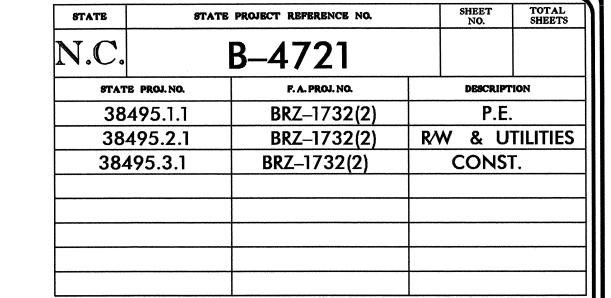
# VICINITY MAP Out 1348 Houck 1364 Rd. 90 PROJECT 1300 PROJECT 165 Out 165 Out 1348 PROJECT 165 Out 165 Out 1348 Out 1

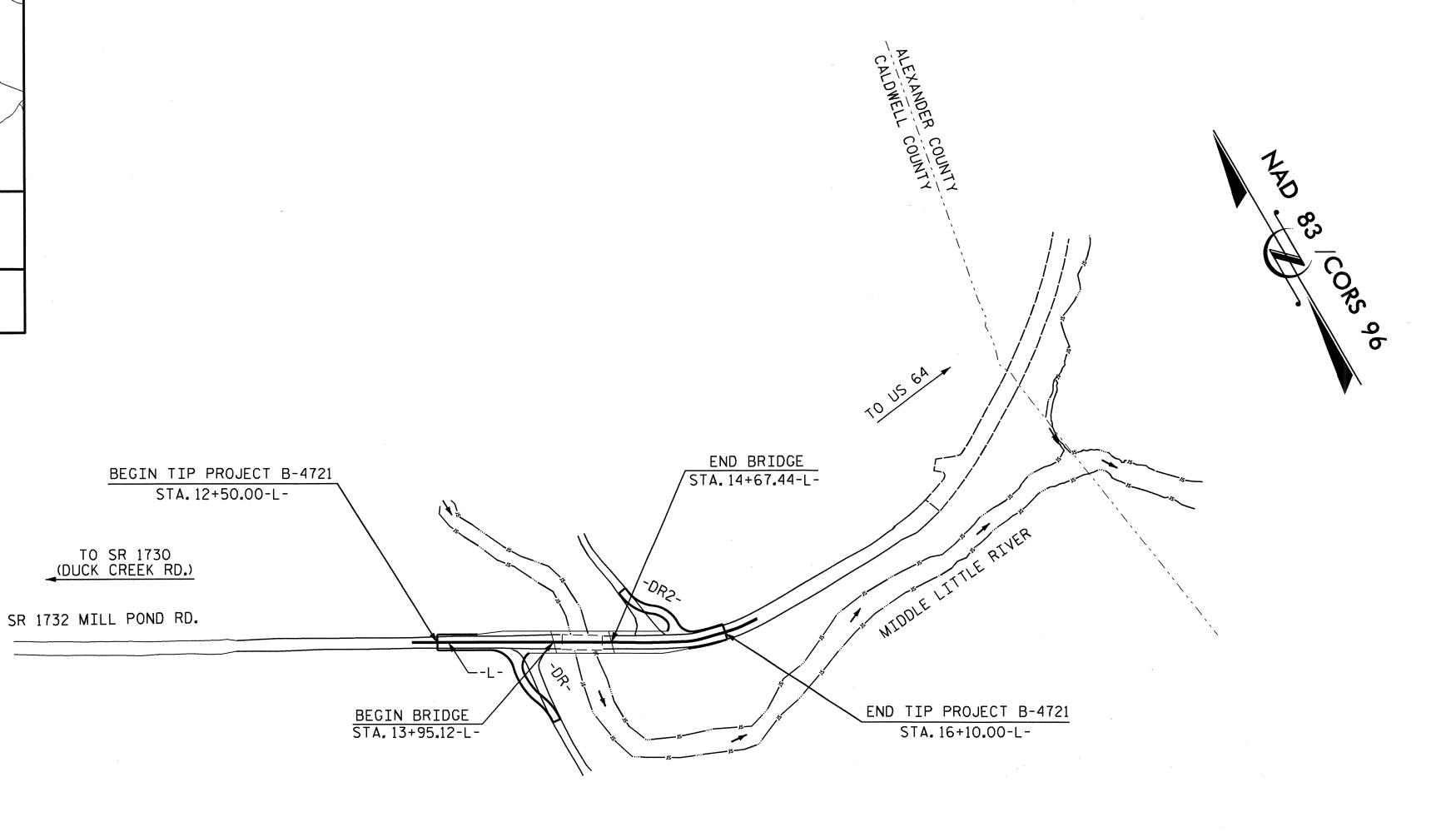
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

# CALDWELL COUNTY

LOCATION: REPLACE BRIDGE NO. 57. OVER THE MIDDLE LITTLE RIVER ON SR 1732

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE





# TRUCTUR

C203083

M

# NORTH CAROLINAS ROLLINAS ROLLI

# DESIGN DATA ADT 2013 = 30 ADT 2033 = 50

ADT 2033 = 50 DHV = 10 % D = 60 %

T = 2 % \* V = 40 MPH

\* (TTST 0% + DUAL 2%)

FUNC CLASS=RURAL LOCAL
SUBREGIONAL TIER

### PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT B-4721 = 0.054 MI

LENGTH STUCTURES TIP PROJECT B-4721 = 0.014 MI

TOTAL LENGTH TIP PROJECT B-4721 = 0.068 MI

## Prepared in the Office of: DIVISION OF HIGHWAYS

1000 Birch Ridge Dr., Raleigh NC, 27610

2012 STANDARD SPECIFICATIONS

*LETTING DATE:*MARCH 19, 2013

Q. H. NGUYEN, PE PROJECT ENGINEER

B. D. KLAPPENBACH, PE PROJECT DESIGN ENGINEER

# STRUCTURE MANAGEMENT UNIT 1000 BIRCH RIDGE DR. RALEIGH, N.C. 27610

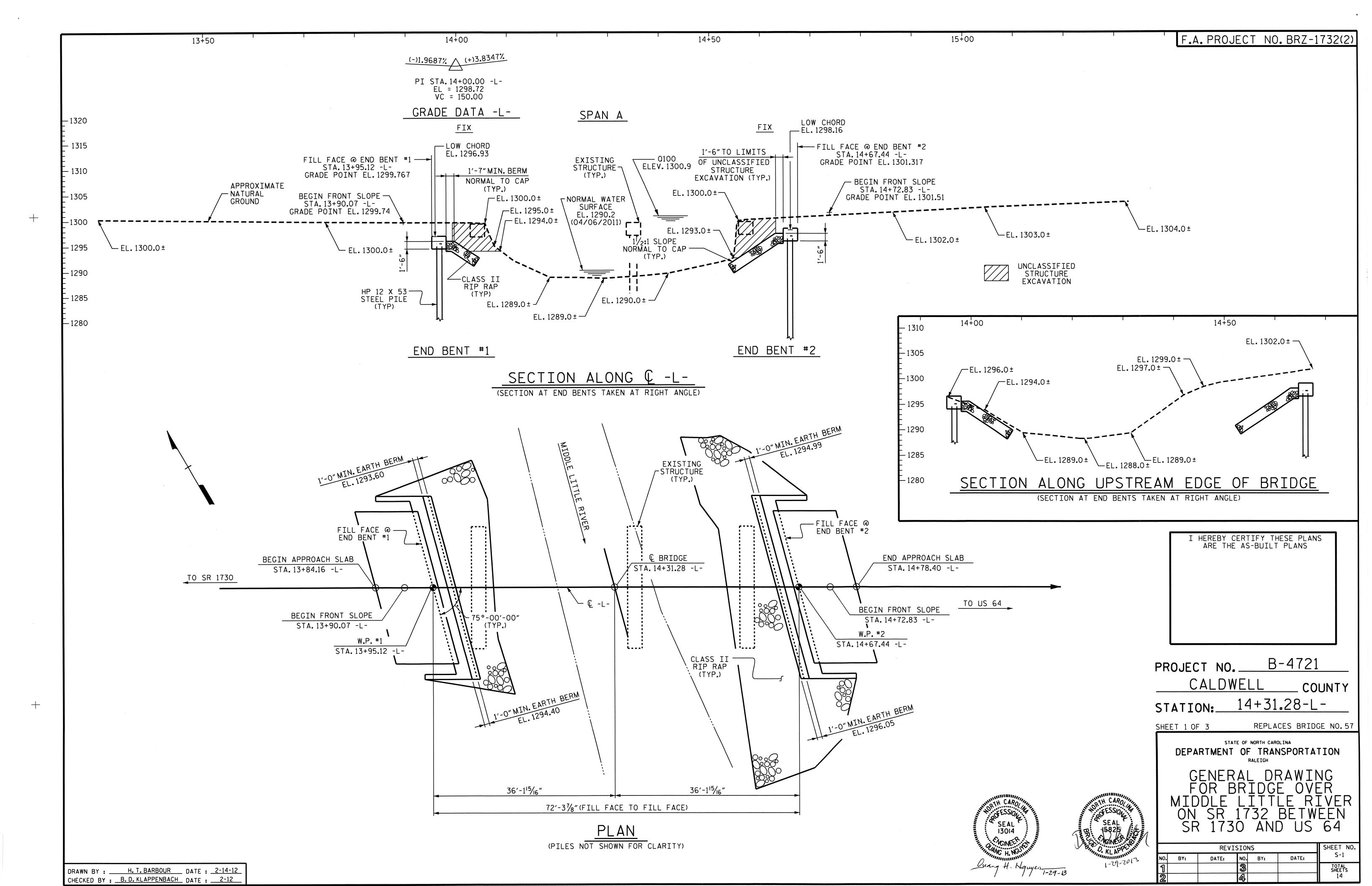
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

