.

PROJECT REFERENCE NO. P-5206A	SHEET NO. 8
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



7  
WILLIAM MARSHALL  
SENHOUR  
DB 647 PG 546

MATCHLINE STA. 12+00.00 -Y3- SEE SHEET NO. 7

-Y3-  
PI Sta 13+77.18  
Δ = 42° 45' 00.8" (RT)  
D = 17' 27" 33.0"  
L = 373.07'  
T = 195.70'  
R = 500.00'  
SE = .06

-Y3-  
PI Sta 22+76.57  
Δ = 0° 48' 07.0" (LT)  
D = 0' 34' 22.6"  
L = 139.96'  
T = 69.98'  
R = 10,000.00'  
SE = NC

FOR -Y3- PROFILE SEE SHEET 20

**LEGEND**

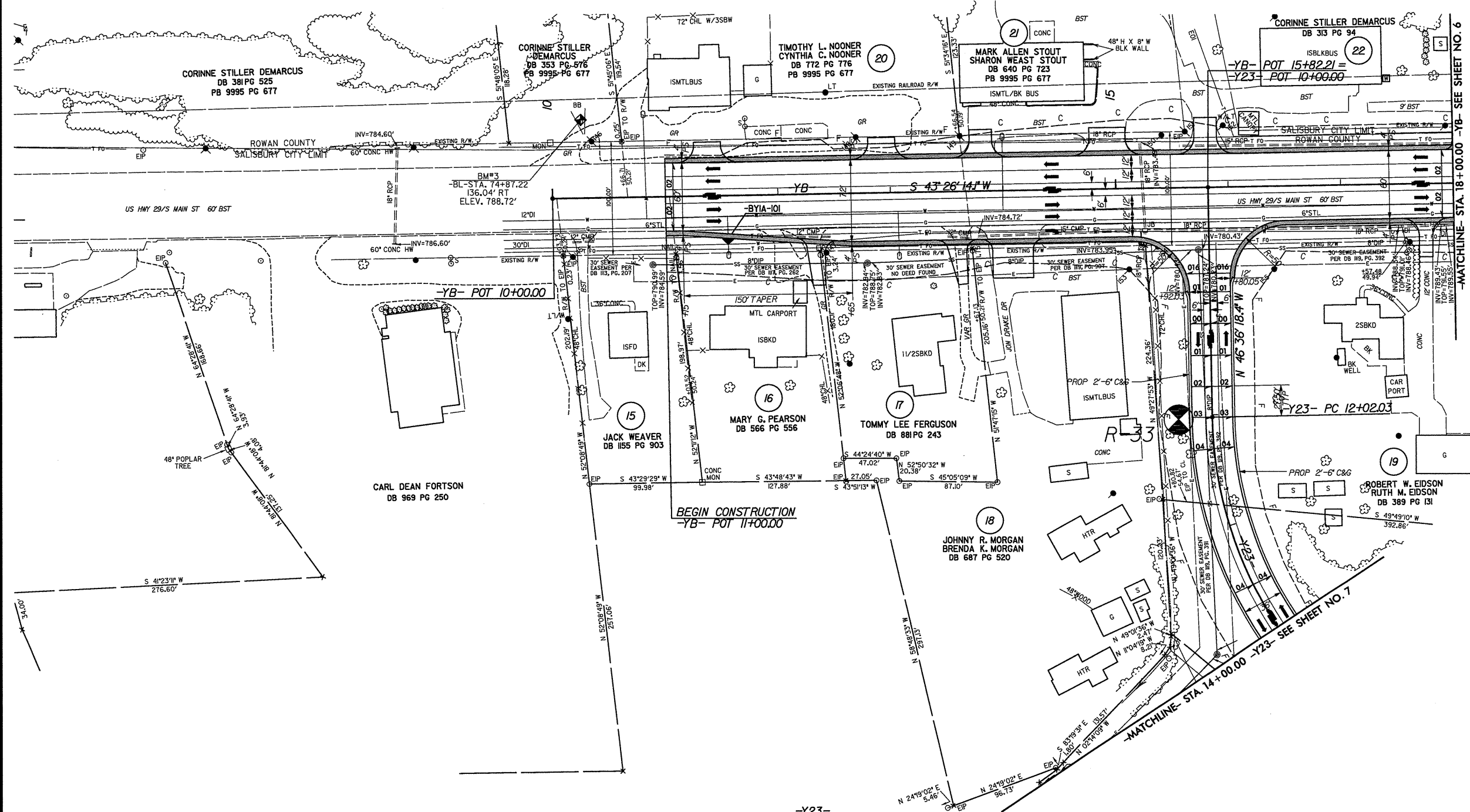
▣ PAVEMENT REMOVAL

**Florence & Hutcheson**  
CONSULTING ENGINEERS  
5121 Kingdom Way, Suite 100 Raleigh, NC 27607  
NC License No. 9-0268

12/5/2012  
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PROJECT REFERENCE NO. P-5206A	SHEET NO. 9
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

DO NOT SCALE DRAWING



-Y23-  
 P/ Sta 14+41.33  
 $\Delta = 68' 43' 17.9''$  (LT)  
 $D = 16' 22' 12.8''$   
 $L = 419.80'$   
 $T = 239.30'$   
 $R = 350.00'$   
 $SE = .04$

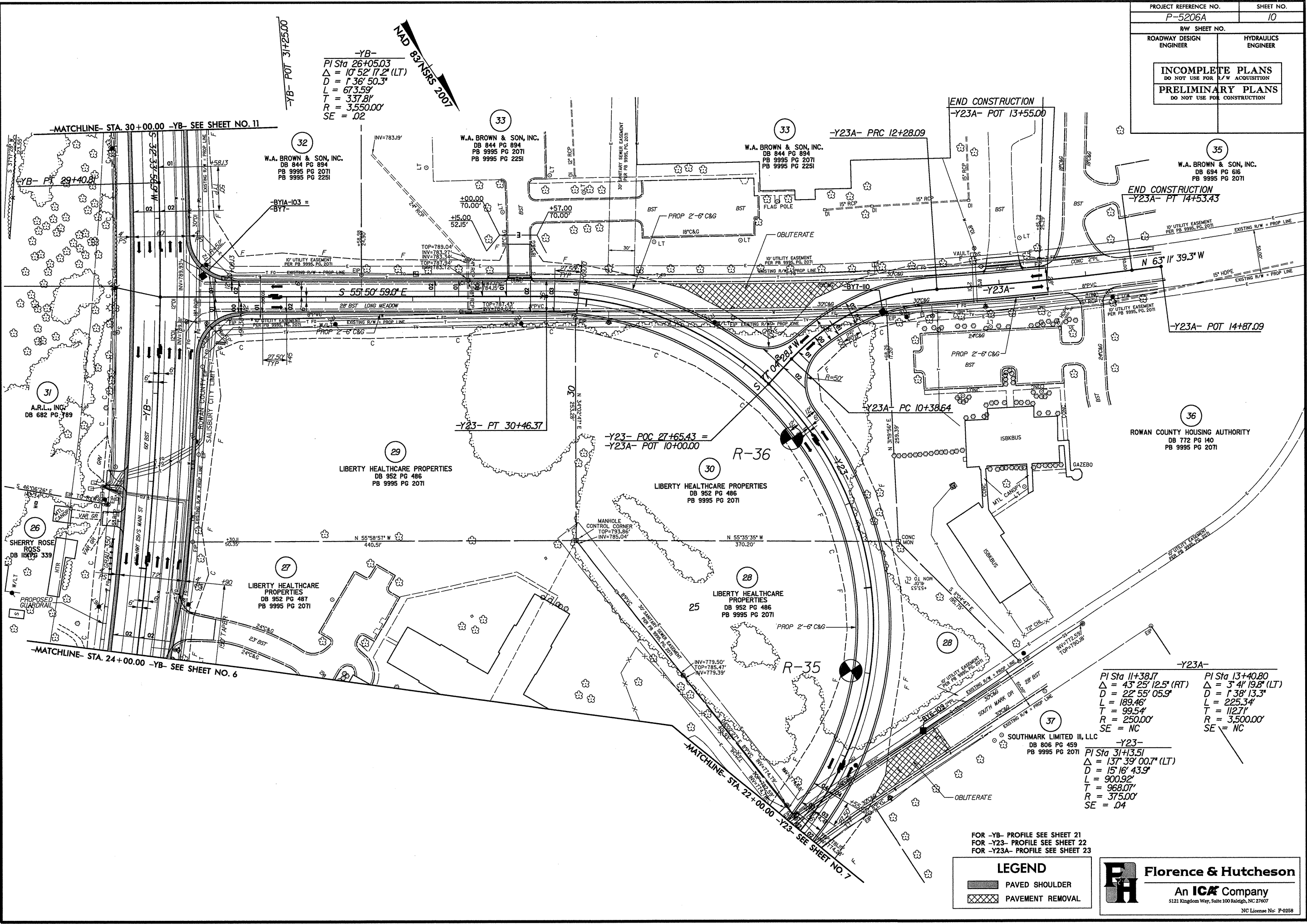
FOR -YB- PROFILE SEE SHEET 21  
 FOR -Y23- PROFILE SEE SHEET 22

LEGEND	
	PAVED SHOULDER

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 Florence & Hutcheson, Inc.

PROJECT REFERENCE NO. P-5206A	SHEET NO. 10
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



-YB-  
 PI Sta 26+05.03  
 $\Delta = 10' 52' 17.2''$  (LT)  
 $D = 1' 36' 50.3''$   
 $L = 673.59'$   
 $T = 337.81'$   
 $R = 3,550.00'$   
 $SE = .02$

-Y23A-  
 PI Sta 11+38.17  
 $\Delta = 43' 25' 12.5''$  (RT)  
 $D = 22' 55' 05.9''$   
 $L = 189.46'$   
 $T = 99.54'$   
 $R = 250.00'$   
 $SE = NC$

-Y23-  
 PI Sta 31+13.51  
 $\Delta = 137' 39' 00.7''$  (LT)  
 $D = 15' 16' 43.9''$   
 $L = 900.92'$   
 $T = 968.07'$   
 $R = 375.00'$   
 $SE = .04$

-Y23A-  
 PI Sta 13+40.80  
 $\Delta = 3' 41' 19.8''$  (LT)  
 $D = 1' 38' 13.3''$   
 $L = 225.34'$   
 $T = 112.71'$   
 $R = 3,500.00'$   
 $SE = NC$

FOR -YB- PROFILE SEE SHEET 21  
 FOR -Y23- PROFILE SEE SHEET 22  
 FOR -Y23A- PROFILE SEE SHEET 23

LEGEND	
	PAVED SHOULDER
	PAVEMENT REMOVAL

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 NC License No: F-0288

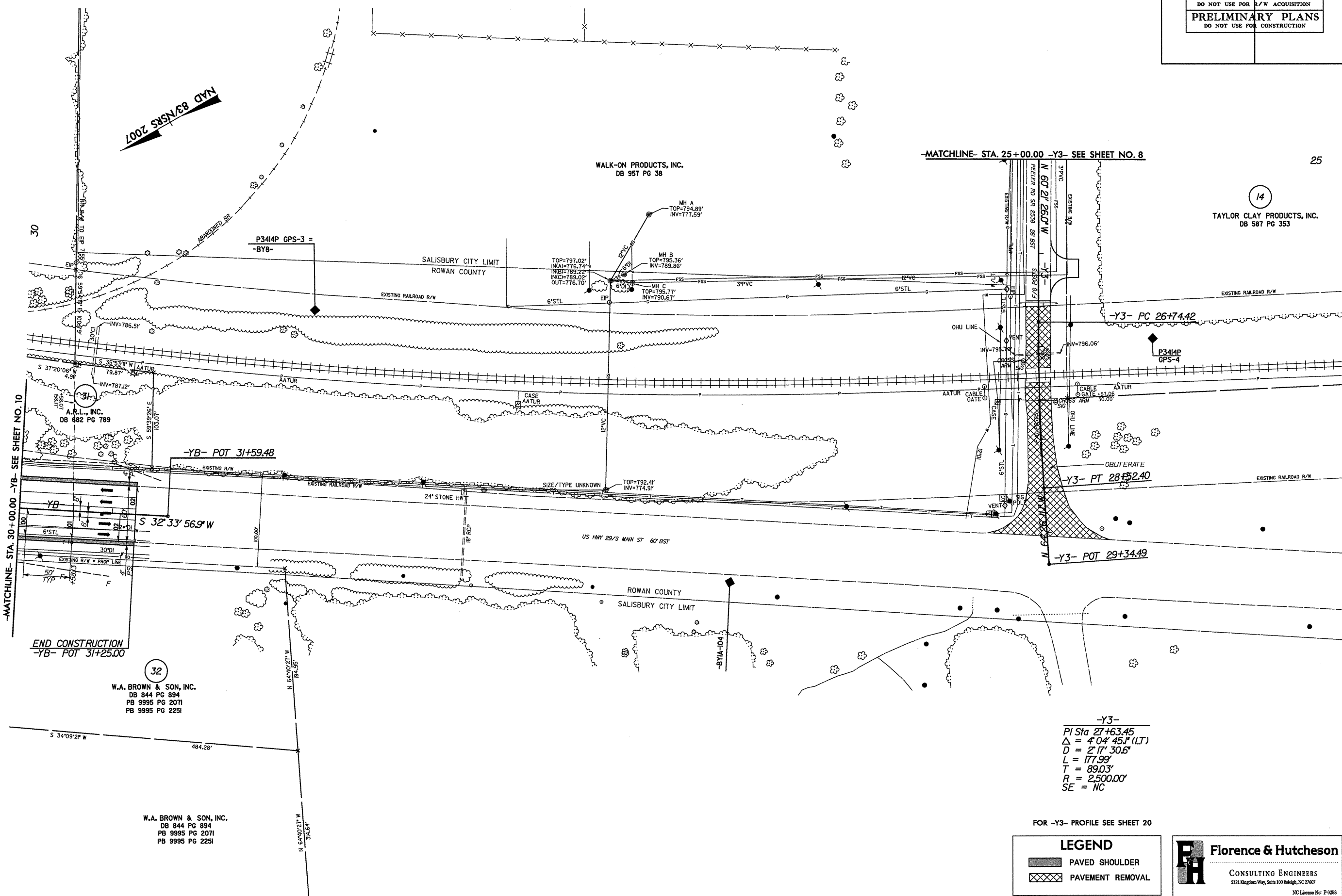
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 Florence & Hutcheson, Inc.

PROJECT REFERENCE NO. P-5206A	SHEET NO. 11
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

25

14

TAYLOR CLAY PRODUCTS, INC.  
DB 587 PG 353



-MATCHLINE- STA. 25+00.00 -Y3- SEE SHEET NO. 8

-MATCHLINE- STA. 30+00.00 -YB- SEE SHEET NO. 10

END CONSTRUCTION  
-YB- POT 31+25.00

-Y3-  
 PI Sta 27+63.45  
 $\Delta = 4' 04'' 45.1'' (LT)$   
 $D = 2' 17'' 30.6''$   
 $L = 177.99'$   
 $T = 89.03'$   
 $R = 2,500.00'$   
 $SE = NC$

FOR -Y3- PROFILE SEE SHEET 20

**LEGEND**

- PAVED SHOULDER
- PAVEMENT REMOVAL

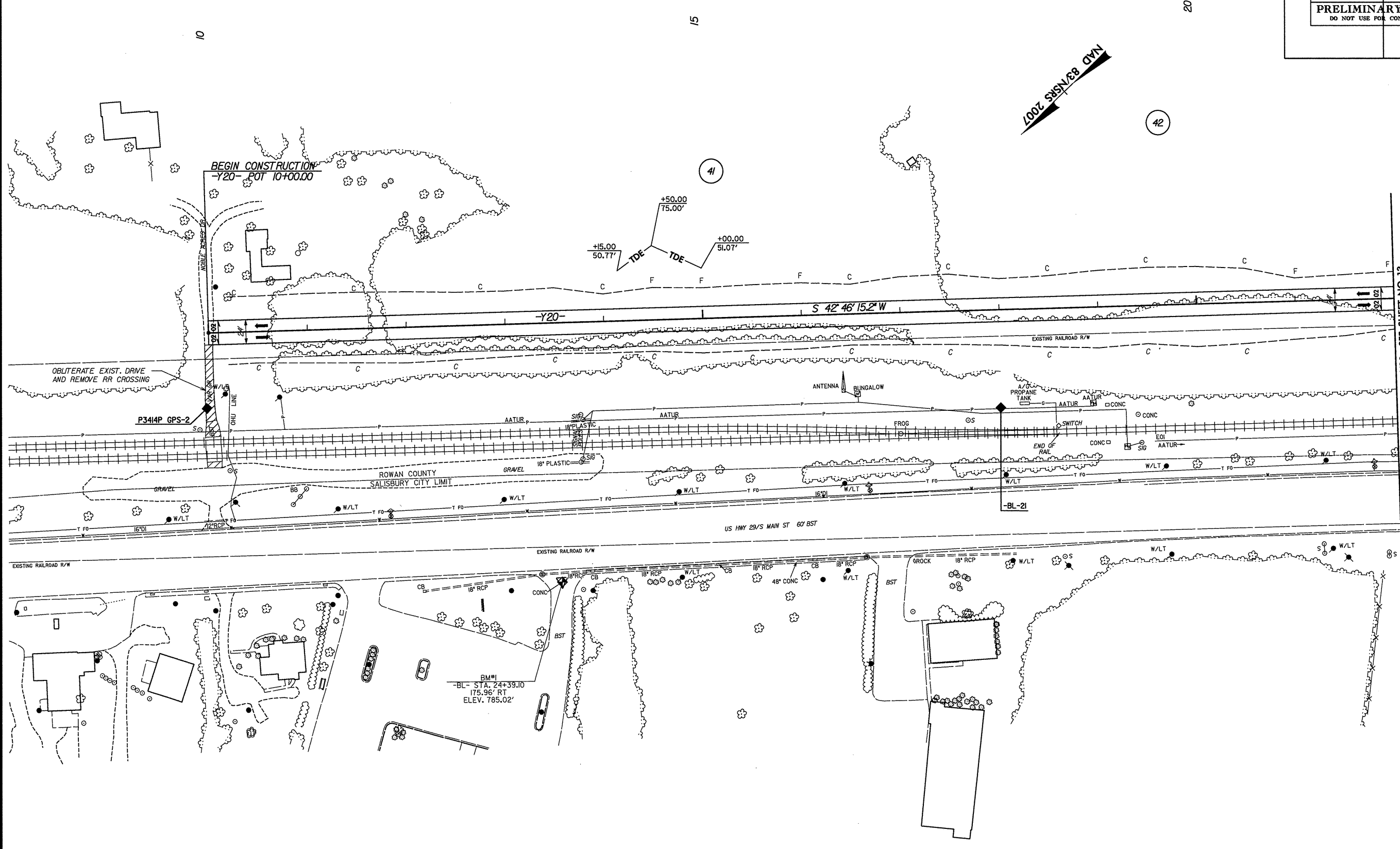
**Florence & Hutcheson**  
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 Florence & Hutcheson, Inc.

32  
 W.A. BROWN & SON, INC.  
 DB 844 PG 894  
 PB 9995 PG 2071  
 PB 9995 PG 2251

W.A. BROWN & SON, INC.  
 DB 844 PG 894  
 PB 9995 PG 2071  
 PB 9995 PG 2251

PROJECT REFERENCE NO. P-5206A	SHEET NO. 12
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



NAD 83/NGRS 2007

42

41

-MATCHLINE- STA. 22+00.00 -Y20- SEE SHEET NO. 13

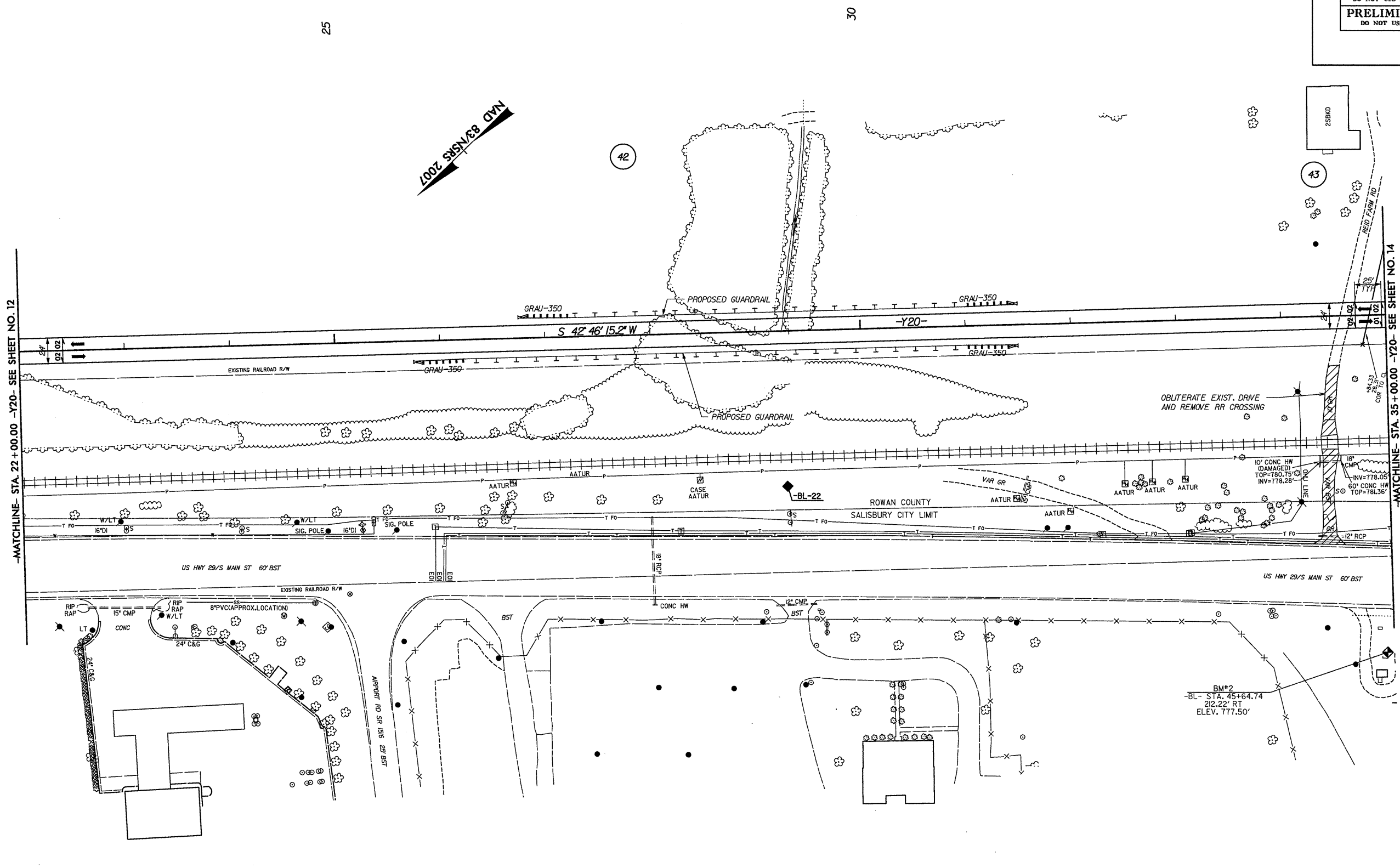
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FOR -Y20- PROFILE SEE SHEET 23

LEGEND	
	PAVEMENT REMOVAL
	DRIVEWAY AND RR CROSSING REMOVAL

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 NC License No: P-0258

PROJECT REFERENCE NO. P-5206A	SHEET NO. 13
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



FOR -Y20- PROFILE SEE SHEET 24

LEGEND	
	PAVEMENT REMOVAL
	DRIVEWAY AND RR CROSSING REMOVAL

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 NC License No. P-0058

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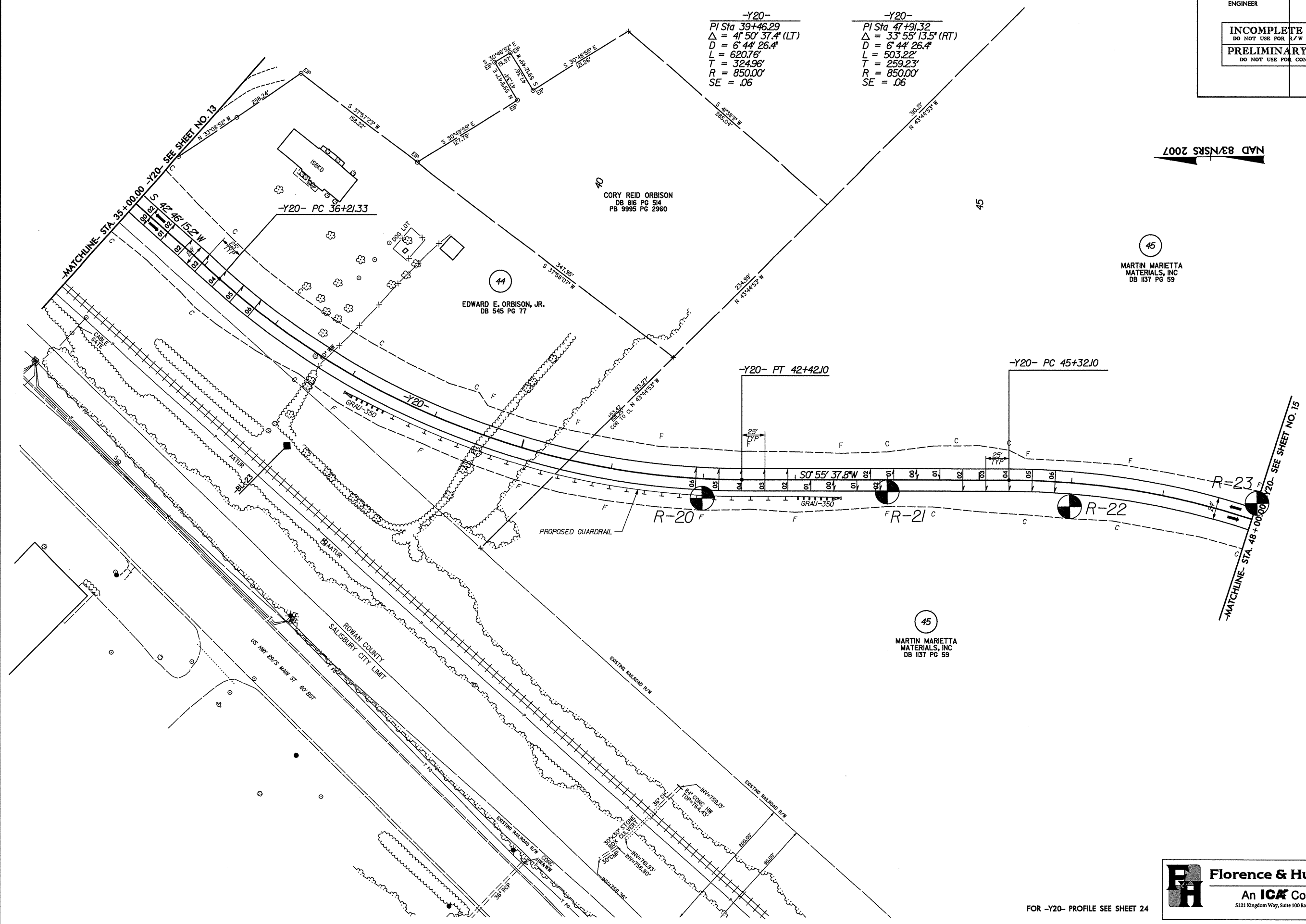


