NOTE: SEE SHEET 1A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

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LINE -L- <u>STATION</u> 13+28.00 to 25+30.00 $\begin{array}{ccc} \underline{PLAN} & \underline{PROFILE} & \underline{XSECT} \\ \underline{4} & \underline{5-6} & \underline{7-9} \end{array}$

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 33630.I.I (B-429I) F.A. PROJ. BRSTP-I504(7)

TRANSYLVANIA
PROJECT DESCRIPTION APPROACHES TO BRIDGE NO. 193 ON
SR-I504 OVER DAVIDSON RIVER

INVENTORY

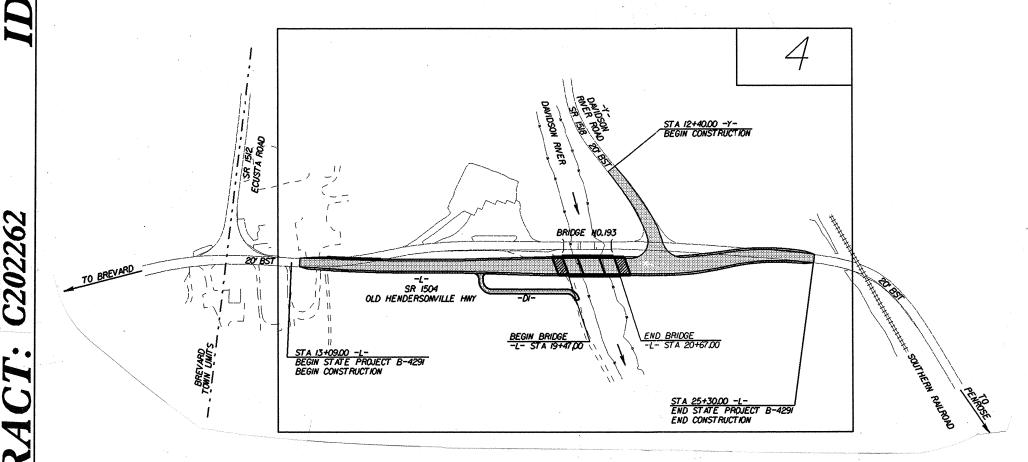


CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORNIS (DOS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVEWED OR INSPECTEE IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 199) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORNING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNIOS OR BETWEEN SAMPLED STRATA WITHIN THE BORENOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU UN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY BINEFART IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS AND TAYEY CONSIDERABLY WITH THE ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMMARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPHINON OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HUNSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS TO BE CONDITIED AT THE SITE DIFFERING FROM THOSE MIGHTED AT THE SUBSURFACE INFORMATION.



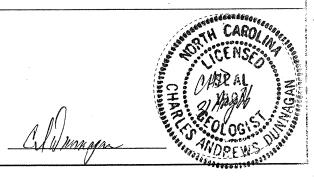
INVESTIGATED BY C A DUNNAGAN

CHECKED BY W D FRYE, Jr

SUBMITTED BY W D FRYE, Jr

DATE AUGUST 2006

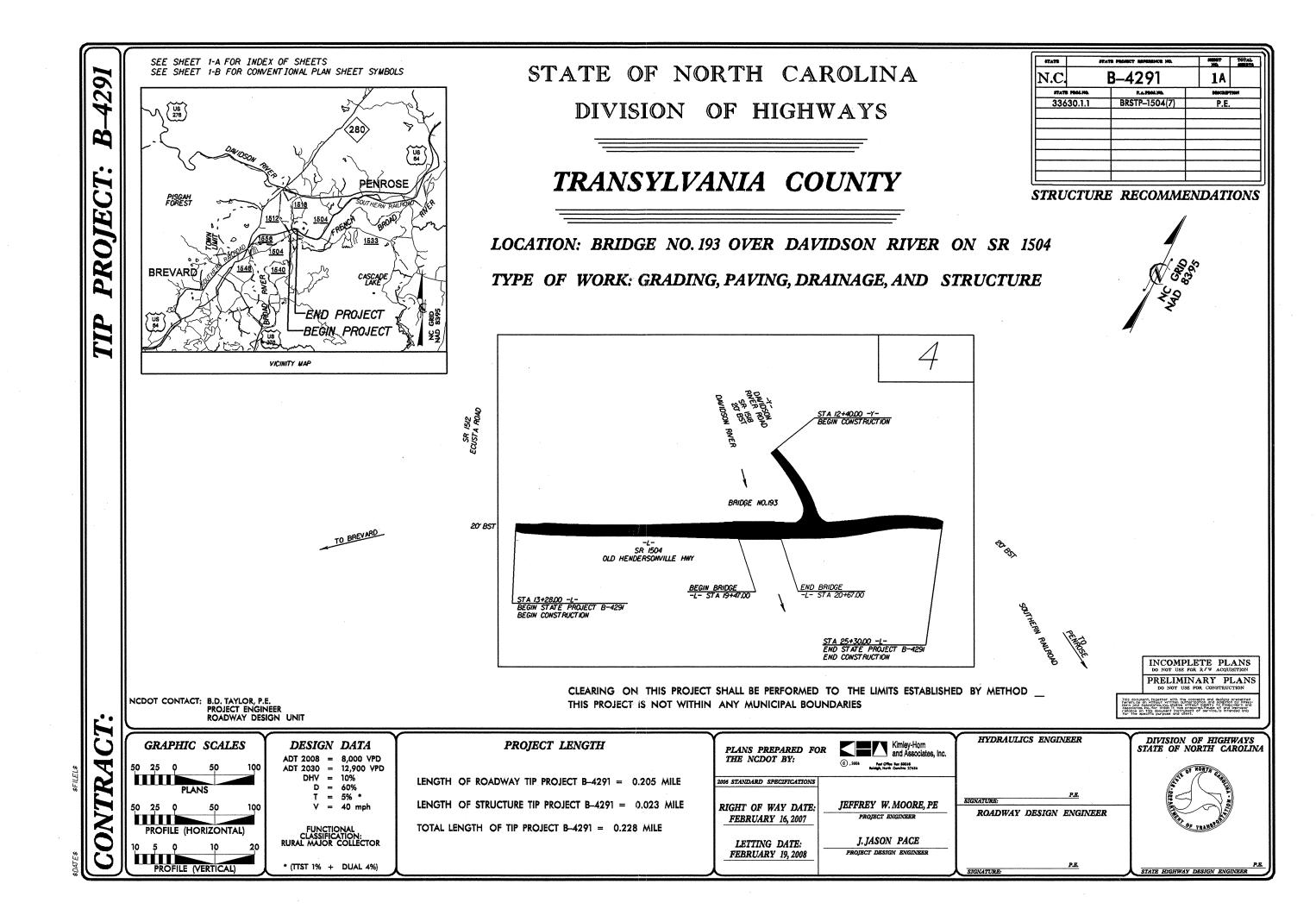
PERSONNEL
T B DANIEL
M M HAGER
C J COFFEY
R D CHILDERS



C A DUNNAGAN

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED SY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