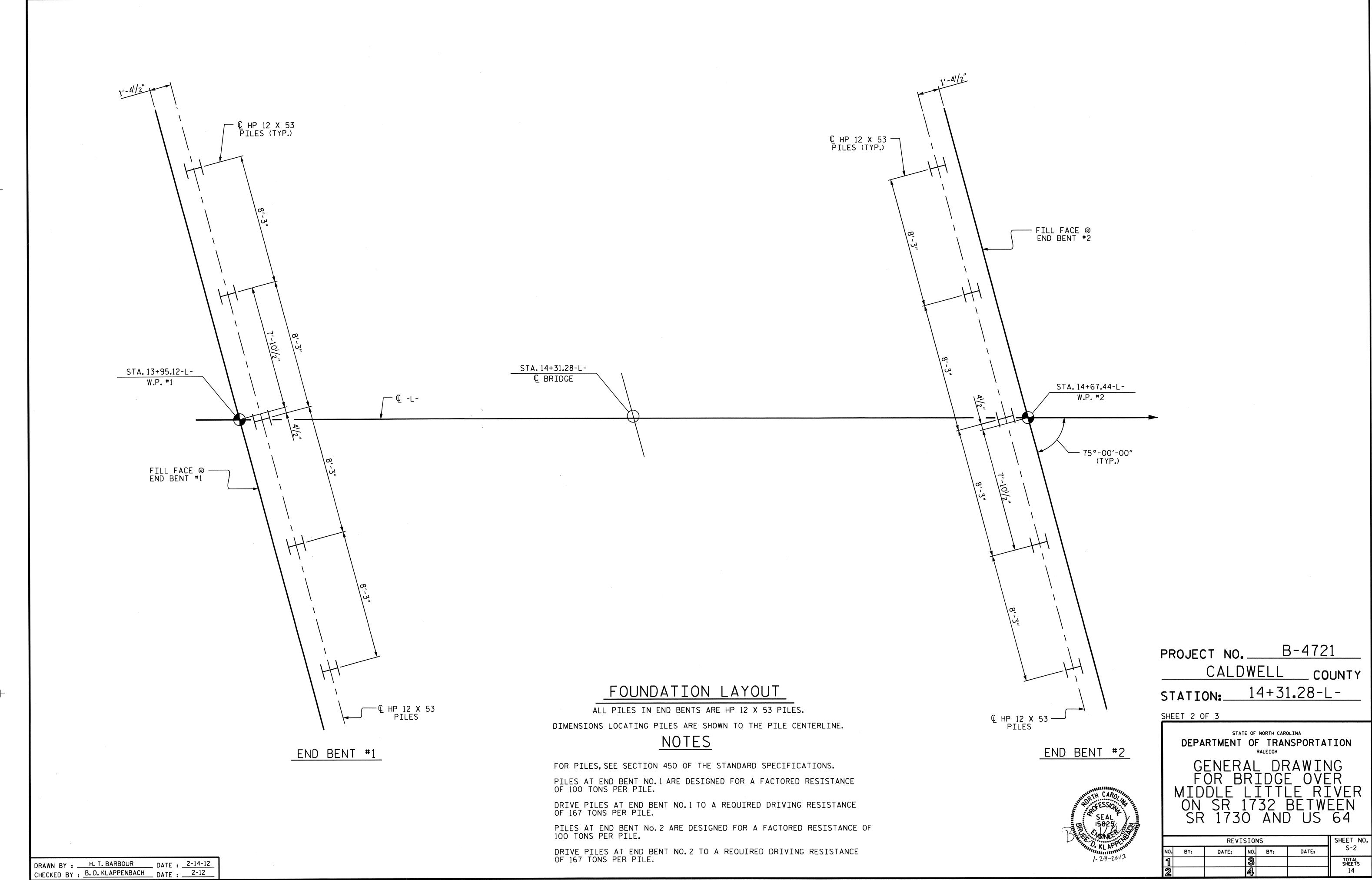
P.E.

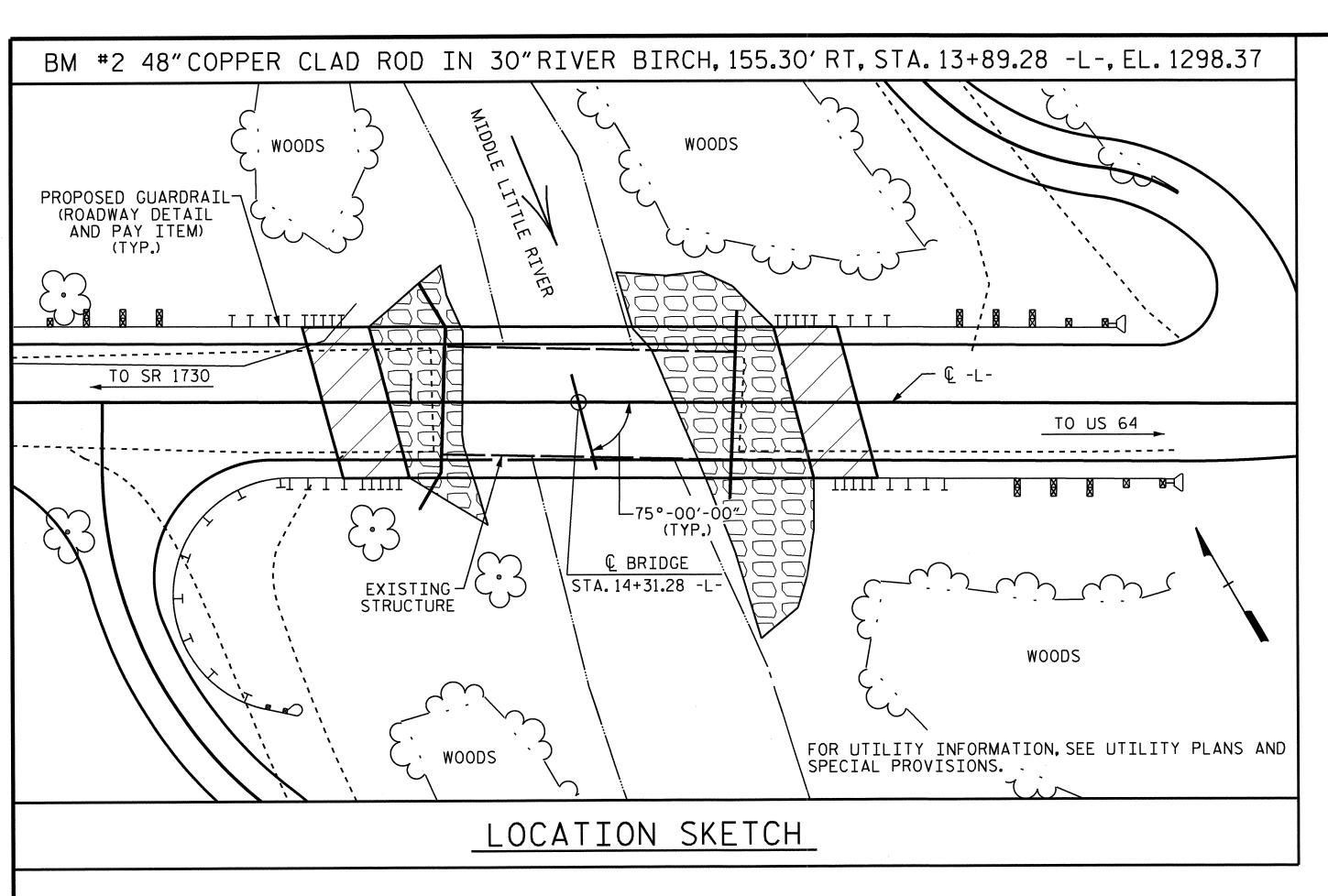
STATE DESIGN ENGINEER

DEPARTMENT OF TRANPORTATION
FEDERAL HIGHWAY ADMINISTRATION

APPROVED ENGINEER DIVISION ADMINISTRATION DA







### HYDRAULIC DATA

DESIGN DISCHARGE = 2100 C.F.S. FREQUENCY OF DESIGN FLOOD = 25 YRS.

DESIGN HIGH WATER ELEVATION = 1299.700 DRAINAGE AREA

BASE DISCHARGE (Q100)

= 9.80 SQ. MI. = 3000 C.F.S. BASE HIGH WATER ELEVATION = 1300.900

### OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 2100 C.F.S.

FREQUENCY OF OVERTOPPING FLOOD = 25 YRS. = 1299.700 OVERTOPPING FLOOD ELEVATION

### NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 2 SPANS, 1 @ 30'-3", 1 @ 20'-3", WITH GRAVEL WEARING SURFACE ON 4" X 8" TIMBER DECK ON 8 LINES OF 18" I-BEAMS @ 2'-7" CENTERS, WITH A CLEAR ROADWAY WIDTH OF 19.0 FT. ON TIMBER CAPS WITH TIMBER POST AND SILL AT BOTH END BENTS AND AT BENT No.1 (CRUTCH BENT) LOCATED AT THE SAME LOCATION AS THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. SEE SPECIAL PROVISIONS FOR REMOVAL OF EXISTING STRUCTURE AT STATION 14+31.28-L-.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. EACH SIDE OF CENTERLINE ROADWAY AT END BENT No. 1 AND 30' TO THE LEFT AND 40'TO THE RIGHT OF CENTERLINE ROADWAY AT END BENT No. 2 AS DIRECTED BY THE ENGINEER. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 14+31.28-L."

FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18, EVALUATING SCOUR AT BRIDGES".

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 X 53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES CO	" X 2'-0" STRESSED NCRETE ED SLABS
	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE				LUMP SUM				140.26			LUMP SUM	10	700.00
END BENT NO. 1			13.7		2039	5	75		80	89			
END BENT NO. 2			13.7		2039	5	75		140	156		.,	
TOTAL	LUMP SUM	LUMP SUM	27.4	LUMP SUM	4078	10	150	140.26	220.0	245.0	LUMP SUM	10	700.00

PROJECT NO. B-4721 CALDWELL \_\_\_\_ COUNTY STATION: 14+31.28 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

REVISIONS NO. BY: DATE:

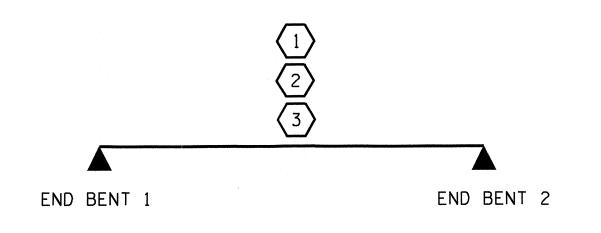
DRAWN BY : \_\_\_\_H. T. BARBOUR \_\_\_ DATE : \_\_\_\_2-14-12 CHECKED BY : B.D. KLAPPENBACH DATE : 2-12

### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE SHEAR MOMENT MOMENT DISTRIBUTION FACTORS (DF) LIVELOAD FACTORS CONTROLI DIST/ LEFT SPAN DIST, LEFT SPAN DISTI FACT DIST LEFT SPAN DIS<sup>-</sup> FAC<sup>-</sup> MIN RAT 0.269 34.482 0.608 1.10 0.80 1.01 70′ 1.04 70′ EL 3.448 34.482 1.75 0.269 N/A 1.01 HL-93(Inv) 1.35 70′ 3.448 N/A 34.482 1.43 EL 1.36 1.35 0.269 HL-93(0pr) N/A DESIGN $\overline{2}$ 47.356 0.269 3.448 0.269 1.32 34,482 1.36 0.608 1.38 70′ 1.75 34.482 70′ EL 0.80 EL 36.000 1.32 HS-20(Inv) LOAD RATING 3.448 34.482 0.608 1.79 63.236 1.35 0.269 1.76 70′ EL N/A ---36.000 HS-20(0pr) 1.76 0.269 0.269 34.482 0.608 4.12 70′ EL 3.448 0.80 2.94 70′ EL 34.482 39.656 13.500 1.40 2.94 SNSH 34.482 2.93 3.448 0.80 0.269 2.20 34.482 70′ 44.052 0.269 2.84 EL EL 2.20 20.000 1.40 SNGARBS2 0.269 3.448 0.80 0.269 2.09 34.482 2.69 34.482 0.608 2.72 70′ 46.016 1.40 70′ EL 70′ EL EL 22.000 2.09 SNAGRIS2 0.80 0.269 1.88 34.482 0.608 2.06 70′ 3.448 1.46 70′ 34.482 0.269 EL 27.250 1.46 39.844 1.40 SNCOTTS3 0.269 34.482 1.71 70′ 3.448 0.80 1.23 70′ 34.482 1.58 0.608 EL 0.269 34.925 1.23 1.40 SNAGGRS4 34.482 0.269 1.20 1.54 70′ 70′ 0.269 0.608 1.73 3.448 0.80 34.482 35.550 1.20 42.646 1.40 70′ EL SNS5A 0.269 34.482 1.58 3.448 0.80 1.10 70' 34.482 70′ 39.950 1.10 0.269 1.42 70′ EL EL 44.058 1.40 SNS6A 0.80 0.269 1.05 70' 34.482 0.269 1.35 34.482 1.55 70′ EL 3.448 1.05 44.113 0.608 42.000 1.40 SNS7B LEGAL 0.269 34.482 3.448 0.80 1.35 70′ 34.482 0.269 1.73 70′ EL 1.345 0.608 1.88 EL LOAD 44.401 1.40 TNAGRIT3 33.000 RATING 0.269 0.269 70′ 44.717 34.482 0.608 70′ 3.448 0.80 1.35 34.482 33.075 1.35 1.74 70′ EL 1.83 EL EL 1.40 TNT4A 34.482 1.65 3.448 0.80 0.269 34.482 0.608 70' 1.11 70′ 0.269 1.43 EL EL EL 41.600 46.073 1.40 70′ TNT6A 0.269 0.80 1.11 70′ 34.482 34.482 0.608 70′ 42.000 46.794 0.269 1.43 1.62 EL 3.448 EL 1.40 TNT7A 0.269 34.482 3.448 0.80 70' 70′ 1.16 EL 48.526 34.482 0.608 1.51 EL 0.269 1.49 1.16 TNT7B 42.000 70′ 34.482 0.269 0.80 0.269 1.10 1.41 34.482 0.608 1.46 3.448 1.40 TNAGRIT4 43.000 1.10 0.269 34.482 0.269 1.33 34.482 46.505 1.40 1.45 0.80 TNAGT5A 45.000 70′ $\langle 3 \rangle$ 1.39 3.448 0.80 0.269 1.02 70′ 34.482

34.482 0.608

70′

EL



1.02

45.000

TNAGT5B

45.905

1.40

0.269

1.31

LRFR SUMMARY

ASSEMBLED BY : D. A. GLADDEN DATE : 8-25-11 CHECKED BY : H. T. BARBOUR DATE : 9-14-11

DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

### LOAD FACTORS:

	)ESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
	LOAD RATING FACTORS	STRENGTH I	1.25	1.50
F		SERVICE III	1.00	1.00

### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

### COMMENTS:

EL

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

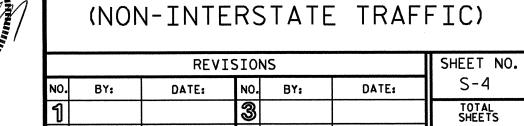
### GIRDER LOCATION

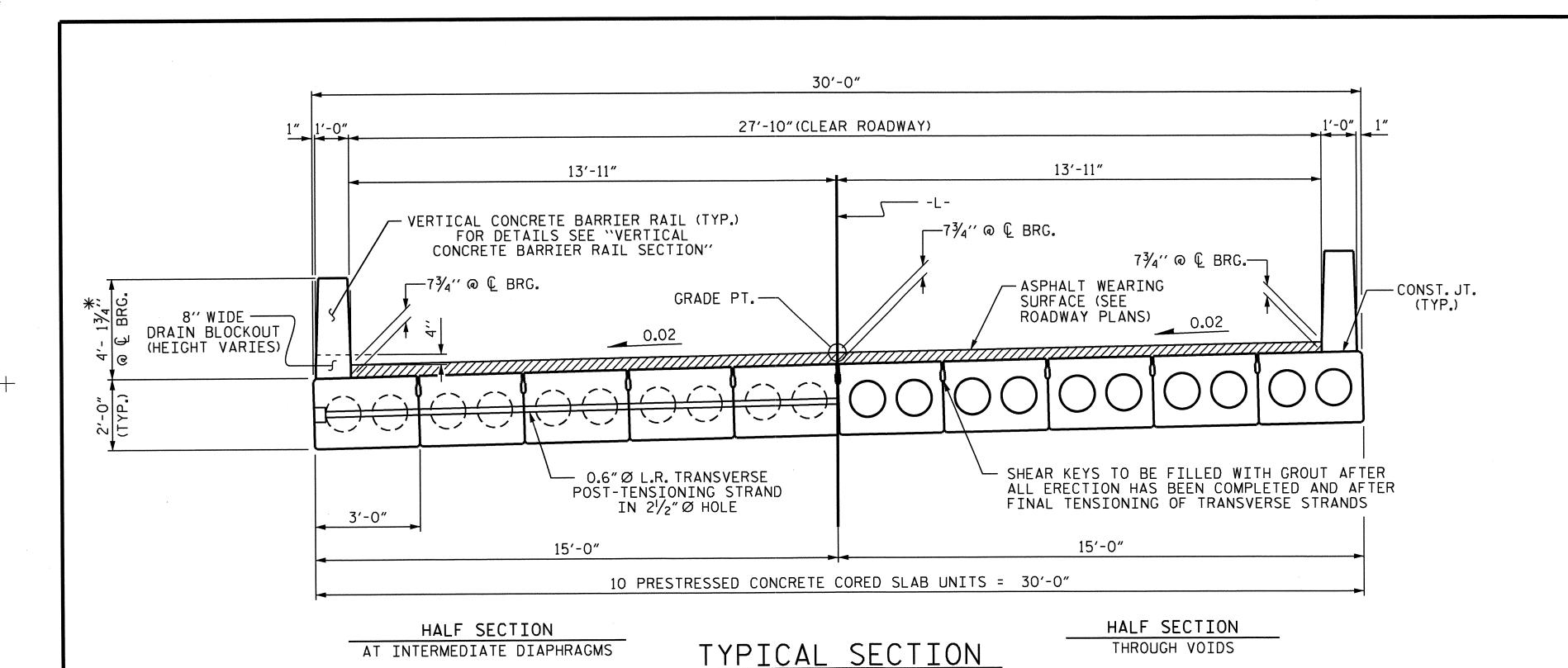
- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. B-4721 CALDWELL COUNTY STATION: 14+31.28 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD LRFR SUMMARY FOR



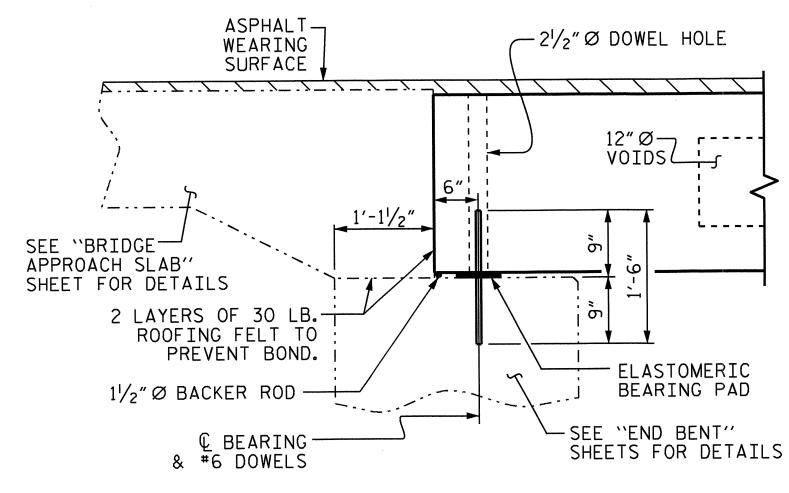


\* - THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE

"VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE

FIXED END

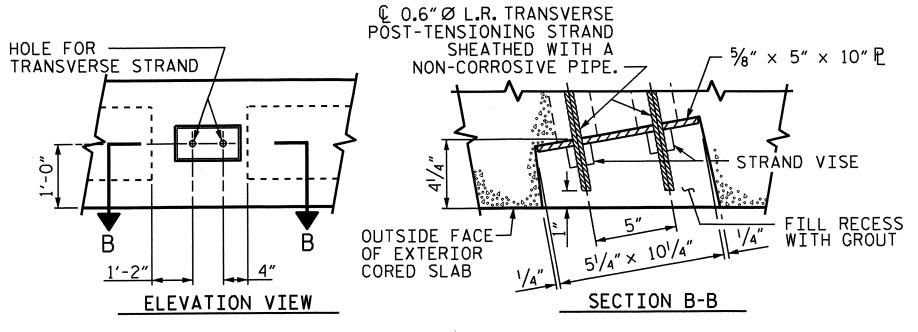


ASSEMBLED BY: D. A. GLADDEN DATE: 8-25-11 CHECKED BY: H. T. BARBOUR DATE: 9-14-11

DRAWN BY: MAA 6/10

CHECKED BY : MKT 7/10

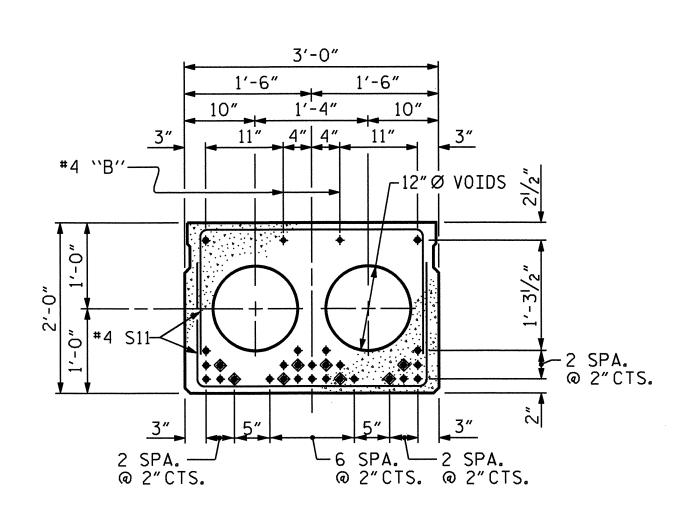
### SECTION AT END BENT



GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

— € 2½″Ø \ DOWEL HOLES #5 S15 #5 S10~ ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

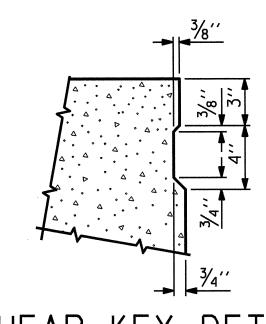


STRAND LAYOUT FOR 70'UNIT (28 STRANDS REQUIRED)