PROJECT REFERENCE NO. P-5206A	SHEET NO. 14
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

**-Y20-**  
 PI Sta 39+46.29  
 $\Delta = 41^{\circ} 50' 37.4" (LT)$   
 $D = 6' 44" 26.4"$   
 $L = 620.76'$   
 $T = 324.96'$   
 $R = 850.00'$   
 $SE = .06$

**-Y20-**  
 PI Sta 47+91.32  
 $\Delta = 33^{\circ} 55' 13.5" (RT)$   
 $D = 6' 44" 26.4"$   
 $L = 503.22'$   
 $T = 259.23'$   
 $R = 850.00'$   
 $SE = .06$

NAD 83/NSRS 2007



44  
 EDWARD E. ORBISON, JR.  
 DB 545 PG 77

40  
 CORY REID ORBISON  
 DB 816 PG 514  
 PB 9995 PG 2960

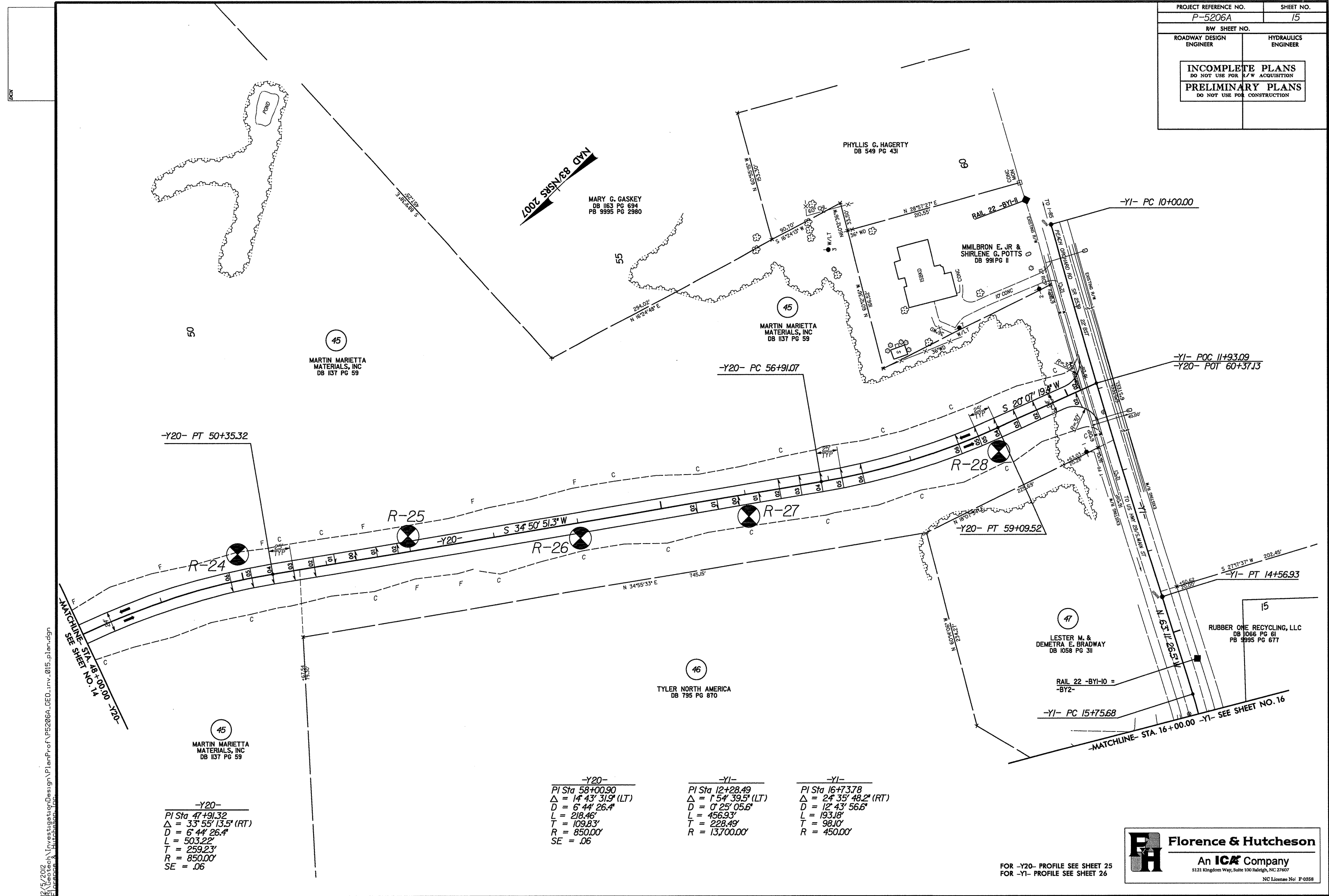
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 MARTIN MARIETTA MATERIALS, INC  
 DB 1137 PG 59

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FOR -Y20- PROFILE SEE SHEET 24

PROJECT REFERENCE NO. P-5206A	SHEET NO. 15
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



-Y20-  
 PI Sta 47+91.32  
 $\Delta = 33^{\circ}55'13.5"$  (RT)  
 $D = 6^{\circ}44'26.4"$   
 $L = 503.22'$   
 $T = 259.23'$   
 $R = 850.00'$   
 $SE = .06$

-Y20-  
 PI Sta 58+00.90  
 $\Delta = 14^{\circ}43'31.9"$  (LT)  
 $D = 6^{\circ}44'26.4"$   
 $L = 218.46'$   
 $T = 109.83'$   
 $R = 850.00'$   
 $SE = .06$

-Y1-  
 PI Sta 12+28.49  
 $\Delta = 1^{\circ}54'39.5"$  (LT)  
 $D = 0^{\circ}25'05.6"$   
 $L = 456.93'$   
 $T = 228.49'$   
 $R = 13700.00'$

-Y1-  
 PI Sta 16+73.78  
 $\Delta = 24^{\circ}35'48.2"$  (RT)  
 $D = 12^{\circ}43'56.6"$   
 $L = 193.18'$   
 $T = 98.10'$   
 $R = 450.00'$

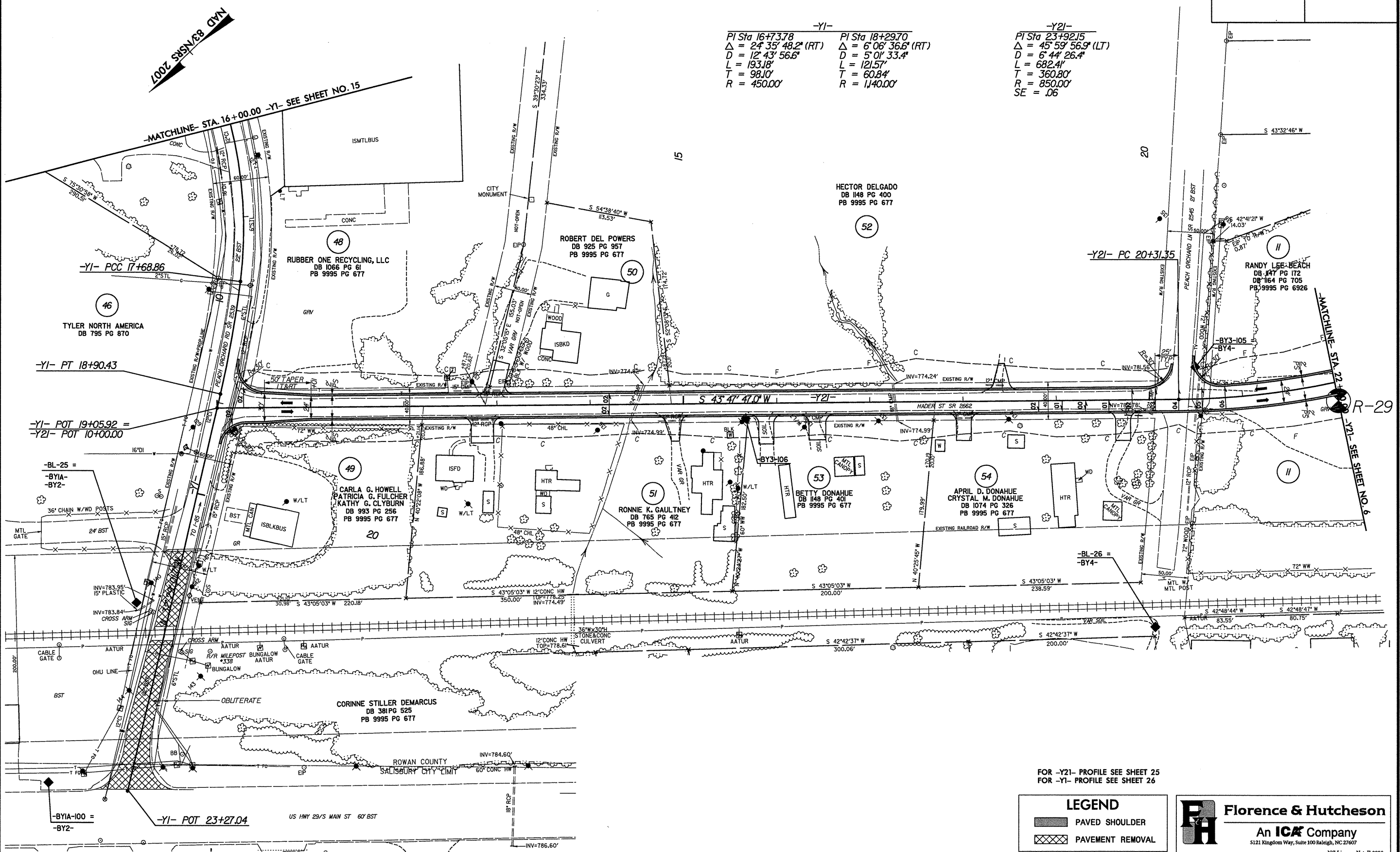
FOR -Y20- PROFILE SEE SHEET 25  
 FOR -Y1- PROFILE SEE SHEET 26

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 NC License No: F-0258

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-Y1-

PI Sta 16+73.78	PI Sta 18+29.70	-Y21-
$\Delta = 24' 35" 48.2" (RT)$	$\Delta = 6' 06" 36.6" (RT)$	PI Sta 23+92.15
$D = 12' 43" 56.6"$	$D = 5' 01" 33.4"$	$\Delta = 45' 59" 56.9" (LT)$
$L = 193.18'$	$L = 121.57'$	$D = 6' 44" 26.4"$
$T = 98.10'$	$T = 60.84'$	$L = 682.41'$
$R = 450.00'$	$R = 1,140.00'$	$T = 360.80'$
		$R = 850.00'$
		$SE = .06$



FOR -Y21- PROFILE SEE SHEET 25  
 FOR -Y1- PROFILE SEE SHEET 26

**LEGEND**

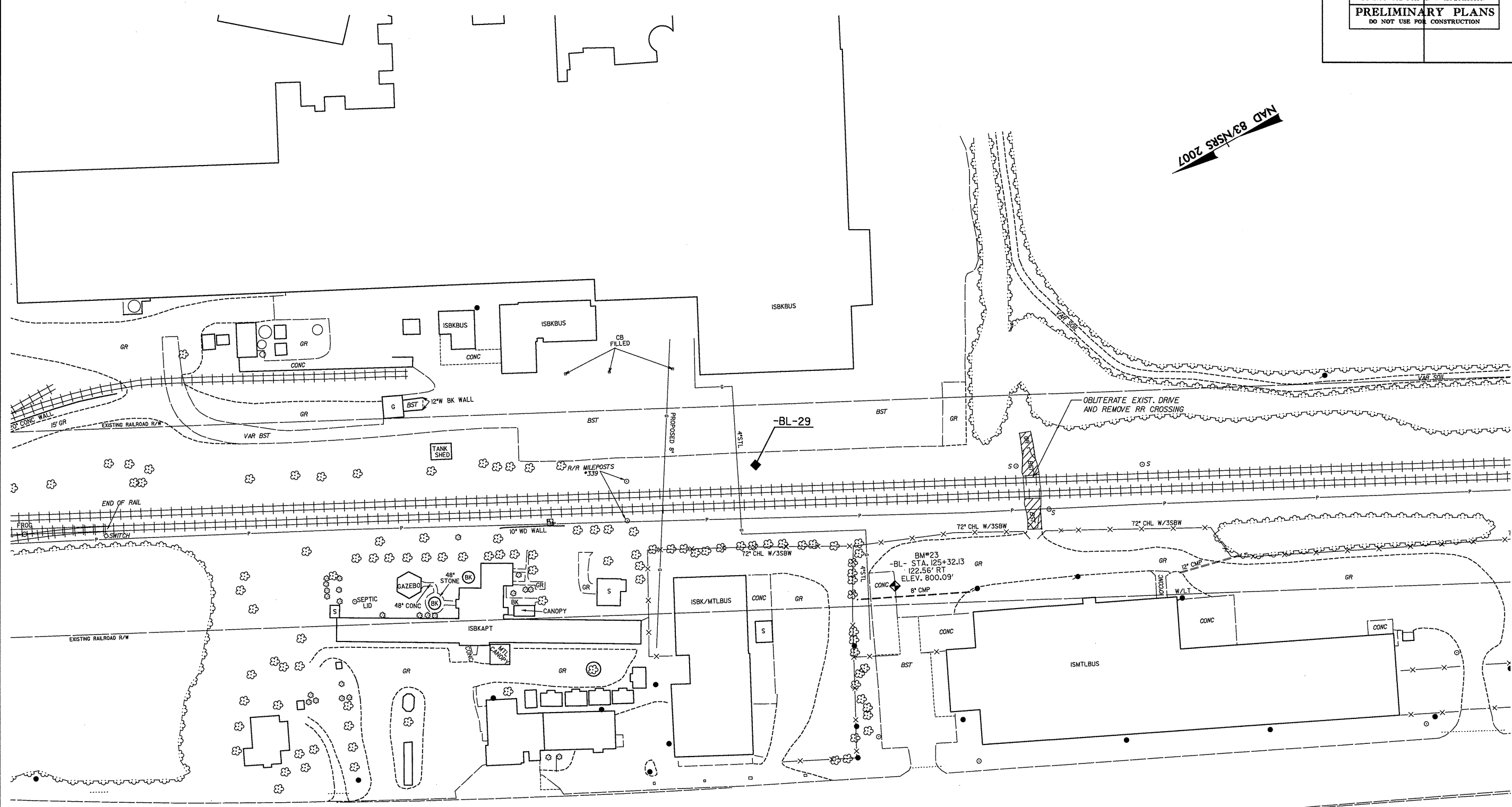
- PAVED SHOULDER
- PAVEMENT REMOVAL

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 NC License No: P-0258

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PROJECT REFERENCE NO. P-5206A	SHEET NO. 17
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

NAD 83/NSRS 2007

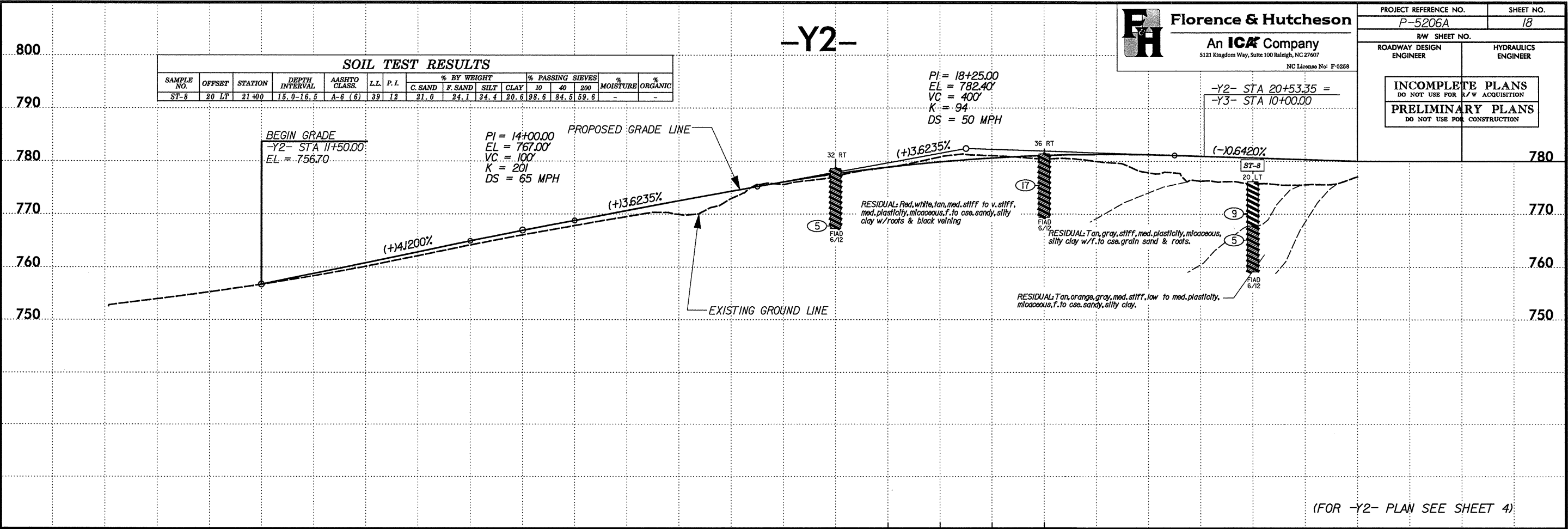


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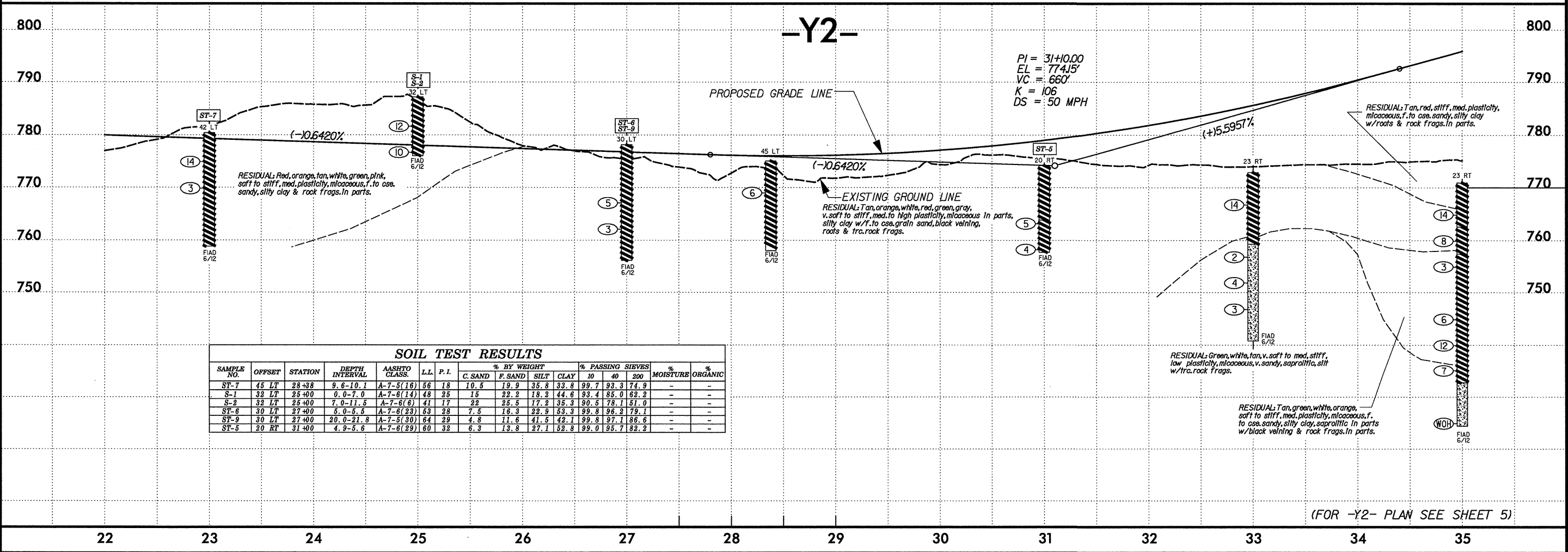
LEGEND	
	PAVEMENT REMOVAL
	DRIVEWAY AND RR CROSSING REMOVAL

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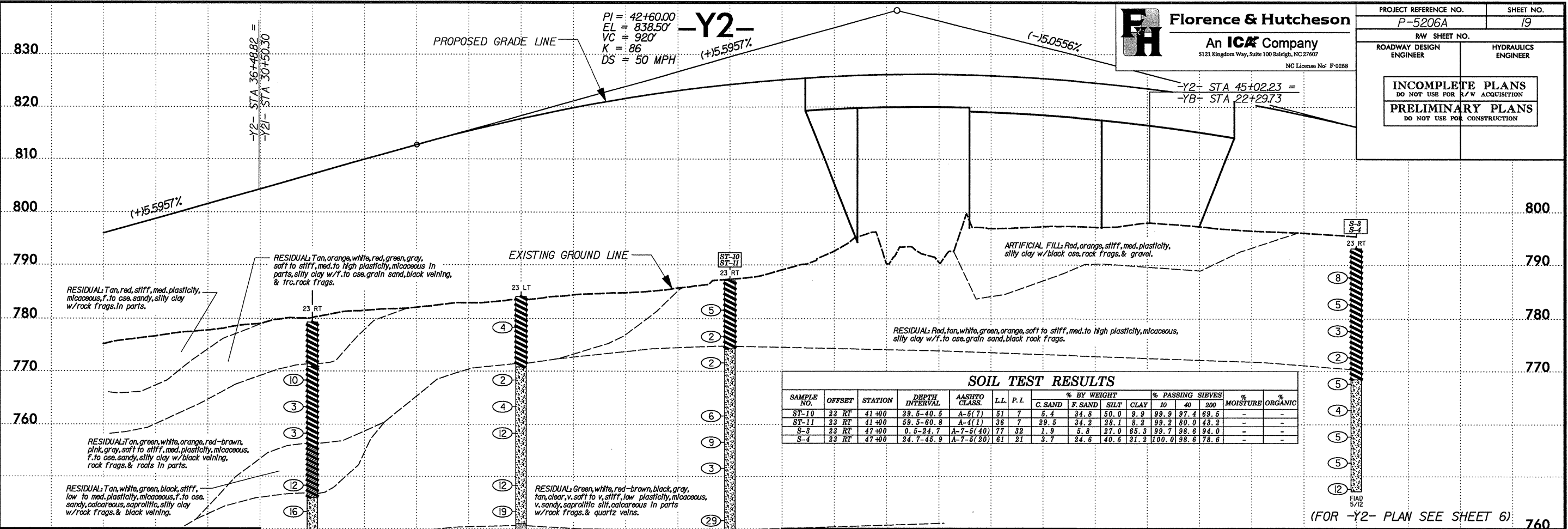
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-8	20 LT	21+00	15.0-16.5	A-6 (6)	39	12	21.0	24.1	34.4	20.6	98.6	84.5	59.6	-	-



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-7	45 LT	23+38	9.6-10.1	A-7-5(16)	56	18	10.5	19.9	35.8	33.8	99.7	93.3	74.9	-	-
S-1	32 LT	25+00	0.0-7.0	A-7-6(14)	48	25	15	22.2	18.2	44.6	93.4	85.0	62.2	-	-
S-2	32 LT	25+00	7.0-11.5	A-7-6(6)	41	17	22	35.5	17.2	35.3	90.5	78.1	51.0	-	-
ST-6	30 LT	27+00	5.0-5.5	A-7-6(23)	53	28	7.5	16.3	22.9	53.3	99.8	96.2	79.1	-	-
ST-9	30 LT	27+00	20.0-21.8	A-7-5(30)	64	29	4.8	11.6	41.5	42.1	99.8	97.1	86.6	-	-
ST-5	20 RT	31+00	4.9-5.6	A-7-6(29)	60	32	6.3	13.8	27.1	52.8	99.0	95.7	82.2	-	-

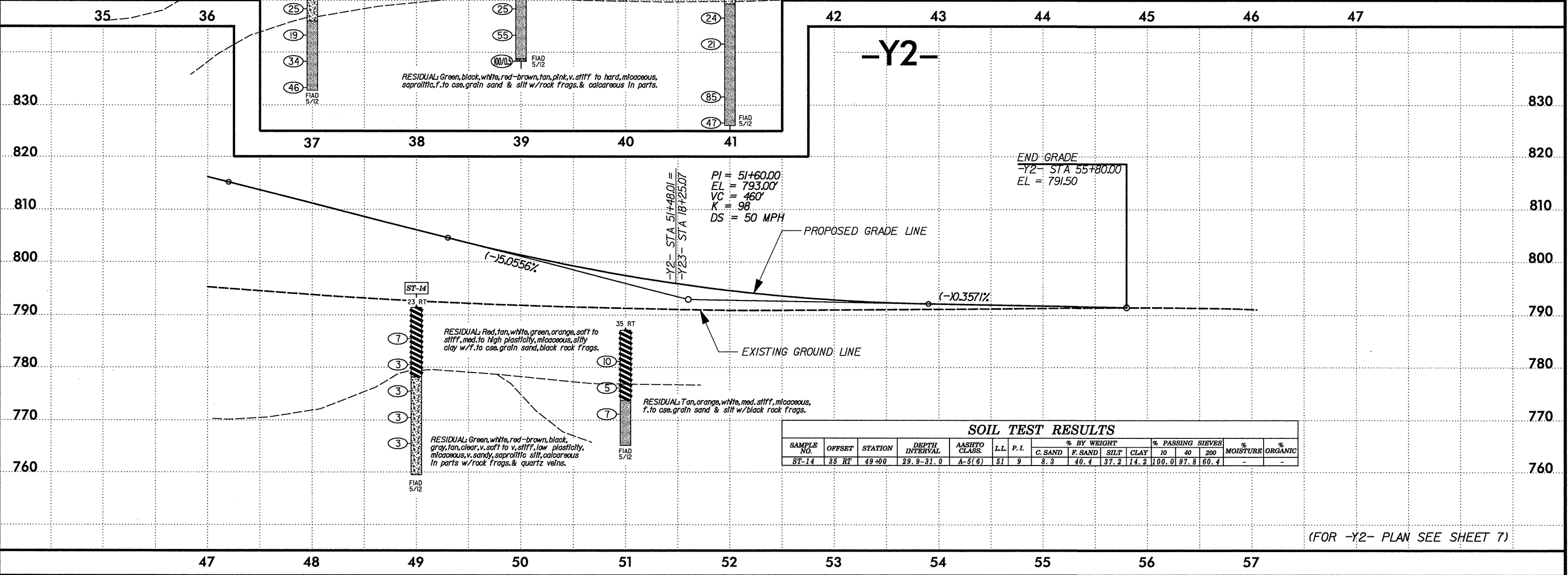


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**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	200		
ST-10	23 RT	41+00	39.5-40.5	A-5(7)	51	7	5.4	34.8	50.0	9.9	99.9	97.4	69.5	-
ST-11	23 RT	41+00	59.5-60.8	A-4(1)	36	7	29.5	34.2	28.1	8.2	99.2	80.0	43.2	-
S-3	23 RT	47+00	0.5-24.7	A-7-5(40)	77	32	1.9	5.8	27.0	65.3	99.7	98.6	94.0	-
S-4	23 RT	47+00	24.7-46.9	A-7-5(20)	61	21	3.7	24.6	40.5	31.2	100.0	98.6	78.6	-