	SOIL AND ROCK LEGEND, TERM	MS, SYMBOLS, AND ABBREVIATIONS			
SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS		
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUYIUM (ALLUY,) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.		
THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 190 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO 1206, ASTM D-1586). SOIL	POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE	AQUIFER - A WATER BEARING FORMATION OR STRATA.		
CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE:	ANGULARITY OF GRAINS	OF WEATHERED ROCK.	ARENACEUUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED PROM SHIPD ON THAT CONTINUE		
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR,	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.		
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL		
SOIL LEGEND AND AASHTO CLASSIFICATION	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.		
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS (≤ 35% PASSING *200) (> 35% PASSING *200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.		
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE	COLLUYIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM		
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31	ROCK (NCR) INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. COASTAL PLAIN L COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.		
SYMBOL 000000000000000000000000000000000000	MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
% PASSING SILT-	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT		
# 10 50 MX GRANULAR CLAY MUCK,	ORGANIC MATERIAL GRANULAR SILT - CLAY SOILS SOILS OTHER MATERIAL		ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE		
40 30 MX 50 MX 51 MN 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN S0ILS S0ILS	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	HORIZONTAL.		
LIQUID LIMIT 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN 48 MX 41 MN SOILS WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI,) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF		
PLASTIC INDEX 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN LITTLE OR HIGHLY	HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE		
AMOUNTS OF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.		
OF MAJOR GRAVEL, AND GRAVEL AND GRAVEL AND SAND SOLLS SOLLS MATTER	■ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ■ STATIC WATER LEVEL AFTER 24 HOURS	(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.		
MATERIALS SAND SHIND SHI		MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELOSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM		
AS A EXCELLENT TO GOOD FAIR TO POOR PAIR TO POOR UNSUITABLE	E PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY		
SUBGRADE 1.55% 1.	SPRING OR SEEP	WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	THE STREAM.		
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN		
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION SPT *** TEST BORING DESIGNATIONS	(MOD, SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL	THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.		
CONSISTENCY (N-VALUE) (TONS/FT2)	WITH SOIL DESCRIPTION S - BULK SAMPLE	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO		
GENERALLY VERY LOOSE 4 CRANII AR LOOSE 4 TO 10	SOIL SYMBOL AUGER BORING SS - SPLIT SPOON	(SEV.) IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	ITS LATERAL EXTENT.		
MATERIAL MEDIUM DENSE 10 TO 30 N/A	ARTIFICIAL FILL (AF) OTHER SAMPLE	IF TESTED, YIELDS SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN		
(NON-COHESIVE) DENSE 30 TO 50 VERY DENSE >50	ST - SHELBY TUBE	VERY SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT (V SEV.) THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.		
VERY SOFT (2 (0.25	- INFERRED SOIL BOUNDARY MONITORING WELL DE DOCK CAMPLE	REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.		
GENERALLY SOFT 2 TO 4 0.25 TO 0.50 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE PIEZOMETER	VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, VIELDS SPT N VALUES < 100 BPF AL COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.		
MATERIAL STIFF 8 TO 15 1 TO 2	ALLUVIAL SOIL BOUNDARY	SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF		
(COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD >30 >4	25/025 DIP & DIP DIRECTION OF SLOPE INDICATOR INSTALLATION CBR - CALIFORNIA BEARING	ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
TEXTURE OR GRAIN SIZE	ROCK STRUCTURES — SPT N-VALUE RATIO SAMPLE	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE		
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	SOUNDING ROD REF— SPT REFUSAL	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	PARENT ROCK,		
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	ABBREVIATIONS	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL		
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	AR - AUGER REFUSAL HI HIGHLY W - MOISTURE CONTENT	TO DETACH HAND SPECIMEN.	TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.		
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	BT - BORING TERMINATED MED MEDIUM V - VERY CL CLAY MICA MICACEOUS VST - VANE SHEAR TEST	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.		
GRAIN MM 305 75 2.0 0.25 0.05 0.005	CPT - CONE PENETRATION TEST MOD MODERATELY WEA WEATHERED	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF		
SIZE IN. 12 3	CSE COARSE NP - NON PLASTIC 7 - UNIT WEIGHT DMT - DILATOMETER TEST ORG ORGANIC 7/4- DRY UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS		
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHIPS FOR FIELD MOISTURE OF SCHOOL	DPT - DYNAMIC PENETRATION TEST PMT - PRESSUREMETER TEST	POINT OF A GEOLOGIST'S PICK.	THAN 0.1 FOOT PER 60 BLOWS.		
(ATTERBERG LIMITS) OESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION OESCRIPTION	e - VOID RATIO SAP SAPROLITIC F - FINE SD SAND, SANDY	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH		
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	FOSS FOSSILIFEROUS SL SILT, SILTY	PIECES CAN BE BROKEN BY FINGER PRESSURE.	OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY		
(SAT.) FROM BELOW THE GROUND WATER TABLE	FRAC FRACTURED, FRACTURES SLI SLIGHTLY FRAGS FRAGMENTS TCR - TRICONE REFUSAL	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.		
PLASTIC CEMISOLID. PEGUIDES DEVING TO		FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.		
RANGE S - WET - (W) ATTAIN OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	FRACTURE SPACING BEDDING TERM SPACING IERM THICKNESS			
PLL PLASTIC LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	VERY THICKLY REDDED > 4 FFFT	BENCH MARK:		
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	I I I CIAV RITS	WIDE 3 TO 10 FEET THINKY DEDDED 0.16 - 15 FEET	ELEVATION: FT.		
SL _ SHRINKAGE LIMIT	MOBILE B- 6 CONTINUOUS FLIGHT AUGER CORE SIZE:	MODERATELY CLOSE 1 TO 3 FEET VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:		
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	BK-51 8 HOLLOW AUGERS	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	10125		
PLASTICITY	—) <i>_</i>	INDURATION			
PLASTICITY INDEX (PI) DRY STRENGTH	TING CAPOIDS INCOME	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.			
NONPLASTIC 0-5 VERY LOW	X CME-550 X CASING X W/ ADVANCER	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.			
LOW PLASTICITY 6-15 SLIGHT MED. PLASTICITY 16-25 MEDIUM	HAND TOULS:	20110 011 05 0500 011 011 015 015 015 015			
HIGH PLASTICITY 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH POST HOLE DIGGER	MODERATELY INDURATED CHAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: BREAKS EASILY WHEN HIT WITH HAMMER.			
COLOR	TRICONETUNGCARB. HAND AUGER	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT SOCIOLING NOD	DIFFICULT TO BREAK WITH HAMMER.			
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.			