### INTERIOR SLAB SECTION 0.6" Ø LOW RELAXATION

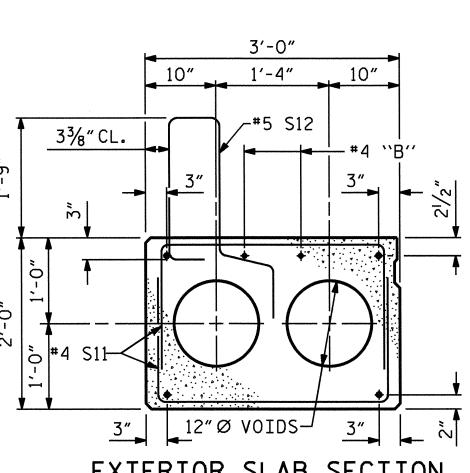
BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



EXTERIOR SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

B-4721 PROJECT NO.\_\_\_\_ CALDWELL \_ COUNTY STATION: 14+31.28 -L-

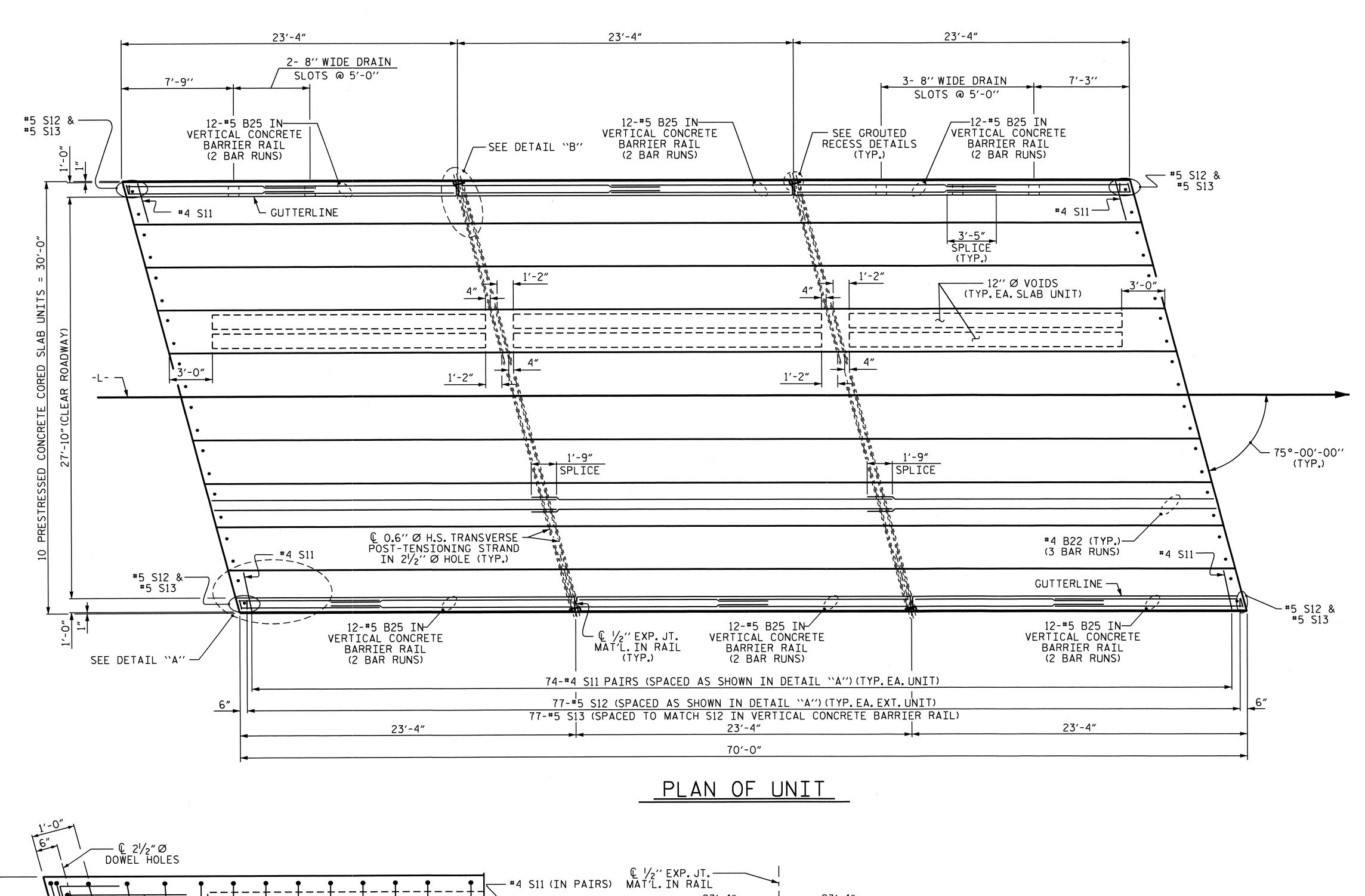
SHEET 1 OF 3

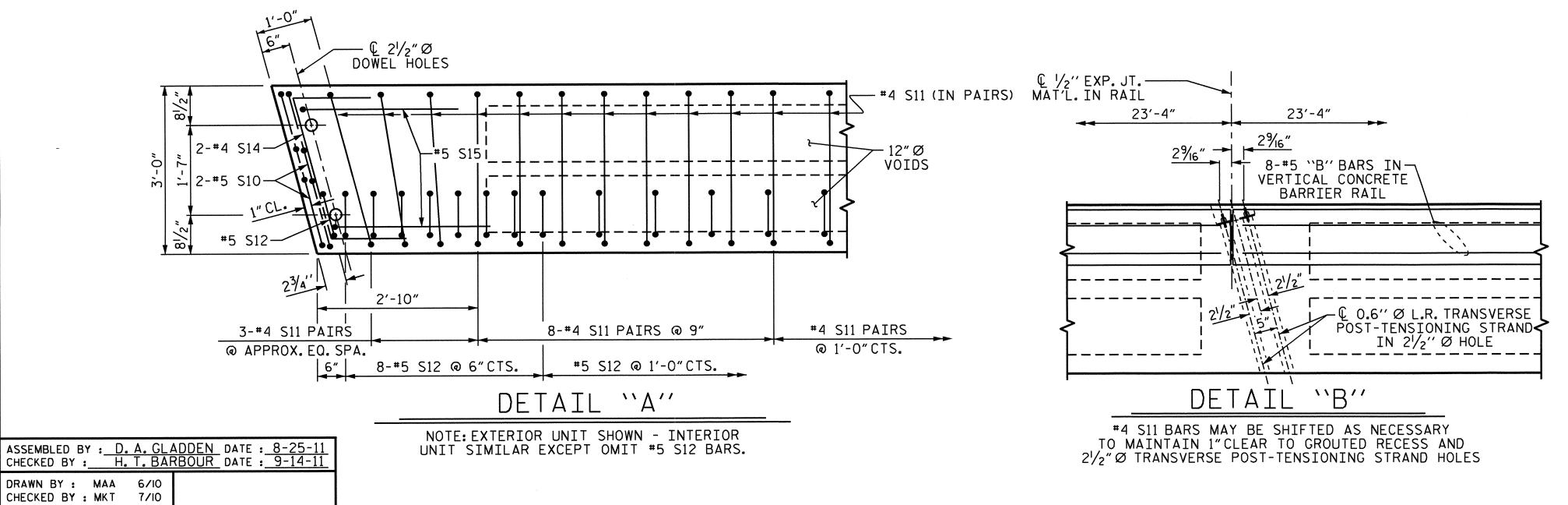
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT

SHEET NO. **REVISIONS** S-5 DATE: DATE: BY: BY: TOTAL SHEETS

SEAL 15825





PROJECT NO. B-4721

CALDWELL COUNTY

STATION: 14+31.28 -L-

SHEET 2 OF 3

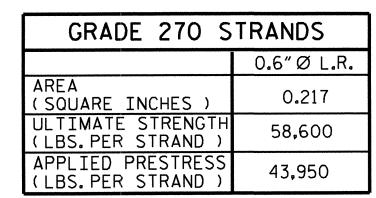
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

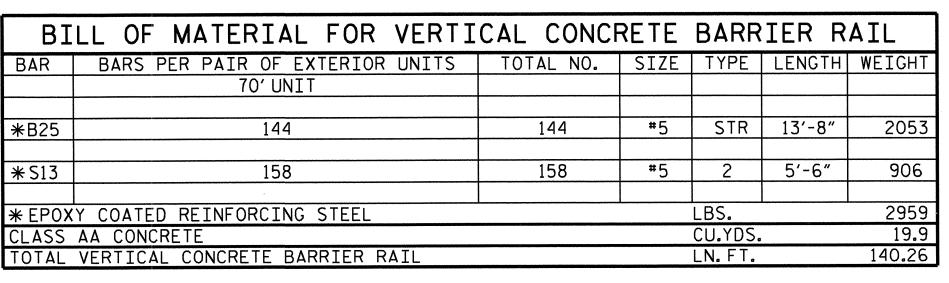
PLAN OF 70'UNIT 27'-10"CLEAR ROADWAY 75° SKEW

		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	S-6
1			3			TOTAL SHEETS
2			4			14



DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	45⁄ <sub>16</sub> " <b>∮</b>
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	13/16″ ♦
FINAL CAMBER	3 <sup>1</sup> /2″ <b>∮</b>

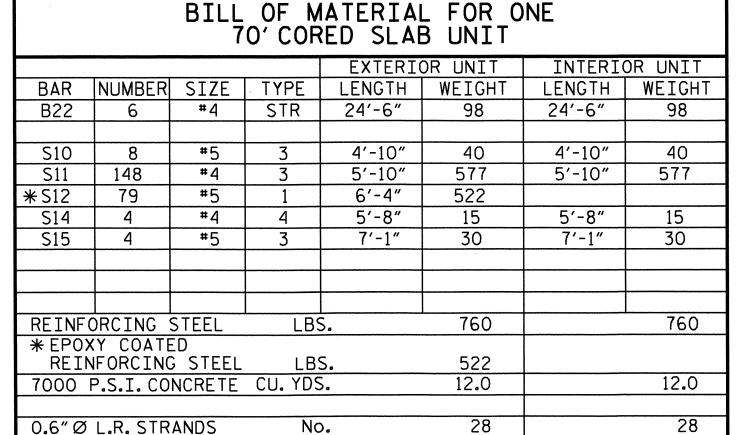
L				
**	INCLUDES	FUTURE	WEARING	SURFACE



\* THICKNESS AT MID-SPAN REFLECTS THE EFFECTS OF THE VERTICAL CURVE. VERTICAL CURVE ORDINATE =  $2\frac{3}{4}$ "