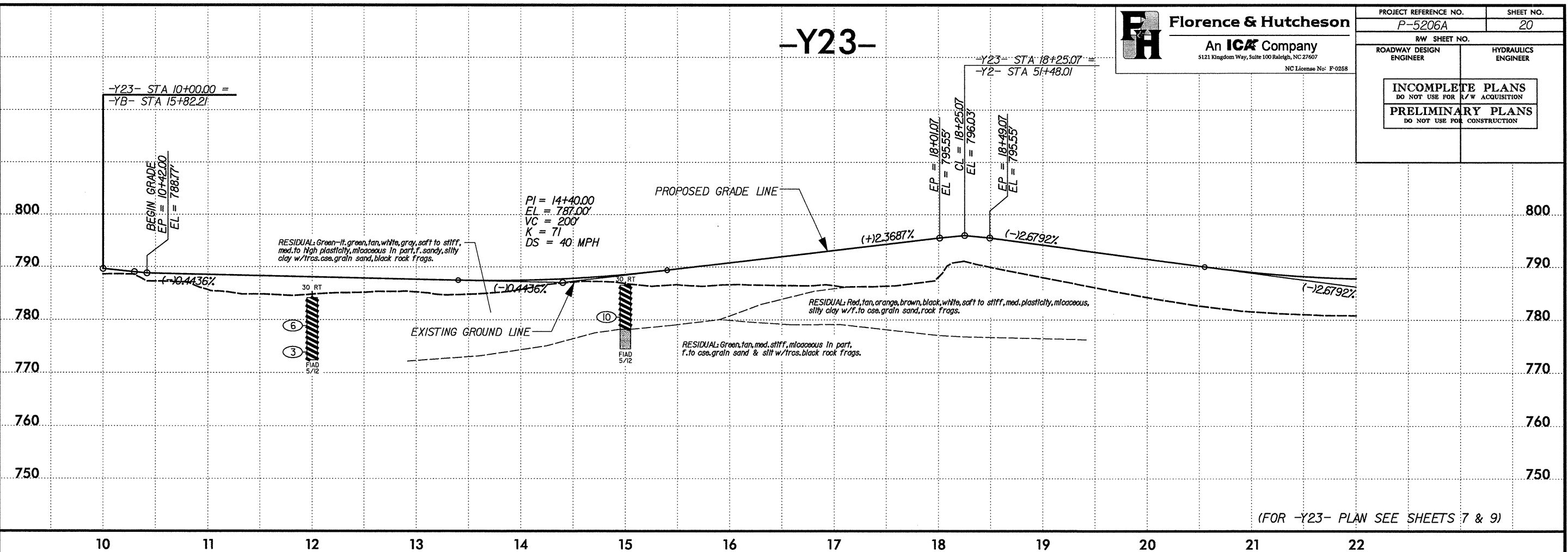
**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	200		
ST-14	35 RT	49+00	29.9-31.0	A-5(6)	51	9	8.3	40.4	37.2	14.2	100.0	97.8	60.4	-

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 Florence & Hutcheson

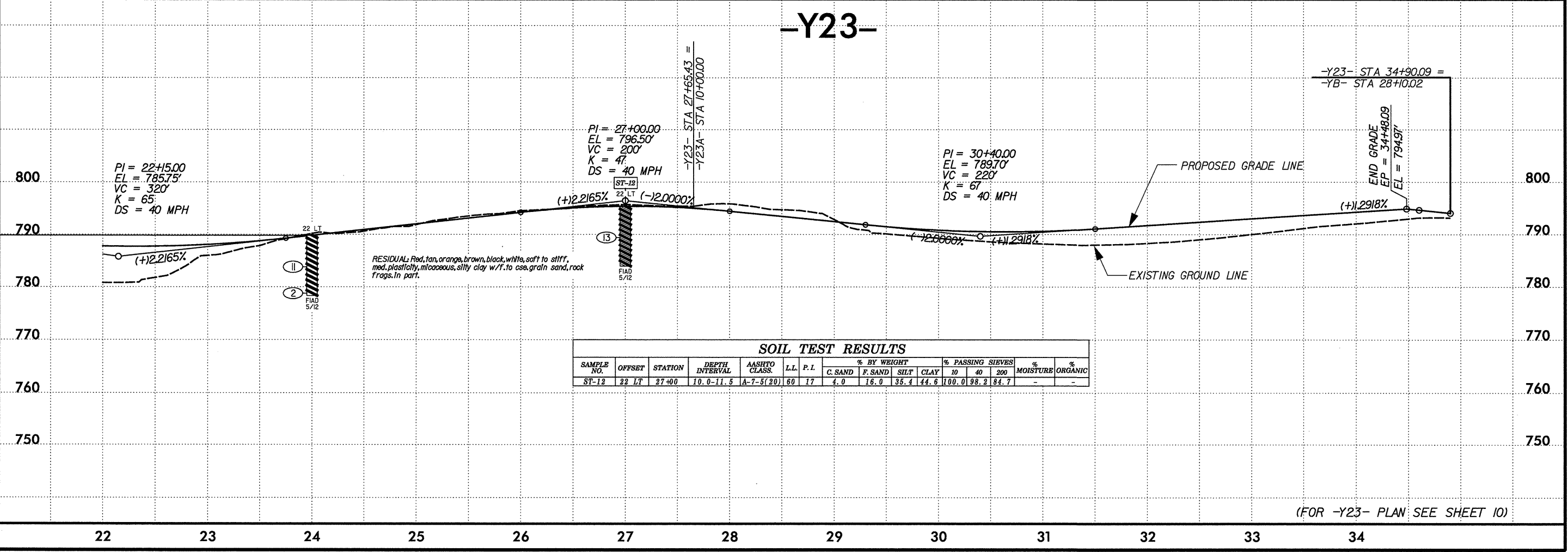
(FOR -Y2- PLAN SEE SHEET 7)

**-Y23-**



(FOR -Y23- PLAN SEE SHEETS 7 & 9)

**-Y23-**



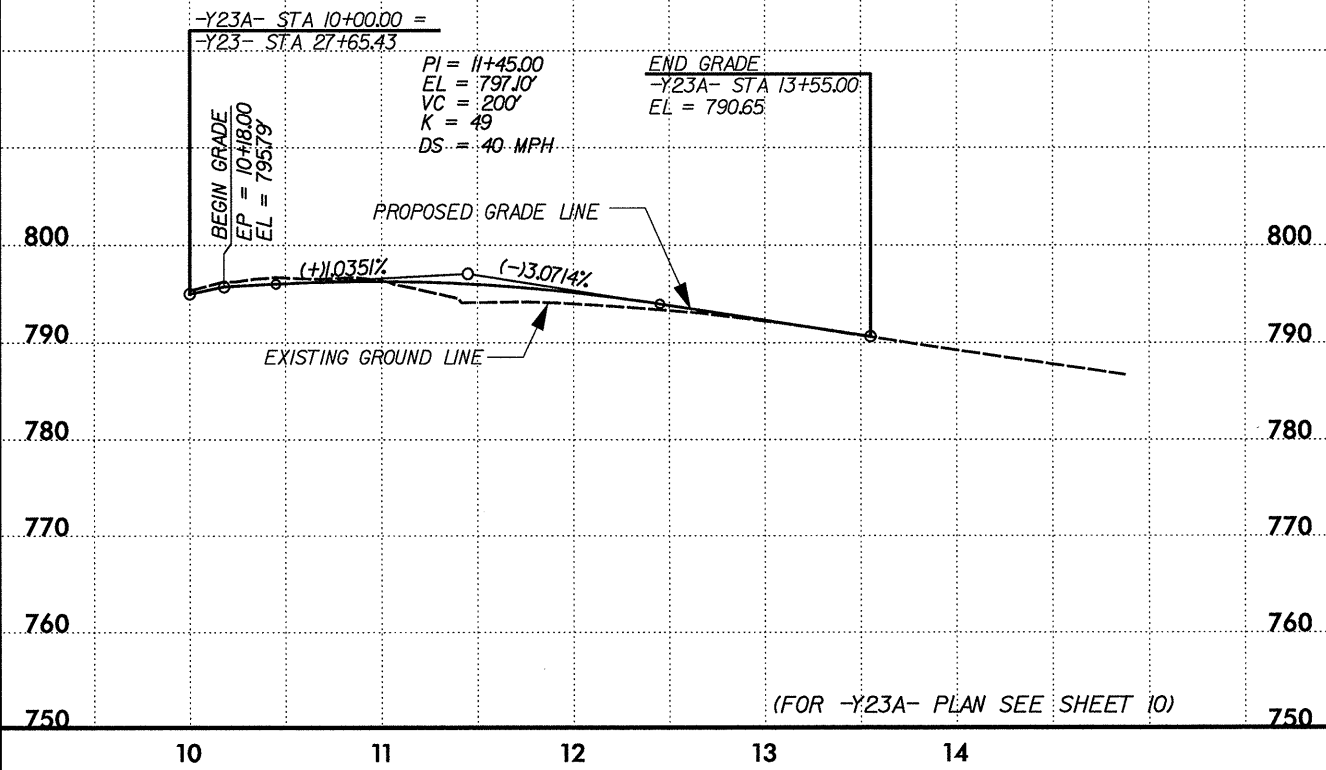
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SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-12	22 LT	27+00	10.0-11.5	A-7-5(20)	60	17	4.0	16.0	35.4	44.6	100.0	98.2	84.7	-	-

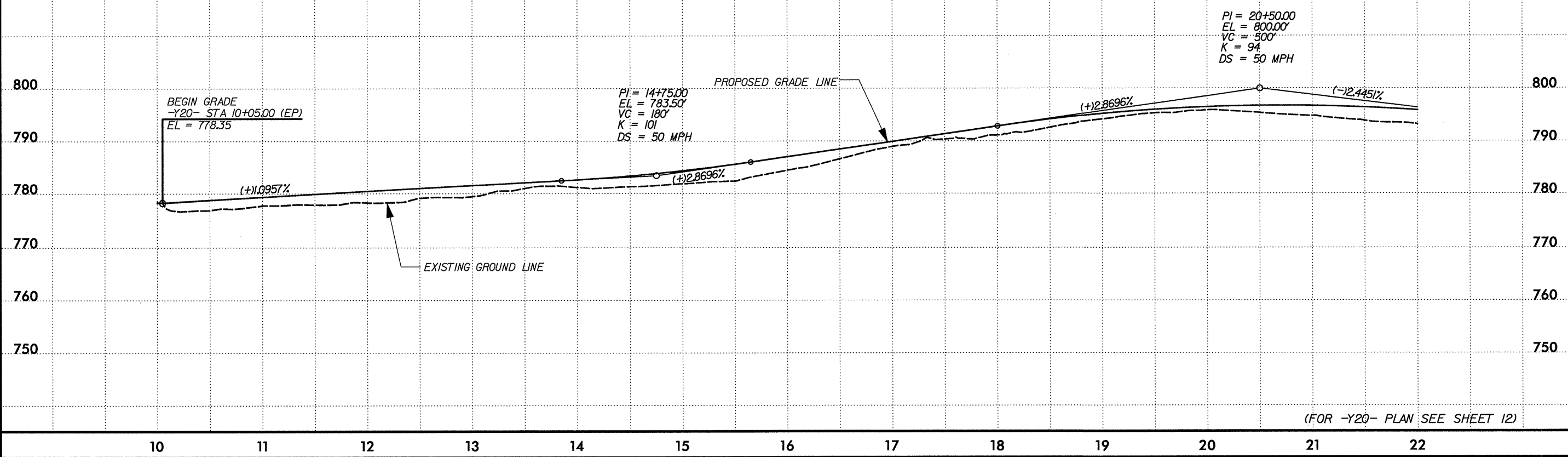
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 Florence & Hutcheson, Inc.

# -Y23A-

PROJECT REFERENCE NO. P-5206A	SHEET NO. 21
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	



# -Y20-



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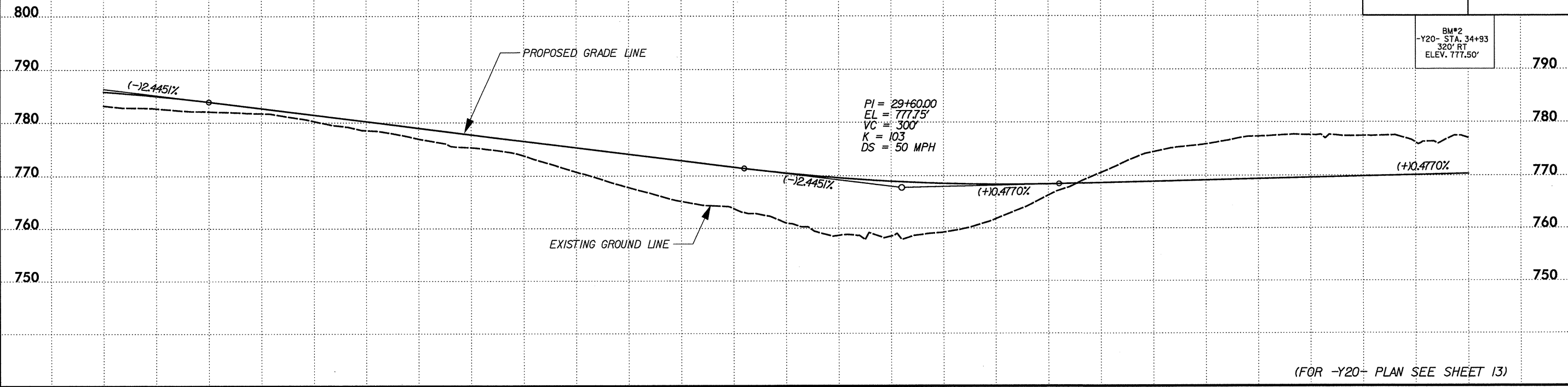


# -Y20-

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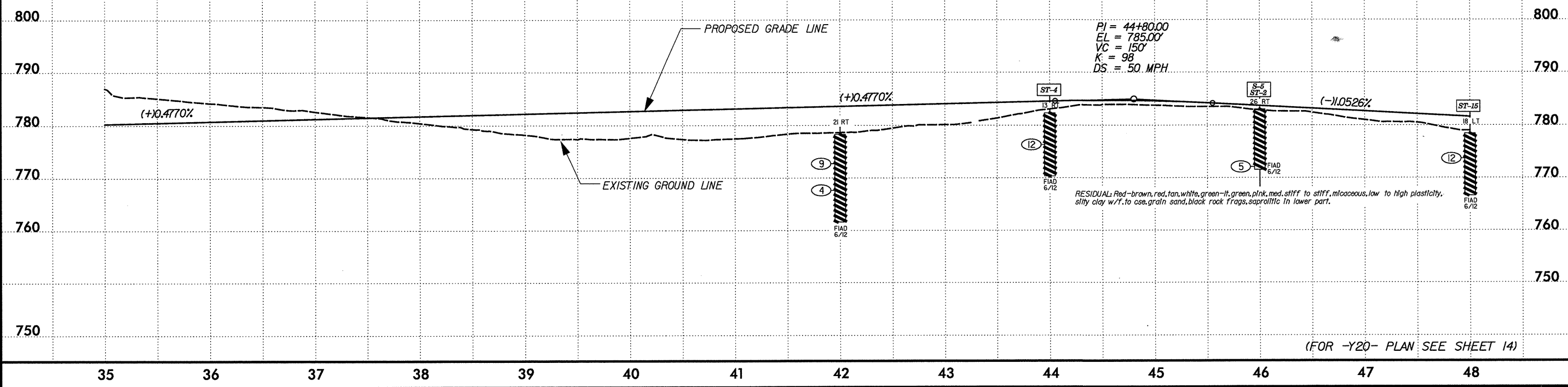
PROJECT REFERENCE NO. P-5206A	SHEET NO. 22
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

BM#2  
 -Y20- STA. 34+93  
 320' RT  
 ELEV. 777.50'



# -Y20-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-4	13 RT	44+00	10.0-11.5	A-7-5(9)	53	12	16.1	23.3	37.3	23.3	99.5	90.7	66.1	-	-
S-5	26 RT	46+00	0.0-11.0	A-7-5(29)	69	30	8.5	11.9	24.8	54.8	98.2	94.5	80.4	-	-
ST-2	26 RT	46+00	5.0-6.2	A-7-5(30)	67	26	3.1	11.4	37.0	48.6	100.0	98.9	89.0	-	-
ST-15	18 LT	48+00	9.8-10.3	A-7-6(11)	46	16	16.5	18.1	36.8	28.6	99.8	90.5	70.4	-	-



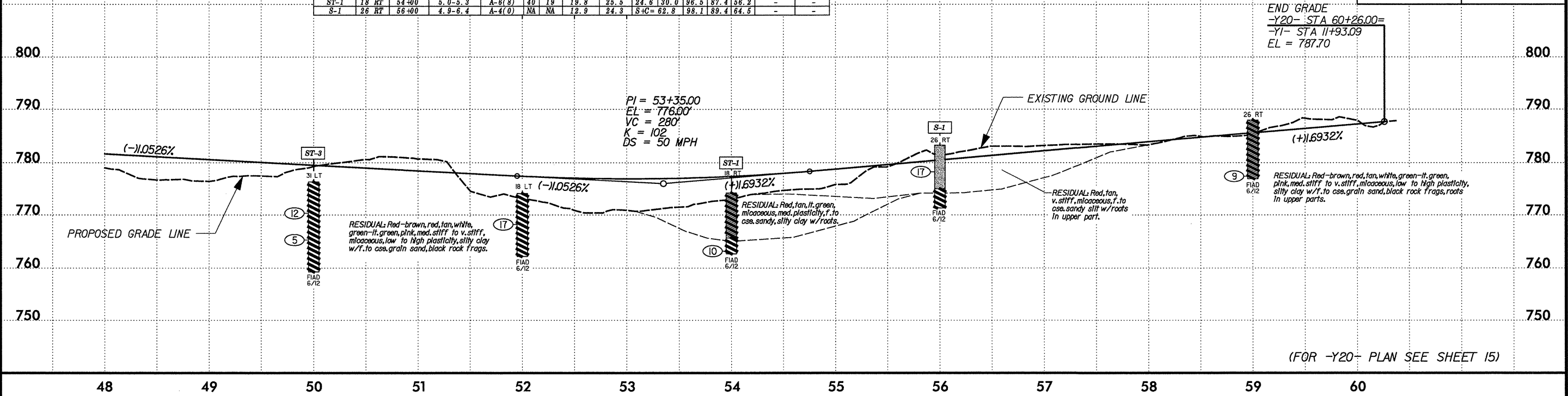
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# -Y20-

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 NC License No: P-0258

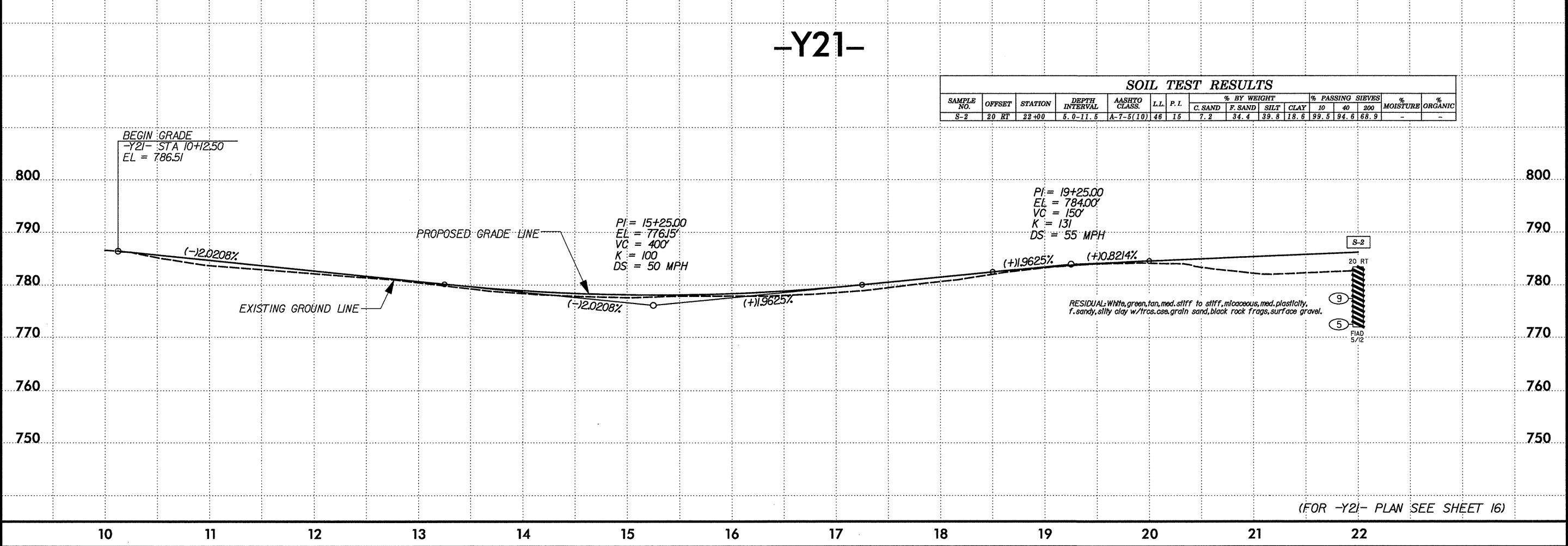
PROJECT REFERENCE NO. P-5206A	SHEET NO. 23
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-3	31 LT	50+00	15.0-16.4	A-7-5(16)	62	13	8.5	17.0	42.2	32.3	100.0	96.6	79.6	-	-
ST-1	18 RT	54+00	5.0-5.3	A-6(8)	40	19	19.8	25.5	24.8	30.0	96.6	87.4	56.2	-	-
S-1	26 RT	56+00	4.9-8.4	A-4(0)	NA	NA	12.9	24.3	S+C= 62.8	98.1	89.4	64.5	-	-	



# -Y21-

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-2	20 RT	22+00	5.0-11.5	A-7-5(10)	46	15	7.2	34.4	39.8	18.6	99.5	94.6	68.9		

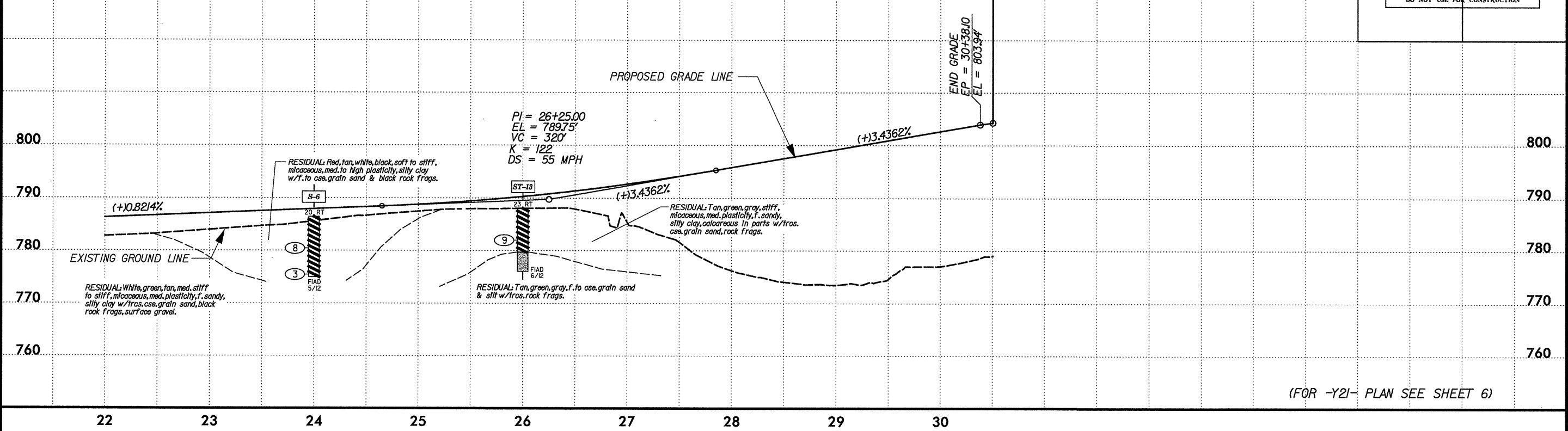


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**-Y21-**

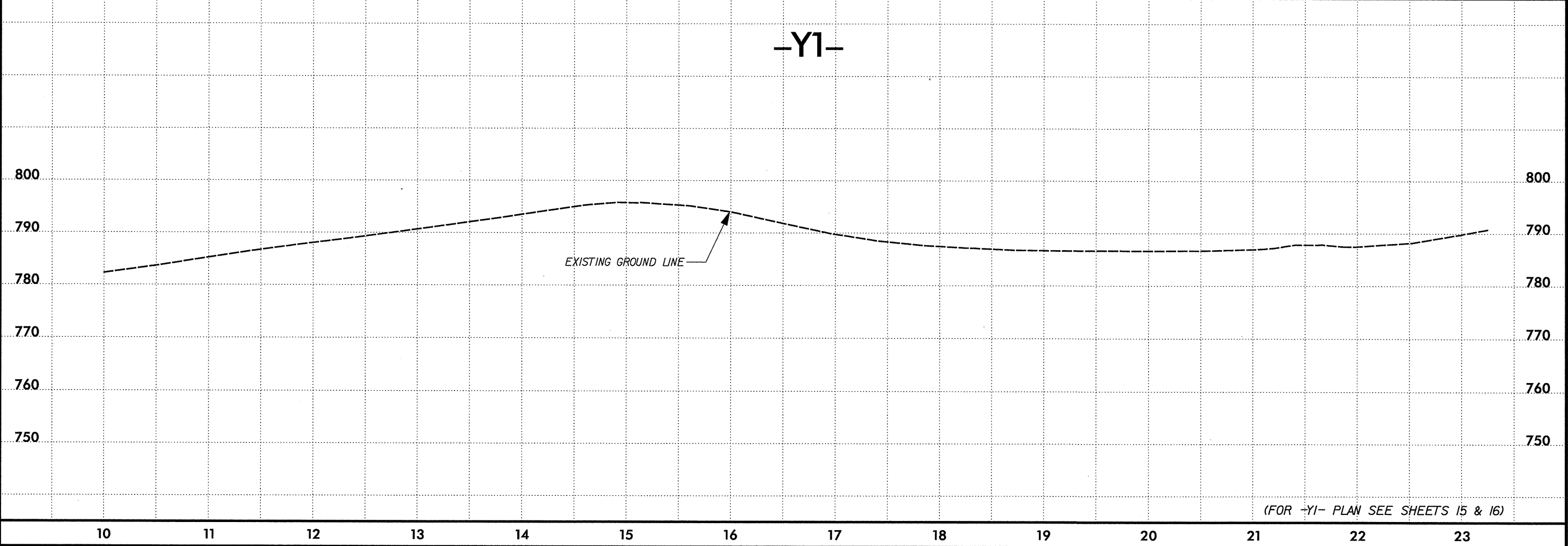
PROJECT REFERENCE NO. P-5206A	SHEET NO. 24
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
<b>INCOMPLETE PLANS</b> DO NOT USE FOR R/W ACQUISITION <b>PRELIMINARY PLANS</b> DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-6	20 RT	24+00	1.5-10.0	A-7-5(56)	91	48	2.8	5.9	24.0	67.3	100.0	98.2	93.0	-	-
ST-13	23 RT	26+00	10.0-11.0	A-4(1)	40	4	8.4	44.1	27.2	20.3	90.8	87.7	52.2	-	-