PROJECT REFERENCE NO.

33630.1.1 (B-4291)

EARTHWORK BALANCE SHEET

Volumes in Cubic Yards

PROJECT B-4291 COUNTY Transylvania DATE 1/29/2012 TIP#

SHEET XOF X SHEETS

			TOTAL													
LINE	STATION	STATION	EXCAV.	ROCK	UNDERCUT	UNSUIT.	SUITABLE	TOTAL	ROCK	UNDERCU	EARTH	EMBANK.	BORROW	SUITABLE	UNSUIT.	TOTAL
			(UNCL.)	EXCAV.	EXCAV.	EXCAV.	EXCAV.	ЕМВ.	EMB.	EMB.	ЕМВ.	15%		WASTE	WASTE	WASTE
PHASE1																
L	13+09.00	19+47.00	372	0	0	205	167	1217	0	0	1217	1400	1233	0	205	205
L	20+67.00	25+30.00	20	0	0	0	20	521	0	0	521	599	579	0	0	0
Y	12+40.00	13+50.00	8	0	0	0	8	25	0	0	25	29	21	0	0	0
		SUBTOTAL	400	0	0	205	195	1763	0	0	1763	2028	1833	0	205	205
PHASE2			· · · · · · · · · · · · · · · · · · ·			***************************************										
L	13+28.00	19+47.00	284	0	0	156	128	52	0	0	52	60	0	68	156	224
L	20+67.00	25+30.00	555	0	0	305	250	23	0	0	23	26	0	224	305	529
Y	13+50.00	14+79.41	200	0	0	0	200	10	0	0	10	12	0	188	0	188
		SUBTOTAL	1039	0	0	461	578	85	0	0	85	98	0	480	461	941
		TOTAL	1.420				770	1040			1040	0106	1022	400		1116
		TOTAL	1439	0	0	666	773	1848	0	0	1848	2126	1833	480	666	1146
LOSS DU	E TO CLEARING	& GRUBBING	-175				-175						175			
	PROJECT TOT.	ALS	1264	0	0	666	598	1848	0	0	1848	2126	2008	480	666	1146
EST 5% FOR R	L REPLACING TOP:	SOIL ON BORROW														
	PITS												100			
	GRAND TOTA	LS	1264										2108			
SAY		1300										2200				
CONTINGENC	CY SHALLOW UN	NDERCUT = 500 CY														
CLASS IV SUB	GRADE STABILIZ	ZATION = 475 TONS														
ES	STIMATED DBE =	= 90 CY														

^{*} EARTHWORK QUANTITIES ARE CALCULATED BY THE ROADWAY DESIGN UNIT. THESE EARTHWORK QUANTITIES ARE BASED IN PART ON SUBSURFACE DATA PROVIDED BY THE GEOTECHNICAL ENGINEERING UNIT.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

MICHAEL F. EASLEY
GOVERNOR

LYNDO TIPPETT SECRETARY

August 25, 2006

STATE PROJECT:

33630.1.1 (B-4291)

F. A. PROJECT:

BRSTP-1504(7)

COUNTY:

Transylvania

DESCRIPTION:

Approaches to Bridge No. 193 on SR-1504 over the Davidson River

SUBJECT:

Geotechnical Report - Inventory

Project Description

This project is located approximately 2.0 miles east of Brevard. The existing bridge will be replaced with a structure 50.0 feet downstream (South). A subsurface investigation was performed on the planned approaches to the new bridge. The investigation was conducted using a CME-550 and 8-inch hollow stem augers. Standard Penetration Tests were performed at intervals of 5.0 feet using an auto-hammer. Soil samples were collected and submitted for testing of quality.

Areas of Special Geotechnical Interest

1) <u>Soft Foundation Material</u>: Two areas of artificial fill were encountered on the project. The locations and type of material are listed below.

<u>Line</u>	Station Interval	<u>Material</u>
-L-	17+25 to 19+50	Dark gray to black silty sand and brown silty sand with gravel and brick fragments
-L-	21+40 to (Approx.) 24+00	Dark gray to black sandy silt

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MAILING ADDRESS: NC DEPARTMENT OF TRANSPORTATION GEOTECHNICAL UNIT 1589 MAIL SERVICE CENTER RALEIGH NC 27699-1589 TELEPHONE: 919-250-4088 FAX: 919-250-4237

WEBSITE: WWW.DOH.DOT.STATE.NC.US

LOCATION: CENTURY CENTER COMPLEX BUILDING B 1020 BIRCH RIDGE DRIVE RALEIGH NC 27610 2) <u>Debris and Waste</u>: Construction debris was encountered at the following bore hole locations.

Line	Station Interval	Material
-L- -L-	17+50, CL	Bricks and Gravel
-L-	17+75, CL 18+50, CL	Bricks and Gravel Bricks and Gravel

3) Potential Hazardous Site: The Geo-Environmental Section of the Geotechnical Engineering Unit is completing a preliminary assessment of the potential for hazardous material within soils found on this project. Specifically, within the approximate -L- Station intervals of 17+25 to 19+50 and 21+40 and 24+00. These soils would underlie one to 5 feet of proposed embankment.