GUTTERLINE ASP	HALT THICKNESS & RA	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
70' UNITS	* 11/2"	3′-101/4′′

ALL BAR DIMENSIONS ARE OUT TO OUT



BAR TYPES

6"

\$10 \$11 " \$15

S15, 1'-8<sup>1</sup>/<sub>2</sub>"

S11 2'-8"

S10 1'-10"

73/4"

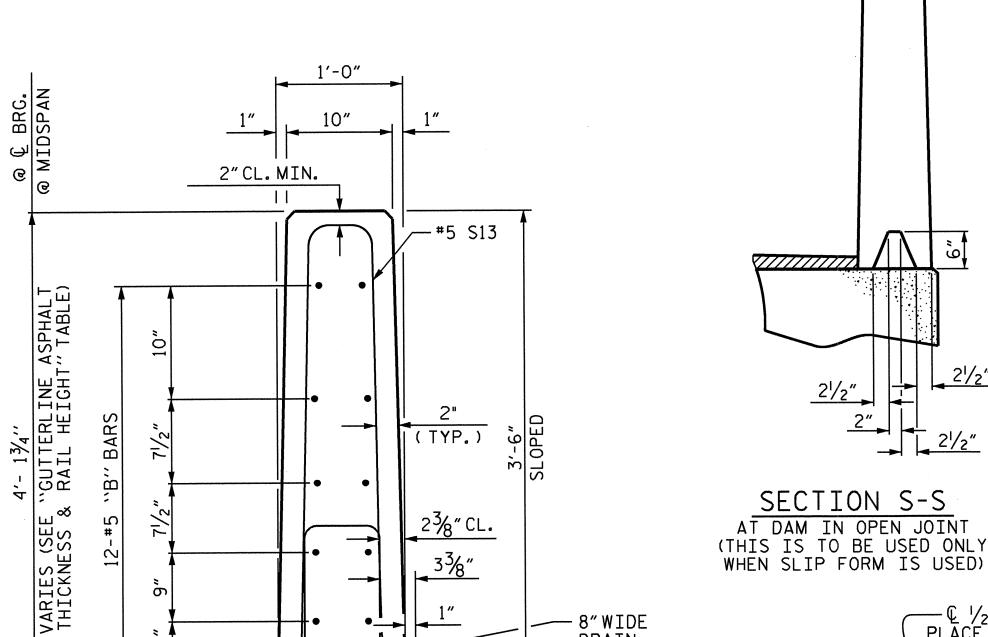
81/4" 1'-6"

1'-6"

# - C BEARING PAD -€ 1″Ø HOLES -BEARING PAD - TYPE I -FIXED END

### (TYPE I - 20 REQ'D) ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.



- #5 S12 SEE "PLAN OF

UNIT" FOR SPACING

CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
70' UNIT			
EXTERIOR C.S.	2	70′-0″	140'-0''
INTERIOR C.S.	8	70′-0″	560'-0"
TOTAL	10		700′-0″
		3-3-3-	

CONCRETE	RELEASE	STRENGTH
UNIT		PSI
70' UNITS		5500

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)		2'-0" 4-#5 S12 6" 4-#5 S12 #5 S12 & S13
PLACE WITH GALVANIZED NAILS (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED)	1" 10" 1" FIELD BEND— "B" BARS  *5 S13  *5 S13  *5 S13  *5 S13	
AUTTON UT EVI VIOTON OOTIVIO		

VERTICAL CONCRETE BARRIER RAIL DETAILS

8"WIDE DRAIN

**BLOCKOUT** (HEIGHT VARIES)

END OF RAIL DETAILS

SIDE VIEW

### NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

THE BOTTOM TWO #5 B25 BARS IN THE VERTICAL CONCRETE BARRIER RAIL MAY BE FIELD CUT TO AVOID DRAINS.

PROJECT NO. B-4721 CALDWELL \_ COUNTY STATION: 14+31.28 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 2'-0"

PRESTRESSED CONCRETE CORED SLAB UNIT

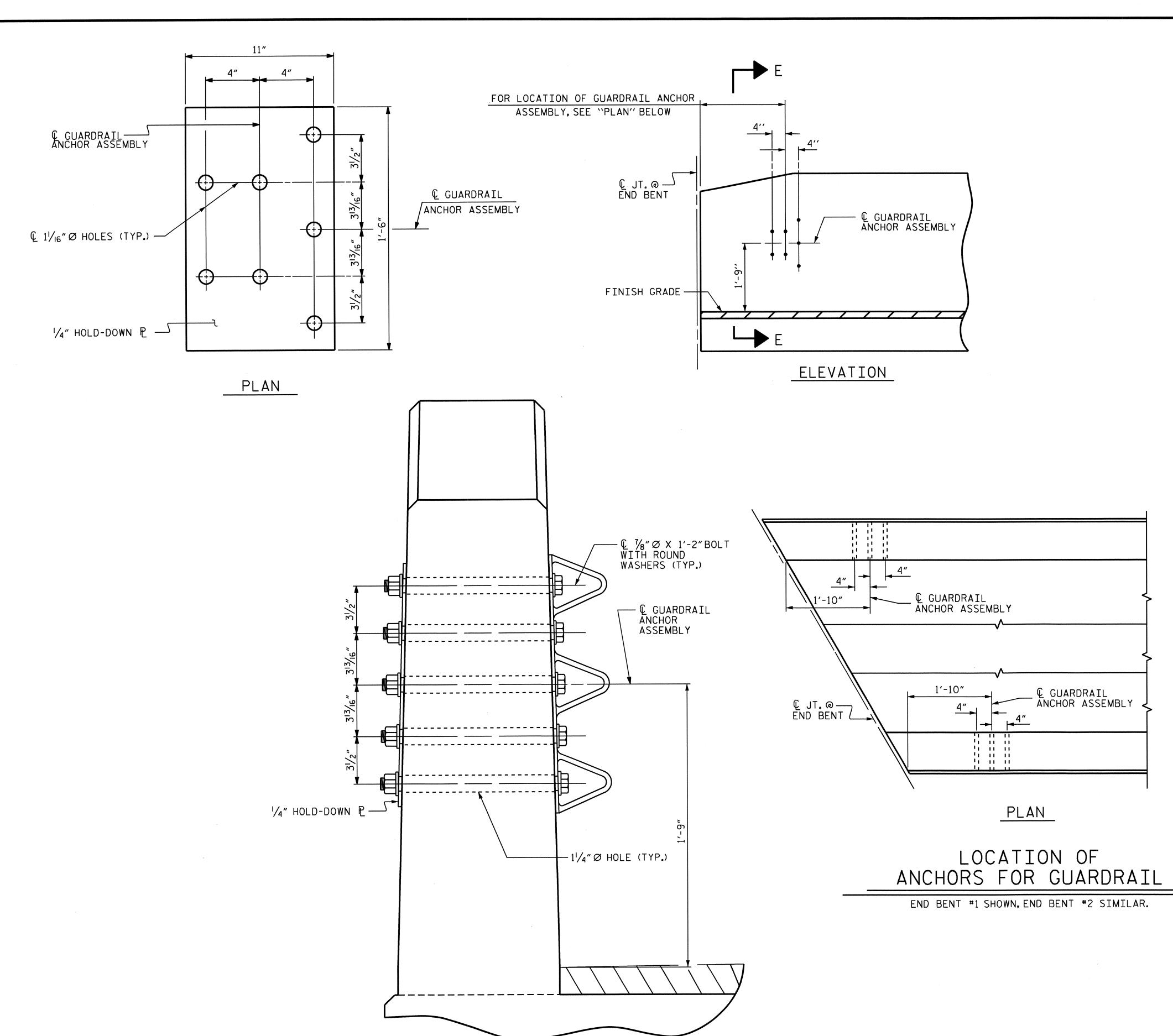
SHEET NO **REVISIONS** S-7 NO. BY: DATE: DATE: TOTAL SHEETS

ASSEMBLED BY : D. A. GLADDEN DATE : 8-25-11 CHECKED BY : H. T. BARBOUR DATE : 9-14-11 DRAWN BY : MAA 6/10 REV. 12/11 CHECKED BY : MKT 7/10

CONST. JT. —

SECTION THRU RAIL

END VIEW



SECTION E-E

ASSEMBLED BY: D. A. GLADDEN DATE: 8-25-11 CHECKED BY: H. T. BARBOUR DATE: 3-8-12

MAA/GM MAA/GM

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10

GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $1/4^{\prime\prime}$  HOLD DOWN PLATE AND 7 -  $1/8^{\prime\prime}$  Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

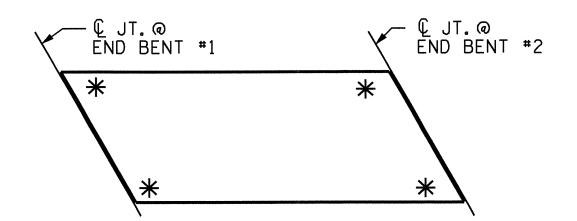
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



### SKETCH SHOWING POINTS OF ATTACHMENT

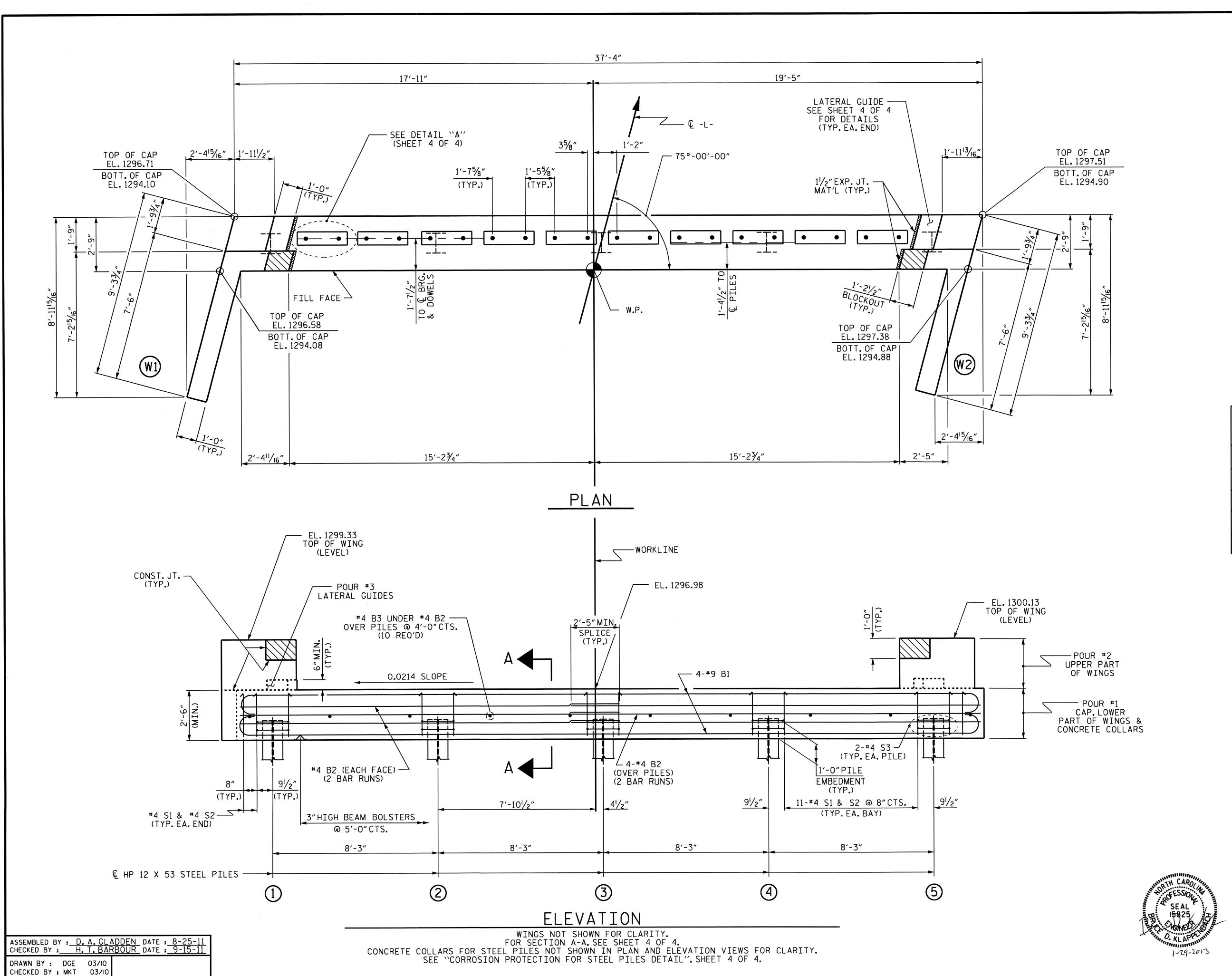
\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4721 CALDWELL \_ COUNTY STATION: 14+31.28 -L-



GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL

SHEET NO. REVISIONS NO. BY: DATE: BY: TOTAL SHEETS



### NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

TOP OF PILE ELEVATIONS					
1	1295.13				
2	1295.31				
3	1295.49				
4	1295.67				
5	1295.84				

PROJECT NO. B-4721

CALDWELL COUNTY

STATION: 14+31.28 -L-

SHEET 1 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No.1

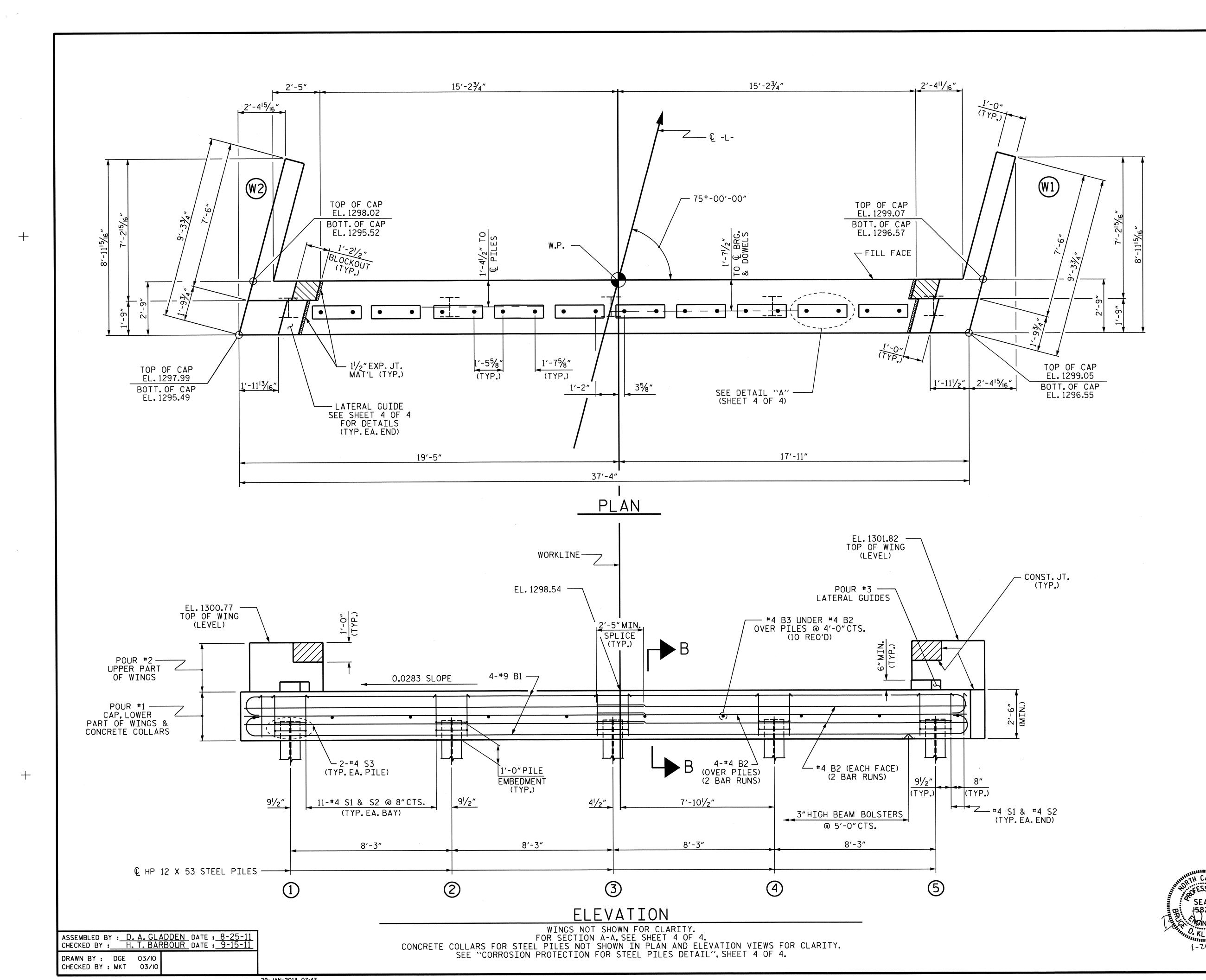
REVISIONS

SHEET NO. S-9

SOLUTION STATE: NO. BY: DATE: STATE

TOTAL SHEETS

14



### NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

TOP OF PILE ELEVATIONS					
1	1296.55				
2	1296.78				
3	1297.01				
4	1297.25				
5	1297.48				

B-4721 PROJECT NO. \_\_\_ CALDWELL COUNTY

14+31.28 -L-STATION:

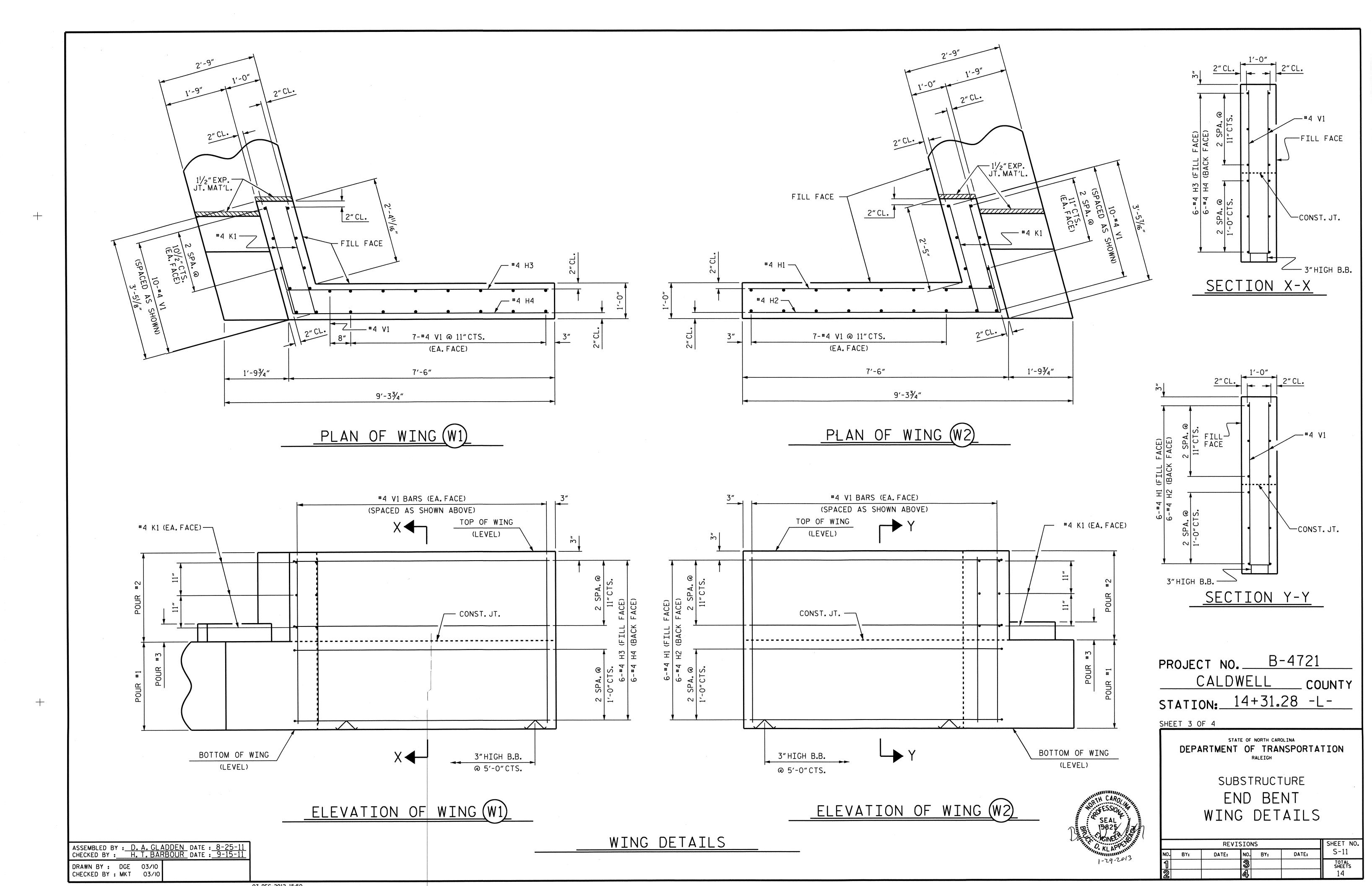
SHEET 2 OF 4

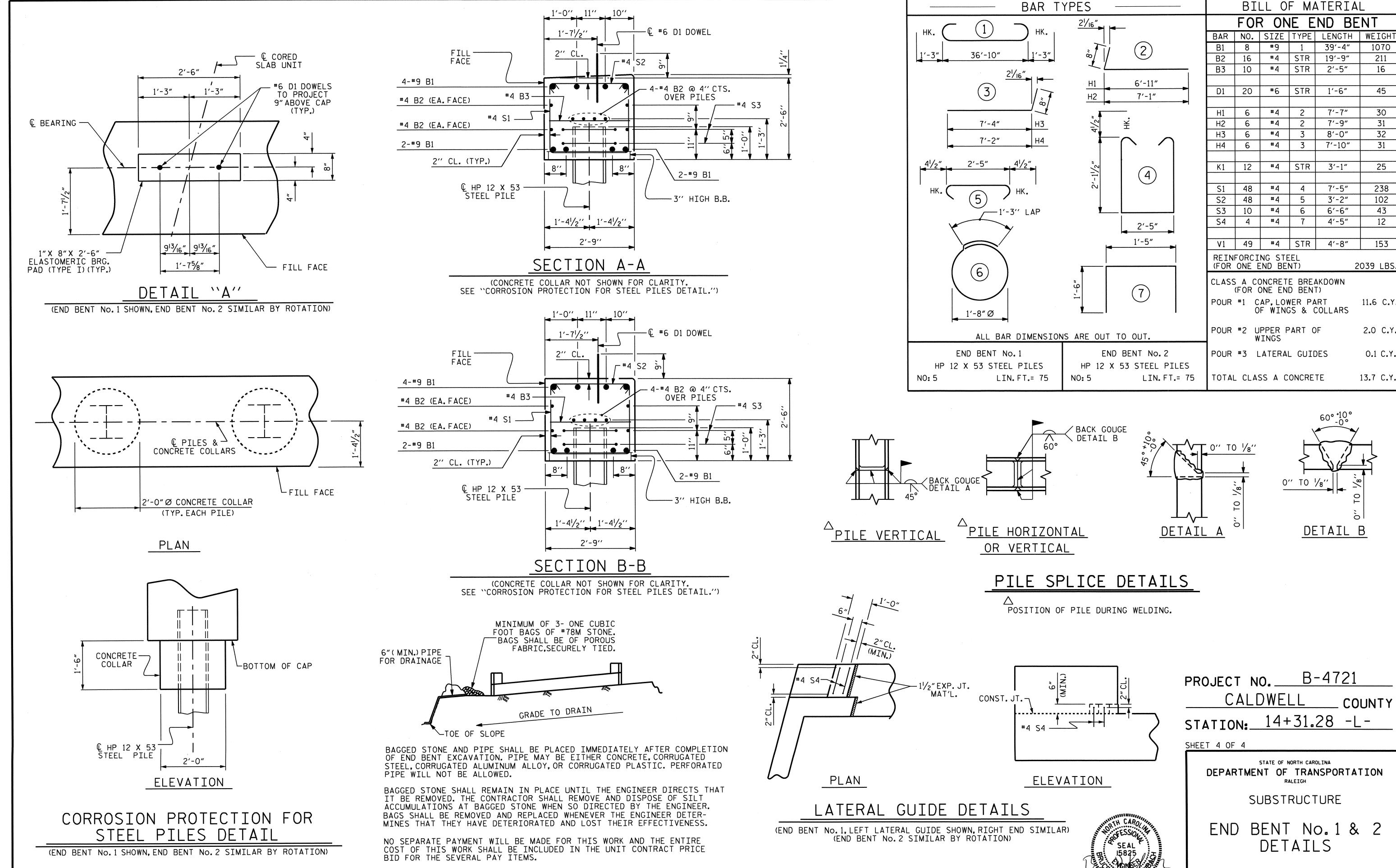
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

END BENT No. 2

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2	l		<u>A</u> l			14





ASSEMBLED BY: D. A. GLADDEN DATE: 8-25-11 CHECKED BY: H. T. BARBOUR DATE: 9-15-11

DRAWN BY: DGE 03/10 CHECKED BY : MKT 03/10

TEMPORARY DRAINAGE AT END BENT

(FOR ONE END BENT) POUR #1 CAP, LOWER PART 11.6 C.Y. OF WINGS & COLLARS POUR #2 UPPER PART OF 2.0 C.Y. POUR #3 LATERAL GUIDES 0.1 C.Y. 13.7 C.Y. TOTAL CLASS A CONCRETE

39′-4″

8'-0"

7′-10″

7′-5″

3′-2″

6′-6″

4'-5"

4′-8″

211

16

45

30

31

32

31

25

238

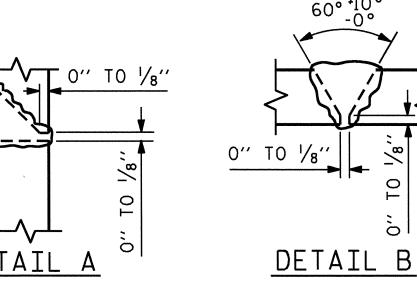
102

43

12

153

2039 LBS.





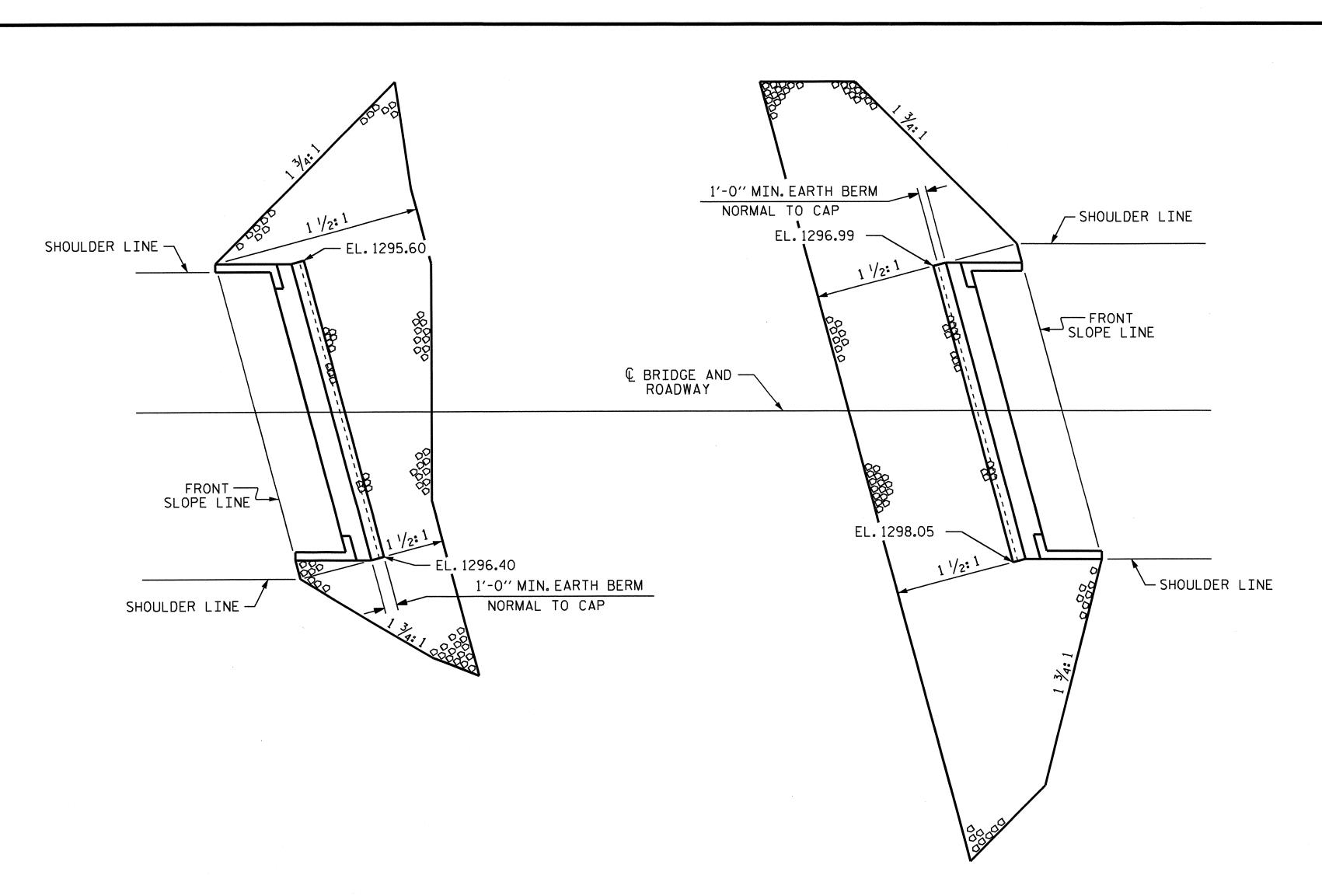
B-4721 PROJECT NO. \_\_ CALDWELL COUNTY 14+31.28 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

		SHEET NO.				
Ţ	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS
			4			14



NOTES:

FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

ESTIMA	ESTIMATED QUANTITIES				
BRIDGE @ STA.14+31.28 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	80	89			
END BENT 2	140	156			
TOTAL	220	245			

1'-7" MIN. BERM NORMAL TO CAP EL.1295.60 (END BT.1, LEFT SIDE) EL.1296.40 (END BT.1, RIGHT SIDE) PROJECT NO. B-4721 EL.1296.99 (END BT.2, LEFT SIDE) EL.1298.05 (END BT.2, RIGHT SIDE) CALDWELL SLOPE 1 1/2 :1 STATION: 14+31.28 -L-- GROUND LINE

RIP RAP AT END BENT 2

SECTION

2'-0''