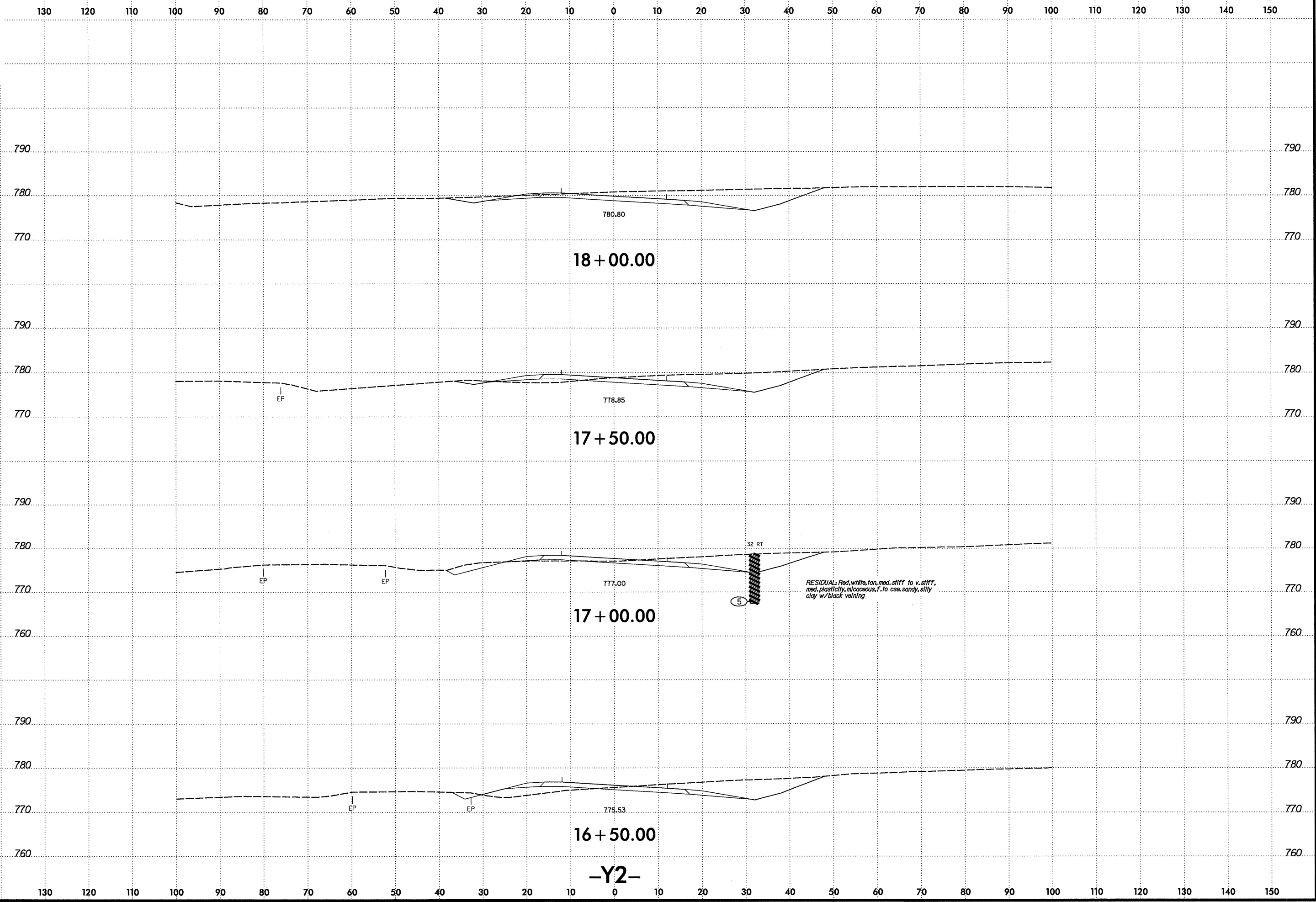
(FOR -Y21- PLAN SEE SHEET 6)

**-Y1-**

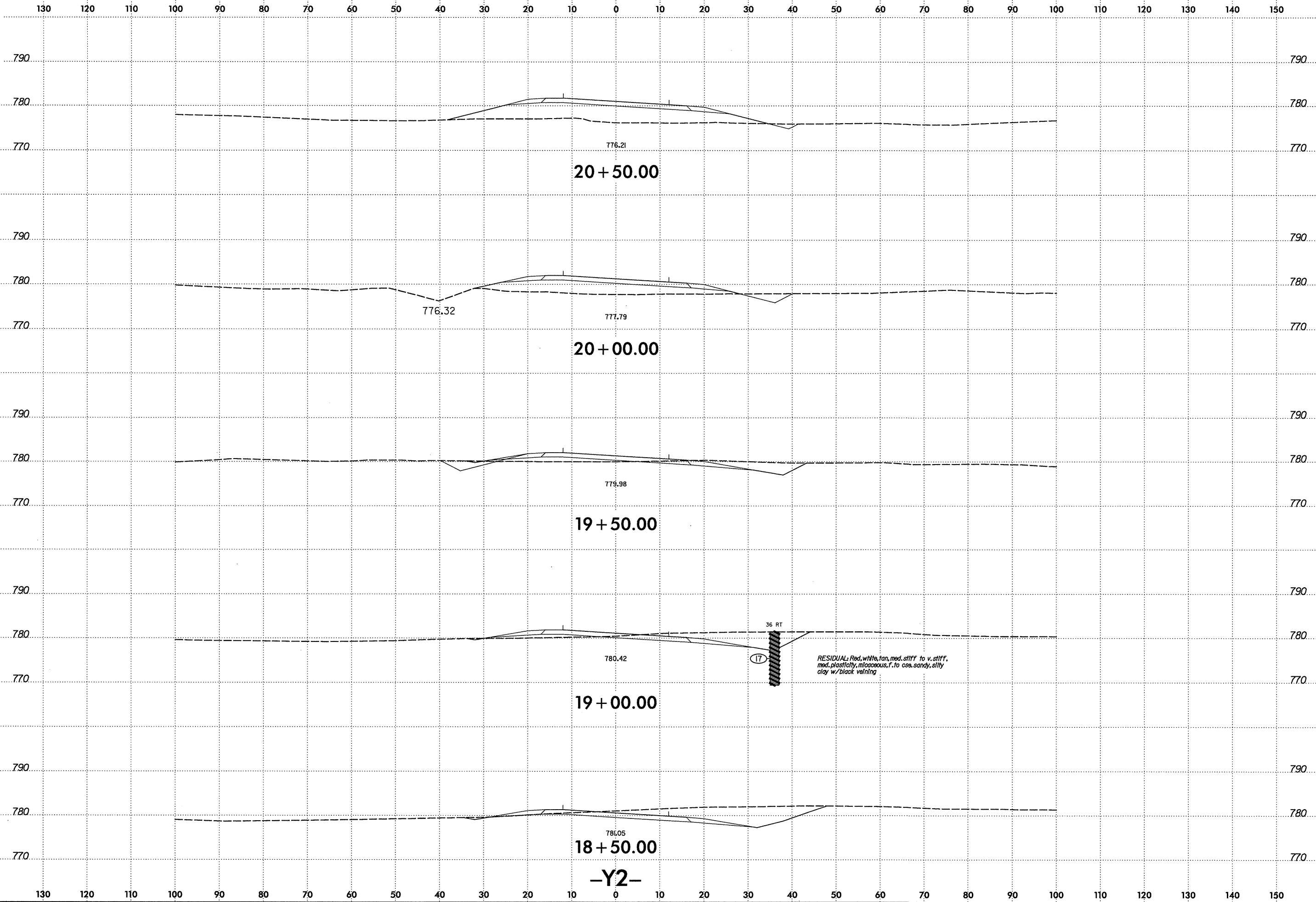


(FOR -Y1- PLAN SEE SHEETS 15 & 16)

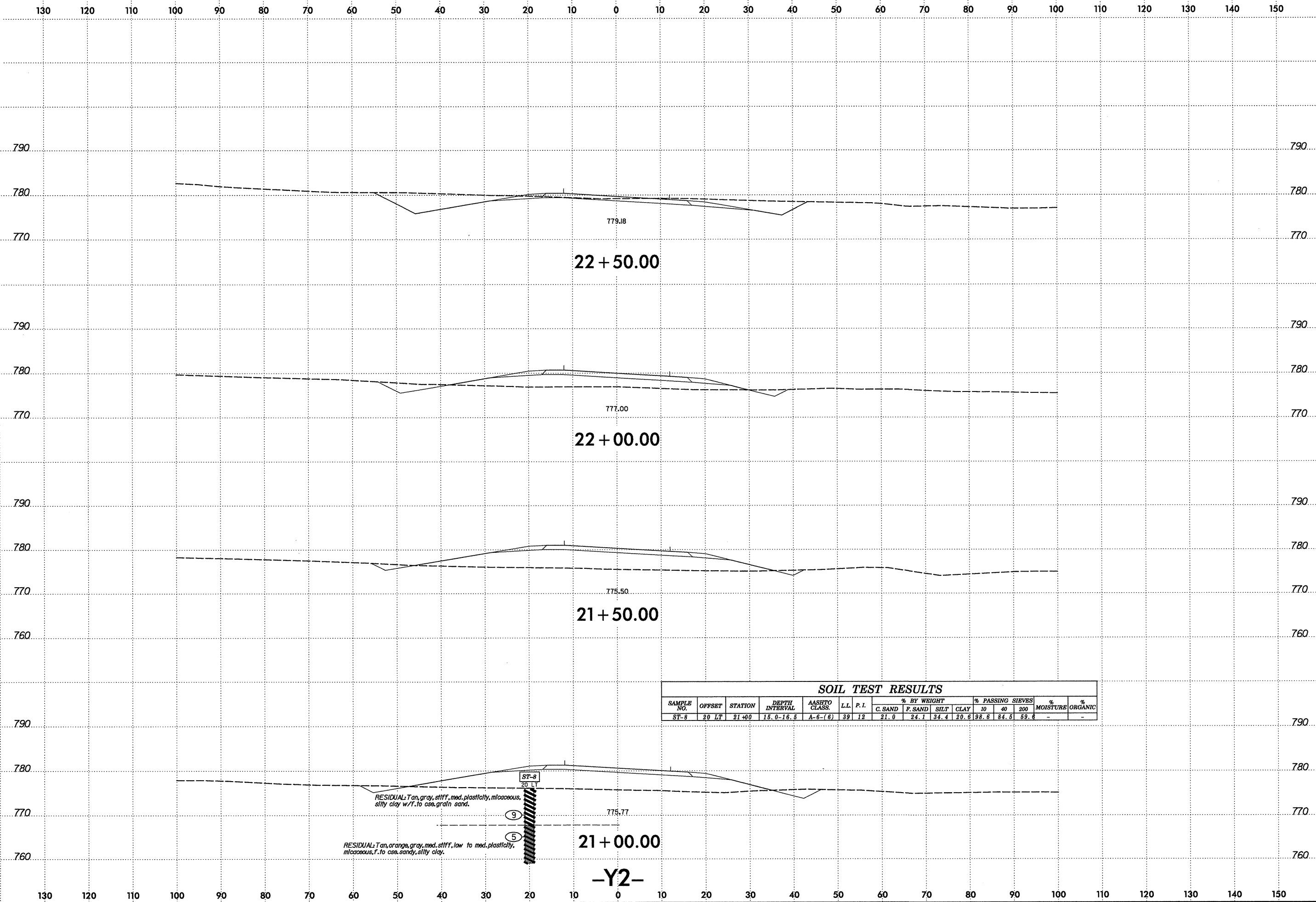
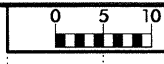
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 Florence & Hutcherson, Inc.



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 Florence & Hutcheson, Inc.

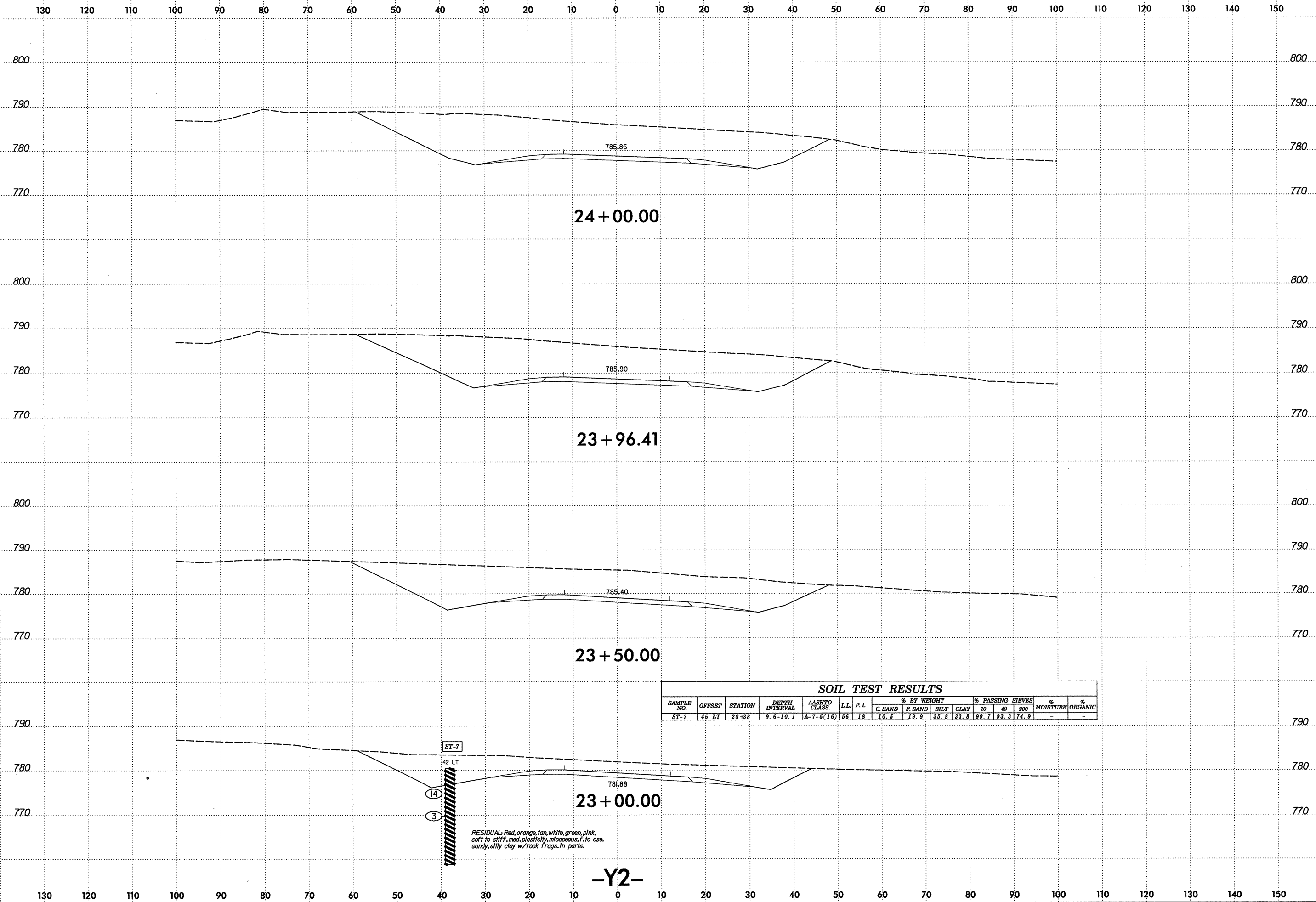


SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40		
ST-8	20 LT	21+00	15.0-16.5	A-6-(6)	39	12	21.0	24.1	34.4	20.6	98.6	84.5	69.6	-

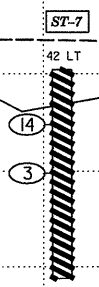
RESIDUAL: Tan, gray, stiff, med. plasticity, micaceous, silty clay w/ f. to csa. grain sand.

RESIDUAL: Tan, orange, gray, med. stiff, low to med. plasticity, micaceous, f. to csa. sandy, silty clay.



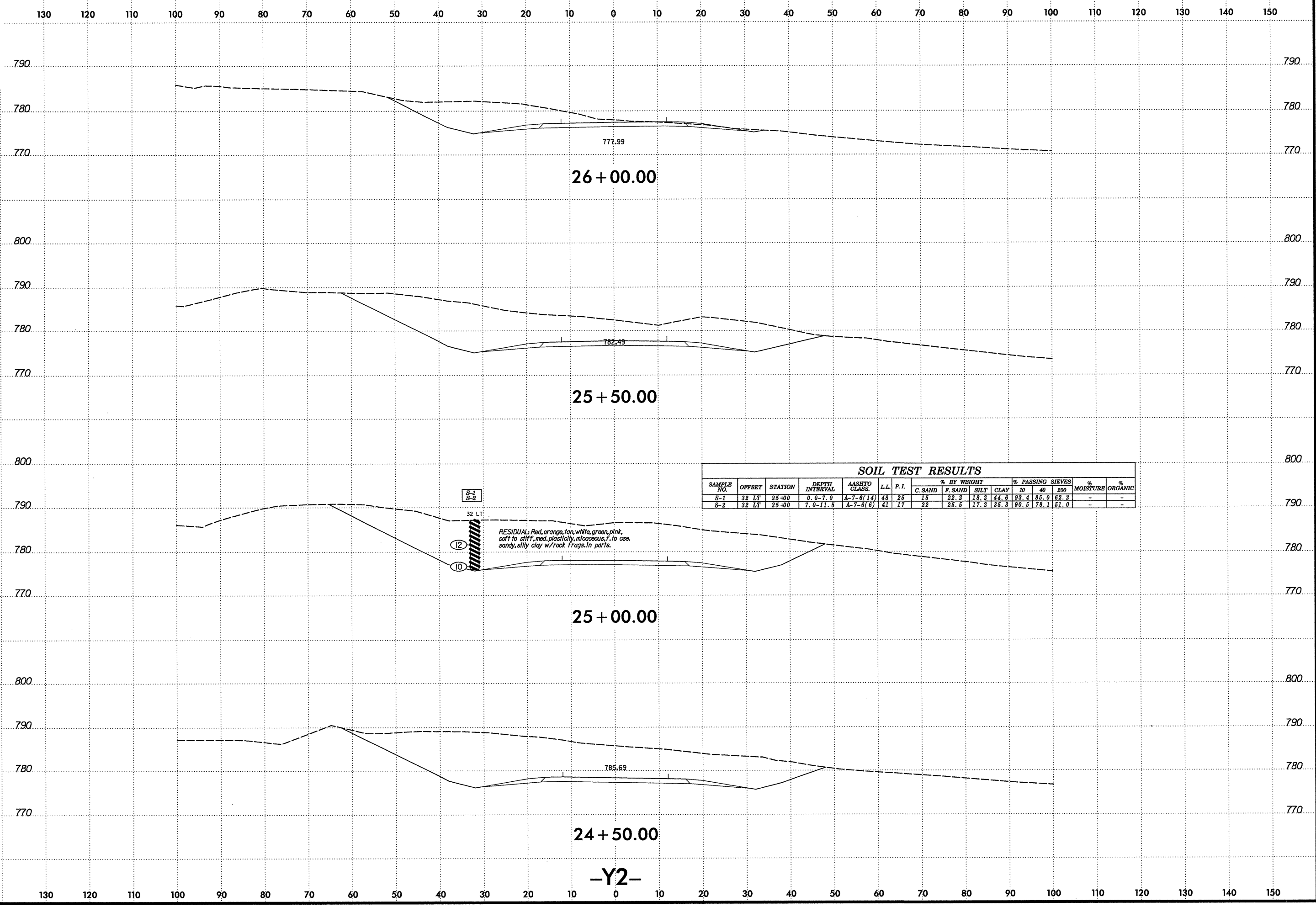
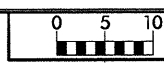


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-7	45 LT	28+38	9.6-10.1	A-7-5(16)	56	18	10.5	19.9	35.8	33.8	99.7	93.3	74.9	-	-



RESIDUAL: Red, orange, tan, white, green, pink, soft to stiff, med. plasticity, micaceous, f. to csa, sandy, silty clay w/rock frags. In parts.

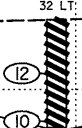
-Y2-



**SOIL TEST RESULTS**

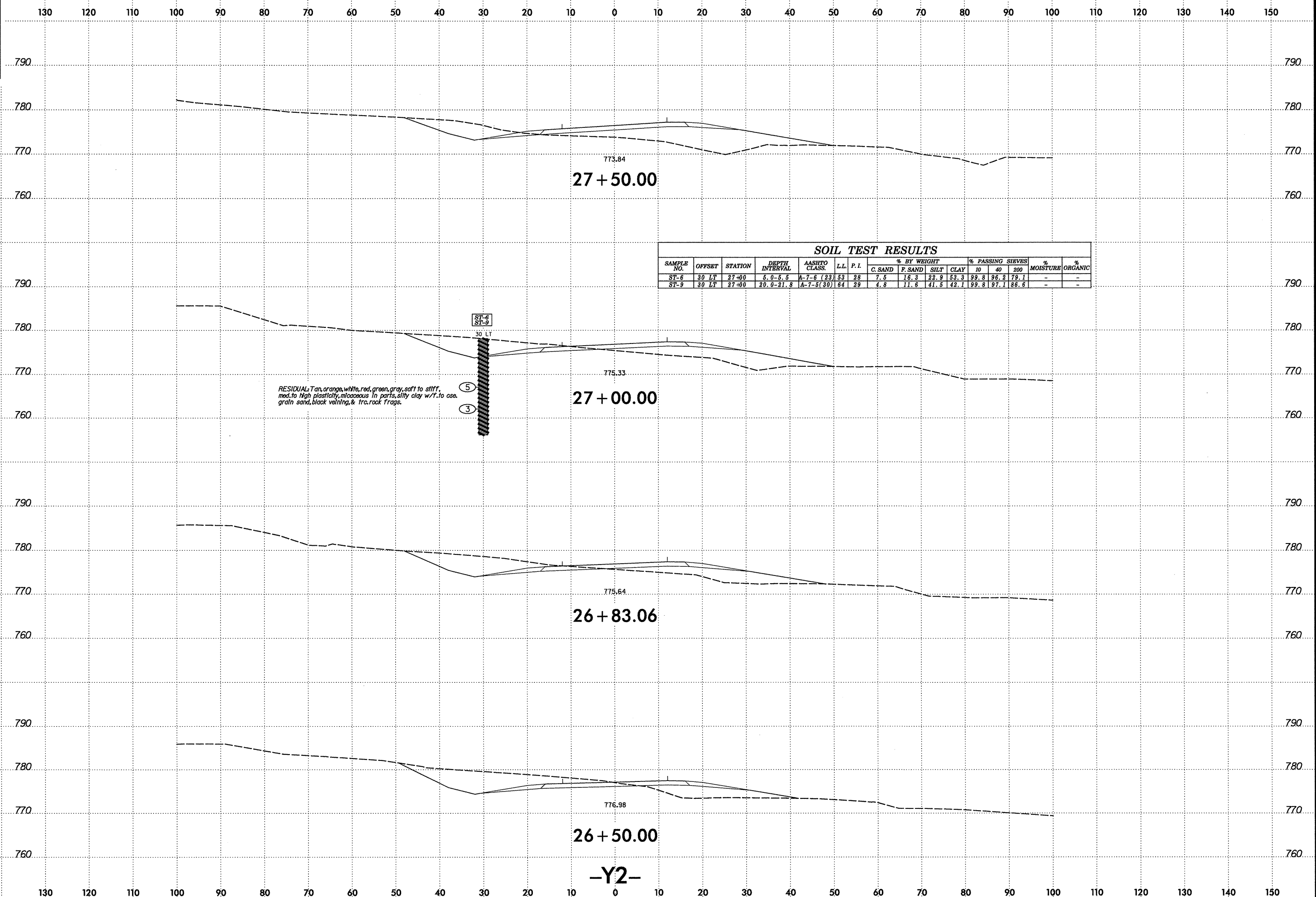
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-1	32 LT	25+00	0.0-7.0	A-7-6(14)	48	26	15	22.2	18.2	44.6	93.4	85.0	62.2	-	-
S-2	32 LT	25+00	7.0-11.5	A-7-6(6)	41	17	22	25.5	17.2	35.3	90.5	78.1	51.0	-	-