Physiology and Land Use

The area involved in this project is the flat, floodplain of the Davidson River. Major portions have been filled in with construction materials (gravel and bricks) and a peculiar black sandy silt and silty sand of unknown origin. Relative to this black material, it should be noted that a defunct paper mill is located near this project.

The project corridor crosses woodlands and a fallow field.

Groundwater

Static groundwater was not measured in any of the borelogs. However, it is presumed to be at about the surface elevation of the Davidson River. This elevation is at approximately 2092.5.

Geotechnical Descriptive Analysis

For descriptive purposes, this project is divided into two segments. Separating the segments is the Davidson River.

Segment 1: <u>-L- Station 13+28.00 to 19+47.00</u>

Construction in this segment will consist mostly of embankments. These will be less than 5.0 feet high. Cuts are confined to the left of centerline. These are to create a ditchline, and are less than 5.0 feet deep. Five borelogs were advanced in this segment. Alluvium is present at the ground surface to approximate Station 17+40. This is a soft sandy silt with a trace of organic matter. A basal gravel layer is usually present, separating the alluvium and the saprolite. This contact is between 9.0 and 12.0 feet below the ground surface.

From approximate Station 17+40 to approximate Station 19+25, fill material is present at the surface. This is dark gray to black loose silty sand over a layer of dark gray to brown silty sand with gravel and brick fragments. The black horizon contains trace amounts to no organic matter, and has not discernable odor.

Because of the questionable nature (and potential hazards) of this material, drilling was halted before the limits of the fill could be more accurately delineated.

The saprolite in this segment is a hard sandy silt.

Weathered rock of mylonite and phylonite was encountered at -L- Station 17+00, at a depth of 12.1 feet.

Segment 2: <u>-L- Station 20+67.00 to 25+30.00</u>

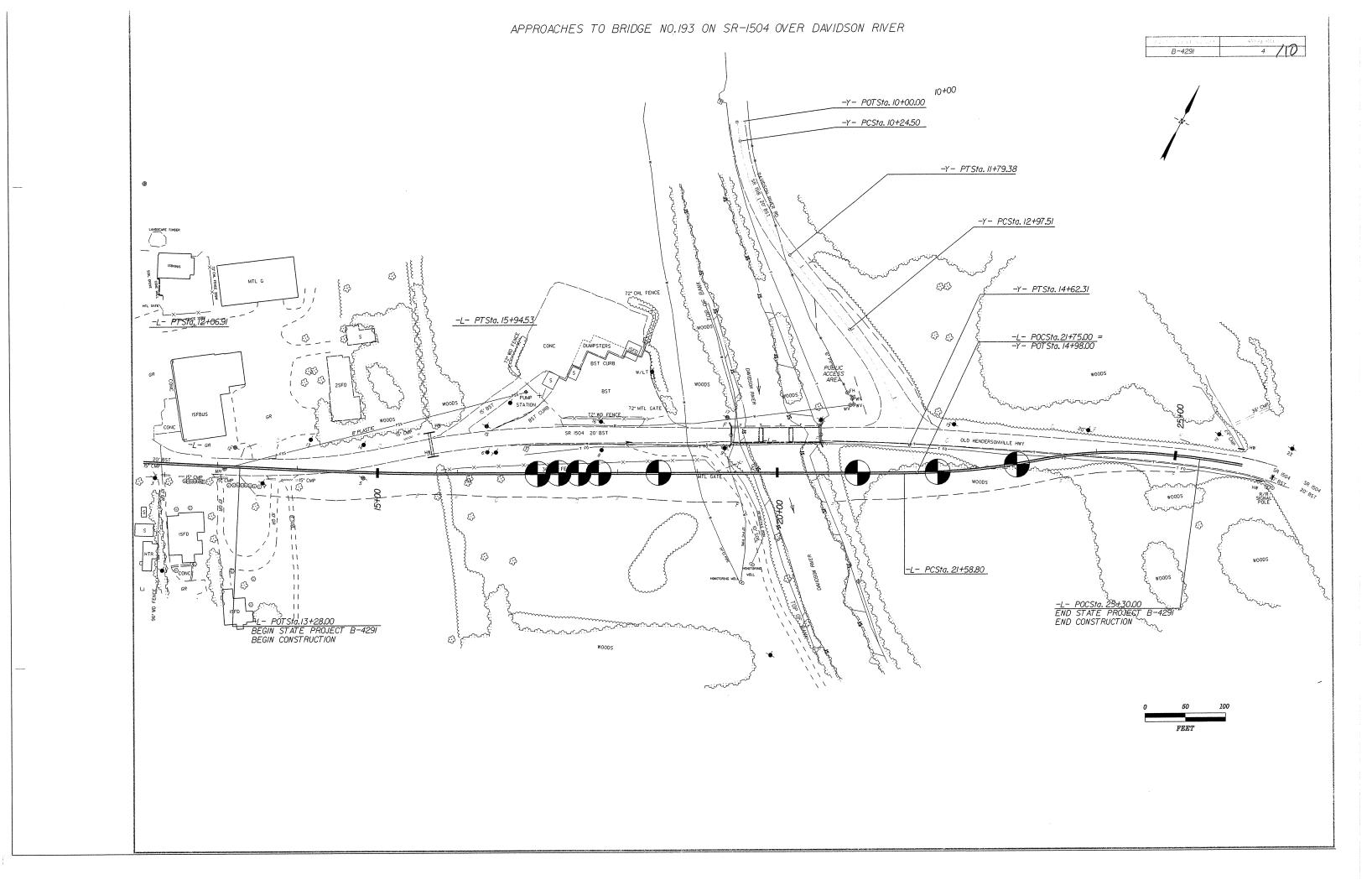
Construction in this segment is very much like that of Segment 1: embankments of 5.0 feet or less and cuts to establish a ditchline. Three borings were advanced in this segment.