1'-0" MIN. EARTH BERM
NORMAL TO CAP

RIP RAP AT END BENT 1

BERM RIP RAPPED

GEOTEXTILE ---

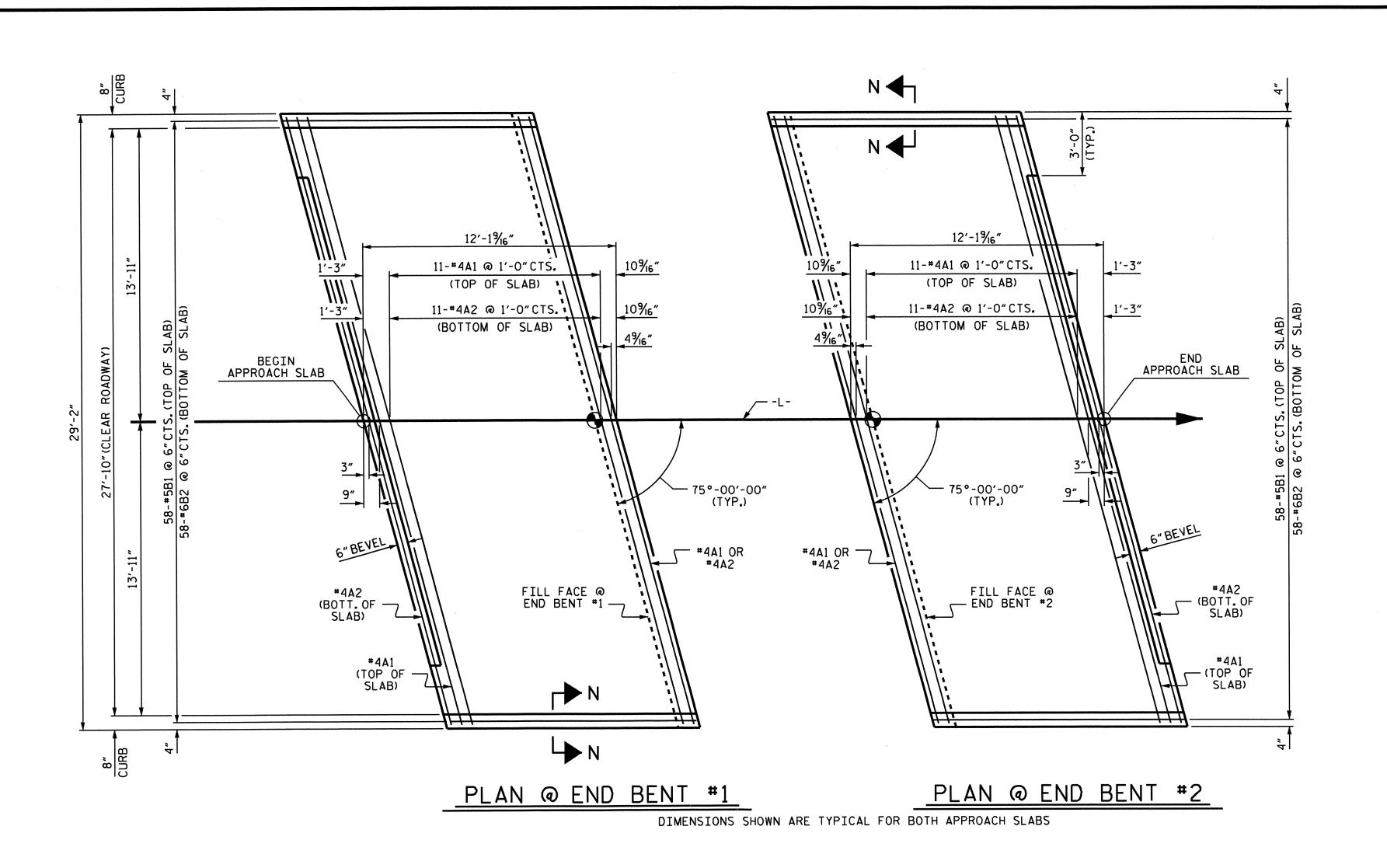
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD

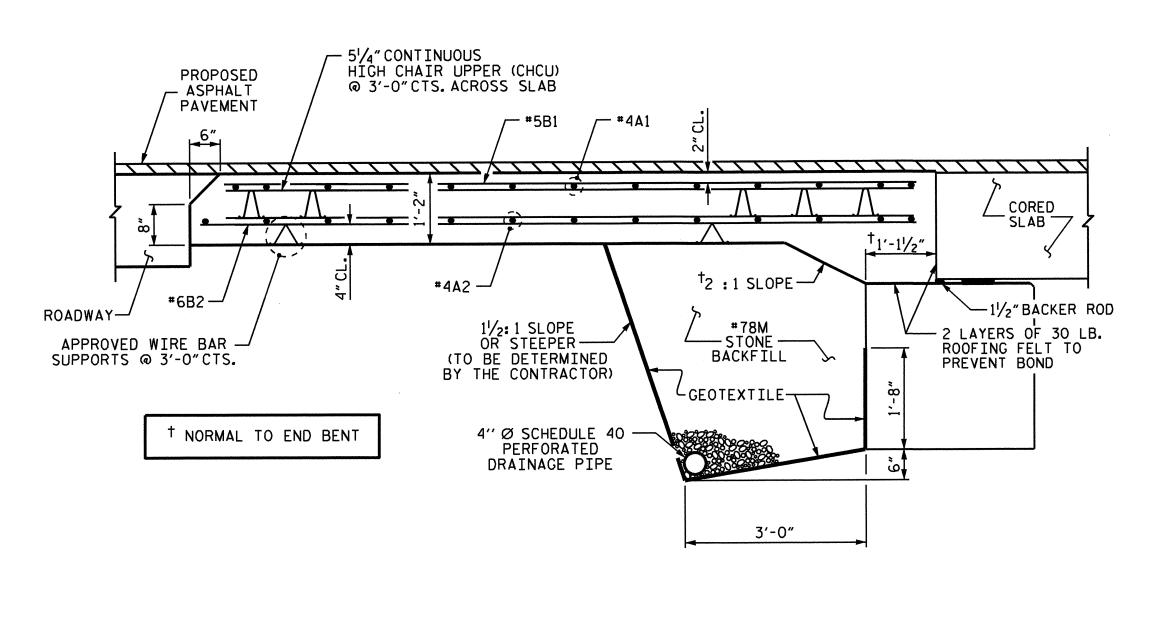
-RIP RAP DETAILS-

\_\_\_ COUNTY

SHEET NO. REVISIONS S-13 NO. BY: DATE: DATE: TOTAL SHEETS 14

ASSEMBLED BY: D. A. GLADDEN DATE: 8-25-11 CHECKED BY: H. T. BARBOUR DATE: 8-25-11 REV. 8/16/99 REV. 10/17/00 REV. 5/1/06R RWW/LES RWW/LES TLA/GM DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84





ASSEMBLED BY: D. A. GLADDEN DATE: 8-25-11 CHECKED BY: H. T. BARBOUR DATE: 9-14-11 DRAWN BY : SHS/MAA 5-09 REV. 12-11 MAA/AAC CHECKED BY : BCH 5-09

SECTION THRU SLAB

### NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

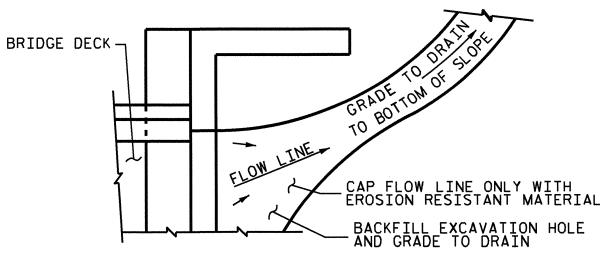
\*78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

\*78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS. AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL

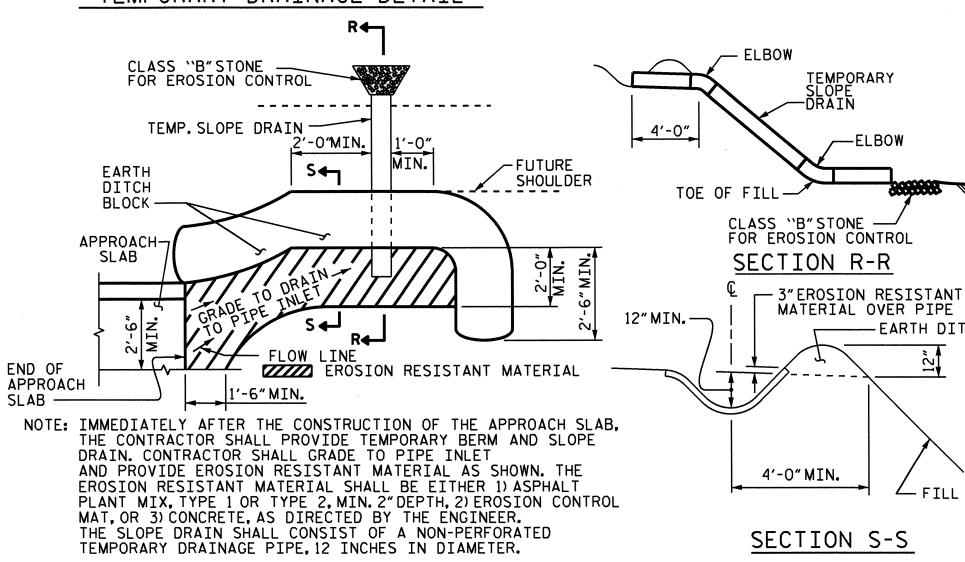
APPROACH SLAB GROOVING IS NOT REQUIRED.

BE PAVED. SEE ROADWAY PLANS.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

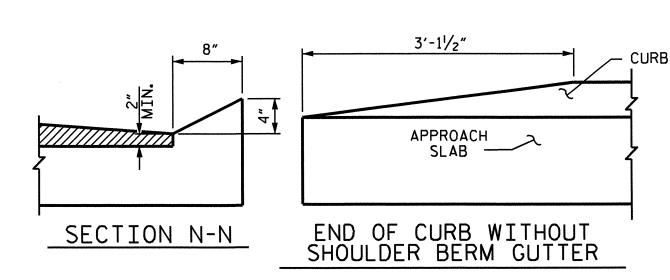
### TEMPORARY DRAINAGE DETAIL



PLAN VIEW

### TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



### CURB DETAILS

SPLICE LENGTHS							
BAR SIZE	EPOXY COATED	UNCOATED					
#4	2'-0"	1'-9"					
#5	2′-6″	2'-2"					
#6	3'-10"	2'-7"					



B-4721 PROJECT NO. CALDWELL COUNTY

SECTION S-S

BILL OF MATERIAL

13 | #4 | STR | 29'-10"

BAR NO. SIZE TYPE LENGTH

\* A1 | 13 | #4 | STR | 29'-10"

\*B1 | 58 | \*5 | STR | 11'-1"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

REINFORCING STEEL

\* EPOXY COATED

B2 | 58 | #6 | STR | 11'-7"

\* A1 | 13 | #4 | STR | 29'-10"

A2 | 13 | #4 | STR | 29'-10"

\*B1 | 58 | #5 | STR | 11'-1"

B2 | 58 | #6 | STR | 11'-7"

APPROACH SLAB AT EB #1

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

WEIGHT

259

259

670

1009

259

670

1009

1268

LBS.

LBS.

C.Y.

LBS.

LBS.

C. Y.

- FILL SLOPE

14+31.28 -L-STATION:\_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 75° SKEW

SHEET NO. REVISIONS S-14 NO. BY: DATE: DATE: TOTAL SHEETS

### STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS - - - - - - - - - - - - - - - A.A.S.H.T.O. (CURRENT)

LIVE LOAD - - - - - - - - - - - - - SEE PLANS

IMPACT ALLOWANCE - - - - - - - - - - SEE A.A.S.H.T.O.

STRESS IN EXTREME FIBER OF

STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN.

- AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN.

REINFORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SQ. IN.

CONCRETE IN COMPRESSION - - - - - - - - - - - 1,200 LBS. PER SQ. IN.

CONCRETE IN SHEAR - - - - - - - - - SEE A.A.S.H.T.O.

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS.PER SQ. IN.

COMPRESSION PERPENDICULAR TO GRAIN

EQUIVALENT FLUID PRESSURE OF EARTH - - - - 30 LBS. PER CU. FT.

OF TIMBER ----

(MINIMUM)

375 LBS. PER SQ. IN.

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990