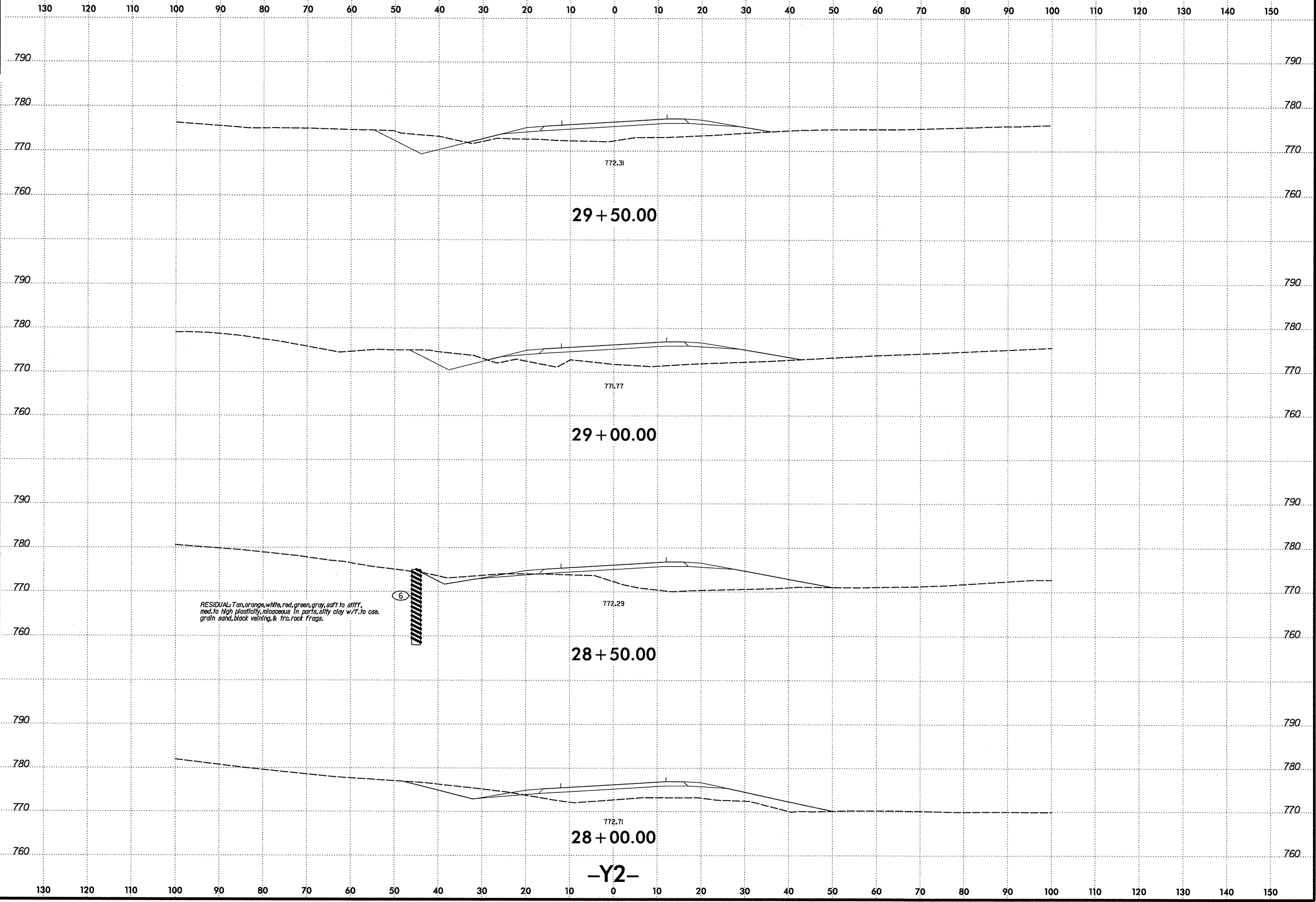
S-1  
S-2



RESIDUAL: Red, orange, tan, white, green, pink, soft to stiff, med. plasticity, micaceous, f. to csa. sandy, silty clay w/rock frags. In parts.

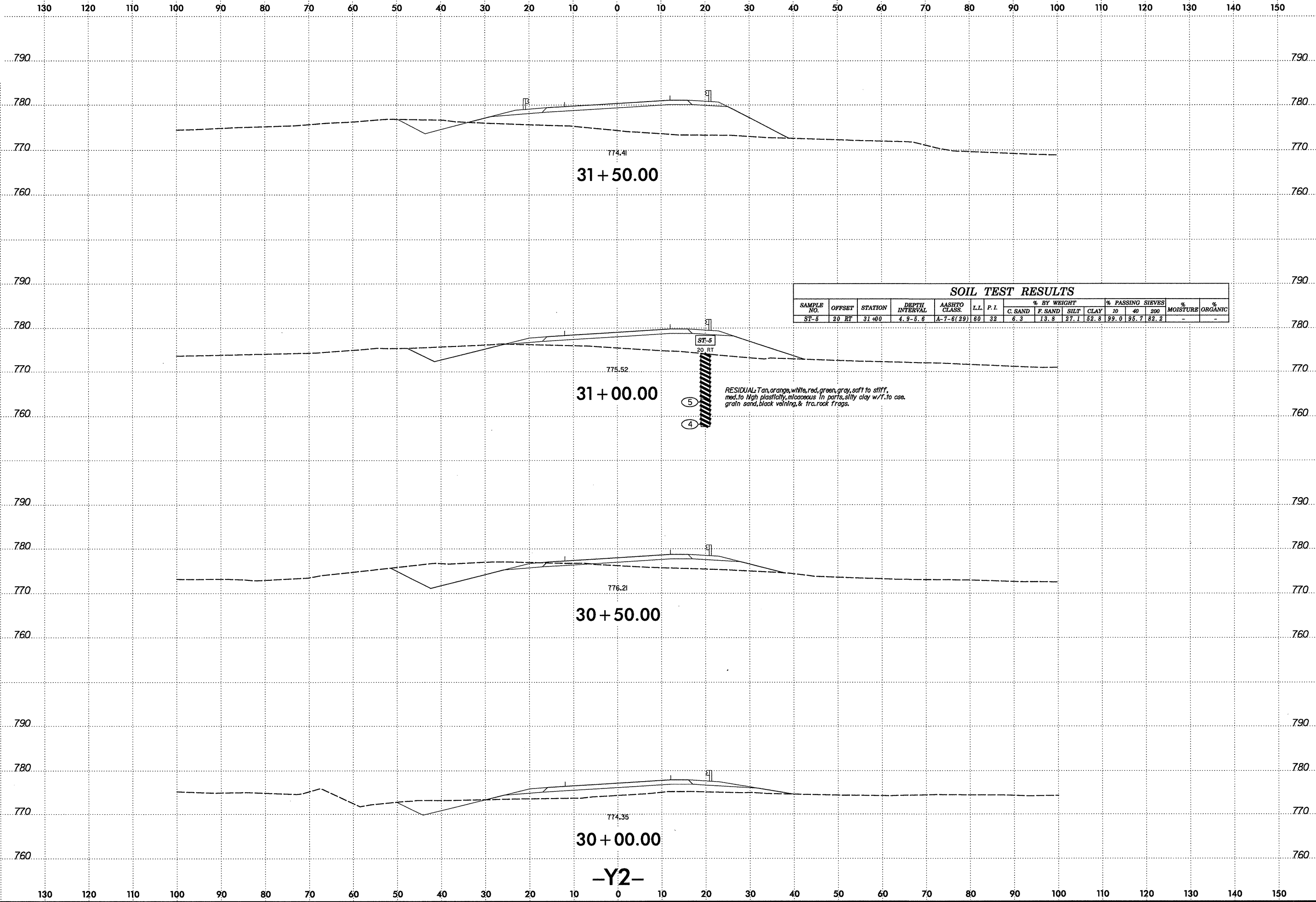






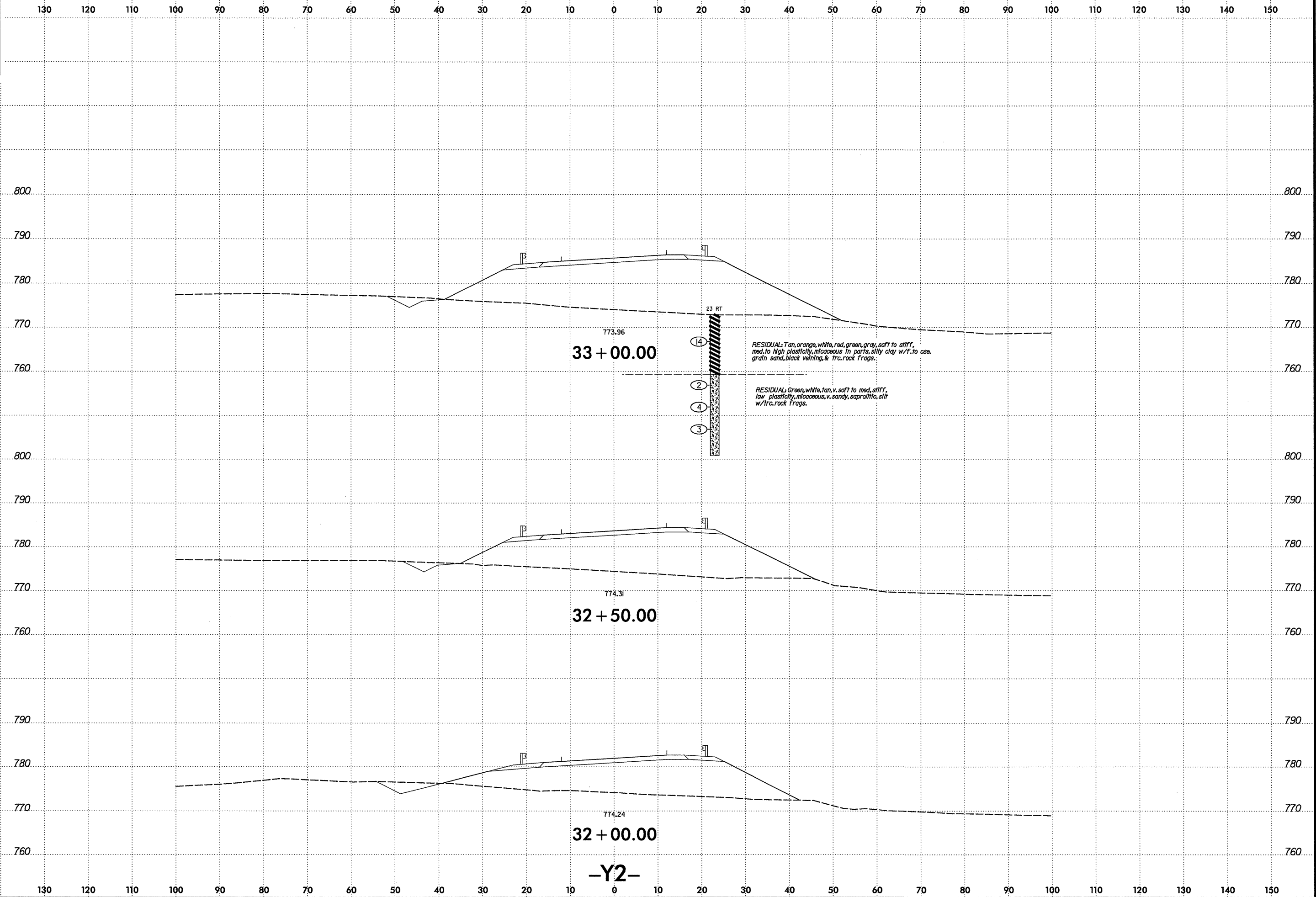
RESIDUAL: Tan, orange, white, red, green, gray, soft to stiff, med. to high plasticity, micaceous. In parts, silty clay w/f. to coarse grain sand, black veining, & tra. rock frags.

-Y2-

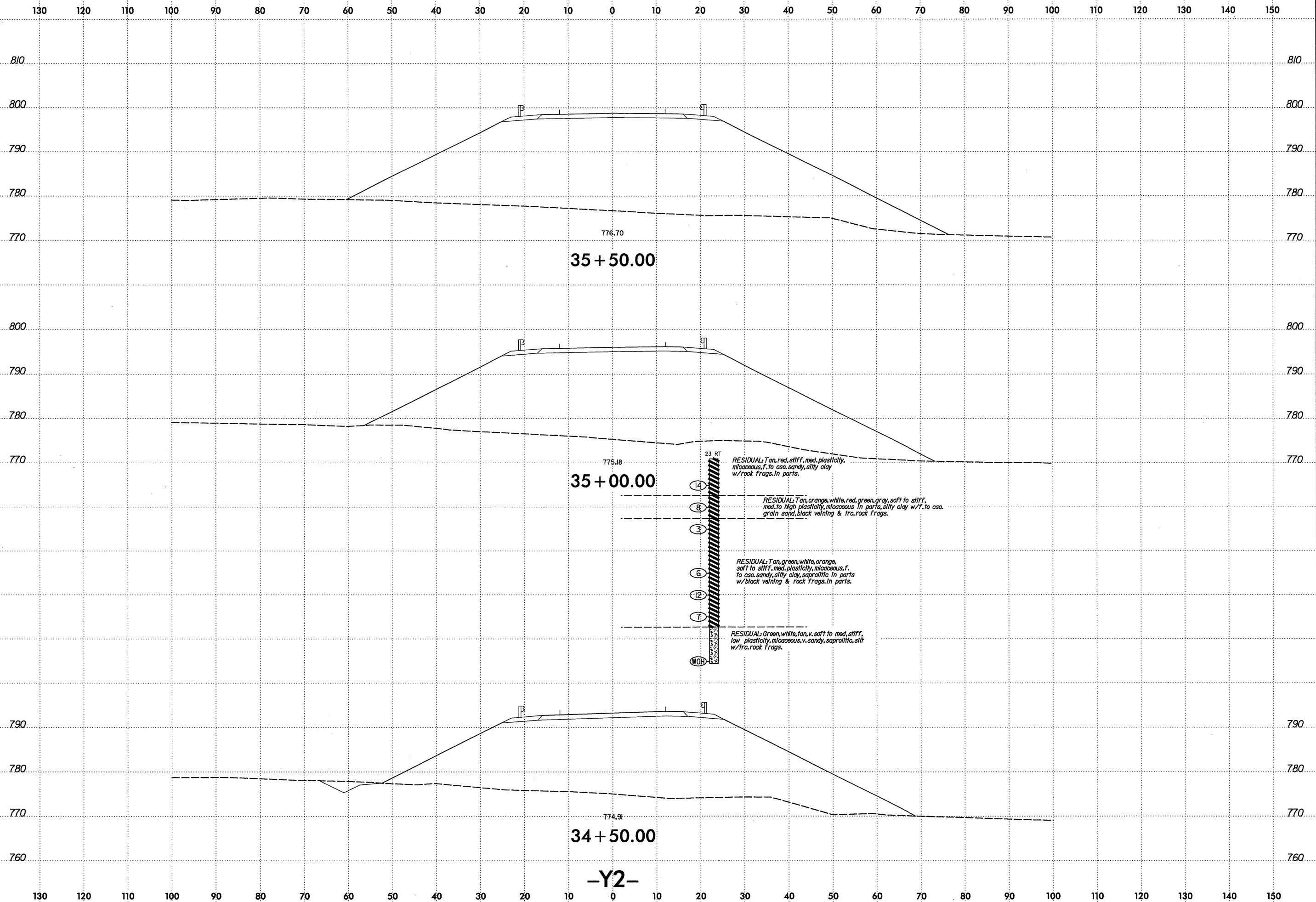


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 Florence & Hutcheson, Inc.

Rock



Rock

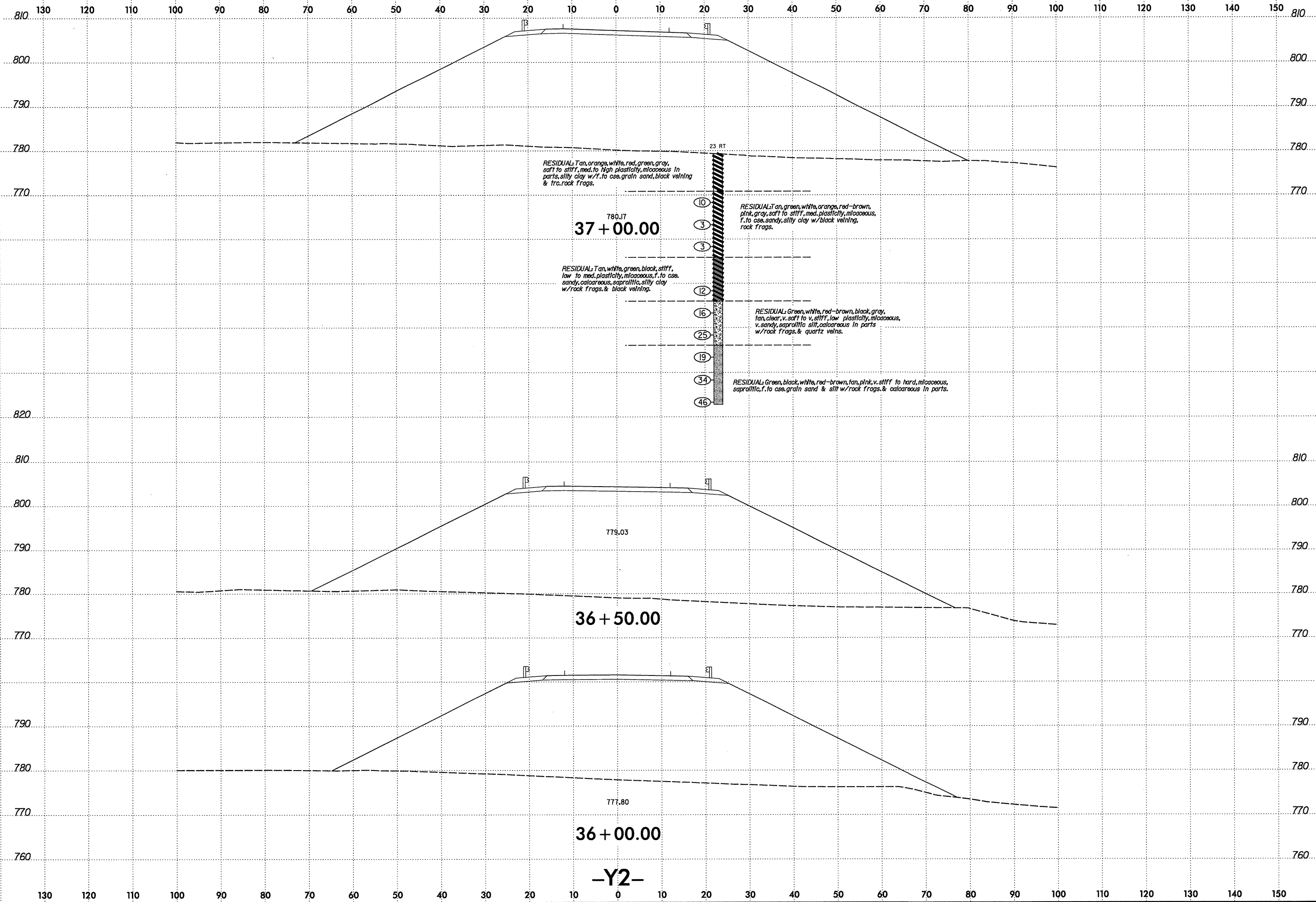


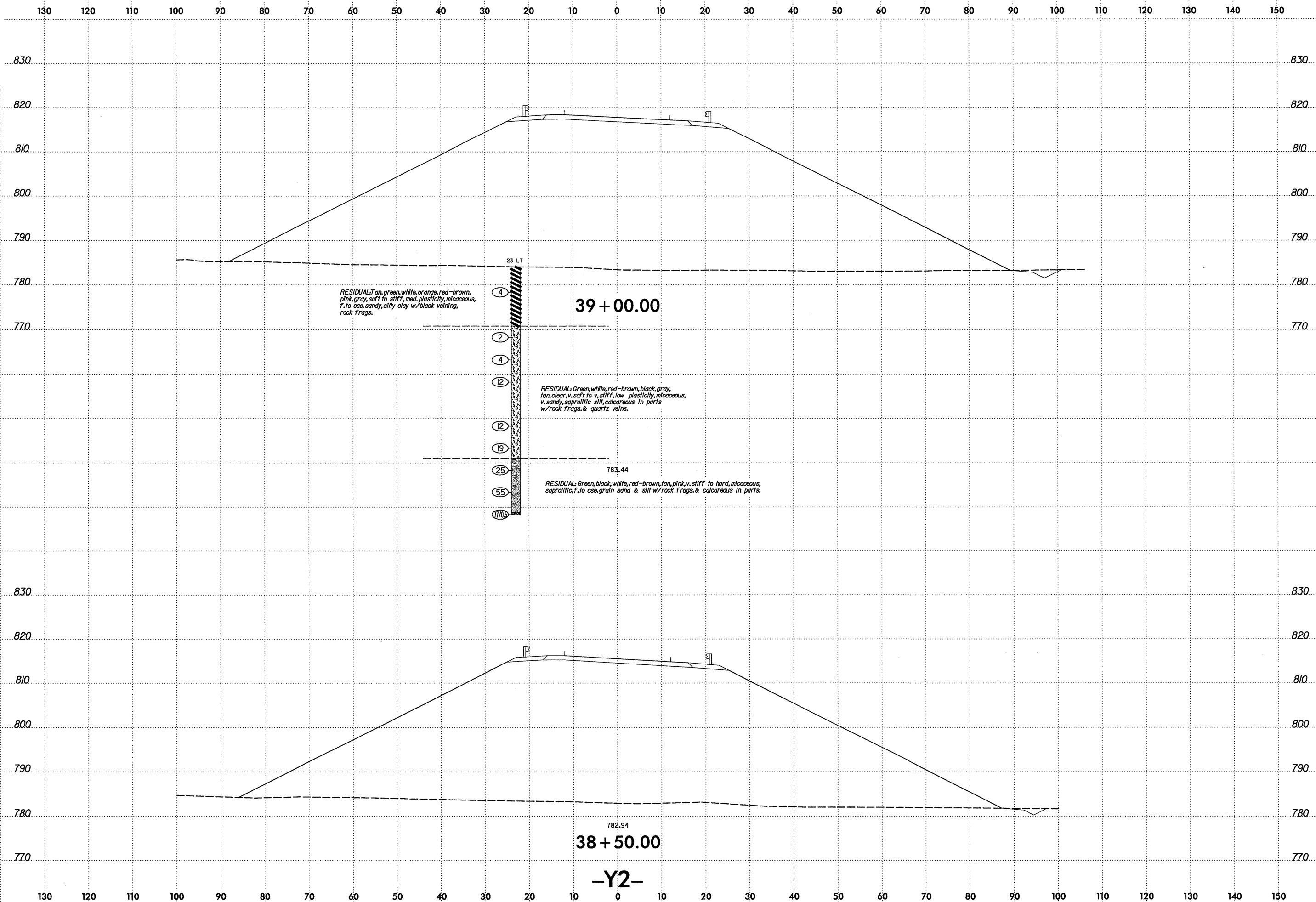
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-Y2-



DOCK



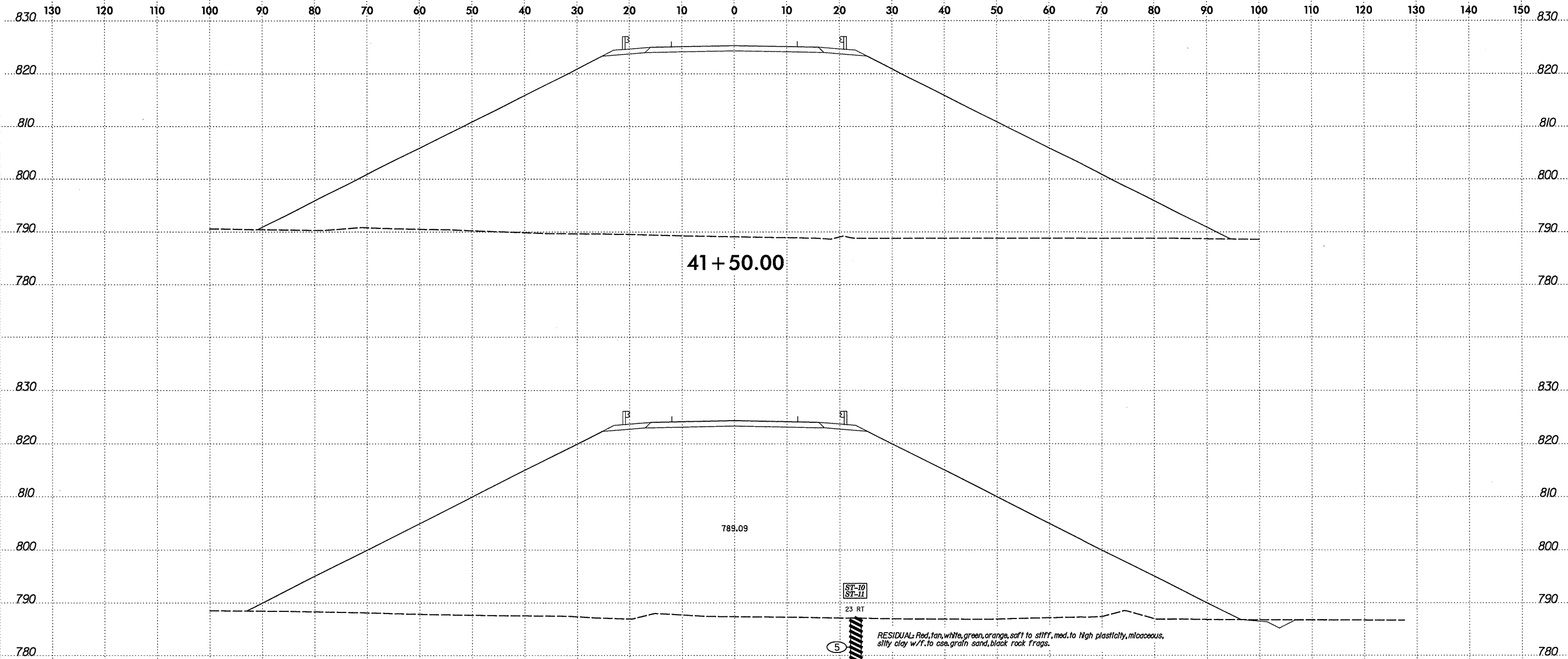


RESIDUAL: tan, green, white, orange, red-brown, pink, gray, soft to stiff, med. plasticity, micaceous, f. to cse. sandy, silty clay w/ black veining, rock frags.