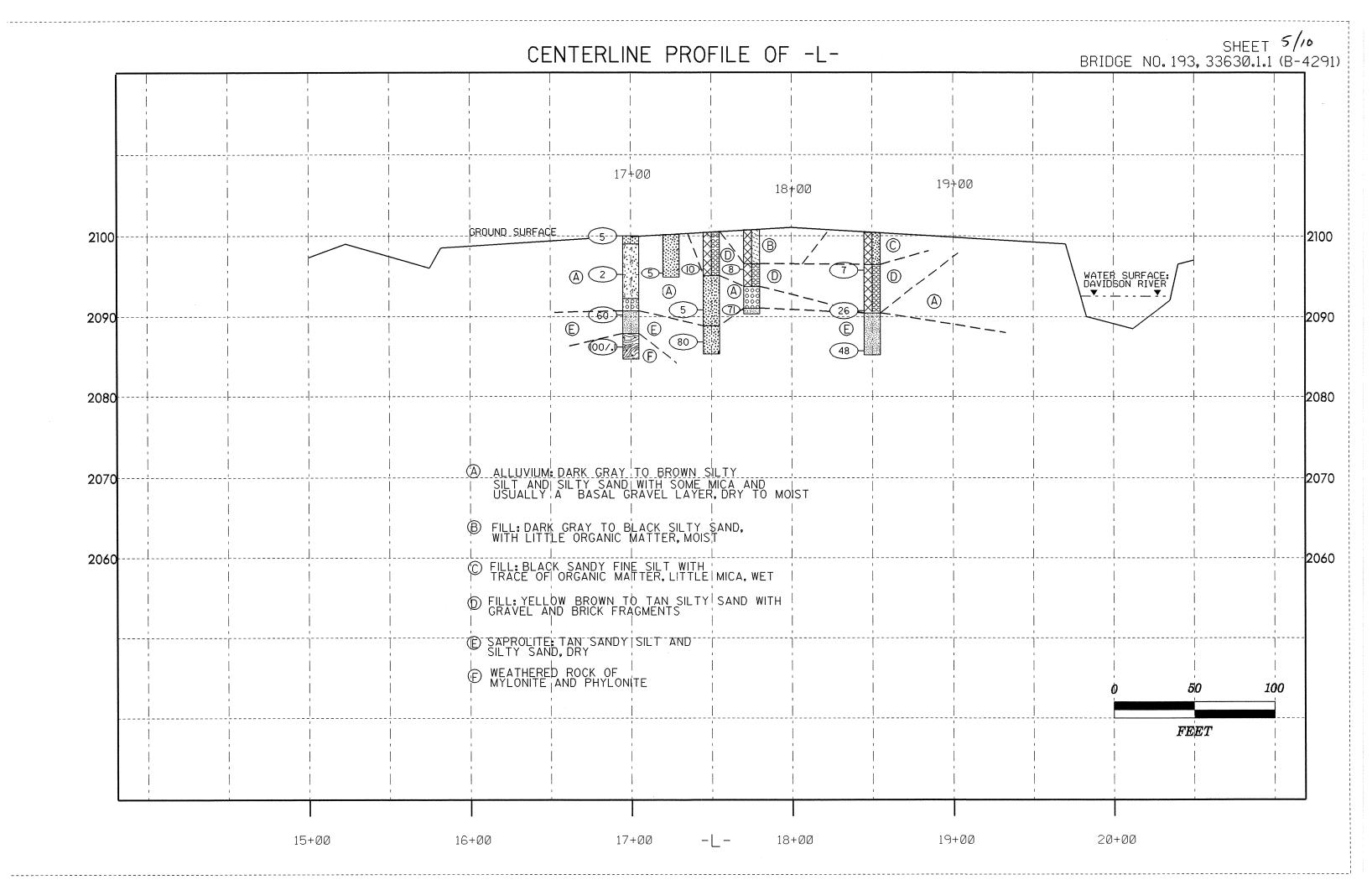
Fill material is present in this segment too. It is between approximate Stations 21+50 and 23+50. On this side of the river, the fill consists of 8.0 feet or less of soft sandy silt. Here too, organic matter is present in trace amounts to none at all; no odor was noted.

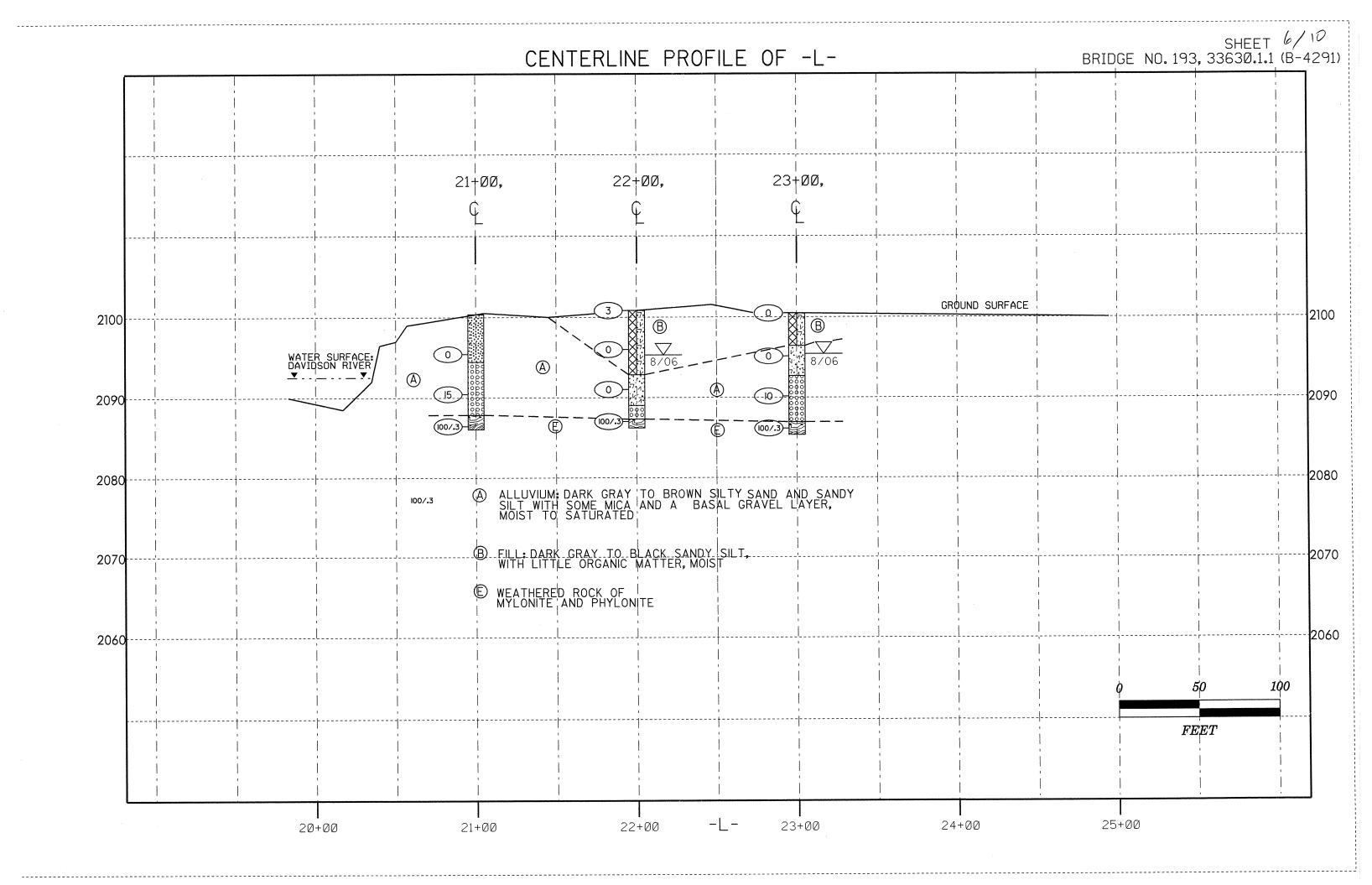
Outside of the fill interval, alluvium is present at the ground surface. This is comprised of very loose to medium dense silty sands and gravels, and very soft sandy silt. Saprolite was not noted in any of the borings. The alluvium was deposited on weathered rock of mylonite and phylorite at a uniform elevation 2087.5. This is approximately 12.0 feet below ground surface.

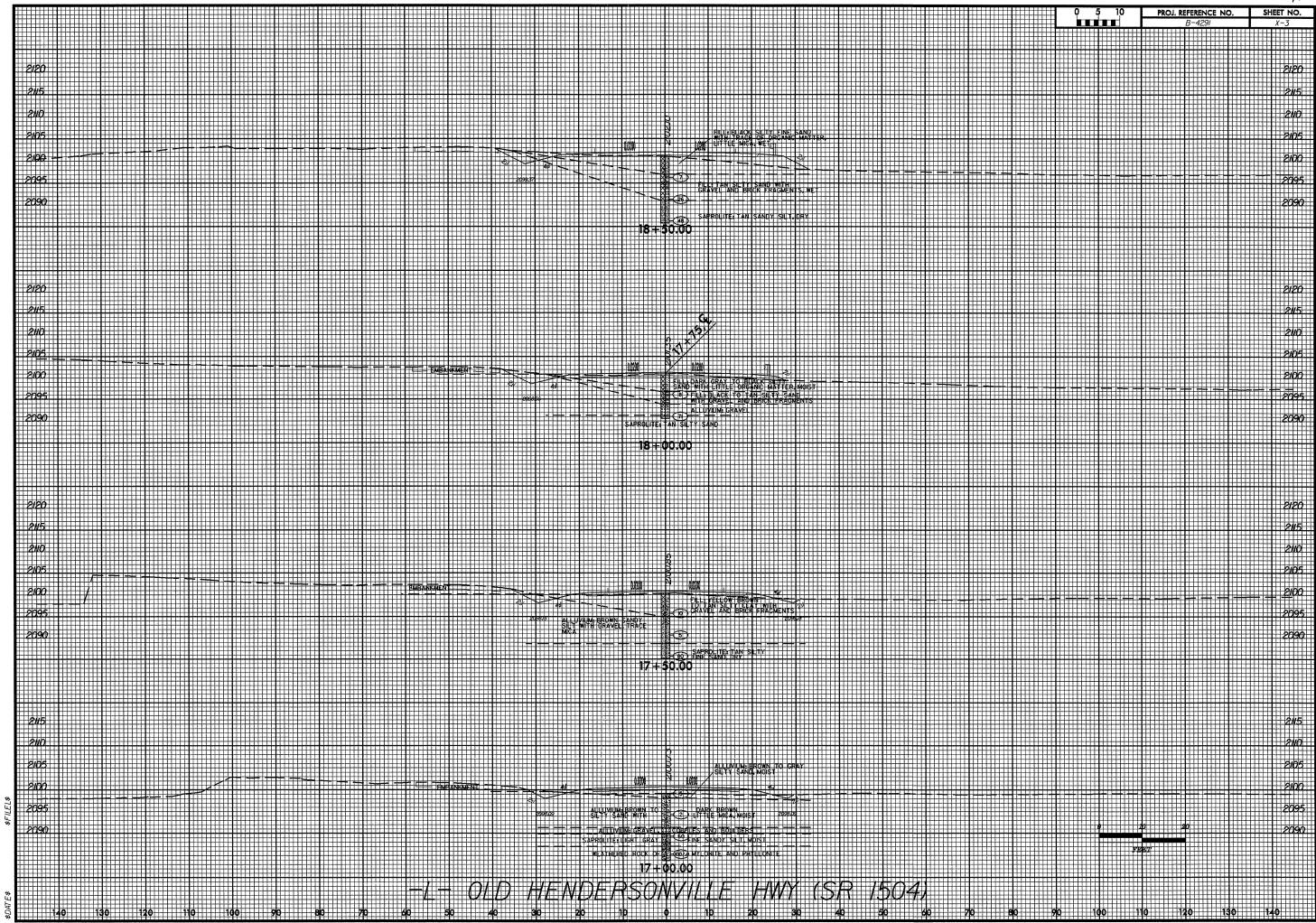
Respectfully Submitted,

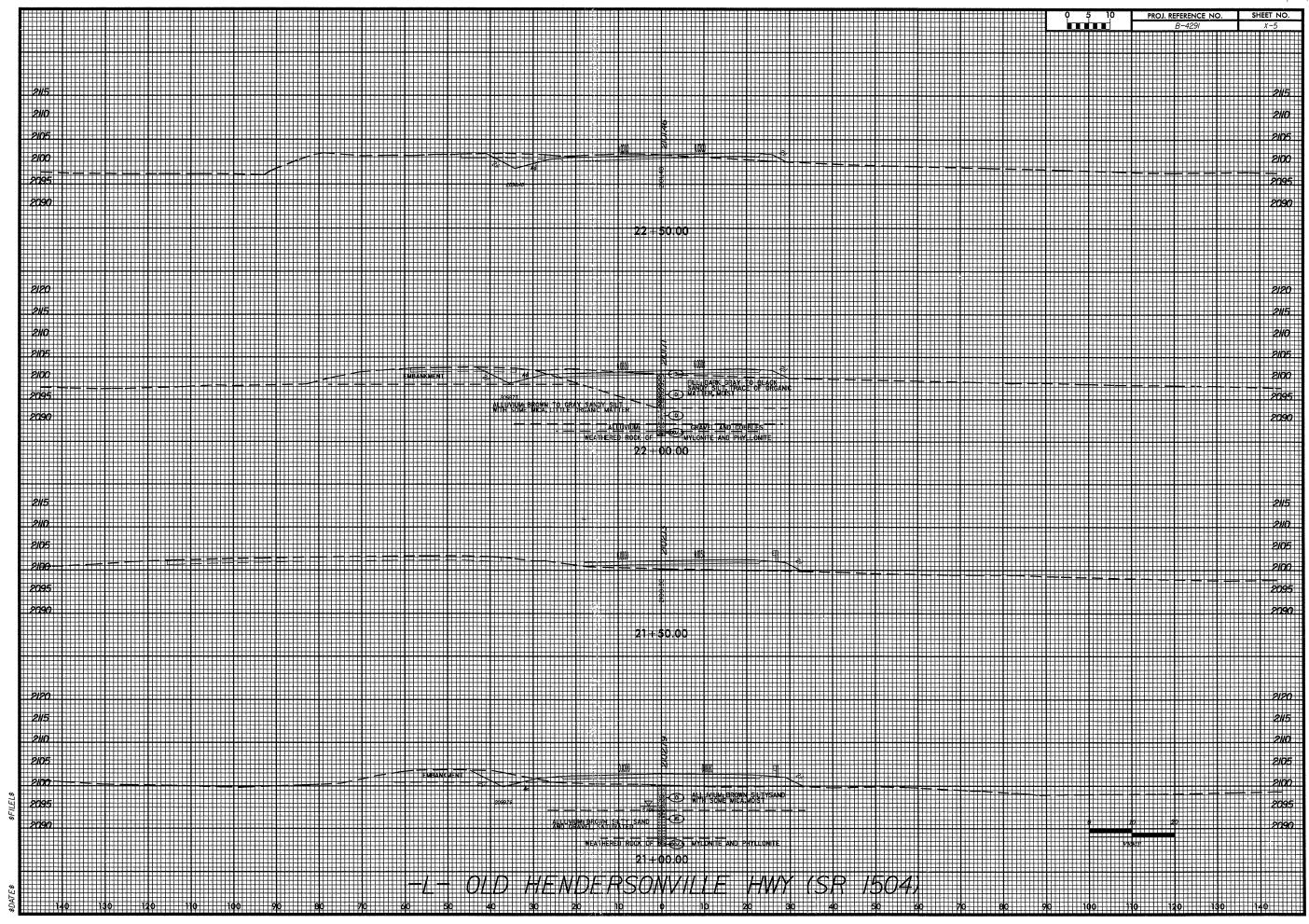
Charles A. Dunnagan, LG Project Geological Engineer