RESIDUAL: Green, white, red-brown, black, gray, tan, clear, v. soft to v. stiff, low plasticity, micaceous, v. sandy, saprolitic silt, calcareous in parts w/ rock frags. & quartz veins.

RESIDUAL: Green, black, white, red-brown, tan, pink, v. stiff to hard, micaceous, saprolitic, f. to cse. grain sand & silt w/ rock frags. & calcareous in parts.

- 23 LT
- ④
- ②
- ④
- ⑫
- ⑫
- ⑰
- ⑳
- ⑤⑤
- ①①⑤



**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-10	23 RT	41+00	39.5-40.5	A-5(7)	51	7	5.4	34.8	50.0	9.9	99.9	97.4	69.5	-	-
ST-11	23 RT	41+00	59.5-60.8	A-4(1)	36	7	29.5	34.2	28.1	8.2	99.2	80.0	43.2	-	-

RESIDUAL: Red, tan, white, green, orange, soft to stiff, med. to high plasticity, micaceous, silty clay w/f. to csa. grain sand, black rock frags.

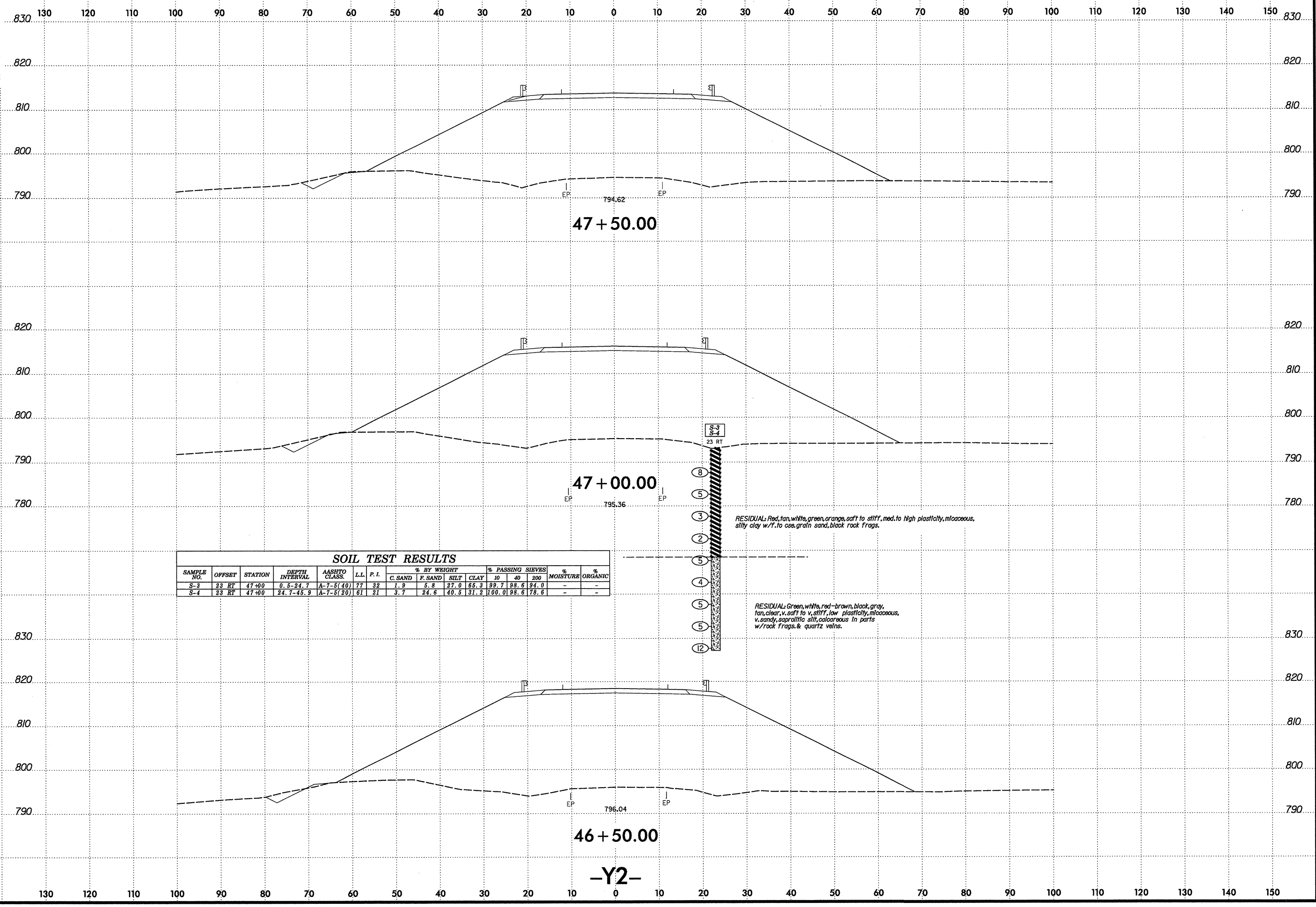
RESIDUAL: Green, white, red-brown, black, gray, tan, clear, v. soft to v. stiff, low plasticity, micaceous, v. sandy, saprolite silt, calcareous in parts w/rock frags. & quartz veins.

RESIDUAL: Green, black, white, red-brown, tan, pink, v. stiff to hard, micaceous, saprolite, f. to csa. grain sand & silt w/rock frags. & calcareous in parts.

41+00.00

-Y2-





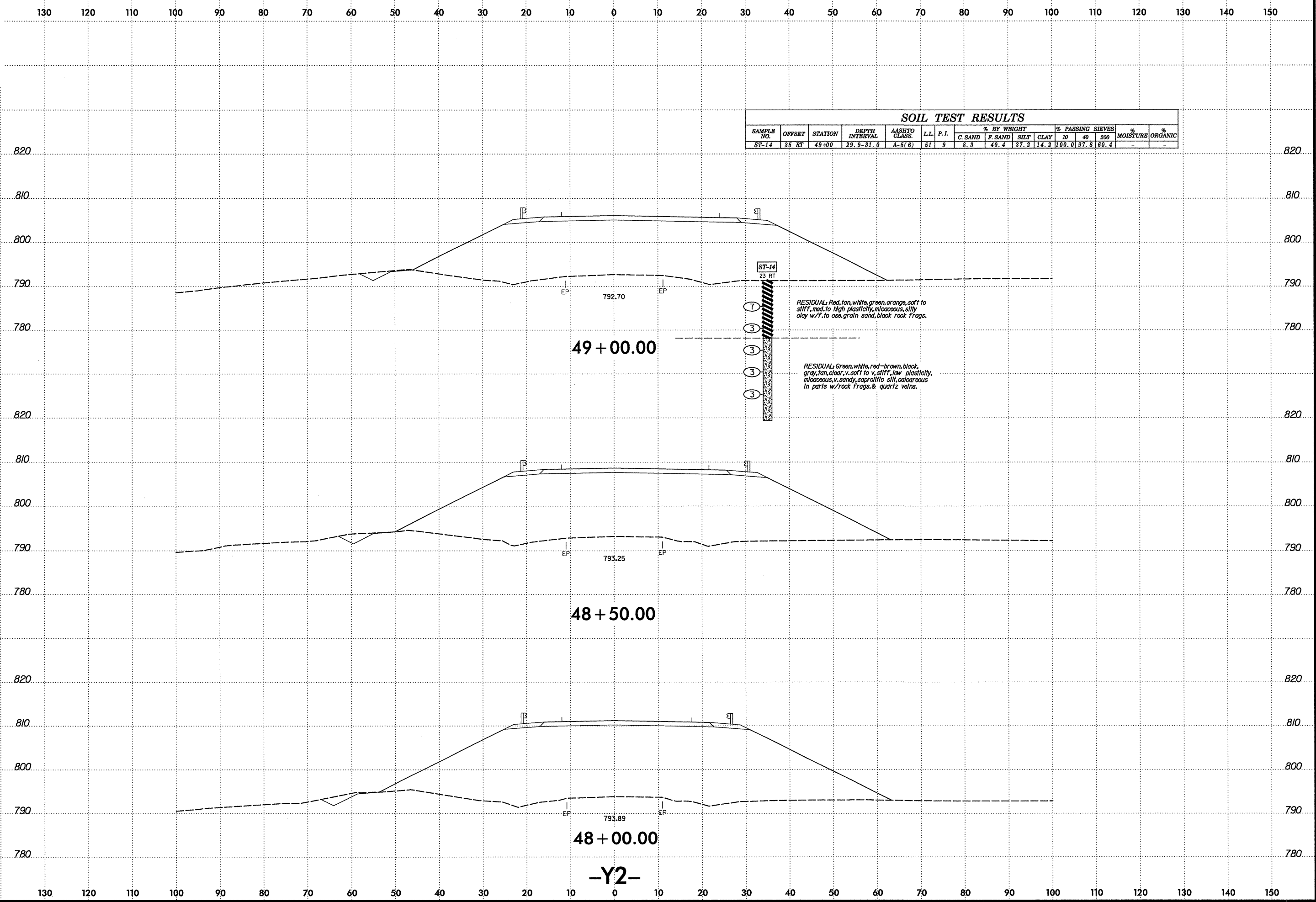
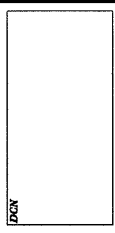
**SOIL TEST RESULTS**

SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-3	23 RT	47+00	0.5-24.7	A-7-5(10)	77	32	1.9	5.8	27.0	65.3	99.7	98.6	94.0	-	-
S-4	23 RT	47+00	24.7-45.9	A-7-5(20)	61	21	3.7	24.6	40.5	31.2	100.0	98.6	78.6	-	-

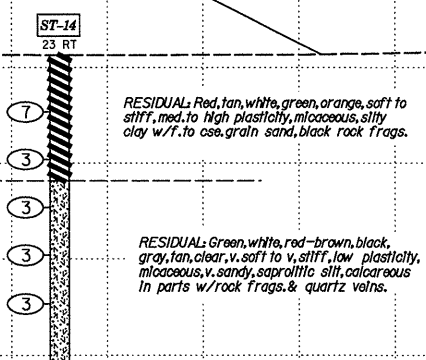
RESIDUAL: Red, tan, white, green, orange, soft to stiff, med. to high plasticity, micaceous, silty clay w/f. to csa. grain sand, black rock frags.

RESIDUAL: Green, white, red-brown, black, gray, tan, clear, v. soft to v. stiff, low plasticity, micaceous, v. sandy, saprolite silt, calcareous in parts w/rock frags. & quartz veins.

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 Florence & Hutcheson, Inc.

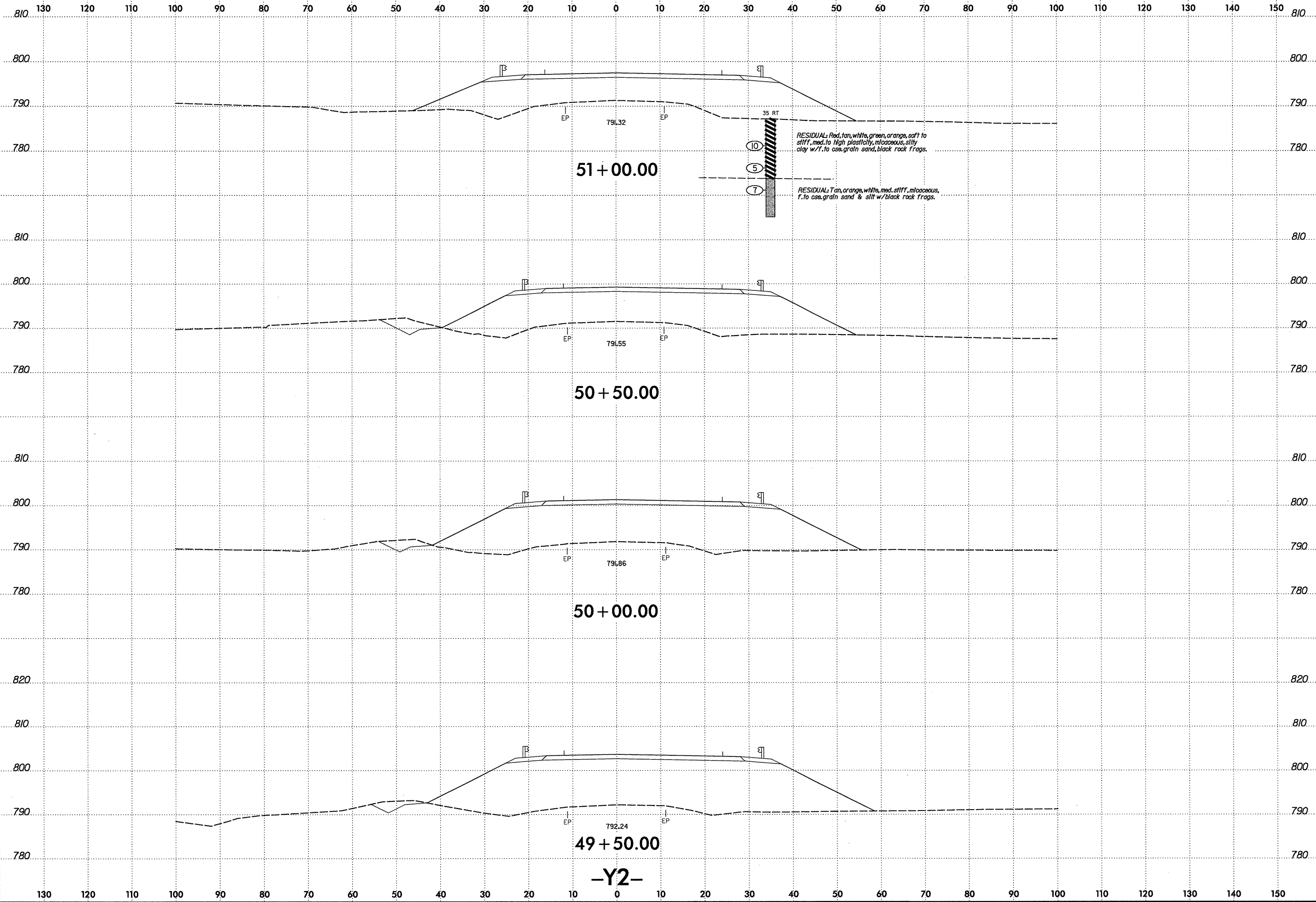


SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P. I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							G. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-14	35 RT	49+00	29.9-31.0	A-5(6)	51	9	8.3	40.4	37.2	14.2	100.0	97.8	60.4	-	-

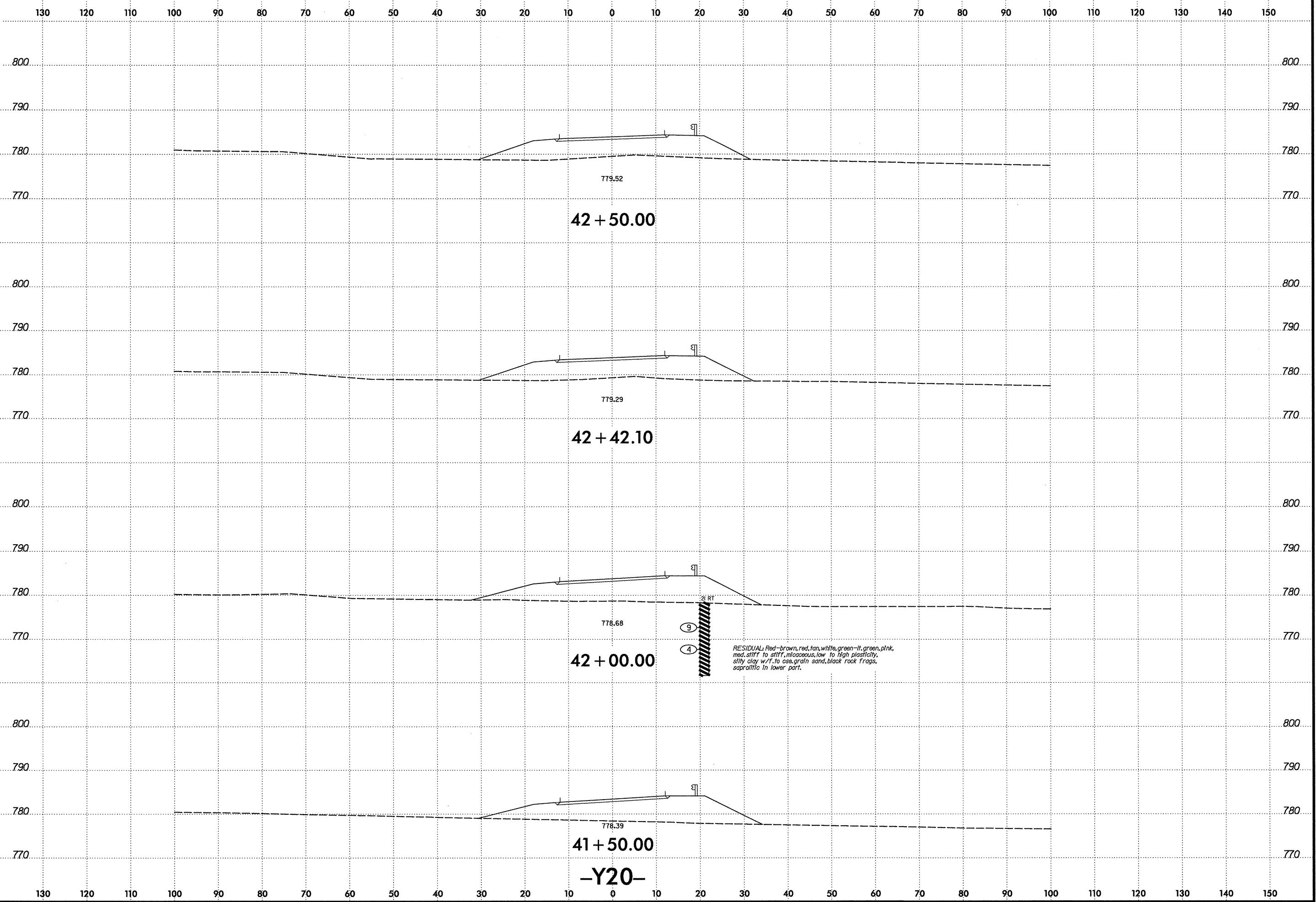




BACK



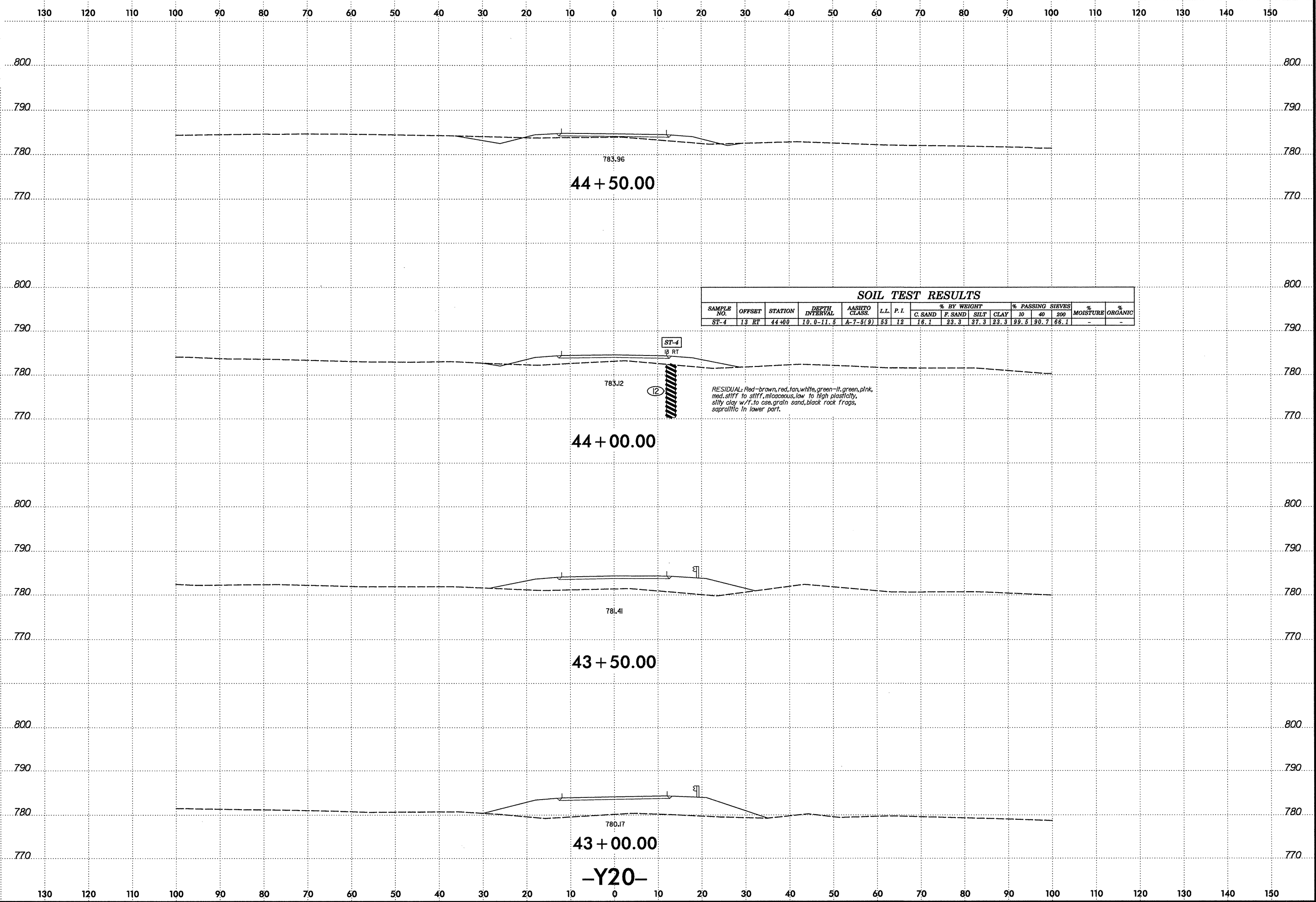
PACK



RESIDUAL: Red-brown, red, tan, white, green-lt, green, pink, med. stiff to stiff, micaceous, low to high plasticity, silty clay w/f. to csa, grain sand, black rock frags, saprolite in lower part.



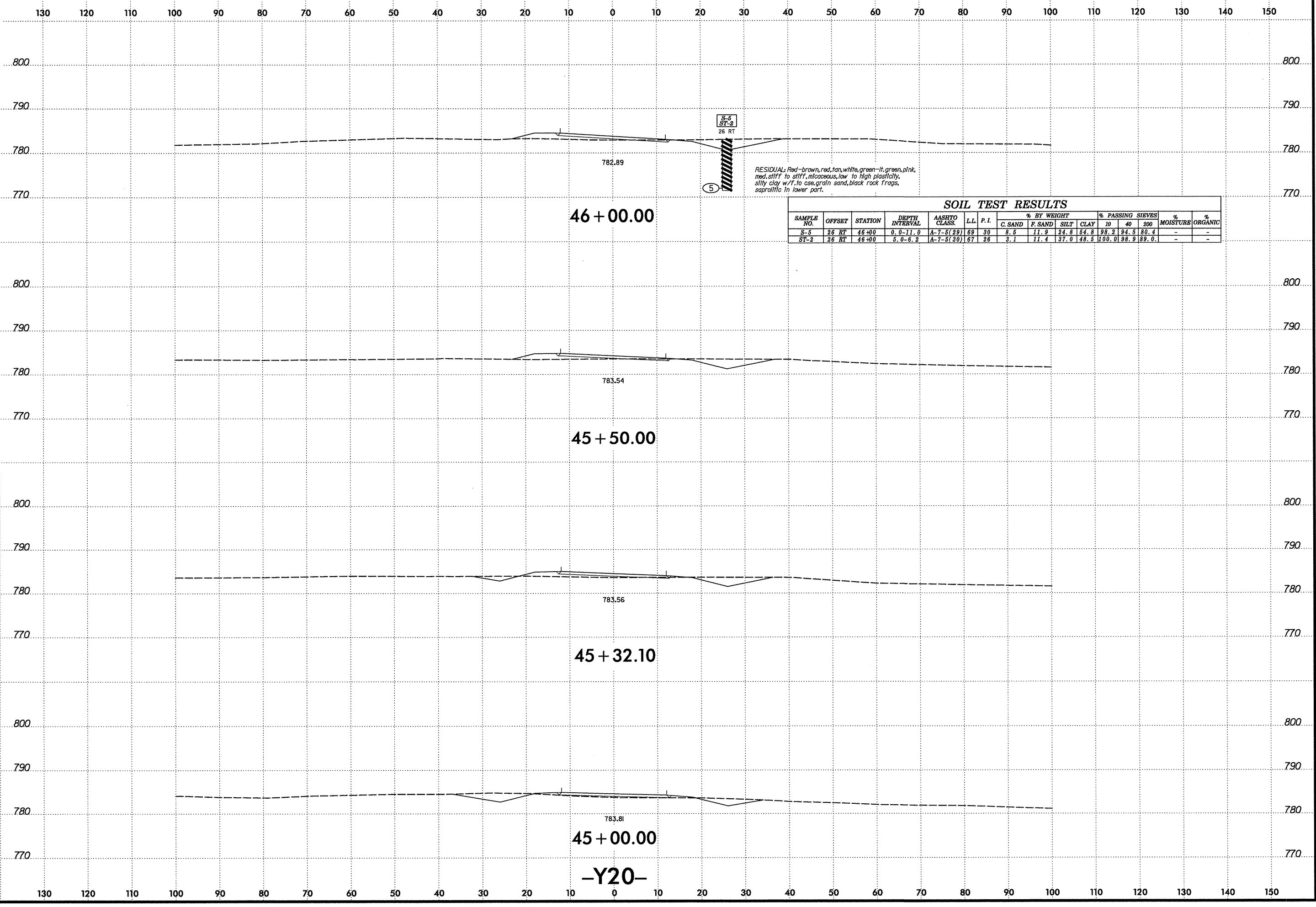
rock



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-4	13 RT	44+00	10.0-11.5	A-7-5(9)	53	12	16.1	23.3	37.3	23.3	99.5	90.7	66.1	-	-

RESIDUAL: Red-brown, red, tan, white, green-lt. green, pink, med. stiff to stiff, micaceous, low to high plasticity, silty clay w/f. to cse. grain sand, black rock frags, saprolitic in lower part.

DOOR



S-5  
ST-2  
26 RT  
5

RESIDUAL: Red-brown, red, tan, white, green-lt. green, pink, med. stiff to stiff, micaceous, low to high plasticity, silty clay w/f. to css. grain sand, black rock frags, saprolite in lower part.

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	75	40	200		
S-5	26 RT	46+00	0.0-11.0	A-7-5(29)	69	30	8.6	11.9	24.8	54.8	98.2	94.6	80.4	-	-
ST-2	26 RT	46+00	5.0-6.2	A-7-5(30)	67	26	3.1	11.4	37.0	48.5	100.0	98.9	89.0	-	-

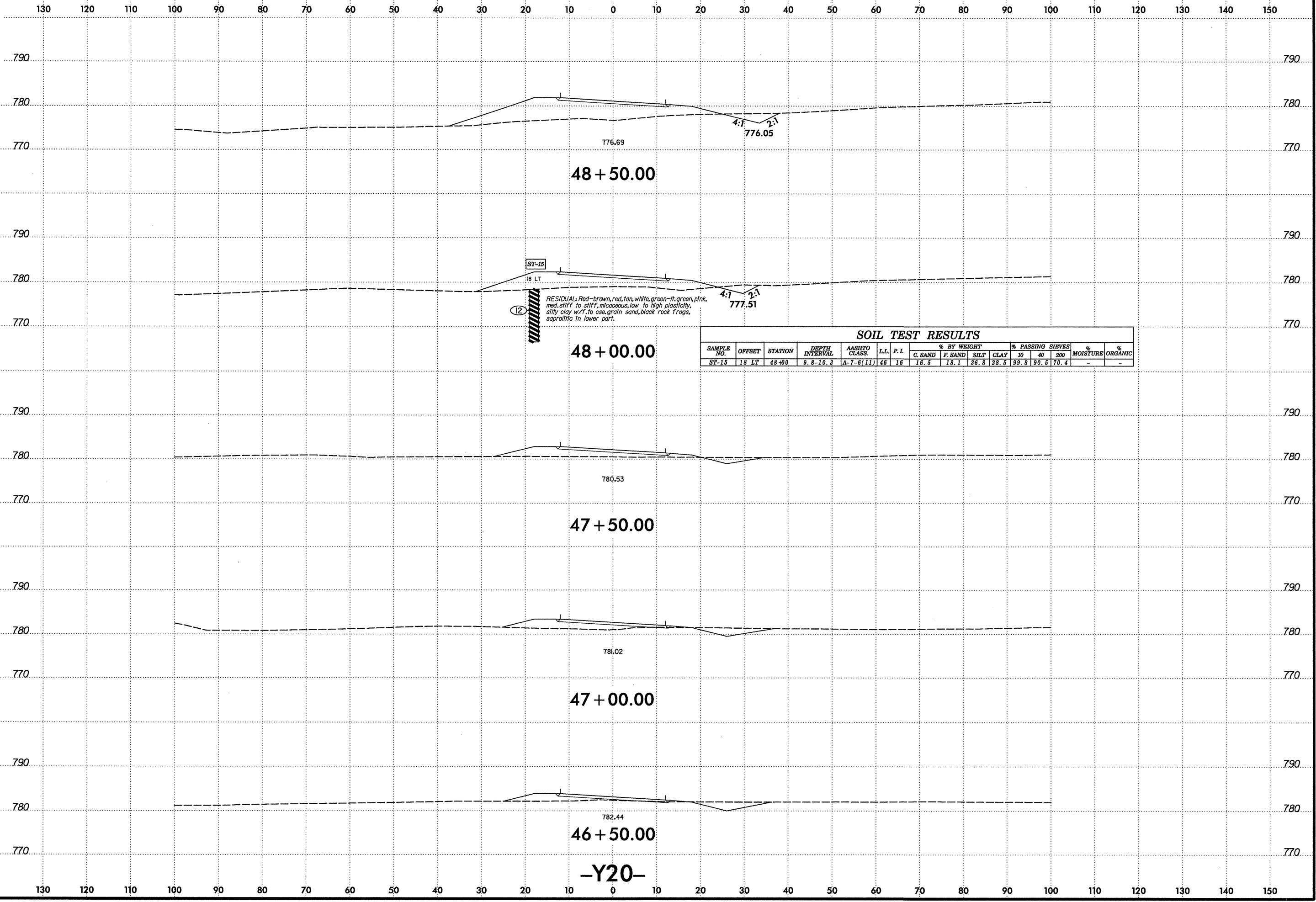
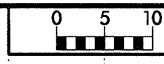
46 + 00.00

45 + 50.00

45 + 32.10

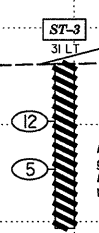
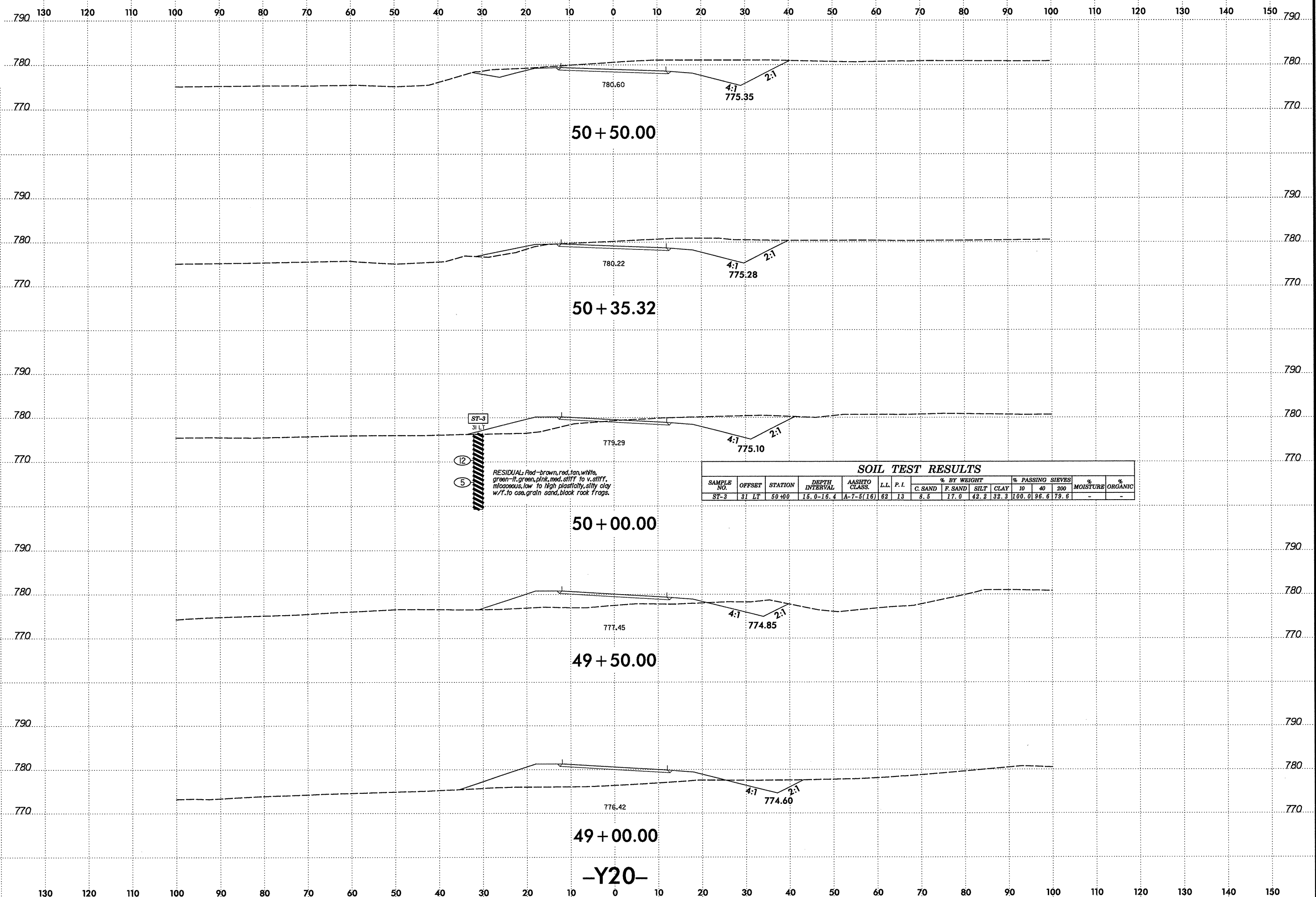
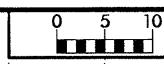
45 + 00.00

-Y20-



ST-15  
18 LT  
RESIDUAL: Red-brown, red, tan, white, green-ll, green, plnk, med. stiff to stiff, micaceous, low to high plasticity, silty clay w/f. to csa. grain sand, black rock frags, saprolitic in lower part.

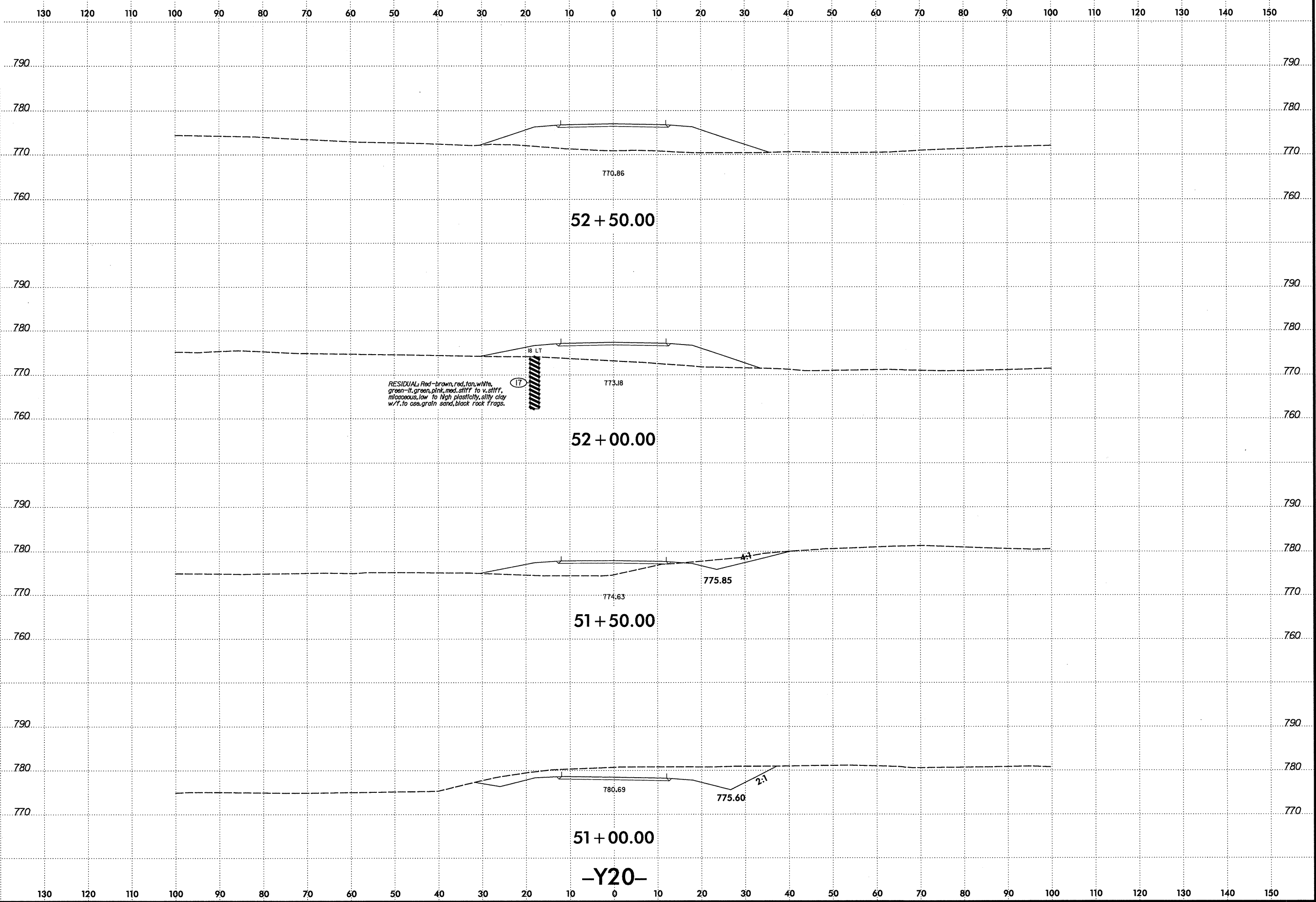
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-15	18 LT	48+00	9.8-10.3	A-7-6(11)	48	16	16.8	18.1	36.8	28.5	99.8	90.5	70.4	-	-



RESIDUAL: Red-brown, red, tan, white, green-ft. green, pink, med. stiff to v. stiff, micaceous, low to high plasticity, silty clay w/f. to csa. grain sand, black rock frags.

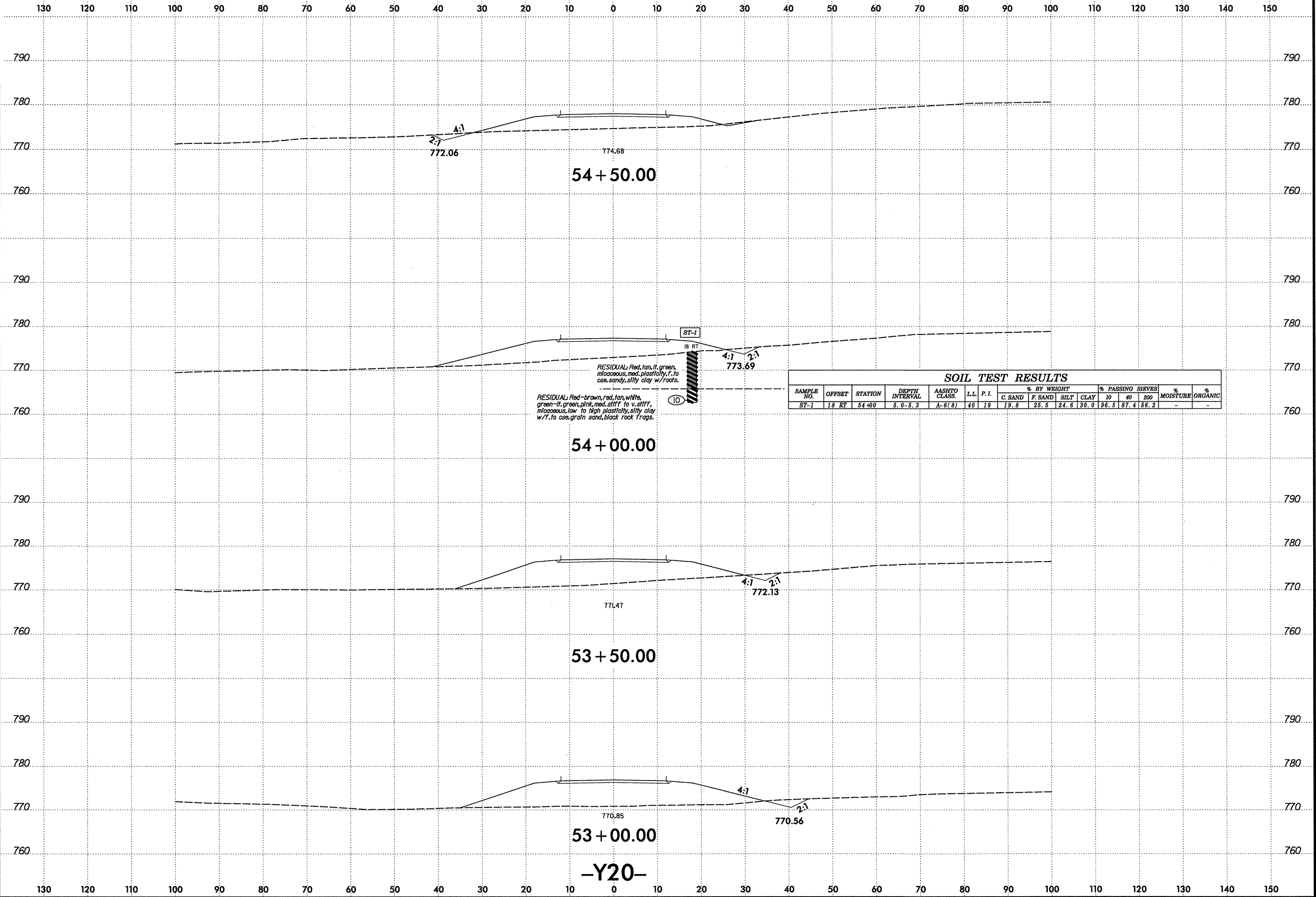
SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-3	31 LT	50+00	15.0-16.4	A-7-5(16)	62	13	8.5	17.0	42.2	32.3	100.0	96.6	79.6	-	-





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-Y20-

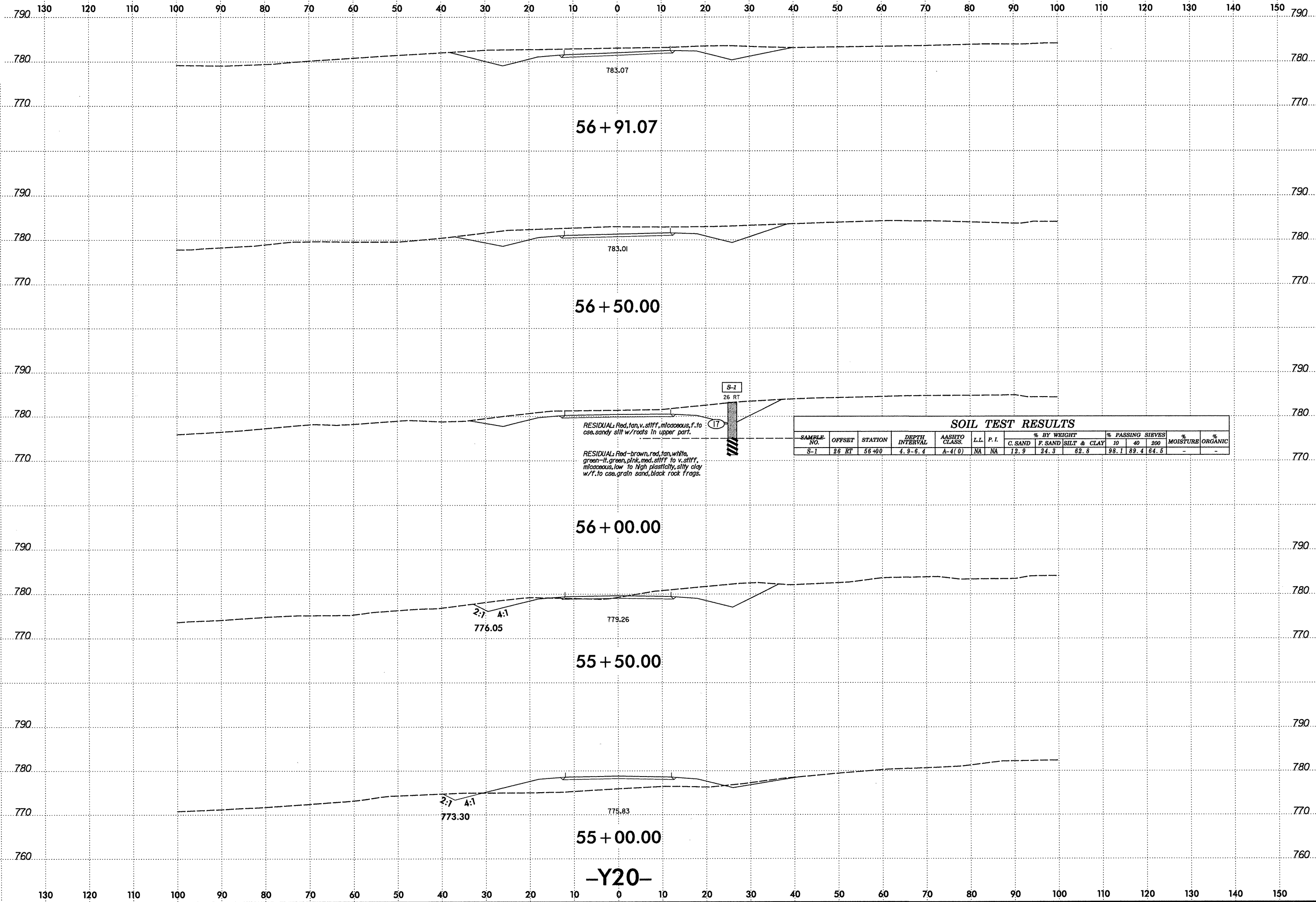
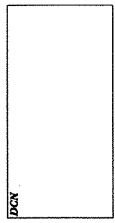


RESIDUAL: Red, tan, lt. green, micaceous, med. plastfoly, f. to csa, sandy, silty clay w/ roots.

RESIDUAL: Red-brown, red, tan, white, green-lt. green, pink, med. stiff to v. stiff, micaceous, low to high plastfoly, silty clay w/f. to csa, grain sand, black rock frags.

SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES		% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	200		
ST-1	18 RF	54+00	5.0-5.3	A-6(8)	40	19	19.8	25.5	24.6	30.0	96.5	87.4	56.2	-

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 Florence & Hutcheson, Inc.



56 + 91.07

56 + 50.00

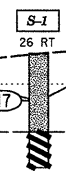
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55 + 50.00

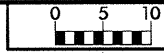
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-Y20-

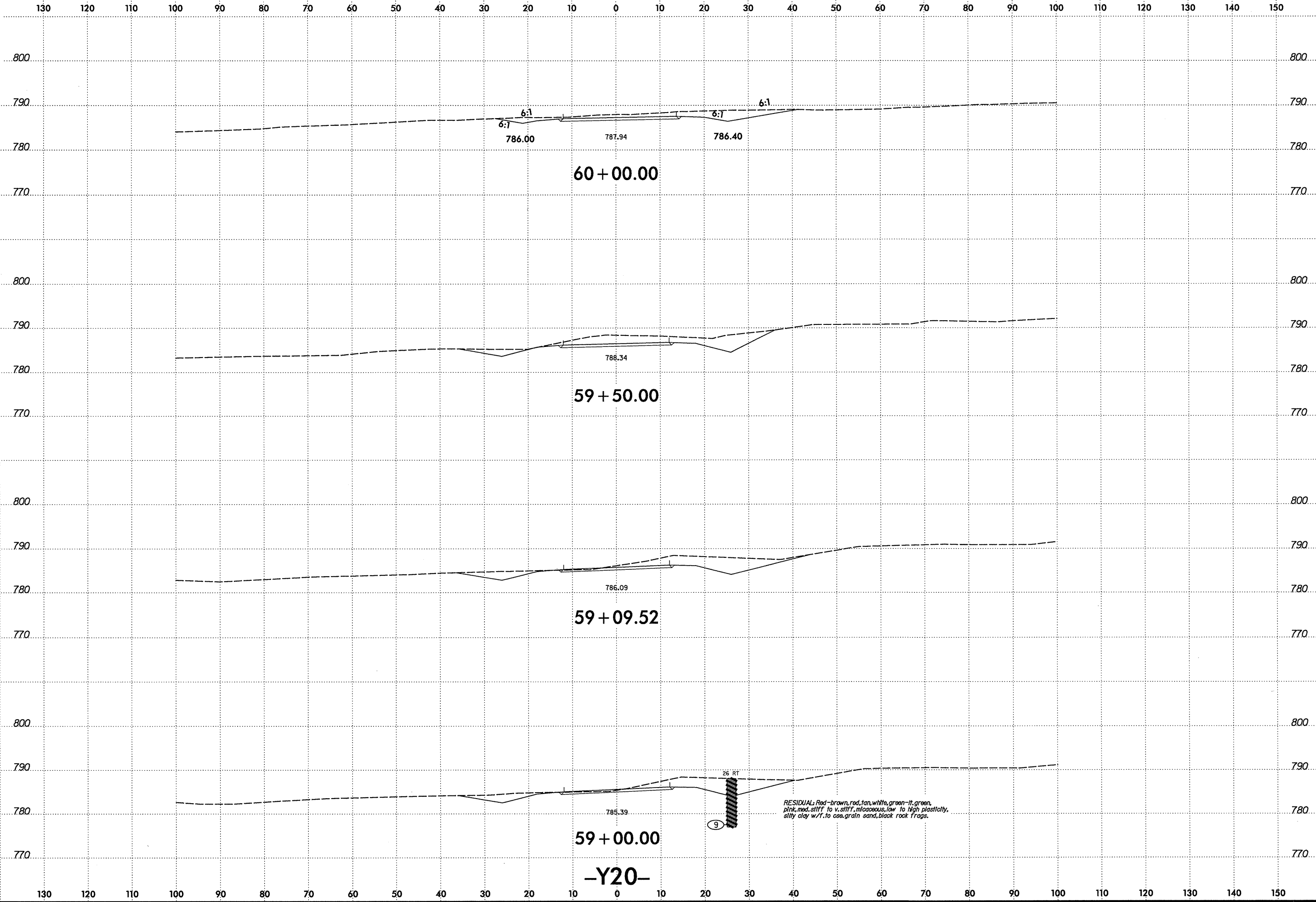
RESIDUAL: Red, tan, v. stiff, micaceous, f. to cse. sandy silt w/ roots in upper part.  
 RESIDUAL: Red-brown, red, tan, white, green-ll. green, plnk, med. stiff to v. stiff, micaceous, low to high plasticity, silty clay w/ f. to cse. grain sand, black rock frags.



SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT & CLAY	10	40	200		
S-1	26 RT	56+00	4.9-6.4	A-4(0)	NA	NA	12.9	24.3	62.8	98.1	89.4	64.6	-	-



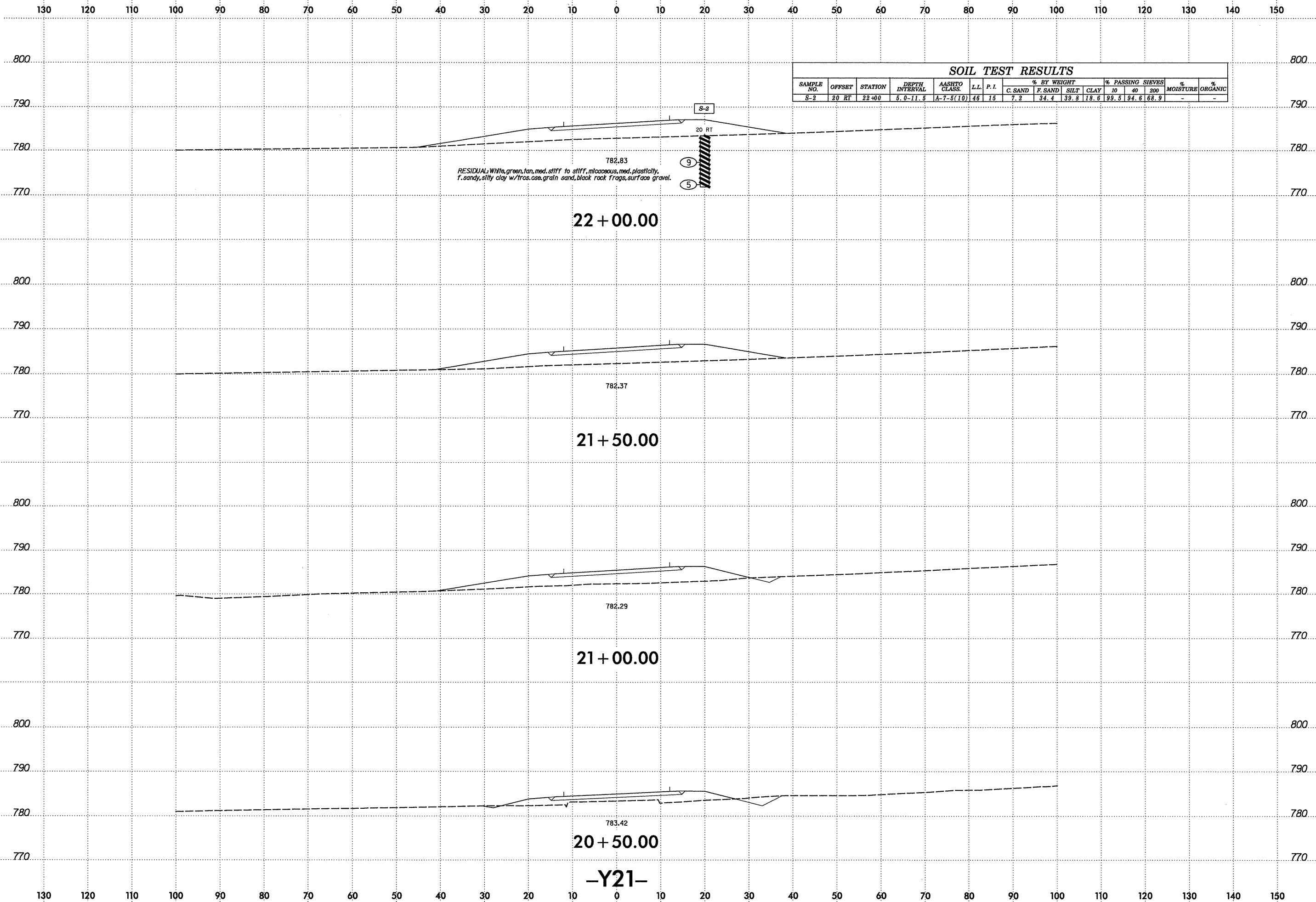
DOOR

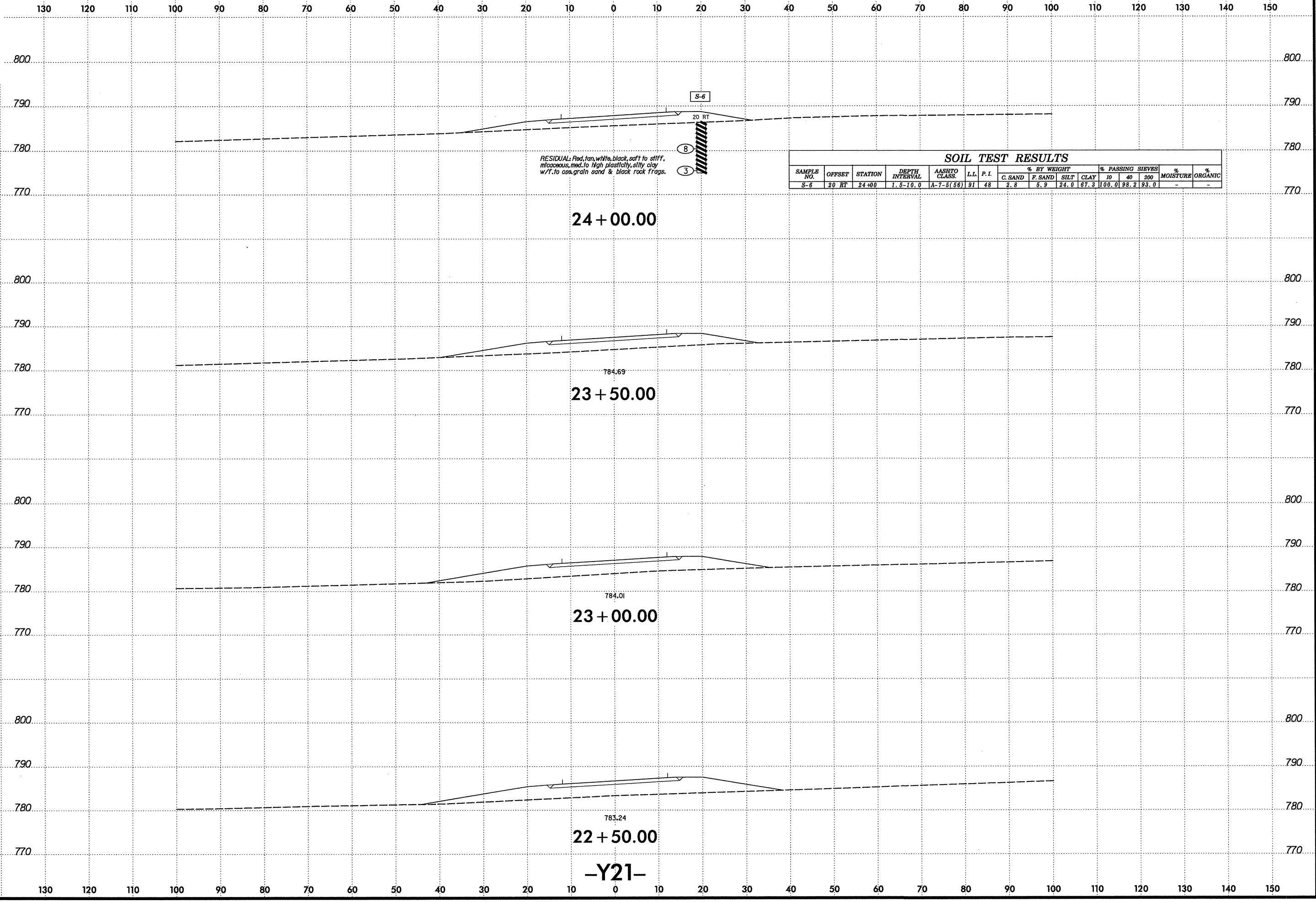


**SOIL TEST RESULTS**

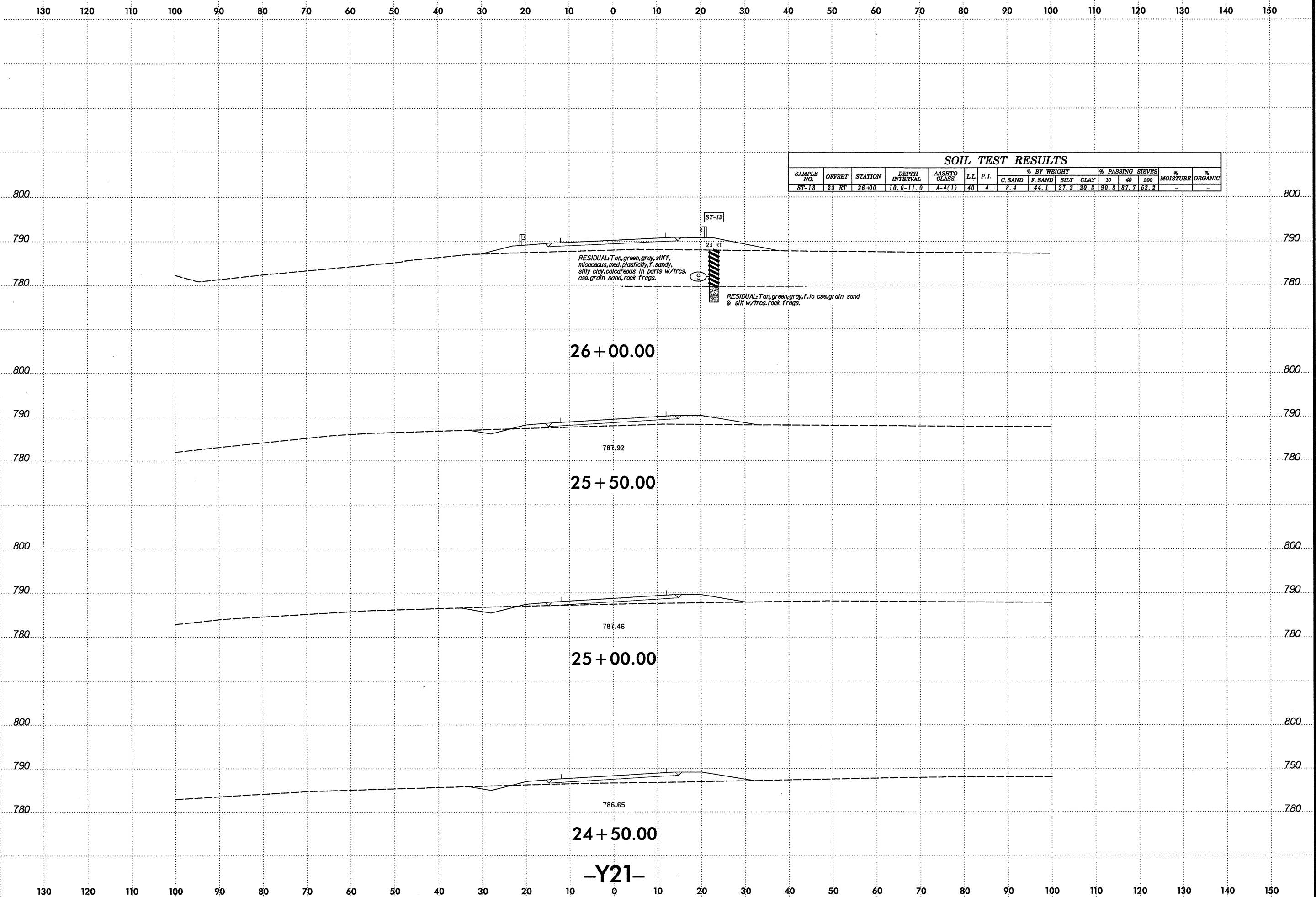
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
S-2	20 RT	22+00	5.0-11.5	A-7-5(10)	46	15	7.2	34.4	39.8	18.6	99.5	94.6	68.9	-	-

Rock





SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-13	23 RT	26+00	10.0-11.0	A-4(1)	40	4	8.4	44.1	27.2	20.3	90.8	87.7	52.2	-	-



26 + 00.00

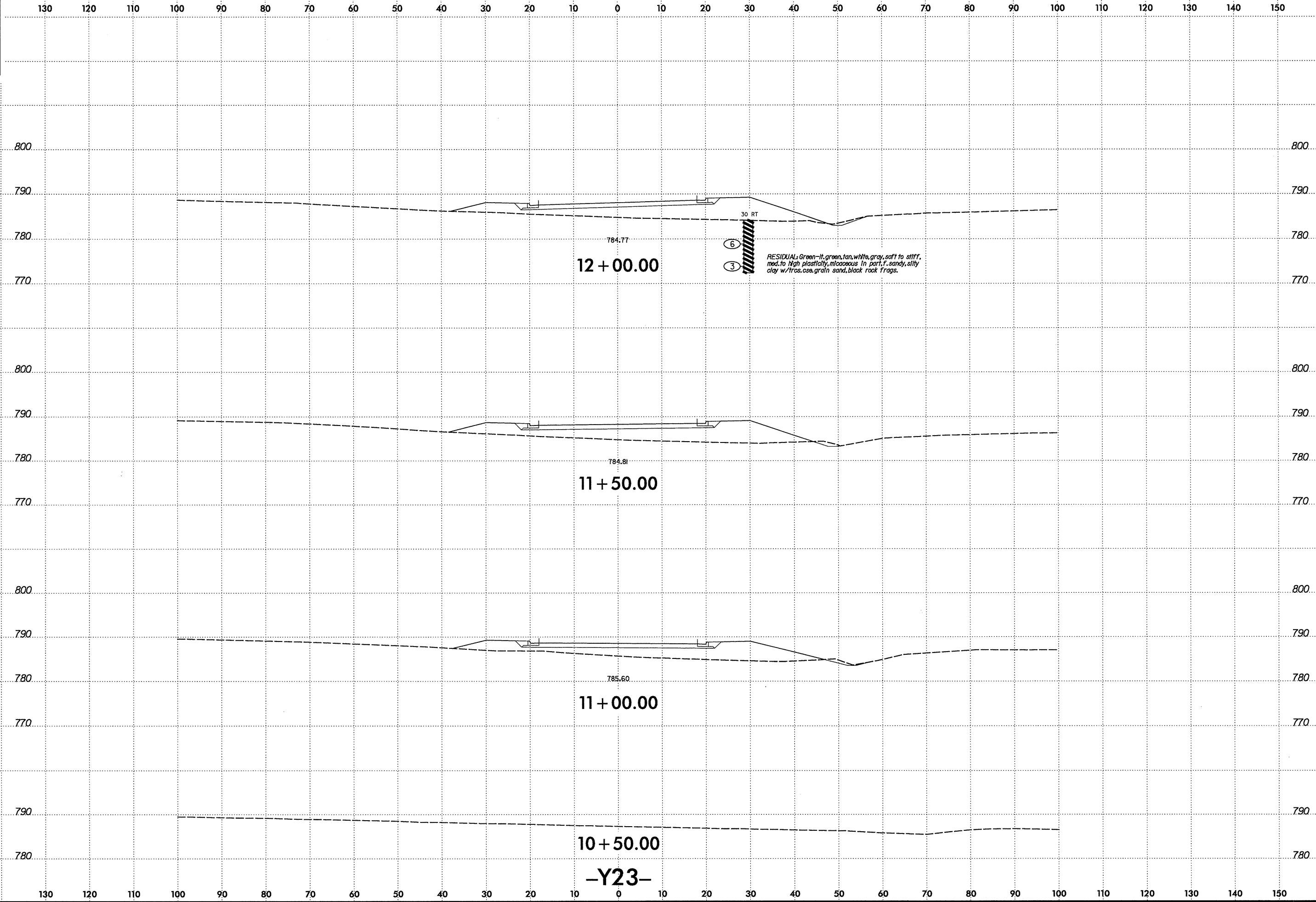
25 + 50.00

25 + 00.00

24 + 50.00

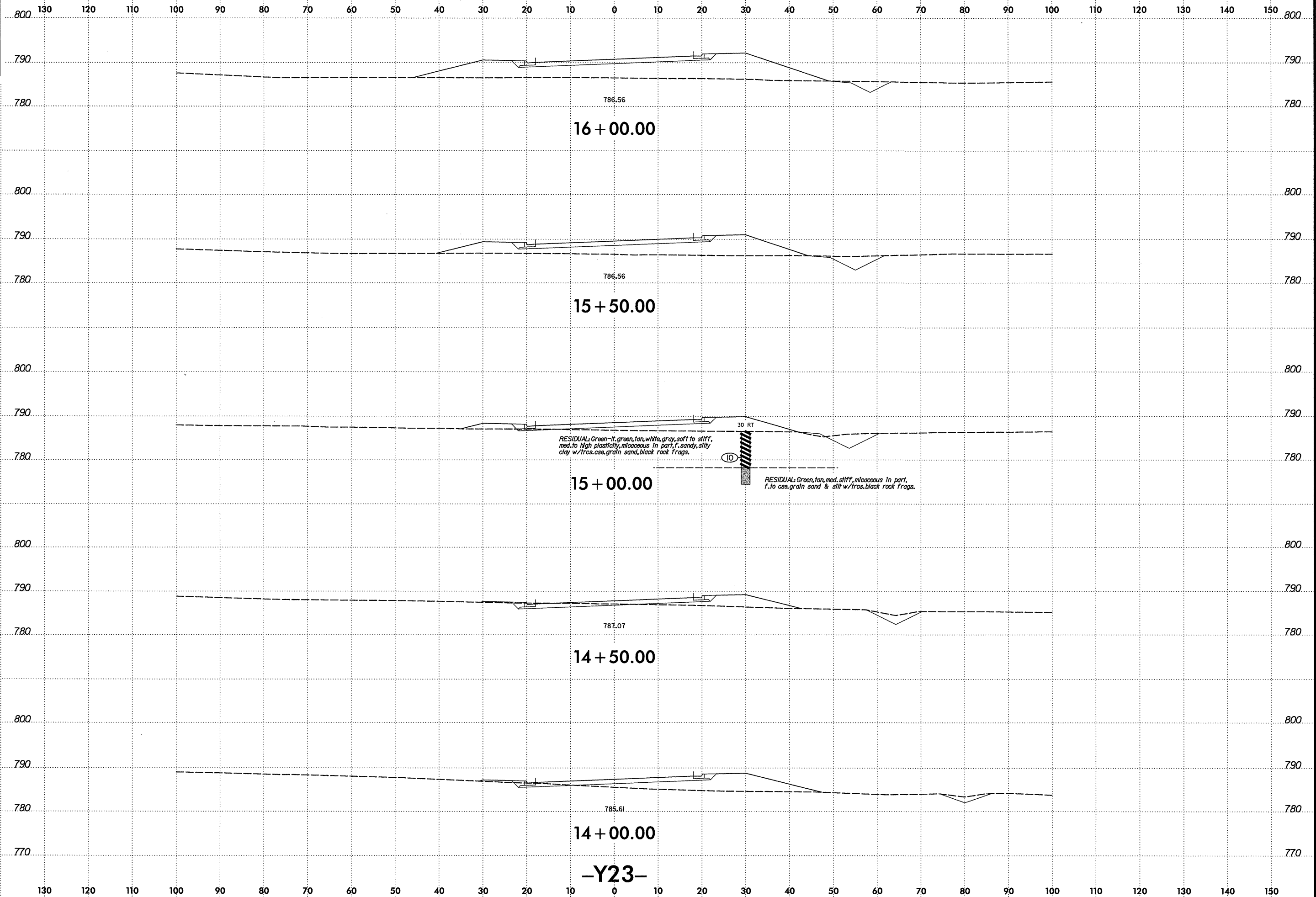
-Y21-

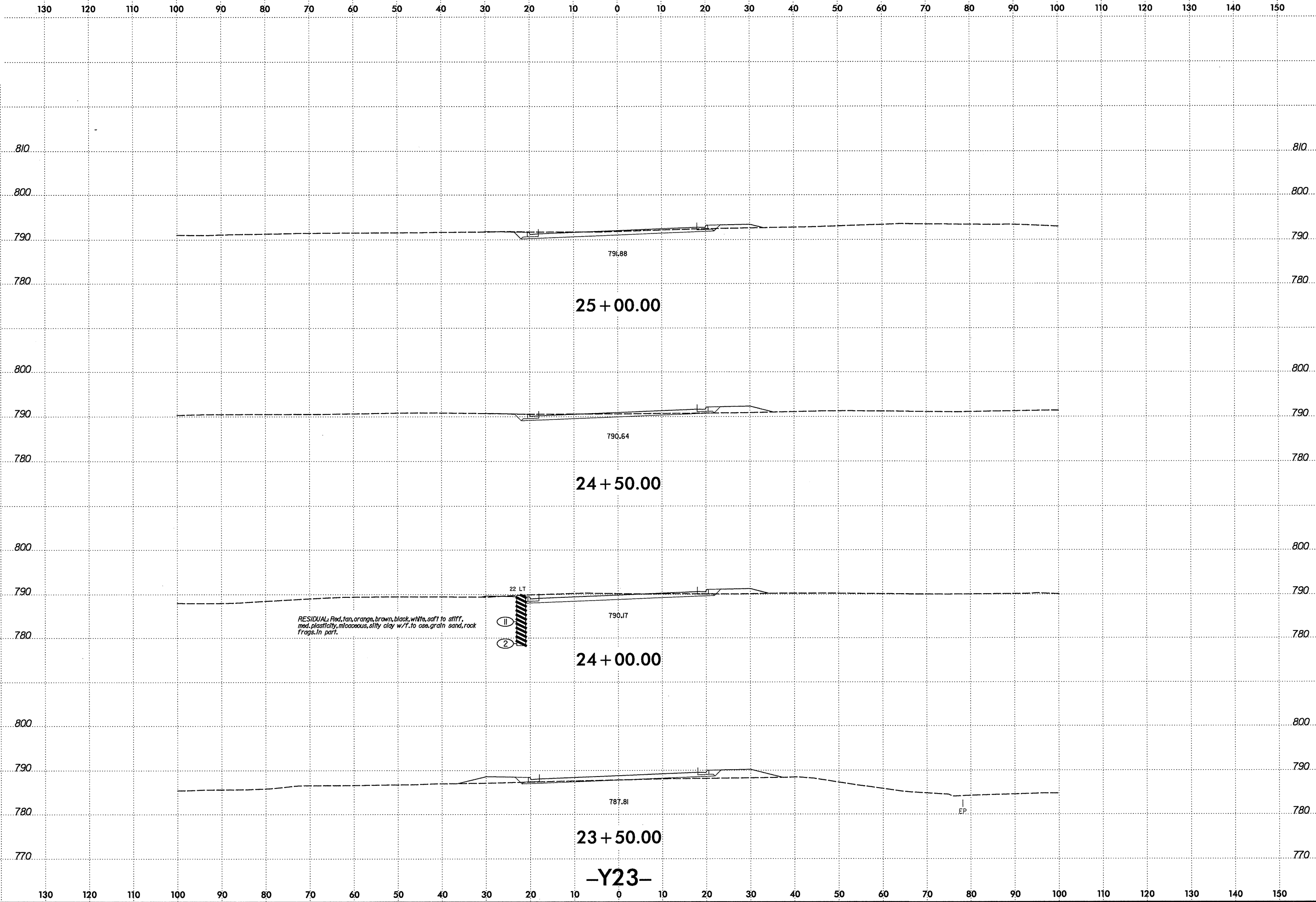
Rock



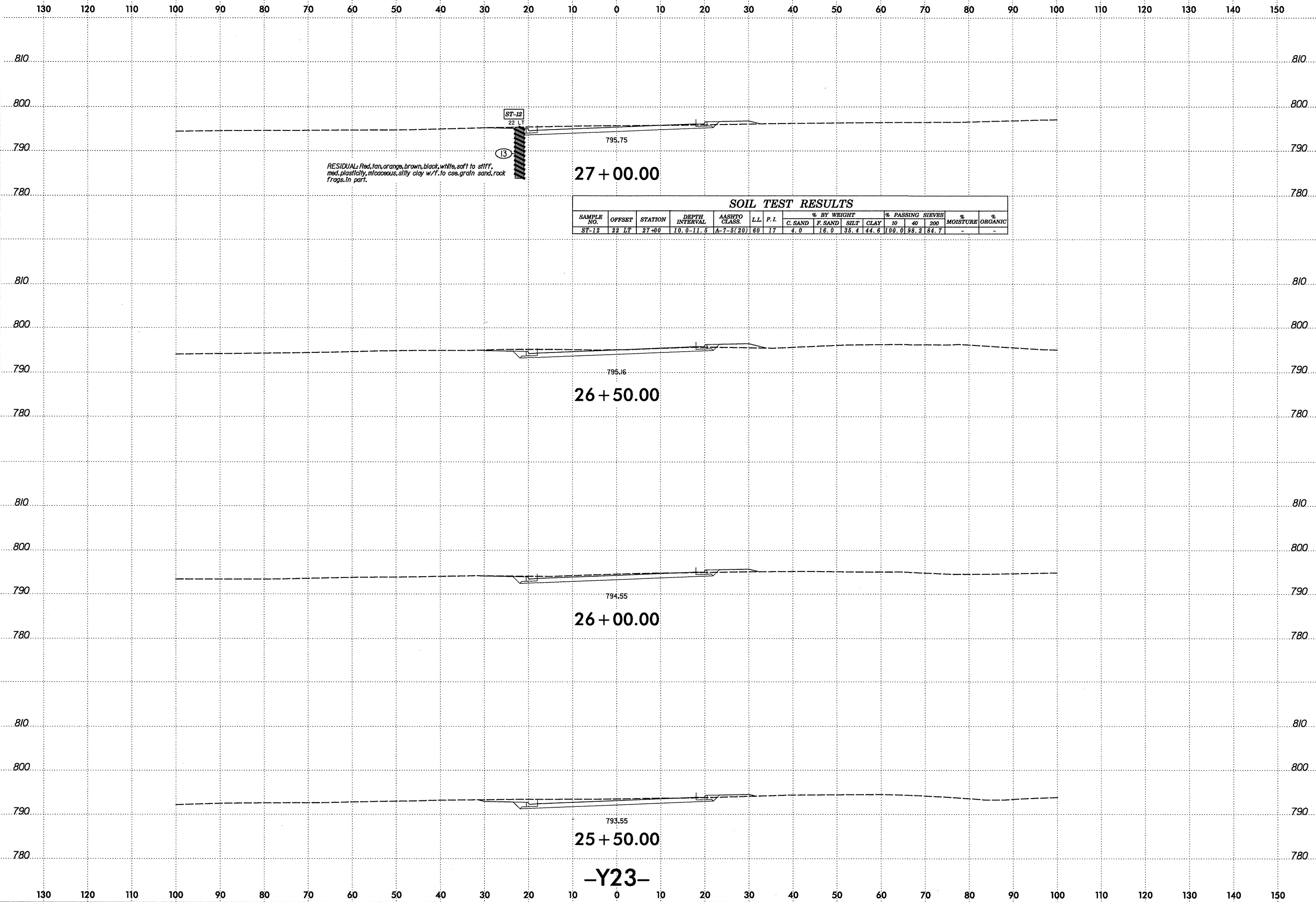
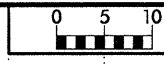


Rock





12/5/2012  
 RA\Geotech\InvestigationDesign\XSC P-5206A\_GEO\_xsl Y23\_sections.dgn  
 Florence & Hutcheson, Inc.



RESIDUAL: Red, tan, orange, brown, black, white, soft to stiff, med. plasticity, micaceous, silty clay w/f. to cse. grain sand, rock frags. in part.

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	LL	P.I.	% BY WEIGHT				% PASSING SIEVES			% MOISTURE	% ORGANIC
							C. SAND	F. SAND	SILT	CLAY	10	40	200		
ST-12	22 LT	27+00	10.0-11.5	A-7-5(20)	60	17	4.0	16.0	35.4	44.6	100.0	98.2	84.7	-	-

12/5/2012  
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Florence & Hutcheson, Inc.

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