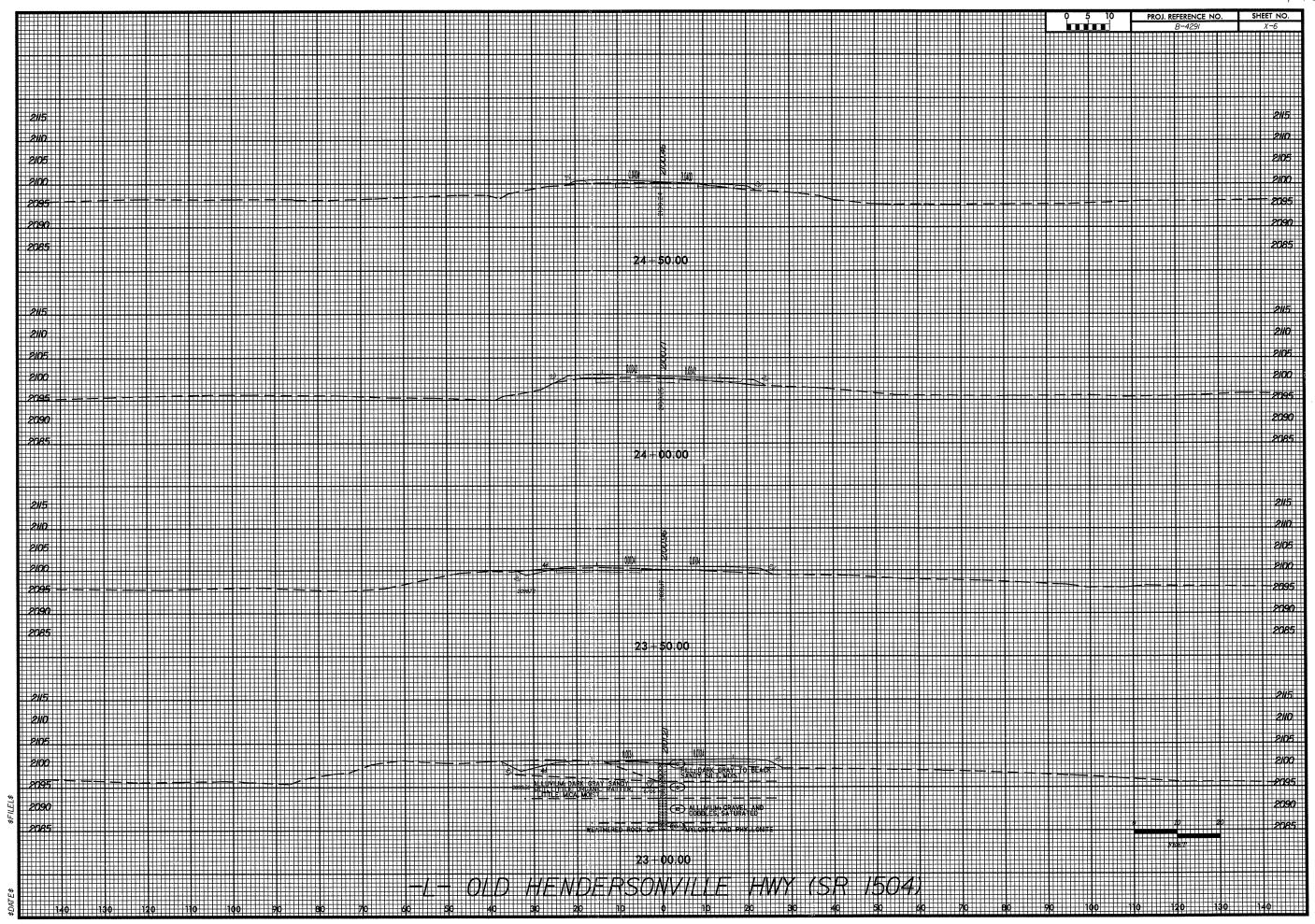












JCS NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY

B-4291

T.I.P. ID #:

REPORT ON SAMPL	ES OF: Soil	s for Quality						
PROJECT:	33630.1.1			ansylvania		/ner:	TO 110	
DATE SAMPLED:	8.7.06	DATE	RECEIVED:	8.9.06	,	E REPORTE	CD: 8.11.0	16
SAMPLED FROM:	Roadway		SAME	LED BY:	C. A. Dunnag			
SUBMITTED BY:	W. D. Frye				2002	STANDARD	SPECIFICA	TION
LABORATORY:	Asheville						,	
							•	
			TEST RI	SULTS				
Project Sample No.	SS-1	SS-2	SS-3	SS-4	SS-5		T	T
Lab Sample No. A	153407	153408	153409	153410	153411			
HiCAMS Sample #								
Retained #4 Sieve %	0.0	0.0	0.0	0.0	0.0			
Passing #10 Sieve %	100	38	96	. 99	100		, r -	
Passing #40 Sieve %	91	22	93	96	97			
Passing #200 Sieve %	23	7	65	61	47			· ·
I tasing "200 sieve ve				<u> </u>				
		M	INUS #10 1	ED A CTIC)N			ē
1000/		171	11108 #101	MACIN		T	T	T
Soil Mortar - 100%	1 07	57	7 .	9	9	 	 	
Coarse Sand -Ret. #60	27	57	38	43	56		 	·
Fine Sand - Ret. #270	57	30	39	36	21	<u> </u>	<u> </u>	
Silt 0.05-0.005 mm %	4	3		12	14	<u> </u>		
Clay < 0.005 mm %	12	10	16				<u> </u>	
Passing # 40 Sieve %						 		
Passing # 200 Sieve %	<u> </u>			L				
			·			T :.	т	T
Liquid Limit	28	22	53	58	51			
Plastic Index	NP	NP	NP	NP	NP NP	<u> </u>	 	<u> </u>
AASHTO Classification	A-2-4 (0)	A-1-a (0)	A-5 (8)	A-5 (7)	A-5 (3)	<u> </u>		-
Quantity							1	
Texture		21.00	22100	22100	22100			
Station	21+00	21+00	22+00	22+00	22+00			
Hole No.	1	0.2	0.5	12	9.3		-	
Depth (ft) From:	4.3	9.3	1.5	5.3	10.3			-
То:	5.3	10.3	1.5	3.3	10.5	 	 	
		<u> </u>	L	1		<u> </u>		
Remarks:								
A-153407 - 153411								
CC:								
C. A. Dunnagan								
File								
		,						
SOILS ENGINEER:								

8-19-2000

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS-MATERIALS AND TESTS UNIT SOILS TEST REPORT-SOILS LABORATORY										
T.I.P. ID #: B-4291										
REPORT ON SAMPLES OF: Soils for Quality										
PROJECT:	33630.1.1	CC	OUNTY:	Fransylvania		Owner:				
DATE SAMPLED:	8.11.06		RECEIVED			DATE RE	PORTED	8.16.0	6	
SAMPLED FROM:	Roadway				C. A. D	unnagan				
SUBMITTED BY:	W. D. Frye				2002		NDARD S	PECIFICA	TION	
LABORATORY:	Asheville									
				ESULTS						
Project Sample No.	SS-6	SS-7	SS-8	SS-9						
Lab Sample No. A	153425	153426	153427	153428						
HiCAMS Sample #										
Retained #4 Sieve %	0.0	0.0	0.0	0.0						
Passing #10 Sieve %	87	98	96	82						
Passing #40 Sieve %	. 80		95	63						
Passing #200 Sieve %	55	28	69	36					<u> </u>	
		M	INUS #10	FRACTIC	N	—————————————————————————————————————				
Soil Mortar - 100%		·				·				
Coarse Sand -Ret. #60	15	26	5	33					·	
Fine Sand - Ret. #270	31	51	32	31						
Silt 0.05-0.005 mm %	36	11	51	22						
Clay < 0.005 mm %	18	12	12	14						
Passing # 40 Sieve %									<u> </u>	
Passing # 200 Sieve %		<u></u>				L			<u></u>	
· ·					<u> </u>					
Liquid Limit	28	35	61	25						
Plastic Index	NP	NP	NP	NP					·	
AASHTO Classification	A-4 (4)	A-2-4 (0)	A-5 (10)	A-4 (0)						
Quantity										
Texture	·									
Station	18+50	17+00	17+00	17+00				.,		
Hole No.										
Depth (ft) From:	14.2	0.1	1.0	14.5						
To:	15.2	1.0	1.5	14.6						
									<u> </u>	

Remarks:

A-153425 - 153428

CC:

C. A. Dunnagan	
File	

SOILS	EN	GIN	EER:
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